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Dear Mr. Hoagland.

Enclosed is a copy of the final report entitled: "Review of Subsurface Geologic Conditions in the Vicinity to the Zell Building, Norwalk, CT with respect to the Kellogg-Deering Well Field."

This report is submitted as public comment in review of the RI/FS Report entitled: Final Supplemental Remedial Investigation and Feasibility Study, Kellogg-Deering Superfund Site, Norwalk, Connecticut (July 1989).

Sincerely

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**REVIEW OF SUBSURFACE GEOLOGIC CONDITIONS  
IN THE VICINITY OF THE ZELL BUILDING  
NORWALK, CT**

**WITH RESPECT TO THE  
KELLOGG-DEERING WELL FIELD**

**Prepared For:**

**MURTHA, CULLINA, RICHTER AND PINNEY**

**AUGUST 1989**

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## 1.0 INTRODUCTION

### 1.1 PROJECT BACKGROUND

Pitney Bowes was notified by Region 1 of the U.S. EPA that the property leased by Pitney Bowes at 276 Main Street in Norwalk, CT, Zell Buildings 1 and 2 and surrounding property, may have contributed to the Kellogg-Deering well field contamination and that they were now considered a Potentially Responsible Party (PRP) as described in Section 122 of the Superfund Amendments and Reauthorization Act of 1986.

This notification was based on several speculative comments made in a consulting engineers report, prepared by HRP Associates of New Britain, CT, for the Elinco Corporation, a company located at 272 Main Street, next door to the building leased by Pitney Bowes. The Elinco Corporation has a documented history of TCE usage, (including a former 500-gallon aboveground TCE tank), and has also been named by U.S. EPA Region 1 as a PRP for the Kellogg-Deering site. Further information provided to Dames & Moore has identified a former degreasing pit inside the Elinco plant near the northeast corner of the building. Trichloroethene (TCE) and Tetrachloroethene (PCE) are common degreasing agents.

The December 4, 1987 report prepared by Dames & Moore for Murtha, Cullina, Richer and Pinney concluded that Pitney Bowes operations at 276 Main Street could not have been responsible for, or contributed to, the ground water contamination detected in the Kellogg-Deering well field. This report confirms the conclusions of the 1987 report. More recent information developed by NUS, an EPA contractor, is the subject of this report. Dames & Moore has reviewed this information in the context of the geologic setting of the site, and conclude that the Zell buildings currently leased by Pitney Bowes are downgradient of the degreasing pit located in the northeast corner of the Elinco building.

### 1.2 OBJECTIVES OF THE REPORT

The objectives of this study are to (1) review data compiled to date (2) define subsurface geologic conditions, and (3) address the issue of the Zell Buildings and neighboring buildings as sources of contamination of the Kellogg-Deering Well Field.

The compilation of subsurface data to date includes monitoring well logs water elevations, tracer studies, pump tests, soil gas survey, and analyses of ground water and soil samples. Subsurface conditions that can be defined from these data sets include characterization of the overburden and bedrock geologic units, determination of ground water gradients and flow directions, and the concentration of contaminants in ground water and soils. The locations and concentrations of contaminants in conjunction with subsurface flow gradients, directions, and barriers are useful in identifying the primary source of contamination.

This report includes the following information:

- A daily field log documenting EPA contractor field activities (Appendix C), and site photographs (Appendix B);
- A discussion of the topography, geology and hydrogeology of the area along with graphic illustrations of the site's physical attributes;
- A discussion of the present and historical problems of local ground water contamination within the context of the topographic, geologic and hydrogeologic character of the site;
- The results of the soil sampled from the borehole for monitoring well K-19B, which is located between the two segments of the Zell building complex leased by Pitney-Bowes, (Zell 1 & Zell 2), as well as the results from soil boring ESB-1 from within the Elinco plant;
- The analytical results of the ground water samples that were split with NUS corporation, EPA contractor, comparisons with the EPA results and the analytical results from previous rounds of sampling;
- Results and analyses of the long term and short term tracer studies;
- Results and analyses of the ground water sampling before and after the interceptor well pumping test;

- Results and interpretations of the indoor and outdoor soil gas surveys performed by NUS are discussed and related to the ground water data.

SECTION 2

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## 2.0 SITE CHARACTERISTICS

This section of the report provides information on the physiographic setting of the Zell Building leased by Pitney Bowes, as well as the site's geologic and hydrogeologic characteristics. *Pitney Bowes on-site operations as well as their past and current waste handling practices* are discussed in the previous report prepared by Dames & Moore on December 4, 1987 for Murtha, Cullina, Richer and Pinney.

### 2.1 PHYSIOGRAPHIC SETTING

The buildings leased by Pitney Bowes are located at 276 Main Street, the Zell Building complex, in Norwalk, Connecticut (Figure 1) in an area dominated by light industrial and commercial establishments (Figure 2). The Zell Building complex consists of two buildings (Figure 3) which are connected by a common lobby and are surrounded by a paved parking area with loading docks.

To the east of the Zell Building complex is an elevated bedrock ridge upon which a small housing development has been built. To the south is the manufacturing operation of the Elinco Corporation. Along the Zell Building's northern border is an undeveloped wooded area and to the west, across Main Street, is a row of small commercial (retail) businesses. The Kellogg-Deering well field is located within one half mile of the Zell Building complex, west-southwest of the site (Figure 2).

### 2.2 TOPOGRAPHY

The topography of the area encompassing the Pitney-Bowes/Elinco properties is irregular and ranges from 78.45 feet, well K-18B to 105.23 feet, well MW-1 above mean sea level (MSL) (Figure 1 and Table 1). MW-1 is the highest upgradient point on the overburden surface and K-18B is the lowest downgradient point located on these properties (Table 1).

The elevations of the monitor well and soil boring locations relative to MSL were used in the construction of Figure 4 (Table 1). This data was recorded in the soil boring and well construction logs provided to Dames & Moore by U.S. EPA-Region 1. These data show a component of the topographic gradient from the Elinco property to the Zell buildings.

The site-scale topography of the Zell/Elinco complex is shown in Figure 4. In terms of regional topographic characteristics, the overburden surface, which controls the surface drainage of the area, slopes radially from the southwest to the northwest, toward the regional drainage system of Deering Pond and the Norwalk River (Figure 1).

### 2.3 GEOLOGY

A study of the soil boring logs (Appendix A) and geologic data provided to Dames & Moore by U.S. EPA-Region 1 has identified two distinct geologic units on site:

- Overburden--this unit consists of various unconsolidated, glacially-derived sediments which range in thickness from 5.5 feet at MW-1 to 38.5 feet at K-11. These sediments are primarily very coarse grained, poorly sorted outwash deposits (sand, gravel, cobbles and boulders) with some interbedded layers of glacial till which are very poorly sorted and finer grained (sand, silt and clay). In general, this unit exhibits high porosity and permeability. Figure 4 depicts the surface contours of this unit with respect to mean sea level (MSL).
- Bedrock--this unit is comprised primarily of felsic and feldspathic gneiss, granite and micaceous schists. The bedrock is heavily fractured in the uppermost sections and gains structural integrity with depth. An elevated bedrock ridge occurs along the eastern and southern extremes of the Pitney-Bowes/Elinco properties and is a prominent feature of the region's structural geology. The surface relief of the bedrock is a strong influence on the area's topography (Figure 1). The site-scale relief of the bedrock surface is shown in Figure 5. A bedrock slab is apparent beneath the northeast corner of the Elinco building, (underneath the location of the former degreasing pit), and the top of this bedrock slab appears to slope to the northwest toward Zell 1 and monitor wells K-11 and MW-100 (Figure 5).

There is a strong inverse correlation between the bedrock surface elevation and the thickness of the overburden (Table 1). The influence of the bedrock topography and overburden thickness on the site's hydrogeology is discussed later.

The predominance of feldspar in the bedrock was determined by the U.S. EPA contractor's field geologist and by correlation with information recorded in the boring logs, and is consistent with the description of the bedrock (Appendix A). Specifically, the presence of albite (sodium feldspar), anorthite (calcium feldspar) and orthoclase (potassium feldspar) was observed by the field geologists. As feldspar weathers at a rapid rate relative to other igneous and metamorphic mineral assemblages, ions of sodium ( $\text{Na}^{+1}$ ), calcium ( $\text{Ca}^{+2}$ ) and potassium ( $\text{K}^{+1}$ ) are released in the bedrock water bearing zone. These cations readily combine with chloride ( $\text{Cl}^{-1}$ ) and bromide ( $\text{Br}^{-1}$ ), (the ions used in the tracer study), to form common salts which precipitate as cement matrices within fractures in the bedrock. The presence of these types of mineral cements was also noted by the field geologists in the boring logs. The impact of this information on the results of the tracer study conducted by NUS, the U.S. EPA-Region 1 contractor, is discussed in section 2.7.

## 2.4 HYDROGEOLOGY

### 2.4.1 Background

As previously mentioned, the main contaminants of interest to U.S. EPA-Region 1 within the Kellogg-Deering Well Field are volatile organic compounds (VOCs). Specifically, the EPA has identified two common industrial solvents, trichloroethene (TCE) and tetrachloroethene (PCE), as the problem contaminants within the overburden and bedrock aquifers of this area. As noted in the Dames & Moore report of 1987, the Zell manufacturing activities which previously occurred in both the Zell buildings currently occupied by Pitney Bowes and what is now the Elinco building included metal plating operations and the manufacture of electronic components. Soil borings and soil gas samples taken from Zell's former processing areas in the three buildings have showed anomalously high concentrations of TCE. In addition, Elinco has a documented history of extensive TCE usage and associated potential release, and their property is suspected to be a possible local source of TCE contamination. The presence of a degreasing pit near the northeast corner of the Elinco plant supports this conclusion. Both TCE (specific gravity = 1.47) and PCE (specific gravity = 1.62) are "sinkers;" therefore, the direct downward migration of TCE and PCE from the degreasing pit in the Elinco plant is most likely the highest source term of this contamination. TCE and PCE would infiltrate the shallow overburden ground water at this location and continue to migrate downward to the bedrock surface, where it could pass through fractures or move



along the bedrock surface gradient. Additionally, TCE & PCE could migrate from the degreasing pit in the Elinco building with surface water (storm water or "washings" from degreasing operations) and/or by entering floor drains.

Both compounds, TCE and PCE, prefer to exist as themselves, liquid or gas phase, rather than in solution, because their solubilities are low (0.01 and 0.002 parts per 100, respectively). Within ground water, these compounds migrate downward through the water column until they reach a physical barrier to their movement or the density of the host ground water-mineral solution matches their own. A common mode of TCE and PCE migration is to "coat" a pathway along a topographic or bedrock surface gradient and to continue to migrate along this route, "pooling" in depressions and sinking through fractures along the way. Flat locations on the bedrock surface and horizontal planes within bedrock fractures which are positioned so that in the downgradient flow direction they have a slight "upward" slope are common collection points for the dense non-aqueous phase liquid (DNAPL) TCE.

The contour diagrams of the overburden and bedrock surfaces (Figures 4 & 5, Table 1) clearly show that components of both the topographic and bedrock surface gradients are from the northeast corner of the Elinco building toward the Zell 1 building occupied by Pitney-Bowes. Specifically, the bedrock slab beneath the Elinco building slopes toward the positions of monitor wells K-11 and MW-100. The downgradient migration of TCE in product form or as a constituent of the ground water in both the overburden and bedrock may have substantially contributed to the TCE contamination present in the soil, ground water and soil gas beneath the Zell buildings leased by Pitney-Bowes.

TCE is lighter and five times more soluble than PCE, it tends to migrate with surface and ground water much more easily than PCE. Additionally, PCE has a pronounced affinity for the fine grained (silt and clay) fraction of sediments, which are less prevalent within the overburden than coarse-grained material (Appendix A).

#### 2.4.2 Hydrogeology and Contaminant Migration in the Overburden

As discussed in section 2.3, the overburden is dominated by very coarse-grained, unconsolidated glacial sediments. These sediments generally exhibit very high porosity and permeability as well, which makes this unit quite conductive to ground water

movement in both the vertical and horizontal flow directions. Aquifer test data provided to Dames & Moore by U.S. EPA-Region 1 corroborate the information present in the boring logs and confirm the generally high hydraulic conductivity of the overburden aquifer (Appendix G). In addition, Dames & Moore's well sampling logs indicate that water levels in the overburden wells varied by as much as four (4) feet between sampling events (Appendix E), which suggests a strong meteorological influence on the hydrogeology of this aquifer. The variability of the water table in the overburden suggests a relatively short residence time for the ground water in this zone, which behaves much like a surface water body in that its volume responds rapidly to meteorological events and its flow characteristics are related more to the pumpage at the Kellogg-Deering Well Field, the geometry of the bedrock surface and the geomorphology of the overburden than simply to the area's topography. Water table contour maps based on the water level data from two rounds sampling discussed by NUS (4/27/88 and 5/2/88; Appendix F) are presented in Figures 6 and 7, respectively. Ground water movement within the overburden appears to have two major components of flow:

- Horizontal-flow responds to the horizontal component of the hydraulic gradient related to the surface elevations of the water table and to pathway control from the geometry of the bedrock and overburden surfaces. The boring logs show the majority of the overburden material to be coarse-grained and to contain no dominant confining layers throughout the site. Therefore, stratigraphic control on ground water flow appears to be less important than the influence from the bedrock surface relief (Figure 5) and the geomorphology of the overburden. The direction of the horizontal component of the hydraulic gradient spreads radially (southwest to northwest) from the complex:
- Vertical-flow in response to gravity and the vertical component of the hydraulic gradient. Coarse material in the overburden provides little resistance to the downward infiltration of ground water through the overburden to the bedrock aquifer; overburden sediments also provide an efficient pathway for the injection of surface water into this zone from storms. In addition, the overburden is as little as 5.5 feet thick in some locations, allowing nearly direct transfer of storm water/runoff between the overburden surface and the bedrock at these

locations. Since TCE and PCE are both "sinkers", this mechanism is an important source of contamination to the bedrock aquifer. The aquifer test data in Appendix G suggest the vertical components of the hydraulic gradients present in the overburden aquifer are negligible.

#### 2.4.3 Hydrogeology and Contaminant Migration in the Bedrock

As discussed in section 2.3, the bedrock consists primarily of felsic and feldspathic gneiss and granite. Geochemical data from soil borings provided to Dames & Moore by U.S. EPA-Region 1 show significant concentrations of the common feldspar cations (sodium ( $\text{Na}^+$ ), calcium ( $\text{Ca}^{+2}$ ) and potassium ( $\text{K}^+$ )), which supports the observations of the EPA contractors' field geologists. Information contained in the soil boring logs provided to Dames & Moore suggest that the bedrock is appreciably fractured in the uppermost sections and gains structural integrity with depth.

Aquifer test data provided to Dames & Moore by U.S. EPA-Region 1 show that the ground water flow in the bedrock aquifer is complex. However, a review of the information made available to Dames & Moore and the application of basic geologic principles shows that the ground water within the bedrock has two major components of flow:

- *Horizontal-flow responds to the horizontal component of the hydraulic gradient related to the pressure gradient and the surface elevations of the water table and to pathway control from the network of fissures and fractures present within the bedrock. Pressure gradients within the bedrock aquifer would respond in part to the local differentials in the downward force due to the weight of the overburden and from communication with the ground water in the overburden. The aquifer test data presented in Appendix G suggest that the bedrock aquifer is fairly well confined. The regional horizontal flow of the bedrock aquifer is thought to be in the radial direction (southwest to northwest) of the regional drainage system; however, the full effects of the pumping of the overburden aquifer for drinking water purposes on the horizontal movement of the bedrock aquifer is not fully understood.*

- Vertical--flow in response to gravity and the vertical component of the hydraulic gradient. Stress from the weight of the overburden adds to the pressure head in the bedrock aquifer. Aquifer test data provided in Appendix G suggest that the importance of the vertical components of the hydraulic gradient is important in some locations. Ground water can move downward until it reaches locations where the bedrock integrity is high and prevents its further migration, or where another less conductive geologic unit is encountered and complicates or blocks flow. Large-scale structural features such as faults, complex regional jointing and massive intrusions dikes, sills, plutons etc., would also complicate the vertical migration of deep ground water. Since TCE and PCE are both "sinkers", their migration through the bedrock aquifer is primarily driven by gravity, with pathway control from the system of fissures and fractures.

Both a short term and long term tracer study were conducted in an attempt to resolve the flow characteristics of the bedrock aquifer--the results of these studies are discussed in section 2.7.

## 2.5 CHARACTERISTICS OF GROUND WATER CONTAMINATION

TCE contamination was first discovered in the Kellogg-Deering Well Field in 1975. Ground water sampling on the Zell/Elinco complex began in 1985. Analytical data and figures developed by NUS (the U.S. EPA-Region 1 contractor) and provided to Dames & Moore support the deduction that the degreasing pit in the Elinco building is a likely local source of soil, soil gas and ground water TCE contamination (Figure 1-3; Ebasco, 1989 [1]). The chlorinated solvent degreasing pit was previously used by Zell Products Corporation and is enclosed in a concrete vault within the northeast section of the Elinco building. Elinco currently has a degreasing pit which is located in the southeast corner of the former Zell processing area (Figure 1-3; Ebasco, 1989 [1]). This degreasing pit/former processing area is the single most likely high-volume source term of TCE contamination located on the Zell/Elinco complex.

Former process areas operated by the Zell Products Corporation which were located in Zell 1 and Zell 2 were filled in and capped with concrete by Pitney Bowes when

they leased and moved into these buildings in 1969 and 1974, respectively. Pitney Bowes has not conducted processing or manufacturing activities in these buildings, which have been used for storage, office space and a research and development laboratory. Therefore, the presence of solvent contamination in the soil, soil gas and ground water in and around Zell 1 and Zell 2 could not be related to Pitney Bowes' operations.

TCE is known to readily permeate and corrode concrete, and the concrete trenches associated with the former process area and degreasing pit in the Elinco building were "observed to be deteriorated, apparently from past chemical attack" (Ebasco, 1989 [1]). Therefore, the total volume of TCE likely to have leaked from the degreasing pit to the soil and ground water below the complex may have been substantial. In addition, the previous hydrogeologic investigations of the Zell/Elinco complex have shown a historical problem of VOCs within the ground water. The previous operations of the Zell Products Corporation, (which occupied the three "complex" buildings), are suspected to have included the use of large amounts of chlorinated solvents in their metal plating and degreasing operations.

Although a discussion of contaminant migration within the context of the area's topography, geology and hydrogeologic character is included in the Final Supplemental RI/FS report prepared by Ebasco for U.S. EPA Region 1, the hydrogeologic data are discussed on a regional scale: the Zell/Elinco complex, (as opposed to the degreasing pit and/or TCE tank), is identified as the source of the TCE contamination of the Kellogg-Deering Well field Superfund Site. Dames & Moore has reviewed and interpreted a portion of this data on a finer scale as it applies to the complex, which suggests that the degreasing pit is the most likely source of the TCE contamination. The quantity of TCE in the study area ground water (in the aqueous phase alone) was calculated by Ebasco to be 19,400 pounds; this is equivalent to 1,663 gallons of "free product" (DNAPL) TCE. Since the presence of DNAPL TCE in the subsurface of the source area is highly likely, and as the concentrations of this low-solubility chemical in the ground water below the "complex" have not decreased since monitoring began in 1985, the total quantity of TCE in the ground water in the study area may be many times that estimated by Ebasco. In order to have dissolved such a significant quantity of TCE (19,400 pounds/1,663 gallons) in the site's ground water, a location within the source area must have released at least this quantity of TCE over a period of time long enough to produce the concentrations of aqueous TCE observed to persist in the area.. This observation further

suggests that the principal source of TCE contamination that has originated from the "complex" is the degreasing pit in the Elinco building. This deduction is likely for two reasons:

#### Migration Potential and Pathway:

- The degreasing pit was reported by HRP Associates to be contained enclosed "in a concrete vault-like structure" (HRP, 1984?) TCE is known to readily permeate and corrode concrete, and could pass as free product through the base of the concrete vault under the influence of gravity. Deterioration of the concrete vault around and below the former Zell processing area may provide a significant pathway for the continued release of TCE from Elinco's degreasing pit (Ebasco, 1989 [1]). Bedrock is shallow beneath the degreasing pit at Elinco, and dips significantly in a radial pattern to the north-northwest and west (HRP, 1984?; Dames & Moore, 1989-Figure 5). Therefore, there is a significant component of the bedrock surface gradient from the degreasing pit in the direction of the highly contaminated wells in the source area and downgradient area (Figure 5, Figures 10A-D).

#### Quantity of TCE vs. Time:

- The estimated quantity of TCE within the source area alone (18,200 pounds/1560 gallons) necessitates a source capable of providing at least this amount of TCE over a long enough period of time to permit its solution. The degreasing pit located in the northeast corner of the Elinco building is the only current potential source of this volume of TCE identified within the "complex." Continued release of TCE from this area through the corroded concrete vault may explain the persistent levels of TCE in the ground water below the "complex."

Information regarding the contamination of both the overburden and bedrock ground water by TCE, PCE and other VOCs is discussed in the following sections within the context of the information presented above in sections 2.2 through 2.4. Information on ground water conditions in the area which were obtained from previous U.S. EPA-Region 1 investigations, and which were subsequently made available to Dames & Moore, is discussed and compared to the most recent data. A more detailed discussion of the

1988 ground water data from both samples collected by Dames & Moore and samples split with the U.S. EPA-Region 1 contractor, NUS, is presented in sections 2.6.4 and 3.1.

#### 2.5.1 Ground Water Contamination in the Overburden

In addition to the ground water data collected since 1985, a shallow (1'-3' deep) soil boring sample obtained from the Elinco property in 1988 (discussed in section 2.6.3) showed concentrations of TCE more than two times greater than in a similar sample obtained from K-19B on the property leased by Pitney Bowes; this further suggests that the principal source of TCE within the "complex" is the degreasing pit on the Elinco property.

Figures 10A and 10B show the approximate concentration contours of TCE in the overburden ground water for 1985 and 1988, respectively. These Figures were constructed using the ground water data obtained by NUS during the supplemental RI/FS (Ebasco, 1989 [1]). As shown in Figures 10A and 10B, the elongated area of highly contaminated ground water in the overburden generally follows the dip of the bedrock surface from the northeast corner of the Elinco building (degreasing pit) to the northwest corner of Zell 1 (location of MW-101).

The highest concentrations of TCE in the overburden have consistently occurred in the downgradient well MW-101 (23,000 ppb--Dames & Moore). MW-101 occupies the second lowest downgradient position on the bedrock surface, and is screened approximately 30 feet lower than the bedrock surface below the degreasing pit in the Elinco building (Figure 5; Figure 9). Although overburden well K-11 is both closer to the Elinco property and 5 feet lower on the bedrock surface, TCE was detected at much lower concentrations (3,700 ppb--Dames & Moore) in 1988. These data might suggest that MW-101 is in more direct communication with the subsurface below the degreasing pit than K-11 and/or the thickness of the overburden (38.5 feet) and the local stratigraphy limits hydrologic communication between the degreasing pit and K-11. However, additional data provided to Dames & Moore by U.S. EPA from the field laboratory analysis of ground water from K-11 showed substantially higher concentrations of TCE and PCE (Appendix E). Overburden wells MW-104 (1,300 ppb--Dames & Moore) and K-19A (120 ppb--Dames & Moore) showed considerably lower concentrations of TCE. Although information available to Dames & Moore on the previous

ground water sampling events is incomplete, TCE concentrations in overburden wells MW-104 (2,200-3,342 ppb-NUS) and MW-101 (16,000-37,660-NUS) reported by NUS in 1985 are consistent with the most recent data.

The 1988 ground water data obtained by Dames & Moore is provided in Tables 3 through 6 and a more detailed discussion of this data is presented in sections 2.6.4 and 3.1 of this report.

### 2.5.2 Ground Water Contamination In the Bedrock

As discussed in the previous sections 2.4.2, 2.4.3, 2.5 and 2.5.1, the thin veneer of overburden material above the bedrock well locations near the Elinco building (MW-1, MW-2, MW-3) would permit the recurrent and relatively rapid downward infiltration of TCE and PCE and the downward vertical migration of DNAPL and contaminated ground water from the overburden to the bedrock aquifer. Therefore, it is logical that from 1985 until recently, the highest concentrations of TCE in the bedrock ground water have occurred in MW-3 (Figure 10C), adjacent to the northwest corner of the Elinco building and close to the location of the degreasing pit ( ~100' west-southwest and ~10' lower along the bedrock surface; Figure 5). However, bedrock ground water data from the 1988 sampling events (Figure 10D) indicate that the center of the plume of TCE contamination appears to have migrated from MW-3 to MW-100, "downhill" approximately 20' along the bedrock surface (Figure 5). This conclusion is supported by two separate but related observations:

- TCE concentrations in MW-3 have decreased from as high as 73,950 ppb (NUS) in December, 1985 to as low as 53,000 ppb (NUS) in 1988;
- TCE concentrations in MW-100 have increased from as low as 30,000 ppb (NUS) in November, 1985 to as high as 170,000 (NUS) ppb in 1988.



## 2.6 MONITORING WELL INSTALLATION & SAMPLING/ SOIL BORING PROGRAM

### 2.6.1 Activities

Two (2) soil samples were collected from monitoring well K-19B which is located between the Zell 1 and Zell 2.

Four (4) ground water samples were collected from the monitoring wells surrounding the Zell Building (K-11, MW-100, K-19A, K-19B). These samples were collected on March 24, 1988 and the samples were split with NUS Corporation (EPA Contractor). On March 29, 1989 and December 23, 1989, ground water samples were collected from wells MW-100, MW-101, MW-104 and MW-105 before and after the interceptor well pumping test conducted by Fuss & O'Neil: this data is discussed in Section 3.

### 2.6.2 Results of Soil Samples Collected from K-19B

The soil samples were collected from monitoring well K-19B which is located between the Zell Buildings. The samples were collected over (2) intervals .5-2.5 feet and 5-7 feet below the surface and analyzed for the parameters on the Target Compound List. The results of the analyses are summarized in Tables 2 and 3. The following is a summary of the results. In addition, results of the analysis of a surface soil sample adjacent to the Elinco building are presented and discussed.

1. Inorganic Parameters - The values for most metals (Table 3) were within the typical background concentrations of metals in soils (Booz-Allen & Hamilton, 1983). Values reported for certain metals such as arsenic, barium, vanadium and zinc and are most likely related to the former metal plating operations conducted by Zell Products Corporation. Pitney Bowes has not conducted processing or manufacturing activities in Zell 2, which has been used since 1974 for engineering office space and a research and development laboratory. Therefore, the presence of these metals can not be related to Pitney Bowes' operations.

2. Volatile Organics - Detectable amounts of acetone, methylene chloride and two phthalates (Di-n-butylphthalate and bis (z-ethylhexyl) phthalate) were found in the soil samples and in the laboratory blank. These compounds are common laboratory reagents and are not suspected to be related to the area's ground water contamination;

- Detectable amounts of 1,2-Dichloroethene (15 ppb), TCE (207 ppb) and PCE (110 ppb) were found in the shallow soil sample. These compounds were not detected in the deeper soil samples, which suggests that their presence in the shallow sample is probably related to the former plating operations of Zell Products Corporation: this location is believed to be near a former unlined metal processing pit operated by Zell. Pitney Bowes filled in and paved the former processing and pit areas prior to moving into the building and has not conducted operations at Zell 2 which could have resulted in this contamination;
- The remaining nine (9) compounds detected are Polynuclear Aromatic Hydrocarbons (PAH). PAH's were not detected in the deeper sample. These compounds are generally associated with coal tars and asphalt, and may be related to the downward infiltration of surface water through the parking lot. These data are provided in Table 2.

### 2.6.3 Results of Soil Samples Collected from the Elinco Property

The analytical results of soil boring ESB-1 (NUS corporation) from the Elinco property in 1988 were made available to Dames & Moore, and are summarized below:

1. Inorganic Parameters -The detected values for most metals were comparable to those from the 1-3' sample from K-19B.
2. Volatile Organics - Detectable amounts of acetone and methylene chloride were found in the soil samples and in the laboratory blank. These compounds are common laboratory reagents and are not suspected to be

related to the local ground water contamination. A breakdown of the pertinent VOC data is as follows;

- Trichloroethene (TCE) and Tetrachloroethene (PCE) were found at concentrations of 510 ppb and 97 ppb, respectively, in the shallow soil sample. This concentration of TCE is over twice as high as that found in K-19B. Unfortunately, deeper soil samples were not obtained from ESB-1.

The analytical results of a soil sample taken from a previous boring from within the Elinco building showed TCE concentrations an order of magnitude higher (2,000 ppb) than those detected in ESB-1: this data was reported by U.S. EPA (Appendix F).

#### 2.6.4 Results of Ground Water Samples Collected

The first round of ground water samples were collected on March 24, 1988 by York Laboratories (Dames & Moore Contractor). These samples were split with NUS corporation (EPA Contractor). The analytical results of the sampling are presented in Table 3. The following is a summary of the results.

- Well location K-11 (overburden): detectable concentrations (0-120 ppb range) of 1,1,1-trichloroethane (TCA) and tetrachloroethene (PCE) were found in the ground water sample of both splits. Trichloroethene (TCE) concentrations of 1000-5500 ppb were also found in the sample. TCE is a potential breakdown product of PCE, one of the original contaminants suspected on site. As seen in Figures 5, K-11 is downgradient along the bedrock surface (Figure 5) of the suspected source of contamination (the degreasing pit on the Elinco property). Although TCE concentrations have decreased in the ground water from this overburden well since ground water sampling began in 1985, TCE concentrations have increased in MW-100 during this period; MW-100 is a bedrock well adjacent to the overburden well K-11 (Figure 9). This observation may indicate that contaminated overburden ground water and/or DNAPL TCE is infiltrating the bedrock aquifer through a fracture in the bedrock surface at this location.

- Well location MW-100 (bedrock): low concentrations of PCE and 1,2-dichloroethene were found in both of the split samples. Elevated levels of TCE (up to 170,000 ppb-NUS) were also reported in both analyses of the split sample. This well is located within a few feet of K-11 and monitors the bedrock water aquifer (Figure 9). TCE concentrations in the ground water from this bedrock well have increased since ground water analyses began in 1985, which indicates that the center of the VOC contaminant plume has migrated downward along the bedrock surface from MW-3 on the Elinco property (Figure 10C, Figure 10D).
- Well location K-19A (overburden): detectable concentrations of TCE, PCE and 1,1,1-trichloroethane were found in the ground water samples analyzed by York Laboratories and NUS Corporation. Detectable amounts of these compounds were reported in the overburden soil samples collected from the installation of monitor well K-19B. As previously discussed, these contaminants can not be related to Pitney Bowes' operations.
- Well location K-19B (bedrock): PCE was found at a concentration of 9,700 ppb and elevated levels of TCE (47,000) were detected in the ground water sample analyzed by York Laboratories. PCE and TCE were detected at high levels (17,000 and 130,000 ppb, respectively) in the sample analyzed by NUS Corporation. These anomalous concentrations are probably related to the former metal processing area operated by Zell Products Corporation, and can not be related to Pitney Bowes' operations.

## 2.7 TRACER STUDY

### 2.7.1 Background

The objective of the tracer study was to estimate the flow rate, direction and dispersion characteristics of the ground water and the contaminants it contains within the region encompassing the Pitney Bowes/Elinco complex. A short term tracer study was initiated on April 11, 1988 and a long term study began on April 12, 1988.

### 2.7.2 Short Term Tracer Study

The short term test utilized a sodium chloride (NaCl) ionic tracer solution. Chloride, like bromide, is a conservative tracer which poses no health risk in ground water. Two different ions were used to facilitate the segregation and interpretation of the data from the two separate tracer studies.

To initiate the short term test, a slug consisting of 50 gallons of a 10,000 mg/l solution of NaCl was injected over a 45 minute period into monitoring well MW-1. Injection of the chloride was performed on April 11, 1988. After injection, monitoring well MW-100, MW-103 and MW-104 were sampled on a near daily basis through May 11, 1988 and analyzed for the presence of the chloride ion. After May 11, the sampling interval was reduced to once a week until June 2, 1988, when the short term tracer study was terminated.

The results of the short term tracer study are presented in Table 7. The four monitoring wells sampled periodically did not show a significant increase of the chloride ion concentrations in the ground water. In fact, no observable trend in chloride concentration appears to have occurred in these wells over the course of this study. As discussed in section 2.3, the felspar cations sodium ( $\text{Na}^{+1}$ ), calcium ( $\text{Ca}^{+2}$ ) and potassium ( $\text{K}^{+1}$ ) are present in great abundance in the overburden soils and bedrock. These conditions are ideal to scavenge the chloride ion from solution, and therefore greatly complicate the behavior of chloride in this aquifer. Therefore, Dames & Moore views the short term tracer study to be inconclusive, and to provide no additional information as to the hydrogeologic conditions of the site.

### 2.7.3 Long Term Tracer Study

The long term study involved the injection of sodium bromide (NaBr) in the two upgradient bedrock wells near the Elinco Building, MW-1 and K-10.

One hundred gallons of 10,000 mg/l concentration NaBr solution was introduced into the ground water at both well K-10 and well MW-3. Several wells, including K-8, K-14, K-15, K-21, K-24 have been periodically sampled and analyzed for the presence of the bromide ion. The long term tracer study was discontinued on July 6, 1988. Results of the long term tracer study are included in Table 8 as well.

Results of the long term tracer are mostly inconclusive. Isolated anomalous concentrations of bromide were detected in bedrock wells located in directions ranging from north (K-20B, 7/19/88; K-9B, 8/09/88) to northwest (K-6B, 8/09/88; K-18B, 8/19/88) to southwest (K-8, 5/03/88), although no recognizable migration pattern is apparent (Table 8). However, not enough information has been made available to determine the hydrologic relationship of many of these well locations relative to the injector wells. Detectable levels of bromide were detected at least once in all of the wells monitored; however, the data does not show a clear trend in bromide concentration in the wells monitored (Table 8). As discussed in the previous sections, the geochemical conditions present within the bedrock (abundance of weathered feldspar and free sodium ( $\text{Na}^{+1}$ ), calcium ( $\text{Ca}^{+2}$ ) and potassium ( $\text{K}^{+1}$ )) are ideal to scavenge the bromide ion from solution, and therefore greatly complicate the behavior of bromide in this aquifer. Therefore, in Dames & Moore's professional opinion, a tracer study using a conservative (*non-interactive*) tracer compound relative to the region's geochemistry would provide more useful information on the flow characteristics of the bedrock aquifer.

### 3.0 GROUND WATER SAMPLING BEFORE AND AFTER INTERCEPTOR WELL PUMP TEST

#### 3.1 ACTIVITIES

Ground water sampling before and after the interceptor well pump test performed by Fuss & O'Neil was conducted to evaluate whether the ground water pumping from the interceptor well has induced contamination under the area surrounding the Zell Building presently leased by Pitney Bowes.

##### 3.1.1 Results of Ground Water Sampling Before Pump Test

Ground water sampling The Fuss & O'Neil pump test was conducted on March 29, 1988. These results are presented in Table 5. The following is a summary of the results:

- Well location MW-101 (overburden) - Elevated concentrations (23,000 ppb) of TCE and a lower concentration of 1,2-dichloroethene (1,400 ppb) were found in the ground water sample. These concentrations are higher (by one order of magnitude) than those reported in ground water samples from the other overburden wells on site (Figure 10A, Figure 10B). This elevated level of TCE is most likely related to the collection of TCE along the bedrock surface at this location, as it is one of the lowest bedrock surface points sampled downdip of the degreasing pit on the Elinco property. Figure 5 depicts the bedrock surface relief of the Elinco-Zell "complex," and illustrates the relative locations of the site's wells with respect to one another and MSL.
- Well location MW-104 (overburden) - Detectable amounts of PCE (62 ppb) and 1,1,1-trichloroethane (34 ppb) were reported in this ground water sample. TCE (1,300 ppb) was also found in this sample. Since the beginning of ground water analyses in 1985, TCE levels have decreased in this well;
- Well location MW-100 (bedrock) - PCE (5,100 ppb) and 1,2-dichloroethene (4,800 ppb) were found in the sample. Elevated levels of

TCE (140,000 ppb) were also found in the sample. This well monitors the bedrock zone and is located within a few feet of K-11, an overburden well. TCE concentrations in the ground water of this bedrock have increased since ground water analyses were started in 1985. This suggests that TCE contamination is continuing to migrate downdip from the degreasing pit in Elinco building along the bedrock surface. MW-100 is screened below the lowest point on the site in terms of bedrock surface elevation (Figure 9);

- Well location MW-105 (bedrock) - no contaminants were detected in the ground water sample collected at this location. These results are consistent with past analytical results.

### 3.1.2 Results of Ground Water Sampling After Pump Test

On December 23, 1988, after Fuss & O'Neil completed pumping the interceptor well on the Pitney Bowes property, ground water samples were collected by Dames & Moore from three monitoring wells (MW-101, MW-104, MW-105) surrounding the Pitney Bowes building. A fourth well (MW-100), which was proposed to be sampled, was locked and inaccessible to field personnel. A discussion of these analyses are summarized below and are presented in Tables 5 and 6.

- Well location MW-101 (overburden) - an elevated concentration (18,000 ppb) of TCE and lower concentrations of 1,2-dichloroethene (2,000 ppb) and acetone (2,500 ppb) were detected in the ground water sample.
- Well location MW-104 (overburden) - similar concentrations of TCE (1,400 ppb) and PCE (63 ppb) were detected after the pump test as compared to those before. This suggests that a source of TCE still exists at the "complex," (i.e. Elinco's degreasing pit and/or DNAPL below the "complex");
- Well location MW-105 (bedrock) - no elevated concentrations of solvents were detected in the ground water at this location. These results are consistent with past ground water data from this location.



Table 7 summarizes the December 23, 1988 analytical results along with the results of the March 29, 1988 sampling event. Upon comparison of the analytical results of the ground water sampling prior to and after completion of the Fuss & O'Neil field work, we can conclude that the concentrations of the contaminants in the ground water have more or less remained consistent. As previously mentioned, the pumping of the interceptor well appears to have no significant affect on the concentration of TCE present in the overburden ground water.

#### 4.0 SOIL GAS SURVEYS AND RESULTS

Soil gas surveys have recently become a common screening technique for volatile organic compounds in the shallow subsurface. Soil gas analysis is a relatively new tool which has only become commonplace in the past few years. Since there is a wide degree of variability in both the sampling and analytical methodologies employed by different soil gas investigators, soil gas surveys have so far been limited in usefulness to a screening technique for VOCs. The results of soil gas surveys are dependent on two major factors:

- The Henry's Law constant of each VOC analyte (i.e., the relative tendency of a compound to prefer to exist in the vapor phase rather than in the liquid or dissolved phase) and;
- The stratigraphic "tightness" of the soil unit at the sampling depth--this is dependent on the percentage of fine-grained material (silt, clay) present in the soil, the soil's moisture content and the soil's organic content. Therefore, a coarse-grained (high sand, gravel content), low-organic content, dry soil would be well suited for a soil gas survey, whereas a fine-grained (high silt, clay content), high-organic content, wet soil would be relatively "tight" and poorly suited for a soil gas survey. From the description of the geologic setting of the site presented in the previous sections, the Elinco-Zell building complex is well suited for a soil gas survey.

The soil gas surveys were performed by NUS corporation, the U.S. EPA-Region 1 contractor. Soil gas samples were generally taken at uniform depths of 2-3 feet below grade, except in locations where a depth profile was desired (Table 9A, Appendix F). After approximately ten (10) sample tube volumes of air were purged, soil gas samples were taken directly from the PVC sampling tube with a syringe and injected into a portable gas chromatograph for analysis. A description of the soil gas sampling and analysis work plan utilized by NUS is included in Appendix F.

Since the soil gas samples taken from the Elinco-Zell building complex were taken from shallow depths within the overburden, these samples represent the presence of the analyte VOCs within the unsaturated zone above the overburden ground water table.

In effect, the VOCs present in the soil gas samples represent a composite of both the vapor phase of the "product" which has entered the overburden directly via infiltration, and the vapor phase fraction of these VOCs which enters the unsaturated zone by volatilization from the dissolved VOCs in overburden ground water.

#### 4.1 INDOOR SOIL GAS SURVEY AND RESULTS

Soil gas samples were taken through the floors of the Elinco plant and Zell buildings by NUS corporation, the U.S. EPA-Region 1 contractor (Figure 11, Table 9B). This soil gas survey detected TCE, PCE and 1,1,1 TCA beneath the Elinco plant's degreasing pit and TCE beneath both Zell 1 and 2. A summary of the analytical results of the indoor soil gas survey is as follows:

1 ) Elinco plant: three soil gas samples (and one duplicate) were taken from beneath areas adjacent to the degreasing pit. TCE (4040 mg/M<sup>3</sup>), PCE (115 mg/M<sup>3</sup>) and 1,1,1 TCA (50 mg/M<sup>3</sup>) were detected in ESG-1 and its duplicate. This sample was taken adjacent to and north (downgradient) of the degreasing pit (Table 9B, Figure 11). ESG-2 and ESG-3 did not test positively for these parameters.:

2 ) Zell 1 and Zell 2:

Zell 1: three soil gas samples, and one duplicate, were taken from areas beneath Zell 1. These samples were only analyzed for TCE. TCE was detected in PSG-1 (6,233.6 mg/M<sup>3</sup>) and its duplicate PSG-1A (4,315.2 mg/M<sup>3</sup>), PSG-2 (4,029.2 mg/M<sup>3</sup>) and PSG-3 (3,945.5 mg/M<sup>3</sup>). PSG-1 is close to the west wall of Zell 1, near the location of wells K-11 and MW-100. PSG-2 and PSG-3 are located approximately 30 and 50 feet NNE of PSG-1, respectively (Figure 11).

Zell 2: two soil gas samples were taken from areas beneath Zell 2. These samples were only analyzed for TCE. TCE was detected in both PSG-4 (14,890.2 mg/M<sup>3</sup>) and PSG-5 (1595.4 mg/M<sup>3</sup>). These sample locations are along the west wall of Zell 2, and are directly downgradient of the degreasing pit near the NE corner of the Elinco

plant. PSG-5 and PSG-4 and within 30 and 75 feet of wells K-19A and K-19B, which are between Zell 1 and 2 (Figure 11).

The analytical results of the indoor soil gas survey data are consistent with the trends observed in the analytical data from the overburden ground water wells. Since the soil gas samples are drawn from the unsaturated zone directly above the overburden ground water table, the correlation with concentrations of TCE within the overburden ground water near these sample locations observed is understandable. While a comparison of the indoor soil gas survey data (Table 9A) and the overburden ground water data (Tables 4 through 7) generally support this correlation, supplementary analytical data from well K-19A (near PSG-4 & 5) and well K-11 (near PSG-1, 2 & 3) provided to Dames & Moore by U.S. EPA show a strong correlation between the concentrations of TCE in soil gas and shallow ground water (Appendix F). This correlation suggests that a substantial fraction of the VOCs detected in the soil gas of the unsaturated zone beneath the "complex" is due to vapor-phase volatilization of the VOCs in the shallow ground water.

#### 4.2 OUTDOOR SOIL GAS SURVEY AND RESULTS

Soil gas samples were taken by NUS corporation, the U.S. EPA-Region 1 contractor from the area surrounding the Elinco plant and the Zell buildings (Figure 11, Table 9A). Soil gas samples were analyzed for TCE, PCE, 1,1,1 TCA and selected petroleum hydrocarbons. TCE was detected in several samples in three general locations:

- Northeast corner of the Elinco plant near the degreasing pit: samples 19, 66, 67 and 68 showed between 23.9 and 320.1 mg/M<sup>3</sup> of TCE (Figure 11, Table 9A). Samples 54A, 54B, and 54C were taken as a soil gas depth profile near the northeast corner of the Elinco plant. This sample location is close to the former location of Elinco's aboveground TCE tank, and suggests surface contamination of soils at this location are related to this tank.
- Between Elinco and Zell Buildings: sample 94 (641.6 mg/M<sup>3</sup> of TCE and 14.0 mg/M<sup>3</sup> of 1,1,1 TCA) is located midway between the center of the north wall of the Elinco plant and the south wall of Zell 1. PCE was

detected in sample 39 (123.0 mg/M<sup>3</sup>), which is adjacent to the center of the west wall of the Elinco building and is close to the location of MW-3.

- Zell 1 & Zell 2: TCE was detected in several soil gas samples located between Zell 1 and 2 and PCE was also detected in two of these samples. Sample 56 showed both PCE and TCE (2206.5 mg/M<sup>3</sup> of PCE and 1121.1 mg/M<sup>3</sup> of TCE, respectively) at levels that correlate to the concentrations observed in K19-A. Sample 37 showed similar concentrations of PCE to TCE, (6640.0 mg/M<sup>3</sup> of PCE and 229 mg/M<sup>3</sup> of TCE, respectively). These sample locations are adjacent to one another, and both are close to the positions of wells K-19A and K-19B. Only TCE was detected in sample 38 (29.0 mg/M<sup>3</sup>), which is approximately 50 feet south of sample 37. VOCs were not detected in other samples taken between Zell 1 and 2. This information supports the findings of both the indoor soil gas survey and the ground water data, and suggests that the VOC contamination at these locations is residual due to a combination of the former metal processing operations of Zell Products Corporation and the upward migration of vapor phase PCE and TCE from the shallow ground water.

## 5.0 CONCLUSIONS

The findings of this report are based on the interpretation of analytical data provided to Dames & Moore by U.S. EPA-Region 1 which was obtained from samples of soil, soil gas and ground water from the Elinco-Zell building complex. Based on the conclusions of the the December 4, 1987 report prepared by Dames & Moore for Murtha, Cullina, Richer and Pinney together with the information presented in this report, the following conclusions regarding the site characteristics and contamination present at the Elinco-Zell building complex can be made:

- Pitney-Bowes does not store, use or dispose of significant quantities of TCE in The Zell Building complex located at 276 Main Street in Norwalk, CT, and did not have TCE on site until 1983-1984: their entire inventory of TCE since they have occupied the Zell building complex has amounted to approximately 56 gallons (one 1-gallon container was purchased in 1983 and one 55-gallon drum was purchased by Pitney Bowes in 1984).
- Elinco has a history of extensive TCE use and potential release. A degreasing pit located near the NE corner of the Elinco plant is the probable source of local TCE and PCE contamination.
- The Elinco plant degreasing operations area has the greatest degree of soil contamination by VOCs as reported by U.S. EPA.
- VOC contamination in the overburden ground water is the highest in downgradient well locations which occupy the lowest positions along the bedrock surface relative to MSL. Analytical data from the overburden wells show that aqueous TCE concentrations are remaining relatively constant in these wells (MW-101, K-11). This suggests that either the degreasing pit in the Elinco building is continuing to release TCE, (which subsequently migrates down the bedrock surface gradient), and/or that DNAPL TCE present in the bedrock fractures continues to dissolve and replace the aqueous TCE being removed by the natural flushing of the ground water below the "complex."

- The center of the plume of TCE contamination in the bedrock aquifer has apparently migrated from beneath and adjacent to the Elinco plant in 1985 (well MW-3; Figure 10C) to MW-100 in 1938 (Figure 10D).
- Data obtained from the indoor and outdoor soil gas surveys show TCE and PCE contamination both near the degreasing pit, and near both Zell 1 and Zell 2. The soil gas data shows a strong correlation with the overburden ground water data.
- The short term and long term tracer studies were inconclusive and do not resolve the ground water flow patterns within the bedrock aquifer.
- The pump test of a downgradient interceptor well on the Zell property leased by Pitney Bowes did not significantly affect the ground water quality within the bedrock aquifer. As previously discussed, this suggests that a source of TCE still exists at the "complex," (i.e. Elinco's degreasing pit and/or DNAPL below the "complex")
- Both TCE and PCE have been detected in ground water from monitor wells MW-101, MW-104, and K-11 and bedrock wells MW-100 and MW-3 near Zell Building 1 and the Elinco Building. A likely source of TCE and PCE is the degreasing operation near the northeast corner of the Elinco Building. From the Elinco degreasing operation, the probable pathways of migration are northward along the paved surface entering the ground near Zell Building 1 or directly into the subsurface near the Elinco Building and subsequent migration of TCE down the bedrock surface gradient.
- The Zell building complex located at 276 Main Street in Norwalk, CT, (currently occupied by Pitney Bowes) is downdip of the degreasing pit in the northeast corner of the Elinco plant in terms of the bedrock surface. TCE and PCE (both common degreasing solvents) have specific gravities of 1.47 and 1.62, respectively, and tend to "sink" downgradient under the influence of gravity; therefore, solvents in groundwater travel in a northwest-west direction from the subsurface below the degreasing pit toward and under the Zell Building 1, accumulating in bedrock surface depressions, sinking

through fractures, and continuing along the regional hydraulic gradient toward the Kellogg-Deering Well Field.

- The information presented in the Final Supplemental RI/FS report (Ebasco, 1989, [1]), Dames & Moore's 1987 report (Appendix H) and this report strongly support the conclusion that Pitney Bowes' operations in the Zell buildings 1 and 2 could not have contributed to the TCE contamination found in the Kellogg-Deering Well Field Superfund Site.



TABLE 1

## GEOPHYSICAL DATA

PITNEY BOWES, NORWALK

Well, Soil Boring #	Overburden Sfc. Elev.	Bedrock Sfc. Elev.	Overburden Thickness
K-11	78.45	39.95	38.50
MW-100	81.00	43.00	38.00
MW-101	80.00	44.10	35.90
K-18B*	77.29	56.79	20.50
K-18A*	77.56	57.56	20.00
K-19B	80.17	63.07	17.10
K-19A	80.20	63.70	16.50
MW-105	83.00	66.00	17.00
MW-106	83.00	66.50	16.50
MW-103	83.00	73.00	10.00
K-10*	82.16	73.16	9.00
MW-104	81.00	75.00	6.00
MW-3*	78.28	60.28	18.0
MW-2*	76.57	65.57	11.0
MW-102	101.00	95.00	6.00
MW-1*	105.23	99.73	5.50

\* Wells, soil borings on Elinco property

Elevations corrected to feet above MSL

Elevations of ground surface, bedrock sfc and calculations of overburden thickness based on these data were obtained from the boring logs & monitor well construction diagrams included in Appendix H.

TABLE 2  
VOLATILE ORGANIC ANALYSES  
SOIL BORING K-19B

VOLATILE ORGANICS PARAMETER	K-19B 0.5-2.5 DEPTH		K-19B 5.0-7.0 DEPTH		TRIP BLANK	
	CONC. (ppb)	FLAG	CONC. (ppb)	FLAG	CONC. (ppb)	FLAG
Acetone	64		76	B	16	B
Methylene Chloride	59		17	B	5	B
1,2 dichloroethene (Total)	15					
Trichloroethene	207					
Tetrachloroethene	110					
Di-n-butylphthalate	350	JB	610	JB		
Phenanthrene	570	J				
Fluoranthene	1100					
Pyrene	1100					
Benzo (a) anthracene	580	J				
Chrysene	680	J				
Bis(2-Ethylhexyl) phthalate	2700		340	J		
Benzo (b) fluoranthene	1300					
Benzo (a) pyrene	830	J				
Indeno (1, 2, 3-cd) pyrene	490	J				
Benzo (g, h, i) perylene	470	J				
Tentatively ID Compounds	22		15			

Notes on Lab Flags

B = Analyte was found in the blank as well as in the sample.

J = The concentration listed is an estimated value, which is less than the specified minimum detection limit but is greater than zero.

TABLE 3  
 INORGANIC ANALYSES  
 SOIL BORING K-19B

INORGANICS PARAMETER	K-19B 0.5-2.5 DEPTH		K-19B 5.0-7.0 DEPTH	
	CONC (ppb)	FLAG	CONC (ppb)	FLAG
Aluminum	20419.4		14205.9	
Antimony	19.4	UN	17.6	U
Arsenic	9.2	S	3	S
Barium	107.1		60.3	
Beryllium	1.3	B	0.9	B
Cadmium	1.6	U	1.5	U
Calcium	3225.8		1694.1	
Chromium	29		26.2	
Cobalt	13.9	B	10.9	B
Copper	31.3		15.9	
Iron	23515.1		18941.2	
Lead	9.7	N*	5.6	N*
Magnesium	5483.9		5323.5	
Manganese	658.1		368.2	
Mercury	0.2	C	0.2	C
Nickel	19.7		16.2	
Potassium	1190.3	B	1802.9	
Selenium	1.3	U	1.2	U
Silver	3.2	UN	2.9	UN
Sodium	645.2	U	588.2	U
Thallium	1.3	UN	1.2	UN
Vanadium	41.3	P	27.9	
Zinc	112.3	E	52.6	E
Cyanide	0.81	U	0.74	U

Notes on Lab Flags

- B = Analyte was found in the blank as well as in the sample.
- J = The concentration listed is an estimated value, which is less than the specified minimum detection limit but is greater than zero.
- U = Indicates compound was analyzed for but not detected.
- N = Indicates spike sample recovery is not within control limits.
- E = Indicates a value estimated or not reported due to the presence of interference.
- C = Pesticide parameter confirmed by GC/MS.
- \* = Indicates duplicate analysis is not within control limits.
- S = Indicates a value determined by method of standard addition.

**TABLE 4**  
**GROUND WATER ANALYTICAL DATA**  
 March 24, 1988

VOLATILE ORGANICS PARAMETERS (ppb)	K-11		MW-100		K-19A		K-19B		BLANK #1		BLANK #2	
	VALUE	FLAG	VALUE	FLAG	VALUE	FLAG	VALUE	FLAG	VALUE	FLAG	VALUE	FLAG
Methylene chloride	725 *	B	3,500 *	B	13	B	1,950 *	B	9.5		14.0	
Acetone	2,550 *	B	*		13	B	*		19.0		16.0	
1,1,1-trichloroethane	67	J	4,500		67	J						
Trichloroethene	3,700		130,000	J	120		47,000				4.5	J
1,2-dichloroethene (total)	61	J	5,200	J	13		4,700					

Notes on Lab Flags

B = Analyte was found in the blank as well as in the sample.

J = The concentration listed is an estimated value, which is less than the specified minimum detection limit but is greater than zero.

\* = denotes that value was blank subtracted

TABLE 5  
GROUND WATER ANALYTICAL DATA  
MARCH 29, 1988

EPA TCL VOLATILE ORGANICS PARAMETER	STATE OF CT ACTION LEVEL	MW-101		MW-104		MW-100		MW-105		TRIP BLANK	
		CONC. (ppb)	FLAG	CONC. (ppb)	FLAG	CONC. (ppb)	FLAG	CONC. (ppb)	FLAG	CONC. (ppb)	FLAG
Methylene chloride	25					*				8	JB
Acetone	NONE	*		*		*					
1,2-dichloroethene (total)	NONE	1,400				4,800	J				
1,1,1-trichloroethane				34	J						
Trichloroethene	25	23,000		1,300		140,000					
Tetrachloroethene	20			62		5,100					
Toluene	1,000			*		200*	JB				

Notes on Lab Flags

B = Analyte was found in the blank as well as in the sample.

J = The concentration listed is an estimated value, which is less than the specified minimum detection limit but is greater than zero.

\* = denotes that value was blank subtracted

TABLE 6  
GROUND WATER ANALYTICAL DATA  
DECEMBER 23, 1988

<u>VOLATILE ORGANICS PARAMETERS (ppb)</u>	<u>STATE OF CT ACTION LEVEL</u>	<u>MW-101</u>	<u>MW-104</u>	<u>MW-105</u>	<u>TRIP BLANK</u>
Methylene Chloride	25	U	U	U	U
Acetone	None	2,500	130	6J	6J
1,2 dichloroethene (total)	None	2,000	U	U	U
Trichloroethene (TCE)	25	18,000	1,400	U	U
Tetrachloroethene (PCE)	20	U	63	U	U
Toluene	1,000	150J	U	5	2J

U = The compound was analyzed for but not detected.

B = Analyte was found in the blank as well as in the sample.

J = The concentration listed is an estimated value, which is less than the specified minimum detection limit but is greater than zero.

TABLE 7  
GROUND WATER DATA COMPARISON  
ZELL BUILDING WELLS

VOLATILE ORGANICS PARAMETERS (ppb)	MW-101		MW-104		MW-105	
	12/23/88 - 3/29/88		12/23/88 - 3/29/88		12/23/88 - 3/29/88	
Methylene Chloride	U	U	U	U	U	U
Acetone	2,50J	B	130	B	6J	U
1,2 dichloroethene (Total)	2,000	1,400	U	U	U	U
Trichloroethane (TCA)	U	U	U	34	U	U
Trichloroethene (TCE)	18,000	23,000	1,400	1,300	U	U
Tetrachloroethene (PCE)	U	U	63	62	U	U
Toluene	150J	U	U	B	5	U

U = The compound was analyzed for but not detected.

B = Analyte was found in the blank as well as in the sample.

J = The concentration listed is an estimated value, which is less than the specified minimum detection limit but is greater than zero.

TABLE 8

SUPPLEMENTAL RI - TRACER TEST RESULT  
OBSERVATION WELLS  
KELLOGG DEERING WELL FIELD SITE  
NORWALK, CONNECTICUT

Date Sampled	Short term Tracer Test Observation Wells Chloride Concentration (mg/l)				Long term Tracer Test Observation Wells Bromide Concentration (mg/l)														
	MW 1 <sup>1</sup>	MW 100	MW-103	MW-104	MW-3 <sup>2</sup>	MW-102	K-3A	K-3B	K-6B	K-8	K-9B	K-11	K-14	K-15	K-18A	K-18B	K-20B	K-21	K-24
4/10/88	1.85	2.90	4.26	2.30	<0.79	--	<0.79	<0.79	--	<0.79	--	<0.79	--	<0.79	--	--	--	--	--
4/12/88	--	2.90	4.26	2.30	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--
4/13/88	--	2.98	4.26	2.70	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--
4/14/88	--	2.60	3.48	1.85	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--
4/15/88	--	2.40	--	1.78	--	--	<0.79	--	--	--	--	<0.79	--	<0.79	--	--	--	--	--
4/17/88	--	3.37	>3.55	2.30	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--
4/18/88	--	3.30	3.55	2.41	--	--	<0.79	<0.79	--	--	--	<0.79	--	<0.79	--	--	--	--	--
4/19/88	--	3.12	3.73	2.27	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--
4/20/88	--	3.20/3.41 <sup>4</sup>	3.90/3.72	2.20/2.49	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--
4/21/88	--	3.34/3.26	3.51/3.51	2.41/2.27	--	--	<0.79	<0.79	--	--	--	<0.79	--	<0.79	--	--	--	--	--
4/22/88	--	3.26	3.55	2.27	--	--	--	--	--	--	--	--	--	--	--	--	--	<0.79	--
4/24/88	--	3.83	3.72	2.13	--	--	<0.79	<0.79	--	--	--	<0.79	--	<0.79	--	--	--	--	--
4/25/88	--	2.52/2.41	3.72	1.74	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--
4/26/88	--	--	3.48	1.70	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--
4/27/88	--	2.66	3.48	1.78	>100	--	<0.79	<0.79	--	--	--	<0.79	--	<0.79	--	--	--	<0.79	--
4/28/88	--	3.49	3.62	1.92	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--
4/29/88	--	--	3.55	1.86	--	--	<0.79	<0.79	--	--	--	--	--	<0.79	--	--	--	<0.79	--
5/03/88	9.94	2.63	3.12	1.85	--	--	<0.79	--	--	14.6	--	--	<0.79	<0.79	--	--	--	<0.79	--
5/06/88	--	3.10	2.80	2.19	--	--	<0.79	--	--	<0.79	--	--	<0.79	<0.79	--	--	--	<0.79	<0.79
5/10/88	--	3.55	3.20	2.16	--	--	--	--	--	<0.79	--	--	<0.79	<0.79	--	--	--	<0.79	<0.79
5/19/88	--	2.41	1.60	1.80	--	--	--	--	--	<0.79	--	--	0.95	0.95	--	--	--	<0.79	<0.79
5/25/88	--	3.48	4.60	2.55	--	--	--	--	--	<0.79	--	--	<0.79	<0.79	--	--	--	<0.79	<0.79
6/02/88	--	2.55	4.26	2.67	--	--	--	--	--	<0.79	--	--	<0.79	<0.79	--	--	--	<0.79	<0.79
6/09/88	--	--	--	--	--	--	--	--	0.43	--	--	--	0.49	0.38	--	--	--	0.51	0.24
6/15/88	--	--	--	--	--	--	--	--	0.49	--	--	--	0.57	0.46	--	--	--	0.47	0.55
6/22/88	--	--	--	--	--	--	--	--	0.46	--	--	--	0.60	0.45	--	--	--	0.48	0.36
6/30/88	--	--	--	--	--	--	--	--	0.37	--	--	--	0.47	0.36	--	--	--	0.12	0.24
7/06/88	--	--	--	--	--	--	--	--	0.41	--	--	--	6.52	0.35	--	--	--	0.35	0.53
7/19/88	--	--	--	--	--	0.33	--	--	--	--	--	--	0.47	0.49	--	--	0.49	0.51	0.44
7/28/88	--	--	--	--	75.84	--	--	--	0.49	--	--	--	--	0.36	--	--	--	0.38	0.35
8/09/88	--	--	--	--	--	--	--	--	0.44	0.33	0.29	--	--	0.30	--	--	--	0.40	--
8/19/88	--	--	--	--	<7.9	--	--	--	0.29	--	--	--	--	0.27	0.62	1.03	--	0.37	0.31
8/29/88	--	--	--	--	--	--	--	--	0.32	--	--	--	--	0.26	--	--	--	0.34	0.18
9/08/88	--	--	--	--	--	--	--	--	0.32	--	--	--	--	0.32	--	--	--	0.23	0.33
9/20/88	--	--	--	--	--	--	--	--	0.69	--	--	--	--	0.62	--	--	--	0.47	0.87
9/29/88	--	--	--	--	--	--	--	--	0.32	--	--	--	--	0.32	--	--	--	0.39	0.38
10/26/88	--	--	--	--	--	--	--	--	0.21	--	--	--	--	0.12	--	--	--	0.17	0.15

1. Injection well MW 1; injection conducted on April 11, 1988.  
 2. Injection wells MW 3 and K-10 injection conducted from April 12 through April 14, 1988.  
 3. Background levels  
 4. Sampled twice on date indicated  
 5. Short term tracer study terminated after 6/02/88 samples were collected.



TABLE 9A

NUS Corporation Outdoor Soil Gas Survey Results					
Pitney Bowes-Elinco Complex					
Norwalk, Connecticut					
Soil Gas Concentrations					
in mg/M3					
Sample ID	Sample Location	PCE	TCE	1,1,1 TCA	Ethylbenzene
1	East of Zell 2				
2	East of Zell 2				
3	East of Zell 2				
4	East of Zell 2				
5	East of Zell 2				
6	East of Zell 2				
7	Southeast of Zell 2				
8	Southeast of Zell 2				
10A	S of Zell 2/N of Matheis				
11	S of Zell 2/N of Matheis				NA
11A	S of Zell 2/N of Matheis				
12	S of Zell 2/N of Matheis				NA
16	S of Zell 2/N of Matheis				NA
17	S of Zell 2/N of Matheis				NA
19	E of Elinco/N of Matheis		320.10		NA
20	SE corner of Zell 1				NA
21	East of Zell 1				NA
23	Between Elinco & Zell 1				
24	Between Elinco & Zell 1				
25	Between Elinco & Zell 1				
26	SW corner of Zell 1				12.00
27	West of Zell 1				
28	West of Zell 1				
29	West of Zell 1				
30	West of Zell 1				
31	West of Zell 1				
32	NW corner of Zell 1				
34	North of Zell 1				
35	NE corner of Zell 1				
36	Between Zell 1 & 2				
37	Between Zell 1 & 2	6640.00	229.00		
38	Between Zell 1 & 2		29.00		
39	West of Elinco	123.00			
40	West of Elinco				
41	SW corner of Elinco				
42	South of Elinco				
43	South of Elinco				
44	South of Elinco				
45	South of Elinco				
46	S of Elinco/W of Matheis				
47	S of Elinco/W of Matheis				
48	S of Elinco/W of Matheis				
50	SW corner of Matheis Ct.				
51	S of Matheis				

Sample locations are shown in Figure 6.

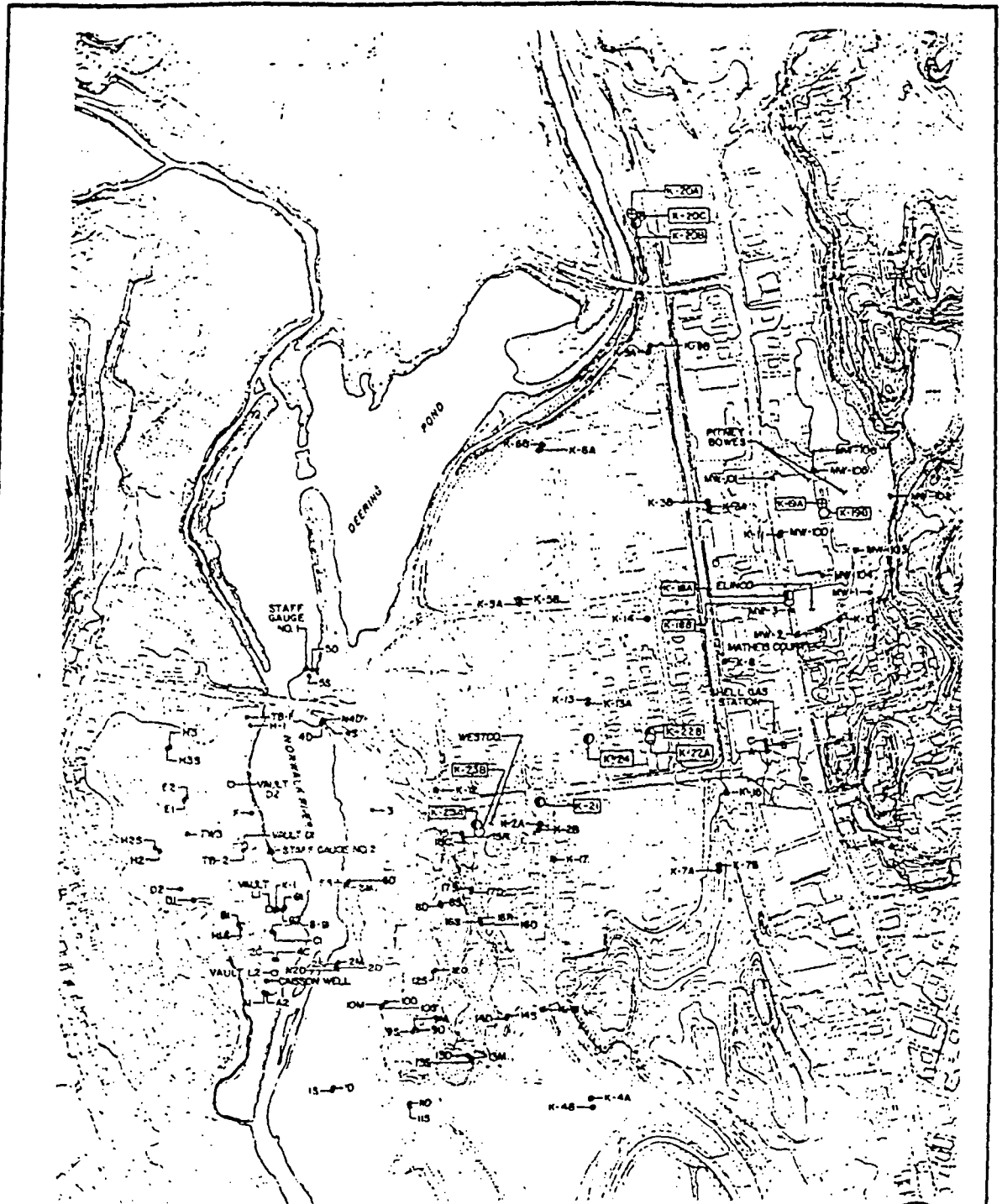
NA indicates that the soil gas sample was not analyzed for the parameter noted.

Most samples were analyzed for Benzene and Toluene, but neither compound was detected in the survey.

TABLE 9A

NUS Corporation Soil Gas Survey Results						
Pitney Bowes-Elinco Complex						
Norwalk, Connecticut						
Soil Gas Concentrations						
Sample ID	Sample Location	in mg/M3				
		PCE	TCE	1,1,1 TCA	Ethylbenzene	
5 2	SE corner of Matheis					
5 3	E of Matheis					
5 4	NE corner of Elinco					NA
5 4 A	NE corner of Elinco		797.00			
5 4 B	NE corner of Elinco		609.00			
5 4 C	NE corner of Elinco		489.00			
5 5	E of Zell 1					
5 6	Between Zell 1 & 2	2206.50	1121.10			
6 6	NE corner of Elinco		130.60			
6 7	NE corner of Elinco		23.90			
6 8	N of Elinco/S of Zell 1		78.90			
6 9	N of Elinco/S of Zell 1					
7 0	NE corner of Elinco					
7 1	North of Elinco					
7 2	North of Elinco					
8 0	N of Elinco/S of Zell 1					
8 8	S of Elinco/W of Matheis					
8 9	S of Elinco/W of Matheis					
9 0	S of Elinco/W of Matheis					
9 1	S of Elinco/W of Matheis					
9 2	S of Elinco/W of Matheis		21.80			
9 3	S of Elinco/W of Matheis					
9 4	Between Elinco & Zell 1		641.60		14.00	
9 5	West of Zell 1					
9 6	NW corner of Elinco					
9 7	West of Zell 1					
9 8	West of Zell 1					
9 9	Unknown					
1 0 0	SW corner of Zell 2					
1 0 1	E of Elinco/N of Matheis					
1 0 2	E of Elinco/N of Matheis					





0 300 600  
SCALE IN FEET

LEGEND

- ⊕ OVERBURDEN WELL
- ⊙ DEEP BEDROCK WELL
- SHALLOW BEDROCK WELL
- EXISTING M&S MONITORING WELL
- WELL TO BE SURVEYED
- EXISTING MONITORING WELL (BY OTHERS)
- EXISTING VAULT
- ▲ EXISTING STAFF GAUGE
- EXISTING CAISSON WELL

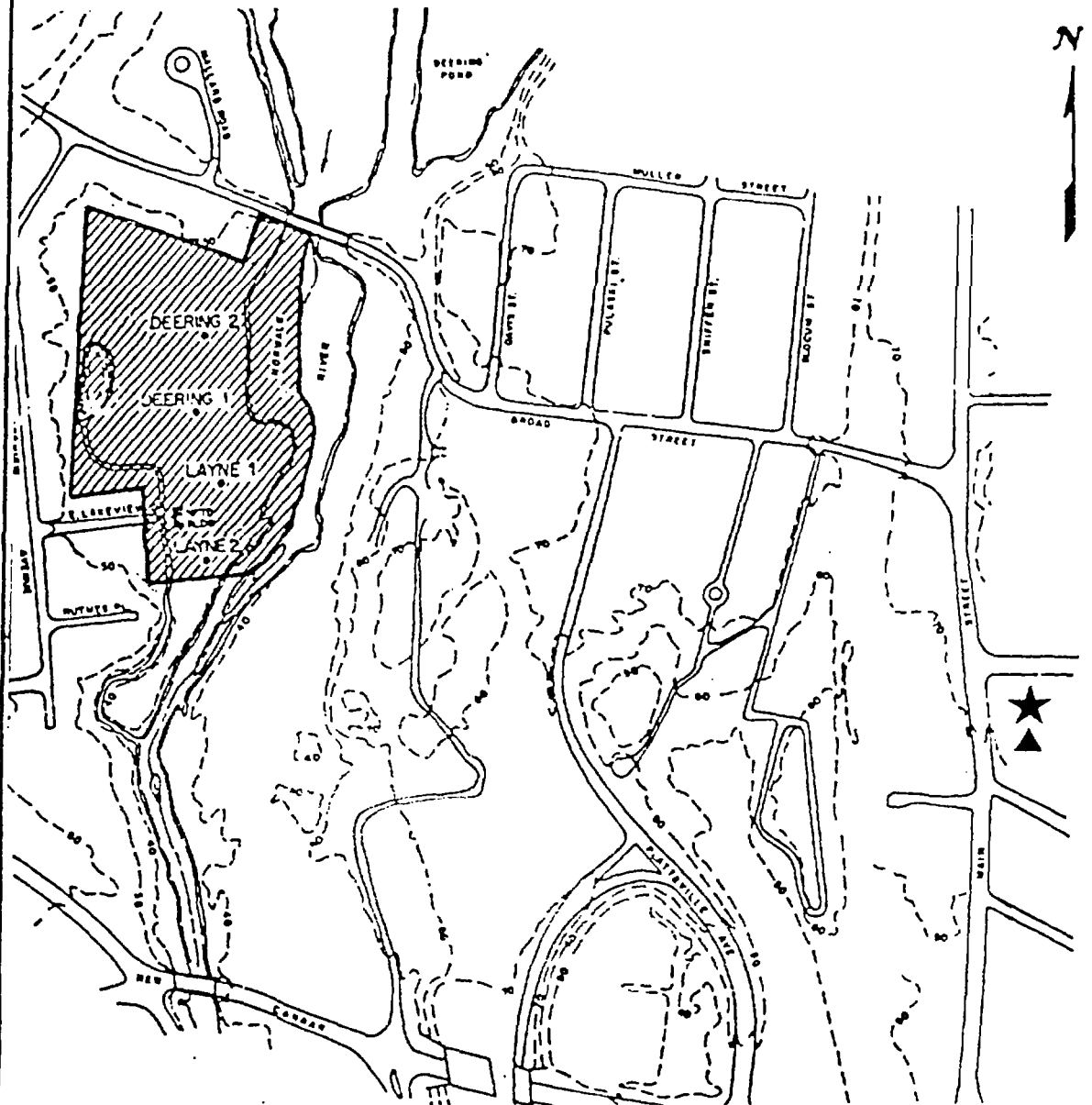


KELLOGG-DEERING WELDFIELD/VICINITY TOPOGRAPHIC MAP

NOTE: PROVIDED TO DAMES & MOORE BY U.S. EPA - REGION 1

Dames & Moore  
FIGURE 1

# SITE LOCATION MAP



KELLOGG-DEERING WELL FIELD



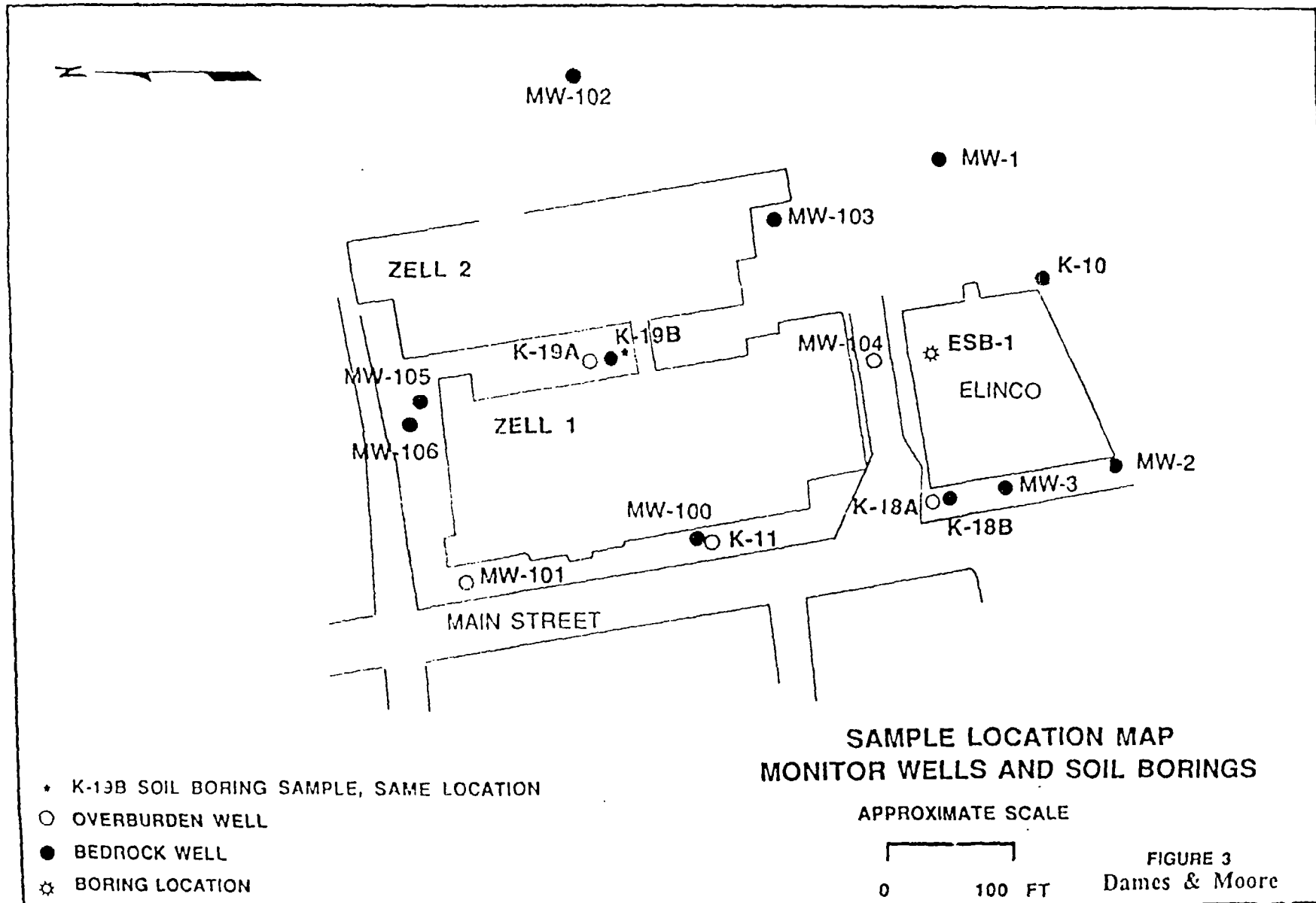
ZELL BUILDING

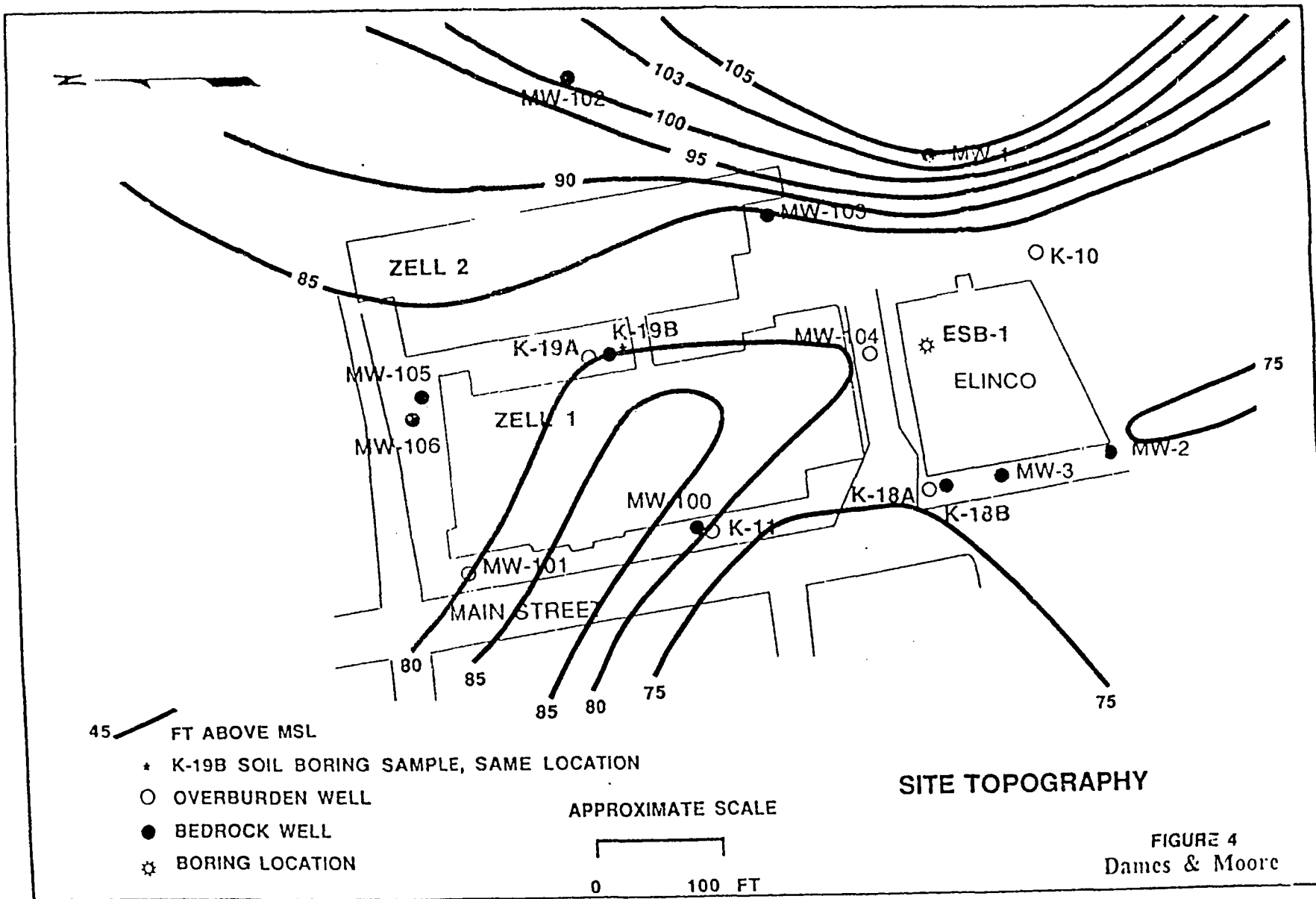


ELINCO PLANT

SCALE: 1" = 400'

