

**Surface Water, Groundwater, and
Sediment Transport Monitoring,
Water Year 2010, Grady Ranch,
Marin County, California**

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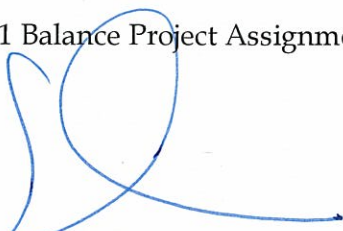
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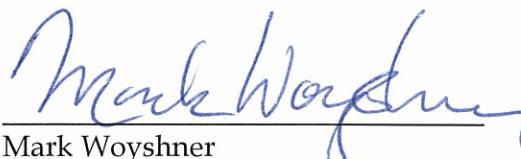
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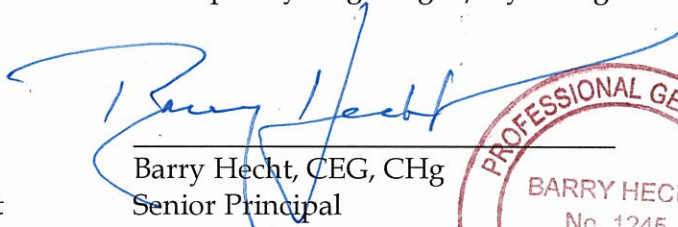
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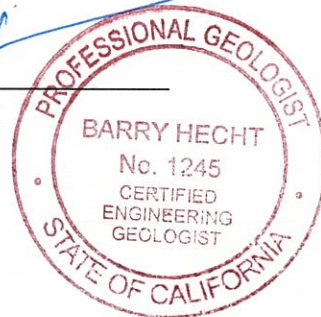
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TABLE OF CONTENTS

1. INTRODUCTION1

1.1 PURPOSE1

1.2 PRIOR AND CONCURRENT WORK.....2

1.3 COMMENCEMENT OF WORK.....3

2. SETTING5

2.1 CLIMATE AND HYDROLOGIC SETTING5

2.2 GEOLOGIC AND GEOMORPHIC SETTING.....5

3. METHODS.....7

3.1 RAINFALL.....7

3.2 STREAM GAGING.....7

3.3 SEDIMENT TRANSPORT9

 3.3.1 *Types of sediment sampled*9

 3.3.2 *Field methods for sampling sediment*9

 3.3.3 *Sediment-rating curves*10

3.4 GROUNDWATER MONITORING11

3.5 WATER QUALITY.....12

3.6 GEOPHYSICAL SURVEYS AND GEOTECHNICAL BORINGS.....13

4. DISCUSSION.....15

4.1 HYDROLOGIC SUMMARY15

4.2 SEDIMENT TRANSPORT16

4.3 WATER QUALITY.....17

4.4 SURFACE-WATER AND GROUNDWATER INTERACTIONS18

5. CONCLUSIONS AND RECOMMENDATIONS.....20

6. LIMITATIONS.....22

7. REFERENCES23

LIST OF TABLES

- Table 1. Gaging station and monitoring well descriptors, Grady Ranch, Marin County, California.
- Table 2. Summary of water-quality field measurements and laboratory analyses, Grady Ranch, Marin County, California.
- Table 3. Total monthly flow in Miller Creek and its tributaries, Grady Ranch, Marin County, California.
- Table 4. Runoff intensity and duration in Miller Creek at Grady Bridge, Marin County, California.

LIST OF FIGURES

- Figure 1. Location of Grady Ranch, Marin County, California
- Figure 2. Upper Miller Creek watershed and tributaries, Grady Ranch, Marin County, California
- Figure 3. Monitoring stations at Grady Ranch, Marin County, California
- Figure 4. Rainfall at Grady Ranch, water year 2010, Marin County, California
- Figure 5. Annual hydrograph for Miller Creek above Grady Bridge, water year 2010, Marin County, California
- Figure 6. Annual hydrograph for Grady Creek, water year 2010, Marin County, California
- Figure 7. Annual hydrograph for Landmark Creek, water year 2010, Marin County, California
- Figure 8. Annual hydrograph for S3 Tributary, water year 2010, Marin County, California
- Figure 9. Annual hydrograph for S4 Tributary, water year 2010, Marin County, California
- Figure 10. Wet and dry dates for Miller Creek and its tributaries on Grady Ranch, water year 2010, Marin County, California
- Figure 11. Annual unit hydrographs at Grady Ranch, water year 2010, Marin County, California

LIST OF FIGURES (CONTINUED)

- Figure 12. Annual unit hydrographs for Miller Creek and San Geronimo Creek, water year 2010, Marin County, California
- Figure 13. Sediment-discharge measurements for Miller Creek and tributaries water year 2010, Grady Ranch, Marin County, California
- Figure 14. Water temperature record at the Miller Creek above Grady Bridge station, water year 2010, Marin County, California
- Figure 15. Specific conductance record at the Miller Creek above Grady Bridge station, water year 2010, Marin County, California
- Figure 16. Piper diagram illustrating different ionic signatures of water samples collected from wells and surface waters on Grady Ranch, Marin County, California
- Figure 17. Groundwater elevations at Grady Ranch, water year 2010, Marin County, California
- Figure 18. Surface-groundwater interactions on Miller Creek above Grady Bridge, water year 2010, Marin County, California
- Figure 19. Surface-groundwater interactions on Grady Creek, water year 2010, Marin County, California
- Figure 20. Surface-groundwater interactions on S4 Tributary, water year 2010, Marin County, California

LIST OF FORMS

- Form 1. Annual rainfall record, Grady Ranch, water year 2010, Marin County, California
- Form 2. Annual hydrologic record, Miller Creek above Grady Bridge, water year 2010, Marin County, California
- Form 3. Annual hydrologic record, Grady Creek, water year 2010, Marin County, California
- Form 4. Annual hydrologic record, Landmark Creek, water year 2010, Marin County, California
- Form 5. Annual hydrologic record, S3 Tributary, water year 2010, Marin County, California

LIST OF FORMS (CONTINUED)

- Form 6. Annual hydrologic record, S4 Tributary, water year 2010, Marin County, California
- Form 7. Annual sediment-discharge record, Miller Creek above Grady Bridge, water year 2010, Marin County, California

APPENDICES

- Appendix A. Log of hydrologic observations
- Appendix B. Stage-to-discharge rating curves
- Appendix C. Sediment-discharge measurements, Grady Ranch, water year 2010, Marin County, California
- Appendix D. Monitoring well logs, Grady Ranch, Marin County, California
- Appendix E. Maximum daily water temperature in Miller Creek and tributaries, Grady Ranch, Marin County, California
- Appendix F. Analytical laboratory reports, Grady Ranch, Marin County, California
- Appendix G. Geology Plate C1.1A (AMEC Geomatrix, November 2008), boring logs (August 2009) and supplemental seismic refraction lines (October 2009), Grady Ranch, Marin County, California
- Appendix H. Seismic refraction survey (November 2009), Grady Ranch, Marin County, California
- Appendix I. Electrical resistivity survey (January 2010), Grady Ranch, Marin County, California
- Appendix J. Bedrock well completion reports (November 2009, February 2010, and August 2010)
- Appendix K. Annual Hydrologic Record and Sediment-Transport Measurements for San Geronimo Creek at Lagunitas Road, Marin County, California: Data Report for Water Year 2010

1. INTRODUCTION

1.1 Purpose

The Grady Ranch project lies within Marin County, California, approximately 6 miles northwest of City of San Rafael and 4.5 miles southwest of the City of Novato, in the headwaters of Miller Creek watershed (Figure 1). Upper Miller Creek watershed has a rural, sparsely developed character. South-facing valley slopes of Big Rock Ridge comprise the relatively natural portion of Grady Ranch, donated as a conservation easement to the Marin County Open Space District by Skywalker Properties in the mid 1990's, and north-facing valley slopes of Loma Alta drain the Rocking H Ranch (formerly Luiz Ranch), of which the upper portion is also relatively undisturbed and in an Open Space District conservation easement (Figure 2). The most significant existing improvement to the property is the bridge over Miller Creek (installed 1941), which allows dirt road access leading up Grady Creek to former dairy and homestead.

The Miller Creek valley has incised during historical times, and continues to incise. The upstream knickpoint is currently stabilized at Grady Bridge by the concrete footer spanning the channel beneath the bridge and concrete rubble. The creek bed elevation drops 11 feet immediately downstream of Grady bridge, and remnants of an intermediate (inset) terrace level, up to 12 feet above the existing thalweg, are still present along portions of left bank of the creek (Brown and Hecht, 2011). Substantial incision and channel change over the past 17 years can be noted from a detailed 1993 topographic map (see Vandivere, 1994).

A stream and valley restoration plan is proposed on Grady Ranch as part of the proposed facility and associated infrastructure to be used primarily for advanced, digital technology-based entertainment production. The proposed project, planned for the Grady Creek watershed, is outlined in the precise development plan (PDP) (CSW/ST2, 2009a and 2009b) and is tiered off of the Grady Ranch Master Plan (Nichols-Berman, 1996).

Balance Hydrologics, Inc. (Balance) was asked to conduct multi-year pre-construction hydrologic monitoring on Grady Ranch that includes: a) rainfall, b) stream gaging of Miller Creek and its primary tributaries, c) sediment transport, d) groundwater level monitoring, and e) water quality sampling. This report presents the findings from the first year of monitoring

during water year 2010¹, and develops initial conclusions subject to refinement and revision as a longer, more robust set of observations are developed.

1.2 Prior and Concurrent Work

Balance prepared a series of reports related to restoration planning and hydrologic and geomorphic assessment. As part of the PDP submittal, Balance prepared a geomorphic assessment of upper Miller Creek, Grady Creek, and their tributaries on Grady Ranch (Brown and others, 2008), and then an updated summary of the existing channel stability in upper Miller Creek on Grady Ranch was subsequently conducted by Brown and Hecht (2011a). A broad understanding of the stream and valley floor restoration vision was illustrated in the memo by Brown and Hecht (2009) as a response to comments to the PDP. A summary the proposed restoration effort was then presented by Brown and Hecht (2011) with discussion of several often-posed questions and aspects of the project in relation to other restoration efforts within northern California. The restoration plans and the feasibility were described by Owens and others (2008, 2011), a project alternatives analysis by (Brown and others, 2011), and stormwater changes related specifically to the project by Ballman and Cayot (2008) and CSW/ST2 (2009a). To document channel conditions downstream of Grady Ranch and to summarize planning efforts during recent years, Balance, in response to comments to the PDP, conducted a baseline reconnaissance characterization of the reaches of Miller Creek extending downstream from the project site to San Pablo Bay (Woyshner and others, 2011). Separate analyses were conducted concurrently to assess biologic issues (WRA, 2008, 2010) and a geotechnical investigation of the site was conducted by AMEC Geomatrix (2008).

Unlike most watersheds in the North Bay, a watershed assessment has not been completed for the Miller Creek catchment, although important preliminary drafts of sections were prepared prior the economic disruptions which have affected the region beginning in 2008 (see Woyshner and others for details). At an individual site level, previous work related to stream restoration has been conducted within the upper Miller Creek watershed. Philip Williams and Associates (PWA) with David Gates and Associates (DGA) prepared a report related to watershed assessment and planning (PWA and DGA, 1981). PWA and Clearwater Hydrology prepared a series of reports and plans related to hydrologic analysis and restoration design

¹ Most hydrologic and geomorphic monitoring occurs for a period defined as a water year, which begins on October 1 and ends on September 30 of the named year. For example, water year 2010 (WY2010) began on October 1, 2009 and ended on September 30, 2010.

(Vandivere, 1984; Vandivere, 1985; Vandivere and Mock, 1985). These studies were the basis for rehabilitation of an incised reach of Miller Creek downstream of Grady Ranch, which included in-stream grade stabilization and laying back and revegetating banks to reduce flow velocities and associated erosion. Summaries of the restoration are included in post-construction assessments by Haltiner and others (1996), Yin and Pope-Daum (2004), and Woyshner and others (2011).

Data collected in Miller Creek is compared in this report to similar data collected in other streams. The primary comparison is with San Geronimo Creek at the community of Lagunitas, where Balance staff have operated a stream gage for MMWD since 1979. Data for that gage have been recently summarized through 2008 (Hecht, Strudley and Brown, 2010). Streamflow and sediment-transport data have also been collected and analyzed at the gage in 2009 and 2010 (Owens and Hecht, 2010). Earlier flow and sediment-transport data for San Geronimo Valley tributaries were collected during the early 1980s (Hecht, 1983), but no sustained data collection has occurred since. Useful and significant geomorphic analysis of the San Geronimo Valley has also been developed as part of recent watershed-planning efforts sponsored by Marin County (Stillwater Sciences, 2009, and Prunuske Chatham, 2010). Other sediment studies in central Marin or in the east-draining watershed have occurred on regulated tributaries, where dams and releases have altered the channels, and limit the comparability to Miller Creek.

1.3 Commencement of Work

On December 22, 2009, Balance was authorized to proceed with an aquifer characterization and surface-water and groundwater monitoring program, as part of multi-year pre-construction hydrologic monitoring on Grady Ranch that included the following tasks:

- Install a tipping-bucket rain gage and monitor rainfall during water year 2010;
- Install 2 stream gages on Miller Creek, one at Grady Bridge and one at the property line, and 4 tributary gages, one each on Grady Creek, Landmark Creek, S3 Tributary, and S4 Tributary, gage flows during water year 2010;
- Sample bedload and suspended sediment and specific conductance at all the stream gages and establish preliminary rating curves, used to calculate sediment loads;
- Install 4 monitoring wells and monitor water levels during water year 2010, in addition to monitoring groundwater level in an existing monitoring well on the ranch;

- Subcontract with Norcal Geophysical Consultants to conduct seismic refraction survey lines across the valley floor to evaluate depth to bedrock; and,
- Collect water quality samples from the wells and at the gaging stations for analyses of general mineral composition as a method of fingerprinting the source of the samples.

Table 1 describes the wells and gaging stations we visited throughout water year 2010 and Figure 3 maps their locations.

2. SETTING

2.1 Climate and Hydrologic Setting

Grady Ranch has a Mediterranean climate, with wet winters and generally dry summers. The site receives an average of 34 inches of rain (Rantz, 1971). Summer high temperatures routinely reach 90°F, with lows of approximately 50°F. Winter daily temperature range is smaller, with highs between 50 and 60°F and lows between 40 and 50°F. Miller Creek, where it runs through Grady Ranch, is an intermittent stream, flowing only during the winter, spring and early summer (occasionally) in response to precipitation events and the discharge of shallow groundwater. Flow seldom, if ever, persists beyond early July. The watershed area of Miller Creek at the downstream property line of Grady Ranch is 2.8 square miles. As is typical of small, coastal streams in California, the creek's discharge is extremely variable, sometimes changing rapidly over several orders of magnitude during a day.

2.2 Geologic and Geomorphic Setting

Grady Ranch lies within the Coast Range geomorphic province of California and is underlain by Mesozoic rocks of the Franciscan Complex. A number of primary faults, including the San Andreas, are largely responsible for the present form of the Coast Range province. Grady Ranch, along with much of the rest of eastern Marin County, is found within a relatively coherent block of crust known as the Bay Block. It is bounded on the east and west by right-lateral strike-slip faults: the Rogers Creek/Hayward and San Andreas respectively. Although most of the motion along these two faults is in a shearing sense, a small component of the motion along these faults is compressional, which has generated the mountains and generally steep topography of the project site, Marin County and the Coast Range at large. Grady Ranch is approximately 8.5 miles east of the trace of the San Andreas Fault and 6 miles west of the Rogers Creek/Hayward Fault.

Within the project area, along Big Rock Ridge, the Franciscan Complex is dominated by graywackes and shales of the Novato Quarry terrane that are Jurassic and Cretaceous in age (Blake and others, 2000). To the south and within discrete bands surrounding the more intact Novato Quarry terrane is Franciscan *mélange*, a complex mixture of highly fractured rocks bound within a soft matrix of crushed shale (and other fine-grained sediments) or serpentinite.

Where exposed on the surface, the *mélange* matrix erodes easily, leaving behind a rounded landscape, with subtle rolling hills and knobs. At Big Rock Ranch, the topographic saddle through which the Lucas Valley Road passes and which separates the Miller Creek watershed from the Nicasio Creek watershed, a block of Franciscan serpentinite outcrops. It is associated with a section of former oceanic crust (Sloan, 2006).

Below Big Rock Ranch, Quaternary alluvium overlies the Franciscan Complex bedrock within the valley bottom along Miller Creek. This alluvium is the result of hundreds of thousands to millions of years of net valley filling with sediment transported downstream by Miller Creek. These unconsolidated, stratified sands, silts, clays, gravels and cobbles form a veneer of up to 100 feet thick over the bedrock below. In a number of places, Miller Creek has re-incised into these sediments, leaving cliffs of up to 35 feet (Figure 3).

In addition to the erosion, transport and deposition of sediment associated with Miller Creek, landslides and debris flows also dominate geomorphic processes within the steep portions of the project area. Slumping and other types of failure of the incised creek banks can also radically, rapidly alter the landscape, and have significant effects on the channel form both up and downstream of the channel failure. Earthquakes and intense, high-magnitude rain events can both serve as triggers for these episodic, geomorphic events.

3. METHODS

3.1 Rainfall

We installed a tipping bucket rain gage on October 11, 2009 on the old dairy trough located approximately 165 feet northwest of the property gate at Lucas Valley Road (Figure 3). The gage recorded the time of each bucket tip corresponding to 0.01 inches of rain. The recorded rainfall total for water year 2010 was 35.65 inches², which is about 105 percent of the long-term average annual rainfall, estimated to be approximately 34 inches (Rantz, 1971). Daily and cumulative rainfall data for water year 2010 are shown in Figure 2 and Form 1. For comparison, rainfall during water year 2010 at a nearby Balance station, located on the west side of Loma Alta in Lagunitas, Marin County, was 110 percent of the average annual rainfall (Owens and Hecht, 2010). Antecedent to water year 2010 was three years of below normal rainfall (<http://marinwater.org/controller?action=menuclick&id=221>). Water year 2006 was wetter than normal, including a major storm on Dec. 31, 2005 which generated flooding and mudflows throughout the Miller Creek watershed and Lucas Valley.

3.2 Stream Gaging

We installed six stream gaging stations on Grady Ranch (Figure 1): one real-time station on Miller Creek above Grady Bridge³, one station on each of the four main tributaries flowing into Miller Creek, and one station on Miller Creek at the property line. Two of the tributaries, Landmark Creek and S3 Tributary, are located upstream of Grady Bridge, and the other two tributaries are located downstream of Grady Bridge, Grady Creek and S4 Tributary. The watershed area of the real-time station on Miller Creek above Grady Bridge is 2.1 square miles; the watershed area of Grady Creek, Landmark Creek, and S3 Tributary is 0.4 square miles; and, the watershed area of S4 Tributary is 0.3 square miles. The watershed area of Miller Creek at the downstream property line of Grady Ranch is 2.8 square miles.

² NOAA's Big Rock meteorological station located 1.8 miles east of our rain gage confirms that there was no rainfall in water year 2010 prior to our rain gage installation, e.g., between October 1 and October 10 (<http://www.raws.dri.edu/cgi-bin/rawMAIN.pl?caCBIR>).

³ Real-time stage, streamflow, specific conductance, and temperature data are available at www.balancehydro.com/onlinegaging.php. Private gage login required.

We verified our gaging records during at Grady Ranch during 2010 with gaged flows from MMWD's San Geronimo Creek at Lagunitas station, with a watershed area of 8.7 square miles.

We installed a staff plate and datalogger at each gaging station and collected a continuous record of water level (stream stage) for water year 2010. The stage record was converted to streamflow with a stage-discharge rating curve developed with periodic hand streamflow measurements at a range of stages. We used modified U. S. Geological Survey methods to measure streamflow by measuring depth and velocity at many verticals across a cross section of the creek (Rantz and others, 1982). Based on our staff plate readings and streamflow measurements (Appendix A), we created an empirical stage-to-discharge relationship referred to as a stage-discharge "rating curve" (Appendix B). As is typically done, we calibrated the stage record to observations of gage height and then applied stage shifts to the data to account for local fill and scour associated with sediment-transporting storm events. After the stage record is corrected, we apply the stage-discharge rating curve to convert each stage value to streamflow. Our results are presented as daily flow, which are totaled from data recorded and calculated every 15 minutes (Forms 2 - 6) and plotted as annual hydrographs (Figures 5 - 9).

At all of our gages except on Landmark Creek, we measured flows up to 30 to 40 percent of the seasonal peak flow (on January 20th). Landmark Creek is more difficult to access than the other gages and due to time constraints it was not visited during the peak flow event on January 20.⁴ To extrapolate beyond the range of measured flow, we extended the rating curves based on professional judgment and verified with extrapolated stage to velocity and stage to cross sectional area rating curves. As with all open-channel gaging of natural streams, some uncertainty remains (especially at high flows) in spite of efforts to be as precise as possible, and data is regarded preliminary and subject to revision as additional measurements are collected.

Three stations were relocated after the streams dried down in preparation for continued gaging during water year 2011:

- The gaging station on Miller Creek at the property line was severely damaged from the first high flows of the season. No data are available for this station during water year 2010. It was relocated to an eddy pool about 600 feet upstream of the property line.

⁴ Our highest measurement on Landmark Creek was ~2 percent of the seasonal peak.

- During water year 2010, sediment filled in the pool at the real-time gage above Grady Bridge. We moved the instrumentation to a pool just upstream on the opposite bank on October 7, 2010.
- At the Grady Creek station, the bank slumped on March 2, 2010 and buried the staff plate and datalogger sensor. We moved the instrumentation just upstream to a pool on the opposite bank on October 19, 2010. The new Grady Creek gage is located upstream of the slump, so as not to be affected by this sediment transported downstream.

3.3 Sediment Transport

3.3.1 Types of sediment sampled

We distinguish two types of sediment in transport: bedload sediment and suspended sediment. *Bedload sediment* is supported by the bed; it rolls and saltates along the bed, commonly within the lowermost 3 inches. Movement can be either continuous or intermittent, but is generally much slower than the mean velocity of the stream. In the Miller Creek channels, as elsewhere in the Bay Area, bedload consists primarily of medium and coarse sands and gravels.

Suspended sediment is supported by the turbulence of the water, and is transported at a rate approaching the mean velocity of flow. In these streams, suspended sediment consists of fine sands, silts, and clays, and tends to be entrained at lower flows than bedload. As a result, fine sediment may be deposited on top of the coarse sands and gravels used for fish spawning further downstream, or can be deposited in pools that are used for summer rearing downstream of Bridgegate Drive.⁵

3.3.2 Field methods for sampling sediment

Standard methods and equipment reviewed by the Federal Interagency Sedimentation Project (FISP) were used to make measurements of sediment transport. Field measurements of sediment discharge are made either by hand samplers applied in transects across the channel at wadeable flows or with cable-suspended samplers from the bridge railing at high flows. We use Helley-Smith 3-inch bedload samplers, and DH-48, DH-81 and D-74 suspended-sediment

⁵ At Grady Ranch, spawning and rearing occurs in the bedrock headwater canyons upstream of the valley. Elsewhere in the Miller Creek watershed, almost nothing is known about where steelhead spawn and rear (Liz Lewis, pers. comm., 2010), which was the subject of a NOAA Fisheries field study of the stream system during 2010. Comments re spawning and rearing downstream are based on local accounts, presently unsubstantiated.

samplers. Bedload- and suspended-sediment samples are taken at multiple verticals across the creek to collect a representative sample (Emmett, 1980; Edwards and Glysson, 1999, and older references cited therein). For bedload-sediment sampling we first establish the active-bed width by observation and/or preliminary sampling, then sample within that portion of the creek. For suspended-sediment sampling, we use two sampling methods depending on conditions; both methods are used and endorsed by the USGS to collect suspended-sediment samples that are representative of the mean sediment concentration of a stream. The two methods are the equal-discharge-increment method (EDI) and the equal-width-increment method (EWI) (Edwards and Glysson, 1999). With both methods we collect depth-integrated samples at multiple verticals across the creek.

Bedload samples were dried and weighted at Balance's office. Suspended-sediment samples are analyzed by Soil Control Lab in Watsonville, California, a state-certified laboratory.

3.3.3 Sediment-rating curves

The principal purpose of sediment sampling is to develop an annual empirical relationship between the amount of sediment transported at a given flow. These "sediment-rating curves" are valuable for year-to-year comparisons since these curves are diagnostic of the processes of sediment movement through the stream system. As the position or shape of the curve changes, a different relationship between streamflow and sediment transport is expressed, indicating decreases or increases in sediment supply (c.f., Hecht and Owens, 2006). Water year 2010 rating curves provide a clear, rigorous baseline against which post-project conditions can be quantified.

These rating curves are the basis for calculating the volume of sediment transported past the gaging station for each 15-minute period and hence for each day. This continuous record of sediment discharge is vastly simplified from the many individual events, processes, and occurrences that influence the actual discharge of sediment, but experience has shown it to be a useful and reasonably accurate approximation of this complex reality (c.f., Edwards and Glysson, 1999; Emmett, 1980).

For the purposes of this study, we measured bedload- and suspended-sediment discharge at all five gages (Appendix C) and developed preliminary sediment-rating curves at Miller Creek

above Grady Bridge based on samples collected at flows ranging from 2.50 to 80.8 cfs (Figure 13). We recognize that extrapolating our curves above 80.8 cfs may be overestimating sediment transport, as some streams have a diminishing rate of increasing transport at highest flows; this may not be the case for Miller Creek, particularly downstream of Grady Bridge where vertical banks are actively collapsing. While future sampling during high flows may elucidate this relationship, water year 2010 sediment data and totals should be considered preliminary and subject to revision due to the irregular and supply-driven nature of sediment discharge in small streams (c.f., Edwards and Glysson, 1999). We will collect another year of sediment transport data before attempting to develop preliminary sediment-rating curves at tributary gages, given fewer samples were collected and the number of samples can limit the precision of the results.

3.4 Groundwater Monitoring

On October 20 - 21, 2009, we observed the installation of four monitoring wells drilled by Taber Consultants to depths ranging from 30 to 40 feet (Appendix D). MW-1, -2, and -3 were located at sites we selected near Miller Creek, and MW- 4 near Grady Creek (Table 3). Prior to installing the monitoring wells, AMEC Geomatrix had conducted a geotechnical investigation of the site in 2008 and, in the course of that study, had installed one monitoring well (RW-6). The well log for RW-6 is also located in Appendix D.

Monitoring well locations are shown on Figure 3. MW-1 and MW-2 are upstream of Grady Bridge. MW-1 is located furthest upstream between the Landmark Creek confluence and S3 confluence, about 1000 feet from Grady Bridge. MW2 is about 100 upstream of Grady Bridge. MW-3 is roughly 350 feet downstream from Grady Bridge and MW-4 is approximately 400 feet upstream from Miller Creek (and Grady Bridge). RW-6 is between Miller Creek and S4 Tributary about 300 feet downstream from Grady Bridge. The wells are located on the valley floor, which is an older stream terrace. They are drilled into alluvium and intersect the shallow water table in the alluvium. We screened the wells to document how and how far groundwater rises during the wet season, falls during the dry season, and interacts with surface water.

Monitoring wells MW-1, -2, -3, and -4 were drilled to bedrock with an 8-inch hollow-stem auger using track mounted CME-55 drill rig.⁶ We logged the excavated soils and screened the

⁶ Monitoring well RW-6 was drilled an additional 16 feet into bedrock to a depth of 52 feet with a track-mounted CME-55 4-inch diameter rotary wash (AMEC, 2008).

wells to monitor the most permeable horizon. The wells were constructed of 2-inch, schedule 40 PVC pipe screened with 0.02-inch slots and blank riser consisting of the remaining pipe length to approximately 3 feet above ground surface. We chose to fill the annulus with 30-mesh Monterey sand overlain by a bentonite plug, which was then overlain by a concrete grout slurry seal. Cement was also mounded on the ground surface around the wells to drain radially away from the wells, inhibiting ponding of surface water around the casing. All of the wells were vented and secured with a locking cap and "stove pipe."

We collected a continuous record of groundwater water level in all five monitoring wells. Depth-to-water measurements were taken at each monitoring well on each site visit. We calibrated the water surface elevation record to these observations (Figure 17).

3.5 Water Quality

Our limited 2010 water-quality monitoring program had two objectives:

- 1) to assess aquatic habitat and wetting and drying conditions along Miller Creek and its tributaries using temperature, and
- 2) to evaluate surface-groundwater interactions, dynamically using specific conductance and temperature, and (on a one-time basis) assess recharge areas and flow paths qualitatively using tracing with general minerals analysis ("major-ion fingerprinting").

We collected continuous temperature data at all the gages, making calibrative hand measurements during site visits. Figure 14 shows the daily water temperature record at the Miller Creek above Grady Bridge real-time station. Additional temperature loggers were also placed in pools on Miller Creek upstream and downstream of Grady Bridge to investigate how this knickpoint affects seasonal wet-up and dry-down dates (Figure 10). For data reference, the maximum daily temperatures in Miller Creek above and below the bridge and in Grady Creek are tabulated in Appendix E.

We also collected continuous specific conductance⁷ data at the Miller Creek above Grady Bridge real-time station (Figure 15) and made measurements with hand meters recently calibrated

⁷ Specific conductance measures the ability of the water to conduct electricity, and is a widely used index for salinity or total dissolved solids (TDS). Rainwater has very low specific conductance and, as water passes over and through the ground, salts are dissolved, increasing the specific conductance. Higher

with potassium hydroxide standards at this and all the tributary gages. Specific conductance was converted to an equivalent value at 25 °C according to the accepted polynomial relationship between specific conductance and temperature (see observer log footnotes).

Water-quality samples were collected from the five monitoring wells (MW-1 through -4 and RW-6), from Grady Creek and Landmark Creek during dry-season baseflow, and from bedrock well #B. On November 11, 2009, Forster Pump and Engineering had drilled well #B to a depth of 400 feet and installed perforated casing below a depth of 160 feet (Appendix J). While it was being tested for yield, we collected a water quality sample on November 17, 2009. Bedrock well #B is located near MW-2. The water-quality samples for general-minerals analysis were field filtered through 0.45 micron glass-fiber filter, then preserved and transported per Standard Methods to a California-certified analytical laboratory to be analyzed within acceptable holdtimes for general minerals (Table 2). Laboratory reports are compiled in Appendix F (and Appendix J for bedrock well #B).

3.6 Geophysical Surveys and Geotechnical Borings

As part of the geotechnical investigation (AMEC Geomatrix, 2008), seismic refraction lines⁸ and borehole logging were used to develop a geologic map and cross sections across the property where the proposed commercial building and associate infrastructure is planned (Appendix G). Supplemental seismic refraction lines were conducted in October 2009 by Norcal Geophysical Consultants (Norcal) to further characterize the lower Grady Creek and the ridge east to the property boundary (Appendix G). Tiered off this survey, we requested to extend lines across the valley bottom to evaluate the depth of alluvium (Appendix H). Electrical resistivity⁹ lines of the lower Grady Creek, S4 Tributary fan, and interconnecting Miller Creek terrace areas were then conducted in January 2010 by Norcal as part of a groundwater supply exploration directed by others (Appendix I). A water-well drilling campaign was executed by Forster Pump and Engineering in November 2009, February 2010, and August 2010. The driller's well completion reports (logs) are compiled in Appendix J.

specific conductance indicates transmittal through salt-bearing geologic formations or longer residence times in the ground.

⁸ The seismic refraction method utilizes the refraction of seismic waves on geologic layers and rock/soil units to characterize the subsurface geologic conditions and geologic structure.

⁹ Electrical resistivity (ER) of soils is a function of porosity, water content, ionic concentration of pore water, clay content, and permeability. ER survey are commonly used to map vertical extent of soil types, stratigraphy, clay aquitards, and saline water, and evaluate depth to groundwater and to bedrock.

At a later date, the findings of these subsurface investigations will be used in correspondence with the monitoring data to develop a hydrogeologic framework model that refers to the model grid location and size, model layering, and assignment of property zones to replicate the conceptual understanding of the site. This is the basis of a groundwater flow model and hydrogeologic assessment that will evaluate the effects of the proposed creek and valley restoration plan on groundwater elevations, and eventually on the likely duration of flow in channels used for steelhead migration.

4. DISCUSSION

4.1 Hydrologic Summary

Water year 2010 (WY2010) began with an unusually large storm early in the season, the largest intensity storm of WY2010 that produced 4.74 inches of rain on October 13, 2009 at Grady Ranch (Figure 4). It generated baseflow and wetted pools downstream of Grady Bridge (Figure 10). It was followed by two months of sub-normal rainfall, although the baseflow continued downstream of the bridge, though it receded at times and places, and in places to near-negligible levels. A moderate storm occurred on December 12, sufficient to generate surface runoff and cause very short-lived flow at Landmark Creek and S3 and S4 tributaries. Isolated light rain fell until another moderate storm took place on January 12. This 1.1-inch event pushed the cumulative rainfall total to 10.99 inches and resulted in flow at all the gages, except Grady Creek. A series of large storms followed during the third week of January, which initiated flow at the Grady Creek gage and produced the seasonal peak flow on January 20¹⁰ at all gages. Several moderate storms typical of the region sustained flows in February, but then March was relatively dry and all of the streams except for Grady Creek and S4 Tributary dried down towards the end of the month. More rain fell in April than had fallen in March, and these spring showers wetted up the creeks, some of which had gone temporarily dry at the gage sites, and sustained flows in the creeks that remained wet. Very light showers continued into May, but all the gages were dry by May 2.

Annual hydrographs for all five creeks are plotted in Figure 5 through 9. S3 Tributary was the only stream to dry up in the middle of February and therefore represents the most intermittent stream onsite. The total monthly flow (the volume discharged) at each of the five gages are summarized in Table 3. Of the four tributaries, Grady Creek had the highest unit discharge and generated considerably more flow during WY2010, particularly later in the season (Table 3). Unit hydrographs are plotted in Figure 15. "Unit flow" is calculated by dividing the daily mean flow by the watershed area and allows for comparison of the response to rainfall among different watersheds. In general, the magnitude of streamflow is governed by the size of the watershed, so that a larger watershed produces higher flows. However, differences among

¹⁰ High water marks observed in the field indicate that the January 20 event was approximately a bankfull event, which roughly corresponds to a 1.5-year recurrence flood or a flood with an exceedance probability of 67 percent.

streams in wet- and dry-season baseflows also reflect variations in the topography, geology, and land use within their watersheds. In March and April, Grady Creek and S4 Tributary exhibit higher falling-limb flows than the other streams, which suggests these stations may be augmented by groundwater discharge.¹¹ Grady Creek also need slightly more precipitation to generate flow, the watershed ostensibly requiring more groundwater recharge to initially fill the alluvial aquifer which subsequently allowed the flow in Grady Creek to persist a few days longer than in other tributaries.

We also compared unit flows at Miller Creek above Grady Bridge to the San Geronimo Creek station at Lagunitas (Figure 12). Coincident timing of the peaks corroborated our gaging, but most notably, Miller Creek flowed intermittently – dry for more than eight months of the year. In contrast, San Geronimo Creek had continuous flow.¹² Miller Creek is higher in its watershed with mudflow deposits and deeper alluvium than San Geronimo Creek,, which drains over bedrock at the gage beneath Lagunitas Road, where all groundwater and baseflow is forced to the surface. We also found of note that the two streams have similar magnitudes of *unit-flow* peaks, even though Miller Creek receives ~25 percent less rainfall, attesting to the ‘flashiness’ of the flows on Miller Creek. San Geronimo Creek has four times the watershed area and considerably more conifer-forested slopes with deep soils, which tends to delay runoff and sustain longer baseflows.

4.2 Sediment Transport

Bedload and suspended sediment transport was measured in the Miller Creek because (a) they are factors impairing habitat quality downstream from Grady Ranch, and (b) because – when noticeably greater than in adjoining streams -- they help identify channels which are rapidly incising or which are otherwise disturbed.

As occurs universally, sediment discharge increased as flow increased (Figure 13). On Miller Creek, the sediment-rating curves for bedload and for suspended-sediment are similar to each other, characterizing a stream with abundant supply, and with sediment readily available for

¹¹ Big Rock Ridge is stratigraphically capped by weathered fine-grained sandstone that respond as an aquifer and, at times, the source of landslides and mudflows in the tributaries, as observed following high duration-intensity storms such as December 31, 2005 and January 4, 1982.

¹² Continuous flow and wetted bedrock pools were noted in the canyon further upstream of Grady Creek, Landmark Creek, and N1 Tributary.

transport. The magnitude of sediment transported is similar in contrast to San Geronimo Creek, which had 17 times more suspended-sediment load than bedload during WY2010 (Table 4). Furthermore, Miller Creek transported considerably more sediment per unit volume of flow than San Geronimo Creek - 5 times the concentration of suspended sediment and 64 times the concentration of bedload (Table 4). Roughly half of the sediment load was discharged during the 24-hour seasonal peak flows on January 20, 2010. The peak flow on that date rose to a level which we had previously identified as morphologic bankfull.

Landmark Creek, S3 Tributary, Grady Creek, and S4 Tributary have higher rates of transport at any given flow than Miller Creek, a much larger watershed (Figure 13). This condition is a typical and nearly universal throughout Marin County (c.f., Hecht, 1983) and the Bay Area (c.f., Hecht and Owens, 2006) given that smaller watersheds have briefer, flashier hydrographs flowing through smaller, steeper channels.

For streams of a given size, a much higher rate of transport at a given flow can be an indicator of active channel incision. Hecht and Owens (2006) showed that incising channels elsewhere in the Bay Area manifested transport rates typically 5 to 10 greater at a given flow than streams from otherwise similar watersheds which are not incising.

4.3 Water Quality

We found that water temperatures in Miller Creek and Grady Creek were within the reported acceptable range for steelhead habitat (Appendix E). Maximum daily temperatures increased as streamflows receded, the creeks dried down, and weather became seasonally warmer, but remained in an acceptable level for steelhead habitat (Figure 14). Specific conductance (plotted in Figure 15) typically remained stable at roughly 200 umhos per centimeter (@ 25°C) during periods of baseflow, diminishing sharply as rainfall runoff increased during storm events. The specific conductance values are similar to other central Marin streams, such as San Geronimo Creek and the tributaries of Lagunitas Creek, suggesting that flow emanates from the alluvial aquifer rather than deeper bedrock sources (see discussion in Hecht and others, 2010). The ionic signatures of the samples support this finding, as illustrated when plotted on a Piper diagram (Figure 13); monitoring wells MW-1, -2, -3, and RW-6 are similar to the surface water samples, and can be characterized as a calcium carbonate groundwater source with a significant portion of magnesium, which is typical for the region.

In contrast to the alluvial groundwater and surface-water samples, the ionic signature of the sample collected from the bedrock well #B is dominated by sodium and chloride. This signature is also apparent MW-4 (near upper Grady Creek), which either suggests groundwater contributions from bedrock or from the recently placed bentonite (clay) in the well annulus when completing the well. Re-sampling during dry-season 2011 could address the likely source of groundwater from MW-4.

Well #B also has elevated concentrations of iron, aluminum, and fluoride (above California Title 22 drinking water standards, as shown in Table 2). The boron concentration was also elevated (8.38 mg/L), a level higher than tolerated by nearly all ornamental plants and California natives¹³. The contrasting low levels of boron in stream water is a second line of evidence pointing toward alluvium as the overwhelming source of groundwater entering the streams, in contrast to bedrock.

4.4 Surface-water and Groundwater Interactions

At the end of the dry season, groundwater elevations were considerably lower than during mid-winter (Figure 17). The largest seasonal fluctuation in groundwater elevation was observed in the upstream monitoring well MW-1 (18 feet), and the smallest furthest downstream in MW-3 (8 feet). MW-2 near Grady Bridge and MW-4 near Grady Creek fluctuated 16 feet, and RW-6 fluctuated 15 feet. This pattern reflects the potential for groundwater storage in the valley aquifer. When the storms commenced during WY2010, aquifer recharge elevated the groundwater levels in the valley alluvium. Groundwater elevations rebounded significantly with each storm, then receded between storms. Once 11 inches of rain had fallen, flow became continuous on Miller Creek, with groundwater levels stabilizing at a high winter level. Water levels for MW-1, -3, and -4 remained relatively stable during the following mid-winter storms, while MW-2 (closest to Grady Bridge knickpoint) and particularly RW-6 declined over time. RW-6 also showed the largest storm rebound, owing to its higher declining rates water levels. In addition to intersecting sands and gravels, RW-6 is located between Miller Creek and the S4 Tributary, which may account for the greater and more rapid rates of decline. The dry-season recession began in May for all wells except MW-3,

¹³ Boron is important in agriculture. Small amounts are essential to plant growth. Greater concentrations in soil and irrigation water are harmful, however, and the toxic concentration for some plants such as lemon or orange trees is as low as 1 mg/L. (Hem, 1985). Boron at these levels is not thought to negatively affect mammals, including humans.

which began its decline in July. MW-3 is overlain by 15 feet of a silty clay mudflow deposit, which confines the alluvium.

At three sites, water elevations at the gaging stations were compared to an adjacent monitoring well:

- Miller Creek above Grady Bridge (Figure 18). Largely driven by the 11-foot knickpoint immediately downstream, this reach loses water to the alluvial aquifer. Groundwater recharge keeps groundwater levels high until a mid-winter dry spell or the onset of the dry season when streamflow recedes, feeding the groundwater which discharges at and beneath the foot of the Grady Bridge knickpoint. This artificial and anthropogenic condition induces infiltration above the bridge, prematurely desiccating the channel early in the drydown season. .
- Grady Creek (Figure 19) and S4 Tributary (Figure 20). Grady Creek and S4 Tributary gain water from the aquifer and dries down in response to receding groundwater levels.

5. CONCLUSIONS AND RECOMMENDATIONS

Grady Ranch lies in the headwaters of Miller Creek watershed where flows in the alluvial channels are intermittent, but perennial in portions of the bedrock headwater canyons.¹⁴

The rainfall total and seasonal distribution of storms were roughly normal during water year 2010, except for an unusually large early season storm on October 13th that initiated flow for the season downstream of Grady Bridge 11-foot knickpoint. Upstream of Grady Bridge, the channel was dry until mid-January after 11 inches of cumulative rainfall. Flows also dried back briefly during the March dry spell and then on May 1st for the remainder of the dry season, while flows downstream of Grady Bridge were continuous until July 4th. The seasonal pattern of flow observed on Miller Creek above Grady Bridge was similar to pattern of flow monitored the four principal tributaries on Grady Ranch.

Grady Creek needed the most rain to generate continuous streamflow to its confluence with Miller Creek, owing to a higher rate of alluvial groundwater recharge in its watershed. Of the four tributaries we monitored, Grady Creek had the highest unit discharge and generated considerably more flow during WY2010, particularly later in the season. Flow in Grady Creek and the S4 Tributary persisted longer into the spring than others, while S3 Tributary was most intermittent. Grady Creek generate more runoff than the other gaged tributaries, which are of nearly equal area (Table 3); this will be further assessed with gaging data from year-two (WY2011).

The analysis of surface-water and groundwater interaction shows that Miller Creek above Grady Bridge is a losing reach, while tributaries Grady Creek and S4 are gain water from groundwater discharge. Miller Creek below Grady Bridge also appears to be a gaining reach. Results of water quality sampling and a general mineral analysis suggests that flow emanates primarily from the alluvial aquifer rather than deeper bedrock sources, with the possible exception of contributions from deeper bedrock groundwater at the mouth Grady Creek (which would need re-testing to confirm).

¹⁴ Perennial flows were observed on bedrock canyon channel of Grady Creek, Landmark Creek and N1 Tributary. Tributary watershed south of Lucas Valley Road were on off site private property and not investigated.

Groundwater levels in the valley alluvium fluctuated annually – from wet-season recharge to dry-season recession – 15 to 18 feet across most of the valley. Groundwater flow closely parallels the main streams. Depth to water below stream channels was 9 to 15 feet during late dry season, reflecting a potential for aquifer storage if the high groundwater discharge rates to the incised channels were retarded by restoration. In addition, restoring higher groundwater during the months of spring will result in a more persistent flow through the creek system.

Sediment transport rates were high relative to San Geronimo Creek, particularly bedload transport. Rates were also elevated relative to those in other Marin County streams of similar size and geomorphic location. The elevated rates may be associated with ongoing channel incision, as the watershed is presently well-vegetated, with minimal disruption or bare surfaces to account for the high loads observed. Rates may also be episodically elevated due to the effects of the Dec. 31, 2005 storm, one of the largest during the past century or two in this watershed.

A stream and valley restoration plan proposed on Grady Ranch is related to a proposed digital technology-based entertainment production facility and associated infrastructure planned for the Grady Creek watershed. This report presents findings from year-one of a multi-year pre-construction hydrologic monitoring on Grady Ranch. In addition to data collected and presented in this report, we intend to use data collected from year-two monitoring to establish a conceptual understanding of groundwater flow at the site. A groundwater flow model will then be developed to evaluate the effects of the proposed creek and valley restoration plan on groundwater levels and streamflow persistence.

6. LIMITATIONS

This report was prepared in general accordance with the accepted standard of practice existing in Northern California at the time the investigation was performed. No other warranties, expressed or implied, are made. It should be recognized that interpretation and evaluation of dynamic flow and subsurface conditions is a difficult and inexact art. More extensive studies, including those recommended above, can reduce some of the uncertainties associated with this study.

Balance Hydrologics has prepared this report for the client's exclusive use on this particular groundwater study. Analyses and information included in this report are intended for use at the watershed scale and for the planning purposes described above. Analyses of channels and other water bodies, rocks, earth properties, topography and/or environmental processes are generalized to be useful at the scale of a watershed, both spatially and temporally. Information and interpretations presented in this report should not be applied to specific projects or sites without the expressed written permission of the authors, nor should they be used beyond the particular area to which we have applied them.

This study was conducted partly to help calibrate work done by others, which has not been independently verified. Our conclusions and any implied or inferred recommendations are based on a limited range of surface water and groundwater data in a region of relatively complex geology. They are limited to planning purposes and should not be used for design or site-specific work. Even with these limitations, all work should be cited with the specific cautions listed in the report, given the brief period of record. If readers are aware of additional data, observations, conditions, or forthcoming changes to the bases of our computations or conclusions, please let us know at the first opportunity, such that this report may be promptly revised. Contacts and responsible individuals are given, such that such notifications can occur easily and quickly.

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TABLES

Table 1. Gaging station and monitoring well descriptors, Grady Ranch, Marin County, California.

	Gaging Stations					Monitoring Wells				
	Landmark Creek	S3 Tributary	Miller Creek above Grady Bridge	Grady Creek	S4 Tributary	MW-1	MW-2	MW-3	MW-4	RW-6
ID	N3	S3	M1	N4	S4	--	--	--	--	--
Latitude, WGS84 (degrees N)	38.04357	38.04049	38.03958	38.04052	38.03871	38.04121	38.04002	38.03901	38.04049	38.03878
Longitude, WGS84 (degrees W)	122.60661	122.60454	122.60253	122.60103	122.60323	122.60544	122.60269	122.6015	122.60134	122.60247
Date installed	11/16/2009	11/16/2009	11/4/2009	11/4/2009	11/16/2009	10/20/2009	10/20/2009	10/21/2009	10/21/2009	8/24/2009
Watershed area (sq. mi.)	0.4	0.4	2.1	0.4	0.3	--	--	--	--	--
Reference elevation, feet msl (gage height 0.0 or top of well casing)	255.84	227.49	224.01	228.21	207.94	255.61	241.79	236.26	248.99	236.16
Channel bed elevation or ground surface elevation near well, feet msl	262.54	234.24	225.86	235.06	214.54	252.69	238.44	234.27	245.82	233.7
Depth of well seal, feet below ground surface	--	--	--	--	--	17	19	15	15	17.5
Screened interval, feet below ground surface	--	--	--	--	--	20-30	25-35	30-40	20-30	20-52
Bottom of casing, feet below ground surface	--	--	--	--	--	30	35	40	30	52
Bottom of casing elevation, feet msl	--	--	--	--	--	222.69	203.44	194.27	215.82	181.70

Table 2. Summary of field measurements and water quality analyses, Grady Ranch, Marin County, California.

PARAMETER	UNITS	DETECTI ON LIMIT	MCL	Groundwater Samples							Surface Water Samples				
DESCRIPTORS				091021:1110	091021:1352	091104:1632	091104:0957	091104:1150	091104:1709	091117:1630	091119	091202:1300	091117:1600	091117:1700	
Sample I.D.				MW-3	MW-4	MW-1	MW-2	MW-3	MW-4	Test Well B	Test Well B	RW-6	Landmark Creek	Grady Creek	
Site				164-310-15,-17,-19	164-310-15,-17,-19	164-310-15,-17,-19	164-310-15,-17,-20	164-310-15,-17,-21	164-310-15,-17,-22	164-310-15,-17,-20	164-310-15,-17,-20	164-310-15,-17,-22	164-310-15,-17,-22	164-310-15,-17,-22	
Assessors parcel number				N38.0391	N38.04049	N38.04121	N38.04002	N38.0391	N38.04049	N38.041288	N38.041288	N38.038784	N38.043994	N38.042869	
Latitude, WGS84	degrees			W122.60150	W122.60134	W122.60544	W122.60269	W122.60150	W122.60134	W122.604357	W122.604357	W122.602470	W122.606615	W122.598375	
Longitude, WGS84	degrees			234	244	252	239	234	244	291	291	235	284	336	
Elevation, NGVD29	feet			Soil Control	Soil Control	MBAS	MBAS	MBAS	MBAS	MBAS	Analytical Sciences	MBAS	MBAS	MBAS	
Lab used				sr, gp	sr, gp	sr, tb	sr, tb	sr, tb	sr, tb	mw	Forester P&E	mw	mw	mw	
Sample collected by				yes	yes	no	no	no	no	no		no	no	no	
Sample filtering															
FIELD MEASUREMENTS				10/21/09	10/21/09	11/4/09	11/4/09	11/4/09	11/4/09	11/17/09	11/19/09	12/2/09	11/17/09	11/17/09	
Date	MM/DD/YY			11:10	13:52	16:32	9:57	11:50	17:09	16:30		13:00	16:00	17:00	
Time	HH:MM			276 (top),	491	357.4	322.0	306.8							
Specific conductance (@ 25 C°)	umhos/cm			531(bottom)											
Conductance (@ field temp)	umhos/cm			220, 360	401.4	290.3	261.5	250.4							
Temperature	deg C			15, 9	15	15.1	15.2	15.3							
WATER QUALITY INDICATORS				210	170	140	121	122	139	190	170	148	171	163	
Alkalinity (total)	ng/L CaCO ₃	1		160	140						28				
Hardness (total)	ng/L CaCO ₃	5		7.1	6.9	6.8	6.9	7.0	8.2	8.5	8.22	7.3	7.7	8.2	
pH	pH Units	0.1	10.6	450	490	349	317	309	866	1033		407	405	394	
Specific conductance (@ 25 C°)	umhos/cm	1	1600	250	260	243	240	218	518	638		253	250	225	
Total dissolved solids (TDS)	mg/L	10	1000												
GENERAL MINERALS				205	164	140	121	122	139	190	170	148	171	163	
Bicarbonate (as CaCO ₃)	mg/L	10		250	200	171	148	149	170	232	207	181	209	199	
Bicarbonate (as HCO ₃)	mg/L	1		37	33	35	26	26	34	10	9	31	45	50	
Calcium (Ca)	mg/L	0.5		0	0						0				
Carbonate (as CaCO ₃)	mg/L	6	120	0	0						0				
Carbonate (as CO ₃)	mg/L	1	120	27	43	11	12	10	140	194		20	12	10	
Chloride (Cl)	mg/L	1	250	0.31	0						2.1				
Iron (Fe)	mg/L	0.05	0.3	15	14	15	15	14	12	2	1.2	20	14	13	
Magnesium (Mg)	mg/L	0.5		0.19	0.055						0.031				
Manganese (Mn)	mg/L	0.02	0.05	5	6.3	1	1.4	2.1	5	4.4		2.4	0.8	0.8	
Potassium (K)	mg/L	0.5		27	36	16	16	13	120	212	210	22	19	16	
Sodium (Na)	mg/L	0.5		20	26	19	14	11	56	5		22	23	29	
Sulfate (SO ₄)	mg/L	1	250												
TITLE 22 PRIMARY STANDARDS, INORGANIC											1.7				
Aluminum (Al)	mg/L	0.05	1								0				
Antimony (Sb)	mg/L	0.006	0.01								0				
Arsenic (As)	mg/L	0.002	0.010								0				
Barium (Ba)	mg/L	0.1	1								0.068				
Beryllium (Be)	mg/L	0.001	0								0				
Cadmium (Cd)	mg/L	0.001	0.01								0				
Chromium (Cr)	mg/L	0.001	0.05								0.0056				
Fluoride (F)	mg/L	0.1	1	0.24	0.24						2.2				
Mercury (Hg)	mg/L	0.0002	0								0				
Nickel (Ni)	mg/L	0.01	0.1								0				
Nitrate as (NO ₃)	mg/L	1	45	2.6	5.1	1	3	2	1	1	0	0.2	0	1	
Selenium (Se)	mg/L	0.005	0.05								0				
Thallium (Tl)	mg/L	0.001	0								0				
OTHER CONSTITUENTS						0.05	0.06	0.06	0.26	8.38		0.3	0.06	0.14	
Boron (B)	mg/L	0.1													
Copper (Cu)	mg/L	0.05	1	0	0										
Cyanide (CN)	mg/L	0.05									0.16				
Lead (Pb)	mg/L	0.005	0.02												
Sliver (Ag)	mg/L	0.01													
Zinc (Zn)	mg/L	0.05	5	0	0										
Gross Alpha	pCi/L		15												
LAB CHECK				4.38	4.53	3.70	3.26	3.07	8.03	10.00		4.21	4.24	4.28	
Major Cations (Ca+Mg+K+Na)	meq/L	--	--	5.28	5.03	3.51	3.06	2.95	7.90	9.38		3.99	4.24	4.15	
Major Anions (HCO ₃ +CO ₃ +Cl+SO ₄)	meq/L	--	--	0.83	0.90	1.06	1.07	1.04	1.02	1.07		1.06	1.00	1.03	
Ion Balance (Cations/Anions)	--	--	--												
NOTES															
Observer key: sr = Sarah Richmond; gp = Gustavo Porras; tb = Travis Baggett; mw = Mark Woysner															
Lab results: 0 = not detected; blank value = not tested															
MCL = Title 22 Maximum Contaminant Level as of June 12, 2003; the MCL of Lead is the Regulatory Action Level															

**Table 3. Total monthly surface flow in Miller Creek and tributaries,
Grady Ranch, Marin County, California.**

Month	Landmark Creek (N3) (acer-feet)	S3 Tributary (acer-feet)	Miller Creek above Grady Bridge (M1) (acer-feet)	Grady Creek (N4) (acer-feet)	S4 Tributary (acer-feet)	Miller Creek below S4 (M2=M1+N4+S4) ^a (acer-feet)
Water Year 2010						
October	no data	no data	no data	no data	no data	no data
November	no data	no data	0	0	no data	no data
December	1	1	0	0	3	3
January	113	120	709	170	98	977
February	64	61	454	96	67	617
March	34	27	200	71	39	310
April	29	40	257	78	40	375
May	0	0	0	0	0	0
June	0	0	0	0	0	0
July	0	0	0	0	0	0
August	0	0	0	0	0	0
September	0	0	0	0	0	0
Annual	241	249	1620	415	247	2282
Watershed area (miles ²)	0.4	0.4	2.1	0.4	0.3	2.8
Unit discharge (cfs/mile ²)	0.83	0.86	1.1	1.4	1.1	1.1

Note:

a. Flow in Miller Creek below S4 Tributary was calculated as the sum of Miller Creek above Grady Bridge (station M1), Grady Creek (station N4), and S4 Tributary.

Table 4. Discharge intensity and duration in Miller Creek above Grady Bridge, Marin County, California.

Storm Event		Rainfall (inches)	Runoff Volume (ac-ft)	Mean Flow (cfs)	Mean Unit Flow ^a (cfs/sq.mi.)	Sediment Load		Sediment Concentration	
						Suspended (tons)	Bedload (tons)	Suspended (tons/ac-ft)	Bedload (tons/ac-ft)
<i>Miller Creek above Grady Bridge</i>									
24 hours	January 20, 2010	2.38	135	68.3	32.5	323	242	2.39	1.79
72 hours	January 19-21, 2010	5.78	329	55.3	26.3	477	358	1.45	1.09
7 days	January 18-24, 2010	7.84	512	36.9	17.6	532	399	1.04	0.779
10 days	January 18-27, 2010	9.56 ^b	651	32.8	15.6	555	416	0.852	0.639
30 days	January 18 - February 16, 2010	13.20 ^c	959	16.1	7.67	585	439	0.610	0.458
60 days	January 17 - March 17, 2010	18.45 ^d	1343	11.3	5.37	619	464	0.461	0.345
Total flow	January 12 - April 30, 2010	35.65	1620	7.49	3.57	639	480	0.395	0.296
	Water Year 2010	35.65	1620	2.24	1.07	639	480	0.395	0.296
								<i>Multiples of San Geronimo Creek =</i>	
								<i>5.1</i>	
								<i>64</i>	
<i>San Geronimo Creek at Lagunitas Road</i>									
Total flow	Water Year 2010	50.03	9070	12.50	1.44	706	42	0.078	0.005
								<i>Percent of Miller Creek above Grady Bridge =</i>	
								<i>20%</i>	
								<i>2%</i>	

Notes;

- a. Watershed area above gaging station is 2.1 square miles above the Miller Creek station and 8.7 square miles above the San Geronimo Creek station.
- b. Rainfall on January 17-26. No rain on January 27.
- c. Rainfall on January 16 - February 13. No rain on February 14-16.
- d. Rainfall on January 16 - March 12. No rain on February 13-17.
- e. Values shown with more than 2 or 3 significant figures are the result of electronic calculations and do not imply increased precision.

FIGURES

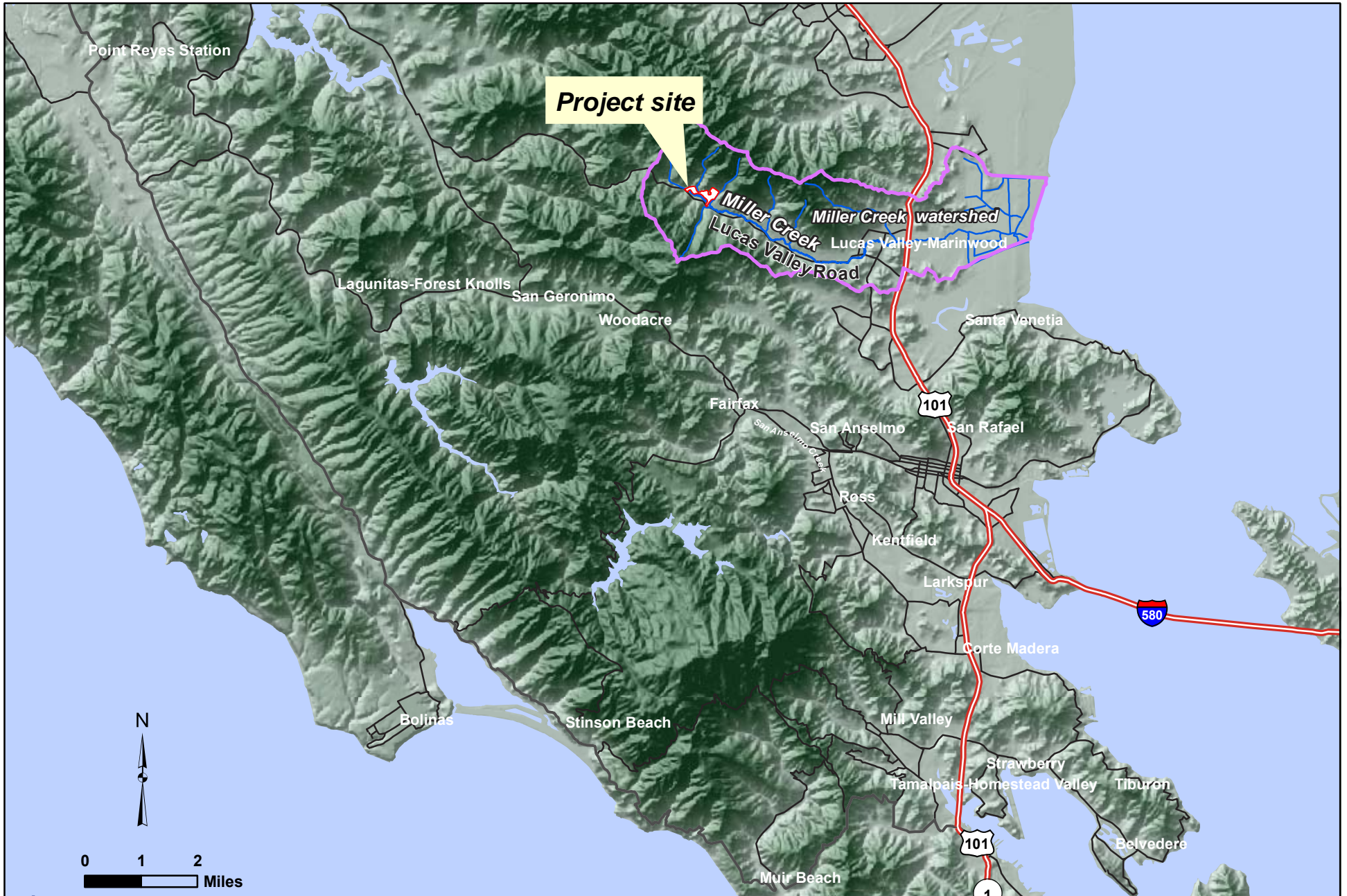


Figure 1. Location of Grady Ranch, Marin County, California

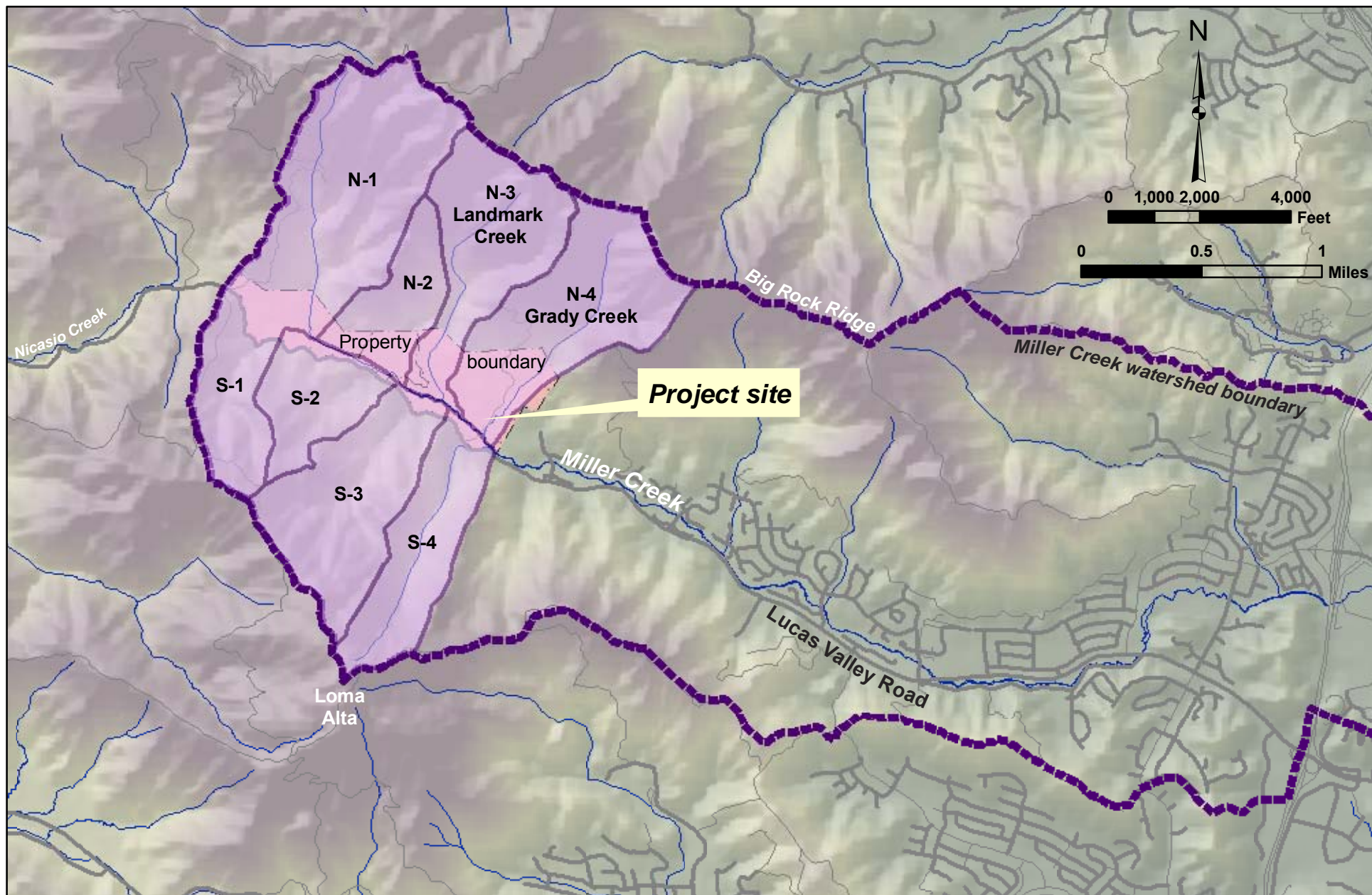


Figure 2. Map of the upper Miller Creek watershed and tributaries, Grady Ranch, Marin County, California.



Aerial photo source: Digital Globe, dated 4-1-09.

Figure 3. Monitoring stations at Grady Ranch, Marin County, California.

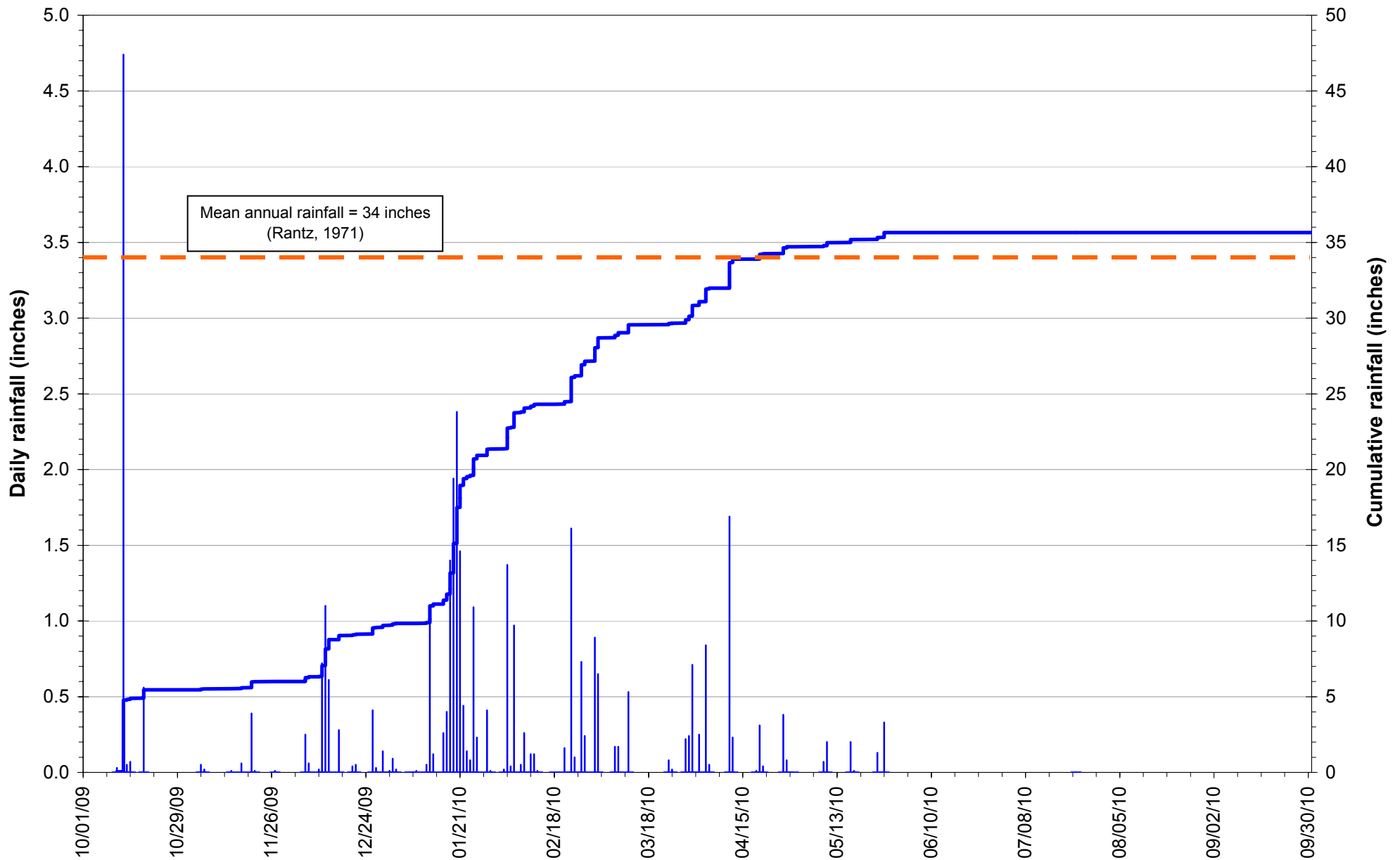
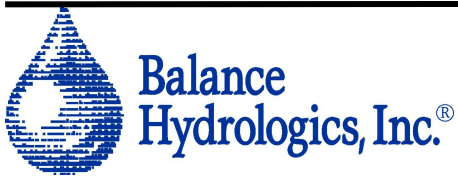


Figure 4. Rainfall at Grady Ranch, water year 2010, Marin County, California. The cumulative rainfall for water year 2010 at Grady Ranch was 35.65 inches, which is approximately 105 percent of the 1906-56 long-term average (Rantz, 1971).



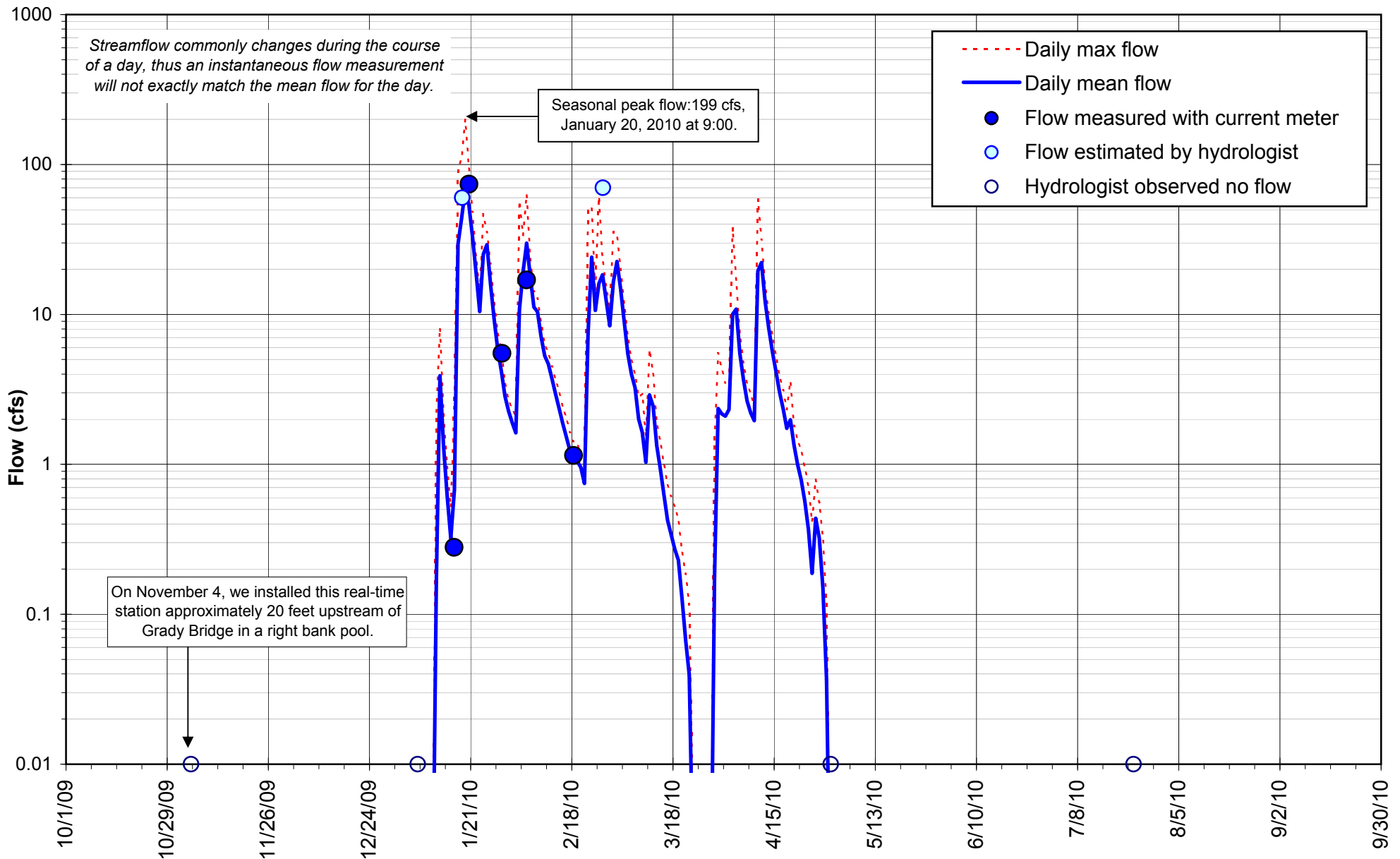
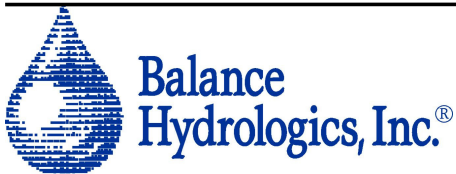


Figure 5. Annual hydrograph for Miller Creek above Grady Bridge, water year 2010, Marin County, California. The channel briefly wetted following the first significant early-season October 13th storm. Flows were continuous from January 12th through March 23rd, and again from March 30th through April 30th. The channel was dry from May 1st through the end of the water year.



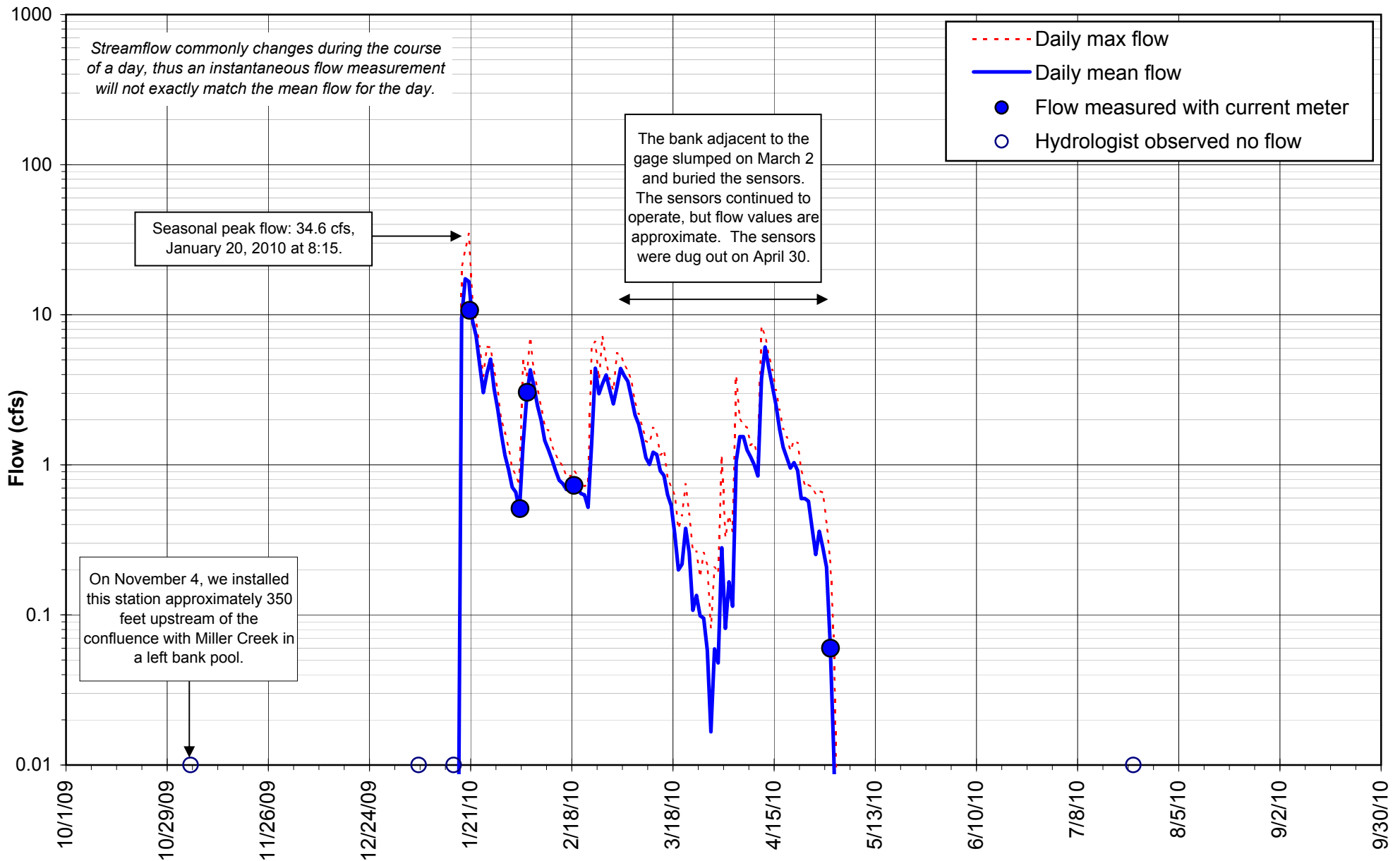
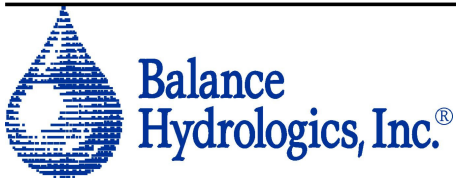


Figure 6. Annual hydrograph for Grady Creek, water year 2010, Marin County, California. The channel briefly wetted following the first significant early-season October 13th storm. Flows were continuous from January 18th through May 1st.



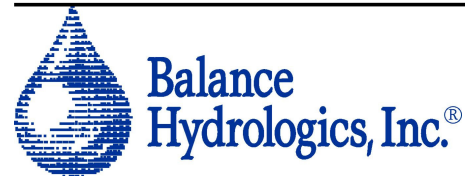
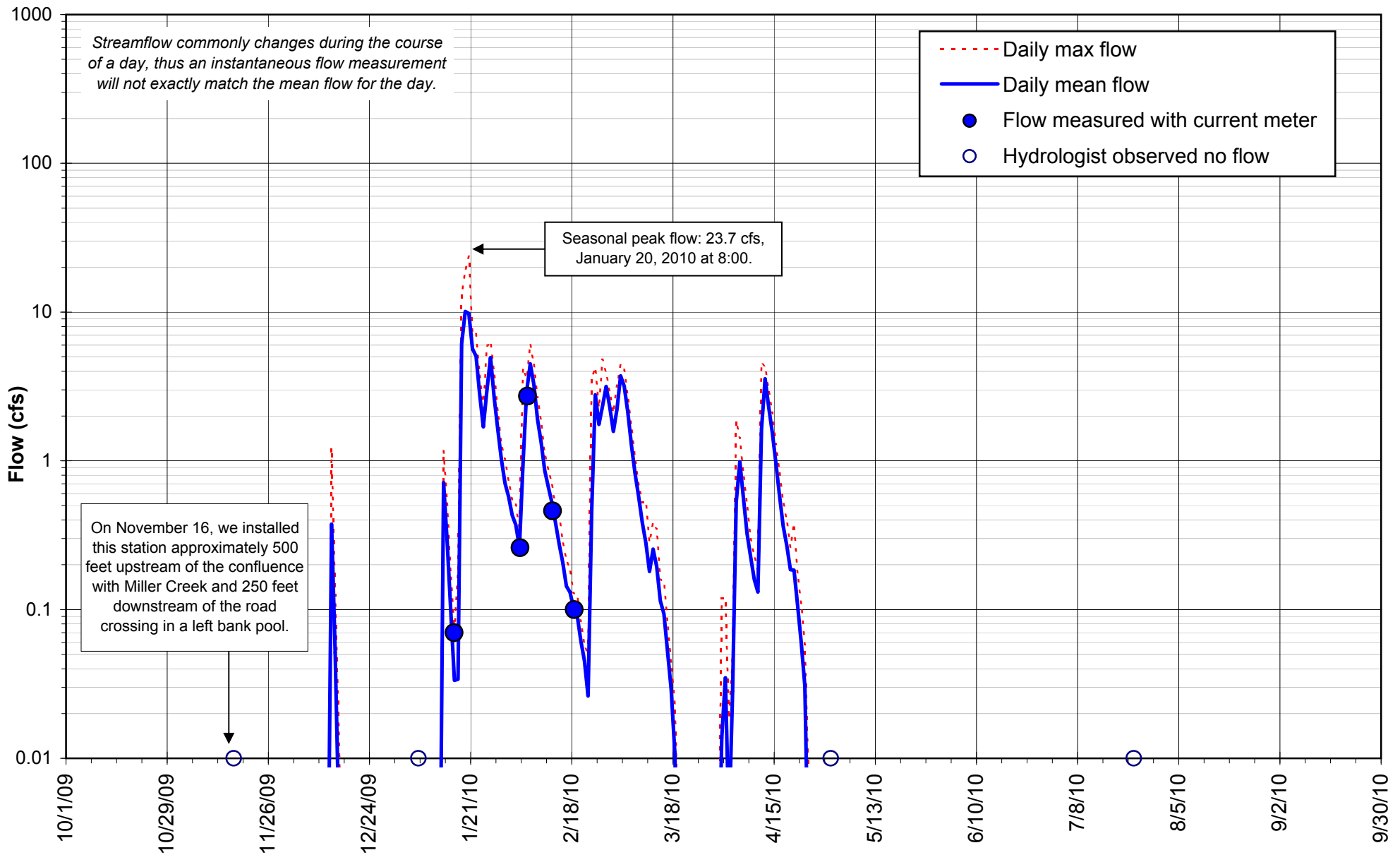


Figure 7. Annual hydrograph for Landmark Creek, water year 2010, Marin County, California. The channel briefly wetted following the first significant early-season October 13th storm. Flows were continuous from January 13th through March 18th, and then from March 31st through April 23rd.

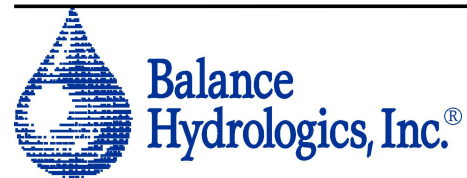
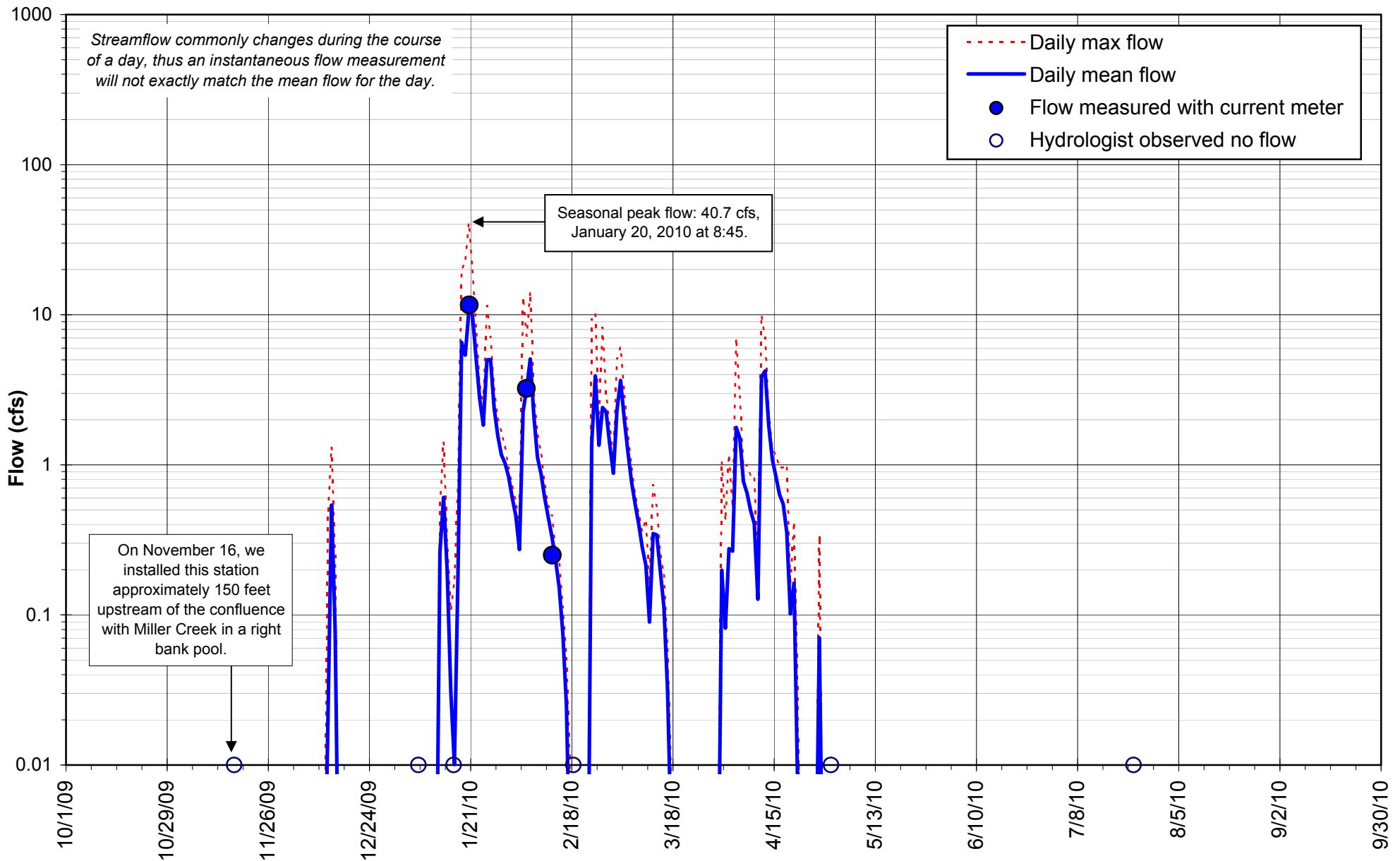


Figure 8. Annual hydrograph for S3 tributary, water year 2010, Marin County, California. The channel briefly wetted following the first significant early-season October 13th storm. Flows periodically dried down throughout the wet season.

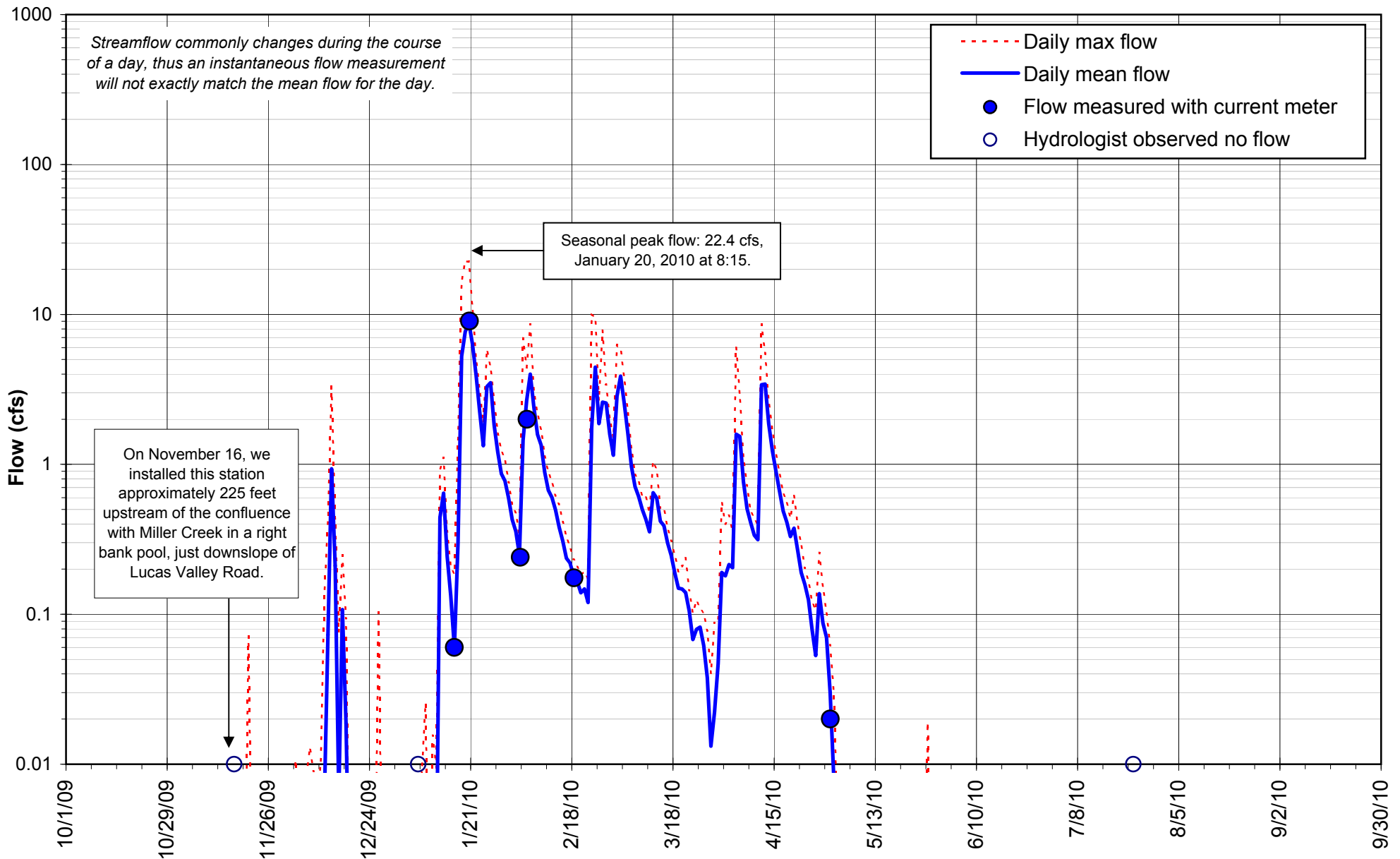
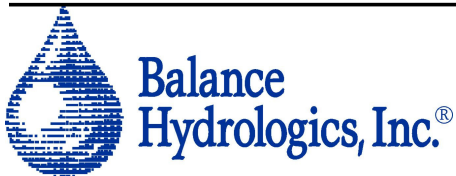


Figure 9. Annual hydrograph for S4 Tributary, water year 2010, Marin County, California. The channel briefly wetted following the first significant early-season October 13th storm. Flows were sustained from January 12th through May 1st.



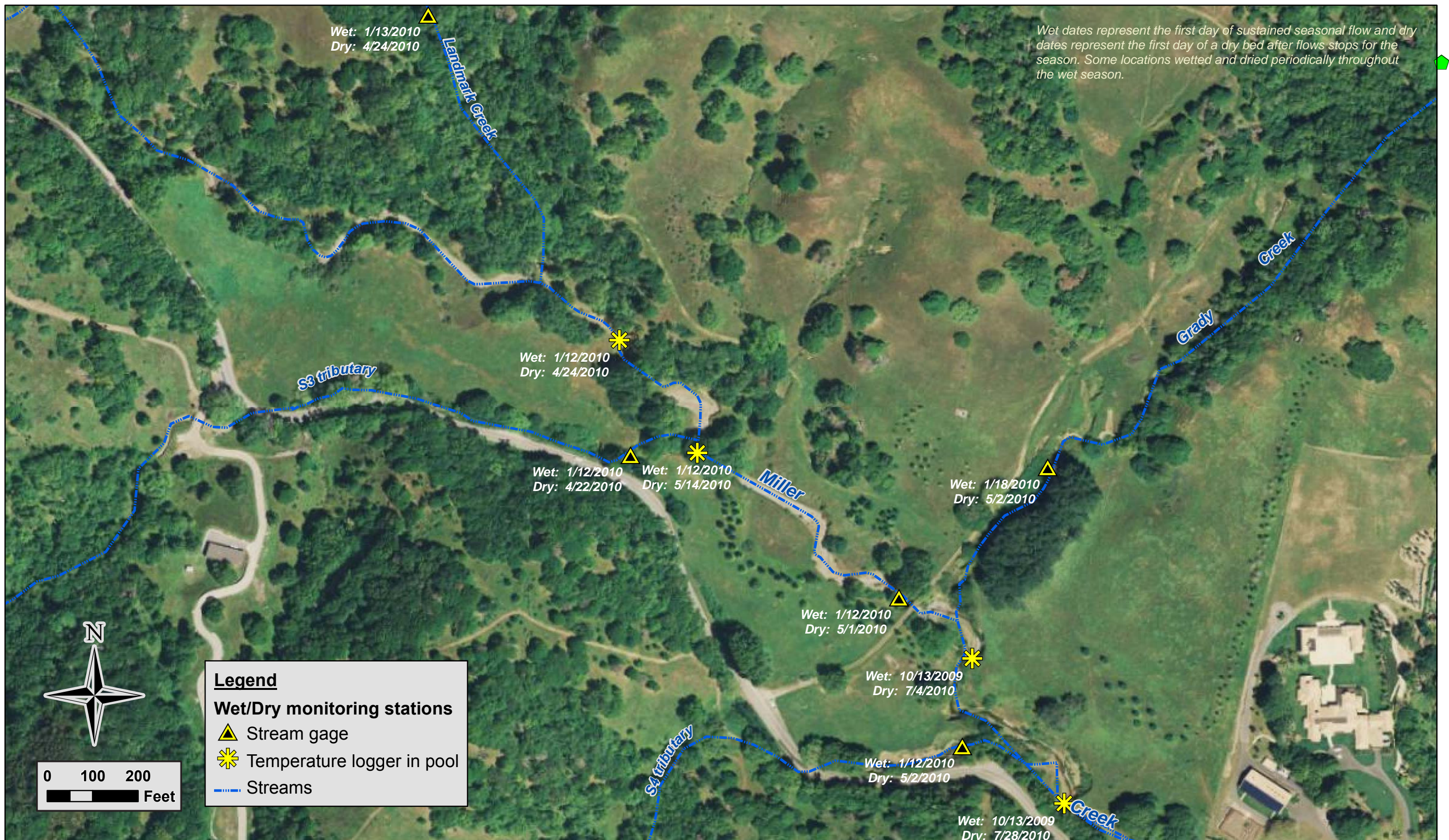


Figure 10. Wet and dry dates for Miller Creek and its tributaries on Grady Ranch, water year 2010, Marin County, California. Miller Creek below Grady Bridge wetted up 3 months before Miller Creek above Grady Bridge and dried down 2 months after Miller Creek above Grady Bridge.

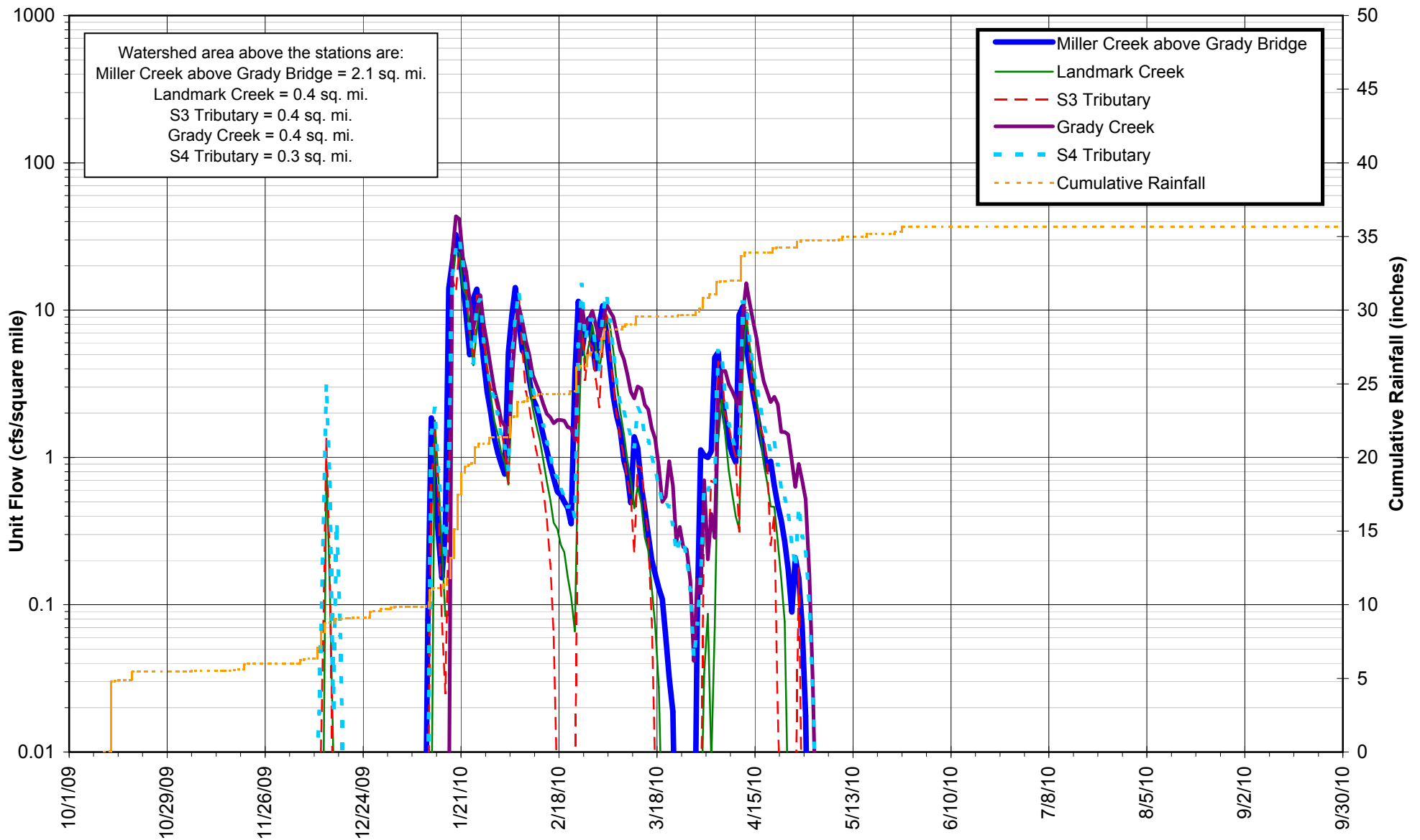
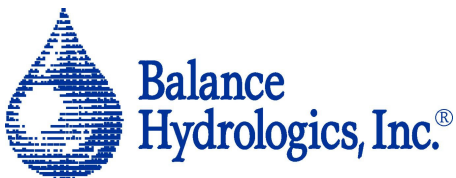


Figure 11. Annual unit hydrographs at Grady Ranch, water year 2010, Marin County, California. Unit flow is calculated by normalizing flow by watershed area. Note that Grady Creek and S4 Tributary have continuous flow throughout the wet season and noticeably higher falling limb flows than other streams in March and April.



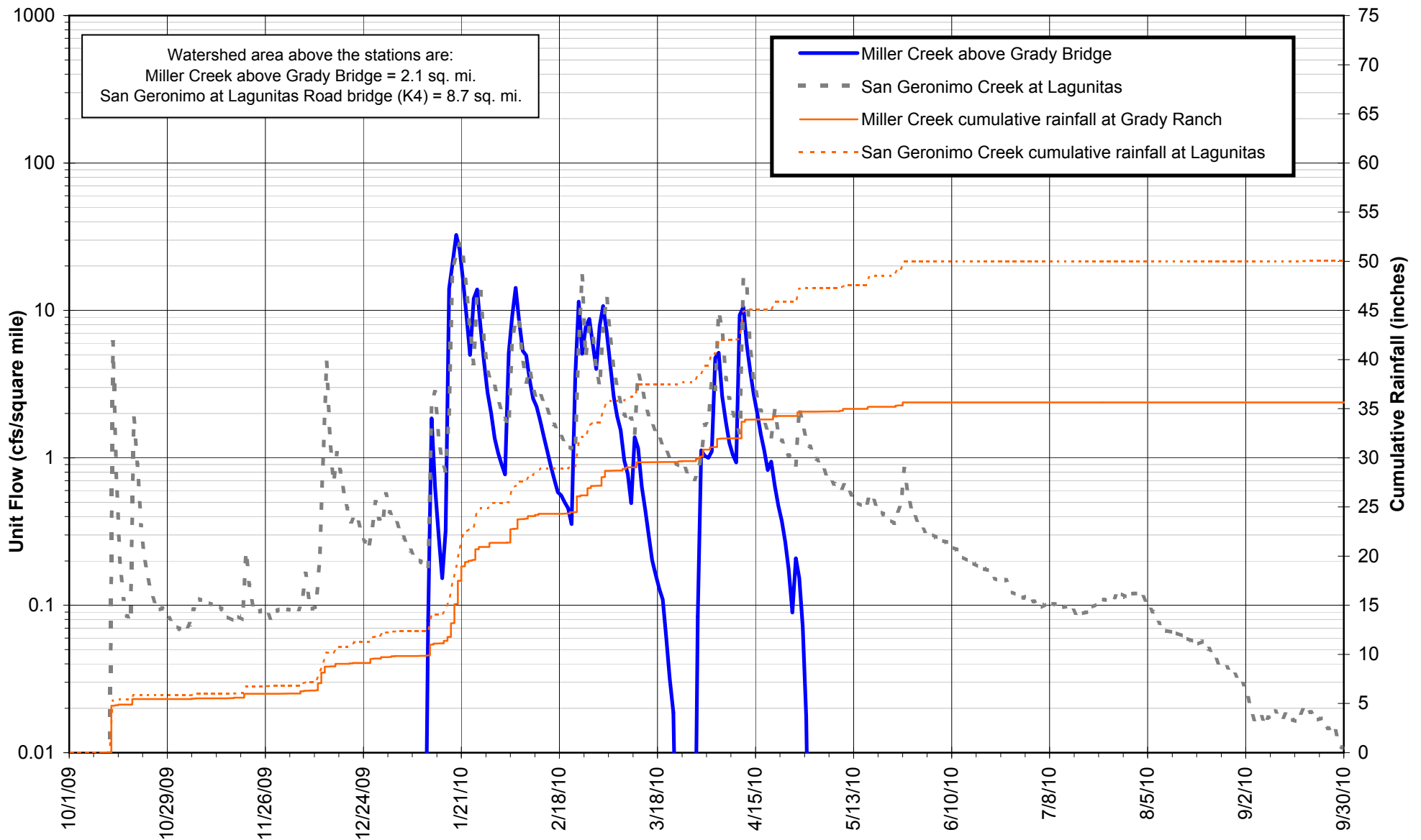
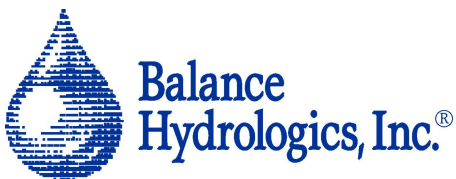


Figure 12. Annual unit hydrographs for Miller Creek and San Geronimo Creek, water year 2010, Marin County, California. The Miller Creek above Grady Bridge station is higher in its watershed with deeper alluvium than the San Geronimo Creek gage. The Miller Creek gage is also at the incision knickpoint which drains groundwater and depletes streamflow. Thus, the Miller Creek station was dry for more than 8 months of the year. In contrast, San Geronimo at Lagunitas Road is a bedrock-dominated channel, which had continuous flow.



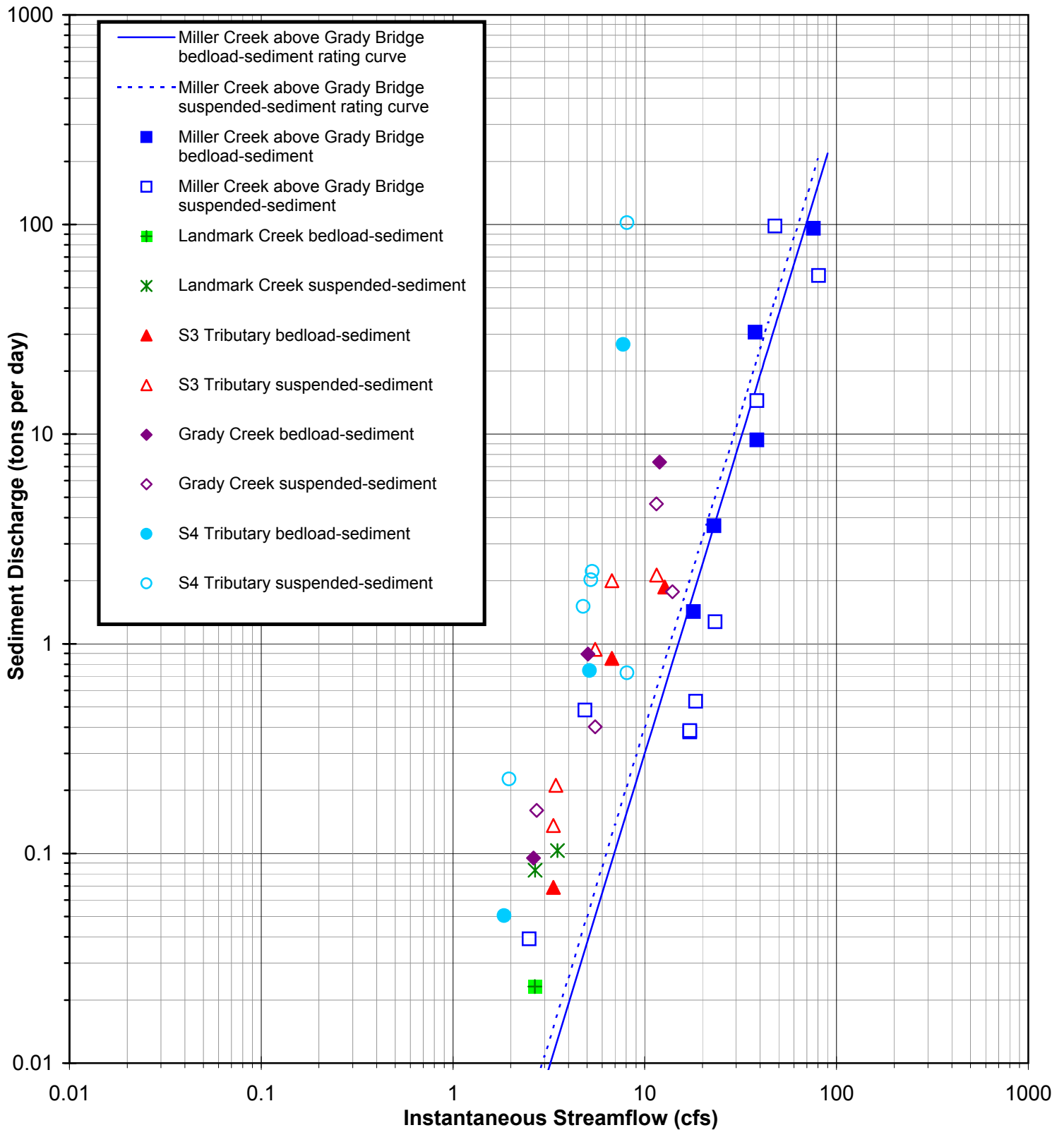
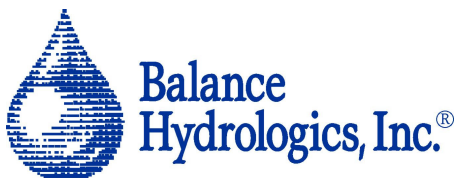


Figure 13. Sediment-discharge measurements for Miller Creek and tributaries water year 2010, Grady Ranch, Marin County, California. These first-year measurements suggest that S4 tributary transports higher rates of sediment than the other tributaries, potentially when upstream banks fail. Preliminary rating curves are illustrated for the Miller Creek station.



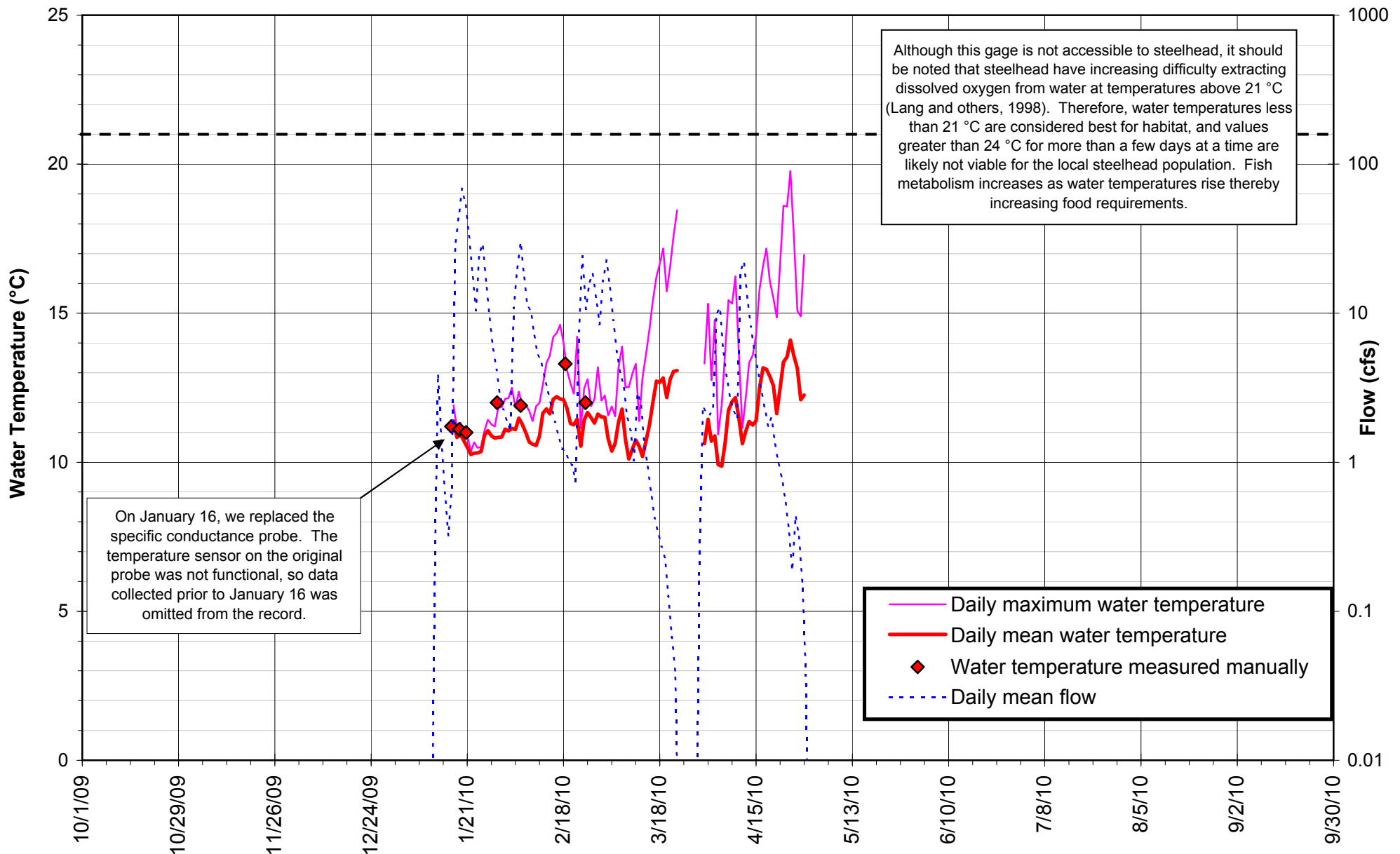
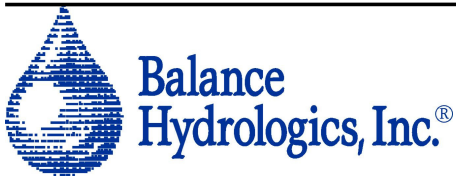


Figure 14. Water temperature at the Miller Creek above Grady Bridge station, water year 2010, Marin County, California. Water temperatures were within the reported acceptable range for steelhead habitat. The flow record is plotted for reference.



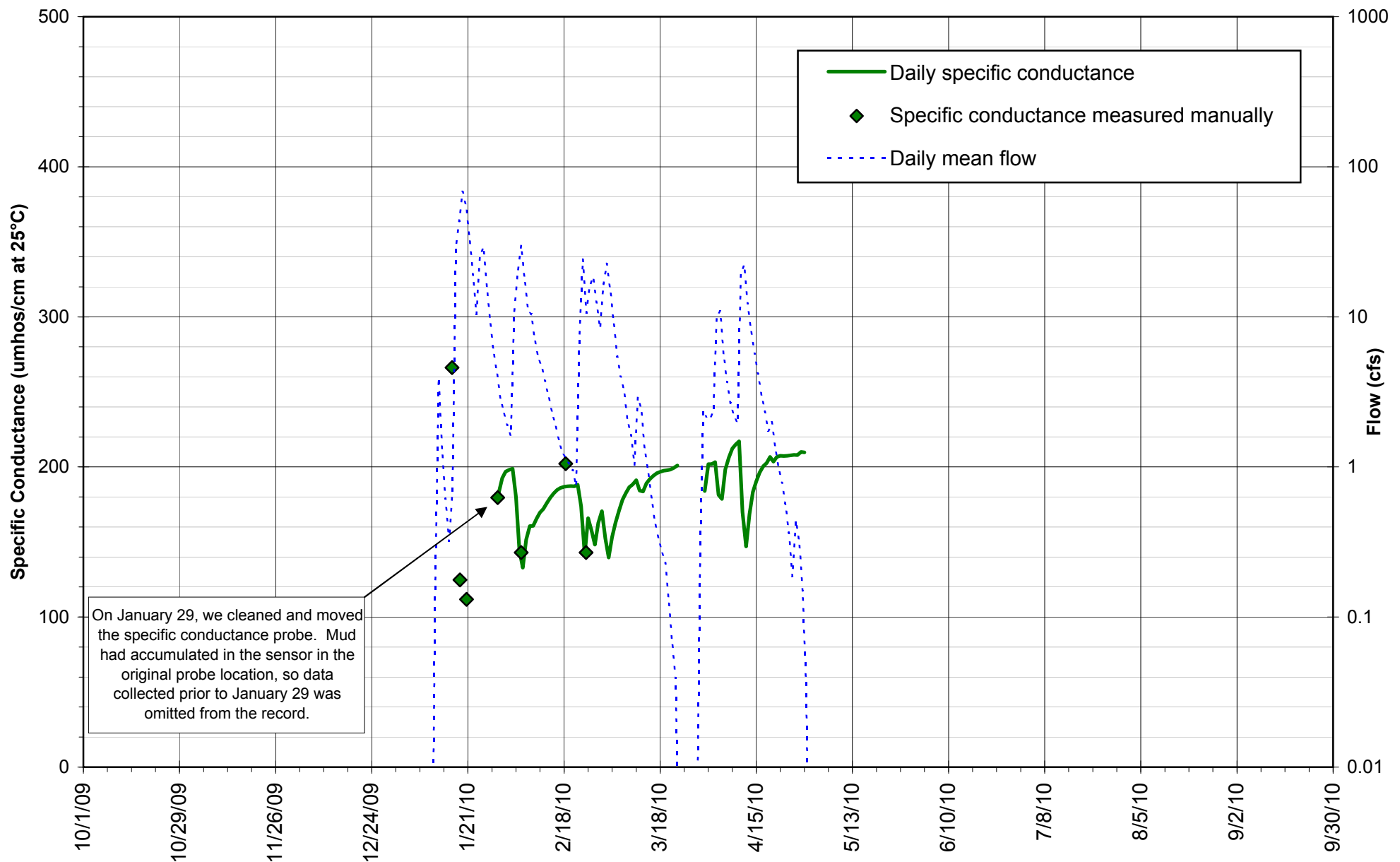
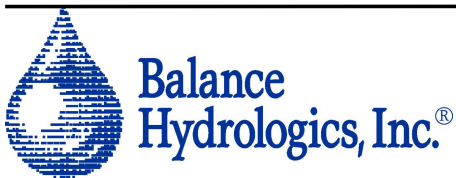
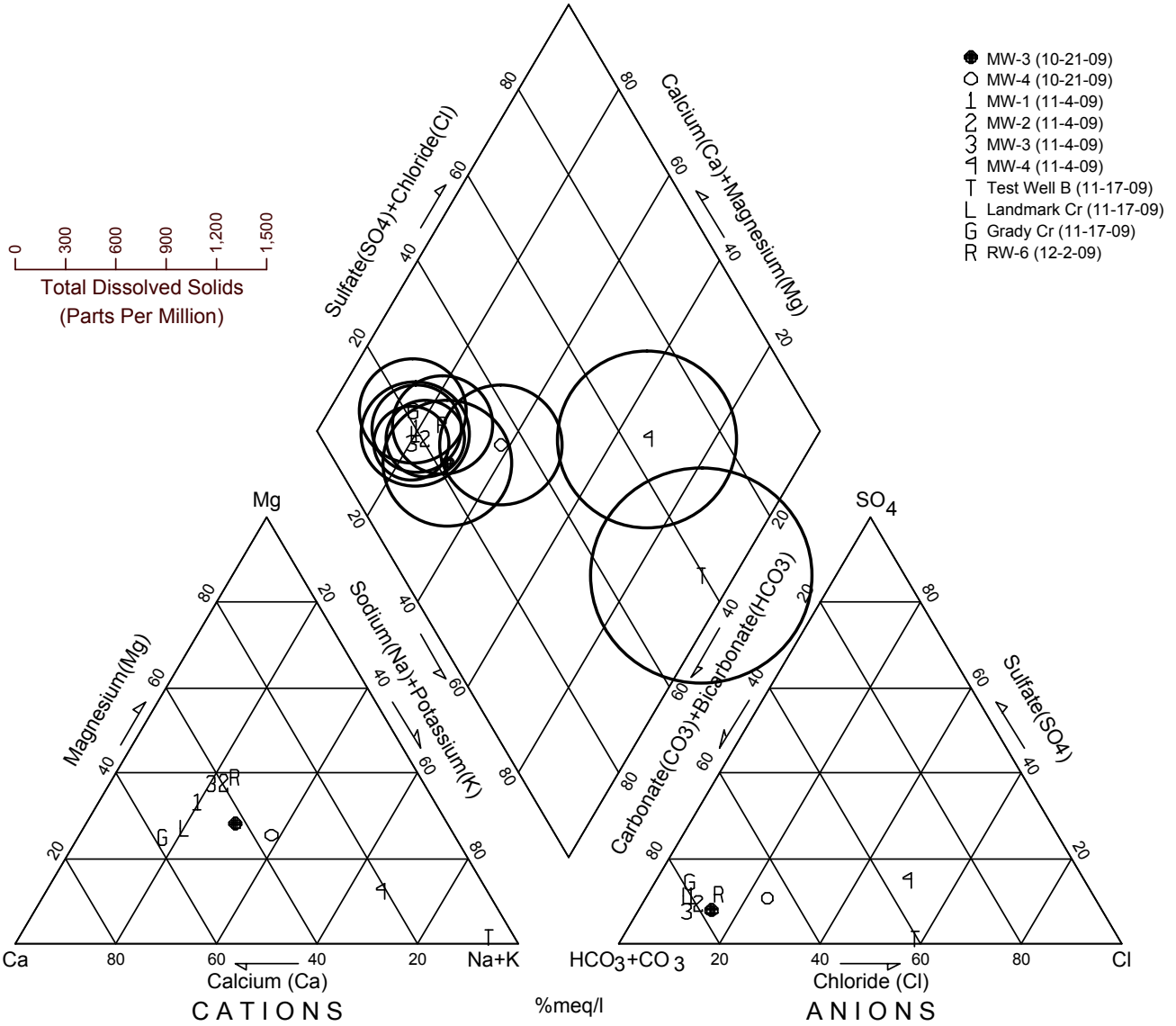


Figure 15. Specific conductance record at the Miller Creek above Grady Bridge station, water year 2010, Marin County, California. Specific conductance during baseflow recession is similar to other central Marin streams, such as San Geronimo Creek, suggesting that flow emanates from the alluvial aquifer rather than deeper bedrock sources. During storms, specific conductance recedes with runoff.



Grady Ranch

Marin County, California



This diagram shows cations in the ternary graph on the left and anions on the right graph. The diamond graph in the center illustrates both cations and anions. Hardness dominated water plots to the left and top of the diamond graph, soft monovalent-salt dominated water to the right, and soft alkaline water towards the bottom. The radius of circle around the plotted points represents the concentration of dissolved solids, calibrated to the scale shown.

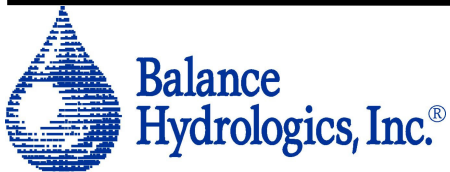


Figure 16. Piper diagram illustrating different ionic signatures of water samples collected from wells and surface waters on Grady Ranch, Marin County, California.

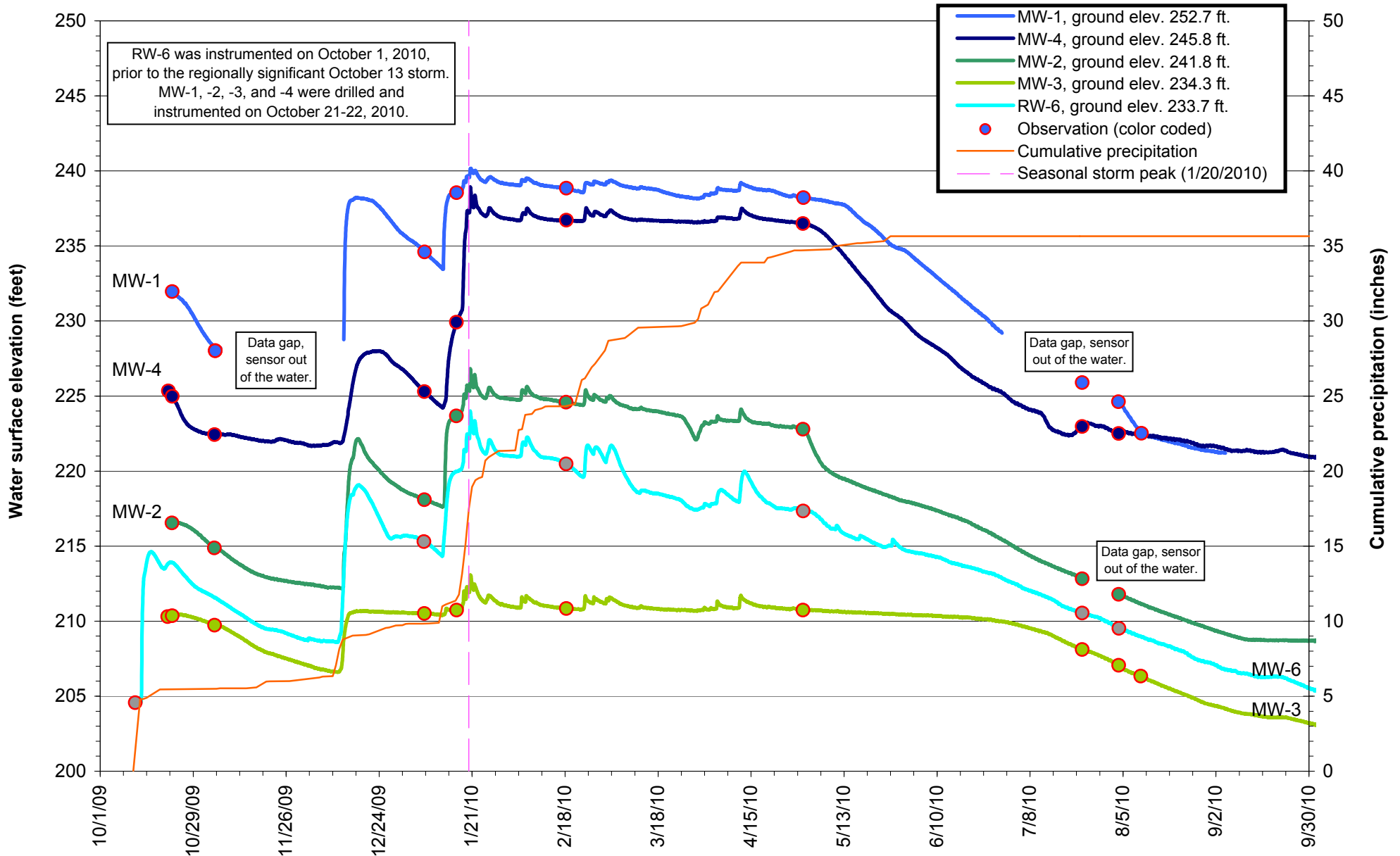
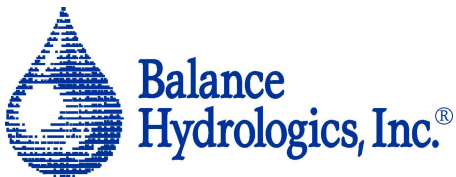


Figure 17. Groundwater elevations at Grady Ranch, water year 2010, Marin County, California.

Water levels for MW-1, -3, and -4 remained relatively stable during the wet season, while MW-2 (closest to Grady Bridge knickpoint) and particularly RW-6 declined over time. RW-6 also showed largest storm rebound (see text for discussion). Dry-season recession began in May for all wells except MW-3, which began in July. MW-3 is overlain by mudflow deposits and shows the least recharge.



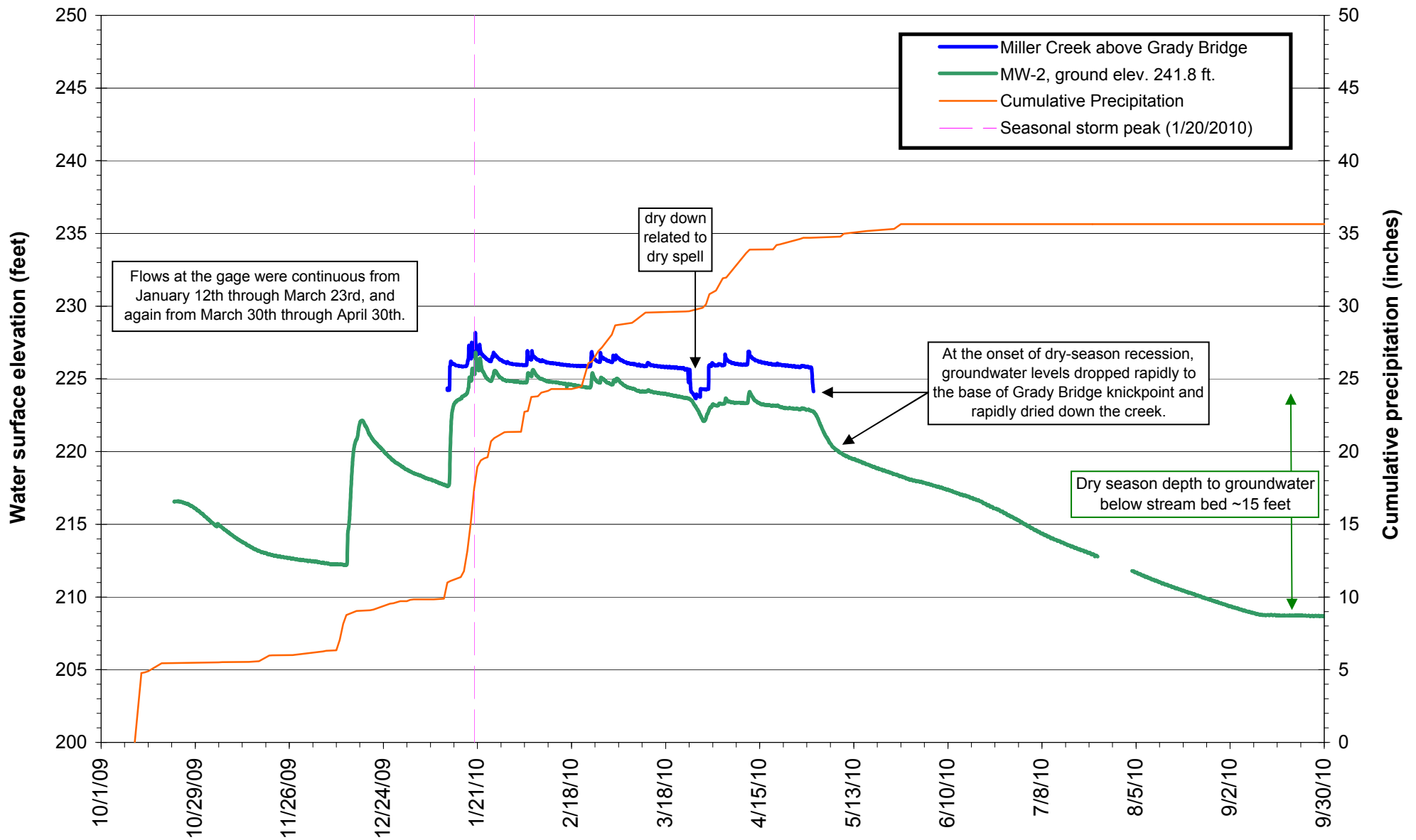
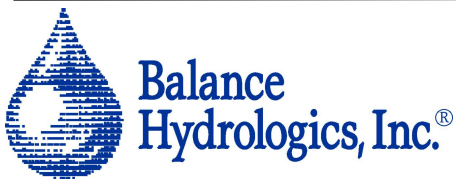


Figure 18. Surface-groundwater interactions on Miller Creek above Grady Bridge, water year 2010, Marin County, California. Water-surface elevations show that the reach above Grady Bridge loses water to the alluvial aquifer. This groundwater recharge keeps groundwater levels high until a mid-winter dry spell or the onset of the dry season when streamflow recedes and relatively rapid groundwater discharge at the foot of the Grady Bridge knickpoint draws down groundwater and dries down the stream.



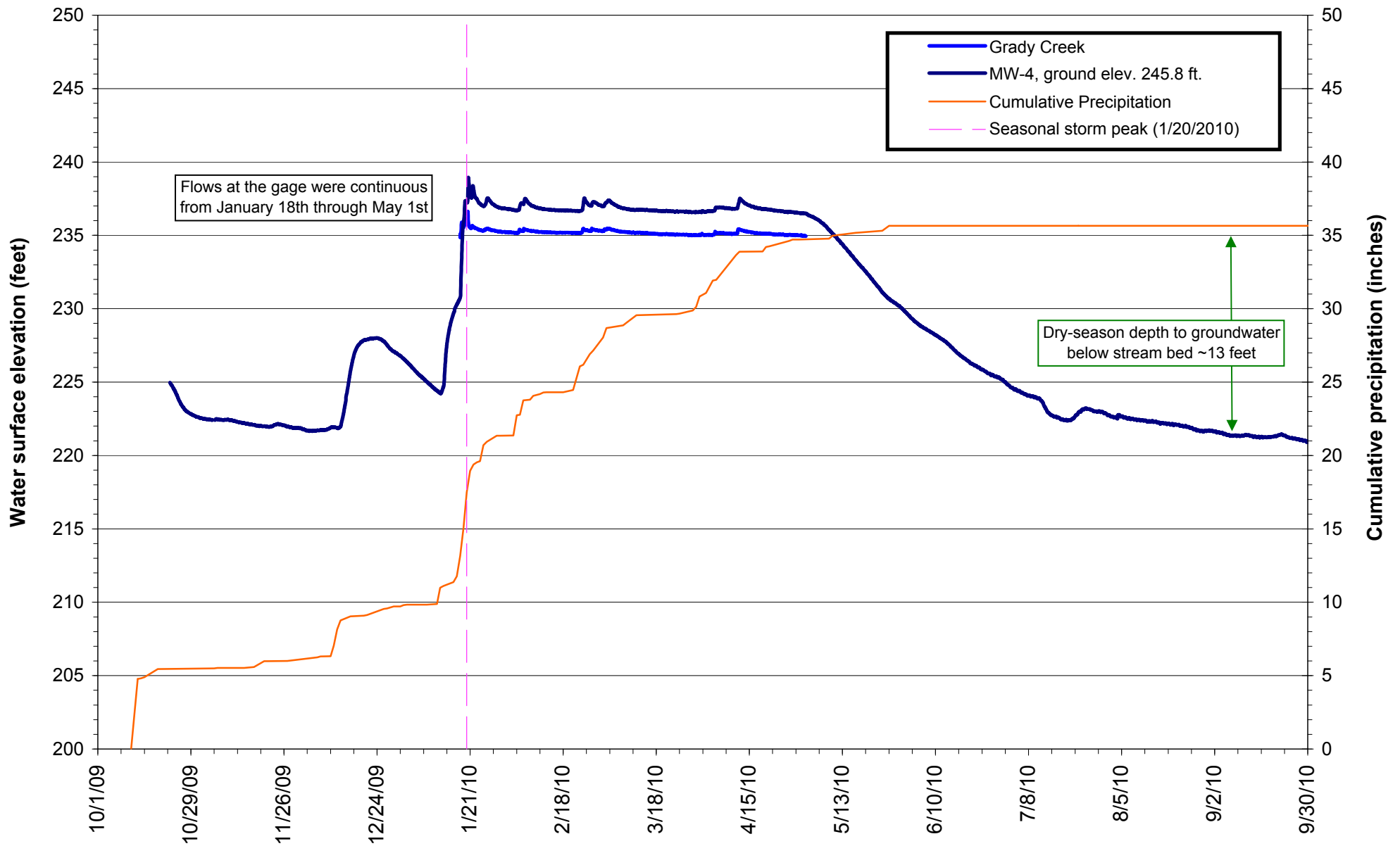
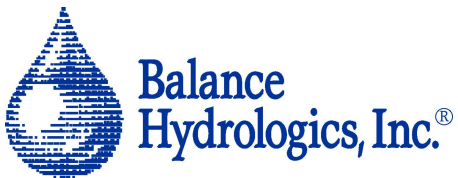


Figure 19. Surface-groundwater interactions on Grady Creek, water year 2010, Marin County, California. Water-surface elevations show that Grady Creek gains water from the aquifer and dries back along with receding groundwater levels.



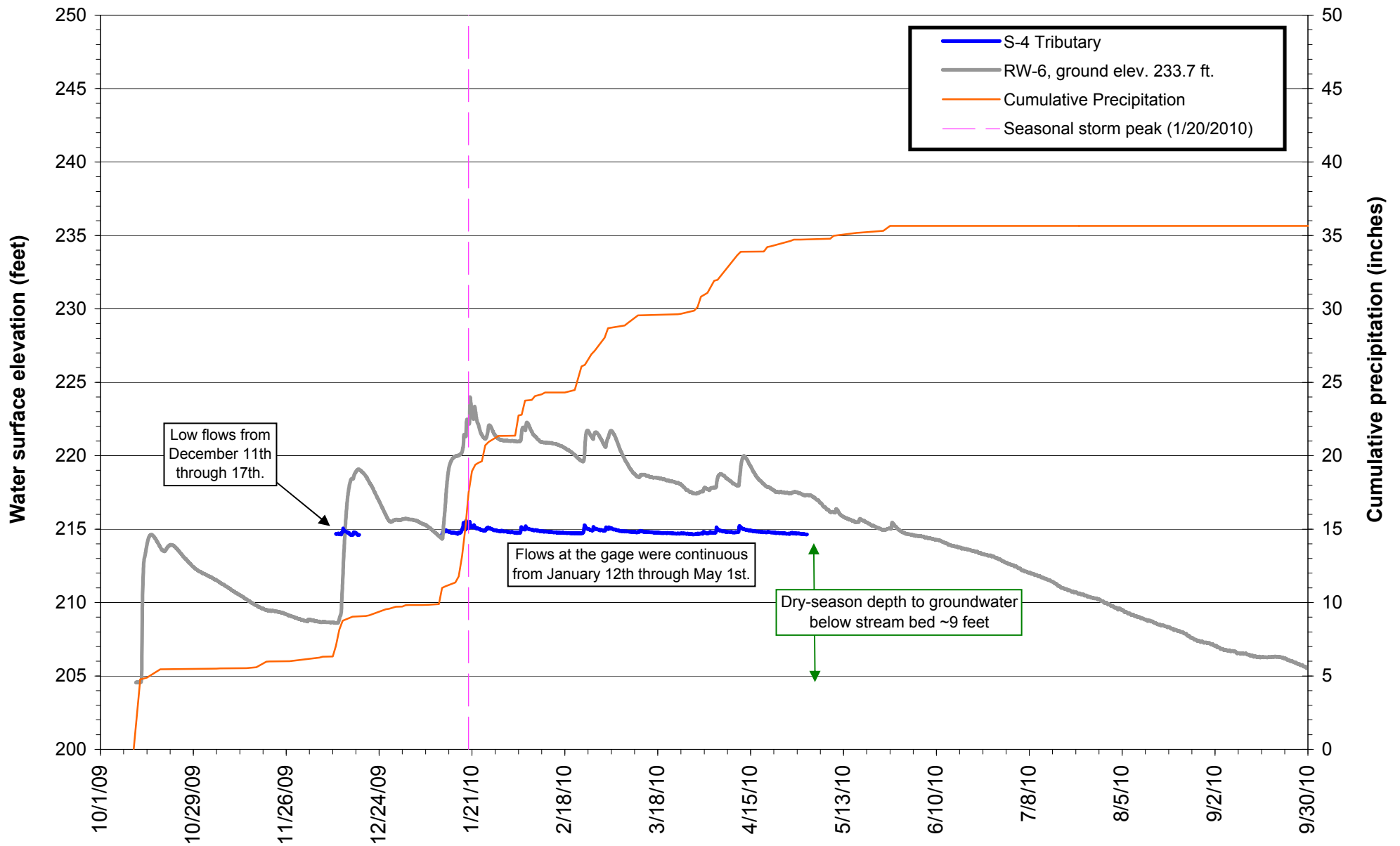
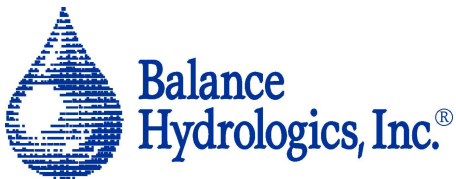


Figure 20. Surface-groundwater interactions on Grady Creek, water year 2010, Marin County, California. Water-surface elevations show that S4 Tributary gains water from the aquifer and dries back along with receding groundwater levels.



FORMS

Form 1. Annual Rainfall Record: Miller Creek Upper Watershed

Station Location / Watershed Descriptors

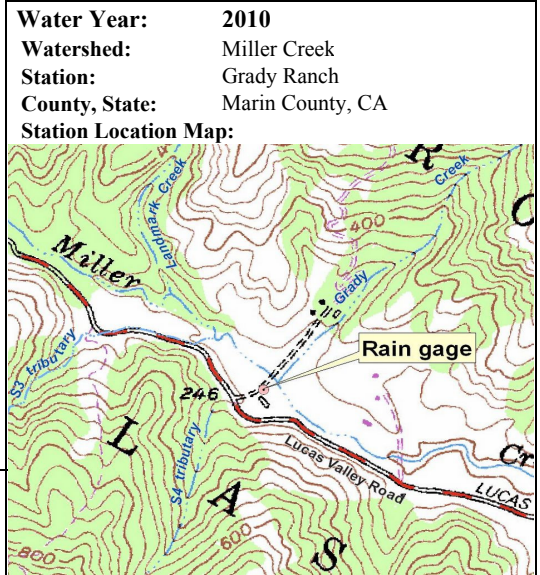
Located on old dairy trough 165 ft northwest of the property gate on access road to Grady Ranch
 Latitude: 38°2'21.24"N, Longitude: 122°36'9.69"W (WGS84)
 Elevation: 237 feet (WGS84)

Period of Record

Tipping-bucket rain gage installed 10/11/2009
 Sponsored by Skywalker Properties, Ltd.

Peak Daily Rainfall (period of record)

Date	Inches	Date	Inches	Date	Inches	Date	Inches
10/13/09	4.74	-	-	-	-	-	-
1/20/10	2.38	-	-	-	-	-	-
-	-	-	-	-	-	-	-



Water Year 2010 Daily Total Rainfall (inches)

DAY	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEPT
1	0.00	0.00	0.00	0.09	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	0.00	0.00	0.00	0.02	0.00	0.89	0.25	0.00	0.00	0.00	0.00	0.00
3	0.00	0.00	0.00	0.00	0.02	0.65	0.00	0.00	0.00	0.00	0.00	0.00
4	0.00	0.00	0.00	0.00	1.37	0.00	0.84	0.00	0.00	0.00	0.00	0.00
5	0.00	0.05	0.00	0.00	0.04	0.00	0.05	0.00	0.00	0.00	0.00	0.00
6	0.00	0.02	0.25	0.00	0.97	0.00	0.00	0.00	0.00	0.00	0.00	0.00
7	0.00	0.00	0.06	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
8	0.00	0.00	0.00	0.01	0.05	0.17	0.00	0.00	0.00	0.00	0.00	0.00
9	0.00	0.00	0.00	0.00	0.26	0.17	0.00	0.07	0.00	0.00	0.00	0.00
10	0.00	0.00	0.02	0.00	0.00	0.00	0.00	0.20	0.00	0.00	0.00	0.00
11	0.03	0.00	0.72	0.05	0.12	0.00	1.69	0.00	0.00	0.00	0.00	0.00
12	0.00	0.00	1.10	1.10	0.12	0.53	0.23	0.00	0.00	0.00	0.00	0.00
13	4.74	0.00	0.61	0.12	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.00
14	0.05	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
15	0.07	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
16	0.00	0.00	0.28	0.26	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
17	0.00	0.06	0.00	0.40	0.00	0.00	0.00	0.20	0.00	0.00	0.00	0.00
18	0.00	0.00	0.00	1.40	0.00	0.00	0.00	0.01	0.00	0.00	0.00	0.00
19	0.56	0.00	0.00	1.94	0.00	0.00	0.01	0.00	0.00	0.00	0.00	0.00
20	0.00	0.39	0.04	2.38	0.00	0.00	0.31	0.00	0.00	0.00	0.00	0.00
21	0.00	0.01	0.05	1.46	0.16	0.00	0.04	0.00	0.00	0.00	0.00	0.00
22	0.00	0.00	0.00	0.44	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
23	0.00	0.00	0.00	0.14	1.61	0.00	0.00	0.00	0.00	0.00	0.00	0.00
24	0.00	0.00	0.00	0.08	0.10	0.08	0.00	0.00	0.00	0.00	0.00	0.00
25	0.00	0.00	0.00	1.09	0.00	0.02	0.00	0.13	0.00	0.00	0.00	0.00
26	0.00	0.00	0.41	0.23	0.73	0.00	0.00	0.00	0.00	0.00	0.00	0.00
27	0.00	0.01	0.03	0.00	0.24	0.00	0.38	0.33	0.00	0.00	0.00	0.00
28	0.00	0.00	0.00	0.00	0.00	0.00	0.08	0.00	0.00	0.00	0.00	0.00
29	0.00	0.00	0.14	0.41	0.00	0.22	0.00	0.00	0.00	0.00	0.00	0.00
30	0.00	0.00	0.00	0.01	--	0.24	0.00	0.00	0.00	0.00	0.00	0.00
31	0.00	--	0.01	0.00	--	0.71	--	0.00	--	0.00	0.00	--
Total	5.45	0.55	3.72	11.63	5.80	3.68	3.88	0.94	0.00	0.00	0.00	0.00
Max	4.74	0.39	1.10	2.38	1.61	0.89	1.69	0.33	0.00	0.00	0.00	0.00

Water Year 2010		
Total Annual	35.65	(inches)
Maximum Daily Total	4.74	(inches)

Balance Hydrologics, Inc. 101 Lucas Valley Rd., Suite 229, San Rafael, CA 94903 (415) 472-7584
 Balance Hydrologics, Inc. 800 Bancroft Way, Suite 101, Berkeley, CA 94710 (510) 704-1000; fax: (510) 704-1001; www.balancehydro.com

Form 2. Annual Hydrologic Record: Miller Creek above Grady Bridge

Station Location / Watershed Descriptors
 Approximately 20 feet upstream Grady Bridge (right bank)
 Latitude: 38°22.58"N, Longitude: 122°36'9.31"W (WGS84)
 Drainage area is 1344 acres or 2.10 square miles.
 Regulation: County open space

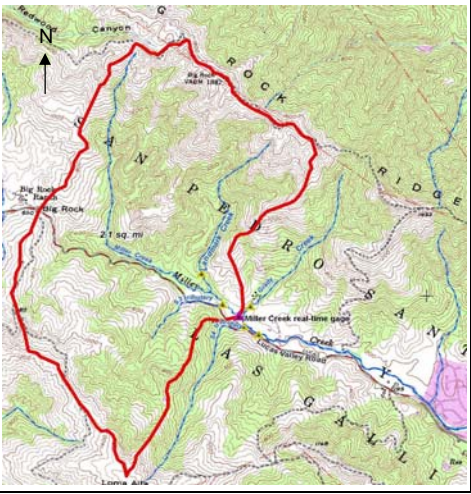
Period of Record
 Gage was installed on 11/4/09 by Balance Hydrologics.
 Record 11/24/09 through 9/30/10.
 Gaging sponsored by Skywalker Properties, Ltd.

Mean Flows
 Monthly mean flows are presented below.

Seasonal Peak Flows (period of record)

Date	Time (24-hr)	Gage Ht. (feet)	Discharge (cfs)	Date	Time (24-hr)	Gage Ht. (feet)	Discharge (cfs)
1/20/10	9:00	4.43	198.80				

Water Year: 2010
Stream: Miller Creek
Station: above Grady Bridge
County, State: Marin County, CA
Station Location Map



Water Year Daily Mean Flow (cubic feet per second)

DAY	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEPT
1	no data	no data	0.00	0.00	2.27	8.41	2.17	0.00	0.00	0.00	0.00	0.00
2	no data	no data	0.00	0.00	1.89	16.53	2.10	0.00	0.00	0.00	0.00	0.00
3	no data	no data	0.00	0.00	1.62	22.53	2.32	0.00	0.00	0.00	0.00	0.00
4	no data	0.00	0.00	0.00	10.77	14.79	10.01	0.00	0.00	0.00	0.00	0.00
5	no data	0.00	0.00	0.00	19.21	8.96	10.86	0.00	0.00	0.00	0.00	0.00
6	no data	0.00	0.00	0.00	29.84	5.46	5.50	0.00	0.00	0.00	0.00	0.00
7	no data	0.00	0.00	0.00	17.89	4.00	3.69	0.00	0.00	0.00	0.00	0.00
8	no data	0.00	0.00	0.00	11.18	3.24	2.66	0.00	0.00	0.00	0.00	0.00
9	no data	0.00	0.00	0.00	10.40	2.01	2.21	0.00	0.00	0.00	0.00	0.00
10	no data	0.00	0.00	0.00	7.08	1.63	1.95	0.00	0.00	0.00	0.00	0.00
11	no data	0.00	0.00	0.00	5.29	1.03	19.47	0.00	0.00	0.00	0.00	0.00
12	no data	0.00	0.00	0.17	4.68	2.90	22.15	0.00	0.00	0.00	0.00	0.00
13	no data	0.00	0.00	3.90	3.73	2.44	12.26	0.00	0.00	0.00	0.00	0.00
14	no data	0.00	0.00	1.32	2.94	1.33	8.10	0.00	0.00	0.00	0.00	0.00
15	no data	0.00	0.00	0.62	2.35	0.93	5.67	0.00	0.00	0.00	0.00	0.00
16	no data	0.00	0.00	0.32	1.87	0.62	4.16	0.00	0.00	0.00	0.00	0.00
17	no data	0.00	0.00	0.67	1.52	0.42	3.03	0.00	0.00	0.00	0.00	0.00
18	no data	0.00	0.00	29.47	1.23	0.34	2.34	0.00	0.00	0.00	0.00	0.00
19	no data	0.00	0.00	43.86	1.17	0.27	1.74	0.00	0.00	0.00	0.00	0.00
20	no data	0.00	0.00	68.27	1.05	0.23	1.98	0.00	0.00	0.00	0.00	0.00
21	no data	0.00	0.00	53.87	0.95	0.13	1.34	0.00	0.00	0.00	0.00	0.00
22	no data	0.00	0.00	33.13	0.74	0.07	0.99	0.00	0.00	0.00	0.00	0.00
23	no data	0.00	0.00	18.94	7.55	0.04	0.78	0.00	0.00	0.00	0.00	0.00
24	no data	0.00	0.00	10.46	24.08	0.00	0.56	0.00	0.00	0.00	0.00	0.00
25	no data	0.00	0.00	25.21	10.65	0.00	0.37	0.00	0.00	0.00	0.00	0.00
26	no data	0.00	0.00	29.12	16.09	0.00	0.19	0.00	0.00	0.00	0.00	0.00
27	no data	0.00	0.00	15.88	18.41	0.00	0.44	0.00	0.00	0.00	0.00	0.00
28	no data	0.00	0.00	9.29	12.43	0.00	0.32	0.00	0.00	0.00	0.00	0.00
29	no data	0.00	0.00	5.82	...	0.00	0.15	0.00	0.00	0.00	0.00	0.00
30	no data	0.00	0.00	4.18	...	0.18	0.04	0.00	0.00	0.00	0.00	0.00
31	no data	...	0.00	2.85	...	2.36	...	0.00	...	0.00	0.00	...
MEAN	NA	0.00	0.00	11.53	8.17	3.25	4.32	0.00	0.00	0.00	0.00	0.00
MAX. DAY	NA	0.00	0.00	68.27	29.84	22.53	22.15	0.00	0.00	0.00	0.00	0.00
MIN. DAY	NA	0.00	0.00	0.00	0.74	0.00	0.04	0.00	0.00	0.00	0.00	0.00
cfs days	NA	0.00	0.00	357.33	228.85	100.87	129.55	0.00	0.00	0.00	0.00	0.00
ac-ft	NA	0.00	0.00	708.77	453.92	200.07	256.96	0.00	0.00	0.00	0.00	0.00

Monitor's Comments

- Multiple stage shifts were applied to the rating equation for local scour or fill.
- Daily values with more than 2 to 3 significant figures result from calculations. No additional precision is implied.
- Minor runoff may have occurred during the rains of mid-October, prior to installation of the gages. We believe such flows are negligible, and have computed mean daily flow for the year without adding any flows for those dates.

Water Year Summary

Mean daily discharge	2.24	(cfs)
Max. daily discharge	68.3	(cfs)
Min. daily discharge	0.00	(cfs)
Total	817	(cfs-days)
Total Volume	1620	(ac-ft)

Form 3. Annual Hydrologic Record: Grady Creek

Station Location / Watershed Descriptors

Approximately 350 feet upstream of the confluence with Miller Creek (left bank)
 Latitude: 38°2'25.49"N, Longitude: 122°36'5.44"W (WGS84)
 Drainage area is 256 acres or 0.40 square miles.
 Regulation: County open space

Period of Record

Gage was installed on 11/4/09 by Balance Hydrologics.
 Preliminary record 11/4/09 through 9/30/10.
 Gaging sponsored by Skywalker Properties, Ltd.

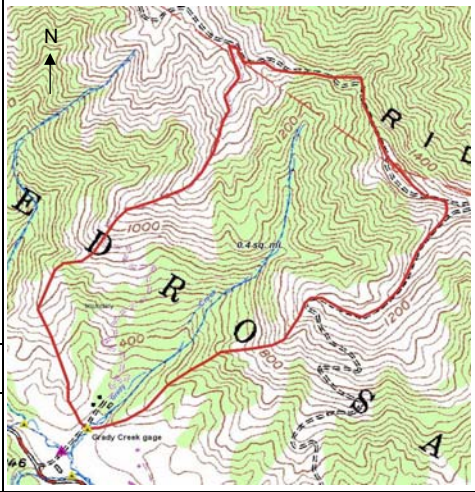
Mean Flows

Monthly mean flows are presented below.

Seasonal Peak Flows (period of record)

Date	Time (24-hr)	Gage Ht. (feet)	Discharge (cfs)	Date	Time (24-hr)	Gage Ht. (feet)	Discharge (cfs)
1/20/10	8:15	8.55	34.60				

Water Year: 2010
Stream: Grady Creek
Station: ...
County, State: Marin County, CA
Station Location Map



Water Year Daily Mean Flow (cubic feet per second)

DAY	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEPT
1	no data	no data	0.00	0.00	0.71	2.55	0.08	0.01	0.00	0.00	0.00	0.00
2	no data	no data	0.00	0.00	0.66	3.30	0.17	0.00	0.00	0.00	0.00	0.00
3	no data	no data	0.00	0.00	0.49	4.39	0.11	0.00	0.00	0.00	0.00	0.00
4	no data	0.00	0.00	0.00	1.37	3.93	1.05	0.00	0.00	0.00	0.00	0.00
5	no data	0.00	0.00	0.00	2.79	3.60	1.54	0.00	0.00	0.00	0.00	0.00
6	no data	0.00	0.00	0.00	4.29	2.81	1.54	0.00	0.00	0.00	0.00	0.00
7	no data	0.00	0.00	0.00	3.34	2.15	1.25	0.00	0.00	0.00	0.00	0.00
8	no data	0.00	0.00	0.00	2.47	1.86	1.13	0.00	0.00	0.00	0.00	0.00
9	no data	0.00	0.00	0.00	1.98	1.48	1.00	0.00	0.00	0.00	0.00	0.00
10	no data	0.00	0.00	0.00	1.45	1.12	0.84	0.00	0.00	0.00	0.00	0.00
11	no data	0.00	0.00	0.00	1.26	1.01	3.75	0.00	0.00	0.00	0.00	0.00
12	no data	0.00	0.00	0.00	1.09	1.21	6.09	0.00	0.00	0.00	0.00	0.00
13	no data	0.00	0.00	0.00	0.92	1.17	4.45	0.00	0.00	0.00	0.00	0.00
14	no data	0.00	0.00	0.00	0.79	0.91	3.34	0.00	0.00	0.00	0.00	0.00
15	no data	0.00	0.00	0.00	0.75	0.85	2.52	0.00	0.00	0.00	0.00	0.00
16	no data	0.00	0.00	0.00	0.68	0.63	1.73	0.00	0.00	0.00	0.00	0.00
17	no data	0.00	0.00	0.00	0.72	0.54	1.31	0.00	0.00	0.00	0.00	0.00
18	no data	0.00	0.00	9.46	0.72	0.34	1.12	0.00	0.00	0.00	0.00	0.00
19	no data	0.00	0.00	17.36	0.71	0.20	0.95	0.00	0.00	0.00	0.00	0.00
20	no data	0.00	0.00	16.64	0.64	0.22	1.03	0.00	0.00	0.00	0.00	0.00
21	no data	0.00	0.00	9.15	0.63	0.38	0.92	0.00	0.00	0.00	0.00	0.00
22	no data	0.00	0.00	7.25	0.52	0.26	0.60	0.00	0.00	0.00	0.00	0.00
23	no data	0.00	0.00	4.64	1.41	0.11	0.60	0.00	0.00	0.00	0.00	0.00
24	no data	0.00	0.00	3.02	4.40	0.14	0.57	0.00	0.00	0.00	0.00	0.00
25	no data	0.00	0.00	4.06	2.98	0.10	0.38	0.00	0.00	0.00	0.00	0.00
26	no data	0.00	0.00	5.05	3.48	0.09	0.25	0.00	0.00	0.00	0.00	0.00
27	no data	0.00	0.00	3.20	3.97	0.06	0.36	0.00	0.00	0.00	0.00	0.00
28	no data	0.00	0.00	2.34	3.17	0.02	0.28	0.00	0.00	0.00	0.00	0.00
29	no data	0.00	0.00	1.60	...	0.06	0.21	0.00	0.00	0.00	0.00	0.00
30	no data	0.00	0.00	1.16	...	0.05	0.07	0.00	0.00	0.00	0.00	0.00
31	no data	...	0.00	0.93	...	0.28	...	0.00	...	0.00	0.00	...
MEAN	NA	0.00	0.00	2.77	1.73	1.15	1.31	0.00	0.00	0.00	0.00	0.00
MAX. DAY	NA	0.00	0.00	17.36	4.40	4.39	6.09	0.01	0.00	0.00	0.00	0.00
MIN. DAY	NA	0.00	0.00	0.00	0.49	0.02	0.07	0.00	0.00	0.00	0.00	0.00
cfs days	NA	0.00	0.00	85.88	48.39	35.79	39.23	0.01	0.00	0.00	0.00	0.00
ac-ft	NA	0.00	0.00	170.35	95.99	70.99	77.82	0.02	0.00	0.00	0.00	0.00

Monitor's Comments

- Multiple stage shifts were applied to the rating equation for local scour or fill.
- Daily values with more than 2 to 3 significant figures result from calculations. No additional precision is implied.
- Minor runoff may have occurred during the rains of mid-October, prior to installation of the gages. We believe such flows are negligible, and have computed mean daily flow for the year without adding any flows for those dates.
- The bank adjacent to the gage slumped on March 2 and buried the sensors. The sensors continued to operate, but flow values are approximate. The sensors were dug out on April 30.