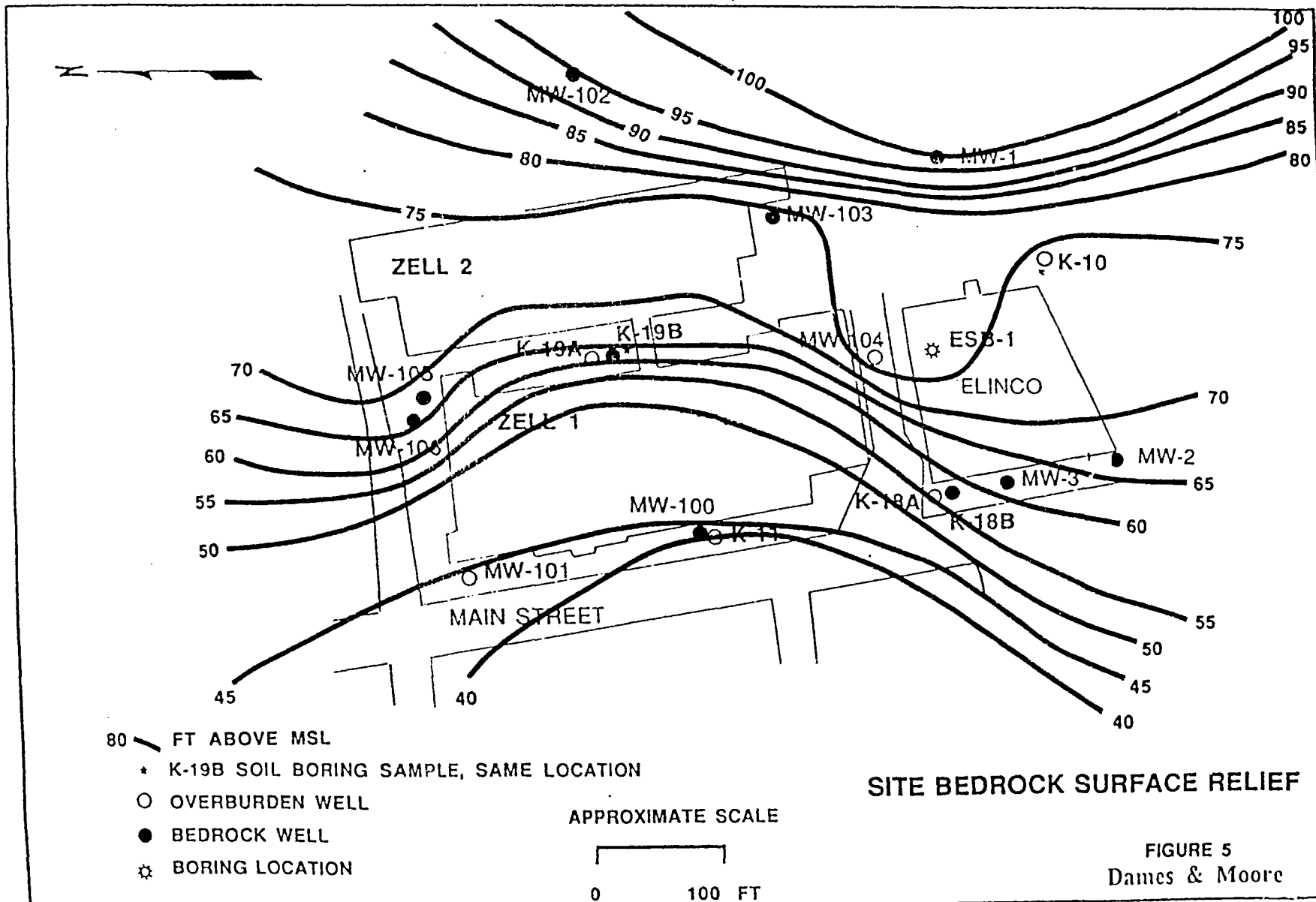
Dames & Moore  
FIGURE 2





**SITE TOPOGRAPHY**

**FIGURE 4**  
Dames & Moore



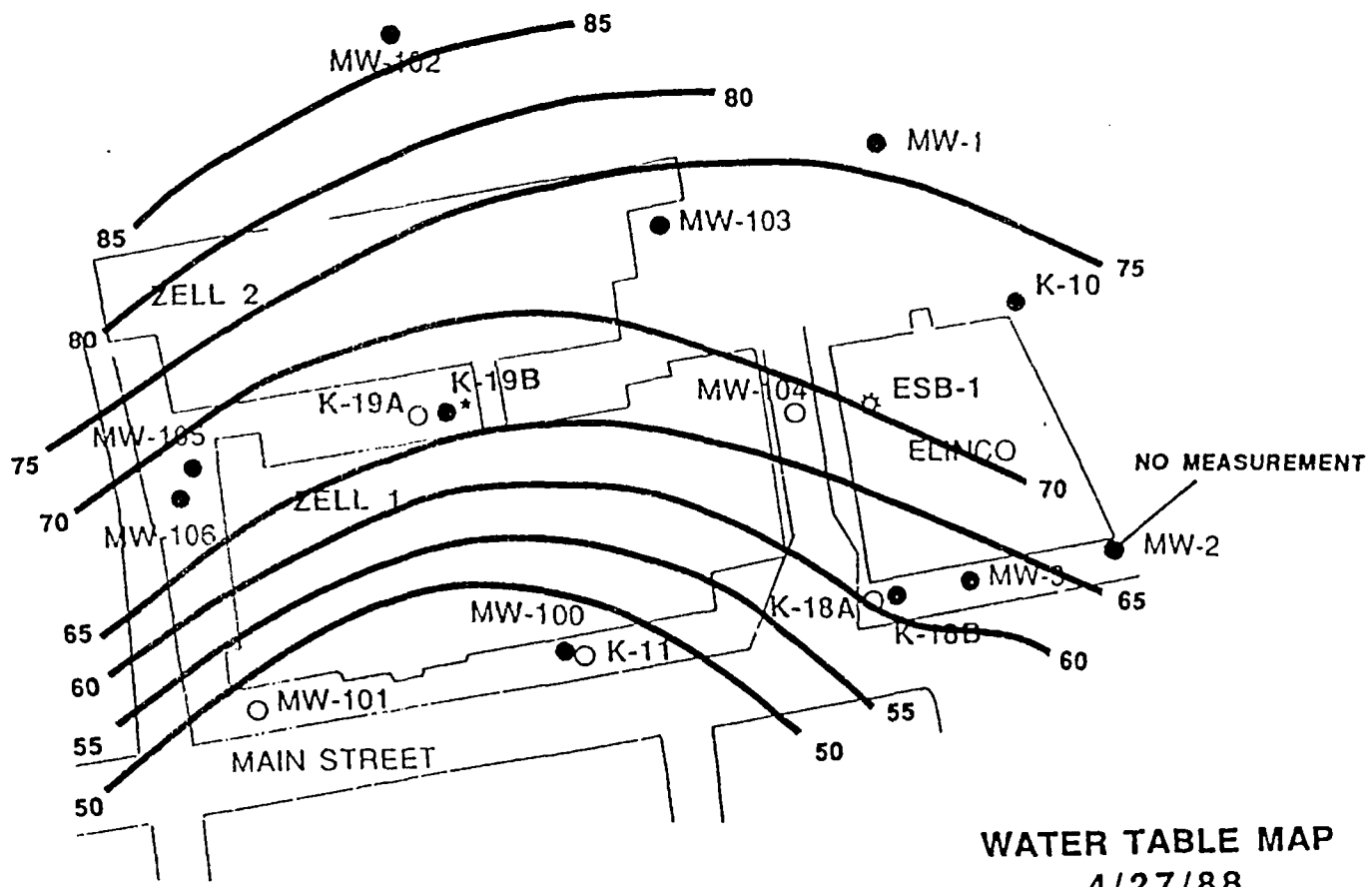
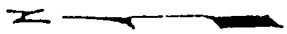
- 80 — FT ABOVE MSL
- \* K-19B SOIL BORING SAMPLE, SAME LOCATION
- OVERBURDEN WELL
- BEDROCK WELL
- ⊛ BORING LOCATION

APPROXIMATE SCALE  
 0 ————— 100 FT

**SITE BEDROCK SURFACE RELIEF**

FIGURE 5  
 Dames & Moore





**WATER TABLE MAP  
4/27/88**

- OVERBURDEN WELL
- BEDROCK WELL
- ☆ BORING LOCATION

80 — WATER TABLE CONTOUR

APPROXIMATE SCALE

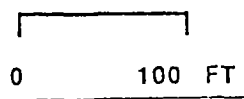
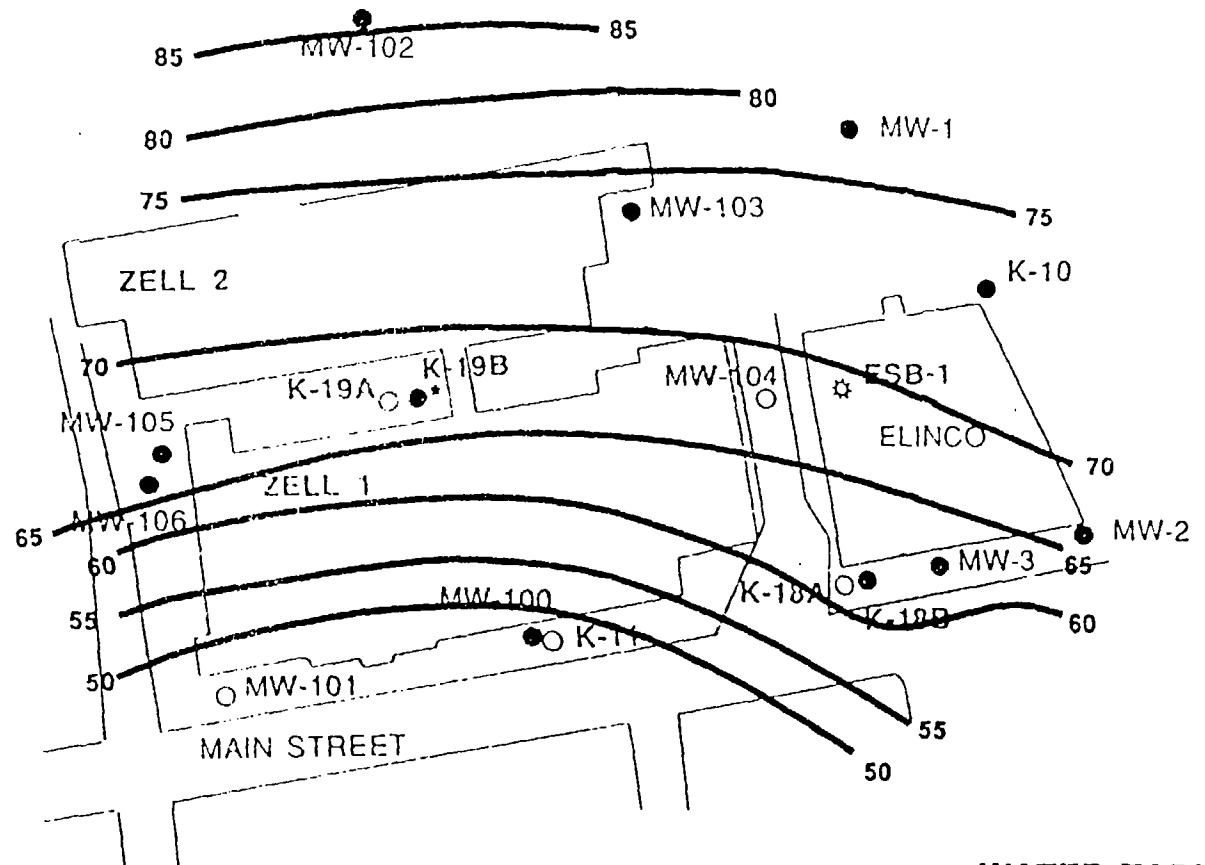
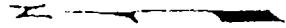


FIGURE 6  
Dames & Moore



**WATER TABLE MAP**  
**5/2/88**

- OVERBURDEN WELL
- BEDROCK WELL
- ☆ BORING LOCATION

1 — WATER TABLE CONTOUR

APPROXIMATE SCALE

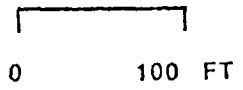
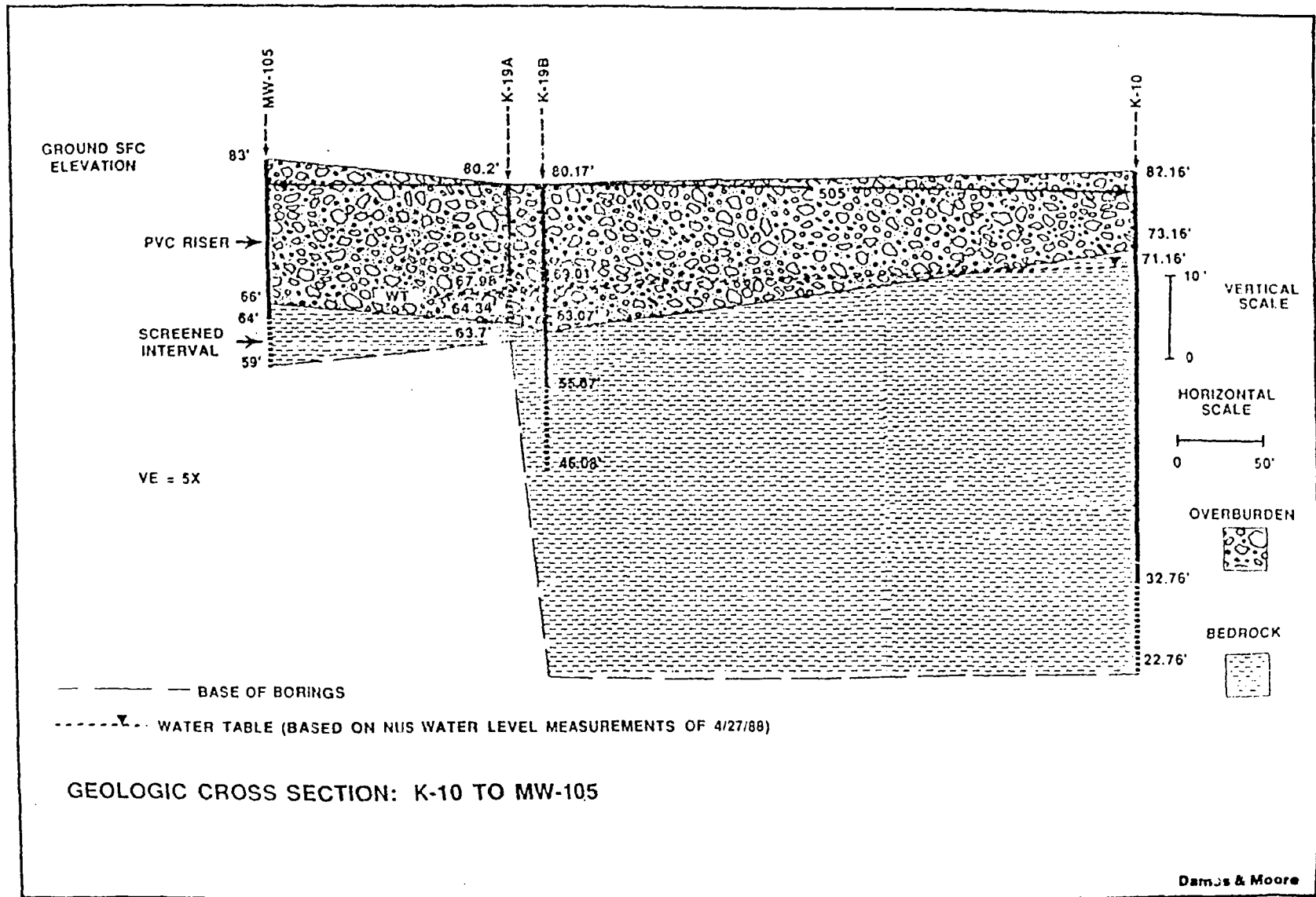
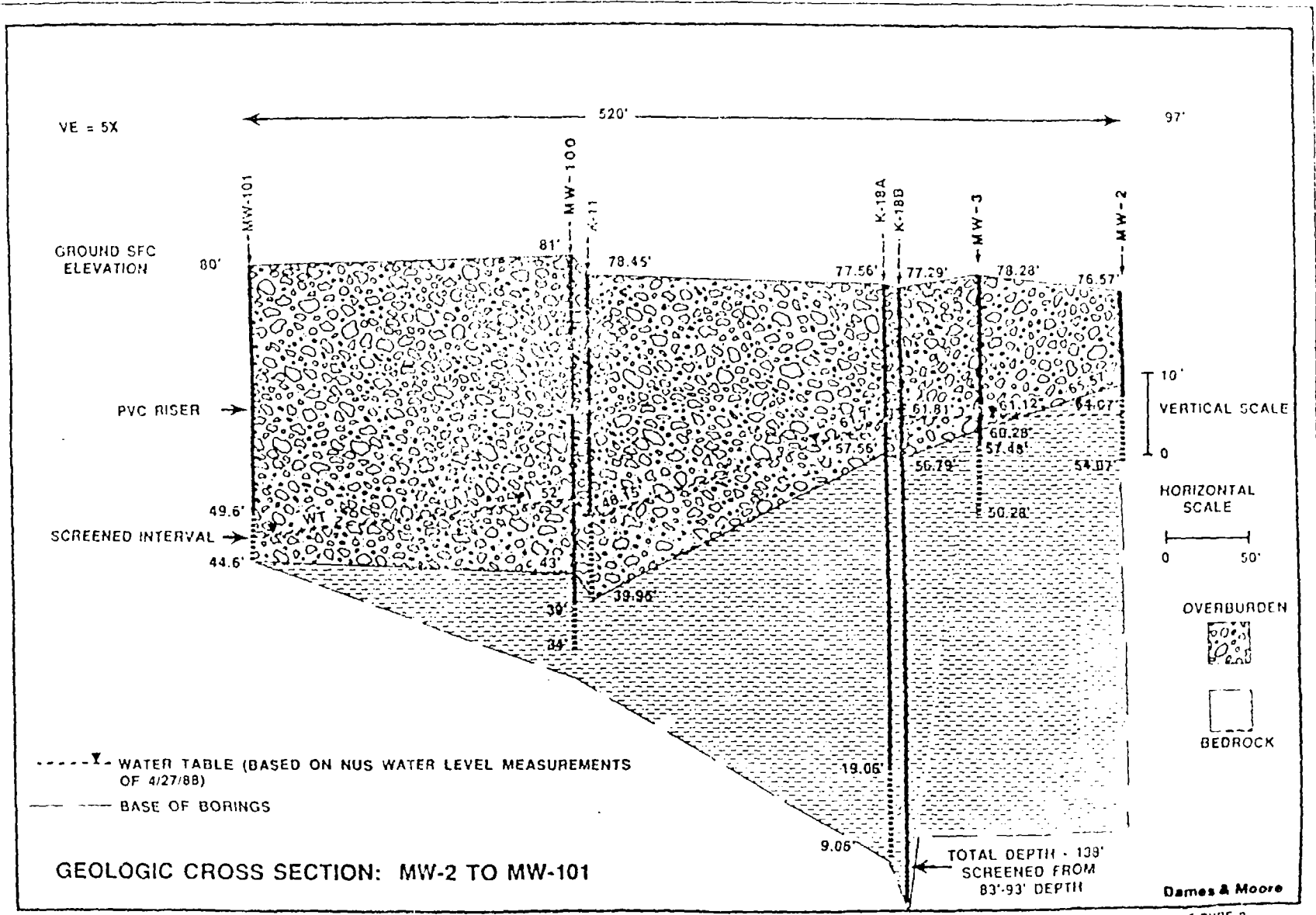


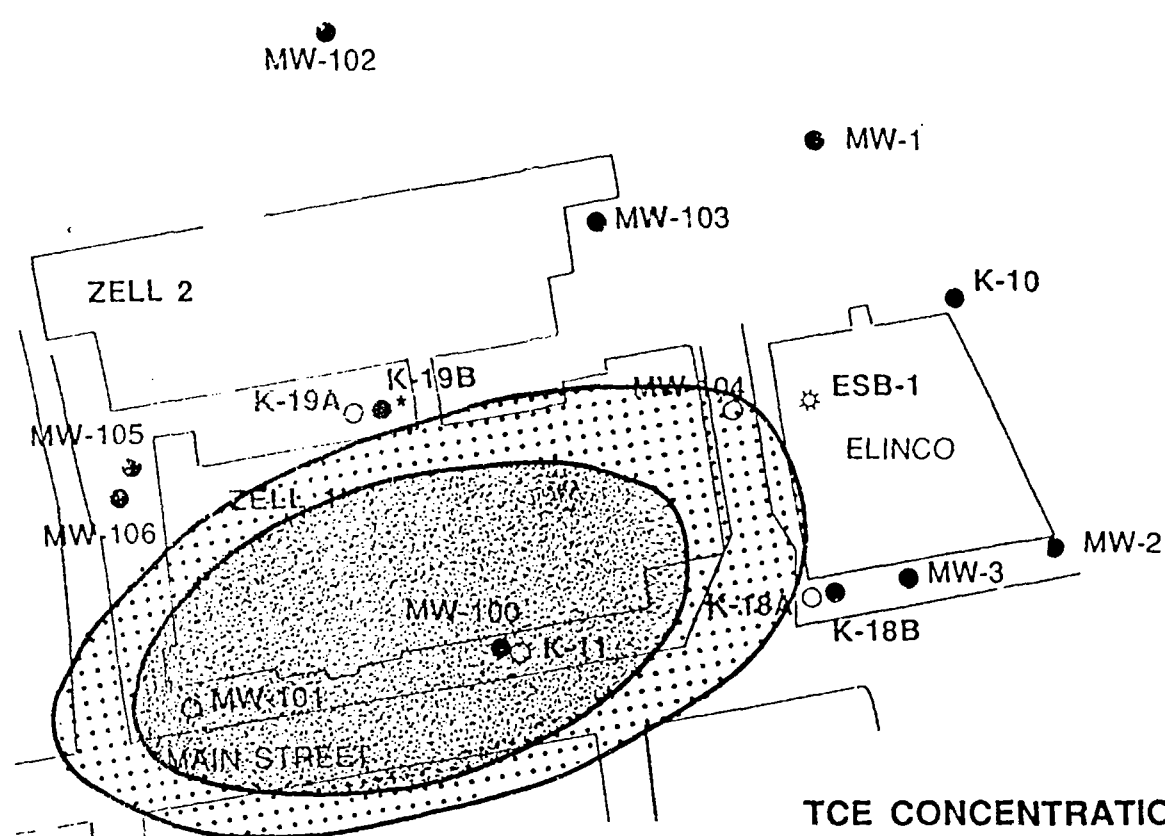
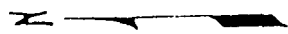
FIGURE 7  
Dames & Moore



GEOLOGIC CROSS SECTION: K-10 TO MW-105

FIGURE 8





5000 - 50,000 ppb  
1000 - 5000 ppb

○ OVERBURDEN WELL  
● BEDROCK WELL  
☆ BORING LOCATION

TCE CONCENTRATIONS IN  
OVERBURDEN GROUNDWATER  
DECEMBER, 1985  
ELINCO/ZELL COMPLEX

APPROXIMATE SCALE

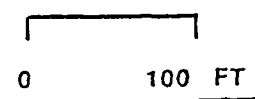
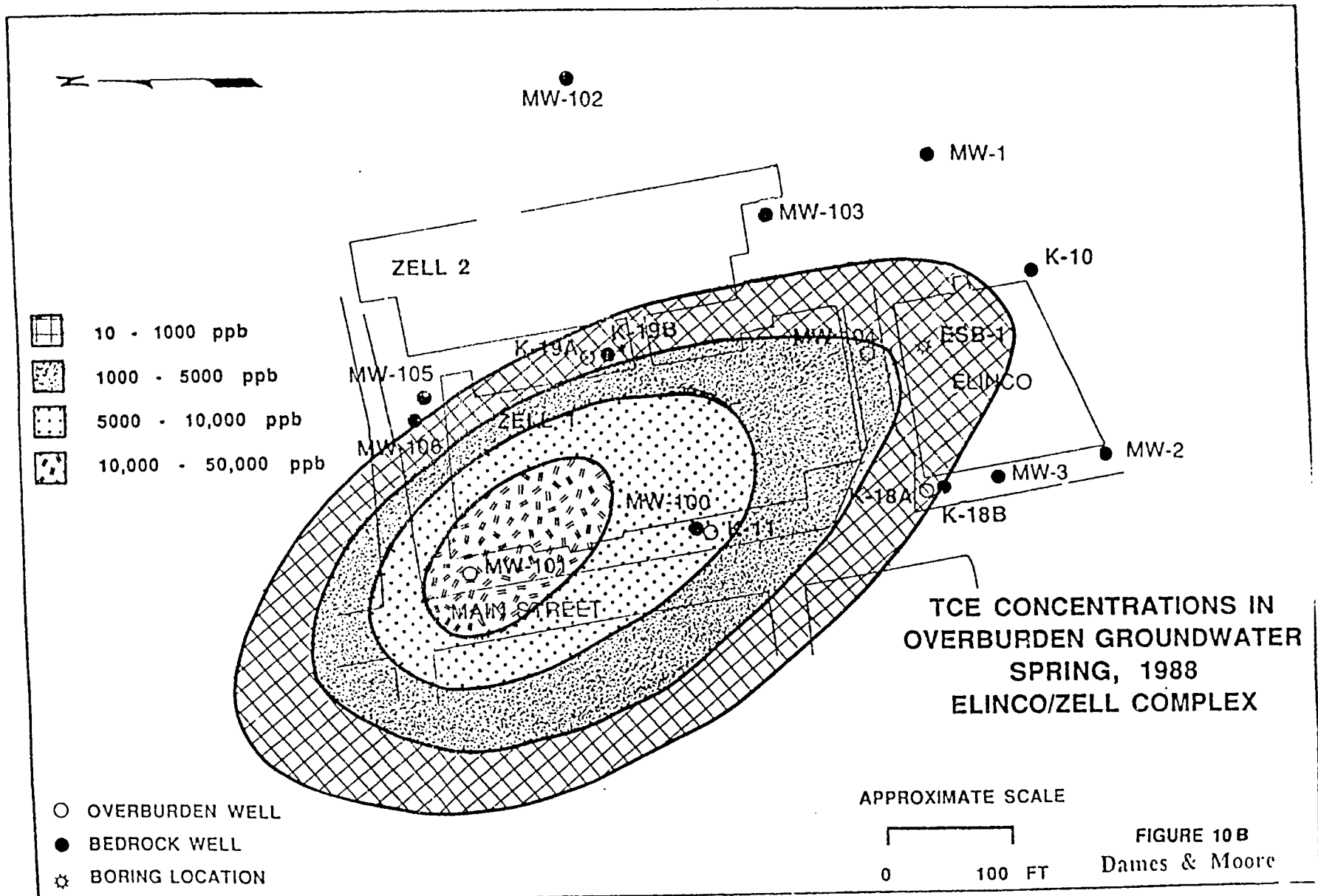
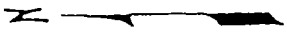


FIGURE 10A  
Dames & Moore



**\*TCE CONCENTRATIONS IN  
BEDROCK GROUNDWATER  
DECEMBER, 1985**



MW-102

MW-1

ZELL 2

MW-105

MW-106

ZELL 1

MW-100

MW-101

MW-104





MAIN STREET




ESB-1

ESB-2

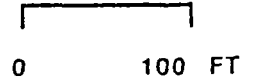
K-10

MW-103

-  10 - 1000 ppb
-  1000 - 5000 ppb
-  5000 - 50,000 ppb
-  50,000 - 100,000 ppb

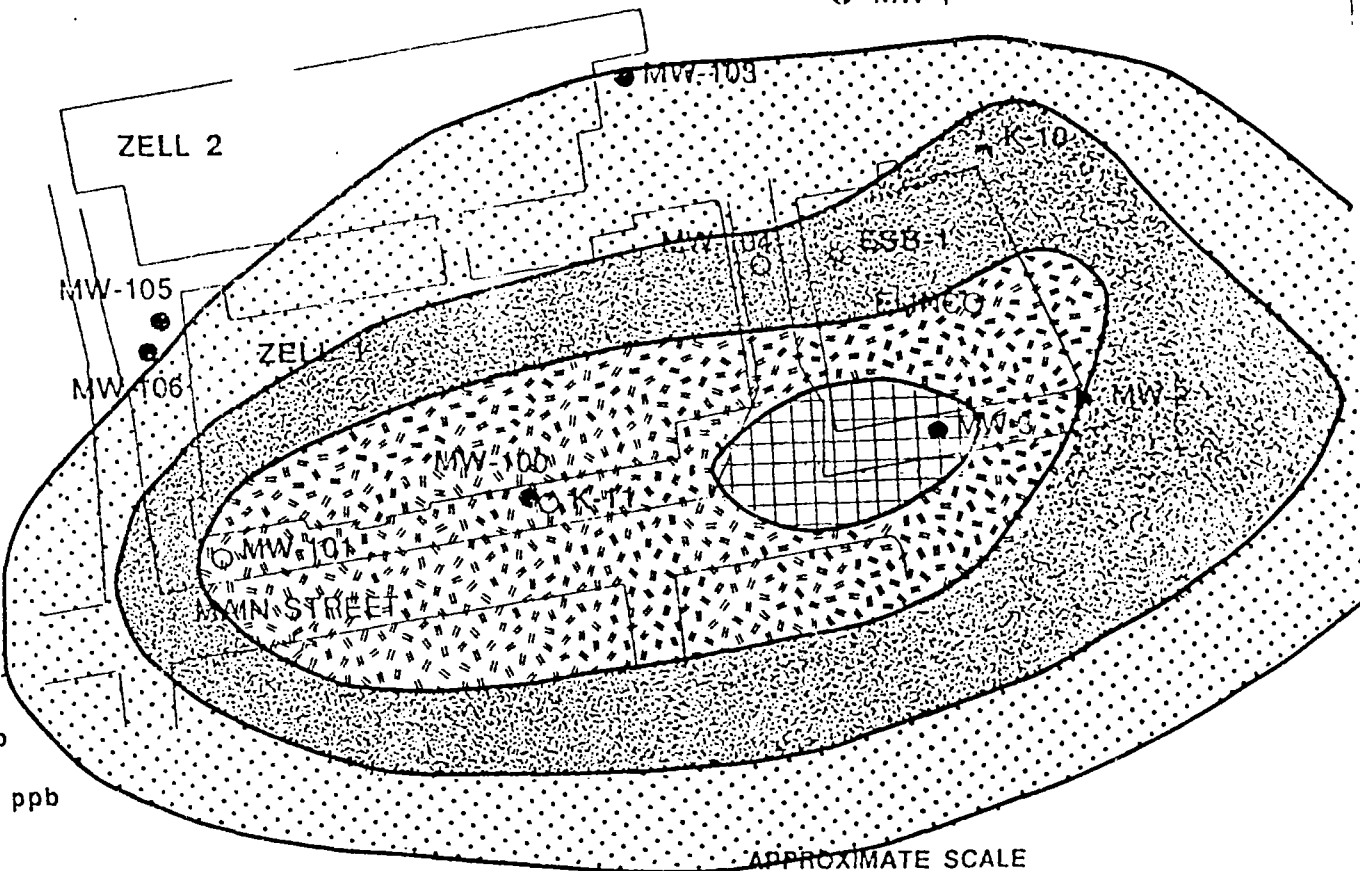
-  OVERBURDEN WELL
-  BEDROCK WELL
-  BORING LOCATION

APPROXIMATE SCALE

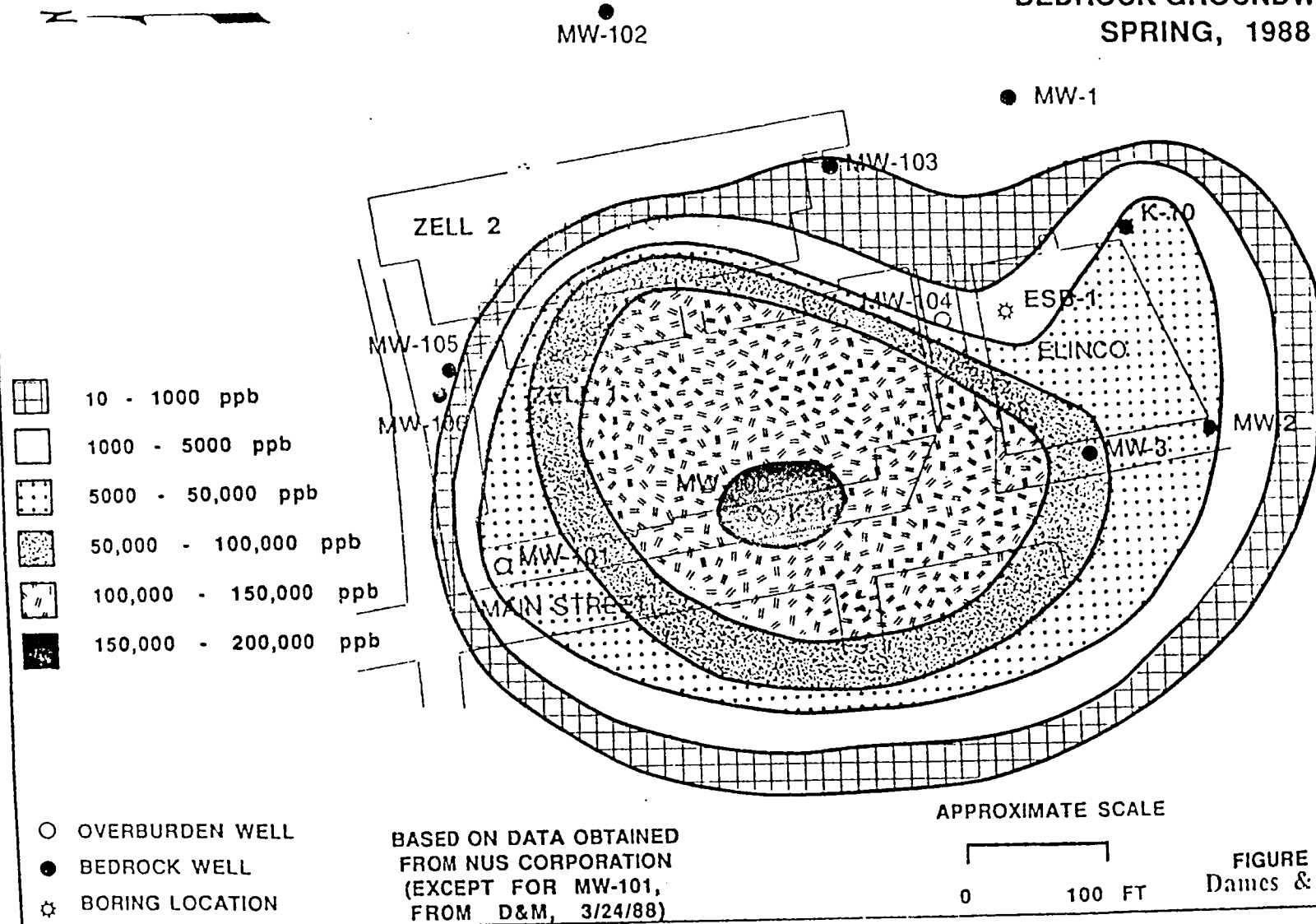
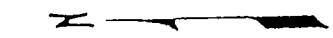








\*BASED ON DATA OBTAINED  
FROM NUS CORPORATION




**FIGURE 10 C**  
Dames & Moore



**TCE CONCENTRATIONS IN  
BEDROCK GROUNDWATER  
SPRING, 1988**

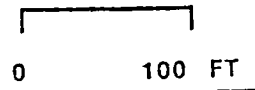


-  10 - 1000 ppb
-  1000 - 5000 ppb
-  5000 - 50,000 ppb
-  50,000 - 100,000 ppb
-  100,000 - 150,000 ppb
-  150,000 - 200,000 ppb

-  OVERBURDEN WELL
-  BEDROCK WELL
-  BORING LOCATION

BASED ON DATA OBTAINED  
FROM NUS CORPORATION  
(EXCEPT FOR MW-101,  
FROM D&M, 3/24/88)

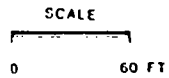
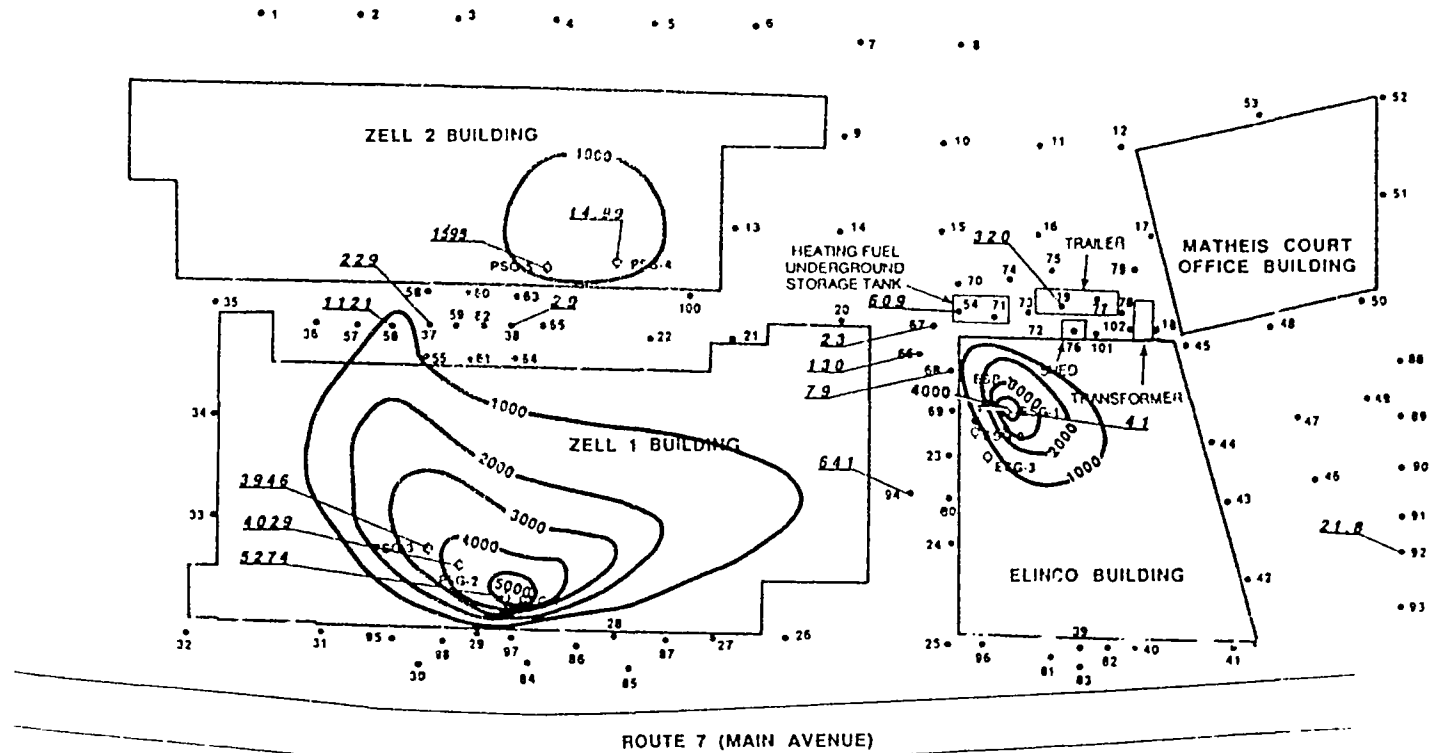
APPROXIMATE SCALE



**FIGURE 10D**  
Dames & Moore



SOIL GAS SURVEY  
SAMPLING LOCATIONS AND  
CONCENTRATION CONTOUR MAP



1000 TCE CONCENTRATIONS  
2.6 ISOLATED TCE CONCENTRATIONS DETECTED

LEGEND  
93 • SOIL GAS SAMPLING LOCATIONS (OUTSIDE)  
ESG-1 • SOIL GAS SAMPLING LOCATIONS (INSIDE)  
ESB-1 • BORING LOCATIONS

FIGURE 11  
Dames & Moore

## REFERENCES

- Dames & Moore, Kellogg-Deering Well Field Zell Building Evaluation, Norwalk, CT. Prepared for Murtha, Cullina, Richter and Pinney (December 4, 1987).
- Day, Berry, and Howard (letter) "Re: Public Comment Kellogg-Deering Site Operable Unit No. 1 Norwalk, Connecticut EPA Work Assignment Number 58-1656" (August 7, 1986).
- Ebasco, 1989; Final Supplemental Remedial Investigation and Feasibility Study, Kellogg-Deering Superfund Site, Norwalk, Connecticut. Volume I.
- EPA Region I Administrator, Michael R. Deland, *Record of Decision Remedial Alternative Selection Operable Unit 1* (data illegible).
- Geraythy & Miller, Inc., "Hydrogeology and Ground Water Quality Study 1982-1983 Connecticut Light and Power Company Landfill Smith Well Field Norwalk, Connecticut" (June, 1983).
- HRP Associates, Inc., "Hydrogeologic and Engineering Report on the Extent of Contamination and Remedial Options Available for the Elinco Division of EDO Corporation 272 Main Avenue, Norwalk, Connecticut".
- Versar, Inc., "Evaluation of Hydrogeologic Aspects of the Remedial Investigation Conducted by the U.S. Environmental Protection Agency for the Kellogg-Deering Site, Norwalk, Connecticut" (July, 1986).

**APPENDIX A**

**BORING AND WELL CONSTRUCTION LOGS**



**FUSS & O'NEILL**  
consulting engineers

210 Main Street, Manchester, Conn. 06040/(203) 646-2469

LETTER OF TRANSMITTAL

Date <u>3/9/88</u>	Project No. <u>86-100</u>
Attention:	
RE:	

TO GREGORY A. DeMASTRO  
HYDROGEOLOGIST  
DAMES AND MOORE  
ONE BLUE HILL PLAZA, SUITE 530  
PEARL RIVER, NEW YORK 10965-8668

We are sending you the following items:

Attached

Under separate cover via \_\_\_\_\_

Shop Drawings  
 Copy of Letter

Prints  
 Change Order

Plans

Samples  
 Other

Specifications

Copies	Date	No.	Description
1	6/86		ATTACHMENT II OF JUNE 1986 HYDROGEOLOGIC INVESTIGATION - 272 MAIN AVE., NEWALK, CT.

These are transmitted as checked below:

- |  |   |   |
|--|---|---|
| <input type="checkbox"/> For approval            | <input type="checkbox"/> Approved as submitted            | <input type="checkbox"/> Resubmit ___ copies for approval   |
| <input checked="" type="checkbox"/> For your use | <input type="checkbox"/> Approved as noted                | <input type="checkbox"/> Submit ___ copies for distribution |
| <input checked="" type="checkbox"/> As requested | <input type="checkbox"/> Returned for corrections         | <input type="checkbox"/> Return ___ corrected prints        |
| <input type="checkbox"/> For review and comment  |   |   |
| <input type="checkbox"/> For Bids Due _____ 19__ | <input type="checkbox"/> Prints returned after loan to us |   |

REMARKS \_\_\_\_\_

AS WE DISCUSSED. I HOPE THESE WILL ASSIST YOU  
IN YOUR EVALUATION.

to KARL BURDEAU, ESQ.

SIGNED: Bob Pattato

ATTACHMENT II  
GEOLOGIC LOGS AND MONITOR WELL COMPLETION REPORTS

# East Coast Drilling, Inc.

APR 16 1984

Bl # \_\_\_\_\_ Fig \_\_\_\_\_

P. O. BOX 961 - WALLINGFORD, CONN. 06492

Bl # \_\_\_\_\_ Fig \_\_\_\_\_

TO HRP Associates

ADDRESS New Britain, Connecticut

PROJECT NAME Elinco

LOCATION - Norwalk, Connecticut

REPORT SENT TO Client

PROJ. NO. 83-86-10

SAMPLES SENT TO Taken at Site

OUR JOB NO. 83-176A

SHEET 1 OF 1  
 DATE 4/9/84  
 HOLE NO. MW-1  
 LINE & STA. \_\_\_\_\_  
 OFFSET \_\_\_\_\_

### GROUND WATER OBSERVATIONS

At 6.5' after 24 Hours

Type

CASING

SAMPLER

CORE BAR.

SURFACE ELEV. \_\_\_\_\_

Size I.D.

Hammer Wt.

Hammer Fall

NW

3"

\_\_\_\_\_

\_\_\_\_\_

S-S

1 3/8"

140lb.

30"

BXS'f

\_\_\_\_\_

BIT

Diamond

DATE STARTED 4/3/84

DATE COMPL. 4/3/84

BORING FOREMAN Quagliaroli II

INSPECTOR P. Misluk

S'LS ENGR. \_\_\_\_\_

### LOCATION OF BORING:

DEPTH	Casing Blows per foot	Sample Depths From-To	Type of Sample	Blows per 6" on Sampler			Moisture Density or Consist.	Strata Change Elev.	SOIL IDENTIFICATION Remarks include color, gradation, Type of soil etc. Rock-color, type, condition, hardness, Drilling time, seams and etc.	SAMPLE		
				From 0-6'	To 6-12'	To 12-18'				No	Pen	Rec.
		0.5'-2.0'	D	15	10	9	Moist	.5'	Macadam.	1	1.5'	1.0'
							Medium					
							Dense		Brown fine-medium Sand, some medium-fine Gravel, trace Silt.			
		5.0'	D	50/0				5.5'				
		5.5'-8.5'	C				3		Cored White Gneiss, weathered.	C1	3.0'	.6'
							4					
		8.5'-10.5'	C				4		Cored Gray weathered Gneiss.	C2	2.0'	.8'
		10.5'-13.5'	C				6.5		"	C3	3.0'	.6'
							5		"			
		13.5'-16.5'	C				4		"	C4	3.0'	3.0'
							5		"			
		16.5'-18.5'	C				7		"	C5	2.0'	1.5'
							6		"			
		18.5'-22.5'	C				5		"	C6	4.0'	2.6'
							5		"			
		22.5'-24.5'	C				6		"	C7	2.0'	1.7'
							3		"			
		24.5'-26.5'	C				5		(reamed in 15.0' of casing)	C8	2.0'	1.6'
							5		"			
							4		"			
							6		"			
								26.5'	Bottom of Boring 26.5'			
									Installed 2" PVC Rock Well @ 26.5'			
									17.0' Riser Pipe			
									1 Curb Box			

GROUND SURFACE TO 5.5' USED NW CASING: THEN Cored to 26.5'

Sample Type  
 Dry C=Cored W=Washed  
 UP=Undisturbed Piston  
 TP=Test Pit A=Auger V=Vane Test  
 UT=Undisturbed Thinwall

Proportions Used  
 trace 0 to 10%  
 little 10 to 20%  
 some 20 to 35%  
 and 35 to 50%

140lb Wt. x 30" fall on 2" O.D. Sampler  
 Cohesionless Density Cohesive Consistency  
 0-10 Loose 0-4 Soft 30+ Hard  
 10-30 Med Dense 4-8 M/Stiff  
 30-50 Dense 8-15 Stiff  
 50+ Very Dense 15-30 V-Stiff

SUMMARY:  
 Earth Boring 5.5'  
 Rock Coring 21.0'  
 Samples 1  
 HOLE NO MW-1

MONITOR WELL COMPLETION REPORT

GENERAL INFORMATION

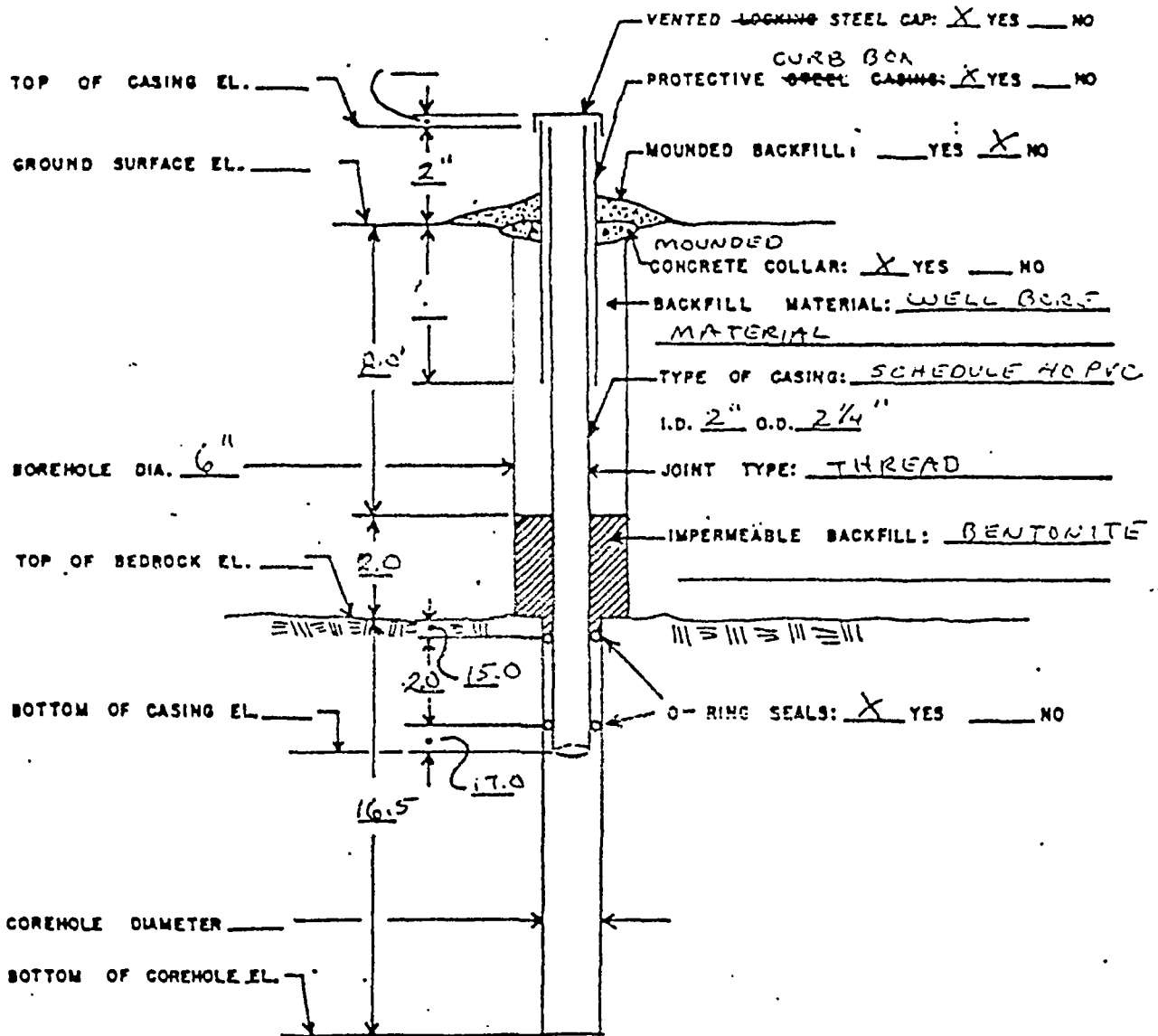
Town: NORWALK Site: ELINCO  
Monitoring Point I.D. No.: MW-1 Date of completion: 4/4/84  
DEP/WPC I.D. No:  
Monitoring Point Location (relative to site features): UPGRADEMENT  
SEE SITE MAP  
Drilling Contractor: EAST COAST DRILLING INC. Supervising Engineer/Geologist: P. MISLUK  
Well Construction Method: AUGERED AND CORED

WELL INFORMATION (ELEVATIONS TO NEAREST 0.1 FEET)

Ground surface elevation (MSL): Well depth below ground surface: 26.5  
Refusal: \_\_\_ Yes \_\_\_ No  
Top of casing elevation (MSL): Screened interval:  
Length of Screen:  
Length of riser pipe: 17.0'  
Screen type: Screen slot size:  
Filter fabric: \_\_\_ Yes \_\_\_ No Screen packing: \_\_\_ Yes \_\_\_ No  
If yes, Thickness:  
Well inside diameter: 2" Material:  
grain size:  
Impermeable Backfill: Bentonite  
Well casing material and schedule: 40 PVC Estimated K screened interval:  
Method of well development: PUMP Time spent developing: 1 hr  
Locking \_\_\_ or threaded cap X Impermeable backfill: CONCRETE COLLAR  
Curb Box

# MONITOR WELL INSTALLATION DETAIL FOR WELL IN BEDROCK

mw-1





Bedrock wells

Casing length: 17.0'

Water-bearing rock unit: FRACTURED FELSIC GNEISS

Water bearing sections (depths and approximate yields): 10' - 26.5'

Length of rock core: 16.5'

Diameter of core hole: 2 7/8"

Thickness and depth of impermeable backfill: 2' 8' - 10' 2' 1' - 3'

O-rings seals:  Yes  No

GEOLOGIC INFORMATION

Aquifer:

Inferred relationship to plume:  Within  Outside  Edge

Watershed (plume discharge watercourse):

Aquifer materials (attach boring log):

Attach maps and plans required of G.1.j. and G.4.

# East Coast Drilling, Inc.

P. O. BOX 961 - WALLINGFORD, CONN. 06492

TO HRP Associates

PROJECT NAME Eliaco

REPORT SENT TO Client

COPIES SENT TO Taken at Site

Bit # \_\_\_\_\_ Fig \_\_\_\_\_

Bit # \_\_\_\_\_ Fig \_\_\_\_\_

ADDRESS New Britain, Connecticut

LOCATION Norwalk, Connecticut

PROJ. NO. 83-86-10

OUR JOB NO. 83-176A

SHEET 1 of 1  
 DATE 4/9/84  
 HOLE NO. MW-2  
 LINE & STA. \_\_\_\_\_  
 OFFSET \_\_\_\_\_

GROUND WATER OBSERVATIONS		CASING	SAMPLER	CORE BAR	SURFACE ELEV.
At _____ after _____ Hours	Type	<u>USA</u>	<u>S-S</u>		DATE STARTED <u>4/4/84</u>
At _____ after _____ hours	Size I.D.	<u>3 1/2"</u>	<u>1 3/8"</u>		DATE COMPL. <u>4/4/84</u>
	Hammer Wt.		<u>140lb</u>	BIT	BORING FOREMAN <u>Quagliaroli II</u>
	Hammer Fall		<u>30"</u>		INSPECTOR <u>P. Misluk</u>
					SOILS ENGR. _____

## LOCATION OF BORING:

DEPTH	Casing Blows per foot	Sample Depths From-To	Type of Sample	Blows per 6" on Sampler			Moisture Density or Consist.	Strata Change Elev.	SOIL IDENTIFICATION Remarks include color, gradation, Type of soil etc. Rock-color, type, condition, hardness, Drilling time, seams and etc.	SAMPLE		
				From 0-6	6-12	To 12-18				No	Pen	Rec.
		0.5'-2.0'	D	10	4	6	Moist Loose	.5'	Macadam.	1	1.5'	1.3'
									Light Brown fine Sand, little Silt.			
		5.0'-6.5'	D	10	28	37	Moist Very Dense	5.0'	Multi-color fine-coarse Sand and fine-medium Gravel.	2	1.5'	1.3'
									Gray Brown fine-coarse Sand, some fine-coarse Gravel, trace Silt, Cobbles.			
		10.0'-10.5'	D	6	1	5	Moist V/Dense	11.0'	Grayish White Gneiss, (from 14.0'-14.5' seams).	3	1.5'	.5'
		11.0'-16.0'	C							C1	5.0'	4.6'
							7 5 8 7 6 5 4 4 3 5		Gray Gneiss with fractured joints.	C2	5.0'	5.0'
		15.0'-21.0'	C									
								21.0'	Roller Bit to 23.0'			
								23.0'	Bottom of Boring 23.0'  Installed 2" PVC Monitor Well @ 22.5'  12.5' Riser Pipe 10.0' Screen 1 Curb Box			

GROUND SURFACE TO 11.0' USED USA "CASING: THEN Roller Bit to 23.0'

Soil Type  
 D=Dry C=Cored W=Washed  
 UP=Undisturbed Piston  
 TP=Test Pit A=Auger V=Vane Test  
 UT=Undisturbed Thruwall

Proportions Used  
 trace 0 to 10%  
 little 10 to 20%  
 some 20 to 35%  
 and 35 to 50%

140lb Wt. x 30" fall on 2" O.D. Sampler  
 Cohesionless Density Cohesive Consistency  
 0-10 Loose 0-4 Soft 30+ Hard  
 10-30 Med. Dense 4-8 M/Stiff  
 30-50 Dense 8-15 Stiff  
 50+ Very Dense 15-30 V-Stiff

SUMMARY  
 Earth Boring 13.0'  
 Rock Coring 10.0'  
 Samples 3  
 HOLE NO MW-2

MONITOR WELL COMPLETION REPORT

GENERAL INFORMATION

Town: NORWALK Site: ELINCO  
Monitoring Point I.D. No.: MW-2 Date of completion: 4/4/84  
DEP/WPC I.D. No:  
Monitoring Point Location (relative to site features): DOWNGRAIENT  
Drilling Contractor: EAST COAST DRILLING, INC. Supervising Engineer/Geologist: <sup>SEE SITE MAP</sup>  
Well Construction Method: AUGERED

WELL INFORMATION (ELEVATIONS TO NEAREST 0.1 FEET)

Ground surface elevation (MSL): Well depth below ground surface: 22.5'  
Refusal:  Yes  No  
Top of casing elevation (MSL): Screened interval: 12.5 - 22.5'  
Length of Screen: 10.0'  
Length of riser pipe: 17.5'  
Screen type: PVC Screen Slot size: .010"  
Filter fabric:  Yes  No Screen packing:  Yes  No  
Well inside diameter: 2" If yes, Thickness: 10.0'  
Material: OTTOWA SAND  
grain size: #12 SAND  
Impermeable Backfill: Bentonite  
Well casing material and schedule: 40 PVC Estimated R screened interval:  
Method of well development: PUMP Time spent developing: 1 HR  
Locking  or threaded cap  Impermeable backfill: CONCRETE COLLAR  
Curb Box

Bedrock wells

Casing length: 22.5'  
+ SCREEN

Water-bearing rock unit: FRACTURED FELSIC GNEISS

Water bearing sections (depths and approximate yields): 13.0' - 22.5'

Length of rock core: 12.0'

Diameter of core hole: 2 1/2"

Thickness and depth of impermeable backfill: 2' 10.5 - 12.5' 2' 1 - 3'

O-rings seals: \_\_\_ Yes X No

GEOLOGIC INFORMATION

Aquifer: ~

Inferred relationship to plume: \_\_\_ Within \_\_\_ Outside \_\_\_ Edge

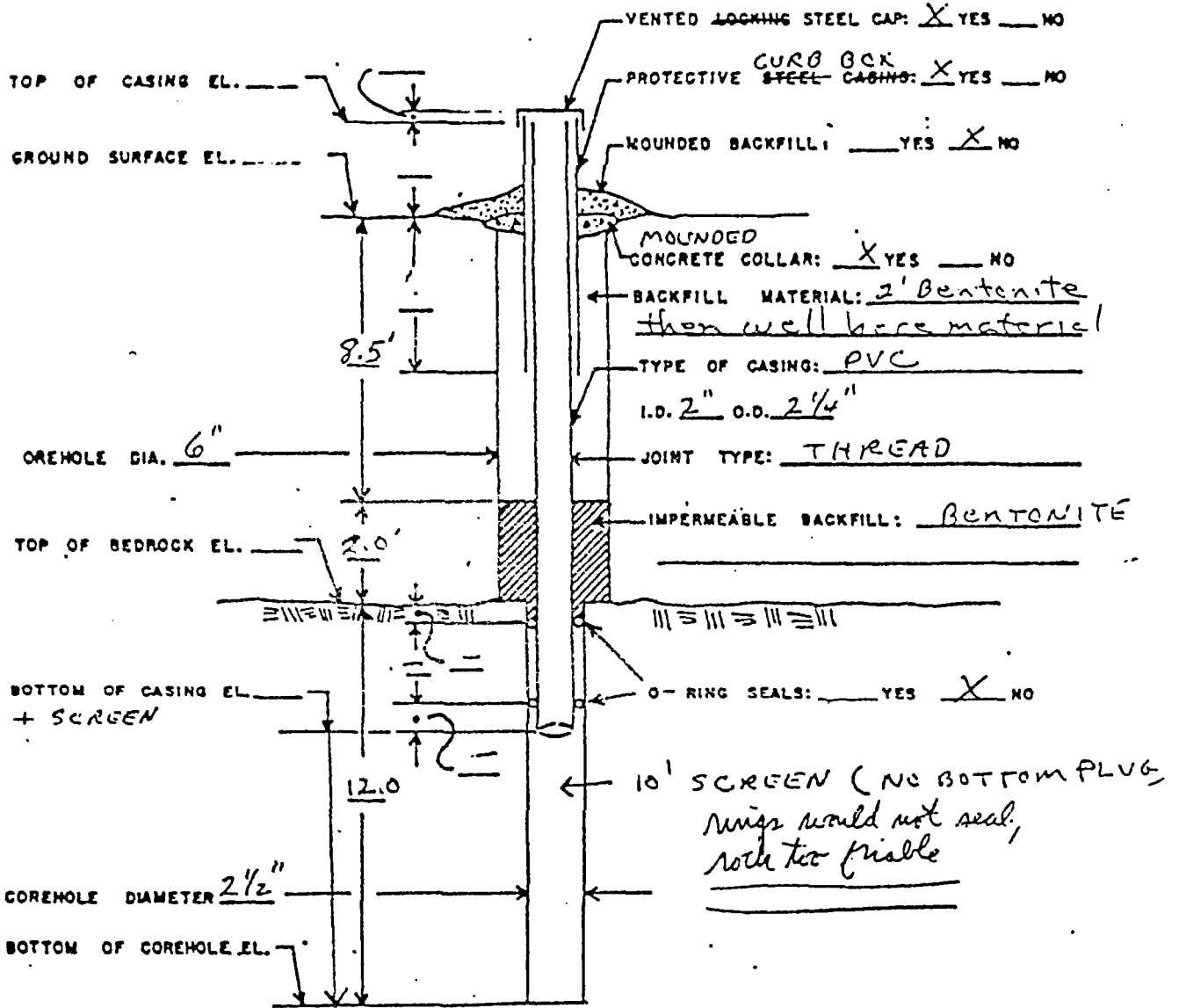
Watershed (plume discharge watercourse):

Aquifer materials (attach boring log):

Attach maps and plans required of G.1.j. and G.4.

# MONITOR WELL INSTALLATION DETAIL FOR WELL IN BEDROCK

mw-2



# East Coast Drilling, Inc.

P. O. BOX 961 - WALLINGFORD, CONN. 06492

TO HRP Associates

PROJECT NAME Elinco

ART SENT TO Client

ES SENT TO Taken at Site

Bit # \_\_\_\_\_ Fig \_\_\_\_\_

Bit # \_\_\_\_\_ Fig \_\_\_\_\_

ADDRESS New Britain, Connecticut

LOCATION Norwalk, Connecticut

PROJ. NO. 83-86-10

OUR JOB NO. 83-176A

SHEET 1 OF 1

DATE 4/9/84

HOLE NO. MW-3

LINE & STA. \_\_\_\_\_

OFFSET \_\_\_\_\_

GROUND WATER OBSERVATIONS		CASING	SAMPLER	CORE BAR.	SURFACE ELEV.
At <u>19.0'</u>	after <u>0</u> Hours	Type <u>HSA</u>	<u>S-5</u>	_____	DATE STARTED <u>4/4/84</u>
At _____	after _____ Hours	Size: D. <u>2 1/2"</u>	<u>1 3/8"</u>	_____	DATE COMPL. <u>4/5/84</u>
		Hammer Wt. _____	<u>140lb.</u>	BIT	BORING FOREMAN <u>Quagliaroli II</u>
		Hammer Fall _____	<u>30"</u>	_____	INSPECTOR <u>P. Misluk</u>
					SOILS ENGR. _____

## LOCATION OF BORING:

DEPTH	Casing Blows per foot	Sample Depths From - To	Type of Sample	Blows per 6" on Sampler			Moisture Density or Consist.	Strata Change Elev.	SOIL IDENTIFICATION Remarks include color, gradation, Type of soil etc. Rock-color, type, condition, hardness, Drilling time, seams and etc.	SAMPLE		
				0-6	6-12	12-18				No	Pen	Rec.
		0.5'-2.0'	D	6	5	6	Moist Mddium Dense	.5'	Macadam.	1	1.5'	.5'
		5.0'-6.5'	D	18	58	51	Moist Very Dense		Brown fine-medium Sand, little fine Gravel, little Silt.			
		10.0'-11.5'	D	12	31	27	Moist Very Dense		Brown fine-coarse Sand, some fine-coarse Gravel, trace Silt.	2	1.5'	1.0'
		15.0'-16.5'	D	12	42	85	Moist Very Dense		Brown fine-medium Sand, little fine-medium Gravel.	3	1.5'	1.2'
		18.0'-23.0'	C					18.0'	" " "	4	1.5'	1.5'
		23.0'-28.0'	C						Gray Gneiss. (Reamed Casing to 20.0')	C1	5.0'	4.7'
									" "	C2	5.0'	4.7'
								28.0'	Bottom of Boring 28.0' Installed 2" PVC Rock Well @ 28.0' 20.8' Riser Pipe 1 Curb Box			

GROUND SURFACE TO 18.0'

USED HSA CASING: THEN Cored to 28.0'

<p>Core Type</p> <p>U=Dry C=Cored W=Washed</p> <p>UP=Undisturbed Piston</p> <p>TP=Test Pit A=Auger V=Vane Test</p> <p>UT=Undisturbed Thin-wall</p>	<p>Proportions Used</p> <p>trace 0 to 10%</p> <p>fine 10 to 20%</p> <p>some 20 to 35%</p> <p>and 35 to 50%</p>	<p>140lb Wt. x 30" fall on 2" O.D. Sampler</p> <p>Cohesionless Density</p> <p>0-10 Loose</p> <p>10-30 Med. Dense</p> <p>30-50 Dense</p> <p>50+ Very Dense</p>	<p>Cohesive Consistency</p> <p>0-4 Soft</p> <p>4-8 M/Stiff</p> <p>8-15 Stiff</p> <p>15-30 V-Stiff</p>	<p>SUMMARY</p> <p>Earth Boring <u>18.0'</u></p> <p>Rock Coring <u>10.0'</u></p> <p>Samples <u>4</u></p>
HOLE NO <u>MW-3</u>				

MONITOR WELL COMPLETION REPORT

GENERAL INFORMATION

Town: NORWALK

Site: ELINCO

Monitoring Point I.D. No.: MW-3

Date of completion: 4/6/84

DEP/WPC I.D. No:

Monitoring Point Location (relative to site features): DOWNGRADIENT  
(see site map)

Drilling Contractor: EAST COAST  
DRILLING, INC.

Supervising Engineer/Geologist: P. MISLUK

Well Construction Method:  
AUGERED AND CORED

WELL INFORMATION (ELEVATIONS TO NEAREST 0.1 FEET)

Ground surface elevation (MSL):

Well depth below ground surface: 28.0'

Refusal: \_\_\_ Yes \_\_\_ No

Top of casing elevation (MSL):

Screened interval: 13' - 22'

Length of Screen:

Length of riser pipe: 20.8'

Screen type: PVC

Screen Slot size: .010"

Filter fabric: \_\_\_ Yes  No

Screen packing:  Yes \_\_\_ No

If yes, Thickness: 10.0'

Well inside diameter: 2"

Material: Ottawa sand

grain size: # 12 sand

Impermeable Backfill: BENTONITE

Well casing material and schedule: 40 PVC

Estimated K screened interval:

Method of well development: PUMP

Time spent developing: 1 hr.

Locking \_\_\_ or threaded cap   
Curb Box

Impermeable backfill: CONCRETE  
COLLAR

Bedrock wells

Casing length: 20.8'

Water-bearing rock unit: FRACTURED FELSIC GNEISS

Water bearing sections (depths and approximate yields): 10' - 28'

Length of rock core: 10'

Diameter of core hole: 2 3/8"

Thickness and depth of impermeable backfill: 2', 16' to 12' and 2', 1' to 3'

O-rings seals:  Yes  No

GEOLOGIC INFORMATION

Aquifer:

Inferred relationship to plume:  Within  Outside  Edge

Watershed (plume discharge watercourse):

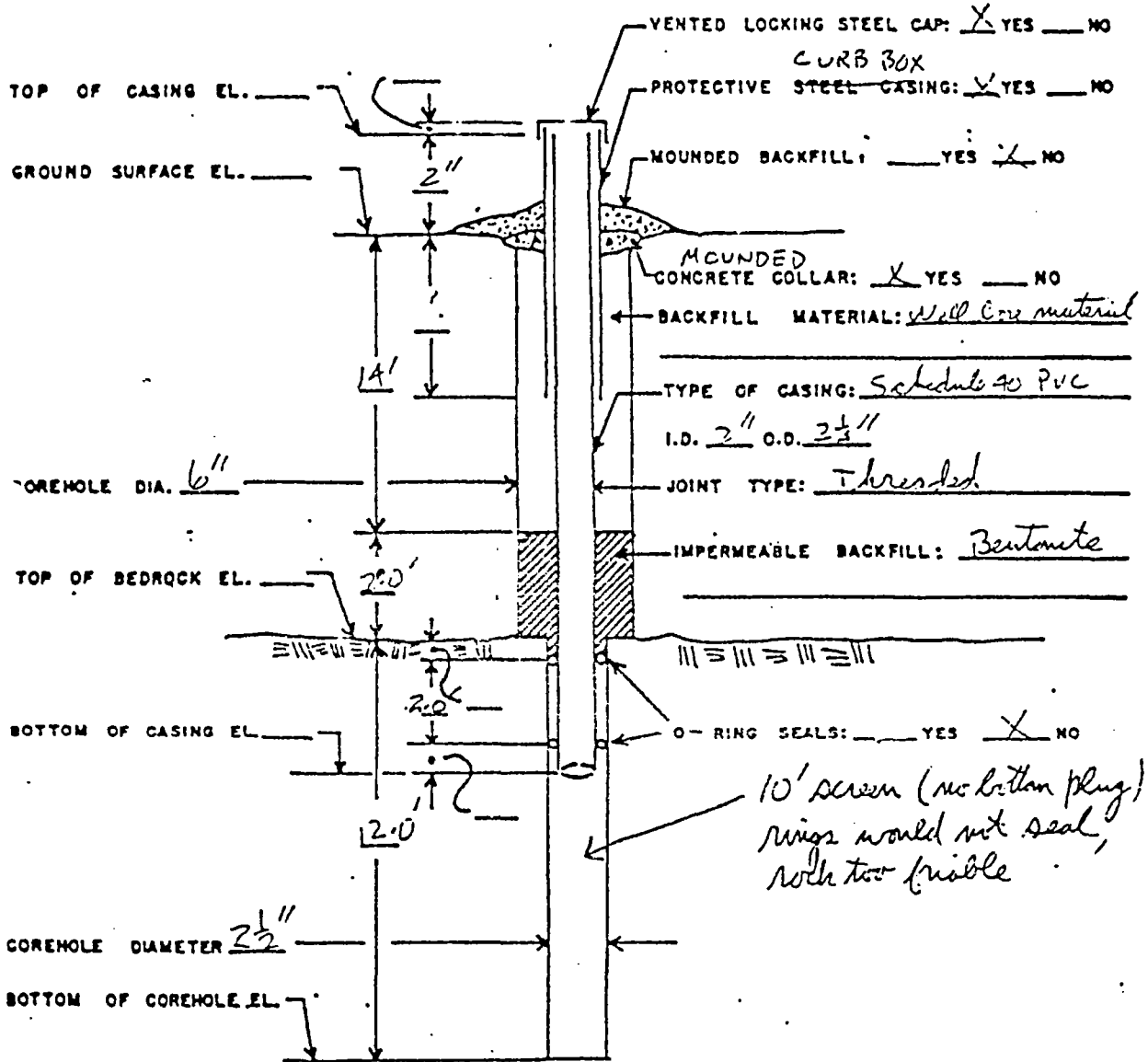
Aquifer materials (attach boring log):

Attach maps and plans required of G.1.j. and G.4.



# MONITOR WELL INSTALLATION DETAIL FOR WELL IN BEDROCK

mw-3



**Glenn Drilling Inc.**

R.F.D. #1, Lake Road  
Fitchville, CT 06334  
(203) 887-3621

CLIENT Russ & O'Neill  
PROJECT NAME KESLOFF / NORWALK  
LOCATION Norwalk, Ct.

BORING NUMBER  
**MW-100**

SHEET  
No. 1  
of 2

DRILLER Michael Deane  
INSPECTOR Bob Potterton  
DATE START Sept. 9, 1985  
DATE FINISH Sept. 11, 1985

ARCHITECT  
ENGINEER

	Casing	Sampler	Core Barrel
TYPE	<u>NW</u>	<u>SS</u>	<u>NX</u>
SIZE I.D.	<u>3"</u>	<u>1 1/2"</u>	
HAMMER WT.	<u>300#</u>	<u>140#</u>	
HAMMER FALL	<u>24"</u>	<u>30"</u>	

FILE NO. 926-8540

SURFACE ELEV. \_\_\_\_\_  
LINE & STATION \_\_\_\_\_  
OFFSET \_\_\_\_\_

DEPTH	SAMPLE					COL. A	STRATA CHANGE	FIELD CLASSIFICATION AND REMARKS	
	NO.	DEPTH RANGE	BLOWS PER 6" ON SAMPLER						REC.
			0-6	6-12	12-18				
5'	1	5.0'-6.5'	16	36	100	1.1		Brown fine-medium sand. Cobbles Fine-coarse Gravel.	
10'	2	10.0'-10.8'	100	100	3	.7		Boulders. Brown fine-coarse Sand and Gravel.	
20'	3	20.0'-22.0'	76 25	67	56	.7		Brown fine-coarse Sand. Medium- coarse Gravel. Cobbles and Boulders	
25'	4	25.0'-27.0'	38 27	49	56	.6			
30'	5	30.0'-32.0'	55 38	71	61	.5		Brown fine-medium Sand. Fine-coarse Gravel. Cobbles	
35'	6	35.0'-37.0'	33 12	16	12	.5		Brown fine-medium Sand. Fine-medium gravel.	
	R-1	38.0'-43.0'				1.3	10	38.0'	Cored Marble like Rock.
							8		

**SAMPLE IDENTIFICATION**

S	— SPLIT SPOON
T	— THIN WALL TUBE
U	— UNDISTURBED PISTON
O	— OPEN END ROD
W	— WASH SAMPLE
A	— AUGER SAMPLE

**PENETRATION RESISTANCE**  
140 lb. Wt. falling 30" on 2" O.D. Sampler

Cohesionless Density		Cohesive Consistency	
0-4	Very Loose	0-2	Very Soft
5-9	Loose	3-4	Soft
10-29	Med. Dense	5-8	M/Stiff
30-49	Dense	9-15	Stiff
50 -	Very Dense	16-30	V-Stiff
		31 -	Hard

**PROPORTIONS USED**

trace	0 to 10%
little	10 to 20%
some	20 to 35%
and	35 to 50%

**REMARKS:**  
COL. A Coring Time

<b>Glenn Drilling Inc.</b> R.F.D. #1, Lake Road Fitchville, CT 06334 (203) 887-3621	CLIENT <u>Fuss &amp; O'Neill</u> PROJECT NAME <u>Kesloff / NORWALK</u> LOCATION <u>Norwalk, Ct.</u>	BORING NUMBER <u>MW 100</u> SHEET No. <u>2</u> of <u>2</u>
--	---	--

DRILLER <u>Michael Deane</u> INSPECTOR <u>Bob Potterton</u> DATE START <u>Sept. 9, 1985</u> DATE FINISH <u>Sept. 11, 1985</u>	ARCHITECT ENGINEER TYPE <u>NW</u> Casing <u>3"</u> Sampler <u>SS</u> Core Barrel <u>NX</u> SIZE I.D. <u>3"</u> <u>1 1/2"</u> <u>2"</u> HAMMER WT. <u>300#</u> <u>140#</u> HAMMER FALL <u>24"</u> <u>30"</u>	FILE NO. <u>926-8570</u> SURFACE ELEV. _____ LINE & STATION _____ OFFSET _____
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DEPTH	SAMPLE						COL. A	STRATA CHANGE	FIELD CLASSIFICATION AND REMARKS
	NO.	DEPTH RANGE	BLOWS PER 6" ON SAMPLER			REC.			
			0-6	6-12	12-18				
40'							8	Cored Marble like Rock.	
							11		
							13		
	R-2	43.0'-45.0'			1.9		7		
45'							8		
	R-3	45.0'-50.0'			3.6		7		
							9		
							9		
							11		
							50.0'		
									End of Boring: 50.0'
								Installed 2" PVC Monitor Well Bottom set at 47.0' 5.0' Screen 42.0' Riser 1 Protective Cover.  Drilled 10.0' with Augers Moved Hole and Drove Casing.	

<b>SAMPLE IDENTIFICATION</b> S — SPLIT SPOON T — THIN WALL TUBE U — UNDISTURBED PISTON O — OPEN END ROD W — WASH SAMPLE A — AUGER SAMPLE	<b>PENETRATION RESISTANCE</b> 140 lb. Wt. falling 30" on 2" O.D. Sampler <table style="width:100%; font-size: small;"> <tr> <th colspan="2">Cohesionless Density</th> <th colspan="2">Cohesive Consistency</th> </tr> <tr> <td>0-4</td><td>Very Loose</td> <td>0-2</td><td>Very Soft</td> </tr> <tr> <td>5-9</td><td>Loose</td> <td>3-4</td><td>Soft</td> </tr> <tr> <td>10-29</td><td>Med. Dense</td> <td>5-8</td><td>M/Stiff</td> </tr> <tr> <td>30-49</td><td>Dense</td> <td>9-13</td><td>Stiff</td> </tr> <tr> <td>50 +</td><td>Very Dense</td> <td>16-30</td><td>V-Stiff</td> </tr> <tr> <td></td><td></td> <td>31 +</td><td>Hard</td> </tr> </table>	Cohesionless Density		Cohesive Consistency		0-4	Very Loose	0-2	Very Soft	5-9	Loose	3-4	Soft	10-29	Med. Dense	5-8	M/Stiff	30-49	Dense	9-13	Stiff	50 +	Very Dense	16-30	V-Stiff			31 +	Hard	<b>PROPORTIONS USED</b> trace 0 to 10% little 10 to 20% some 20 to 35% and 35 to 60%	<b>REMARKS:</b> COL. A <u>Coring Time</u>
Cohesionless Density		Cohesive Consistency																													
0-4	Very Loose	0-2	Very Soft																												
5-9	Loose	3-4	Soft																												
10-29	Med. Dense	5-8	M/Stiff																												
30-49	Dense	9-13	Stiff																												
50 +	Very Dense	16-30	V-Stiff																												
		31 +	Hard																												

MONITOR WELL COMPLETION REPORTGENERAL INFORMATION

Town: NORWALK  
 Monitoring Point I.D. No.: MW-100  
 DEP/WPC I.D. No: 103-092  
 Monitoring Point Location (relative to site features): WEST OF BUILDING #1 BETWEEN BUILDING & RTE 7  
 Drilling Contractor: GLENN DRILLING, INC. Supervising Engineer/Geologist: R.S. PATTERTON  
 Well Construction Method: HOLLOW STEM AUGERS / 3-INCH DRIVEN CASING

BARDANISE BUILDINGS  
 Site: 276 MAIN AVE.

Date of completion: 9/11/85

WELL INFORMATION (ELEVATIONS TO NEAREST 0.1 FEET)

Ground surface elevation (MSL): 81'  
 Well depth below ground surface: 47.7 FEET  
 Refusal: \_\_\_ Yes  No  
 Top of casing elevation (MSL): 81'  
 Screened interval: 42.7' - 47.7' BELOW GRADE  
 Length of Screen: 5 FEET  
 Length of riser pipe: 42 FEET  
 Screen type: SLOTTED PVC  
 Screen Slot size: 10 SLOT  
 Filter fabric: \_\_\_ Yes  No  
 Screen packing: \_\_\_ Yes  No  
 If yes, Thickness:  
 Material:  
 grain size:  
 Well inside diameter: 2 - INCHES  
 Impermeable Backfill: BENTONITE PELLETS CEMENT/BENTONITE GROUT  
 Well casing material and schedule: SCHEDULE 40 PVC  
 Estimated K screened interval: N/A  
 Method of well development: BAILING  
 Time spent developing: 15 MINS.  
 Locking \_\_\_ or threaded cap \_\_\_  
 Impermeable backfill: CEMENT  
 CURB BOX W/PENTAGONAL BOLT

Bedrock wells

Casing length: 42 FEET

Water-bearing rock unit: GNEISS

Water bearing sections (depths and approximate yields): NA

Length of rock core: 12 FEET

Diameter of core hole:  $2\frac{3}{8}$  - INCHES

Thickness and depth of impermeable backfill:

O-rings/seals:  Yes  NoGEOLOGIC INFORMATION

Aquifer: BEDROCK (GNEISS)

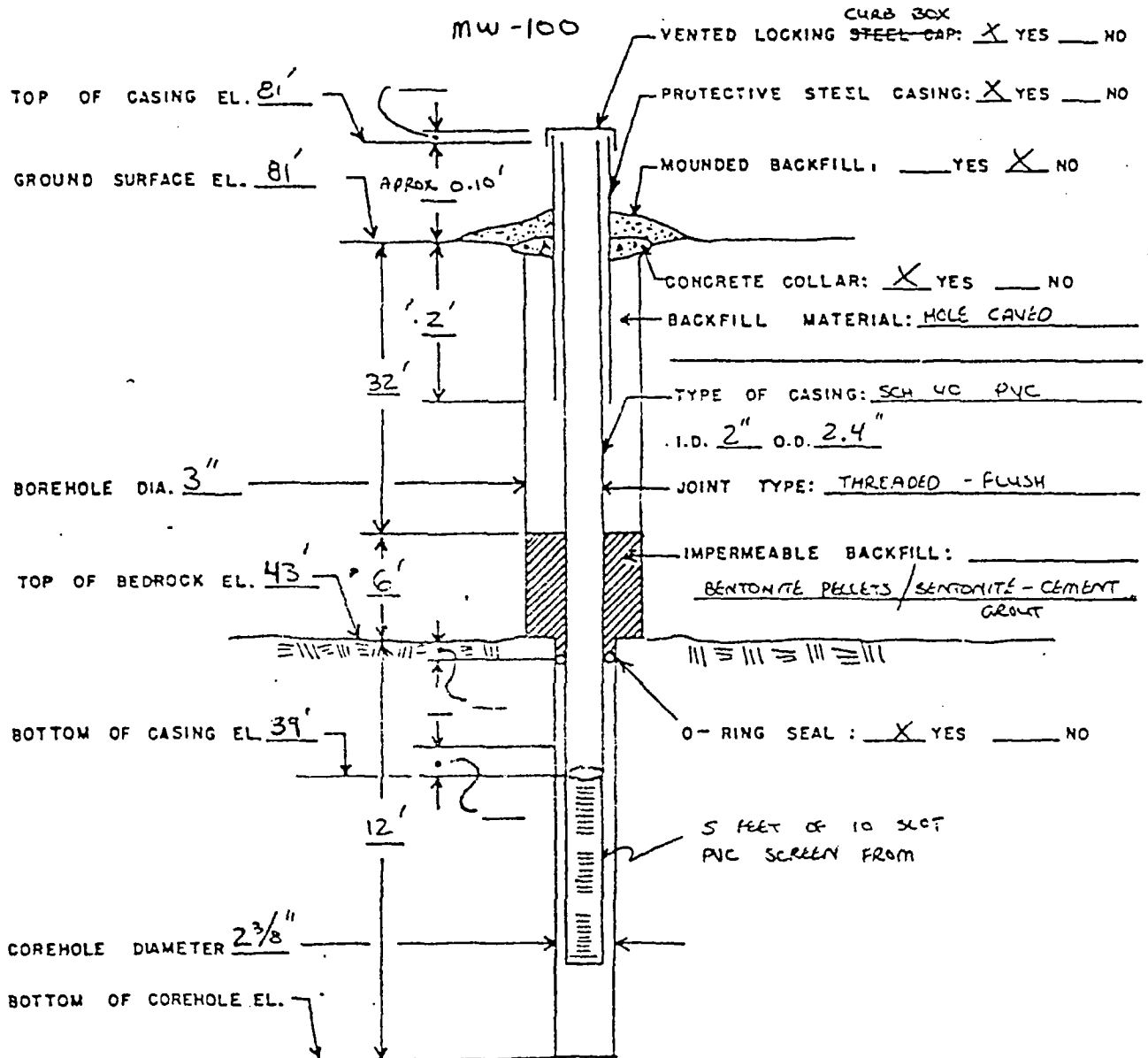
Inferred relationship to plume:  Within  Outside  Edge UNKNOWN

Watershed (plume discharge watercourse): NORWALK RIVER

Aquifer materials (attach boring log): <sup>BEDROCK</sup> (SEE LOG)

Attach maps and plans required of G.1.j. and G.4.