Water Year Summary

Mean daily discharge	0.57	(cfs)
Max. daily discharge	17.4	(cfs)
Min. daily discharge	0.00	(cfs)
Total	209	(cfs-days)
Total Volume	415	(ac-ft)

Form 4. Annual Hydrologic Record: Landmark Creek

Station Location / Watershed Descriptors

Approximately 250 feet downstream road crossing (500 feet upstream Miller Creek)
 Latitude: 38°2'34.59"N, Longitude: 122°36'23.98"W (WGS84)
 Drainage area is 256 acres or 0.40 square miles.
 Regulation: County open space

Period of Record

Gage was installed on 11/16/09 by Balance Hydrologics.
 Preliminary record 11/16/09 through 9/30/10.
 Gaging sponsored by Skywalker Properties, Ltd.

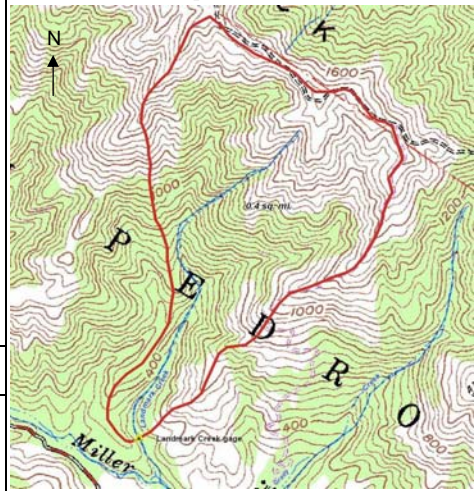
Mean Flows

Monthly mean flows are presented below.

Seasonal Peak Flows (period of record)

Date	Time (24-hr)	Gage Ht. (feet)	Discharge (cfs)	Date	Time (24-hr)	Gage Ht. (feet)	Discharge (cfs)
1/20/10	8:00	8.51	23.65				

Water Year: 2010
Stream: Landmark Creek
Station: ...
County, State: Marin County, CA
Station Location Map



Water Year Daily Mean Flow (cubic feet per second)

DAY	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEPT
1	no data	no data	0.00	0.00	0.43	1.58	0.03	0.00	0.00	0.00	0.00	0.00
2	no data	no data	0.00	0.00	0.37	2.22	0.00	0.00	0.00	0.00	0.00	0.00
3	no data	no data	0.00	0.00	0.26	3.73	0.03	0.00	0.00	0.00	0.00	0.00
4	no data	no data	0.00	0.00	0.97	3.17	0.53	0.00	0.00	0.00	0.00	0.00
5	no data	no data	0.00	0.00	2.94	2.12	0.99	0.00	0.00	0.00	0.00	0.00
6	no data	no data	0.00	0.00	4.49	1.27	0.56	0.00	0.00	0.00	0.00	0.00
7	no data	no data	0.00	0.00	3.26	0.82	0.32	0.00	0.00	0.00	0.00	0.00
8	no data	no data	0.00	0.00	1.90	0.57	0.23	0.00	0.00	0.00	0.00	0.00
9	no data	no data	0.00	0.00	1.33	0.38	0.16	0.00	0.00	0.00	0.00	0.00
10	no data	no data	0.00	0.00	0.86	0.28	0.13	0.00	0.00	0.00	0.00	0.00
11	no data	no data	0.00	0.00	0.67	0.18	1.63	0.00	0.00	0.00	0.00	0.00
12	no data	no data	0.00	0.00	0.52	0.26	3.57	0.00	0.00	0.00	0.00	0.00
13	no data	no data	0.37	0.71	0.38	0.19	2.29	0.00	0.00	0.00	0.00	0.00
14	no data	no data	0.05	0.29	0.28	0.11	1.55	0.00	0.00	0.00	0.00	0.00
15	no data	no data	0.00	0.10	0.21	0.09	0.96	0.00	0.00	0.00	0.00	0.00
16	no data	0.00	0.00	0.03	0.14	0.05	0.58	0.00	0.00	0.00	0.00	0.00
17	no data	0.00	0.00	0.03	0.13	0.03	0.36	0.00	0.00	0.00	0.00	0.00
18	no data	0.00	0.00	6.11	0.10	0.01	0.27	0.00	0.00	0.00	0.00	0.00
19	no data	0.00	0.00	10.08	0.09	0.00	0.19	0.00	0.00	0.00	0.00	0.00
20	no data	0.00	0.00	9.80	0.06	0.00	0.18	0.00	0.00	0.00	0.00	0.00
21	no data	0.00	0.00	5.67	0.05	0.00	0.11	0.00	0.00	0.00	0.00	0.00
22	no data	0.00	0.00	5.04	0.03	0.00	0.06	0.00	0.00	0.00	0.00	0.00
23	no data	0.00	0.00	2.79	0.46	0.00	0.03	0.00	0.00	0.00	0.00	0.00
24	no data	0.00	0.00	1.69	2.78	0.00	0.00	0.00	0.00	0.00	0.00	0.00
25	no data	0.00	0.00	2.98	1.75	0.00	0.00	0.00	0.00	0.00	0.00	0.00
26	no data	0.00	0.00	4.94	2.34	0.00	0.00	0.00	0.00	0.00	0.00	0.00
27	no data	0.00	0.00	2.68	3.16	0.00	0.00	0.00	0.00	0.00	0.00	0.00
28	no data	0.00	0.00	1.63	2.30	0.00	0.00	0.00	0.00	0.00	0.00	0.00
29	no data	0.00	0.00	1.03	...	0.00	0.00	0.00	0.00	0.00	0.00	0.00
30	no data	0.00	0.00	0.71	...	0.00	0.00	0.00	0.00	0.00	0.00	0.00
31	no data	...	0.00	0.57	...	0.01	...	0.00	...	0.00	0.00	...
MEAN	NA	NA	0.01	1.84	1.15	0.55	0.49	0.00	0.00	0.00	0.00	0.00
MAX. DAY	NA	NA	0.37	10.08	4.49	3.73	3.57	0.00	0.00	0.00	0.00	0.00
MIN. DAY	NA	NA	0.00	0.00	0.03	0.00	0.00	0.00	0.00	0.00	0.00	0.00
cfs days	NA	NA	0.46	56.89	32.26	17.11	14.76	0.00	0.00	0.00	0.00	0.00
ac-ft	NA	NA	0.91	112.84	63.98	33.93	29.27	0.00	0.00	0.00	0.00	0.00

Monitor's Comments

1. A stage shift were applied to the rating equation for local fill following the Jan. 20 event.
2. Daily values with more than 2 to 3 significant figures result from calculations. No additional precision is implied.
3. Minor runoff may have occurred during the rains of mid-October, prior to installation of the gages. We believe such flows are negligible, and have computed mean daily flow for the year without adding any flows for those dates.

Water Year Summary

Mean daily discharge	0.33	(cfs)
Max. daily discharge	10.1	(cfs)
Min. daily discharge	0.00	(cfs)
Total	121	(cfs-days)
Total Volume	241	(ac-ft)

Form 5. Annual Hydrologic Record: S3 Tributary

Station Location / Watershed Descriptors

Approximately 150 feet upstream of the confluence with Miller Creek (right bank)
 Latitude: 38°2'25.55"N, Longitude: 122°36'16.66"W (WGS84)
 Drainage area is 256 acres or 0.40 square miles.
 Regulation: County open space

Period of Record

Gage was installed on 11/16/09 by Balance Hydrologics.
 Record 11/16/09 through 9/30/10.
 Gaging sponsored by Skywalker Properties, Ltd.

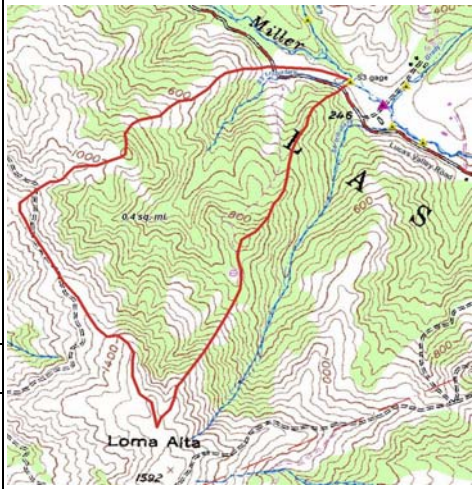
Mean Flows

Monthly mean flows are presented below.

Seasonal Peak Flows (period of record)

Date	Time (24-hr)	Gage Ht. (feet)	Discharge (cfs)	Date	Time (24-hr)	Gage Ht. (feet)	Discharge (cfs)
1/20/10	8:45	8.64	40.67				

Water Year: 2010
Stream: S3 Tributary
Station: below Lucas Valley Road
County, State: Marin County, CA
Station Location Map



Water Year Daily Mean Flow (cubic feet per second)

DAY	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEPT
1	no data	no data	0.00	0.00	0.61	0.88	0.08	0.00	0.00	0.00	0.00	0.00
2	no data	no data	0.00	0.00	0.46	2.31	0.28	0.00	0.00	0.00	0.00	0.00
3	no data	no data	0.00	0.00	0.27	3.65	0.27	0.00	0.00	0.00	0.00	0.00
4	no data	no data	0.00	0.00	2.25	2.04	1.78	0.00	0.00	0.00	0.00	0.00
5	no data	no data	0.00	0.00	3.23	1.23	1.51	0.00	0.00	0.00	0.00	0.00
6	no data	no data	0.00	0.00	5.07	0.78	0.77	0.00	0.00	0.00	0.00	0.00
7	no data	no data	0.00	0.00	2.11	0.55	0.65	0.00	0.00	0.00	0.00	0.00
8	no data	no data	0.00	0.00	1.12	0.40	0.49	0.00	0.00	0.00	0.00	0.00
9	no data	no data	0.00	0.00	0.87	0.28	0.41	0.00	0.00	0.00	0.00	0.00
10	no data	no data	0.00	0.00	0.60	0.21	0.13	0.00	0.00	0.00	0.00	0.00
11	no data	no data	0.00	0.00	0.44	0.09	3.85	0.00	0.00	0.00	0.00	0.00
12	no data	no data	0.02	0.26	0.33	0.35	4.22	0.00	0.00	0.00	0.00	0.00
13	no data	no data	0.54	0.61	0.23	0.34	1.83	0.00	0.00	0.00	0.00	0.00
14	no data	no data	0.08	0.20	0.15	0.20	1.08	0.00	0.00	0.00	0.00	0.00
15	no data	no data	0.00	0.03	0.08	0.11	0.83	0.00	0.00	0.00	0.00	0.00
16	no data	0.00	0.00	0.01	0.03	0.03	0.63	0.00	0.00	0.00	0.00	0.00
17	no data	0.00	0.00	0.19	0.00	0.00	0.55	0.00	0.00	0.00	0.00	0.00
18	no data	0.00	0.00	6.56	0.00	0.00	0.35	0.00	0.00	0.00	0.00	0.00
19	no data	0.00	0.00	5.39	0.00	0.00	0.10	0.00	0.00	0.00	0.00	0.00
20	no data	0.00	0.00	10.54	0.00	0.00	0.16	0.00	0.00	0.00	0.00	0.00
21	no data	0.00	0.00	9.94	0.00	0.00	0.01	0.00	0.00	0.00	0.00	0.00
22	no data	0.00	0.00	5.18	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
23	no data	0.00	0.00	2.75	1.38	0.00	0.00	0.00	0.00	0.00	0.00	0.00
24	no data	0.00	0.00	1.84	3.91	0.00	0.00	0.00	0.00	0.00	0.00	0.00
25	no data	0.00	0.00	5.01	1.35	0.00	0.00	0.00	0.00	0.00	0.00	0.00
26	no data	0.00	0.00	5.02	2.41	0.00	0.00	0.00	0.00	0.00	0.00	0.00
27	no data	0.00	0.00	2.39	2.25	0.00	0.07	0.00	0.00	0.00	0.00	0.00
28	no data	0.00	0.00	1.57	1.36	0.00	0.00	0.00	0.00	0.00	0.00	0.00
29	no data	0.00	0.00	1.17	...	0.00	0.00	0.00	0.00	0.00	0.00	0.00
30	no data	0.00	0.00	1.03	...	0.00	0.00	0.00	0.00	0.00	0.00	0.00
31	no data	...	0.00	0.84	...	0.20	...	0.00	...	0.00	0.00	...
MEAN	NA	NA	0.02	1.95	1.09	0.44	0.67	0.00	0.00	0.00	0.00	0.00
MAX. DAY	NA	NA	0.54	10.54	5.07	3.65	4.22	0.00	0.00	0.00	0.00	0.00
MIN. DAY	NA	NA	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
cfs days	NA	NA	0.66	60.51	30.52	13.66	20.06	0.00	0.00	0.00	0.00	0.00
ac-ft	NA	NA	1.32	120.02	60.53	27.10	39.79	0.00	0.00	0.00	0.00	0.00

Monitor's Comments

- Multiple stage shifts were applied to the rating equation for local scour or fill.
- Daily values with more than 2 to 3 significant figures result from calculations. No additional precision is implied.
- Minor runoff may have occurred during the rains of mid-October, prior to installation of the gages. We believe such flows are negligible, and have computed mean daily flow for the year without adding any flows for those dates.

Water Year Summary

Mean daily discharge	0.34	(cfs)
Max. daily discharge	10.5	(cfs)
Min. daily discharge	0.00	(cfs)
Total	125	(cfs-days)
Total Volume	249	(ac-ft)

Form 6. Annual Hydrologic Record: S4 Tributary

Station Location / Watershed Descriptors

Approximately 225 feet upstream of confluence with Miller Creek (right bank)
 Latitude: 38°2'18.92"N, Longitude: 122°36'9.98"W (WGS84)
 Drainage area is 192 acres or 0.30 square miles.
 Regulation: County open space

Period of Record

Gage was installed on 11/16/09 by Balance Hydrologics.
 Preliminary record 11/16/09 through 9/30/10.
 Gaging sponsored by Skywalker Properties, Ltd.

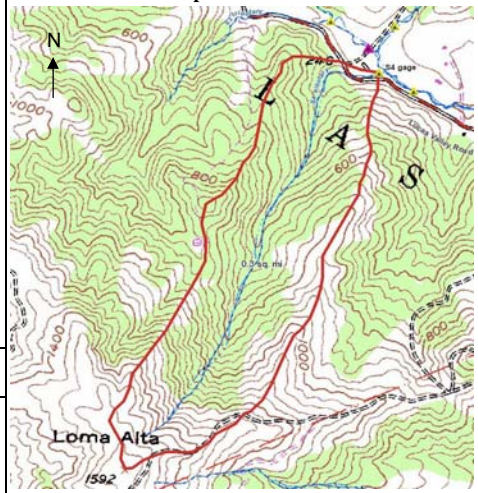
Mean Flows

Monthly mean flows are presented below.

Seasonal Peak Flows (period of record)

Date	Time (24-hr)	Gage Ht. (feet)	Discharge (cfs)	Date	Time (24-hr)	Gage Ht. (feet)	Discharge (cfs)
1/20/10	8:15	7.65	22.43				

Water Year: 2010
Stream: S4 Tributary
Station: below Lucas Valley Road
County, State: Marin County, CA
Station Location Map



Water Year Daily Mean Flow (cubic feet per second)

DAY	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEPT
1	no data	no data	0.00	0.00	0.43	1.15	0.18	0.01	0.00	0.00	0.00	0.00
2	no data	no data	0.00	0.00	0.36	2.85	0.21	0.00	0.00	0.00	0.00	0.00
3	no data	no data	0.00	0.00	0.25	3.88	0.20	0.00	0.00	0.00	0.00	0.00
4	no data	no data	0.00	0.00	1.44	2.56	1.59	0.00	0.00	0.00	0.00	0.00
5	no data	no data	0.00	0.00	2.66	1.62	1.53	0.00	0.00	0.00	0.00	0.00
6	no data	no data	0.00	0.00	4.00	0.97	0.77	0.00	0.00	0.00	0.00	0.00
7	no data	no data	0.00	0.00	2.40	0.71	0.51	0.00	0.00	0.00	0.00	0.00
8	no data	no data	0.00	0.00	1.58	0.61	0.41	0.00	0.00	0.00	0.00	0.00
9	no data	no data	0.00	0.00	1.34	0.50	0.34	0.00	0.00	0.00	0.00	0.00
10	no data	no data	0.00	0.00	0.88	0.43	0.31	0.00	0.00	0.00	0.00	0.00
11	no data	no data	0.01	0.00	0.67	0.35	3.39	0.00	0.00	0.00	0.00	0.00
12	no data	no data	0.07	0.44	0.60	0.65	3.44	0.00	0.00	0.00	0.00	0.00
13	no data	no data	0.93	0.64	0.50	0.59	1.86	0.00	0.00	0.00	0.00	0.00
14	no data	no data	0.21	0.24	0.38	0.42	1.24	0.00	0.00	0.00	0.00	0.00
15	no data	no data	0.01	0.13	0.31	0.39	0.90	0.00	0.00	0.00	0.00	0.00
16	no data	0.00	0.11	0.06	0.24	0.30	0.66	0.00	0.00	0.00	0.00	0.00
17	no data	0.00	0.02	0.30	0.22	0.25	0.49	0.00	0.00	0.00	0.00	0.00
18	no data	0.00	0.00	5.26	0.18	0.19	0.41	0.00	0.00	0.00	0.00	0.00
19	no data	0.00	0.00	7.67	0.17	0.15	0.33	0.00	0.00	0.00	0.00	0.00
20	no data	0.00	0.00	8.82	0.14	0.15	0.38	0.00	0.00	0.00	0.00	0.00
21	no data	0.00	0.00	6.35	0.15	0.14	0.27	0.00	0.00	0.00	0.00	0.00
22	no data	0.00	0.00	3.95	0.12	0.10	0.19	0.00	0.00	0.00	0.00	0.00
23	no data	0.00	0.00	2.25	1.76	0.07	0.16	0.00	0.00	0.00	0.00	0.00
24	no data	0.00	0.00	1.33	4.46	0.08	0.13	0.00	0.00	0.00	0.00	0.00
25	no data	0.00	0.00	3.30	1.87	0.08	0.08	0.00	0.00	0.00	0.00	0.00
26	no data	0.00	0.00	3.50	2.60	0.06	0.05	0.00	0.00	0.00	0.00	0.00
27	no data	0.00	0.00	1.83	2.56	0.04	0.14	0.00	0.00	0.00	0.00	0.00
28	no data	0.00	0.00	1.21	1.59	0.01	0.09	0.00	0.00	0.00	0.00	0.00
29	no data	0.00	0.00	0.86	...	0.02	0.07	0.00	0.00	0.00	0.00	0.00
30	no data	0.00	0.00	0.77	...	0.05	0.03	0.00	0.00	0.00	0.00	0.00
31	no data	...	0.00	0.59	...	0.19	...	0.00	...	0.00	0.00	...
MEAN	NA	NA	0.04	1.60	1.21	0.63	0.68	0.00	0.00	0.00	0.00	0.00
MAX. DAY	NA	NA	0.93	8.82	4.46	3.88	3.44	0.01	0.00	0.00	0.00	0.00
MIN. DAY	NA	NA	0.00	0.00	0.12	0.01	0.03	0.00	0.00	0.00	0.00	0.00
cfs days	NA	NA	1.38	49.51	33.85	19.56	20.35	0.01	0.00	0.00	0.00	0.00
ac-ft	NA	NA	2.74	98.21	67.14	38.79	40.36	0.02	0.00	0.00	0.00	0.00

Monitor's Comments

- Multiple stage shifts were applied to the rating equation for local scour or fill.
- Daily values with more than 2 to 3 significant figures result from calculations. No additional precision is implied.
- Minor runoff may have occurred during the rains of mid-October, prior to installation of the gages. We believe such flows are negligible, and have computed mean daily flow for the year without adding any flows for those dates.

Water Year Summary

Mean daily discharge	0.34	(cfs)
Max. daily discharge	8.82	(cfs)
Min. daily discharge	0.00	(cfs)
Total	125	(cfs-days)
Total Volume	247	(ac-ft)

Water Year: 2010

Stream: Miller Creek

Station: Above Grady Bridge

County: Marin County, CA

Form 7. Annual Sediment-Discharge Record

Daily Suspended-Sediment Discharge (tons)

DAY	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEPT	
1	no data	no data	0.0	0.0	0.0	0.3	0.0	0.0	0.0	0.0	0.0	0.0	
2	no data	no data	0.0	0.0	0.0	3.0	0.0	0.0	0.0	0.0	0.0	0.0	
3	no data	no data	0.0	0.0	0.0	5.3	0.0	0.0	0.0	0.0	0.0	0.0	
4	no data	0.0	0.0	0.0	7.5	1.4	2.7	0.0	0.0	0.0	0.0	0.0	
5	no data	0.0	0.0	0.0	3.5	0.3	0.6	0.0	0.0	0.0	0.0	0.0	
6	no data	0.0	0.0	0.0	15.2	0.1	0.1	0.0	0.0	0.0	0.0	0.0	
7	no data	0.0	0.0	0.0	2.5	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
8	no data	0.0	0.0	0.0	0.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
9	no data	0.0	0.0	0.0	0.5	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
10	no data	0.0	0.0	0.0	0.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
11	no data	0.0	0.0	0.0	0.1	0.0	11.2	0.0	0.0	0.0	0.0	0.0	
12	no data	0.0	0.0	0.0	0.0	0.0	4.8	0.0	0.0	0.0	0.0	0.0	
13	no data	0.0	0.0	0.0	0.0	0.0	0.8	0.0	0.0	0.0	0.0	0.0	
14	no data	0.0	0.0	0.0	0.0	0.0	0.2	0.0	0.0	0.0	0.0	0.0	
15	no data	0.0	0.0	0.0	0.0	0.0	0.1	0.0	0.0	0.0	0.0	0.0	
16	no data	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
17	no data	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
18	no data	0.0	0.0	34.7	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
19	no data	0.0	0.0	68.4	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
20	no data	0.0	0.0	323	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
21	no data	0.0	0.0	85.7	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
22	no data	0.0	0.0	16.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
23	no data	0.0	0.0	3.0	4.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
24	no data	0.0	0.0	0.5	9.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
25	no data	0.0	0.0	11	0.5	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
26	no data	0.0	0.0	10.6	5.8	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
27	no data	0.0	0.0	1.8	2.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
28	no data	0.0	0.0	0.4	0.8	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
29	no data	0.0	0.0	0.1	...	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
30	no data	0.0	0.0	0.0	...	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
31	no data	...	0.0	0.0	...	0.0	...	0.0	...	0.0	0.0	...	Annual
TOTAL	0	0	0	555	53	10	21	0	0	0	0	0	639
Max.day	0	0	0	323	15	5	11	0	0	0	0	0	323

Daily Bedload-Sediment Discharge (tons)

DAY	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEPT	
1	no data	no data	0.0	0.0	0.0	0.2	0.0	0.0	0.0	0.0	0.0	0.0	
2	no data	no data	0.0	0.0	0.0	2.3	0.0	0.0	0.0	0.0	0.0	0.0	
3	no data	no data	0.0	0.0	0.0	4.0	0.0	0.0	0.0	0.0	0.0	0.0	
4	no data	0.0	0.0	0.0	5.6	1.1	2.0	0.0	0.0	0.0	0.0	0.0	
5	no data	0.0	0.0	0.0	2.7	0.2	0.5	0.0	0.0	0.0	0.0	0.0	
6	no data	0.0	0.0	0.0	11.4	0.1	0.1	0.0	0.0	0.0	0.0	0.0	
7	no data	0.0	0.0	0.0	1.9	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
8	no data	0.0	0.0	0.0	0.5	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
9	no data	0.0	0.0	0.0	0.4	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
10	no data	0.0	0.0	0.0	0.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
11	no data	0.0	0.0	0.0	0.0	0.0	8.4	0.0	0.0	0.0	0.0	0.0	
12	no data	0.0	0.0	0.0	0.0	0.0	3.6	0.0	0.0	0.0	0.0	0.0	
13	no data	0.0	0.0	0.0	0.0	0.0	0.6	0.0	0.0	0.0	0.0	0.0	
14	no data	0.0	0.0	0.0	0.0	0.0	0.2	0.0	0.0	0.0	0.0	0.0	
15	no data	0.0	0.0	0.0	0.0	0.0	0.1	0.0	0.0	0.0	0.0	0.0	
16	no data	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
17	no data	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
18	no data	0.0	0.0	26.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
19	no data	0.0	0.0	51.3	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
20	no data	0.0	0.0	242	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
21	no data	0.0	0.0	64.3	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
22	no data	0.0	0.0	12.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
23	no data	0.0	0.0	2.3	3.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
24	no data	0.0	0.0	0.4	6.9	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
25	no data	0.0	0.0	7.9	0.4	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
26	no data	0.0	0.0	7.9	4.3	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
27	no data	0.0	0.0	1.3	2.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
28	no data	0.0	0.0	0.3	0.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
29	no data	0.0	0.0	0.1	...	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
30	no data	0.0	0.0	0.0	...	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
31	no data	...	0.0	0.0	...	0.0	...	0.0	...	0.0	0.0	...	Annual
TOTAL	0	0	0	416	40	8	16	0	0	0	0	0	480
Max.day	0	0	0	242	11	4	8	0	0	0	0	0	242

Monitor's Comments

- Daily values are based on calculations of sediment discharge at 15-minute intervals.
- Daily values with more than 2 to 3 significant figures result from electronic calculations. No additional precision is implied.

Total annual sediment discharge (suspended plus bedload sediment)	
WY 2010:	1,119 tons

Balance Hydrologics, Inc. 101 Lucas Valley Rd., Suite 229, San Rafael, CA 94903 (415) 472-7584
 Balance Hydrologics, Inc. 800 Bancroft Way, Suite 101, Berkeley, CA 94710 (510) 704-1000; fax: (510) 704-1001; www.balancehydro.com

APPENDICES

APPENDIX A

Log of hydrologic observations

Appendix A. Summary of hydrologic monitoring results for Water Year 2010, Grady Ranch, Marin County, California

Site Conditions		Groundwater			Streamflow					High-water Marks		Water Quality Observations				Remarks	
Date/Time	Observer	Top of casing to water	WSE elevation (NGVD 29)	Depth below ground surface	Cage Height	WSE elevation (NGVD 29)	Measured Discharge	Estimated Discharge	Instrument Used	Estimated Accuracy	Estimated stage at staff plate	Inferred dates?	Temperature	Specific Conductance	Specific Conductance	Additional sampling?	
		(ft)	(ft)	(ft)	(ft)	(cfs)	(cfs)	(AA/PY)		(e/g/l/p)			(°C)	(µmhos)	at 25 (°C)	(Obed, etc.)	
GROUND WATER MONITORING POINTS																	
MW-1 (Terrace S of Miller Cr, 400 ft u/s of S3 trib, near Landmark Cr)																	
Latitude: 38.04121° N																	
Longitude: 122.60544° W																	
Depth to bottom (from GS) = 30.00																	
Depth to bottom (from RP) = 32.75																	
Reference point (RP) elevation = 255.61																	
Stickup of RP = 2.92																	
Ground surface elevation = 252.69																	
10/22/09 17:35	mw	23.65	231.96	20.73													
11/4/09 16:17	sr, tb	27.58	228.03	24.66													installed F30 #2157 at 27 feet bgs
11/4/09 16:40	sr, tb	27.60	228.01	24.68									15.1	290.3	357.4		Measured initial DTW and then set pump ~2 ft above bottom of well and turned on at max rate.
1/6/10 16:52	sr	21.01	234.60	18.09													Purged 7.5 gal (0.85 gal = 1 well casing). Water was initially very brown/turbid and cleared around 4 gal, such that water was faintly brown/turbid. Measured SCT in 2nd (last) 5-gal bucket. Sampled for irrigation suitability analysis. Downloaded and relaunched.
1/16/10 8:17	tb	17.07	238.54	14.15													F15 may not be sufficient (water level fluct. 2-13 ft from Nov to present). Pulled up levellogger 1.4 ft so that it was 5 ft below water surface.
2/18/10 9:52	sr	16.76	238.85	13.84													swapped the F15 for the F30, string length unchanged.
4/30/10 18:23	sr, jo	17.40	238.21	14.48													Downloaded data and relaunched; string length unchanged.
7/23/10 18:07	sr	29.72	225.89	26.80													Downloaded data and relaunched (changed to continuous logging); string length unchanged.
8/3/10 16:00	gp	31.00	224.61	28.08													Downloaded
8/10/10 13:15	bh	33.10	222.51	30.18													Lengthened string (25.7 ft --> 34.0 ft).
10/7/10 17:27	sr, tb	32.75	222.86	29.83													Downloaded
MW-2 (Terrace N of Miller Cr, 100 ft u/s of Grady bridge)																	
Latitude: 38.04002° N																	
Longitude: 122.60269° W																	
Depth to bottom (from GS) = 35.00																	
Depth to bottom (from RP) = 38.25																	
Reference point (RP) elevation = 241.79																	
Stickup of RP = 3.35																	
Ground surface elevation = 238.44																	
10/22/09 16:00	mw	25.25	216.54	21.90													
11/4/09 9:18	sr, tb	26.91	214.88	23.56									15.30	259.6	319.20		installed F30 # 2185 at 29.3 feet from RP
11/4/09 9:56	sr, tb	27.05	214.74	23.70									15.2	261.5	322.0		Measured initial DTW and SCT. SCT measured at top of water column. No more measurements of SCT will be made in well because a) reading represents well water, not GW and b) SCT increases with depth. Instead measure SCT after purging. Set pump ~2 ft above
1/6/10 16:11	sr	23.70	218.09	20.35													Purged 10 gal (1.85 gal = 1 well casing). Water was initially very brown/turbid and cleared around 5 gal, such that water was faintly brown/turbid. Measured SCT in 2nd (last) 5-gal bucket. Sampled for irrigation suitability analysis. Downloaded and relau
1/16/10 7:08	tb	18.12	223.67	14.77													F15 may not be sufficient (water level fluct. 2-13 ft from Nov to present). Pulled up levellogger 0.6 ft so that it was 5 ft below water surface.
2/18/10 8:48	sr	17.21	224.58	13.86													swapped the F15 for the F30, string length unchanged.
4/30/10 16:00	sr, jo	19.01	222.78	15.66													Downloaded data and relaunched; string length unchanged.
7/23/10 17:31	sr	28.97	212.82	25.62													Downloaded data and relaunched (changed to continuous logging); string length unchanged.
8/3/10 17:50	gp	30.00	211.79	26.65													Downloaded
10/7/10 17:12	sr, tb	33.10	208.69	29.75													Lengthened string (28.6 ft --> 36.0 ft).
WW-2 (Near MW-2)																	
Latitude: 38. ___ ° N																	
Longitude: 122. ___ ° W																	

Site Conditions		Groundwater			Streamflow					High-water Marks		Water Quality Observations				Remarks	
Date/Time	Observer	Top-of-casing to water (ft)	WSE elevation (NGVD 29) (ft)	Depth below ground surface (ft)	Gage Height (ft)	WSE elevation (NGVD 29) (ft)	Measured Discharge (cfs)	Estimated Discharge (cfs)	Instrument Used (AA/PY)	Estimated Accuracy (e/g/lp)	Estimated stage at staff plate	Inferred dates?	Temperature (°C)	Specific Conductance (µmhos)	Specific Conductance at 25 (°C)	Additional sampling? (Obed, etc.)	
Depth to bottom (from GS) =	307.70																
Depth to bottom (from RP) =	310.00																
Reference point (RP) elevation =	Not surveyed																
Stickup of RP =	2.30																
Ground surface elevation =	?																
10/19/10 15:22	gp	25.40	?	?													Well instrumented with F30 levellogger, SN 31019270, Bal# 1833. LL is 36.65 ft. from RP.(or 36.0 ft. from top of internal 4" casing)
MW-3 (Terrace N of Miller Cr, 350 ft d/s of Grady bridge, near S4 trib)																	
Latitude:	38.03901° N																
Longitude:	122.60150° W																
Depth to bottom (from GS) =	40.00																
Depth to bottom (from RP) =	42.33																
Reference point (RP) elevation =	236.26																
Stickup of RP =	1.99																
Ground surface elevation =	234.27																
10/21/09 8:07	sr, gp	25.97	210.29	23.98									15.0	220	...		SC measured at top of water column
10/21/09 8:07	sr, gp												9.0	360	...		SC measured at bottom of water column
10/21/09 10:41	sr	26.10	210.16	24.11													After bailing 1 well casing, water still brown/turbid; sampled for general mineral analysis
10/22/09 16:50	mw	25.90	210.36	23.91													installed F30 #2159 at 30 feet below RP
11/4/09 11:23	sr, tb	26.53	209.73	24.54													Measured initial DTW and then set pump ~2 ft above bottom of well and turned on at max rate.
11/4/09 11:50	sr, tb	26.91	209.35	24.92									15.3	250.4	306.8		Purged 10 gal (2.56 gal = 1 well casing). Water was initially very brown/turbid and cleared around 3 gal, such that water was faintly brown/turbid. Measured SCT in 2nd (last) 5-gal bucket. Sampled for irrigation suitability analysis. Downloaded and retau
1/6/10 15:01	sr	25.76	210.50	23.77													F15 seems sufficient (water level fluc. 3-8 ft from Nov to present).
1/16/10 7:47	tb	25.52	210.74	23.53													swapped the F15 for the F30, string length unchanged.
2/18/10 9:22	sr	25.42	210.84	23.43													Downloaded data and relaunched; string length unchanged.
4/30/10 15:39	sr, jp	25.52	210.74	23.53													Downloaded data and relaunched (changed to continuous logging); string length unchanged.
7/23/10 17:08	sr	28.16	208.10	26.17													Downloaded
8/3/10 16:49	gp	29.20	207.06	27.21													Lengthened string (30.2 ft --> 38.0 ft).
8/10/10 9:45	bh	29.93	206.33	27.94													Downloaded
10/7/10 16:24	sr, tb	33.23	203.03	31.24													
MW-4 (Terrace E of Grady Cr, 400 ft u/s of Miller Cr, near ranch rd ford)																	
Latitude:	38.04049° N																
Longitude:	122.60134° W																
Depth to bottom (from GS) =	30.00																
Depth to bottom (from RP) =	33.17																
Reference point (RP) elevation =	248.99																
Stickup of RP =	3.17																
Ground surface elevation =	245.82																
10/21/09 13:50	sr	23.65	225.34	20.48									15.5	401.4	491		SC measured in 4th 5-gal bucket bailed (5 5-gal buckets ~ 1 well casing); Bailed 3.5 buckets (drillers ready to seal well); water still brown/turbid; sampled for general mineral analysis
10/22/09 16:30	mw	24.02	224.97	20.85													installed F30 #2160 at 28 feet from RP
11/4/09 12:05	sr, tb	26.57	222.42	23.40													Measured initial DTW and then set pump ~2 ft above bottom of well and turned on at max rate.
11/4/09 12:25	sr, tb	28.02	220.97	24.85													Purged 1 gal (1.10 gal = 1 well casing). Water was initially very brown/turbid. Pump stopped working in the well. Pump worked in the bucket, but perhaps too much sediment in the water to work in the well? DTW indicates there is still water in the well.
11/4/09 15:41	sr, tb	27.10	221.89	23.93													Measured DTW and then began bailing.
11/4/09 15:55	sr, tb	30.10	218.89	26.93													Bailed 2.5 gal. Water very brown/turbid with fine sand in it. Bailer was not filling up. Bottom of well is 31.21 ft. Well casing turns when spun from the top (doesn't feel as secure as other wells). Left well to recover.

Site Conditions		Groundwater			Streamflow					High-water Marks		Water Quality Observations				Remarks			
Date/Time	Observer	Top-of-casing to water (ft)	WSE elevation (NGVD 29) (ft)	Depth below ground surface (ft)	Gage Height (ft)	WSE elevation (NGVD 29) (ft)	Measured Discharge (cfs)	Estimated Discharge (cfs)	Instrument Used (AA/PY)	Estimated Accuracy (e/g/lp)	Estimated stage at staff plate	Inferred dates?	Temperature (°C)	Specific Conductance (µmhos)	Specific Conductance at 25 (°C)	Additional sampling? (Qbed, etc.)			
11/4/09 17:08	sr, tb	28.37	220.62	25.20														MW asked us to sample without any more bailing. Sampled for irrigation suitability analysis.	
1/6/10 15:24	sr	23.69	225.30	20.52														F15 seems sufficient (water level fluct. 3-10 ft from Nov to present).	
1/16/10 7:35	tb	19.07	229.92	15.90														swapped the F15 for the F30, string length unchanged.	
2/18/10 9:08	sr	12.28	236.71	9.11														Downloaded data and relaunched; string length unchanged.	
4/30/10 15:25	sr, jp	12.50	236.49	9.33														Downloaded data and relaunched (changed to continuous logging); string length unchanged.	
7/23/10 16:48	sr	26.02	222.97	22.85														Downloaded	
8/3/10 17:15	gp	26.49	222.50	23.32														Lengthened string (28 ft --> 30.0 ft).	
10/7/10 16:31	sr, tb	28.05	220.94	24.88														Downloaded	
RW-6 (Terrace W Miller Cr; 250 ft S bridge, near S-4)																			
Latitude:		38.03878° N																	
Longitude:		122.60247° W																	
Depth to bottom (from GS) =		52.00																	
Reference point (RP) elevation =		236.16																	
Stickup of RP =		2.46																	
Ground surface elevation =		233.70																	
10/11/09 14:30	mw	31.60	204.56	29.14															
1/6/10 12:51	sr	20.86	215.30	18.40															
2/18/10 8:16	sr	15.69	220.47	13.23															
4/30/10 18:00	sr, jp	18.83	217.33	16.37															
7/23/10 17:45	sr	25.63	210.53	23.17															
8/3/10 18:46	gp	28.65	209.51	24.19															
10/7/10 16:13	sr, tb	30.75	205.41	28.29															
SURFACE WATER MONITORING POINTS																			
Miller Creek above Grady bridge																			
Latitude:		38.03958° N																	
Longitude:		122.60253° W																	
Reference point (RP) elevation =		224.01																	
Ground surface elevation =		225.81																	
11/4/09 14:30	mw, tb																		
1/6/10 8:15	sr				dry			
1/16/10 9:37	tb				1.84	225.85	0.279	...	PY	f	2.50	01/12/10	11.2	197.1	266.2	...			
1/18/10 16:05	gp, mw				3.03	227.04	...	60.00	...	p	3.70	Not specified	11.1	91.7	124.8	1 Qbed, 1 Qss			
1/20/10 14:38	gp, mw				3.24	227.25	73.9	...	AA	e/g	4.15	01/20/10	11.0	80.0	111.8	1Qbed			
1/29/10 16:15	mw				2.11	226.12	5.49	...	PY	...	3.50	Not specified	12.0	132.0	179.6	1 Qss			
2/3/10 15:00	mw				1.94	225.95			
2/5/10 12:15	sr, tb				2.40	226.41	17.01	20.00	PY	g	3.40	02/04/10	11.9	107	142.9	2 Qbed, 2 Qss			
2/18/10 12:15	sr				1.89	225.90	1.15	1.00	PY	g	2.40	2/6-2/7	13.3	157	202.2	...			
2/24/10 9:45	mw				2.50	226.51	2.80	2/18/10	12.0	105	143.0	1 Qbed, 1 Qss			
2/26/10 14:48	gp				3.12	227.13	...	70	1 Qbed, 1 Qss			
2/26/10 16:14	gp				2.78	226.79			
4/30/10 17:00	sr, jp				0.46	...	PY	g	2.10	Mar-Apr 2010	15.6	182	222.0	...			
																			Installed datalogger (TB - see inventory for #) and solar panel (Bal #1152) into pool w/s Grady bridge (real-time station).
																			No flow at gage; Trouble-shooted SCT probe (checked wiring, cables -- didn't find any problems) and called SB to look into program/real-time data
																			Flow measurement on concrete in very shallow water, may be underestimate, water clear.
																			Storm duty - Headphones broke so no flow meas.
																			I removed the SCT probe from the still well, brushed out the mud, and placed it in the creek outside of the casing somewhat in the current so it would not accumulate mud within the sensor.
																			measured Q at S3, S4 and Landmark but only read a GH at Miller Cr
																			Rained ~1.6" last night - left bank has small trib running more turbid than Miller Ck w/s meas.; Miller Ck is as turbid S3, though much more bedload moving in Miller Ck than S3.
																			Gage pool filled in so that bed is now at GH 1.86; water clear, flow through both sides of bridge
																			wake at staff dipping down to 1.48
																			Approximate time of Qss is 15:50
																			No flow at gage (pool completely filled in with sediment), but water in adjacent LB pool. Dug down to 1.4 on staff plate - moist water but no ponded water. Walked us to upper side of straight reach (adj. to unnamed LB trib - not flowing) to minimize effects of losing reach (measured 0.46 cfs). Water clear.

Site Conditions		Groundwater			Streamflow					High-water Marks		Water Quality Observations				Remarks		
Date/Time	Observer	Top-of-casing to water	WSE elevation (NGVD 29)	Depth below ground surface	Gage Height	WSE elevation (NGVD 29)	Measured Discharge	Estimated Discharge	Instrument Used	Estimated Accuracy	Estimated stage at staff plate	Inferred dates?	Temperature	Specific Conductance	Specific Conductance at 25 (°C)	Additional sampling?		
		(ft)	(ft)	(ft)	(ft)	(ft)	(cfs)	(cfs)	(AA/PY)	(e/g/lp)			(°C)	(µmhos)		(Qbed, etc.)		
7/23/10 12:25	sr				Dry at gage (bed = 1.85), no repairs needed. Gage pool filled in, pool us along LB. Downloaded and relaunched temp loggers d/s Grady Bridge, d/s S3, d/s Landmark Ck.
10/7/10 13:00	sr, tb				Moved real-time gage to LB pool just u/s previous location (left stilling well in case bed conditions change).
S4 Tributary																		
Latitude: 38.03871° N																		
Longitude: 122.60323° W																		
Reference point (RP) elevation = 207.94																		
Ground surface elevation = 214.64																		
11/16/09 13:40	mw, sr				Installed Levelogger (Bal #1894) into S-4 - No flow
1/6/10 12:06	sr				dry	No flow at gage; No isolated pools betw gage and road; Slight flow out of gravels at confluence with Miller Ck (lower 6')
1/16/10 11:22	tb				6.78	214.72	0.03	...	PY	g	7.05	01/12/10	10.4	155.3	215.7	water very slightly cloudy, bed silty and sandy with small cobbles.
1/18/10 17:10	gp, mw				7.17	215.11	7.70	Not specified	Storm duty - Headphones broke so no flow meas.
1/20/10 14:30	gp, mw				7.25	215.19	9.03	...	AA	...	7.80	01/20/10	10.5	78.0	110.5	1 Qbed, 1 Qss	...	
2/3/10 18:00	mw				6.80	214.74	0.24	...	PY	f	11.5	120.0	165.5	
2/5/10 14:15	sr, tb				7.02	214.96	2.00	1.50	PY	g	7.80	02/04/10	11.4	95.1	128.3	1 Qbed, 2 Qss	...	
2/18/10 14:00	sr				6.78	214.72	0.18	0.10	PY	g	7.25	02/06/10	12.6	134.2	176	...	Observer may be artificially elevating the water level. Downloaded levelogger and relaunched.	
2/26/10 16:43	gp				7.17	215.11	1 Qbed, 1 Qss	Water clear, downloaded levelogger (but didn't stop and relaunch).	
4/30/10 13:30	sr, jp				6.71	214.65	0.02	0.03	PY	f	7.50	Jan. 2010	17.3	178.0	210	...	Also 1 Qss sample taken slightly u/s of Lucas Valley Rd.	
7/23/10 10:00	sr				Water was turbid for first half-hour (upstream erosion), but then cleared up. Found juvenile resident trout/steelhead in small pool ~5' us confluence with Miller Ck (S4 not flowing into Miller Ck - soaking into gravels). Base of stilling well was above lowest possible water level, so pushed 0.06 ft. at 13:50. Downloaded data and relaunched (changed to continuous logging).
Grady Creek																		
Latitude: 38.04052° N																		
Longitude: 122.60103° W																		
Reference point (RP) elevation = 228.21																		
Ground surface elevation = 235.06																		
10/22/09 17:15	mw				15.0	330	415	installed temp logger in bedrock pool; flow further downstream ~100 ft than before 10/13 storm
11/4/09 12:39	mw, tb				Installed Levelogger F15 Gold (Bal #1783) into Grady Ck
1/6/10 15:44	sr				dry	No flow at gage; Isolated pool at base of canyon
1/16/10 7:33	tb				dry	no water, no sign of recent flow.
1/18/10 17:37	gp, mw				7.50	235.71	Storm duty - Headphones broke so no flow meas.
1/20/10 17:15	gp, mw				7.42	235.63	10.69	...	AA	...	8.40	01/20/10	11.0	90.0	125.8	1 Qbed, 1 Qss	...	
2/3/10 17:00	mw				6.97	235.18	0.51	...	PY	f	7.70	poor	13.0	178.0	235.8	
2/5/10 16:00	sr, tb				7.10	235.31	3.034	2.75	PY	e/g	8.00	02/04/10	12.6	130.5	170.9	1 Qbed, 1 Qss	Water clear -- can see bed through Q meas. Fresh sand deposited at tail end of pool. Downloaded levelogger and relaunched.	
2/18/10 15:30	sr				6.96	235.17	0.729	0.40	PY	e	7.15	02/06/10	13.9	174.0	221.2	...	Water clear, downloaded levelogger and relaunched because levelogger time and PC time were very off.	
2/26/10 17:17	gp				7.16	235.37	1 Qbed, 1 Qss	...	
2/26/10 17:30	gp				7.16	235.37	Water clear. Bank failed on gage. JO removed dirt and reconnected gage with use in stream. Downloaded data and relaunched (changed to continuous logging). LL gained time during this period (extra 15 min., which is only half-min. per day; I deleted last 15 min. record rather than adjust by such a small amt). Bank failure may have triggered issue, regardless time resynched so moving forward should be okay.
4/30/10 15:00	sr, jp				6.85	235.06	0.06	0.10	PY	g	6.95, 7.45, 8.65	4/27/2010, Feb 2010, Jan-2010	16.4	199.0	238.0	
7/23/10 10:30	sr				Dry at gage (bed = 6.8), connected to channel despite large bank failure u/s gage. Downloaded and relaunched (this levelogger gains time?). GPS first flow u/s gage (increased from 0.25 to 1 gpm moving further u/s) and deployed new temp logger with MW (old one gone).
Grady Creek (u/S qage)																		

Site Conditions		Groundwater			Streamflow					High-water Marks		Water Quality Observations				Remarks		
Date/Time	Observer	Top-of-casing to water	WSE elevation (NGVD 29)	Depth below ground surface	Gage Height	WSE elevation (NGVD 29)	Measured Discharge	Estimated Discharge	Instrument Used	Estimated Accuracy	Estimated stage at staff plate	Inferred dates?	Temperature	Specific Conductance	Specific Conductance	Additional sampling?		
		(ft)	(ft)	(ft)	(ft)	(cfs)	(cfs)	(AA/PY)		(e/g/l/p)			(°C)	(µmhos)	at 25 (°C)	(Qbed, etc.)		
Latitude: 38. ___ ° N																		
Longitude: 122. ___ ° W																		
Reference point (RP) elevation = 228.21																		
Ground surface elevation = 235.06																		
10/19/10 15:00	gp, mw				Dry	Staff plate range 3.33 to 6.66 installed. 9.00 on the old staff plate equals 6.52 on the new one. F15 levellogger SN 21045668, Bal # 2179. LL start time 10/10/10 15:00 PDT; sample rate is 15 min.; continuous logging mode.
S3 Tributary																		
Latitude: 38.04049° N																		
Longitude: 122.60454° W																		
Reference point (RP) elevation = 227.49																		
Ground surface elevation = 234.29																		
11/16/09 13:40	mw, sr				Installed Levellogger (Bal #???) into S-3 - No flow
1/6/10 13:44	sr				dry	No flow at gage; No isolated pools betw gage and road
1/16/10 8:11	tb				dry	7.25	01/12/10	No flow at gage.
1/20/10 15:30	gp, mw sr, tb				7.90	235.39	11.63	...	AA	11.0	78.0	109.0	1Qbed	...	At threshold for bedload transport. Downloaded levellogger and relaunched.
2/5/10 10:45					7.56	235.05	3.22	2.00	PY	e/g	8.00	02/04/10	10.9	97.3	133.2	1 Qbed, 1 Qss	...	
2/12/10 15:05	mw				7.11	234.60	0.25	...	PY	g	12.0	133.0	181.0	
2/18/10 10:30	sr				dry	7.00	02/06/10	No flow at gage (bed GH ~6.8), pool downstream flowing ~2 gpm, but NOT into Miller Ck (SC 175.8 us @ 12.0C, 234.9 us @ 25C). Downloaded levellogger (but didn't stop and relaunch).	
2/26/10 15:30	gp				7.81	235.30	1 Qbed, 1 Qss	...	Also 1 Qss sample taken slightly u/s of Lucas Valley Rd.
4/30/10 17:19	sr, jo				dry	8, 8.65	Jan. 2010	No flow at gage (bed GH at 6.76). Downloaded data and relaunched (changed to continuous logging).	
7/23/10 12:15	sr				Dry at gage (bed = 6.7), no repairs needed. Downloaded.
Landmark Creek																		
Latitude: 38.04357° N																		
Longitude: 122.60661° W																		
Reference point (RP) elevation = 255.84																		
Ground surface elevation = 262.64																		
11/16/09 10:00	mw, sr				dry	10.0	222.9	311.8	Installed Levellogger (Bal #1895) into Landmark Ck - No flow
1/6/10 14:00	sr				No flow at gage; Flow just upstream road crossing (visual est 0.05 cfs; SCT values taken here); Didn't download temp logger
1/16/10 10:48	tb				7.28	263.12	0.07	...	float	f	8.00	01/12/10	10.5	182.3	252.3	small flow test among cobbles and roots, water clear
2/3/10 16:00	mw sr, tb				7.44	263.28	0.26	...	PY	f	8.00	poor	12.0	150.0	204.0	Water clear (clearest of all stations visited today). Downloaded levellogger and relaunched.
2/5/10 17:30					7.68	263.52	2.72	...	AA	e	8.30	02/04/10	12.2	107.7	142.6	1 Qbed, 1 Qss	...	
2/12/10 15:45	mw				7.48	263.32	0.46	...	AA	g/f	12.0	150.0	204.0	
2/18/10 16:45	sr				7.38	263.22	0.10	0.10	PY	g	8.00	02/06/10	12.8	162.7	212.1	Created weir to focus flow (had to remove cobbles to minimize turbulence); water clear; downloaded levellogger (but didn't stop and relaunch).
2/26/10 17:58	gp				7.73	263.57	1 Qss	...	No flow at gage (bed GH at 6.6). Leaves in bottom of pools suggest no flow has occurred recently. Downloaded data and relaunched (changed to continuous logging).
4/30/10 16:15	sr, jo				dry	8.00	Jan. 2010	Dry at gage (bed = 6.7), no repairs needed. Downloaded.
7/23/10 12:45	sr				
Miller Creek at property line																		
Latitude: 38.03811° N																		
Longitude: 122.60085° W																		
Reference point (RP) elevation = Not surveyed																		
Ground surface elevation = ?																		
10/21/09 17:23	mw, gp, sr				1.42	20 gpm	visual est.	med.

Site Conditions		Groundwater			Streamflow					High-water Marks		Water Quality Observations				Remarks	
Date/Time	Observer	Top-of-casing to water	WSE elevation (NGVD 29)	Depth below ground surface	Gage Height	WSE elevation (NGVD 29)	Measured Discharge	Estimated Discharge	Instrument Used	Estimated Accuracy	Estimated stage at staff plate	Inferred dates?	Temperature	Specific Conductance	Specific Conductance	Additional sampling?	
		(ft)	(ft)	(ft)	(ft)		(cfs)	(cfs)	(AA/PY)	(e/g/l/p)			(°C)	(µmhos)	at 25 (°C)	(Qbed, etc.)	
1/6/10 11:00	sr				1.49	...	0.20	...	PY	g	12.3	212.7	280.7	...	Flow measured 20' upstream gage (not sufficient flow in pool adj. to dancefloor); Created weir and removed rocks from section prior to meas. to approx. laminar flow; No HWMs visible on gage (algae growing on staff plate and stilling well); Sediment erodi
1/16/10 12:08	tb				1.62	...	1.18	...	PY	g	1.7-1.8	01/12/10	12.9	199.8	259.8	...	signs of sediment from road drainage pipe at staff plate, flow measurement upstream in sandy run.
1/18/10 16:30	gp, mw				Gage blew out sometime before our visit.
4/30/10 12:37	sr, jo				NA	...	1.07	...	PY	g	wse +1, +1.1	4/27/2010, Jan. 2010	16.0	174	210	...	Water clear, "dancefloor" covered with sand, gravels, and cobbles, pool filled in with coarse sediment in the bottom, with a sand patch in the lee bedrock. "Bath tub" ring marks this year's HWM. Bed seems armored d/s bridge - fines winnowed away.
7/23/10 9:45	sr				Dry at gage, deployed temp logger (no rock). Observed several isolated pools with tadpoles on the way down to this gage -- deployed 2 temp loggers with MW later in the afternoon.
10/7/10 14:30	sr, tb				Installed levellogger (BAL #1975) into pool just u/s previous location -- stilling well mounted into bedrock - continuous logging. Pool dry. No staff plate; observer will need to take laser level to shoot benchmark and then WSE relative to benchmark. All temp loggers downloaded and relaunched (currently 4 d/s MC bridge, 2 u/s bridge, and 1 in Grady Ck pool).

Notes:

Observer Key: mw = Mark Woysner; gp = Gustavo Porras; sr = Sarah Richmond, tb = Travis Baggett

Stage: Water level observed at staff plate

Hydrograph: Describes stream stage as rising (R), falling (F), steady (S), baseflow (B), or uncertain (U).

Instrument: If measured, typically made using a standard (AA) or pygmy (PY) bucket-wheel ("Price-type") current meter. If estimated, from rating curve (R) or visual (V).

Estimated measurement accuracy: Excellent (E) = +/- 2%; Good (G) = +/- 5%; Fair (F) = +/- 9%; Poor (P) estimated percent accuracy given

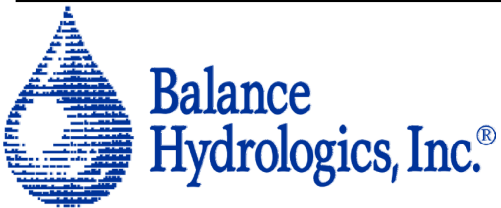
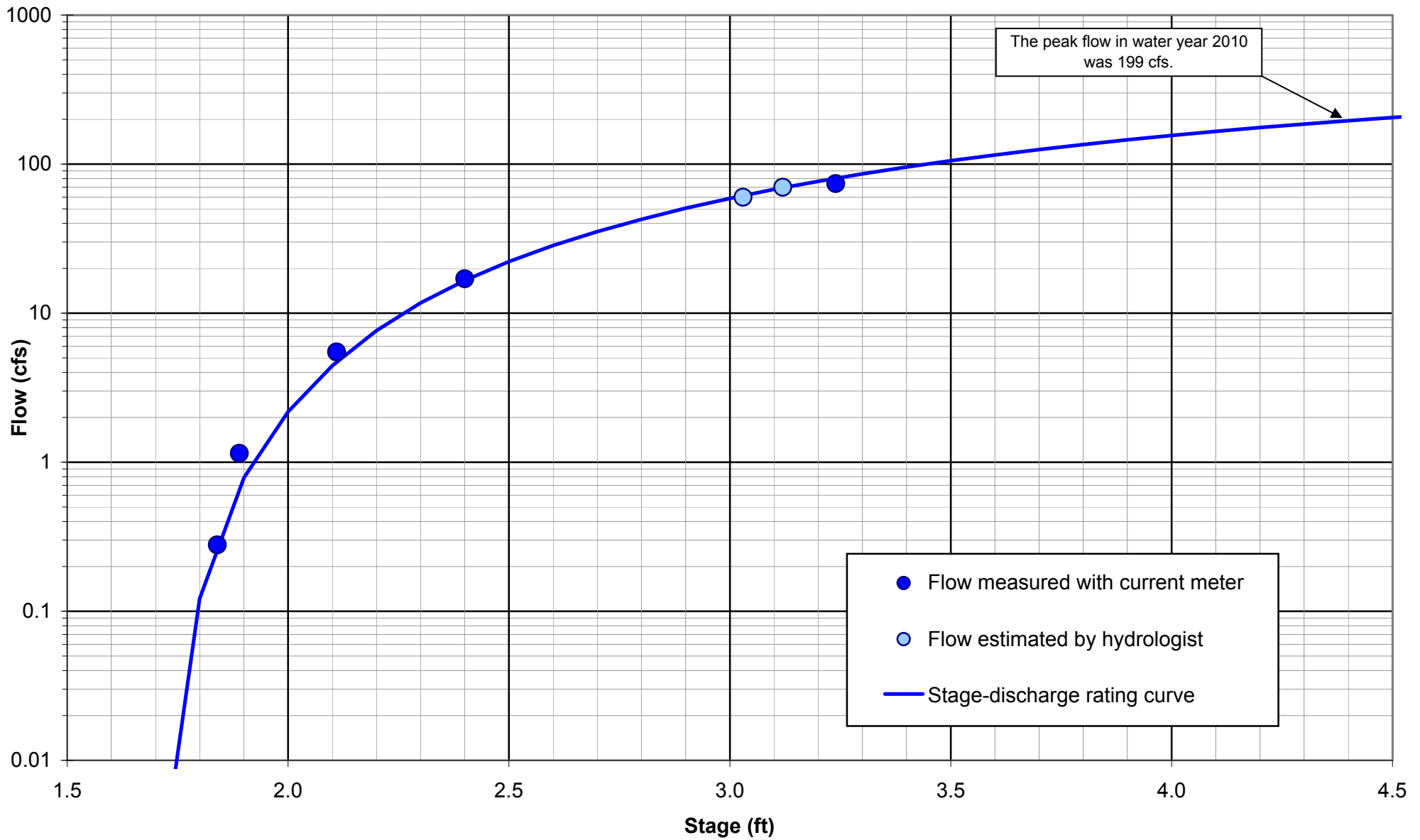
High-water mark (HWM): Measured or estimated at location of the staff plate

Specific conductance: Measured in micromhos/cm in field; then adjusted to 25degC by equation $(1.8813774452 - [0.050433063928 * \text{field temp}] + [0.00058561144042 * \text{field temp}^2]) * \text{Field specific conductance}$

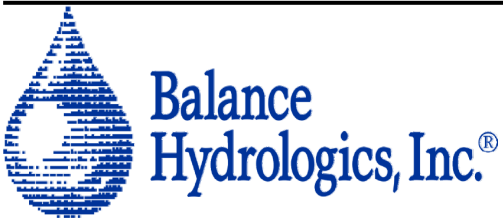
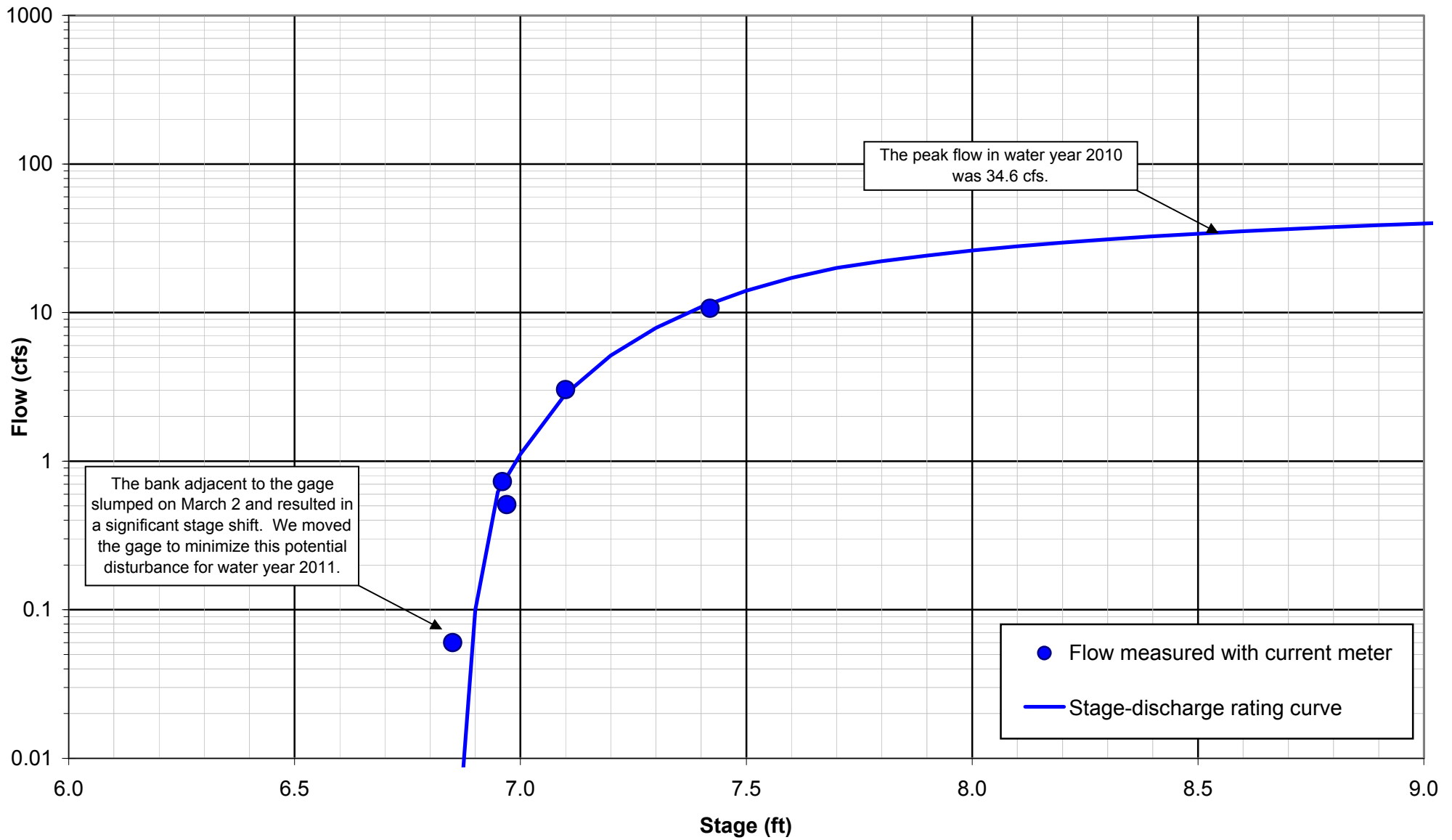
Additional Sampling: Qbed = Bedload, Qss = Suspended sediment, Nutr = nutrients; other symbols as appropriate

APPENDIX B

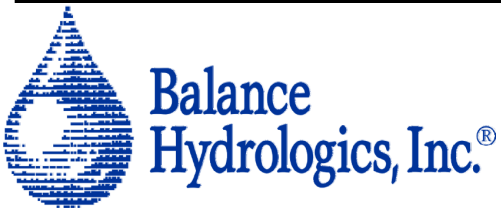
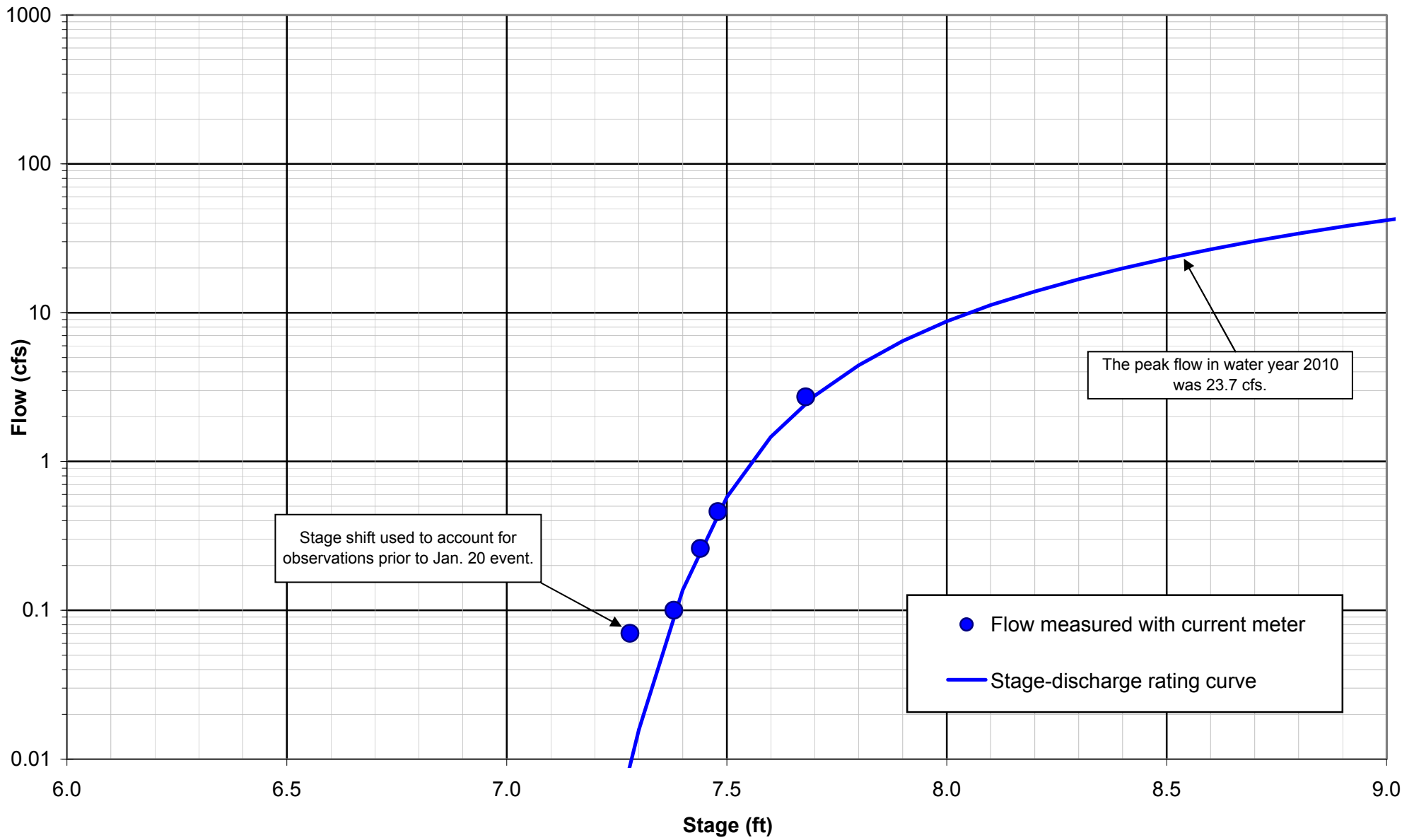
Stage-to-discharge rating curves



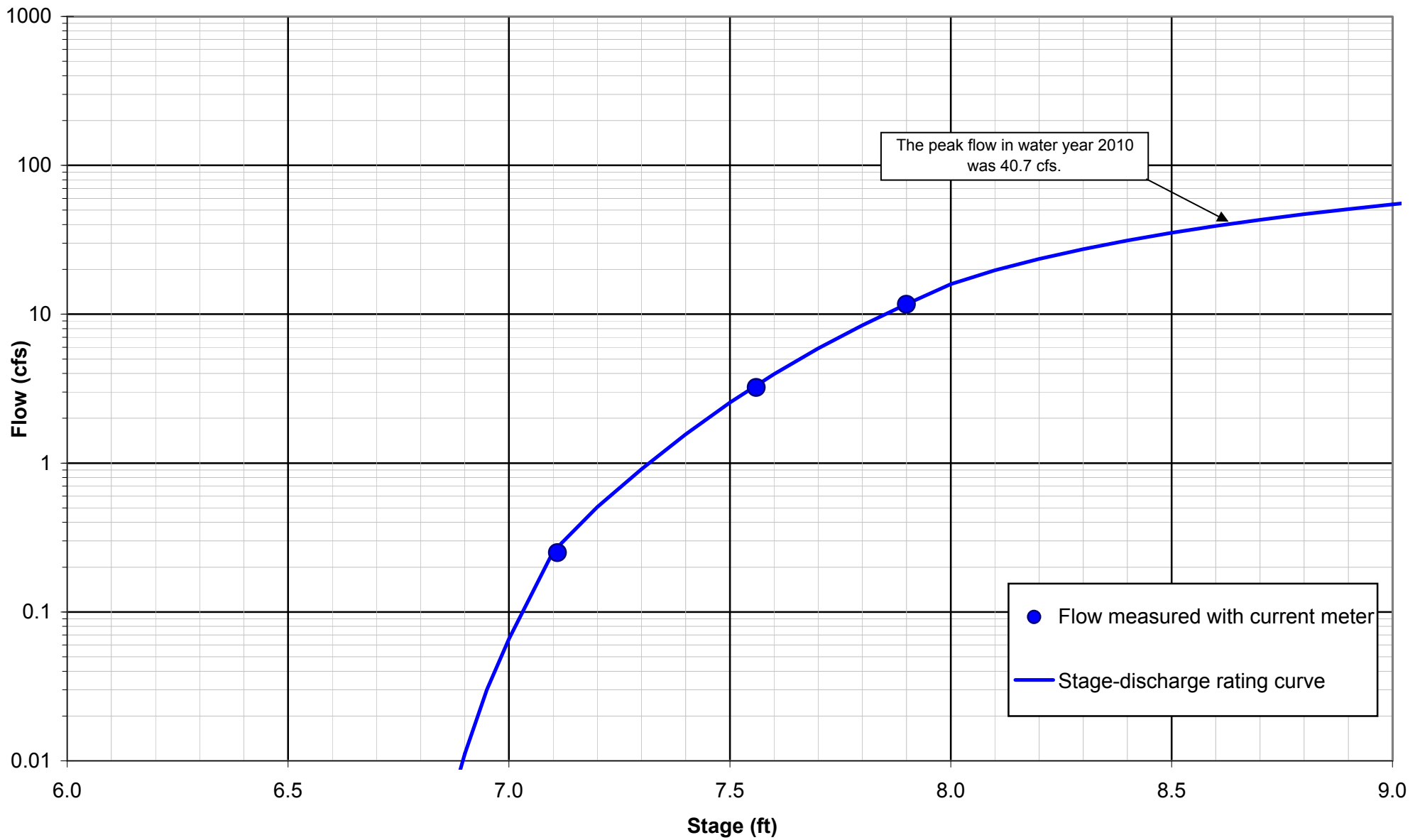
Stage-discharge rating curve: Miller Creek above Grady Bridge, water year 2010, Grady Ranch, Marin County, California.



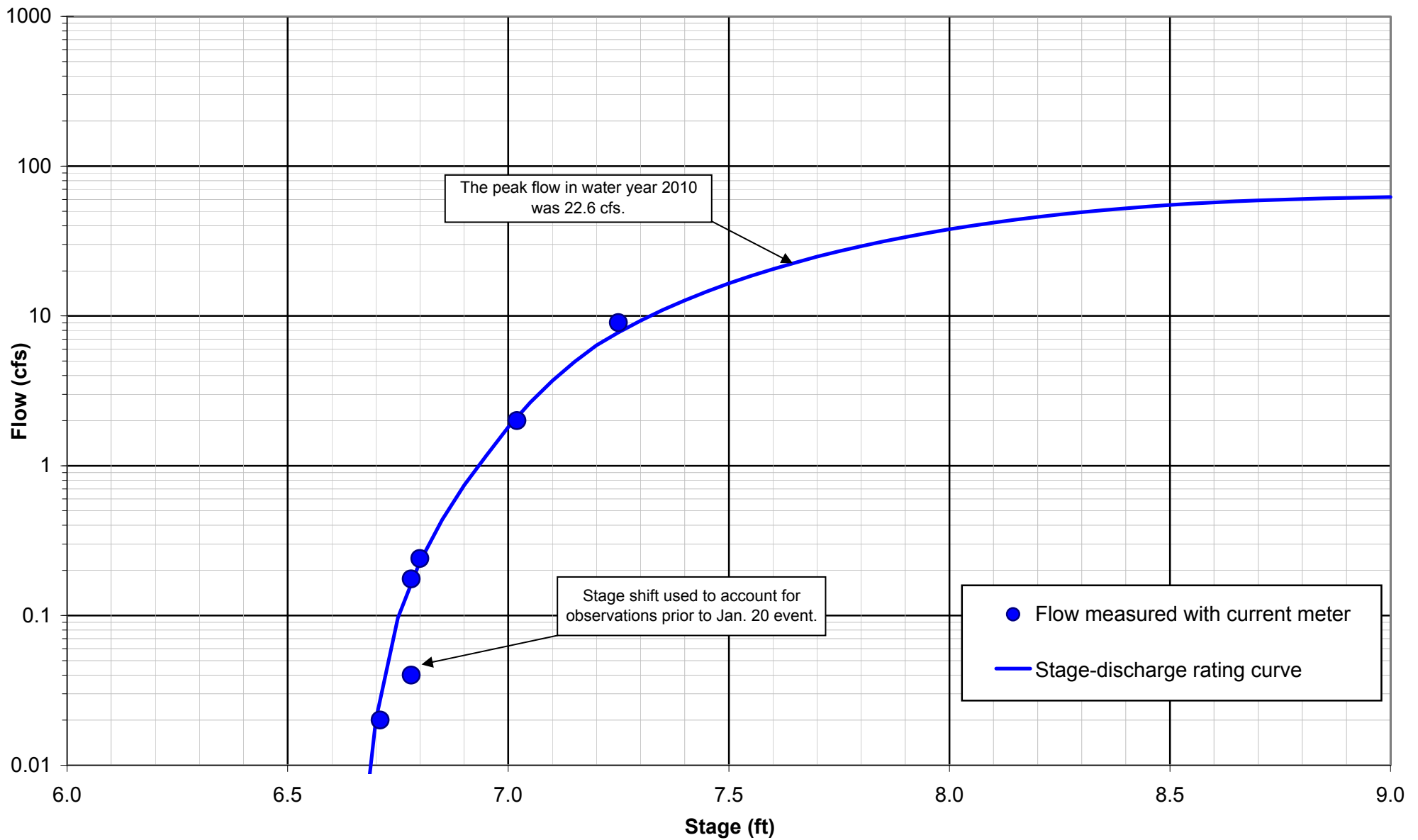
Stage-discharge rating curve: Grady Creek, water year 2010, Grady Ranch, Marin County, California.



Stage-discharge rating curve: Landmark Creek, water year 2010, Grady Ranch, Marin County, California.



Stage-discharge rating curve: S3 Tributary, water year 2010, Grady Ranch, Marin County, California.



Stage-discharge rating curve: S4 Tributary, water year 2010, Grady Ranch, Marin County, California.

APPENDIX C

Sediment-discharge measurements

Appendix C. Sediment-discharge measurements, Grady Ranch, water year 2010, Marin County, California

Site Conditions						Bedload Sampling Details						Sediment Transport			
Location	Sample	Observer(s)	Gage Height	Streamflow Discharge	Stream Condition	Active Bed Width	Sampler Width	No. of Verts.	Time/Vert.	Total Time	Sample Dry Weight	Bedload-Sediment Discharge Rate	Bedload-Sediment Discharge Rate	Suspended-Sediment Concentration	Suspended-Sediment Discharge Rate
	Date:Time		(ft)	(cfs)	R,F,B,U	(ft)	(ft)		(sec)	(sec)	(gm)	(lb/sec)	(tons/day)	(mg/l)	(tons/day)
Landmark Ck	100205:1720	sr, tb	7.69	2.67	F	2	0.25	2	60	120	4	0.001	0.0
Landmark Ck	100205:1700	sr, tb	7.69	2.68	F	11.5	0.1
Landmark Ck	100226:1758	gp	7.75	3.51	F	10.9	0.1
S3 trib	100120:1530	mw, gp	7.91	12.72	F	4	0.25	3	60	180	220	0.043	2
S3 trib	100205:1018	sr, tb	7.56	3.34	F	4	0.25	4	60	240	11	0.002	0
S3 trib	100226:1530	gp	7.74	6.74	R	6.5	0.25	5	30	150	52	0.020	1
S3 trib	100120:1545	mw, gp	7.90	11.53	F	68.4	2.1
S3 trib	100205:1010	sr, tb	7.56	3.34	F	15.1	0.1
S3 trib	100205:1057	sr, tb	7.57	3.44	F	22.8	0.2
S3 trib	100226:1540	gp	7.71	6.74	R	110.0	2.0
S3 trib (across road, offsite)	100226:1600	gp	7.68	5.51	R	63.3	1
Miller Ck above Grady Bridge	100118:1615	mw, gp	2.74	38.50	F	10	0.25	5	60	300	739	0.217	9.4
Miller Ck above Grady Bridge	100120:1415	mw, gp	3.19	75.78	F	17	0.25	7	30	210	3108	2.219	95.8
Miller Ck above Grady Bridge	100205:1145	sr, tb	2.41	17.97	F	11	0.25	6	60	360	122	0.033	1.4
Miller Ck above Grady Bridge	100205:1226	sr, tb	2.39	17.17	F	6	0.25	4	60	240	40	0.009	0.4
Miller Ck above Grady Bridge	100218:1215	sr	1.88	1.06	F	Visual observation of no bedload transport						...	0.0
Miller Ck above Grady Bridge	100224:1000	mw	2.47	22.99	F	10	0.25	5	60	300	288	0.085	3.7
Miller Ck above Grady Bridge	100226:1550	gp	2.69	37.59	F	18	0.25	5	30	150	670	0.709	30.6
Miller Ck above Grady Bridge	100118:1610	mw, gp	2.74	38.50	F	139.0	14.4
Miller Ck above Grady Bridge	100120:1330	mw, gp	3.24	80.77	F	262.0	57.1
Miller Ck above Grady Bridge	100129:1600	mw	2.10	4.88	F	36.7	0.5
Miller Ck above Grady Bridge	100201:0930	mw	2.00	2.50	F	5.8	0.0
Miller Ck above Grady Bridge	100205:1120	sr, tb	2.41	18.42	F	10.7	0.5
Miller Ck above Grady Bridge	100205:1235	sr, tb	2.39	17.17	F	8.3	0.4
Miller Ck above Grady Bridge	100224:0940	gp	2.48	23.27	F	20.3	1.3
Miller Ck above Grady Bridge	100226:1510	gp	2.83	47.85	F	762.0	98.3
Grady Ck	100120:1706	mw, gp	7.43	11.94	F	4	0.25	3	60	180	869	0.170	7
Grady Ck	100205:1535	sr, tb	7.10	2.63	F	4.5	0.25	3	60	180	10	0.002	0
Grady Ck	100226:1717	gp	7.21	5.05	F	3	0.25	5	60	300	235	0.021	1
Grady Ck	100118:1737	mw, gp	7.50	13.95	F	47.1	1.8
Grady Ck	100120:1706	mw, gp	7.43	11.49	F	150.0	4.6
Grady Ck	100205:1515	sr, tb	7.10	2.73	F	21.8	0.2
Grady Ck	100226:1720	gp	7.16	5.52	F	27.0	0.4
S4 trib	100120:1645	mw, gp	7.26	7.72	F	4	0.25	3	20	60	1057	0.621	27
S4 trib	100205:1347	sr, tb	7.02	1.85	F	4.5	0.25	3	60	180	5	0.001	0
S4 trib	100226:1700	gp	7.18	5.16	F	3	0.25	5	40	200	131	0.017	1
S4 trib	100118:1705	mw, gp	7.19	4.78	F	117.0	1.5
S4 trib	100120:1613	mw, gp	7.29	8.09	F	4680.0	102.1
S4 trib	100205:1331	sr, tb	7.03	8.09	F	33.3	0.7
S4 trib	100205:1439	sr, tb	7.06	1.96	F	42.9	0.2
S4 trib	100226:1650	gp	7.18	5.24	F	143.0	2.0
S4 trib (across road, offsite)	100226:1630	gp	7.18	5.33	F	154.0	2.2

Notes:

Observer Key: mw = Mark Woysner; gp = Gustavo Porras; sr = Sarah Richmond; tb = Travis Baggett
 Streamflow discharge is the measured or estimated instantaneous flow when sediment was sampled, and usually differs from the mean flow for the day.
 Stream Condition: R = rising, F = falling, B = baseflow, U = uncertain
 Values for sediment discharge having more than two to three digits displayed are the result of calculations; increased precision is not implied.
 Active Bed Width: The width thought by the field observer to be transporting significant amounts of bedload based on observations and/or measurements.
 Sampler Width and Type: 0.25 = 3-inch Helley Smith; 0.50 = 6-inch Helley Smith
 Bedload Discharge (lbs/sec) = [active bed width (ft) * sample dry weight (gm) * 0.002205 (lbs)] / [sampler width (ft) * sampling time (sec)]
 Bedload Discharge (tons/day) = [active bed width (ft) * sample dry weight (gm) * 86,400 (sec)] / [sampler width (ft) * sampling time (sec) * 907,200 (gm)]
 When water is very clear we generally do not take a suspended-sediment sample, because from past experience, those clear samples are below the detection limit.

APPENDIX D

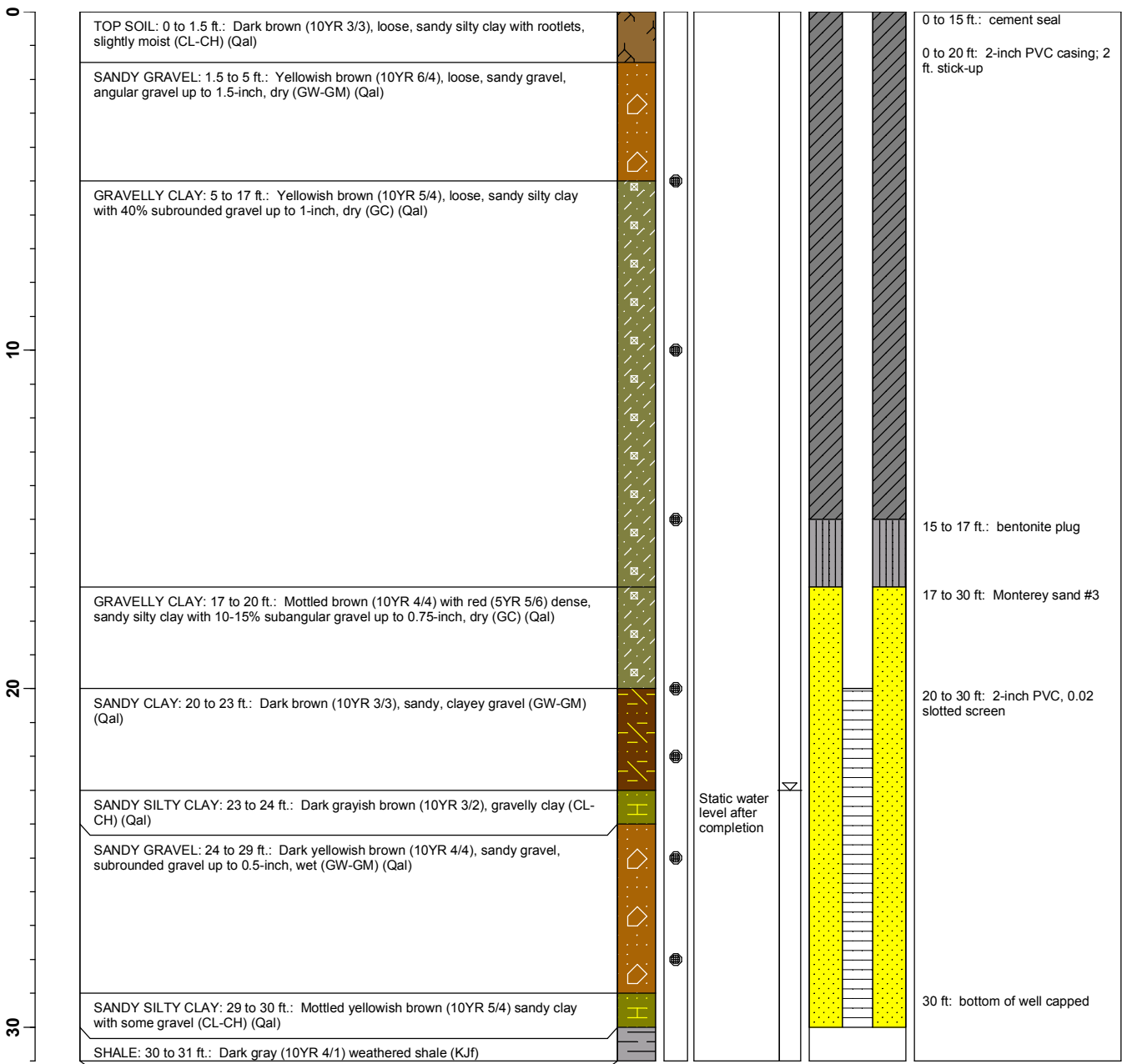
Borehole logs and monitoring well construction



Log of boring and well construction of MW-1, Grady Ranch, Marin County, California

Property Owner and Mailing Information: Skywalker U: [redacted] 5858 Lucas Valley Road Nicasio, CA 94946	Client: Skywalker Properties Ltd.	Well completion date: October 21, 2009 Borehole geologist: Gustavo Porras Drilling company: Taber Consultants	
Site location: Vega upper terrace S of Miller Cr. about 400 ft u/s of S3 trib, near Landmark Cr.	Driller: Andrew Elbon Drilling rig: Track mounted CME-55	Drilling equipment: 8-inch hollow stem auger Depth of borehole: 31 feet	
APN: 164-310-15, -17, and -19	Depth of casing: 30 feet Diameter of casing: 2-inch		
Latitude, Longitude: N38.04121°, W122.60544°			
Ground surface elevation: 252 feet Start drilling date: October 21, 2009			

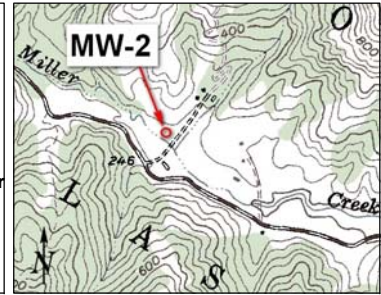
Depth feet	Lithology	Sample	Hydrology	Well Construction	Remarks
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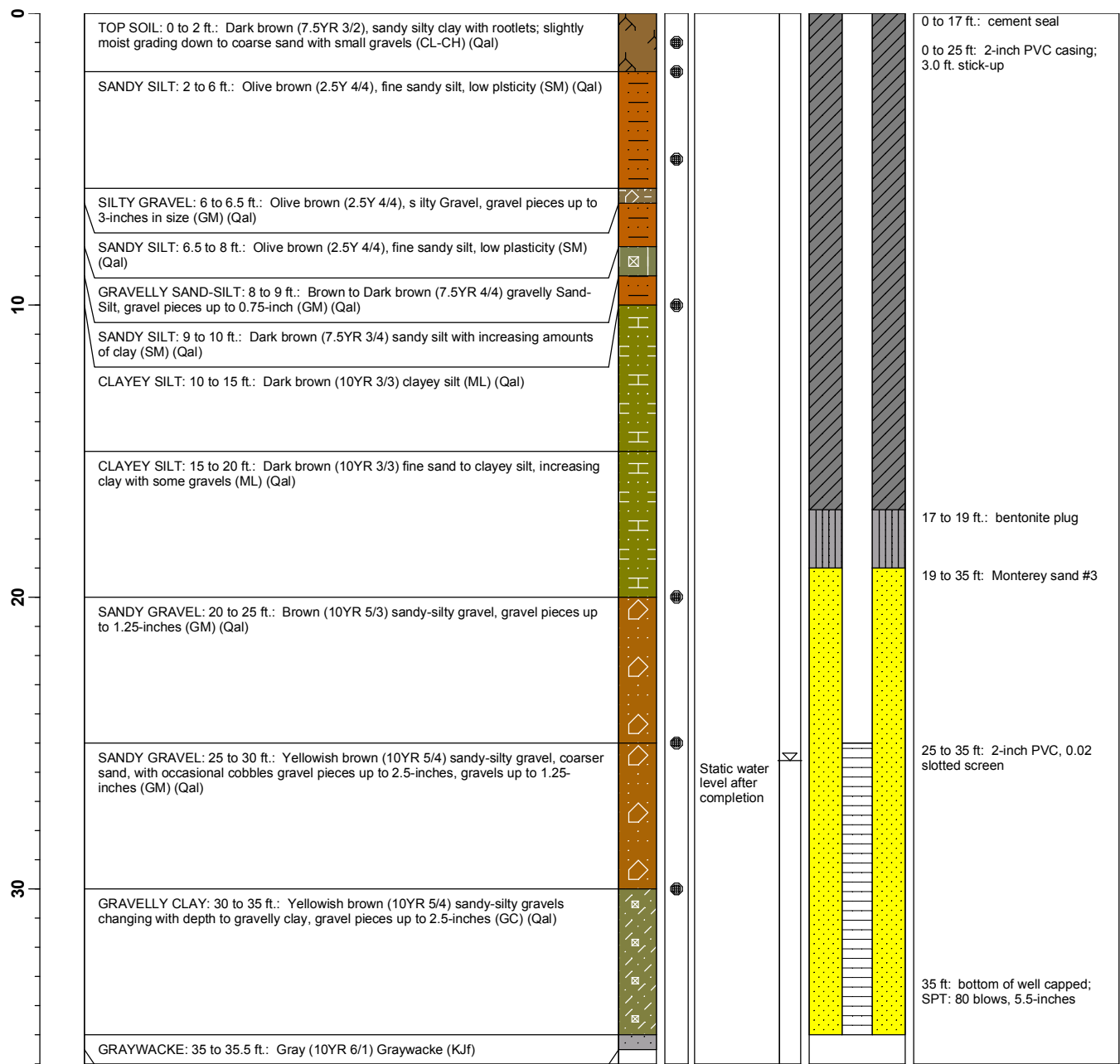


Log of boring and well construction of MW-2, Grady Ranch, Marin County, California

Property Owner and Mailing Information Skywalker Properties, Ltd. 5858 Lucas Valley Road Nicasio, CA 94946	Client: Skywalker Properties, Ltd.	Well completion date: October 20, 2009	
		Borehole geologist: Gustavo Porras	
		Drilling company: Taber Consultants	
Site location:	Vega upper terrace N of Miller Cr about 100 ft u/s of Grady bridge	Driller: Andrew Elbon	
APN:	164-310-15, -17, and -19	Drilling rig: Track mounted CME-55	
Latitude, Longitude:	N38.04002°, W122.60269°	Driling equipment: 8-inch hollow stem auger	
Ground surface elevation:	239 feet	Depth of borehole: 35.5	
Start drilling date:	October 20, 2009	Depth of casing: 35	
		Diameter of casing: 2-inch	



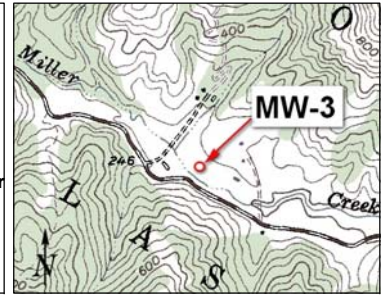
Depth feet	Lithology	Sample	Hydrology	Well Construction	Remarks
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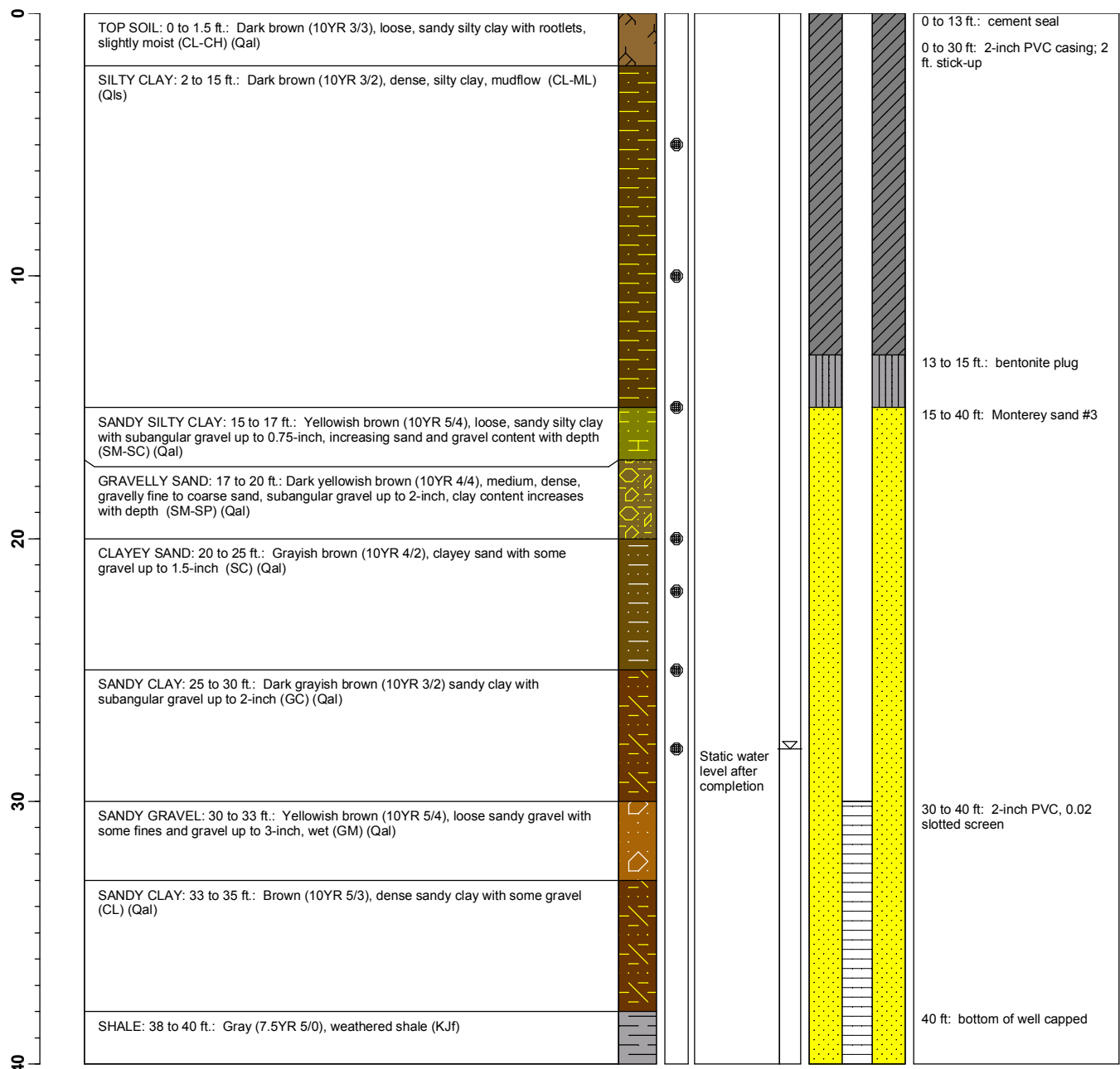


Log of boring and well construction of MW-3, Grady Ranch, Marin County, California

Property Owner and Mailing Information Skywalker Properties, Ltd. 5858 Lucas Valley Road Nicasio, CA 94946	Client: Skywalker Properties, Ltd.	Well completion date: October 20, 2009 Borehole geologist: Gustavo Porras Drilling company: Taber Consultants Driller: Andrew Elbon Drilling rig: Track mounted CME-55 Driling equipment: 8-inch hollow stem auger Depth of borehole: 40 Depth of casing: 40 Diameter of casing: 2-inch
Site location: Vega upper terrace N of Miller Cr about 350 ft d/s of Grady bridge, near S4 trib		
APN: 164-310-15, -17, and -19 Latitude, Longitude: N38.0391°, W122.60150° Ground surface elevation: 234 feet Start drilling date: October 20, 2009		



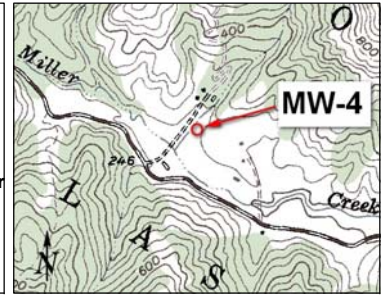
Depth feet	Lithology	Sample	Hydrology	Well Construction	Remarks
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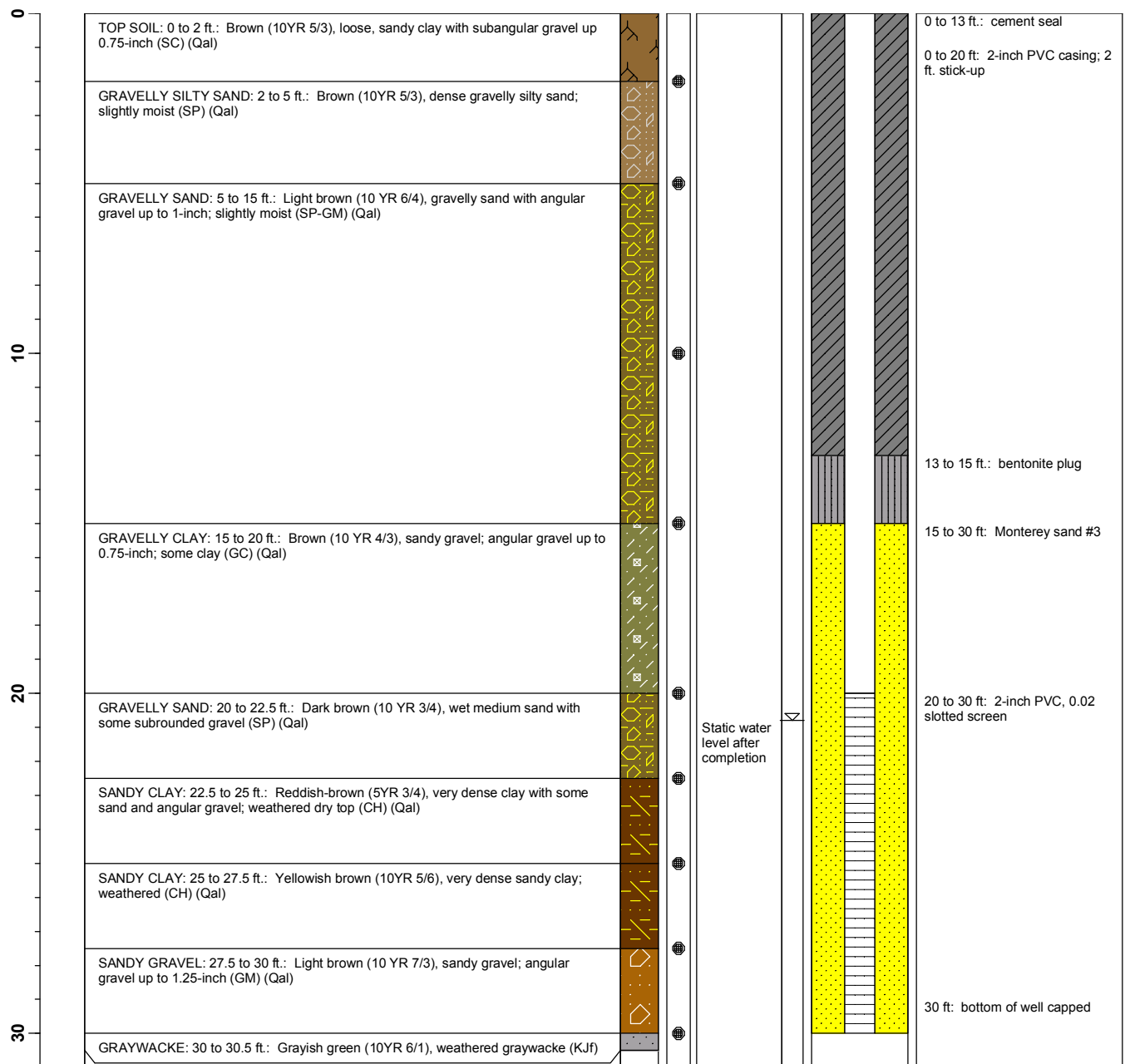


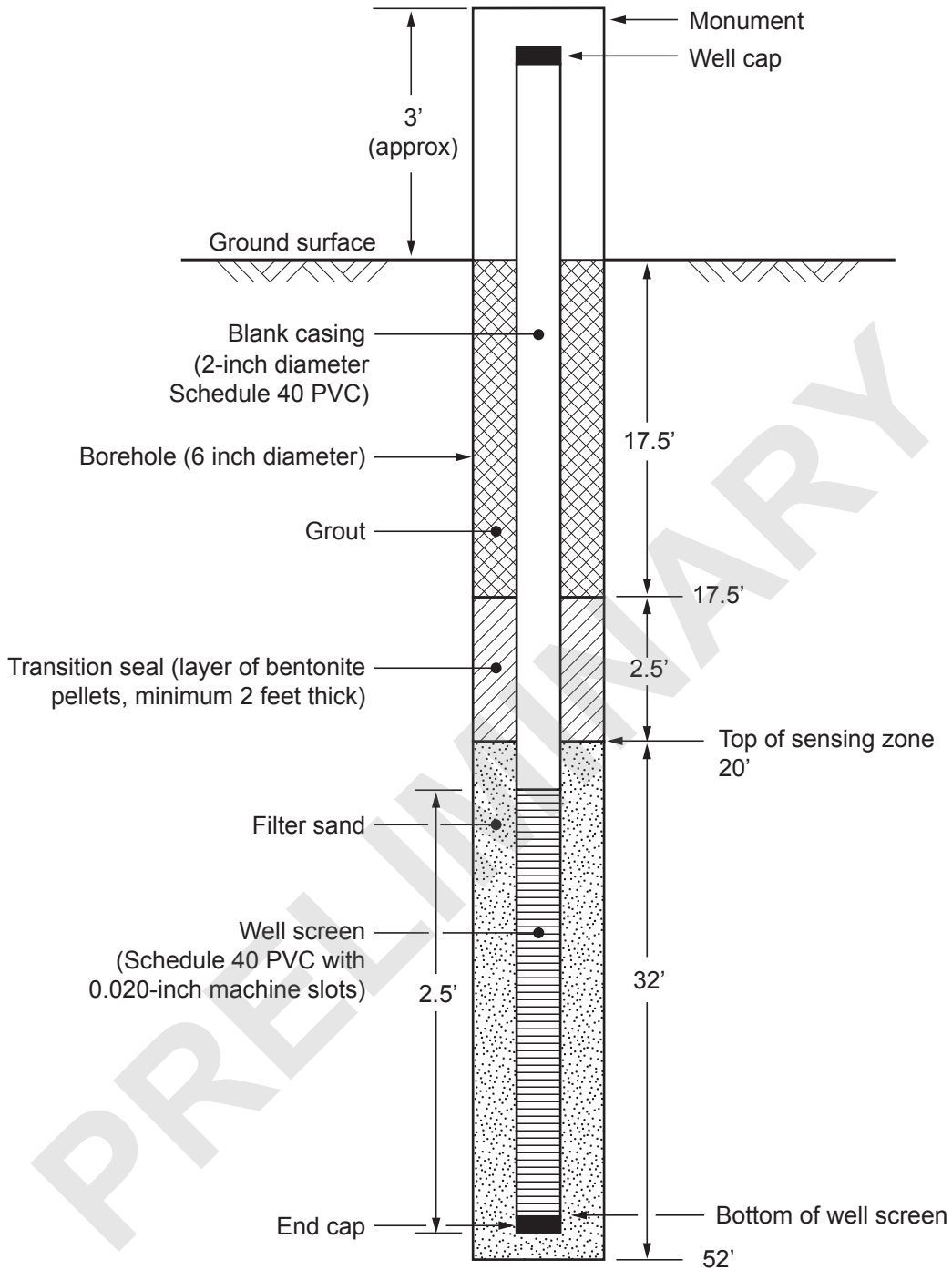
Log of boring and well construction of MW-4, Grady Ranch, Marin County, California

Property Owner and Mailing Information Skywalker Properties, Ltd. 5858 Lucas Valley Road Nicasio, CA 94946	Client: Skywalker Properties, Ltd.	Well completion date: October 20, 2009 Borehole geologist: Gustavo Porras Drilling company: Taber Consultants Driller: Andrew Elbon Drilling rig: Track mounted CME-55 Driling equipment: 8-inch hollow stem auger Depth of borehole: 30.5 Depth of casing: 30 Diameter of casing: 2-inch
Site location: Upper terrace E of Grady Cr. about 400 ft u/s of Miller Cr., near ranch road ford		
APN: 164-310-15, -17, and -19		
Latitude, Longitude: N38.04049°, W122.60134°		
Ground surface elevation: 244 feet		
Start drilling date: October 20, 2009		



Depth feet	Lithology	Sample	Hydrology	Well Construction	Remarks
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Not to scale

MONITORING WELL RW-6(P)
CONSTRUCTION DIAGRAM
Grady Ranch Development

By: _____ Date: 08/24/2009 Project No. 14648.000

AMEC Geomatrix

Figure **A3-1**

PROJECT: Grady Ranch Development Marin County, California		Log of Boring No. RW-6(P)	
BORING LOCATION: Road realignment		ELEVATION AND DATUM: 232	
DRILLING CONTRACTOR: Taber Consultants		DATE STARTED: 8/6/2009	DATE FINISHED: 8/6/2009
DRILLING EQUIPMENT: Track mounted CME-55		TOTAL DEPTH (feet): 52	MEASURING POINT: Ground Surface
DRILLING METHOD: 4-inch diameter auger		DEPTH TO FREE WATER FIRST ENCOUNTERED:	
SAMPLING METHOD: See Boring Log Explanation, Figure A2-1		DEPTH TO FREE WATER AT COMPLETION:	
HAMMER WEIGHT: 140 lbs.	HAMMER DROP: 30 inches (Auto Hammer)	LOGGED BY: C. Johnson	

DEPTH (feet)	SAMPLES			MATERIAL DESCRIPTION	LABORATORY TESTS			
	Sample No.	Sample	Blows/foot		Moisture Content (%)	Dry Density (pcf)	Other	
1				SILTY SAND (SM) loose, dark yellowish brown (10YR 4/4), dry, sand is fine to coarse, includes local subrounded gravel up to 1" max. diameter, coarse sand is rounded [ALLUVIUM]			11:50 AM start no liners sample bagged S1 13"/18" PP at 2.5= 1; <0.25; <0.25 (tsf) (sample crumbling) S2: 12"/18"	
2	1		15					
3				CLAYEY SAND (SC) loose, very dark grayish brown (10YR 3/2), moist to wet, sand is fine, fines have medium plasticity and medium toughness, scattered gravels up to 1/2" max. diameter [ALLUVIUM]			S3: 14"/18" PP at 6'= 0.75; 1; 1 (tsf) S4: 13"/18" sample pushed 18"	
4	2		2					
5								
6	3		2					
7				10': Local red and strong brown, friable sandstone up to ~1/4" diameter			S5: 11"/18" PP at 11'= 0.25; <0.25; 0.5; 0.5 (tsf)	
8	4		0					
9				11.5': Sand is mostly very fine, fines content increases			S6: 18"/18"	
10	5		6					
11				CLAYEY SAND with Gravel (SC) medium dense, dark brown (10YR 3/3), moist to slightly wet, sand mostly fine to medium, gravel mostly subrounded to rounded, variably weathered sandstone up to 1.5" max. diameter [ALLUVIUM]			S7: 16.5"/18" PP at 15.5'= 1; 1.25; >4.5 (tsf) (gravels) S8: 13"/18"	
12	6		3					
13								
14								
15								
16	7		17					
17	8		19					

GEES-SOIL_12/03_GRADY_SOIL_LOGS.GPJ GES32003-7.GDT 8/18/09

GT-1 (12/03)

DEPTH (feet)	SAMPLES			MATERIAL DESCRIPTION	LABORATORY TESTS		
	Sample No.	Sample	Blows/foot		Moisture Content (%)	Dry Density (pcf)	Other
18	8			CLAYEY SAND with Gravel (SC) (cont.)			
19							
20				20': Fines content decreases slightly, medium to coarse sand content increases, becomes mostly dark yellowish brown (10YR 4/4)			S9: 14.5"/18" PP at 21'= 1; 1; 0.5; 0.5 (tsf)
21	9		21				S10: 18"/18"
22							
23	10		15				
24							
25				CLAYEY GRAVEL with Sand (GC) medium dense, dark yellowish brown (10YR 4/4), wet, gravel is rounded to subrounded sandstone and shale clasts up to 1" max. diameter [ALLUVIUM]			S11: 15.5"/18" PP at 26'= >4.5 (tsf) (gravels)
26	11		20				S12: 18"/18"
27							
28	12		10	CLAYEY SAND (SC) medium dense, dark yellowish brown (10YR 3/4), moist, sand is fine, includes medium plasticity clay bed/layer 27.2-27.8' [ALLUVIUM]			
29							
30				30': Increase in coarse sand includes rounded gravel up to 1.5" diameter			S13: 18"/18" PP at 31.5'= 2.5; 2.5; 2; 2.25; 1.5 (tsf)
31	13		22	SANDY CLAY (CL) firm, dark yellowish brown (10YR 3/4), moist, sand is fine, fines have medium plasticity and medium to high toughness, includes local black organic flecks, locally mottled with strong brown [ALLUVIUM]			S14: 18"/18"
32							
33	14		8				
34							
35				SANDSTONE dark olive gray (5Y 3.2), moderately hard, moderately strong, dark oxide-staining on fracture surfaces [FRANCISCAN (KJf)]			S15: 7"/11"
36	15		62 11"				S16= 4"/4"
37	16		50* 4"				
38				Continued on rock log			
39							

PROJECT: Grady Ranch Development Marin County, California		Log of Boring No. RW-5	
BORING LOCATION: Planned fill mound		ELEVATION AND DATUM: 236	
DRILLING CONTRACTOR: Taber Consultants		DATE STARTED: 8/6/2009	DATE FINISHED: 8/6/2009
DRILLING EQUIPMENT: Track mounted CME-55		TOTAL DEPTH (feet): 50	MEASURING POINT: Ground Surface
DRILLING METHOD: 4-inch diameter rotary wash		DEPTH TO FREE WATER FIRST ENCOUNTERED: 31.0feet (9:00 AM 8/6/2009)	
SAMPLING METHOD: See Boring Log Explanation, Figure A2-1		DEPTH TO FREE WATER AT COMPLETION:	
HAMMER WEIGHT: --		HAMMER DROP: --	
		LOGGED BY: C. Johnson	

RT-1 (3/03)

GEES-ROCK-3/03 GRADY ROCK LOGS.GPJ GES32003-7.GDT 8/18/09

DEPTH (feet)	RUN NO.	RECOVERY (%)	RQD (%)	FRACTURING	HARDNESS	STRENGTH	WEATHERING	LITHOLOGIC DESCRIPTION	GRAPHIC LOG	DISCONTINUITY DESCRIPTION	REMARKS
38											
39											
40											
41											
42	1	60	0	VC	So	Fr	Se-Mo	SHALE (continued) crushed, soft, friable, moderately weathered, locally interbedded with sandstone			Run 1: 42-43' 10:15-10:19 AM
43				NR	NR	NR	NR		N/R		
44								43.4': Includes dark gray, very fine sandstone bed 6" thick		Jo 60, Op, Pl -Jo, 40. ss/sh contact	Run 2: 43-47' 10:26-10:33 AM
45	2	70	0	VC	So	Fr	Se-Mo	45.2': Includes greenstone(?) bed or inclusion 0.4' thick		-altered, chloritized nodule Jo 30, Op, Pl	
46				NR	NR	NR	NR		N/R		
47								47-47.8': Shear zone(?), slickensides perpendicular to shear plane, waxy green chloritized shale or serpentinite		Sh, 70, Pl, Po	Run 3: 47-50' 10:37-10:42 AM
48	3	100	0	VC	So	Fr	Se-Mo				
49					Lo	We				Jo? 50, Ir, Ro Mc	
50								Bottom of boring 50.0'			
51											
52											

FRACTURING: VC-Very Close (<0.01'), CI-Close (0.1'-0.3'), Mo-Moderate (0.3'-1'), Wi-Wide (1'-3'), and VW-Very Wide (3'-10'). HARDNESS: So-Soft, Lo-Low, Mo-Moderate, Ha-Hard, and VH-Very Hard. STRENGTH: Fr-Friable, We-Weak, Mo-Moderate, St-Strong, and Ex-Extremely Strong. WEATHERING: Fr-Fresh, SI-Slight, Mo-Moderate, and Se-Severe. DISCONTINUITY: Type (Be-Bedding, Jo-Joint, Fo-Foliation, Sh-Shear, Me-Mechanical Break, and Ve-Vein), Dip Angle, Aperture (Ti-Tight, Op-Open, He-Healed, and Fi-Filled), Surface Shape (Ir-Irregular, Pl-Planar, and Wa-Wavy), Roughness (St-Stepped, Ro-Rough, Mo-Moderately Rough, SI-Slightly Rough, Sm-Smooth, and Po-Polished).											Project No. 14648.000
											Figure A2-6

DEPTH (feet)	RUN NO.	RECOVERY (%)	RQD (%)	FRACTURING	HARDNESS	STRENGTH	WEATHERING	LITHOLOGIC DESCRIPTION	GRAPHIC LOG	DISCONTINUITY DESCRIPTION	REMARKS
51	4	100	0	Cl-Mo	Lo	Mo	Fr	INTERBEDDED SANDSTONE and SHALE (cont.)			
52								Bottom of boring 52.0'			
53											
54											
55											
56											
57											
58											
59											
60											
61											
62											
63											
64											
65											
66											
67											
68											

RT-2 (3/03)

GEES-ROCK-3/03 GRADY ROCK LOGS.GPJ GES32003-7.GDT 8/18/09

PRELIMINARY

FRACTURING: VC-Very Close (<0.01'), Cl-Close (0.1'-0.3'), Mo-Moderate (0.3'-1'), Wi-Wide (1'-3'), and VW-Very Wide (3'-10'). HARDNESS: So-Soft, Lo-Low, Mo-Moderate, Ha-Hard, and VH-Very Hard. STRENGTH: Fr-Friable, We-Weak, Mo-Moderate, St-Strong, and Ex-Extremely Strong. WEATHERING: Fr-Fresh, SI-Slight, Mo-Moderate, and Se-Severe. DISCONTINUITY: Type (BJ-Bedding Joint, Be-Bedding, Jo-Joint, Fo-Foliation, Sh-Shear, Me-Mechanical Break, and Ve-Vein), Dip Angle, Aperture (Ti-Tight, Op-Open, He-Healed, and Fi-Filled), Surface Shape (Ir-Irregular, Pl-Planar, and Wa-Wavy), Roughness (St-Stepped, Ro-Rough, Mo-Moderately Rough, SI-Slightly Rough, Sm-Smooth, and Po-Polished).

APPENDIX E

Maximum daily water temperature in Miller Creek and Grady Creek

**Appendix E. Maximum daily water temperature in Miller Creek
and tributaries, Grady Ranch, Marin County, California.**

Day	Miller Creek above Grady Bridge (at gage) (°C)	Grady Creek ¹ (1,250 feet upstream Grady Creek gage) (°C)	Miller Creek below Grady Creek (150 feet downstream Grady Bridge) (°C)
Water Year 2010			
1-Oct	dry	no data	no data
2-Oct	dry	no data	no data
3-Oct	dry	no data	no data
4-Oct	dry	no data	no data
5-Oct	dry	no data	no data
6-Oct	dry	no data	no data
7-Oct	dry	no data	no data
8-Oct	dry	no data	no data
9-Oct	dry	no data	no data
10-Oct	dry	no data	no data
11-Oct	dry	no data	no data
12-Oct	dry	no data	no data
13-Oct	dry	no data	no data
14-Oct	dry	no data	no data
15-Oct	dry	no data	no data
16-Oct	dry	no data	no data
17-Oct	dry	no data	no data
18-Oct	dry	no data	no data
19-Oct	dry	no data	no data
20-Oct	dry	no data	no data
21-Oct	dry	no data	15.748
22-Oct	dry	no data	15.676
23-Oct	dry	no data	15.652
24-Oct	dry	no data	15.533
25-Oct	dry	no data	15.485
26-Oct	dry	no data	15.247
27-Oct	dry	no data	14.385
28-Oct	dry	no data	13.594
29-Oct	dry	no data	14.266
30-Oct	dry	no data	14.745
31-Oct	dry	no data	14.697
1-Nov	dry	no data	14.888
2-Nov	dry	no data	14.768
3-Nov	dry	no data	14.601
4-Nov	dry	no data	14.098
5-Nov	dry	no data	17.272
6-Nov	dry	no data	18.747
7-Nov	dry	no data	12.775
8-Nov	dry	no data	12.147
9-Nov	dry	no data	15.724
10-Nov	dry	no data	16.63
11-Nov	dry	no data	17.796
12-Nov	dry	no data	11.516
13-Nov	dry	no data	13.016
14-Nov	dry	no data	10.222
15-Nov	dry	no data	9.977

**Appendix E. Maximum daily water temperature in Miller Creek
and tributaries, Grady Ranch, Marin County, California.**

Day	Miller Creek above Grady Bridge (at gage) (°C)	Grady Creek ¹ (1,250 feet upstream Grady Creek gage) (°C)	Miller Creek below Grady Creek (150 feet downstream Grady Bridge) (°C)
16-Nov	dry	no data	8.792
17-Nov	dry	no data	15.629
18-Nov	dry	no data	9.139
19-Nov	dry	no data	7.795
20-Nov	dry	no data	12.703
21-Nov	dry	no data	7.87
22-Nov	dry	no data	14.409
23-Nov	dry	no data	8.045
24-Nov	dry	no data	8.668
25-Nov	dry	no data	10.149
26-Nov	dry	no data	12.268
27-Nov	dry	no data	14.625
28-Nov	dry	no data	11.783
29-Nov	dry	no data	9.41
30-Nov	dry	no data	8.22
1-Dec	dry	no data	8.965
2-Dec	dry	no data	7.645
3-Dec	dry	no data	8.717
4-Dec	dry	no data	6.661
5-Dec	dry	no data	8.668
6-Dec	dry	no data	8.891
7-Dec	dry	no data	7.192
8-Dec	dry	no data	8.045
9-Dec	dry	no data	3.88
10-Dec	dry	no data	9.139
11-Dec	dry	no data	6.458
12-Dec	dry	no data	13.04
13-Dec	dry	no data	14.05
14-Dec	dry	no data	11.394
15-Dec	dry	no data	13.642
16-Dec	dry	no data	14.146
17-Dec	dry	no data	14.721
18-Dec	dry	no data	14.816
19-Dec	dry	no data	14.481
20-Dec	dry	no data	14.481
21-Dec	dry	no data	15.031
22-Dec	dry	no data	13.81
23-Dec	dry	no data	13.69
24-Dec	dry	no data	13.522
25-Dec	dry	no data	13.353
26-Dec	dry	no data	13.377
27-Dec	dry	no data	13.618
28-Dec	dry	no data	13.642
29-Dec	dry	no data	13.233
30-Dec	dry	no data	13.522
31-Dec	dry	no data	13.666

**Appendix E. Maximum daily water temperature in Miller Creek
and tributaries, Grady Ranch, Marin County, California.**

Day	Miller Creek above Grady Bridge (at gage) (°C)	Grady Creek ¹ (1,250 feet upstream Grady Creek gage) (°C)	Miller Creek below Grady Creek (150 feet downstream Grady Bridge) (°C)
1-Jan	dry	no data	13.666
2-Jan	dry	no data	13.906
3-Jan	dry	no data	13.185
4-Jan	dry	no data	13.209
5-Jan	dry	no data	12.92
6-Jan	dry	no data	13.016
7-Jan	dry	no data	12.799
8-Jan	dry	no data	12.92
9-Jan	dry	no data	12.968
10-Jan	dry	no data	12.534
11-Jan	dry	no data	12.799
12-Jan	no data	no data	13.257
13-Jan	no data	no data	12.727
14-Jan	no data	no data	12.509
15-Jan	no data	no data	12.654
16-Jan	11.73	no data	12.799
17-Jan	11.91	no data	12.582
18-Jan	11.14	no data	11.248
19-Jan	11.32	no data	11.662
20-Jan	10.91	no data	11.516
21-Jan	10.91	no data	10.98
22-Jan	10.36	no data	10.663
23-Jan	10.67	no data	11.662
24-Jan	10.49	no data	11.005
25-Jan	10.51	no data	11.102
26-Jan	11.06	no data	11.492
27-Jan	11.43	no data	12.219
28-Jan	11.27	no data	11.686
29-Jan	11.2	no data	11.856
30-Jan	11.83	no data	12.001
31-Jan	11.85	no data	12.122
1-Feb	12.13	no data	12.461
2-Feb	12.15	no data	12.509
3-Feb	12.5	no data	12.654
4-Feb	11.83	no data	12.171
5-Feb	12.37	no data	12.437
6-Feb	11.91	no data	12.001
7-Feb	11.92	no data	12.001
8-Feb	11.73	no data	11.953
9-Feb	11.38	no data	11.686
10-Feb	11.88	no data	12.074
11-Feb	11.99	no data	12.074
12-Feb	12.6	no data	12.727
13-Feb	13.33	no data	13.257
14-Feb	13.57	no data	13.305
15-Feb	14.2	no data	13.738

**Appendix E. Maximum daily water temperature in Miller Creek
and tributaries, Grady Ranch, Marin County, California.**

Day	Miller Creek above Grady Bridge (at gage) (°C)	Grady Creek ¹ (1,250 feet upstream Grady Creek gage) (°C)	Miller Creek below Grady Creek (150 feet downstream Grady Bridge) (°C)
16-Feb	14.34	no data	13.69
17-Feb	14.61	no data	13.954
18-Feb	14.02	no data	13.546
19-Feb	13.07	no data	12.944
20-Feb	12.65	no data	12.678
21-Feb	12.3	no data	12.461
22-Feb	14.21	no data	13.329
23-Feb	11.15	no data	11.88
24-Feb	12.47	no data	12.534
25-Feb	12.78	no data	12.871
26-Feb	11.88	no data	11.953
27-Feb	12.12	no data	12.243
28-Feb	13.19	no data	13.185
1-Mar	12.07	no data	12.243
2-Mar	12.24	no data	12.316
3-Mar	11.57	no data	11.71
4-Mar	11.87	no data	12.001
5-Mar	11.54	no data	11.783
6-Mar	13.17	no data	13.209
7-Mar	13.88	no data	13.762
8-Mar	12.52	no data	12.461
9-Mar	12.52	no data	12.485
10-Mar	12.98	no data	12.799
11-Mar	13.3	no data	13.401
12-Mar	11.38	no data	11.662
13-Mar	12.84	no data	12.775
14-Mar	13.65	no data	13.377
15-Mar	14.47	no data	13.954
16-Mar	15.42	no data	14.673
17-Mar	16.22	no data	15.008
18-Mar	16.66	no data	15.055
19-Mar	17.17	no data	15.031
20-Mar	15.73	no data	14.218
21-Mar	16.56	no data	14.673
22-Mar	17.58	no data	14.792
23-Mar	18.45	no data	14.792
24-Mar	dry	no data	14.792
25-Mar	dry	no data	14.481
26-Mar	dry	no data	14.816
27-Mar	dry	no data	14.936
28-Mar	dry	no data	15.008
29-Mar	dry	no data	13.834
30-Mar	21.29	no data	14.721
31-Mar	13.31	no data	14.05

**Appendix E. Maximum daily water temperature in Miller Creek
and tributaries, Grady Ranch, Marin County, California.**

Day	Miller Creek above Grady Bridge (at gage) (°C)	Grady Creek ¹ (1,250 feet upstream Grady Creek gage) (°C)	Miller Creek below Grady Creek (150 feet downstream Grady Bridge) (°C)
1-Apr	15.32	no data	15.055
2-Apr	12.75	no data	13.016
3-Apr	14.78	no data	14.792
4-Apr	10.93	no data	11.783
5-Apr	11.93	no data	12.316
6-Apr	13.69	no data	13.762
7-Apr	15.44	no data	15.127
8-Apr	15.32	no data	14.912
9-Apr	16.24	no data	15.414
10-Apr	13.69	no data	13.161
11-Apr	11.15	no data	11.394
12-Apr	12.16	no data	12.243
13-Apr	13.34	no data	13.473
14-Apr	13.59	no data	13.618
15-Apr	14.27	no data	14.17
16-Apr	15.78	no data	15.438
17-Apr	16.59	no data	15.986
18-Apr	17.17	no data	16.415
19-Apr	16.09	no data	15.438
20-Apr	15.51	no data	15.008
21-Apr	14.86	no data	14.577
22-Apr	16.64	no data	15.843
23-Apr	18.61	no data	16.63
24-Apr	18.58	no data	16.296
25-Apr	19.77	no data	16.558
26-Apr	17.62	no data	15.223
27-Apr	15.08	no data	14.601
28-Apr	14.9	no data	14.649
29-Apr	16.96	no data	15.247
30-Apr	17.66	no data	15.438
1-May	dry	no data	15.629
2-May	dry	no data	16.034
3-May	dry	no data	16.201
4-May	dry	no data	15.963
5-May	dry	no data	15.867
6-May	dry	no data	16.249
7-May	dry	no data	16.511
8-May	dry	no data	16.201
9-May	dry	no data	15.652
10-May	dry	no data	14.936
11-May	dry	no data	16.606
12-May	dry	no data	16.892
13-May	dry	no data	16.82
14-May	dry	no data	17.13
15-May	dry	no data	17.463

**Appendix E. Maximum daily water temperature in Miller Creek
and tributaries, Grady Ranch, Marin County, California.**

Day	Miller Creek above Grady Bridge (at gage) (°C)	Grady Creek ¹ (1,250 feet upstream Grady Creek gage) (°C)	Miller Creek below Grady Creek (150 feet downstream Grady Bridge) (°C)
16-May	dry	no data	16.368
17-May	dry	no data	13.714
18-May	dry	no data	15.867
19-May	dry	no data	15.963
20-May	dry	no data	17.486
21-May	dry	no data	16.487
22-May	dry	no data	16.796
23-May	dry	no data	17.772
24-May	dry	no data	16.534
25-May	dry	no data	13.666
26-May	dry	no data	18.438
27-May	dry	no data	18.723
28-May	dry	no data	16.915
29-May	dry	no data	19.651
30-May	dry	no data	20.055
31-May	dry	no data	18.628
1-Jun	dry	no data	19.246
2-Jun	dry	no data	20.246
3-Jun	dry	no data	20.793
4-Jun	dry	no data	19.365
5-Jun	dry	no data	21.795
6-Jun	dry	no data	21.652
7-Jun	dry	no data	20.674
8-Jun	dry	no data	20.103
9-Jun	dry	no data	20.198
10-Jun	dry	no data	20.579
11-Jun	dry	no data	21.819
12-Jun	dry	no data	22.944
13-Jun	dry	no data	23.472
14-Jun	dry	no data	22.298
15-Jun	dry	no data	20.365
16-Jun	dry	no data	19.793
17-Jun	dry	no data	19.579
18-Jun	dry	no data	17.772
19-Jun	dry	no data	17.772
20-Jun	dry	no data	18.39
21-Jun	dry	no data	18.937
22-Jun	dry	no data	19.008
23-Jun	dry	no data	18.081
24-Jun	dry	no data	16.225
25-Jun	dry	no data	19.508
26-Jun	dry	no data	20.007
27-Jun	dry	no data	21.485
28-Jun	dry	no data	22.776
29-Jun	dry	no data	21.819
30-Jun	dry	no data	20.865

**Appendix E. Maximum daily water temperature in Miller Creek
and tributaries, Grady Ranch, Marin County, California.**

Day	Miller Creek above Grady Bridge (at gage) (°C)	Grady Creek ¹ (1,250 feet upstream Grady Creek gage) (°C)	Miller Creek below Grady Creek (150 feet downstream Grady Bridge) (°C)
1-Jul	dry	no data	20.793
2-Jul	dry	no data	22.657
3-Jul	dry	no data	25.453
4-Jul	dry	no data	dry
5-Jul	dry	no data	dry
6-Jul	dry	no data	dry
7-Jul	dry	no data	dry
8-Jul	dry	no data	dry
9-Jul	dry	no data	dry
10-Jul	dry	no data	dry
11-Jul	dry	no data	dry
12-Jul	dry	no data	dry
13-Jul	dry	no data	dry
14-Jul	dry	no data	dry
15-Jul	dry	no data	dry
16-Jul	dry	no data	dry
17-Jul	dry	no data	dry
18-Jul	dry	no data	dry
19-Jul	dry	no data	dry
20-Jul	dry	no data	dry
21-Jul	dry	no data	dry
22-Jul	dry	no data	dry
23-Jul	dry	17.08	dry
24-Jul	dry	16.44	dry
25-Jul	dry	15.82	dry
26-Jul	dry	14.39	dry
27-Jul	dry	15.15	dry
28-Jul	dry	15.75	dry
29-Jul	dry	15.99	dry
30-Jul	dry	16.11	dry
31-Jul	dry	16.06	dry
1-Aug	dry	16.30	dry
2-Aug	dry	16.20	dry
3-Aug	dry	16.37	dry
4-Aug	dry	16.08	dry
5-Aug	dry	15.70	dry
6-Aug	dry	15.99	dry
7-Aug	dry	16.27	dry
8-Aug	dry	16.08	dry
9-Aug	dry	16.23	dry
10-Aug	dry	15.94	dry
11-Aug	dry	15.20	dry
12-Aug	dry	15.99	dry
13-Aug	dry	16.15	dry
14-Aug	dry	15.61	dry
15-Aug	dry	15.51	dry

**Appendix E. Maximum daily water temperature in Miller Creek
and tributaries, Grady Ranch, Marin County, California.**

Day	Miller Creek above Grady Bridge (at gage) (°C)	Grady Creek ¹ (1,250 feet upstream Grady Creek gage) (°C)	Miller Creek below Grady Creek (150 feet downstream Grady Bridge) (°C)
16-Aug	dry	16.20	dry
17-Aug	dry	15.96	dry
18-Aug	dry	16.20	dry
19-Aug	dry	16.70	dry
20-Aug	dry	16.34	dry
21-Aug	dry	14.98	dry
22-Aug	dry	15.96	dry
23-Aug	dry	17.44	dry
24-Aug	dry	18.82	dry
25-Aug	dry	19.98	dry
26-Aug	dry	18.11	dry
27-Aug	dry	16.65	dry
28-Aug	dry	15.01	dry
29-Aug	dry	15.13	dry
30-Aug	dry	15.03	dry
31-Aug	dry	16.20	dry
1-Sep	dry	17.39	dry
2-Sep	dry	18.56	dry
3-Sep	dry	17.96	dry
4-Sep	dry	17.15	dry
5-Sep	dry	16.75	dry
6-Sep	dry	17.23	dry
7-Sep	dry	16.46	dry
8-Sep	dry	15.25	dry
9-Sep	dry	15.37	dry
10-Sep	dry	15.94	dry
11-Sep	dry	16.49	dry
12-Sep	dry	16.11	dry
13-Sep	dry	15.41	dry
14-Sep	dry	15.51	dry
15-Sep	dry	15.10	dry
16-Sep	dry	16.49	dry
17-Sep	dry	16.51	dry
18-Sep	dry	16.51	dry
19-Sep	dry	16.39	dry
20-Sep	dry	16.27	dry
21-Sep	dry	15.39	dry
22-Sep	dry	14.63	dry
23-Sep	dry	15.18	dry
24-Sep	dry	16.06	dry
25-Sep	dry	17.03	dry
26-Sep	dry	18.79	dry
27-Sep	dry	18.01	dry
28-Sep	dry	18.75	dry
29-Sep	dry	20.46	dry
30-Sep	dry	18.06	dry

Notes;

1. Temperature logger installed in November 2009 was later not found and a replaced in July 2010. Data shows that there was flow in the pool year-round.

APPENDIX F

Analytical laboratory reports

SOIL CONTROL LAB

42 HANGAR WAY
WATSONVILLE
CALIFORNIA
95076
USA

Balance Hydrologics - San Rafael
101 Lucas Valley Road - Suite 229
San Rafael, CA 94901
Attn: Mark Woysner

Work Order #: 9100688
Reporting Date: November 2, 2009

Date Received: October 23, 2009
Project # / Name: 208164 / Grady Ranch
Water System #: NA
Sample Identification: MW-3, sampled 10/21/2009 11:10:00AM
Sampler Name / Co.: Sarah Richmond / Balance Hydrologics
Matrix: Water
Laboratory #: 9100688-01

	Results	Units	RL	State Drinking Water Limits 1	Analysis Method	Date Analyzed	Flags
General Mineral							
pH	7.1	pH Units	0.1	-	EPA 150.1	10/23/09	
Specific Conductance (EC)	450	uS/cm	1.0	1600	EPA 120.1	10/23/09	
Hydroxide as OH	ND	mg/L	2.5	-	EPA 310.1	10/23/09	
Carbonate as CO3	ND	mg/L	2.5	-	EPA 310.1	10/23/09	
Bicarbonate as HCO3	250	mg/L	2.5	-	EPA 310.1	10/23/09	
Total Alkalinity as CaCO3	210	mg/L	2.5	-	EPA 310.1	10/23/09	
Hardness	160	mg/L	5.0	-	SM 2340 B	10/30/09	
Total Dissolved Solids	250	mg/L	10	1000	EPA 160.1	10/27/09	
Nitrate as NO3	2.6	mg/L	1.0	45	EPA 300.0	10/24/09	
Chloride	27	mg/L	1.0	500	EPA 300.0	10/24/09	
Sulfate as SO4	20	mg/L	1.0	500	EPA 300.0	10/24/09	
Fluoride	0.24	mg/L	0.10	2	EPA 300.0	10/24/09	
Calcium	37	mg/L	0.50	-	EPA 200.7	10/30/09	
Magnesium	15	mg/L	0.50	-	EPA 200.7	10/30/09	
Potassium	5.0	mg/L	0.50	-	EPA 200.7	10/30/09	
Sodium	27	mg/L	5.0	-	EPA 200.7	10/30/09	
* Iron	310	ug/L	50	300	EPA 200.7	10/30/09	
* Manganese	190	ug/L	20	50	EPA 200.7	10/30/09	
Copper	ND	ug/L	50	1000	EPA 200.7	10/30/09	
Zinc	ND	ug/L	50	5000	EPA 200.7	10/30/09	

RL - are levels down to which we can quantify with reliability, a result below this level is reported as "ND" for Not Detected.

State Drinking Water Limits - as listed by California Administrative Code, Title 22.

* - a * in the left hand margin of the report means that particular constituent is above the California Drinking Water Limits.

Mike Galloway

SOIL CONTROL LAB

42 HANGAR WAY
WATSONVILLE
CALIFORNIA
95076
USA

Balance Hydrologics - San Rafael
101 Lucas Valley Road - Suite 229
San Rafael, CA 94901
Attn: Mark Woyshner

Work Order #: 9100688
Reporting Date: November 2, 2009

Date Received: October 23, 2009
Project # / Name: 208164 / Grady Ranch
Water System #: NA
Sample Identification: MW-4, sampled 10/21/2009 1:52:00PM
Sampler Name / Co.: Sarah Richmond / Balance Hydrologics
Matrix: Water
Laboratory #: 9100688-02

	Results	Units	RL	State Drinking Water Limits ¹	Analysis Method	Date Analyzed	Flags
General Mineral							
pH	6.9	pH Units	0.1	-	EPA 150.1	10/23/09	
Specific Conductance (EC)	490	uS/cm	1.0	1600	EPA 120.1	10/23/09	
Hydroxide as OH	ND	mg/L	2.5	-	EPA 310.1	10/23/09	
Carbonate as CO ₃	ND	mg/L	2.5	-	EPA 310.1	10/23/09	
Bicarbonate as HCO ₃	200	mg/L	2.5	-	EPA 310.1	10/23/09	
Total Alkalinity as CaCO ₃	170	mg/L	2.5	-	EPA 310.1	10/23/09	
Hardness	140	mg/L	5.0	-	SM 2340 B	10/30/09	
Total Dissolved Solids	260	mg/L	10	1000	EPA 160.1	10/27/09	
Nitrate as NO ₃	5.1	mg/L	1.0	45	EPA 300.0	10/24/09	
Chloride	43	mg/L	1.0	500	EPA 300.0	10/24/09	
Sulfate as SO ₄	26	mg/L	1.0	500	EPA 300.0	10/24/09	
Fluoride	0.24	mg/L	0.10	2	EPA 300.0	10/24/09	
Calcium	33	mg/L	0.50	-	EPA 200.7	10/30/09	
Magnesium	14	mg/L	0.50	-	EPA 200.7	10/30/09	
Potassium	6.3	mg/L	0.50	-	EPA 200.7	10/30/09	
Sodium	36	mg/L	5.0	-	EPA 200.7	10/30/09	
Iron	ND	ug/L	50	300	EPA 200.7	10/30/09	
* Manganese	55	ug/L	20	50	EPA 200.7	10/30/09	
Copper	ND	ug/L	50	1000	EPA 200.7	10/30/09	
Zinc	ND	ug/L	50	5000	EPA 200.7	10/30/09	

RL - are levels down to which we can quantify with reliability, a result below this level is reported as "ND" for Not Detected.

State Drinking Water Limits¹ - as listed by California Administrative Code, Title 22.

* - a * in the left hand margin of the report means that particular constituent is above the California Drinking Water Limits.

Mike Galloway



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Balance Hydrologics
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Tuesday, December 01, 2009

Lab Number: AA61876

Collection Date/Time: 11/4/2009 16:32 Sample Collector: RICHMOND, S
 Submittal Date/Time: 11/5/2009 11:00 Sample ID

Sample Description: MW1

Analyte	Method	Unit	Result	Qual	PQL	Date Analyzed
Alkalinity, Total (as CaCO ₃)	2320B	mg/L	140		2	11/5/2009
Bicarbonate (as HCO ₃ ⁻)	2320B	mg/L	171		10	11/6/2009
Boron	4500B-B	mg/L	0.05		0.05	11/23/2009
Calcium	3111B	mg/L	35		1	11/13/2009
Chloride	300.0	mg/L	11		1	11/6/2009
Magnesium	3111B	mg/L	15		1	11/13/2009
Nitrate as NO ₃	300.0	mg/L	1		1	11/6/2009
Nitrite as NO ₂ -N	300.0	mg/L	Not detected		0.1	11/6/2009
o-Phosphate-P	300.0	mg/L	Not detected		0.05	11/6/2009
pH (Laboratory)	4500-H+B	STD. Units	6.8			11/5/2009
Potassium	3111B	mg/L	1.0		0.5	11/13/2009
QC Anion Sum x 100	Calculation	%	101%			11/13/2009
QC Anion-Cation Balance	Calculation	%	2			11/29/2009
QC Cation Sum x 100	Calculation	%	106%			11/29/2009
QC Ratio TDS/SEC	Calculation		0.7			11/30/2009
SAR (Sodium Adsorption Ratio)	Suarez, 1981		0.6			11/13/2009
SAR, Adjusted	Suarez, 1981		0.5			11/30/2009
Sodium	3111B	mg/L	16		1	11/13/2009
Specific Conductance (E.C)	2510B	umhos/cm	349		1	11/5/2009
Sulfate	300.0	mg/L	19		1	11/6/2009
Total Diss. Solids	2540C	mg/L	243		10	11/11/2009

Sample Comments:

Report Approved by:

David Holland, Laboratory Director

mg/L: Milligrams per liter (=ppm)
 H = Analyzed outside of hold time
 J = Result is less than PQL

ug/L : Micrograms per liter (=ppb)
 E = Analysis performed by External Laboratory; See External Laboratory Report attachments.

PQL : Practical Quantitation Limit



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Tuesday, December 01, 2009

Lab Number: AA61877

Collection Date/Time: 11/4/2009 9:57
Submittal Date/Time: 11/5/2009 11:00

Sample Collector: RICHMOND, S
Sample ID

Sample Description: MW2

Analyte	Method	Unit	Result	Qual	PQL	Date Analyzed
Alkalinity, Total (as CaCO ₃)	2320B	mg/L	121		2	11/5/2009
Bicarbonate (as HCO ₃ ⁻)	2320B	mg/L	148		10	11/6/2009
Boron	4500B-B	mg/L	0.06		0.05	11/23/2009
Calcium	3111B	mg/L	26		1	11/13/2009
Chloride	300.0	mg/L	12		1	11/6/2009
Magnesium	3111B	mg/L	15		1	11/13/2009
Nitrate as NO ₃	300.0	mg/L	3		1	11/6/2009
Nitrite as NO ₂ -N	300.0	mg/L	Not detected		0.1	11/6/2009
o-Phosphate-P	300.0	mg/L	Not detected		0.05	11/6/2009
pH (Laboratory)	4500-H+B	STD. Units	6.9			11/5/2009
Potassium	3111B	mg/L	1.4		0.5	11/13/2009
QC Anion Sum x 100	Calculation	%	98%			11/13/2009
QC Anion-Cation Balance	Calculation	%	3			11/13/2009
QC Cation Sum x 100	Calculation	%	103%			11/29/2009
QC Ratio TDS/SEC	Calculation		0.66			11/30/2009
SAR (Sodium Adsorption Ratio)	Suarez, 1981		0.6			11/13/2009
SAR, Adjusted	Suarez, 1981		0.5			11/30/2009
Sodium	3111B	mg/L	16		1	11/13/2009
Specific Conductance (E.C)	2510B	umhos/cm	317		1	11/5/2009
Sulfate	300.0	mg/L	14		1	11/6/2009
Total Diss. Solids	2540C	mg/L	210		10	11/11/2009

Sample Comments:

Report Approved by:

David Holland, Laboratory Director

mg/L: Milligrams per liter (=ppm)

ug/L : Micrograms per liter (=ppb)

PQL : Practical Quantitation Limit

H = Analyzed outside of hold time

E = Analysis performed by External Laboratory; See External Laboratory Report attachments.

J = Result is less than PQL



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Tuesday, December 01, 2009

Lab Number: AA61878

Collection Date/Time: 11/4/2009 11:50
Submittal Date/Time: 11/5/2009 11:00

Sample Collector: RICHMOND, S
Sample ID

Sample Description: MW3

Analyte	Method	Unit	Result	Qual	PQL	Date Analyzed
Alkalinity, Total (as CaCO ₃)	2320B	mg/L	122		2	11/5/2009
Bicarbonate (as HCO ₃ ⁻)	2320B	mg/L	149		10	11/6/2009
Boron	4500B-B	mg/L	0.06		0.05	11/23/2009
Calcium	3111B	mg/L	26		1	11/13/2009
Chloride	300.0	mg/L	10		1	11/6/2009
Magnesium	3111B	mg/L	14		1	11/13/2009
Nitrate as NO ₃	300.0	mg/L	2		1	11/6/2009
Nitrite as NO ₂ -N	300.0	mg/L	Not detected		0.1	11/6/2009
o-Phosphate-P	300.0	mg/L	Not detected		0.05	11/6/2009
pH (Laboratory)	4500-H+B	STD. Units	7.0			11/5/2009
Potassium	3111B	mg/L	2.1		0.5	11/13/2009
QC Anion Sum x 100	Calculation	%	97%			11/13/2009
QC Anion-Cation Balance	Calculation	%	1			11/29/2009
QC Cation Sum x 100	Calculation	%	99%			11/29/2009
QC Ratio TDS/SEC	Calculation		0.71			11/30/2009
SAR (Sodium Adsorption Ratio)	Suarez, 1981		0.5			11/13/2009
SAR, Adjusted	Suarez, 1981		0.4			11/30/2009
Sodium	3111B	mg/L	13		1	11/13/2009
Specific Conductance (E.C)	2510B	umhos/cm	309		1	11/5/2009
Sulfate	300.0	mg/L	11		1	11/6/2009
Total Diss. Solids	2540C	mg/L	218		10	11/11/2009

Sample Comments:

Report Approved by:

David Holland, Laboratory Director

mg/L: Milligrams per liter (=ppm)

ug/L : Micrograms per liter (=ppb)

PQL : Practical Quantitation Limit

H = Analyzed outside of hold time

E = Analysis performed by External Laboratory; See External Laboratory Report attachments.

J = Result is less than PQL



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Tuesday, December 01, 2009

Lab Number: AA61879

Collection Date/Time: 11/4/2009 17:09
Submittal Date/Time: 11/5/2009 11:00

Sample Collector: RICHMOND, S
Sample ID

Sample Description: MW4

Analyte	Method	Unit	Result	Qual	PQL	Date Analyzed
Alkalinity, Total (as CaCO ₃)	2320B	mg/L	139		2	11/6/2009
Bicarbonate (as HCO ₃ ⁻)	2320B	mg/L	170		10	12/1/2009
Boron	4500B-B	mg/L	0.26		0.05	11/23/2009
Calcium	3111B	mg/L	34		1	11/13/2009
Chloride	300.0	mg/L	140		1	11/6/2009
Magnesium	3111B	mg/L	12		1	11/13/2009
Nitrate as NO ₃	300.0	mg/L	1		1	11/6/2009
Nitrite as NO ₂ -N	300.0	mg/L	Not detected		0.1	11/6/2009
o-Phosphate-P	300.0	mg/L	Not detected		0.05	11/6/2009
pH (Laboratory)	4500-H+B	STD. Units	8.2			11/5/2009
Potassium	3111B	mg/L	5.0		0.5	11/13/2009
QC Anion Sum x 100	Calculation	%	91%			12/1/2009
QC Anion-Cation Balance	Calculation	%	1			12/1/2009
QC Cation Sum x 100	Calculation	%	93%			12/1/2009
QC Ratio TDS/SEC	Calculation		0.6			11/30/2009
SAR (Sodium Adsorption Ratio)	Suarez, 1981		4.5			12/1/2009
SAR, Adjusted	Suarez, 1981		4.7			12/1/2009
Sodium	3111B	mg/L	120		1	11/13/2009
Specific Conductance (E.C)	2510B	umhos/cm	866		1	11/5/2009
Sulfate	300.0	mg/L	56		1	11/6/2009
Total Diss. Solids	2540C	mg/L	518		10	11/11/2009

Sample Comments:

Report Approved by:

David Holland, Laboratory Director

mg/L: Milligrams per liter (=ppm)

ug/L : Micrograms per liter (=ppb)

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H = Analyzed outside of hold time

E = Analysis performed by External Laboratory; See External Laboratory Report attachments.

J = Result is less than PQL



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Tuesday, December 15, 2009

Lab Number: AA62174

Collection Date/Time: 11/17/2009 16:00
Submittal Date/Time: 11/18/2009 14:00

Sample Collector: WOYSHNER, M
Sample ID: 208164

Sample Description: Landmark Cr.

Analyte	Method	Unit	Result	Qual	PQL	Date Analyzed
Alkalinity, Total (as CaCO ₃)	2320B	mg/L	171		2	11/18/2009
Bicarbonate (as HCO ₃ ⁻)	2320B	mg/L	209		10	11/19/2009
Boron	4500B-B	mg/L	0.06		0.05	12/2/2009
Calcium	3111B	mg/L	45		1	12/4/2009
Chloride	300.0	mg/L	12		1	11/19/2009
Magnesium	3111B	mg/L	14		1	12/4/2009
Nitrate as NO ₃	300.0	mg/L	Not detected		1	11/19/2009
Nitrite as NO ₂ -N	300.0	mg/L	Not detected		0.1	11/19/2009
o-Phosphate-P	300.0	mg/L	Not detected		0.1	11/19/2009
pH (Laboratory)	4500-H+B	STD. Units	7.7			11/18/2009
Potassium	3111B	mg/L	0.8		0.5	12/4/2009
QC Anion Sum x 100	Calculation	%	105%			12/14/2009
QC Anion-Cation Balance	Calculation	%	0			12/14/2009
QC Cation Sum x 100	Calculation	%	105%			12/14/2009
QC Ratio TDS/SEC	Calculation		0.62			12/2/2009
SAR (Sodium Adsorption Ratio)	Suarez, 1981		0.6			12/4/2009
SAR, Adjusted	Suarez, 1981		0.7			12/4/2009
Sodium	3111B	mg/L	19		1	12/4/2009
Specific Conductance (E.C)	2510B	umhos/cm	405		1	11/18/2009
Sulfate	300.0	mg/L	23		1	11/19/2009
Total Diss. Solids	2540C	mg/L	250		10	11/30/2009

Sample Comments:

Report Approved by:

David Holland, Laboratory Director

mg/L: Milligrams per liter (=ppm)

ug/L : Micrograms per liter (=ppb)

PQL : Practical Quantitation Limit

H = Analyzed outside of hold time

E = Analysis performed by External Laboratory; See External Laboratory Report attachments.

J = Result is less than PQL



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841 Folger Ave
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Tuesday, December 15, 2009

Lab Number: AA62175

Collection Date/Time: 11/17/2009 16:30
Submittal Date/Time: 11/18/2009 14:00

Sample Collector: WOYSHNER, M
Sample ID: 208164

Sample Description: Test Well B

Analyte	Method	Unit	Result	Qual	PQL	Date Analyzed
Alkalinity, Total (as CaCO ₃)	2320B	mg/L	190		2	11/18/2009
Bicarbonate (as HCO ₃ ⁻)	2320B	mg/L	232		10	12/14/2009
Boron	4500B-B	mg/L	8.38		0.05	12/11/2009
Calcium	3111B	mg/L	10		1	12/4/2009
Chloride	300.0	mg/L	194		1	11/19/2009
Magnesium	3111B	mg/L	2		1	12/4/2009
Nitrate as NO ₃	300.0	mg/L	1		1	11/19/2009
Nitrite as NO ₂ -N	300.0	mg/L	Not detected		0.1	11/19/2009
o-Phosphate-P	300.0	mg/L	Not detected		0.1	11/19/2009
pH (Laboratory)	4500-H+B	STD. Units	8.5			11/18/2009
Potassium	3111B	mg/L	4.4		0.5	12/4/2009
QC Anion Sum x 100	Calculation	%	91%			12/14/2009
QC Anion-Cation Balance	Calculation	%	3			12/14/2009
QC Cation Sum x 100	Calculation	%	97%			12/4/2009
QC Ratio TDS/SEC	Calculation		0.62			12/2/2009
SAR (Sodium Adsorption Ratio)	Suarez, 1981		16.0			12/4/2009
SAR, Adjusted	Suarez, 1981		15.5			12/14/2009
Sodium	3111B	mg/L	212		1	12/4/2009
Specific Conductance (E.C)	2510B	umhos/cm	1033		1	11/18/2009
Sulfate	300.0	mg/L	5		1	11/19/2009
Total Diss. Solids	2540C	mg/L	638		10	11/30/2009

Sample Comments:

Report Approved by:

David Holland, Laboratory Director

mg/L: Milligrams per liter (=ppm)

ug/L : Micrograms per liter (=ppb)

PQL : Practical Quantitation Limit

H = Analyzed outside of hold time

E = Analysis performed by External Laboratory; See External Laboratory Report attachments.

J = Result is less than PQL



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Balance Hydrologics
Mark Woysner
841 Folger Ave
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Tuesday, December 15, 2009

Lab Number: AA62176

Collection Date/Time: 11/17/2009 17:00
Submittal Date/Time: 11/18/2009 14:00

Sample Collector: WOYSHNER, M
Sample ID: 208164

Sample Description: Grady Cr.

Analyte	Method	Unit	Result	Qual	PQL	Date Analyzed
Alkalinity, Total (as CaCO ₃)	2320B	mg/L	163		2	11/18/2009
Bicarbonate (as HCO ₃ ⁻)	2320B	mg/L	199		10	11/19/2009
Boron	4500B-B	mg/L	0.14		0.05	12/2/2009
Calcium	3111B	mg/L	50		1	12/4/2009
Chloride	300.0	mg/L	10		1	11/19/2009
Magnesium	3111B	mg/L	13		1	12/4/2009
Nitrate as NO ₃	300.0	mg/L	1		1	11/19/2009
Nitrite as NO ₂ -N	300.0	mg/L	Not detected		0.1	11/19/2009
o-Phosphate-P	300.0	mg/L	Not detected		0.1	11/19/2009
pH (Laboratory)	4500-H+B	STD. Units	8.2			11/18/2009
Potassium	3111B	mg/L	0.8		0.5	12/4/2009
QC Anion Sum x 100	Calculation	%	106%			12/4/2009
QC Anion-Cation Balance	Calculation	%	1			12/14/2009
QC Cation Sum x 100	Calculation	%	109%			12/14/2009
QC Ratio TDS/SEC	Calculation		0.57			12/14/2009
SAR (Sodium Adsorption Ratio)	Suarez, 1981		0.5			12/4/2009
SAR, Adjusted	Suarez, 1981		0.6			12/4/2009
Sodium	3111B	mg/L	16		1	12/4/2009
Specific Conductance (E.C)	2510B	umhos/cm	394		1	11/18/2009
Sulfate	300.0	mg/L	29		1	11/19/2009
Total Diss. Solids	2540C	mg/L	225		10	11/30/2009

Sample Comments:

Report Approved by:

David Holland, Laboratory Director

mg/L: Milligrams per liter (=ppm)

ug/L : Micrograms per liter (=ppb)

PQL : Practical Quantitation Limit

H = Analyzed outside of hold time

E = Analysis performed by External Laboratory; See External Laboratory Report attachments.

J = Result is less than PQL



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ELAP Certification Number: 2385

Lab Number: AA62502

Collection Date/Time: 12/2/2009 13:00 Sample Collector: WOYSHNER, M
 Submittal Date/Time: 12/3/2009 12:50 Sample ID: 208164

Sample Description: RW-6

Analyte	Method	Unit	Result	Qual	PQL	Date Analyzed
Alkalinity, Total (as CaCO ₃)	2320B	mg/L	148		2	12/3/2009
Bicarbonate (as HCO ₃ ⁻)	2320B	mg/L	181		10	12/14/2009
Boron	4500B-B	mg/L	0.30		0.05	12/11/2009
Calcium	3111B	mg/L	31		1	12/8/2009
Chloride	300.0	mg/L	20		1	12/3/2009
Magnesium	3111B	mg/L	20		1	12/8/2009
Nitrate as NO ₃	300.0	mg/L	0.2		1	12/3/2009
Nitrite as NO ₂ -N	300.0	mg/L	Not detected		0.1	12/3/2009
o-Phosphate-P	300.0	mg/L	Not detected		0.1	12/3/2009
pH (Laboratory)	4500-H+B	STD. Units	7.3			12/3/2009
Potassium	3111B	mg/L	2.4		0.5	12/8/2009
QC Anion Sum x 100	Calculation	%	98%			12/14/2009
QC Anion-Cation Balance	Calculation	%	3			12/14/2009
QC Cation Sum x 100	Calculation	%	103%			12/14/2009
QC Ratio TDS/SEC	Calculation		0.62			12/11/2009
SAR (Sodium Adsorption Ratio)	Suarez, 1981		0.8			12/14/2009
SAR, Adjusted	Suarez, 1981		0.7			12/14/2009
Sodium	3111B	mg/L	22		1	12/8/2009
Specific Conductance (E.C)	2510B	umhos/cm	407		1	12/3/2009
Sulfate	300.0	mg/L	22		1	12/3/2009
Total Diss. Solids	2540C	mg/L	253		10	12/4/2009

Sample Comments:

Report Approved by:

David Holland
 Laboratory Director

mg/L: Milligrams per liter (=ppm)

ug/L : Micrograms per liter (=ppb)

PQL : Practical Quantitation Limit

H = Analyzed outside of hold time

E = Analysis performed by External Laboratory; See External Laboratory Report attachments.