# MONITOR WELL INSTALLATION DETAIL FOR WELL IN BEDROCK



<b>Glenn Drilling Inc.</b> R.F.D. #1, Lake Road Fitchville, CT 06334 (203) 887-3621	CLIENT <u>Fuss &amp; O'Neill</u>	BORING NUMBER <u>MW-101</u>
	PROJECT NAME <u>KOSLOFF / NORWALK</u>	
	LOCATION <u>Norwalk, Ct.</u>	SHEET

OPERATOR <u>Michael Deane</u>	ARCHITECT ENGINEER	FILE NO. <u>926-8540</u>
INSPECTOR <u>Bob Potterton</u>	Casing <u>NW</u> Sampler <u>SS</u> Corr Barrel <u>NX</u>	SURFACE ELEV. _____
DATE START <u>Sept. 12, 1985</u>	TYPE _____ SIZE I.D. <u>3"</u> <u>1 1/2"</u> <u>2"</u>	LINE & STATION _____
DATE FINISH <u>Sept. 13, 1985</u>	HAMMER WT. <u>300#</u> <u>140#</u>	OFFSET _____
	HAMMER FALL <u>24"</u> <u>30"</u>	

DEPTH	SAMPLE				COL. A	STRA TA CHANGE	FIELD CLASSIFICATION AND REMARKS		
	NO.	DEPTH RANGE	BLOWS PER 6" ON SAMPLER					REC.	
			0-6	6-12					12-18
5'	1	5.0'-5.8'	61	100/13	.8		Brown fine-coarse Sand and Gravel. Cobbles and Boulders.		
10'	2	10.0'-10.5'	113		.5		Brown fine-coarse sand. Fine-medium Gravel. Cobbles and Boulders.		
	3	15.0'-16.0'	53	100	.9		Augered 12.0'. Moved Hole and Drove Casing.		
20'		20.0'-					No Sample Recovery.		
25'	4	25.0'-27.0'	33	41	29	.9	25.0' Brown fine-coarse Sand. Fine Gravel.		
30'	5	31.0'-33.0'	31	29	31	1.1	Brown fine-medium Sand. Cobbles		
35'	6	35.0'-35.9'	51	100/14	.5				
	R-1	36.0'-37.0'			.9	10	36.0' Cored Combination of Mica, Granite and Quartz.		
							End of Bring: 37.0'		

<b>SAMPLE IDENTIFICATION</b> SPLIT SPOON THIN WALL TUBE UNDISTURBED PISTON OPEN END ROD WASH SAMPLE AUGER SAMPLE	<b>PENETRATION RESISTANCE</b> 140 lb. Wt. falling 30" on 2" O.D. Sampler <table border="1"> <tr> <th>Cohesiveness</th> <th>Density</th> <th>Cohesive</th> <th>Consistency</th> </tr> <tr> <td>0-4</td> <td>Very Loose</td> <td>0-2</td> <td>Very Soft</td> </tr> <tr> <td>5-9</td> <td>Loose</td> <td>3-4</td> <td>Soft</td> </tr> <tr> <td>10-29</td> <td>Med. Dense</td> <td>5-8</td> <td>M/Stiff</td> </tr> <tr> <td>30-49</td> <td>Dense</td> <td>9-15</td> <td>Stiff</td> </tr> <tr> <td>50 -</td> <td>Very Dense</td> <td>16-30</td> <td>V-Stiff</td> </tr> <tr> <td></td> <td></td> <td>31 -</td> <td>Hard</td> </tr> </table>	Cohesiveness	Density	Cohesive	Consistency	0-4	Very Loose	0-2	Very Soft	5-9	Loose	3-4	Soft	10-29	Med. Dense	5-8	M/Stiff	30-49	Dense	9-15	Stiff	50 -	Very Dense	16-30	V-Stiff			31 -	Hard	<b>PROPORTIONS USED</b> Trace 0 to 10% little 10 to 20% some 20 to 35% and 35 to 60%	<b>REMARKS:</b> Installed 2" PVC Monitor Well Bottom at 35.4' 5.0' Screen: 35.0' Riser 1 Protective Casing COL. A <u>Coring Time</u>
Cohesiveness	Density	Cohesive	Consistency																												
0-4	Very Loose	0-2	Very Soft																												
5-9	Loose	3-4	Soft																												
10-29	Med. Dense	5-8	M/Stiff																												
30-49	Dense	9-15	Stiff																												
50 -	Very Dense	16-30	V-Stiff																												
		31 -	Hard																												

MONITOR WELL COMPLETION REPORTGENERAL INFORMATION

Town: NORWALK  
 Monitoring Point I.D. No.: MW-101  
 DEP/WPC I.D. No: 103-092  
 Monitoring Point Location (relative to site features):  
 Drilling Contractor: GLENN DRILLING, INC. Supervising Engineer/Geologist: R.S. POTTERTON  
 Well Construction Method: HOLLOW STEM AUGERS - 3-INCH DRIVEN CASING

BARDANISE BUILDINGS  
 Site: 276 MAIN AVE.

Date of completion: 9/13/85

NORTHWEST CORNER OF BUILDING # 1

WELL INFORMATION (ELEVATIONS TO NEAREST 0.1 FEET)

Ground surface elevation (MSL): 80 FEET  
 Well depth below ground surface: 35.4'  
 Refusal:  Yes  No  
 Top of casing elevation (MSL): 80 FEET  
 Screened interval: 30.4' - 35.4' BELOW GRADE  
 Length of Screen: 5 FEET  
 Length of riser pipe: 35 FEET  
 Screen type: SLOTTED PVC  
 Screen Slot size: 10 SLOT  
 Filter fabric:  Yes  No  
 Screen packing:  Yes  No  
 If yes, Thickness: 1/2 - INCH  
 Well inside diameter: 2 - INCHES  
 Material: QUARTZ SAND  
 grain size: MEDIUM  
 Impermeable Backfill: BENTONITE PELLETS / CEMENT / BENTONITE GROUT  
 Well casing material and schedule: SCHEDULE 40 PVC  
 Estimated K screened interval: NA  
 Method of well development: BAILING  
 Time spent developing: 15 MINS.  
 Locking  or threaded cap   
 Impermeable backfill: CEMENT  
 CURB BOX W/PENTAGONAL BOLT



Bedrock wells

Casing length:

Water-bearing rock unit:

Water bearing sections (depths and approximate yields):

Length of rock core:

Diameter of core hole:

Thickness and depth of impermeable backfill:

C-rings seals:  Yes  NoGEOLOGIC INFORMATION

Aquifer: STRATIFIED GLACIAL DEPOSITS

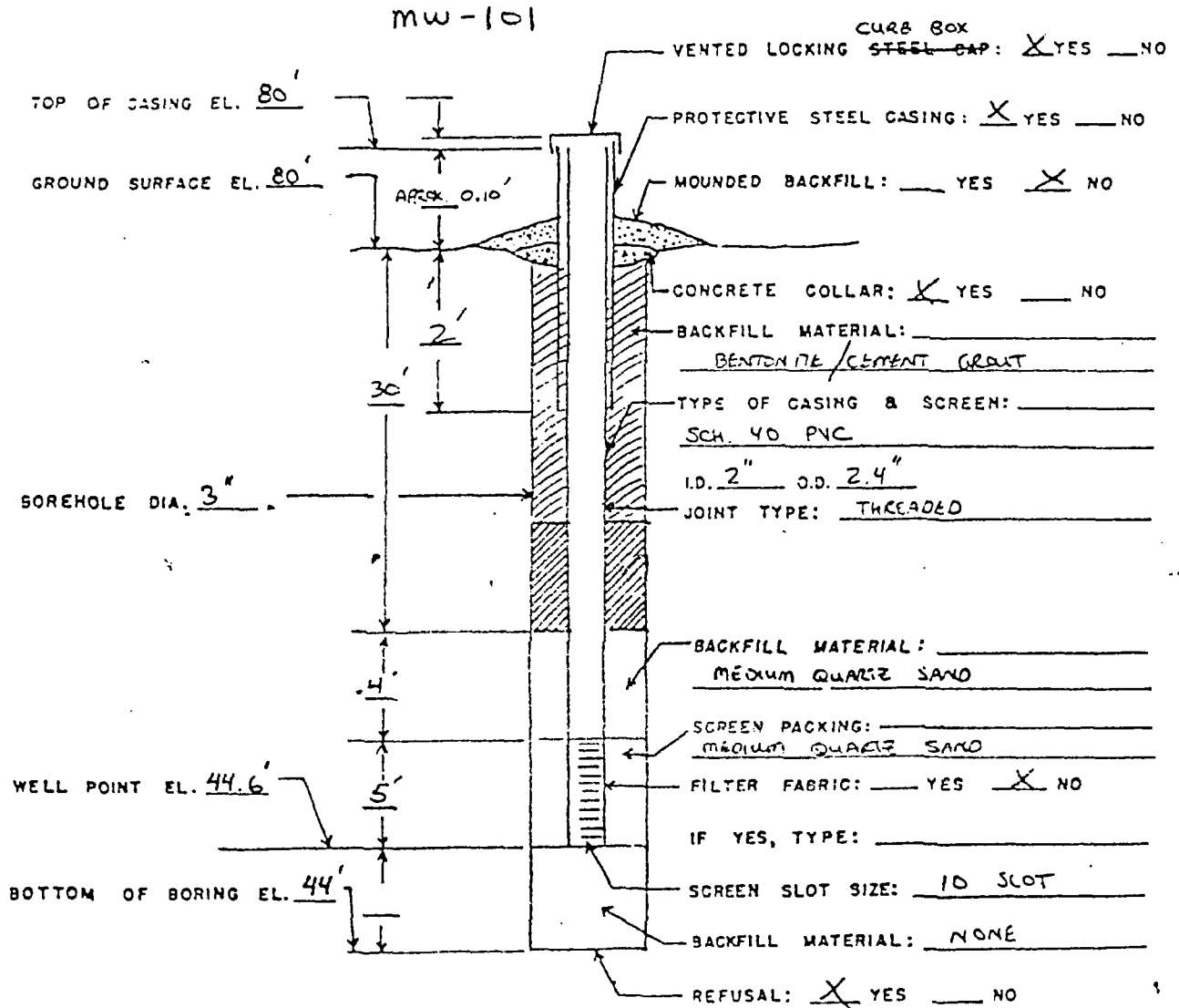
Inferred relationship to plume:  Within  Outside  Edge UNKNOWN

Watershed (plume discharge watercourse): NORWALK RIVER

Aquifer materials (attach boring log): SAND &amp; GRAVEL

Attach maps and plans required of G.1.j. and G.4.

# MONITOR WELL INSTALLATION DETAIL FOR WELL IN UNCONSOLIDATED DEPOSIT



**Glenn Drilling Inc.**

R.F.D. #1, Lake Road  
Fitchville, CT 06334  
(203) 887-3621

CLIENT Eves & O'Neill

PROJECT NAME Kesoff/Norwalk

LOCATION Norwalk, Ct.

BORING NUMBER  
MW-102

SHEET  
No. 1  
of 1

LER Michael Deane

ARCHITECT  
ENGINEER

FILE NO. 926-8540

INSPECTOR Bob Potterton

TYPE Casing HW Sampler SS Core Barrel NX

SURFACE ELEV. \_\_\_\_\_

DATE START Sept. 16, 1985

SIZE I.D. 1" 1 1/2" 2"

LINE & STATION \_\_\_\_\_

DATE FINISH Sept. 17, 1985

HAMMER WT. 300# 140#

HAMMER FALL 24" 30"

OFFSET \_\_\_\_\_

DEPTH	SAMPLE					COL. A	STRATA CHANGE	FIELD CLASSIFICATION AND REMARKS	
	NO.	DEPTH RANGE	BLOWS PER 6" ON SAMPLER						REC.
			0-6	6-12	12-18				
5'	1	5.0'-6.0'	76	103		.9	5.0'	Brown fine-coarse Sand. Weathered Rock. Auger to 6.0'. Moved Hole and Drove Casing. Cored Rock @ 6'	
	R-1	5.0'-10.0'				1.1			
10'								19.0'	
	R-2	10.0'-15.0'				1.5			
	R-3	15.0'-19.0'				1.3			
20'								End of Boring: 19.0'	
								Installed 2" PVC Monitor Well	
								Bottom set at 15.5'	
								5.0' Screen	
								10.0 Riser	
								1 Protective Casing and Plug	

**SAMPLE IDENTIFICATION**

- SPLIT SPOON
- THIN WALL TUBE
- U UNDISTURBED PISTON
- O OPEN END ROD
- W WASH SAMPLE
- A AUGER SAMPLE

**PENETRATION RESISTANCE**  
140 lb. Wt. falling 30" on 2" O.D. Sampler

Cohesiveness	Density	Cohesive Consistency
0-4	Very Loose	0-2 Very Soft
5-9	Loose	3-4 Soft
10-29	Med. Dense	5-8 M/Still
30-49	Dense	9-15 Stiff
50 +	Very Dense	16-30 V-Stiff
		31 + Marg

**PROPORTIONS USED**

trace	0 to 10%
little	10 to 20%
some	20 to 35%
and	35 to 50%

**REMARKS:**  
COL. A Coring Time

MONITOR WELL COMPLETION REPORTGENERAL INFORMATION

Town: NORWALK

BARDANISE BUILDINGS  
Site: 276 MAIN AVE.

Monitoring Point I.D. No.: MW-102

Date of completion: 9/17/85

DEP/WPC I.D. No: 103-092

Monitoring Point Location (relative to site features): 114 FEET EAST OF BUILDING #2

Drilling Contractor: GLENN DRILLING, INC. Supervising Engineer/Geologist: R.S. POTTERTON

Well Construction Method: HOLLOW STEM AUGERS - AND 4-INCH DRIVEN CASING

WELL INFORMATION (ELEVATIONS TO NEAREST 0.1 FEET)

Ground surface elevation (MSL):

101 FEET

Well depth below ground surface: 15.5'

Refusal: \_\_\_ Yes X No

Top of casing elevation (MSL): 101 FEET

Screened interval:

Length of Screen: 5 FEET

10.5 - 15.5 FEET BELOW GRADE

Length of riser pipe: 10 FEET

Screen type: SLOTTED PVC

Screen Slot size: 10 SLOT

Filter fabric: \_\_\_ Yes X NoScreen packing: \_\_\_ Yes X No

If yes, Thickness:

Well inside diameter: 2-INCHES

Material:

grain size:

Impermeable Backfill: BENTONITE PELLETS  
CEMENT/BENTONITE GROUTWell casing material and schedule:  
SCHEDULE 40 PVC

Estimated K screened interval: NA

Method of well development: BAILING

Time spent developing: 15 MINS.

Locking \_\_\_ or threaded cap \_\_\_  
CURB BOX W/PENTAGONAL BOLT

Impermeable backfill: CEMENT

Bedrock wells

Casing length: 10 FEET

Water-bearing rock unit: GNEISS

Water bearing sections (depths and approximate yields): NA

Length of rock core: 13 FEET

Diameter of core hole: 2 3/8"

Thickness and depth of impermeable backfill: 0.8" THICK FROM 9 FEET BELOW GRADE TO GRADE.

O-ring seals:  Yes  NoGEOLOGIC INFORMATION

Aquifer: BEDROCK (GNEISS)

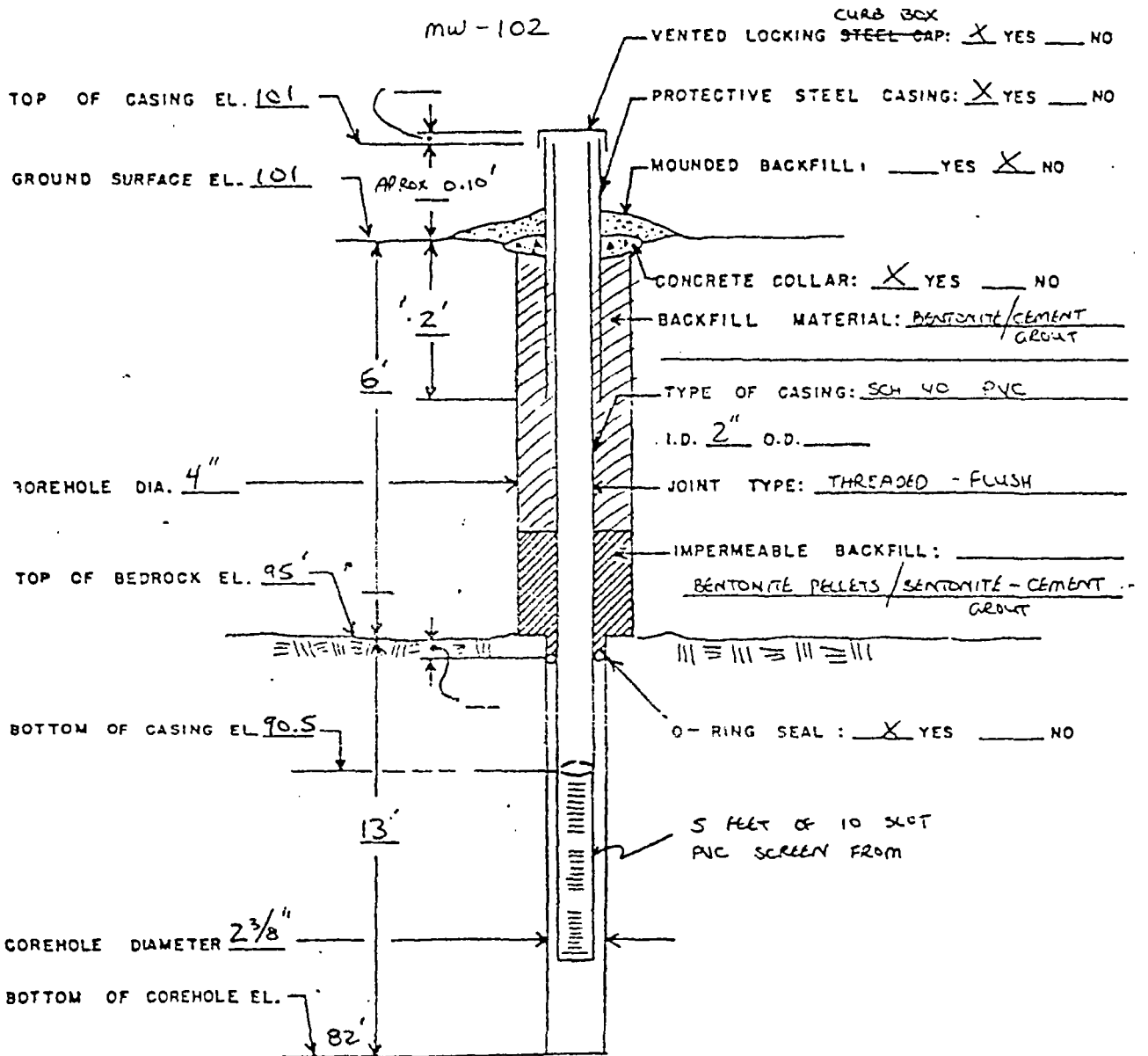
Inferred relationship to plume:  Within  Outside  Edge UNKNOWN

Watershed (plume discharge watercourse): NORWALK RIVER

Aquifer materials (attach boring log): BEDROCK

Attach maps and plans required of G.1.j. and G.4.

# MONITOR WELL INSTALLATION DETAIL FOR WELL IN BEDROCK



**Glenn Drilling Inc.**

R.F.D. #1, Lake Road  
Fitchville, CT 06334  
(203) 887-3621

CLIENT Fuss & O'Neill

PROJECT NAME KOYLOFF / NORWALK

LOCATION Norwalk, Ct.

BORING NUMBER  
**MW-103**

SHEET  
No. 1  
of 1

DRILLER Michael Deane

ARCHITECT  
ENGINEER

FILE NO. 926-8570

INSPECTOR Bob Potterton

	Casing	Sampler	Core Barrel
TYPE	<u>HW</u>	<u>SS</u>	<u>VX</u>
SIZE I.D.	<u>4"</u>	<u>1 1/2"</u>	<u>2"</u>
HAMMER WT.	<u>300#</u>	<u>140#</u>	
HAMMER FALL	<u>24"</u>	<u>30"</u>	

SURFACE ELEV. \_\_\_\_\_

DATE START Sept 17, 1985

LINE & STATION \_\_\_\_\_

DATE FINISH Sept 18, 1985

OFFSET \_\_\_\_\_

DEPTH	SAMPLE						COL. A	STRATA CHANGE	FIELD CLASSIFICATION AND REMARKS
	NO.	DEPTH RANGE	BLOWS PER 6" ON SAMPLER			REC.			
			0-6	6-12	12-18				
5'	1	5.0'-7.0'	6	8	16	1.2		Brown fine sand.	
			37						
10'	R-1	10.0'-15.0'				2.5	2	10.0'	Cored Granite, Quartz.
							5		
							8		
							13		
							5		
	R-2	15.0'-20.0'				1.2	1	20.0'	
							9		
							7		
							11		
							1		
									End of Boring: 20.0'
									Installed 2" PVC Monitor Well Bottom set at 15.8' 5.0' Screen 10.8' Riser 1 Protective Casing with Plug

- SAMPLE IDENTIFICATION**
- T — SPLIT SPOON
  - U — THIN WALL TUBE
  - O — UNDISTURBED PISTON
  - W — OPEN END ROD
  - A — WASH SAMPLE
  - A — AUGER SAMPLE

**PENETRATION RESISTANCE**  
140 lb. Wt. falling 30" on 2" O.D. Sampler

Cohesionless Density		Cohesive Consistency	
0-4	Very Loose	0-2	Very Soft
5-9	Loose	3-4	Soft
10-29	Med. Dense	5-8	M/Still
30-49	Dense	9-15	Stiff
50 -	Very Dense	16-30	V-Still
		31 -	Hard

**PROPORTIONS USED**

trace 0 to 10%  
little 10 to 20%  
some 20 to 35%  
and 35 to 50%

**REMARKS:**

COL. A Coring Time

MONITOR WELL COMPLETION REPORTGENERAL INFORMATION

Town: NORWALK  
 Monitoring Point I.D. No.: MW-103  
 DEP/WPC I.D. No: 103 - 092  
 Monitoring Point Location (relative to site features): 17' SOUTH OF BUILDING 2  
 Drilling Contractor: GLENN DRILLING, INC. Supervising Engineer/Geologist: R.S. POTTERTON  
 Well Construction Method: HOLLOW STEM AUGERS AND 4-INCH DRIVEN CASING

BARDANISE BUILDINGS

Site: 276 MAIN AVE.

Date of completion: 9/18/05

WELL INFORMATION (ELEVATIONS TO NEAREST 0.1 FEET)

Ground surface elevation (MSL): 83'  
 Well depth below ground surface:  
 Refusal: \_\_\_ Yes X No  
 Top of casing elevation (MSL): 83'  
 Screened interval:  
 Length of Screen: 5 FEET 10.8 - 15.8 FEET BELOW GRADE  
 Length of riser pipe: 10.8 FEET  
 Screen type: SLOTTED PVC  
 Screen Slot size: 10 SLOT  
 Filter fabric: \_\_\_ Yes X No  
 Screen packing: \_\_\_ Yes X No  
 Well inside diameter: 2 -INCHES  
 If yes, Thickness:  
 Material:  
 grain size:  
 Well casing material and schedule: SCHEDULE 40 PVC  
 Impermeable Backfill: BENTONITE PELLETS CEMENT/BENTONITE GROUT  
 Method of well development: BAILING  
 Estimated K screened interval:  
 Time spent developing: 15 MINS.  
 Locking \_\_\_ or threaded cap \_\_\_  
 Impermeable backfill: CEMENT  
 CURB BOX W/PENTAGONAL BOLT



Bedrock wells

Casing length: 10.8 FEET

Water-bearing rock unit: GNEISS

Water bearing sections (depths and approximate yields): NA

Length of rock core: 10 FEET

Diameter of core hole: 2 3/8 - INCHES

Thickness and depth of impermeable backfill: 10.5' THICK (FROM 10.5 FEET TO GRADE)

O-rings/seals:  Yes  NoGEOLOGIC INFORMATION

Aquifer: BEDROCK (GNEISS)

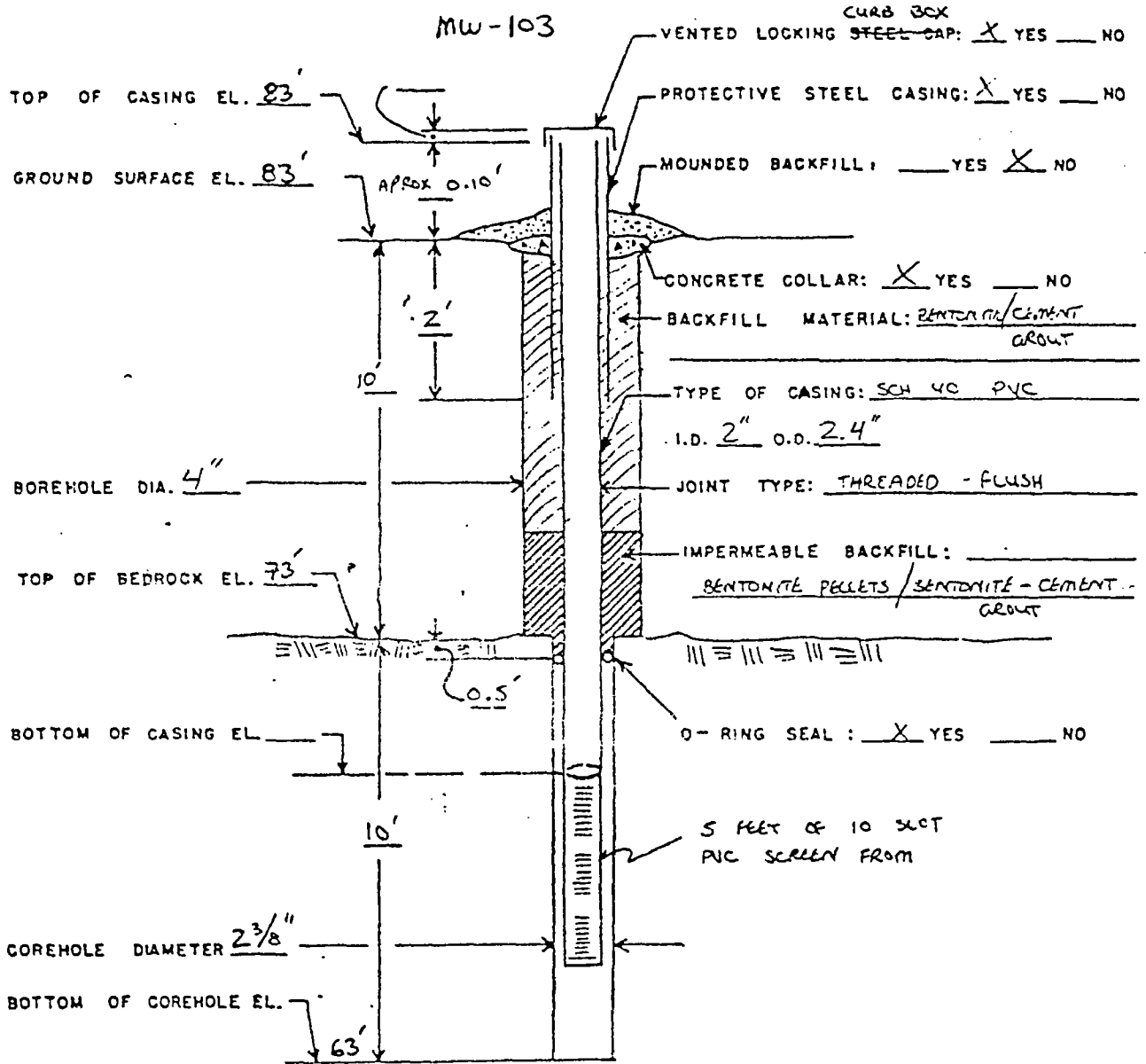
Inferred relationship to plume:  Within  Outside  Edge UNKNOWN

Watershed (plume discharge watercourse): NEWALK RIVER

Aquifer materials (attach boring log): BEDROCK

Attach maps and plans required of G.1.j. and G.4.

# MONITOR WELL INSTALLATION DETAIL FOR WELL IN BEDROCK



**Glenn Drilling Inc.**  
 R.F.D. #1, Lake Road  
 Fitchville, CT 06334  
 (203) 887-3821

CLIENT Fuss & O'Neill  
 PROJECT NAME KOSLOFF/NORWALK  
 LOCATION Norwalk, Ct.

BORING NUMBER  
MW-104  
 SHEET  
 No. 1  
 of 1

DRILLER Michael Deane  
 INSPECTOR Bob Potterton  
 DATE START Sept. 17, 1985  
 DATE FINISH Sept. 18, 1985

ARCHITECT ENGINEER  
 TYPE HW Casing SS Core Barrel NX  
 SIZE I.D. 4" 1 1/2" 2"  
 HAMMER WT. 300# 140#  
 HAMMER FALL 22" 30"

FILE NO. 926-8540  
 SURFACE ELEV. \_\_\_\_\_  
 LINE & STATION \_\_\_\_\_  
 OFFSET \_\_\_\_\_

DEPTH	SAMPLE							COL. A	STRATA CHANGE	FIELD CLASSIFICATION AND REMARKS
	NO.	DEPTH RANGE	BLOWS PER 6" ON SAMPLER			REC.	A			
			0-6	6-12	12-18					
5'	1	5.0'-5.6'	65	100	1	.6		6.0'	Brown fine-coarse Sand. Fine-medium gravel. Cobbles and Boulders.	
	R-1	6.0'-11.0'				1.7	5			
10'							11	15.7'	Cored Rock.	
							15			
							13			
	R-2	11.0'-13.0'				.8	25			
							21			
15'	R-3	13.0'-15.7'				1.0	17		End of Boring: 15.7'  Installed 2" PVC Monitor Well Bottom set at 14.2' 5.0' Screen 9.8' Riser 1 Protective Casing and Plug.	
							10			
							19			

<b>SAMPLE IDENTIFICATION</b> S — SPLIT SPOON T — THIN WALL TUBE U — UNDISTURBED PISTON O — OPEN END ROD W — WASH SAMPLE A — AUGER SAMPLE	<b>PENETRATION RESISTANCE</b> 140 lb. Wt. falling 30" on 2" O.D. Sampler Cohesionless Density      Consesive Consistency				<b>PROPORTIONS USED</b> trace 0 to 10% little 10 to 20% some 20 to 35% and 35 to 50%	<b>REMARKS:</b>  COL. A <u>Coring Time</u>
	0-4	Very Loose	0-2	Very Soft		
	5-9	Loose	3-4	Soft		
	10-29	Med. Dense	5-8	M/Stiff		
30-49	Dense	9-15	Stiff			
50+	Very Dense	16-30	V-Stiff			
		31+	Hard			

MONITOR WELL COMPLETION REPORTGENERAL INFORMATION

Town: NORWALK

BARDANISE BUILDINGS  
Site: 276 MAIN AVE.

Monitoring Point I.D. No.: MW-104

Date of completion: 9/18/85

DEP/WPC I.D. No: 103-092

Monitoring Point Location (relative to site features): 5 FEET SOUTH OF  
BUILDING # 1

Drilling Contractor: GLENN DRILLING, INC. Supervising Engineer/Geologist: R.S. PATTISON

Well Construction Method: HOLLOW STEM AUGERS - AND 4-INCH DRIVEN CASING

WELL INFORMATION (ELEVATIONS TO NEAREST 0.1 FEET)

Ground surface elevation (MSL):

81 FEET

Well depth below ground surface:

Refusal: \_\_\_ Yes X No

Top of casing elevation (MSL): 81 FEET

Screened interval:

Length of Screen: 5 FEET

9.2 - 14.2 FEET BELOW GRADE

Length of riser pipe: 9.8 FEET

Screen type: SLOTTED PVC

Screen slot size: 10 SLOT

Filter fabric: \_\_\_ Yes X NoScreen packing: \_\_\_ Yes X No

If yes, Thickness:

Well inside diameter: 2-INCHES

Material:

grain size:

Impermeable Backfill: BENTONITE PELLETS  
CEMENT/BENTONITE GROUTWell casing material and schedule:  
SCHEDULE 40 PVC

Estimated K screened interval: NA

Method of well development: BAILING

Time spent developing: 15 MINS.

Locking \_\_\_ or threaded cap \_\_\_  
CURB BOX W/PENTAGONAL BOLT

Impermeable backfill: CEMENT

Bedrock wells

Casing length: 9.8 FEET

Water-bearing rock unit: GNEISS

Water bearing sections (depths and approximate yields): NA

Length of rock core: 9.7 FEET

Diameter of core hole: 2<sup>3</sup>/<sub>8</sub> - INCHES

Thickness and depth of impermeable backfill: 4 FEET (FROM 7' TO 3' BELOW GRADE)

O-ring seal :  Yes  NoGEOLOGIC INFORMATION

Aquifer: BEDROCK (GNEISS)

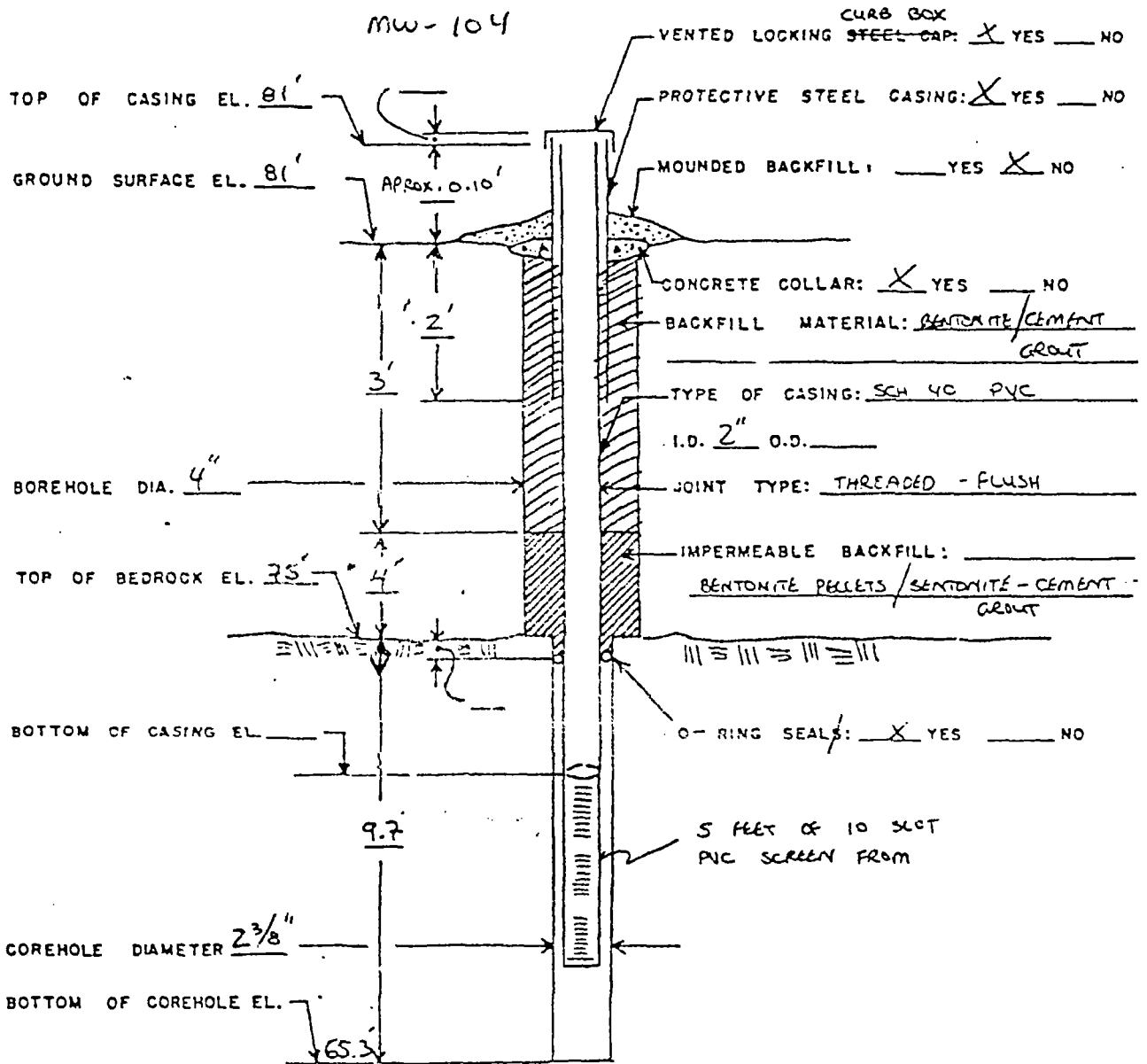
Inferred relationship to plume:  Within  Outside  Edge

Watershed (plume discharge watercourse): NORWALK RIVER

Aquifer, materials (attach boring log): BEDROCK

Attach maps and plans required of G.1.j. and G.4.

# MONITOR WELL INSTALLATION DETAIL FOR WELL IN BEDROCK



**Glenn Drilling Inc.**  
 R.F.D. #1, Lake Road  
 Fitchville, CT 06334  
 (203) 887-3621

CLIENT Fuss & O'Neill  
 PROJECT NAME KCSLOFF/NORWALK  
 LOCATION Norwalk, Ct.

BORING NUMBER MA-105  
 SHEET No. 1 of 1

DRILLER Michael Deane  
 INSPECTOR B&B POTTERSON / Tim BINGHAM  
 DATE START Sept. 20, 1985  
 DATE FINISH Sept. 23, 1985

ARCHITECT ENGINEER  
 TYPE HW Casing SS Sampler NX Core Barrel  
 SIZE I.D. 4" 1 1/2"  
 HAMMER WT. 300# 140#  
 HAMMER FALL 24" 30"

FILE NO. 926-85/0  
 SURFACE ELEV. \_\_\_\_\_  
 LINE & STATION \_\_\_\_\_  
 OFFSET \_\_\_\_\_

DEPTH	SAMPLE						COL. A	STRATA CHANGE	FIELD CLASSIFICATION AND REMARKS
	NO.	DEPTH RANGE	BLOWS PER 6" ON SAMPLER			REC.			
			0-6	6-12	12-18				
5'	1	5.0'-7.0'	48	90	56	1.2		Brown fine-coarse sand and gravel Weathered Rock.	
10'	2	10.0'-10.8'	5	100/13		9		Brown fine-coarse sand and gravel. Weathered Rock.	
15'	3	15.0'-16.5'	31	33	102	9		Dark Brown fine-coarse Sand and Gravel. Weathered Rock.	
20'	R-1	18.0'-23.0'				1.9	17	ROCK AT 17'	
25'	R-2	23.0'-27.5'					23		
							27.5'	End of Boring: 27.5'	
								Installed 2" PVC Monitor Well Bottom set at 24.0' 5.0' Screen 19.0' Riser 1 Protective Casing and Plug Reamed hole with Roller Bit to 25.0' Telescoped NW Casing to maintain Hole.	

- SAMPLE IDENTIFICATION**
- S — SPLIT SPOON
  - T — THIN WALL TUBE
  - U — UNDISTURBED PISTON
  - O — OPEN END ROD
  - W — WASH SAMPLE
  - A — AUGER SAMPLE

**PENETRATION RESISTANCE**  
 140 lb. Wt. falling 30" on 2" O.D. Sampler

Cohesionless Density		Cohesive Consistency	
0-4	Very Loose	0-2	Very Soft
5-9	Loose	3-4	Soft
10-29	Med. Dense	5-8	M/Stiff
30-49	Dense	9-15	Stiff
50 +	Very Dense	16-30	V-Stiff
		31 +	Hard

**PROPORTIONS USED**

trace 0 to 10%  
 little 10 to 20%  
 some 20 to 35%  
 and 35 to 50%

**REMARKS:**

COL. A Corring Time

MONITOR WELL COMPLETION REPORTGENERAL INFORMATION

Town: NORWALK

BARDANISE BUILDINGS  
Site: 276 MAIN AVE.

Monitoring Point I.D. No.: MW-105

Date of completion: 9/23/85

DEP/WPC I.D. No: 103-092

Monitoring Point Location (relative to site features): 10 FEET NORTH OF BUILDING 1

Drilling Contractor: GLENN DRILLING, INC. Supervising Engineer/Geologist: R.S. POTTERTON

Well Construction Method: DRIVE 4-INCH CASING / NX CORING IN SEDUCK

WELL INFORMATION (ELEVATIONS TO NEAREST 0.1 FEET)

Ground surface elevation (MSL):

83'

Well depth below ground surface:

Refusal: \_\_\_ Yes X No

Top of casing elevation (MSL): 83'

Screened interval:

Length of Screen: 5 FEET

19.5' - 24.5' BELOW GRADE

Length of riser pipe: 19 FEET

Screen type: SLOTTED PVC

Screen Slot size: 10 SLOT

Filter fabric: \_\_\_ Yes X NoScreen packing: \_\_\_ Yes X No

If yes, Thickness:

Well inside diameter: 2 - INCHES

Material:

grain size:

Impermeable Backfill: BENTONITE PELLETS  
CEMENT/BENTONITE GROUTWell casing material and schedule:  
SCHEDULE 40 PVC

Estimated K screened interval: NA

Method of well development: BAILING

Time spent developing: 15 MINS.

Locking \_\_\_ or threaded cap \_\_\_  
CURB BOX W/PENTAGONAL BOLT

Impermeable backfill: CEMENT



Bedrock wells

Casing length: 19 FEET

Water-bearing rock unit: GNEISS

Water bearing sections (depths and approximate yields): NA

Length of rock core: 8 FEET

Diameter of core hole: 2 <sup>3</sup>/<sub>8</sub> - INCHES

Thickness and depth of impermeable backfill: 9 FEET (10'-19' BELOW GRADE)

O-rings seals:  Yes  NoGEOLOGIC INFORMATION

Aquifer: BEDROCK (GNEISS)

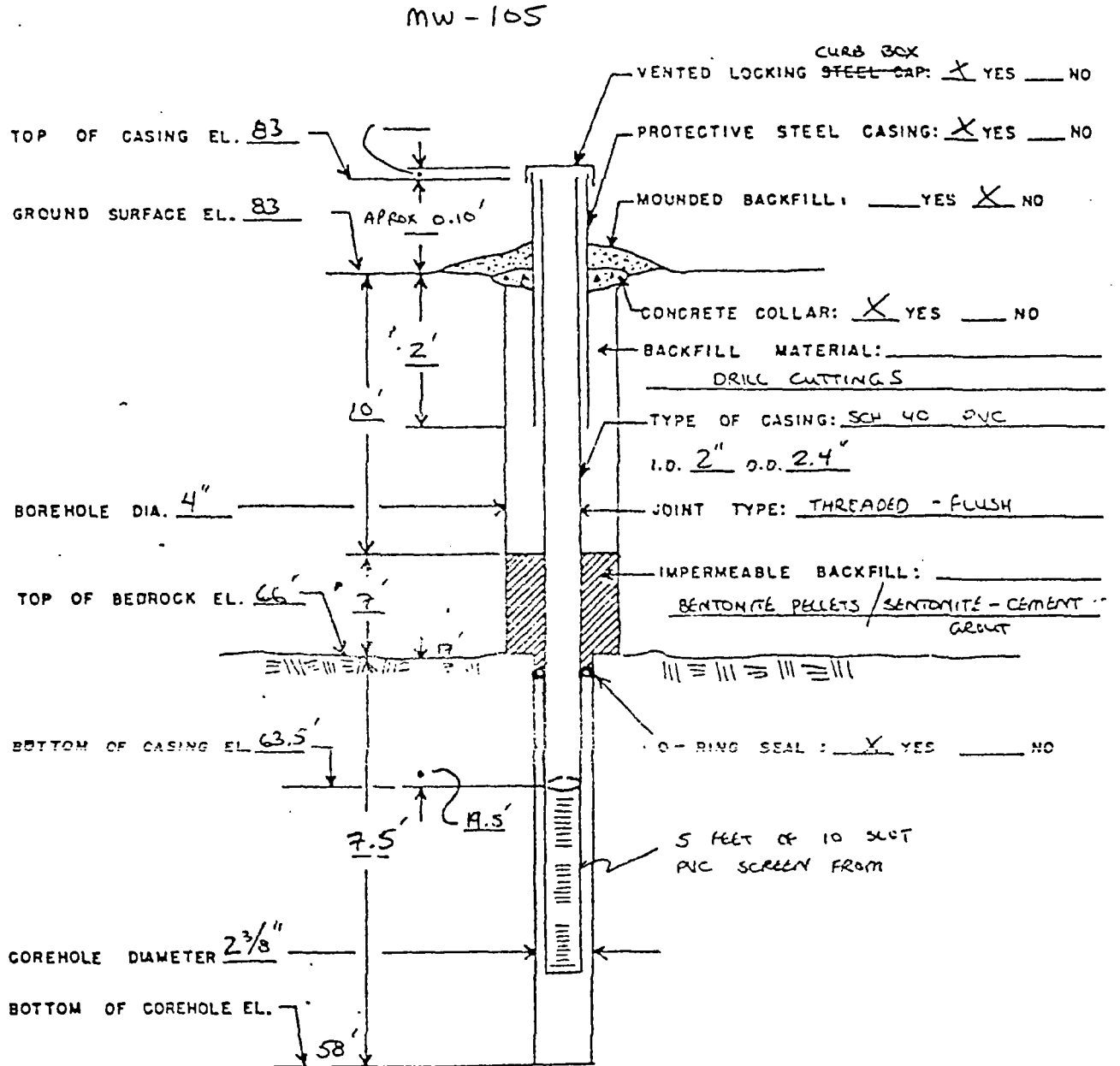
Inferred relationship to plume:  Within  Outside  Edge UNKNOWN

Watershed (plume discharge watercourse): NORWALK RIVER

Aquifer materials (attach boring log): SAND &amp; GRAVEL / BEDROCK

Attach maps and plans required of G.1.j. and G.4.

# MONITOR WELL INSTALLATION DETAIL FOR WELL IN BEDROCK



**Glenn Drilling Inc.**

R.F.D. #1, Lake Road  
Fitchville, CT 06334  
(203) 887-3621

CLIENT Engg & O'Neill

PROJECT NAME KCSLCH/NORWALK

LOCATION Norwalk, Ct.

BORING NUMBER  
MW-106

SHEET  
No. 1  
of 1

OWNER Michael Deane

ARCHITECT  
ENGINEER

FILE NO. 926-8540

INSPECTOR Bob Potterton

Casing HW Sampler          Core Barrel         

SURFACE ELEV.         

DATE START Sept. 23, 1985

TYPE         

LINE & STATION         

DATE FINISH Sept. 24, 1985

SIZE I.D. 4"

HAMMER WT. 300#

HAMMER FALL 24"

OFFSET         

DEPTH	SAMPLE					COL. A	STRATA CHANGE	FIELD CLASSIFICATION AND REMARKS	
	NO.	DEPTH RANGE	BLOWS PER 6" ON SAMPLER						REC.
			0-6	6-12	12-18				
5								Drove Casing to 16.5'.	
10								End of Boring: 16.5'  Installed 2" PVC Monitor Well Bottom set at 16.0' 5.0' Screen 11.0' Riser 1 Protective Casing and Plug.	

<b>SAMPLE IDENTIFICATION</b> . — SPLIT SPOON — THIN WALL TUBE U — UNDISTURBED PISTON O — OPEN END ROD W — WASH SAMPLE A — AUGER SAMPLE	<b>PENETRATION RESISTANCE</b> 140 lb. Wt. falling 30" on 2" O.D. Sampler		<b>PROPORTIONS USED</b>		<b>REMARKS:</b>  COL. A _____
	Cone/penetration Density		Cohesive Consistency		
	0-4	Very Loose	0-2	Very Soft	
	5-9	Loose	3-4	Soft	
	10-29	Med. Dense	5-8	M/Stiff	
30-49	Dense	9-15	Stiff		
50 -	Very Dense	16-30	V-Stiff		
		31 -	Hard		
			trace 0 to 10%		
			little 10 to 20%		
			some 20 to 25%		
			and 35 to 50%		

MONITOR WELL COMPLETION REPORTGENERAL INFORMATION

Town: NORWALK BARDANISE BUILDINGS  
 Site: 276 MAIN AVE.  
 Monitoring Point I.D. No.: MW-106 Date of completion: 9/24/85  
 DEP/WPC I.D. No: 103-092  
 Monitoring Point Location (relative to site features): NORTH OF BUILDING #1  
 Drilling Contractor: GLENN DRILLING, INC. Supervising Engineer/Geologist: R.S. PATTISON  
 Well Construction Method: DRY 4-INCH CASING

WELL INFORMATION (ELEVATIONS TO NEAREST 0.1 FEET)

Ground surface elevation (MSL): 83'	Well depth below ground surface: Refusal: <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No
Top of casing elevation (MSL): 83'	Screened interval: 11.5' - 16.5' BELOW GRADE
Length of Screen: 5 FEET	Screen Slot size: 10 SLOT
Length of riser pipe: 11 FEET	Screen packing: <input checked="" type="checkbox"/> Yes <input type="checkbox"/> No
Screen type: SLOTTED PVC	If yes, Thickness: 5 1/2 - FEET
Filter fabric: <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No	Material: QUARTZ SAND
Well inside diameter: 2 - INCHES	grain size: MEDIUM
Well casing material and schedule: SCHEDULE 40 PVC	Impermeable Backfill: <sup>BENTONITE PELLETS</sup> CEMENT/BENTONITE GROUT
Method of well development: BAILING	Estimated K screened interval: NA
Locking <input type="checkbox"/> or threaded cap <input type="checkbox"/>	Time spent developing: 15 MINS.
CURB BOX w/PENTAGONAL BOLT	Impermeable backfill: CEMENT

Bedrock wells

Casing length:

Water-bearing rock unit:

Water bearing sections (depths and approximate yields):

Length of rock core:

Diameter of core hole:

Thickness and depth of impermeable backfill:

O-rings seals:  Yes  NoGEOLOGIC INFORMATION

Aquifer: SAND &amp; GRAVEL

Inferred relationship to plume:  Within  Outside  Edge UNKNOWN

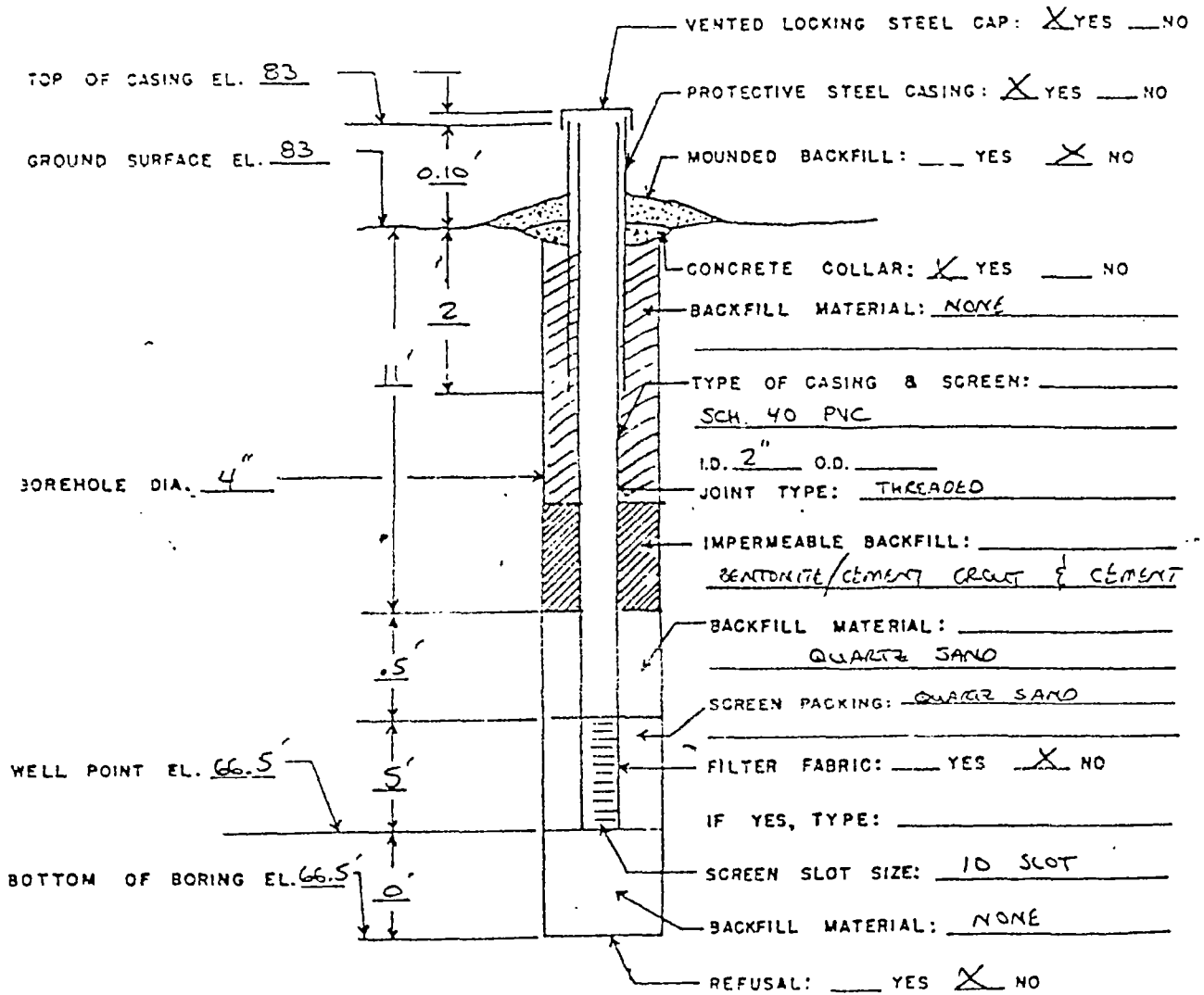
Watershed (plume discharge watercourse): NORWALK RIVER

Aquifer materials (attach boring log): SAND &amp; GRAVEL (ASSUMED SIZES)

Attach maps and plans required of G.1.j. and G.4.

# MONITOR WELL INSTALLATION DETAIL FOR WELL IN UNCONSOLIDATED DEPOSIT

mw-106





PROJECT KELLOGG-DEERING

PROJECT NO. 3781-16 BORING K-10

ELEVATION 82.16' ±1 DATE 2/20/95

FIELD GEOLOGIST JEFF ORIENT

SAMPLE NO, TYPE & DEPTH (ft)	BLOWS/SIX INCHES OR RQD (%)	SAMPLE RECOVERY/ SAMPLE LENGTH (ft)	MATERIAL MOISTURE & WATER DEPTH (ft)	MATERIAL DESCRIPTION*			USCS OR ROCK BROKENNESS	REMARKS
				SOIL DENSITY/ CONSISTENCY OR ROCK HARDNESS	COLOR	MATERIAL CLASSIFICATION		
						SAND GRAVEL SCORLES		
								DRILLED TO 16.0' THEN MOVED RE-DRILLED - GROUPED UP ORIGINAL BORING
						9.0' ↓ ↓ 9.0'		
						FELSIC GNEISS		
								CASING SET TO 15.0'

REMARKS BORING LOCATED BEHIND NEW ENGLAND QUARTZ

BORING K-10

\*SEE LEGEND ON BACK

PAGE 1 OF 3

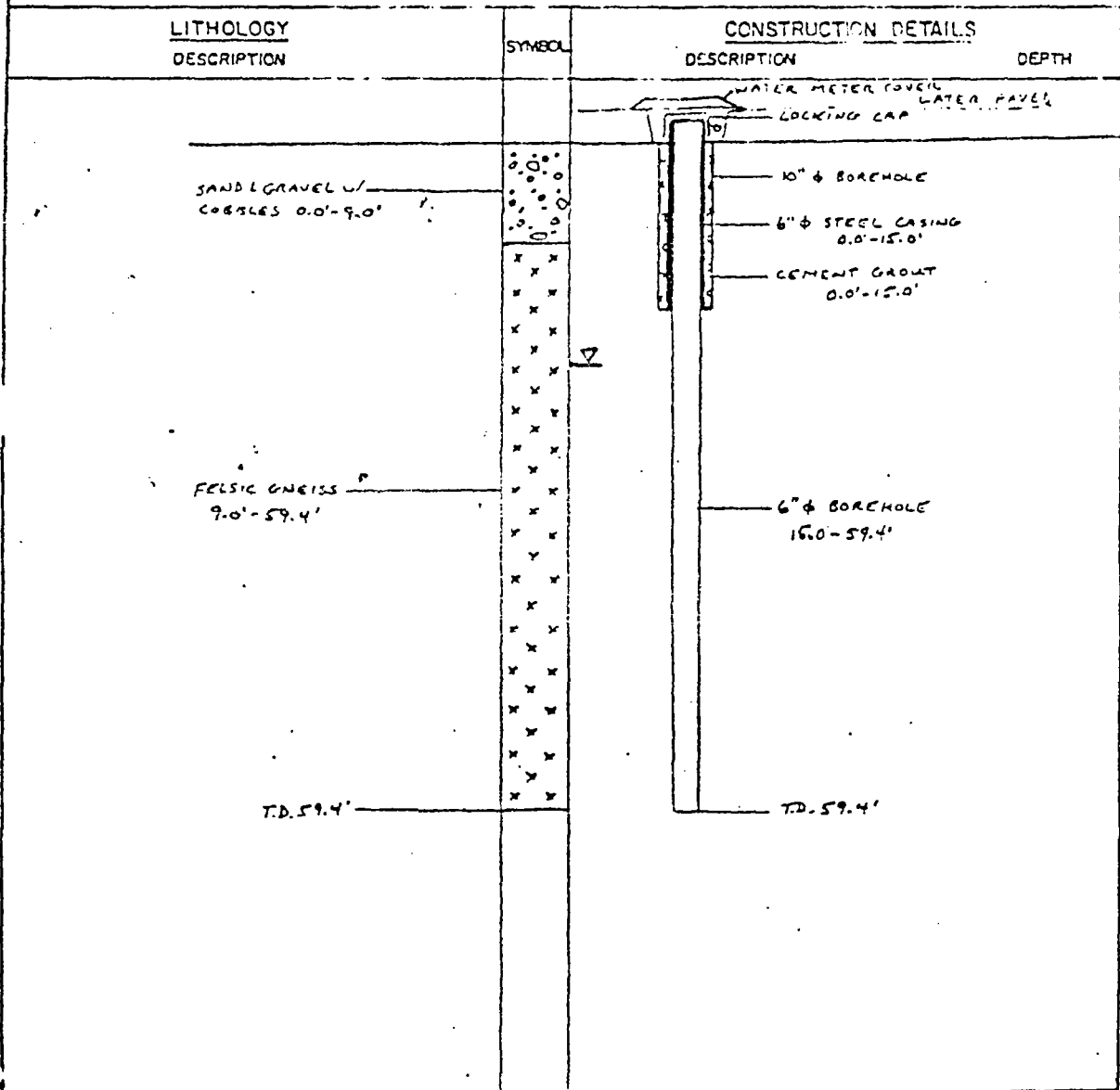






# WELL LOG SHEET

WELL NO. <i>K-10</i>	PROJECT NO. <i>5781.16</i>	PROJECT NAME <i>KELLOGG - DERRING</i>
LOCATION <i>BEHIND NEW ENGLAND QUARTZ</i>	GEOLOGIST <i>JEFF ORIENT</i>	
DRILLING DATE <i>2/20/85</i>	DRILLING CONTRACTOR <i>EARTH ENGINEERING &amp; SCIENCES, INC.</i>	
DRILLING METHOD <i>AIR ROTARY/AIR HAMMER</i>		DRILLER <i>L. FINDERMAN</i>
INSTALLATION DATE <i>2/20/85</i>		
WATER LEVEL BEFORE INSTALLATION <i>20.85'</i>	WATER LEVEL AFTER INSTALLATION <i>20.85'</i>	
DEVELOPMENT METHOD <i>PILE LIFT</i>	GROUND ELEVATION	





PROJECT KELLOGG-DEERING  
 PROJECT NO. 3791.16 BORING K-11  
 ELEVATION 78.45' msl DATE 2/20-2/21/85  
 FIELD GEOLOGIST JESS ORIENT

SAMPLE NO. TYPE # @ DEPTH (ft)	BLOWS/SIX INCHES OR ROD (%)	SAMPLE RECOVERY, SAMPLE LENGTH (ft)	MATERIAL MOISTURE @ WATER DEPTH (ft)	MATERIAL DESCRIPTION*			USCS OR ROCK BROKENNESS	REMARKS
				SOIL DENSITY/ CONSISTENCY OR ROCK HARDNESS	COLOR	MATERIAL CLASSIFICATION		
0								
						1.5-2.0' - 2.0-4.0'		
						MED TO COARSE SAND GRAIN		
						GRAVEL ROUNDED TO SUBANGULAR		
						UP TO 1" Ø		
5								
10								
15								
20								
25								

REMARKS BORING LOCATED IN FRONT OF PITNEY BOWS BORING K-11

\*SEE LEGEND ON BACK PAGE 1 OF 2



PROJECT KELLOGG - DEERING  
 PROJECT NO. 5781.16 BORING K-11  
 ELEVATION 78.45' msl DATE 2/20 - 2/21/85  
 FIELD GEOLOGIST JEFF ORIENT

SAMPLE NO. TYPE & DEPTH (ft)	BLOWS/SIX INCHES OR ROD (%)	SAMPLE RECOVERY/ SAMPLE LENGTH (ft)	MATERIAL MOISTURE & WATER DEPTH (ft)	MATERIAL DESCRIPTION*			USCS OR ROCK BROKENNESS	REMARKS
				SOIL DENSITY/ CONSISTENCY OR ROCK HARDNESS	COLOR	MATERIAL CLASSIFICATION		
25						SAND & GRAVEL (CONT)		
30								
35								
40								BELOW 38.5'
45								
50								

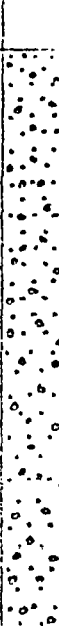
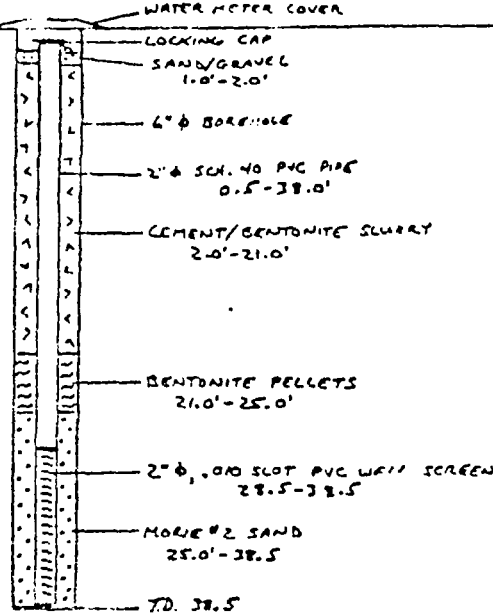
REMARKS INSTALLED WELL K-11 IN BORING BORING K-11

\*SEE LEGEND ON BACK

PAGE 2 OF 2

# WELL LOG SHEET

WELL NO. *K-11* PROJECT NO. *5781.16* PROJECT NAME *KELLOGG-DEERING*  
 LOCATION IN FRONT OF PITNEY-BOWES GEOLOGIST *JEFF. ORIENT/M. COCHRAN*  
 DRILLING DATE *2/20-2/21/85* DRILLING CONTRACTOR *FOR EARTH ENGINEERING & SCIENCES, INC*  
 DRILLING METHOD *AIR ROTARY* DRILLER *L. FIMBORAK* INSTALLATION DATE *2/21/85*  
 WATER LEVEL BEFORE INSTALLATION *28'* WATER LEVEL AFTER INSTALLATION  
 DEVELOPMENT METHOD \_\_\_\_\_ GROUND ELEVATION \_\_\_\_\_

LITHOLOGY DESCRIPTION	SYMBOL	CONSTRUCTION DETAILS DESCRIPTION	DEPTH
<p>SAND &amp; GRAVEL 0.0' - 38.5'</p> <p>T.D. 38.5' (B.C. ROCK)</p>		 <p>WATER TOWER COVER</p> <p>LOCKING CAP</p> <p>SAND/GRAVEL 1.0' - 2.0'</p> <p>6" Ø BOREHOLE</p> <p>2" SCH. 40 PVC PIPE 0.5' - 38.0'</p> <p>CEMENT/BENTONITE SLURRY 2.0' - 21.0'</p> <p>BENTONITE PELLETS 21.0' - 25.0'</p> <p>2" Ø, .010 SLOT PVC WELL SCREEN 28.5' - 38.5'</p> <p>MOBILE #2 SAND 25.0' - 38.5'</p> <p>T.D. 38.5'</p>	

SOIL BORING LOGS

PROJECT: Kellogg - Deering  
 PROJECT NO.: L670  
 ELEVATION:  
 WATER LEVEL DATA:  
 (Date, Time & Conditions)

BORING NO.: SS B1  
 DRILLER: Vince Price  
 DATE: 4-18-88  
 FIELD GEOLOGIST: Scott M. Kfall  
 Empire Sta. 6

SAMPLE NO. & DEPTH	DEPTH (ft.)	FLOW 1" OR ROD (in.)	SAMPLE RECOVERY SAMPLE LENGTH	LITHOLOGY CHANGE (DEPTH IN FT.)	MATERIAL DESCRIPTION*			USCS	REMARKS
					LOG DENSITY, CONSISTENCY OR POCE HARDNESS	COLOR	MATERIAL CLASSIFICATION		
5-1 113.2	0-2'	7	0.3	20	LOOSE	BR. TN	SAND (fine) trace gravel (f. 11)	SW	ASPHALT 2-3 HNU=app.
1135	2-4'	10	1.1	20	LOOSE		PPS bricks		HNU=app DAMP TO DRY
114	4-6'	12	1.2	24	5.0 LOOSE	DR. GR.	CAECYSIT GRAVEL (f. 11)	SM	HNU=app DAMPTULI
115	6-8'	13	0.2	20	LOOSE				HNU=app DRY
1205	8-10'	15	1.0	20	LOOSE				HNU=app DRY
122.5	10-12'	16	1.2	20	M DENSE				HNU=app DRY
5-3 1425	12-14'	17	1.1	20	DENSE		SAND AND GRAVEL 1/4" - 2"	SW	HNU=app DAMP-DRY
1445	14-16'	18	1.4	20	M DENSE				HNU=app DAMP DRY
1455	16-18'	19	1.6	20	V DENSE				HNU=app DAMP
1520	18-20'	20	0.4	20	V DENSE		HIGH GRAVEL ZONE 1" - 2" 18'-20'		HNU=0 NO SAMPLE OBTAINED GRAVEL
155	20-22'	21	0.7	20	V DENSE				HNU=app
1610	22-24'	22	1.2	20	DENSE				HNU=app DAMP
1630	24-26'	23	1.1	20	DENSE				HNU=0 DAMP

REMARKS 2" 00 Split Spoon Driven w/140 lb hammer falling 30"  
 Photo # 15 Roll # 2 ps 207 F01017 After Soil Meth Pig  
 Stopps for 1 1/2 HR DUE TO RAIN

\* See Legend on Back

BORING SS B1  
 PAGE 1 OF 2

BORING LOG

NUS CORPORATION

PROJECT: Kellogg - DEWEES

BORING NO. SS 81

PROJECT NO.: L670

DATE: 4-18-88

DRILLER: VINCE PRUE

ELEVATION:

FIELD GEOLOGIST: Scott M. Kroll

WATER LEVEL DATA:  
(Date, Time & Conditions)

SAMPLE NO & TYPE	DEPTH (ft)	BLOWS 1' OF ROD (ft)	SAMPLE RECOVERY SAMPLE LENGTH	LITHOLOGY CHANGE (Depth Ft)	MATERIAL DESCRIPTION*			USCS	REMARKS
					SOIL DENSITY CONSISTENCY OR POCE HARDNESS	COLOR	MATERIAL CLASSIFICATION		
		78							
170	26'2"	77	0.6		V DENS				100 / 3' HWS
		71	1.7						
		95							
		100							
171S	29'3"	27	1.1		V DENS				100 / 3'
		3-1	2.0						
		21							
		21							
		21	0.3		V DENS				110 / 4'
		100	0.9						STOPPED AT

REMARKS No sample obtained after 30'

BORING SS 81

PAGE 2 OF 2

\* See Legend on Back



PROJECT: Kellogg - Deerling  
 PROJECT NO.: L670  
 ELEVATION:  
 WATER LEVEL DATA:  
 (Date, Time & Conditions)

BORING NO.: SS82  
 DATE: 4-19-84  
 DRILLER: VINCE PRUE  
 FIELD GEOLOGIST: Scott M. Krall  
 Empire Soils

SAMPLE NO & DEPTH	DEPTH (ft)	BLOW COUNT (1' OR 2')	SAMPLE RECOVERY (%)	LITHOLOGY CHANGE (Describe)	MATERIAL DESCRIPTION*		USCS	REMARKS
					SOIL DENSITY, CONSISTENCY OR ROCK HARDNESS	COLOR		
S-1 0905	05'-20"	16	0.6 / 20		M DENSE	BE TO SILTY SAND AND GRAVEL (FILL) GRAVEL 2'-7"	SM	AS FACT TO 6 HNU = Open DAMP
		14						
		14						
0907	2'-4"	7	0.6 / 20		LOOSE			HNU = Open DAMP
		4						
0910	4'-6"	7	1.0 / 20		LOOSE			HNU = Open DAMP
		4						
0915	6'-8"	7	0.7 / 20		LOOSE			HNU = Open DAMP
		6						
S-2 0918	8'-10"	8	0.7 / 20		LOOSE			HNU = 0.3 HNU = Open DAMP
		5						
		7						
		4						
0940	10'-12"	10	0.4 / 20		DC: -			HNU = Open DAMP TOOK
		7						
		15						
0950	12'-14"	10	0.7 / 20		M DENSE			HNU = Open LOW STAINING ON GRAVEL AT 1/3
		9						
		4						
S-3 1020	14'-16"	13	1.4 / 20	15'	DENSE	CONCRETE PAD AT 14' ✓	✓	HNU = Open DAMP
		10						
		10						
		36						
1030	14'-18"	15	2.0 / 20		DENSE	Si. H. SAND (M-C). GRAVEL 1/2" ✓	SLD	HNU = Open DAMP
		10						
		21						
S-4 1085	18'-20"	16	1.2 / 20		DENSE			HNU = Open DAMP
		23						
		22						
		23						
1110	20'-22"	15	0.6 / 20		DENSE			HNU = Open DAMP TOOK
		11						
		11						
1115	22'-24"	11	1.1 / 20		DENSE	GRAVEL ZONE 19'-20" 23'-24"		HNU = Open DAMP TOOK
		11						
		23						
S-5 1140	24'-26"	14	0.7 / 20		M DENSE			HNU = Open DAMP TOOK
		11						

REMARKS: 3" OD SPLIT SPOON DESIGN W/100" Hammers falling 30"  
 AKer Soil Max Rig

BORING SS82  
 PAGE 1 of 2

\* See Legend on Back

PROJECT: Kellogg - Deepening BORING NO.: SSB2  
 PROJECT NO.: L670 DATE: 4-19-88 DRILLER: VINCE PRUE  
 ELEVATION: \_\_\_\_\_ FIELD GEOLOGIST: Scott M Krahl *Empire Soils*  
 WATER LEVEL DATA: \_\_\_\_\_  
 (Date, Time & Conditions) \_\_\_\_\_

SAMPLE NO. & TYPE	DEPTH (ft.)	BLOWS 1" OF ROD (ft.)	SAMPLE RECOVERY SAMPLE LENGTH	LITHOLOGY CHANGE (DEPTH)	MATERIAL DESCRIPTION*			REMARKS
					TON, BENEATH, CONSISTENCY OR ROCK HARDNESS	COLOR	MATERIAL CLASSIFICATION	
		23						
1145	26.78	23	1.7		DENSE			H <sub>2</sub> O DAMP
		41	2.0					
		52						
		67						

REMARKS \_\_\_\_\_  
 \_\_\_\_\_  
 \_\_\_\_\_

BORING SSB2  
 PAGE 2 OF 2

\* See Legend on Back

PROJECT: Kellogg - Openings  
 PROJECT NO.: L670 DATE: 4-19-88 BORING NO.: SSB3  
 ELEVATION: DRILLER: Vince Pave  
 WATER LEVEL DATA: FIELD GEOLOGIST: Scott M Krall Empire Soils  
 (Date, Time & Conditions)

SAMPLE NO. & TYPE	DEPTH (ft)	HOWS 1" OR 800 (ft)	SAMPLE RECOVERY SAMPLE LENGTH	LITHOLOGY CHANGE (ft)	MATERIAL DESCRIPTION*		VICI	REMARKS
					SOIL BENEATH CONSISTENCY OR ROCK HARDNESS	CLASS		
						GR SILTY SAND (F-C) GRAVEL (FILL) 6" - 1/2" - 1"	5m	Asphalt - 0.5'
S-1 1418	6'-8"	12 12	0.2 20		m Dense			HNU = 0 ppm DAMP
1420	8'-10"	12 8	0.8 20		Loose			HNU = 1 ppm DAMP
1430	10'-12"	12 8	1.1 20		Loose			HNU = 1 ppm DAMP
S-2 1435	12'-14"	14 4	1.7 20		Dense			HNU = 0 ppm
1450	14'-16"	12 2	1.2 20		m Dense	Concrete at 135'		HNU = 0 ppm
1500	18'-14"	14 100	0.7 6.7	15.0	Dense	ST BR SILTY SAND (MC) GRAVEL 1/2" - 2"	SW	100/1 HNU = 4 ppm
S-3 1510	18'-20"	20 7	1.6 20		Dense			HNU = 0 DAMP
1535	20'-22"	12 105	0.6 9.6		v Dense	Hard Gravelly Sand 20" - 2 1/2"		100/0
1350	22'-24"	17 11	1.7 20		m Dense			HNU 150 ppm Strong Petrol Odor
S-4 1600	24'-26"	16 22	1.7 20		m Dense			HNU 20 ppm

REMARKS: Aggered to 6' with 6 1/4" OD hollow stem  
 - High HNU readings at 24'  
 Sample obtained with a 2" SS Driven w/ 140" Hammer  
 \* See Legend on Back  
 Ficker soil Max Rig falling 30"

BORING SSB3  
 PAGE 1 OF 2

PROJECT: Kellogg - Deer Run  
 PROJECT NO.: L670  
 ELEVATION:  
 WATER LEVEL DATA:  
 (Date, Time & Conditions)

BORING NO.: SSB3  
 DRILLER: Vinko Pice Engineer  
 DATE: 4-19-88  
 FIELD GEOLOGIST: S. C. H. M. Krall

SAMPLE NO. & TYPE	DEPTH (ft)	BLOW COUNT (1' or 2')	SAMPLE RECOVERY (%)	LITHOLOGY CHANGE (DEPTH)	MATERIAL DESCRIPTION*		USC	REMARKS
					SOIL STRENGTH, CONSISTENCY OR ROCK HARDNESS	MATERIAL CLASSIFICATION		
		20						
1700	26.2	27	0.8		DENSE			HWL = 20 ppm (AMP)
		19						HWL 70 ppm
1715	28-30	43	0.7		DENSE			100/1 (AMP)
5-5 1730	2-2	37	0.8 1.7		DENSE			100/3 (AMP)
		100						31.4 BOH AUGER REFUSAL 24-31.4: Level 2' to 3'

REMARKS: B2 At 24' 20ppm recommended to drillers to back off hole and let it vent. Drillers wearing respirators (Leak) while working from 24' to BOH

BORING SSB3  
 PAGE 2 OF 2

\* See Legend on Back

31.4

PROJECT: Kellogg - Deeping  
 PROJECT NO.: 670  
 ELEVATION:  
 WATER LEVEL DATA:  
 (Date, Time & Conditions)

BORING NO.: 5584  
 DRILLER: V. L. P. J.  
 DATE: 4-20-88  
 FIELD GEOLOGIST: Scott M. Krahl

SAMPLE NO. & DEPTH	DIST. IN	BLOWS 6" OF 140 LB.	SAMPLE RECOVERY LENGTH	LITHOLOGY CHANGE (DEPTH)	MATERIAL DESCRIPTION*			USC	REMARKS
					FOR BENTON. CONSISTENCY DEGREE PARADOX	COLOR	MATERIAL CLASSIFICATION		
				0.5		BR	SANDY SILT TRACER GRAM - (Fill)	SM	ASHPACT TOP IS
S-1 0925	6'-8"	16 17 18 19 20 21 22 23 24 25 26 27 28 29 30	0.3 0.2	7.0	M DENSE	DE-TN	SAND (GRAVEL) GRAVEL 1/2" - 1"	SW	HNU=0.000 MOIST
0926	8'-10"	24 25 26 27 28 29 30	0.9 2.0		M DENSE				HNU=0.000 - 0.000
0927	10'-4"	20 21 22 23 24 25 26 27 28 29 30	1.1 2.0		M DENSE				HNU=0.000 DAMP
S-2 0940	12'-11"	27 28 29 30	1.2 2.0		DE-TN		GRAVEL 1/2" - 2"		HNU=0.000
							AVOID REFUSAL 14 Y		
							END OF HOLE		

REMARKS Auger to 6' with 64.00 Hollow Stem  
 Sample obtained w 2" OD SS DRIVEN w 140" Hammer falling 30"  
 After soil MAX Pig

BORING 5584  
 PAGE 1 OF 1

\* See Legend on Back

PROJECT: KEHOGS - DEERING BORING NO: WSB1  
 PROJECT NO: L 670 DATE: 11 APRIL 1988 DRILLER: E GIE OF EMPIRE S-13  
 ELEVATION: FIELD GEOLOGIST: K T McCREADOR  
 WATER LEVEL DATA: HOLE COLLAPSED TO 12.5' (DRY)  
 (Date, Time & Conditions)

SAMPLE NO & DEPTH	DEPTH (ft)	BLOWS # OF 100	SAMPLE RECOVERY SAMPLE LENGTH	LITHOLOGY CHANGE (DEPTH IN FT)	MATERIAL DESCRIPTION*			USCS	REMARKS
					FOR DENSITY CONSISTENCY OR ROCK HARDNESS	COLOR	MATERIAL CLASSIFICATION		
S-1	1.0			0.5		BLACK	ASPHALT		DRILLING STARTED 1335 NOT SAMPLED
		6	7/24	4.1'	UP TO 1/2" TO 1" CLAY		COARSE SAND - SOME GRAVEL	SW	DRY TO DAMP
		7	9/24				TRACE CLAY		
	4.0	8	11/24						
S-2		9	13/24			F. SENSITIVE TO 1/2" SENSITIVE	GRAY TO TAN	MEDIUM TO COARSE SAND - TRACE	SW
		10	15/24				SMALL (1/8") GRAVEL		
	7.0	11	17/24				- GRAVEL Ø INCREASING UP TO 2/8"		DRY TO DAMP
S-3		12	19/18						
		13	21/18						
	10.0	14	23/18						
S-4		15	25/18				- GRAVEL COBBLES BEING FRAGMENTED BY SPAUN		DAMP
		16	27/18						
	13.0	17	29/18						
S-5		18	31/18						
		19	33/18						
	16.0	20	35/18				- MATERIAL IS MORE WELL GRADED		
S-6		21	37/18						
		22	39/18						
	19.0	23	41/24				- TRACE DECOMPOSED S.H.C. - ORANGE CLAY		
S-7		24	43/24				- SAND Ø DECREASING		MOIST
		25	45/24						
	23.0	26	47/24						
S-8		27	49/24				- ORANGE COLORED MOTTLED		
		28	51/24						
	25.0	29	53/24						

REMARKS DRILLER USE FALLING F-10 TRUCK MOUNTED RIG w/ 6 3/8" O.D.  
HOLLOW STEM AUGERS BETWEEN SAMPLES  
1405 -

BORING WSB1  
 PAGE 1 OF 2

\* See Legend on Back

PROJECT: KENOGG - DEERING

BORING NO. WS31

PROJECT NO.:

DATE: 11 APRIL 1988

DRILLER E. COLE/EMARC

ELEVATION:

FIELD GEOLOGIST: KT Mc CREANOR

WATER LEVEL DATA

(Date, Time & Conditions)

SAMPLE NO & TYPE	DEPTH (ft)	BLOWS 1" DE 100 (ft)	SAMPLE RECOVERY SAMPLE LENGTH	LITHOLOGY CHANGE (DEPTH IN FT)	MATERIAL DESCRIPTION*		USCS	REMARKS
					FOR DENSITY CONSISTENCY OR SOLE HARDNESS	COLOR		
	26.0	18 21	13 24		1/2 DENSE TO 1/2 DENSE	SPERM TO TAN		MEDIUM TO COARSE SAND - SW TRACE SMALL (1" φ) GRAVEL
S-9	26.0	16 17	12 24					- SAND φ INCREASING WET
	30.0	55 28	17 18	30.0				BORING COMPLETED TO 30.0' 4/11 BACKFILLED TO SURFACE 4/12 TD 30'

REMARKS \_\_\_\_\_  
 \_\_\_\_\_  
 \_\_\_\_\_

BORING WS31  
 PAGE 2 OF 2

\* See Legend on Back

PROJECT: KEHOOG-DEERING BORING NO. WSB2  
 PROJECT NO.: L 670 DATE: 12 APRIL 1988 DRILLER: EGIE OF EMPRE  
 ELEVATION: FIELD GEOLOGIST: KT MCCREANOR  
 WATER LEVEL DATA: NO WATER ENCOUNTERED  
 (Date, Time & Conditions)

LAYER NO & TYPE	DEPTH (ft)	DOWN HOLE LOG	LAYER THICKNESS (ft)	LITHOLOGY CHANGE (Depth ft)	MATERIAL DESCRIPTION*			VICI	REMARKS	NO OF P
					SOIL DENSITY CONSISTENCY OR ROCK HARDNESS	COLOR	MATERIAL CLASSIFICATION			
S-1	2.0	15 20 14	10 24	4.0'	DENSE TO V DENSE	LT BRN TO GRAY	COARSE SAND - SOME SMALL GRAVEL (3/4" φ) TRACE CLAY	SM	DAMP "Fill" SLIGHT ODOOR	C
S-2	6.0	12 10 6 5 12 22	6 24	8.0'	M DENSE TO DENSE	LT BRN TO BRN	SANDY CLAY AND ROCK FRAGMENTS (CRUSHED BY DRIVE SHOES)	GC	DAMP DEANT REMAINS "Fill"	C
S-3	10.0	12 14 4 5	5 24	12.0'	M DENSE TO DENSE	BRN	SAND - SOME GRAVEL (UP TO 2" φ)	SW	DAMP TO MOIST, NATURAL MATERIAL, WELL GRADED MEDIUM SAND	O
S-4	14.0	1 2 14 22 19 12 9	10 24	12.9'	V loose TO DENSE	LT BRN TO BRN	CLAYEY SILT/FINE SAND SAND - TRACE GRAVEL	SM	MOIST	O
S-5	18.0	5 7 4 5 6	6 24				- φ DEEPENSING TO FINE SAND		MOIST	O
S-6	22.0	2 35 20	13 24							O
S-7	24.0	27 90 10 43	16 24		DENSE TO V DENSE	BRN	SAND AND GRAVEL (UP TO 2" φ)	SW		O

REMARKS Failing Fill 15'  
140' Hammer - 20" Fall

BORING WSB2  
 PAGE 1 OF 2

\* See Legend on Back



PROJECT: KENOGG DEEPING BORING NO. WSB 2  
 PROJECT NO.: L 670 DATE: 12 APRIL 1988 DRILLER: E. G. LEPIRE  
 ELEVATION: FIELD GEOLOGIST: K. McCREARER  
 WATER LEVEL DATA: NO WATER ENCOUNTERED  
 (Date, Time & Conditions)