D = Method deviates from standard method due to insufficient sample for MS/MSD

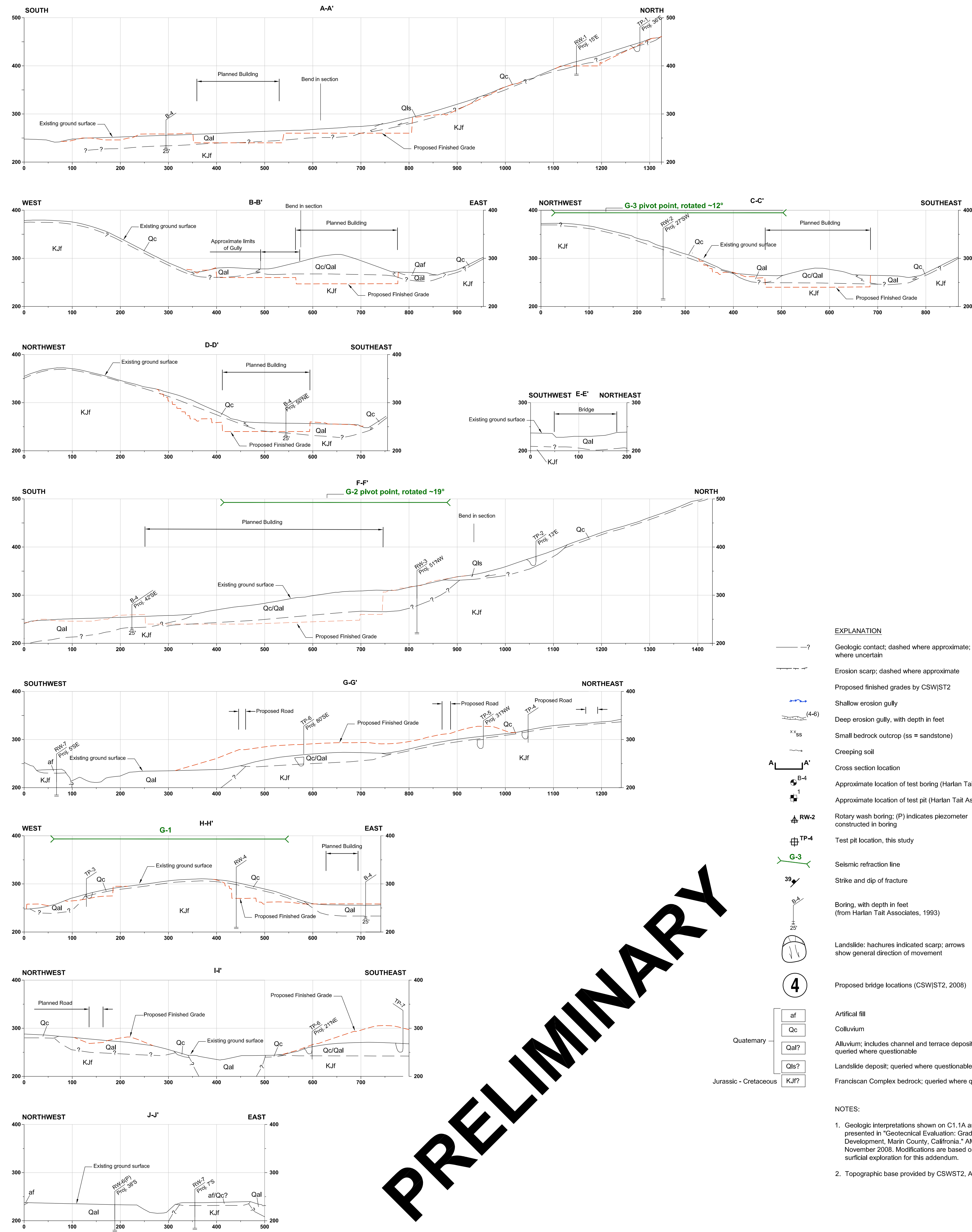
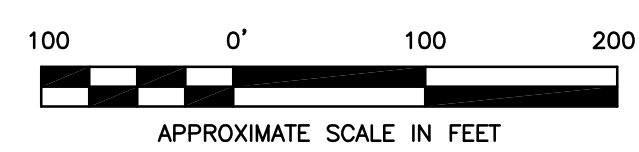
J = Result is less than PQL

APPENDIX G

**Geology Plate (AMEC Geomatrix, November 2008),
boring logs (August 2009), and
supplemental seismic refraction lines (October 2009)**



GEOLOGIC MAP, SITE PLAN, GRADING PLAN



CROSS-SECTIONS

- EXPLANATION**
- - - - - ? Geologic contact; dashed where approximate; queried where uncertain
 - - - - - Erosion scarp; dashed where approximate
 - - - - - Proposed finished grades by CSWST2
 - - - - - (4-6) Shallow erosion gully
 - - - - - Deep erosion gully, with depth in feet
 - - - - - Small bedrock outcrop (ss = sandstone)
 - - - - - Creeping soil
 - - - - - Cross section location
 - - - - - Approximate location of test boring (Harlan Tait Associates, 1993)
 - - - - - Approximate location of test pit (Harlan Tait Associates, 1993)
 - - - - - RW-2 Rotary wash boring; (P) indicates piezometer constructed in boring
 - - - - - TP-4 Test pit location, this study
 - - - - - G-3 Seismic refraction line
 - - - - - 39° Strike and dip of fracture
 - - - - - Boring, with depth in feet (from Harlan Tait Associates, 1993)
 - - - - - Landslide: hachures indicated scarp; arrows show general direction of movement
 - - - - - 4 Proposed bridge locations (CSWST2, 2008)
 - - - - - af Artificial fill
 - - - - - Qc Colluvium
 - - - - - Qal? Alluvium; includes channel and terrace deposits; queried where questionable
 - - - - - Qls? Landslide deposit; queried where questionable
 - - - - - KJf? Franciscan Complex bedrock; queried where questionable
- Quaternary
Jurassic - Cretaceous

- NOTES:**
- Geologic interpretations shown on C1.1A are based on C1.1 presented in "Geotechnical Evaluation: Grady Ranch Development, Marin County, California." AMEC Geomatrix, November 2008. Modifications are based on additional surficial exploration for this addendum.
 - Topographic base provided by CSWST2, August 3, 2009.

PRELIMINARY

CSWST2

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Rev	Date	Description	Designed	Drawn	Checked

GRADY RANCH PRECISE DEVELOPMENT PLAN
**SITE GEOLOGY, CROSS SECTIONS, AND
SLOPE STABILIZATION PLAN (1 OF 2)**

Skywalker Properties Ltd.

County Of
Marin

State Of
California

Prepared Under the Direction of:

REGISTERED PROFESSIONAL ENGINEER
No. 2018
James S. French
EXP. 06/30/2010
STATE OF CALIFORNIA

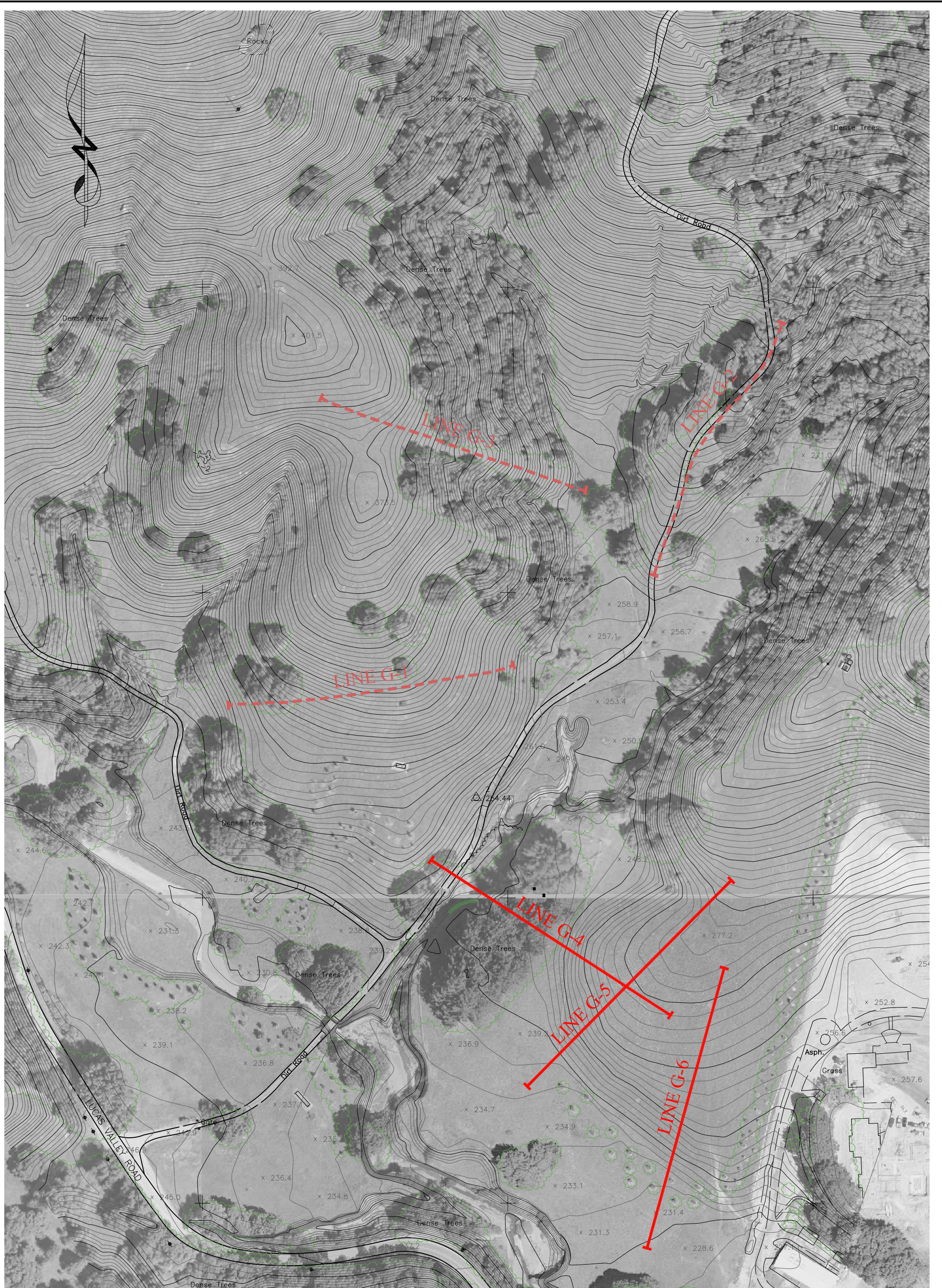
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Date: November 21, 2008

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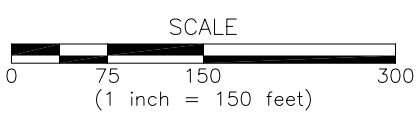
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LEGEND

	SEISMIC REFRACTION PROFILE (10/2009 SURVEY)
	SEISMIC REFRACTION PROFILE (07/2009 SURVEY)

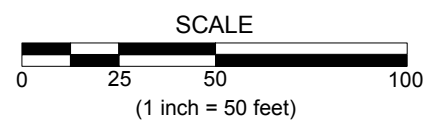
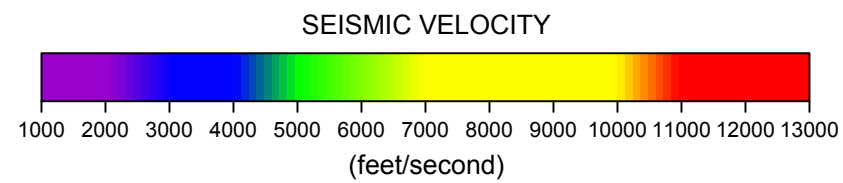
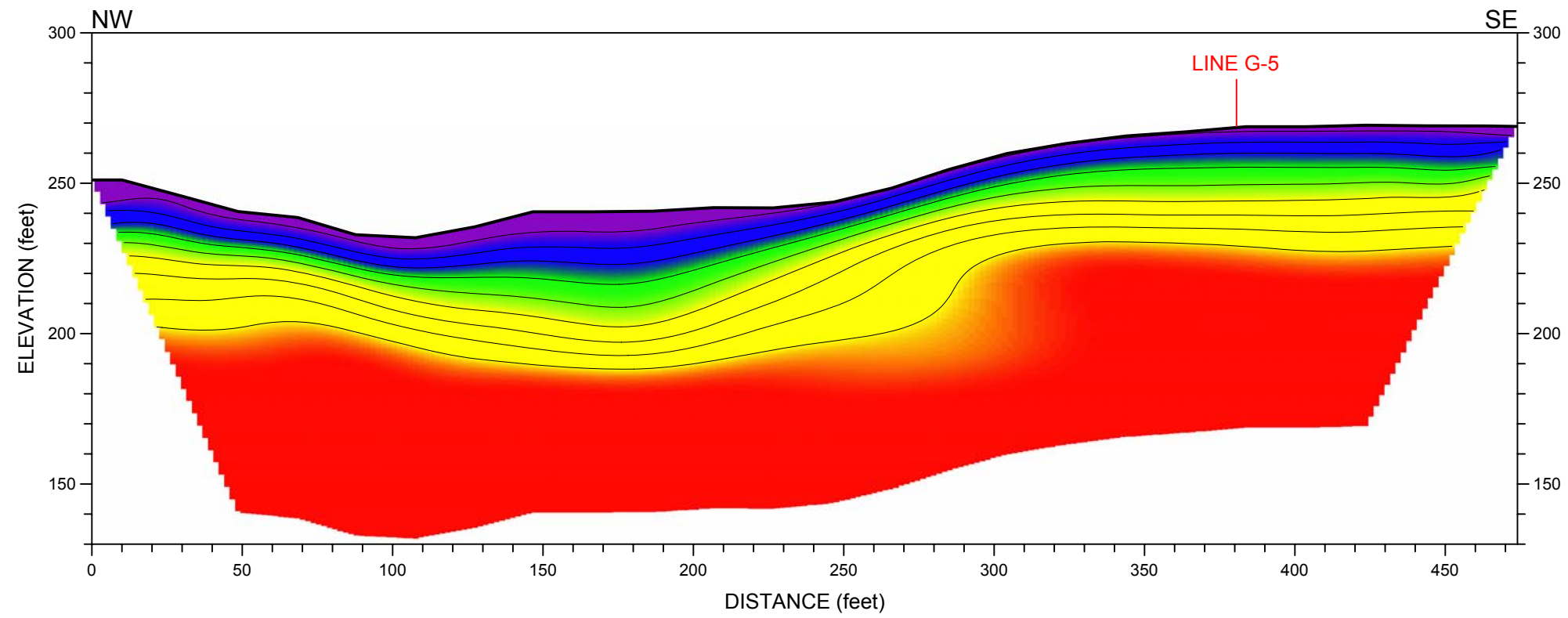
NOTE: BASE MAP PROVIDED BY AMEC GEOMATRIX




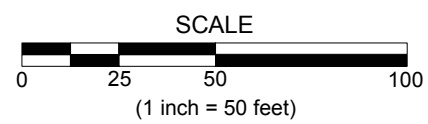
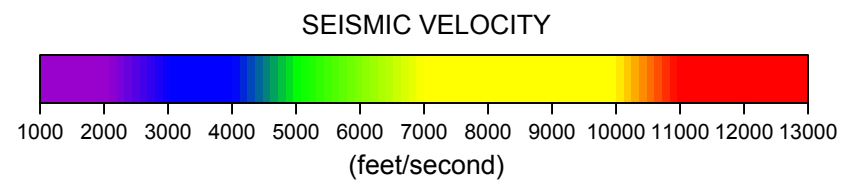
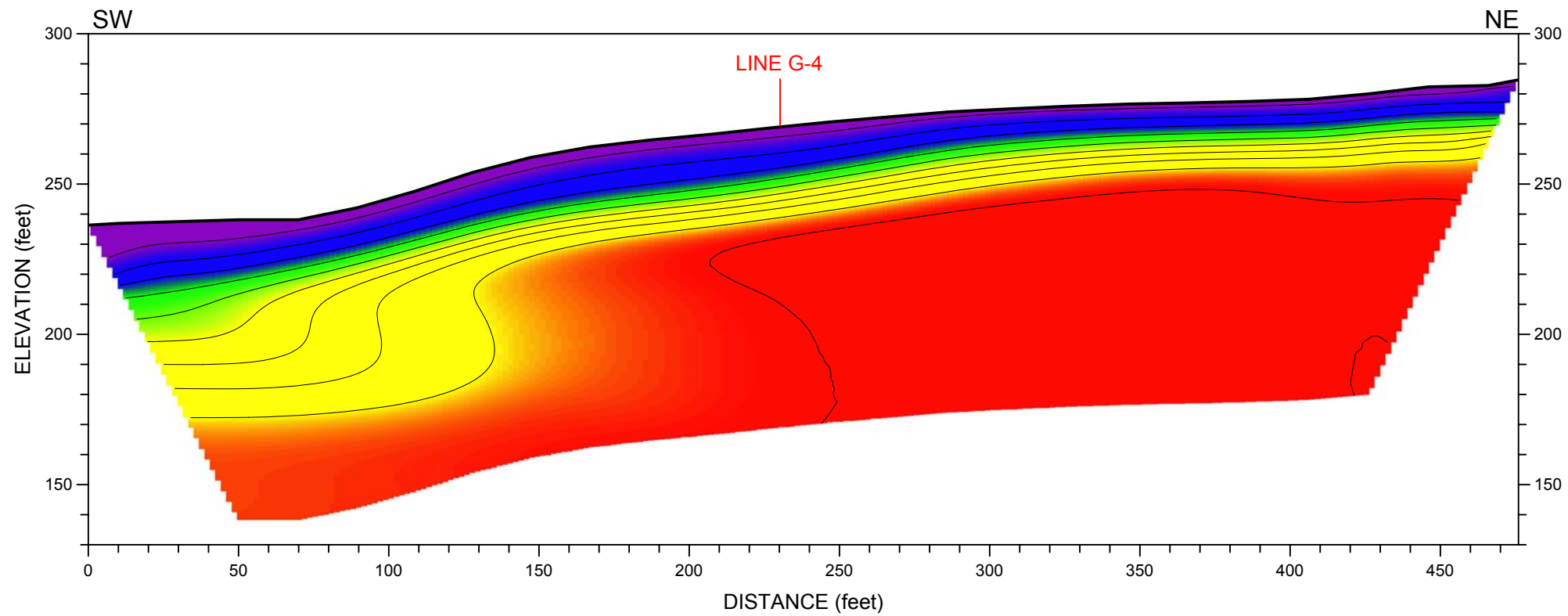
NORCAL


**SITE LOCATION MAP
GRADY RANCH**

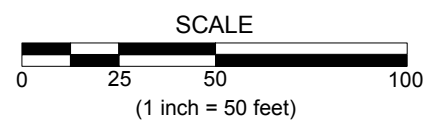
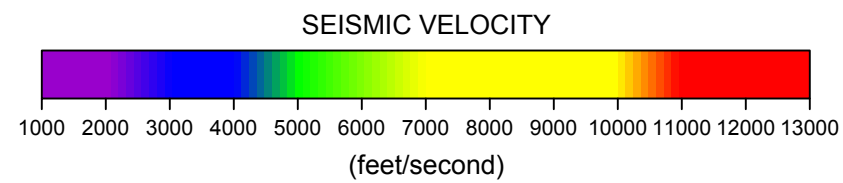
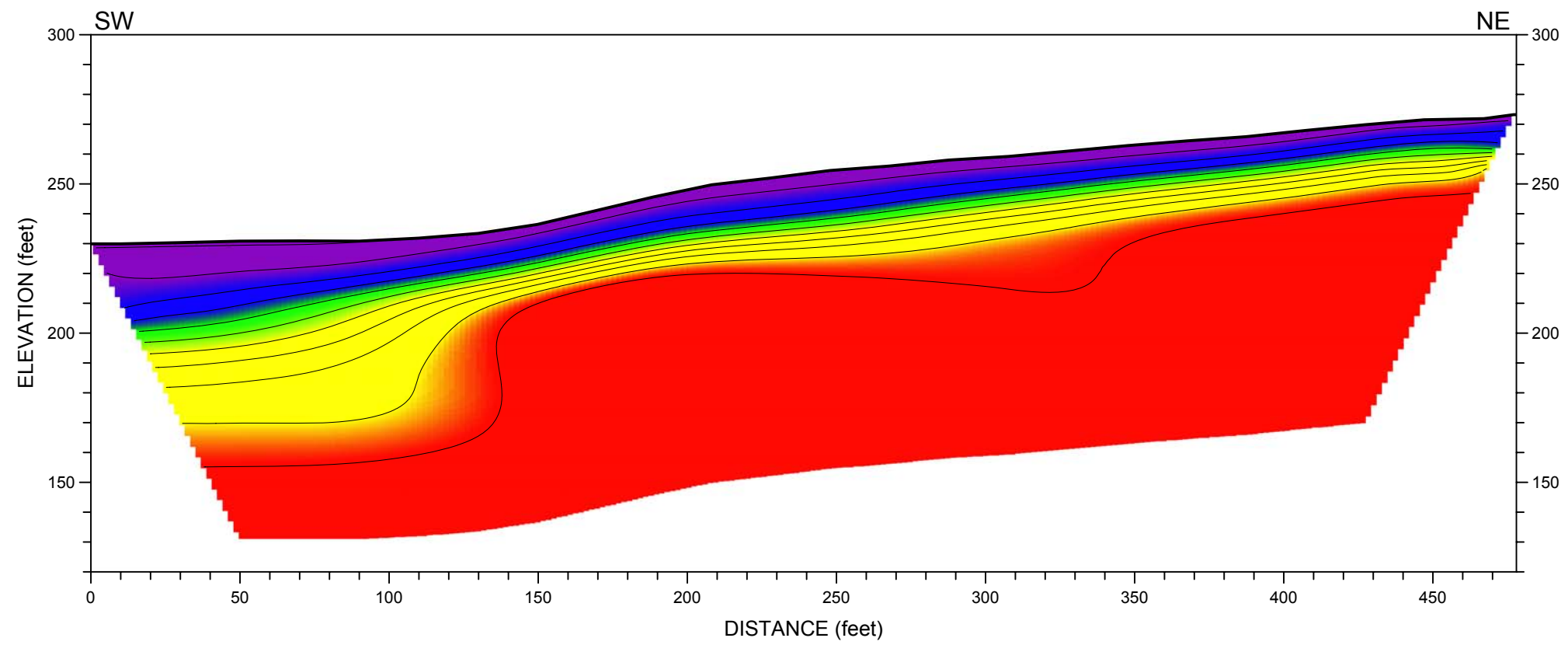
LOCATION: LUCAS VALLEY, CALIFORNIA		PLATE 1
CLIENT: AMEC GEOMATRIX		
JOB #: 09-325.55	NORCAL GEOPHYSICAL CONSULTANTS INC.	
DATE: OCT. 2009	DRAWN BY: G.RANDALL	APPROVED BY: DJK




 NORCAL	SEISMIC REFRACTION PROFILE LINE G-4 GRADY RANCH		PLATE 2
	LOCATION: LUCAS VALLEY, CALIFORNIA		
	CLIENT: AMEC GEOMATRIX		
	NORCAL GEOPHYSICAL CONSULTANTS INC.		
JOB #: 09-325.55	DATE: OCT. 2009	DRAWN BY: G.RANDALL	APPROVED BY: DJK



 NORCAL	SEISMIC REFRACTION PROFILE LINE G-5 GRADY RANCH	
	LOCATION: LUCAS VALLEY, CALIFORNIA	
	CLIENT: AMEC GEOMATRIX	3
	JOB #: 09-325.55	
DATE: OCT. 2009	DRAWN BY: G.RANDALL	APPROVED BY: DJK



 NORCAL	SEISMIC REFRACTION PROFILE LINE G-6 GRADY RANCH		PLATE 4
	LOCATION: LUCAS VALLEY, CALIFORNIA		
	CLIENT: AMEC GEOMATRIX		
	JOB #: 09-325.55	NORCAL GEOPHYSICAL CONSULTANTS INC.	
DATE: OCT. 2009	DRAWN BY: G.RANDALL	APPROVED BY: DJK	

PROJECT: Grady Ranch Development Marin County, California		Log of Boring No. RW-1	
BORING LOCATION: Planned water tank		ELEVATION AND DATUM: 408	
DRILLING CONTRACTOR: Taber Consultants		DATE STARTED: 8/5/2009	DATE FINISHED: 8/5/2009
DRILLING EQUIPMENT: Track mounted CME-55		TOTAL DEPTH (feet): 26	MEASURING POINT: Ground Surface
DRILLING METHOD: 4-inch diameter auger		DEPTH TO FREE WATER FIRST ENCOUNTERED:	
SAMPLING METHOD: See Boring Log Explanation, Figure A2-1		DEPTH TO FREE WATER AT COMPLETION:	
HAMMER WEIGHT: 140 lbs.	HAMMER DROP: 30 inches (Auto Hammer)	LOGGED BY: C. Johnson	

DEPTH (feet)	SAMPLES			MATERIAL DESCRIPTION	LABORATORY TESTS		
	Sample No.	Sample	Blows/foot		Moisture Content (%)	Dry Density (pcf)	Other
1				CLAYEY SAND (SC) dark yellowish brown (10YR 3/4), dry, loose to medium dense, sand is fine, contains abundant rootlets, includes scattered coarse sand/fine gravel-sized clasts of friable red and yellowish brown sandstone [COLLUVIUM]			~3:30 PM start
2	1		10				S1 14.5"/18" PP at 2"= 2.0; >4.5; >4.5; 3.5 (tsf)
3				SANDSTONE dark yellowish brown (10YR 3/4), soft, friable, severely weathered, local rootlets, local weathered clay fractures(?), locally oxidized to strong brown, slightly silty [FRANCISCAN (KJf)]			PP at 2.5"= >4.5; 3.75; 2.5 (tsf) S2 15.5"/18"
4							
5							S3 15"/18" PP at 5.5"= 3.25; >4.5 (tsf)
6	3		20				S4 16"/18"
7				6.5': Rootlets grade out			
8							
9							
10							S5 5"/5" S6 12"/11"
11	5		50				Switch to mud rotary
12	6		73 11"				
13				Continued on rock log			
14							
15							
16							
17							

GEES-SOIL 12/03 GRADY_SOIL_LOGS.GPJ GES32003-7.GDT 8/18/09

PROJECT: Grady Ranch Development
Marin County, California

Log of Boring No. RW-1

BORING LOCATION: Planned water tank

ELEVATION AND DATUM:
408

DRILLING CONTRACTOR: Taber Consultants

DATE STARTED:
8/5/2009

DATE FINISHED:
8/5/2009

DRILLING EQUIPMENT: Track mounted CME-55

TOTAL DEPTH (feet):
26

MEASURING POINT:
Ground Surface

DRILLING METHOD: 4-inch diameter rotary wash

DEPTH TO FREE WATER FIRST ENCOUNTERED:

SAMPLING METHOD: See Boring Log Explanation, Figure A2-1

DEPTH TO FREE WATER AT COMPLETION:

HAMMER WEIGHT: --

HAMMER DROP: --

LOGGED BY:
C. Johnson

DEPTH (feet)	RUN NO.	RECOVERY (%)	RQD (%)	FRACTURING	HARDNESS	STRENGTH	WEATHERING	LITHOLOGIC DESCRIPTION	GRAPHIC LOG	DISCONTINUITY DESCRIPTION	REMARKS
1											5" diameter casing driven to 5 feet
2											
3											
4											
5											
6											
7											
8											
9											
10											
11	1	120	0	Cl	So	Fr	Se	Continued from soil log SANDSTONE (continued) mostly dark yellowish brown (10YR 4/4) 11.5': Sand becomes fine to medium, increase in strength and hardness, includes local black shaley coarse sand			Run 1: 11-11.5' 4:16-4:18 PM
12											Run 2: 11.5-16.5' 4:32-4:26 PM
13											
14	2	82	8	Cl	Lo	We	Mo-Se				
15											

FRACTURING: VC-Very Close (<0.01'), Cl-Close (0.1'-0.3'), Mo-Moderate (0.3'-1'), Wi-Wide (1'-3'), and VW-Very Wide (3'-10'). HA RDNESS: So-Soft, Lo-Low, Mo-Moderate, Ha-Hard, and VH-Very Hard. STRENGTH: Fr-Friable, We-Weak, Mo-Moderate, St-Strong, and Ex-Extremely Strong. WEATHE RING: Fr-Fresh, Sl-Slight, Mo-Moderate, and Se-Severe. DISCONTINUITY: Type (Be-Bedding, Jo-Joint, Fo-Foliation, Sh-Shear, Me-Mechanical Break, and Ve-Vein), Dip Angle, Aperture (Ti-Tight, Op-Open, He-Healed, and Fi-Filled), Surface Shape (Ir-Irregular, Pl-Planar, and Wa-Wavy), Roughness (St-Ste pped, Ro-Rough, Mo-Moderately Rough, Sl-Slightly Rough, Sm-Smooth, and Po-Polished).

Project No.
14648.000

Figure A2-2

RT-1 (3/03)

GEES-ROCK-3/03 GRADY_ROCK_LOGS.GPJ GES32003-7.GDT 8/18/09

RT-2 (3/03)

GEES-ROCK-3/03 GRADY_ROCK_LOGS.GPJ GES32003-7.GDT 8/18/09

DEPTH (feet)	RUN NO.	RECOVERY (%)	ROD (%)	FRACTURING	HARDNESS	STRENGTH	WEATHERING	LITHOLOGIC DESCRIPTION	GRAPHIC LOG	DISCONTINUITY DESCRIPTION	REMARKS
16	2			NR	NR	NR	NR	SANDSTONE (cont.)	N/R		
17									Jo 30 Fi Pl clay-lined		Run 3: 16.5-21.5' 4:31-4:37 PM
18									Jo 30, Op Pl Sm Jo 20, Op Pl		
19	3	84	0	Cl-VC	Lo	We	Mo-Se	19.5': Becomes mostly olive brown (2.5Y 4/4)	Jo 40 Op Pl Be, 30		
20									Jo, 65, Op Pl, Sm, Mg-ox Jo, 5, Op Pl Mg-ox		
21				NR	NR	NR	NR		N/R		
22									Me Jo, 40, Op Pl, Sm		Run 4: 21.5-26' 4:39-4:44 PM
23									Jo, 40, Op Pl, Sm, Mg-ox stained Jo 50 Op Pl. Jo 30 Op Pl		
24	4	84	0	Cl-VC	Lo	We	Mo-Se		Jo 60 Op Pl Sm Jo 65 Op Pl Mg-ox stained		
25				NR	NR	NR	NR		N/R		
26								Bottom of boring 26.0'			
27											
28											
29											
30											
31											
32											
33											

FRACTURING: VC-Very Close (<0.01'), Cl-Close (0.1'-0.3'), Mo-Moderate (0.3'-1'), Wi-Wide (1'-3'), and VW-Very Wide (3'-10'). HA RDNESS: So-Soft, Lo-Low, Mo-Moderate, Ha-Hard, and VH-Very Hard. STRENGTH: Fr-Friable, We-Weak, Mo-Moderate, St-Strong, and Ex-Extremely Strong. WEATHE RING: Fr-Fresh, Sl-Slight, Mo-Moderate, and Se-Severe. DISCONTINUITY: Type (BJ-Bedding Joint, Be-Bedding, Jo-Joint, Fo-Foliation, Sh-Shear, Me- Mechanical Break, and Ve-Vein), Dip Angle, Aperture (Ti-Tight, Op-Open, He-Healed, and Fi-Filled), Surface Shape (Ir-Irregular, Pl-Planar, and Wa-Wav y), Roughness (St-Stepped, Ro-Rough, Mo-Moderately Rough, Sl-Slightly Rough, Sm-Smooth, and Po-Polished).

Project No.
14648.000

Figure A2-2 Cont.

PROJECT: Grady Ranch Development Marin County, California		Log of Boring No. RW-2	
BORING LOCATION: Building cut slope - West		ELEVATION AND DATUM: 324	
DRILLING CONTRACTOR: Taber Consultants		DATE STARTED: 8/4/2009	DATE FINISHED: 8/4/2009
DRILLING EQUIPMENT: Track mounted CME-55		TOTAL DEPTH (feet): 107.5	MEASURING POINT: Ground Surface
DRILLING METHOD: 4-inch diameter auger		DEPTH TO FREE WATER FIRST ENCOUNTERED:	
SAMPLING METHOD: See Boring Log Explanation, Figure A2-1		DEPTH TO FREE WATER AT COMPLETION:	
HAMMER WEIGHT: 140 lbs.	HAMMER DROP: 30 inches (Auto Hammer)	LOGGED BY: C. Johnson	

DEPTH (feet)	SAMPLES			MATERIAL DESCRIPTION	LABORATORY TESTS		
	Sample No.	Sample	Blows/foot		Moisture Content (%)	Dry Density (pcf)	Other
1				CLAYEY SAND (SC) medium dense, olive brown (2.5Y 4/3), dry sand is fine to medium, fines have medium plasticity, trace gravel up to 3/4" max diameter [COLLUVIUM]			7:29 start S1 15.5"/18"
2	1		64	SHALE very dark gray (2.5Y 3/1), crushed, low hardness, weak, moderately weathered, locally oxidized along fractures, local clay films on fracture surfaces [FRANCISCAN (KJf)]			S2 6"/11"
3	2		81 11"				
4							
5	3		94	5': Locally severely weathered and friable; interbedded with dark gray, friable to weak, fine-grained sandstone			S3 5"/9"
6	4		94				S4 0"/3" some fragments collected
7				Continued on rock log			Switch to mud rotary 8:00 AM
8							
9							
10							
11							
12							
13							
14							
15							
16							
17							


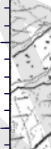
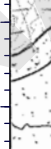
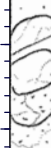
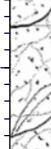
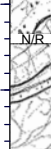
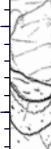

GEES-SOIL 12/03 GRADY_SOIL_LOGS.GPJ GES32003-7.GDT 8/18/09

PROJECT: Grady Ranch Development Marin County, California		Log of Boring No. RW-2	
BORING LOCATION: Building cut slope - West		ELEVATION AND DATUM: 324	
DRILLING CONTRACTOR: Taber Consultants		DATE STARTED: 8/4/2009	DATE FINISHED: 8/4/2009
DRILLING EQUIPMENT: Track mounted CME-55		TOTAL DEPTH (feet): 107.5	MEASURING POINT: Ground Surface
DRILLING METHOD: 4-inch diameter rotary wash		DEPTH TO FREE WATER FIRST ENCOUNTERED:	
SAMPLING METHOD: See Boring Log Explanation, Figure A2-1		DEPTH TO FREE WATER AT COMPLETION:	
HAMMER WEIGHT: --		HAMMER DROP: --	
		LOGGED BY: C. Johnson	

DEPTH (feet)	RUN NO.	RECOVERY (%)	RQD (%)	FRACTURING	HARDNESS	STRENGTH	WEATHERING	LITHOLOGIC DESCRIPTION	GRAPHIC LOG	DISCONTINUITY DESCRIPTION	REMARKS
1											
2											
3											
4											
5											
6								Continued from soil log			
7	1	100	0					SANDSTONE dark olive gray (5Y 3/2), local clay-lined fractures, slightly oxidized fracture surfaces, includes local blocks/blebs of shale up to 1.5" long [FRANCISCAN (KJf)]	Jo, 50, Op, Pl, Mo Jo, 30, Op, Pl, Si Jo, 50, Fi, Pl, Si, clay-lined		Run 1: 6-7.5' 8:19-8:24 AM
8				Cl	Lo	Mo			Jo, 40, Fi, Pl, Si, Mg-Ox Jo, 50, Fi, Pl, Ir, Mo, Mg-ox Jo, 30, Fi, Pl, clay-lined Jo, 50, Fi, Pl, clay-lined		Run 2: 7.5-12.5' 8:31-8:38 AM
9							Mo		Jo, 50, Fi, Pl, Si, clay-lined Mg-Ox		
10	2	96	20					INTERBEDDED SANDSTONE & SHALE black (5Y 2.5/2) to very dark grayish brown (10YR 3/2), very closely spaced fractures, moderately weathered, sandstone is fine to very fine grained, generally low hardness and weak, shale is generally friable to weak and in beds up to 6" thick, may be slightly sheared [FRANCISCAN (KJf)]	Me Be, 45, ss/sh contact Be, 40, Pl		
11				VC-Cl	So/Lo	We			Jo, 50, Fi, Pl, clay-lined		
12				NR	NR	NR	NR		N/R		
13								12.5-17.2': Primarily sandstone, mostly crushed			Run 3: 12.5-17.5' 8:43-8:50 AM
14	3	94	34	VC	Lo	We	Mo		Jo, 35, Fi, Pl, clay-lined > Be, 40, fine ss. Jo, 10, Fi, Pl, clay		
15									- oxide vein		

FRACTURING: VC-Very Close (<0.01'), Cl-Close (0.1'-0.3'), Mo-Moderate (0.3'-1'), Wf-Wide (1'-3'), and VW-Very Wide (3'-10'). HA RDNESS: So-Soft, Lo-Low, Mo-Moderate, Ha-Hard, and VH-Very Hard. STRENGTH: Fr-Friable, We-Weak, Mo-Moderate, St-Strong, and Ex-Extremely Strong. WEATHE RING: Fr-Fresh, Sl-Slight, Mo-Moderate, and Se-Severe. DISCONTINUITY: Type (Be-Bedding, Jo-Joint, Fo-Foliation, Sh-Shear, Me-Mechanical Break, and Ve-Vein), Dip Angle, Aperture (Ti-Tight, Op-Open, He-Healed, and Fi-Filled), Surface Shape (Ir-Irregular, Pl-Planar, and Wa-Wavy), Roughness (St-Ste pped, Ro-Rough, Mo-Moderately Rough, Sl-Slightly Rough, Sm-Smooth, and Po-Polished).

Project No. 14648.000
Figure A2-3

DEPTH (feet)	RUN NO.	RECOVERY (%)	ROD (%)	FRACTURING	HARDNESS	STRENGTH	WEATHERING	LITHOLOGIC DESCRIPTION	GRAPHIC LOG	DISCONTINUITY DESCRIPTION	REMARKS
16	3	94	34	VC	Lo	We	Mo	INTERBEDDED SANDSTONE and SHALE (cont.)			
17				NR	NR	NR	NR				
18					Lo	We		18-20.9': Primarily shale, includes local blocks (beds?) of very fine black sandstone up to 1/4" across		Be, 70, ss/sh contact	Run 4: 17.5-20.9' 8:54-8:59 AM
19	4	100	24								
20					So	Fr		22.4-24': Sandstone, includes local medium sand beds		Be(?) 45, sh/ss contact	Run 5: 20.9-22.5' 9:03-9:06 AM
21	5	100	0								
22				VC			SI	24-24.5': Very fine-grained sandstone and shale 24.5-27.5': Mostly sandstone with interbeds of shale up to 1" thick, grain size of sand increases with depth		Be(?) 80, sh/ss contact	Run 6: 22.5-27.5' 9:08-9:14 AM
23											
24								27.5-29.5': Mostly shale, includes local sandstone beds up to 1" thick		Be, 70, med ss/fine ss	Run 7: 27.5-32.5' 9:34-9:38 AM
25	6	98	52		Lo	We					
26								29.5-32.3': Mostly sandstone, sand is very fine, includes 0.6' thick low hardness, weak shale bed		Be, 60, sh bed 1/4" thick	Run 8: 32.5-37.5' 10:20-10:30 AM
27											
28								32.8-35.4': Mostly shale with local sandstone beds		Be, 45, ss/sh contact	
29					So	Fr					
30	7	96	0	VC			SI			Be, 50	
31					Lo	We					
32										Be, 60, sh/ss contact	
33											

RT-2 (3/03)

GEES-ROCK-3/03 GRADY_ROCK_LOGS.GPJ GES32003-7.GDT 8/18/09

FRACTURING: VC-Very Close (<0.01'), Cl-Close (0.1'-0.3'), Mo-Moderate (0.3'-1'), Wi-Wide (1'-3'), and VW-Very Wide (3'-10'). HA RDNESS: So-Soft, Lo-Low, Mo-Moderate, Ha-Hard, and VH-Very Hard. STRENGTH: Fr-Friable, We-Weak, Mo-Moderate, St-Strong, and Ex-Extremely Strong. WEATHERING: Fr-Fresh, SI-Slight, Mo-Moderate, and Se-Severe. DISCONTINUITY: Type (BJ-Bedding Joint, Be-Bedding, Jo-Joint, Fo-Foliation, Sh-Shear, Me-Mechanical Break, and Ve-Vein), Dip Angle, Aperture (Ti-Tight, Op-Open, He-Healed, and Fi-Filled), Surface Shape (Ir-Irregular, Pl-Planar, and Wa-Wavy), Roughness (St-Stepped, Ro-Rough, Mo-Moderately Rough, SI-Slightly Rough, Sm-Smooth, and Po-Polished).

DEPTH (feet)	RUN NO.	RECOVERY (%)	ROD (%)	FRACTURING	HARDNESS	STRENGTH	WEATHERING	LITHOLOGIC DESCRIPTION	GRAPHIC LOG	DISCONTINUITY DESCRIPTION	REMARKS
34								INTERBEDDED SANDSTONE and SHALE (cont.)			
35	8	98	24	VC-CI	Lo-So	We-Fr	SI				
36											
37				NR	NR	NR	NR				
38								37.5-42.5': Fine-grained, dark gray sandstone			Run 9: 37.5-42.5' 10:36-10:39 AM
39											
40	9	100	56	Mo	Lo-Mo	We-Mo					
41											
42							Fr				
43								43.5-46.8': Shale, generally crushed			Run 10: 42.5-47.5' 10:42-10:51 AM
44											
45	10	86	36	VC	So-Lo	Fr-We					
46											
47				NR	NR	NR	NR				
48				Wi	Mo	Mo		47.5-48.5': Sandstone			Run 11: 47.5-51.3' 10:58-11:09 AM
49	11	89	29	VC-CI	So-Lo	Fr-We	Fr	48.5-50.9': Shale, includes very thin sandstone beds (<0.1" thick) and beds 1.2" thick			
50											
51				NR	NR	NR	NR				
52	12	83	83	VC	Lo	We	Fr	51.3': Shale			Run 12: 51.3-52.5'

RT-2 (3/03)
GEES-ROCK-3/03 GRADY_ROCK_LOGS.GPJ GES32003-7.GDT 8/18/09

FRACTURING: VC-Very Close (<0.01'), CI-Close (0.1'-0.3'), Mo-Moderate (0.3'-1'), Wi-Wide (1'-3'), and VW-Very Wide (3'-10'). HA RDNESS: So-Soft, Lo-Low, Mo-Moderate, Ha-Hard, and VH-Very Hard. STRENGTH: Fr-Friable, We-Weak, Mo-Moderate, St-Strong, and Ex-Extremely Strong. WEATHERING: Fr-Fresh, SI-Slight, Mo-Moderate, and Se-Severe. DISCONTINUITY: Type (BJ-Bedding Joint, Be-Bedding, Jo-Joint, Fo-Foliation, Sh-Shear, Me-Mechanical Break, and Ve-Vein), Dip Angle, Aperture (Ti-Tight, Op-Open, He-Healed, and Fi-Filled), Surface Shape (Ir-Irregular, Pl-Planar, and Wa-Wavy), Roughness (St-Stepped, Ro-Rough, Mo-Moderately Rough, SI-Slightly Rough, Sm-Smooth, and Po-Polished).



Log of Boring No. RW-2 cont.

RT-2 (3/03)

GEES-ROCK-3/03 GRADY ROCK LOGS.GPJ GES32003-7.GDT 8/18/09

DEPTH (feet)	RUN NO.	RECOVERY (%)	RQD (%)	FRACTURING	HARDNESS	STRENGTH	WEATHERING	LITHOLOGIC DESCRIPTION	GRAPHIC LOG	DISCONTINUITY DESCRIPTION	REMARKS
12				NR	NR	NR	NR	INTERBEDDED SANDSTONE and SHALE (cont.)	N/R		11:22-11:27 AM
53				VC	So	Fr-We		52.5-53.3': Crushed shale			Run 13: 52.5-57.5' 11:30-11:38 AM
54								53.5-57.5': Sandstone, sand is very fine to 56.4' then sand becomes mostly fine and lighter gray	Me		
55	13	100	58	Cl	Lo	We-Mo			Jo 50, Op, Pl, Sm - Sl		
56									Jo 45 Fr, Pl, Sm, qtz/calcite Jo 50, Op, Pl, Sm Be 60		
57							Fr		Be 55 Jo 40, Fr, Pl, Sl, qtz/calcite Be 50, ss/sh contact		Run 14: 57.5-62.5' 11:44 AM-12:02 PM
58								57.5-62': Shale with very thin interbeds of fine sandstone throughout, dipping at ~50°			
59											
60	14	90	0	VC-Cl	So-Lo	Fr-We			Jo 40 Op Pl Sm		
61									Jo 40 Op Pl Sm		
62									Be 50, ss bed 1" thick		
63				NR	NR	NR	NR	62.5-66.5': Shale as above, shale contains minor amounts of very fine sand to 63.8'	N/R		
64				Cl	Lo	We			Jo 45, Op, Ir, Ro		Run 15: 62.5-67.5' 12:08 PM Start
65				VC	So	Fr			Me		
66	15	80	20	Cl	Lo-Mo	We	Fr		Jo 40, Op, Ir, Sl Be 40, ss bed 1/4" thick Jo 45 Op, Pl, Sm Be 50 ss bed 1" thick Jo 40, Op, Pl, Sm		
67				NR	NR	NR	NR		Jo 40, Op, Pl, Sm - Sl		
68					So	Fr		67.5-67.9': Shale as above	Be, Ir, Sh/ss contact		Run 16: 67.5-72.5' 12:26-12:36 PM
69	16	98	76	VC	Lo	We-Mo	Fr	67.9-69.8': Fine-grained gray sandstone, includes local blebs of quartz	-qtz Be 50		
70								69.8-72.4': Shale, includes some very fine sand	Be 50, ss/sh contacts Me		

FRACTURING: VC-Very Close (<0.01'), Cl-Close (0.1'-0.3'), Mo-Moderate (0.3'-1'), Wi-Wide (1'-3'), and VW-Very Wide (3'-10'). HA RDNESS: So-Soft, Lo-Low, Mo-Moderate, Ha-Hard, and VH-Very Hard. STRENGTH: Fr-Friable, We-Weak, Mo-Moderate, St-Strong, and Ex-Extremely Strong. WEATHERING: Fr-Fresh, Sl-Slight, Mo-Moderate, and Se-Severe. DISCONTINUITY: Type (BJ-Bedding Joint, Be-Bedding, Jo-Joint, Fo-Foliation, Sh-Shear, Me-Mechanical Break, and Ve-Vein), Dip Angle, Aperture (Ti-Tight, Op-Open, He-Healed, and Fi-Filled), Surface Shape (Ir-Irregular, Pl-Planar, and Wa-Wavy), Roughness (St-Stepped, Ro-Rough, Mo-Moderately Rough, Sl-Slightly Rough, Sm-Smooth, and Po-Polished).

Project No.
14648.000

Figure A2-3 Cont.

Log of Boring No. RW-2 cont.

DEPTH (feet)	RUN NO.	RECOVERY (%)	RQD (%)	FRACTURING	HARDNESS	STRENGTH	WEATHERING	LITHOLOGIC DESCRIPTION	GRAPHIC LOG	DISCONTINUITY DESCRIPTION	REMARKS
71	16	98	76	VC	Lo	We-Mo	Fr	INTERBEDDED SANDSTONE and SHALE (cont.)			
72				NR	NR	NR	NR			Jo 45, Op Pl, Ro Be 60, ss bed 1/2" thick	
73								72.6-77.2': Shale with very fine sand, local green sandstone beds	N/R	Jo 55, Op Pl, Po	Run 17: 72.5-77.5' 12:40-12:58 PM
74										Be 60, ss bed 1" thick Jo 40 Op Pl Po	
75	17	94	20	VC-CI	Lo-So	Mo-Fr	Fr			Be 50 ss bed 1/4" thick Jo 50, Op Pl	
76											
77				NR	NR	NR	NR		N/R		
78								77.5-82.5': Shale as above		Jo 50, Op, Pl - qtz blebs	Run 18: 77.5-82.5' 1:40-1:52 PM
79											
80	18	100	0								
81											
82				VC-CI	Lo-So	Mo-Fr		82.5-84.5': Very fine-grained sandstone		Me Jo 60, Op, Pl, Sm Ve 40, Fi, qtz/calcite	Run 19: 82.5-87.5' 1:56-2:08 PM
83							Fr				
84								84.5-87.5': Shale with minor fine sand			
85	19	100	32								
86											
87											
88	20	96	72	CI	Lo	Mo		87.5-92.5': Sandstone, very fine-grained		Jo 60, Op, Pl, Po Me? Jo 60 Op Pl Sm Jo 60 Fi Pl qtz/calcite	Run 20: 87.5-92.5' 2:14-2:25 PM

RT-2 (3/03)

GES-ROCK-3/03 GRADY_ROCK_LOGS.GPJ GES32003-7.GDT 8/18/09

FRACTURING: VC-Very Close (<0.01'), CI-Close (0.1'-0.3'), Mo-Moderate (0.3'-1'), Wi-Wide (1'-3'), and VW-Very Wide (3'-10'). HA RDNESS: So-Soft, Lo-Low, Mo-Moderate, Ha-Hard, and VH-Very Hard. STRENGTH: Fr-Friable, We-Weak, Mo-Moderate, St-Strong, and Ex-Extremely Strong. WEATHERING: Fr-Fresh, SI-Slight, Mo-Moderate, and Se-Severe. DISCONTINUITY: Type (BJ-Bedding Joint, Be-Bedding, Jo-Joint, Fo-Foliation, Sh-Shear, Me-Mechanical Break, and Ve-Vein), Dip Angle, Aperture (Ti-Tight, Op-Open, He-Healed, and Fi-Filled), Surface Shape (Ir-Irregular, Pl-Planar, and Wa-Wavy), Roughness (St-Stepped, Ro-Rough, Mo-Moderately Rough, SI-Slightly Rough, Sm-Smooth, and Po-Polished).

RT-2 (3/03)

GEES-ROCK-3/03 GRADY_ROCK_LOGS.GPJ_GES32003-7.GDT 8/18/09

DEPTH (feet)	RUN NO.	RECOVERY (%)	ROD (%)	FRACTURING	HARDNESS	STRENGTH	WEATHERING	LITHOLOGIC DESCRIPTION	GRAPHIC LOG	DISCONTINUITY DESCRIPTION	REMARKS
90								INTERBEDDED SANDSTONE and SHALE (cont.)		Jo 30 Op Pl Sm	
91	20	96	72	Cl	Lo	Mo	Fr			Jo 30 Fi qtz/calcite Be 40	
92										Jo 50 Fi qtz/calcite N/R	
93								92.5-99.2': Sandstone, very fine-grained, local quartz/calcite veins		Jo 30 Fi qtz/calcite	Run 21: 92.5-97.5' 2:28-2:39 PM
94										Me	
95	21	100	66							Jo 60 Fi qtz/calcite Jo 60 Fi qtz/calcite Be 70	
96										Jo 30 Fi qtz/calcite	
97										Jo 65 Op Pl Sm Jo 50 Op Pl Sm	
98										Bc(?) Ir, ss/sh contact	Run 22: 97.5-102.5' 2:46-2:59 PM
99										Bc 40 friable bed 3/4" thick	
100	22	100	44	Cl	Lo	Mo	Fr	99.2-107.5': Mostly shale, includes abundant thin (0.1" thick and less) sandstone interbeds, local smooth to polished fracture surfaces dipping at ~50°		Jo 50 Op Pl Sl	
101										Me	
102										Be 40 Be 40 ss bed 1/2" thick	
103										Me Be 20, ss bed 0.4' thick	Run 23: 102.5-107.5' 3:05-3:21 PM
104										Be 35 ss bed 3/4" thick	
105	23	96	70							Me Be 35 ss bed 3/4" thick	
106										N/R	
107										N/R	

FRACTURING: VC-Very Close (<0.01'), Cl-Close (0.1'-0.3'), Mo-Moderate (0.3'-1'), Wi-Wide (1'-3'), and VW-Very Wide (3'-10'). HA RDNESS: So-Soft, Lo-Low, Mo-Moderate, Ha-Hard, and VH-Very Hard. STRENGTH: Fr-Friable, We-Weak, Mo-Moderate, St-Strong, and Ex-Extremely Strong. WEATHERING: Fr-Fresh, Sl-Slight, Mo-Moderate, and Se-Severe. DISCONTINUITY: Type (BJ-Bedding Joint, Be-Bedding, Jo-Joint, Fo-Foliation, Sh-Shear, Me- Mechanical Break, and Ve-Vein), Dip Angle, Aperture (Ti-Tight, Op-Open, He-Healed, and Fi-Filled), Surface Shape (Ir-Irregular, Pl-Planar, and Wa-Wavy), Roughness (St-Stepped, Ro-Rough, Mo-Moderately Rough, Sl-Slightly Rough, Sm-Smooth, and Po-Polished).

Project No.
14648.000

Figure A2-3 Cont.

PROJECT: Grady Ranch Development Marin County, California		Log of Boring No. RW-3	
BORING LOCATION: Building cut slope - North		ELEVATION AND DATUM: 314	
DRILLING CONTRACTOR: Taber Consultants		DATE STARTED: 8/5/2009	DATE FINISHED: 8/5/2009
DRILLING EQUIPMENT: Track mounted CME-55		TOTAL DEPTH (feet): 97	MEASURING POINT: Ground Surface
DRILLING METHOD: 4-inch diameter auger		DEPTH TO FREE WATER FIRST ENCOUNTERED:	
SAMPLING METHOD: See Boring Log Explanation, Figure A2-1		DEPTH TO FREE WATER AT COMPLETION:	
HAMMER WEIGHT: 140 lbs.	HAMMER DROP: 30 inches (Auto Hammer)	LOGGED BY: C. Johnson	

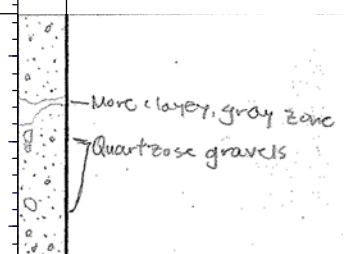
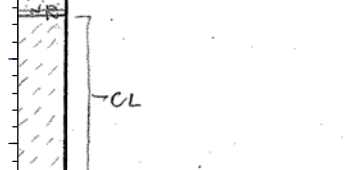
DEPTH (feet)	SAMPLES			MATERIAL DESCRIPTION	LABORATORY TESTS		
	Sample No.	Sample	Blows/foot		Moisture Content (%)	Dry Density (pcf)	Other
1				SILTY SAND (SM) medium dense, dark brown (10YR 3/3), dry, sand is fine, includes abundant roots and rootlets [COLLUVIUM]			7:23 AM start S1 16"/18" recovered
2	1		30	CLAYEY SAND (SC) medium dense, dark yellowish brown (10YR 4/6), dry to slightly moist, sand fine to medium, fines have medium plasticity, includes local rootlets and abundant degraded sandstone gravels that are dark yellowish brown to dark red and up to 1" max. diameter [COLLUVIUM]			S2 17"/18" PP at 4"=>4.5 (tsf)
3	2		20				
4							
5				5': Includes local light gray clayey pockets 1"-2" thick			S3 16"/18"
6	3		61				S4 18"/18"
7	4		38	6.0': Increase in dark red degraded friable sandstone, becomes yellowish red (5YR 4/6), rootlets grade out, becomes dense			
8				6.5': Abundantly mottled strong brown, yellowish red, and light grayish brown			
9							
10							S5 15"/18" PP at 11.5"=>4.5 (tsf)
11	5		57	10': Fines content increases, becomes moist and mostly dark brown (7.5YR 3/4), includes local moderately hard very fine sandstone clasts in fragments up to 1/2" max. diameter			S6 18"/18"
12	6		36				
13							
14							
15							S7 14.5"/18" PP at 16.5"=>4.5 (tsf)
16	7		70	15': Fines content decreases slightly relative to above, includes local friable shale clasts up to 1/2" max. diameter			S8 18"/18"
17	8		28				

GEES-SOIL_12/03_GRADY_SOIL_LOGS.GPJ GES32003-7.GDT 8/18/09

Log of Boring No. RW-3 cont.

DEPTH (feet)	SAMPLES			MATERIAL DESCRIPTION	LABORATORY TESTS		
	Sample No.	Sample	Blows/foot		Moisture Content (%)	Dry Density (pcf)	Other
18	8			CLAYEY SAND (SC) (cont.)			
19							
20				20': Increase in dark yellowish brown, friable sandstone gravels up to 2" max diameter			S9 15"/18" PP at 21.4=>4.5 (tsf)
21	9	X	89 10.5"	21.4': Becomes medium dense			S10 18"/18"
22							
23	10		28				
24							
25				25': Includes fine-grained sandstone gravels up to 1.5" max diameter that are subangular, weak, and covered with dark brown clay films			S11 14.5"/18"
26	11	X	77				
27	12		50* 4"	26.5': No recovery, slough material includes dark yellowish brown, friable sandstone and clayey sand similar to above			S12 0"/18"
28							28.5' Driller noted that the drilling became gradually harder and got drilling refusal with auger at 28.5'
29				Continued on rock log			
30							
31							9:00 AM switch to mud rotary and HQ coring
32							
33							
34							
35							
36							
37							
38							
39							

PROJECT: Grady Ranch Development Marin County, California		Log of Boring No. RW-3	
BORING LOCATION: Building cut slope - North		ELEVATION AND DATUM: 314	
DRILLING CONTRACTOR: Taber Consultants		DATE STARTED: 8/5/2009	DATE FINISHED: 8/5/2009
DRILLING EQUIPMENT: Track mounted CME-55		TOTAL DEPTH (feet): 97	MEASURING POINT: Ground Surface
DRILLING METHOD: 4-inch diameter rotary wash		DEPTH TO FREE WATER FIRST ENCOUNTERED:	
SAMPLING METHOD: See Boring Log Explanation, Figure A2-1		DEPTH TO FREE WATER AT COMPLETION:	
HAMMER WEIGHT: --		HAMMER DROP: --	
		LOGGED BY: C. Johnson	

DEPTH (feet)	RUN NO.	RECOVERY (%)	RQD (%)	FRACTURING	HARDNESS	STRENGTH	WEATHERING	LITHOLOGIC DESCRIPTION	GRAPHIC LOG	DISCONTINUITY DESCRIPTION	REMARKS
19											
20											
21											
22											
23											
24											
25											
26											
27											
28								Continued from soil log			
29								CLAYEY SAND (SC) (continued) sand is fine to coarse, mostly fine to medium			Run 1: 28.5-31.5' 9:45-9:48 AM 29' PP: >4.5 (tsf)
30	1	97		NA	NA	NA	NA				
31				NR	NR	NR	NR				
32	2	92		NA	NA	NA	NA	31.5-33.5': SANDY CLAY (CL), dark yellowish brown (10YR 3/4) to grayish brown (2.5Y 5/2), fines have medium plasticity			Run 2: 31.5-36.5' 10:20-10:25 AM
33											

RT-1 (3/03)
GEES-ROCK-3/03 GRADY_RANCH_LOGS.GPJ GES32003-7.GDT 8/18/09

FRACTURING: VC-Very Close (<0.01'), CI-Close (0.1'-0.3'), Mo-Moderate (0.3'-1'), Wi-Wide (1'-3'), and VW-Very Wide (3'-10'). HA-RDNESS: So-Soft, Lo-Low, Mo-Moderate, Ha-Hard, and VH-Very Hard. STRENGTH: Fr-Friable, We-Weak, Mo-Moderate, St-Strong, and Ex-Extremely Strong. WEATHERING: Fr-Fresh, SI-Slight, Mo-Moderate, and Se-Severe. DISCONTINUITY: Type (Be-Bedding, Jo-Joint, Fo-Foliation, Sh-Shear, Me-Mechanical Break, and Ve-Vein), Dip Angle, Aperture (Ti-Tight, Op-Open, He-Healed, and Fi-Filled), Surface Shape (Ir-Irregular, Pl-Planar, and Wa-Wavy), Roughness (St-Stepped, Ro-Rough, Mo-Moderately Rough, SI-Slightly Rough, Sm-Smooth, and Po-Polished).		Project No. 14648.000
		Figure A2-4

RT-2 (3/03)

GEES-ROCK-3/03 GRADY ROCK LOGS.GPJ GES32003-7.GDT 8/18/09

DEPTH (feet)	RUN NO.	RECOVERY (%)	RQD (%)	FRACTURING	HARDNESS	STRENGTH	WEATHERING	LITHOLOGIC DESCRIPTION	GRAPHIC LOG	DISCONTINUITY DESCRIPTION	REMARKS
34								CLAYEY SAND (SC) (cont.)			
35	2	92		NA	NA	NA	NA				
36				NR	NR	NR	NR		N/R		
37				NA	NA	NA	NA	36.9-38.4: Metamorphic boulder (?), crushed, polished, clay-lined fractures dipping 30°, local oxidized staining on fracture surfaces, shoe contains dark red sandstone clast and clay			Run 3: 36.5-41.5' 10:31-10:43 AM
38											Driller noted no major changes in drilling
39	3	38									
40				NR	NR	NR	NR		N/R		
41											
42	4	94	0	VC	So/Lo	We-Mo	SI	SHALE (META MUDSTONE?) very dark gray (2.5Y 3/1), crushed, soft to low hardness, weak to moderate strength, slightly weathered [FRANCISCAN (KJf)]	Jo, 60, Op, Pl, Sm Me Jo, 30 Fi, Pl, Ir, clay-lined Jo, 40 Fi Pl clay-lined Me		Run 4: 41.5-43.1' 10:55-11:01 AM
43				NR	NR	NR	NR		N/R		
44					Lo	We		INTERBEDDED SANDSTONE and SHALE very dark greenish gray (10GY 3/) to black (10YR 2/1), variable bed thickness, mostly crushed to closely spaced fractures 43.1-44.6': Sandstone/greenstone? 44.6-46': Shale	Be, 50		Run 5: 43.1-47' 11:06-11:19 AM
45	5	74	23	VC	So	Fr	SI		Be, 50, Ir, ss/sh contact Be, 60, ss bed 0.1" thick Jo, 50, Op, Pl, Sm		
46				NR	NR	NR	NR		N/R		
47								47-53.4': Shale; includes scattered anastomosing quartz/calcite veinlets throughout, increase in sand and fine sandstone interbeds with depth	Jo 70 up Pl Sm Jo, 45, Op, Pl, Sm Jo 65 Fi Pl clay-lined		Run 6: 47-52' 11:23-11:33 AM
48											
49											
50	6	90	30	VC-CI	So-Lo	Fr	SI		Me(?) Be 50, ss bed 1/4" thick		
51											
52				NR	NR	NR	NR		N/R		

FRACTURING: VC-Very Close (<0.01'), CI-Close (0.1'-0.3'), Mo-Moderate (0.3'-1'), Wi-Wide (1'-3'), and VW-Very Wide (3'-10'). HA RDNESS: So-Soft, Lo-Low, Mo-Moderate, Ha-Hard, and VH-Very Hard. STRENGTH: Fr-Friable, We-Weak, Mo-Moderate, St-Strong, and Ex-Extremely Strong. WEATHE RING: Fr-Fresh, SI-Slight, Mo-Moderate, and Se-Severe. DISCONTINUITY: Type (BJ-Bedding Joint, Be-Bedding, Jo-Joint, Fo-Foliation, Sh-Shear, Me- Mechanical Break, and Ve-Vein), Dip Angle, Aperture (Ti-Tight, Op-Open, He-Healed, and Fi-Filled), Surface Shape (Ir-Irregular, Pl-Planar, and Wa-Wav y), Roughness (St-Stepped, Ro-Rough, Mo-Moderately Rough, SI-Slightly Rough, Sm-Smooth, and Po-Polished).

Project No.
14648.000

Figure A2-4 Cont.

Log of Boring No. RW-3 cont.

DEPTH (feet)	RUN NO.	RECOVERY (%)	RQD (%)	FRACTURING	HARDNESS	STRENGTH	WEATHERING	LITHOLOGIC DESCRIPTION	GRAPHIC LOG	DISCONTINUITY DESCRIPTION	REMARKS
53						We		INTERBEDDED SANDSTONE and SHALE (cont.)		Be 70, ss bed 1/2" thick me	Run 7: 52-57' 11:38-11:39 AM and 11:51-11:58 AM
54	7	100	16	VC-CI			SI-Fr	53.4-63.7': Sandstone, very dark gray (N 3/), abundant quartz/calcite veinlets throughout		Be 55, sh/ss contact Jo 50 Fi qtz/calcite Jo 45 Fi, clay lined	
55										Jo 40 Fi qtz/calcite	
56					Lo	Mo				Jo 65 Op, Pl, Si	
57										me	Run 8: 57-62' 12:02-12:09 PM
58										vc 70, qtz/calcite ~1mm thick	
59	8	100	42			So Fr				Be 50, Ir Be 55, Pl	
60				CI							
61										Jo 40 Op Pl Sm Jo 30 op Pl Si	
62					Lo	Mo				Jo 50 Fi Pl qtz/calc. me	Run 9: 62-67' 12:12-12:21 PM
63										Jo 55 Op Pl Sm Be 70, ss/sh contact	
64	9	100	22				Fr	63.7-72': Shale, local quartz/calcite nodules and veinlets		Jo 50 Op Pl Si	
65											
66											
67				VC	So	Fr		67-69.5': Shale has local sandstone inclusions up to 2" in diameter		>45 inclusions Jo 40 Op, Pl - wa, Po	Run 10: 67-72' 12:29-12:40 PM
68										Jo 40 Op, Ir, Sm	
69	10	96	44							me(?) Be(?) Jo 60 Op Pl Sm	
70					Lo	We		69.5-71.8': Shale includes some very fine sand, becomes slightly harder and stronger		me	

RT-2 (3/03)

GEES-ROCK-3/03 GRADY ROCK LOGS.GPJ GES32003-7.GDT 8/18/09

FRACTURING: VC-Very Close (<0.01'), CI-Close (0.1'-0.3'), Mo-Moderate (0.3'-1'), WI-Wide (1'-3'), and VW-Very Wide (3'-10'). HA RDNESS: So-Soft, Lo-Low, Mo-Moderate, Ha-Hard, and VH-Very Hard. STRENGTH: Fr-Friable, We-Weak, Mo-Moderate, St-Strong, and Ex-Extremely Strong. WEATHERING: Fr-Fresh, SI-Slight, Mo-Moderate, and Se-Severe. DISCONTINUITY: Type (BJ-Bedding Joint, Be-Bedding, Jo-Joint, Fo-Foliation, Sh-Shear, Me-Mechanical Break, and Ve-Vein), Dip Angle, Aperture (Ti-Tight, Op-Open, He-Healed, and Fi-Filled), Surface Shape (Ir-Irregular, Pl-Planar, and Wa-Wavy), Roughness (St-Stepped, Ro-Rough, Mo-Moderately Rough, SI-Slightly Rough, Sm-Smooth, and Po-Polished).



DEPTH (feet)	RUN NO.	RECOVERY (%)	RQD (%)	FRACTURING	HARDNESS	STRENGTH	WEATHERING	LITHOLOGIC DESCRIPTION	GRAPHIC LOG	DISCONTINUITY DESCRIPTION	REMARKS
71	10	96	44					INTERBEDDED SANDSTONE and SHALE (cont.)			
72				NR	NR	NR	NR	72.5-82': Sandstone			Run 11: 72-77' 12:55-1:02 PM
73				VC	Lo	We-Fr					
74											
75	11	100	70								
76											
77											Run 12: 77-82' 1:10-1:17 PM
78											
79											
80	12	100	44	Cl-Wi	Mo	Mo	Fr				
81											Run 13: 82-87' 1:22-1:28 PM
82								82-90': Sandstone, includes quartz or calcite veins up to 1/2" thick			
83											
84											
85	13	96	78								
86											
87				NR	NR	NR	NR				Run 14: 87-92' 1:59-2:07 PM
88	14	94	8	Cl-VC	Mo	Mo	Fr				
89											

RT-2 (3/03)

GEES-ROCK-3/03 GRADY_ROCK_LOGS.GPJ GES32003-7.GDT 8/18/09

FRACTURING: VC-Very Close (<0.01'), Cl-Close (0.1'-0.3'), Mo-Moderate (0.3'-1'), Wi-Wide (1'-3'), and VW-Very Wide (3'-10'). HARDNESS: So-Soft, Lo-Low, Mo-Moderate, Ha-Hard, and VH-Very Hard. STRENGTH: Fr-Friable, We-Weak, Mo-Moderate, St-Strong, and Ex-Extremely Strong. WEATHERING: Fr-Fresh, Sl-Slight, Mo-Moderate, and Se-Severe. DISCONTINUITY: Type (BJ-Bedding Joint, Be-Bedding, Jo-Joint, Fo-Foliation, Sh-Shear, Me-Mechanical Break, and Ve-Vein), Dip Angle, Aperture (Ti-Tight, Op-Open, He-Healed, and Fi-Filled), Surface Shape (Ir-Irregular, Pl-Planar, and Wa-Wavy), Roughness (St-Stepped, Ro-Rough, Mo-Moderately Rough, Sl-Slightly Rough, Sm-Smooth, and Po-Polished).

Project No.
14648.000
Figure A2-4 Cont.

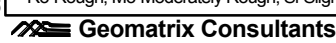
DEPTH (feet)	RUN NO.	RECOVERY (%)	RQD (%)	FRACTURING	HARDNESS	STRENGTH	WEATHERING	LITHOLOGIC DESCRIPTION	GRAPHIC LOG	DISCONTINUITY DESCRIPTION	REMARKS
90	14	94	8	Cl-VC	So-Lo	Fr-We	Fr	INTERBEDDED SANDSTONE and SHALE (cont.)			
91								90-91.7': Shale			
92				NR	NR	NR	NR	92-93.2': Fine-grained sandstone			
93	15	100	0	Cl	Mo	Mo		93.2-97': Shale, abundant polished fracture or bedding surfaces			Run 15: 92-97' 2:10-2:19 PM
94				VC	So	Fr		Fr			
95											
96											
97								Bottom of boring 97.0'			
98											
99											
100											
101											
102											
103											
104											
105											
106											
107											

RT-2 (3/03)

GEES-ROCK-3/03 GRADY ROCK LOGS.GPJ_GES32003-7.GDT 8/18/09

PRELIMINARY

FRACTURING: VC-Very Close (<0.01'), Cl-Close (0.1'-0.3'), Mo-Moderate (0.3'-1'), Wi-Wide (1'-3'), and VW-Very Wide (3'-10'). HARDNESS: So-Soft, Lo-Low, Mo-Moderate, Ha-Hard, and VH-Very Hard. STRENGTH: Fr-Friable, We-Weak, Mo-Moderate, St-Strong, and Ex-Extremely Strong. WEATHERING: Fr-Fresh, SI-Slight, Mo-Moderate, and Se-Severe. DISCONTINUITY: Type (BJ-Bedding Joint, Be-Bedding, Jo-Joint, Fo-Foliation, Sh-Shear, Me-Mechanical Break, and Ve-Vein), Dip Angle, Aperture (Ti-Tight, Op-Open, He-Healed, and Fi-Filled), Surface Shape (Ir-Irregular, Pl-Planar, and Wa-Wavy), Roughness (St-Stepped, Ro-Rough, Mo-Moderately Rough, SI-Slightly Rough, Sm-Smooth, and Po-Polished).



PROJECT: Grady Ranch Development Marin County, California		Log of Boring No. RW-4	
BORING LOCATION: Proposed wine cave - East		ELEVATION AND DATUM: 304	
DRILLING CONTRACTOR: Taber Consultants		DATE STARTED: 8/3/2009	DATE FINISHED: 8/3/2009
DRILLING EQUIPMENT: Track mounted CME-55		TOTAL DEPTH (feet): 92.5	MEASURING POINT: Ground Surface
DRILLING METHOD: 4-inch diameter auger		DEPTH TO FREE WATER FIRST ENCOUNTERED:	
SAMPLING METHOD: See Boring Log Explanation, Figure A2-1		DEPTH TO FREE WATER AT COMPLETION:	
HAMMER WEIGHT: 140 lbs.	HAMMER DROP: 30 inches (Auto Hammer)	LOGGED BY: C. Johnson	

DEPTH (feet)	SAMPLES			MATERIAL DESCRIPTION	LABORATORY TESTS		
	Sample No.	Sample	Blows/foot		Moisture Content (%)	Dry Density (pcf)	Other
1				SILTY SAND (SM) medium dense, dark yellowish brown (10YR 3/4), dry to slightly moist, sand is fine to medium, includes abundant rootlets in upper 1' [COLLUVIUM]			8:05 AM start S1 15"/18"
2	1		13	2': Becomes mostly strong brown (7.5YR 4/6), includes local severely weathered sandstone clasts up to 1/2" max. diameter, includes abundant decomposing rootlets and local charcoal bits			S2 18"/18"
3	2		17				
4							
5				5': Locally mottled with dark yellowish brown, becomes very dense			S3 14.5"/18"
6	3		56	SANDSTONE yellowish brown (10YR 5/6), soft, friable, moderately to severely weathered, locally weathered to strong brown, local black oxidized fracture surfaces, slightly silty [FRANCISCAN (KJf)]			S4 19"/18"
7	4		56				
8							
9							
10	5		50* 1.5"	Continued on rock log			S5 0"/1.5" Switch to mud rotary at 8:50 AM
11							
12							
13							
14							
15							
16							
17							

GEES-SOIL 12/03 GRADY_SOIL_LOGS.GPJ GES32003-7.GDT 8/18/09

GT-1 (12/03)

PROJECT: Grady Ranch Development Marin County, California		Log of Boring No. RW-4	
BORING LOCATION: Proposed wine cave - East		ELEVATION AND DATUM: 304	
DRILLING CONTRACTOR: Taber Consultants		DATE STARTED: 8/3/2009	DATE FINISHED: 8/3/2009
DRILLING EQUIPMENT: Track mounted CME-55		TOTAL DEPTH (feet): 92.5	MEASURING POINT: Ground Surface
DRILLING METHOD: 4-inch diameter rotary wash		DEPTH TO FREE WATER FIRST ENCOUNTERED:	
SAMPLING METHOD: See Boring Log Explanation, Figure A2-1		DEPTH TO FREE WATER AT COMPLETION:	
HAMMER WEIGHT: --		HAMMER DROP: --	
		LOGGED BY: C. Johnson	

DEPTH (feet)	RUN NO.	RECOVERY (%)	RQD (%)	FRACTURING	HARDNESS	STRENGTH	WEATHERING	LITHOLOGIC DESCRIPTION	GRAPHIC LOG	DISCONTINUITY DESCRIPTION	REMARKS
1											
2											
3											
4											
5											
6											
7											
8											
9											
10								Continued from soil log			
11	1	63	40	Mo	So-Lo	Fr-We	Mo	SANDSTONE yellowish brown (10YR 5/6), local clay-lined fractures, fracture surfaces oxidized to dark brown, sand is fine and mostly quartz [FRANCISCAN (KJf)]	<i>Mc Jo, 30, Fi, Pl, Sl, clay-lined</i>		Run 1: 10.1-12.5' 9:06-9:10 AM
12				NR	NR	NR	NR		<i>Jo, 50, Fi, Pl, Mo, clay-lined Jo, 35, Fi, Pl, Sl, clay-lined</i>		
13									<i>N/R</i>		
14	2	96	0	Cl	Lo-So	Fr	Mo		<i>Jo, 35, Fi, Pl, Sl, Mo, Mg-ox Jo, 40, Fi, Pl, Sl, Mg-ox Jo, 80, Fi, Pl, Sm, Mg-ox</i>		Run 2: 12.5-17.5' 9:12-9:17 AM
15									<i>Mc Jo, 70, Op, Pl, Sm-Sl Jo, 85, Op, Pl, Sl-Ro Jo, 45, Fi, Pl, Sm, clay</i>		

FRACTURING: VC-Very Close (<0.01'), Cl-Close (0.1'-0.3'), Mo-Moderate (0.3'-1'), Wf-Wide (1'-3'), and Vw-Very Wide (3'-10'). HA RDNESS: So-Soft, Lo-Low, Mo-Moderate, Ha-Hard, and VH-Very Hard. STRENGTH: Fr-Friable, We-Weak, Mo-Moderate, St-Strong, and Ex-Extremely Strong. WEATHE RING: Fr-Fresh, Sl-Slight, Mo-Moderate, and Se-Severe. DISCONTINUITY: Type (Be-Bedding, Jo-Joint, Fo-Foliation, Sh-Shear, Me-Mechanical Break, and Ve-Vein), Dip Angle, Aperture (Ti-Tight, Op-Open, He-Healed, and Fi-Filled), Surface Shape (Ir-Irregular, Pl-Planar, and Wa-Wavy), Roughness (St-Stepped, Ro-Rough, Mo-Moderately Rough, Sl-Slightly Rough, Sm-Smooth, and Po-Polished).

Project No. 14648.000
Figure A2-5

RT-1 (3/03)

GEES-ROCK-3/03 GRADY_RANCH_LOGS.GPJ GES32003-7.GDT 8/18/09

Log of Boring No. RW-4 cont.

RT-2 (3/03)

GEES-ROCK-3/03 GRADY ROCK LOGS.GPJ GES32003-7.GDT 8/18/09

DEPTH (feet)	RUN NO.	RECOVERY (%)	RQD (%)	FRACTURING	HARDNESS	STRENGTH	WEATHERING	LITHOLOGIC DESCRIPTION	GRAPHIC LOG	DISCONTINUITY DESCRIPTION	REMARKS
16	2	96	0	Cl	Lo-So	Fr	Mo	SANDSTONE (cont.)			
17				NR	NR	NR	NR		N/R		
18								18': Becomes olive gray (5Y 5/2), strength and hardness increase, includes local crushed zones up to 0.6' thick			Run 3: 17.5-22.5' 9:22-9:28 AM
19											Driller noted bottom foot dropped from barrel; recovered in following run
20	3	100	0								
21											
22				Cl	Lo	Mo-We	Mo				Run 4: 22.5-26.5' 9:32-9:34 AM
23											
24	4	100	0								Run 5: 26.5-27.5'
25											
26											
27	5	40	0	NR	NR	NR	NR		N/R		Run 6: 27.5-32.5' 10:15-10:22 AM
28					Lo	Mo-We	Mo				
29											
30	6	100	26	Cl	Mo	Mo-St	Mo-Sl	29.3': Becomes dark bluish gray (10B 4/), strength and hardness increase			Run 7: 32.5-37.5' 10:28-10:36 AM
31											
32											
33											

FRACTURING: VC-Very Close (<0.01'), Cl-Close (0.1'-0.3'), Mo-Moderate (0.3'-1'), W-Wide (1'-3'), and VW-Very Wide (3'-10'). HA RDNESS: So-Soft, Lo-Low, Mo-Moderate, Ha-Hard, and VH-Very Hard. STRENGTH: Fr-Friable, We-Weak, Mo-Moderate, St-Strong, and Ex-Extremely Strong. WEATHERING: Fr-Fresh, Sl-Slight, Mo-Moderate, and Se-Severe. DISCONTINUITY: Type (BJ-Bedding Joint, Be-Bedding, Jo-Joint, Fo-Foliation, Sh-Shear, Me-Mechanical Break, and Ve-Vein), Dip Angle, Aperture (Ti-Tight, Op-Open, He-Healed, and Fi-Filled), Surface Shape (Ir-Irregular, Pl-Planar, and Wa-Wavy), Roughness (St-Stepped, Ro-Rough, Mo-Moderately Rough, Sl-Slightly Rough, Sm-Smooth, and Po-Polished).

Project No.
14648.000

Figure A2-5 Cont.

DEPTH (feet)	RUN NO.	RECOVERY (%)	ROD (%)	FRACTURING	HARDNESS	STRENGTH	WEATHERING	LITHOLOGIC DESCRIPTION	GRAPHIC LOG	DISCONTINUITY DESCRIPTION	REMARKS
34				CI	Mo	Mo-St	Mo-SI	SANDSTONE (cont.)		Jo, 50, Fi, Pl, Sl, clay-lined Me	
35	7	46	20							Jo, 30, Fi, Pl-Ir, Mo clay	
36				NR	NR	NR	NR		N/R		
37											
38											Run 8: 37.5-40' 10:41-10:48 AM
39	8	0	0	NR	NR	NR	NR		N/R		Some gravel-sized fragments recovered; saved in bag
40				CI	Mo	St	SI	40': Includes local blebs of quartz/calcite up to 1/2" long		Me	Run 9: 40-42.5'
41	9	28	16	NR	NR	NR	NR		N/R		Core has been pushing inner barrel up; collected in bucket - up direction?
42											
43								42.5': Becomes mostly dark bluish gray (10B 4/)		Jo, 45, Fi, Pl, clay-lined Jo 30, Fi, Pl Jo 30, Pl Jo 25, Pl	Run 10: 42.5-47.5' 11:36-11:43 AM
44										Ve, 60, Fi, Pl, Si, Qtz/calcite Me	
45	10	100	64		Mo	St	SI			Jo, 50, Fi, Pl, Sl, clay	
46											
47				Mo						Jo, 60, Op, Pl, Sl Jo, 60, Op, Pl, Sm Jo, 40, Op, Pl, Sm Jo, 40, Op, Pl, Sl Ve, Ir, Qtz/calcite Jo, 50, Op, Pl, Sl	
48								47.5': Grades stronger and harder			Run 11: 47.5-52.5' 11:47 AM Start
49											
50	11	100	96		Ha	St	Fr			Jo, 40, Op, Pl, Ro Jo, 80, Fi, Qtz/calcite Me	
51											
52											

RT-2 (3/03)

GES-ROCK-3/03 GRADY_ROCK_LOGS.GPJ GES32003-7.GDT 8/18/09

FRACTURING: VC-Very Close (<0.01'), CI-Close (0.1'-0.3'), Mo-Moderate (0.3'-1'), WI-Wide (1'-3'), and VW-Very Wide (3'-10'). HA RDNESS: So-Soft, Lo-Low, Mo-Moderate, Ha-Hard, and VH-Very Hard. STRENGTH: Fr-Friable, We-Weak, Mo-Moderate, St-Strong, and Ex-Extremely Strong. WEATHE RING: Fr-Fresh, SI-Slight, Mo-Moderate, and Se-Severe. DISCONTINUITY: Type (BJ-Bedding Joint, Be-Bedding, Jo-Joint, Fo-Foliation, Sh-Shear, Me- Mechanical Break, and Ve-Vein), Dip Angle, Aperture (TI-Tight, Op-Open, He-Healed, and Fi-Filled), Surface Shape (Ir-Irregular, Pl-Planar, and Wa-Wav y), Roughness (St-Stepped, Ro-Rough, Mo-Moderately Rough, SI-Slightly Rough, Sm-Smooth, and Po-Polished).

Project No.
14648.000
Figure A2-5 Cont.

DEPTH (feet)	RUN NO.	RECOVERY (%)	RQD (%)	FRACTURING	HARDNESS	STRENGTH	WEATHERING	LITHOLOGIC DESCRIPTION	GRAPHIC LOG	DISCONTINUITY DESCRIPTION	REMARKS
53	11							SANDSTONE (cont.)			Run 12: 52.5-57.5' 12:06-12:12 PM
54											
55	12	100	90	Mo							Run 13: 57.5-62.5' 12:17-12:19 PM and 12:36-12:42 PM
56											
57					Ha	Mo	Fr				
58											
59											
60	13	96	48	Mo-CI							Run 14: 62.5-67.5' 12:48-12:55 PM
61											
62											
63											
64											
65	14	100	62	CI-Wi							Run 15: 67.5-72.5' 1:09-1:16 PM
66											
67					Ha	Mo	Fr				
68											
69	15	94	52	CI-Mo				68.9': Medium-grained sand increases			
70											

RT-2 (3/03)

GEES-ROCK-3/03 GRADY ROCK LOGS.GPJ GES32003-7.GDT 8/18/09

FRACTURING: VC-Very Close (<0.01'), CI-Close (0.1'-0.3'), Mo-Moderate (0.3'-1'), Wi-Wide (1'-3'), and VW-Very Wide (3'-10'). HA RDNESS: So-Soft, Lo-Low, Mo-Moderate, Ha-Hard, and VH-Very Hard. STRENGTH: Fr-Friable, We-Weak, Mo-Moderate, St-Strong, and Ex-Extremely Strong. WEATHERING: Fr-Fresh, SI-Slight, Mo-Moderate, and Se-Severe. DISCONTINUITY: Type (BJ-Bedding Joint, Be-Bedding, Jo-Joint, Fo-Foliation, Sh-Shear, Me-Mechanical Break, and Ve-Vein), Dip Angle, Aperture (Ti-Tight, Op-Open, He-Healed, and Fi-Filled), Surface Shape (Ir-Irregular, Pl-Planar, and Wa-Wavy), Roughness (St-Stepped, Ro-Rough, Mo-Moderately Rough, SI-Slightly Rough, Sm-Smooth, and Po-Polished).

Project No.
14648.000
Figure A2-5 Cont.

PROJECT: Grady Ranch Development Marin County, California		Log of Boring No. RW-5	
BORING LOCATION: Planned fill mound		ELEVATION AND DATUM: 236	
DRILLING CONTRACTOR: Taber Consultants		DATE STARTED: 8/6/2009	DATE FINISHED: 8/6/2009
DRILLING EQUIPMENT: Track mounted CME-55		TOTAL DEPTH (feet): 50	MEASURING POINT: Ground Surface
DRILLING METHOD: 4-inch diameter auger		DEPTH TO FREE WATER FIRST ENCOUNTERED: 31.0 feet (9:00 AM 8/6/2009)	
SAMPLING METHOD: See Boring Log Explanation, Figure A2-1		DEPTH TO FREE WATER AT COMPLETION:	
HAMMER WEIGHT: 140 lbs.	HAMMER DROP: 30 inches (Auto Hammer)	LOGGED BY: C. Johnson	

DEPTH (feet)	SAMPLES			MATERIAL DESCRIPTION	LABORATORY TESTS		
	Sample No.	Sample	Blows/foot		Moisture Content (%)	Dry Density (pcf)	Other
1				SAND with Silt and Gravel (SP-SM) loose to medium dense, very dark grayish brown (10YR 3/2), dry to slightly moist, sand fine to coarse, mostly fine, gravel is subangular to subrounded dark gray and brownish gray sandstone up to 2" max diameter, local rootlets [COLLUVIUM] 2.5': Includes local shale gravels up to 1.2" max diameter, some sandstone oxidized to strong brown, rootlets grade out			7:21 AM start S1 10"/18" PP at 2.5'=3.5; 2.0; 1.75; 3.75 (tsf)
2	1		12				S2 8.5"/18"
3				SILTY SAND (SM) loose, mostly dark brown (10YR 3/3), wet from driller adding water, sand fine to coarse, mostly fine to medium, some fine gravels that are rounded to subangular [ALLUVIUM]			Driller added some water to hole S3 11.5"/18" PP at 5'= 0.25; 0.5; <0.25; 0.5 (tsf) higher number of gravels S4 11"/18" abundant fine gravels
4	2		12				
5							
6				SAND with Clay and Gravel (SW-SC) loose, brown (10YR 4/3), moist, sand is well-graded, gravel is fine and mostly subrounded to rounded sandstone and quartzose volcanic clasts up to 1.5" max diameter [ALLUVIUM]			S5 15.5"/18" PP at 11'=1.0; 1.0; 1.2 (tsf)
7	3		4				
8				SILTY SAND (SM) loose, dark brown (10YR 3/3), moist, to slightly wet, sand is fine, some coarse sand and scattered fine gravel [ALLUVIUM] 15': Fines content decreases			S6 15"/18" S7 15"/18" PP at 15.5'= 0.75; 1.0; 0.5; 1.25 (tsf) PP at 16.5'= 2.5; 2.5; 2.0; 3.0 (tsf) S8 13"/18"
9	4		2				
10							
11				SAND with Silt and Gravel (SP-SM) medium dense, dark brown (10YR 3/3), slightly wet, sand fine to coarse, mostly medium, gravel rounded to subangular up to 3/4" max diameter [ALLUVIUM]			
12	5		11				
13							
14	6		2				
15							
16	7		8				
17							
	8		12				

GEES-SOIL_12/03_GRADY_SOIL_LOGS.GPJ GES32003-7.GDT 8/18/09

GT-1 (12/03)

Log of Boring No. RW-5 cont.

DEPTH (feet)	SAMPLES			MATERIAL DESCRIPTION	LABORATORY TESTS		
	Sample No.	Sample	Blows/foot		Moisture Content (%)	Dry Density (pcf)	Other
18	8			16.5': Coarse sand increases			
19							
20				20': Abundant dark red, dark yellowish brown, and gray gravels, up to 1.5" max diameter			S9 15.5"/18" PP at 21'=4.0; 3.5; >4.5 (tsf)
21	9		19				S10 11"/18"
22				21.5': Includes chert gravel and sandstone gravel broken up by sampler to angular fragments up to 1.5" max diameter, becomes moist			
23	10		22				
24							
25							S11 14"/18" PP at 26.5'= >4.5; >4.5; 4.25; 3.0 (tsf) (gravels)
26	11		28				
27				26.5': Grades to dark yellowish brown (10YR 4/4)			
28	12		22				
29							
30				CLAYEY SAND with Gravel (SC) medium dense, olive brown (2.5Y 4/4), moist to wet, sand is fine to coarse, mostly medium, gravel is subrounded to subangular sandstone, with minor chert, shale, and quartzose volcanic clasts up to 1.5" max diameter [ALLUVIUM]			S13 14"/18" PP at 30.5'= >4.5 (tsf) (gravels)
31	13		19		ATD ∇		S14 13"/18"
32	14		24				
33							
34							
35				35': Includes abundant gravels, degraded dark yellowish brown sandstone clast 2" diameter, becomes dense			S15 16"/18" PP at 36.5'= >4.5 (tsf) (gravels)
36	15		98				S16 14.5"/18"
37							
38	16		41				Driller noted harder drilling at 38.5'; refusal at 39.5'
39				SHALE			

Log of Boring No. RW-5 cont.

DEPTH (feet)	SAMPLES			MATERIAL DESCRIPTION	LABORATORY TESTS		
	Sample No.	Sample	Blows/foot		Moisture Content (%)	Dry Density (pcf)	Other
40	17		82 9"	SHALE dark olive gray (5Y 3/2) to black (5Y 2.5/1), crushed, soft, weak, moderately weathered, abundant polished bedding or fracture surfaces, sheared, contains nodules of more competent shale surrounded by more friable material, locally pale olive [FRANCISCAN (KJf)]			S17 14"/15" PP at 40.75'= >4.5 (tsf) S18 15.5"/11" driller set sampler prior to counting blows
41	18		76 11"				
42				Continued on rock log			
43							
44							
45							
46							
47							
48							
49							
50							

PRELIMINARY

PROJECT: Grady Ranch Development Marin County, California		Log of Boring No. RW-5	
BORING LOCATION: Planned fill mound		ELEVATION AND DATUM: 236	
DRILLING CONTRACTOR: Taber Consultants		DATE STARTED: 8/6/2009	DATE FINISHED: 8/6/2009
DRILLING EQUIPMENT: Track mounted CME-55		TOTAL DEPTH (feet): 50	MEASURING POINT: Ground Surface
DRILLING METHOD: 4-inch diameter rotary wash		DEPTH TO FREE WATER FIRST ENCOUNTERED: 31.0feet (9:00 AM 8/6/2009)	
SAMPLING METHOD: See Boring Log Explanation, Figure A2-1		DEPTH TO FREE WATER AT COMPLETION:	
HAMMER WEIGHT: --		HAMMER DROP: --	
		LOGGED BY: C. Johnson	

RT-1 (3/03)

GEES-ROCK-3/03 GRADY ROCK LOGS.GPJ GES32003-7.GDT 8/18/09

DEPTH (feet)	RUN NO.	RECOVERY (%)	RQD (%)	FRACTURING	HARDNESS	STRENGTH	WEATHERING	LITHOLOGIC DESCRIPTION	GRAPHIC LOG	DISCONTINUITY DESCRIPTION	REMARKS
38											
39											
40											
41											
42	1	60	0	VC	So	Fr	Se-Mo	SHALE (continued) crushed, soft, friable, moderately weathered, locally interbedded with sandstone			Run 1: 42-43' 10:15-10:19 AM
43				NR	NR	NR	NR		N/R		
44								43.4': Includes dark gray, very fine sandstone bed 6" thick		Jo 60, Op, Pl -Jo, 40. 55/sh contact	Run 2: 43-47' 10:26-10:33 AM
45	2	70	0	VC	So	Fr	Se-Mo	45.2': Includes greenstone(?) bed or inclusion 0.4' thick		-altered, chloritized nodule Jo 30, Op, Pl	
46				NR	NR	NR	NR		N/R		
47								47-47.8': Shear zone(?), slickensides perpendicular to shear plane, waxy green chloritized shale or serpentinite		Sh, 70, Pl, Po	Run 3: 47-50' 10:37-10:42 AM
48	3	100	0	VC	So	Fr	Se-Mo				
49					Lo	We				Jo? 50, Ir, Ro Mc	
50								Bottom of boring 50.0'			
51											
52											

FRACTURING: VC-Very Close (<0.01'), CI-Close (0.1'-0.3'), Mo-Moderate (0.3'-1'), Wi-Wide (1'-3'), and VW-Very Wide (3'-10'). HARDNESS: So-Soft, Lo-Low, Mo-Moderate, Ha-Hard, and VH-Very Hard. STRENGTH: Fr-Friable, We-Weak, Mo-Moderate, St-Strong, and Ex-Extremely Strong. WEATHERING: Fr-Fresh, SI-Slight, Mo-Moderate, and Se-Severe. DISCONTINUITY: Type (Be-Bedding, Jo-Joint, Fo-Foliation, Sh-Shear, Me-Mechanical Break, and Ve-Vein), Dip Angle, Aperture (Ti-Tight, Op-Open, He-Healed, and Fi-Filled), Surface Shape (Ir-Irregular, Pl-Planar, and Wa-Wavy), Roughness (St-Stepped, Ro-Rough, Mo-Moderately Rough, SI-Slightly Rough, Sm-Smooth, and Po-Polished).										Project No. 14648.000
										Figure A2-6

PROJECT: Grady Ranch Development Marin County, California		Log of Boring No. RW-6(P)	
BORING LOCATION: Road realignment		ELEVATION AND DATUM: 232	
DRILLING CONTRACTOR: Taber Consultants		DATE STARTED: 8/6/2009	DATE FINISHED: 8/6/2009
DRILLING EQUIPMENT: Track mounted CME-55		TOTAL DEPTH (feet): 52	MEASURING POINT: Ground Surface
DRILLING METHOD: 4-inch diameter auger		DEPTH TO FREE WATER FIRST ENCOUNTERED:	
SAMPLING METHOD: See Boring Log Explanation, Figure A2-1		DEPTH TO FREE WATER AT COMPLETION:	
HAMMER WEIGHT: 140 lbs.	HAMMER DROP: 30 inches (Auto Hammer)	LOGGED BY: C. Johnson	

DEPTH (feet)	SAMPLES			MATERIAL DESCRIPTION	LABORATORY TESTS		
	Sample No.	Sample	Blows/foot		Moisture Content (%)	Dry Density (pcf)	Other
1				SILTY SAND (SM) loose, dark yellowish brown (10YR 4/4), dry, sand is fine to coarse, includes local subrounded gravel up to 1" max. diameter, coarse sand is rounded [ALLUVIUM]			11:50 AM start no liners sample bagged S1 13"/18" PP at 2.5= 1; <0.25; <0.25 (tsf) (sample crumbling) S2: 12"/18"
2	1		15				
3	2		2				
4							
5				CLAYEY SAND (SC) loose, very dark grayish brown (10YR 3/2), moist to wet, sand is fine, fines have medium plasticity and medium toughness, scattered gravels up to 1/2" max. diameter [ALLUVIUM]			S3: 14"/18" PP at 6'= 0.75; 1; 1 (tsf) S4: 13"/18" sample pushed 18"
6	3		2				
7	4		0				
8							
9							
10				10': Local red and strong brown, friable sandstone up to ~1/4" diameter 11.5': Sand is mostly very fine, fines content increases			S5: 11"/18" PP at 11'= 0.25; <0.25; 0.5; 0.5 (tsf) S6: 18"/18"
11	5		6				
12	6		3				
13							
14							
15				CLAYEY SAND with Gravel (SC) medium dense, dark brown (10YR 3/3), moist to slightly wet, sand mostly fine to medium, gravel mostly subrounded to rounded, variably weathered sandstone up to 1.5" max. diameter [ALLUVIUM]			S7: 16.5"/18" PP at 15.5'= 1; 1.25; >4.5 (tsf) (gravels) S8: 13"/18"
16	7		17				
17	8		19				

GEES-SOIL_12/03_GRADY_SOIL_LOGS.GPJ GES32003-7.GDT 8/18/09

GT-1 (12/03)

DEPTH (feet)	SAMPLES			MATERIAL DESCRIPTION	LABORATORY TESTS		
	Sample No.	Sample	Blows/foot		Moisture Content (%)	Dry Density (pcf)	Other
18	8			CLAYEY SAND with Gravel (SC) (cont.)			
19							
20				20': Fines content decreases slightly, medium to coarse sand content increases, becomes mostly dark yellowish brown (10YR 4/4)			S9: 14.5"/18" PP at 21'= 1; 1; 0.5; 0.5 (tsf)
21	9		21				S10: 18"/18"
22							
23	10		15				
24							
25				CLAYEY GRAVEL with Sand (GC) medium dense, dark yellowish brown (10YR 4/4), wet, gravel is rounded to subrounded sandstone and shale clasts up to 1" max. diameter [ALLUVIUM]			S11: 15.5"/18" PP at 26'= >4.5 (tsf) (gravels)
26	11		20				S12: 18"/18"
27							
28	12		10	CLAYEY SAND (SC) medium dense, dark yellowish brown (10YR 3/4), moist, sand is fine, includes medium plasticity clay bed/layer 27.2-27.8' [ALLUVIUM]			
29							
30				30': Increase in coarse sand includes rounded gravel up to 1.5" diameter			S13: 18"/18" PP at 31.5'= 2.5; 2.5; 2; 2.25; 1.5 (tsf)
31	13		22	SANDY CLAY (CL) firm, dark yellowish brown (10YR 3/4), moist, sand is fine, fines have medium plasticity and medium to high toughness, includes local black organic flecks, locally mottled with strong brown [ALLUVIUM]			S14: 18"/18"
32							
33	14		8				
34							
35				SANDSTONE dark olive gray (5Y 3.2), moderately hard, moderately strong, dark oxide-staining on fracture surfaces [FRANCISCAN (KJf)]			S15: 7"/11"
36	15		62 11"				S16= 4"/4"
37	16		50* 4"	Continued on rock log			
38							
39							

PROJECT: Grady Ranch Development Marin County, California		Log of Boring No. RW-6(P)	
BORING LOCATION: Road realignment		ELEVATION AND DATUM: 232	
DRILLING CONTRACTOR: Taber Consultants		DATE STARTED: 8/6/2009	DATE FINISHED: 8/6/2009
DRILLING EQUIPMENT: Track mounted CME-55		TOTAL DEPTH (feet): 52	MEASURING POINT: Ground Surface
DRILLING METHOD: 4-inch diameter rotary wash		DEPTH TO FREE WATER FIRST ENCOUNTERED:	
SAMPLING METHOD: See Boring Log Explanation, Figure A2-1		DEPTH TO FREE WATER AT COMPLETION:	
HAMMER WEIGHT: --		HAMMER DROP: --	
		LOGGED BY: C. Johnson	

RT-1 (3/03)

GEES-ROCK-3/03 GRADY ROCK LOGS.GPJ GES32003-7.GDT 8/18/09

DEPTH (feet)	RUN NO.	RECOVERY (%)	RQD (%)	FRACTURING	HARDNESS	STRENGTH	WEATHERING	LITHOLOGIC DESCRIPTION	GRAPHIC LOG	DISCONTINUITY DESCRIPTION	REMARKS
36								Continued from soil log			
37	1	0	0	NR	NR	NR	NR	INTERBEDDED SANDSTONE and SHALE shale is generally black, crushed, soft to low hardness and friable to weak, locally beds are softer/harder; sandstone is dark gray (N 4/), with closely spaced fractures, low hardness, and weak to moderately strong [FRANCISCAN (KJf)]	N/R		Run 1: 36.3-37' 1:50-1:52 PM
38				VC	So-Lo	Fr-We		37-39.1': Shale with abundant very thin (~0.1" thick) sandstone interbeds, cross-bedding locally	Jo 70 Op Pl Sl Be(?) 40 Jo 25 Fi, clay-lined Be, 50, cross-bedding 40' bed		Run 2: 37-42' 1:55-2:04 PM
39							Fr	39.1-41.1': Sandstone, includes abundant quartz/calcite veinlets	Be? 40 sh/ss contact Jo 55, Op, Pl-Ir, Sl		
40	2	82	50	Cl	Lo	We-Mo			Me		
41											
42				NR	NR	NR	NR	42-47.8': Dark greenish gray sandstone	N/R		Run 3: 42-47' 2:08-2:13 PM
43									Ve, 40, qtz/calcite Jo, 40, Fi, Pl-Ir, Rq, qtz lined		
44											
45	3	100	74	Cl-Mo	Lo	Mo			Jo, 70, Op, Pl, Rq		
46							Fr		Jo, 45, Op, Pl, Sl		
47									Jo, 50, Fi, thin shale bed, polished Be 40, shale bed 1/2" thick		Run 4: 47-52' 2:16-2:21 PM
48								47.8-49': Friable, soft shale	Be? Ir, sh/sh contact		
49	4	100	0	VC	So	Fr			Be 50, sh/ss contact		
50				Cl-Mo	Lo	Mo			Me Be 55		

FRACTURING: VC-Very Close (<0.01'), Cl-Close (0.1'-0.3'), Mo-Moderate (0.3'-1'), Wf-Wide (1'-3'), and VW-Very Wide (3'-10'). HA RDNESS: So-Soft, Lo-Low, Mo-Moderate, Ha-Hard, and VH-Very Hard. STRENGTH: Fr-Friable, We-Weak, Mo-Moderate, St-Strong, and Ex-Extremely Strong. WEATHE RING: Fr-Fresh, Sl-Slight, Mo-Moderate, and Se-Severe. DISCONTINUITY: Type (Be-Bedding, Jo-Joint, Fo-Foliation, Sh-Shear, Me-Mechanical Break, and Ve-Vein), Dip Angle, Aperture (Ti-Tight, Op-Open, He-Healed, and Fi-Filled), Surface Shape (Ir-Irregular, Pl-Planar, and Wa-Wavy), Roughness (St-Ste pped, Ro-Rough, Mo-Moderately Rough, Sl-Slightly Rough, Sm-Smooth, and Po-Polished).

Project No. 14648.000
Figure A2-7

DEPTH (feet)	RUN NO.	RECOVERY (%)	RQD (%)	FRACTURING	HARDNESS	STRENGTH	WEATHERING	LITHOLOGIC DESCRIPTION	GRAPHIC LOG	DISCONTINUITY DESCRIPTION	REMARKS
51	4	100	0	Cl-Mo	Lo	Mo	Fr	INTERBEDDED SANDSTONE and SHALE (cont.)			
52								Bottom of boring 52.0'			
53											
54											
55											
56											
57											
58											
59											
60											
61											
62											
63											
64											
65											
66											
67											
68											

RT-2 (3/03)

GEES-ROCK-3/03 GRADY ROCK LOGS.GPJ GES32003-7.GDT 8/18/09

PRELIMINARY

FRACTURING: VC-Very Close (<0.01'), Cl-Close (0.1'-0.3'), Mo-Moderate (0.3'-1'), Wi-Wide (1'-3'), and VW-Very Wide (3'-10'). HARDNESS: So-Soft, Lo-Low, Mo-Moderate, Ha-Hard, and VH-Very Hard. STRENGTH: Fr-Friable, We-Weak, Mo-Moderate, St-Strong, and Ex-Extremely Strong. WEATHERING: Fr-Fresh, SI-Slight, Mo-Moderate, and Se-Severe. DISCONTINUITY: Type (BJ-Bedding Joint, Be-Bedding, Jo-Joint, Fo-Foliation, Sh-Shear, Me-Mechanical Break, and Ve-Vein), Dip Angle, Aperture (Ti-Tight, Op-Open, He-Healed, and Fi-Filled), Surface Shape (Ir-Irregular, Pl-Planar, and Wa-Wavy), Roughness (St-Stepped, Ro-Rough, Mo-Moderately Rough, SI-Slightly Rough, Sm-Smooth, and Po-Polished).

PROJECT: Grady Ranch Development Marin County, California		Log of Boring No. RW-7	
BORING LOCATION: Lucas Valley Road (shoulder)		ELEVATION AND DATUM: 236	
DRILLING CONTRACTOR: Taber Consultants		DATE STARTED: 8/7/2009	DATE FINISHED: 8/7/2009
DRILLING EQUIPMENT: Track mounted CME-55		TOTAL DEPTH (feet): 52.5	MEASURING POINT: Ground Surface
DRILLING METHOD: 4-inch diameter auger		DEPTH TO FREE WATER FIRST ENCOUNTERED:	
SAMPLING METHOD: See Boring Log Explanation, Figure A2-1		DEPTH TO FREE WATER AT COMPLETION:	
HAMMER WEIGHT: 140 lbs.	HAMMER DROP: 30 inches (Auto Hammer)	LOGGED BY: C. Johnson	

DEPTH (feet)	SAMPLES			MATERIAL DESCRIPTION	LABORATORY TESTS		
	Sample No.	Sample	Blows/foot		Moisture Content (%)	Dry Density (pcf)	Other
1				GRAVEL ROAD FILL			
2	1		33	SILTY SAND (SM) medium dense, dark brown (10YR 3/3), dry to slightly moist local rootlets and degraded sandstone gravels up to 2" max diameter, sand is mostly fine [FILL?]			7:49 AM start driller noted "crunchy gravel" down to 1.5"
3							S1: 14.5"/18" PP at 2.5' = >4.5 (tsf)
4	2		15				S2: 12"/18"
5				5.0': Gravel content increases, includes some rounded sandstone clasts up to ~1/2" max diameter, may be colluvium?			S3: 6"/18" PP at 6' = >4.5 (tsf) (gravels)
6	3		8				S4: 4"/18"
7				6.5': Increasing dark yellowish brown, severely weathered sandstone clasts/fragments			
8							
9				SANDSTONE?			
10	5		50* 1.25"	No recovery [FRANCISCAN (KJf)]			S5: 0"/1.25"
11				Continued on rock log			switch to mud rotary 8:12 AM
12							
13							
14							
15							
16							
17							

GEES-SOIL 12/03 GRADY_SOIL_LOGS.GPJ GES32003-7.GDT 8/18/09

GT-1 (12/03)

PROJECT: Grady Ranch Development Marin County, California		Log of Boring No. RW-7	
BORING LOCATION: Lucas Valley Road (shoulder)		ELEVATION AND DATUM: 236	
DRILLING CONTRACTOR: Taber Consultants		DATE STARTED: 8/7/2009	DATE FINISHED: 8/7/2009
DRILLING EQUIPMENT: Track mounted CME-55		TOTAL DEPTH (feet): 52.5	MEASURING POINT: Ground Surface
DRILLING METHOD: 4-inch diameter rotary wash		DEPTH TO FREE WATER FIRST ENCOUNTERED:	
SAMPLING METHOD: See Boring Log Explanation, Figure A2-1		DEPTH TO FREE WATER AT COMPLETION:	
HAMMER WEIGHT: --		HAMMER DROP: --	
		LOGGED BY: C. Johnson	

DEPTH (feet)	RUN NO.	RECOVERY (%)	RQD (%)	FRACTURING	HARDNESS	STRENGTH	WEATHERING	LITHOLOGIC DESCRIPTION	GRAPHIC LOG	DISCONTINUITY DESCRIPTION	REMARKS
1											
2											
3											
4											
5											
6											
7											
8											
9											
10								Continued from soil log			
11	1	58	0	Cl	Lo	Fr-We	Se-Mo	SANDSTONE olive brown (2.5Y 4/4), intensely fractured, Mg-oxide stained fractures abundant, sand is mostly fine [FRANCISCAN (KJf)]	 Jo 40, Op Pl Sl Jo 80 Fr, Pl, Sl, Mg-ox	Run 1: 10.1-12.5' 8:29-8:33 AM	
12				NR	NR	NR	NR		 Jo 75 Fr, Pl, Sm, Mg-ox Jo 50 Fr, Pl, Sm, Mg-ox		
13								12.5-15.4': Variably weathered, includes 1/4" diameter root and soft, clayey, mottled zone surrounding it, locally oxidized to strong brown	 Jo 70, Fr, Pl, Sm, Mg-ox -1/4" diam. root	Run 2: 12.5-17' 8:39-8:44 AM	
14	2	64	0	VC	So-Lo	Fr-We	Se		 Jo 75 Op Pl Sm		
15											

FRACTURING: VC-Very Close (<0.01'), Cl-Close (0.1'-0.3'), Mo-Moderate (0.3'-1'), Wi-Wide (1'-3'), and VW-Very Wide (3'-10').
 HARDNESS: So-Soft, Lo-Low, Mo-Moderate, Ha-Hard, and VH-Very Hard. STRENGTH: Fr-Friable, We-Weak, Mo-Moderate, St-Strong, and Ex-Extremely Strong. WEATHERING: Fr-Fresh, Sl-Slight, Mo-Moderate, and Se-Severe. DISCONTINUITY: Type (Be-Bedding, Jo-Joint, Fo-Foliation, Sh-Shear, Me-Mechanical Break, and Ve-Vein), Dip Angle, Aperture (Ti-Tight, Op-Open, He-Healed, and Fi-Filled), Surface Shape (Ir-Irregular, Pl-Planar, and Wa-Wavy), Roughness (St-Stepped, Ro-Rough, Mo-Moderately Rough, SI-Slightly Rough, Sm-Smooth, and Po-Polished).

Project No. 14648.000
 Figure A2-8

RT-1 (3/03)

GEES-ROCK-3/03 GRADY_ROCK_LOGS.GPJ GES32003-7.GDT 8/18/09

RT-2 (3/03)

GEES-ROCK-3/03 GRADY ROCK LOGS.GPJ GES32003-7.GDT 8/18/09

DEPTH (feet)	RUN NO.	RECOVERY (%)	ROD (%)	FRACTURING	HARDNESS	STRENGTH	WEATHERING	LITHOLOGIC DESCRIPTION	GRAPHIC LOG	DISCONTINUITY DESCRIPTION	REMARKS
16	2	64	0	VC	So	Fr	Se	SANDSTONE (cont.)			
17				NR	NR	NR	NR		N/R		
18				VC	Lo	We	Se-Mo	17-18.2': Sandstone, generally crushed and pulverized by coring		Jo 85, Fi, Pl, clay-lined	Run 3: 17-20' 8:49-8:56 AM
19	3	40	0	NR	NR	NR	NR		N/R		
20								20': Sandstone becomes slightly harder and stronger than above		Jo 80 Op Pl Sm Jo 90 Fi clay-lined	Run 4: 20-22.5' 8:59-9:05 AM
21	4	92	16	Cl	Lo	We	Mo			Me, 25 .55 Jo, Op, Pl, Ir, Sl, Mg-ox	
22				NR	NR	NR	NR		N/R	Jo 70, Op, Pl, Sm, Mg-ox	
23										Jo 70, Fi, clay-lined	Run 5: 22.5-27.5' 9:09-9:15 AM
24										Jo, 60, Fi, clay-lined	
25	5	96	14	Cl	Lo	We	Mo			Jo 25 Op Pl Ro Jo 25 Op Pl Ro	
26											
27				NR	NR	NR	NR		N/R	Me	
28								27.5': Grades to dark greenish gray (10Y 4/), less weathered, becomes slightly stronger		Jo 40 Op Pl Sl	Run 6: 27.5-32.5' 9:19-9:26 AM
29											
30	6	100	58	Cl	Lo	We	Sl			Jo 50 Fi clay-lined Jo 30 Op, Ir, Ro Be, 35, 2" thick bed v. fine	
31										Jo, 70, Fi, clay-lined	
32										Jo 30 Op Pl Sm Mg-ox Jo 25 Op Pl Sm	
33										Jo 40, Ti, Pl, Ro Jo, 30, Op Pl Sl	Run 7: 32.5-37.5' 9:56-10:01 AM Driller noted total fluid loss starting at ~32.5'

FRACTURING: VC-Very Close (<0.01'), Cl-Close (0.1'-0.3'), Mo-Moderate (0.3'-1'), Wl-Wide (1'-3'), and VW-Very Wide (3'-10'). HA RDNESS: So-Soft, Lo-Low, Mo-Moderate, Ha-Hard, and VH-Very Hard. STRENGTH: Fr-Friable, We-Weak, Mo-Moderate, St-Strong, and Ex-Extremely Strong. WEATHERING: Fr-Fresh, Sl-Slight, Mo-Moderate, and Se-Severe. DISCONTINUITY: Type (BJ-Bedding Joint, Be-Bedding, Jo-Joint, Fo-Foliation, Sh-Shear, Me-Mechanical Break, and Ve-Vein), Dip Angle, Aperture (Ti-Tight, Op-Open, He-Healed, and Fi-Filled), Surface Shape (Ir-Irregular, Pl-Planar, and Wa-Wavy), Roughness (St-Stepped, Ro-Rough, Mo-Moderately Rough, Sl-Slightly Rough, Sm-Smooth, and Po-Polished).

Project No.
14648.000

Figure A2-8 Cont.

RT-2 (3/03)

GEES-ROCK-3/03 GRADY_ROCK_LOGS.GPJ GES32003-7.GDT 8/18/09

DEPTH (feet)	RUN NO.	RECOVERY (%)	RQD (%)	FRACTURING	HARDNESS	STRENGTH	WEATHERING	LITHOLOGIC DESCRIPTION	GRAPHIC LOG	DISCONTINUITY DESCRIPTION	REMARKS
34								SANDSTONE (cont.)		Jo 65 op PI Fr, Ro	
35	7	96	10			We-Mo				Jo 60, Pi clay-lined Jo 75, Op PI SI	
36				Cl	Lo		SI			Jo 65, Op PI Sm Jo 65 Pi clay-lined	
37								37': Grades to dark greenish gray (10BG 4/)		Jo 50 Op PI SI	
38				NR	NR	NR	NR			Jo 70 op PI SI Jo 45 op PI SI	Run 8: 37.5-42.5' 10:06-10:12 AM
39										Jo 50 Op Ir SI	10:20 AM: Driller has inner barrel stuck in casing rods; has to pull all out to retrieve
40	8	96	18	Cl	Mo	Mo	Fr			Jo 40 Op Ir-PI SI	
41										Me	
42				NR	NR	NR	NR			Jo 70 Pi clay-lined Be 30 shale bed 3/4" th	Run 9: 42.5-45.4' 10:44-10:49 AM
43								42.5-44.9': Sandstone is locally interbedded with black, friable shale beds up to 4" thick		Jo 70 Op PI Ro	
44	9	83	21	Cl	Mo	Mo	Fr			Be 40 ss/sh contact	
45				NR	NR	NR	NR			Be 80 3" thick sh bed Jo 50 op PI Sm-Po	Run 10: 45.4-47.5' 10:53-10:59 AM
46	10	100	62		Mo-Lo	Mo-Fr		45.4-47.3': Includes abundant shale interbeds up to 3" thick		Be 30 ss/sh contact Jo 25 Op PI Sm/PA	
47					Lo	Fr		47.3-47.6': Shale, abundant polishes fracture/bedding surfaces		Jo 40 op PI Sm	Run 11: 47.5-52.5' 11:03-11:12 AM
48										Be 45, sh bed 2" thick Be 30 sh bed 4" thick Jo 40 Op PI Ro	
49				Cl-Mo			Fr			Be 30 soft shale bed 1" thick Jo 35 Op PI Sm	
50	11	100	56		Mo	Mo		50': Sand becomes very fine			
51											
52											

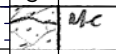
FRACTURING: VC-Very Close (<0.01'), Cl-Close (0.1'-0.3'), Mo-Moderate (0.3'-1'), Wi-Wide (1'-3'), and VW-Very Wide (3'-10'). HA RDNESS: So-Soft, Lo-Low, Mo-Moderate, Ha-Hard, and VH-Very Hard. STRENGTH: Fr-Friable, We-Weak, Mo-Moderate, St-Strong, and Ex-Extremely Strong. WEATHERING: Fr-Fresh, SI-Slight, Mo-Moderate, and Se-Severe. DISCONTINUITY: Type (BJ-Bedding Joint, Be-Bedding, Jo-Joint, Fo-Foliation, Sh-Shear, Me-Mechanical Break, and Ve-Vein), Dip Angle, Aperture (Ti-Tight, Op-Open, He-Healed, and Fi-Filled), Surface Shape (Ir-Irregular, PI-Planar, and Wa-Wavy), Roughness (St-Stepped, Ro-Rough, Mo-Moderately Rough, SI-Slightly Rough, Sm-Smooth, and Po-Polished).

Project No.
14648.000

Figure A2-8 Cont.

PROJECT: Grady Ranch Development
Marin County, California

Log of Boring No. RW-7 cont.

DEPTH (feet)	RUN NO.	RECOVERY (%)	ROD (%)	FRACTURING	HARDNESS	STRENGTH	WEATHERING	LITHOLOGIC DESCRIPTION	GRAPHIC LOG	DISCONTINUITY DESCRIPTION	REMARKS
11								SANDSTONE (cont.)			
53								Bottom of boring 52.5'			
54											
55											
56											
57											
58											
59											
60											
61											
62											
63											
64											
65											
66											
67											
68											
69											
70											

RT-2 (3/03)

GEES-ROCK.3/03 GRADY ROCK LOGS.GPJ GES32003-7.GDT 8/18/09

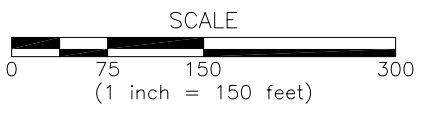
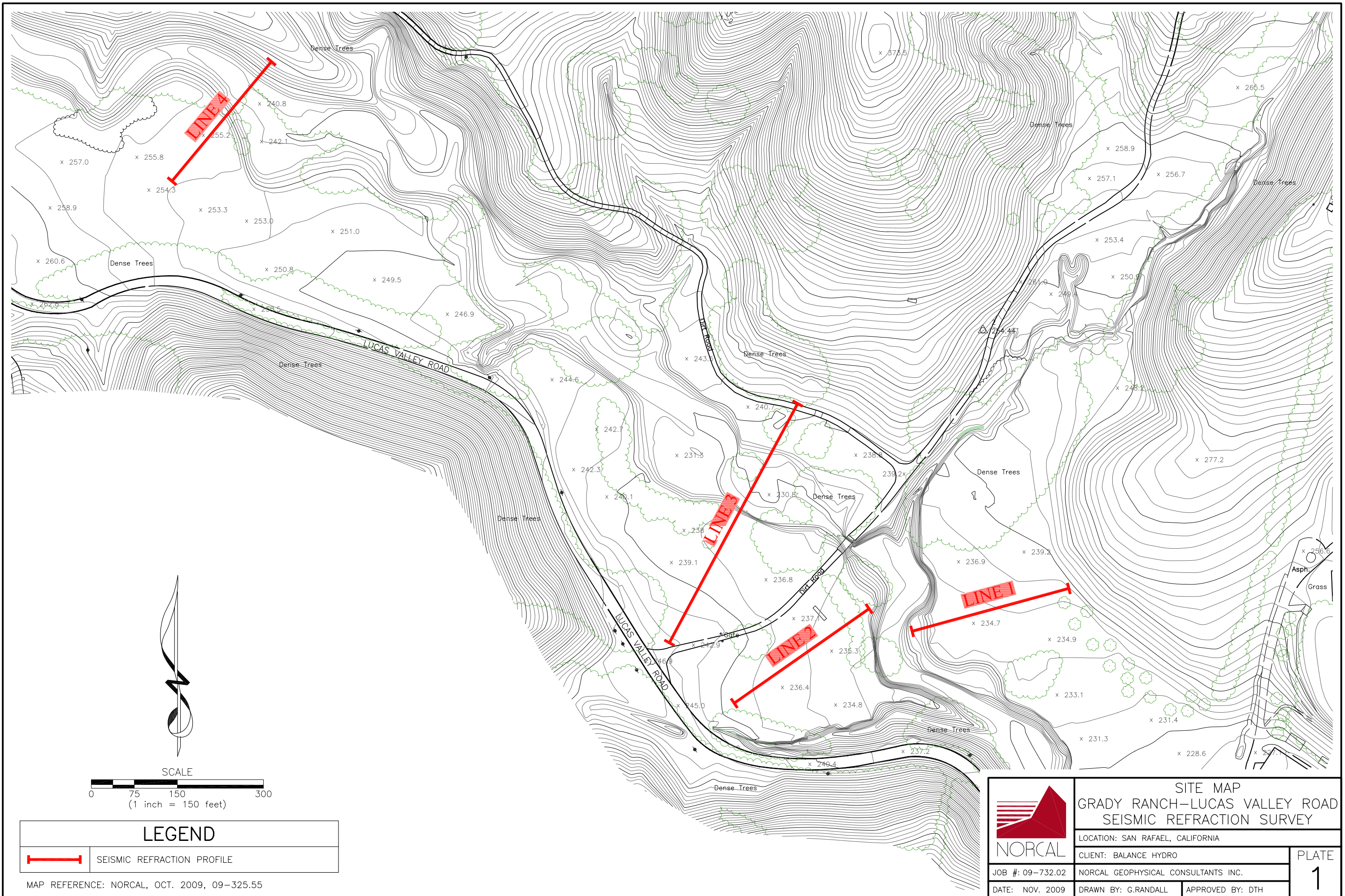
PRELIMINARY

FRACTURING: VC-Very Close (<0.01'), CI-Close (0.1'-0.3'), Mo-Moderate (0.3'-1'), WI-Wide (1'-3'), and VW-Very Wide (3'-10'). HARDNESS: So-Soft, Lo-Low, Mo-Moderate, Ha-Hard, and VH-Very Hard. STRENGTH: Fr-Friable, We-Weak, Mo-Moderate, St-Strong, and Ex-Extremely Strong. WEATHERING: Fr-Fresh, SI-Slight, Mo-Moderate, and Se-Severe. DISCONTINUITY: Type (BJ-Bedding Joint, Be-Bedding, Jo-Joint, Fo-Foliation, Sh-Shear, Me-Mechanical Break, and Ve-Vein), Dip Angle, Aperture (TI-Tight, Op-Open, He-Healed, and Fi-Filled), Surface Shape (Ir-Irregular, Pl-Planar, and Wa-Wavy), Roughness (St-Stepped, Ro-Rough, Mo-Moderately Rough, SI-Slightly Rough, Sm-Smooth, and Po-Polished).

Project No.
14648.000
Figure A2-8 Cont.


APPENDIX H

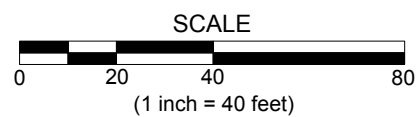
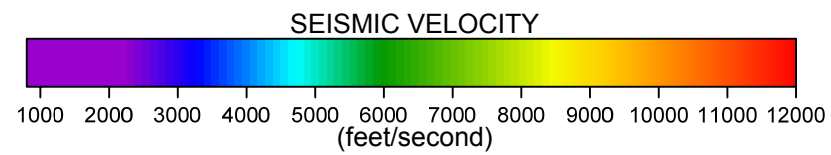
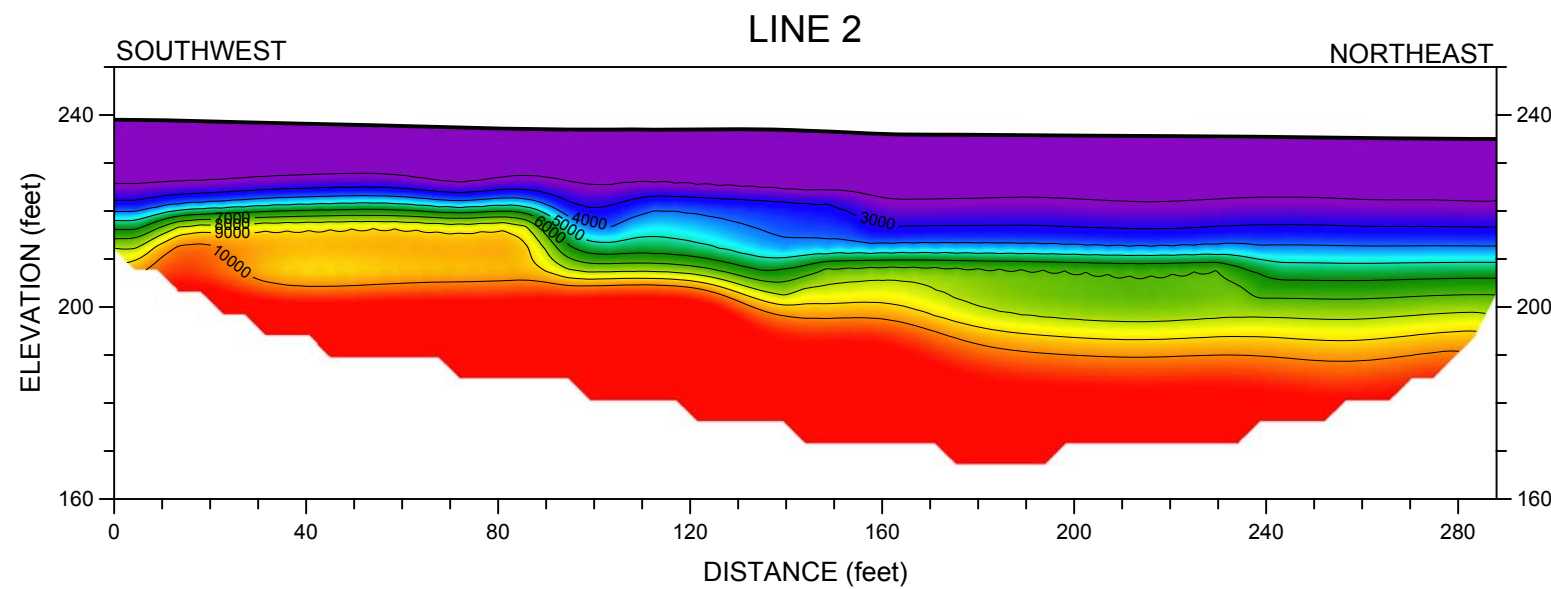
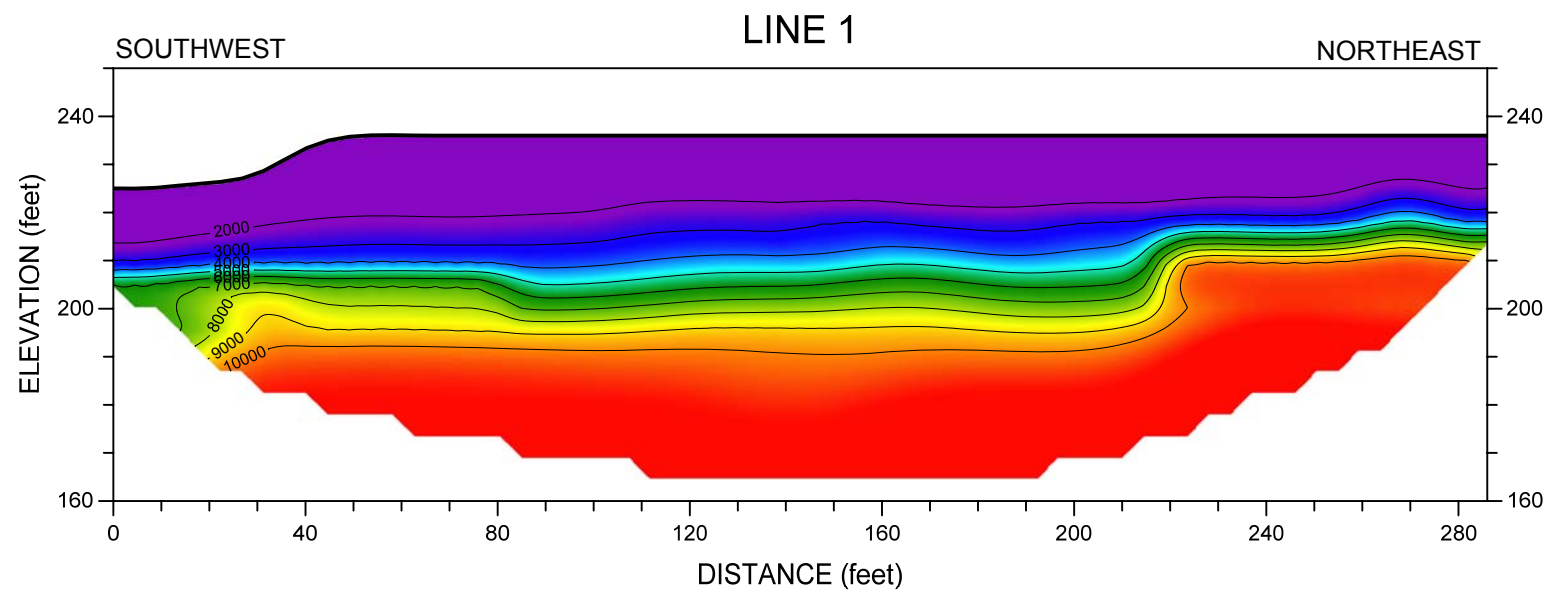
Seismic refraction survey (November 2009)



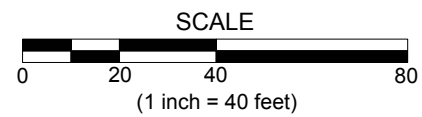
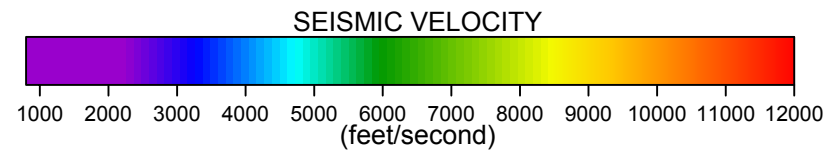
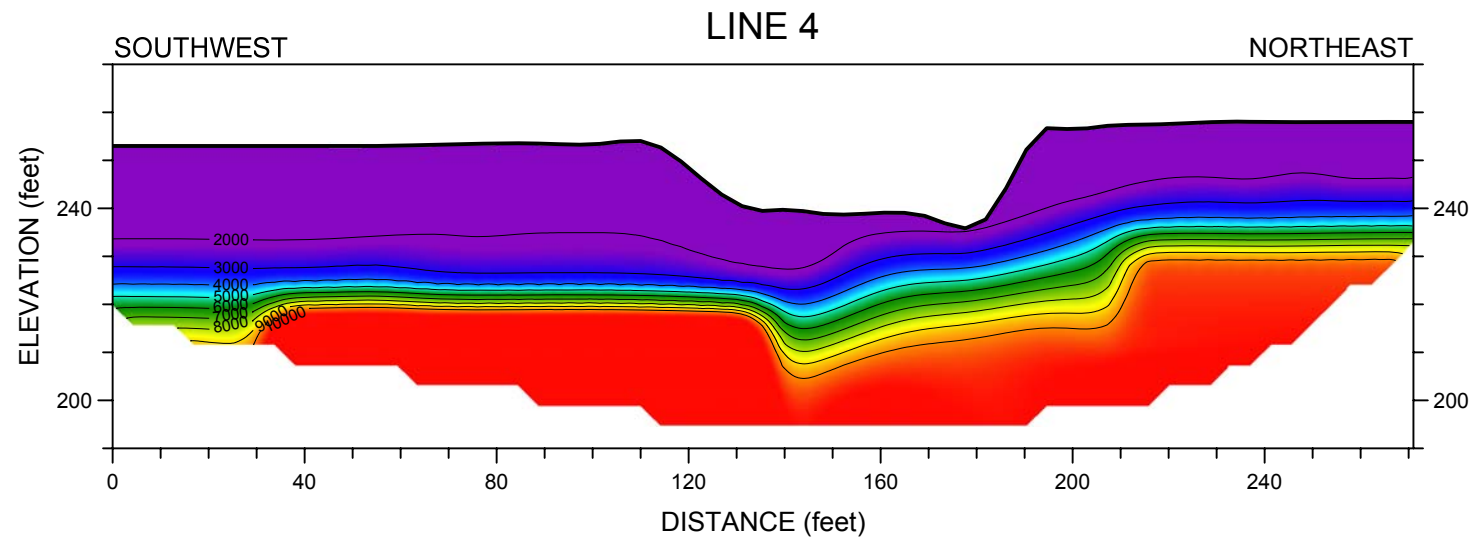
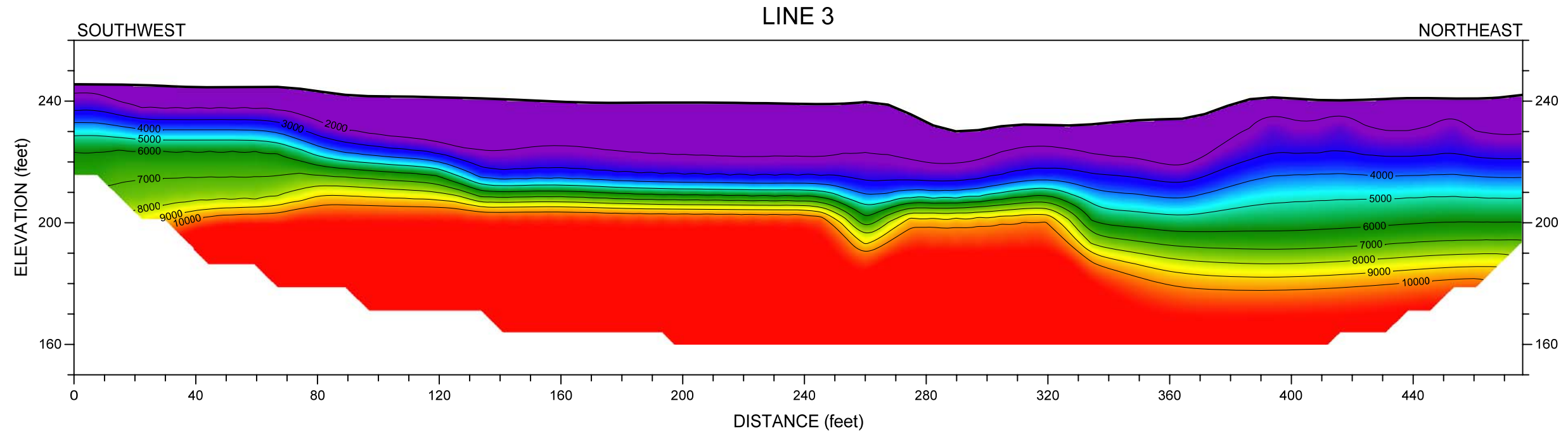
LEGEND	
	SEISMIC REFRACTION PROFILE


MAP REFERENCE: NORCAL, OCT. 2009, 09-325.55

	SITE MAP GRADY RANCH—LUCAS VALLEY ROAD SEISMIC REFRACTION SURVEY	
	LOCATION: SAN RAFAEL, CALIFORNIA	
JOB #: 09-732.02	CLIENT: BALANCE HYDRO	
DATE: NOV. 2009	DRAWN BY: G.RANDALL	APPROVED BY: DTH
		PLATE 1



 NORCAL	SEISMIC REFRACTION PROFILES		2
	LINES 1 & 2		
	GRADY RANCH - LUCAS VALLEY ROAD		
	LOCATION: SAN RAFAEL, CALIFORNIA		
JOB #: 09-732.02	CLIENT: BALANCE HYDRO		
DATE: NOV. 2009	NORCAL GEOPHYSICAL CONSULTANTS INC.		
	DRAWN BY: G.RANDALL	APPROVED BY: DTH	



 NORCAL	SEISMIC REFRACTION PROFILES		PLATE 3
	LINES 3 & 4		
	GRADY RANCH - LUCAS VALLEY ROAD		
	LOCATION: SAN RAFAEL, CALIFORNIA		
JOB #: 09-732.02	CLIENT: BALANCE HYDRO		
DATE: NOV. 2009	NORCAL GEOPHYSICAL CONSULTANTS INC.		
	DRAWN BY: G.RANDALL	APPROVED BY: DTH	

APPENDIX I

Electrical resistivity survey (January 2010)

January 19, 2010

Mr. Gregg Grubin
CSW/ST2
45 Leveroni Court
Novato, CA 94949

Subject: Electrical Resistivity Survey
Grady Ranch Project, Marin County, CA
NORCAL Job # 09-732.03

Dear Mr. Grubin:

This letter presents the findings of an electrical resistivity (ER) survey performed by NORCAL Geophysical Consultants, Inc. at the Grady Ranch in Marin County, California. The survey was performed on December 29 and 30, 2009 by NORCAL California Professional Geophysicist Donald J. Kirker and Geophysical Technician Travis Black. Logistical support was provided by Lou Bouc of Lucasfilm Ltd. and Steve Korbay of Geokor.

1.0 SITE DESCRIPTION

The Grady Ranch Project is a proposed development located off of Lucas Valley Road northwest of San Rafael, CA. It is characterized by relatively steep hills covered with low grass and trees. The parcel is accessed by a gravel/dirt road that traverses the property along the east boundary. Outcropping rock is evident at various locations on the property. The local geology consists of Franciscan Complex bedrock overlain by Quaternary colluvium that includes stream deposits from Lucas Valley Creek. Past surveys in this area by NORCAL defined bedrock at depths of 20- to 35-ft. near the creek.

The ER survey was conducted along three lines, as determined by Lou Bouc and Steve Korbay. They are generally located across a wide drainage near the entrance to the property. These locations were selected due to the creek proximity and the possibility of water bearing stream deposits or deeper zones within the bedrock that may also be water-bearing. The locations of these lines, and the site topography, are shown on Plate 1. Surface elevations along these lines range from approximately 228 to over 290 feet above mean sea level (msl).

2.0 PURPOSE

The purpose of the ER survey is to measure variations in the electrical properties of the subsurface to aid in determining lithologic changes that may be related to relative permeability. We understand that this information will be used to aid in the placement of a water supply well.

3.0 FIELD INVESTIGATIONS

3.1 ELECTRICAL RESISTIVITY

The groundwater bearing potential for a given site can be dependent on inherent lithologic variations. These variations may be defined by characterizing the changes in specific electrical resistivity values both laterally and with depth. Generally, fine-grained low-permeable sedimentary deposits will exhibit low electrical resistivities, whereas coarser grained materials will have higher values and generally higher permeability. Therefore electrical resistivity surveys provide a means for defining increased permeability zones within a sedimentary section. Within a bedrock section, as the degree of weathering and fracturing increases, the electrical resistivity values will generally decrease. However, the actual rock lithology also influences the electrical resistivity values. Shear zones or lateral changes in rock characteristics may also manifest abrupt changes in resistivity. Therefore, characterizing the electrical properties both laterally and with depth can provide subsurface information that may be related to water-bearing potential both within the sedimentary and bedrock sections. Interpreted potential drill targets, however, are typically based on a qualitative assessment of the character and/or configuration of the modeled resistivity values, and not based on the detection of groundwater. Therefore there is no guarantee that water will be encountered, only that the chances of encountering water are increased due to the targeting of certain subsurface features.

For this investigation, we used the electrical resistivity (ER) method with the dipole-dipole electrode configuration. The dipole-dipole array consists of four electrodes that are placed in the ground in a collinear arrangement. One pair of adjacent electrodes is used to transmit electrical current into the earth. The second pair of electrodes is used to measure the resulting potential drop (voltage). This electrode configuration provides information on both the depth and lateral extent of subsurface electrical properties.

3.2 DATA ACQUISITION AND ANALYSIS

We obtained high resolution ER data along 3 profiles, as shown on Plate 1. They are designated as Lines 1 through 3 and are oriented either southwest to northeast or northwest to southeast. The dipole-dipole resistivity data were collected using an electrode spacing of 10-feet, resulting in total profile lengths of 550-ft. The positions of these lines were recorded using our hand-held portable Global Positioning System (GPS).

ER data were collected using a Supersting/Swift electrical resistivity system manufactured by Advanced Geoscience, Inc. (AGI) of Austin, Texas. This system was configured to collect ER data using an array of 56 stainless steel electrodes distributed in a collinear array. The electrodes were connected to the Swift switch-box by four multi-conductor cables with 14-connectors (take-outs) per cable. The Supersting was programmed to control the Swift switch-box to turn on and off specific electrodes (four at a time) while automatically collecting the ER data.

Upon completion of the ER survey, we downloaded the apparent resistivity data to a lap-top computer using the software *AGI Administrator* by Advanced Geoscience, Incorporated (AGI). We also used this software to convert the data to a format suitable for inversion. We then used the computer program *EarthImager*, also by AGI, to invert the data and derive a model that provides an appropriate fit to the measured data. The computer program *Surfer 9.0* by Golden Software was then used to contour the

calculated data, producing a 2-D model showing the variation of the electrical resistivity values both laterally and with depth. Further descriptions of the ER methodology, data acquisition, and analysis are provided in Appendix A.

4.0 RESULTS

The results of the ER survey are illustrated on Plates 1 through 3. Plate 1 is the Site Location Map showing the location of Lines 1 through 3, and the pertinent site features. Plates 2 and 3 contain model resistivity profiles showing variations of the electrical resistivity values both laterally and with depth along each line. The resistivity variations are manifested by contours and gradational color shading that represents specific resistivity values. A graduated scale relating resistivity to color is included at the bottom of each plate.

4.1 ELECTRICAL RESISTIVITY PROFILES AND INTERPRETATION

The resistivity profiles illustrate general variations in electrical resistivity to depths of about 110- to 115-ft below ground surface (bgs). The resistivities range from less than 5 ohm-m to over 700 ohm-m. Based on the distribution of values we have divided the resistivities into three ranges consisting of low, moderate, and high. Low resistivities range from 2- to 79-ohm meters and are represented by dark blue to light blue colors. Moderate resistivities range from 79- to 209-ohm meters and are represented by dark to light green colors. High resistivities range from 209- to over 700-ohm meters and are represented by yellow to red colors.

Specific resistivity values can be related to various soil types and bedrock conditions. Regarding soil types, low values (blue shading) are generally indicative of fine grained silts and clays. Moderate values (green shading) are typically indicative of poorly sorted sands with lesser fine grained materials. The highest resistivity values (yellow to red shading) are representative of coarse grained materials such as sands and gravels with minor amounts of fine grained materials.

The wide range of resistivity values along Lines 1 through 3 can also be manifested by variations in the character of bedrock. Rock formations that are deeply buried and not exposed to chemical weathering are generally impermeable, contain little water, and have a relatively high electrical resistivity. Conversely, highly weathered and fractured rock that contains moisture typically has a lower resistivity.

4.1.1 Alluvium and Bedrock Layering

Based on our review of this data and the general depth to bedrock as defined from our previous surveys in this area, we have made an interpretation of the electrical resistivity variations as they may relate to the alluvium and bedrock layers. This is shown as a hatched line and is referred to as 'Interpreted Bedrock Interface' on Plates 2 and 3.

The surface layer of alluvium varies in thickness from 10- to 33-ft and is generally thickest along Lines 1 and 2. It consists primarily of moderate and high resistivities with some isolated zones of low resistivity material. The high resistivity values (yellow to red) form small isolated zones at the northeast end of Line 1 and in the center of Lines 2 and 3. As mentioned above, these zones are indicative of coarse grained materials such as sand and gravel, and represent the most permeable material in the alluvium. The low resistivities (blue) throughout the center of Line 1 and at the northwest end of Line 3 are indicative of fine grained material and represent the least permeable material.



CSW/ST2
January 19, 2010
Page 4

The bedrock is generally characterized by large alternating zones of moderate and low resistivity values. The moderate values (green) at the center of Line 1 and the northwest ends of Lines 2 and 3, probably represent more competent bedrock that is slightly weathered and/or fractured. The low resistivity bedrock (blue) may indicate an increase in the fracturing and moisture content of the bedrock.

4.1.2 Recommended Well Locations

Based on our interpretations of the character of the alluvium and bedrock, we have marked four areas as potential well locations. They are referred to as "Recommended Well Locations 1 through 4" on Plates 1 through 3. Location #1 is within the center of Line 1 and targets the thicker zone of alluvium and the underlying moderately high resistivity bedrock zone. Location #2 is at the southwest end of Line 1 and targets the very large zone of low resistivity values within the bedrock at a depth of 30- to 60-ft bgs. This zone may represent fractured rock that could be water bearing. Location #3 is located at a profile distance of 200 ft along Line 2 and targets both the shallow zone of higher resistivity alluvium and the underlying lateral bedrock change that may represent a variation in the bedrock weathering or fracturing characteristics. Location #4 is located at a profile distance of 245 ft along Line 3 and targets the zone of low resistivity values at a depth of 20- to 40-ft bgs. These values form a small zone in the shallow bedrock section that may represent a water bearing fracture zone. This location should be considered the lowest priority of the four recommended drill locations.

5.0 STANDARD OF CARE

The scope of NORCAL's services for this project consisted of using geophysical methods to characterize the subsurface. The accuracy of our findings is subject to specific site conditions and limitations inherent to the techniques used. We performed our services in a manner consistent with the standard of care ordinarily exercised by members of the profession currently employing similar methods. No warranty, with respect to the performance of services or products delivered under this agreement, expressed or implied, is made by NORCAL.

We appreciate having the opportunity to provide you with this information.

Respectfully,

NORCAL Geophysical Consultants, Inc.

A handwritten signature in black ink that reads "Donald J. Kirker".

Donald J. Kirker
Geophysicist, PGp-997

DJK/tt

Enclosures: Plates 1 through 3



Appendix A

GEOPHYSICAL METHODOLOGY



Appendix A

ELECTRICAL RESISTIVITY SURVEYS

Rational

Electrical resistivity is the physical property of a material that resists the flow of electrical current. The electrical resistivity of earth materials is directly affected by moisture content and permeability. Typically, electrical resistivity decreases as permeability and moisture content increases. The resistivity of earth materials is also greatly affected by the concentration of dissolved salts or free ions in the saturating fluid. Generally, fine-grained materials such as clays have a lower electrical resistivity than coarse grained materials such as sands and gravels. The presence of fluids that have a high concentration of dissolved salts or free ions can significantly decrease the electrical resistivity of both fine and coarse-grained materials.

Electrical properties of rock can vary greatly depending upon degree of weathering and fracturing, as well as composition. Rock formations that are deeply buried and not exposed to chemical weathering are generally impermeable, contain little water, and have a relatively high electrical resistivity. Conversely, highly weathered and fractured rock that contains moisture typically has lower resistivity.

Based on the above relationships, geophysical methods that measure the electrical resistivity of the subsurface can be used to determine lateral changes due to possible faulting, the depth and/or lateral extent of possible water-bearing formations, and the depth to bedrock.

Methodology

The electrical resistivity of the subsurface is measured using a galvanic resistivity method. This consists of transmitting electrical current into the earth through a pair of grounded metal electrodes, and measuring the resulting potential drop across the second pair of grounded metal electrodes. There are a variety of electrode arrangements (arrays) that can be used. The dipole-dipole electrode configuration is typically used because it provides information on both the depth and lateral extent of subsurface electrical properties.

The dipole-dipole array consists of four electrodes that are placed in the ground in a collinear arrangement. One pair of adjacent electrodes is used to transmit current into the earth and is referred to as the current dipole. The second pair of electrodes is used to measure the resulting potential drop, and is referred to as the potential dipole. Both dipoles have the same length.

To begin a profile, a reading is taken with the dipoles separated by their common length. Subsequent readings are taken as the potential dipole is moved along the profile while the current dipole remains stationary. The separation between dipoles is always a multiple of the dipole length. As the separation between dipoles increases, so does the depth of investigation. Once the maximum separation is reached, the current dipole is moved along the profile one dipole length and the entire procedure is repeated.

For each reading, a value is calculated that represents the apparent resistivity of the volume of earth that the current flows through. The term, apparent, is used because the value represents the resistivity of a volume rather than an individual layer. The apparent resistivity values are then plotted in cross-section and contoured to form what is referred to as a "pseudo-section". The term "pseudo" is used because the vertical scale is not scalar but is proportional to the dipole separation. In addition, the resistivities are apparent rather than true. However, the pseudo-section can be inverted to generate a 2-D model showing the depth and true resistivity of subsurface layers.

Instrumentation

Apparent resistivity data is typically acquired using a SuperSting R1 Resistivity meter with the Swift automatic multi-electrode system. Both systems are manufactured by Advanced Geosciences Incorporated (AGI). The Sting is a self-contained unit that transmits current at outputs ranging from 1 to 500 milliamps (mA). The unit also measures the potential drop and converts the data to values of apparent resistivity for a number of electrode arrays. The data are stored in internal memory and can be downloaded to a computer for processing. The Swift consists of an electrode interface console, four cables, and 56 stainless steel electrodes. Each cable has 14 individual take-outs that can be connected to electrodes at intervals up to 10 meters (33 feet). Depending on the objective of the survey, the Swift can operate using 28 to 56 electrodes.

Data Acquisition

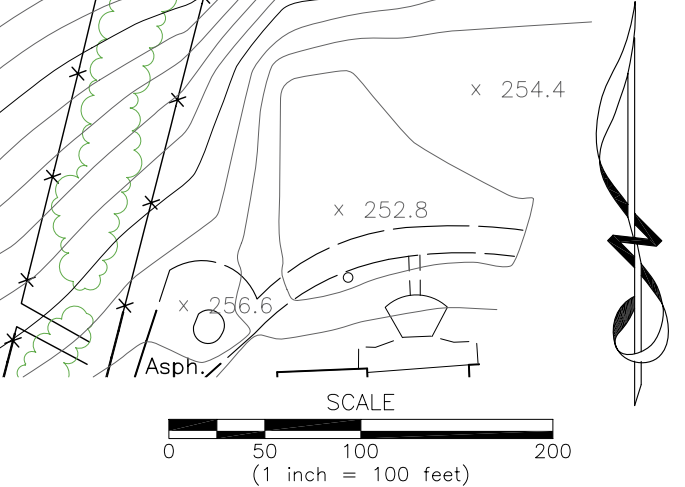
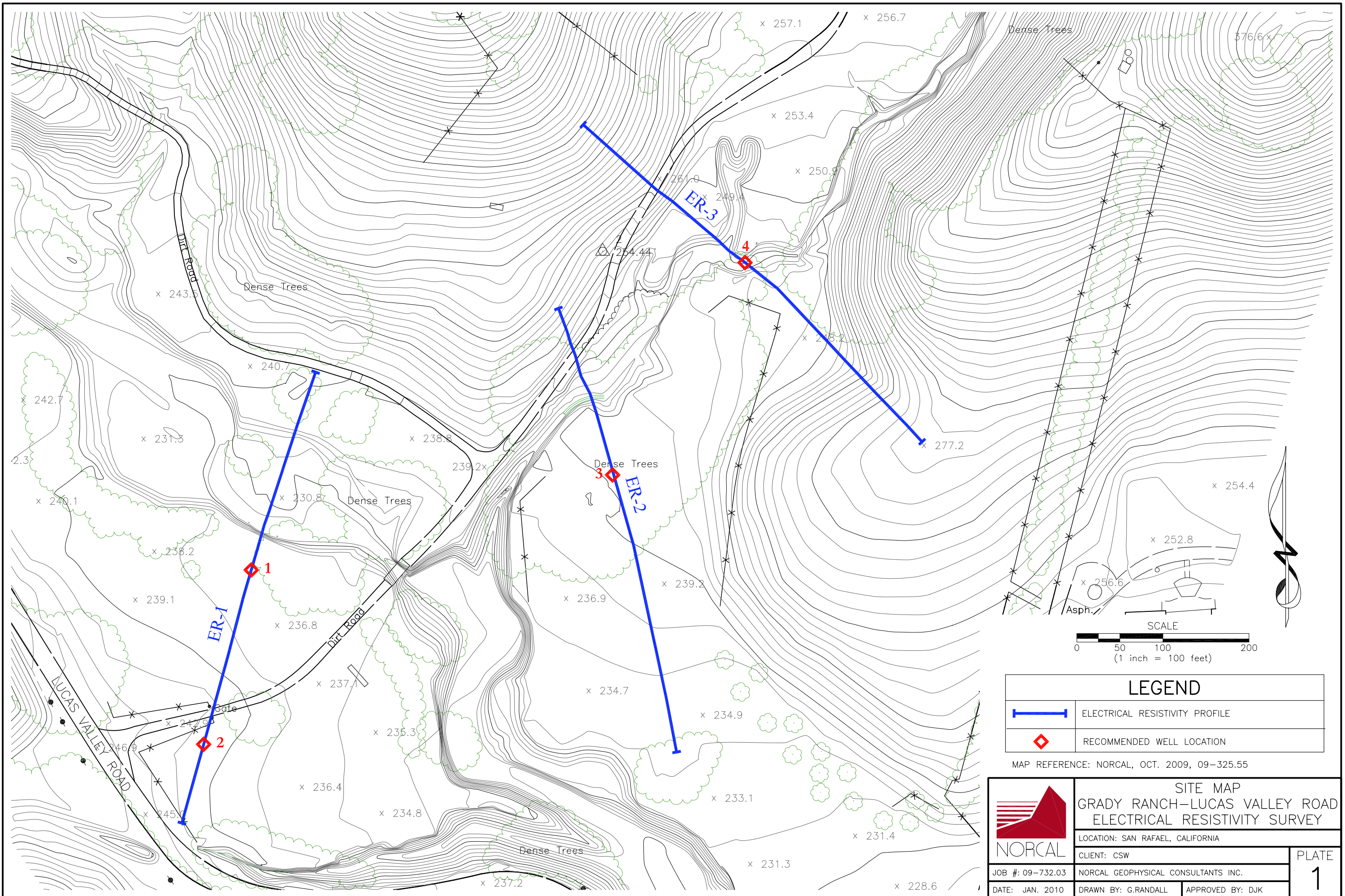
ER surveys using the Sting/Swift resistivity system are initiated by laying out the cables, end-to-end, along each profile. The Swift console is then connected between the two cables and to the Sting ER meter. At each take-out in the cable, stainless steel electrodes are driven into the ground and then fastened to the respective take-out. To begin the survey, the ER meter tests the contact resistance of each electrode. If any of the values are abnormally high, the electrode plant as well as the connection between the electrode and the switch is inspected, and if necessary, improved. The survey is begun once all of the electrode contacts tested satisfactory. To start out, readings are taken with the dipoles separated by their common length and moved along the length of the array. For example, if the length between two electrodes (referred to as a dipole) is 10 meters (33 feet), then the distance between the current and potential dipoles (two electrodes each) will also be 10 meters (33 feet). Since each of the switches are individually addressable by the Sting, the instrument is able to move this configuration down the array by turning the appropriate switches on and off, as necessary, to switch from one dipole to another. Subsequent readings are then taken by increasing the distance between dipoles, up to eight times the dipole separation, along the array. It then repeats the entire procedure using dipole lengths typically two to three times the length of the initial dipole. For example, if the initial dipole was 10 meters (33 feet), then the Sting/Swift system repeats the process using dipole lengths of 20 and 30 meters (66 and 98 feet).

Data Analysis

Upon completion of a dipole-dipole survey, apparent resistivity data are downloaded from the Sting to a lap-top computer using the program STINGDMP. The data are inverted to true resistivity versus depth and distance using the program EARTHIMAGER 2D. Both programs are written by AGI. The data generated by the EARTHIMAGER 2D program are then gridded and contoured using the computer program Surfer 9.0 by Golden Software to produce 2-D models.

Limitations

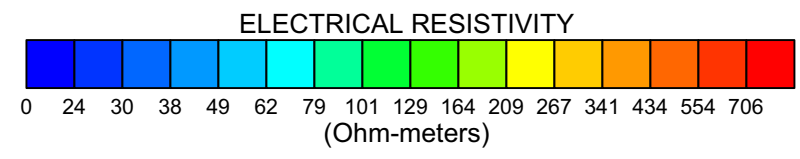
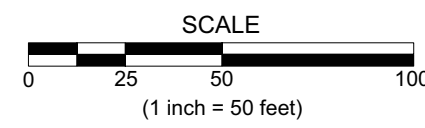
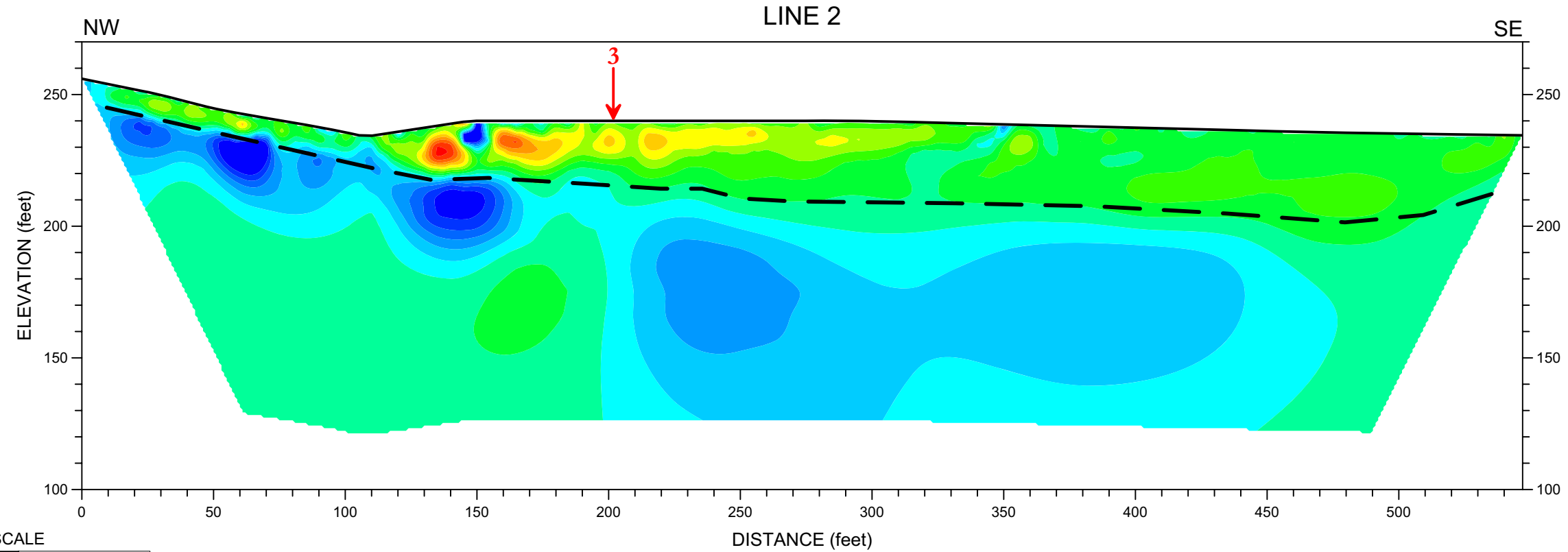
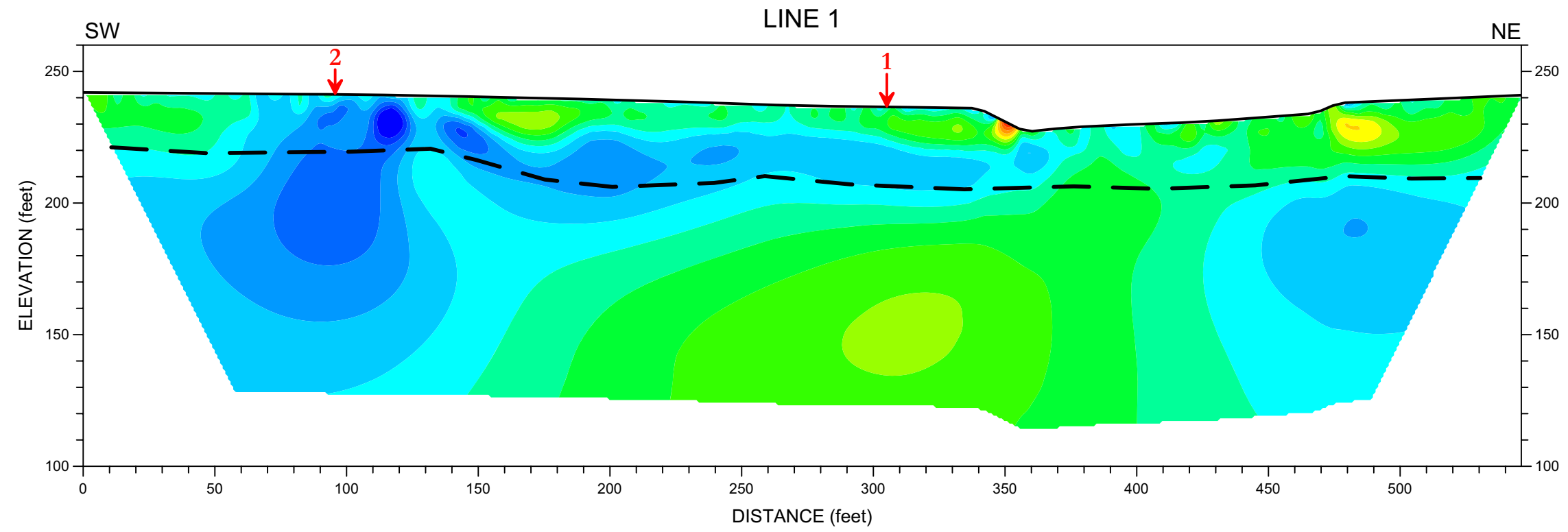
A common feature of all electrical methods is that the models derived from the electric profiling are not unique. That is, depending on the subsurface geo-electric structure, there may be many models that will produce essentially the same apparent resistivities. This is known as the *principal of equivalence*. To overcome this limitation, computer software programs include routines for evaluating the equivalence of a given model relative to the observed resistivity values, resulting in a model that provides the closest fit to the observed data.