SAMPLE NO. & PIPE	DEPTH (ft)	ROWS OF ROD (ft)	SAMPLE RECOVERY SAMPLE LENGTH	LITHOLOGY CHANGE (Down ft)	MATERIAL DESCRIPTION*			VIC	REMARKS
					SOIL DENSITY CONSISTENCY OR ROCK HARDNESS	COLOR	MATERIAL CLASSIFICATION		
	26.0	53			LENS TO V. DENSE	BRN	SAND AND GRAVEL	SW	MOIST
S-8		31	3						
	28.0	27							
		27	12						
	30.0	26		30.0'	↓	↓	↓	↓	
									BORING COMPLETED TO 30.0' 4/12
									BACKFILLED TO SURFACE 4/12
									TD 30'

REMARKS \_\_\_\_\_  
 \_\_\_\_\_  
 \_\_\_\_\_

BORING WSB 2  
 PAGE 2 OF 2

\* See Legend on Back

PROJECT: **K-D**  
 PROJECT NO: **154-1156 L670**  
 ELEVATION:  
 WATER LEVEL DATA  
 (Date, Time & Conditions)

DATE: **4/12/88**  
 FIELD GEOLOGIST: **KTM**

BORING NO: **KD WSB-3**  
 DRILLER: **ED GIES**

SAMPLE NO & TIME	DEPTH (ft)	BLANK 6" DR ROD (ft)	SAMPLE RECOVERY (ft)	LITHOLOGY CHANGE (Down ft)	MATERIAL DESCRIPTION*		VICIS	REMARKS
					TEXTURE / CONSISTENCY / COLOR	MATERIAL CLASSIFICATION		
S-1	5	0.5	9/24		ADJUSTED DR SHIP GREY	WELL GRADED SAND, SOME GRAVEL	DAMP	"Fill"
1540	20	6			LOOSE	5" TR CLAY		
1555		7	3/24		HARD	- CUT FRAGS (SCHIST)		
	40	6			HARD			
S-2		7	7/24			- INCR. CLAY ND		SAND TO MOIST
1533	60	2				GRAVEL		
1705		7	10/24	6.6'				
	8.0	8		80'	DR BRN	BOTTOM 4" - NATURAL - NO. 7 - TR GRAVEL UP TO 2" 6"		CLAYEY SILT / FINE SAND
S-3		2	10/24					
1710	10.0	10			M	MED TO COARSE SAND TR GRAVEL 1/4" 50"		
1720		23	12/24		RUST	COARSE SAND 40% FINES		DAMP-MOIST
	12.0	27		120'				
S-4		20	13/24		ADJUSTED DR SHIP GREY	WELL GRADED SAND TR CUT FRAGS		DAMP
1735	14.0	18						
		20	10/24			- SOME SMALL (1/4") GRAVEL		
	16.0	1						
S-5		14	10/24					- DIFFICULTY PENETRATING
1755	18.0	15						
1805		24	0/24					- GRAVEL STUCK IN SHOE
	20.0	24						
1815		31	10/24					
	22.0	10						END 4/12
S-6	0715	12	0/24					- IRON SHOE PLUGGED w/ SCHIST
	24.6	11						
	0725	10	10/24					

BIOREMARKS: 26.0 17 27 7/24 FAILING F DR S MOIST - DRIVESHAW WET  
 28.0 31 28 25 4/24 RUSTY - TR GR 5 2" 6" WET  
 See legend on page 38

H-0 @ 27.8' BORING WSB-3 PAGE OF

DONE 0830 BACKFILL 4/13

TD 30'

PROJECT: K-D BORING NO. KD-WSB-4  
 PROJECT NO.: 154-1256 2670 DATE: 4/13/89 DRILLER: ED GIE  
 ELEVATION: \_\_\_\_\_ FIELD GEOLOGIST: KTM  
 WATER LEVEL DATA: \_\_\_\_\_  
 (Date, Time & Conditions) \_\_\_\_\_

SAMPLE NO DEPTH	DEPTH (ft)	HOWS S' GA 400 1"	SAMPLE RECOVERY SAMPLE LENGTH	LITHOLOG- CHANGE (DEPTH)	MATERIAL DESCRIPTION*		REMARKS
					SOIL DENSITY CONSISTENCY OR SOIL WEARNESS	COLOR	
S-1	20	10	10/24		V. DENSE DAN CR RUST	WELL GRADED SAND TR GRAVEL (5 1/8") & CLAY	DAMP TR ROOTS
1020	20	16					
1035	40	22	8/24			- CUT FRACS (WHITE) - NO CLAY	DRY TO DAMP
S-2	60	25	14/24				
1045	60	30					
S-3	80	34	8 1/2/24			TR CLAY/SILT	DAMP
1055	80	43					
1100	100	52	16/24		BRN RST	ANGULAR SM (1 1/8") GRAVEL	DAMP <u>DUPLICATED</u>
S-4	120	61	10/24				
1110	120	70					
1130	140	80	8/24			3' OF FINE BRN DAMP SAND	
S-5	160	90	9/24		BRN RST	SILTY SAND TR CLAY NO GRAVEL	DAMP
1140	160	100					
1150	180	110	12/24				
S-6	200	120	15/24	10.2 11.2		TAN BRN SAND	DAMP
1200	220	130	18/24			BRN SILTY SAND TR CLAY LAMINATION ~ 20° TO HORIZ	
S-7	240	140	10/24				DAMP
1305	240	150					
1310	260	160	20/24			BOTTOM 14" WET	

SPARKS 260 4 4  
 1310 286 4 4  
 1302 2 3 22/24  
 \* See log of on file

Filing F.O.R. 140# hammer  
 30" Fall  
 - LAMINATIONS WHERE SANDY  
 ARE SOFTER

BACKFILLED 4/13  
 2 - 10 MI

26.2' - H<sub>2</sub>O  
 BORING WSB-4  
 PAGE \_\_\_\_ OF \_\_\_\_  
 TD 30'

LOG

NUS CORPORATION

PROJECT: KENLOGG DEERING

PROJECT NO.: L 670

ELEVATION:

WATER LEVEL DATA

(Date, Time & Conditions)

DATE: 13 APRIL 1988

FIELD GEOLOGIST: K.T. MCCREANOR

BORING NO. KD CSB1

DRILLER: EMER OF EMPIRE SOILS

SAMPLE NO. & TYPE	DEPTH (ft)	FLOW 1" OR 2" ID	SAMPLE RECOVERY (SAMPLE LENGTH)	LITHOLOGY CHANGE (DEPTH)	MATERIAL DESCRIPTION*			VICI	REMARKS
					SOIL DENSITY (CONSISTENCY OR ROCK HARDNESS)	COLOR	MATERIAL CLASSIFICATION		
S-1	2.0	4	9 / 22	2.2	M DENSE	DRY GRAY	COARSE SAND - SOME GRAVEL	SP	TOP 2" ASPHALT. DAMP
		6	9 / 1640 MS						
	4.0	7	11 / 24	4.0	M STIFF	BRN	SILTY CLAY	CL	DAMP "FILL"
		15	11 / 1688 MS		M DENSE	DRY GRAY	COARSE SAND AND GRAVEL	SP	DAMP TO DRY "FILL"
S-2	6.0	11	15 / 20		M DENSE TO	DRY TAN	COARSE SAND - SOME GRAVEL	SW	DAMP TO DRY "NATURAL
		13	15 / 1705 MS		V DENSE				
	8.0	16	16 / 24						MOIST (AT TOP OF AUGER)
		17	16 / 1700 MS						
S-3	10.0	18	10 / 24						MOIST
		20	10 / 1730 MS						
	12.0	22	8 / 24						DAMP TO MOIST
		23	8 / 1740 MS						
S-4	14.0	23	7 / 20		V DENSE	BRN	COARSE SAND AND FINE TO MED	SW	MOIST (DRIVE SHOE WET)
		27	7 / 1745 MS				(1 1/2" φ) GRAVEL		
		27	0 / 2	14.6					DRILLER REPORTS TOP OF
									BEARING COMPLETED TO 14.6' 4/13/88
									ROCK @ 14.6' TD
									BACKFILLED/GROUTED TO SURFACE 4/13

REMARKS DRILLED W 6" φ HOLLOW AUGERS USING A FAIRING F-10 TRUCK-MOUNTED PIG  
150# HAMMER WITH 30" FALL

BORING CSB1

\* See Legend on Back

PAGE 1 OF 1

PROJECT: Kellogg - DEERING BORING NO: KD-CSB2  
 PROJECT NO.: L670 DATE: 4-14-88 DRILLER: E.O. Cole Empressville  
 ELEVATION: FIELD GEOLOGIST: McCaman / Kral  
 WATER LEVEL DATA:  
 (Date, Time & Conditions)

SAMPLE NO & DEPTH	DEPTH (ft)	NO. OF 2" OR 1 1/2" SAMPLES	RECOVERY SAMPLES LENGTH	LITHOLOG CHANGE (DEPTH IN)	MATERIAL DESCRIPTION		REMARKS
					LOG DENSITY CONSISTENCY OR ROCK HARDNESS	MATERIAL CLASSIFICATION	
S-1						TOP 1" ASPHALT 1-6" CONCRETE	
0755	20	2	9/18		M DENSE	COARSE SAND TR GRAVEL S/S W	DAMP "Fill"
0805		2	6/24				
	40	2			LOOSE		
0813		2	11/24			- TR CLAY & ROCK CUT FRAGS	DAMP "Fill"
	60	2					
S-2		2	16/24		M DENSE		
0822	80	2					
S-3		2	2/22		V DENSE		100/3'
0835	100	2					
0850	10-12'	2	11/20'		V DENSE		DAMP
		2					
0900	12-13.7'	2					
		2	11.2		V DENSE		100/2' SAT (2')
S-4		2	1.0		V DENSE		100/3' SAT
0920	14-15'	2	1.0				
0930	16-16.3'	2	.3		V DENSE		150/3' SAT
						↓	↓
						6.5 ROCK 16.3 DEE DRILLER	STOPPED
							TD 16.3

REMARKS Split Spool sampling 2" SS w 140" hammer falling 30"  
 Hollow stem augers 6 1/4" OD Falling F-10 Rig

BORING KD-CSB2  
 PAGE 1 OF 1

\* See Legend on Back

NUS CORPORATION

PROJECT: Kellogg - Deering BORING NO CSB-3  
 PROJECT NO.: 2670 DATE: 4-14-88 DRILLER: Ed Cole Eng'g Soils  
 ELEVATION: FIELD GEOLOGIST: Scott M Krall  
 WATER LEVEL DATA:  
 (Date, Time & Conditions)

SAMPLE NO & DEPTH	DEPTH (ft)	FLOW 1" OF 800 PSI	SAMPLE RECOVERY %	LITHOLOGY CHANGE (DOWN IN)	MATERIAL DESCRIPTION*			USCS	REMARKS
					SOIL DENSITY, CONSISTENCY OR ROCK HARDNESS	COLOR	MATERIAL CLASSIFICATION		
S-1	0.3-2'	6	0.7/1.5	0.5	M DENSE	BR, GR	ASPHALT		H <sub>2</sub> O = 0ppm DRY
1245		6		2.0			SAND MED GRAINED, FILL GRAVEL 1/2" - 1"	SP	
1250	20-40	12	0.9/2.0		M DENSE	BR, GR	SAND MED GRAINED GRAVEL 1/2" - 1"	SW	H <sub>2</sub> O = 0ppm DRY
1255	40-60	12	1.4/2.0		DENSE				H <sub>2</sub> O = 0ppm DRY
1300	60-80	12	0.5/0.5		DENSE		GRAVEL LARGER IN SIZE 1" - 2"		100/0, H <sub>2</sub> O = 0ppm DRY - DW
S-2									
1300	80-100	12	1.6/2.0		V DENSE				H <sub>2</sub> O = 0ppm DAMP
1330	100-120	11	1.7/2.0		V DENSE				H <sub>2</sub> O = 0ppm DAMP
S-3									
1345	120-140	12	0.8/1.6		DENSE				100/1' H <sub>2</sub> O = 0ppm DAMP
1355	140-160	12	1.4/2.0		M DENSE				H <sub>2</sub> O = 0.4ppm DAMP A 135'
S-4									
1400	160-170	12	0.6/0.6		V DENSE				100/1' NO RECOVERY DUE TO GRAVEL
1415	180-200	11	1.3/2.0		DENSE				H <sub>2</sub> O = 0.8ppm DAMP
1430	200-220	12	1.7/2.0		DENSE				H <sub>2</sub> O = 2ppm MOIST
S-5				20.0					
1445	220-240	11	1.1/2.0		M DENSE	BR	SAND COARSE GRAINED TRACT 1/2" - 1" GRAVEL	SW	H <sub>2</sub> O = 9ppm MOIST
1500	240-260	12	1.2/2.0		M DENSE				H <sub>2</sub> O = 12ppm MOIST

REMARKS: LOCATED IN FRONT OF PITNEY BOWES (33' NORTH OF K11 5' FROM BUILDING) CSB-3  
 2" SS DENSE 20" w/ 110' H<sub>2</sub>O  
 Faling Field Rig 140 = hammer with 30" Fall  
 \* See Legend on Back

PROJECT: Kellogg - Operating

PROJECT NO.: 4670

DATE: 4-14-88

BORING NO: CSB-3

DRILLER: ED Cole Empire Soils

ELEVATION:

FIELD GEOLOGIST: Scott M. Kyall

WATER LEVEL DATA:  
(Date, Time & Conditions)

SAMPLE NO. & TYPE	DEPTH (ft)	BLOWS # OF ROD (ft)	SAMPLE RECOVERY SAMPLE LENGTH	LITHOLOGY CHANGE (Describe)	MATERIAL DESCRIPTION*		WATER	REMARKS
					100% DENSITY, CONSISTENCY OR ROCK HARDNESS	COLOR		
		21		26.5				
5-6 / 1520	26-27	9	1.4	20	m DENS	BR Silty Sand	SM	
		11				BR Fine Grain Sand	SM	HNU = 9 ppm max
		33						
1525	28-29	130	.4		V DENS			
		100						100% HNU = 1 ppm
						RED ROCK PER DRILLER		STOPPED
								TD = 24.5

REMARKS \_\_\_\_\_  
\_\_\_\_\_  
\_\_\_\_\_  
\* See Legend on Back





PROJECT: Kellogg - Deerfield  
 PROJECT NO: 6670 DATE: 4-14-88 BORING NO: CSB4A  
 ELEVATION: FIELD GEOLOGIST: Scott M. Krahl DRILLER: Ed Cole (Empire Soil)  
 WATER LEVEL DATA:  
 (Date, Time & Conditions)

SAMPLE NO & DEPTH	DEPTH (ft)	FLOW RATE (ft <sup>3</sup> /min)	SAMPLE RECOVERY (ft)	LITHOLOGY CHANGE (Depth)	MATERIAL DESCRIPTION*		VICI	REMARKS
					SOIL DENSITY CONSISTENCY OR ROCK HARDNESS	MATERIAL CLASSIFICATION		
S-1	05-20	31		05		BL ASPHALT SAND COARSEGRAINED FILL		
	20	33	0.7	35	m. DENSE			100/300m DAMP
	1730	2-4	1.1		DENSE			HNU = 0
	1735	4-6	0.9		DENSE	DR SILTY SAND; GRAVEL 1/2"-1"	GM	100/3' HNU = 1ppm DAMP
	1750	6-8	1.7	75	V DENSE			HNU = 38 ppm
S-2						BR-TM SAND (MED-COURSE GR)	GP	100/3 MOIST AT 75'
0745	8-10	45	0.8		V DENSE	GRAVEL 1/2"-1"		150/3' SAT
0735	10-12	12	1.5		V DENSE			HNU = 9 ppm SAT
S-3	0810	12-14	1.9		V DENSE	NOTABLE TRENCH FROM STAINLESS STEEL GRAVEL		HNU = 15 ppm MOIST AT 75'
0830	14-16	4	0.7			TD - 15.7		100/2 HNU 2 ppm

REMARKS LOCATED ~ 10' North of KD-5544  
 STOPPED AT 8' ON 4-14-88 STOPPED AT 15.7  
 2" SS Drilled 30" w/ 140" Hammer Filling Field Log

\* See Legend on Back

BORING CSB-4A

PAGE 1 OF 1



PROJECT: Kellogg - Decline BORING NO: KDCSG-6  
 PROJECT NO: 4670 DATE: 4-18-88 DRILLER: ED COLE, Empire Drilling  
 ELEVATION: FIELD GEOLOGIST: Kim KAL  
 WATER LEVEL DATA NOT ENCOUNTERED  
 (Date, Time & Conditions) 4-18-88, 10:30 AM, Cloudy Rain ~ 50°

SAMPLE NO & TYPE	DEPTH (ft)	BOWS ft OF ROD ft	SAMPLE RECOVERY SAMPLE LENGTH	LITHOLOGY CHANGE (Depth ft)	MATERIAL DESCRIPTION*		USCS	REMARKS
					SOIL DENSITY CONSISTENCY OR ROCK HARDNESS	COLOR		
	0-5					ASPHALT		
245 S-1	0.5	11	14"		GENUINE	BROWN FINE TO COARSE SAND AND	SW-	LOW MOISTURE
	2-2	18	18"		LOOSE	GRAVEL - LITTLE TO NO	GW	HANU 8PPM ABOVE
						ORGANICS HOMOGENEOUS		BACKGROUND
00 S-1	5.0	19 15	16"		GENUINE	BROWN FINE TO COARSE SAND AND	SW-	LOW-MEDIUM MOISTURE
	4.0	20	24"		MEDIUM	GRAVEL, SOME SILT TRACE	GW	HANU 8PPM ABOVE
					DENSE	BLACK ORGANICS		BACKGROUND
						↓		
15 S-2	4.0	7 11.2	13"		GENUINE	BROWN FINE TO COARSE SAND AND	SW-	MEDIUM MOIST
	5.4	13.5 10.7	16"		DENSE	GRAVEL - LITTLE TO NO	GW	HANU 8PPM ABOVE
						ORGANICS		BACKGROUND
						↓		
30 S-1	6.0	150 1.2	0"	ANOMAL 6.6'		NO SAMPLE RECOVERED		
	8.0		24"					TO 6.8'

REMARKS SOIL 6.8' Falling Flat  
140# Hammer with 20" Fall

BORING KDCSG6

\* See legend on page 1

PROJECT: Kelco Decline BORING NO: KD-CSB-7  
 PROJECT NO.: 6670 DATE: 4-12-88 DRILLER: ED COLE  
 ELEVATION: FIELD GEOLOGIST: KIM KRAL  
 WATER LEVEL DATA: Groundwater at 15.5'  
 (Date, Time & Conditions) 4-12-88 @ 1240 Rain 7.5"

SAMPLE NO. & TYPE	DEPTH (ft)	BLOWS 1' OF ROD (1")	SAMPLE RECOVERY SAMPLE LENGTH	LITHOLOGY CHANGE (Depth Ft)	MATERIAL DESCRIPTION*			VIC	REMARKS
					TOE DENSITY, CONSISTENCY OR SOLE HARDNESS	COLOR	MATERIAL CLASSIFICATION		
	0-5'								
40 S-1	0.5'	11	12"		GLUWIK	MED	FINE TO COARSE SAND AND	SW	LOW MOISTURE,
	2.0'	12	10"		LOOSE	BRN	GRAVEL LOW ORGANICS	GW	HNU OPPM ABOVE
							↓		BACKGROUND
S-1	2.0'	4	2"		SOIL	DRK	FINE TO COARSE SAND AND	SW	LOW-MEDIUM MOISTURE
	4.0'	3	24"		COARSE	GLAY	SILT LOW ORGANICS	UL	HNU OPPM ABOVE
					LOOSE		MICAIFOUS		BACKGROUND
							↓		
S-1	4.0'	4	5"		COHESIVE	DRK	FINE TO COARSE SAND AND	SW	MEDIUM MOISTURE
	6.0'	3	24"		SOFT	GLAY	GRAVEL SAME SILT	GW	HNU OPPM ABOVE
							↓		BACKGROUND
S-1	6.0'	4	10"		GLUWIK	MOTIC	FINE TO COARSE SAND AND	SW	MEDIUM MOISTURE
	8.0'	5	24"		MEDIUM	BRN	GRAVEL NO ORGANICS	GW	HNU OPPM ABOVE
					DENSE	M-DW	MICAIFOUS		BACKGROUND
							↓		
S-1	8.0'	15	7"		LEAN	MOTIC	FINE TO COARSE SAND AND	SW	MOIST SAMPLE
	8.8'	13	9"		MED	BRN	GRAVEL	GW	HNU OPPM ABOVE BACKGROUND
							↓		
30 S-1	10.0'	21	19"		LEAN	MOTIC	FINE TO COARSE SAND AND	SW	MOIST SAMPLE
	11.4'	19	29"		MEDIUM	BRN	GRAVEL WITH SILT	GW	HNU OPPM ABOVE
					DENSE	AND			BACKGROUND
						GLAY			
							↓		
S-2	12.0'	28	16" / 4"		LEAN	MOTIC	FINE TO COARSE SAND AND	SW	MOIST

REMARKS Falling F-10 7 1/2 lbs hammer with 30" Fall

BORING KD-CSB-7

\* See Legend on Back

PROJECT: Kellon - Design BORING NO: KDC5B-7  
 PROJECT NO.: 6670 DATE: 4-18-88 DRILLER: ED COLE  
 ELEVATION: FIELD GEOLOGIST: Kim Kahl  
 WATER LEVEL DATA: WATER AT 16.0  
 (Date, Time & Conditions) 4-18-88, 14:00, Rain 50'

SAMPLE NO. & TYPE	DEPTH (ft)	ROWS 1" OR 2" HOD 1" H	SAMPLE RECOVERY SAMPLE LENGTH	LITHOLOGY CHANGE (Depth)	MATERIAL DESCRIPTION*			VICI	REMARKS
					SOIL DENSITY, CONSISTENCY OR ROCK HARDNESS	COLOR	MATERIAL CLASSIFICATION		
	14.0	25			DENSE	BROWN	GRAVEL	LOW	NEW OFFICE BLDG. CONC. CURB
S-1	14.0	25	2"		GRAVEL	MIXED	FINE TO COARSE SAND AND	LOW	2' DIST. WEST
	16.0	27	2"		DENSE	BROWN	GRAVEL	LOW	NEW OFFICE BLDG.
									ROCK FOUND
									TO 16.0'

REMARKS Bottom of Hole = 16.0'

BORING KD-C5B7

PAGE 2 OF 2

\* See Legend on Back

BORING NUMBER

BORING LOG

NUS CORPORATION

PROJECT: KE 11054 DEELING BORING NO.: KDCSB-8  
 PROJECT NO.: 1670 DATE: 4-19-88 DRILLER: EDDIE  
 ELEVATION: \_\_\_\_\_ FIELD GEOLOGIST: Kim Karl  
 WATER LEVEL DATA: \_\_\_\_\_  
 (Date, Time & Conditions) 4-19-88, 0800, SUNNY 40°

SAMPLE NO. & DEPTH	DEPTH (FT)	DOWN S' OF JOB 1:1	SAMPLE RECOVERY SAMPLE LENGTH	LITHOLOGY CHARGE (DETAILED)	MATERIAL DESCRIPTION*			USCS	REMARKS
					SOIL DENSITY, CONSISTENCY OR POLE HARDNESS	COLOR	MATERIAL CLASSIFICATION		
	0-5						1" ASPHALT, 5" CONCRETE		
375	S-1	0-5	0.51		LIQ. SAND	MOIST	FINE TO COARSE SAND + SILT	SW	HI-MOIST
		2-0	1.7		SEM. SAND	MOD. MOIST	GRAVEL MIXED WITH SILT	GW	MOIST
					CLAY		FILL		
320	S-1	2-0	0.71		LIQ. SAND	MOD. MOIST	FINE TO COARSE SAND	SW	HI-MOIST
		4-0	1.20		SEM. SAND	MOD. MOIST	AND GRAVEL, SOME SILT	GW	AS
					CLAY		SILT NO ORGANICS		MOIST
							MICROBIAL FILL		
330	S-2	4-0	1.51		LIQ. SAND	MOD. MOIST	FINE TO COARSE SAND	SW	HI-MOIST
	S-1	6-0	1.20		CLAY	MOD. MOIST	GRAVEL WITH SILT	GW	MOIST-WET
							CLAY SILT HIGHLY PLASTIC		ML
							ORGANICS FILL		
345	S-2	6-0	0.51		CLAY	MOD. MOIST	FINE TO COARSE SAND AND SILT	SW	HI-MOIST
		6-5	1.25		SEM. SAND	MOD. MOIST	GRAVEL	GW	AS LOW MOIST
		6-							
							↓		
355	S-2	7-	1.51		LIQ. SAND	MOD. MOIST	FINE TO COARSE SAND + SILT	SW	HI-MOIST
		9	1.20		SEM. SAND	MOD. MOIST	GRAVEL WITH SILT	GW	MOD. MOIST
					CLAY		SILT - HIGHLY PLASTIC		
							CLAY MICROBIAL FILL		TO 9'

REMARKS 2 1/2" CORE - 3 PM ON 4/19/88

9-0' BOTTOM OF HOLE

Using F-117 & 140# Hammer with 30" Fall

\* See Legend on Back

BORING KDCSB-8

PAGE 1 OF \_\_\_\_\_

BORING LOG

POCKET LOG

NUS CORPORATION

PROJECT: KELLOGG DEERINK BORING NO: KDCS39  
 PROJECT NO.: L670 DATE: 4-19-58 DRILLER: E COLE  
 ELEVATION: ..... FIELD GEOLOGIST: KIM KALL  
 WATER LEVEL DATA: NOT ENGAGED  
 (Date, Time & Conditions) 4-19-58 C.S.D. OVERCAST 45°

SAMPLE NO. & TYPE	DEPTH (ft)	BLOW COUNT (100 SPS)	SAMPLE RECOVERY (SAMPLE LENGTH)	LITHOLOGICAL CHANGE (DEPTH)	MATERIAL DESCRIPTION*		USCS	REMARKS
					SOIL BENEATH CONSISTENCY OR SOIL HARDNESS	COLOR		
	0.5					ASPHALT		
020 S-1	0.5	5	1.0		Semi-CLAYEY	FINE TO COARSE SAND AND SILT	SM	MED MOIST
	2.0	12	1.5		Coarse CLAYEY	GRAVEL, SOME SILT	SM	HNU 500PM
						FILL LOW ORGANICS		A2
025 S-1	2.0	10	1.1		CLAYEY	FINE TO COARSE SAND	SM	LOW MOISTURE
	4.0	17	12.0		CLAYEY	GRAVEL, SILT	SM	HNU 1000PM
						LOW ORGANICS		A2
015 S-2	4.0	33	2.0		CLAYEY	FINE TO COARSE SAND	SM	HNU 500PM AS
	6.0	47	2.0		Semi-CLAYEY	GRAVEL, MISCANEOUS	SM	LOW MOISTURE
						LOW ORGANICS FILL		
025 S-2	6.0	41	1.5		CLAYEY	FINE TO COARSE SAND	SM	HNU 1000PM AS
	8.0	61	2.0		CLAYEY	GRAVEL, NO ORGANICS	SM	LOW MOIST.
035 S-3	2-10	32	2.0		CLAYEY	FINE TO COARSE SAND	SM	HNU 1000PM AS
		47	12.0		CLAYEY	GRAVEL, NO ORGANICS	SM	LOW MOIST
050 S-3	10-	37	1.5		CLAYEY	FINE TO COARSE SAND	SM	HNU 300PM AS
	12-	41	2.0		CLAYEY	GRAVEL, NO ORGANICS	SM	LOW MOIST
								TO 12'

REMARKS BACKGROUND OPEN ON HNU BOH = 12'  
Failure of A.F.C. 12# hammer with 30' Fall

BORING KDCS39

PAGE 1 OF 1

\* See Legend on Back





BORING LOG

NUS CORPORATION

PROJECT: KEILOG - OBSERV BORING NO.: KDCS-11  
 PROJECT NO.: LC 70 DATE: 7-19-98 DRILLER: ED.C.E.  
 ELEVATION: ..... FIELD GEOLOGIST: Kim KAAL  
 WATER LEVEL DATA: NOT ENCOUNTERED  
 (Date, Time & Conditions) 4-19-88, 11:30, OVERCAST, 50°

SAMPLE NO. & TYPE	DEPTH (ft)	BLOWS 1' OF LOG (1")	SAMPLE RECOVERY SAMPLE LENGTH	LITHOLOGY CHANGE (Describe)	MATERIAL DESCRIPTION*		VICIS	REMARKS
					SOIL BEHAVIOR, CONSISTENCY OR ROCK HARDNESS	COLOR		
	0-0.5					1" REINACT 5" CONCRETE		
130 S-1	0.5	11	0.5		CLAYEY MUD	FINE TO COARSE SAND	SW	4% H <sub>2</sub> O
	2.0	6	1.5		LOOSE BRN	SAND + GRAVEL	SW	MOIST SAMPLE
						NO ORGANICS		
35 S-1	2.0	3	0.6		CLAYEY MUD	FINE TO COARSE SAND	SW	4% H <sub>2</sub> O
	4.0	10	1.5		CLAYEY MUD	GRAVEL WITH SILT	SW	MOIST SAMPLE
						NO ORGANICS FILL	SW	
40 S-1	4	10	1.8		CLAYEY MUD	FINE TO COARSE SAND + GRAVEL	SW	4% H <sub>2</sub> O
	5.8	10	1.8		CLAYEY MUD	GRAVEL	SW	MOIST SAMPLE
500 S-1	6.0	10	1.6		CLAYEY MUD	FINE TO COARSE SAND	SW	1% H <sub>2</sub> O
S-2	6.0	10	1.0		CLAYEY MUD	GRAVEL	SW	MOIST
2 S-2	8.0	10	1.0		CLAYEY MUD	FINE TO COARSE SAND	SW	1% H <sub>2</sub> O
	9.0		1.0		BRN	GRAVEL	SW	TD 9'

REMARKS Bottom 9.0 Feet of Borehole  
15' = Remaining 10' of Borehole

BORING KDCS-11  
 PAGE 1 OF 1

\* See Legend on Back

PROJECT: Kellogg DRAIN BORING NO.: KD CSB-12  
 PROJECT NO.: 6670 DATE: 7-17-88 DRILLER: LDONE  
 ELEVATION: \_\_\_\_\_ FIELD GEOLOGIST: K. R. K. L.  
 WATER LEVEL DATA: WATER AT 12.5  
 (Date, Time & Conditions) 4-19-88 1600, OVERCAST, YD

SAMPLE NO. & TYPE	DEPTH (FT)	ROWS 1" OF LOG	SAMPLE RECOVERY LENGTH	LITHOLOGY CHANGE (DIAMETER)	MATERIAL DESCRIPTION*		VICI	REMARKS
					SOIL BEING CONSISTENCY OR SOCE HARDNESS	COLOR		
	0.3					1 ASPHALT		
600 S-1	1.5	2	1.4		GRANULAR MTD	FINE TO COARSE SAND	SW	
	2.0	4	1.5		SPN	AND FINE GRAVEL	SP	HIGH OPEN AB
								LOW MOISTURE
610 S-1	2.0	5	1.5		GRANULAR MTD	SILT WITH MEDIUM		HNU - 5 BPM AB
	4.0	10	2.0		FINE AGNT	SAND, MOD. PLASTIC		MOD. MOISTURE
					GRN	MICACEOUS, HIGH ORGANICS		
620 S-1	4.0		1.8		GRANULAR MTD	FINE TO COARSE SAND +	SW	8 BPM AB
	5.2		1.2		BRN	GRAVEL, WITH SILT	UN	LOW - MOD MOIST
630 S-2	6.0	11	4.4		GRANULAR MTD	FINE TO COARSE SAND GRAVEL	SW	OPEN AB, LOW MOIST
645 S-2	8.10	15	1.1		GRANULAR MTD	FINE TO COARSE SAND +	SW	5 BPM AB, MOD. MOIST
			2.0		GRANULAR MTD	GRAVEL	UN	LOW MOIST
655 S-2	10.2		1.5		GRANULAR MTD	FINE TO COARSE SAND +	SW	5 BPM AB
			2.0		BRN	GRAVEL	UN	LOW MOIST
705 S-3	12.2		1.4		GRANULAR MTD	FINE TO COARSE SAND + GRAVEL	SW	OPEN AB, MOD. MOIST TDM

REMARKS BOH = 14 Falling Head 140# hammer with  
30" Fall

BORING KD CSB-12  
 PAGE 1 OF 1

\* See Legend on Back

PROJECT: REINFORCED SPRING NO: LD-52-13  
 PROJECT NO: 1730 DATE: 6-20-48 DRILLER: ED  
 ELEVATION: 11.8 FIELD GEOLOGIST: R. M. ...  
 WATER LEVEL DATA: ...  
 (Date, Time & Conditions) ...

SAMPLE NO. & TYPE	DEPTH (FT)	BLOW COUNT (60#)	SAMPLE RECOVERY (%)	LITHOLOGY CHANGE (DEPTH)	MATERIAL DESCRIPTION*			REMARKS
					SOIL DENSITY, CONSISTENCY OR ROCK HARDNESS	COLOR	MATERIAL CLASSIFICATION	
	2-5						ASPHALT, 4" SOIL	
1730 E-N	5-	13	0.5				CONCRETE	2 1/2" Voids
	2-0	7	1.5				NO SAMPLE RECOVERED	NO SAMPLE
							EXPOSED TO WEATHERING	NO SAMPLE
								MAILED
740 S-1	2-0	5	1.0		UNSATURATED		SILT AND FINE TO MEDIUM SAND	ML - MUST WET
	4-0	8	1.5		UNSATURATED		MEDIUM SAND	OL - LOW PLASTICITY
					UNSATURATED		UNSATURATED MEDIUM TO FINE SAND	3 1/2" Voids
50 S-2	4-0	13	1.3		UNSATURATED		SILT AND FINE TO MEDIUM SAND	ML - MUST WET
	6-0	11	1.2		DENSE	GRN	COARSE SAND WITH SILT	CL - SOME PLASTICITY
							COARSE SAND	ABOVE 2 1/2" Voids
20 S-2	4-0	10	1.5		UNSATURATED		FINE TO COARSE SAND	3 1/2" Voids, 2-3" Voids
	8-0	15	1.2		UNSATURATED		SAND WITH SILT AND CLAY	3 1/2" Voids, 1-2" Voids
5-3	1-0	12	1.8		UNSATURATED		FINE TO COARSE SAND	SW - HIGH PLASTICITY
	0-0	15	1.2				SAND AND FINE GRAVEL	SP - 3 1/2" Voids
50 S-3	10-0	30	2.8		UNSATURATED		FINE TO COARSE SAND	SW - HIGH PLASTICITY
	11-0	20	1.2				FINE GRAVEL	SP - 2 1/2" Voids
								3 1/4" - 8" Voids

REMARKS: Bottom 11.8' Falling F-10 3.5  
11.8' Falling F-10 3.5

BORING LD-52-13

\* See Legend on Back

PROJECT: KEITHLY DEERING BORING NO: KDCSB-14  
 PROJECT NO.: 4670 DATE: 4-20-88 DRILLER: COOLE  
 ELEVATION: \_\_\_\_\_ FIELD GEOLOGIST: K.M. KAC  
 WATER LEVEL DATA: \_\_\_\_\_  
 (Date, Time & Conditions) 4-20-88, 0945, Sunny, Windy 40°

SAMPLE NO. & TYPE	DEPTH (ft)	FLOWMETER NO. & LOG	SAMPLE SECURITY SAMPLE LENGTH	LITHOLOGY CHANGE (BASICALLY)	MATERIAL DESCRIPTION*			USCS	REMARKS
					SOIL DENSITY, CONSISTENCY OR ROCK HARDNESS	COLOR	MATERIAL CLASSIFICATION		
	0-5						GRAVEL	HI	
945 S-1	5-1		5/1		GRAVEL		FINETOCARSE	SW	HNU 57PM AB
	2.0		2.5		SEN		SAND AND GRAVEL	GW	LOW MOISTURE
							LOW OCCASIONAL MUDS	SW	1/4 VOR
00 S-1	2.0		3/1		GRAVEL		FINETOCARSE SAND	SW	HNU OPEN AB
	4.0		2.5		SEN		LOW OCCASIONAL FILL	GW	LOW MOISTURE
									1/4 VOR
105 S-2	4.0		4/1		GRAVEL		FINETOCARSE SAND AND GRAVEL	SW	HNU OPEN AB
	6.0		6.5		SEN		COARSE GRAVEL FILL	GP	LOW MOISTURE
									1/2 VOR
110 S-2	6.0		6/1		GRAVEL		FINETOCARSE SAND	SW	HNU OPEN
	8.0		12.5		SEN		FILL		LOW MOISTURE
									1/2 VOR
	5.0		3/1		GRAVEL		FINETOCARSE SAND	SW	HNU 57PM
	7.0		11.0		SEN		FILL		LOW MOISTURE
									TO 9'

REMARKS Bottom 9.0 Fill of Fine Sand  
14.0 = Fine sand with 30% fill

BORING KDCSB-14

PAGE 1 OF 1

\* See Legend on Back

*KMK*

PROJECT: KELLON: DEELIN  
 PROJECT NO.: 4670 DATE: 4-20-83 BORING NO.: KDCSB-15  
 ELEVATION: FIELD GEOLOGIST: Kim Kasel DRILLER: EDCME  
 WATER LEVEL DATA: WATER 30.0  
 (Date, Time & Conditions) 4-20-83 11:00, SUNNY, WINDY, 50.0

SAMPLE NO & DEPTH	DEPTH (ft)	FLOW 1" OF ROD (ft)	SAMPLE RECOVERY SAMPLE LENGTH	LITHOLOGICAL CHANGE (DIP/ANGLE)	MATERIAL DESCRIPTION*		VICIS	REMARKS
					SOIL DENSITY, CONSISTENCY OF SOIL, HARDNESS	COLOR		
	0-0.5					3" ASPHALT		
110	5-1 0.5	4	8/		GLAUC. MUD	FINE TO COARSE SAND, SOME SW		HNU OPPM AB
	2.0	5	12.0		LOOSE SILT	COARSE WELL ROUNDED		LOW MOISTURE
115	5-1 2.0	4	0.5/		SILT	COARSE. NO ORGANICS		3 UOAS
	4.0	4	12.0		GLAUC. MUD	FINE TO COARSE SAND	SW	HNU OPPM AB
120	5-2 4.0	11	0.3/		LOOSE SILT	WHITE FINE GRAVEL		LOW MOISTURE
	5.0	10.5	11.2		GLAUC. MUD	SILT AND FINE TO COARSE SAND	ML	HNU OPPM AB
130	5-2 6.0	10.5	2.0/		MED. SILT	SAND	SW	2 UOAS
	8.0	10.5	12.0		GLAUC. MUD	FINE TO COARSE SAND		MOIST
145	5-3 8.0	10.5	2.0/		GLAUC. MUD	FINE TO COARSE SAND	SW	HNU OPPM AB
	9.0	10.5	11.0		LOOSE SILT		SW	3 UOAS, 3/4 METAL
210	5-3 10.0	10.5	2.0/		GLAUC. MUD	FINE TO COARSE SAND	SW	HNU OPPM AB
	2.0	10.5	12.0		LOOSE SILT	GRAVEL	SW	2-8.5
305	5-4 12.0	10.5	11.0/A		GLAUC. MUD	FINE TO COARSE SAND +	SW	11 PPM AB

REMARKS: 12.9  
 Failure Fine Silty  
 4.5# Hammer with 30" Fall

BORING KDCSB-15  
 PAGE 1 OF 2

\* See Legend on Back

4-26

PROJECT: FEIRUA DEE... BORING NO: KP05B-15  
 PROJECT NO.: 4-20 DATE: 4-20-88 DRILLER: EDDIE  
 ELEVATION: \_\_\_\_\_ FIELD GEOLOGIST: Ron Kitch  
 WATER LEVEL DATA: WATER 30.0'  
 (Date, Time & Conditions) 4-20-88 1305 Sunny, 60-70°F

SAMPLE NO. & TYPE	DEPTH (ft)	FLOW S OF TUB (ft)	SAMPLE RECOVERY SAMPLE LENGTH	WATER LOSS CHARGE (percent)	MATERIAL DESCRIPTION*			VIC	REMARKS
					SOIL STRAT. CONSISTENCY OR ROCK HARDNESS	COLOR	MATERIAL CLASSIFICATION		
305 S-4	14.0	10.0	2.0		BRN	GRN	GRAVEL	LOW	MOIST 3 VOL. 1/2 bag
320 S-4	14.0	10.0	2.0		GRN	GRN	FINE TO COARSE SAND	LOW	Hvy 0 ppm H <sub>2</sub> O
	16.0	12.0	2.0		GRN	GRN	GRAVEL	LOW	MOIST 3-5 bag
350 S-5	16.0	12.0	4.0		GRN	GRN	FINE TO COARSE S+G	LOW	Hvy 0 ppm: 3/4 vol
40 S-5	17.0	13.0	1.3		GRN	GRN	FINE TO COARSE SAND	LOW	Hvy 0 ppm AS
	19.0	15.0	2.0		GRN	GRN	GRAVEL	LOW	2 1/2 vol 1 metal
	19.0	15.0	1.0		GRN	GRN	FINE TO COARSE SAND	LOW	2 1/2 vol 1/4 vol
	20.0	16.0	1.0		GRN	GRN	GRAVEL	LOW	
420 S-6	20.0	16.0	2.0		GRN	GRN	FINE TO COARSE SAND	LOW	Hvy 2 ppm AS
	22.0	18.0	2.0		GRN	GRN	GRAVEL	LOW	3 VOL. 1 metal
130 S-6	22.0	18.0	2.0		GRN	GRN	FINE TO COARSE SAND	LOW	Hvy 5 ppm AS
	24.0	20.0	2.0		GRN	GRN	GRAVEL	LOW	2-5 bag
S-7	24.0	20.0	0				NO SAMPLE RECOVERED		

REMARKS \_\_\_\_\_  
 \_\_\_\_\_  
 \_\_\_\_\_

BORING 4-20-88  
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\* See Legend on Back

PROJECT: Kenmore Park BORING NO: KDCSS 15  
 PROJECT NO: 4670 DATE: 4-22-68 DRILLER: ED COLE  
 ELEVATION: \_\_\_\_\_ FIELD GEOLOGIST: KIM L. M.  
 WATER LEVEL DATA: WATER 30.0'  
 (Date, Time & Conditions) 4:30 PM, 4:50, Sunny, Windy

SAMPLE NO. & TYPE	DEPTH (ft)	FLOW ST. OR LOG (ft)	SAMPLE RECOVERY SAMPLE LENGTH	LITHOLOGY CHANGE (DEPTH)	MATERIAL DESCRIPTION*		VICI	REMARKS
					CON. BENTON. CONSISTENCY OR SOIL HARDNESS	COLOR		
450	S-7	25'	10'		(CALCULATED)	FINE TO COARSE SAND	Su	4 in. 2.5 ppm
		28'	12.0'		(LOSS)	FINE - GRAVEL	Su	4 in. 2.5 ppm
								1/2 - 5/8"
50	S-3	25'	15'		(CALC.)	MID FINE TO COARSE SAND	Su	13 ppm ARB. TV
		30'	12.0'		(LOSS)	FINE - GRAVEL	Su	4 in. 2.5 ppm
60	S-3	30'	15'		(CALC.)	FINE TO COARSE SAND	Su	4 in. 2.5 ppm
			12.0'		(LOSS)	FINE - GRAVEL	Su	4 in. 2.5 ppm
								TO 32'

REMARKS RE 4 32.0'

BORING KDCSS 15  
PAGE 2 OF 2

\* See Legend on Back

PROJECT: Kellogg - Deering BORING NO: CSB16  
 PROJECT NO: 4670 DATE: 4-20-88 DRILLER: VINCE PRUE Emp. re Soils  
 ELEVATION: \_\_\_\_\_ FIELD GEOLOGIST: Geo. H. M. Kral  
 WATER LEVEL DATA: \_\_\_\_\_  
 (Date, Time & Conditions)

SAMPLE NO. & TYPE	DEPTH (ft)	BLOWS 1' OR 400 (ft)	SAMPLE RECOVERY SAMPLE LENGTH	LITHOLOGY CHANGE (ft)	MATERIAL DESCRIPTION*		VICIS	REMARKS
					SOIL DENSITY, CONSISTENCY OR POOR HARDNESS	COLOR		
SP1 1216	0-2'	13	1.7	0.3	MED. DENSE	Gr B1	SM	ASHBURY TO 3' HARDENED MUD DAMP
1220	2'-4'	21	0.3	1.1	CLAYE			FIN. 1 ppm 100/1
								STOPPED AT 39 (TD) Auger Refusal Poss. blr Boulder

REMARKS Location Behind Elwco  
2" SS Drive 30" w 140" Hammer  
AKer Soil Max Rig

\* See Legend on Back

BORING CSB16

PAGE 1 OF 1



PROJECT: Kellogg - Drilling  
 PROJECT NO.: 2670 DATE: 4-20-84 BORING NO.: CSB18  
 ELEVATION: FIELD GEOLOGIST: Scott M. Knoll DRILLER: VINCE PRUE  
 WATER LEVEL DATA: Engue Soils  
 (Date, Time & Conditions)

SAMPLE NO. & TYPE	DEPTH (ft)	FLOW 1" DIA. ROD (ft)	SAMPLE RECOVERY SAMPLE LENGTH	LITHOLOGY CHANGE (ft)	MATERIAL DESCRIPTION*			VICI	REMARKS
					SOIL DENSITY, CONSISTENCY OR POCE HARDNESS	COLOR	MATERIAL CLASSIFICATION		
S-1 1530	0-2'	12	0/1.5		MEDIUM	BROWN	SAND GRAVEL (Fill)	SM	Humid - 3
		16							HNU = 0 DAMP
1538	2'-4'	19	14/20	25/30	DENSE	TAN	SILT NO GRAVEL (Fill)	SM	HNU = 0.4 DAMP
		11.5					SILTY SAND (F-6) GRAVEL 1/2-1"	SM	
S-2 1540	4-6'	42	12/20		DRIVE	BROWN			HNU = Open
1615	6-8'	11.5	12/20		NO DRIVE				100/2 HNU = Open
		10.8					ANGRE REFRA 7.9 TD		
S-3	4-10'								

REMARKS TRIPOD METHOD FOR SAMPLING DUE TO  
High Tension wire above w 2" SS DRIVEN  
30" w 140" Hammer AKAC Soil Max Rig

\* See Legend on Back

BORING CSB18  
 PAGE 1 OF 1

PORING LOG

NUS CORPORATION

PROJECT: Kellogg - Dredging

PROJECT NO.: L670

BORING NO.: CSB18

ELEVATION:

DATE: 4-20-84

DRILLER: VINCE PRUE

WATER LEVEL DATA:

FIELD GEOLOGIST: Scott M. Krahl

Empire Soils

(Date, Time & Conditions)

SAMPLE NO. & TYPE	DEPTH (ft)	FLOW ROD NO. & LENGTH (ft)	SAMPLE RECOVERY SAMPLE LENGTH	LITHOLOGY CHANGE (ft)	MATERIAL DESCRIPTION*		VICI	REMARKS
					LOG BEHAVIOR, CONSISTENCY OR SOIL HARDNESS	MATERIAL CLASSIFICATION		
5-1 1530	0-2'	12	0.7 1.5	5	MDAYE BROWN	SAND GRAVEL (Fill)	SM	MINIMAL - 3
1538	2-4'	19	1.4 2.0	20 30	DEAFE TAN	SILT NO GRAVEL (Fill)	SM	HNU = 0 DAMP HNU = 0.4 DAMP
3-2 1546	4-6'	22	1.9 2.0		DEAFE BROWN	SILTY SAND (FILL) GRAVEL 1/2-1"	SM	HNU = 0 DAMP
1615	6-8'	24	1.3 1.3		V. DEAFE			100/2 HNU = 0 DAMP
5-3	8-10'					AS PER RECORD - 7.9' TD		

REMARKS TRIPOD METHOD FOR SAMPLING DUE TO  
High Tension wire above w 2" SS DRIVEN  
30" w 140" Hammer AKer Soil Max Rig

\* See Legend on Back

BORING CSB18

PAGE 1 OF 1

PROJECT: Kellogg - Opening BORING NO.: CSB 19  
 PROJECT NO.: L670 DATE: 4-21-88 DRILLER: Vince Pire  
 ELEVATION: \_\_\_\_\_ FIELD GEOLOGIST: Scott M. Kelli Empire Soils  
 WATER LEVEL DATA: \_\_\_\_\_  
 (Date, Time & Conditions) \_\_\_\_\_

SAMPLE NO. & TYPE	DEPTH (ft)	BLOWS 1" OR 300 (ft)	SAMPLE RECOVERY SAMPLE LENGTH	LITHOLOGY CHANGE (Depth ft)	MATERIAL DESCRIPTION*			VICI	REMARKS
					SOIL DENSITY, CONSISTENCY OR POLE HARDNESS	COLOR	MATERIAL CLASSIFICATION		
5-1 OS00	0-2'	21	0.6 / 1.5	2.5	M. DENSE	BR	SILTY SAND TO CLAY GRAVEL (F. 11)	Sm	Amount of sand HNV = 15
OS05	2-4'	7	0.1 / 2.0	2.5	LOOSE	TAN	SILT (F. 11)		HNV = 0 per Dam?
3-2 OS10	4-6'	42	1.0 / 2.0	30	V. DENSE	BR	Silty sand and gravel 1/2" (F. 11) (with debris)	Sm	HNV = 0 Dam?
OS15	6-8'	92	0.7 / 0.8	70	V. DENSE	TAN	SAND (no coarse) gravel 1/2" (F. 11)	Sm	HNV = 0 per Dam 150/3
5-3 OS15	8-10'	100	0.8 / 0.9		V. DENSE		89 TO		100% 7-8' stopped HNV = 0 per

REMARKS 2" SS DRIVEN 30" w 140' Hammer  
Aker Soil Test Rig

BORING CSB 19

PAGE 1 OF 1

\* See Legend on Back

PROJECT: KAYAK DEEPENING BORING NO.: SDCSB20  
 PROJECT NO.: 1270 DATE: 4-21-83 DRILLER: ED COLE  
 ELEVATION: \_\_\_\_\_ FIELD GEOLOGIST: Kim KAAL  
 WATER LEVEL DATA: \_\_\_\_\_  
 (Date, Time & Conditions) 4:21:55, DECO, OVERCAST 45°

SAMPLE NO. & DEPTH	DEPTH (ft)	DEPTH OF LOG (ft)	SAMPLE RECOVERY (ft)	LITHOLOGY CHANGE (ft)	MATERIAL DESCRIPTION*		VICI	REMARKS
					SOIL DENSITY, CONSISTENCY OR SOIL CLASSIFICATION	COLOR		
	00-0.5					ASPHALT		
85	S-1 1.5-2.0	1.5	0.5	1.5	loose	fine to coarse sand + gravel	SW	4% H <sub>2</sub> O, 1% silt
		2.0	1.5	1.5	loose	fine to coarse sand + gravel	SW	3-1/2% H <sub>2</sub> O, 1/2% silt
						loose		low moist.
820	S-1 2.0-4.0	2.0	1.4	2.0	loose	fine to coarse sand + gravel	SW	4% H <sub>2</sub> O, 1% silt
		4.0	2.0	2.0	loose	fine to coarse sand + gravel	SW	3-1/2% H <sub>2</sub> O, 1% silt
						loose		low moist 1/2-80%
830	S-2 4.0-6.0	4.0	0.6	10.9	loose	fine to coarse sand + gravel	SW	4% H <sub>2</sub> O, 1% silt
		6.0	0.6	10.9	loose	fine to coarse sand + gravel	SW	low moist 1/2-80%
8	S-2 6.0-8.0	6.0	2.0	12.0	loose	fine to coarse sand + gravel	SW	4% H <sub>2</sub> O, 3% silt
		8.0	2.0	12.0	loose	fine to coarse sand + gravel	SW	low moist 1/2-80%
900	S-3 8.0-9.0	8.0	1.0	1.0	loose	fine to coarse sand + gravel	SW	4% H <sub>2</sub> O, 3% silt
		9.0	1.0	1.0	loose	fine to coarse sand + gravel	SW	low moist 1/2-80%
930	S-3 9.0-12.0	9.0	2.0	2.0	loose	fine to coarse sand + gravel	SW	4% H <sub>2</sub> O, 2% silt
		12.0	2.0	2.0	loose	fine to coarse sand + gravel	SW	low moist 1/2-80%

REMARKS: BOH 12.0' 9:45 - DECKS COMPLETE  
TRANSFER SAMPLES (KR)  
4-21-83

BORING SDCSB20  
 PAGE 1 OF 1

\* See Legend on Back  
 Failing F-10 R-3 10# hammer with 30" fall





PROJECT: 110106 - 100706 BORING NO: K05203  
 PROJECT NO: 6670 DATE: 8-21-68 DRILLER: WILLIAM KYLE  
 ELEVATION: ..... FIELD GEOLOGIST: Ken K. K.  
 WATER LEVEL DATA: 11 0' @ 10:00 AM  
 (Date, Time & Conditions) 4-21-68 1:00 PM, Windy, Sunny, SE

SAMPLE NO. & DEPTH	DEPTH (ft)	BLOWS (1' OR LESS)	SAMPLE RECOVERY (%)	ATTACHED TAGS (IF ANY)	MATERIAL DESCRIPTION*		VICI	REMARKS
					SOIL BEING EXAMINED OR SOIL HARDNESS	MATERIAL CLASSIFICATION		
	00-5					ASPHALT		
300	S-1	2.5	14/		GR	FINE TO COARSE SAND	SW	HAN 0' FROM TOP
	2.0		12.0		LOOSE SAND	SAND + GRAVEL	SW	2 1/2 Vials 30-40%
					GR	NO ORGANICS		MOIST
10	S-1	3.0	15/		GR	SAND + GRAVEL	SW	HAN 0' FROM TOP
	4.0		13.0		LOOSE SAND	FINE TO COARSE	SW	7 1/2 Vials
					GR	NO ORGANICS		MOIST
320	S-2	4.0	6/		GR	SAND + GRAVEL	SW	HAN 0' FROM TOP
	5.0		12.0		LOOSE SAND	FINE TO COARSE	SW	2 Vials
						NO ORGANICS		MOIST
3	S-2	3.0	17/		GR	SAND + GRAVEL	SW	HAN 2' FROM TOP
	4.0		13.0		LOOSE SAND	COARSE	SW	MOIST
						NO ORGANICS		
345	S-3	5.0	00/			NO RECOVERY		NO SAMPLE RECOVERED
	10.0		12.0					4-21-68
200	S-3	2.0	1.5/		GR	FINE TO COARSE SAND	SW	WET SPIN
	2.0		12.0		LOOSE SAND	SAND + GRAVEL METAL	SW	SATURATED SAMPLE
					GR	NO ORGANICS		HAN 10' FROM TOP
						NO ORGANICS		1.80% 3 Vials
								TD 12'

REMARKS: NO SAMPLE RECOVERED AT 5.0 FT  
ENTERED IN LOG

BORING C6823  
 PAGE 1 OF 1

\* See Legend on Back

PROJECT: KE 166 - DEWIND BORING NO.: KDCS B 24  
 PROJECT NO.: 4670 DATE: 12-15-58 DRILLER: WALTER F. RICE  
 ELEVATION: ..... FIELD GEOLOGIST: KIM K. KRAL  
 WATER LEVEL DATA: .....  
 (Date, Time & Conditions) 4-21-59 1600, Sunny, WINDY, 50°

SAMPLE NO. & DEPTH	DEPTH (FEET)	BLOW COUNT (60 SEC)	SAMPLE DEPTH (FEET)	SAMPLE LENGTH	LITHOLOGY CHANGE (DEPTH)	MATERIAL DESCRIPTION*		VICI	REMARKS
						LOG NUMBER (CONSISTENCY OR SOIL HARDNESS)	COLOR		
	0-1.5						ASPHALT		
620 S-1	1.5-2.0	4	1.6	1.5		CLAY MUD	SILT AND FINE TO COARSE SAND LOW PLASTICITY, LOW SHRINKAGE	ML	HOW COARSE AS MOIST
S-1	2.0-4.0	3	1.0	2.0		CLAY MUD	SILT, SOME FINE TO COARSE SAND, LOW PLASTICITY	ML	HOW COARSE AS MOIST
620 S-2	4.0-6.0	2					CLAY MUD (COARSE)		
S-2	6.0-8.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND, LOW PLASTICITY	ML	HOW COARSE AS MOIST - WET
620 S-3	8.0-10.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
S-3	10.0-12.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
620 S-4	12.0-14.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
S-4	14.0-16.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
620 S-5	16.0-18.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
S-5	18.0-20.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
620 S-6	20.0-22.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
S-6	22.0-24.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
620 S-7	24.0-26.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
S-7	26.0-28.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
620 S-8	28.0-30.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
S-8	30.0-32.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
620 S-9	32.0-34.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
S-9	34.0-36.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
620 S-10	36.0-38.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
S-10	38.0-40.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
620 S-11	40.0-42.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
S-11	42.0-44.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
620 S-12	44.0-46.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
S-12	46.0-48.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
620 S-13	48.0-50.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
S-13	50.0-52.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
620 S-14	52.0-54.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
S-14	54.0-56.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
620 S-15	56.0-58.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
S-15	58.0-60.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
620 S-16	60.0-62.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
S-16	62.0-64.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
620 S-17	64.0-66.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
S-17	66.0-68.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
620 S-18	68.0-70.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
S-18	70.0-72.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
620 S-19	72.0-74.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
S-19	74.0-76.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
620 S-20	76.0-78.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
S-20	78.0-80.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
620 S-21	80.0-82.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
S-21	82.0-84.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
620 S-22	84.0-86.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
S-22	86.0-88.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
620 S-23	88.0-90.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
S-23	90.0-92.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
620 S-24	92.0-94.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
S-24	94.0-96.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
620 S-25	96.0-98.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST
S-25	98.0-100.0	2	2.0	2.0		CLAY MUD	SILT AND FINE TO COARSE SAND	SW	HOW COARSE AS MOIST

REMARKS: 3-4 13:0 TD  
Sample size 1/2" dia taken from 0'-12.0'  
After Soil Dry 2 1/2 40# hammer falling 30"  
 \* See Legend on Page

BORING CSE  
 PAGE 1 OF 1



PROJECT Kellogg Deering  
 PROJECT NO 1670  
 ELEVATION  
 WATER LEVEL DATA  
 (Date, Time & Conditions)

DATE 5/2/88  
 FIELD GEOLOGIST Blackritz

BORING NO KDC0805  
 DRILLER

SAMPLE NO & DATE	DEPTH (ft)	FLOWS (ft/hr)	SAMPLE RECOVERY (ft)	LITHOLOGICAL CHANGE (depth)	MATERIAL DESCRIPTION*		VICI	REMARKS
					10% DENSITY CONSISTENCY OR POOR MATERIALS	COLOR		
S-1	2-3	0	0.7		Ben	Brown m. to br. sand and gravel		0-0.5 ft. sample no. 1 1.5-2.0 ft. sample no. 2 2.0-2.5 ft. sample no. 3
	2-4	0	0.5		Ben			8 PPM 9 PPM 10 PPM 11 PPM 12 PPM 13 PPM 14 PPM 15 PPM 16 PPM 17 PPM 18 PPM 19 PPM 20 PPM 21 PPM 22 PPM 23 PPM 24 PPM 25 PPM 26 PPM 27 PPM 28 PPM 29 PPM 30 PPM 31 PPM 32 PPM 33 PPM 34 PPM 35 PPM 36 PPM 37 PPM 38 PPM 39 PPM 40 PPM 41 PPM 42 PPM 43 PPM 44 PPM 45 PPM 46 PPM 47 PPM 48 PPM 49 PPM 50 PPM 51 PPM 52 PPM 53 PPM 54 PPM 55 PPM 56 PPM 57 PPM 58 PPM 59 PPM 60 PPM 61 PPM 62 PPM 63 PPM 64 PPM 65 PPM 66 PPM 67 PPM 68 PPM 69 PPM 70 PPM 71 PPM 72 PPM 73 PPM 74 PPM 75 PPM 76 PPM 77 PPM 78 PPM 79 PPM 80 PPM 81 PPM 82 PPM 83 PPM 84 PPM 85 PPM 86 PPM 87 PPM 88 PPM 89 PPM 90 PPM 91 PPM 92 PPM 93 PPM 94 PPM 95 PPM 96 PPM 97 PPM 98 PPM 99 PPM 100 PPM
S-2	4-6	0	0.5		Ben	lt. Ben med. to fine sand w. some gravel		4 PPM
	6-8	0	0.5		Ben			3 PPM
S-3	8-10	0	0.5		Ben	Med. to fine sand and gravel		20 PPM None in bottom
	10-12	0	0.1		Ben	Very small recovery Ben med. to coarse sand		production refusal After refusal 1/15/88

REMARKS After Soil Max. Rig using 140 lb hammer  
 falling 30" - Tripod used for hammer dead  
 due to hole location beneath power lines.  
 \* See Legend on back  
 2" Stainless split spoon sampling device

BORING KDC08-25  
 PAGE 01

PROJECT: Kellogg - Demans BORING NO: KD-65826-  
 PROJECT NO: 4670 DATE: 3/3/88 DRILLER: V. P. R  
 ELEVATION: FIELD GEOLOGIST: Blackert  
 WATER LEVEL DATA:  
 (Date, Time & Conditions)

SAMPLE NO. & DEPTH	DEPTH IN FEET	SAMPLE LENGTH	LITHOLOGY (CHANGE INDICATED)	MATERIAL DESCRIPTION*		VISC	REMARKS
				CON. BINDER, EQUIPMENT OR OTHER MATERIALS	MATERIAL CLASSIFICATION		
S-1	0-2	0.7			Brn	Fin. Red to tan sand	Top of pit 0.5 PPM Duplicate taken
	2-4	0.4			Brn	subrounded	0.0 PPM
S-2	4-6	0.2			Brn	Med. to coarse sand	0.2 PPM
	6-8	0.2				subrounded	Higher Percent at 6.7 2.0 at 7
S-3	8-10						

REMARKS: After Soil Max Rig and  
 140# hammer with 30" fall  
 2" Stainless Steel split spoon sampling device

BORING KD-65826  
 PAGE \_\_\_ OF \_\_\_



PROJECT: Kallogg - Diering BORING NO.: 10-CSB 28  
 PROJECT NO.: 1670 DATE: 5/3/86 DRILLER: \_\_\_\_\_  
 ELEVATION: \_\_\_\_\_ FIELD GEOLOGIST: Blackett  
 WATER LEVEL DATA: \_\_\_\_\_  
 (Date, Time & Conditions) \_\_\_\_\_

SAMPLE NO. & TYPE	DEPTH (ft)	STONS 1" dia	SAMPLE RECOVERY (%)	LITHOLOGY CHANGE (ft)	MATERIAL DESCRIPTION*		VICI	REMARKS
					TONS PER MINUTE (approx)	COLOR		
S-1	0-2	9	26	1.5		tan Med to coarse sand		0-0.8 movement, 2 PPM MAS-2 test
	2-4	31	2.2	2.0		GRAY tan Gray fine material crushed gravel?		CFPM
S-2	4-6	27	0.7	2.0		tan Med to coarse sand and gravel		5 PPM
	6-8	52	1.1	2.0		tan Med to coarse sand and gravel		22 PPM
S-3	8-10	49	0.2	1.2		tan Med to coarse sand and gravel		21 PPM
	10-12	31	4.7	1.0		tan Gravel & pebbles with med to coarse sand		7 PPM
S-3	12-14	124	2.50	2.0				Reusal at 12.6 lett from 12.2 Nugget to 15 very hard 15' TD

REMARKS After Soil Max  
140# weight over 30"  
2" split spacers (stainless steel)

BORING CSB 28  
 PAGE \_\_\_\_\_ OF \_\_\_\_\_

\* See legend on back

PROJECT: Kellogg-Dearing BORING NO.: CSB 29  
 PROJECT NO.: 167D DATE: 5/3/88 DRILLER:  
 ELEVATION: FIELD GEOLOGIST: Blackett  
 WATER LEVEL DATA: (Obs. Time & Conditions)

SAMPLE NO. & TYPE	DEPTH (ft)	BLOWS (7' or 10')	SAMPLE RECOVERY (%)	CORRECTION CHANGE (ft/min)	MATERIAL DESCRIPTION*		VICIS	REMARKS
					FOR BRINNY, CONSISTENCY OR SOIL HARDNESS	COLOR		
S-1	0-2	7	0%		Brn	Sandy clay and med		2 PPM
	2-4	4	10%		Brn	to coarse sand		2 PPM
S-2	4-6	11	0%		Brn	Dark to brown sand		1 PPM
	6-8	17	0%		Brn	fine to coarse sand and gravel		2 PPM
S-3	8-10	22	0%		Brn	med to coarse sand & gravel		1 PPM
	10-12	28	0%					2 PPM
								Rough & Spoon R-Fluse at 11'
								Below? 12.2-12.20
								removed @ 11' - rough
								to 12.2-TD

REMARKS: After Soil Max drill rig with 140# weight falling over dist. of 30" - 2" OD stainless steel split spools.

\* See Legend on Back

PROJECT: Kellogg-Deering  
 PROJECT NO. LG 98  
 ELEVATION:  
 WATER LEVEL DATA  
 (Date, Time & Conditions)

DATE: 5/9/88  
 FIELD GEOLOGIST: Blackert

BORING NO. KD-5830  
 DRILLER: V. Prue

SAMPLE NO & TIME	DEPTH (ft)	BLOWS # OF 400 FT	SAMPLE RECOVERY SAMPLE LENGTH	LITHOLOGICAL CHANGE (Describe)	MATERIAL DESCRIPTION*		VICI	REMARKS
					100 PENNY CONSISTENCY DEGREE HARDNESS	COLOR		
S-1	0-2	1	0.1	1.5	Ben	Med to coarse sand		Top of formation - 2 PPM water content
	2-4	13	0.3	2.0	Ben	Sandy clay and med to coarse fine sand		2.85% From 2 PPM
S-2	4-6	7	0.4	2.0	Ben	Seal and clay with some gravel		Residual H <sub>2</sub> O 2 PPM
	6-8	18	0.1	2.0	Ben	Sand, gravel and cobbles		*250 PPM*
S-3	8-10	20	0.1	2.0		Sand, gravel and cobbles		*200 PPM*
	10-12	71	0.1	2.0		Sand, gravel and cobbles		30 PPM Water at bottom - Refusal Refusal at 10.6' Auger to 14.3 Refusal TO 14.9

REMARKS: Baker Soil Maxwell Rig 15# weight falling 30" Stainless Steel 2" split spoons. Typical Used for weight drop due to basehole location under power lines.

BORING KD-5830  
 PAGE \_\_\_ OF \_\_\_

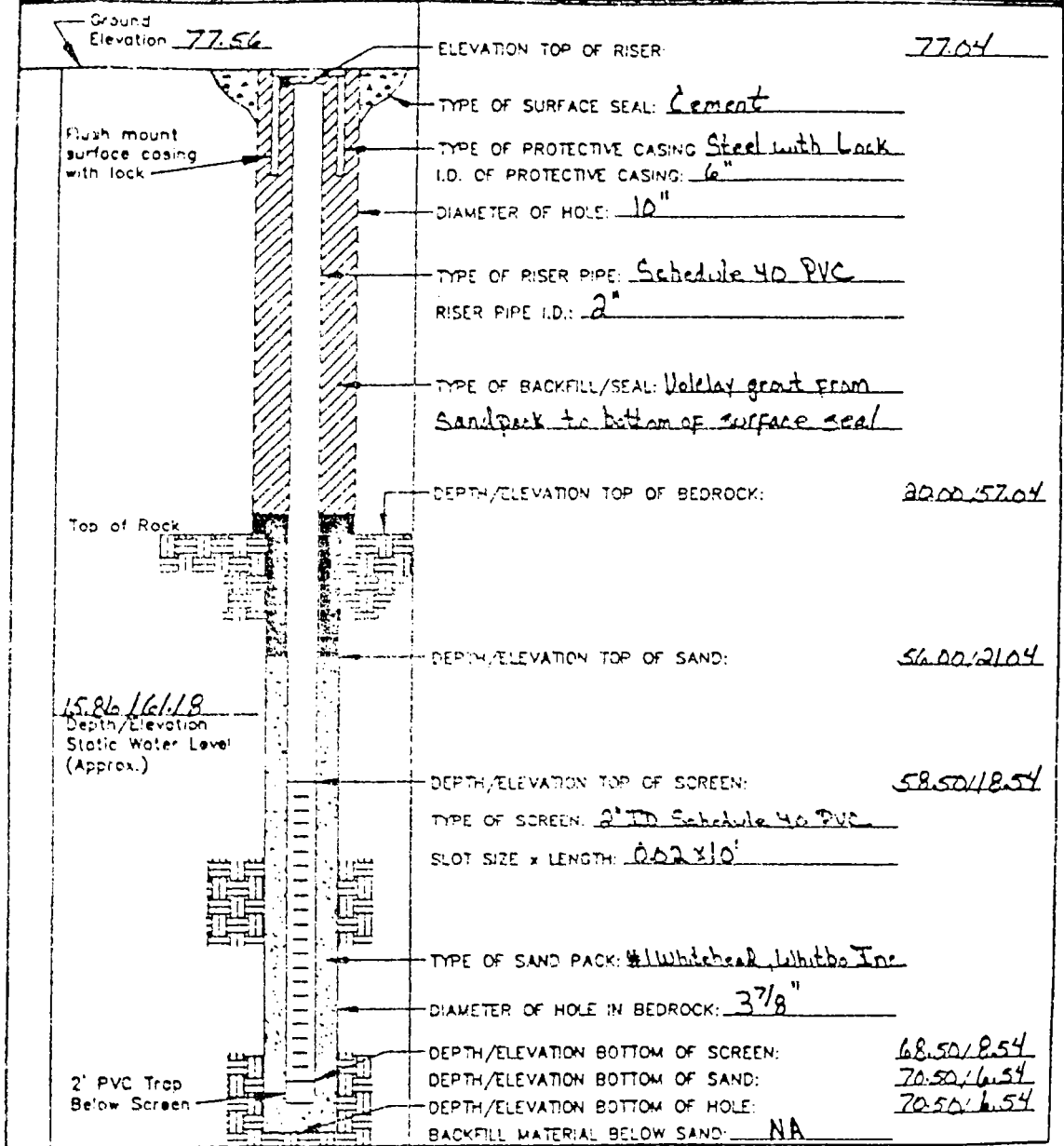
MONITORING WELL  
CONSTRUCTION DIAGRAMS



WELL NO.: K18A

**BEDROCK  
MONITORING WELL SHEET**  
WELL INSTALLED IN BEDROCK

PROJECT: <u>Kellogg-Dearing</u>	LOCATION: <u>Norwalk CT</u>	DRILLER: <u>E. Cole/Empire</u>
PROJECT NO.: <u>1670</u>	BORING: <u>K18A</u>	DRILLING METHOD: <u>W.S. Auger/Core &amp; Beam</u>
ELEVATION: <u>77.56 (Ground)</u>	DATE: <u>3/15/88</u>	DEVELOPMENT METHOD: <u>Air Lift</u>
FIELD GEOLOGIST: <u>Dan Blackert</u>		



SCALE: 1/4" = 1'-0"

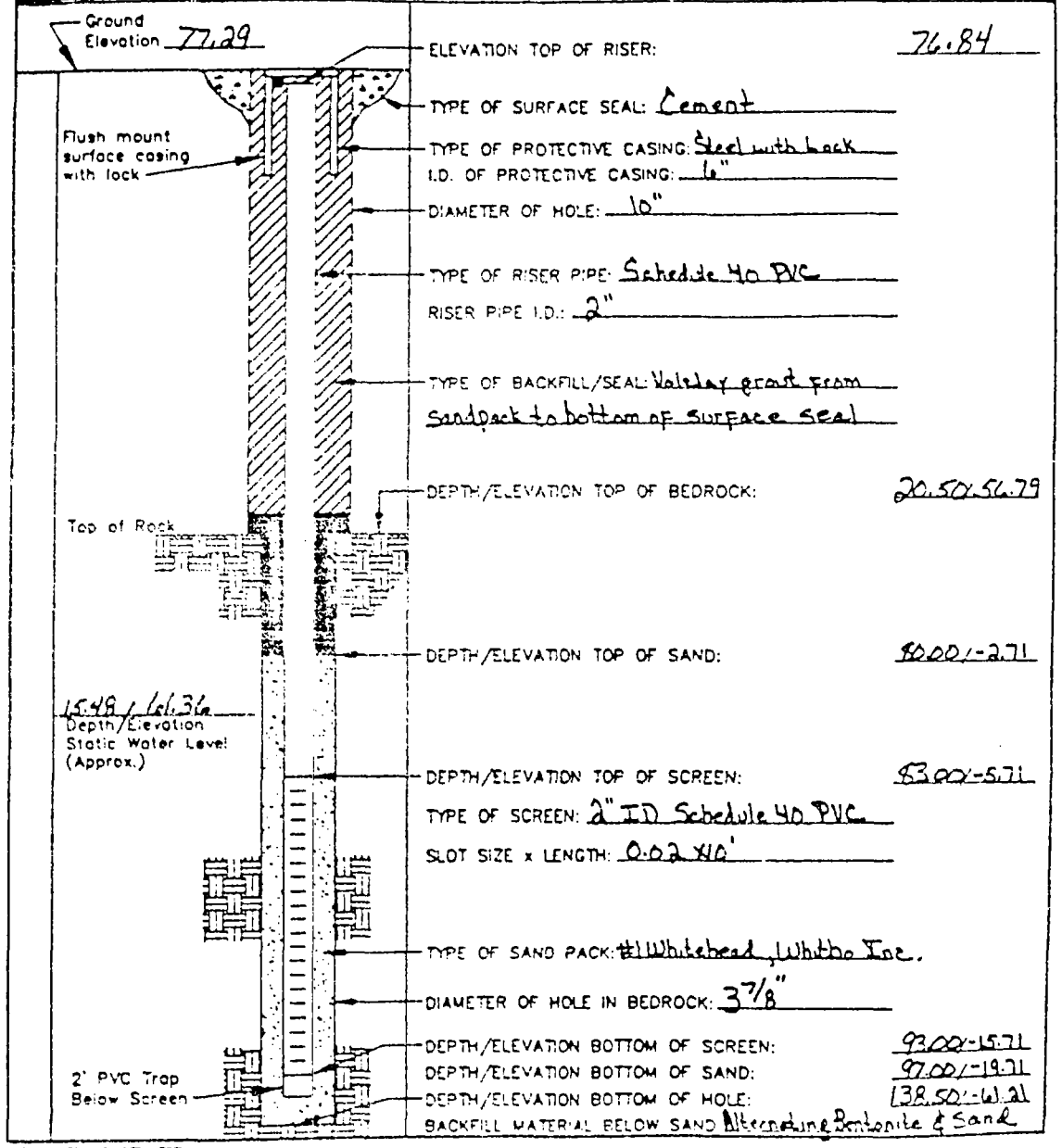




WELL NO.: K18B

**BEDROCK  
MONITORING WELL SHEET**  
WELL INSTALLED IN BEDROCK

PROJECT: <u>Kellogg-Deering</u>	LOCATION: <u>Norwalk CT</u>	DRILLER: <u>E. Cole/Empire</u>
PROJECT NO.: <u>L670</u>	BORING: <u>K18B</u>	DRILLING METHOD: <u>U.S. Auger/Case/Ream</u>
ELEVATION: <u>77.29 (Ground)</u>	DATE: <u>3/22/88</u>	DEVELOPMENT METHOD: <u>Air Lift</u>
FIELD GEOLOGIST: <u>Don Blackett</u>		



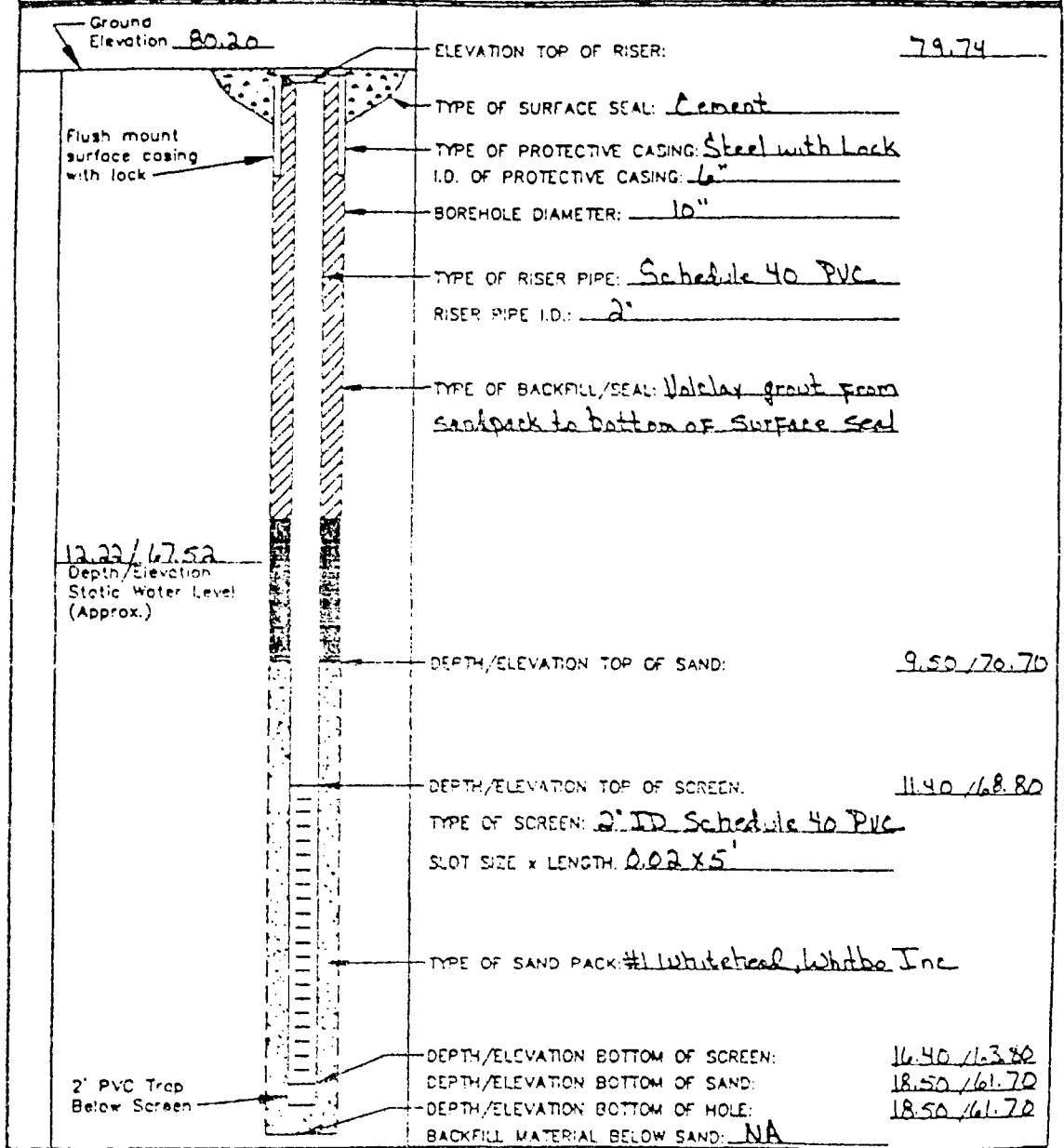
SCALE: 1/4" = 1'-0"



WELL NO.: K19A

### OVERBURDEN MONITORING WELL SHEET

PROJECT: <u>Kellogg-Deering</u>	LOCATION: <u>Norwalk 2T</u>	DRILLER: <u>V. Puse / Empire</u>
PROJECT NO.: <u>L670</u>	BORING: <u>K19A</u>	DRILLING METHOD: <u>Hollow Stem Auger</u>
ELEVATION: <u>80.20 (Ground)</u>	DATE: <u>3/16/88</u>	DEVELOPMENT METHOD: <u>Air Lift</u>
FIELD GEOLOGIST: <u>S. Krall</u>		

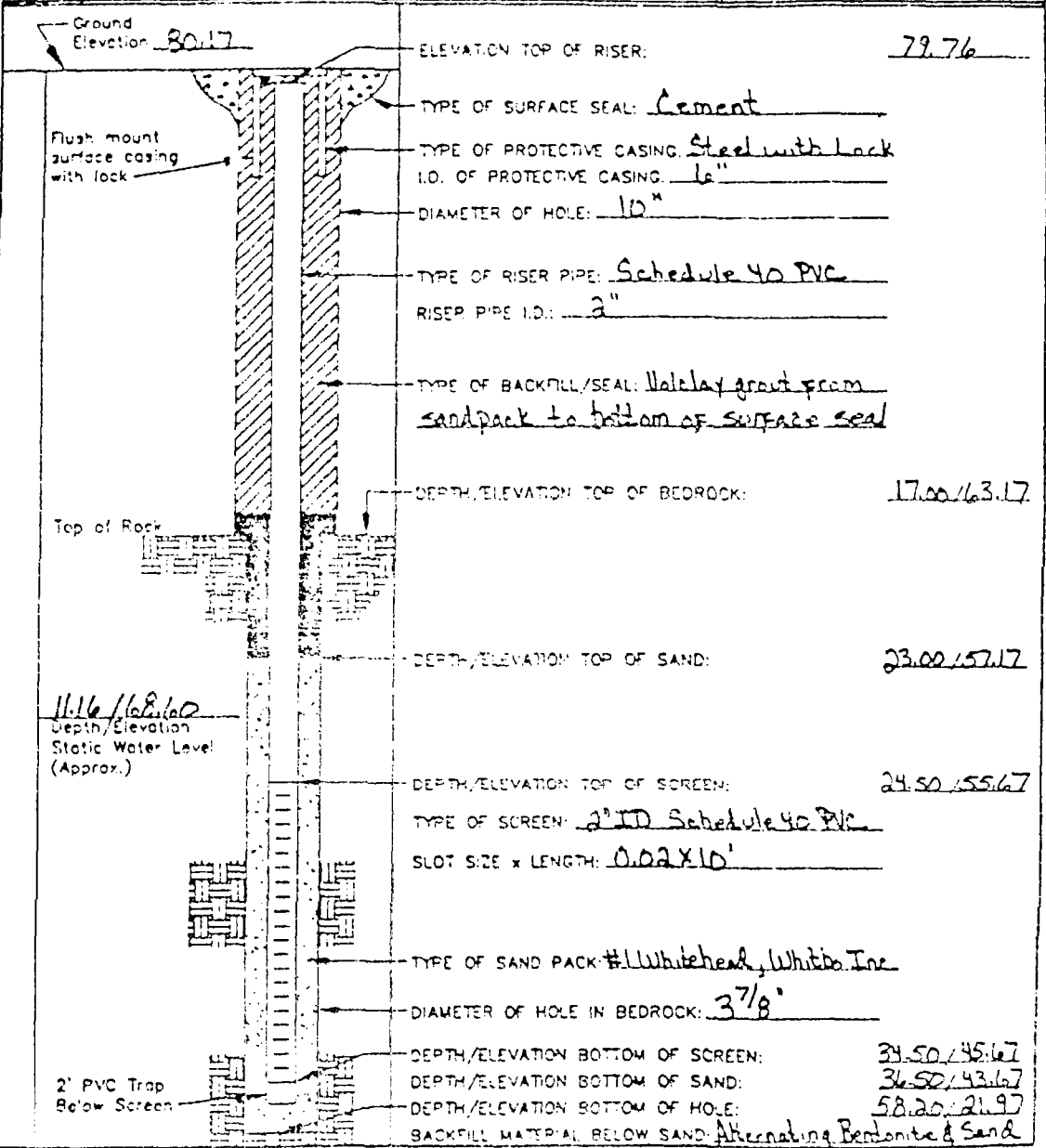




WELL NO.: K19B

BEDROCK  
MONITORING WELL SHEET  
WELL INSTALLED IN BEDROCK

PROJECT: <u>Kellogg-Decree</u>	LOCATION: <u>Norwalk CT</u>	DRILLER: <u>V. Peave / Empire</u>
PROJECT NO.: <u>1670</u>	BORING: <u>K19B</u>	DRILLING METHOD: <u>H.S. Auger / Core &amp; Beam</u>
ELEVATION: <u>80.17 (Ground)</u>	DATE: <u>3/15/88</u>	DEVELOPMENT METHOD: <u>Air Lift</u>
FIELD GEOLOGIST: <u>S. Keall</u>		



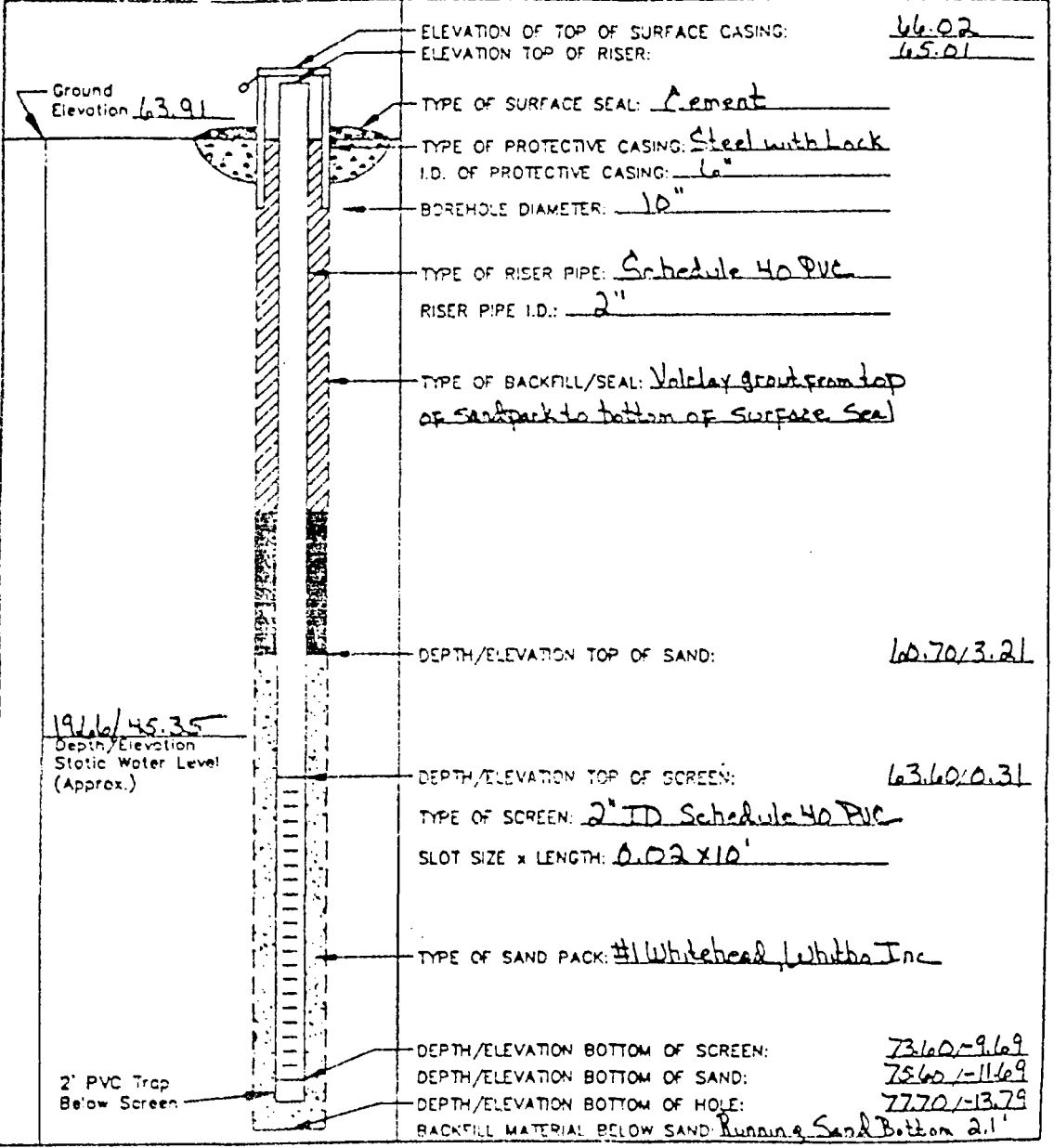
SCALE: 1/4" = 1'-0"



WELL NO.: K20A

## OVERBURDEN MONITORING WELL SHEET

PROJECT: <u>Kellogg-Deering</u>	LOCATION: <u>Norwalk CT</u>	DRILLER: <u>E. Cole / Empire</u>
PROJECT NO.: <u>1670</u>	BORING: <u>K20A</u>	DRILLING METHOD: <u>Hollow Stem Auger</u>
ELEVATION: <u>63.91 (Ground)</u>	DATE: <u>3/29/88</u>	DEVELOPMENT METHOD: <u>Air Lift</u>
FIELD GEOLOGIST: <u>S. Krall</u>		



ELEVATION OF TOP OF SURFACE CASING: 66.02  
 ELEVATION TOP OF RISER: 65.01  
 TYPE OF SURFACE SEAL: Cement  
 TYPE OF PROTECTIVE CASING: Steel with Lock  
 I.D. OF PROTECTIVE CASING: 6"  
 BOREHOLE DIAMETER: 10"  
 TYPE OF RISER PIPE: Schedule 40 PVC  
 RISER PIPE I.D.: 2"  
 TYPE OF BACKFILL/SEAL: Volclay grout from top of sand pack to bottom of surface seal  
 DEPTH/ELEVATION TOP OF SAND: 60.70 / 3.21  
 DEPTH/ELEVATION TOP OF SCREEN: 63.60 / 0.31  
 TYPE OF SCREEN: 2" ID Schedule 40 PVC  
 SLOT SIZE x LENGTH: 0.02 x 10'  
 TYPE OF SAND PACK: #1 Whitehead Whiteba Inc  
 DEPTH/ELEVATION BOTTOM OF SCREEN: 73.60 / -9.69  
 DEPTH/ELEVATION BOTTOM OF SAND: 75.60 / -11.69  
 DEPTH/ELEVATION BOTTOM OF HOLE: 77.70 / -13.79  
 BACKFILL MATERIAL BELOW SAND: Running Sand Bottom 2.1'

19.66 / 45.35  
 Depth/Elevation  
 Static Water Level  
 (Approx.)