LEGEND	
	ELECTRICAL RESISTIVITY PROFILE
	RECOMMENDED WELL LOCATION

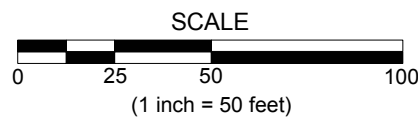
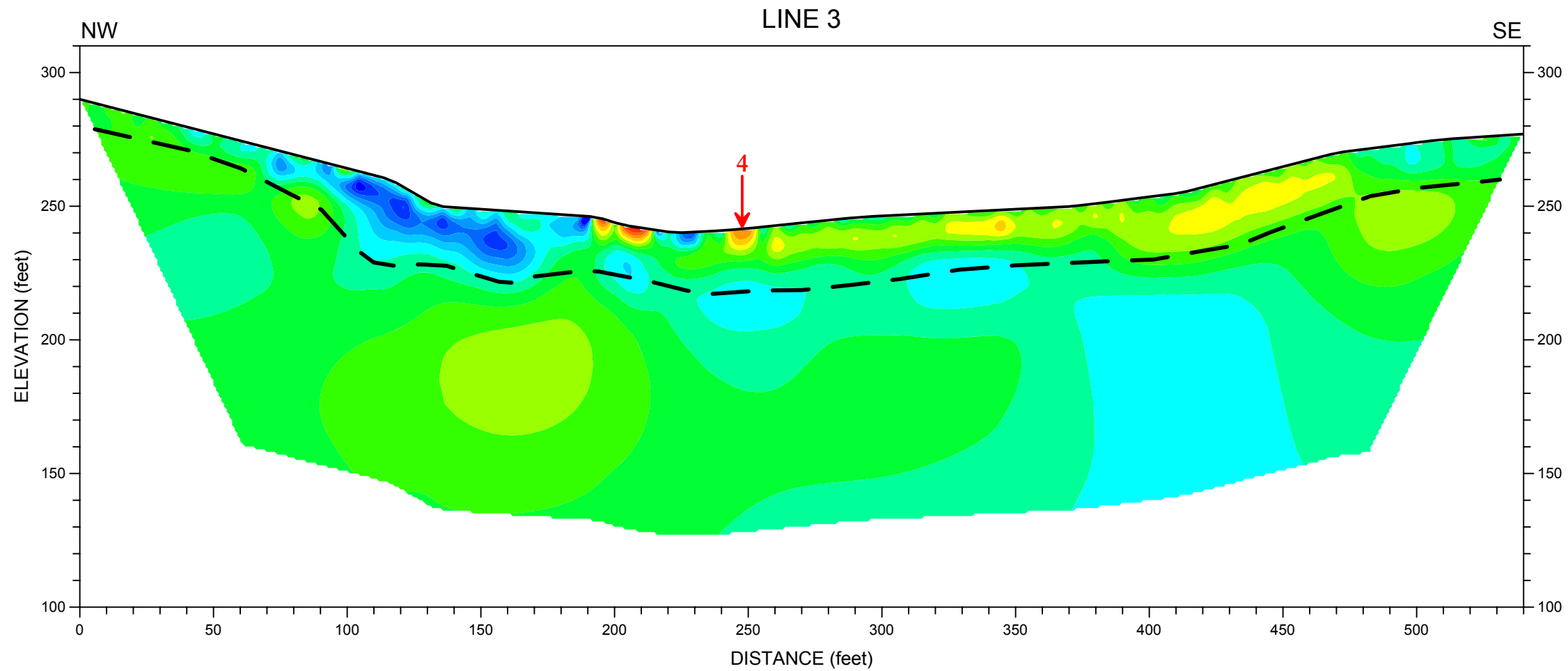
MAP REFERENCE: NORCAL, OCT. 2009, 09-325.55

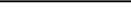

	SITE MAP GRADY RANCH—LUCAS VALLEY ROAD ELECTRICAL RESISTIVITY SURVEY		PLATE 1
	LOCATION: SAN RAFAEL, CALIFORNIA		
JOB #: 09-732.03	CLIENT: CSW		
DATE: JAN. 2010	NORCAL GEOPHYSICAL CONSULTANTS INC.	DRAWN BY: G.RANDALL	
		APPROVED BY: DJK	

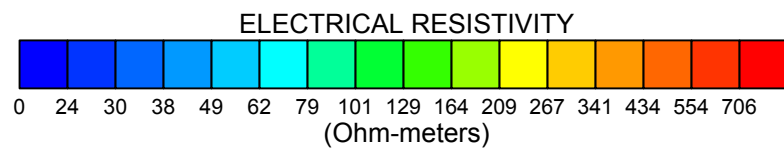



LEGEND	
	INTERPRETED BEDROCK INTERFACE BASED ON ELECTRICAL RESISTIVITY & PREVIOUS SEISMIC REFRACTION SURVEYS (2009) CONDUCTED BY NORCAL
	RECOMMENDED WELL LOCATION

	ELECTRICAL RESISTIVITY PROFILES		PLATE 2
	LINES 1 & 2		
	GRADY RANCH-LUCAS VALLEY ROAD		
	LOCATION: SAN RAFAEL, CALIFORNIA		
CLIENT: CSW			
JOB #: 09-732.03		NORCAL GEOPHYSICAL CONSULTANTS INC.	
DATE: JAN. 2010		DRAWN BY: G.RANDALL	APPROVED BY: DJK



LEGEND	
	INTERPRETED BEDROCK INTERFACE BASED ON ELECTRICAL RESISTIVITY & PREVIOUS SEISMIC REFRACTION SURVEYS (2009) CONDUCTED BY NORCAL
	RECOMMENDED WELL LOCATION



	ELECTRICAL RESISTIVITY PROFILE		PLATE 3
	LINE 3		
	GRADY RANCH-LUCAS VALLEY ROAD		
	LOCATION: SAN RAFAEL, CALIFORNIA		
JOB #: 09-732.03	NORCAL GEOPHYSICAL CONSULTANTS INC.		
DATE: JAN. 2010	DRAWN BY: G.RANDALL	APPROVED BY: DJK	

APPENDIX J

Bedrock well completion reports



**Forster
PUMP &
Engineering Inc.**
Water Systems • Wells • Tanks • Drainage

FAX TRANSMISSION COVER SHEET

This message is intended only for the use of the individual to which it is addressed and may contain information that is privileged, confidential and exempt from disclosure under applicable law. If the reader of this messages is not the intended recipient, or the employee or agent responsible for delivering the message to the intended recipient, you are hereby notified that any dissemination, distribution or copying of this communication is strictly prohibited. If you have received this communication in error, please notify us immediately by telephone and return the original message to us at the above address. Thank you.

DATE: Dec 1, 10

FAX NO.: 662-1903

PLEASE DELIVER THE FOLLOWING PAGES TO:

To: LOW BOUC

THIS FASCIMILE IS BEING SENT BY: Bob Lexas

NUMBER OF PAGES: 20 INCLUDING COVER SHEET)

Grandy Ranch Drillers logs and Flow Test Reports for well B.

IT looks like the only other well cased is RT Side F. I did not test this well yet. I'll send EST.

If you have any questions to verify regarding this fax please call the number below.

ORIGINAL			
<input type="checkbox"/> Not being sent			
<input type="checkbox"/> To follow via:	<input type="checkbox"/> Regular U.S. Mail	<input type="checkbox"/> Overnight Mail	<input type="checkbox"/> Federal Express

415 459-4770
56 Woodland Avenue, San Rafael, CA 94901-5344 * Lic. No. 426152
Fax 415 459-0607 * www.forsterpump.com

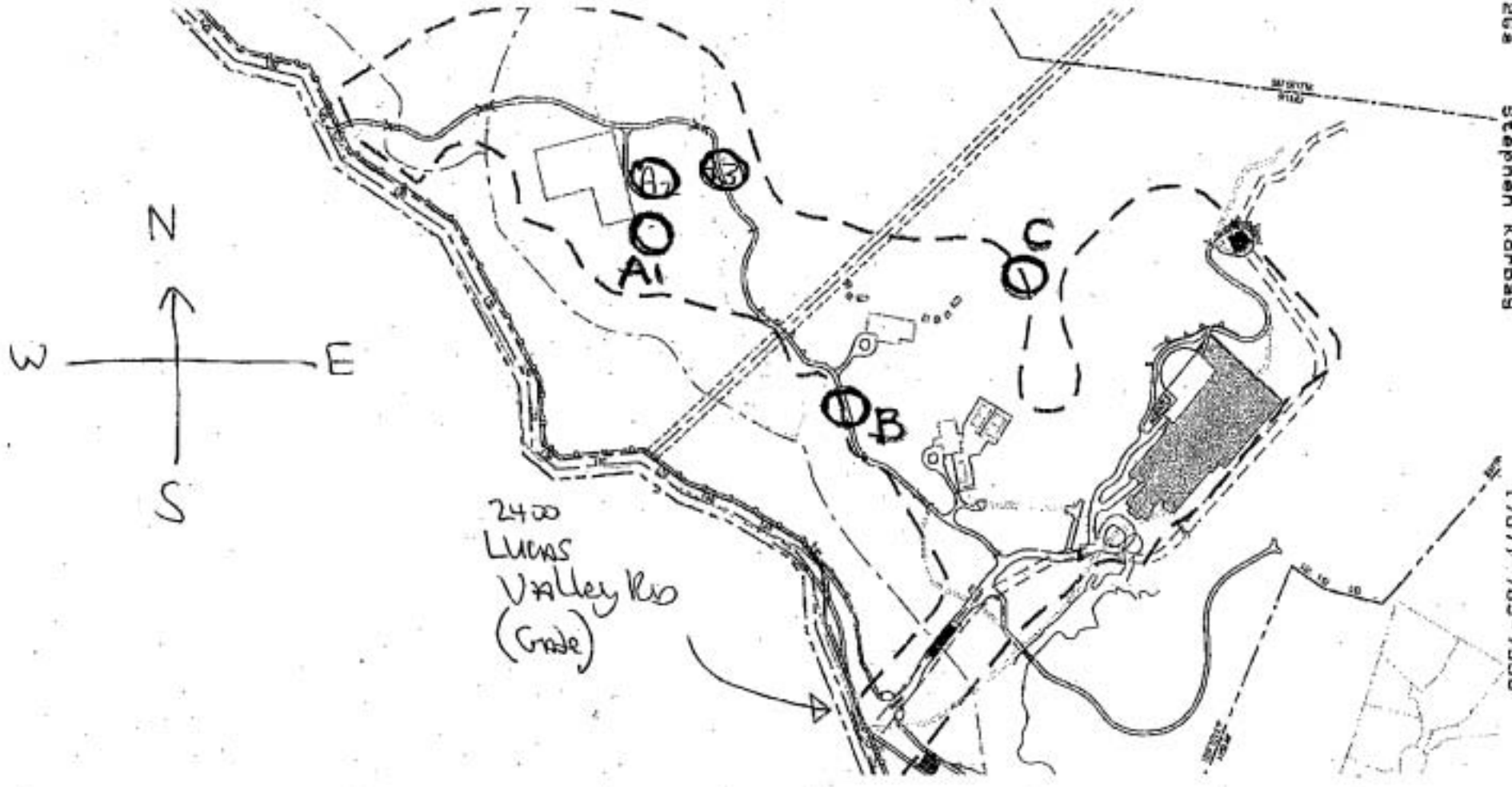
GRADY RANCH

06V 05 09 10:266

Stephen Korbar

(707) 763-7998

P-1



DUPLICATE
Driller's Copy

STATE OF CALIFORNIA
WELL COMPLETION REPORT

OWNER USE ONLY - DO NOT FILL IN

Page 1 of A-1

Owner's Well No. _____

No. **0946213**

Date Work Began 11/12/09

Ended 11/12/09

STATE WELL NO./STATION NO.	
LATITUDE	LONGITUDE
APPROXIMATES	

Local Permit Agency **MARIN COUNTY ENVIRONMENTAL HEALTH**

Permit No. 09/10-19A(1) Permit Date 11/09/09

GEOLOGIC LOG

ORIENTATION (°)		VERTICAL	HORIZONTAL	ANGLE	(SPECIFY)
DEPTH FROM SURFACE		DRILLING METHOD			
FE	TO	FLUID			
0	40	BROWN CLAY W/WEATHERED SANDSTONE			
40	60	BROWN & BLUE CLAY			
60	110	SANDSTONE, SHALE & CLAY			
110	200	SANDSTONE			
200	300	SANDSTONE, SHALE & CLAY			
TEST HOLE					

WELL OWNER

Name: **SKYWALKER PROPERTIES LTD**

Mailing Address: **P.O. BOX 10877**

SAL RAFAEL CA 94903

WELL LOCATION

Address: **2400 LUCAS VALLEY ROAD**

City: **MESASO CA 94946**

County: **MARIN COUNTY**

APN Book: **164** Page: **340** Parcel: **14**

Township: _____ Range: _____ Section: _____

East: _____ North: _____ West: _____ Long: _____

LOCATION SKETCH

NORTH

WEST

EAST

SOUTH

Describe or Identify (Distance of Well from Road, Building, Fence, etc) and sketch a map. (Use additional paper if necessary. PLEASE BE ACCURATE & COMPLETE)

AGREEMENT (✓)

New Well

Modification/Repair

Draper

Other (Specify)

Drilling/Service

Packout and Washout

Other (Specify)

DIAGNOSTIC (✓)

Water Sample

Corrosion

Pump

Rodded

Material

Nonflowing

Test Well

Cathodic Protection

Well Treatment

Direct Push

Slurrier

Water Extraction

Spacing

Remediation

Other (Specify)

TOTAL DEPTH OF BORING 300 (Feet)

TOTAL DEPTH OF COMPLETED WELL 300 (Feet)

WATER LEVEL & YIELD OF COMPLETED WELL

DEPTH TO FIRST WATER _____ (ft) BELOW SURFACE

DEPTH OF STATIC WATER LEVEL _____ (ft) & DATE MEASURED _____

ESTIMATED YIELD _____ (GPM) & YIELD TYPE _____

TEST LENGTH _____ (MIN) TOTAL DRAWDOWN _____ (ft)

* May not be representative of a well's long-term yield.

DEPTH FROM SURFACE	BORE-HOLE DIA. (Inches)	CASING (5)							
		TYPE (✓)				MATERIAL / GRADE	INTERNAL DIAMETER (Inches)	GAUGE OR WALL THICKNESS	SLOT SIZE (F AM) Gauge
FE	TO	BLANK	SCREEN	PIPE	PIPE				
0	20								
20	BOTTOM								

DEPTH FROM SURFACE	ANNULAR MATERIAL			
	TYPE	THICKNESS	PERCENT	OTHER
0 - 20	X BENTONITE			
20 - BOTTOM	1/8 XI/4			

ATTACHMENTS (✓)

Geologic Log

Well Construction Diagram

Geophysical Log(s)

Soil/Water Chemical Analysis

Other **MAP LOCATION OF WELL**

ATTACH ADDITIONAL INFORMATION (P. 1-2)

CERTIFICATION STATEMENT

I, the undersigned, certify that this report is complete and accurate to the best of my knowledge and belief.

FORSTER PUMP & ENGINEERING, INC.

NAME: _____ (PERSON, FIRM, OR CORPORATION) (PRINT OR PRINTED)

56 WOODLAND AVE SAN RAFAEL CA 94901

ADDRESS: _____ CITY: _____ STATE: _____ ZIP: **94915**

Signature: Robert Forster DATE SIGNED: 11/19/09 C-7 LICENSE NUMBER: 426152

C-7 LICENSE WHEN REC. CONTRACTOR: _____

DUPLICATE
Driller's Copy

STATE OF CALIFORNIA
WELL COMPLETION REPORT

Refer to Instruction Pamphlet

No. **0946215**

Page of **A-2**

Owner's Well No.

Date Work Began 11/13/09

Ended 11/13/09

Local Permit Agency **MARIN COUNTY ENVIRONMENTAL HEALTH**

Permit No. 09/10-19A (1)

Permit Date 11/09/09

DO NOT USE ONLY DO NOT FILL IN

STATE WELL NO./STATION NO.

LATITUDE

LONGITUDE

AMTRON/OWNER

ORIENTATION (±)			GEOLOGIC LOG	
			DRILLING METHOD	FLUID
DEPTH FROM SURFACE	FL	TO	DESCRIPTION	
FL	TO	FL	Describe material, grain size, color, etc.	
0	20		BROWN CLAY W/WEATHERED SANDSTONE	
20	50		SANDSTONE & SHALE	
50	100		SANDSTONE	
100	140		SHALE & CLAY	
140	160		SANDSTONE & SHALE	
160	280		SANDSTONE, SHALE & CLAY	
			TEST HOLE	
			MELANGE	

WELL OWNER

Name **SKYWALKER PROPERTIES LTD**

Mailing Address **P.O. BOX 10877**

City **SAL RAFAEL CA 94903**

Address **2400 LUCAS VALLEY ROAD**

City **NICASIO CA 94946**

County **MARIN COUNTY**

APN Book **164** Page **310** Parcel **11**

Township Range Section

Lat N Long W

LOCATION SKETCH

NORTH

WEST

EAST

See attached

ACTIVITY (±)

NEW WELL

MODIFICATION/REPAIR

Deepen

Other (specify)

OTHER JOB(S)

WATER SUPPLY

Domestic

Public

Irrigation

OTHER (specify)

SEARCHING

TEST WELL

CANNON PROTECTION

HEAT EXCHANGE

DIRECT PUSH

EXTRACTION

BACK EXTRACTION

REFILLING

REPLETION

OTHER (specify)

WATER LEVEL & YIELD OF COMPLETED WELL

DEPTH TO FIRST WATER (ft) BELOW SURFACE

DEPTH OF STAGE

WATER LEVEL (ft) & DATE MEASURED

ESTIMATED YIELD (GPM) & TEST TYPE

TEST LENGTH (hrs) TOTAL DRAWDOWN (ft)

* May not be representative of a well's long-term yield

DEPTH FROM SURFACE	BORE-HOLE DIA. (Inches)	CASING (S)				MATERIAL / GRADE	INTERNAL DIAMETER (Inches)	GAUGE OR WALL THICKNESS	SLOT SIZE IF ANY (Inches)
		TYPE (±)	BLANK	SCREEN	DOWN THE HOLE				
0	20								
20	BOTTOM								

DEPTH FROM SURFACE	ANNULAR SEALANT			
	OCY- MENT (±)	SPR- TONRE (±)	PSL (±)	WATER BACK (OVERSIZES)
0	20			X BENTONITE
20	BOTTOM			1/8 X 1/2

ATTACHMENTS (±)

Geologic Log

Well Construction Diagram

Geophysical Log(s)

Soil/Water Chemical Analysis

Other **MAP-LOCATION OF WELL**

ATTACH ADDITIONAL INFORMATION IF APPLICABLE

CERTIFICATION STATEMENT

I, the undersigned, certify that this report is complete and accurate to the best of my knowledge and belief

FORSTER PUMP & ENGINEERING, INC.

NAME (PERSON, FIRM, OR CORPORATION; TITLED OR INITIALED)

ADDRESS **56 WOODLAND AVE SAN RAFAEL CA 94901**

Signed DATE 11/19/09 STATE CA PERMIT NO. 426152

DO NOT WRITE IN THESE SPACES

DUPLICATE
Driller's Copy

STATE OF CALIFORNIA
WELL COMPLETION REPORT

Page 1 of A-3

Refer to Instructions Pamphlet

No. **0946214**

Owner's Well No. _____

Date Work Began 11/18/09

Ended 11/16/09

MARIN COUNTY ENVIRONMENTAL HEALTH

Local Permit Agency

Permit No. 09710-15A (1)

Permit Date 11/09/09

OWNER USE ONLY - DO NOT FILL IN

STATE WELL REGISTRATION NO. _____

LATITUDE _____ LONGITUDE _____

GEOLOGIC LOG

ORIENTATION (±)		VERTICAL	HORIZONTAL	ANGLE	(SPECIFY)
DEPTH FROM SURFACE		DESCRIPTION			
FL	TO FL	Describe material, grain size, color, etc.			
0	50	BROWN CLAY W/WEATHERED SANDSTONE			
50	200	SANDSTONE & SHALE			
200	300	SANDSTONE, SHALE & CLAY			
		MELANGE			
		TEST HOLE			

WELL OWNER

Name **SKYWALKER PROPERTIES LTD**

Mailing Address **P. O. BOX 10877**

City **SAN RAFAEL** CA **94903**

Address **2400 LUCAS VALLEY ROAD**

City **NICASIO** CA **94946**

County **MARIN COUNTY**

APN Book **184** Page **310** Parcel **11**

Township _____ Range _____ Section _____

Lat _____ N Long _____

LOCATION SKETCH

WEST EAST

See attached MAP

Illustrate or Describe Distance of Well from Roads, Railways, Paces, Rivers, etc. and attach a map. Use additional paper if necessary. **PLEASE BE ACCURATE & COMPLETE.**

WATER LEVEL & YIELD OF COMPLETED WELL

DEPTH TO FIRST WATER _____ (ft.) BELOW SURFACE

DEPTH OF STATIC WATER LEVEL _____ (ft.) & DATE MEASURED _____

ESTIMATED YIELD _____ (GPM) & TEST TYPE _____

TEST LENGTH _____ (hrs) TOTAL TRANSDUCER _____ (ft)

* May not be representative of a well longer than 100 ft.

DEPTH FROM SURFACE	BORE-HOLE DIA. (inches)	CASING (S)				
		TYPE (K)	MATERIAL / GRADE	INTERNAL DIAMETER (inches)	GAUGE OR WALL THICKNESS	SLOT SIZE IF ANY (inches)
FL	TO FL	BLANK				

DEPTH FROM SURFACE	ANNULAR SPACE (S)		
	TYPE	GR	PER
0	20	X BENTONITE	
20	BOTTOM	1/8" X 1/4"	

ATTACHMENTS (±)

— Geologic Log

— Well Construction Diagram

— Geophysical Log(s)

— Soil/Water Chemical Analyses

Other: MAP

ATTACH ADDITIONAL INFORMATION IF IT EXISTS

CERTIFICATION STATEMENT

I, the undersigned, certify that this report is complete and accurate to the best of my knowledge and belief.

FORSTER PUMP & ENGINEERING, INC.

NAME _____

ADDRESS **56 WOODLAND AVE** CITY **SAN RAFAEL** STATE **CA 94901**

PHONE **426-152**

DATE _____

DUPLICATE
Driller's Copy

STATE OF CALIFORNIA
WELL COMPLETION REPORT

OWNER USE ONLY - DO NOT FILL IN

STATE WELL NO./SPATIOID NO.

LATITUDE LONGITUDE

APN/PROVIDER

Page 1 of 8

Owner's Well No. _____

No. **0946211**

Date Work Began 11/11/09 Ended 11/11/09

Local Permit Agency **MARIN COUNTY ENVIRONMENTAL HEALTH**

Permit No. 09/10-19A (1) Permit Date 11/09/09

GEOLOGIC LOG

ORIENTATION (±)		DRILLING METHOD	FLUID	DESCRIPTION
<input checked="" type="checkbox"/> VERTICAL	<input type="checkbox"/> HORIZONTAL	<input type="checkbox"/> ANGLE	(SPECIFY)	Describe material, grain size, color, etc.
FL	TO	FL		
0	30			BROWN CLAY W/WEATHERED SANDSTONE
30	100			SANDSTONE
100	120			SHALE
120	240			SANDSTONE & SHALE
240	300			SANDSTONE
300	400			SANDSTONE & SHALE

WELL OWNER
SKYWALKER PROPERTIES LTD

Name _____
Mailing Address P. O. BOX 10877
SAN RAFAEL CA 94903
City State ZIP

Address 2400 LOCAS VALLEY ROAD
City NICASIO CA 94946
County MARIN COUNTY

APN Book 164 Page 310 Parcel 11
Township _____ Range _____ Section _____
Elev. _____ N. Long _____

LOCATION SKETCH

ACTIVITY (±)

NEW WELL

MODIFICATION/REPAIR
 Clean
 Other (Specify)

DESTROY (Describe Procedure And Method Under GEOLOGIC LOG)

TESTS (±)

WATER SUPPLY
 Gravity Pump
 Pressure Injection

MONITORING
 TEST WELL

CATHODIC PROTECTION
 HEAT EXCHANGE
 ELECTROLYSIS
 INJECTION
 VAPOR EXTRACTION
 AERATION
 BRACINATION
 OTHER (Specify)

WEST EAST

See attached MAP

Illustrate or Describe Distance of Well from Roads, Buildings, Fences, etc. and attach a map. Use additional paper if necessary. **PLEASE BE ACCURATE & COMPLETE.**

WATER LEVEL & YIELD OF COMPLETED WELL

DEPTH TO FIRST WATER _____ (FL) BELOW SURFACE

DEPTH OF STATIC WATER LEVEL 120 (FL) & DATE MEASURED 11/11/2009

ESTIMATED YIELD 5 (GPM) & TEST TYPE AIR

TEST LENGTH 2 (Hrs) TOTAL DRAWDOWN 380 (FL)

* May not be representative of a well's long-term yield.

DEPTH FROM SURFACE	BORE-HOLE DIA (Inches)	CASING (S)						DEPTH FROM SURFACE	ANNULAR SPACE FILL		
		TYPE (±)	MATERIAL / GRADE	INTERNAL DIAMETER (Inches)	GAUGE OR WALL THICKNESS	SLOT SIZE IF ANY (Inches)	FL		TO	FL	TYPE
0	80	10	X	PVC/F480	4 1/2		0	60	X	BENTONITE	
80	160	7"	X	PVC/F480	4 1/2		60	400		1/8 X 1/4	
160	320	7"	X	PVC	4 1/2	032"					
320	340	7"	X	PVC/F480	4 1/2						
340	400	7"	X	PVC	4 1/2	032"					

CAP ON BOTTOM

ATTACHMENTS (±)

Geologic Log

Well Construction Diagram

Geophysical Log(s)

Soil/Water Chemical Analysis

Other MAP

ATTACH ADDITIONAL INFORMATION, IF IT EXISTS.

CERTIFICATION STATEMENT

I, the undersigned, certify that this report is complete and accurate to the best of my knowledge and belief.

FORSTER PUMP & ENGINEERING, INC.

NAME (PERSON, FIRM, OR CORPORATION, PARTNERSHIP OR TRUST)
56 WOODLAND AVE SAN RAFAEL CA 94901

ADDRESS _____ CITY 11/19/09 STATE 426152

Signed Robert Forster DATE SIGNED _____ CAP LOG NUMBER _____

6-57 USE THIS WHEN WELL CONTRACTOR

DUPLICATE
Driller's Copy

STATE OF CALIFORNIA
WELL COMPLETION REPORT
Refer to Instruction Pamphlet

DWR USE ONLY - DO NOT FILL IN

STATE WELL REGISTRATION NO. _____

LATITUDE _____ LONGITUDE _____

APN/TRACKER _____

Page 1 of C
Owner's Well No. _____
Date Work Began 11/17/09 Ended 11/17/09
Local Permit Agency **MARIN COUNTY ENVIRONMENTAL HEALTH**
Permit No. 09/10-19A (1) Permit Date 11/09/09
No. **0946212**

ORIENTATION (±)			GEOLOGIC LOG	
X VERTICAL _____ HORIZONTAL _____ ANGLE _____ (SPECIFY)			Name _____	
DRILLING METHOD _____ FLUID _____			Mailing Address _____	
DESCRIPTION			City _____	
Describe material, grain size, color, etc.			State _____	
DEPTH FROM SURFACE	PL	IN	FT	DESCRIPTION
0			10	BROWN CLAY
10			30	SHALE
30			60	SANDSTONE & SHALE
60			140	SHALE & CLAY
140			190	SANDSTONE & SHALE
190			315	SHALE
315			340	SANDSTONE
340			400	SANDSTONE & SHALE

Name **SKYWALKER PROPERTIES LTD**
Mailing Address **P.O. BOX 10877**
City **SAN RAFAEL** State **CA** Zip **94903**
Address **2400 LUCAS VALLEY ROAD**
City **NICASIO** State **CA** Zip **94946**
County **MARIN COUNTY**
APN Book **154** Page **310** Parcel **11**
Township _____ Range _____ Section _____
East _____ Sec _____ N' _____ Long _____

LOCATION SKETCH NORTH

WEST SOUTH

Describe or Diagram Distance of Well from Road, Buildings, Fences, Lines, etc and attach a map. Use additional paper if necessary. PLEASE BE ACCURATE & COMPLETE.

ACTIVITY (X)
 NEW WELL
 MODIFICATION
 REPAIR
 OTHER (Specify) _____

PROTECTIVE DEVICES (Specify and Attach to Well Log if Necessary)

PIES (±)
WATER SUPPLY _____
Recharge _____
Infiltration _____

WATER LEVEL & YIELD OF COMPLETED WELL

DEPTH TO FIRST WATER _____ (Ft) BELOW SURFACE
DEPTH OF STABLE WATER LEVEL _____ (Ft) & DATE MEASURED _____
ESTIMATED YIELD _____ (GPM) & TEST TYPE _____
TEST LENGTH _____ (Ft) TOTAL DRAWDOWN _____ (Ft)
* May not be representative of a well's long-term yield.

WATER LEVEL & YIELD OF COMPLETED WELL

DEPTH TO FIRST WATER _____ (Ft) BELOW SURFACE
DEPTH OF STABLE WATER LEVEL _____ (Ft) & DATE MEASURED _____
ESTIMATED YIELD _____ (GPM) & TEST TYPE _____
TEST LENGTH _____ (Ft) TOTAL DRAWDOWN _____ (Ft)
* May not be representative of a well's long-term yield.

DEPTH FROM SURFACE	BORE HOLE DIA. (INCHES)	CASING (S)				
		TYPE (Ft)	MATERIAL / GRADE	INTERNAL DIAMETER (INCHES)	GAUGE OR WALL THICKNESS	SLOT SIZE IF ANY (INCHES)
0 - 20		BLANK				
20 - 400		SCREEN				

DEPTH FROM SURFACE	ANNULAR MATERIAL		
	TYPE	THICKNESS (INCHES)	SPACING (INCHES)
0 - 20	X BENTONITE		
20 - 400	1/8 X 1/4		

ATTACHMENTS (±)

____ Geologic Log
____ Well Construction Diagram
____ Geophysical Log(s)
____ Soil/Water Chemistry Analyses
 Other **MAP**

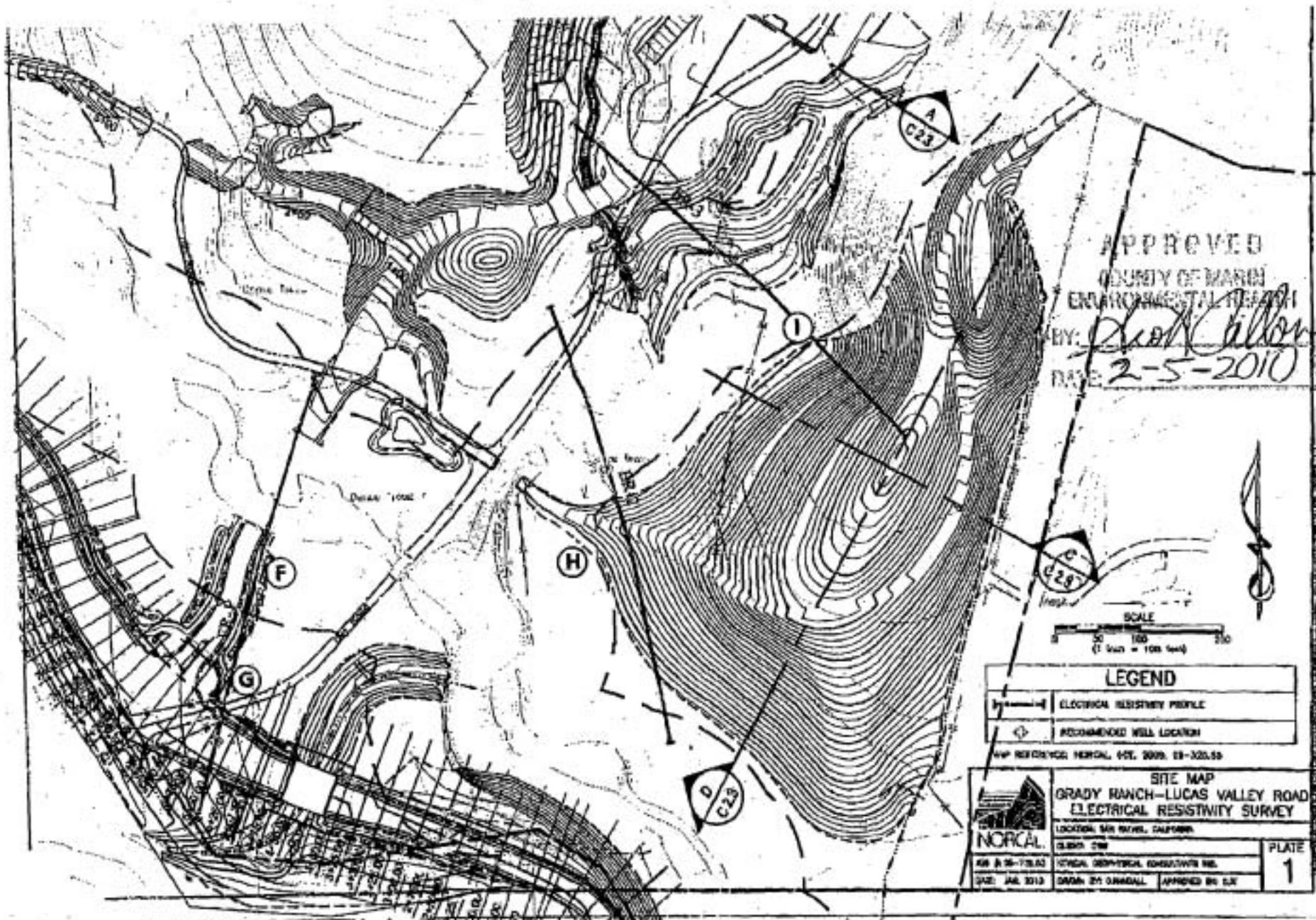
ATTACH ADDITIONAL INFORMATION, IF IT EXISTS.

CERTIFICATION STATEMENT

I, the undersigned, certify that this report is complete and accurate to the best of my knowledge and belief.

FORSTER PUMP & ENGINEERING, INC.
56 WOODLAND AVE SAN RAFAEL CA 94901
Signed Robert Forster Date 11/19/09
D-57 LICENSED WATER WELL CONTRACTOR

Grady Ranch Drill Sites - F, G, H, & I.



APPROVED
 COUNTY OF MARIN
 ENVIRONMENTAL HEALTH
 BY: *Don Cotton*
 DATE: 2-5-2010

LEGEND	
	ELECTRICAL RESISTIVITY PROFILE
	RECOMMENDED WELL LOCATION

MAP REVISIONS: NONE, 0-CE, 2009, 08-300, 09

 NORCAL 400 N 25th St SAN RAFAEL, CA 94901 TEL: 415-452-7342 FAX: 415-452-7343	SITE MAP GRADY RANCH - LUCAS VALLEY ROAD ELECTRICAL RESISTIVITY SURVEY LOCATION: SAN RAFAEL, CALIFORNIA CLIENT: CTR	PLATE 1
	DATE: JUL 2010 DRAWN BY: ORRISALL APPROVED BY: EJR	

DUPLICATE
Driller's Copy

STATE OF CALIFORNIA
WELL COMPLETION REPORT

Refer to Instruction Pamphlet

No. **0346216**

DWG USE ONLY - DO NOT FILL IN

STATE WELL NO./STATION NO.

LATITUDE LONGITUDE

APN/OTHER

Page of **Site F**

Owner's Well No.

Date Work Began 02/18/10, Ended 02/18/10

Local Permit Agency **MARIN COUNTY ENVIRONMENTAL HEALTH**

Permit No. 09/10-19A (2) Permit Date 02/05/10

GEOLOGIC LOG

ORIENTATION (±)		DRILLING METHOD	FLUID	DESCRIPTION
VERTICAL _____ HORIZONTAL _____ ANGLE _____ (SPECIFY)				
DEPTH FROM SURFACE		Describe material, grain size, color, etc.		
FL.	TO			
0	20	BROWN CLAY W/ GRAVELS		
20	40	WEATHERED SANDSTONE & BROWN CLAY		
40	60	SANDSTONE & SHALE		
60	80	SHALE & CLAY		
80	300	SANDSTONE & SHALE		

WELL OWNER
SKYWALKER PROPERTIES LTD

Name:
Mailing Address: P.O. BOX 10877
SAL RAFAEL CA 94903
City: State: CA ZIP:

Address: 2400 LUCAS VALLEY ROAD
City: NICASIO CA 94946
County: MARIN COUNTY

APN Book 164 Page 310 Parcel 11
Township Range Section
Lat. Long.

LOCATION SKETCH

Illustrate or Describe Distance of Well from roads, Buildings, Fences, Rivers, etc. and attach a map. Use additional paper if necessary. **PLEASE BE ACCURATE & COMPLETE.**

ACTIVITY (±)

NEW WELL

MODIFICATION/REPAIR

Deepen
 Other (Specify)

DESTROY (Describe Procedures and Materials Under "GEOLOGIC LOG")

USES (±)

WATER SUPPLY

Domestic Public
 Irrigation Industrial

MONITORING
 TEST WELL
 CATHODIC PROTECTION
 HEAT EXCHANGE
 DIRECT RUSH
 INJECTION
 VAPOR EXTRACTION
 SPARGING
 REMEDIATION
 OTHER (SPECIFY)

TOTAL DEPTH OF BORING 300 (Feet)
TOTAL DEPTH OF COMPLETED WELL 300 (Feet)

WATER LEVEL & YIELD OF COMPLETED WELL

DEPTH TO FIRST WATER (FL) BELOW SURFACE

DEPTH OF STATIC WATER LEVEL 40 (FL) & DATE MEASURED 02/18/2010

ESTIMATED YIELD 5 (GPM) & TEST TYPE AIR

TEST LENGTH 2 (Hrs.) TOTAL DRAWDOWN 280 (FL)

* May not be representative of a well's long-term yield.

DEPTH FROM SURFACE	BORE-HOLE DIA. (Inches)	CASING (S)				INTERNAL DIAMETER (Inches)	GAUGE OR WALL THICKNESS	SLOT SIZE IF ANY (Inches)
		TYPE (±)	MATERIAL / GRADE					
FL. TO	FL.	BLANK	SCREEN	CORR. SHEATH	FILL PIPE			
0	20	12	X			PVC/F480	8	
20	80	10"	X			PVC/F480	4 1/2	
80	300	7"	X			PVC	4 1/2	032'

DEPTH FROM SURFACE	ANNULAR MATERIAL			
	CEMENT (±)	BENTONITE (±)	FILL (±)	FILTER PACK (TYPE/SIZE)
FL. TO	FL.	FL.	FL.	
0	50			X BENTONITE
50	300			1/8 X 1/4

ATTACHMENTS (±)

Geologic Log
 Well Construction Diagram
 Geophysical Log(s)
 Soil/Water-Chemical Analyses
 Other SITE MAP

ATTACH ADDITIONAL INFORMATION, IF IT EXISTS.

CERTIFICATION STATEMENT

I, the undersigned, certify that this report is complete and accurate to the best of my knowledge and belief.

FORSTER PUMP & ENGINEERING, INC.

NAME: PERSON, FIRM, OR CORPORATION, (TYPE OR NUMBER)
56 WOODLAND AVE SAN RAFAEL CA 94901

ADDRESS: CITY: STATE: 426152

Signature: TITLE: C.S.T. LICENSE NUMBER:

DUPLICATE
Driller's Copy

STATE OF CALIFORNIA
WELL COMPLETION REPORT

Refer to Instruction Pamphlet

DWR USE ONLY - DO NOT FILL IN

STATE WELL NO./STATION NO.

LATITUDE LONGITUDE

APN/TRA/OTHER

Page 1 of 1 **SKG**
 Owner's Well No. SKG No. 0946217
 Date Work Began 02/10/10 Ended 02/10/10
 Local Permit Agency MARIN COUNTY ENVIRONMENTAL HEALTH
 Permit No. 09/10-19A (2) Permit Date 02/05/10

DEPTH FROM SURFACE		DESCRIPTION
FL.	TO FL.	
0	25	BROWN CLAY W/SOME GRAVELS
25	55	WEATHERED SANDSTONE W/ BROWN CLAY & SHALE
55	80	MELANGE
80	100	SANDSTONE & SHALE
100	190	SANDSTONE
190	230	SANDSTONE & SHALE
230	240	SANDSTONE
240	260	SHALE W/SANDSTONE

WELL OWNER
SKYWALKER PROPERTIES LTD
 Name: SKYWALKER PROPERTIES LTD
 Mailing Address: P. O. BOX 10877
 City: SAN RAFAEL CA 94903
 Address: 2400 LUCAS VALLEY ROAD
 City: NICASIO CA 94946
 County: MARIN COUNTY
 APN Book: _____ Page: _____ Parcel: _____
 Township: _____ Range: _____ Section: _____
 Lat: _____ Long: _____

LOCATION SKETCH
 NORTH
 SOUTH
 WEST
 EAST
see attached

ACTIVITY ()
 NEW WELL
 MODIFICATION/REPAIR
 Draper _____
 Other (Specify) _____
 DESTROY (Describe Procedure and Advise Under GEOLOGIC LOG)

USES ()
 WATER SUPPLY
 Domestic Public
 Irrigation Industrial
 MONITORING
 TEST WELL
 CATHODIC PROTECTION
 HEAT EXCHANGE
 DIRECT FUSH
 INJECTION
 VAPOR EXTRACTION
 SPARGING
 REMEDIATION
 OTHER (SPECIFY) _____

TOTAL DEPTH OF BORING 260 (Feet)
 TOTAL DEPTH OF COMPLETED WELL 260 (Feet)

WATER LEVEL & YIELD OF COMPLETED WELL
 DEPTH TO FIRST WATER _____ (FL) BELOW SURFACE
 DEPTH OF STATIC _____
 WATER LEVEL _____ (FL) & DATE MEASURED _____
 ESTIMATED YIELD * _____ (GPM) & TEST TYPE _____
 TEST LENGTH _____ (hrs) TOTAL DRAWDOWN _____ (FL)
 * May not be representative of a well's long-term yield.

DEPTH FROM SURFACE	BORE-HOLE DIA. (Inches)	TYPE ()				MATERIAL GRADE	INTERNAL DIAMETER (Inches)	GAUGE OR WALL THICKNESS	SLOT SIZE IF ANY (Inches)
		PIPE	SCREEN	COCK	OTHER				
0	20								
20	BOTTOM								

DEPTH FROM SURFACE	ANNULAR MATERIAL TYPE			
	CE-MENT ()	BEN-TONITE ()	FILL ()	FILTER PACK (TYPE/SIZE)
0		X		BENTONITE
20				1/4 X 1/8

ATTACHMENTS ()
 Geologic Log
 Well Construction Diagram
 Geophysical Log(s)
 Soil/Water Chemical Analysis
 Other Site Map
 ATTACH ADDITIONAL INFORMATION, IF IT EXISTS

CERTIFICATION STATEMENT
 I, the undersigned, certify that this report is complete and accurate to the best of my knowledge and belief.
FORSTER PUMP & ENGINEERING, INC.
 NAME: FORSTER PUMP & ENGINEERING, INC.
56 WOODLAND AVE SAN RAFAEL CA 94901
 ADDRESS: _____ CITY: _____ STATE: _____ ZIP: 426152
 Signed: _____ DATE SIGNED: 2/16/10 C-37 LICENSE NUMBER: _____
 I am a Licensed Water Well Contractor

DUPLICATE
Driller's Copy

STATE OF CALIFORNIA
WELL COMPLETION REPORT
Refer to Instruction Pamphlet

DO NOT FILL IN

STATE WELL NO./STATION NO.

LATITUDE LONGITUDE

APN/RS/OTHER

Page of **SITE H**

Owner's Well No. No. **0945115**

Date Work Began 08/06/10 Ended 08/06/10

Local Permit Agency **MARIN COUNTY ENVIRONMENTAL HEALTH**

Permit No. 09/10-19A(2) Permit Date 02/05/10

GEOLOGIC LOG

ORIENTATION (±)		VERTICAL	HORIZONTAL	ANGLE	(SPECFY)
DEPTH FROM SURFACE		DRILLING METHOD			
FL TO FL		FLUID			
		DESCRIPTION			
<i>Describe material, grain size, color, etc.</i>					
0	20	BROWN CLAY W/ GRAVELS			
20	30	BROWN CLAY			
30	50	SANDSTONE, SHALE & CLAY			
50	80	SHALE & CLAY			
80	300	SANDSTONE & SHALE			

WELL OWNER

Name: **SKYWALKER PROPERTIES LTD**

Mailing Address: **P. O. BOX 10877**

City: **SAN RAFAEL** State: **CA** Zip: **94903**

Address: **2400 LUCAS VALLEY ROAD**

City: **NICASIO** State: **CA** Zip: **94946**

County: **MARIN COUNTY**

APN Book Page Parcel

Township Range Section

Lat Long

LOCATION SKETCH

WEST EAST

SOUTH

Illustrate or Describe Distance of Well from Roads, Buildings, Fences, Rivers, etc. and attach a map. Use additional paper if necessary. PLEASE BE ACCURATE & COMPLETE.

ACTIVITY (±)

NEW WELL

MODIFICATION/REPAIR

Deepen

Other (Specify)

DESTROY (Describe Process and Materials Under "GEOLOGIC LOG")

USES (±)

WATER SUPPLY

Domestic Pools

Irrigation Industrial

MONITORING

TEST WELL

CATHODIC PROTECTION

HEAT EXCHANGE

DIRECT PUSH

INJECTION

VAPOR EXTRACTION

SPARGING

REMEDIATION

OTHER (SPECIFY)

WATER LEVEL & YIELD OF COMPLETED WELL

DEPTH TO FIRST WATER (FL) BELOW SURFACE

DEPTH OF STATIC WATER LEVEL (FL) & DATE MEASURED

ESTIMATED YIELD (GPM) & TEST TYPE

TEST LENGTH (MIN) TOTAL DRAWDOWN (FL)

** May not be representative of a well's long-term yield.*

DEPTH FROM SURFACE	BORE-HOLE DIA. (Inches)	CASING (S)					
		TYPE (±)		MATERIAL / GRADE	INTERNAL DIAMETER (Inches)	GAUGE OR WALL THICKNESS	SLOT SIZE IF ANY (Inches)
FL TO FL		BLANK	SCREEN				

DEPTH FROM SURFACE	ANNULAR MATERIAL			
	TYPE			
FL TO FL	CE- MENT (±)	BEN- TONITE (±)	FILL (±)	FILTER PACK (TYPE/SIZE)
0				X BENTONITE
				1/4 X 1/8
33				BOTTOM

ATTACHMENTS (±)

Geologic Log

Well Construction Diagram

Geophysical Log(s)

Soil/Water Chemical Analyses

Other

ATTACH ADDITIONAL INFORMATION, IF IT EXISTS.

CERTIFICATION STATEMENT

I, the undersigned, certify that this report is complete and accurate to the best of my knowledge and belief.

FORSTER PUMP & ENGINEERING, INC.

NAME PERSON, FIRM, OR CORPORATION (TYPE OF PRINTED)

Address 56 WOODLAND AVE City SAN RAFAEL State CA Zip 94901

Signed [Signature] Date 8/12/10 State License Number 426152

C-ST LICENSE WATER WELL CONTRACTOR

DUPLICATE
Driller's Copy

STATE OF CALIFORNIA
WELL COMPLETION REPORT

Refer to Instruction Pamphlet

No. **0945118**

Page of **SITE 1**

Owner's Well No.

Date Work Began 08/10/10 Ended 08/10/10

Local Permit Agency **MARIN COUNTY ENVIRONMENTAL HEALTH**

Permit No. 09/10-18A (2) Permit Date 02/05/10

DWR USER ONLY - DO NOT FILL IN

STATE WELL NO./STATION NO.

LATITUDE LONGITUDE

APN/TRACT/OTHER

ORIENTATION (±)		VERTICAL	HORIZONTAL	ANGLE	(SPECIFY)
DEPTH FROM SURFACE		FLUID			
Ft.	to	DESCRIPTION			
Describe material, grain size, color, etc.					
0	10	BROWN CLAY			
10	50	SANDSTONE			
50	100	SANDSTONE & SHALE			
100	120	SANDSTONE			
120	180	SANDSTONE & SHALE			
180	200	SANDSTONE			
200	260	SANDSTONE & SHALE			
260	300	SANDSTONE, SHALE & CLAY			
TOTAL DEPTH OF BORING <u>300</u> (Feet)					
TOTAL DEPTH OF COMPLETED WELL <u>300</u> (Feet)					

WELL OWNER

Name: **SKYWALKER PROPERTIES LTD**

Mailing Address: **P. O. BOX 10877**

City: **SAN RAFAEL** CA **94903** STATE **CA** ZIP **94903**

Address: **2400 LUCAS VALLEY ROAD**

City: **NICASIO** CA **94946**

County: **MARIN COUNTY**

APN Book Page Parcel

Township Range Section

Lot DES. MIN. SEC. N Long DES. MIN. SEC. W

LOCATION SKETCH

NORTH

SOUTH

WEST

EAST

Illustrate or Describe Distance of Well from Roads, Buildings, Fences, Rivers, etc. and attach a map. Use additional paper if necessary. **PLEASE BE ACCURATE & COMPLETE.**

ACTIVITY (±)

NEW WELL

MODIFICATION/REPAIR

Deepen

Other (Specify)

DESTROY (Describe Procedure and Materials Under 'GEOLOGIC LOG')

USES (±)

WATER SUPPLY

Domestic Public

Irrigation Industrial

MONITORING

TEST WELL

CATHODIC PROTECTION

HEAT EXCHANGE

DIRECT PUSH

INJECTION

VAPOR EXTRACTION

SPRINKLING

REMEDIATION

OTHER (SPECIFY)

WATER LEVEL & YIELD OF COMPLETED WELL

DEPTH TO FIRST WATER (Ft.) BELOW SURFACE

DEPTH OF STATIC WATER LEVEL (Ft.) & DATE MEASURED

ESTIMATED YIELD * (GPM) & TEST TYPE

TEST LENGTH (Hrs.) TOTAL DRAWDOWN (Ft.)

* May not be representative of a well's long-term yield.

DEPTH FROM SURFACE	BORE-HOLE DIA. (Inches)	CASING (S)				MATERIAL / GRADE	INTERNAL DIAMETER (Inches)	GAUGE OR WALL THICKNESS	SLOT SIZE IF ANY (Inches)
		TYPE (±)							
Ft.	to	BLANK	SOBER	COIN	DRILLER	FILL PIPE			
0	40								
40	300								

DEPTH FROM SURFACE	ANNULAR MATERIAL				
	TYPE				
Ft.	to	CE- MENT (±)	BEN- TONITE (±)	FILL (±)	FLTR PACK (TYPE/SIZE)
0	40		X		BENTONITE CHIPS
40	300			X	GRAVEL

ATTACHMENTS (±)

Geologic Log

Well Construction Diagram

Geophysical Log(s)

Soil/Water Chemical Analysis

Other

ATTACH ADDITIONAL INFORMATION, IF IT EXISTS.

CERTIFICATION STATEMENT

I, the undersigned, certify that this report is complete and accurate to the best of my knowledge and belief.

FORSTER PUMP & ENGINEERING, INC.

NAME (PERSON, FIRM, OR CORPORATION) (TYPE OR PRINT)

66 WOODLAND AVE SAN RAFAEL CA 94901

ADDRESS CITY STATE **426152**

Signed DATE SIGNED C-51 LICENSE NUMBER



Forster Pump & Engineering Inc.

Water Systems - Wells - Tanks - Drainage

Water Well Flow Test

Project FLOW TEST
 Location GRADY RANCH WELL #B
2400 LUCAS VALLEY
NICASIO, CA
 Owner SKYWALKER PROP

By BOB & SCOTT Date 11/16/09 Well Diameter 5" Casing PVC

Depth 400' Static Level 46' Pump: Type GOULDS SUBMERSIBLE

Model No. 10GS15412L GPM Rating 10 Depth 380' Voltage 230V

PG&E Power Generator X H. P. 1 1/2

<u>Date</u>	<u>Time</u>	<u>Static Level</u>	<u>Pumping Rate</u>	<u>Meter Reading</u>
11/16/09	11:00AM	46'	10 GPM	0166610
11/17/09	11:00AM	380'	1.4	0169420
11/18/09	10:30AM	380'	1.2	0170760
11/19/09	9:00AM	380'	1.2	0171840
11/24/09	1:30 PM	60'		

Total Test Period in Minutes 4,200 Total Gallons Pumped 5,230

GPM during Test Period 1.20 Gallons of Chlorine Used 1

Bacteria Sample to Lab YES Mineral Analysis to Lab YES

Additional comments

THIS REPORT IS FOR INFORMATIONAL PURPOSES ONLY. IT IS IN LIEU OF, AND SUPERSEDES ANY OTHER REPRESENTATIONS OR STATEMENTS OF THE AGENT OF EMPLOYEES OF THE COMPANY, AND ALL OTHER SUCH REPRESENTATIONS OR STATEMENTS SHALL BE RELIED UPON AT THE CUSTOMER'S OWN RISK.

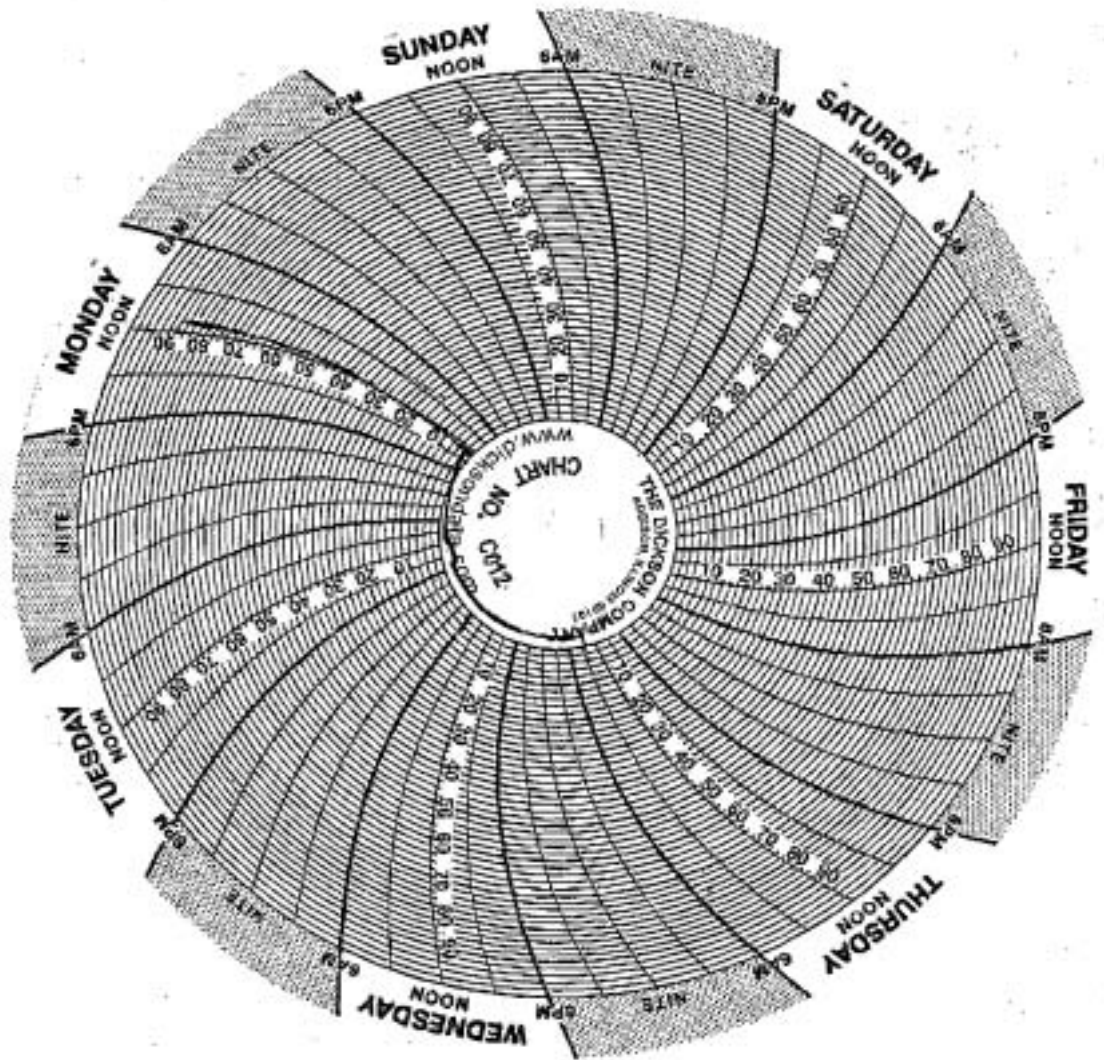
THE DATA AND CONCLUSIONS PROVIDED HEREIN ARE BASED UPON THE TESTS AND OBSERVATIONS OF THE COMPANY USING STANDARD AND ACCEPTED PRACTICES OF THE GROUNDWATER INDUSTRY. HOWEVER, CONDITIONS IN WATER WELLS ARE SUBJECT TO CONSIDERABLE ERROR DUE TO FACTORS WITHIN THE WELL AND GROUNDWATER FORMATION WHICH ARE BEYOND THE COMPANY'S IMMEDIATE CONTROL.

THEREFORE, THE DATA AND CONCLUSIONS ARE VALID ONLY AS OF THE DATE AND WITHIN THE LIMITATIONS OF THE TEST OR THE INSTALLATION INDICATED, AND SHOULD NOT BE RELIED UPON TO PREDICT EITHER THE FUTURE QUANTITY OR QUALITY OF THE WATER THAT THE WELL WILL PRODUCE. THE COMPANY MAKES NO WARRANTIES, EITHER EXPRESSED OR IMPLIED AS TO SUCH FUTURE WATER PRODUCTION. FURTHER, IT EXPRESSLY DISCLAIMS AND EXCLUDES LIABILITY FOR CONSEQUENTIAL OR INCIDENTAL DAMAGES ARISING OUT OF THE BREACH OF AN EXPRESS OR IMPLIED WARRANTY OF THE FUTURE WATER PRODUCTION, OR ARISING OUT OF ANY FURTHER USE OF THIS REPORT BY THE CUSTOMER OR THIRD PARTIES.

IN PRESENTING THE DATE AND CONCLUSIONS CONTAINED HEREIN, THE COMPANY, UNLESS EXPRESSLY STATED TO THE CONTRARY, DOES NOT REPRESENT THE FOLLOWING FACTORS. 1) THAT THE WELL OR PUMP SYSTEM IS IN ANY PARTICULAR STATE OF REPAIR. 2) THAT THE WATER PRODUCED WILL SATISFY ANY GOVERNMENTAL ORDINANCES OR REGULATIONS. 3) THAT THE WATER IS ADEQUATE FOR A PARTICULAR USE CONTEMPLATED BY THE CUSTOMER.

SIGNATURE ROBERT DATE November 20, 2009
 56 Woodland Ave. San Rafael, Ca. 94901-5344 • Lic. No. 426152
 FAX 459-0607 • www.forsterpump.com

Grady Ranch Well Site B
Flow Test Scale
11-19-09





Analytical Sciences

GRADY Ranch
Well # B
LAB Report

December 14, 2009

Bob Lerios
Forster Pump & Engineering, Inc.
56 Woodland Avenue
San Rafael, CA 94901-5344

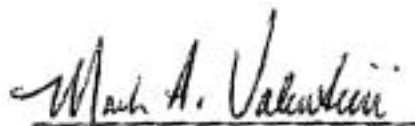
Dear Bob,

Enclosed you will find Analytical Sciences' final report 9111908 for your Skywalker Properties project. An invoice for this work is enclosed.

Should you or your client have any questions regarding this report please contact me at your convenience. We appreciate you selecting Analytical Sciences for this work and look forward to serving your analytical chemistry needs on projects in the future.

Sincerely,

Analytical Sciences



Michele Peters
Laboratory Manager



Report Date: December 14, 2009

Laboratory Report

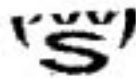
Bob Lerios
Forster Pump & Engineering, Inc.
56 Woodland Avenue
San Rafael, CA 94901-5344

Project Name: **Skywalker Properties**
Lab Project: **9111908**

**Grady Ranch - Well Site
B**

This 5 page report of analytical data has been reviewed and approved for release.

Michele Peters
Laboratory Manager



Total Coliform & E. Coli

Lab#	Sample ID	Compound Name	Result (MPN/100 mL)	RDL (MPN/100 mL)
9111908-01	Grady Ranch - Well B	Total Coliform	110	1
		E. Coli	<1 QT	1

Date Sampled:	11/19/09	Date Analyzed:	11/20/09	QC Batch:	B006674
Date Received:	11/19/09	Method:	SM 9223 B		

Graphite Furnace Metals

Lab#	Sample ID	Compound Name	Result (µg/L)	RDL (µg/L)
9111908-01	Grady Ranch - Well B	Antimony (Sb)	ND	5.0
		Arsenic (As)	ND	2.0
		Selenium (Se)	ND	5.0
		Thallium (Tl)	ND	2.0

Date Sampled:	11/19/09	Date Analyzed:	12/03/09	QC Batch:	B006684
Date Received:	11/19/09	Method:	EPA 200.9		

Drinking Water Metals (µg/L)

Lab#	Sample ID	Compound Name	Result (µg/L)	RDL (µg/L)
9111908-01	Grady Ranch - Well B	Aluminum (Al)	1700	50
		Barium (Ba)	68	50
		Beryllium (Be)	ND	1.0
		Cadmium (Cd)	ND	1.0
		Chromium (Cr)	5.6	2.5
		Iron (Fe)	2100	100
		Manganese (Mn)	31	20
		Nickel (Ni)	ND	10

Sampled:	11/19/09	Date Analyzed:	11/24/09	QC Batch:	B006637
Received:	11/19/09	Method:	EPA 200.7		

Drinking Water Metals (mg/L)

Lab#	Sample ID	Compound Name	Result (mg/L)	RDL (mg/L)
9111908-01	Grady Ranch - Well B	Sodium (Na)	210	5.0

Date Sampled:	11/19/09	Date Analyzed:	11/23/09	QC Batch: B006637
Date Received:	11/19/09	Method:	EPA 200.7	

Hardness

Lab#	Sample ID	Compound Name	Result (mg/L)	RDL (mg/L)
9111908-01	Grady Ranch - Well B	Calcium (Ca)	9.0	5.0
		Magnesium (Mg)	1.2	1.0
		Hardness	28	5.0

Date Sampled:	11/19/09	Date Analyzed:	11/23/09	QC Batch: B006637
Date Received:	11/19/09	Method:	SM 2340 B	

Mercury

Lab#	Sample ID	Compound Name	Result (µg/L)	RDL (µg/L)
9111908-01	Grady Ranch - Well B	Mercury (Hg)	ND	0.20

Date Sampled:	11/19/09	Date Analyzed:	11/25/09	QC Batch: B006675
Date Received:	11/19/09	Method:	EPA 245.1	

pH

Lab#	Sample ID	Compound Name	Result (pH Units)	RDL (pH Units)
11908-01	Grady Ranch - Well B	pH	8.22	1.00

Sampled:	11/19/09	Date Analyzed:	11/19/09	QC Batch: B006650
Received:	11/19/09	Method:	SM 4500-B B	



Alkalinity

Lab#	Sample ID	Compound Name	Result (mg CaCO3/L)	RDL (mg CaCO3/L)
9111908-01	Grady Ranch - Well B	Total Alkalinity	170	5.0
		Bicarbonate Alkalinity	170	5.0
		Carbonate Alkalinity	ND	5.0
		Hydroxide Alkalinity	ND	5.0

Date Sampled:	11/19/09	Date Analyzed:	11/30/09	QC Batch:	B006651
Date Received:	11/19/09	Method:	SM 2320 B		

Anions

Lab#	Sample ID	Compound Name	Result (mg/L)	RDL (mg/L)
9111908-01	Grady Ranch - Well B	Fluoride	2.2	0.10
		Nitrite as N	ND	0.15
		Nitrate	ND	0.50
		Nitrate as N	ND	0.15

Date Sampled:	11/19/09	Date Analyzed:	11/19/09	QC Batch:	B006673
Date Received:	11/19/09	Method:	EPA 300.0		

Free Cyanide

Lab#	Sample ID	Compound Name	Result (µg/L)	RDL (µg/L)
9111908-01	Grady Ranch - Well B	Cyanide (free)	160	50

Date Sampled:	11/19/09	Date Analyzed:	12/13/09	QC Batch:	B006766
Date Received:	11/19/09	Method:	SM 4500-CN F		

Notes and Definitions

- QT The bacterial test utilized is a quantitative test. A result of less than 1 (<1) is indicating bacteria are "absent" in 100 milliliters of sample water.
- RDL Reporting Detection Limit
- ND Analyte NOT DETECTED at or above the reporting detection limit (RDL)
- RPD Relative Percent Difference
- NR Not Reported

Please Note: California Department of Health Services recommended drinking water standards are as follows:

- Arsenic (10 ug/L)
- Iron (300 ug/L)
- Manganese (50 ug/L)
- Nitrate (45 mg/L)
- Lead (15 ug/L)
- Total Coliform (<1 MPN/100 mL)

APPENDIX K

**Annual Hydrologic Record and Sediment-Transport
Measurements for San Geronimo Creek at Lagunitas Road,
Marin County, California:
Data Report for Water Year 2010
(December 2010)**

**Annual Hydrologic Record and
Sediment-Transport Measurements for
San Geronimo Creek at Lagunitas Road,
Marin County, California:
Data Report for Water Year 2010**

Report prepared for:

Marin Municipal Water District

Prepared by:

Jonathan Owens

Barry Hecht

Balance Hydrologics, Inc.

December 2010

A report prepared for:

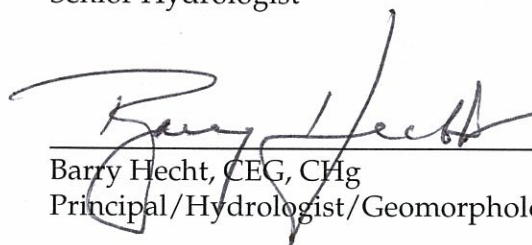
Marin Municipal Water District
220 Nellen Avenue
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Attention: Gregory Andrew
(415) 945-1191

**Annual Hydrologic Record and Sediment-Transport Measurements for San Geronimo Creek at Lagunitas Road, Marin County, California:
Data Report for Water Year 2010**

© 2010 Balance Hydrologics, Inc. Project Assignment: 8801
by



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December 21, 2010

TABLE OF CONTENTS

- 1. PROJECT PURPOSE AND INTRODUCTION.....1**
- 2. STATION DESCRIPTION.....3**
 - 2.1 SAN GERONIMO CREEK AT LAGUNITAS ROAD.....3
 - 2.2 COMPARISONS TO OTHER WATERSHEDS.....4
- 3. HYDROLOGIC SUMMARY.....5**
 - 3.1 WATER YEAR 2010.....5
 - 3.2 COMPARISON TO OTHER WATERSHEDS.....5
 - 3.3 YEAR-TO-YEAR COMPARISONS.....5
 - 3.4 RAINFALL.....6
- 4. DEVELOPING A STREAMFLOW RECORD7**
- 5. SEDIMENT TRANSPORT9**
 - 5.1 IMPORTANCE OF MEASURING SEDIMENT IN SAN GERONIMO CREEK.....9
 - 5.2 FIELD METHODS FOR SAMPLING SEDIMENT10
 - 5.3 DEVELOPING AND USING SEDIMENT-RATING CURVES10
 - 5.3.1 *Discussion of bedload sediment*.....11
 - 5.3.2 *Discussion of suspended sediment*.....12
 - 5.4 CREATING A CONTINUOUS RECORD OF SEDIMENT-DISCHARGE RATES12
 - 5.5 RECONNAISSANCE-LEVEL SEDIMENT-YIELD ESTIMATES.....12
- 6. FUTURE MONITORING14**
- 7. REFERENCES15**

ANNUAL SUMMARY FORMS

- Form 1. Annual Hydrologic Record: water year 2010
- Form 2. Annual Sediment-Discharge Record: water year 2010
- Form 3. Annual Rainfall Record: San Geronimo Creek at Lagunitas

LIST OF TABLES

- Table 1. Stream gaging observer log: San Geronimo Creek at Lagunitas Road, water year 2010
- Table 2. Sediment transport measurements: San Geronimo Creek at Lagunitas Road, water year 2010
- Table 3. Hydrologic summary for recent water years: San Geronimo Creek at Lagunitas Road

LIST OF FIGURES

- Figure 1. Daily flow hydrograph for San Geronimo Creek at Lagunitas Road, water year 2010
- Figure 2. Daily flow comparison: San Geronimo Creek, Walker Creek near Marshall, and Wildcat Creek in Richmond (Contra Costa County), water year 2010
- Figure 3. Flow comparison of water year 2010 to period of record: San Geronimo Creek at Lagunitas Road
- Figure 4. Daily stage hydrograph: San Geronimo Creek at Lagunitas Road, water year 2010
- Figure 5. Rainfall record: San Geronimo Creek at Lagunitas Road, water year 2010
- Figure 6. Sediment discharge and rating curves: San Geronimo Creek at Lagunitas Road, water year 2010
- Figure 7. Comparison of bedload-sediment discharge to previous years: San Geronimo Creek at Lagunitas Road
- Figure 8. Comparison of suspended-sediment discharge to previous years: San Geronimo Creek at Lagunitas Road

1. PROJECT PURPOSE AND INTRODUCTION

The Marin Municipal Water District (MMWD) has requested that Balance Hydrologics, Inc. (Balance) monitor streamflow and sediment transport in San Geronimo Creek at the Lagunitas Road bridge. This report summarizes stream flows in San Geronimo Creek during water year 2010¹ and is a continuation of hydrologic record keeping at this station that has been ongoing since November 1979. Results of stream gaging at this site are used by MMWD and its cooperators for diverse applications:

- Streamflow is the basic influence affecting aquatic habitat and sensitive-species protection for coho and steelhead, which are MMWD priorities, and those of the County, and have been so for many San Geronimo Valley residents since at least the 1977 general plan for the valley.
- Streamflow measurements and records for San Geronimo Creek can be used to estimate flows on other Lagunitas Creek tributaries, as well as on other ungaged streams in western Marin County and the region. This station has the longest period of record of any gage on unregulated streams in relatively non-urbanized portions of Marin County.
- On-line stream flow and rainfall information are frequently consulted by MMWD and others to track conditions on the stream, and by many residents 'simply to stay in touch with what's going on in the creek'.
- Bedload-sediment transport rates, especially at the moderate to high flows which move gravels and other bed material, are an important factor influencing the amount and quality of salmonid rearing habitat, as well as the availability, quality and mobility of spawning gravels for anadromous species – both in San Geronimo Creek and in Lagunitas Creek. Information collected at this gage is essential in understanding the variability of the accumulation and depletion of sand and gravel in Lagunitas Creek, to which San Geronimo Creek flows.
- Bed sediment stored, temporarily or long-term, in Lagunitas Creek below Kent Lake can affect the amount and quality of habitat available for the California freshwater shrimp (*Syncaris pacifica*) and other aquatic biota, in addition to salmonid habitat.

¹Most hydrologic and geomorphic monitoring occurs for a period defined as a water year, which begins on October 1 and ends on September 30 of the named year. For example, water year 2010 (WY2010) began on October 1, 2009, and concluded on September 30, 2010.

- Bedload-sediment transport rates, especially at low to moderate flows (which can only move smaller material like sand and fine gravel), are a good measure of sediment availability in the channel. These rates may also prove to be one measure of the effectiveness of erosion-control programs for the San Geronimo Creek watershed sponsored by Marin County, MMWD, the Marin County Resource Conservation District (MCRCD), and the Marin Open Space Conservation District (MCOSED), among others.

This data report summarizes our work at the gage on San Geronimo Creek during water year 2010. The report:

- Briefly describes where and what measurements and observations were made;
- Summarizes the results of these measurements;
- Reports daily streamflow in San Geronimo Creek during the study period;
- Compares daily and annual streamflow to those gaged during the prior 30 years;
- Records storm peaks affecting salmonid passage and bed-surface material in the Lagunitas Creek/San Geromino Creek system;
- Develops a preliminary estimate of bedload-sediment discharge for San Geronimo Creek during the water year; and
- Develops a preliminary estimate of suspended-sediment discharge for San Geronimo Creek during the water year.

Data collection by MMWD has been suspended at the close of water year 2010. On a very limited basis, Balance Hydrologics ('Balance') may continue to make measurements sustaining the stream gage and to conduct sediment-transport measurements.

2. STATION DESCRIPTION

2.1 San Geronimo Creek at Lagunitas Road

The gage is located in San Geronimo Creek at the Lagunitas Road bridge; this station has been designated as 'K4' since its inception in 1979. The watershed of 8.7 square miles receives long-term average precipitation of approximately 45 to 46 inches per year based on the USGS's isohyetal maps (Rantz, 1971). Balance staff have maintained an automatic creek-level recorder here and have been measuring water levels, streamflow, specific conductance, and bedload sediment since November 1979. The station was converted to continuous-electronic recording at the onset of water year 1998. A new electronic recorder and rain gage² were installed during March 2006; this new equipment includes real-time telemetry of the data to Balance's website³ and is now accessible both to MMWD staff and the general public.

During water year 2007, we switched to referencing a new staff plate at the same location installed on the northern side of the creek, affixed to the north bridge abutment. This staff is at a different datum, offset approximately 4.0 feet lower, such that a stage of 1.0 on the original staff plate corresponds to a stage of 5.0 on the new staff plate. The original 1979 datum is still intact and is cross referenced at high flows. The change addresses a gradual shift northward of the typical low-flow channel at this site.

Bedload transport was monitored intensively from late 1979 through 1982 (Hecht, 1983), and at reduced 'reconnaissance' frequencies during many subsequent years. Beginning in water year 2006, bedload measurements were made with increased frequency.

Suspended-sediment transport rates were measured from late 1979 through June 1982 and then resumed again beginning with water year 2005.

² As often occurs in forested areas, the rain gage is located near taller trees, and may slightly under-report rainfall for some storms. No claim of meeting USWB standards is made.

³ Real-time rainfall, stage, and streamflow data are available in near real time at <http://www.balancehydrologics.com/geronimo/creek/index.php>. Note the conditions posted on this site.

2.2 Comparisons to Other Watersheds

Additional comparisons for validation were made to Wildcat Creek in Contra Costa County, another gaging station operated by Balance about 20 miles to the east, and to Walker Creek near Marshall (USGS station #11460750), about 10 miles to the northwest.

We chose Wildcat Creek for comparison because its watershed size and relief are more similar to the San Geronimo Creek drainage than most other gaging stations that are closer to the San Geronimo Valley. This station is located at Vale Road on the Richmond/ San Pablo border, and has a drainage area of 7.8 square miles, compared to 8.7 square miles at the San Geronimo gage. Soils are, however, considerably more clay rich, the lower watershed is highly urbanized, and rainfall rates are lower within the Wildcat Creek watershed. Wildcat Creek streamflow is also slightly affected by Jewel Lake and Lake Anza, two small impoundments in the upper watershed. Real-time data for Wildcat Creek can presently be obtained on the same Balance website.

Walker Creek near Marshall, with a watershed area of 31.1 square miles, is the nearest operating gage with a long-term record to the Lagunitas watershed. It is substantially regulated by an upstream reservoir (Soulajule Reservoir on Arroyo Sausal) with flow-bypass requirements. Rainfall and geologic substrate in the Walker Creek watershed are more similar to those in the San Geronimo Creek watershed. Real-time data for the Walker Creek gage may be obtained on the USGS website through NWIS (waterdata.usgs.gov/nwis).

3. HYDROLOGIC SUMMARY

We collected a continuous record of water level for water year 2010, and converted this record to streamflow through a stage-discharge rating curve developed with periodic manual streamflow measurements at a range of stages. Daily flows are presented in Form 1 and Figure 1. A comparison to flow in the other creeks discussed above is shown in Figure 2. Measurements and observations from site visits are listed in Table 1.

3.1 Water Year 2010

The water year began with baseflow below normal in October, likely a result of the 3 previous dry years. An unusually large amount of rain fell on October 13 (approximately 4.9 inches), which substantially increased baseflow. Several small and moderate rains occurred in December 2009, then more substantial rain fell in a succession of days in mid January 2010. The peak flow for the year was approximately 640 cfs at 9:30 AM January 20, 2010. Occasional moderate rain continued through mid April and small rain happened periodically through the end of May. Flows during the spring flow recession were slightly higher than usual (Figure 3). Summer baseflow declined in a typical pattern, with September baseflow being close to average.

3.2 Comparison to Other Watersheds

Figure 2 shows the flow hydrograph of San Geronimo Creek plotted with Wildcat Creek and Walker Creek. We used Figure 2 to validate the San Geronimo Creek record in its response to storms and other perturbations; we concluded that the record was reasonable based on the timing and magnitude of the flow peaks, and the pattern of flow recession *after* flow peaks. In addition, we used Figure 2 in conjunction with Figure 5 to evaluate if there were any flow peaks that are not associated with rainfall, and are therefore most likely due to human influence; we did not identify any such peaks for San Geronimo Creek during this water year.

3.3 Year-to-Year Comparisons

Table 3 and Figure 3 show that streamflow on San Geronimo Creek during water year 2010 totaled at the long-term median. Although the instantaneous peak flow was quite

small, the total yearly flow was close to average. An unusually high percentage of rainfall occurred in October, April and May, when much of the rainfall does not contribute to runoff; hence, it is not surprising that rainfall was slightly above the long-term average yet runoff is slightly below average for this year.

3.4 Rainfall

Balance installed a tipping bucket rain gauge during March 2006. The gauge collects 15-minute and hourly data and posts the data to Balance's website. The recorded rainfall total for water year 2010 was 50.0 inches, after adjustments⁴. This is about 110 percent of the estimated long-term average precipitation for this location⁵ of approximately 45 to 46 inches per year (Rantz, 1971). Daily and cumulative rainfall data for water year 2010 are shown in Figure 5.

⁴ The rain gauge became clogged on October 13, 2009. We made adjustments to the data based on the amount of rainfall found in the funnel when it was cleaned, and from nearby rain gauges.

⁵ The estimated long-term average annual rainfall is taken from an isohyetal (rainfall contours) map for the location of our gaging station. S.E. Rantz (1971) of USGS produced a map of average annual rainfall for the entire San Francisco Bay Area that was based on a network of long-term rain gages for the years 1931 to 1970. For zones between rain gages, Rantz's isohyets were constructed based on expected patterns due to terrain elevations.

4. DEVELOPING A STREAMFLOW RECORD

The flow record starts with detailed field measurements; to calculate flow we measure depth and velocity at many verticals across a cross section of the creek (Rantz and others, 1982). Based on our periodic site visits, staff plate readings, and flow measurements (Table 1), we created an empirical stage-to-discharge relationship, also referred to as a stage-discharge “rating curve”. We then applied this rating curve to the datalogger and pressure-transducer record of water levels. The stage record is presented in Figure 4. During the monitoring period, as is typically done, we calibrate the stage record to observations of gage height; we then apply stage shifts to account for local scour and fill, and the effects of leaf dams during low flows. The last step is to apply the stage-discharge rating curve applicable to that date. We have directly measured flow up to 1,610 cfs; above that range we extend the rating curve to our estimate of peak flows (Shaw and others, 2007).

The upper end of the stage-discharge rating curve was established based on the peak flow of water year 2006. That peak flow of December 31, 2005 was estimated using standard ‘indirect methods’ protocols. We surveyed high-water marks left by the peak flow, as well as channel cross-sections, a longitudinal profile, and other measurements and observations⁶ required for indirect peak-discharge estimates. From this surveyed data we applied the slope-area method (Benson and Dalrymple, 1967) using our observations during this storm of flow, slope, and obstruction locations to calculate an estimate of the peak flow (3,940 cfs). We consider our estimate to be within about 10 percent of the peak flow (+/- 400 cfs). We then extended the rating curve to meet this point.

As with all other open-channel gaging of natural streams, some uncertainty remains (especially at high flows) in spite of efforts to be as precise as possible.

⁶ Our staff were at the site a few hours after the peak flow and made observations that no log jams or other transient obstructions to flow had occurred; these observations confirm the assumptions in the slope-area method used to calculate the peak flow.

Most of our results are presented as daily flow, which are averaged from data recorded and calculated every 15 minutes. Upon request, the more detailed 15-minute record can be made available for specific periods of interest.

5. SEDIMENT TRANSPORT

5.1 Importance of Measuring Sediment in San Geronimo Creek

Sediment transport and bed sedimentation are measured at San Geronimo Creek because they are potentially-significant factors in a broader effort to understand the sources and transport of sediment within the San Geronimo Creek and Lagunitas Creek watersheds. The data help in evaluating MMWD's steps to make water supply as compatible as possible with other watershed values and functions, and to interpret conditions in San Geronimo and Lagunitas Creeks. Beginning in water year 2005, we resumed measuring suspended-sediment concentrations and calculating suspended-sediment loads, in addition to the ongoing measurements and calculations of bedload sediment authorized in 2001.

We distinguish two types of sediment in transport: bedload sediment and suspended sediment. *Bedload sediment* is supported by the bed; it rolls and saltates along the bed, commonly within the lowermost 3 inches. Movement can be either continuous or intermittent, but is generally much slower than the mean velocity of the stream. In San Geronimo Creek, bedload consists primarily of coarse sands and gravels. *Suspended sediment* is supported by the turbulence of the water, and is transported at a rate approaching the mean velocity of flow.

For the purposes of this study, we measured and calculated values for bedload sediment, because excess bedload can be an especially impairing portion of the sediment load⁷ to salmonid habitat, bedload transport is closely related to the degree of bed sedimentation, and bedload has been the basis for managing sedimentation and bed conditions in the Lagunitas Creek watershed.

In San Geronimo Creek, as typically occurs elsewhere in unregulated streams, suspended sediment consists of fine sands, silts, and clays, and tends to be entrained at

⁷ Bedload in excess can fill pools used for rearing, can make spawning riffles more prone to scour, or can impede passage. Bedload also fills the undercut banks used by *Syncaris pacifica* (a federally-listed freshwater shrimp). Insufficient bedload can also create habitat-management issues.

lower flows than bedload. As a result, fine sediment may be deposited on top of the coarse sands and gravels used for fish spawning during the flow recession periods, or deposited in pools that are used for summer rearing.

5.2 Field Methods for Sampling Sediment

Sediment measurements are listed in Table 2 and plotted in Figures 6, 7, and 8. Standard methods and equipment reviewed by the Federal Interagency Sedimentation Project (FISP) were used to make measurements of sediment transport. Field measurements of sediment discharge are made either by hand samplers applied in transects across the channel at wadeable flows or with cable-suspended samplers from the bridge railing at high flows. We use Helley-Smith 3-inch bedload samplers, and DH-48, DH-81 and D-74 suspended-sediment samplers. Bedload- and suspended-sediment samples are taken at multiple verticals across the creek to collect a representative sample (Emmett, 1980; Edwards and Glysson, 1999, and older references cited therein). For bedload-sediment sampling we first establish the active-bed width by observation and/or preliminary sampling, then sample within that portion of the creek. For suspended-sediment sampling, we use two sampling methods depending on conditions; both methods are used and endorsed by the USGS to collect suspended-sediment samples that are representative of the mean sediment concentration of a stream. The two methods are the equal-discharge-increment method (EDI) and the equal-width-increment method (EWI) (Edwards and Glysson, 1999). With both methods we collect depth-integrated samples at multiple verticals across the creek.

Bedload samples are dried and weighed at Balance's office. Suspended-sediment samples are analyzed by Soil Control Lab in Watsonville, California, a state-certified laboratory.

5.3 Developing and using Sediment-rating Curves

The principal purpose of the sediment sampling is to develop an annual empirical relationship of the amount of sediment transported at a given flow. These "sediment-rating curves" (see Figures 6, 7 and 8) are the basis for calculating the volume of sediment transported by the creek past the gaging station for each 15-minute period and hence for each day (see Section 5.4).

The rating curves are also diagnostic of the processes of sediment movement through the stream system. As the position of the curve changes, a different relationship between streamflow and sediment transport is expressed, indicating limitations or increases in sediment supply (c.f., Hecht and Owens, 2006). Sediment transport at a given flow may change over short periods, such as during rising and receding hydrograph limbs, and will also generally change whenever watershed or channel conditions upstream make sediment more or less available for mobilization.

Distinguishing when changes affect the nature or position of the sediment-rating curve is a key decision to be made by experienced professionals who regularly observe the channel and maintain familiarity with the watershed; distinguishing when to shift or develop a new sediment-curve is an essential basis for valid calculation of sediment yield (c.f., Benson and Dalrymple, 1967; Hecht, 1983; Hecht and Owens, 2006).

We do not have measurements of sediment transport at very high flows (above 1,620 cfs), yet this is when much of the sediment may be transported in high-flow years. Unless data clearly indicate otherwise, to estimate sediment transport at high flows, we extend the sediment-rating curves using the same slopes as calculated from lower-flow data, a relationship empirically verified during prior years of sampling. Extension to higher flows at rating-curve slopes observed in prior years is a customary and widely used practice. Sediment-transport rates at high flows are therefore considered to be at the reconnaissance level, and are preliminary and subject to revision.

5.3.1 Discussion of bedload sediment

In Figures 6 and 7, the location of the plotted rating curves is an indicator of the mobility of bedload for the period that a curve represents. The lower on the graph that the rating curve plots, the lower bedload sediment transport (or more precisely, 'delivery') has been, at a given flow.

Bedload-transport measurements from water years 1995 through 2010 are plotted in Figure 7. The bedload-sediment rating curve shown for water year 2010 seems to represent a similar rate of transport at a given flow when compared with those previous data. Slight adjustments were made to the curve for water year 2010. This comparison is a major purpose of the bedload measurement program; we are finding that the

position of the rating curve may be a useful indication of bed conditions in San Geronimo and portions of Lagunitas Creeks.

At lower flows (less than 100 cfs), sediment-transport rates seem similar to those observed last year and within the mid-range of previous data, but remain well above the levels observed between 1996 and 2002 (Hecht and others, 2009).

5.3.2 Discussion of suspended sediment

Suspended-sediment measurements were carried out during water year 2010. The measurements are detailed in Table 2 and plotted in Figures 6 and 8. Suspended-sediment data collected during 2010 were at similar concentrations at a given flow when compared to previous years. Figure 6 shows that suspended-sediment discharge is generally greater than bedload discharge, a typical condition for San Geronimo Creek. Figure 6 also shows the expected relationships at low flows (less than 40 cfs); we have observed sediment in suspension (visual turbidity) when bedload is not moving.

5.4 Creating a Continuous Record of Sediment-discharge Rates

Because we have represented our sediment measurements as a function of flow (Figures 6, 7, and 8), we then can use the continuous flow record (15-minute intervals) in conjunction with the sediment-rating curves to create continuous records of sediment discharge. The sediment-rating curves shown in Figure 6 are the “sediment-discharge” functions that we apply to the continuous flow record. This continuous record of sediment discharge is vastly simplified from the many individual events, processes, and occurrences that influence the actual discharge of sediment, but experience has shown it to be a useful and reasonably accurate approximation of this complex reality (c.f., Edwards and Glysson, 1999; Emmett, 1980). This record can serve as a useful tool for year-to-year comparisons. The interpolation from our manual measurements to a continuous record allows us to calculate daily and annual estimates of sediment yield (Form 2 and Table 3).

5.5 Reconnaissance-level Sediment-Yield Estimates

One of our purposes in sampling sediment at this site is to compare sediment-discharge rates as a function of flow in the stream and to detect long-term trends, as in Figures 6

through 8. Nonetheless, we have developed preliminary estimates of sediment discharge from San Geronimo Creek to have been approximately 270 tons of bedload and 920 tons of suspended sediment in water year 2010 (Form 2 and Table 3). Because the number of samples can limit the precision of the results, we developed this pair of reconnaissance-level estimates solely to compare sediment yields under present watershed conditions with the annual average sediment yield computed for conditions prevailing during previous years at this station, using either similar reconnaissance-, or more precise, full-scale methods.

The annual sediment totals that we calculate depend on three main factors: 1) the magnitude of the peak flow of the water year, 2) the amount of total streamflow during the water year, and 3) the relative position of the sediment-rating curves (high vs. low for a given flow). During water year 2010, the peak flow was low, the total flow was average, and the rating curves were in the middle range of previous data.

We emphasize that the sediment data and totals are approximate, due to the irregular and supply-driven nature of sediment discharge in small coastal streams. It is not intended that the yearly estimates of bedload discharge presented above substitute for a full-scale bedload discharge-rate investigation (such as we conducted during the early 1980's) when nearly every storm is to be sampled. Preparation of long-term reconnaissance sediment yield estimates is of necessity done with caution and with the detailed knowledge associated with individual samplings and storm periods. Our recent work has shown that the sediment-rating curves can be a useful, sensitive, and early indicator of bed conditions during the subsequent summer in Lagunitas Creek, immediately downstream (Hecht and others, 2008; 2009).

6. FUTURE MONITORING

MMWD has decided to suspend stream gaging and sediment-transport monitoring on San Geronimo Creek for water year 2011. Balance may perform just enough field visits to maintain the equipment at the site, and collect basic data from which skeletal records might be able to be developed at a later date should monitoring resume in the near future.

Please contact us if you can contribute any observations or measurements, have data that can guide revisions, or have questions concerning this work.

7. REFERENCES

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FORMS

Water Year: **2010**
 Stream: **San Geronimo Creek**
 Station: **at Lagunitas Road bridge (K4)**
 County, State: **Marin County, California**

Form 1. Annual Hydrologic Record

Station Location / Watershed Descriptors

Latitude: 36 00' 40", Longitude: 122 42' 02" in Rancho San Geronimo at Lagunitas. The gage is at Lagunitas Road bridge. Land use includes open space, golf course, pasture, and low- to medium-density residential uses in valleys. Drainage area upstream of gage is 8.7 sq. miles.

Mean Daily Flow

Mean daily flow (MDQ) for WY 2009 is 6.7 cfs; MDQ WY'08 = 10.8 cfs; MDQ WY'07 = 6.4 cfs. Mean annual flow (MAQ) is 15.4 cfs (based on 28 years of record; WYs 1980 to 2009).

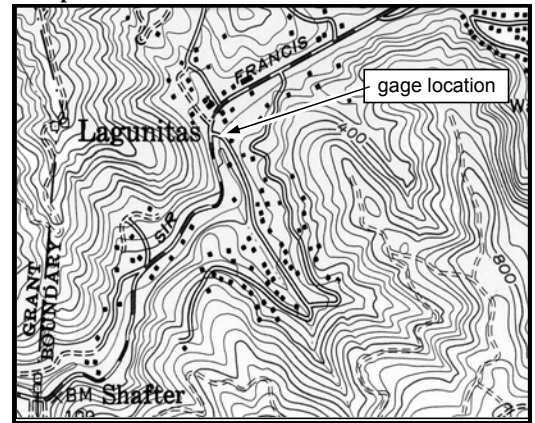
Peak Flows

Date	Time	Peak Stage (feet)	Discharge (cfs)	Date	Time	Peak Stage (feet)	Discharge (cfs)
10/13/09	16:15	6.80	225	2/6/10	5:45	6.08	166
1/18/10	12:15	8.39	481	2/24/10	0:30	7.48	349
1/19/10	7:15	7.85	389	2/26/10	15:30	6.41	201
1/20/10	9:30	9.01	640	3/3/10	5:30	6.41	202
1/21/10	17:00	7.0	287	4/4/10	18:00	7.28	317
1/25/10	19:15	6.52	220	4/11/10	15:15	7.54	356

Peak flow at this station for the period of gaging record was 3,940 +/- 400 cfs, 12/31/2005.

Another high flow at this station for the period of gaging record was 3,800 +/- 300 cfs, 1/4/1982.

Map



Period of Record

Staff plate and crest gage installed 11/17/79. Electronic water-level recorders were installed 10/22/97 (left bank), then 3/20/06 (right bank). Monitoring sponsored by Marin Municipal Water District.

WY 2010 Mean Daily Flow in cubic feet per second (cfs)

DAY	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEPT	
1	0.04	0.60	0.82	3.31	18.21	26.55	15.40	9.38	2.98	1.01	1.05	0.24	
2	0.03	0.58	0.81	3.17	15.94	65.45	29.09	8.53	2.77	0.98	1.02	0.22	
3	0.03	0.60	0.81	2.82	14.41	108.09	25.38	7.70	2.67	0.92	1.02	0.19	
4	0.04	0.65	0.81	2.50	53.41	57.10	85.09	7.32	2.64	0.86	0.99	0.16	
5	0.04	0.82	0.82	2.24	74.89	35.64	63.62	6.36	2.51	0.83	0.89	0.17	
6	0.03	0.98	0.90	2.04	71.94	26.50	30.36	6.20	2.41	0.88	0.82	0.17	
7	0.03	0.94	1.43	1.88	39.56	20.84	22.07	5.80	2.39	0.89	0.73	0.15	
8	0.03	0.95	0.94	1.78	28.50	17.14	17.61	5.48	2.33	0.89	0.68	0.17	
9	0.03	0.91	0.82	1.72	34.76	14.78	14.75	5.37	2.25	0.89	0.66	0.20	
10	0.03	0.88	0.85	1.63	25.74	16.05	12.96	5.78	2.15	0.86	0.59	0.19	
11	0.03	0.89	1.70	1.55	21.69	12.77	141.45	5.31	2.07	0.84	0.58	0.18	
12	0.03	0.86	8.59	19.96	23.21	32.73	126.80	4.90	1.96	0.85	0.58	0.18	
13	52.93	0.82	39.00	24.28	21.08	27.45	47.97	4.55	1.84	0.87	0.56	0.19	
14	6.38	0.78	11.34	11.86	18.48	20.32	29.94	4.28	1.75	0.82	0.54	0.18	
15	1.66	0.72	5.94	8.11	16.43	17.06	22.66	4.07	1.72	0.77	0.52	0.18	
16	0.96	0.69	9.36	7.18	14.59	14.75	18.16	3.97	1.67	0.76	0.51	0.18	
17	0.73	0.71	6.99	17.00	13.18	13.22	15.21	4.76	1.59	0.76	0.49	0.20	
18	0.68	0.73	5.03	169.52	12.07	12.01	13.33	4.78	1.54	0.78	0.47	0.21	
19	16.36	0.69	3.92	198.74	11.18	10.67	11.93	4.08	1.53	0.77	0.46	0.22	
20	7.74	2.00	3.21	245.39	10.33	9.68	18.32	3.77	1.50	0.85	0.47	0.21	
21	3.02	1.19	3.50	201.48	10.13	8.96	13.00	3.61	1.42	0.89	0.45	0.20	
22	1.80	0.87	3.25	122.11	9.62	8.46	11.41	3.44	1.32	0.93	0.42	0.19	
23	1.31	0.80	2.66	63.64	52.34	7.94	9.98	3.29	1.27	0.95	0.38	0.20	
24	1.06	0.79	2.34	38.18	152.36	7.59	9.06	3.14	1.26	0.94	0.35	0.19	
25	0.91	0.82	2.09	117.14	42.01	7.82	8.39	3.75	1.29	0.95	0.31	0.17	
26	0.81	0.80	2.88	116.42	65.93	7.01	7.82	4.12	1.18	0.99	0.30	0.17	
27	0.83	0.71	4.36	54.27	60.06	6.51	19.08	7.33	1.07	1.09	0.30	0.18	
28	0.77	0.77	3.34	33.85	36.34	6.23	14.59	5.02	1.02	1.04	0.28	0.15	
29	0.71	0.80	3.10	28.35		6.88	11.85	3.95	0.98	1.00	0.28	0.14	
30	0.66	0.82	4.92	26.45		8.84	10.36	3.47	0.98	1.01	0.25	0.13	
31	0.63		3.74	21.17		14.53		3.16		1.04	0.27		
Monthly	MEAN	3.24	0.84	4.53	49.99	34.58	20.96	29.25	5.05	1.80	0.90	0.55	0.18
	MAX	52.93	2.00	39.00	245.39	152.36	108.09	141.45	9.38	2.98	1.09	1.05	0.24
	MIN	0.03	0.58	0.81	1.55	9.62	6.23	3.14	0.98	0.76	0.25	0.13	
	cfs days	100.3	25.2	140.3	1549.8	968.4	649.6	877.6	156.7	54.1	27.9	17.2	5.5
	ac-ft	199	50	278	3074	1921	1288	1741	311	107	55	34	11

Monitor's Comments

- Data collection was continuous for the entire water year.
- Starting in water year 2007 the stage datum references the staff plate on the north side of the creek. The new, north-bank staff plate reads about 4.0 feet higher than the old staff plate.
- Multiple stage shifts were applied to account for scour and fill, as well as leaf dams.
- Peak flows are based on the 15-minute electronic record.
- Values with more than 2 or 3 significant figures are the result of calculations; no additional precision is implied.

Water Year 2010

Mean Daily Flow	12.5	(cfs)
Max. Daily Flow	245	(cfs)
Min. Daily Flow	0.03	(cfs)
Total Flow	4,573	(cfs-days)
Total Flow	9,070	(ac-ft)

Balance Hydrologics, Inc., 800 Bancroft Way, Suite 101, Berkeley, CA 94710 phone:(510) 704-1000 fax:(510) 704-1001

www.balancehydro.com

Water Year: 2010

Stream: San Geronimo Creek
 Station: Lagunitas Road bridge (K4)
 County: Marin County, CA

Form 2. Annual Sediment-Discharge Record

Daily Suspended-Sediment Discharge (tons)

DAY	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEPT	
1	0.0	0.0	0.0	0.0	0.2	0.5	0.1	0.0	0.0	0.0	0.0	0.0	
2	0.0	0.0	0.0	0.0	0.1	6.8	1.4	0.0	0.0	0.0	0.0	0.0	
3	0.0	0.0	0.0	0.0	0.1	19.7	0.5	0.0	0.0	0.0	0.0	0.0	
4	0.0	0.0	0.0	0	14.9	3.4	32.1	0.0	0.0	0.0	0.0	0.0	
5	0.0	0.0	0.0	0.0	8.2	1.0	5.2	0.0	0.0	0.0	0.0	0.0	
6	0.0	0.0	0.0	0.0	7.4	0.5	0.7	0.0	0.0	0.0	0.0	0.0	
7	0.0	0.0	0.0	0.0	1.3	0.3	0.3	0.0	0.0	0.0	0.0	0.0	
8	0.0	0.0	0.0	0.0	0.6	0.2	0.2	0.0	0.0	0.0	0.0	0.0	
9	0.0	0.0	0.0	0.0	1.0	0.1	0.1	0.0	0.0	0.0	0.0	0.0	
10	0.0	0.0	0.0	0.0	0.4	0.1	0.1	0.0	0.0	0.0	0.0	0.0	
11	0.0	0.0	0.0	0.0	0.3	0.1	83.2	0.0	0.0	0.0	0.0	0.0	
12	0.0	0.0	0.0	0.3	0.3	1.4	28.9	0.0	0.0	0.0	0.0	0.0	
13	11.2	0.0	2.0	0.4	0.3	0.5	2.2	0.0	0.0	0.0	0.0	0.0	
14	0.0	0.0	0.1	0.1	0.2	0.2	0.7	0.0	0.0	0.0	0.0	0.0	
15	0.0	0.0	0.0	0.0	0.1	0.2	0.3	0.0	0.0	0.0	0.0	0.0	
16	0.0	0.0	0.0	0.0	0.1	0.1	0.2	0.0	0.0	0.0	0.0	0.0	
17	0.0	0.0	0.0	0.3	0.1	0.1	0.1	0.0	0.0	0.0	0.0	0.0	
18	0.0	0.0	0.0	93.2	0.1	0.1	0.1	0.0	0.0	0.0	0.0	0.0	
19	0.7	0.0	0.0	105.4	0.1	0.0	0.1	0.0	0.0	0.0	0.0	0.0	
20	0.0	0.0	0.0	207.4	0.0	0.0	0.2	0.0	0.0	0.0	0.0	0.0	
21	0.0	0.0	0.0	80.1	0.0	0.0	0.1	0.0	0.0	0.0	0.0	0.0	
22	0.0	0.0	0.0	23.0	0.0	0.0	0.1	0.0	0.0	0.0	0.0	0.0	
23	0.0	0.0	0.0	4.5	23.3	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
24	0.0	0.0	0.0	1.2	64.8	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
25	0.0	0.0	0.0	28	1.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
26	0.0	0.0	0.0	21.1	10.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
27	0.0	0.0	0.0	3.0	3.8	0.0	0.3	0.0	0.0	0.0	0.0	0.0	
28	0.0	0.0	0.0	0.9	1.1	0.0	0.1	0.0	0.0	0.0	0.0	0.0	
29	0.0	0.0	0.0	0.6		0.0	0.1	0.0	0.0	0.0	0.0	0.0	
30	0.0	0.0	0.0	0.5		0.0	0.0	0.0	0.0	0.0	0.0	0.0	
31	0.0		0	0.3		0.2		0.0		0.0			Annual
TOTAL	12	0	2	570	141	36	157	0	0	0	0	0	918
Max.day	11	0	2	207	65	20	83	0	0	0	0	0	207

Daily Bedload-Sediment Discharge (tons)

DAY	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEPT	
1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
2	0.0	0.0	0.0	0.0	0.0	2.1	0.2	0.0	0.0	0.0	0.0	0.0	
3	0.0	0.0	0.0	0.0	0.0	7.2	0.0	0.0	0.0	0.0	0.0	0.0	
4	0.0	0.0	0.0	0.0	4.6	0.5	9.7	0.0	0.0	0.0	0.0	0.0	
5	0.0	0.0	0.0	0.0	2.5	0.0	1.5	0.0	0.0	0.0	0.0	0.0	
6	0.0	0.0	0.0	0.0	2.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
7	0.0	0.0	0.0	0.0	0.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
8	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
9	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
10	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
11	0.0	0.0	0.0	0.0	0.0	0.0	24.6	0.0	0.0	0.0	0.0	0.0	
12	0.0	0.0	0.0	0.0	0.0	0.2	9.5	0.0	0.0	0.0	0.0	0.0	
13	3.4	0.0	0.4	0.0	0.0	0.0	0.2	0.0	0.0	0.0	0.0	0.0	
14	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
15	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
16	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
17	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
18	0.0	0.0	0.0	27.4	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
19	0.1	0.0	0.0	31.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
20	0.0	0.0	0.0	58.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
21	0.0	0.0	0.0	24.8	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
22	0.0	0.0	0.0	7.9	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
23	0.0	0.0	0.0	0.8	6.9	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
24	0.0	0.0	0.0	0.1	19.4	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
25	0.0	0.0	0.0	9.1	0.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
26	0.0	0.0	0.0	7.3	3.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
27	0.0	0.0	0.0	0.4	0.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
28	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
29	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	
30	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	
31	0.0		0.0	0.0		0.0		0.0		0.0			Annual
TOTAL	3	0	0	168	40	10	46	0	0	0	0	0	267
Max.day	3	0	0	59	19	7	25	0	0	0	0	0	59

Daily values are based on calculations of sediment discharge at 15-minute intervals.

Multiple sediment-discharge rating curves were used for different periods of the year and ranges of flow.

Daily values with more than 2 to 3 significant figures result from electronic calculations. No additional precision is implied.

Total annual sediment discharge (suspended plus bedload sediment)	
WY 2010:	1,185 tons

Balance Hydrologics, Inc., 800 Bancroft Way, Suite 101, Berkeley, CA 94710 phone:(510) 704-1000 fax:(510) 704-1001

Form 3. Annual Rainfall Record: San Geronimo Creek at Lagunitas

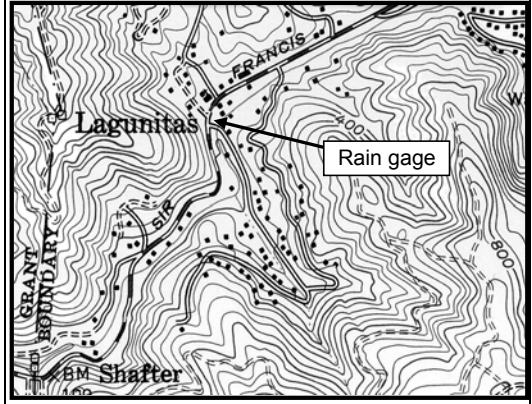
Station Location / Watershed Descriptors

Located on a telephone pole at the north side of the Lagunitas Road bridge
 Latitude: 36 00' 40", Longitude: 122 42' 07"
 Elevation: 239 feet

Period of Record

6-inch, tipping-bucket rain gauge installed March 2006
 Sponsored by Marin Municipal Water District

Water Year: 2010
Watershed: San Geronimo Creek
Station: Lagunitas Road bridge
County, State: Marin County, CA
Station Location Map:



Peak Daily Rainfall

and

Peak Rainfall Intensity

Date	Daily total (in)
10/13/09	4.92
1/18/10	1.74
1/19/10	1.65
1/20/10	1.95
2/23/10	2.34
4/11/10	2.77

Date	Max. Hourly Intensity (in/hr)
10/13/09	0.85
1/12/10	0.42
1/19/10	0.58
2/23/10	0.40
2/26/10	0.62
4/11/10	0.40

Water Year 2010 Daily Total Rainfall (inches)

DAY	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEPT
1	0.00	0.01	0.00	0.08	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	0.00	0.00	0.00	0.04	0.00	1.14	1.24	0.00	0.00	0.00	0.00	0.00
3	0.00	0.00	0.00	0.00	0.08	1.03	0.00	0.00	0.00	0.00	0.00	0.00
4	0.00	0.00	0.00	0.00	1.46	0.01	1.28	0.00	0.00	0.00	0.00	0.00
5	0.00	0.05	0.00	0.00	0.22	0.00	0.11	0.00	0.00	0.00	0.00	0.00
6	0.00	0.12	0.23	0.00	0.43	0.00	0.00	0.00	0.00	0.00	0.00	0.00
7	0.00	0.00	0.16	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
8	0.00	0.00	0.00	0.01	0.12	0.07	0.00	0.00	0.00	0.00	0.00	0.00
9	0.00	0.00	0.00	0.00	0.54	0.16	0.00	0.05	0.00	0.00	0.00	0.00
10	0.00	0.00	0.07	0.00	0.00	0.20	0.04	0.27	0.00	0.00	0.00	0.00
11	0.00	0.00	0.84	0.00	0.32	0.00	2.77	0.00	0.00	0.00	0.00	0.00
12	0.00	0.00	1.11	1.63	0.33	1.22	0.20	0.00	0.00	0.00	0.00	0.00
13	4.92	0.00	0.97	0.08	0.00	0.01	0.00	0.00	0.00	0.00	0.00	0.00
14	0.00	0.00	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
15	0.20	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
16	0.01	0.00	0.61	0.54	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
17	0.01	0.04	0.01	0.89	0.00	0.00	0.00	0.78	0.00	0.00	0.00	0.00
18	0.01	0.01	0.00	1.74	0.00	0.00	0.00	0.12	0.00	0.00	0.00	0.00
19	0.65	0.00	0.00	1.65	0.00	0.00	0.04	0.01	0.00	0.00	0.00	0.00
20	0.00	0.66	0.14	1.95	0.00	0.00	0.77	0.00	0.00	0.00	0.00	0.01
21	0.00	0.00	0.35	1.27	0.21	0.00	0.01	0.00	0.00	0.00	0.00	0.00
22	0.01	0.04	0.00	0.39	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
23	0.00	0.00	0.00	0.32	2.34	0.00	0.00	0.00	0.00	0.00	0.00	0.00
24	0.01	0.00	0.01	0.16	0.64	0.21	0.00	0.00	0.00	0.00	0.00	0.00
25	0.00	0.00	0.00	1.63	0.00	0.05	0.00	0.71	0.00	0.00	0.00	0.00
26	0.00	0.00	0.46	0.30	1.16	0.00	0.00	0.01	0.00	0.00	0.00	0.00
27	0.00	0.03	0.03	0.00	0.33	0.00	1.31	0.80	0.00	0.00	0.00	0.00
28	0.00	0.00	0.01	0.00	0.00	0.00	0.09	0.00	0.00	0.00	0.00	0.00
29	0.00	0.00	0.42	0.49	0.65	0.00	0.00	0.00	0.00	0.00	0.00	0.00
30	0.00	0.00	0.01	0.01	0.26	0.00	0.00	0.00	0.00	0.00	0.00	0.00
31	0.00	0.00	0.01	0.00	0.80	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Total	5.83	0.96	5.45	13.18	8.18	5.81	7.86	2.75	0.00	0.00	0.00	0.01
Max	4.92	0.66	1.11	1.95	2.34	1.22	2.77	0.80	0.00	0.00	0.00	0.01

Notes and comments:

The rain gauge became clogged on 10/13/2009. The amount of rainfall was corrected by the volume of water found in the funnel of the rain gauge on 11/5/09. The pattern of rainfall was correlated a nearby rain gauge that Balance operates.

Water Year 2010

Total Annual	50.03	(inches)
Maximum Daily Total	4.92	(inches)

Balance Hydrologics, Inc. 101 Lucas Valley Rd., Suite 229, San Rafael, CA 94903 (415) 472-7584

Balance Hydrologics, Inc. 800 Bancroft Way, Suite 101, Berkeley, CA 94710 (510) 704-1000; fax: (510) 704-1001; www.balancehydro.com

TABLES

**Table 1. Stream gaging observer log:
San Geronimo Creek at Lagunitas Road bridge, water year 2010**

Site Conditions					Streamflow				Water Quality Observations				High-Water Marks		Remarks
Date/Time (observation time)	Observer	Stage (left bank)	Stage (right bank)	Hydrograph	Measured Discharge	Estimated Discharge	Instrument Used	Estimated Accuracy	Water Temperature	Field Specific Conductance	Adjusted Specific Conductance	Additional sampling?	Estimated stage at left-bank staff plate	Inferred dates?	
(mm/dd/yr)		(feet)	(feet)	(R/F/S/B)	(cfs)		(AA/PY)	(e/g/f/p)	(oC)	(µmhos/cm)	(at 25 oC)	(Qbed, etc.)	(feet)	(mm/dd/yr)	
9/22/09 17:00	jo	...	3.915	B	...	0.05	visual	p	17.6	362	422	DO	low flow very difficult to measure, measurements deemed invalid, so estimated flow is reported; DO = 49%; water clear, many leaf dams
11/5/09 15:47	tb, sr	0.09	4.06	B, R	0.96	1.0	PY	f	13.0	327	425	...	2.3, 0.11	Oct. 13, today	rain today, increasing; cleaned rain gauge
12/9/09 17:00	mw	0.18	...	F, B	0.83	...	PY	f	too dark	...	leaf dam across creek below bridge
1/18/10 13:05	mw, gp	4.00	...	F	3 Qss, 3 Qbed	bridgeboard; left-bank stage fell from 4.4 ft. at 11:00 to 3.1 ft. at 14:40
1/20/10 11:35	mw, gp	3.70	...	F	12.0	95	129	2 Qss, 2 Qbed	bridgeboard; left-bank stage fell from 4.1 ft. at 11:10 to 3.45 ft. at 12:05
1/26/10 11:30	mw	1.81	...	F	11	125	175	Qss, Qbed	not too turbid, not much bedload
2/18/10 13:30	gp	0.38	4.34	B	13.03	...	AA	e	11.7	222	297	...	4.1, 2.6	...	water is clear; HWM are on left-bank staff plate
2/24/10 11:00	mw	1.90	...	F	12	122	166	Qss, Qbed	2.4	today	recessional HWM at ~2.4 feet; seemed to be a higher HWM but not clear
3/19/10 16:00	jo	0.33	4.31	F, B	10.40	...	PY	g	11.8	225	300	...	1.1, 2.3, 4.5	recent, season	many 1-inch fish in creek; water clear
4/5/10 8:45	mw	1.37	...	F	...	70.0	visual	p	Qss, 2 Qbed	no clear high-water marks
4/6/10 8:05	mw	0.88	...	F	...	35.0	visual	p	9.5	160	233	Qss, Qbed	3.0	4/4/2010	baseflow staying high from two days ago
4/30/10 11:00	jo, sr	0.33	4.32	F, B	10.69	9.0	PY	g	10	212	297	...	4.3, 0.8	season, recent	water mostly clear; some coarse sand on bed between gravels and in lee of rocks
6/2/10 16:30	jo	0.08	4.06	B	2.57	2.5	PY	f, g	11.1	6.1, 1.1, 0.6	season, recent	water clear, no fish seen; pebble casings of bug larva on some rocks
6/29/10 15:55	jo	0.020	3.995	B	0.96	1.2	PY	f, g	17.3	328	382	...	1.0, 2.0	spring	water clear; many 2- to 3-inch fish; leaf and twig dams in staff pool
8/3/10 12:17	gp	0.020	4.00	B	1.07	1.0	PY	f, g	15.6	327	398	water is clear, did not see any fish, leaf dams downstream of staff plates were not cleared
9/2/10 17:50	mw	0.04	4.02	B	0.19	...	PY	f	18.0	360	419	Leaf dam at riffle crest below gage affecting water level; dam left in place; all riffles have leaf dams.
9/30/10 12:00	mw	0.01	3.99	B	0.14	...	PY	g	15.0	340	427	Leaf dams everywhere. I was careful not to break dam. Cleaned staff plates.

Observer Key: (mw) is Mark Woyshner; (gp) is Gustavo Porras; (jo) is Jonathan Owens, (tb) is Travis Baggett, (sr) is Sarah Richmond

Stage: Water level observed at outside staff plate

Hydrograph: Describes stream stage as rising (R), falling (F), steady (S), or baseflow (B)

Instrument: If measured, typically made using a standard (AA) or pygmy (PY) bucket-wheel ("Price-type") current meter. If estimated, from rating curve (R) or visual (V).

Estimated measurement accuracy: Excellent (E) = +/- 2%; Good (G) = +/- 5%; Fair (F) = +/- 9%; Poor (P) estimated percent accuracy giver

High-water mark (HWM): Measured or estimated at location of the staff plate

Specific conductance: Measured in micromhos/cm in field; then adjusted to 25degC by equation $(1.8813774452 - [0.050433063928 * \text{field temp}] + [0.00058561144042 * \text{field temp}^2]) * \text{Field specific conductance}$

Additional Sampling: Qbed = Bedload, Qss = Suspended sediment, Nutr = nutrients; other symbols as appropriate

**Table 2. Sediment transport measurements:
San Geronimo Creek at Lagunitas Road bridge (K4), water year 2010**

Sample Date:Time	Site Conditions					Bedload Sampling Details						Sediment Transport			
	Observer(s)	Gage Height (left bank)	Gage Height (right bank)	Stream Discharge	Stream Condition	Active Bed Width	Sampler Width	No. of Verts.	Time/Vert.	Total Time	Sample Dry Weight	Bedload Discharge Rate	Bedload Discharge Rate	Suspended Sediment Concentration	Suspended Sediment discharge
		(ft)	(ft)	(cfs)	R,F,B,P	(ft)	(ft)		(sec)	(sec)	(grams)	(lb/sec)	(tons/day)	(mg/L)	(tons/day)
1/18/2010 12:50	mw, gp	4.4	8.4	440	F	531	630.3
1/18/2010 13:35	mw, gp	3.7	7.7	350	F	22	0.25	5	20	100	616.7	1.197	50.23
1/18/2010 13:48	mw, gp	3.4	7.4	325	F	22	0.25	5	20	100	342.4	0.664	27.89
1/18/2010 13:55	mw, gp	3.3	7.3	315	F	350	297.4
1/18/2010 14:25	mw, gp	3.2	7.2	275	F	330	244.8
1/18/2010 14:40	mw, gp	3.1	7.1	260	F	20	0.25	5	30	150	1,588.7	1.868	78.42
1/20/2010 11:15	mw, gp	3.95	7.9	403	F	408	443.6
1/20/2010 11:35	mw, gp	3.8	7.8	375	F	22	0.25	5	25	125	2,673.0	4.149	174.18
1/20/2010 11:50	mw, gp	3.6	7.6	355	F	22	0.25	5	25	125	834.3	1.295	54.36
1/20/2010 12:05	mw, gp	3.45	7.4	338	F	248	226
1/26/2010 11:30	mw	1.81	5.8	116	F	"not much bedload"					...	1	29.4	9	
2/24/2010 11:00	mw	1.9	5.9	125	F	19	0.25	5	60	300	989.0	0.55	23.2	30.4	11
4/5/2010 8:50	mw	1.37	5.4	66	F	14	0.25	5	120	600	194.3	0.04	1.7	37.3	7
4/5/2010 9:05	mw	1.36	5.3	64	F	14	0.25	5	120	600	85.0	0.02	0.7
4/6/2010 8:00	mw	0.88	4.9	32	F	12	0.25	5	240	1200	12.2	0.00	0.04	13.8	1.3

Notes:

Observer Key: (jo) is Jonathan Owens; (mw) is Mark Woynshner; (gp) is Gustavo Porras

Streamflow discharge is the measured or estimated instantaneous flow when sediment was sampled, and usually differs from the mean flow for the day.

Stream Condition: R = rising, F = falling, B = baseflow, P = near peak of storm.

Values for sediment discharge having more than two to three digits displayed are the result of calculations; increased precision is not implied.

Active Bed Width: The width thought by the field observer to be transporting significant amounts of bedload, based on field observations and sampling.

Sampler Width and Type: 0.25 = 3-inch Helley Smith; 0.50 = 6-inch Helley Smith

Bedload Discharge (lbs/sec) = [active bed width (ft) * sample dry weight (gm) * 0.002205 (lbs)] / [sampler width (ft) * sampling time (sec)]

Bedload Discharge (tons/day) = [active bed width (ft) * sample dry weight (gm) * 86,400 (sec)] / [sampler width (ft) * sampling time (sec) * 907,200 (gm)]

Value of 0.01 tons/day assigned to observations of "no bedload", so that threshold values of bedload sediment transport can be plotted on a logarithmic axis and estimated.

**Table 3. Hydrologic summary for recent water years,
San Geronimo Creek at Lagunitas Road, Marin County, California**

Water Year	Annual Flow				Precipitation		Sediment Discharge				Peak Flow		
	Mean Annual Flow	Maximum Daily Flow	Minimum Daily Flow	Total Flow Volume	Annual Rainfall	percent of long-term average	Suspended Sediment	percent suspended	Bedload Sediment	percent bedload	Peak Flow	Peak Stage	Date Time
	(cfs)	(cfs)	(cfs)	(ac-ft)	(inches)		(tons)		(tons)		(cfs)	(ft)	(24-hr)
<i>period of record commenced November 1979</i>													
1997	14.2	540	0.05	10,315		1,352	7.50	1/1/1997
1998	29.7	570	0.13	21,525		2,049	9.30	2/3/1998 1:30
1999	16.6	575	0.11	12,040		1,103	6.75	2/6/1999 17:00
2000	13.4	558	0.06	9,733	423			1,150	6.89	2/13/2000 7:15
2001	6.4	158	0.03	4,611	71			341	2.97	2/21/2001 6:15
2002	11.9	396	0.05	8,590	3,348			1,595	8.15	12/1/2001 13:00
2003	12.0	391	0.04	8,710	1,174			1,789	8.68	12/16/2002 3:45
2004	13.6	649	0.04	9,864	1,612 e			1,648	8.29	12/29/2003 11:00
2005	15.9	510	0.05	11,516	...		2,452	74%	851	26%	1,264	7.03	12/27/2004 5:00
2006	27.3	1094	0.37	19,756	...		26,395	79%	6,874	21%	3,940	12.30	12/31/2005 5:45
2007	6.4	300	0.05	4,638	34.3	75%	433	65%	229	35%	478	8.20*	12/26/2006 18:45
2008	10.7	578	0.01	7,804	38.4	84%	3,072	83%	623	17%	1,664	11.85*	1/4/2008 12:30
2009	6.7	361	0.01	4,873	37.3	82%	664	64%	374	36%	641	9.17*	2/22/2009 14:00
2010	12.5	245	0.03	9,070	50.0	110%	918	77%	267	23%	640	9.01*	1/20/2010 9:30
mean 1980 to 2010	15.3	583	0.10	11,079			5,656	80%	1,423	20%	1,466		
median 1980 to 2010	12.5	540	0.10	9,070			1,685	76%	523	24%	1,350		

Notes:

The period of record for this station is Nov. 1979 to Sept. 2010. Monitoring was not continued for water year 2011.

A "water year" ends on Sept. 30 of the named year. For example, water year 2010 starts Oct. 1, 2009, and ends Sept. 30, 2010.

For water years 1997, 2004, and 2005 the record was incomplete; annual statistics were calculated using some correlated records.

Daily flow values computed from instantaneous flow calculated at 15-minute intervals. Sediment discharge values totalled from calculations at 15-minute intervals.

* Stage is the staff plate reading; the staff plate is set at an arbitrary datum and does not represent the absolute depth of water in the creek. Starting in water year 2007 we switched datums to a new staff plate on the right (north) side of the creek (looking downstream). The new, right-bank staff plate reads approximately 4.0 feet higher than the old staff plate.

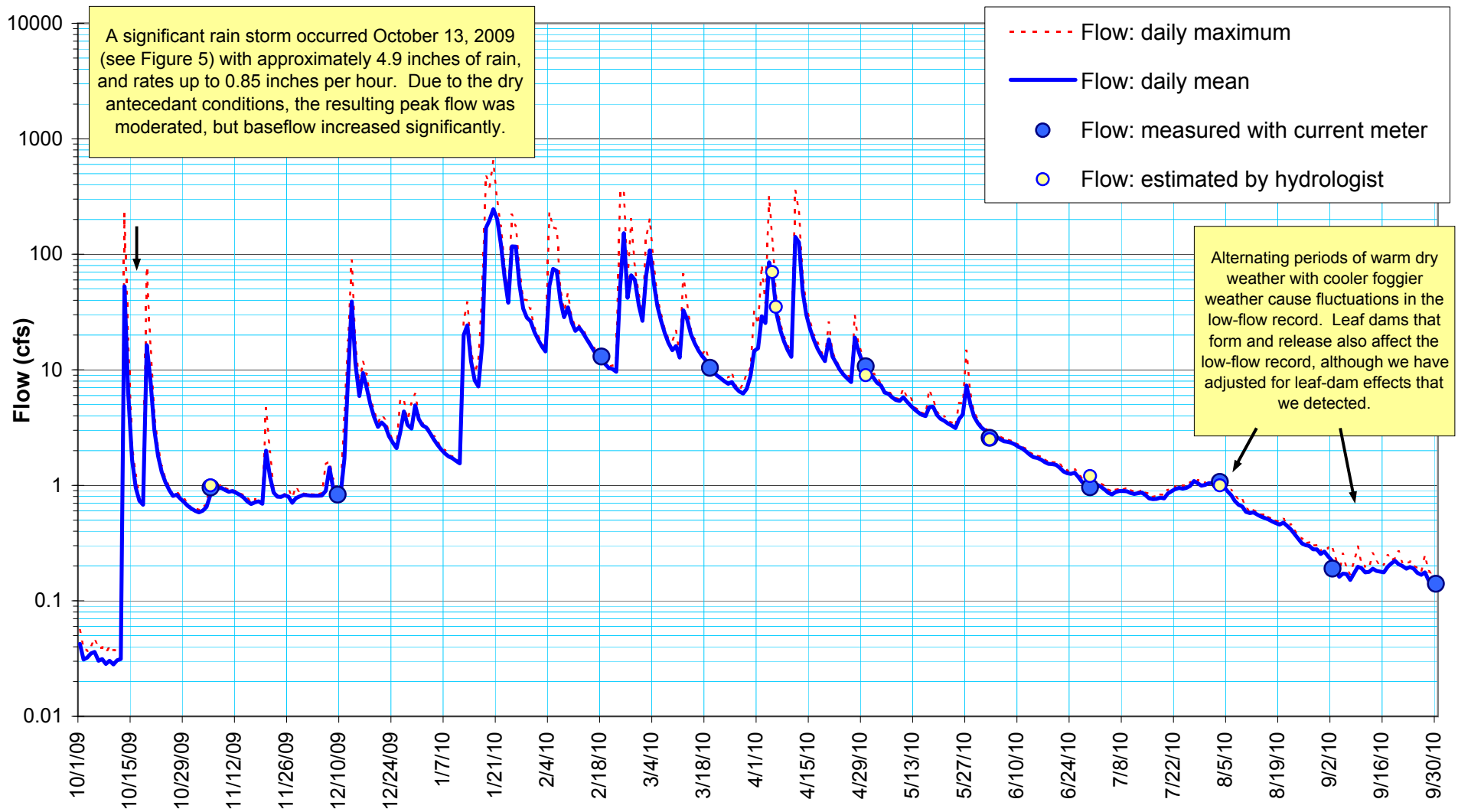
Values displaying more than 2 or 3 significant figures are the result of calculations; no additional precision is implied.

Bedload discharge is based on sampling 4 to 10 times per season, and is for the limited purpose seeking general trends in sediment loads and bed sedimentation over time.

Daily data were not collected during summer 2004. Total flow volumes reported for water years 2004 and 2005 are calculated based on correlated data.

e = Estimated value following partial reactivation of this gage. Data are preliminary, subject to review, and not for publication without consultation.

FIGURES



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Figure 1. Daily flow hydrograph: San Geronimo Creek at Lagunitas Road, water year 2010.

The peak flow of approximately 640 cfs occurred January 20, 2010 at 9:30 AM. The flow peak this year was in the low range compared to previous years (see Table 3). Multiple stage shifts have been applied to this record to account for localized scour and fill, and the effects of leaf dams during low flows.

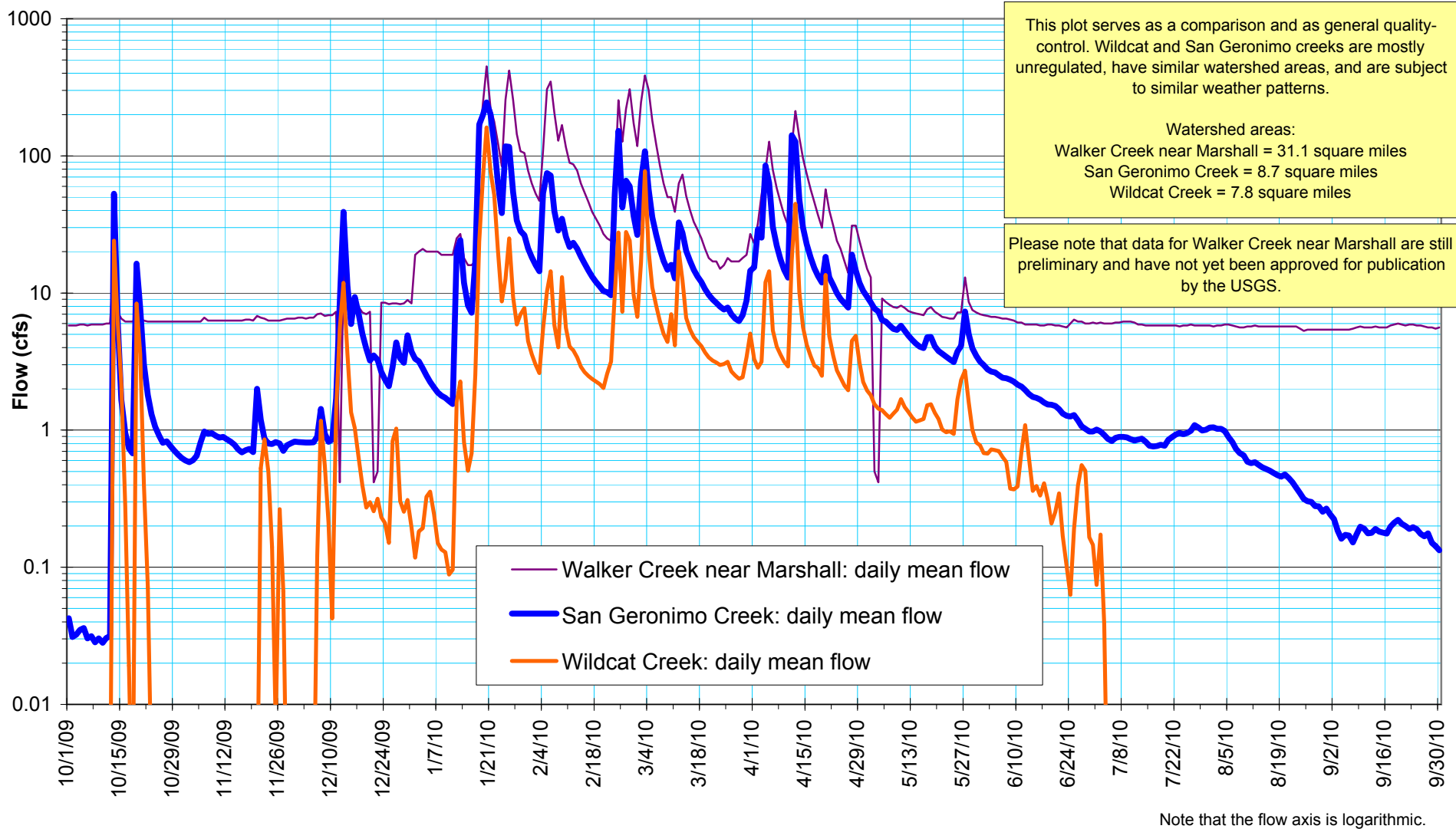


Figure 2. Daily flow comparison: San Geronimo Creek, Walker Creek near Marshall, and Wildcat Creek in Richmond (Contra Costa County), water year 2010. All three creeks behave similarly during winter peaks. Walker Creek receives slightly less rainfall and is affected by SoulaJule Reservoir (evident during low-flow dips and the lack of summer recession). Wildcat Creek receives significantly less rainfall and generally has lower baseflows.



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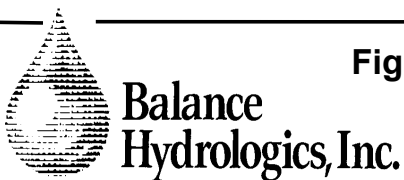
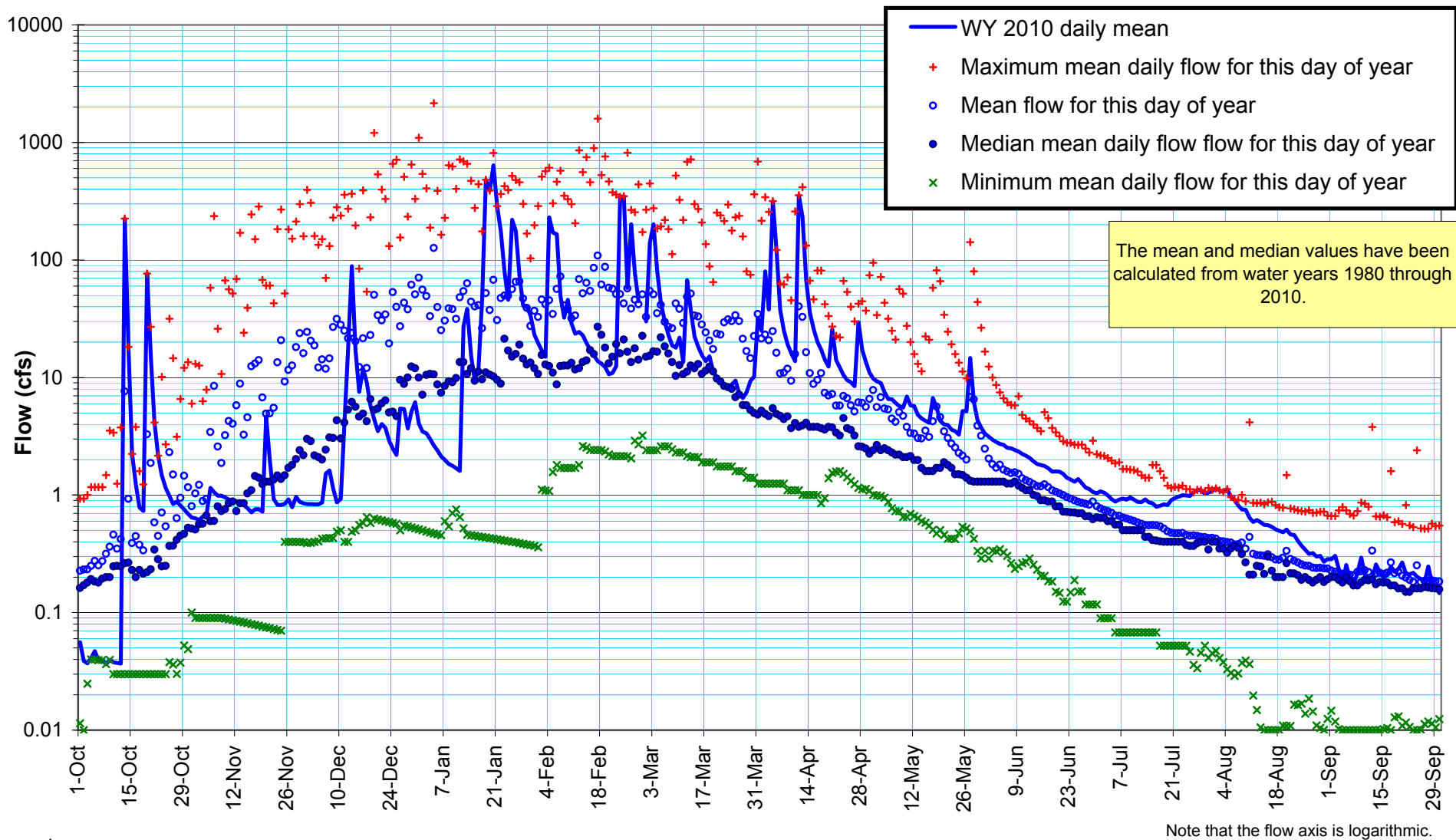


Figure 3. Flow comparison of water year 2010 to period of record: San Geronimo Creek at Lagunitas Road. Due to several moderate storms and sporadic spring rain, total flow for water year 2010 was slightly above the long-term average and long-term median. San Geronimo Creek has been gaged by Balance staff at this location since November 1979.

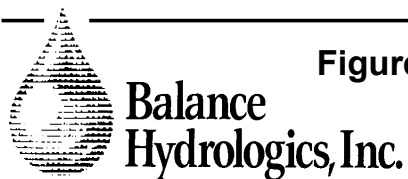
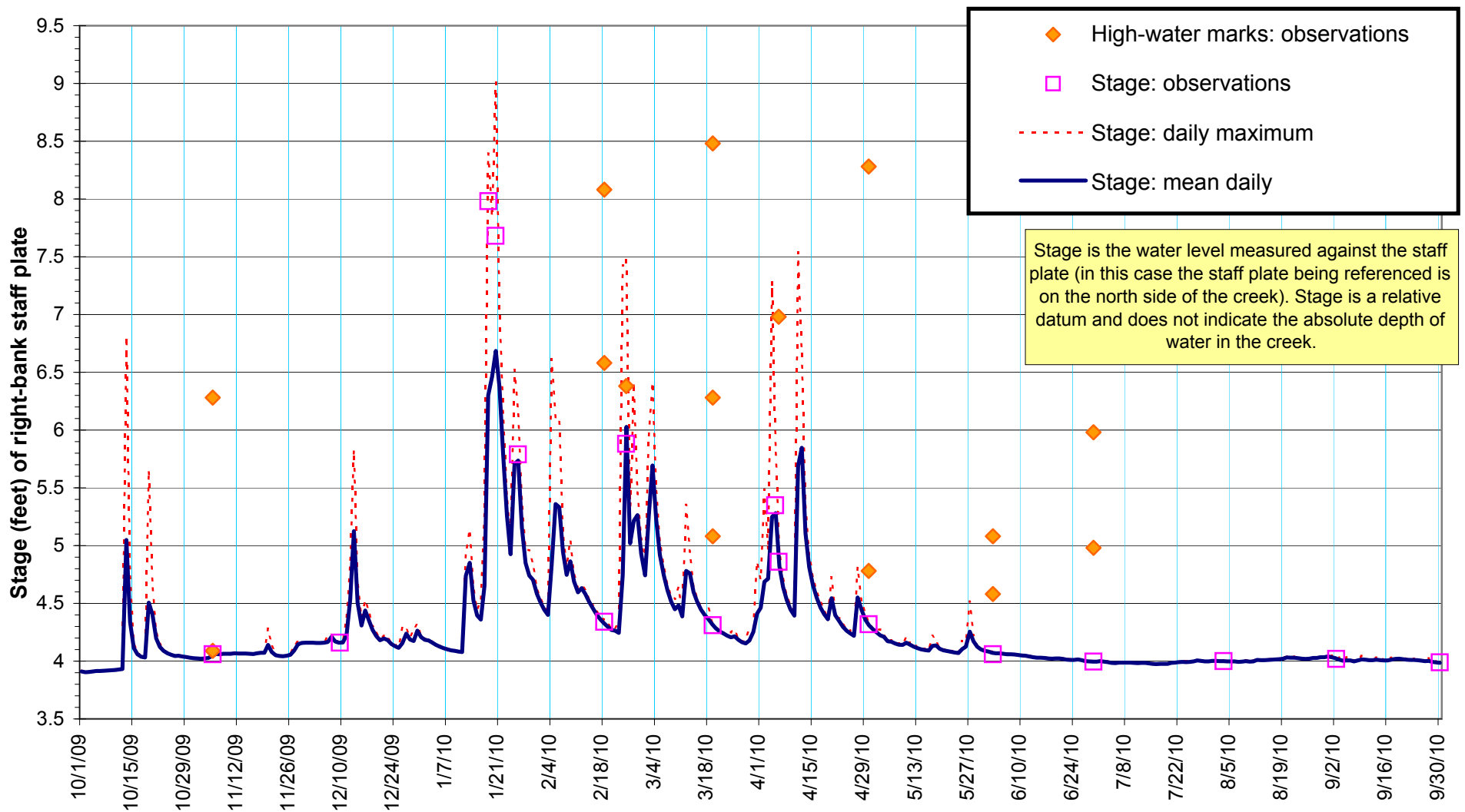
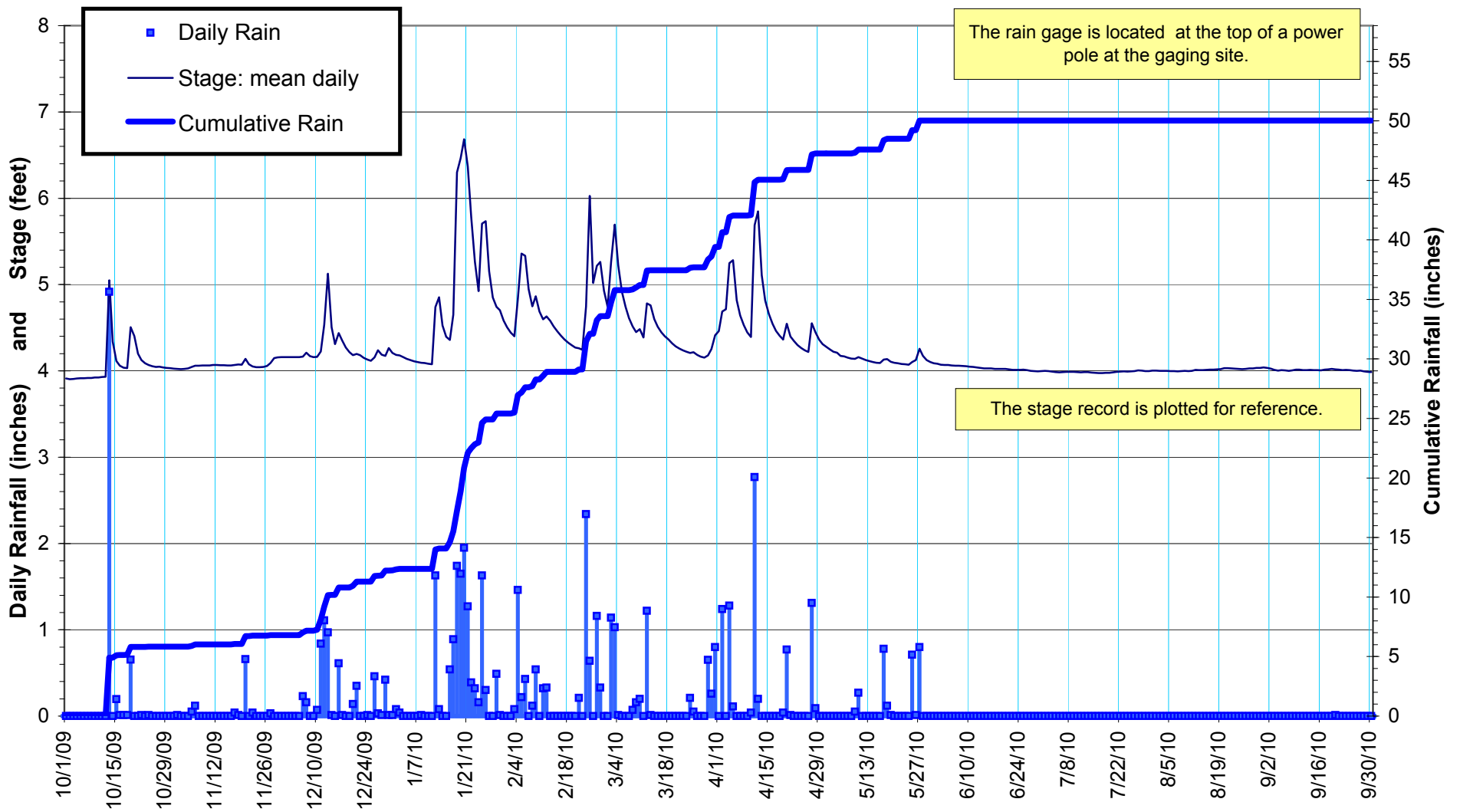
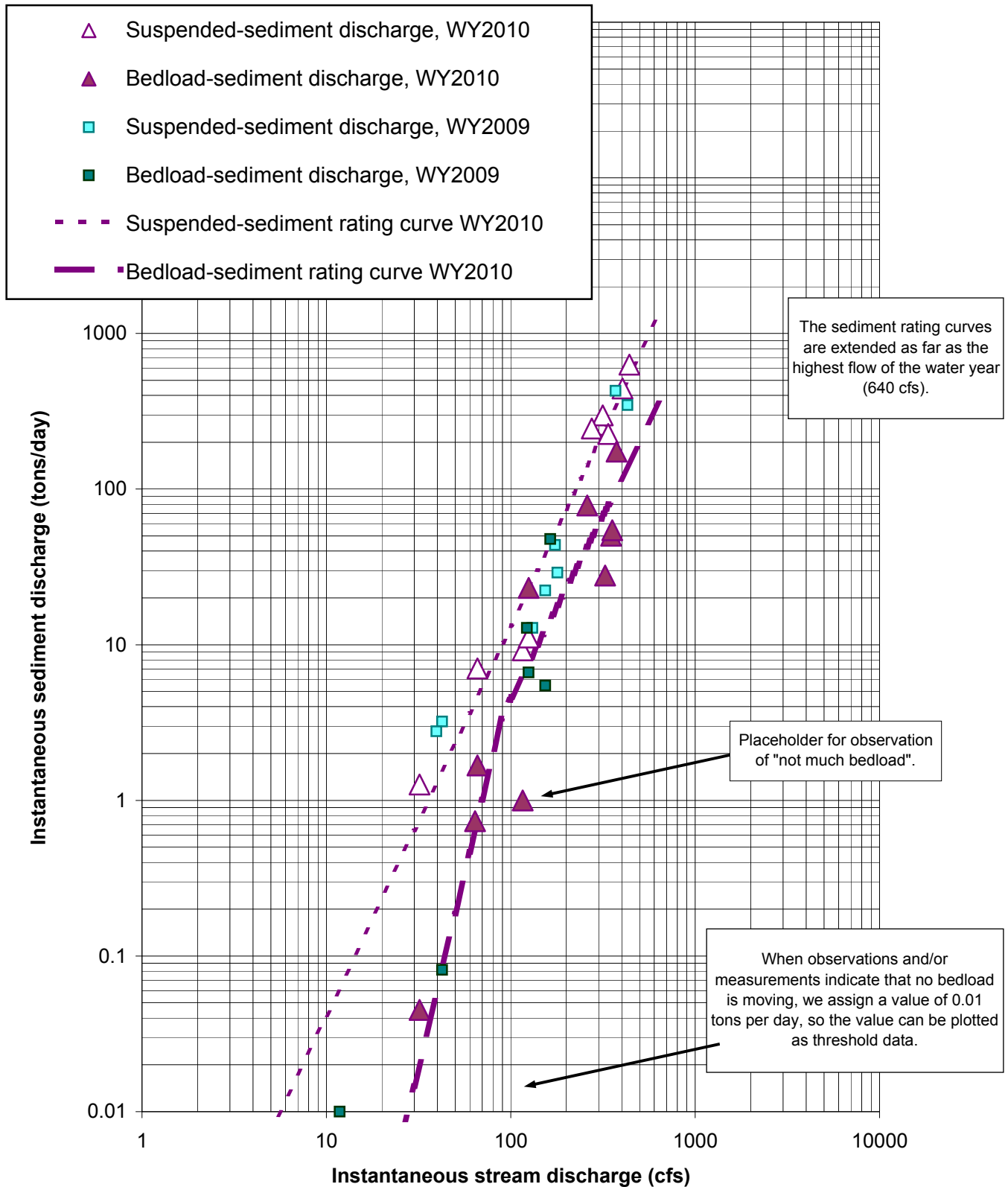


Figure 4. Daily stage record: San Geronimo Creek at Lagunitas Road, water year 2010. The peak stage of 9.01 feet occurred on January 20, 2010 at 9:30 AM. During low flows, naturally-formed leaf dams can temporarily raise water levels, and regularly occur during the fall and late summer.



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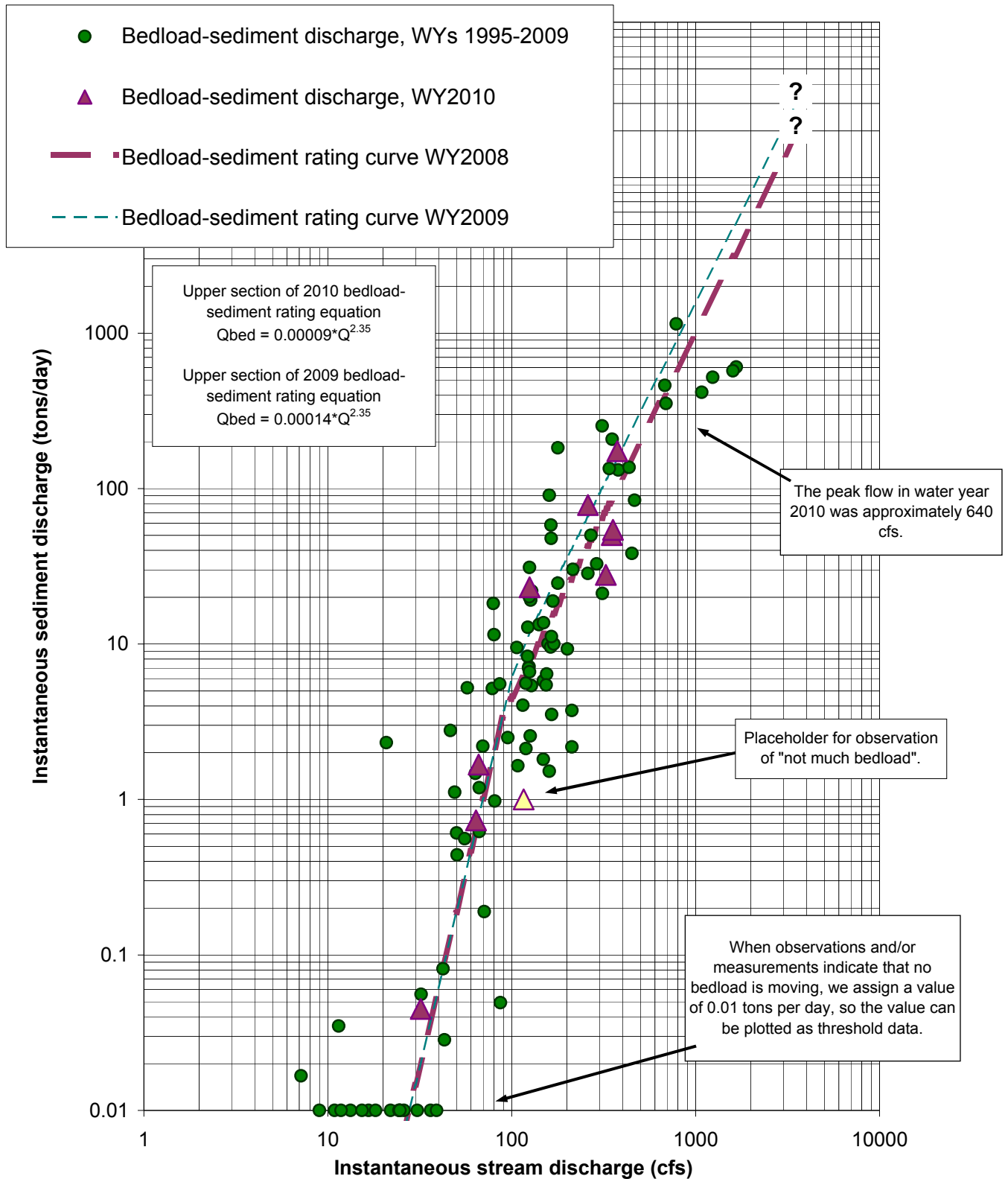
Figure 5. Rainfall record: San Geronimo Creek at Lagunitas Road, water year 2010. The rainfall total for water year 2010 was 50.0 inches, or approximately 110 percent of long-term average precipitation (45 to 46 inches). Water year 2010 is the first above average water year after 3 years in a row with below average rainfall.



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Figure 6. Sediment discharge and rating curves: San Geronimo Creek at Lagunitas Road, water year 2010.

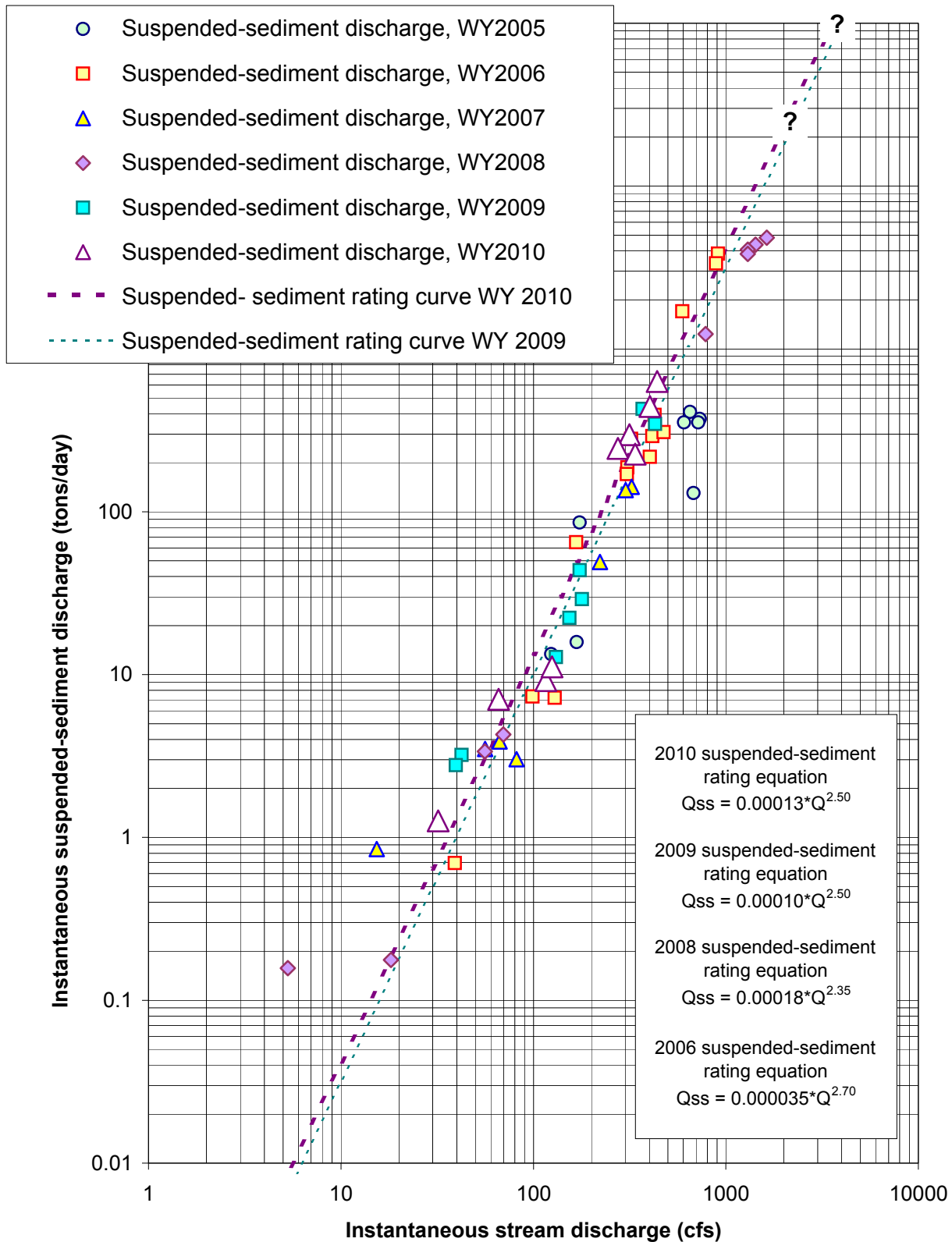
These data suggest that more suspended-sediment is moved by the creek than bedload sediment at all flow levels.



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Figure 7. Comparison of bedload-sediment discharge to previous years: San Geronimo Creek at Lagunitas Road.

Bedload-sediment discharge rates during water year 2010 seem generally similar to previous data, but slightly lower than water year 2009.



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Figure 8. Comparison of suspended-sediment discharge to previous years: San Geronimo Creek at Lagunitas Road. Suspended-sediment discharge rates seem similar to previous data.