2' PVC Trap  
 Below Screen

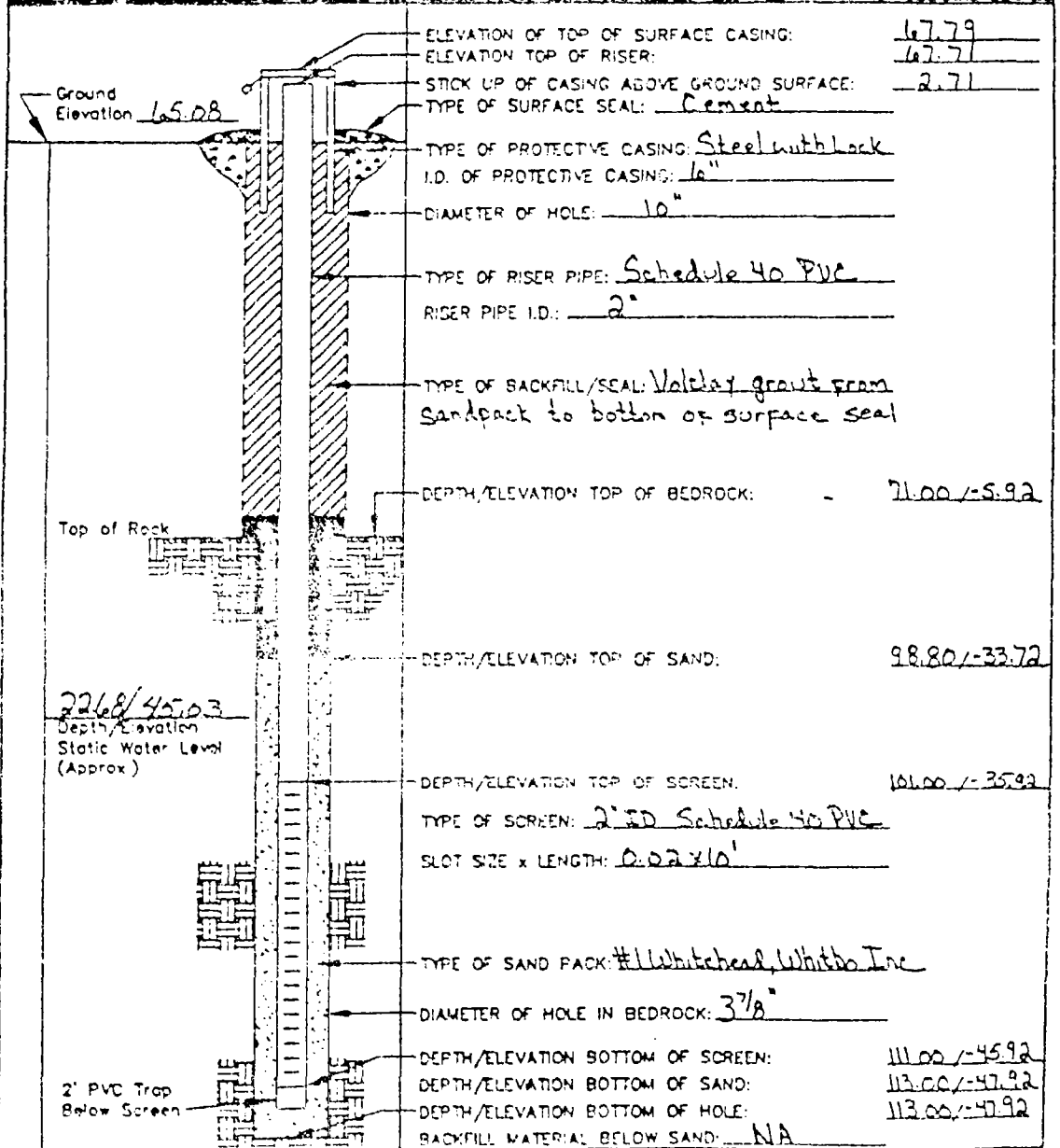
NOTE: SEE DRAWING FOR DETAILS



WELL NO.: K20B

# BEDROCK MONITORING WELL SHEET WELL INSTALLED IN BEDROCK

PROJECT: <u>Bellows-Derwig</u>	LOCATION: <u>Norwalk CT</u>	DRILLER: <u>V. Pous / Empire</u>
PROJECT NO.: <u>L670</u>	BORING: <u>K20B</u>	DRILLING METHOD: <u>H.S. Auger Core &amp; Ream</u>
ELEVATION: <u>65.08</u>	DATE: <u>4/11/88</u>	DEVELOPMENT METHOD: <u>Air Lift</u>
FIELD GEOLOGIST: <u>Don Blackert</u>		



ELEVATION OF TOP OF SURFACE CASING:	<u>67.79</u>
ELEVATION TOP OF RISER:	<u>67.71</u>
STICK UP OF CASING ABOVE GROUND SURFACE:	<u>2.71</u>
TYPE OF SURFACE SEAL:	<u>Cement</u>
TYPE OF PROTECTIVE CASING:	<u>Steel with lock</u>
I.D. OF PROTECTIVE CASING:	<u>6"</u>
DIAMETER OF HOLE:	<u>10"</u>
TYPE OF RISER PIPE:	<u>Schedule 40 PVC</u>
RISER PIPE I.D.:	<u>2"</u>
TYPE OF BACKFILL/SEAL:	<u>Volclay grout from sandpack to bottom of surface seal</u>
DEPTH/ELEVATION TOP OF BEDROCK:	<u>71.00 / -5.92</u>
DEPTH/ELEVATION TOP OF SAND:	<u>98.80 / -33.72</u>
DEPTH/ELEVATION TOP OF SCREEN:	<u>101.00 / -35.92</u>
TYPE OF SCREEN:	<u>2" ID Schedule 40 PVC</u>
SLOT SIZE x LENGTH:	<u>0.02 x 10'</u>
TYPE OF SAND PACK:	<u>#1 Whitehead, Whitbo Inc</u>
DIAMETER OF HOLE IN BEDROCK:	<u>3 7/8"</u>
DEPTH/ELEVATION BOTTOM OF SCREEN:	<u>111.00 / -45.92</u>
DEPTH/ELEVATION BOTTOM OF SAND:	<u>113.00 / -47.92</u>
DEPTH/ELEVATION BOTTOM OF HOLE:	<u>113.00 / -47.92</u>
BACKFILL MATERIAL BELOW SAND:	<u>NA</u>

22.68/45.03  
Depth/Elevation  
Static Water Level  
(Approx)

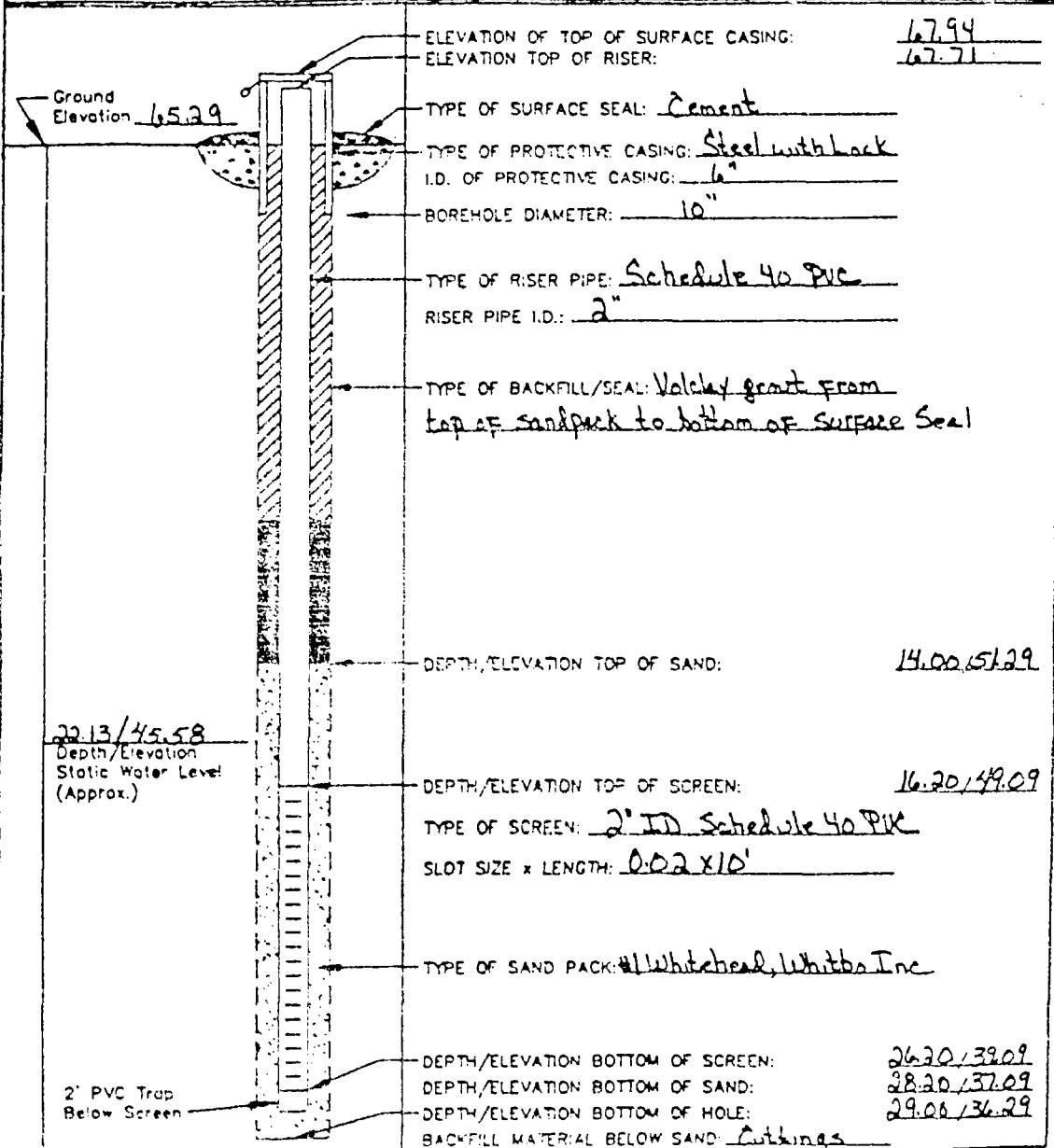
2' PVC Trap  
Below Screen



WELL NO.: K200

## OVERBURDEN MONITORING WELL SHEET

PROJECT: <u>Kellogg-Deering</u>	LOCATION: <u>Norwalk CT</u>	DRILLER: <u>E. Cole/Empire</u>
PROJECT NO.: <u>1670</u>	BORING: <u>K200</u>	DRILLING METHOD: <u>Hollow Stem Auger</u>
ELEVATION: <u>65.29 (Ground)</u>	DATE: <u>4/8/85</u>	DEVELOPMENT METHOD: <u>Air Lift</u>
FIELD GEOLOGIST: <u>S. Krall</u>		



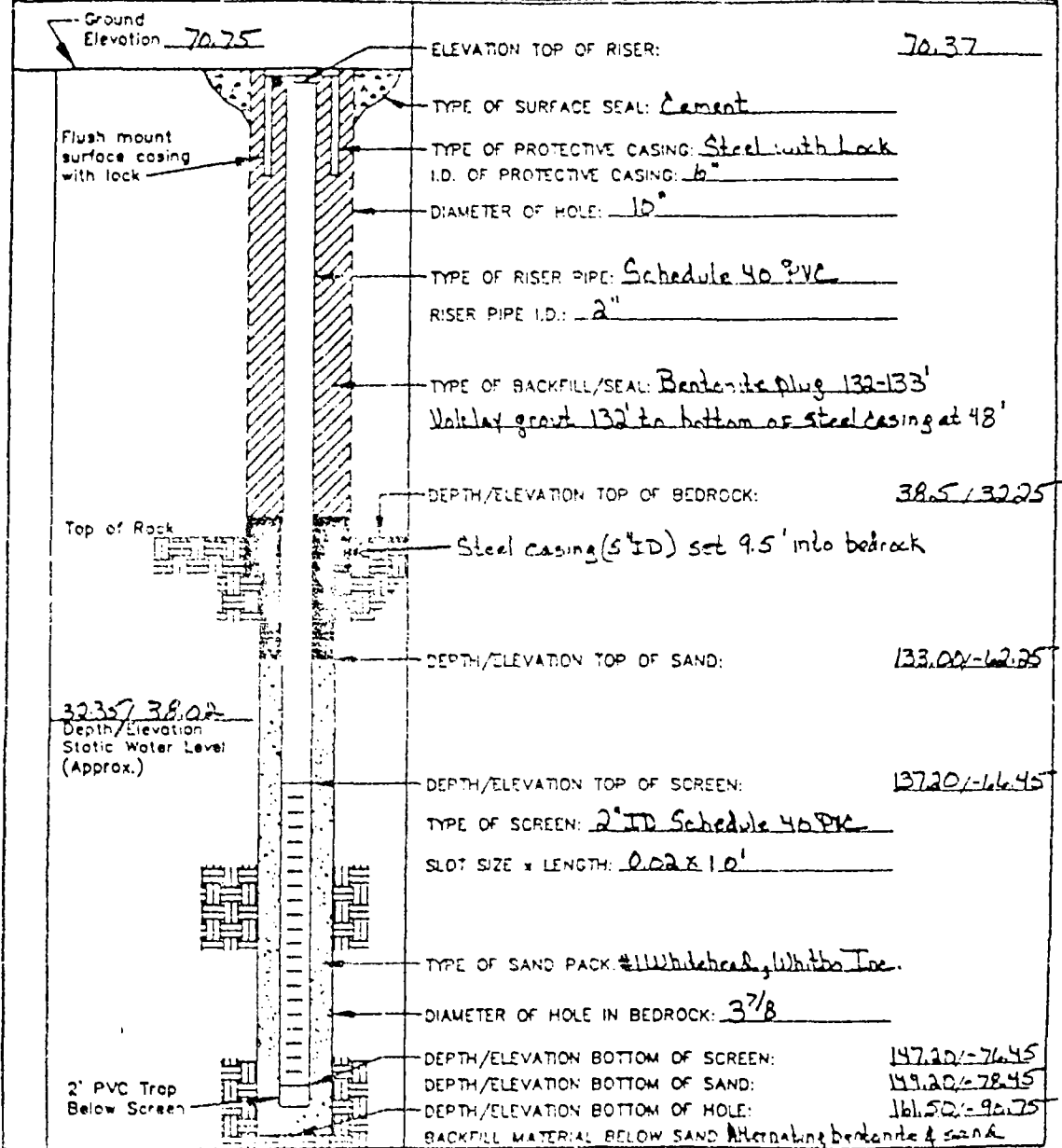
SCALE: 1/4" = 1'-0"



WELL NO.: K21

**BEDROCK  
MONITORING WELL SHEET  
WELL INSTALLED IN BEDROCK**

PROJECT: <u>Kellogg-Deering</u>	LOCATION: <u>Norwalk CT</u>	DRILLER: <u>V. Pave/Empire</u>
PROJECT NO.: <u>1167D</u>	BORING: <u>K21</u>	DRILLING METHOD: <u>H.S. Auger/Cored Beam</u>
ELEVATION: <u>70.75 (Ground)</u>	DATE: <u>4/15/83</u>	DEVELOPMENT METHOD: <u>Pump</u>
FIELD GEOLOGIST: <u>Don Blackett</u>		



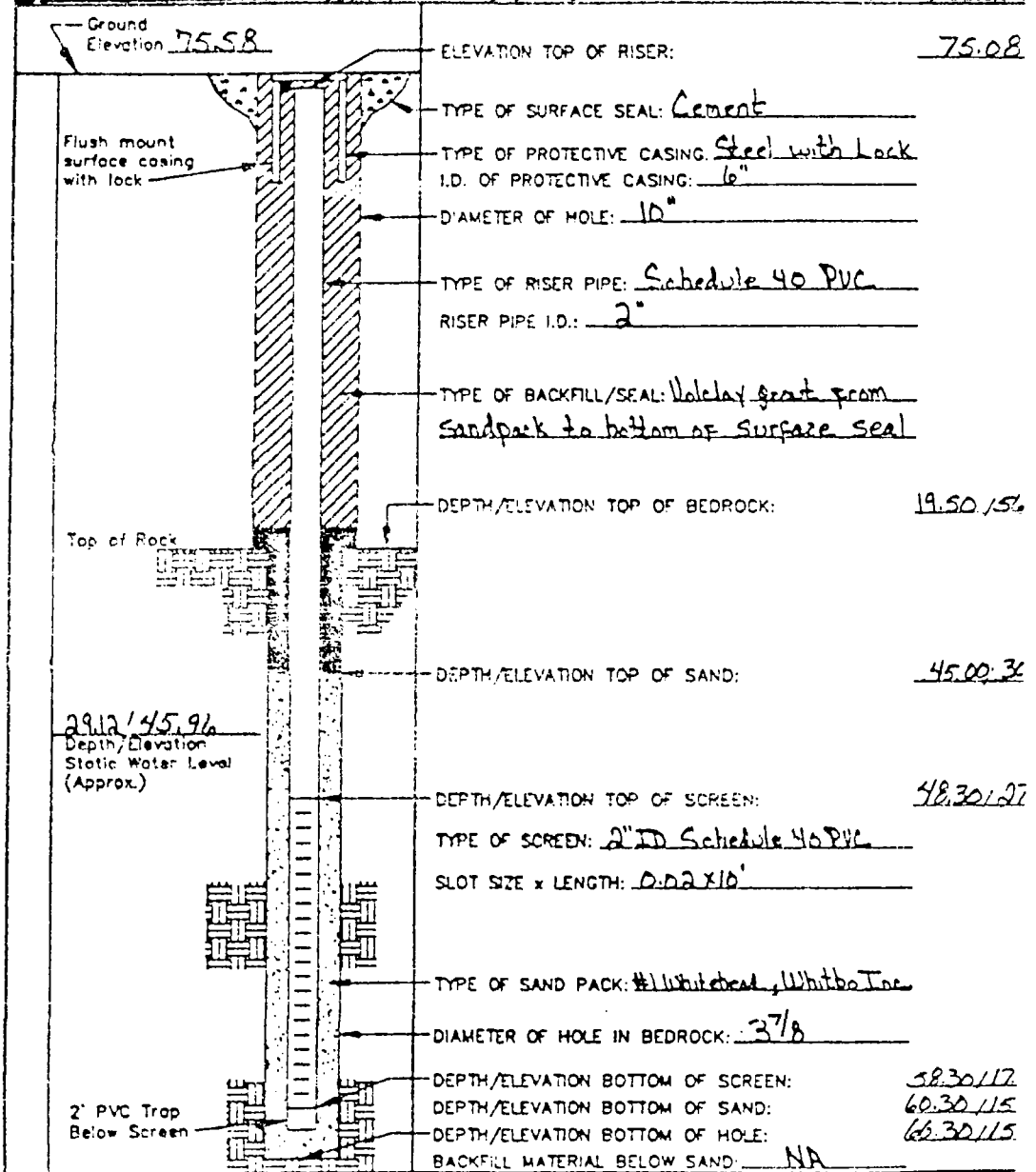
SCALE: 1/4" = 1'-0"



WELL NO.: K22A

**BEDROCK  
MONITORING WELL SHEET  
WELL INSTALLED IN BEDROCK**

PROJECT: <u>Kellogg-Deering</u>	LOCATION: <u>Norwalk CT</u>	DRILLER: <u>E. Cole / Empire</u>
PROJECT NO.: <u>L670</u>	BORING: <u>K22A</u>	DRILLING METHOD: <u>H.S. Auger / Core / F</u>
ELEVATION: <u>75.58 (Ground)</u>	DATE: <u>3/31/88</u>	DEVELOPMENT METHOD: <u>Air Lift</u>
FIELD GEOLOGIST: <u>S. Krall</u>		



SCALE 1/4" = 1'-0"

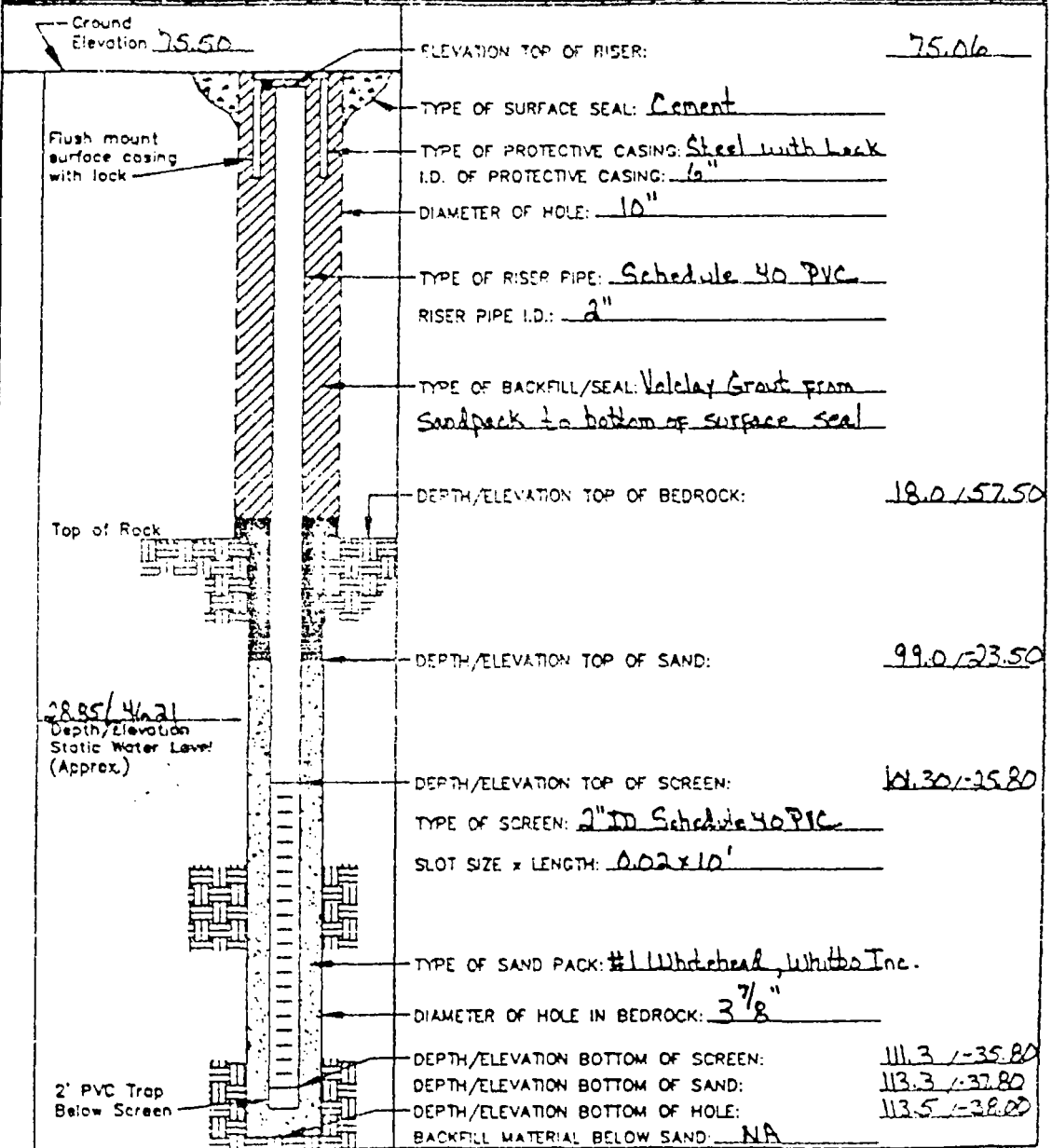




WELL NO.: K22B

**BEDROCK  
MONITORING WELL SHEET  
WELL INSTALLED IN BEDROCK**

PROJECT: <u>Kellogg-Deering</u>	LOCATION: <u>Norwalk, CT</u>	DRILLER: <u>E. G. de Empire</u>
PROJECT NO.: <u>11670</u>	BORING: <u>K22B</u>	DRILLING METHOD: <u>H.S. Auger/Case &amp; Ream</u>
ELEVATION: <u>75.50 (Ground)</u>	DATE: <u>4/7/88</u>	DEVELOPMENT METHOD: <u>Air Lift</u>
FIELD GEOLOGIST: <u>Don Blackett</u>		



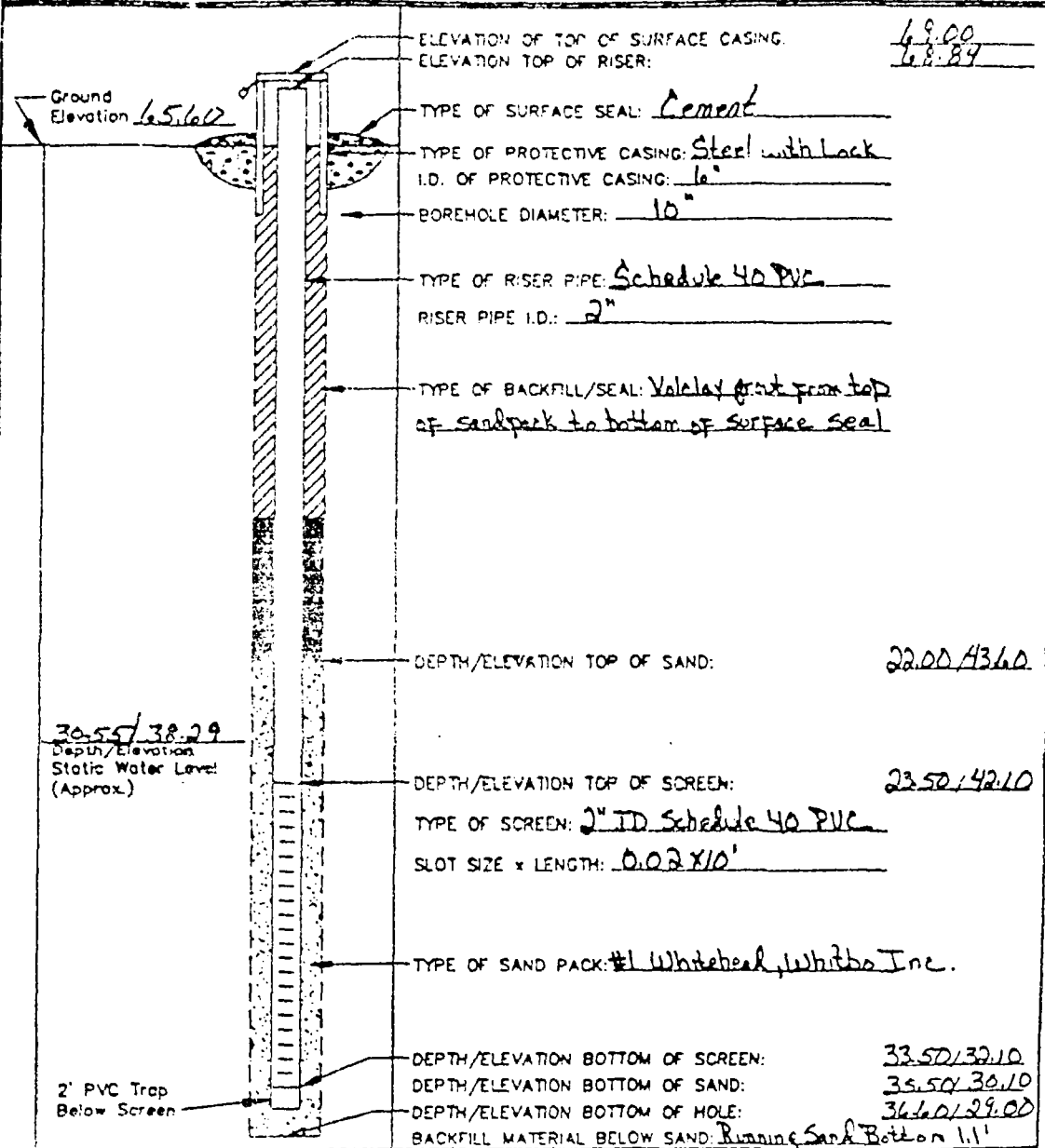
NOTE: USE THIS SCALE FOR ALL DIMENSIONS



WELL NO.: K23A

## OVERBURDEN MONITORING WELL SHEET

PROJECT: <u>Kellogg-Deering</u>	LOCATION: <u>Norwalk CT</u>	DRILLER: <u>V. Puse</u>   <u>Empire</u>
PROJECT NO.: <u>L67D</u>	BORING: <u>K23A</u>	DRILLING METHOD: <u>Hollow Stem Auger</u>
ELEVATION: <u>65.60 (Ground)</u>	DATE: <u>3/17/88</u>	DEVELOPMENT METHOD: <u>Air Lift</u>
FIELD GEOLOGIST: <u>S. Keall</u>		



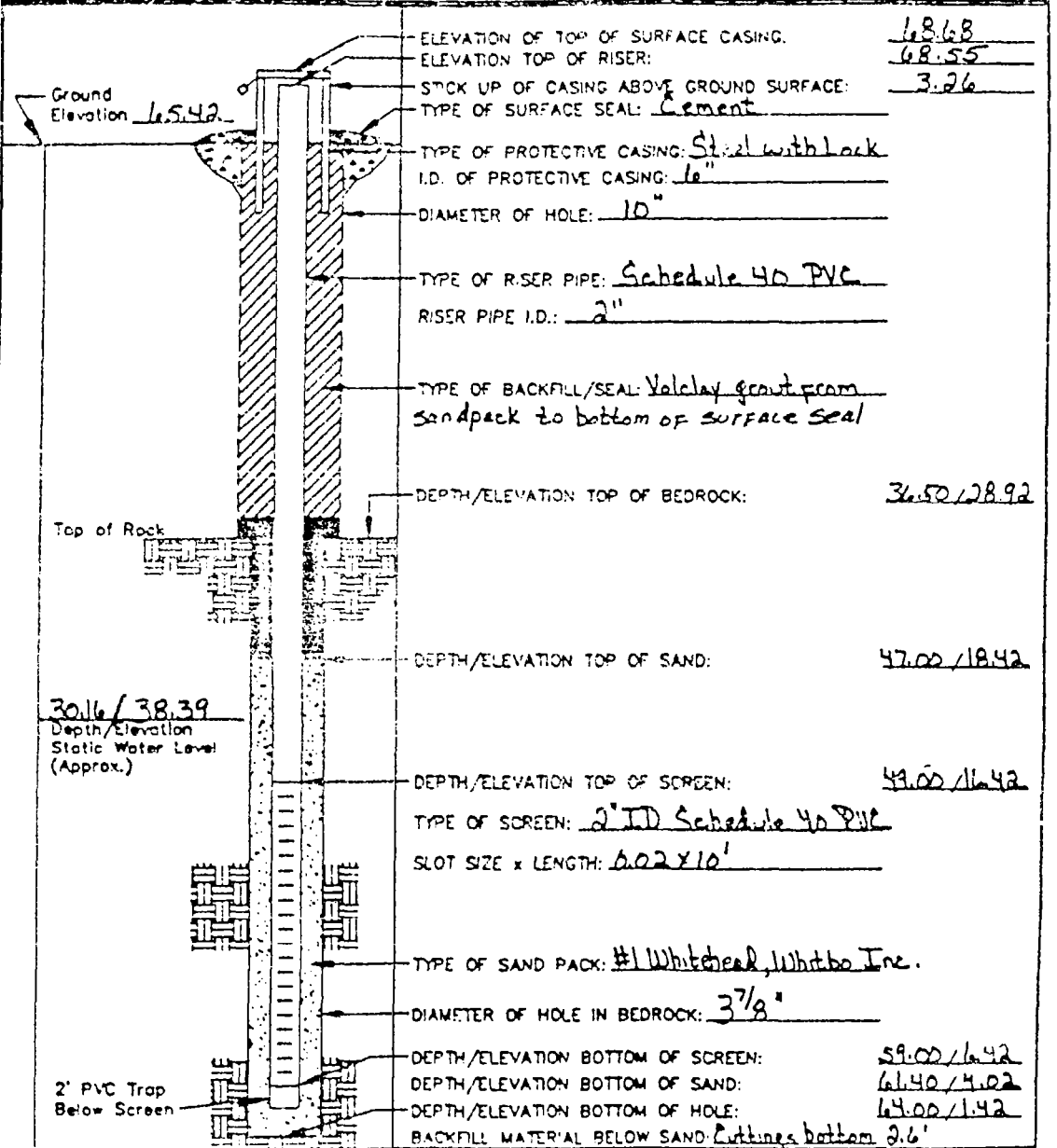
NOTE: USE OF THIS SHEET IS SUBJECT TO THE TERMS AND CONDITIONS OF THE WELL LOGGING CONTRACT.



WELL NO.: K23B

**BEDROCK  
MONITORING WELL SHEET  
WELL INSTALLED IN BEDROCK**

PROJECT: <u>Kellogg-Deering</u>	LOCATION: <u>Normal CT</u>	DRILLER: <u>V. Price / Empire</u>
PROJECT NO.: <u>L670</u>	BORING: <u>K23B</u>	DRILLING METHOD: <u>W.S. Auger/Case &amp; Beam</u>
ELEVATION: <u>65.42</u>	DATE: <u>3/22/88</u>	DEVELOPMENT METHOD: <u>Air Lift</u>
FIELD GEOLOGIST: <u>Rick Bethel</u>		



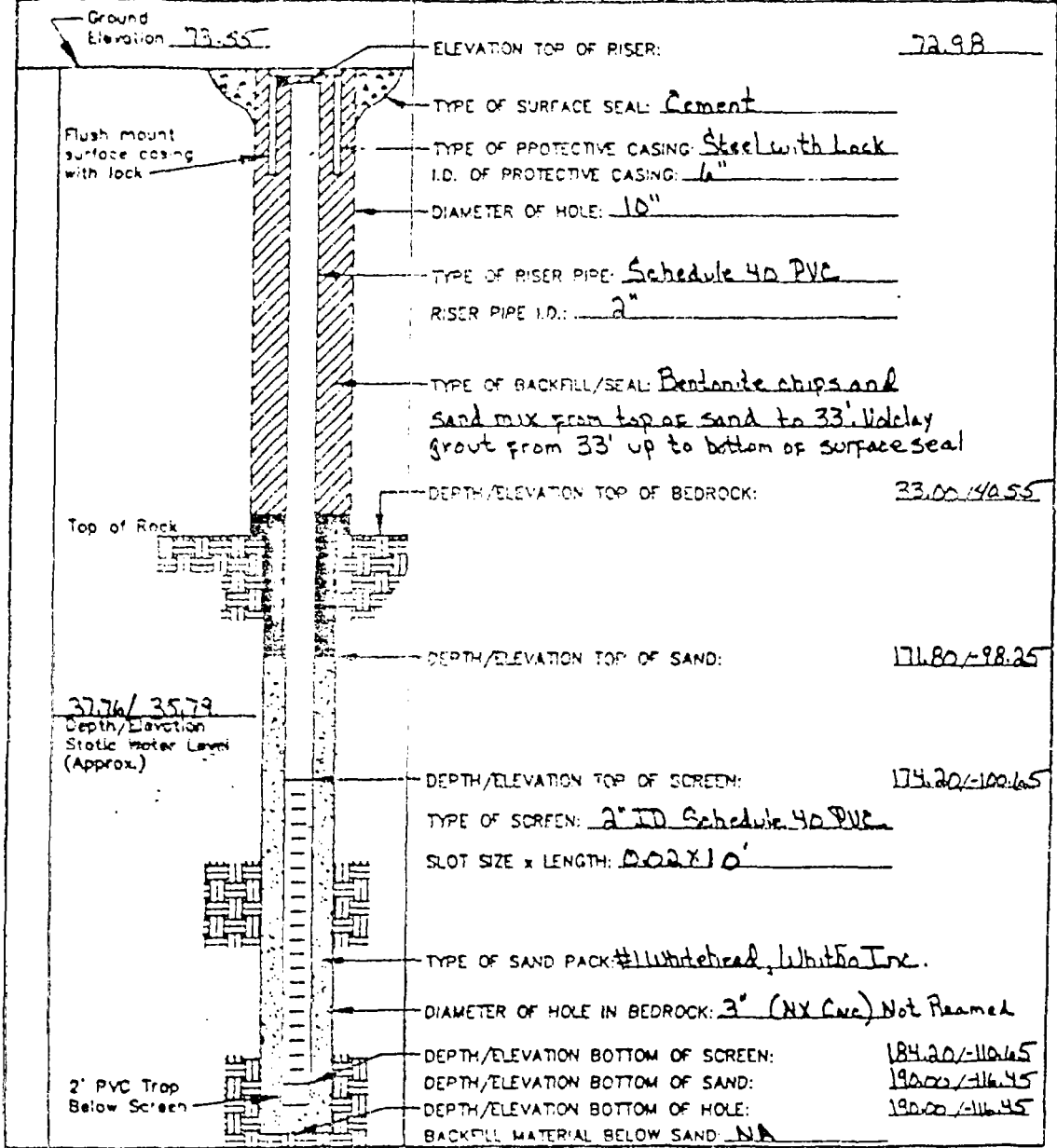
SCALE 1/4" = 1'-0"



WELL NO.: K24

**BEDROCK  
MONITORING WELL SHEET**  
WELL INSTALLED IN BEDROCK

PROJECT: <u>Kellogg-Deering</u>	LOCATION: <u>Norwalk CT</u>	DRILLER: <u>V. Bue/Empire</u>
PROJECT NO.: <u>1670</u>	BORING: <u>K24</u>	DRILLING METHOD: <u>H.S. Auger/Case</u>
ELEVATION: <u>73.55 (Ground)</u>	DATE: <u>4/28/88</u>	DEVELOPMENT METHOD: <u>Air Lift</u>
FIELD GEOLOGIST: <u>Don Blackett</u>		



SCALE: 1/8" = 1'-0" VERTICAL



**FUSS & O'NEILL**  
consulting engineers

210 Main Street, Manchester, Conn. 06040 (203) 646-2469

LETTER OF TRANSMITTAL

Date <u>3/9/88</u>	Project No. <u>86-100</u>
Attention:	
RE:	

TO GREGORY A. DeMASTRO

HYDROGEOLOGIST

DAMES AND MOORE

ONE BLUE HILL PLAZA, SUITE 530

PEARL RIVER, NEW YORK 10965-8668

We are sending you the following items:  Attached  Under separate cover via \_\_\_\_\_

- Shop Drawings     Prints     Plans     Samples     Specifications  
 Copy of Letter     Change Order     Other

Copies	Date	No.	Description
1	6/86		ATTACHMENT II OF JUNE 1986 HYDROGEOLOGIC INVESTIGATION - 272 MAIN AVE, NEWALK, CT.

These are transmitted as checked below:

- For approval     Approved as submitted     Resubmit \_\_\_ copies for approval  
 For your use     Approved as noted     Submit \_\_\_ copies for distribution  
 As requested     Returned for corrections     Return \_\_\_ corrected prints  
 For review and comment  
 For Bids Due \_\_\_\_\_ 19\_\_     Prints returned after loan to us

REMARKS \_\_\_\_\_  
 \_\_\_\_\_  
AS WE DISCUSSED, I HOPE THESE WILL ASSIST YOU  
IN YOUR EVALUATION.  
 \_\_\_\_\_  
 \_\_\_\_\_

COPIES TO KARL BURDEAU, ESQ.

SIGNED: Bob Pattato

ATTACHMENT II  
GEOLOGIC LOGS AND MONITOR WELL COMPLETION REPORTS

# East Coast Drilling, Inc.

APR 16 1984

Bit # \_\_\_\_\_ Fig \_\_\_\_\_

P. O. BOX 961 - WALLINGFORD, CONN. 06492

Bit # \_\_\_\_\_ Fig \_\_\_\_\_

TO HRP Associates

ADDRESS New Britain, Connecticut

PROJECT NAME Elinco

LOCATION Norwalk, Connecticut

PORT SENT TO Client

PROJ. NO. 83-86-10

SAMPLES SENT TO Taken at Site

OUR JOB NO. 83-176A

SHEET 1 of 1  
 DATE 4/9/84  
 HOLE NO. MW-1  
 LINE & STA. \_\_\_\_\_  
 OFFSET \_\_\_\_\_

GROUND WATER OBSERVATIONS		Type Size I.D. Hammer Wt. Hammer Fall	CASING NW 3"	SAMPLER S-S 1 3/8" 140lb. 30"	CORE BAR. BXST	SURFACE ELEV. _____
At <u>6.5'</u> after <u>24</u> Hours	DATE STARTED <u>4/3/84</u>					
At _____ after _____ Hours	DATE COMPL. <u>4/3/84</u>	BORING FOREMAN <u>Quagliaroli JJ</u>				SPECTOR <u>P. Misluk</u>
SOILS ENGR. _____						

## LOCATION OF BORING:

DEPTH	Casing Blows per foot	Sample Depths From - To	Type of Sample	Blows per 6" on Sampler			Moisture Density or Consist.	Strata Change Elev.	SOIL IDENTIFICATION Remarks include color, gradation, Type of soil etc. Rock color, type, condition, hardness, Drilling time, seams and etc.	SAMPLE		
				0-6	6-12	12-18				No	Pen	Rec.
		0.5'-2.0'	D	15	10	9	Moist Medium Dense	.5'	Macadam.	1	1.5'	1.0'
		5.0'	D	50/0				5.5'	Brown fine-medium Sand, some medium-fine Gravel, trace Silt.			
		5.5'-8.5'	C				3		Cored White Gneiss, weathered.	C1	3.0'	.6'
		8.5'-10.5'	C				4		Cored Gray weathered Gneiss.	C2	2.0'	.8'
		10.5'-13.5'	C				4		" "	C3	3.0'	.6'
		13.5'-16.5'	C				6.5		" "	C4	3.0'	3.0'
		16.5'-18.5'	C				4		" "	C5	2.0'	1.5'
		18.5'-22.5'	C				5		" "	C6	4.0'	2.6'
		22.5'-24.5'	C				5		" "	C7	2.0'	1.7'
		24.5'-26.5'	C				5		(reamed in 15.0' of casing)			
							5		" "	C8	2.0'	1.6'
							4					
							6	26.5'	Bottom of Boring 26.5'			
									Installed 2" PVC Rock Well @ 26.5'			
									17.0' Riser Pipe			
									1 Curb Box			

GROUND SURFACE TO 3.5'

USED NW

CASING: THEN Cored ED 26.5'

Sample Type  
 O: Dry C: Cored W: Washed  
 UP: Undisturbed Piston  
 TP: Test Pit A: Auger V: Vane Test  
 UT: Undisturbed Thinwall

Proportions Used  
 trace 0 to 10%  
 fine 10 to 20%  
 some 20 to 35%  
 and 35 to 50%

140lb Wt. x 30" fall on 2" O.D. Sampler  
 Cohesionless Density  
 0-10 Loose  
 10-30 Med. Dense  
 30-50 Dense  
 50+ Very Dense  
 Cohesive Consistency  
 0-4 Soft  
 4-8 M/Stiff  
 8-15 Stiff  
 15-30 V-Stiff

SUMMARY:  
 Earth Boring 3.5'  
 Rock Coring 21.0'  
 Samples \_\_\_\_\_

HOLE NO MW-1

MONITOR WELL COMPLETION REPORT

GENERAL INFORMATION

Town: NORWALK

Site: ELINCO

Monitoring Point I.D. No.: MW-1

Date of completion: 4/4/84

DEP/WPC I.D. No:

Monitoring Point Location (relative to site features): UPGRADEMENT  
SEE SITE MAP

Drilling Contractor: EAST COAST DRILLING INC. Supervising Engineer/Geologist: P. MISLUK

Well Construction Method: AUGERED AND CORED

WELL INFORMATION (ELEVATIONS TO NEAREST 0.1 FEET)

Ground surface elevation (MSL):

Well depth below ground surface: 26.5'

Refusal: \_\_\_ Yes \_\_\_ No

Top of casing elevation (MSL):

Screened interval:

Length of Screen:

Length of riser pipe: 17.0'

Screen type:

Screen Slot size:

Filter fabric: \_\_\_ Yes \_\_\_ No

Screen packing: \_\_\_ Yes \_\_\_ No

If yes, Thickness:

Well inside diameter: 2"

Material:

grain size:

Impermeable Backfill: Bentonite

Well casing material and schedule: 40 PVC Estimated K screened interval:

Method of well development: PUMP

Time spent developing: 1 HR

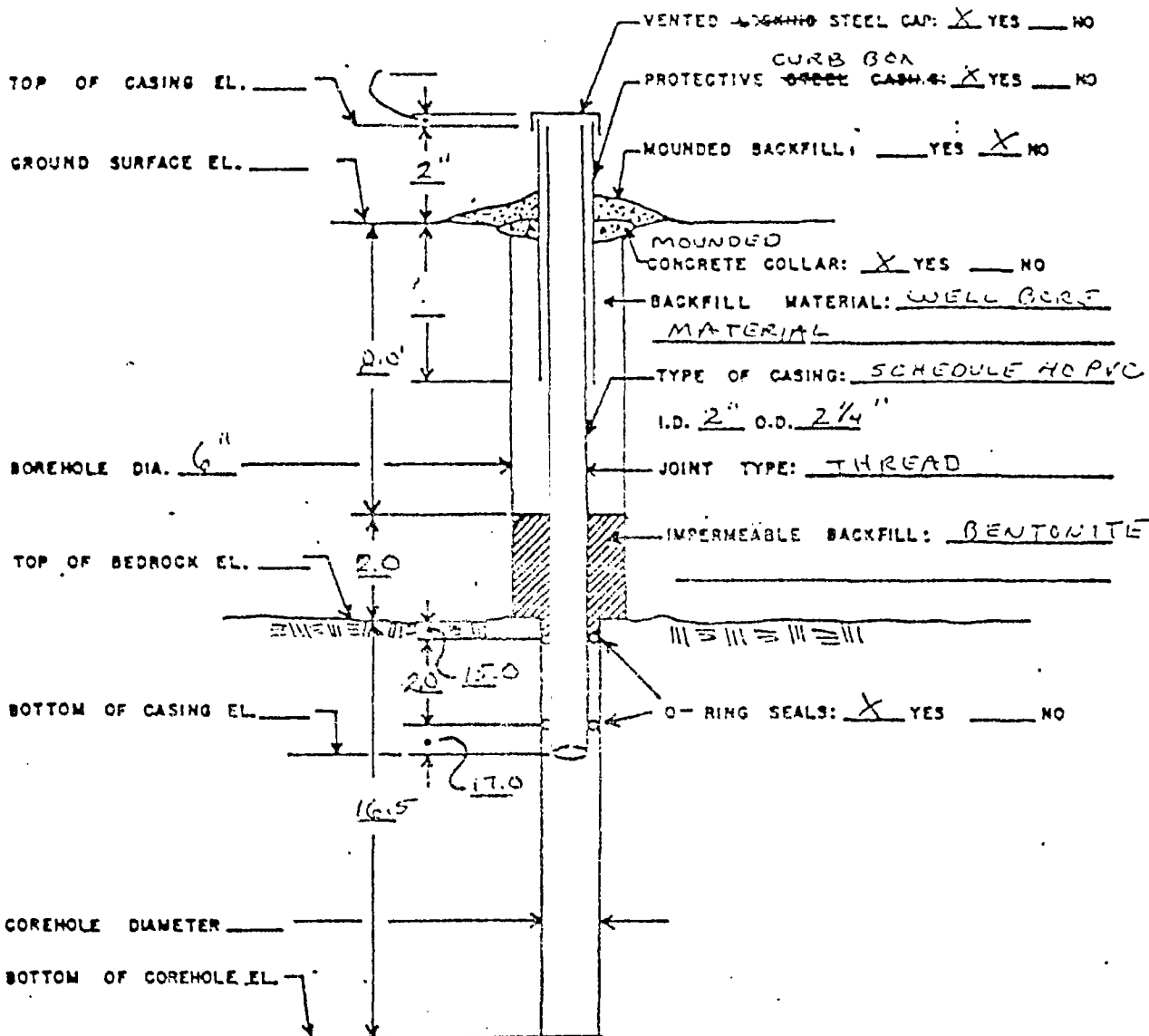
Locking \_\_\_ or threaded cap X  
Curb Box

Impermeable backfill: CONCRETE COLLAR



# MONITOR WELL INSTALLATION DETAIL FOR WELL IN BEDROCK

MW-1



Bedrock wells

Casing length: 17.0'

Water-bearing rock unit: FRACTURED FELSIC GNEISS

Water bearing sections (depths and approximate yields): 10' - 26.5'

Length of rock core: 16.5'

Diameter of core hole: 2 3/8"

Thickness and depth of impermeable backfill: 2' 8" - 10' 2' 1" - 3'

O-rings seals:  Yes  No

GEOLOGIC INFORMATION

Aquifer:

Inferred relationship to plume:  Within  Outside  Edge

Watershed (plume discharge watercourse):

Aquifer materials (attach boring log):

Attach maps and plans required of G.1.j. and G.4.

# East Coast Drilling, Inc.

P. O. BOX 961 - WALLINGFORD, CONN. 06492

Bit # \_\_\_\_\_ Fig \_\_\_\_\_

TO HRP Associates

ADDRESS New Britain, Connecticut

Bit # \_\_\_\_\_ Fig \_\_\_\_\_

PROJECT NAME Elinco

LOCATION Norwalk, Connecticut

PORT SENT TO Client

PROJ. NO. 83-86-10

PLES SENT TO Taken at Site

OUR JOB NO. 83-176A

SHEET 1 OF 1  
 DATE 4/9/84  
 HOLE NO MW-2  
 LINE & STA \_\_\_\_\_  
 OFFSET \_\_\_\_\_

GROUND WATER OBSERVATIONS		CASING	SAMPLER	CORE BAR.	SURFACE ELEV.
At _____	after _____ Hours	Type	HSA	S-S	DATE STARTED <u>4/4/84</u>
At _____	after _____ Hours	Size I.D.	<u>3 1/2"</u>	<u>1 3/8"</u>	DATE COMPL. <u>4/4/84</u>
		Hammer Wt.	<u>140 lb</u>	BIT	BORING FOREMAN <u>Quagliaroli II</u>
		Hammer Fall	<u>30"</u>		INSPECTOR <u>P. Misluk</u>
					SOILS ENGR. _____

## LOCATION OF BORING:

DEPTH	Casing Blows per foot	Sample Depths From - To	Type of Sample	Blows per 6" on Sampler			Moisture Density or Consist.	Strata Change Elev.	SOIL IDENTIFICATION Remarks include color, gradation, Type of soil etc. Rock-color, type, condition, hardness, Drilling time, seams and etc.	SAMPLE		
				From 0-6"	6-12"	12-18"				No	Pen	Rec.
		0.5'-2.0'	D	10	4	6	Moist Loose	.5'	Macadam.	1	1.5'	1.3'
									Light Brown fine Sand, little Silt.			
		5.0'-6.5'	D	10	28	37	Moist Very Dense	5.0'	Multi-color fine-coarse Sand and fine-medium Gravel.	2	1.5'	1.3'
									Gray Brown fine-coarse Sand, some fine-coarse Gravel, trace Silt, Cobbles.			
		10.0'-10.5'	D	6	15		Moist V/Dense	11.0'	Grayish White Gneiss, (from 14.0'-14.5' seams).	3	.5'	.5'
		11.0'-16.0'	C				7 5 8 7 6 5 4 4 3 5		Gray Gneiss with fractured joints.	C1	5.0'	4.6'
		16.0'-21.0'	C							C2	5.0'	5.0'
								21.0'	Roller Bit to 23.0'			
								23.0'	Bottom of Boring 23.0'			
									Installed 2" PVC Monitor Well @ 22.5'			
									12.5' Riser Pipe			
									10.0' Screen			
									1 Curb Box			

GROUND SURFACE TO 11.0' USED HSA CASING: THEN Roller Bit to 23.0'

Sample Type  
 D: Dry C: Cored W: Washed  
 UP: Undisturbed Piston  
 TP: Test Pit A: Auger V: Vane Test  
 UT: Undisturbed Thinwall

Proportions Used  
 trace Cl to 10%  
 white 10 to 20%  
 some 20 to 35%  
 and 35 to 50%

140lb Wt. x 30" fall on 2" O.D. Sampler  
 Cohesionless Density Cohesive Consistency  
 0-10 Loose 0-4 Soft 30 + Hard  
 10-30 Med. Dense 4-8 M/Stiff  
 30-50 Dense 8-15 Stiff  
 50+ Very Dense 15-30 V-Stiff

SUMMARY  
 Earth Boring 13.0'  
 Rock Coring 10.0'  
 Samples 3

HOLE NO MW-2

MONITOR WELL COMPLETION REPORT

GENERAL INFORMATION

Town: NORWALK Site: ELINCO  
Monitoring Point I.D. No.: MW-2 Date of completion: 4/4/84  
DEP/WPC I.D. No:  
Monitoring Point Location (relative to site features): DOWNGRADIENT  
SEE SITE MAP  
Drilling Contractor: EAST COAST DRILLING, INC. Supervising Engineer/Geologist:  
Well Construction Method: AUGERED

WELL INFORMATION (ELEVATIONS TO NEAREST 0.1 FEET)

Ground surface elevation (MSL): Well depth below ground surface: 22.5'  
Refusal:  Yes  No  
Top of casing elevation (MSL): Screened interval: 12.5 - 22.5'  
Length of Screen: 10.0'  
Length of riser pipe: 12.5'  
Screen type: PVC Screen Slot size: .010"  
Filter fabric:  Yes  No Screen packing:  Yes  No  
If yes, Thickness: 10.0'  
Well inside diameter: 2" Material: OTTAWA SAND  
grain size: #12 SAND  
Impermeable Backfill: Bentonite  
Well casing material and schedule: 40 PVC Estimated X screened interval:  
Method of well development: PUMP Time spent developing: 1 HR  
Locking  or threaded cap  Impermeable backfill: CONCRETE COLLAR  
Curb Box

Bedrock wells

Casing length: 22.5'  
+ SCREEN

Water-bearing rock unit: FRACTURED FELSIC GNEISS

Water bearing sections (depths and approximate yields): 13.0' - 22.5'

Length of rock core: 12.0'

Diameter of core hole: 2 1/2"

Thickness and depth of impermeable backfill: 2' 10.5 - 12.5' 2' 1 - 3'

O-rings seals: \_\_\_ Yes X No

GEOLOGIC INFORMATION

Aquifer:

Inferred relationship to plume: \_\_\_ Within \_\_\_ Outside \_\_\_ Edge

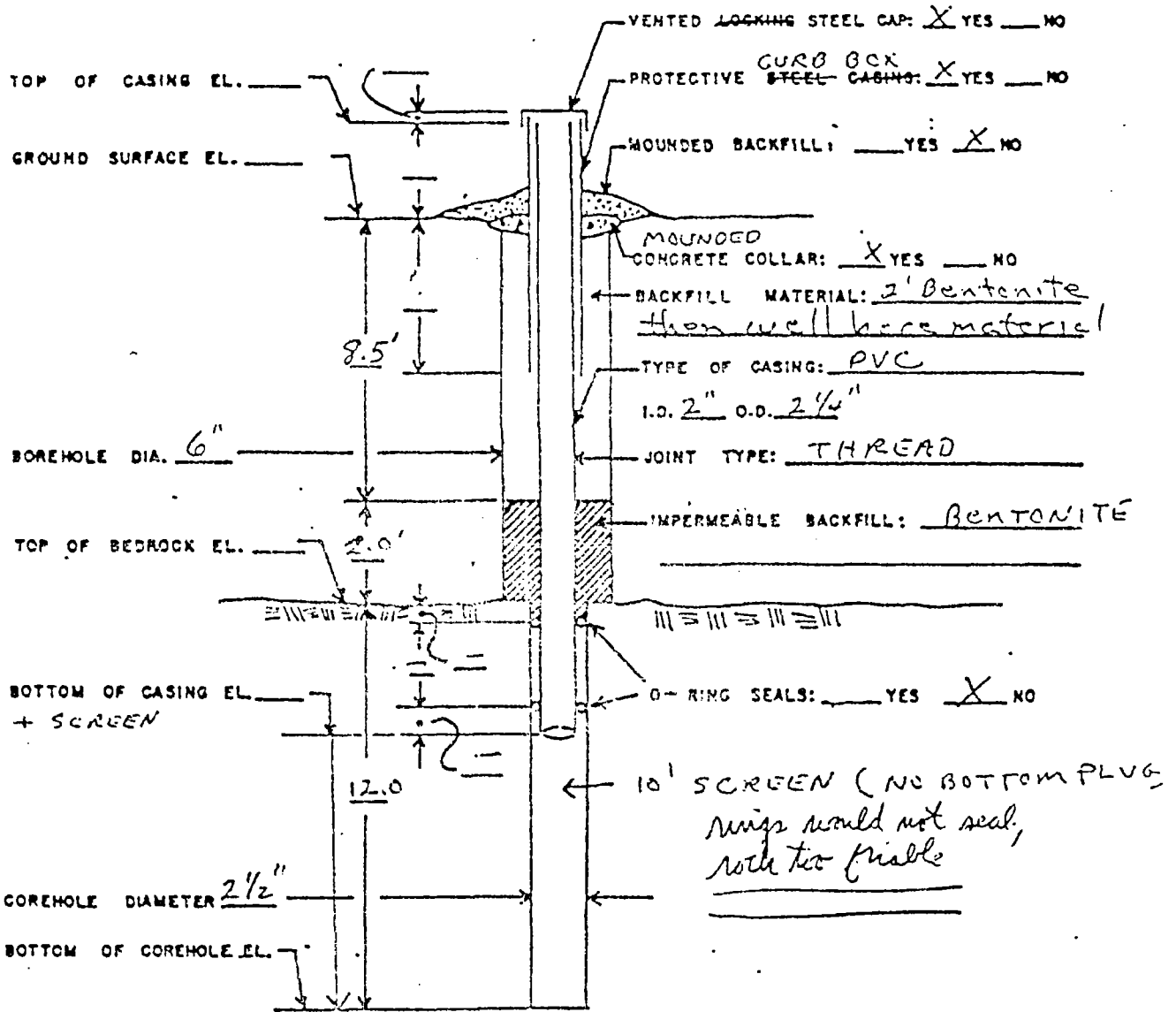
Watershed (plume discharge watercourse):

Aquifer materials (attach boring log):

Attach maps and plans required of G.1.j. and G.4.

# MONITOR WELL INSTALLATION DETAIL FOR WELL IN BEDROCK

mw-2



# East Coast Drilling, Inc.

P. O. BOX 961 - WALLINGFORD, CONN. 06492  
 TO HRP Associates  
 PROJECT NAME Elinco  
 CLIENT SENT TO Client  
 LES SENT TO Taken at Site

Bit # \_\_\_\_\_ Fig \_\_\_\_\_  
 ADDRESS New Britain, Connecticut  
 LOCATION Norwalk, Connecticut  
 PROJ. NO. 83-86-10  
 OUR JOB NO. 83-176A

SHEET 1 OF 1  
 DATE 4/9/84  
 HOLE NO. MW-3  
 LINE & STA. \_\_\_\_\_  
 OFFSET \_\_\_\_\_

GROUND WATER OBSERVATIONS		CASING	SAMPLER	CORE BAR.	SURFACE ELEV.
At <u>19.0'</u>	after <u>0</u> Hours	Type <u>HSA</u>	<u>S-S</u>		<u>4/4/84</u>
At _____	after _____ Hours	Size I.D. <u>3 1/2"</u>	<u>1 3/8"</u>		<u>4/5/84</u>
		Hammer Wt. _____	<u>140lb</u>	<u>BIT</u>	BORING FOREMAN <u>Quagliaroli II</u>
		Hammer Fall _____	<u>30"</u>		INSPECTOR <u>P. Misluk</u>
					SOILS ENGR. _____

## LOCATION OF BORING:

DEPTH	Casing Blows per foot	Sample Depths From - To	Type of Sample	Blows per 6" on Sampler			Moisture Density or Consist.	Strata Change Elev.	SOIL IDENTIFICATION Remarks include color, gradation, Type of soil etc. Rock-color, type, condition, hardness, Drilling time, seams and etc.	SAMPLE		
				From 0-6	6-12	12-18				No	Pen	Rec.
		<u>0.5'-2.0'</u>	<u>D</u>	<u>6</u>	<u>5</u>	<u>6</u>	<u>Moist</u>	<u>.5'</u>	<u>Macadam.</u>	<u>1</u>	<u>1.5'</u>	<u>.5'</u>
							<u>Mddium Dense</u>		<u>Brown fine-medium Sand, little fine Gravel, little Silt.</u>			
		<u>5.0'-6.5'</u>	<u>D</u>	<u>18</u>	<u>58</u>	<u>51</u>	<u>Moist Very Dense</u>		<u>Brown fine-coarse Sand, some fine-coarse Gravel, trace Silt.</u>	<u>2</u>	<u>1.5'</u>	<u>1.0'</u>
		<u>10.0'-11.5'</u>	<u>D</u>	<u>12</u>	<u>31</u>	<u>27</u>	<u>Moist Very Dense</u>		<u>Brown fine-medium Sand, little fine-medium Gravel.</u>	<u>3</u>	<u>1.5'</u>	<u>1.2'</u>
		<u>15.0'-16.5'</u>	<u>D</u>	<u>12</u>	<u>42</u>	<u>85</u>	<u>Moist Very Dense</u>		<u>" "</u>	<u>4</u>	<u>1.5'</u>	<u>1.5'</u>
		<u>18.0'-23.0'</u>	<u>C</u>					<u>18.0'</u>	<u>Gray Gneiss. (Reamed Casing to 20.0')</u>	<u>C1</u>	<u>5.0'</u>	<u>4.7'</u>
		<u>23.0'-28.0'</u>	<u>C</u>						<u>" "</u>	<u>C2</u>	<u>5.0'</u>	<u>4.7'</u>
								<u>28.0'</u>	<u>Bottom of Boring 28.0'</u> <u>Installed 2" PVC Rock Well @ 28.0'</u> <u>20.8' Riser Pipe</u> <u>1 Curb Box</u>			

GROUND SURFACE TO <u>18.0'</u>	USED <u>HSA</u>	CASING: <u>THEN Cored to 28.0'</u>	SUMMARY
Sample Type D: Dry C: Cored W: Washed UP: Undisturbed Piston TP: Test Pit A: Auger V: Vane Test UT: Undisturbed Thinwall	Proportions Used trace 0 to 10% fine 10 to 20% some 20 to 35% and 35 to 50%	140lb Wt. x 30" fall on 2" O.D. Sampler Cohesionless Density 0-10 Loose 10-30 Med. Dense 30-50 Dense 50+ Very Dense	Earth Boring <u>18.0'</u> Rock Coring <u>10.0'</u> Samples <u>4</u> HOLE NO <u>MW-3</u>

MONITOR WELL COMPLETION REPORT

GENERAL INFORMATION

Town: NORWALK

Site: ELINCO

Monitoring Point I.D. No.: MW-3

Date of completion: 4/6/84

DEP/WPC I.D. No:

Monitoring Point Location (relative to site features): DOWNGRADIENT  
(see site map)

Drilling Contractor: EAST COAST  
DRILLING, INC.

Supervising Engineer/Geologist: P. MISLUR

Well Construction Method:  
AUGERED AND CORED

WELL INFORMATION (ELEVATIONS TO NEAREST 0.1 FEET)

Ground surface elevation (MSL):

Well depth below ground surface: 28.0'

Refusal: \_\_\_ Yes \_\_\_ No

Top of casing elevation (MSL):

Screened interval: 18' - 22'

Length of Screen:

Length of riser pipe: 20.8'

Screen type: PVC

Screen Slot size: .010"

Filter fabric: \_\_\_ Yes  No

Screen packing:  Yes \_\_\_ No

If yes, Thickness: 10.0'

Well inside diameter: 2"

Material: Ottawa sand

grain size: # 12 sand

Impermeable Backfill: BENTONITE

Well casing material and schedule: 40 PVC

Estimated K screened interval:

Method of well development: PUMP

Time spent developing: 1 hr.

Locking \_\_\_ or threaded cap   
urb Bsp

Impermeable backfill: CONCRETE  
COLLAR



Bedrock wells

Casing length: 20.8'

Water-bearing rock unit: FRACTURED FELSIC GNEISS

Water bearing sections (depths and approximate yields): 10' - 28'

Length of rock core: 10'

Diameter of core hole: 2 3/8"

Thickness and depth of impermeable backfill: 2', 16' to 18' and 2', 1' to 3'

O-rings seals:  Yes  No

GEOLOGIC INFORMATION

Aquifer:

Inferred relationship to plume:  Within  Outside  Edge

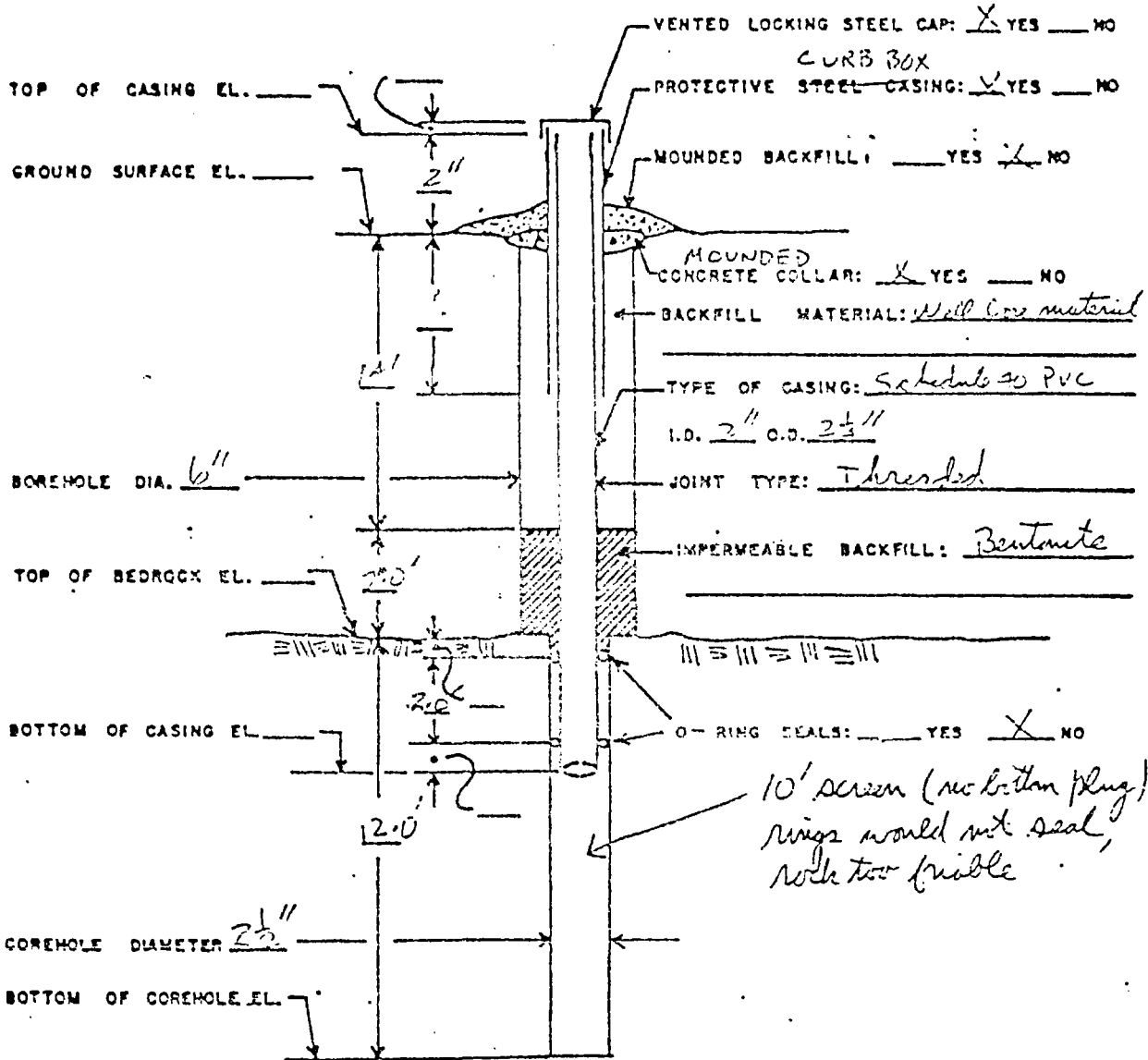
Watershed (plume discharge watercourse):

Aquifer materials (attach boring log):

Attach maps and plans required of G.1.j. and G.4.

# MONITOR WELL INSTALLATION DETAIL FOR WELL IN BEDROCK

mw-3



**Glenn Drilling Inc.**

R.F.D. #1, Lake Road  
 Fitchville, CT 06334  
 (203) 887-3621

CLIENT Diaco & O'Neill  
 PROJECT NAME KESLOFF / NORWALK  
 LOCATION Norwalk, Ct.

BORING  
 NUMBER  
MW-100

SHEET  
 No. 1  
 of 2

DRILLER Michael Deane  
 INSPECTOR Bob Patterton  
 DATE START Sept. 9, 1986  
 DATE FINISH Sept. 11, 1986

ARCHITECT  
 ENGINEER

FILE NO. Q26-8540

	Casing	Semier	Core Barrel
TYPE	<u>NW</u>	<u>SS</u>	<u>NX</u>
SIZE ID	<u>1"</u>	<u>1 1/4"</u>	
HAMMER WT	<u>300#</u>	<u>140#</u>	
HAMMER FALL	<u>22"</u>	<u>30"</u>	

SURFACE ELEV. \_\_\_\_\_

LINE & STATION \_\_\_\_\_

OFFSET \_\_\_\_\_

DEPTH	SAMPLE				COL. A	STRATA CHANGE	FIELD CLASSIFICATION AND REMARKS	
	NO.	DEPTH RANGE	BLOWS PER 6" ON SAMPLER					R.F.C.
			0-6	6-12				
5'	1	5.0'-6.5'	16	36	100	1.1	Brown fine-medium sand. Cobbles Fine-coarse Gravel.	
10'	2	10.0'-10.9'	100	100	13	1.7	Boulders. Brown fine-coarse Sand and Gravel.	
20'	3	20.0'-22.0'	76	67	56	2	Brown fine-coarse Sand. Medium- coarse Gravel. Cobbles and Boulders	
25'	4	25.0'-27.0'	38	49	56	.5		
30'	5	30.0'-32.0'	55	71	61	.5	Brown fine-medium Sand. Fine-coarse Gravel. Cobbles	
35'	6	35.0'-37.0'	32	46	42	.5	Brown fine-medium Sand. Fine-medium gravel.	
	R-1	38.0'-43.0'				1.3	38.0' Cored Marble like Rock.	

SAMPLE IDENTIFICATION	PENETRATION RESISTANCE		PROPORTIONS USED	REMARKS:
	140 lb. Wt. falling 30" on 2" O.D. Sampler			
S — SPLIT SPOON	Cohesiveness Density		trace 0 to 10%	COL. A <u>Coring Time</u>
T — THIN WALL TUBE	0-2	Very Loose	0-2 Very Soft	
U — UNDISTURBED PISTON	3-9	Loose	3-4 Soft	
O — OPEN END ROD	10-29	Med. Dense	5-8 M/Stiff	
W — WASH SAMPLE	30-49	Dense	9-15 Stiff	
A — AUGER SAMPLE	50 -	Very Dense	16-30 V-Stiff	31 -



MONITOR WELL COMPLETION REPORT

GENERAL INFORMATION

Town: NORWALK

BARDANISE BUILDINGS  
Site: 276 MAIN AVE.

Monitoring Point I.D. No.: MW-100

Date of completion: 9/11/85

DEP/WPC I.D. No: 103-092

Monitoring Point Location (relative to site features): WEST OF BUILDING #1  
BETWEEN BUILDING & RTE 7

Drilling Contractor: GLENN DRILLING, INC. Supervising Engineer/Geologist: R.S. POTTERTON

Well Construction Method: HOLLOW STEM AUGERS / 3-INCH DRIVEN CASING

WELL INFORMATION (ELEVATIONS TO NEAREST 0.1 FEET)

Ground surface elevation (MSL):

81'

Well depth below ground surface: 47.7 FEET

Refusal: \_\_\_ Yes X No

Top of casing elevation (MSL): 81'

Screened interval:

42.7' - 47.7' BELOW GRADE

Length of Screen: 5 FEET

Length of riser pipe: 42 FEET

Screen type: SLOTTED PVC

Screen Slot size: 10 SLOT

Filter fabric: \_\_\_ Yes X No

Screen packing: \_\_\_ Yes X No

If yes, Thickness:

Well inside diameter: 2-INCHES

Material:

grain size:

Impermeable Backfill: BENTONITE PELLETS  
CEMENT / BENTONITE GROUT

Well casing material and schedule:  
SCHEDULE 40 PVC

Estimated K screened interval: N/A

Method of well development: BAILING

Time spent developing: 15 MINS.

Locking \_\_\_ or threaded cap \_\_\_  
CURB BOX W/ PENTAGONAL BOLT

Impermeable backfill: CEMENT

Bedrock wells

Casing length: 42 FEET

Water-bearing rock unit: G.1(55)

Water bearing sections (depths and approximate yields): NA

Length of rock core: 12 FEET

Diameter of core hole: 2 3/8 - INCHES

Thickness and depth of impermeable backfill:

O-rings/seals:  Yes  NoGEOLOGIC INFORMATION

Aquifer: BEDROCK (G.1(55))

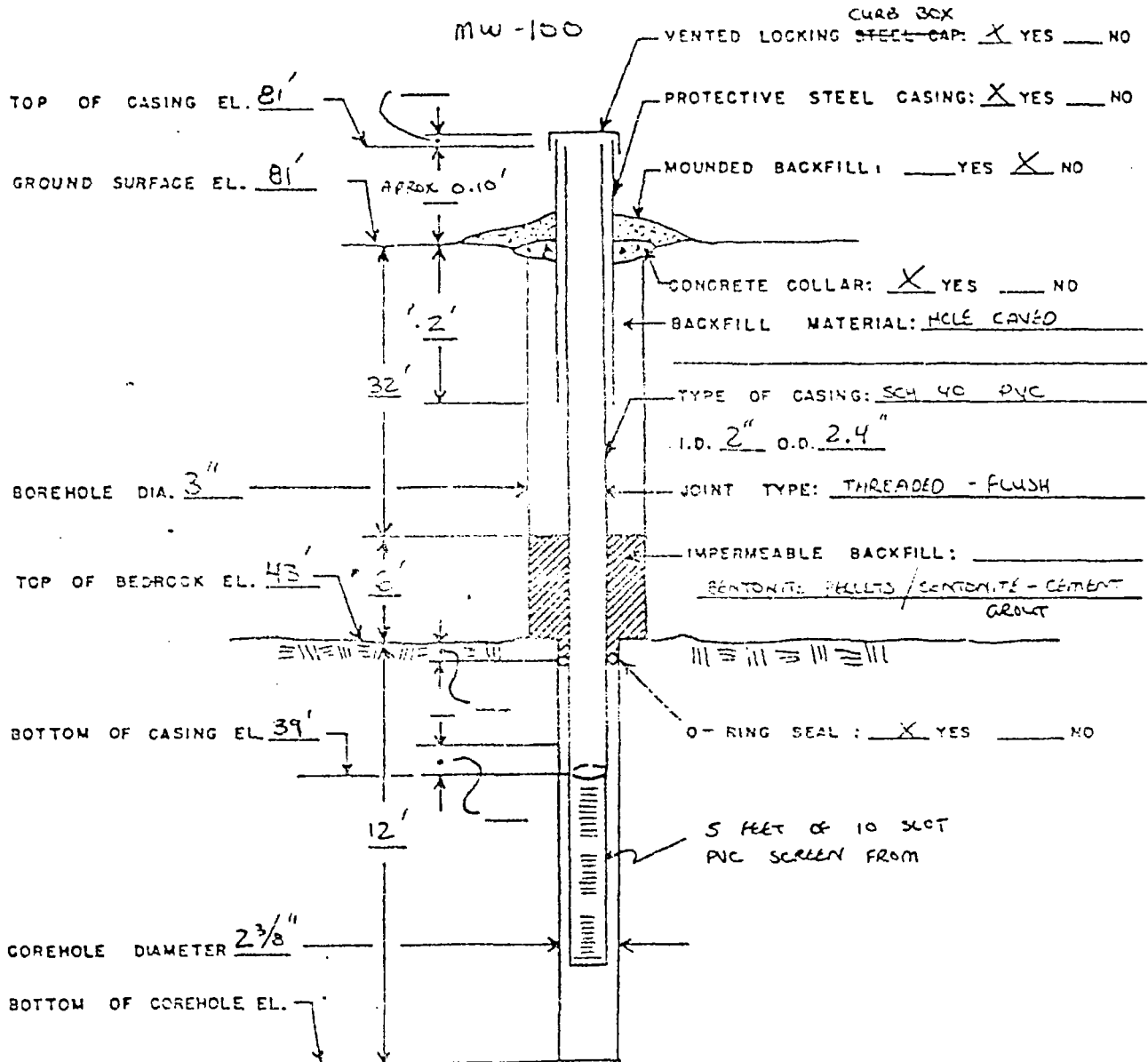
Inferred relationship to plume:  Within  Outside  Edge UNKNOWN

Watershed (plume discharge watercourse): NORWALK RIVER

Aquifer materials (attach boring log): (SEE LOG)  
BEDROCK

Attach maps and plans required of G.1.j. and G.4.

# MONITOR WELL INSTALLATION DETAIL FOR WELL IN BEDROCK







MONITOR WELL COMPLETION REPORTGENERAL INFORMATION

Town: NORWALK  
 Monitoring Point I.D. No.: MW-101  
 DEP/WPC I.D. No: 103-092  
 Monitoring Point Location (relative to site features):  
 Drilling Contractor: GLENN DRILLING, INC. Supervising Engineer/Geologist: R.S. PATTISON  
 Well Construction Method: Hollow Stem Augers - 3-INCH DRIVEN CASING

BARDANISE BUILDINGS  
 Site: 276 MAIN AVE.

Date of completion: 9/13/05

WELL INFORMATION (ELEVATIONS TO NEAREST 0.1 FEET)

Ground surface elevation (NSL):

80 FEET

Well depth below ground surface: 35.4'

Refusal:  Yes  No

Top of casing elevation (NSL): 80 FEET

Screened interval:

30.4' - 35.4' BELOW GRADE

Length of Screen: 5 FEET

Length of riser pipe: 35 FEET

Screen type: SLOTTED PVC

Screen slot size: 10 SLOT

Filter fabric:  Yes  No

Screen packing:  Yes  No

If yes, Thickness: 1/2 - INCH

Well inside diameter: 2 - INCHES

Material: QUARTZ SAND

grain size: MEDIUM

Impermeable Backfill: BENTONITE PELLETS  
 CEMENT/BENTONITE GROUT

Well casing material and schedule:  
 SCHEDULE 40 PVC

Estimated K screened interval: NA

Method of well development: BAILING

Time spent developing: 15 MINS.

Locking  or threaded cap   
 CURB BOX W/PENTAGONAL BOLT

Impermeable backfill: CEMENT

Bedrock wells

Casing length:

Water-bearing rock unit:

Water bearing sections (depths and approximate yields):

Length of rock core:

Diameter of core hole:

Thickness and depth of impermeable backfill:

O-rings seals:  Yes  NoGEOLOGIC INFORMATION

Aquifer: STRATIFIED CRIST DEPOSITS

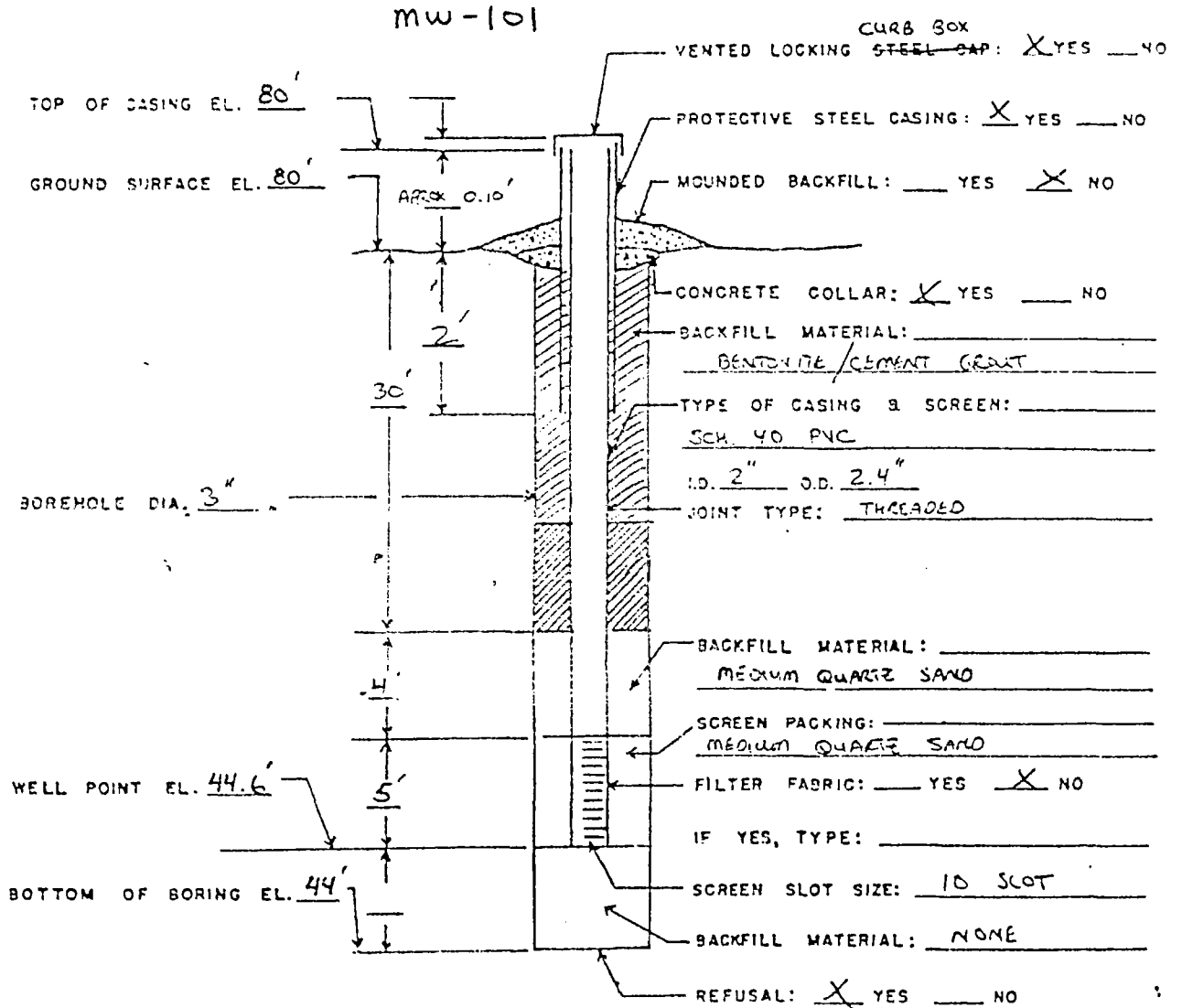
Inferred relationship to plume:  Within  Outside  Edge UNKNOWN

Watershed (plume discharge watercourse): NORWALK RIVER

Aquifer materials (attach boring log): SAND &amp; GRAVEL

Attach maps and plans required of G.1.j. and G.4.

# MONITOR WELL INSTALLATION DETAIL FOR WELL IN UNCONSOLIDATED DEPOSIT



**Glenn Drilling Inc.**

R.F.D. #1, Lake Road  
Fitchville, CT 06334  
(203) 887-3621

CLIENT Fine & O'Neill

PROJECT NAME KESLOFF/NORWALK

LOCATION Norwalk, Ct.

BORING NUMBER  
MW-102

SHEET  
No. 1  
of 1

DRILLER Michael Deane

ARCHITECT  
ENGINEER

FILE NO. 926-8540

INSPECTOR Bob Potterton

Casing HW Sampler SS Core Bitrol NY

SURFACE ELEV. \_\_\_\_\_

DATE START Sept. 16, 1985

SIZE I.D. 1" 1 1/2" 2"

LINE & STATION \_\_\_\_\_

DATE FINISH Sept. 17, 1985

HAMMER WT 300# 170#

OFFSET \_\_\_\_\_

HAMMER FALL 24" 30"

DEPTH	SAMPLE					COL. A	STRATA CHANGE	FIELD CLASSIFICATION AND REMARKS	
	NO.	DEPTH RANGE	BLOWS PER 6" ON SAMPLER						REC.
			0-5	6-12	12-18				
5'	1	5.0'-6.0'	76	103		.9	5.0'	Brown fine-coarse Sand. Weathered Rock. Auger to 6.0'. Moved Hole and Drove Casing.	
	R-1	5.0'-10.0'				1.1			
10'						4	19.0'	Cored Rock @ 6'	
	R-2	10.0'-15.0'				1.5			
20'						6	19.0'	End of Boring: 19.0'	
	R-3	15.0'-19.0'				1.3			

Installed 2" PVC Monitor Well  
Bottom set at 15.5'  
5.0' Screen  
10.0 Riser  
1 Protective Casing and Plug

SAMPLE IDENTIFICATION	PENETRATION RESISTANCE 140 lb. Wt. falling 30" on 2" O.D. Sampler				PROPORTIONS USED	REMARKS:
	Conelessness Density		Coneless Consistency			
— SPLIT SPOON	3-4	Very Loose	0-2	Very Soft	trace	0 to 10%
— THIN WALL TUBE	5-9	Loose	3-4	Soft	little	10 to 20%
— UNDISTURBED PISTON	10-29	Med. Dense	5-8	M/Stiff	some	20 to 35%
— OPEN END ROD	30-49	Dense	9-15	Stiff	and	35 to 50%
— WASH SAMPLE	50+	Very Dense	16-30	V-Stiff		
— AUGER SAMPLE			31+	Hard		

COL. A Coring Time

MONITOR WELL COMPLETION REPORTGENERAL INFORMATION

Town: NORWALK

BARDANISE BUILDINGS  
Site: 276 MAIN AVE.

Monitoring Point I.D. No.: mw-102

Date of completion: 0/17/85

DEP/WPC I.D. No: 103 - 092

Monitoring Point Location (relative to site features):  
114 FEET EAST OF BUILDING #2

Drilling Contracto : GLENN DRILLING, INC. Supervising Engineer/Geologist: R.S. PETERSON

Well Construction Method: HOLLOW STEM AUGERS - AND 4-INCH DRIVEN CASING

WELL INFORMATION (ELEVATIONS TO NEAREST 0.1 FEET)

Ground surface elevation (MSL):

101 FEET

Well depth below ground surface: 15.5'

Refused: \_\_\_ Yes X No

Top of casing elevation (MSL): 101 FEET

Screened interval:

Length of Screen: 5 FEET

10.5 - 15.5 FEET BELOW GRADE

Length of riser pipe: 10 FEET

Screen type: SLOTTED PVC

Screen Slot size: 10 SLOT

Filter fabric: \_\_\_ Yes X NoScreen packing: \_\_\_ Yes X No

If yes, Thickness:

Well inside diameter: 2-INCHES

Material:

grain size:

Impermeable Backfill: BENTONITE PELLETS  
CEMENT/BENTONITE GROUTWell casing material and schedule:  
SCHEDULE 40 PVC

Estimated K screened interval: NA

Method of well development: BAILING

Time spent developing: 15 MINS.

Locking \_\_\_ or threaded cap \_\_\_

Impermeable backfill: CEMENT

CURB BOX w/PENTAGONAL BOLT

Bedrock wells

Casing length: 10 FEET

Water-bearing rock unit: GNEISS

Water bearing sections (depths and approximate yields): NA

Length of rock core: 13 FEET

Diameter of core hole: 2 3/8"

Thickness and depth of impermeable backfill: 0.8" THICK FROM 9 FEET BELOW  
GRADE TO GRADE.O-rings seals:  Yes  NoGEOLOGIC INFORMATION

Aquifer: BEDROCK (GNEISS)

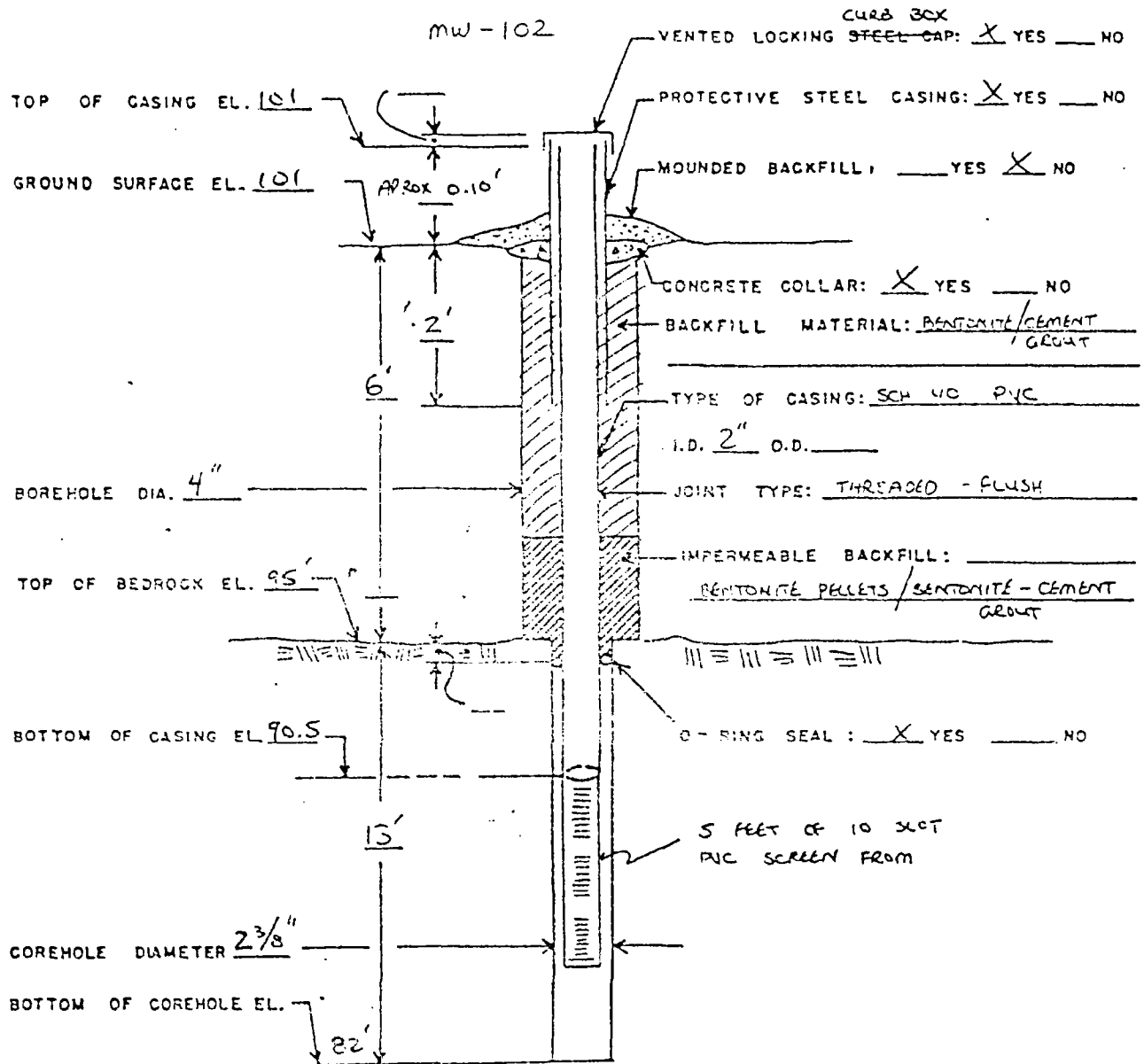
Inferred relationship to plume:  Within  Outside  Edge UNKNOWN

Watershed (plume discharge watercourse): NORWALK RIVER

Aquifer materials (attach boring log): BEDROCK

Attach maps and plans required of G.1.j. and G.4.

# MONITOR WELL INSTALLATION DETAIL FOR WELL IN BEDROCK



<b>Glenn Drilling Inc.</b> R.F.D. #1, Lake Road Fitchville, CT 06334 (203) 887-3621	CLIENT <u>Fuss &amp; O'Neill</u> PROJECT NAME <u>KOSLOFF / NORWALK</u> LOCATION <u>Norwalk, Ct.</u>	BORING NUMBER <b>MW-103</b> SHEET No. <u>1</u> of <u>1</u>
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DRILLER <u>Michael Deane</u> INSPECTOR <u>Bob Petterton</u> DATE START <u>Sept. 17, 1985</u> DATE FINISH <u>Sept. 18, 1985</u>	ARCHITECT ENGINEER TYPE <u>HW</u> SIZE I.D. <u>4"</u> HAMMER WT <u>300#</u> HAMMER FALL <u>24"</u>	Casing <u>HW</u> Sampler <u>SS</u> Core Barrel <u>NY</u> TYPE <u>HW</u> SIZE I.D. <u>4"</u> HAMMER WT <u>300#</u> HAMMER FALL <u>24"</u>	FILE NO. <u>026-9510</u> SURFACE ELEV. _____ LINE & STATION _____ OFFSET _____
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DEPTH	SAMPLE					COL. A	STRATA CHANGE	FIELD CLASSIFICATION AND REMARKS	
	NO.	DEPTH RANGE	BLOWS PER 6" ON SAMPLER						REC.
			0-6	6-12	12-18				
5'	1	5.0'-7.0'	6	8	16	1.2		Brown fine sand.	
			37						
10'	R-1	10.0'-15.0'				2.5	2	10.0'	Cored Granite, Quartz.
							5		
							8		
							13		
							5		
	R-2	15.0'-20.0'				1.2	1		
							6		
							7		
							11		
							7	20.0'	End of Boring: 20.0'
									Installed 2" PVC Monitor Well Bottom set at 15.8' 5.0' Screen 10.8' Riser 1 Protective Casing with Plug

<b>SAMPLE IDENTIFICATION</b> S — SPLIT SPOON T — THIN WALL TUBE U — UNDISTURBED PISTON O — OPEN END ROD W — WASH SAMPLE A — AUGER SAMPLE	<b>PENETRATION RESISTANCE</b> 140 lb. Wt. falling 30" on 2" O.D. Sampler Consistency Density 0-4 Very Loose 5-9 Loose 10-29 Med. Dense 30-49 Dense 50+ Very Dense	Consistency 0-2 Very Soft 3-4 Soft 5-8 M/Stiff 9-15 Stiff 16-30 V-Stiff 31+ Hard	<b>PROPORTIONS USED</b> trace 0 to 10% little 10 to 20% some 20 to 35% and 35 to 50%	<b>REMARKS:</b> COL. A <u>Coring Time</u>
--	--	--	--	--



MONITOR WELL COMPLETION REPORTGENERAL INFORMATION

Town: NORWALK

BARDANOE BUILDINGS  
Site: 276 MAIN AVE.

Monitoring Point I.D. No.: MW-103

Date of completion: 9/18/05

DEP/WPC I.D. No: 103-092

Monitoring Point Location (relative to site features): 17' SOUTH OF BUILDING 2

Drilling Contractor: GLENN DRILLING, INC. Supervising Engineer/Geologist: R.S. PETERSON

Well Construction Method: HOLLOW STEM AUGERS AND 4-INCH DRIVEN CASING

WELL INFORMATION (ELEVATIONS TO NEAREST 0.1 FEET)

Ground surface elevation (MSL):

83'

Well depth below ground surface:

Refusal: \_\_\_ Yes X No

Top of casing elevation (MSL): 83'

Screened interval:

Length of Screen: 5 FEET

10.8 - 15.8 FEET BELOW GRADE

Length of riser pipe: 10.8 FEET

Screen type: SLOTTED PVC

Screen Slot size: 10 SLOT

Filter fabric: \_\_\_ Yes X NoScreen packing: \_\_\_ Yes X No

If yes, Thickness:

Well inside diameter: 2 INCHES

Material:

grain size:

Impermeable Backfill: BENTONITE PELLETS  
CEMENT/BENTONITE GROUTWell casing material and schedule:  
SCHEDULE 40 PVC

Estimated K screened interval:

Method of well development: BAILING

Time spent developing: 15 MINS.

Locking \_\_\_ or threaded cap \_\_\_  
CURB BOX W/PENTAGONAL BOLT

Impermeable backfill: CEMENT

Bedrock wells

Casing length: 10.8 FEET

Water-bearing rock unit: GNEISS

Water bearing sections (depths and approximate yields): NA

Length of rock core: 10 FEET

Diameter of core hole: 2 3/8 - INCHES

Thickness and depth of impermeable backfill: 10.5' THICK (FROM 10.5 FEET TO GRADE)

O-rings/seals:  Yes  NoGEOLOGIC INFORMATION

Aquifer: BEDROCK (GNEISS)

Inferred relationship to plume:  Within  Outside  Edge UNKNOWN

Watershed (plume discharge watercourse): HAZELWALK RIVER

Aquifer materials (attach boring log): BEDROCK

Attach maps and plans required of G.1.j. and G.4.



**Glenn Drilling Inc.**

R.F.D. #1, Lake Road  
Fitchville, CT 06334  
(203) 887-3621

CLIENT Fuss & O'Neill  
PROJECT NAME KOSLOFF/NORWALK  
LOCATION Norwalk, Ct.

BORING NUMBER  
MW-102  
SHEET  
No. 1  
of 1

DRILLER Michael Deane  
INSPECTOR Bob Potterton  
DATE START Sept. 17, 1985  
DATE FINISH Sept. 18, 1985

ARCHITECT ENGINEER  
Casing RW Sampler SS Core Barrel NX  
TYPE RW SIZE I.D. 4" 1 1/2" 2"  
HAMMER WT 300# 140#  
HAMMER FALL 24" 30"

FILE NO. 926-8540  
SURFACE ELEV. \_\_\_\_\_  
LINE & STATION \_\_\_\_\_  
OFFSET \_\_\_\_\_

DEPTH	SAMPLE						COL. A	STRATA CHANGE	FIELD CLASSIFICATION AND REMARKS
	NO.	DEPTH RANGE	BLOWS PER 6" ON SAMPLER			REC.			
			0-6	6-12	12-18				
5'	1	5.0'-5.6'	65	100	1	6	6.0'	Brown fine-coarse Sand. Fine-medium gravel. Cobbles and Boulders.	
	R-1	6.0'-11.0'				1.7			
10'						11	15.7'	Cored Rock.	
						15			
						13			
	R-2	11.0'-13.0'				8			
						21			
	R-3	13.0'-15.7'			1.0	17			
						18			
						19			
								End of Boring: 15.7'	
								Installed 2" PVC Monitor Well Bottom set at 14.2' 5.0' Screen 9.8' Riser 1 Protective Casing and Plug.	

SAMPLE IDENTIFICATION	PENETRATION RESISTANCE	PROPORTIONS USED	REMARKS:
<ul style="list-style-type: none"> <li><u>  </u> SPLIT SPOON</li> <li><u>  </u> THIN WALL TUBE</li> <li><u>  </u> UNDISTURBED PISTON</li> <li><u>  </u> OPEN END ROD</li> <li><u>  </u> WASH SAMPLE</li> <li><u>  </u> AUGER SAMPLE</li> </ul>	140 lb Wt falling 30" on 2" O.D. Sampler Consistency Density      Cohesive Consistency 0-4    Very Loose      0-2    Very Soft 5-9    Loose                3-4    Soft 10-29   Med. Dense        5-15   M/Stiff 30-49   Dense                16-30   Stiff 50+    Very Dense        31+    Hard	trace 0 to 10% little 10 to 20% some 20 to 35% and 35 to 60%	COL. A <u>Coring Time</u>

MONITOR WELL COMPLETION REPORTGENERAL INFORMATION

Town: NORWALK

BARDANISE BUILDINGS  
Site: 276 MAIN AVE.

Monitoring Point I.D. No.: MW-104

Date of completion: 9/18/85

DEB/WPC I.D. No: 103-092

Monitoring Point Location (relative to site features): 5 FEET SOUTH OF BUILDING # 1

Drilling Contractor: GLENN DRILLING, INC. Supervising Engineer/Geologist: R.S. PETERSON

Well Construction Method: HOLLOW STEM AUGERS - AND 4-INCH DRIVEN CASING

WELL INFORMATION (ELEVATIONS TO NEAREST 0.1 FEET)

Ground surface elevation (MSL):

81 FEET

Well depth below ground surface:

Refusal: \_\_\_ Yes X No

Top of casing elevation (MSL): 81 FEET

Screened interval:

Length of screen: 5 FEET

9.2 - 14.2 FEET BELOW GRADE

Length of riser pipe: 9.8 FEET

Screen type: SLOTTED PVC

Screen slot size: 10 SLOTT

Filter fabric: \_\_\_ Yes X NoScreen packing: \_\_\_ Yes X No

If yes, Thickness:

Well inside diameter: 2-INCHES

Material:

grain size:

Impermeable Backfill: BENTONITE PELLETS  
CEMENT/BENTONITE GROUTWell casing material and schedule:  
SCHEDULE 40 PVC

Estimated K screened interval: NA

Method of well development: BAILING

Time spent developing: 15 MINS

Locking \_\_\_ or threaded cap \_\_\_

Impermeable backfill: CEMENT

CURB BOX W/FENTAGONAL BOLT

Bedrock wells

Casing length: 9.8 FEET

Water-bearing rock unit: GNEISS

Water bearing sections (depths and approximate yields): NA

Length of rock core: 9.7 FEET

Diameter of core hole: 2 3/8 - INCHES

Thickness and depth of impermeable backfill: 4 FEET (FROM 7' TO 3' BELOW GRADE)

C-ring seal:  Yes  NoGEOLOGIC INFORMATION

Aquifer: BEDROCK (GNEISS)

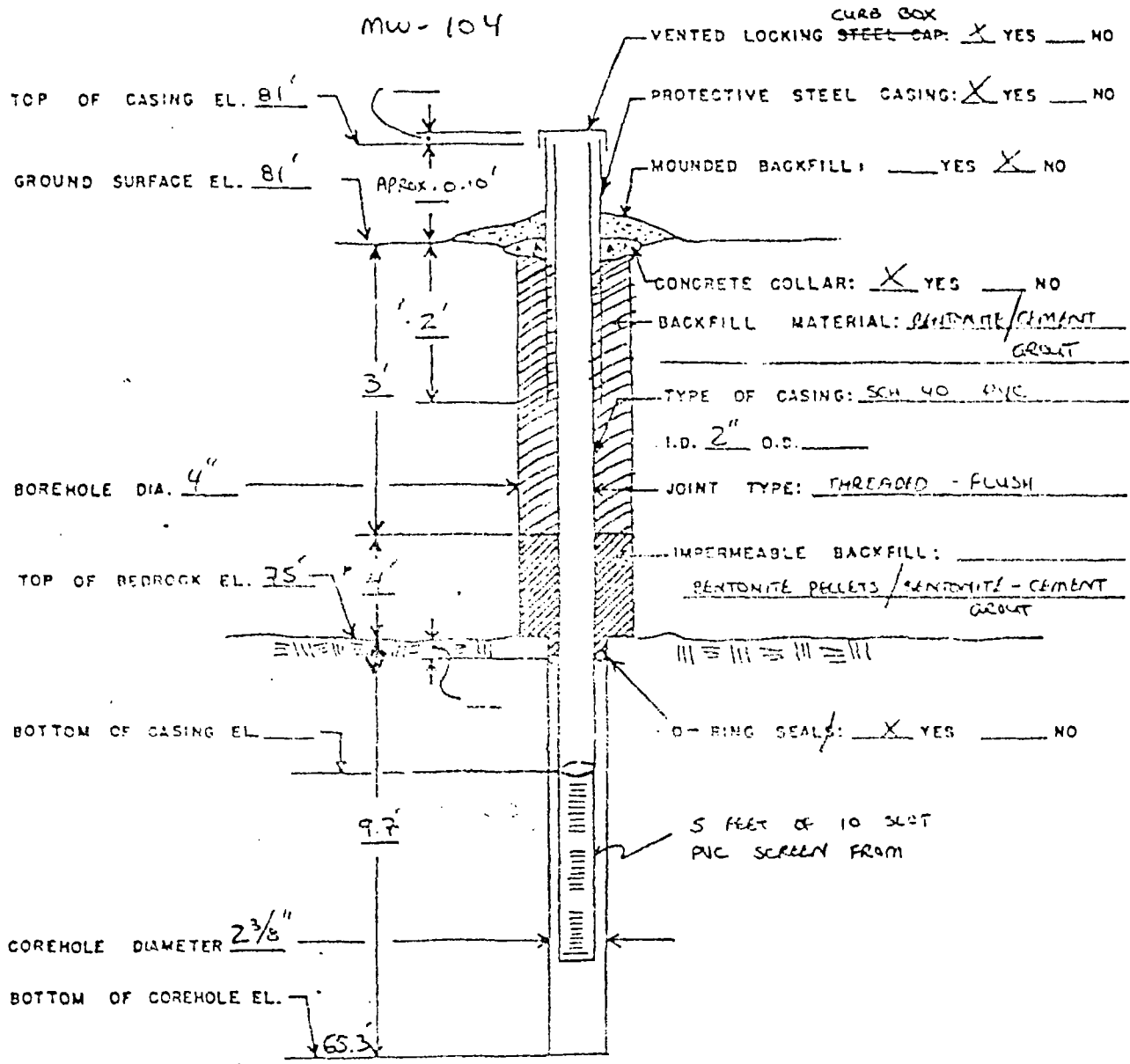
Inferred relationship to plume:  Within  Outside  Edge

Watershed (plume discharge watercourse): NEWJACK RIVER

Aquifer materials (attach boring log): BEDROCK

Attach maps and plans required of G.1.j. and G.4.

# MONITOR WELL INSTALLATION DETAIL FOR WELL IN BEDROCK







MONITOR WELL COMPLETION REPORTGENERAL INFORMATION

Town: NORWALK

BARDANISE BUILDINGS  
Site: 276 MAIN AVE.

Monitoring Point I.D. No.: MW-105

Date of completion: 9/23/85

DEP/WPC I.D. No: 103-092

Monitoring Point Location (relative to site features): 10 FEET NORTH OF BUILDING 1

Drilling Contractor: GLENN DRILLING, INC. Supervising Engineer/Geologist: R.S. POTTERSON

Well Construction Method: DRAVE 4-INCH CASING / NX CORING IN BEDROCK

WELL INFORMATION (ELEVATIONS TO NEAREST 0.1 FEET)

Ground surface elevation (MSL):

83'

Well depth below ground surface:

Refusal: \_\_\_ Yes X No

Top of casing elevation (MSL): 83'

Screened interval:

Length of Screen: 5 FEET

19.5' - 24.5' BELOW GRADE

Length of riser pipe: 19 FEET

Screen type: SLOTTED PVC

Screen Slot size: 10 SLOT

Filter fabric: \_\_\_ Yes X NoScreen packing: \_\_\_ Yes X No

If yes, Thickness:

Well inside diameter: 2 - INCHES

Material:

grain size:

Impermeable Backfill: BENTONITE PELLETS  
CEMENT / BENTONITE GROUTWell casing material and schedule:  
SCHEDULE 40 PVC

Estimated K screened interval: NA

Method of well development: BAILING

Time spent developing: 15 MINS.

Locking \_\_\_ or threaded cap \_\_\_  
CURB BOX w/PENTAGONAL BOLT

Impermeable backfill: CEMENT

Bedrock wells

Casing length: 19 FEET

Water-bearing rock unit: GNEISS

Water bearing sections (depths and approximate yields): NA

Length of rock core: 8 FEET

Diameter of core hole: 2 3/8 - INCHES

Thickness and depth of impermeable backfill: 9 FEET (10'-19' BELOW GRADE)

O-rings seals:  Yes  No

GEOLOGIC INFORMATION

Aquifer: BEDROCK (GNEISS)

Inferred relationship to plume:  Within  Outside  Edge UNKNOWN

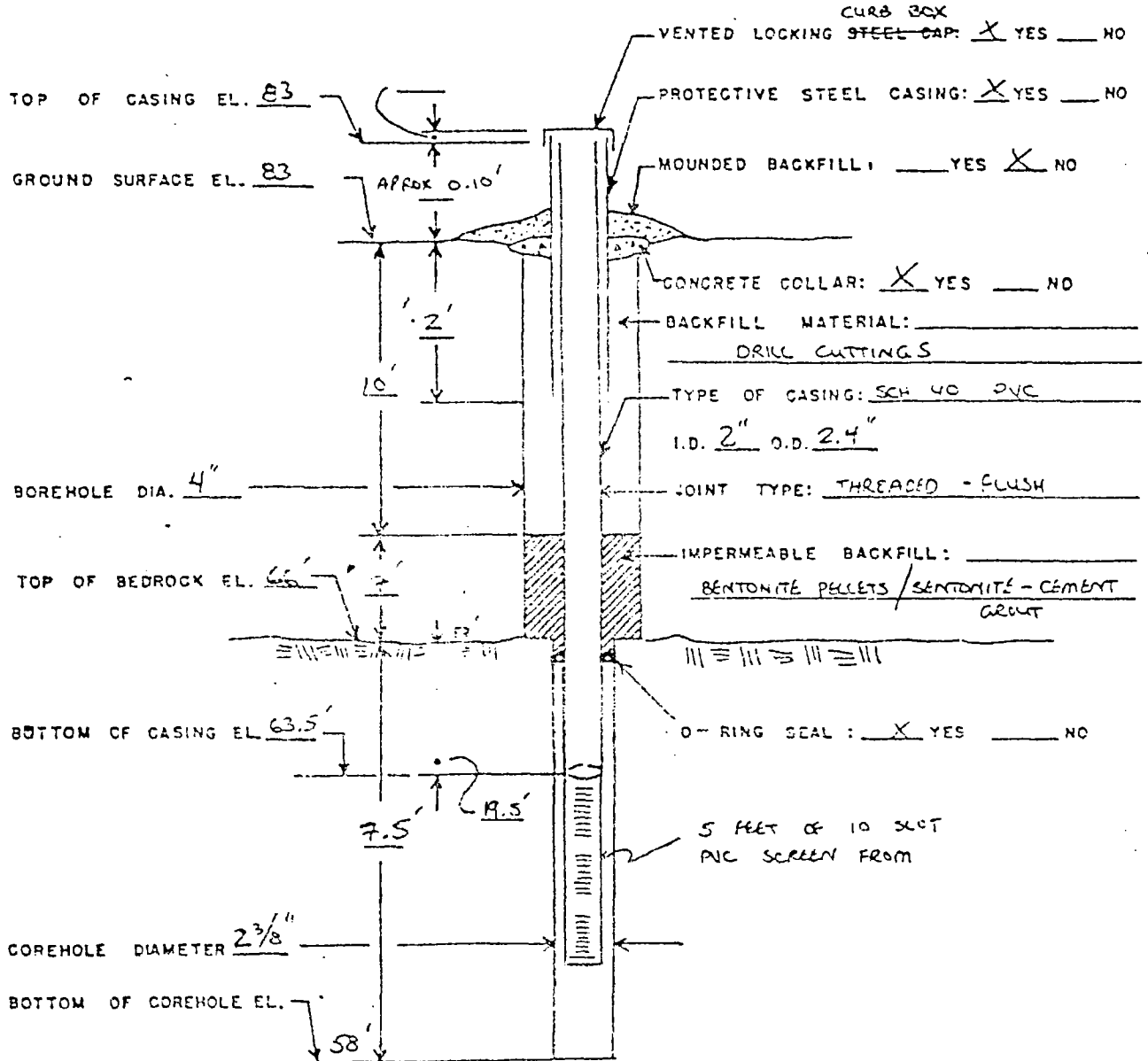
Watershed (plume discharge watercourse): NORWALK RIVER

Aquifer materials (attach boring log): SAND & GRAVEL / BEDROCK

Attach maps and plans required of G.1.3. and G.4.

# MONITOR WELL INSTALLATION DETAIL FOR WELL IN BEDROCK

MW-105



<b>Glenn Drilling Inc.</b> R.F.D. #1, Lake Road Fitchville, CT 06334 (203) 887-3621	CLIENT <u>Rice &amp; O'Neill</u> PROJECT NAME <u>KESLOW/NORWALK</u> LOCATION <u>Norwalk, Ct.</u>	BORING NUMBER <u>MW-106</u> SHEET No. <u>1</u> of <u>1</u>
--	--	--

DRILLER <u>Michael Deane</u>	ARCHITECT ENGINEER	FILE NO. <u>926-8540</u>	
INSPECTOR <u>Bob Potterton</u>	TYPE <u>Casing HW</u>	SAMPLER _____	CORE BARREL _____
DATE START <u>Sept. 23, 1985</u>	SIZE I.D. <u>4"</u>	HAMMER WT. <u>300#</u>	HAMMER FALL <u>24"</u>
DATE FINISH <u>Sept. 24, 1985</u>			
		SURFACE ELEV. _____	LINE & STATION _____
			OFFSET _____

DEPTH	SAMPLE					COL. A	STRATA CHANGE	FIELD CLASSIFICATION AND REMARKS	
	NO.	DEPTH RANGE	BLOWS PER 6" ON SAMPLER						REC.
			0-6	6-12	12-18				
5'							Drove Casing to 16.5'		
10'							End of Boring: 16.5'  Installed 2" PVC Monitor Well Bottom set at 16.0' 5.0' Screen 11.0' Riser 1 Protective Casing and Plug.		

<b>SAMPLE IDENTIFICATION</b> T — SPLIT SPOON U — THIN WALL TUBE O — UNDISTURBED PISTON W — OPEN END ROD W — WASH SAMPLE A — AUGER SAMPLE	<b>PENETRATION RESISTANCE</b> 140 lb. Wt. falling 30" on 1" O.D. Sampler <table style="font-size: small;"> <tr> <th>Cohesiveness</th> <th>Density</th> <th>Cohesive</th> <th>Consistency</th> </tr> <tr> <td>0-4</td> <td>Very Loose</td> <td>0-2</td> <td>Very Soft</td> </tr> <tr> <td>5-9</td> <td>Loose</td> <td>3-6</td> <td>Soft</td> </tr> <tr> <td>10-29</td> <td>Med. Dense</td> <td>7-8</td> <td>M/Stiff</td> </tr> <tr> <td>30-49</td> <td>Dense</td> <td>9-15</td> <td>Stiff</td> </tr> <tr> <td>50 -</td> <td>Very Dense</td> <td>16-30</td> <td>V-Stiff</td> </tr> <tr> <td></td> <td></td> <td>31 -</td> <td>Hard</td> </tr> </table>	Cohesiveness	Density	Cohesive	Consistency	0-4	Very Loose	0-2	Very Soft	5-9	Loose	3-6	Soft	10-29	Med. Dense	7-8	M/Stiff	30-49	Dense	9-15	Stiff	50 -	Very Dense	16-30	V-Stiff			31 -	Hard	<b>PROPORTIONS USED</b> trace 0 to 10% little 10 to 20% some 20 to 35% and 35 to 50%	<b>REMARKS:</b> COL. A _____
Cohesiveness	Density	Cohesive	Consistency																												
0-4	Very Loose	0-2	Very Soft																												
5-9	Loose	3-6	Soft																												
10-29	Med. Dense	7-8	M/Stiff																												
30-49	Dense	9-15	Stiff																												
50 -	Very Dense	16-30	V-Stiff																												
		31 -	Hard																												

MONITOR WELL COMPLETION REPORTGENERAL INFORMATION

Town: NORWALK  
 Monitoring Point I.D. No.: MW-106  
 DEP/WPC I.D. No: 103-092  
 Monitoring Point Location (relative to site features): NORTH OF BUILDING #1  
 Drilling Contractor: GLENN DRILLING, INC. Supervising Engineer/Geologist: R.S. PETERSON  
 Well Construction Method: DROVE 4-INCH CASING

BARCLAY BUILDINGS  
 Site: 276 MAIN AVE.

Date of completion: 9/24/85

WELL INFORMATION (ELEVATIONS TO NEAREST 0.1 FEET)

Ground surface elevation (MSL): 83'  
 Top of casing elevation (MSL): 83'  
 Length of screen: 5 FEET  
 Length of riser pipe: 11 FEET  
 Screen type: SLOTTED PVC  
 Filter fabric: \_\_\_ Yes X No  
 Well inside diameter: 2-INCHES

Well depth below ground surface:  
 Refusal: \_\_\_ Yes X No  
 Screened interval:  
 11.5' - 16.5' BELOW GRADE  
 Screen Slot size: 10 SLOT  
 Screen packing: X Yes \_\_\_ No  
 If yes, Thickness: 5 1/2 - FEET  
 Material: QUARTZ SAND  
 grain size: MEDIUM  
 Impermeable Backfill: <sup>BENTONITE PELLETS</sup> CEMENT/BENTONITE GROUT  
 Estimated R screened interval: NA  
 Time spent developing: 15 MINS.  
 Impermeable backfill: CEMENT

Well casing material and schedule:  
 SCHEDULE 40 PVC  
 Method of well development: BAILING  
 Locking \_\_\_ or threaded cap \_\_\_  
 CURB BOX w/PENTAGONAL BUT

Bedrock wells

Casing length:

Water-bearing rock unit:

Water bearing sections (depths and approximate yields):

Length of rock core:

Diameter of core hole:

Thickness and depth of impermeable backfill:

C-rings seals:  Yes  NoGEOLOGIC INFORMATION

Aquifer: SAND &amp; GRAVEL

Inferred relationship to plume:  Within  Outside  Edge  UNKNOWN

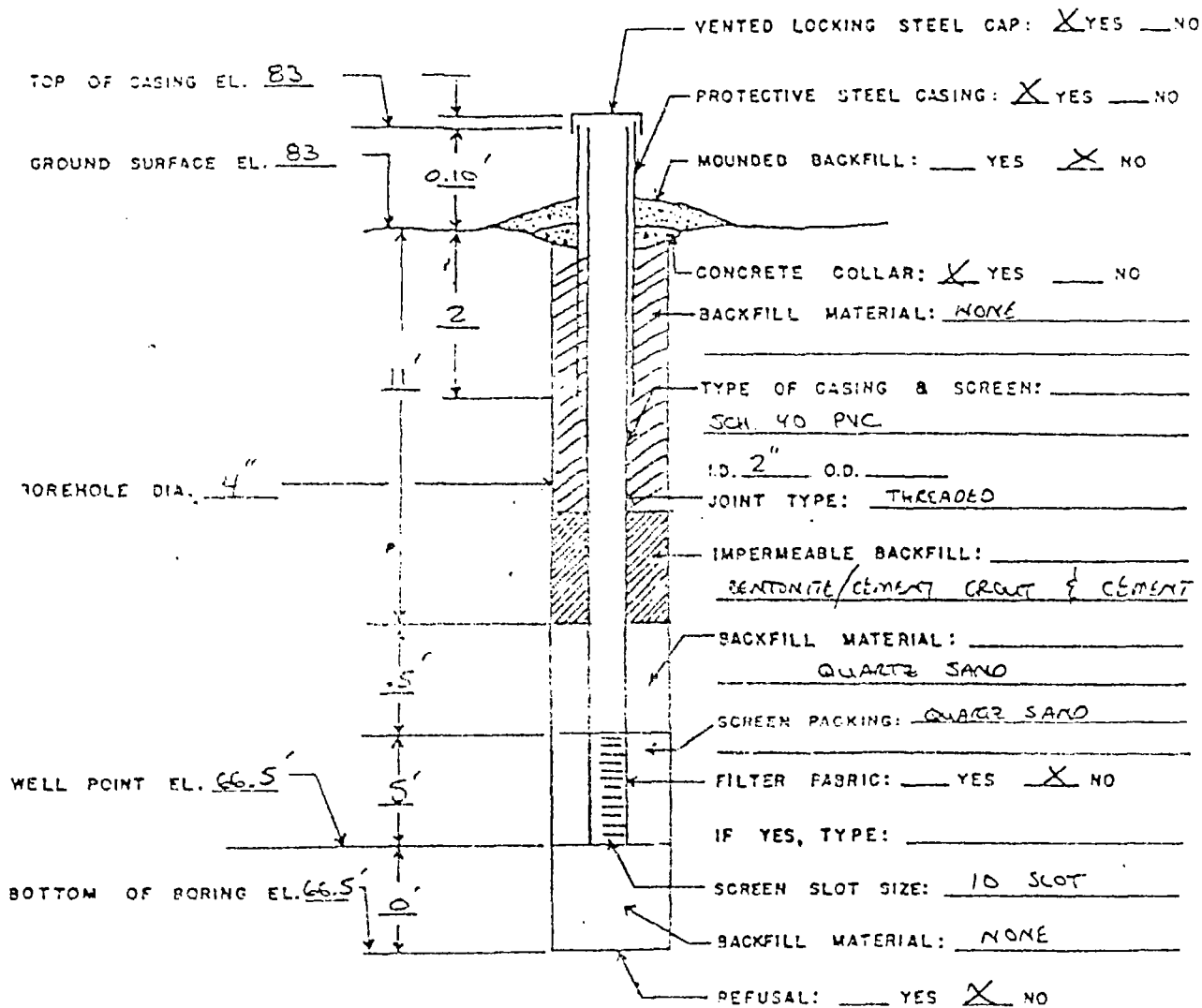
Watershed (plume discharge watercourse): NORWALK RIVER

Aquifer materials (attach boring log): SAND &amp; GRAVEL (ASSORTED SIZES)

Attach maps and plans required of 3.1.j. and G.4.

# MONITOR WELL INSTALLATION DETAIL FOR WELL IN UNCONSOLIDATED DEPOSIT

MW-106





PROJECT KELLOGG-DERRING  
 PROJECT NO. 5781-16 BORING K-10  
 ELEVATION 82.16' ± DATE 2/20/95  
 FIELD GEOLOGIST JEFF ORIENT

SAMPLE NO. TYPE & DEPTH (ft)	BLOWS/SIX INCHES OR R20 (%)	SAMPLE RECOVERY/SAMPLE LENGTH (ft)	MATERIAL MOISTURE & WATER DEPTH (ft)	MATERIAL DESCRIPTION*			USCS OR ROCK BROKENNESS	REMARKS
				SOIL DENSITY/CONSISTENCY OR ROCK HARDNESS	COLOR	MATERIAL CLASSIFICATION		
						SAND GRAVEL & CORBLES		
								DRILLED TO 16.0' THEN MOVED & RE-DRILLED - GROUTED UP ORIGINAL BORING
						9.0' ↓ ↓ 9.0'		
						ELLSIC GNEISS		
								CASING SET TO 15.0'

REMARKS BORING LOCATED BEHIND NEW ENGLAND QUARTZ BORING K-10

SEE LEGEND ON BACK PAGE 1 OF 3







# WELL LOG SHEET

WELL NR. K-10 PROJECT NR. 5781.16 PROJECT NAME KELLOGG - DEERING

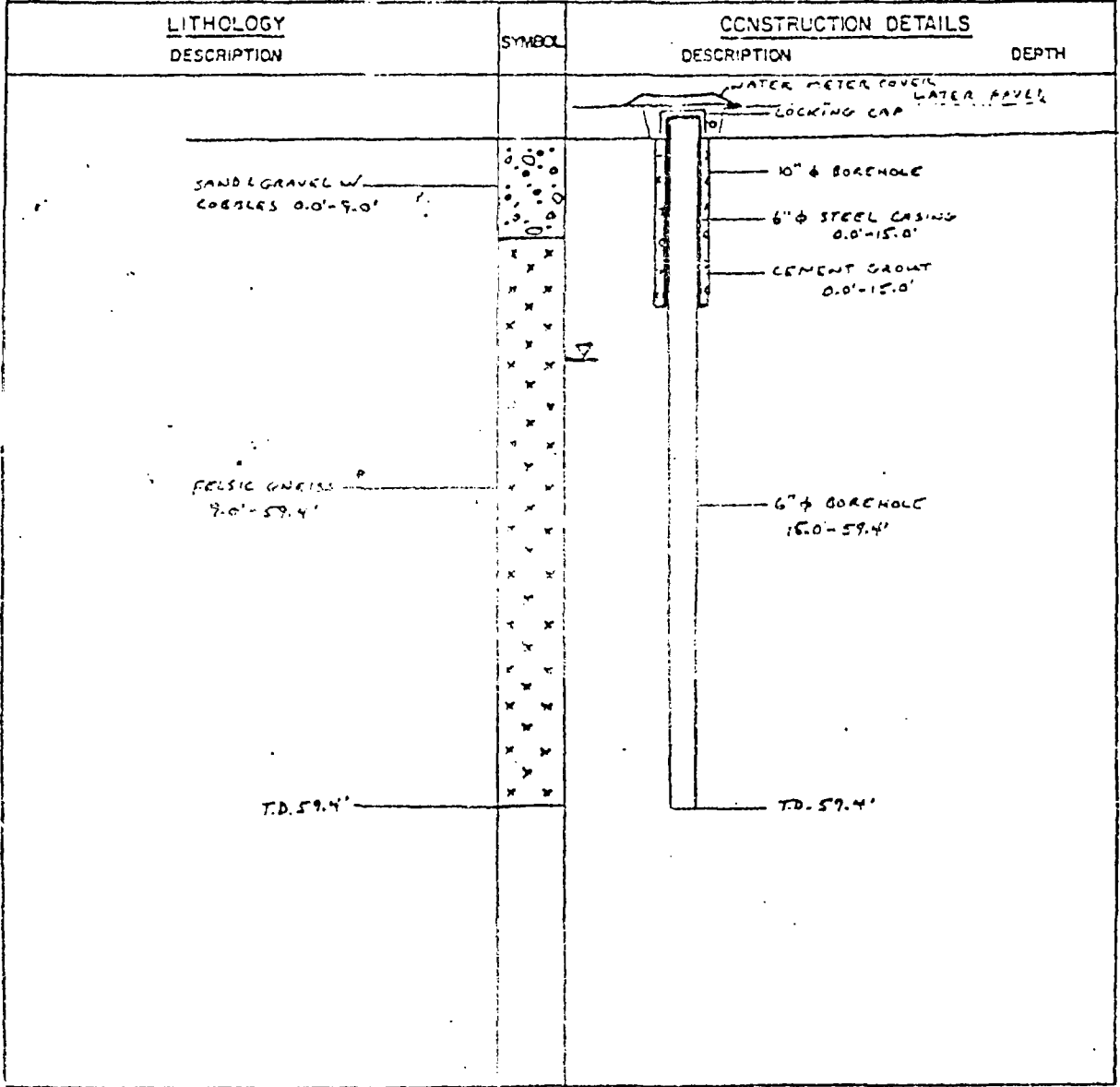
LOCATION BEHIND NEW ENGLAND QUARTZ GEOLOGIST JEFF ORIENT

DRILLING DATE 2/20/85 DRILLING CONTRACTOR EARTH ENGINEERING & SCIENCES, INC.

DRILLING METHOD AIR ROTARY / AIR HAMMER DRILLER L. FIDORAK INSTALLATION DATE 2/20/85

WATER LEVEL BEFORE INSTALLATION 20.85' WATER LEVEL AFTER INSTALLATION 20.85'

DEVELOPMENT METHOD MIL LIFT GROUND ELEVATION





PROJECT KELLOGG-DEERING  
 PROJECT NO. 3781.16 BORING K-11  
 ELEVATION 78.45'-11 DATE 2/20-2/21/85  
 FIELD GEOLOGIST JEFF ORIENT

SAMPLE NO. TYPE & DEPTH (ft)	BLOWS/SIX INCHES OR ROD (%)	SAMPLE RECOVERY/ SAMPLE LENGTH (ft)	MATERIAL MOISTURE & WATER DEPTH (ft)	MATERIAL DESCRIPTION*			USCS OR ROCK BROKENNESS	REMARKS
				SOIL DENSITY/ CONSISTENCY OR ROCK HARDNESS	COLOR	MATERIAL CLASSIFICATION		
0						ASPHALT 0.0-1.0'		
						MED TO COARSE SAND & GRAVEL		
						GRAVEL ROUNDED TO SUBANGULAR		
						UP TO 1" Ø		
5								
10								
15								
20								
25								

REMARKS BORING LOCATED IN FRONT OF PITNEY BOWES

BORING K-11

\*SEE LEGEND ON BACK



# WELL LOG SHEET

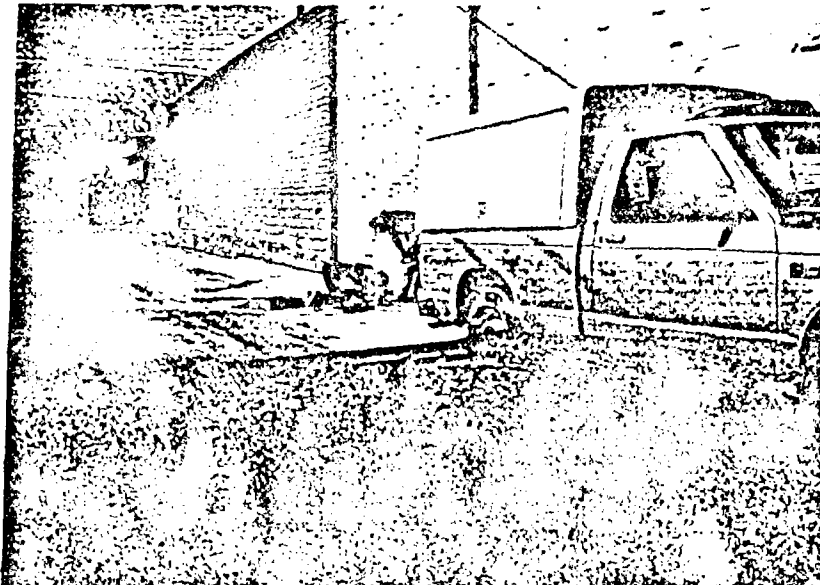
WELL NO. <i>K-11</i>	PROJECT NO. <i>5781.16</i>	PROJECT NAME <i>KELLOGG-DEERING</i>
LOCATION IN FRONT OF PITNEY-COWES		GEOLOGIST <i>JEFF ORIENT/M. COCHRAN</i>
DRILLING DATE <i>2/20-2/21/85</i> DRILLING CONTRACTOR <i>EARTH ENGINEERING &amp; SCIENCES, INC</i>		
DRILLING METHOD <i>AIR ROTARY</i>	DRILLER <i>L. FINGORAK</i>	INSTALLATION DATE <i>2/21/85</i>
WATER LEVEL BEFORE INSTALLATION <i>28'</i>		WATER LEVEL AFTER INSTALLATION
DEVELOPMENT METHOD		GROUND ELEVATION

LITHOLOGY DESCRIPTION	SYMBOL	CONSTRUCTION DETAILS	
		DESCRIPTION	DEPTH
SAND & GRAVEL 0.0'-38.5'			
			TD. 38.5'
			(BEDROCK)

APPENDIX B

PROJECT PHOTO LOG

PROJECT PHOTO LOG



CLIENT P. Jay - Bowes JOB # 15262-005

SITE LOCATION Zell 1 Bldg, Norwalk, CT

DATE OF PHOTO 5-8-97 TYPE OF CAMERA 110 35mm

OTHER \_\_\_\_\_

DESCRIPTION

INCLUDE NAMES/ACTIVITY OF PEOPLE IN PHOTO, WHY PHOTO WAS TAKEN, ORIENTATION OF PHOTO (LOOKING EAST, ETC.), WHERE ON-SITE PHOTO WAS TAKEN, PERTINENT PHYSICAL FEATURES, OTHER INFORMATION AS NEEDED TO EXPLAIN THE PHOTO.

*Two Fess + O'Neil present installing a new flush water casing on Well-104. The well is located on the south side of the Zell 1 Bldg. The photographer is looking west in this photo.*



PROJECT PHOTO LOG



CLIENT Pitney - Bowes JOB # 15267-005

SITE LOCATION Between 2nd Buildings New York, C.T.

DATE OF PHOTO 3-9-39 TYPE OF CAMERA 110 35mm

OTHER \_\_\_\_\_

DESCRIPTION

INCLUDE NAMES/ACTIVITY OF PEOPLE IN PHOTO, WHY PHOTO WAS TAKEN, ORIENTATION OF PHOTO (LOOKING EAST, ETC.), WHERE ON-SITE PHOTO WAS TAKEN, PERTINENT PHYSICAL FEATURES, OTHER INFORMATION AS NEEDED TO EXPLAIN THE PHOTO.

This photo is taken between the 2nd & 3rd Buildings. Concrete  
Drillings in progress with well K-11B, a bithead well.  
Notice the cutting being struck under the drilling rig, they  
were not put in ground. Notice the fragments of the  
wash & some crushed (blm) right next to the drilling  
rig. Contaminated types of glass lying on the ground.  
Most of the work is coming from a walking crane back  
to the left of the photograph.

PROJECT PHOTO LOG



CLIENT Wagner-Dawson JOB # 15767-005

SITE LOCATION Between 2nd and 3rd Bldgs

DATE OF PHOTO 3-9-58 TYPE OF CAMERA 110  35mm

OTHER \_\_\_\_\_

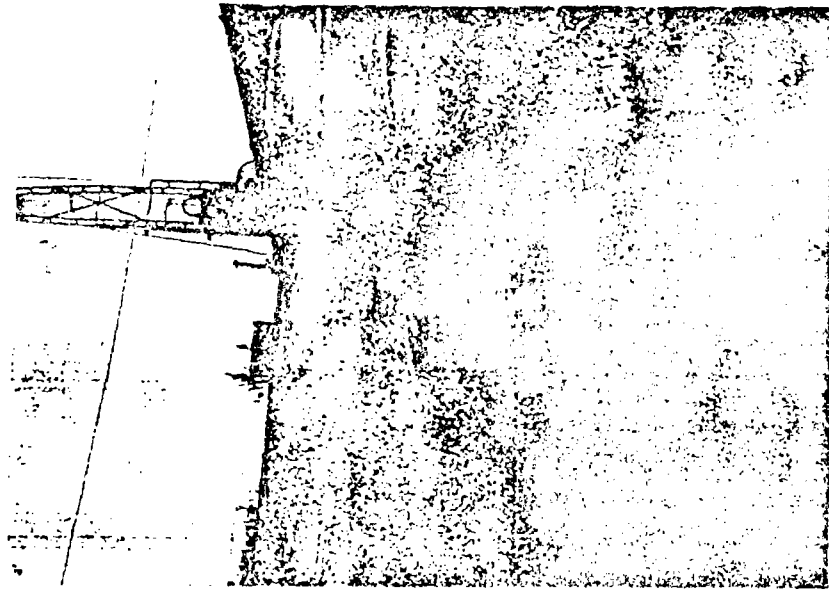
DESCRIPTION

INCLUDE NAMES/ACTIVITY OF PEOPLE IN PHOTO, WHY PHOTO WAS TAKEN, ORIENTATION OF PHOTO (LOOKING EAST, ETC.), WHERE ON-SITE PHOTO WAS TAKEN, PERTINENT PHYSICAL FEATURES, OTHER INFORMATION AS NEEDED TO EXPLAIN THE PHOTO.

This is a close-up from behind the building sign. Again  
observing the column of the beam ducts (black) in  
relation to the sign. Note cutting of duct under the sign  
and tyres on the ground.

Checked photo page 2 K-19B location

PROJECT PHOTO LOG



CLIENT P. Terry - Beves JOB # 15262-005

SITE LOCATION Between 2-11-1+2 Merrill St

DATE OF PHOTO 3/1/77 TYPE OF CAMERA 110 35mm

OTHER \_\_\_\_\_

DESCRIPTION

INCLUDE NAMES/ACTIVITY OF PEOPLE IN PHOTO, WHY PHOTO WAS TAKEN, ORIENTATION OF PHOTO (LOOKING EAST, ETC.), WHERE ON-SITE PHOTO WAS TAKEN, PERTINENT PHYSICAL FEATURES, OTHER INFORMATION AS NEEDED TO EXPLAIN THE PHOTO.

*Photo taken back (southward) to the drilling rig on  
K-14B. There are sitting casing at the joint  
Mudline from bottom is observed showing down  
drilling to the sewer*

PROJECT PHOTO LOG



CLIENT John Deere - Ames JOB # 15763-005

SITE LOCATION Water 2nd 1+2 Newark, CT

DATE OF PHOTO 3-6-68 TYPE OF CAMERA 110 35mm

OTHER \_\_\_\_\_

DESCRIPTION

INCLUDE NAMES/ACTIVITY OF PEOPLE IN PHOTO, WHY PHOTO WAS TAKEN, ORIENTATION OF PHOTO (LOOKING EAST, ETC.), WHERE ON-SITE PHOTO WAS TAKEN, PERTINENT PHYSICAL FEATURES, OTHER INFORMATION AS NEEDED TO EXPLAIN THE PHOTO.

*Close up of concrete work from setting corner at  
K-118. Water is muddy and used a slight film  
on it.*

*See page 4 photo on page 4*

PROJECT PHOTO LOG



CLIENT Pittman-Bones JOB # 15262-005

SITE LOCATION Site 20102 North CT

DATE OF PHOTO 1/11/71 TYPE OF CAMERA 110 35mm

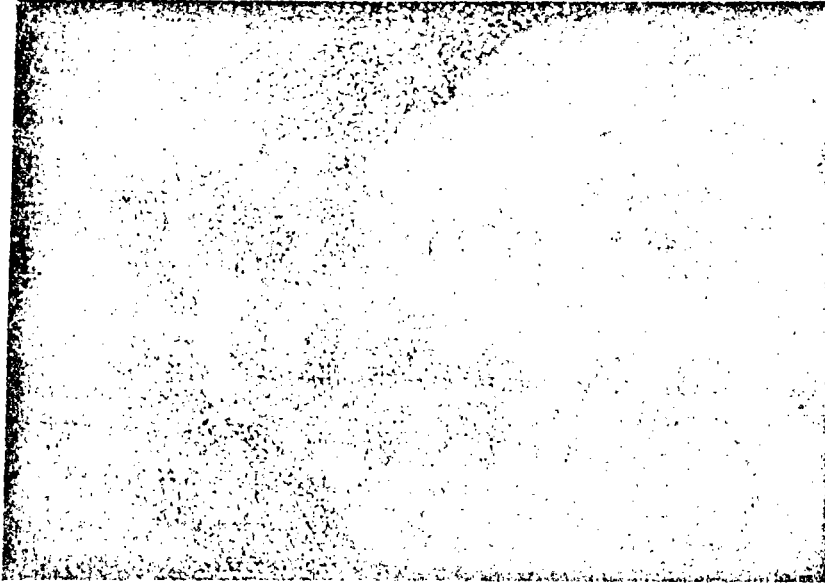
OTHER \_\_\_\_\_

DESCRIPTION

INCLUDE NAMES/ACTIVITY OF PEOPLE IN PHOTO, WHY PHOTO WAS TAKEN, ORIENTATION OF PHOTO (LOOKING EAST, ETC.), WHERE ON-SITE PHOTO WAS TAKEN, PERTINENT PHYSICAL FEATURES, OTHER INFORMATION AS NEEDED TO EXPLAIN THE PHOTO.

This is a black and white photograph taken from the site. The photograph was taken around 10:00 AM on page 5. The subject of the photograph is a person going to a river approx. 46 yds down driveway. The photo is taken from the subject's perspective.

PROJECT PHOTO LOG



CLIENT Pitney-Bowes JOB # 15262-005

SITE LOCATION 1000 E. 10th St. Wash DC

DATE OF PHOTO 7/1/82 TYPE OF CAMERA 110 35mm

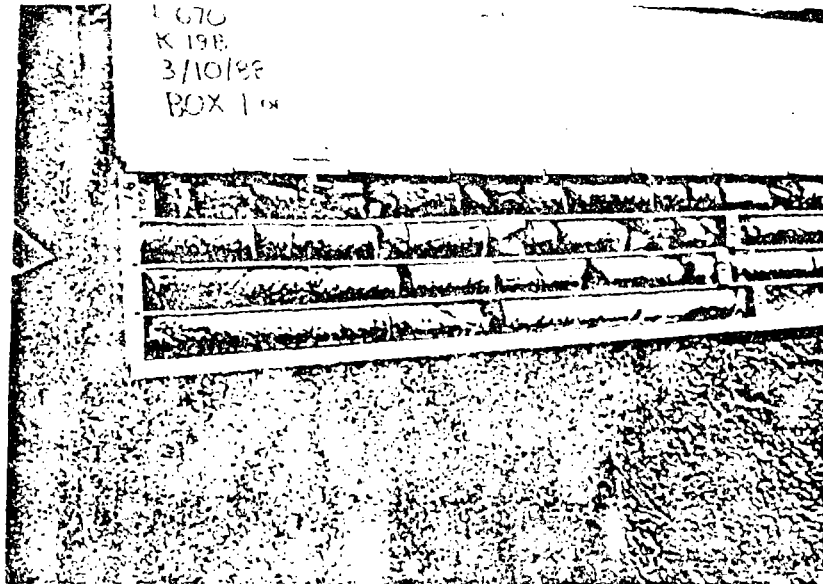
OTHER \_\_\_\_\_

DESCRIPTION

INCLUDE NAMES/ACTIVITY OF PEOPLE IN PHOTO, WHY PHOTO WAS TAKEN, ORIENTATION OF PHOTO (LOOKING EAST, ETC.), WHERE ON-SITE PHOTO WAS TAKEN, PERTINENT PHYSICAL FEATURES, OTHER INFORMATION AS NEEDED TO EXPLAIN THE PHOTO.

*This photo shows the contaminated water flowing into a sewer basin. The photo was taken when the basin was being set. Notice the amount of dark - brown water entering the basin.*

PROJECT PHOTO LOG



CLIENT P. Green - Brown JOB # 15267-005

SITE LOCATION K-19B Meriden, CT

DATE OF PHOTO 3-10-88 TYPE OF CAMERA 110 35mm

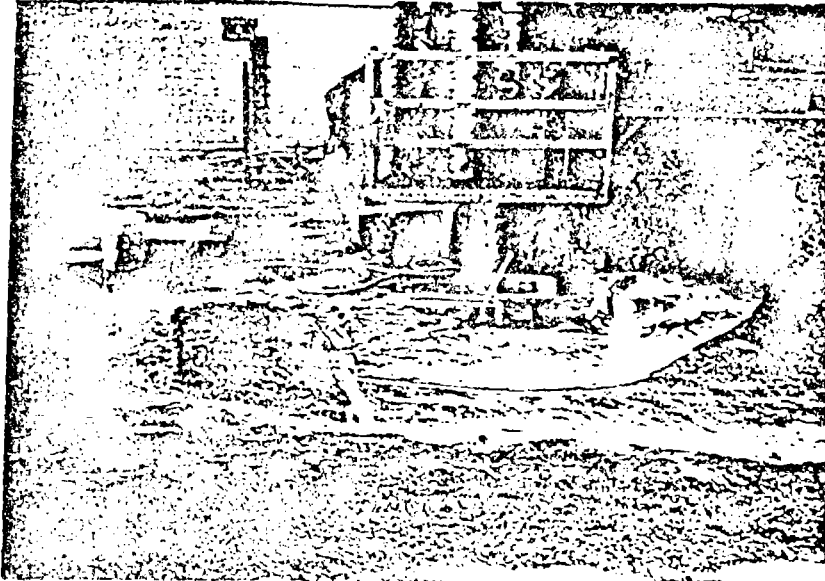
DESCRIPTION

OTHER \_\_\_\_\_

INCLUDE NAMES/ACTIVITY OF PEOPLE IN PHOTO, WHY PHOTO WAS TAKEN, ORIENTATION OF PHOTO (LOOKING EAST, ETC.), WHERE ON-SITE PHOTO WAS TAKEN, PERTINENT PHYSICAL FEATURES, OTHER INFORMATION AS NEEDED TO EXPLAIN THE PHOTO.

*only at the very end of C-1 & C-2 (14-32) on K-19B.  
Photo on page 10 shows the other end of the box.  
Back in a row, hand-drawn picture.  
Some fractured areas toward the top (weird). See sketch  
log for explanation.*

PROJECT PHOTO LOG



CLIENT Palmyra - Beavers JOB # 15262-005

SITE LOCATION Palmyra, Alaska

DATE OF PHOTO 3/10-88 TYPE OF CAMERA 110 35mm

OTHER \_\_\_\_\_

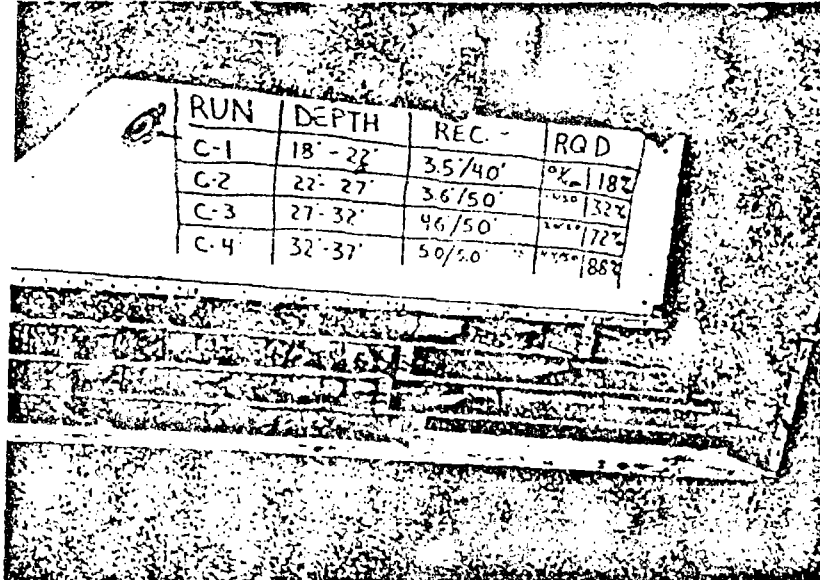
DESCRIPTION

INCLUDE NAMES/ACTIVITY OF PEOPLE IN PHOTO, WHY PHOTO WAS TAKEN, ORIENTATION OF PHOTO (LOOKING EAST, ETC.), WHERE ON-SITE PHOTO WAS TAKEN, PERTINENT PHYSICAL FEATURES, OTHER INFORMATION AS NEEDED TO EXPLAIN THE PHOTO.

*Photograph is taken directly west. They are coming the  
well! 3-198. Within the extent of the sandy water  
around the drill rig. There are all kinds of plants  
with various shells and bones scattered. It is also  
under the rig where all the things are sitting.*



PROJECT PHOTO LOG



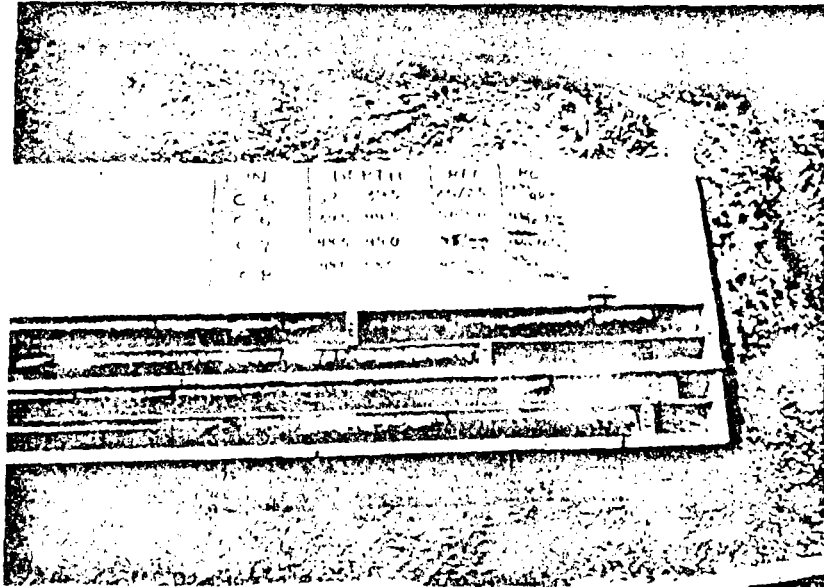
CLIENT Pelham - Dunes JOB # 15262-005  
 SITE LOCATION K-19B Yonkers CT  
 DATE OF PHOTO 3-11-88 TYPE OF CAMERA 110 35mm   
 OTHER \_\_\_\_\_

DESCRIPTION

INCLUDE NAMES/ACTIVITY OF PEOPLE IN PHOTO, WHY PHOTO WAS TAKEN, ORIENTATION OF PHOTO (LOOKING EAST, ETC.), WHERE ON-SITE PHOTO WAS TAKEN, PERTINENT PHYSICAL FEATURES, OTHER INFORMATION AS NEEDED TO EXPLAIN THE PHOTO.

Looking at core runs C-1 & C-4 (18'-37') on K-19B. Photo # 9 shows top end of the box. Run, depth, recovery and Rock Quality Description listed on box. Rock is fine grained. Banded.

# PROJECT PHOTO LOG



CLIENT U.S. Army Corps of Engineers JOB # 33570-1-1

SITE LOCATION 1000 N. 1st St. / W. 1st St.

DATE OF PHOTO 3/16/11 TYPE OF CAMERA 110 — 35mm S

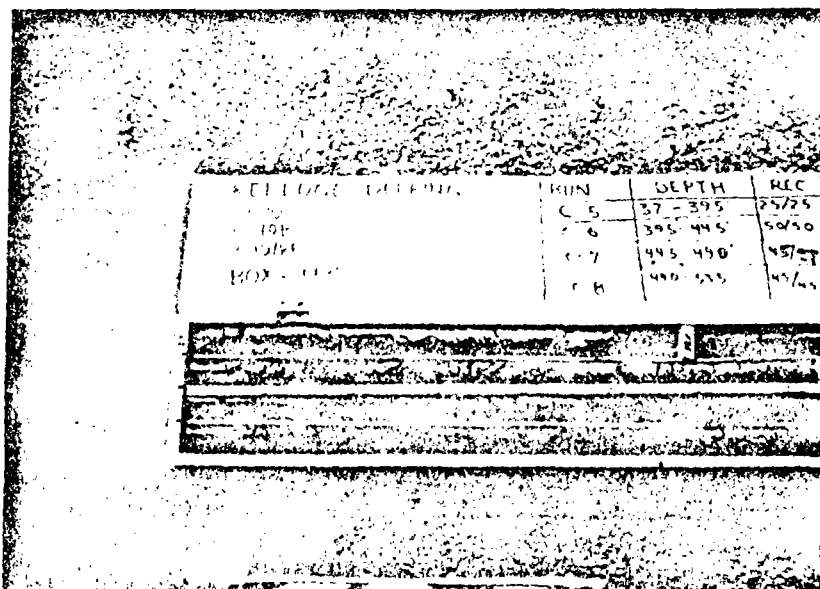
## DESCRIPTION

OTHER \_\_\_\_\_

INCLUDE NAMES/ACTIVITY OF PEOPLE IN PHOTO, WHY PHOTO WAS TAKEN, ORIENTATION OF PHOTO (LOOKING EAST, ETC.), WHERE ON-SITE PHOTO WAS TAKEN, PERTINENT PHYSICAL FEATURES, OTHER INFORMATION AS NEEDED TO EXPLAIN THE PHOTO.

*Handwritten notes in cursive script, mostly illegible due to the high contrast and slant of the handwriting. The notes appear to describe the site location and possibly the purpose of the photograph.*

PROJECT PHOTO LOG



CLIENT W. B. Jones JOB # 101-101

SITE LOCATION 101-101

DATE OF PHOTO 11 TYPE OF CAMERA 110 35mm

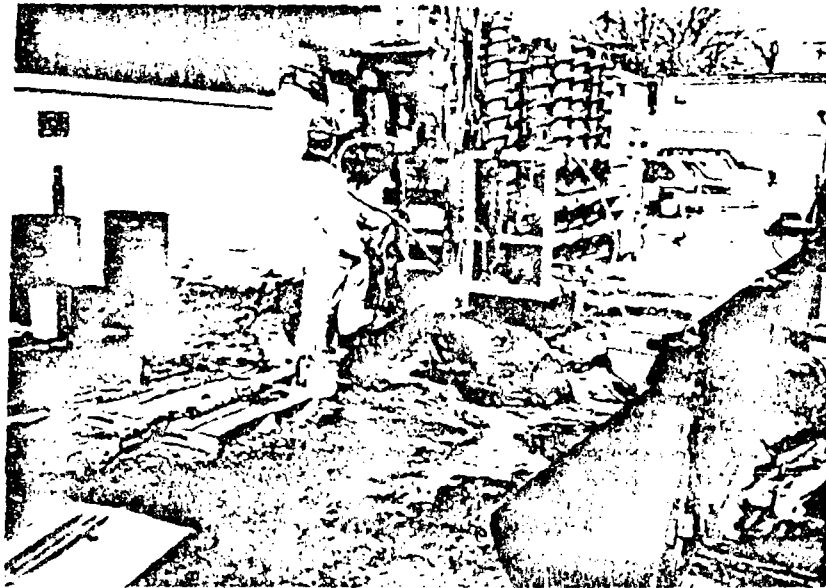
DESCRIPTION

OTHER \_\_\_\_\_

INCLUDE NAMES/ACTIVITY OF PEOPLE IN PHOTO, WHY PHOTO WAS TAKEN, ORIENTATION OF PHOTO (LOOKING EAST, ETC.), WHERE ON-SITE PHOTO WAS TAKEN, PERTINENT PHYSICAL FEATURES, OTHER INFORMATION AS NEEDED TO EXPLAIN THE PHOTO.

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*101-101*  
*101-101*

PROJECT PHOTO LOG



CLIENT \_\_\_\_\_ JOB # \_\_\_\_\_

SITE LOCATION \_\_\_\_\_

DATE OF PHOTO 11 \_\_\_\_\_ TYPE OF CAMERA 110 \_\_\_\_\_ 35mm \_\_\_\_\_

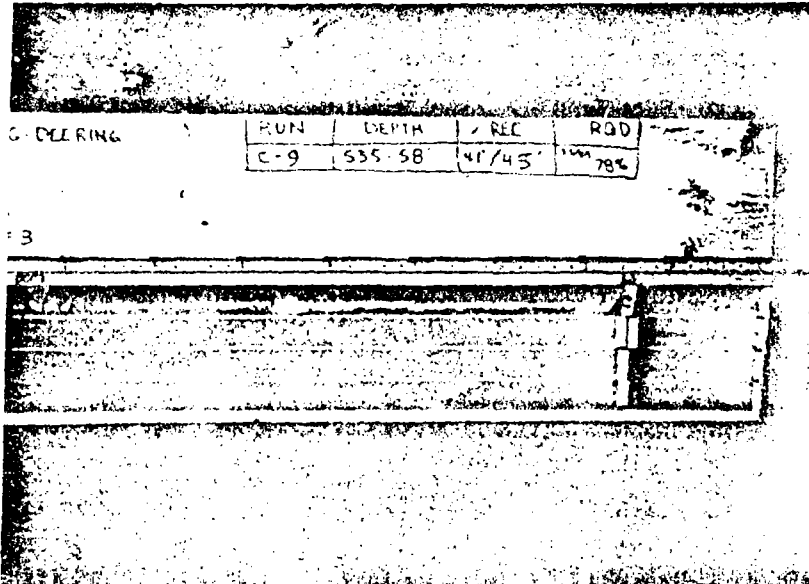
DESCRIPTION

OTHER \_\_\_\_\_

INCLUDE NAMES/ACTIVITY OF PEOPLE IN PHOTO, WHY PHOTO WAS TAKEN, ORIENTATION OF PHOTO (LOOKING EAST, ETC.), WHERE ON-SITE PHOTO WAS TAKEN, PERTINENT PHYSICAL FEATURES, OTHER INFORMATION AS NEEDED TO EXPLAIN THE PHOTO.

*Handwritten description of the construction site, mentioning a worker and building details.*

PROJECT PHOTO LOG



RUN	DEPTH	REC	ROD
C-9	535-58	41/45	78%

#3

CLIENT \_\_\_\_\_ JOB # \_\_\_\_\_

SITE LOCATION \_\_\_\_\_

DATE OF PHOTO \_\_\_\_\_ TYPE OF CAMERA 110 \_\_\_\_\_ 35mm \_\_\_\_\_

OTHER \_\_\_\_\_

DESCRIPTION

INCLUDE NAMES/ACTIVITY OF PEOPLE IN PHOTO, WHY PHOTO WAS TAKEN, ORIENTATION OF PHOTO (LOOKING EAST, ETC.), WHERE ON-SITE PHOTO WAS TAKEN, PERTINENT PHYSICAL FEATURES, OTHER INFORMATION AS NEEDED TO EXPLAIN THE PHOTO.

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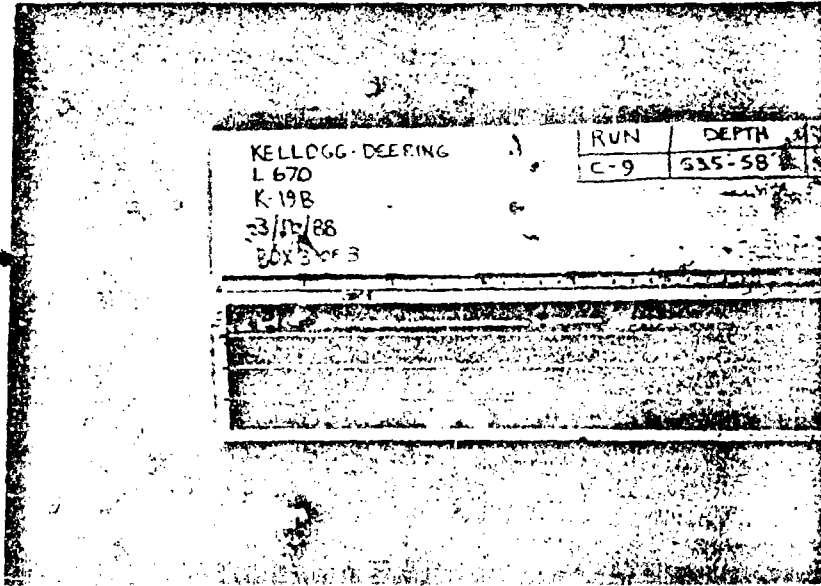
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PROJECT PHOTO LOG



CLIENT \_\_\_\_\_ JOB # \_\_\_\_\_

SITE LOCATION \_\_\_\_\_

DATE OF PHOTO \_\_\_\_\_ TYPE OF CAMERA 110 \_\_\_\_\_ 35mm

OTHER \_\_\_\_\_

DESCRIPTION

INCLUDE NAMES/ACTIVITY OF PEOPLE IN PHOTO, WHY PHOTO WAS TAKEN, ORIENTATION OF PHOTO (LOOKING EAST, ETC.), WHERE ON-SITE PHOTO WAS TAKEN, PERTINENT PHYSICAL FEATURES, OTHER INFORMATION AS NEEDED TO EXPLAIN THE PHOTO.

*View of Kellogg-Deering area looking north from site. Kellogg  
is visible to the left and Deering to the right.  
Location of photo is to the right.  
Photo was taken from a tripod  
on a rocky ledge of the site.*

APPENDIX C

DAMES & MOORE FIELD OBSERVATION LOGS  
FEBRUARY 24 - APRIL 14, 1988

Pitney-Bowers Site Visit

2-24-88

REVISIONS  
BY \_\_\_\_\_  
DATE \_\_\_\_\_

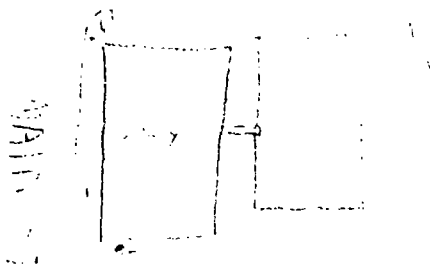
Arrived on site at 2:00 PM. [unclear] [unclear] [unclear]  
Called water office. Talked w/ Blawie and said they (Frank  
Drilling) is putting in a well.

I spoke with Mr Bob Poller of Frisco / O'Neil's Manufacturing  
(203) 646 2469. They are installing 2 interceptors wells  
as part of a DEP Consent Order a so called response action  
since they do not want to use the words remedial action  
The order was issued to LLINCO.

The wells are

1. 6" - 10" deep intercept well 2' from [unclear]

2. 6" - 10" deep intercept well 10' from [unclear] (Drilling)



The wells will be purged &  
the ground water beneath  
any solid waste found  
before the EPA as the  
purging water do not interfere  
with the [unclear] study [unclear]

BY \_\_\_\_\_  
CHECKED BY \_\_\_\_\_  
DATE \_\_\_\_\_  
FILE \_\_\_\_\_



March 8, 1988

Re. Observation of USEPA contractors

Pitney-Bowes

Norwalk, CT

G. DiMastro

- 12:05 pm Arrive at the Pitney-Bowes Zell Building. Drive to north side of building where the proposed K-19A, B wells are to be installed. Area is blocked off but no sign of drillers, contractors or EPA.
- 12:10 p. Drive around to south side of Pitney-Bowes Zell Building. Two field personnel for (Fuss + O'Neil)? are doing work on MW-104. Seem to be clearing an area down around the PVC pipe where they are installing a protective pipe. (see photo)
- 12:20 p. Called Bob Potterton of Fuss + O'Neil. Asked him what the status was on the well logs (MW-100 to 106) around the Pitney-Bowes building. He had contacted the attorney and then he missed the attorney's call but he again interceded that it shouldn't be a problem. He had already made the copies and would put them in the mail after talking again w/ the attorney.
- 1:50 P Two men from EPA arrive at Pitney-Bowes K-19 site and look around. They leave at 1:55 P.
- 2:30 P Get message at P-B front desk. To call West office. Call Mr. Don Blackert at NUS Trailer to discuss drilling schedule 203 849-9721

3:00 P

Found NUS ~~Trailer~~ (Don Blackett) and met other NUS representatives - Driller from Empire also.

The steam cleaner broken down and they are bringing one down from upstate. Should begin P-B drilling on Wednesday around 12:00 (noon) or later.

Directions to Trailer:

Go North on Main St.

Turn by some Auto Body Place of big yellow arrow  
1/4 mile after underpass

Brown RTAEP mobile home - next to it.

3:30 P.

Left and went and called D-M.

Get 6 sets of Bellows

For TCL soil + a QA/QC

Ship to P-B Wednesday.

3:40

Then stopped at P-B and told them to expect Fed Exp package (cooler) for Charlie Brundhuist

3:45

Left for D-M office

Pitney-Bowes  
2nd Bldg.  
Middletown, CT  
3-9-88  
G DeMastro

- 10:00 A Meeting going thru 7-11 1 + 7-11 2 (see meeting notes)
- 11:10 H. Has been given message that cookies from Nancy has arrived
- 11:15 H call D&M office + talk w Blamelt. I am going to start sampling K-19B and leave the meeting. I will take 2 samples. (0-2' + 5-7')
- 11:25 A Engine drilling crew (David Bartlett - <sup>Union</sup> True.) suiting up and getting ready to drill K-19B. Drilling crew dressed in Level D (yellow tyres, overboots, surgical gloves, taped ankles and wrists, hard hats and eyewear. green outer gloves)
- Rick Bethel, NUS field geologist
- Steam cleaned augers on wooden pallet
- Wash + rinse buckets taped out on plastic.
- I put on a white tyre. NUS wears nothing (tyres).
- 11:55 Encountered 1/2 foot of asphalt + concrete. 1<sup>st</sup> Sample from 0.5-2.5'
- 12:15 Took 2<sup>nd</sup> Sample 5-7'. They sample every 5 feet. The cuttings are piled under the rig no drums.
- 2:00 Called in D&M office
- 2:35 Auger refusal 17.1' They (drillers) take a break.
- 3:50 Begin to set casing 4" into bedrock. During the setting of the casing water comes out of augers (5 gal) there is nothing to capture water. Water makes out and sprays over

Pitney-Cross  
211 Bldg  
3-9-88  
Norwalk, CT

cuttings, piled under rig. The water is able to flow down the driveway and mess w/ water from melting snow.

4:15p Begin setting 4" casing and as soon as few tenths of a foot water flows out of top of augers and also coming up outside of augers. Water is muddy brown. They stop after about a gallon comes out. The water flowing down driveway about 40 yds and runs down sewer.

4:45p They plug outside of augers, between augers & hole, with bentonite pellets. Before then they pull up augers 6 inches. After applying bentonite pellets they put mud tub in place.

I called Dick Butchell (NWS) about getting a copy of the drilling log. He tells me to call Joan Piro to ask him.

5:00 After trying plastic around augers & let water flow into mud tub they begin setting casing. About (>10gal) of water flows out outside of auger flights and runs into sewer. Water is muddy brown w/ a little film on it.

5:45p They are still setting casing & are down to 1' into soil. Approximately 75-100 gal of water has washed down the sewer. Firehose also been observed coming out of trench. Bottom of casing is at 18'

6:08 Left site

FIELD MEMORANDUM

ACTION	7-8	INFO
To: Present	Red Walsh T-D	File: P. Tracy-Bowes
	Tuan Riis CR	
	Don Blackett	Drilling - Zell Building
	Sir Ferrelli (us)	
	Edna Johnson	X-Rel:
	myself	
	Kevin Conway MS	
		Date: 3-9-88

From: G. DeMaestro

Reply Required By:

Subject: MEETING to discuss Drilling in Buildings

Reference(s): 10 holes each building

\* Chrome, copper, nickel, gold etc.  
cosmetic compacts.

Zell 2 Bldg (1957)

① Three doors to the right Plating operations. Was a pit.

Filled in w/ fill and then concrete. Will have to penetrate through

2 layers. Original outline of plating will have wooden ceiling above.

Zell 1 Bldg (1957)

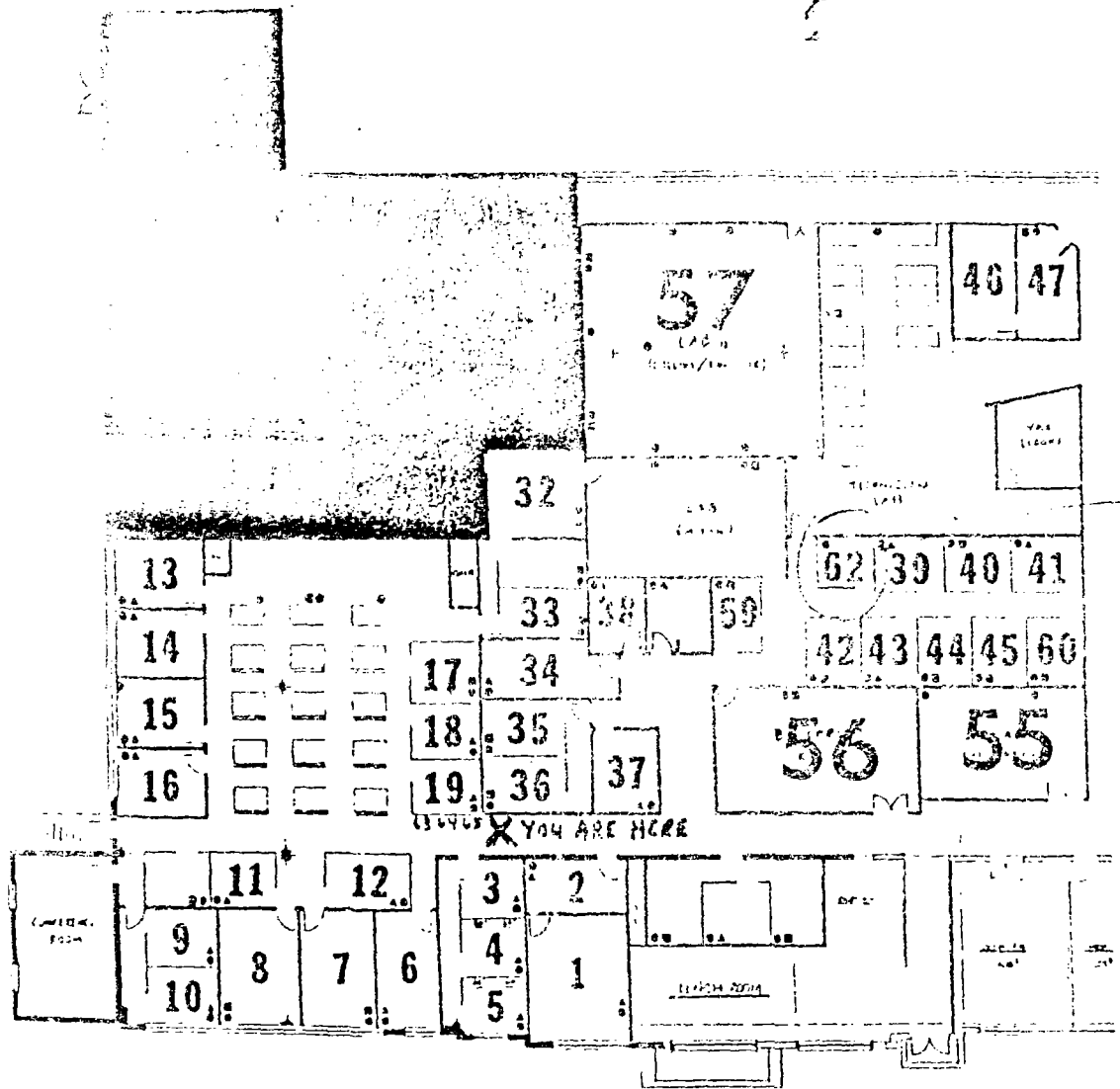
② North central section of Zell 1. Another plating operation. no wooden deck over.  
Smaller plating operation than Zell 2.

Trenches were open and so were basements. All these (plating, cover, etc) fed a

drain and exited the building.

I leave the transportation at 11:15a to home Engine drilling, K-19B.

ROUTING



10-1-60  
 10-1-60  
 10-1-60

Pitney-Bowes  
Zell Bldg  
Norwalk, CT  
3-10-88  
(1)  
G. DiMoro

7:00 A Arrive at Pitney-Bowes Zell Building (K-19B) Location  
Empire is here suited up and getting set up for <sup>ream</sup> reaming

7:15 NYS Corp Rock Borehole field geologist shows up on site  
Weather cond. Cloudy cold, 30s. Ground wet, alot of drilling fines  
and mud still present, leading to sewer. Didnt wash away from rain.

7:40 A Begin <sup>reaming</sup> ~~drilling~~ K-19B

7:45 Jim Forcella (NYS) shows up. I ask him about copy of drilling log - tells  
me I should check w/ John Rios (EPA).

I also inquired about yesterday meeting among the whole site-go survey.  
He + Rios have to sit down + think about it tomorrow. They  
probably will make some changes, go with fewer holes. Forcella will call me.

7:50.11 Finished reaming and put core barrel together. Will start coring at 18'  
1<sup>st</sup> Run 18-22' Ream 3.5/4.0'

RQD - Rock Quality Designation

Measure the lengths of cored rock > 4" at each run up. ~~Measure in units~~ Divid by length/corrun

10:00 Begin 2<sup>nd</sup> Rock coring run.  
22-27' Rec. 3 4/5.0'

10:30 Begin 3<sup>rd</sup> Rock coring run

Spoke w/ John Rios earlier and he said I could make copies of the ~~drilling~~ well  
logs from these wells. Rock Borehole told Rios that they were going to wash

P. Inoy - Bows  
2011 Bldg  
C7  
MILWAUKEE, CT  
3-10-88

the driveway down after they were finished. He said that was ok.

11:35 coring water is being recirculated for the coring. Empire just pumped the water from the mud tub into a 55 gal drum. They are using clean coring water now.

During the coring, the coring water splashes out of the mud tub and flows down the driveway. (see photo)

3rd Run 27'-32' @ 4.6/5.0

1:15 Have begun 4th coring run

2:00 Finished up 4th coring run 32-37' Rec. 5/5.0'

2:15 Began coring run 5  
Barrel out coring Pull out of hole and try to free trapped rock in end of core barrel.

3:30 Call D-M office

3:45 Begin recing run - 5

4:30 Pull out core barrel on run 5  
27-29.5 2.5/2.5 Rec.

Coring water is grey + clear. Very easy to core hit a large wood.

4:55 Begin another coring run to retrieve what was lost in hole from last run.

5:15 Finished run 6 39.5-44.5 5/5 Rec.



Pitney - Bowes  
Zell Bldg  
Newark, CT  
3-11-88  
G DeMastro

- 6:55 A Arrive at Pitney-Bowes Zell Bldg. All soil + water on ground from 3-10-88 in frozen.
- 7:10 A Empire Soils Arrives + sets up.
- 7:40 NUS Field Geologist shows up - then leaves.
- 7:45 Begin coring run 7
- 8:25 Finished coring run 7 44.5' - 49.0' Rec  $\frac{4.5}{4.5}$
- 8:35 Begin coring run 8
- 8:45 NUS field geol (Rich Bull) shows up w/ Don Blackett.
- 9:05 Finished coring run 8 49 - 53.5 Rec 4.5/4.5 (see photo)
- 9:30 Begin coring run 9. (see photo) Differs during coring operation
- 10:05 A Finished last coring run #9 53.5 - 58' 4.1/4.5 Rec.
- Begin breaking down drilling rods. Also going to packer test the formation. Don missing 1 piece and will postpone test to Monday.
- 11:15 Begin ream hole
- 12:20 Finished ream hole 18 - 28' w/ 3 7/8" roller bit
- 12:35 Leave Site Will begin work around 8 AM on Monday

Pitney - Bowes  
2-11 Bldg.  
Norwalk, CT  
3-14-68

G. DellMastio

- 7:50A. Arrive at Zell Bldg.
- 8:15A Dow Blackert (NUS Corp) meets me at K-19B.
- 8:50A Empire Drilling shows up, leaves to drop off supplies
- 9:30A Begin to set up to ream out hole after 1<sup>st</sup> washing it out.
- 9:45A Begin to clean out hole
- 10:15A Begin to ream out the borehole, using a  $3\frac{7}{8}$ " roller bit.  
Progressing at approx. 6 min/ft.  
Reaming water is being recirculated into a mud tub.  
A fair amt. of water is splashing outside of tub.  $\approx 1 \text{ gal}/10 \text{ min.}$
- 10:50A Reaming is progressing at about 6-7 min/ft. Rock is very hard!  
Met Scott (NUS field geologist) sitting sig.  
Ebasco will be on-site the week of 3-21-68.
- 11:45A Have reamed down to 34 feet
- 12:15P Break for Lunch. Had to change roller bit because the first bit was badly worn. Outside ring of teeth was worn away. New bit was  $3\frac{7}{8}$ " but a fraction larger than worn bit, therefore had to re-ream hole and are presently down to 34'. Drillers are not pleased with the progress and would like to change and use a button bit.
- 1:30P Change to a button bit.
- 2:00P Making progress, down to approx. 41-42'

Pitney-Bowes  
2011 Bldg  
Norwalk, CT  
3-17-83

- 2:15 P Down to 45' another 13' to run. K-19B
- 3:10 P Have advanced button bit down to 55' on K-19B.
- 3:40 P Finished reaming out hole to 57.7' Begin to set up for packer test on K-19B.
- 4:20 P D.T.W 9.3' erroneous value since the water level hasn't stabilized from reaming  
Packer Test - reamed hole
- 4:50 P Set Packer bottom at 46'. Above interval that is fractured.  
Begin pumping water into hole until it starts flowing out of casing. Inflate Packer (220 psi) after it seals borehole flow of water out of casing should cease. Higher psi in reamed hole because of upwash borehole. Adjust water flow to desired pressure (20 psi).  
Then take readings of water volume being pumped into formation  
30 sec for 10 min
- Raw test at 20-40 psi. No flow.
- 5:20 P Possible problem with flow meter. Clean it out.  
A lot of fine sand found inside. Rerun test at 20 psi  
No flow observed on meter. Bottom 12' of hole (46-58') must be tight. Tomorrow will raise packer 10' and rerun test.
- 5:50 P Left from Pitney-Bowes.

Vitwey-Bowes  
Zell Bldg  
Norwalk, CT  
3-15-88

G. DeMastro

7:05A Arrive at Zell Bldg K-19B  
Empire setting up.

7:20A NUS Arrives

7:45A Pull Packer setup 10' uphole. Bottom of packer is at 36'

7:55A Begin test at 20psi. No movement in the flow meter.

8:10A Shutdown packer test after 15 minutes at 20psi. No flow.  
In discussion w/ Empire (Vince) Drilling, he believes fractures are up near the top at the bedrock. This is where he lost water previously, not near the bottom.

8:20A NUS decides to abort packer test. They propose to core another 10' into the bedrock, run this interval out and then rerun the packer test.

9:00A NUS changes their earlier decision about casing deeper.  
They are going to set up a double packer from 25-35' interval

9:20A Call into D+M

9:55A Setting up packer in the borehole.

10:00A Begin packer test. Flow of water into the rock is very slow at 20psi.

10:20A Finished packer test at 20psi. Formation was taking water at about 0.6 gal/min. Log shows a possible fault at 39.5'. This

11111-11111  
211 Bldg  
Norwalk, CT  
3-15-88

9:20  
cont)

was written down incorrectly. Fault? is at approx 29.5. The packer is set over this zone now

10:25A

Increase water pressure to 40psi and begin test again.

10:35A

Final packer test at 40psi. Flow approx 1 1/2 gal/min. into rock. They are going to set the well from 25-35' (screen). ~~Install a 2' trap below screen: 35-37'~~ <sup>Install a 2' trap below screen: 35-37'</sup> from 57.7 up to sand pack.

Install a 2' trap below screen: 35-37'

Screened interval: 25-35'

Sand pack: 23-39

Volclay: ~~ABC-522~~ + 23 up to <sup>top</sup> motor (3')

11:15A

Begin to add a sand + bentonite balls into hole.

12:10P

Broke for lunch, leaving sand + bentonite to settle out.

1:00P

Installed screen and casing (view paper) + trap

1:30P

Mixing volclay to grout hole.

1:45P

Finished tamping grout hole to surface w/ volclay  
Trying to remove 4" casing which was spun into bedrock  
Water displaced in barrel during tamping grout was not contained and flow out into driveway.

2:00P

Left site to return to D-M office

Pitney - Bowes  
Zell Building  
Norwalk, CT  
3-16-88  
G. DeMastro

7:20 A Drillers have setup on K-19A. On 3-15-88 they advanced the augers to 14 feet on K-19A.

7:40 A Meet Nus Field geologist (Scott) and Empire at K-19A.

8:00 A On observation of drilling site, Empire has changed auger sizes  $6\frac{1}{4}$ " ID to  $4\frac{1}{4}$ " ID for K-19A. The cuttings and soil was removed and barrelled from K-19B on 3-15-88.

Water in hole (K-19A) at 12.2' Cuttings are drummed as they come out <sup>of</sup> barrel  
DTW 11.22' on K-19B K-19A is  $3\frac{1}{2}$ ' North of K-19B.

8:10 A ~~Augers refused~~ <sup>Augers refused</sup> at bedrock 15.5'  
The borehole will be advanced 2' into the bedrock w/ a button bit. A 2' trap will be installed from the bedrock surface and below. The trap will be surrounded w/ bentonite balls. A 5' screen will be installed w/ a surrounding sand pack, a layer of bentonite balls on top w/ the rest of the borehole filled w/ wetclay to w/in 3' of surface.  
Drillers leave to get supplies.

9:05 A Drillers setup to ream 2' into bedrock. Auger refused at 16.5'  
Ream from 16.5' - 18.5'

10:00 Begin reaming out a borehole in the bedrock. Progress is very slow. Leaking slot of water, the water is coming up the outside of the augers, unable to contain. Water is running into sewer

Pitney Bowes  
211 Building  
Newark, CT  
3-16-88

10:30 A Finished reaming out bedrock to 18.5'  
Began cleaning out cuttings in borehole. Approx 80-100 gal of water lost  
cuttings also observed on ground leading away from borehole. <sup>ground</sup> to sewer

10:50 A Finished cleaning out borehole and began to pull rods. and install well  
Discussion of Scott + Don Blackcut  
Soil-Gas - Mon  
GC lab - Mon afternoon  
Indoor Soil-Gas - March 26<sup>th</sup>  
Tracer study late next week or at least 4 days after FurioD well put in

K-19A  
TD 18.5'  
Trap 16.4 - 18.4  
Screen 11.4 - 16.4  
Sand Pack 9.4 - 11.4

11:50 A Sand Pack and screen to 3.9' Pulling auger flights.

12:00 Left to go to NUS truck for logs. Well finished w/ installation  
except to put in velocity w/in 3' of surface.

Pitney-Bowes  
Observation of EPA Co.  
March 24, 1988 ①

Greg DeMastro

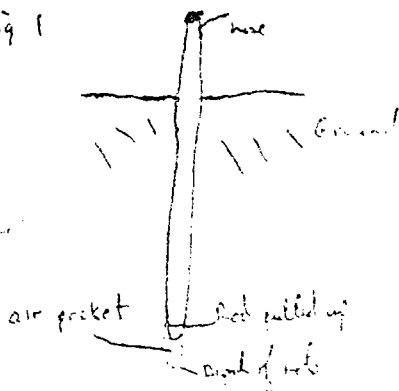
7:15 A Arrive at Pitney-Bowes Zello Building. Go to NUS Trailer  
Meet Empire drillers on the way (<sup>Don</sup> Viny). The backhoe on NW  
corner of Elinor Bldg they drilled to approx 160' re fractures when  
moved back up hole + did packer test around 90'. NUS was satisfied  
with results and set screen. Backfilled from 90-160'.

7:30 A Arrive at NUS trailer. They are getting ready to go to Elinor +  
do some soil-gas work. K-19A,B and MW-100 + K-11 were developed  
on 3-23-88. They will take ground water samples today.

7:45 A Setup for soil gas survey behind Elinor bldg. The following  
explains the procedure they are using which I observed:

Take an approx. 3' long steel rod and using an electric hammer  
pound it into the asphalt until it goes through the asphalt and  
chills into the soil. Total depth  $\approx 4"$ . Remove steel rod and place  
another rod  $\frac{5}{8}"$  diameter  $\approx 5'$  long into hole. This rod has a  
tapered tip w/ holes in the end. A long plastic hose is connected to the  
tip inside the rod and comes out on top. End of plastic hose is not covered.  
Drive this rod  $\approx 1-1.5$  feet into the ground w/ the electric hammer. Then  
pull the rod up a few inches to create an air pocket in the ground. See Fig 1.

Fig 1



Connect hose from rod to a glass jar and also  
hook up bicycle pump to the jar. Pump as  
used to see if there is a vacuum and that  
the top and hose are not clogged.



## Soil-Gas Survey Observation

Take a sample bag and place in a tupperware bowl and connect up hoses  
See Fig 2. Open valve on sampling bag and begin pumping very

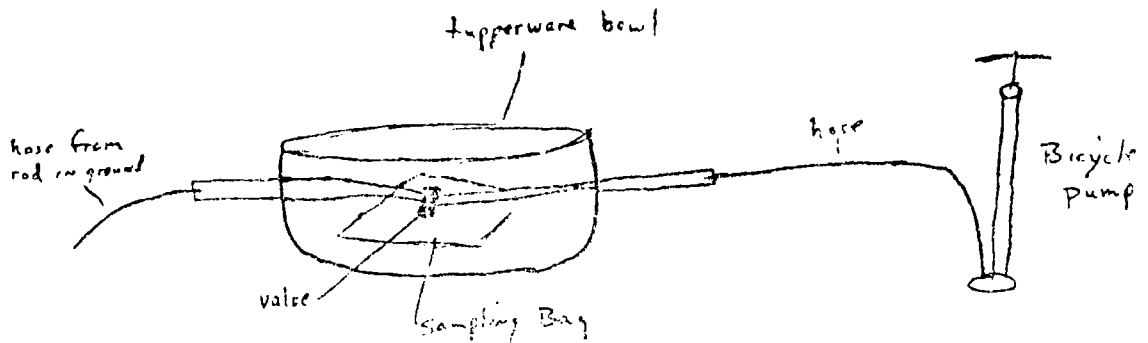


Fig 2. Sampling Bag Setup.

slowly. Tupperware bowl acts like a vacuum. Sample Bag fills up.  
Disconnect outside hose. Open bowl and close valve on bag and  
disconnect hoses. Take a background reading of H<sub>2</sub>O then take a reading  
of sample in the bag. Record and label sample bag.

Remove the rod from the ground. Clean off rod with paper towel.  
unscrew top on rod and disconnect hose. Clean top w/ alcohol (soapy water),  
distilled water, methanol and distilled water. Remaining liquids are not collected.  
Put top together. hook up hose to bottle and the bottle is hooked up to bicycle  
pump. Give pump a few pushes (10) to clear air in tube and top. You  
will see condensation in bottle. Remove tubing from bottle and begin  
process at next sampling point.

Pitney-Bowes  
March 24, 1988

(3)

- 9:15 A. NUS has finished three sampling locations for soil-gas.  
SG-17, 12 & 11. SG-17 (20 ppm / H<sub>2</sub>O)  
and are returning to their trailer.
- 9:30 A Return and take a few more soil-gas samples. A few ppm in a few  
holes but others were 0.
- 10:00 A Return to NUS trailer to get ground water sampling equipment.
- 10:30 A Setup for Sampling K-11 MW-150
- 12:15 P Break for Lunch
- 1:15 P NUS arrives at K-11A. Do not setup for sampling,  
waiting until K-11A samples 7 VOA = 3 MS/MSD samples.

Pitney - Bowes  
Zell Bldg  
Newville, CT  
4-14-88 ①  
C. DiMunzio

6:45 A Arrive on site, go for breakfast

7:00 A Drive back to Pitney-Bowes and speak with Vinny from Engine. They are on the 300' hole by the truck solar company. Hit a large fracture, vertical  $\approx 3'$  and is having trouble keeping enough water in the hole w/ the valves wide open. They have been on stand-by because last sample was 1000 ppm. <sup>(2185)</sup> Don't know if they (EPA) want to continue, possibly causing cross-contamination.

7:15 A. Back Engine Side crew show up and begin setting - next boring location, approx 3' East of K-148. Finished a boring last night approx 30' west of K-148. Auger refusal at 14.6' SB-1

7:50 A. NUS representative arrives at site. Engine Drilling begins taking split open samples. Don't know, taking samples for NUS. SB-2

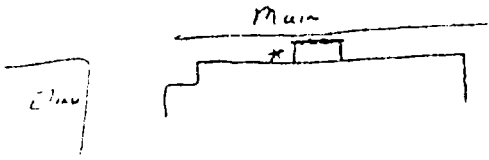
8:20 A Spoke w/ Scott Keath of NUS. Said they are just GW sampling, first boring and the trace study is what is left. They will be doing borings also on Monday. Asked about the other Engine drilling crew, & Scott said they were on stand-by. They are on the 300' hole at Westco and around 150' they hit a large fracture. EPA wants NUS to sample these fractures. The GW sample was analyzed & resulted in a reading of 1000 ppb TCE. Fracture zone 147-155' filled w/ calcite, pyrite. Debris below fracture is solid. Card band dropped shot over interval. First District in the Dept. has 7' to verify water level around the Zell Bldg.

P. Long - Boxes  
2-11 bldg ②  
4-14-88  
Normal/LET  
C. De/Mo/str:

9:50 A Finished boring and began to cement up borehole.

10:30 A Left site to steam clean tools, augers, etc.

11:15 A Settling up along Main St  
See star on map.



Go to lunch

12:30 p Begin the next soil boring. SB-3

1:30 p Finished taking the 10-12' spore

± 25 left site they were down to 20'

APPENDIX D

DAMES & MOORE FIELD SAMPLING RECORDS

**DAMES & MOORE**  
**FIELD SAMPLING RECORD**

Job \_\_\_\_\_ Date 2-26-88  
Job # 0262  
Well # 131V-104 Well Diameter 2"  
Location Pine Bush Newark CT

I Water level measurements (From Top of Casing) in Feet.  
Total Well Depth: 14.60  
Depth to Water: 8.35' Conversion Factor  
Height of Water Column: 6.25' 0.16 = 2" ID  
Volume Conversion Factor: 0.16 0.65 = 4" ID  
Gallons in the Well: 1.60 1.47 = 6" ID

II Well Evacuation  
Pumping: Submersible Bailing: Kemmerer  
Nitrogen (Stainless Steel)  
Centrifugal Bucket Bailer  
Other (Describe)  
Pump On: Bailing Started: 1:40 P  
Pump Off: Bailing Stopped: 1:45 P  
Pumping Time: Gallons Removed: 3 gal  
Pumping Rate:  
Gallons Removed:

III Sampling  
Withdrawal: Pumped (describe)  
Bailed (describe)  
Time: 1:52 P Date: 2-26-88  
Sample I.D. #: 131V-104  
Trip Blank #:  
No. of Containers Filled (p-rimary 12b.): 2  
No. of Containers Filled (replicate samples):     
Physical appearance and odor: No odor - slightly cloudy  
Refrigerated: Date: 2-26-88  
Time: 1:54 P

Field Tests (Before After Evacuation)  
Temperature (C / F) 4.5  
pH: 6.3  
Spec. Cond (umhos / cm) 430  
Dissolved oxygen (mg / l)  
Weather: Clear - 60 - 70 F  
Comments: Disinfectant residue present

DAMES & MOORE  
FIELD SAMPLING RECORD

Job \_\_\_\_\_ Date 2-20-88  
Job# 15264  
Well# 10W-175 Well Diameter 2"  
Location Pinon Bluffs - 20 W. 100 E.T.

I Water level measurements (From Top of Casing) in Feet.  
Total Well Depth: 16  
Depth to Water: 10.9 Conversion Factor  
Height of Water Column: 5.1' 0.16 = 2" ID  
Volume Conversion Factor: 0.16 0.65 = 4" ID  
Gallons in the Well: 0.816 1.47 = 6" ID

II Well Evacuation  
Pumping: Submersible Bailing: Kemmerer  
          Nitrogen                               : Stainless Steel  
          Centrifugal                            : Bucket Bailer  
  : Other (Describe)  
Pump On:   Bailing Started: 11:50 A  
Pump Off:                                        Bailing Stopped: 11:55 A  
Pumping Time:                                 Gallons Removed: 1.6  
Pumping Rate:                                   
Gallons Removed

III Sampling  
Withdrawal: Pumped (describe)  
              Bailed (describe) manual bailed  
Time: 12:10 P Date: 2-20-88  
Sample I.D. #: 10W-175  
Trip Blank #: \_\_\_\_\_  
No. of Containers Filled (primary lab.): 3  
No. of Containers Filled (replicate samples):     
Physical appearance and odor: clear, colorless  
Refrigerated: Date: 2-24-88  
                  Time: 12:14 P

Field Tests (Before After Evacuation)

Temperature (C / F) \_\_\_\_\_ °C  
pH: 7.5  
Spec. Cond (umhos / cm) : 530  
Dissolved oxygen (mg / l) \_\_\_\_\_

Weather cloudy  
Comments \_\_\_\_\_

**DAMES & MOORE  
FIELD SAMPLING RECORD**

Job \_\_\_\_\_ Date 2-26-88  
Job# 15262  
Well# HW-101 Well Diameter 2"  
Location Prince Georges County, MD

I Water level measurements (From Top of Casing) in Feet.

Total Well Depth: 35.1  
Depth to Water: 28.41' Conversion Factor  
Height of Water Column: 6.19 0.16 = 2" ID  
Volume Conversion Factor: 0.16 0.65 = 4" ID  
Gallons in the Well: 0.9964 1.47 = 6" ID

II Well Evacuation

Pumping: Submersible Bailing: Kemmerer  
Nitrogen : Stainless Steel  
Centrifugal : Bucket Bailer  
Other (Describe) \_\_\_\_\_  
Pump On : Bailing Started: 12:25 P  
Pump Off : Bailing Stopped: 12:30 P  
Pumping Time : Gallons Removed: 3.57 gal  
Pumping Rate :  
Gallons Removed:

III Sampling

Withdrawal: Pumped (describe)  
Bailed (describe) 4.2' 5.1'  
Time: 12:35 P Date: 2-26-88  
Sample I.D. #: \_\_\_\_\_  
Trip Blank #: \_\_\_\_\_  
No. of Containers Filled (primary lab.): 3  
No. of Containers Filled (replicate samples):      
Physical appearance and odor: clear, colorless, no odor  
Refrigerated: Date: 2-26-88  
Time: 12:40 P

Field Tests (Before After Evacuation)

Temperature (C / F) 4.9 C  
pH:      
Spec. Cond. (umhos / cm) 45  
Dissolved oxygen (mg / l)      
Weather cloudy, 50-55 F, NW breeze  
Comments \_\_\_\_\_



**DAMES & MOORE  
FIELD SAMPLING RECORD**

Job \_\_\_\_\_ Date 2-26-88  
 Job# 15242  
 Well# 1111-100 Well Diameter 2"  
 Location Salmon River, Marshall, CT

I Water level measurements (From Top of Casing) in Feet.  
 Total Well Depth: 46.1  
 Depth to Water: 28.15' Conversion Factor  
 Height of Water Column: 17.95' 0.16 = 2" ID  
 Volume Conversion Factor: 0.16 0.65 = 4" ID  
 Gallons in the Well: 3.0 1.47 = 6" ID

II Well Evacuation  
 Pumping: Submersible Bailing: Kemmerer  
           Nitrogen : Stainless Steel  
           Centrifugal : Bucket Bailer  
                               : Other (Describe)  
 Pump On: Bailing Started: 1:05 P  
 Pump Off: Bailing Stopped: 1:20 P  
 Pumping Time: Gallons Removed 90  
 Pumping Rate:  
 Gallons Removed:

III Sampling  
 Withdrawal: Pumped (describe)  
               Bailed (describe)  
 Time: 1:20 P Date: 2-26-88  
 Sample I.D. #: 1111-100  
 Trip Blank #: None  
 No. of Containers Filled (primary lab.): 2  
 No. of Containers Filled (replicate samples):     
 Physical appearance and odor: W. color light brown sandy  
 Refrigerated: Date: 2-26-88  
                   Time: 1:25 P

Field Tests (Before After Evacuation)  
 Temperature (C / F) 4°C  
 pH: 7  
 Spec. Cond. (umhos / cm) 150  
 Dissolved oxygen (mg / l)  
 Weather: Overcast, mild breeze  
 Comments: \_\_\_\_\_



# DAMES & MOORE FIELD SAMPLING RECORD

Job # 1-12-87 - 8117 Date 3-24-88  
 Job # 15-241-002  
 Well # K-17H Well Diameter 2"  
 Location Newark, CT

I Water level measurements (From Top of Casing) in Feet.  
 Total Well Depth: 18.1'  
 Depth to Water: 16.05' Conversion Factor  
 Height of Water Column: 6.35' 0.16 = 2" ID  
 Volume Conversion Factor: 0.16 0.65 = 4" ID  
 Gallons in the Well: 1.02 gal 1.47 = 6" ID

II Well Evacuation  
 Pumping: Submersible Bailing: Kemmerer  
           Nitrogen Stainless Steel  
           Centrifugal : Bucket Bailer  
                             : Other (Describe)  
 Pump On: Bailing Started: 1:40 P  
 Pump Off: Bailing Stopped: 1:55 P  
 Pumping Time: Gallons Removed: 6 gal  
 Pumping Rate:  
 Gallons Removed:

III Sampling  
 Withdrawal: Pumped (describe)  
                    Bailed (describe) : 1 bucket  
 Time: 1:55 P Date: 3-24-88  
 Sample ID #: K-17A  
 Trip Blank #: \_\_\_\_\_  
 No. of Containers Filled (primary lab.): 6 7  
 No. of Containers Filled (replicate samples): \_\_\_\_\_  
 Physical appearance and odor: no odor  
 Refrigerated: Date: \_\_\_\_\_ light br/tan  
 Time: 2:00 P

Field Tests (Before/After Evacuation)  
 Temperature (C / F): 19 C  
 pH: 6.0  
 Spec. Cond (umhos / cm): 240  
 Dissolved Oxygen (mg / l): \_\_\_\_\_  
 Weather: Cloudy  
 Comments: Water level 1.02 gal

DAMES & MOORE  
FIELD SAMPLING RECORD

Job Ph. 202 - 6000 Date 3-24-88  
Job # 15-202-015  
Well # MN-100 Well Diameter 2"  
Location Norwalk, CT

I Water level measurements (From Top of Casing) in Feet.  
Total Well Depth: 47'  
Depth to Water: 24.5' Conversion Factor  
Height of Water Column: 18.5' 0.16 = 2" ID  
Volume Conversion Factor: 0.16 0.65 = 4" ID  
Gallons in the Well: 2.45 1.47 = 6" ID

II Well Evacuation  
Pumping: Submersible Bailing: Kemmerer  
Nitrogen Stainless Steel  
Centrifugal : Bucket Bailer  
: Other (Describe)  
Pump On: Bailing Started: 11:30 A  
Pump Off: Bailing Stopped: 12:05 P  
Pumping Time: Gallons Removed 10 gal  
Pumping Rate:  
Gallons Removed:

III Sampling  
Withdrawal: Pumped (describe)  
Bailer (describe) w/ hand held bailer  
Time: 12:10 P Date: 3-24-88  
Sample I.D. #: MN-100  
Trip Blank #:  
No. of Containers Filled (primary lab.): 3  
No. of Containers Filled (replicate samples):  
Physical appearance and odor: Clear from water  
Refrigerated: Date: 3-24-88  
Time: 12:15 P

Field Tests (Before After Evacuation)

Temperature (C / F) 20°C  
pH 6.70  
Sp. Cond (umhos / cm) 340  
Dissolved oxygen (mg / l)

Weather Cloudy 8-10 110-65  
Comments: \_\_\_\_\_

DAMES & MOORE  
FIELD SAMPLING RECORD

Job 10 Hwy - Bents Date 3-24-88  
Job # 15742-005  
Well # K-11 Well Diameter 2"  
Location Norwalk, CT

I Water level measurements (From Top of Casing) in Feet.  
Total Well Depth: 38.5'  
Depth to Water: 37.5' Conversion Factor  
Height of Water Column: 10.7' 0.16 = 2" ID  
Volume Conversion Factor: 0.16 0.65 = 4" ID  
Gallons in the Well: 1.71 1.47 = 6" ID

II Well Evacuation  
Pumping: Submersible Bailing: Kemmerer  
Nitrogen Stainless Steel  
Centrifugal : Bucket Bailer  
: Other (Describe)  
Pump On: Bailing Started: 11:05A  
Pump Off: Bailing Stopped: 11:20A  
Pumping Time: Gallons Removed 9.7 gal  
Pumping Rate:  
Gallons Removed:

III Sampling  
Withdrawal: Pumped (describe)  
Bailed (describe) By hand 100 ml beaker  
Time: 11:25A Date: 3-24-88  
Sample I.D. #: K-11  
Trip Blank #:  
No. of Containers Filled (primary lab.): 3  
No. of Containers Filled (replicate samples): -  
Physical appearance and odor: medium brown no odor  
Refrigerated: Date: 3-24-88  
Time: 11:30A

Field Tests (Before After Evacuation)

Temperature (C / F) 20°C  
pH: 6.50  
Spec Cond (umhos / cm) 230  
Dissolved oxygen (mg / l)  
Weather cloudy - overcast 60-65°F  
Comments Very silty

DAMES & MOORE  
FIELD SAMPLING RECORD

Job Philly Blvd Date 3-24-88  
Job # 15262-C02  
Well # K-19B Well Diameter 2"  
Location Lawrence CT

I Water level measurements (From Top of Casing) in Feet.  
Total Well Depth: 36.5  
Depth to Water: 10.7' Conversion Factor  
Height of Water Column: 25.8' 0.16 = 2" ID  
Volume Conversion Factor: 0.16 0.65 = 4" ID  
Gallons in the Well: 4.13 gal 1.47 = 6" ID

II Well Evacuation  
Pumping: Submersible Bailing: Kemmerer  
Nitrogen: Stainless Steel  
Centrifugal: Bucket Bailer  
Other (Describe):  
Pump On: Bailing Started: 2:25 P  
Pump Off: Bailing Stopped: 2:50 P  
Pumping Time: Gallons Removed: 12 gal  
Pumping Rate:  
Gallons Removed:

III Sampling  
Withdrawal: Pumped (describe)  
Bailed (describe) and bailed / filtered back  
Time: 2:55 P Date: 3-24-88  
Sample I.D. #: K-19B  
Trip Blank #:  
No. of Containers Filled (primary lab.): 3  
No. of Containers Filled (replicate samples): -  
Physical appearance and odor: no odor clear sample  
Refrigerated: Date: 3-24-88  
Time: 3:00 P

Field Tests (Before After Evacuation)

Temperature (C / F) 19 C  
pH: 7.1  
Spec Cond (umhos / cm) 320  
Dissolved oxygen (mg / l)  
Weather: Partly Cloudy 50-65 F  
Comments:

## DAMES & MOORE CHAIN-OF-CUSTODY RECORD

Sample Source & Client <i>Pitney Bowes Merwalk CT</i>						Field Personnel (Signature) <i>Dugan (A.M. West)</i>					
Project Title <i>Kellogg - Dewatering Project</i>						Job No. <i>15262-005</i>					
Date	Time	Sample I.D. No.	Sample Type	No. of Containers	Sampling Site	Remarks					
<i>3/27/88</i>	<i>11:25 A</i>	<i>K-11</i>	<i>GW</i>	<i>3</i>	<i>K-11</i>	<i>TCL test</i>					
<i>3/27/88</i>	<i>12:10 P</i>	<i>MW-100</i>	<i>GW</i>	<i>3</i>	<i>MW-100</i>	<div style="display: flex; justify-content: space-around; width: 100%;"> <div style="text-align: center;">↓</div> <div style="text-align: center;">↓</div> </div>					
<i>3/27/88</i>	<i>1:55 P</i>	<i>K-19A</i>	<i>GW</i>	<i>3</i>	<i>K-19A</i>						
<i>3/27/88</i>	<i>2:55 P</i>	<i>K-19B</i>	<i>GW</i>	<i>3</i>	<i>K-19B</i>						
<i>3/27/88</i>	<i>1:55 P</i>	<i>MS/MSD</i>	<i>GW</i>	<i>3</i>							
Relinquished by:	Date	Time	Received by:	Date	Time	Relinquished by:	Date	Time	Received by:	Date	Time
<i>(Signature)</i>			<i>(Signature)</i>			<i>(Signature)</i>			<i>(Signature)</i>		
<i>(Signature)</i>	<i>3-24-88</i>	<i>3:05P</i>	<i>(Signature)</i>			<i>(Signature)</i>			<i>(Signature)</i>		
Relinquished by:	Date	Time	Received by:	Date	Time	Relinquished by:	Date	Time	Received by:	Date	Time
<i>(Signature)</i>			<i>(Signature)</i>			<i>(Signature)</i>			<i>(Signature)</i>		
Relinquished by:	Date	Time	Received by:	Date	Time	Relinquished by:	Date	Time	Received by:	Date	Time
<i>(Signature)</i>			<i>(Signature)</i>			<i>(Signature)</i>			<i>(Signature)</i>		

CHAIN OF CUSTODY RECORD

PROJ NO		PROJECT NAME				NO. OF CONTAINERS	REMARKS				
SAMPLERS (Signature)											
SEA. NO.	DATE	TIME	COMP.	STATION	LOCATION						
	Kellogg Decring										
Mark A. Mangel											
K-11	3/24	1120	X	KDM	K-11	3	3				
MW-100	3/24	1205	X		MW-100	3	3				
K-19A	3/24	1350	X		K-19A	6	6				
K-19B	3/24	1450	X		K-19B	3	3				
Relinquished by: (Signature)		Date / Time		Received by: (Signature)		Relinquished by: (Signature)		Date / Time		Received by: (Signature)	
Mark A. Mangel		3/24/88 1455		Darryl R. Hunt							
Relinquished by: (Signature)		Date / Time		Received by: (Signature)		Relinquished by: (Signature)		Date / Time		Received by: (Signature)	
Relinquished by: (Signature)		Date / Time		Received for Laboratory by: (Signature)		Date / Time		Remarks			
								split samples collected for Dames & Moore.			

Distribution: Original Accompanies Shipment, Copy to Coordinator Field Files



DAMES & MOORE  
FIELD SAMPLING RECORD

Job Pitney-Bowes

Date 3-29-88

Job # 15202

Well # MW-104

Well Diameter 2"

Location Pitney-Bowes  
Well Building

I Water level measurements (From Top of Casing) in Feet.

Total Well Depth: 14.6'

Depth to Water: 8.7'

Height of Water Column: 5.9'

Volume Conversion Factor: 0.16

Gallons in the Well: .94

Conversion Factor

0.16 = 2" ID

0.65 = 4" ID

1.47 = 6" ID

II Well Evacuation

Pumping: Submersible  
Nitrogen  
Centrifugal

Bailing: Kemmerer

Stainless Steel

: Bucket Bailer

: Other (Describe)

Pump On:

Bailing Started: 2:25 P

Pump Off:

Bailing Stopped: 2:35

Pumping Time:

Gallons Removed: 3 gal

Pumping Rate:

Gallons Removed:

III Sampling

Withdrawal: Pumped (describe)

Bailed (describe) 3rd 4th Bailer

Time: 2:35 P

Date: 3-29-88

Sample I.D. #: MW-104

Trip Blank:

No. of Containers Filled (primary lab.): 3

No. of Containers Filled (replicate samples):   

Physical appearance and odor:

Refrigerated: Date: 3-29-88

Time: 2:40 P

Field Tests (Before After Evacuation)

Temperature (C / F) 13

pH: 7.1

Spec Cond (umhos / cm)

Dissolved oxygen (mg / l)

Weather Sunny 70°

Comments

**DAMES & MOORE  
FIELD SAMPLING RECORD**

Job Pitts - Bows Date 3-29-88  
 Job# 15262  
 Well# MW-105 Well Diameter 2"  
 Location Pitts-Bows Normal CT  
Cell Building

I Water level measurements (From Top of Casing) in Feet.  
 Total Well Depth: 11.0'  
 Depth to Water: 12.2' Conversion Factor  
 Height of Water Column: 3.7' 0.16 = 2" ID  
 Volume Conversion Factor: 0.16 0.65 = 4" ID  
 Gallons in the Well: .59 1.47 = 6" ID

II Well Evacuation  
 Pumping: Submersible Bailing: Kemmerer  
Nitrogen : Stainless Steel  
Centrifugal : Bucket Bailer  
: Other (Describe)  
 Pump On: Bailing Started: 2.05 P  
 Pump Off: Bailing Stopped: 2.10 P  
 Pumping Time: Gallons Removed 2 gal.  
 Pumping Rate:  
 Gallons Removed:

III Sampling  
 Withdrawal: Purged (describe)  
Bailed (describe) 0.5 liter bailer  
 Time: 2.10 P Date: 3-29-88  
 Sample I.D. #: MW-105  
 Trip Blank #:  
 No. of Containers Filled (primary lab.): 2  
 No. of Containers Filled (replicate samples): -  
 Physical appearance and odor: Brown in color, no odor, sand, silt and  
 Refrigerated: Date: 3-29-88 small pebbles.  
 Time: 2.15 P

Field Tests (Before After Evacuation)  
 Temperature (C / F) 12°C  
 pH: 7.0  
 Spec Cond (umhos / cm) N/C  
 Dissolved oxygen (mg / l)  
 Weight: 1.00 g 20°C  
 Comments: \_\_\_\_\_

# DAMES & MOORE FIELD SAMPLING RECORD

Job Pitney-Bowes Date 3-29-88  
 Job# 14762  
 Well# MW-101 Well Diameter 2"  
 Location Normal CT Pitney-Bowes  
                   Cell Building

I Water level measurements (From Top of Casing) in Feet  
 Total Well Depth: 35.1  
 Depth to Water: 29.6 Conversion Factor  
 Height of Water Column: 5.3 0.16 = 2" ID  
 Volume Conversion Factor: 0.16 0.65 = 4" ID  
 Gallons in the Well: .85 gal 1.47 = 6" ID

II Well Evacuation  
 Pumping: Submersible Bailing: Kemmerer  
                   Nitrogen Stainless Steel  
                   Centrifugal : Bucket Bailer  
   : Other (Describe)  
 Pump On: Bailing Started: 1:35 P  
 Pump Off: Bailing Stopped: 1:45  
 Pumping Time: Gallons Removed 3 gal  
 Pumping Rate:  
 Gallons Removed:

III Sampling  
 Withdrawal: Pumped (describe)  
                   Bailed (describe) Submersible Bailer  
 Time: 1:50 P Date: 3-29-88  
 Sample I.D. #: MW-101  
 Trip Blank #:  
 No. of Containers Filled (primary lab.): 3  
 No. of Containers Filled (replicate samples):     
 Physical appearance and odor: Two in color have silt  
 Refrigerated: Date: 3-29-88  
                   Time: 1:58 P

Field Tests (Before After Evacuation)

Temperature (C / F) 16.5  
 pH: 8  
 Spec Cond (umhos / cm) 150  
 Dissolved Oxygen (mg / l)

Weather \_\_\_\_\_  
 Comments TRAP

**GAMES & MOORE  
FIELD SAMPLING RECORD**

Job 2000-10-100 Date 3-29-88  
 Job # 2000-100  
 Well # 2000-100 Well Diameter 2"  
 Location P. H. & B. Co. Farm, Wall, CT  
Bell Building

I Water level measurements (From Top of Casing) in Feet.  
 Total Well Depth: 46.9  
 Depth to Water: 28.7 Conversion Factor  
 Height of Water Column: 18.2 0.16 = 2" ID  
 Volume Conversion Factor: 0.16 0.65 = 4" ID  
 Gallons in the Well: 2.91 gal 1.47 = 6" ID

II Well Evacuation  
 Pumping: Submersible Bailing: Kemmerer  
Nitrogen Stainless Steel  
Centrifugal : Bucket Bailor  
 : Other (Describe)  
 Pump On: Bailing Started: 12:45 P  
 Pump Off: Bailing Stopped: 1:05 P  
 Pumping Time: Gallons Removed: 3 gal  
 Pumping Rate:  
 Gallons Removed:

III Sampling  
 Withdrawal: Pumped (describe)  
Bailed (describe) 2000-100  
 Time: 1:10 P Date: 3-29-88  
 Sample I.D. #: 2000-100  
 Trip Blank #:  
 No. of Containers Filled (primary lab.): 1  
 No. of Containers Filled (replicate samples): 0  
 Physical appearance and odor: no odor  
 Refrigerated. Date: 3/29/88  
 Time: 1:15 P

Field Tests (Before After Evacuation)  
 Temperature (C / F) 7.5 C  
 pH: 7.7  
 Spec Cond (umhos / cm) LLC  
 Dissolved Oxygen (mg / l)  
 Weather: Cloudy  
 Comments: 1-10-14

**DAMES & MOORE CHAIN-OF-CUSTODY RECORD**

Sample Source & Client <i>Pitney Bowes</i>						Field Personnel (Signature)
Project Title <i>Pitney Bowes Zell Building</i>					Job No. <i>15262</i>	
Date	Time	Sample I.D. No.	Sample Type	No. of Containers	Sampling Site	Remarks
<i>3-27-88</i>	<i>1:10P</i>	<i>MW-100</i>	<i>GW</i>	<i>6</i>	<i>MW-100</i>	<i>MS/MSO TSC UGA CLP Delimitation</i>
<i>3-27-88</i>	<i>1:50P</i>	<i>MW-101</i>	<i>GW</i>	<i>3</i>	<i>MW-101</i>	
<i>3-27-88</i>	<i>2:10P</i>	<i>MW-105</i>	<i>GW</i>	<i>3</i>	<i>MW-105</i>	
<i>3-27-88</i>	<i>2:50P</i>	<i>MW-104</i>	<i>GW</i>	<i>3</i>	<i>MW-104</i>	

Relinquished by: (Signature)	Date	Time	Received by: (Signature)	Date	Time	Relinquished by: (Signature)	Date	Time	Received by: (Signature)	Date	Time
Relinquished by: (Signature)	Date	Time	Received by: (Signature)	Date	Time	Relinquished by: (Signature)	Date	Time	Received by: (Signature)	Date	Time
Relinquished by: (Signature)	Date	Time	Received by: (Signature)	Date	Time	Relinquished by: (Signature)	Date	Time	Received by: (Signature)	Date	Time

DAMES & MOORE  
FIELD SAMPLING RECORD

Client PITNEY BOWES Date 12-23-82  
Job# 15-262  
Well # 10W-100 Well Diameter 2"  
Location 25-4 Clark, NORWALK, CT

I. Water level measurements (from top of casing) in Feet.

Total Well Depth :  
Depth to Water : Conversion Factor  
Height of Water Column : 0.16 = 2" ID  
Volume Conversion Factor : 0.65 = 4" ID  
Gallons in the Well : 1.47 = 6" ID

II. Well Evacuation

Pumping : Submersible Bailing : Teflon  
Positive Displacement : Stainless Steel  
Centrifugal : Combination  
: Other (Describe)

Pump On : Bailing Started :  
Pump Off : Bailing Stopped :  
Pumping Time : Gallons Removed :  
Pumping Rate :  
Gallons Removed :

III. Sampling

Withdrawal : Pumped (describe)  
Bailed (describe)

Time : Date :  
Sample I.D. #:  
Trip Blank #:  
No. of Containers Filled (primary lab.) :  
No. of Containers Filled (replicate samples) :  
Physical appearance and odor :  
Refrigerated : Date :  
Time :  
Field Tests (Before After Evacuation)  
Temperature ( °C / °F ) :  
pH  
Spec. Cond. (umhos / cm ) :

Weather: \_\_\_\_\_

Comments: CONDUCTED WITH LUSK & P. SC/

DAMES & MOORE  
FIELD SAMPLING RECORD

Client PITNEY BOWES Date 12-23-88  
Job# 15252  
Well # MW-104 Well Diameter 2"  
Location 2500 Bldg, NORWALK, CT

I. Water level measurements (from top of casing) in Feet.

Total Well Depth : 14.60'  
Depth to Water : 1.20' Conversion Factor  
Height of Water Column : 3.0' 0.16 = 2" ID  
Volume Conversion Factor : 0.25 0.65 = 4" ID  
Gallons in the Well : 0.547 1.47 = 6" ID

II. Well Evacuation

Pumping : Submersible Bailing : Teflon  
Positive Displacement : Stainless Steel  
Centrifugal  : Combination  
: Other (Describe)

Pump On : Bailing Started :  
Pump Off : Bailing Stopped :  
Pumping Time : Gallons Removed : 2 gallons  
Pumping Rate :  
Gallons Removed :

III. Sampling

Withdrawal : Pumped (describe)

Bailed (describe) 5 gal. BAILER W/TEFLON VALVE

Time : 1:00 PM Date : 12-23-88  
Sample I.D. # : MW-104  
Trip Blank # : N/A  
No. of Containers Filled (primary lab.) : 3  
No. of Containers Filled (replicate samples) : N/A  
Physical appearance and odor : NO ODOOR, S-LTY  
Refrigerated : Date : 12-23-88  
Time : 2:00 PM

Field Tests (Before/After Evacuation)

Temperature ( °C / °F ) : 15.00 / 59  
pH : 7.5  
Spec. Cond. (umhos / cm ) : 302

Weather: (Partly) BARRY

Comments: \_\_\_\_\_

DAMES & MOORE  
FIELD SAMPLING RECORD

Client PITNEY BOWES Date 12-23-88  
Job# 15262  
Well # MW-105 Well Diameter 2"  
Location ZELL BLDG., NORWALK, CT

I. Water level measurements (from top of casing) in Feet.  
Total Well Depth : 15.90  
Depth to Water : 13.90 Conversion Factor  
Height of Water Column : 2' 0.16 = 2" ID  
Volume Conversion Factor : 0.16 0.65 = 4" ID  
Gallons in the Well : 0.32 1.47 = 6" ID

II. Well Evacuation  
Pumping : Submersible Bailing : Teflon  
Positive Displacement : Stainless Steel  
Centrifugal : Combination  
Other (Describe)

Pump On : Bailing Started :  
Pump Off : Bailing Stopped :  
Pumping Time : Gallons Removed : 1 gallon  
Pumping Rate :  
Gallons Removed :

III. Sampling  
Withdrawal : Pumped (describe)  
Bailed (describe) 3.0 GALLES W/ TEFLON CK. VALVE

Time : 11:00 AM Date : 12-23-88  
Sample I.D. #: MW-105  
Trip Blank #: N/A  
No. of Containers Filled (primary lab.) : 3  
No. of Containers Filled (replicate samples) : N/A  
Physical appearance and odor : NO ODOOR, SLT  
Refrigerated : Date : 12-23-88  
Time : 2:00 PM

Field Tests (Before/After Evacuation)  
Temperature (°C/°F) : 15°C  
pH : 6.8  
Spec. Cond. (µmhos/cm) : 243

Weather: CLOUDY, CALM

Comments: \_\_\_\_\_



DAMES & MOORE  
FIELD SAMPLING RECORD

Client RITNEY BLUES Date 12-23-88  
Job# 15262  
Well # MW-101 Well Diameter 2"  
Location ZELL GARD, HARTFORD, CT

I. Water level measurements (from top of casing) in Feet.

Total Well Depth : 25.0  
Depth to Water : 23.95 Conversion Factor  
Height of Water Column : 1.05 0.16 = 2" ID  
Volume Conversion Factor : 0.65 0.65 = 4" ID  
Gallons in the Well : 0.112 1.47 = 6" ID

II. Well Evacuation

Pumping : Submersible Bailing : Teflon  
Positive Displacement : Stainless Steel  
Centrifugal  Combination  
Other (Describe) :

Pump On : Bailing Started :  
Pump Off : Bailing Stopped :  
Pumping Time : Gallons Removed : 2 gallons  
Pumping Rate :  
Gallons Removed :

III. Sampling

Withdrawal : Pumped (describe)

Bailed (describe) S.S. GALER W/ 1/2" FLOR CN VALVE

Time : 1:10 PM Date :  
Sample I.D. # : MW-101  
Trip Blank # : N/A  
No. of Containers Filled (primary lab.) : 3  
No. of Containers Filled (replicate samples) : N/A  
Physical appearance and odor : NO ODOUR, SILTY  
Refrigerated : Date : 12-23-88

Time : 2:00 PM  
Field Tests (Before After Evacuation)  
Temperature ( °C / °F ) : 14°C  
pH : 6.7  
Spec. Cond. (umhos / cm ) : 24

Weather: RAINY, CLOUDY

Comments: \_\_\_\_\_

LYMAN'S FISH  
BY MARY LYMAN

TABLE OF DESCRIPTION

FOR CLIENT USE			QTY	WEIGHT	LENGTH	GIRTH	SEX	AGE	MARKS	REMARKS
SAMPLE TO	DATE	SAMPLE #								
105	1/1/81	1	1	1.0	1.0	1.0				
106	1/1/81	2	1	1.0	1.0	1.0				
107	1/1/81	3	1	1.0	1.0	1.0				
108	1/1/81	4	1	1.0	1.0	1.0				
109	1/1/81	5	1	1.0	1.0	1.0				
110	1/1/81	6	1	1.0	1.0	1.0				
111	1/1/81	7	1	1.0	1.0	1.0				
112	1/1/81	8	1	1.0	1.0	1.0				
113	1/1/81	9	1	1.0	1.0	1.0				
114	1/1/81	10	1	1.0	1.0	1.0				
115	1/1/81	11	1	1.0	1.0	1.0				
116	1/1/81	12	1	1.0	1.0	1.0				
117	1/1/81	13	1	1.0	1.0	1.0				
118	1/1/81	14	1	1.0	1.0	1.0				
119	1/1/81	15	1	1.0	1.0	1.0				
120	1/1/81	16	1	1.0	1.0	1.0				
121	1/1/81	17	1	1.0	1.0	1.0				
122	1/1/81	18	1	1.0	1.0	1.0				
123	1/1/81	19	1	1.0	1.0	1.0				
124	1/1/81	20	1	1.0	1.0	1.0				

Lab Use Only REF: \_\_\_\_\_

RECEIVED BY: Robert Brown DATE: 1/1/81 TIME: 11:00 CUSTODY TRANSFERRED TO: Robert Brown SEX:   

INITIALS: Robert Brown COVER SERIALIZED: YES NO    SERIAL #: 1228501 SIGNATURE: \_\_\_\_\_

RECEIVED BY: Robert Brown DATE: 1/1/81 TIME: 11:00 COVER SERIALIZED BY: Robert Brown SERIAL #: 1228501 SIGNATURE: \_\_\_\_\_

INITIALS: Robert Brown SERIAL #: 1228501 SIGNATURE: \_\_\_\_\_

RECEIVED BY: Robert Brown DATE: 1/1/81 TIME: 11:00 COVER SERIALIZED BY: Robert Brown SERIAL #: 1228501 SIGNATURE: \_\_\_\_\_

INITIALS: Robert Brown SERIAL #: 1228501 SIGNATURE: \_\_\_\_\_

DOES THIS SPECIES BEAR ANOTHER PARTY'S YES  NO

IF YES IDENTIFY: \_\_\_\_\_

COPIES TO: Robert Brown

APPENDIX E

KELLOGG-DEERING WELL FIELD SITE SUPPLEMENTAL RI, FIELD GC  
LABORATORY, GROUND WATER ANALYSIS RESULTS, SECOND ROUND

NO. 2

KELLOGG-DEERING WELL FIELD SITE  
SUPPLEMENTAL RI

FIELD GC LABORATORY

GROUNDWATER ANALYSIS RESULTS  
SECOND ROUND













KELLOGG DEERING  
ANALYTICAL RESULTS (in ug/l)

MOBILE LABORATORY  
PROJECT NUMBER L670

COMPOUND	TARGET DETECTION LIMITS							
		K1822	K1822	K1822	K1822	K1822	K1822	K1822
Carbon tetrachloride								
Chloroform								
1,1-Dichloroethane								
1,2-Dichloroethane								
1,1-Dichloroethene								
cis-1,2-dichloroethene								
Methylene chloride								
Tetrachloroethene			32000		115000			
1,1,1-Trichloroethane								
Trichloroethene		118000		113000				50
Vinyl chloride								
<b>EPA METHOD 602</b>								
Benzene						13	65	
Ethylbenzene						11	38	
Toluene					1200	31	400 J	
o-Xylene								
DILUTION		x1000	x50	x1000	x100	x1	x1	x1
DATE SAMPLED								
DATE ANALYZED		4-10-88	4-10-88	4-10-88	4-10-88	4-11-88	4-11-88	4-11-88
RUN NUMBER								
REPORT NUMBER								
COMMENTS								

KELLOGG DEERING  
ANALYTICAL RESULTS (in ug/l)

MOBILE LABORATORY  
PROJECT NUMBER L670

COMPOUND	TARGET DETECTION LIMITS							
		K21-93-5	K21-112	K202-0	K20A	K21(12/11/88)	K21(12/11/88)	K21(12/11/88)
Carbon tetrachloride								
Chloroform								
1,1-Dichloroethane								
1,2-Dichloroethane								
1,1-Dichloroethene								
cis-1,2-dichloroethene								
Methylene chloride								
Tetrachloroethene							17.	
1,1,1-Trichloroethane				418.	22.			
Trichloroethene					12.			1070.
Vinyl chloride								
EPA METHOD 802								
Benzene								
Ethylbenzene								
Toluene							9.	
o-Xylene								
DILUTION		x1	x1	x1	x1	x1	x1	x10
DATE SAMPLED								
DATE ANALYZED		4-11-88	4-12-88	4-12-88	4-12-88	4-13-88	4-13-88	4-13-88
RUN NUMBER								
REPORT NUMBER								
COMMENTS								

























**APPENDIX F**

**NUS SOIL SAMPLING DESCRIPTION**

Further discussion on the selection and application of the groundwater model is found in Section 4.5 Task 3 - Data Evaluation.

### 3.2.3 Zone 1 Investigation

Previous studies at the ELINCO property, within Zone 1, have confirmed the presence of TCE in soils adjacent to the building. The lateral and vertical extent of contaminated soils and the locations of these soil contamination areas are presently unknown. In addition, the history of waste disposal operations by Zeil Products Corporation (former occupants of the ELINCO and Pitney Bowes buildings), as well as those of Pitney Bowes and ELINCO, have not been defined. hrp Associates, Inc. indicated that floor drain discharges may have occurred from all three buildings on the property. Also, it is reported that disposal of spent solvents on the adjacent Mathais Court property may have occurred. The NUS FIP investigation lends some support to this allegation (see Section 2.3.5).

The general technical approach to source characterization within Zone 1 includes a ① preliminary external soil gas survey to identify areas warranting further study, ② a refined external soil gas survey to delineate the extent of sources, ③ an internal soil gas survey to identify subsurface soil contamination beneath the buildings, and ④ confirmatory soil boring, sampling and analysis.

A soil-gas survey for tetrachloroethene (PCE), trichloroethene (TCE), and 1,1,1-trichloroethane (TCA) will be conducted at the ELINCO/Pitney Bowes/Mathais Court complex. Previous investigations have shown that these are the primary contaminants in both soil and groundwater in this area. Soil-gas testing is proposed for the entire complex area, and includes sampling inside the ELINCO and Pitney Bowes buildings, as well as through the floor of the parking garage in the Mathais Court building.

The external soil-gas survey at the complex will be conducted in the following manner. The preliminary survey will be based on an unbiased 50-foot grid (Figure 3-2). The survey will commence in the area previously identified as a source of solvent contamination (i.e., behind the ELINCO Building). Data from this initial survey is expected to reveal the efficacy of the soil-gas survey. If inconclusive results are obtained in this known source area, the soil-gas survey will be abandoned, and a soil boring/sampling and analysis program will be implemented instead. If the soil-gas survey provides acceptable results (i.e., a demonstrated soil-gas concentration gradient), the 50-foot soil-gas grid will be completed.

The data from the preliminary survey will be reviewed and isoconcentration plots will be generated. Contaminant source areas should be identifiable, based on the isoconcentration plots. A refined soil-gas survey grid will be generated, and sampling will be conducted to further define source areas. As shown on Figure 3-2, 47 preliminary external soil-gas survey



points have been identified. The number of grid points included in the refined survey cannot be determined at this time. Four background soil samples will be obtained from the soil-gas borings to the east of the complex during drilling. These samples will be submitted for CLP TCL analysis. For costing purposes it has been assumed that 20 additional nodes will be specified. Soil-gas data will be used to focus a soil boring program. The soil-gas data corresponds to DQO Level II.

Once the external soil-gas survey is completed, soil boring will be conducted and subsurface soil samples will be collected at depths of approximately 3, 6, and 9 feet in contaminated areas. For costing purposes it has been estimated that approximately 20 such borings will be completed. Thus a total of 60 subsurface soil samples will be collected. These samples will also be analyzed for the selected volatile organics using the field GC (DQO Level II: engineering purposes). Selected samples will also be split and submitted for confirmatory Contract Laboratory Program (CLP) Target Compound List (TCL) Analysis (DQO Level IV: risk assessment and enforcement purposes). It is proposed that 25 percent of samples showing positive detections, and 25 percent of samples with no detectable analytes be submitted for confirmatory analysis. For costing purposes, it has been assumed that 15 samples will be submitted to CLP laboratories.

In addition to the external soil gas surveys and boring programs, an investigatory program has also been developed for the interior of the ELINCO and Pitney Bowes buildings. Interior soil gas and subsurface soil sampling will commence upon completion of the external soil-gas survey. Internal sampling will be biased; boring will be conducted near floor drains, dry wells, or sumps, or in areas that the external survey shows to be potentially contaminated (e.g., the portion of the ELINCO Building nearest the virgin PCB tank and drum storage areas). To minimize interruption of activities in the buildings, subsurface soil sampling will be conducted at the same time as the interior soil gas survey, via the same boreholes. It has been estimated that a total of 30 soil gas samples (10 per building) and 60 subsurface soil samples (2 per borehole) will be collected during the interior investigation. Soil gas and soil samples will be analyzed for PCE/TCE/DCA using the field GC (DQO Level II). It is anticipated that 15 subsurface soil samples will be submitted for CLP TCL analysis (DQO Level IV: enforcement support/risk assessment).

In the event that a preliminary reconnaissance of the buildings reveals the presence of sumps, etc. holding liquid materials, or the presence of potential contaminated surfaces, wipe or waste sampling may also be conducted. These potential activities are considered outside the present scope of work and would require additional funding. Such activities could be initiated through a Technical Direction Memorandum (TDM), if necessary. Any

APPENDIX G  
AQUIFER TESTING DATA

AQUIFER TESTING

DATA

USEPA	FILE NO.: L670	BY: JPD	PAGE 2 OF 2
SUBJECT: Kellogg Deering Slug Tests		CHECKED BY:	DATE: 6/14/88

$$T = \frac{1.0 r_c^2}{t} \quad (\text{Cooper, et. al, 1967})$$

### K6B Falling Head Test

$$T = \frac{1.0 (.25)^2}{.48}$$

$$= 0.13 \text{ ft}^2/\text{min}$$

$$= 187.5 \text{ ft}^2/\text{day}$$

$$r_c = .25 \text{ ft}$$

$$t = .48 \text{ min}$$

$$K = 187.5 \text{ ft}^2/\text{day} \div 18.2 \text{ ft (Open Interval)}$$

$$= 10.3 \text{ ft}/\text{day}$$

$$= 3.6 \times 10^{-3} \text{ cm}/\text{sec}$$

### K6B Rising Head Test

$$T = \frac{1.0 (.25)^2}{.9}$$

$$= .069 \text{ ft}^2/\text{min.}$$

$$= 100 \text{ ft}^2/\text{day}$$

$$r_c = .25 \text{ ft}$$

$$t = .9 \text{ min.}$$

$$K = 100 \text{ ft}^2/\text{day} \div 18.2 \text{ ft}$$

$$= 5.5 \text{ ft}/\text{day}$$

$$= 1.9 \times 10^{-3} \text{ cm}/\text{sec.}$$

CLIENT: USEPA	FILE NO.: L670	BY: JPO	PAGE 2 OF 2
SUBJECT: Kellogg Deering Slug Tests		CHECKED BY: DWB 6/20/88	DATE: 6/14/88

K 98 Falling Head Test

$$T = \frac{1.0 (r_c)^2}{t} \quad (\text{Cooper, et al., 1967})$$

$$T = \frac{1.0 (0.25)^2}{2.9} \quad \begin{array}{l} r_c = 0.25 \text{ FT} \\ t = 2.9 \text{ min} \end{array}$$

$$= 0.0216 \text{ ft}^2/\text{min}$$

$$= 31 \text{ ft}^2/\text{day}$$

$$K = 31 \text{ ft}^2/\text{day} \div 28.8 \text{ ft (open interval)}$$

$$= 1.1 \text{ ft/day}$$

$$= 3.8 \times 10^{-4} \text{ cm/sec}$$

K 98 Rising Head Test

$$T = \frac{1.0 (r_c)^2}{t} \quad (\text{Cooper, et al., 1967})$$

$$T = \frac{1.0 (0.25)^2}{1.9} \quad \begin{array}{l} r_c = 0.25 \text{ ft} \\ t = 1.9 \text{ min.} \end{array}$$

$$= 0.033 \text{ ft}^2/\text{min}$$

$$= 47.5 \text{ ft}^2/\text{day}$$

$$K = 47.5 \text{ ft}^2/\text{day} \div 28.8 \text{ ft (open interval)}$$

$$= 1.65 \text{ ft/day}$$

$$= 5.8 \times 10^{-4} \text{ cm/sec}$$

USEPA	FILE NO.: L670	BY: JPB	PAGE 1 OF 1
SUBJECT: Kellogg Deering Slug Tests K18A Falling Head Test		CHECKED BY: DWB 4/20/88	DATE: 6/17/88

TIME (MIN)	ΔH (FT)	H/H <sub>0</sub>
0	3.06	1
.1	2.93	.96
.7	2.90	.95
1.7	2.85	.93
5.4	2.65	.87
8.4	2.5	.82
18	2.06	.67
26	1.74	.57
32	1.52	.50
48	1.26	.41

$$T = \frac{1.0 r_c^2}{t}$$

(Cooper, et. al, 1967)

$$= 3.47 \times 10^{-4} \text{ ft}^2/\text{min}$$

$$= 0.5 \text{ ft}^2/\text{day}$$

$$r_c = .0833 \text{ FT}$$

$$t = 20 \text{ MIN}$$

$$K = 0.5 \text{ ft}^2/\text{day} \div 14.5 \text{ Feet (sand pack interval)}$$

$$K = .034 \text{ ft/day}$$

$$= 1.2 \times 10^{-5} \text{ cm/sec}$$

✓

✓

✓

✓

✓

✓

CLIENT USA	FILE NO. L670	BY: DWS	PAGE   OF   1   1
SUBJECT: Kellogg-Deering Packer Test KIBA		CHECKED BY: JFO 6/22/88	DATE: 6/21/88

$$K = c_p \frac{Q}{H}$$

Test Interval  
58.5 - 70.5

$$c_p = \frac{1}{2} \pi r L \ln\left(\frac{L}{r}\right) 70315$$

$$= 0.013 \times 4.56 \times 70315$$

$$= 4168$$

$$H = h_p + h_g$$

$$= (20 \times 2.31) + 16.4 \text{ ft}$$

$$= 62.6 \text{ ft}$$

$$Q = 4.76 \text{ gpm}$$

$$K = 4168 \times \frac{4.76}{62.6}$$

$$= 316.92 \text{ ft/yr}$$

$$= 0.868 \text{ ft/day}$$

or  $3.06 \times 10^{-4} \text{ cm/sec.}$

$L = 12'$   
 $r = 0.125'$   
 $PSI = 20$   
 ✓  
 ✓  
 ✓  
 ✓  
 ✓  
 ✓  
 ✓  
 ✓  
 ✓

CLIENT 125829	FILE NO. 11670	BY: DWB	PAGE	OF
SUBJECT Kellogg-Deering Packer Test KIBA		CHECKED BY: JAO 6/22/88	DATE 6/21/88	

$$K = C_p \frac{Q}{H} \quad C_p = \frac{1}{2\pi L} \ln\left(\frac{L}{r}\right) \quad 70315$$

$$= 0.013 \times 4.56 \times 70315$$

$L = 12'$   
 $r = 1.25'$   
 $PSE = 30$

Test Interval  
58.5 - 70.5

$$= 4168 \quad \checkmark$$

$$H = h_p + h_g$$

$$= (30 \times 2.31) + 16.4 \text{ Ft} \quad \checkmark$$

$$= 85.7 \text{ Ft} \quad \checkmark$$

$$Q = 7.43 \text{ gpm} \quad \checkmark$$

$$K = 4168 \times \frac{7.43}{85.7} \quad \checkmark$$

$$= 361.36 \text{ Ft/Yr} \quad \checkmark$$

$$= 0.99 \text{ Ft/day} \quad \checkmark$$

$$OR \quad 3.49 \times 10^{-4} \text{ cm/sec} \quad \checkmark$$



CLIENT USEPA FILE NO. L670 BY D.R.B.

 SUBJECT Kellogg-Deering Slug Tests Checked By JFO 6/7/88

K19A Rising Head Test (2)

Time (min)	$\Delta H$ (FT)	$\Delta H/H_0$	$H_0 = 1.93$
.01	1.76	.91	
.02	1.62	.84	
.06	1.18	.61	
.11	0.75	.39	
.16	0.45	.23	
.21	0.25	.13	
.26	0.14	.07	
.31	0.07	.04	

$$r = 0.0833 \text{ FT}$$

$$R = 0.0833 \text{ FT}$$

$$L = 4.13 \text{ FT}$$

$$T_0 = 0.107 \text{ min}$$

$$k = \frac{r^2 \ln(L/R)}{2L T_0} \quad (\text{Hvorslev, 1951})$$

$$= \frac{.027}{.834} \quad \checkmark$$

$$= .0305 \text{ FT/min} \quad \checkmark$$

$$= 1.83 \text{ FT/hr} \quad \checkmark$$

$$= 43.98 \text{ FT/day} \quad \checkmark$$

$$= 1.6 \times 10^{-2} \text{ cm/sec.} \quad \checkmark$$



CLIENT USEPA FILE NO. L670 BY DWB

SUBJECT Kellaga Deering Slug Tests Checked By JPO 6/17/88

K19A Rising Head Test (1)

Time (min)	AH (FT)	AH/H <sub>0</sub>	H <sub>0</sub> = 2.1
0	2.1	1	
.01	1.82	.87	
.02	1.70	.81	
.06	1.26	.60	
.11	0.83	.39	
.16	0.52	.25	
.31	0.31	.15	
.36	0.18	.09	
.31	0.10	.05	

$r = 0.0833$  FT  
 $R = 0.0833$  FT  
 $L = 4.13$  FT  
 $T_0 = 0.11$  MIN

$$K = \frac{r^2 \ln(L/R)}{2L T_0} \quad (\text{Dvors'ov, 1951})$$

$$= \frac{6.74 \cdot 10^{-3} \ln(50.48)}{.91} \quad \text{JPO} \quad \checkmark$$

$$= 0.03 \text{ Ft/min} \quad \checkmark$$

$$= 1.79 \text{ Ft/Hr} \quad \checkmark$$

$$= 43 \text{ Ft/day} \quad \checkmark$$

$$= 1.5 \times 10^{-2} \text{ cm/sec} \quad \checkmark$$

CLIENT: USEPA	FILE NO.: L670	BY: DLWB	PAGE 1 OF 1
SUBJECT: Kellogg-Doering Pacler Test K19B		CHECKED BY: JPO 6/20/88	DATE: 6/17/88

$$K = C_p \frac{Q}{H} \quad C_p = \frac{1}{4\pi rL} \ln\left(\frac{r}{r_0}\right) 70315$$

$$= 0.015 \times 4.45 \times 70315 \quad \checkmark$$

$$L = 10.7$$

$$r = 0.125$$

$$r_0 = 40 \quad \checkmark$$

TEST INTERVAL -  
25'-35.7'

$$= 4693.19 \quad \checkmark$$

$$H = h_p + h_g$$

$$= (40 \times 2.31) + (13.8 + 10.24) \quad \checkmark$$

$$= 92.4 + 24.04 \quad \checkmark$$

$$= 116.4 \text{ ft} \quad \checkmark$$

$$Q = 1.22 \text{ gpm} \quad \checkmark$$

$$K = 4693.19 \frac{1.22}{116.4} \quad \checkmark$$

$$= 49.19 \text{ ft/day} \quad \checkmark$$

$$= 0.13 \text{ ft/day} \quad \checkmark$$

$$\text{or } 4.75 \times 10^{-5} \text{ cm/sec} \quad \checkmark$$

CLIENT: USEPA	FILE NO.: L670	BY: DWB	PAGE 1 OF 1
SUBJECT: Kellogg-Deering Packer Test K19B		CHECKED BY: JPD 6/20/88	DATE: 6/17/88

$$K = c_p \frac{Q}{H} \quad c_p = \frac{1}{2\pi r L} \ln\left(\frac{L}{r}\right) 70315$$

$$= 0.015 \times 445 = 70315$$

L = 10.7  
r = 0.125  
PSI = 20 ✓

TEST INTERVAL - 25' - 35.7' = 4693.19 ✓

$$H = h_p + h_g$$

$$= 20 \times 2.31 + 24.04$$

$$= 46.2 + 24.04$$

$$= 70.24 \text{ Ft} \quad \checkmark$$

Q = 0.71 gpm ✓

$$K = 4693.19 \frac{0.71}{70.24} \quad \checkmark$$

$$= 47.44 \text{ Ft/Yr} \quad \checkmark$$

$$= 0.13 \text{ Ft/day} \quad \checkmark$$

$$\text{OR } 4.59 \times 10^{-5} \text{ cm/sec} \quad \checkmark$$



CLIENT USEPA FILE NO. L670 BY DWB

SUBJECT Kellogg-Deering Slug Tests Checked By JPO 6/17/88

K20 A Falling Head Test

Time (min)	ΔH (FT)	ΔH/H <sub>0</sub>	H <sub>c</sub> = 2.20
0	2.20	1	
.05	2.04	.93	
.1	1.91	.87	
.15	1.79	.81	
.2	1.69	.77	
.4	1.35	.61	
.65	1.03	.47	
.9	0.79	.36	
1.15	0.61	.28	
1.45	0.48	.22	
1.65	0.37	.17	
1.90	0.29	.13	
2.40	0.18	.08	
2.90	0.12	.05	

r = 0.0833 FT  
 R = 0.0833 FT  
 L = 10 FT  
 T<sub>0</sub> = 0.90 MIN

$$K = \frac{r^2 \ln L/R}{2 \cdot T_0} \quad (\text{Hvorslev, 1951})$$

$$= \frac{0.033}{18} \quad \checkmark$$

$$= 1.8 \times 10^{-3} \text{ FT/min} \quad \checkmark$$

$$= 0.11 \text{ FT/hr} \quad \checkmark$$

$$= 2.64 \text{ FT/day} \quad \checkmark$$

$$= 9.3 \times 10^{-4} \text{ cm/sec} \quad \checkmark$$

being filmed

CLIENT: USEPA	FILE NO.: L670	BY: DJR	PAGE 1 OF 1
SUBJECT: Kellogg-Deering Slug Tests		CHECKED BY: JPO 6/17/88	DATE: 6/16/88

K20A Rising Head Test

Time (min)	Δ H (FT)	ΔH/H <sub>0</sub>	H <sub>0</sub> = 2.75
0	2.75	1	
.04	2.47	.90	
.09	2.27	.83	
.14	2.12	.77	
.19	1.99	.72	
.24	1.87	.68	
.29	1.76	.64	
.64	1.20	.44	
.97	0.85	.31	
1.14	0.71	.26	
1.39	0.55	.20	
1.64	0.43	.16	
1.97	0.30	.11	
2.97	0.11	.04	

$r = 0.0833$  FT  
 $R = 0.0833$  FT  
 $L = 10$  FT  
 $T_0 = 0.82$  MIN

$$K = \frac{r^2 \ln(L/R)}{2L T_0} \quad (\text{Hvorslev, 1951})$$

$$= \frac{0.033}{16.4} \checkmark$$

$$= 2.03 \times 10^{-3} \text{ FT/Min} \checkmark$$

$$= 0.12 \text{ FT/HR} \checkmark$$

$$= 2.9 \text{ FT/day} \checkmark$$

$$= 1.0 \times 10^{-3} \text{ cm/sec} \checkmark$$

CLIENT: USEBA FILE NO.: L670 BY: DWB PAGE 1 OF 1

SUBJECT: Kellogg-Doering Packer Test K20B CHECKED BY: JPO 6/20/88 DATE: 6/17/88

$L = 12'$   
 $r = 0.125$   
 $PSI = 40$

$$K = C_p \frac{Q}{H} \quad C_p = \frac{1}{2\pi L} \ln\left(\frac{L}{r}\right) 70315 \quad \checkmark$$

$$= 0.013 \times 4.56 \times 70315 \quad \checkmark$$

TEST INTERVAL - 101' ✓  
113'

$$= 4172 \quad \checkmark$$

$$H = h_p + h_g \quad \checkmark$$

$$= (40 \times 2.31) + 20.5 + 10.5 \quad \checkmark$$

$$= 92.4 + 31 \quad \checkmark$$

$$= 123.4 \text{ FT} \quad \checkmark$$

$$Q = 0.24 \text{ gpm} \quad \checkmark$$

$$K = 4172 \frac{0.24}{123.4} \quad \checkmark$$

$$= 8.11 \text{ Ft/Yr} \quad \checkmark$$

$$= 0.022 \text{ Ft/day} \quad \checkmark$$

$$= 7.84 \times 10^{-6} \text{ cm/sec} \quad \checkmark$$

CLIENT: <u>US EPA</u>	FILE NO.: <u>L670</u>	BY: <u>DLSB</u>	PAGE 1 OF 1
SUBJECT: <u>Hallogg-Deering Packer Test K20B</u>		CHECKED BY: <u>JFO 6/20/88</u>	DATE: <u>6/17/88</u>

$$K = C_p \frac{Q}{H}$$

$$C_p = \frac{1}{2\pi r L} \ln\left(\frac{L}{r}\right) 70315$$

$$= 0.013 \times 4.56 \times 70315$$

$$L = 12'$$

$$r = 0.125$$

$$PSI = 60$$

TEST INTERVAL -  
101' - 113'

$$= 4172$$

$$H = h_p + h_g$$

$$= (60 \times 2.31) + 20.5 + 10.5$$

$$= 138.6 + 31$$

$$= 169.6 \text{ Ft}$$

$$Q = 0.32 \text{ gpm}$$

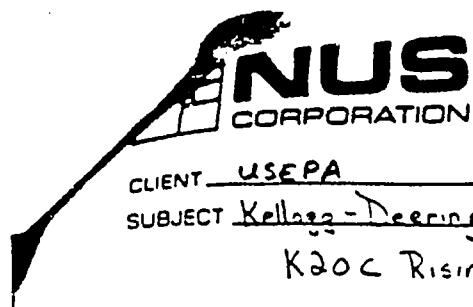
$$K = 4172 \frac{0.32}{169.6}$$

$$= 7.87 \text{ Ft/Yr}$$

$$= 0.022 \text{ Ft/day}$$

$$\text{OR } 7.6 \times 10^{-6} \text{ cm/sec}$$





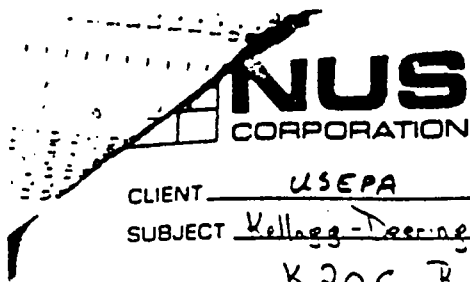
CLIENT USEPA FILE NO. L670 BY DWB  
 SUBJECT Kellogg - Deering Slug Tests Checked By JFO 6/17/88

K20C Rising Head Test (2)

Time (min)	ΔH (FT)	ΔH/H <sub>0</sub>	H <sub>0</sub> = 1.87
0	1.87	1	
.01	1.77	.95	
.05	1.53	.82	
.10	1.34	.72	
.15	1.18	.63	
.2	1.04	.56	
.25	.91	.49	
.30	.81	.43	
.40	.63	.33	
.57	.44	.24	
.65	.36	.19	

r = 0.0833 FT  
 R = 0.0833 FT  
 L = 3.87 FT  
 T<sub>0</sub> = 0.36 min

$$\begin{aligned}
 K &= \frac{r^2 \ln(L/R)}{2L T_0} \quad (\text{Hvorsler, 1951}) \\
 &= \frac{6.94^{-3} \times 3.87}{7.74 \times 0.36} \quad \checkmark \\
 &= 9.6 \times 10^{-3} \text{ FT/min} \quad \checkmark \\
 &= 0.57 \text{ FT/Hr} \quad \checkmark \\
 &= 13.77 \text{ FT/day} \quad \checkmark \\
 &= 4.9 \times 10^{-3} \text{ cm/sec} \quad \checkmark
 \end{aligned}$$



CLIENT USEPA FILE NO L670 BY JPO  
 SUBJECT Kellogg-Dearing Slug Tests Checked By JPO 6/17/88

K 20C Raising Head Test (1)  
 $H_0 = 1.96$

Time (min)	$\Delta H$ (FT)	$\Delta H/H_0$
0	1.96	1
0.01	1.78	.91
0.02	1.66	.85
0.07	1.33	.68
0.12	1.12	.57
0.17	0.95	.48
0.22	0.82	.42
0.32	0.60	.31

$r = 0.0833$  FT  
 $R = 0.0833$  FT  
 $L = 26.20 - 22.33$   
 $= 3.87$  FT  
 $T_0 = 0.23$  MIN

$$K = \frac{r^2 \ln(L/R)}{2L T_0} \quad (\text{Hvorslev, 1951})$$

$$= \frac{6.94^{-3} \ln(3.87/0.0833)}{7.74 \times 0.23} \quad \checkmark$$

$$= \frac{6.94^{-3} \times 3.84}{1.78} \quad \checkmark$$

$$= 0.015 \text{ FT/min} \quad \checkmark$$

$$= 0.90 \text{ JPO FT/Hr} \quad \checkmark$$

$$= 21.56 \text{ FT/day} \quad \checkmark$$

$$= 7.6 \times 10^{-3} \text{ cm/sec} \quad \checkmark$$

CLIENT: USEPA	FILE NO.: L670	BY: DWB	PAGE: 1 OF 1
SUBJECT: Kellogg-Deering Slug Tests		CHECKED BY: JPO 6/17/88	DATE: 6/16/88

K21 Rising Head Test \*

Time (min)	$\Delta H$ (ft)	$\Delta H/H_0$	$H_0 = 2.74$
0	2.74	1	
.01	2.52	.92	
.03	2.05	.75	
.08	1.17	.43	
.13	0.62	.23	
.18	0.31	.11	
.23	0.14	.05	
.28	0.06	.02	

$$\begin{aligned} r &= 0.0833 \text{ FT} \\ R &= 0.0833 \text{ FT} \\ L &= 10.0 \text{ FT} \\ T_0 &= 0.036 \text{ MIN} \end{aligned}$$

$$K = \frac{r^2 \ln(L/R)}{2L T_0} \quad (\text{Hvorslev, 1951})$$

$$= \frac{0.0332}{1.72} \quad \checkmark$$

$$= 0.019 \text{ FT/min} \quad \checkmark$$

$$= 1.16 \text{ FT/hr} \quad \checkmark$$

$$= 27.8 \text{ FT/day} \quad \checkmark$$

$$= 9.8 \times 10^{-3} \text{ cm/sec} \quad \checkmark \checkmark$$

\* Bedrock well  
 \*\* When calculated using  $L=5'$   $K=1.9 \times 10^{-2}$   
 (5' Approx. Fractured interval)

CLIENT: USEPA	FILE NO.: L670	BY: JPO	PAGE 1 OF 1
SUBJECT: Kellogg Deering Slug Tests		CHECKED BY:	DATE: 6/22/88

K 21 Falling Head Test \*

r = .0832 ft  
 R = .0832 ft  
 L = 10.0 ft  
 T<sub>0</sub> = .08 min

$$K = \frac{r^2 \ln(L/R)}{2LT_0} \quad (\text{Dvorsky, 1951})$$

$$= \frac{.0832 \text{ ft}^2}{1.6 \text{ ft/min}}$$

$$= .021 \text{ ft/min}$$

$$= 29.88 \text{ ft/day}$$

$$= 1.05 \times 10^{-2} \text{ cm/sec.}$$

\* - Bedrock well

CLIENT: USEPA	FILE NO.: L670 OPEST head only	BY: DWB	PAGE 1 OF 1
SUBJECT: Hydraulic Conductivity K21		CHECKED BY: JPB 6/21/88	DATE: 6/17/88

$$K = C_p \frac{Q}{H}$$

$$C_p = \frac{1}{2\pi r L} \ln \frac{L}{r} \quad 70315.5 \quad L = 5' \quad r = 1.25'$$

$$= \frac{1}{31.42} (3.69) 70315.5 \quad \checkmark$$

$$= 8257.88 \quad \checkmark$$

$$H = H_g = 32' \quad \checkmark$$

$$Q = 50 \text{ gpm} \quad \checkmark$$

$$K = 8257.88 \frac{93}{32} \quad \checkmark$$

$$= 12903 \text{ ft/yr} \quad \checkmark$$

$$= 35.35 \text{ ft/day} \quad \checkmark$$

$$= 1.2 \times 10^{-2} \text{ cm/sec} \quad \checkmark$$

## Assumes:

$L = 5'$  actual probably less

$Q = 50$  actual probably more

$H = 32$  actual probably less

No appreciable amount entered higher fractures

Based on observation of 4/14/88 in which drillers attempted to wash hole for cutting removal. With rig pump on maximum speed (+50 gpm capacity) no water was returned to the surface during 11 minutes of pumping. Calculations assume that 50 gpm was pumped into the formation through a five foot fractured interval under the pressure of a column of water extending from the original water table to the ground surface.

CLIENT: USEPA	FILE NO.: L670	BY: DWB	PAGE 1 OF 1
SUBJECT: Kellogg-Deering Packer Test K22A		CHECKED BY: JPO 6/20/88	DATE: 6/17/88

$$K = c_p \frac{Q}{H} \quad c_p = \frac{1}{2\pi L} \ln\left(\frac{L}{r}\right) 70315 \quad \begin{matrix} L = 12' \\ r = 0.125 \\ PSI = 40 \end{matrix}$$

$$= 0.013 \times 4.56 \times 70315 \quad \checkmark$$

TEST INTERVAL - 48.3' - 60.3' = 4172 ✓

$$N = h_g + h_p$$

$$= (29 + 3.3) + (40 \times 2.31) \quad \checkmark$$

$$= 32.3 + 92.4 \quad \checkmark$$

$$= 124.7 \text{ FT} \quad \checkmark$$

$$Q = 0.98 \text{ GPM} \quad \checkmark$$

$$K = 4172 \times \frac{0.98}{124.7} \quad \checkmark$$

$$= 32.79 \text{ Ft/yr} \quad \checkmark$$

$$= 0.09 \text{ Ft/day} \quad \checkmark$$

$$\text{OR } 3.17 \times 10^{-5} \text{ cm/sec} \quad \checkmark$$

CLIENT: USEPA	FILE NO.: L670	BY: DJB	PAGE 1 OF 1
SUBJECT: Kellogg-Doering Packard Test K22A		CHECKED BY: JPO 6/20/88	DATE: 6/17/88

$$K = c_p \frac{Q}{H} \quad c_p = \frac{V}{2\pi r L} \ln\left(\frac{L}{r}\right) 70315 \quad \begin{matrix} L = 12' \\ r = 0.125 \\ PSI = 20 \end{matrix}$$

TEST INTERVAL -  
48.3' - 60.3'

$$= 0.013 \times 4.56 \times 70315$$

$$= 4172$$

$$H = H_p + H_g$$

$$= (20 \times 2.31) + 29 + 3.3$$

$$= 46.2 + 32.3$$

$$= 78.5 \text{ Ft}$$

$$Q = 0.47 \text{ gpm}$$

$$K = 4172 \frac{0.47}{78.5}$$

$$= 24.98 \text{ Ft/yr}$$

$$= 0.068 \text{ Ft/day}$$

$$\text{OR } 2.41 \times 10^{-5} \text{ cm/sec}$$

CLIENT: US 824	FILE NO.: L676	BY: DLSB	PAGE 1 OF 1
SUBJECT: Kellogg-Deering Paeker Tests K22B		CHECKED BY: JPO 6/22/88	DATE: 6/21/88

$$K = c_p \frac{Q}{H}$$

$$C_p = \frac{1}{2\pi r L} \ln\left(\frac{r}{r_0}\right) 70315$$

L = 12'  
r = 0.125'  
PSI = 20

$$= 0.013 \times 4.56 \times 70315$$

Test Interval  
101.5 - 113.5

$$= 4168$$

$$H = h_p + h_g$$

$$= (20 \times 231) + 3.8 + 28.85$$

$$= 78.85 \text{ Ft}$$

$$Q = 0.20 \text{ gpm}$$

$$K = 4168 \frac{0.20}{78.85}$$

$$= 10.57 \text{ Ft/Yr}$$

$$= 0.029 \text{ Ft/day}$$

$$\text{OR } 1.02 \times 10^{-5} \text{ cm/sec.}$$



CLIENT: USDO	FILE NO.: L670	BY: DLSB	PAGE   OF   
SUBJECT: Kellogg-Deering Packer Test K22B		CHECKED BY: JFO 6/22/88	DATE: 6/21/88

$$K = c_p \frac{Q}{H} \quad c_p = \frac{1}{2\pi r L} \ln\left(\frac{4}{r}\right) 70315$$

$$= 0.013 \times 4.56 \times 70315$$

$$L = 12'$$

$$r = 0.125'$$

$$PSI = 40$$

Test Interval  
101.5 - 113.5

$$= 4168$$

$$H = h_p + h_g$$

$$= (40 \times 2.31) + 3.80 + 28.85$$

$$= 125.05 \text{ FT}$$

$$Q = 0.25 \text{ gpm}$$

$$K = 4168 \frac{0.25}{125.05}$$

$$= 8.33 \text{ Ft/Yr}$$

$$= 0.02 \text{ Ft/day}$$

$$\text{or } 8.05 \times 10^{-6} \text{ cm/sec}$$



CLIENT USEPA FILE NO. L670 BY DJB

SUBJECT Kellogg-Deerine Slug Tests Checked By JPD 6/17/88

K23 A Rising Head Test (2)

Time (min)	$\Delta H$ (ft)	$\Delta H/H_0$	$H_0 = 1.68$
0	1.68	1	
.01	1.46	.87	
.03	1.26	.75	
.08	0.92	.55	
.13	0.69	.41	
.18	0.52	.31	
.23	0.38	.23	
.23	0.29	.17	
.43	0.11	.07	
.73	0.05	.03	

$r = 0.0833$  FT  
 $R = 0.0833$  FT  
 $L = 2.17$  FT  
 $T_0 = 0.147$  MIN

$$K = \frac{r^2 \ln L/R}{2L T_0} \quad (\text{Dvorsky, 1951})$$

$$= \frac{6.94^2 \times 3.26}{4.34 \times 0.147} \quad \checkmark$$

$$= \frac{0.023}{.628} \quad \checkmark$$

$$= 0.036 \text{ Ft/min} \quad \checkmark$$

$$= 2.16 \text{ Ft/hr} \quad \checkmark$$

$$= 51.9 \text{ Ft/day} \quad \checkmark$$

$$= 1.8 \times 10^{-2} \text{ cm/sec} \quad \checkmark$$

Being Timed

USEPA FILE NO. 6670 BY D. J. Z.  
 SUBJECT Kellogg - Leaking Sludge Tests Checked By JPO 6/17/88

K23A Rising Head Test (1)

Time (MIN)	ΔH (FT)	ΔH/H <sub>0</sub>	H <sub>0</sub> = 1.77
0	1.77	1	
.01	1.48	.84	
.03	1.26	.71	
.08	0.94	.53	
.13	0.71	.40	
.18	0.54	.31	
.23	0.40	.23	
.28	0.30	.17	
.48	0.12	.07	
.73	0.05	.03	

T = 0.0833 FT  
 R = 0.0833 FT  
 L = 2.17 FT  
 T<sub>0</sub> = 0.145 MIN

$$K = \frac{T^2 \ln(L/R)}{2L T_0} \quad (\text{Hvorslev, 1955})$$

$$K = \frac{0.023}{0.629} \quad \checkmark$$

$$= 0.037 \text{ Ft/min} \quad \checkmark$$

$$= 2.19 \text{ Ft/Hr} \quad \checkmark$$

$$= 53.66 \text{ Ft/d.1} \quad \checkmark$$

$$= 1.9 \times 10^{-2} \text{ cm/sec} \quad \checkmark$$



CLIENT USEPA FILE NO. L670 BY DWB

SUBJECT Kollege Deering Slug Tests Checked By JPO 6/17/88

### K23 B Rising Head Test

Time (MIN)	ΔH (FT)	H/H <sub>0</sub> (H <sub>0</sub> = 3.11)
0	3.11	1
0.02	3.01	0.97
0.13	2.75	0.88
0.32	2.53	0.80
0.90	2.01	0.65
1.40	1.49	0.48
1.90	1.10	0.35
2.98	0.57	0.18
3.98	0.31	0.10
6.98	0.05	0.02

$$T = \frac{1.0 r^2}{c}$$

(Cooper, et al., 1967)

$$c = \frac{.00694}{.54}$$

$$r_c = .0833 \text{ ft}$$

$$t = 0.54 \text{ MIN}$$

$$= 0.0128 \text{ ft}^2/\text{min}$$

$$= 18.5 \text{ ft}^2/\text{day}$$

$$K = 18.5 \text{ ft}^2/\text{day} \div 17 \text{ ft (sand pack interval)}$$

$$= 1.1 \text{ ft/day}$$

$$= 3.9 \times 10^{-4} \text{ cm/sec}$$

- ✓
- ✓
- ✓
- ✓
- ✓
- ✓
- ✓
- ✓



CLIENT USEPA FILE NO. 1670 BY DWB  
 SUBJECT Kellogg-Dearing Slug Tests Checked By JPO 6/17/88

K23B Falling Head Test

<u>Time (hr)</u>	<u>ΔH (ft)</u>	<u>H/H<sub>0</sub></u>
<u>0</u>	<u>2.04</u>	<u>1</u>
<u>.1</u>	<u>1.95</u>	<u>.96</u>
<u>.15</u>	<u>1.89</u>	<u>.93</u>
<u>.3</u>	<u>1.76</u>	<u>.86</u>
<u>.63</u>	<u>1.50</u>	<u>.74</u>
<u>.96</u>	<u>1.28</u>	<u>.63</u>
<u>1.13</u>	<u>1.19</u>	<u>.58</u>
<u>1.38</u>	<u>1.06</u>	<u>.52</u>
<u>1.72</u>	<u>0.90</u>	<u>.44</u>
<u>2.88</u>	<u>0.53</u>	<u>.26</u>
<u>4.88</u>	<u>0.21</u>	<u>.10</u>
<u>6.38</u>	<u>0.08</u>	<u>.04</u>
<u>7.88</u>	<u>0.05</u>	<u>.03</u>
<u>9.38</u>	<u>0.02</u>	<u>.01</u>
<u>12.00</u>	<u>0.00</u>	<u>0.00</u>

$T = \frac{1.0 r_w^2}{t}$  (Cooper, et. al., 1967)

$\frac{1.0 r_w^2}{t} = 0.00694$	$r_w = 2.0833 \text{ ft}$	✓
$t = 0.69$	$t = 0.69 \text{ hr}$	✓
$\frac{1.0 r_w^2}{t} = 0.010 \text{ ft}^2/\text{min}$		✓
$\frac{1.0 r_w^2}{t} = 14.5 \text{ ft}^2/\text{day}$		✓
$K = 14.5 \text{ ft}^2/\text{day} \div 17 \text{ ft (sand pack interval)}$		✓
$\frac{1.0 r_w^2}{t} = 0.85 \text{ ft}^2/\text{day}$		✓
$\frac{1.0 r_w^2}{t} = 3.0 \times 10^{-4} \text{ cm}^2/\text{sec}$		✓

CLIENT USEPA FILE NO. L670 BY DWB

SUBJECT Kellogg-Deering Slug Tests Checked By JPD 6/17/88

**K24 Falling Head Tests**

Time (min)	AH (Ft)	H/H <sub>0</sub> (H <sub>0</sub> = 3.07)
0	3.07	1
.05	3.03	.99
.15	3.00	.98
.25	2.98	.97
.45	2.96	.96
.95	2.93	.95
1.95	2.90	.94
4.95	2.83	.92
9.95	2.73	.89
19.95	2.56	.83
29.95	2.37	.77
35.95	2.29	.75

$T = \frac{1.0 r_e^2}{c}$  (Cooper, et. al., 1967)

$r_e = .0833 \text{ Ft}$   
 $t = 95 \text{ min.} \Rightarrow T t / r_e^2 = 1$

$T = \frac{(.0833)^2}{95}$   
 $c = 7.3 \times 10^{-5} \text{ Ft}^2/\text{min}$   
 $= .11 \text{ Ft}^2/\text{day}$

$K = .11 \text{ Ft}^2/\text{day} \div 18.2 \text{ Ft (sand pack interval)}$

$= .006 \text{ Ft/day}$   
 $= 2.1 \times 10^{-6} \text{ cm/sec}$

✓  
 ✓  
 ✓  
 ✓  
 ✓

CLIENT: USEPA	FILE NO.: 1670 JOPSI K24	BY: DRSB	PAGE 1 OF 1
SUBJECT: Hydraulic Conductivity K24		CHECKED BY: JPO 6/2/88	DATE: 6/17/88

$$K = C_p \left( \frac{Q}{H} \right)$$

TEST INTERVAL -  
173.85' - 184.5'

$$C_p = \left( \frac{1}{2\pi r L} \right) \ln \left( \frac{L/r}{r} \right) (70315.5) \quad L = 10.65 \quad r = 0.1432$$

$$= \left( \frac{1}{2\pi \cdot 9.2} \right) (4.31) (70315.5) \quad \checkmark$$

$$= 4528.69 \quad \checkmark$$

$$H = h_p + h_g \quad h_p = 20 + 2.31 \quad h_g = 40.6 \quad \checkmark$$

$$= 46.2 \quad \checkmark$$

$$H = 86.8 \text{ FT} \quad \checkmark$$

$$Q = 3.6 \text{ gpm} \quad \checkmark$$

$$K = (4528.69) \frac{3.6}{86.8} \quad \checkmark$$

$$= (4528.69) (0.0415) \quad \checkmark$$

$$= 187.83 \text{ Ft/Yr} \quad \checkmark$$

$$= 0.512 \text{ Ft/Day} \quad \checkmark$$

$$\text{OR } \frac{1.878}{1.81} \times 10^{-4} \text{ cm/sec.} \quad \checkmark$$

PROJECT: Kellogg-Deering  
 BORING NO.: K18A  
 DEPTH OF CASING: \_\_\_\_\_ FEET

PACKER TEST REPORT  
 PROJECT NO.: L170  
 CONTRACTOR: Empire Soils  
 BY: D. Blackett

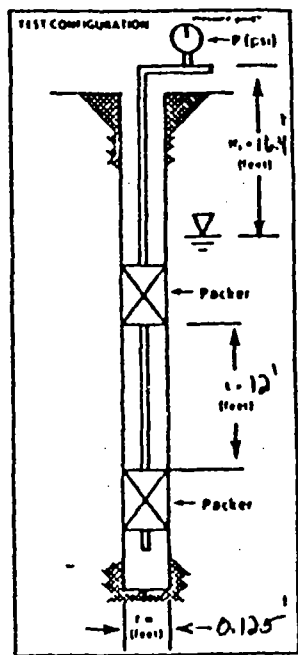
PAGE 1 OF 1

Casing Depth (ft)	Flow Test		Calculated Results									
	Elapsed Time (min)	Flow Reading	Flow Rate Q (gpm)	$H_1$ (ft)	$H_2$ (ft)	$H_1 - H_2$ (ft)	$C_p$	$C_p(Q) = C_p(QH)$	$K$ (cm/sec) = $\frac{1.48 \times 10^{-4}}{C_p(Q)}$			
3.0	0.0	300.0										
	0.5	303.9										
	1.0	307.8										
	1.5	311.6										
	2.0	315.4										
	2.5	319.2										
	3.0	323.0										
	3.5	326.7										
	4.0	330.5										
	4.5	334.1										
	5.0	337.9										
	5.5	341.6										
	6.0	345.1										
	6.5	349.0										
	7.0	352.6										
	7.5	356.5										
	8.0	360.0										
	8.5	363.5										
	9.0	367.3										
	9.5	371.2										
10.0	374.3		71.3	743	16.4	69.3		85.7	4168	361.36	$3.99 \times 10^{-4}$	

$C_p = (1.48 \times 10^{-4}) / (Q(H))$   
 7.48 Gallons = 1 ft<sup>3</sup>  
 1 psi = 2.31 ft head

Remarks: \_\_\_\_\_

$H_1$  is used when the test length is below the water table  
 $H_2$  is used when the test length is above the water table



Length of Test Section (ft)	$C_p$			
	10	20	30	40
1	11.000	22.000	33.000	44.000
2	5.500	11.000	16.500	22.000
3	3.667	7.333	11.000	14.667
4	2.750	5.500	8.250	11.000
5	2.200	4.400	6.600	8.800
6	1.733	3.467	5.167	6.933
7	1.429	2.857	4.286	5.714
8	1.225	2.450	3.675	4.875
9	1.099	2.198	3.297	4.376
10	1.000	2.000	3.000	4.000

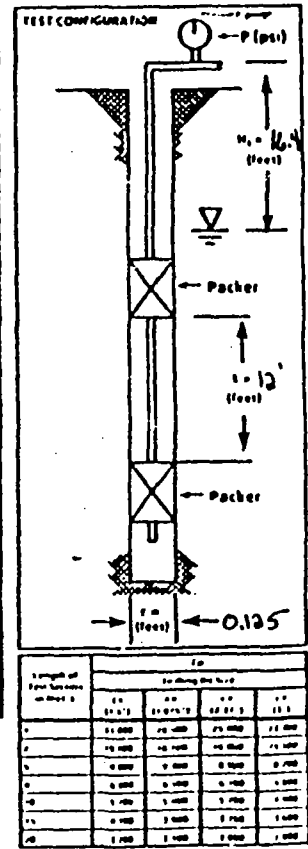


PROJECT: Kellag-Deering  
 BORING NO.: K1AB  
 DEPTH OF CASING: \_\_\_\_\_ FEET

PACKER TEST REPORT  
 PROJECT NO.: L670  
 CONTRACTOR: Empire Sails  
 BY: D. Blackett

PAGE 1 OF 1

Flow Test			Calculated Results									
Pressure (psi)	Elapsed Time (min)	Flow Reading	Δ Flow AQ (ft³)	Δ Flow AQ (gal)	Flow Rate Q (gal/min)	M <sub>1</sub> (ft)	M <sub>2</sub> = 7.31 ft	M <sub>2</sub> (ft)	H = M <sub>2</sub> (ft)	C <sub>p</sub>	Σ (Flow) = C <sub>p</sub> Q <sub>av</sub>	K (number) = $\frac{5.61 \times 10^{-4}}{K (lwy)}$
20	0.0	385.0										
	0.5	387.6										
	1.0	389.6										
	1.5	392.1										
	2.0	394.4										
	2.5	396.8										
	3.0	399.2										
	3.5	401.6										
	4.0	404.0										
	4.5	406.5										
	5.0	408.8										
	5.5	411.3										
	6.0	413.5										
	6.5	416.0										
	7.0	418.3										
	7.5	420.6										
	8.0	423.1										
	8.5	425.3										
	9.0	427.3										
	9.5	430.2										
✓	10.0	432.6	-	47.6	4.76	16.4	46.2	-	162.6	4168	316.92	306x10 <sup>-4</sup>



CP = (1)(12)(1)(1)(4)(1)(70321)  
 748 Gallons = 1 ft<sup>3</sup>  
 1 psi = 2.31 ft head

Remarks: Test Interval 8.5-10.5 Swirl 15.5' Swirl Height 0.9'  
Packer Pressure 180 PSI Borehole Diameter 3"

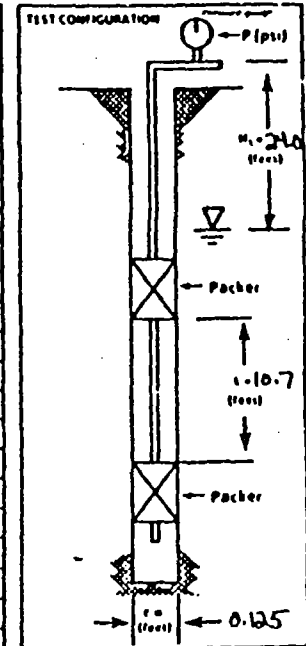
H<sub>1</sub> is used when the test length is below the water table  
 H<sub>2</sub> is used when the test length is above the water table

PROJECT: Kellogg-Deering  
 BORING NO.: K19B  
 DEPTH OF CASING: \_\_\_\_\_ FEET

PACKER TEST REPORT  
 PROJECT NO.: 11670  
 CONTRACTOR: Empire Sails  
 BY: S. Krall

PAGE 1 OF 1

Flow Test			Calculated Results									
CASING ID (in)	Elapsed Time (min)	Flow Reading	A Flow AQ (ft <sup>3</sup> )	B Flow BQ (ft <sup>3</sup> )	Flow Rate Q (gpm)	M <sub>1</sub> (ft)	M <sub>2</sub> = 2.31 p	M <sub>3</sub> (ft)	M = M <sub>1</sub> + M <sub>2</sub> or M <sub>3</sub>	C <sub>p</sub>	Q (ft <sup>3</sup> /d) = C <sub>p</sub> Q <sub>1</sub>	Q (gallons) = 7.48 x 10 <sup>-3</sup> x (ft <sup>3</sup> /d)
40	0.0	550.0										
	0.5	550.7										
	1.0	551.4										
	1.5	552.1										
	2.0	552.7										
	2.5	553.3										
✓	3.0	553.9										
38	3.5	554.5										
↓	4.0	555.1										
↓	4.5	555.8										
36	5.0	556.3										
	5.5	556.8										
	6.0	557.4										
	6.5	558.0										
	7.0	558.6										
	7.5	559.2										
	8.0	559.8										
	8.5	560.4										
	9.0	561.0										
↓	9.5	561.6										
	10.0	562.2										
			12.2	1.22	21.04	92.4		14.44	4693.19	49.19	47580.5	



Length of Test Interval (feet)	C <sub>p</sub>			
	10	20	30	40
1	27.52	31.216	34.912	38.608
2	11.008	19.688	28.176	36.640
3	6.696	11.616	16.320	21.632
4	4.528	7.744	10.880	14.464
5	3.376	5.792	7.936	10.816
6	2.784	4.832	6.528	8.928
7	2.368	4.064	5.440	7.488
8	2.048	3.456	4.608	6.336
9	1.792	2.976	3.968	5.472
10	1.584	2.592	3.408	4.768
11	1.408	2.288	2.912	4.176
12	1.264	2.048	2.496	3.712

C<sub>p</sub> = (11.72 x C) / (11.70321)

7.48 Gallons = 1 ft<sup>3</sup>

1 psi = 2.31 ft head

Remarks

Test Interval = 25-35.7 SWL = 13.80 Swivel Height = 10.24

Packer Pressure 400 PSI

Borehole Diameter 3"

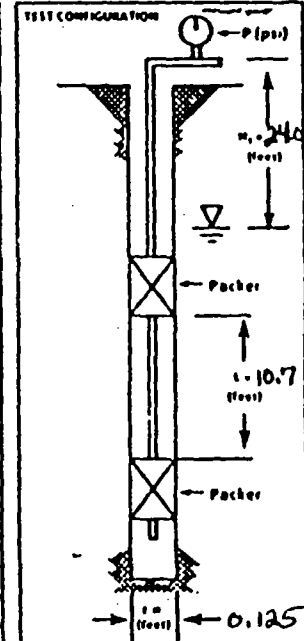
It<sub>1</sub> is used when the test length is below the water table  
 It<sub>2</sub> is used when the test length is above the water table

PROJECT: Kellogg-Deering  
 BORING NO.: R19-B  
 DEPTH OF CASING: \_\_\_\_\_ FEET

PACKER TEST REPORT  
 PROJECT NO.: 1170  
 CONTRACTOR: Empire Soils  
 BY: S. Krall

PAGE 1 OF 1

Flow Test			Calculated Results									
Casings Depth (ft)	Casings Length (ft)	Flow Reading	Flow Rate Q <sub>1</sub> (gpm)	Flow Rate Q <sub>2</sub> (gpm)	Flow Rate Q (gpm)	H <sub>1</sub> (ft)	H <sub>2</sub> = 2.31 p (ft)	H <sub>3</sub> (ft)	H <sub>4</sub> = H <sub>1</sub> + H <sub>2</sub> or H <sub>3</sub> (ft)	C <sub>p</sub>	Q (gpm) = C <sub>p</sub> Q <sub>1</sub>	Q (gpm) = 2.31 p x (ft)
20	0.0	539.0										
↓	0.5	539.5										
↓	1.0	540.0										
18	1.5	540.3										
↓	2.0	540.5										
↓	2.5	540.7										
20	3.0	541.4										
	3.5	541.6										
	4.0	541.9										
	4.5	542.4										
	5.0	542.8										
	5.5	543.2										
	6.0	543.4										
	6.5	543.8										
	7.0	544.2										
	7.5	544.5										
	8.0	544.8										
	8.5	545.2										
	9.0	545.6										
	9.5	545.8										
↓	10.0	546.1										
			7.1	0.71	2404	4612			70124	4693		



Flow Rate (gpm)	Casing Length (ft)			
	0.0	10.0	20.0	30.0
0	0.000	0.000	0.000	0.000
1	0.000	0.000	0.000	0.000
2	0.000	0.000	0.000	0.000
3	0.000	0.000	0.000	0.000
4	0.000	0.000	0.000	0.000
5	0.000	0.000	0.000	0.000
6	0.000	0.000	0.000	0.000
7	0.000	0.000	0.000	0.000
8	0.000	0.000	0.000	0.000
9	0.000	0.000	0.000	0.000
10	0.000	0.000	0.000	0.000
11	0.000	0.000	0.000	0.000
12	0.000	0.000	0.000	0.000
13	0.000	0.000	0.000	0.000
14	0.000	0.000	0.000	0.000
15	0.000	0.000	0.000	0.000
16	0.000	0.000	0.000	0.000
17	0.000	0.000	0.000	0.000
18	0.000	0.000	0.000	0.000
19	0.000	0.000	0.000	0.000
20	0.000	0.000	0.000	0.000

C<sub>p</sub> = (142 + 111) / (10321)  
 748 Gallons = 311  
 1 psi = 2.31 ft head

Remarks: Test Interval = 25-35.7' Soil = 13.80 Summit Height = 10.24

Packer Pressure = 240 PSI Packer Diameter = 3"

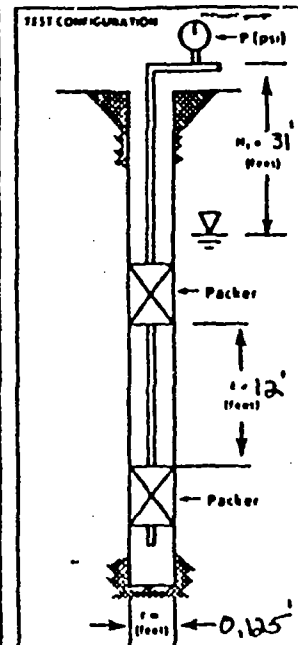
H<sub>1</sub> is used when the test length is below the water table  
 H<sub>2</sub> is used when the test length is above the water table

PROJECT: Kellogg-Deering  
 BORING NO.: K20B  
 DEPTH OF CASING: \_\_\_\_\_ FEET

PACKER TEST REPORT  
 PROJECT NO.: 1670  
 CONTRACTOR: Empire Soils  
 BY: D. Blackert

PAGE 1 OF 1

Packer Depth (ft)	Liquid Level (ft)	Flow Test		Calculated Results								
		Flow Reading	Flow Rate Q (gpm)	Flow Rate Q (gpm)	$n_1$ (ft)	$n_2 = 2.11 n_1$ (ft)	$n_3 = 10 n_1$ (ft)	Cp	$C_p \text{ (psi)} = C_p \text{ DM}$	$\frac{C_p \text{ (psi)}}{1.57 \times 10^4} =$ C (Psi)		
40	0.00	262.2										
	0.25	262.3										
	0.50	262.4										
	0.75	262.4										
	1.00	262.5										
	1.25	262.5										
	1.50	262.6										
	1.75	262.6										
	2.00	262.7										
	2.25	262.8										
	2.50	262.8										
	2.75	262.9										
	3.00	262.9										
	3.25	263.0										
	3.50	263.0										
	3.75	263.1										
	4.00	263.2										
	4.25	263.2										
	4.50	263.3										
	4.75	263.4										
	5.00	263.4										
			1.2	0.24	31	92.4	—	123.4	4168	8.11	78940 <sup>-6</sup>	



Length of Test Section (ft)	Cp			
	10	20	30	40
1	11,000	10,000	9,000	8,000
2	11,000	10,000	9,000	8,000
3	11,000	10,000	9,000	8,000
4	11,000	10,000	9,000	8,000
5	11,000	10,000	9,000	8,000
6	11,000	10,000	9,000	8,000
7	11,000	10,000	9,000	8,000
8	11,000	10,000	9,000	8,000
9	11,000	10,000	9,000	8,000
10	11,000	10,000	9,000	8,000
11	11,000	10,000	9,000	8,000
12	11,000	10,000	9,000	8,000
13	11,000	10,000	9,000	8,000
14	11,000	10,000	9,000	8,000
15	11,000	10,000	9,000	8,000
16	11,000	10,000	9,000	8,000
17	11,000	10,000	9,000	8,000
18	11,000	10,000	9,000	8,000
19	11,000	10,000	9,000	8,000
20	11,000	10,000	9,000	8,000

CP =  $(1/2 + 1) \times (1/1) \times (1/2) \times 1$   
 748 Gallons = 1 ft<sup>3</sup>  
 1 psi = 2.31 ft head

Remarks: Test Interval = 101-113 SWL 20.5 Saturated Height 10.5  
Packer Pressure 150 PSI Borehole Diameter 3"

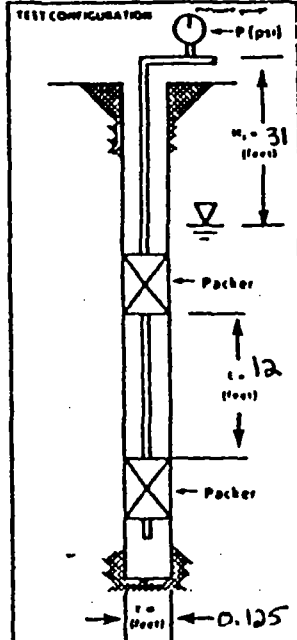
$n_1$  is used when the test length is below the water table  
 $n_2$  is used when the test length is above the water table

PROJECT: Kellogg-Deering  
 BORING NO.: R208  
 DEPTH OF CASING: \_\_\_\_\_ FEET

PACKER TEST REPORT  
 PROJECT NO.: 1170  
 CONTRACTOR: Empire Sails  
 BY: D. Blackett

PAGE 1 OF 1

Flow Test				Calculated Results							
Time (min)	Elapsed Time (min)	Flow Reading	Flow Rate Q (ft <sup>3</sup> /hr)	Flow Rate Q (gpm)	h <sub>1</sub> (ft)	h <sub>2</sub> = 2.31 p (ft)	h <sub>3</sub> (ft)	h <sub>4</sub> = h <sub>1</sub> + h <sub>2</sub> + h <sub>3</sub> (ft)	C <sub>p</sub>	Σ (h <sub>1</sub> h <sub>2</sub> ) = C <sub>p</sub> Q <sup>2</sup>	Σ (h <sub>1</sub> h <sub>2</sub> ) = C <sub>p</sub> Q <sup>2</sup> (ft <sup>3</sup> /hr) <sup>2</sup>
60	0.00	264.0									
	0.25	264.1									
	0.50	264.2									
	0.75	264.2									
	1.00	264.3									
	1.25	264.4									
	1.50	264.5									
	1.75	264.5									
	2.00	264.6									
	2.25	264.7									
	2.50	264.8									
	2.75	264.9									
	3.00	265.0									
	3.25	265.0									
	3.50	265.1									
	3.75	265.2									
	4.00	265.3									
	4.25	265.4									
	4.50	265.4									
	4.75	265.5									
	5.00	265.6									
			1.6	0.32	31	138.6		169.6	4168	7.81e	7.16E10 <sup>-6</sup>



Length of Test Section (feet)	Flow Rate (gpm)			
	10	20	30	40
1	11.000	22.000	33.000	44.000
2	11.000	22.000	33.000	44.000
3	11.000	22.000	33.000	44.000
4	11.000	22.000	33.000	44.000
5	11.000	22.000	33.000	44.000
6	11.000	22.000	33.000	44.000
7	11.000	22.000	33.000	44.000
8	11.000	22.000	33.000	44.000
9	11.000	22.000	33.000	44.000
10	11.000	22.000	33.000	44.000

C<sub>p</sub> = (1/2 \* 1.48 \* 1.48) / (70.32 \* 1)  
 7.48 Gallons = 1 ft<sup>3</sup>  
 1 psi = 2.31 ft head

Remarks: Test Interval 101-113 Swl = 20.5 Summed Height = 16.5  
Packer Pressure = 100 PSI Borehole Diameter = 3"

It<sub>1</sub> is used when the test length is below the water table  
 It<sub>2</sub> is used when the test length is above the water table

PROJECT: Kallogg-Deering  
 BORING NO.: KAA3  
 DEPTH OF CASING: \_\_\_\_\_ FEET

PACKER TEST REPORT  
 PROJECT NO.: 1170  
 CONTRACTOR: Empire Soils  
 BY: S. Krall

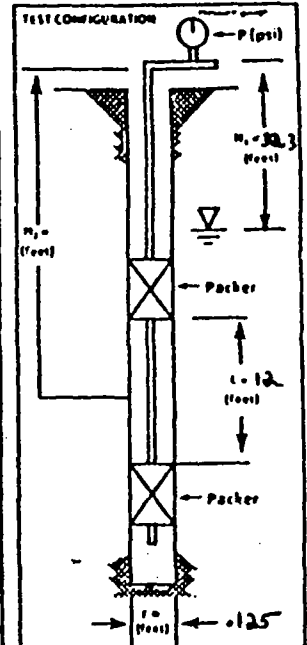
PAGE 1 OF 1

Packer, B (ft)	Flow Test			Calculated Results								
	Elapsed Time (min)	Flow Reading	A flow, AQ (ft <sup>3</sup> )	A flow, AQ (qt)	Flow Rate Q (gpm)	H <sub>1</sub> (ft)	H <sub>2</sub> = 2.31 p (ft)	H <sub>2</sub> (ft)	H = H <sub>1</sub> + H <sub>2</sub> (ft or m)	Cp	E (ft/yr) = Cp OM	K (inches) = $\frac{5.67 \times 10^{-4} E}{K (ft/yr)}$
20PST	0.0	145.2										
	0.5	145.5										
	1.0	145.7										
	1.5	146.0										
	2.0	146.2										
	2.5	146.4										
	3.0	146.7										
	3.5	146.9										
	4.0	147.1										
	4.5	147.3										
	5.0	147.6										
	5.5	147.8										
	6.0	149.0										
	6.5	148.3										
	7.0	148.5										
	7.5	148.7										
	8.0	148.9										
	8.5	149.2										
	9.0	149.5										
	9.5	149.7										
	10.0	149.9										
			4.7	0.47	32.3	46.2		78.5	4168	24.95		$2.41 \times 10^{-5}$

Cp =  $\frac{1172 \times 111 + 111 \times (70221)}{748 \text{ Gallons} \times 111}$

1 pu = 2.31 ft head

Remarks: Test Interval 48.3-10.3 SWL = 29.0' Summit Height = 3.3'  
Packer Pressure 225 PSI Borehole diameter 3"



Length of Test Section in Feet	Cp			
	10	20	30	40
1	11,000	12,000	13,000	14,000
2	10,000	11,000	12,000	13,000
3	9,000	10,000	11,000	12,000
4	8,000	9,000	10,000	11,000
5	7,000	8,000	9,000	10,000
6	6,000	7,000	8,000	9,000
7	5,000	6,000	7,000	8,000
8	4,000	5,000	6,000	7,000
9	3,000	4,000	5,000	6,000
10	2,000	3,000	4,000	5,000

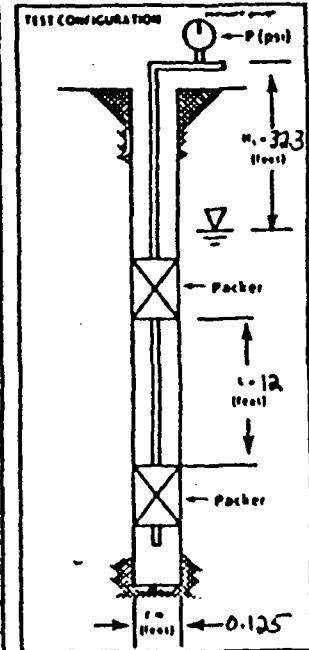
H<sub>1</sub> is used when the test length is below the water table  
 H<sub>2</sub> is used when the test length is above the water table

PROJECT: Kellang-Deering  
 BORING NO.: K22R  
 DEPTH OF CASING: \_\_\_\_\_ FEET

PACKER TEST REPORT  
 PROJECT NO.: L670  
 CONTRACTOR: Empire Soils  
 BY: S. Krall

PAGE 1 OF 1

Flow Test			Calculated Results									
CASING D (in)	UPPER Limit (min)	Flow Reading	Flow Rate Q (gpm)	Flow Rate Q (cfs)	$H_1$ (ft)	$H_2 = 2.31 P$	$H_1 - H_2$ (ft)	$Q^2$ (ft <sup>3</sup> /s <sup>2</sup> )	CP	$Q^2 (H_1 - H_2) = C_p Q^2$	$\frac{Q^2 (H_1 - H_2)}{C_p} = \frac{Q^2 (H_1 - H_2)}{1.87 \times 10^{-5}}$	
40	0	151.0										
	0.5	151.6										
	1.0	152.1										
	1.5	152.6										
	2.0	153.1										
	2.5	153.6										
	3.0	154.0										
	3.5	154.5										
	4.0	155.0										
	4.5	155.4										
	5.0	155.9										
	5.5	156.4										
	6.0	156.9	—	5.9	0.98	32.3	92.4	—	124.7	416.8	32.76	31716.5



Length of Test Section in Feet	Cp			
	0.5	1.0	1.5	2.0
1	11,000	22,000	33,000	44,000
2	22,000	44,000	66,000	88,000
3	33,000	66,000	99,000	132,000
4	44,000	88,000	132,000	176,000
5	55,000	110,000	165,000	220,000
6	66,000	132,000	198,000	264,000
7	77,000	154,000	231,000	308,000
8	88,000	176,000	264,000	352,000
9	99,000	198,000	297,000	396,000
10	110,000	220,000	330,000	440,000

CP =  $(1/12 \times 1)^4 (70321)$   
 7.48 Gallons = 1 cfs  
 1 psi = 2.31 ft head

Remarks: Test Interval = 48.3 - 60.3 Swl = 29' Swivel Height = 3.3'  
Packer Pressure = 225 PSI Basehole Diameter = 3"

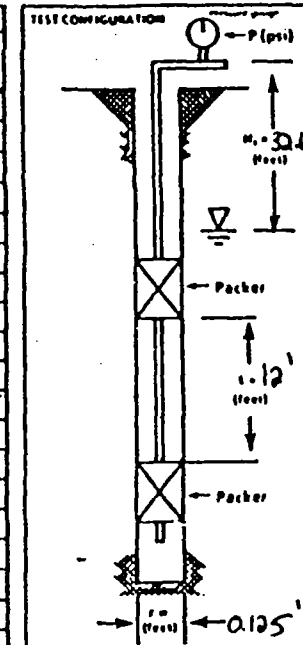
H<sub>1</sub> is used when the test length is below the water table  
 H<sub>2</sub> is used when the test length is above the water table

PROJECT: Kellogg-Deering  
 BORING NO.: K22B  
 DEPTH OF CASING: \_\_\_\_\_ FEET

PACKER TEST REPORT  
 PROJECT NO.: 1670  
 CONTRACTOR: Empire Soils  
 BY: S. Reall

PAGE 1 OF 1

Casing ID (in)	Flow Test			Calculated Results								
	Open End (in)	Flow Reading	Flow Rate Q (ft <sup>3</sup> /hr)	Flow Rate Q (gpm)	M <sub>1</sub> (ft)	M <sub>2</sub> (ft)	M <sub>1</sub> - M <sub>2</sub> (ft)	C <sub>p</sub>	Q (ft <sup>3</sup> /hr) = C <sub>p</sub> Q <sub>out</sub>	Q (ft <sup>3</sup> /hr) = 0.67 x 10 <sup>-4</sup> x Q (ft <sup>3</sup> /hr)		
20	0.0	310.5										
	0.5	310.6										
	1.0	310.7										
	1.5	310.8										
	2.0	310.9										
	2.5	311.0										
	3.0	311.1										
	3.5	311.2										
	4.0	311.3										
	4.5	311.4										
	5.0	311.5	—	1.0	0.20	32.65	46.2	—	78.85	416.8	10.57	



Length of Test Interval (ft)	C <sub>p</sub> Coefficient			
	0.0	0.5	1.0	1.5
1	0.000	0.000	0.000	0.000
2	0.000	0.000	0.000	0.000
3	0.000	0.000	0.000	0.000
4	0.000	0.000	0.000	0.000
5	0.000	0.000	0.000	0.000
6	0.000	0.000	0.000	0.000
7	0.000	0.000	0.000	0.000
8	0.000	0.000	0.000	0.000
9	0.000	0.000	0.000	0.000
10	0.000	0.000	0.000	0.000
11	0.000	0.000	0.000	0.000
12	0.000	0.000	0.000	0.000

C<sub>p</sub> = (1/(2 \* 3.14 \* 10)) \* (70321)  
 2.48 Gallons = 1 ft<sup>3</sup>  
 1 psi = 2.31 ft head

Remarks: Test Interval 10.5-11.5 SWL 28.85 Sewer Height 3.8  
Packer Pressure 210 PSI Borehole Diameter 3"

M<sub>1</sub> is used when the test length is below the water table.  
 M<sub>2</sub> is used when the test length is above the water table.

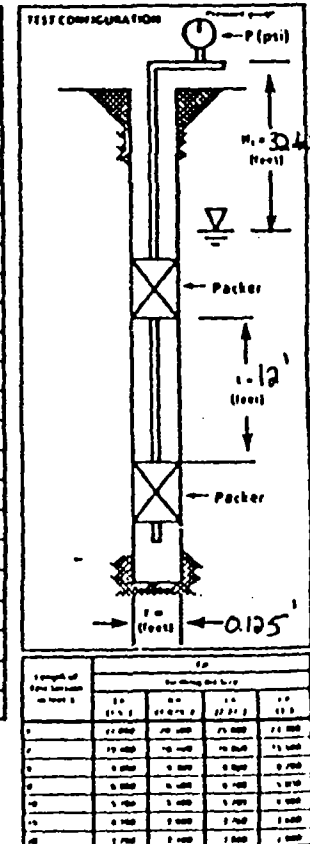


PROJECT: Kellogg-Deering  
 BORING NO.: K22B  
 DEPTH OF CASING: \_\_\_\_\_ FEET

PACKER TEST REPORT  
 PROJECT NO.: 1670  
 CONTRACTOR: Empire Soils  
 BY: S. Keall

PAGE 1 OF 1

Flow Test				Calculated Results								
Casing ID (in)	Depth (ft)	Flow Reading	Flow Rate Q (gpm)	Flow Rate Q (gpm)	Flow Rate Q (gpm)	$H_1$ (ft)	$H_2 = 2.31 P$	$H_2$ (ft)	$C_1 = H_1 + (H_1 \text{ or } H_2)$	CP	$K (Darcy) = C_p QH$	$K (Darcy) = \frac{C_p Q H}{9.87 \times 10^{-5}}$
20	0.0	310.5										
	0.5	310.6										
	1.0	310.7										
	1.5	310.8										
	2.0	310.9										
	2.5	311.0										
	3.0	311.1										
	3.5	311.2										
	4.0	311.3										
	4.5	311.4										
	5.0	311.5	—	1.0	0.20	32.65	46.2	—	78.85	411.8	10.57	102816 <sup>-5</sup>



CP =  $(2.31) \times (H_1) \times (Q)$

7.48 Gallons = 1 ft<sup>3</sup>

1 psi = 2.31 ft head

Remarks

Test Interval 10.5-11.5 SWL 28.85 Swivel Height 3.8

Packer Pressure 310 PSI Borehole Diameter 3"

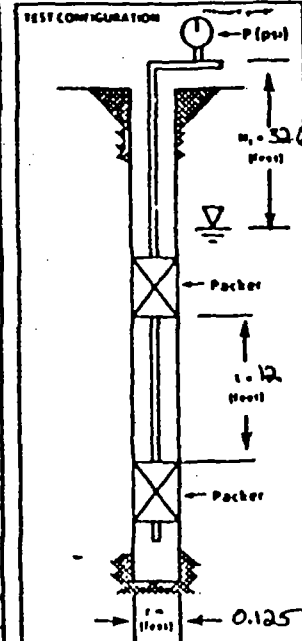
H<sub>1</sub> is used when the test length is below the water table.  
 H<sub>2</sub> is used when the test length is above the water table.

PROJECT: Kellogg-Deering  
 BORING NO.: K23B  
 DEPTH OF CASING: \_\_\_\_\_ FEET

PACKER TEST REPORT  
 PROJECT NO.: 1670  
 CONTRACTOR: Empire Soils  
 BY: S. Keall

PAGE 1 OF 1

Flow Test			Calculated Results									
Casing ID (in.)	Elapsed Time (min)	Flow Reading	8 Flow SG (lb/l)	8 Flow SG (g/cc)	Flow Rate Q (gallons/min)	$H_1$ (ft)	MP = 2.31 p	$H_2$ (ft)	$W = MP \times (D1^2 - D2^2)$	CP	$E (ft/yr) = C_p \times QW$	$C (inches) = \frac{E \times 12 \times 10^7}{K (ft/yr)}$
40	0.0	312.40										
	0.5	312.55										
	1.0	312.65										
	1.5	312.80										
	2.0	312.90										
	2.5	313.05										
	3.0	313.15										
	3.5	313.30										
	4.0	313.40										
	4.5	313.55										
	5.0	313.65	—	1.25	0.25	32.65	92.4	—	125.05	4168	9.33	305810 <sup>6</sup>



Length of Test Interval (ft)	Flow Rate (gpm)			
	1.0	2.0	3.0	4.0
1.0	11.000	22.000	33.000	44.000
2.0	19.000	38.000	57.000	76.000
3.0	27.000	54.000	81.000	108.000
4.0	35.000	70.000	105.000	140.000
5.0	43.000	86.000	129.000	173.000
6.0	51.000	102.000	153.000	206.000
7.0	59.000	118.000	177.000	239.000
8.0	67.000	134.000	201.000	272.000
9.0	75.000	150.000	225.000	305.000
10.0	83.000	166.000	249.000	338.000

CP = (1/12 \* 1.12 \* 1.12) / (170321)  
 7.48 Gallons = 1 ft<sup>3</sup>  
 1 psi = 2.31 ft head

Remarks: Test Interval = 101.5' - 113.5' SWL = 28.85' Summit Height = 3.8'

Packer Pressure = 710 PSI      Borehole Diameter = 3"

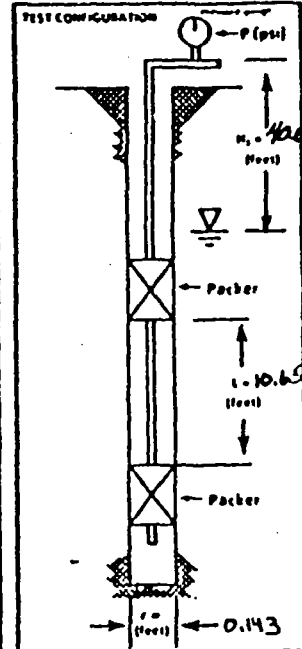
H<sub>1</sub> is used when the test length is below the water table  
 H<sub>2</sub> is used when the test length is above the water table

PROJECT: Kellogg-Deering  
 BORING NO.: K24  
 DEPTH OF CASING: \_\_\_\_\_ FEET

PACKER TEST REPORT  
 PROJECT NO.: L170  
 CONTRACTOR: Empire Soils  
 BY: D. Blackert

PAGE 1 OF 1

Flow Test					Calculated Results							
TIME (min)	UPPER PRESS. (psi)	Flow Reading	Δ Flow, ΔQ (ft³)	Δ Flow, ΔQ (gpm)	Flow Rate Q (gpm)	H <sub>1</sub> (ft)	H <sub>2</sub> = 2.31 p (ft)	H <sub>2</sub> (ft)	H = H <sub>1</sub> - H <sub>2</sub> (ft or m)	C <sub>D</sub>	Σ (H <sub>1</sub> H <sub>2</sub> ) = C <sub>D</sub> QM	Σ (H <sub>1</sub> H <sub>2</sub> ) = 5.67 × 10 <sup>-4</sup> K (H <sub>1</sub> H <sub>2</sub> )
20	0.00	365.0										
	0.25	366.0										
	0.50	367.0										
	0.75	367.8										
	1.00	368.8										
	1.25	369.7										
	1.50	370.6										
	1.75	371.5										
	2.00	372.5										
	2.25	373.4										
	2.50	374.2										
	2.75	375.1										
	3.00	376.1										
	3.25	377.0										
	3.50	377.9										
	3.75	378.5										
	4.00	379.3										
	4.25	380.2										
	4.50	381.1										
	4.75	382.0										
	5.00	383.0										
			18.0	3.60	40.6	46.2			86.8	4528.9	157.83	1.81 × 10 <sup>-4</sup>



Length of Test Interval (feet)	Flow Rate (gpm)			
	10	20	30	40
1	11.000	11.000	11.000	11.000
2	11.000	11.000	11.000	11.000
3	11.000	11.000	11.000	11.000
4	11.000	11.000	11.000	11.000
5	11.000	11.000	11.000	11.000
6	11.000	11.000	11.000	11.000
7	11.000	11.000	11.000	11.000
8	11.000	11.000	11.000	11.000
9	11.000	11.000	11.000	11.000
10	11.000	11.000	11.000	11.000

CP = (1/2 \* 114 \* 11) / (70321)

7.48 Gallons = 1 ft<sup>3</sup>

1 psi = 2.31 ft head

Remarks:

Test Interval 173.85-184.5 SWL = 33.5' Sealed Height = 21'

Packer Pressure = 210 PSI Borehole Diameter = 3.4"

H<sub>1</sub> is used when the test length is below the water table

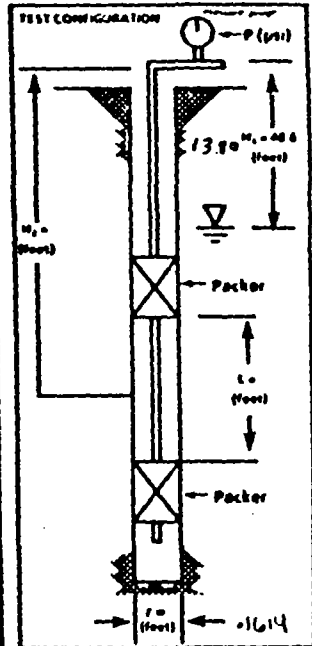
H<sub>2</sub> is used when the test length is above the water table

PROJECT: L670 Kellogg-Drains  
 BORING NO.: K193  
 DEPTH OF CASING: 12 FEET

PACKER TEST REPORT  
 PROJECT NO.: L670  
 CONTRACTOR: RJS  
 BY: Scott M. Kroll 3-15-04

PAGE 1 OF 4

Flow Test					Calculated Results							
Pressure, P (psi)	Elapsed Time (min)	Flow Reading (gpm)	Δ Flow, ΔQ (l/min)	Δ Flow, ΔQ (gal)	Flow Rate Q (gpm)	H <sub>1</sub> (ft)	H <sub>2</sub> - 2.31 P (ft)	H <sub>2</sub> (ft)	H = H <sub>1</sub> + (H <sub>2</sub> or H <sub>1</sub> )	C <sub>P</sub>	K (ft <sup>2</sup> /gpm) = C <sub>P</sub> Q <sup>2</sup>	K (ft <sup>2</sup> /gpm) = 9.87 × 10 <sup>-5</sup> K (ft <sup>2</sup> /gpm)
200	0	10519.5	0.0	0.0								
	05	10519.5	0.0	0.0								
	10	10519.5	0.0	0.0								
	15	10519.5	0.0	0.0								
	20	10519.5	0.0	0.0								
	25	10519.5	0.0	0.0								
	30	10519.5	0.0	0.0								
	35	10519.5	0.0	0.0								
	40	10519.5	0.0	0.0								
	45	10519.5	0.0	0.0								
	50	10519.5	0.0	0.0								
	55	10519.5	0.0	0.0								
	60	10519.5	0.0	0.0								
	65	10519.5	0.0	0.0								
	70	10519.5	0.0	0.0								
	75	10519.5	0.0	0.0								
	80	10519.5	0.0	0.0								
	85	10519.5	0.0	0.0								
	90	10519.5	0.0	0.0								
	95	10519.5	0.0	0.0								
✓	1000	10519.5	0.0	0.0								



Length of Test Section (feet)	C <sub>P</sub>			
	1.0	2.0	3.0	4.0
1	21,000	42,000	63,000	84,000
2	10,500	21,000	31,500	42,000
3	7,000	14,000	21,000	28,000
4	5,250	10,500	15,750	21,000
5	4,200	8,400	12,600	16,800
6	3,500	7,000	10,500	14,000
7	3,000	6,000	9,000	12,000
8	2,625	5,250	7,875	10,500
9	2,333	4,667	7,000	9,333
10	2,100	4,200	6,300	8,400

CP = (142 x LB x L) / (70321)  
 7.48 Gallons = 1 ft<sup>3</sup>  
 1 psi = 2.31 ft head  
 Remarks: PACKER INTERVAL 36'-58' PACKER PRESSURE 240 psi  
Subsided to ground 4.7 ft, packer is not present because packer did not hook from yesterday 2-23-04 (1590)  
NO WATER FLOW TO BE OBSERVED

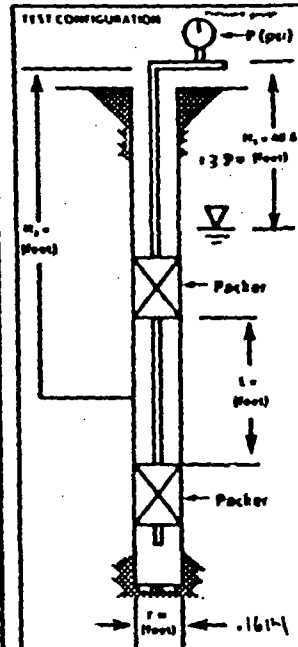
H<sub>1</sub> is used when the test length is below the water table  
 H<sub>2</sub> is used when the test length is above the water table

PROJECT: L 670 Kellogg - Decatur  
 BORING NO: K1913  
 DEPTH OF CASING: 18 FEET

PACKER TEST REPORT  
 PROJECT NO: L670 03  
 CONTRACTOR: NDS  
 BY: Scott M. Krall 3-17-88

PAGE 2 OF 4

Flow Test					Calculated Results							
Pressure, P (psi)	Elapsed Time (min)	Flow Reading (GAL)	Δ Flow, ΔQ (ft³)	Δ Flow, ΔQ (gals)	Flow Rate Q (gpm)	H <sub>1</sub> (ft)	H <sub>2</sub> - 2.31 P (ft)	H <sub>2</sub> (ft)	H = H <sub>2</sub> - H <sub>1</sub> (ft)	C <sub>D</sub>	Σ (H <sub>2</sub> P) - C <sub>D</sub> QM	Σ (gpm)² / K (ft³)
20	0	10476.0	0.0	0.0								
	0.5	10476.0	0.0	0.0								
	1.0	10476.0	0.0	0.0								
	1.5	10476.0	0.0	0.0								
	2.0	10476.0	0.0	0.0								
	2.5	10476.0	0.0	0.0								
	3.0	10476.0	0.0	0.0								
	3.5	10476.0	0.0	0.0								
	4.0	10476.0	0.0	0.0								
	4.5	10476.0	0.0	0.0								
	5.0	10476.0	0.0	0.0								
	5.5	10476.0	0.0	0.0								
	6.0	10476.0	0.0	0.0								
	6.5	10476.0	0.0	0.0								
	7.0	10476.0	0.0	0.0								
	7.5	10476.0	0.0	0.0								
	8.0	10476.0	0.0	0.0								
	8.5	10476.0	0.0	0.0								
	9.0	10476.0	0.0	0.0								
	9.5	10476.0	0.0	0.0								
	10.0	10476.0	0.0	0.0								



Length of Test Section (ft)	C <sub>D</sub>			
	0.5	1.0	1.5	2.0
1	21,000	24,000	27,000	30,000
2	10,500	12,000	13,500	15,000
3	7,000	8,000	9,000	10,000
4	5,250	6,000	6,750	7,500
5	4,200	4,800	5,400	6,000
6	3,500	4,000	4,500	5,000
7	3,000	3,500	3,900	4,300
8	2,625	3,150	3,450	3,750
9	2,325	2,850	3,150	3,450
10	2,100	2,600	2,900	3,200

C<sub>D</sub> = (1/12 \* 3) \* (12) (70321)  
 7.48 Gallons = 1 ft³

0 gpm

1 psi = 2.31 ft head  
 Remarks: 54 in to gravel level 4.7, Packer interval 46'-58', Packer pressure 220 PSI

5.26 13.90

PAVILION TAKES NO WATER

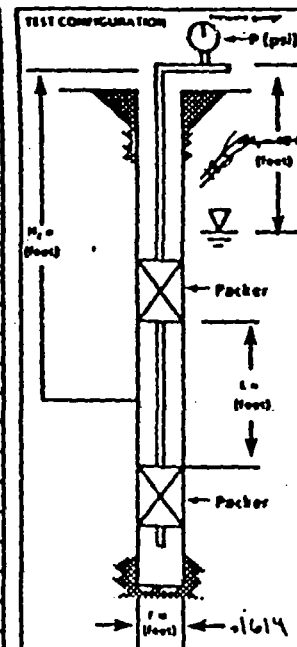
H<sub>1</sub> is used when the test length is below the water table  
 H<sub>2</sub> is used when the test length is above the water table

PROJECT: Kellogg-Dearing  
 BORING NO.: 118  
 DEPTH OF CASING: Packer Bottom 92.5 FEET

PACKER TEST REPORT  
 PROJECT NO.: 1670  
 CONTRACTOR: Empire  
 BY: Don Blakely

PAGE 1 OF 1

Flow Test			Calculated Results									
Packer L.R. (ft)	Losses (ft)	Flow Reading	$\Delta$ Flow, $\Delta Q$ (ft)	$\Delta$ Flow, $\Delta Q$ (ft)	Flow Rate $Q$ (gpm)	$H_1$ (ft)	ESR = 2.31 p	$H_2$ (ft)	$S = \frac{H_1 - H_2}{Q}$ (ft/gpm)	$C_p$	$S (H_1) = C_p Q^{1.85}$	$\frac{S (actual) - S (theor)}{S (theor)}$
20	0	0										
	0.25											
	0.50											
	0.75											
	1.00											
	1.25											
	1.50											
	1.75											
	2.00											
	2.25											
	2.50											
	2.75											
	3.00											
	3.25											
	3.50											
	3.75											
	4.00											
	4.25											
	4.50											
	4.75											
	5.00											



Length of Test Section (ft)	$C_p$			
	0.1	0.2	0.5	1.0
1	11,000	21,000	70,000	21,000
2	19,000	36,000	110,000	33,000
3	25,000	48,000	140,000	42,000
4	30,000	58,000	160,000	48,000
5	34,000	66,000	175,000	52,000
6	37,000	73,000	185,000	55,000
7	40,000	79,000	195,000	58,000
8	42,000	84,000	200,000	60,000
9	44,000	89,000	205,000	62,000
10	45,000	92,000	208,000	63,000

$C_p = (1.42 + 0.114L)(70321)$   
 7.48 Gallons = 1 ft<sup>3</sup>  
 1 psi = 2.31 ft head

0 gpm

Remarks:

Screen height 5ft. Packer Pressure 150PSI Depth to bottom of packer 92.5'  
 FORMATION DID NOT TAKE WATER. Seal 15.3'

H<sub>1</sub> is used when the test length is below the water table.  
 H<sub>2</sub> is used when the test length is above the water table







HYDRAULIC CONDUCTIVITY TESTING DATA SHEET

NUS CORPORATION

PROJECT NAME: Kellogg-Deering  
 PROJECT NO: L670  
 WELL DIAMETER: 6"  
 STATIC WATER LEVEL (Depth/Elevation): 33.37  
 TEST TYPE (Rising/Falling/Constant Head): Rising Head  
 METHOD OF INDUCING WATER LEVEL CHANGE: Solid Slug  
 GEOLOGIST: Don Blackert  
 SCREEN LENGTH/DEPTH: Open Borehole of 3.16 to 4.90  
 WELLBORING NO: K6B  
 TEST NO: 2  
 DATE: 6/9/88  
 CHECKED:  
 PAGE 1 OF 1

TIME	ELAPSED TIME (min or sec)	MEASURED DEPTH TO WATER (ft.)	CORRECTION	DEPTH TO WATER (ft.)	DRAWDOWN OR HEAD (ft.)	REMARKS
1109	0.00 min	33.37	0	33.37	1.20	Data Collected Using
	0.01				0.96	Hermit Environmental Da
	0.06				0.81	Logger, Model SE1000B
	0.11				0.73	
	0.16				0.67	Only Data Used For
	0.26				0.57	Graphing Purposes is
	0.39				0.46	Listed. Additional
	0.56				0.37	Data was Recorded
	0.81				0.26	by the Instrument.
	1.23				0.16	
	1.81	↓	↓	↓	0.08	





HYDRAULIC CONDUCTIVITY TESTING DATA SHEET

NUS CORPORATION

PROJECT NAME: Kellogg-Daering WELLBORING NO.: K23B  
 PROJECT NO.: L670 GEOLOGIST: Don Blackert  
 WELL DIAMETER: 2" SCREEN LENGTH/DEPTH: 10'  
 STATIC WATER LEVEL (Depth/Elevation): 33.48 / 35.20 DATE: 6/8/88  
 TEST TYPE (Rising/Falling/Constant Head): Falling Head CHECKED: JPD 7/11/88  
 METHOD OF INDUCING WATER LEVEL CHANGE: Solid Slug PAGE 1 OF 1

TIME	ELAPSED TIME (min or sec)	MEASURED DEPTH TO WATER (ft.)	CORRECTION	DEPTH TO WATER (ft.)	DRAWDOWN OR HEAD (ft.)	REMARKS
1910	0.00 min	NA <del>33.48</del>	0	NA <del>33.48</del>	2.04	Data Collected Using
	0.10				1.95	Hermit Environmental Data
	0.15				1.89	Logger, Model SF1000B.
	0.30				1.76	
	0.63				1.50	Only Data Used For
	0.96				1.28	Graphing Purposes is
	1.13				1.19	Listed. Additional
	1.38				1.06	Data was Recorded
	1.72				0.90	by the Instrument.
	2.88				0.53	
	4.88				0.21	
	6.38				0.10 JPD <del>0.08</del>	
	7.88				0.05	
	9.38				0.02	
	12.00	↓	↓	↓	0.00	











**HYDRAULIC CONDUCTIVITY TESTING DATA SHEET** **NUS CORPORATION**

PROJECT NAME: Kellogg-Deering WELLBORING NO.: K20A  
 PROJECT NO.: L670 GEOLOGIST: Don Blackert  
 WELL DIAMETER: 2" SCREEN LENGTH/DEPTH: 1 45.10 TEST NO.: 2  
 STATIC WATER LEVEL (Depth/Elevation): 20.92 DATE: 6/8/88  
 TEST TYPE (Rising/Falling/Constant Head): Falling Head CHECKED: JPD 7/11/88  
 METHOD OF INDUCING WATER LEVEL CHANGE: Solid Slug PAGE 1 OF 1

TIME	ELAPSED TIME (min or sec.)	MEASURED DEPTH TO WATER (ft.)	CORRECTION	DEPTH TO WATER (ft.)	DRAWDOWN OR HEAD (ft.)	REMARKS
1000	0.00 Min	NA 20.92	0	NA 20.92	2.20	Data Collected Using
	0.05				2.04	Hermit Environmental Data
	0.10				1.91	Logger, Model SF1000B.
	0.15				1.79	
	0.20				1.69	Only Data Used For
	0.40				1.35	Graphing Purposes is
	0.65				1.03	Listed. Additional
	0.90				0.79	Data was Recorded
	1.15				0.61	by the Instrument.
	1.40 <del>1.45</del> JPD				0.48	
	1.65				0.37	
	1.90				0.29	
	2.40				0.18	
	2.90				0.12	

BEING TYPED



**HYDRAULIC CONDUCTIVITY TESTING DATA SHEET**

**NUS CORPORATION**

PROJECT NAME: Kellogg-Daering WELLBORING NO.: K23A  
 PROJECT NO.: L670 GEOLOGIST: Don Blackert  
 WELL DIAMETER: 2" SCREEN LENGTH/DEPTH: 2.17' Below SWL TEST NO.: 2  
 STATIC WATER LEVEL (Depth/Elevation): 31.33 / 32.67 DATE: 6/8/88  
 TEST TYPE (Rising/Falling/Constant Head): Rising Head CHECKED: JFD 7/1/88  
 METHOD OF INDUCING WATER LEVEL CHANGE: Solid Slug PAGE 1 OF 1

TIME	ELAPSED TIME (min. or sec.)	MEASURED DEPTH TO WATER (ft.)	CORRECTION	DEPTH TO WATER (ft.)	DRAWDOWN OR HEAD (ft.)	REMARKS
2207	0.00 Min	NA 31.33	0	NA 31.33	1.68	Data Collected Using
	0.01				1.46	Hermit Environmental Data
	0.03				1.26	Logger, Model SE1000B
	0.08				0.92	
	0.13				0.79	Only Data Points Used
	0.18				0.52	For Graphing Purposes are
	0.23				0.38	shown. Additional Data
	0.28				0.29	was Recorded by time
	0.48				0.11	Instrument.
	0.73				0.05	



**HYDRAULIC CONDUCTIVITY TESTING DATA SHEET**

**NUS CORPORATION**

PROJECT NAME: Kellogg-Deering WELL/BORING NO.: K20C  
 PROJECT NO.: L670 GEOLOGIST: Don Blackert  
 WELL DIAMETER: 2 SCREEN LENGTH/DEPTH: 3.87' Below SWL TEST NO.: 2  
 STATIC WATER LEVEL (Depth/Elevation): 22.33 (45.61) DATE: 6/18/88  
 TEST TYPE (Rising/Falling/Constant Head): Rising Head CHECKED: Jrs 7/11/88  
 METHOD OF INDUCING WATER LEVEL CHANGE: Solid Slug PAGE 1 OF 1

TIME	ELAPSED TIME (min. or sec.)	MEASURED DEPTH TO WATER (ft.)	CORRECTION	DEPTH TO WATER (ft.)	DRAWDOWN OR HEAD (ft.)	REMARKS
1155	0.00 min	<del>22.33</del> <sup>NA</sup>	0	<del>22.33</del> <sup>NA</sup>	1.87	Data Collected Using
	0.01				1.77	Hermit Environmental Data
	0.05				1.53	Logger, Model SE1000B
	0.10				1.34	
	0.15				1.18	Only Data Used For
	0.20				1.04	Graphing Purposes is
	0.25				0.91	Listed Additional
	0.30				0.81	Data Was Recorded by
	0.40				0.63	the Instrument
	0.57				0.44	
	0.65	↓	↓	↓	0.36	

HYDRAULIC CONDUCTIVITY TESTING DATA SHEET

NUS CORPORATION

PROJECT NAME: Kellogg-Dearing ..... WELLBORING NO.: K206  
 PROJECT NO.: L670 ..... GEOLOGIST: Don Blackert  
 WELL DIAMETER: 2" ..... SCREEN LENGTH/DEPTH: 3.82' Below SWL. TEST NO.: 1  
 STATIC WATER LEVEL (Depth/Elevation): 22.33 ..... 145.61 ..... DATE: 6/8/88  
 TEST TYPE (Rising/Falling/Constant Head): Rising Head ..... CHECKED: JFA 7/11/88  
 METHOD OF INDUCING WATER LEVEL CHANGE: Solid Slug ..... PAGE 1 OF 1

TIME	ELAPSED TIME (min. or sec.)	MEASURED DEPTH TO WATER (ft.)	CORRECTION	DEPTH TO WATER (ft.)	DRAWDOWN OR HEAD (ft.)	REMARKS
1245	0.00 min	<del>22.35</del> NA	0	<del>22.33</del> NA	1.96	Data Collected Using
	0.01	↓	↓	↓	1.78	Hermit Environmental Data
	0.02	↓	↓	↓	1.66	Logger, Model SE1000 B
	0.07	↓	↓	↓	1.33	
	0.12	↓	↓	↓	1.12	Only Data Used For
	0.17	↓	↓	↓	0.95	Graphing Purposes is
	0.22	↓	↓	↓	0.82	Listed. Additional
	0.32	↓	↓	↓	0.60	Data Was Recorded by the Instrument

PROJECT NAME: Kellogg-Deering	WELL/BORING NO.: K19A
PROJECT NO.: L670	GEOLOGIST: Don Blackert
WELL DIAMETER: 2"	SCREEN LENGTH/DEPTH: 4.13' Below SWL
STATIC WATER LEVEL (Depth/Elevation): 12.27	TEST NO.: 2
TEST TYPE (Rising/Falling/Constant Head): Rising Head	DATE: 6/8/88
METHOD OF INDUCING WATER LEVEL CHANGE: Solid Slug	CHECKED: JPO 7/11/88
	PAGE 1 OF 1

TIME	ELAPSED TIME (min or sec)	MEASURED DEPTH TO WATER (ft.)	CORRECTION	DEPTH TO WATER (ft.)	DRAWDOWN OR HEAD (ft.)	REMARKS
1450	0.00	12.27 NA	0	12.27 NA	1.93	Data Collected Using
	0.01				1.76	Hermit Environmental Data
	0.02				1.62	Logger, Model SE1000B
	0.04				1.18	
	0.11				0.75	Only Data Used For
	0.16				0.45	Graphing Purposes as
	0.21				0.25	Listed. Additional
	0.26				0.14	Data Was Recorded by
	0.31	↓	↓	↓	0.07	the Instrument

HYDRAULIC CONDUCTIVITY TESTING DATA SHEET

NUS CORPORATION

PROJECT NAME: Kellogg - Decring ..... WELLBORING NO.: K19A  
PROJECT NO.: L670 ..... GEOLOGIST: Dan Blackett  
WELL DIAMETER: 2" ..... SCREEN LENGTH/DEPTH: 4.13' Below SWL ..... TEST NO.: 1  
STATIC WATER LEVEL (Depth/Elevation): 12.27 / 67.70 ..... DATE: 6/8/88  
TEST TYPE (Rising/Falling/Constant Head): Rising Head ..... CHECKED: JPB 7/11/88  
METHOD OF INDUCING WATER LEVEL CHANGE: Solid Slug ..... PAGE 1 OF 1

TIME	ELAPSED TIME (min or sec.)	MEASURED DEPTH TO WATER (ft.)	CORRECTION	DEPTH TO WATER (ft.)	DRAWDOWN OR HEAD (ft.)	REMARKS
1453	0.00	<del>12.27</del> NA	0	<del>12.27</del> NA	2.10	Data collected using
	0.01				1.82	Hermit Environmental
	0.02				1.70	Data logger Model
	0.06				1.26	SE 1000B
	0.11				0.83	
	0.16				0.52	Only Data Points Used
	0.21				0.31	For Graphing Purposes
	0.26				0.18	is listed. Additional
	0.31	↓	↓	↓	0.10	Data Was Recorded by the Instrument



SE:0006  
Environmental Logger  
06/14 14:25

Units 00000 Tests 9

INPUT 1: Level (F)

Reference 0.00  
Scale factor 10.04  
Offset 0.00

Step 1 06/09 12:17

*K9B Rising Head*

Elapsed Time	Value
0.0000	- 0.41
0.0033	- 0.17
0.0066	- 0.35
0.0099	- 0.50
0.0133	- 0.66
0.0166	- 0.74
0.0200	- 0.79
0.0233	- 0.76
0.0266	- 0.79
0.0300	- 0.77
0.0333	- 0.77
0.0366	- 0.75
0.0400	- 0.75
0.0433	- 0.74
0.0466	- 0.73
0.0500	- 0.73
0.0533	- 0.72
0.0566	- 0.72
0.0600	- 0.71
0.0633	- 0.71
0.0666	- 0.70
0.0700	- 0.69
0.0733	- 0.69
0.0766	- 0.69
0.0800	- 0.66
0.0833	- 0.66
0.0866	- 0.67
0.0900	- 0.67
0.0933	- 0.66
0.0966	- 0.66
0.1000	- 0.64
0.1033	- 0.62
0.1066	- 0.61
0.1100	- 0.59
0.1133	- 0.57
0.1166	- 0.56
0.1200	- 0.55
0.1233	- 0.53
0.1266	- 0.52
0.1300	- 0.51
0.1333	- 0.50
0.1366	- 0.49
0.1400	- 0.48
0.1433	- 0.47
0.1466	- 0.46
0.1500	- 0.45
0.1533	- 0.45
0.1566	- 0.44
0.1600	- 0.43

2.0000	-	0.42
2.5000	-	0.37
3.0000	-	0.32
3.5000	-	0.29
4.0000	-	0.26
4.5000	-	0.23
5.0000	-	0.21
5.5000	-	0.19
6.0000	-	0.17
6.5000	-	0.15
7.0000	-	0.14
7.5000	-	0.13
8.0000	-	0.12
8.5000	-	0.10
9.0000	-	0.09
9.5000	-	0.09
10.0000	-	0.07
12.0000	-	0.05

END

SE10005  
Environmental Logger  
06/14 14:22

Units 00000 Tests 9

INPUT 1: Level (F)

Reference 0.00  
Scale factor 10.00  
Offset 0.00

Steps 0 06/05 11:50

Elapsed Time	Value
0.0000	1.07
0.0033	0.11
0.0066	0.52
0.0099	1.01
0.0133	0.96
0.0166	1.04
0.0200	0.92
0.0233	1.00
0.0266	1.04
0.0300	1.07
0.0333	1.09
0.0366	1.08
0.0400	0.95
0.0433	0.94
0.0466	0.93
0.0500	0.92
0.0533	0.91
0.0566	0.90
0.0600	0.82
0.0633	0.85
0.0666	0.86
0.0700	0.86
0.0733	0.85
0.0766	0.84
0.0800	0.83
0.0833	0.82
0.0866	0.81
0.0900	0.81
0.0933	0.77
0.0966	0.75
0.1000	0.72
0.1033	0.70
0.1066	0.68
0.1100	0.66
0.1133	0.65
0.1166	0.64
0.1200	0.62
0.1233	0.60
0.1266	0.58
0.1300	0.57
0.1333	0.56
0.1366	0.54
0.1400	0.52
0.1433	0.51
0.1466	0.50
0.1500	0.48
0.1533	0.48

*R19B Falling Head*

2.0000	0.45
2.5000	0.35
3.0000	0.35
3.5000	0.31
4.0000	0.27
4.5000	0.24
5.0000	0.21
5.5000	0.19
6.0000	0.17
6.5000	0.15
7.0000	0.14
7.5000	0.12
8.0000	0.11
8.5000	0.10
9.0000	0.09
9.5000	0.08
10.0000	0.08
12.0000	0.06
14.0000	0.04
16.0000	0.03
18.0000	0.02
20.0000	0.02
22.0000	0.02
24.0000	0.01
26.0000	0.01

END

SE:0006  
Environmental Logger  
06/14 14:21

Units 00000 Tests 6

INPUT 1: Level (F)

Reference 0.00  
Scale factor 10.04  
Offset 0.00

*KGB Rising Head*

Step 1 06/09 11:09

Elapsed Time	Value
0.0000	0.00
0.0033	0.50
0.0066	0.55
0.0099	0.00
0.0133	0.95
0.0166	1.01
0.0200	0.92
0.0233	1.20
0.0266	0.72
0.0300	0.79
0.0333	0.96
0.0366	0.68
0.0400	0.64
0.0433	0.61
0.0466	0.78
0.0500	0.75
0.0533	0.73
0.0566	0.71
0.0600	0.69
0.0633	0.67
0.0666	0.65
0.0700	0.63
0.0733	0.62
0.0766	0.60
0.0800	0.58
0.0833	0.57
0.0866	0.56
0.0900	0.54
0.0933	0.53
0.0966	0.46
0.1000	0.42
0.1033	0.37
0.1066	0.33
0.1100	0.30
0.1133	0.26
0.1166	0.24
1.0000	0.22
1.0633	0.19
1.1266	0.16
1.1900	0.16
1.2533	0.14
1.3166	0.13
1.3800	0.12
1.4433	0.11
1.5066	0.10
1.5700	0.09
1.6333	0.08

1.0000	-	0.06
2.5000	-	0.07
3.0000	-	0.07
3.5000	-	0.08
4.0000	-	0.08
4.5000	-	0.08
5.0000	-	0.08
5.5000	-	0.08
6.0000	-	0.08
6.5000	-	0.08
7.0000	-	0.08
7.5000	-	0.08
8.0000	-	0.08
8.5000	-	0.08
9.0000	-	0.08
9.5000	-	0.08
10.0000	-	0.08

END

SE1000B  
 Environmental Logger  
 06/14 14:19

Units 00000 Tests 6

INPUT 1: Level (F)

Reference 0.00  
 Scale factor 10.01  
 Offset 0.00

Step 0 06/09 10:52

*K6B Falling Head*

Elapsed Time	Value
0.0000	0.00
0.0033	0.00
0.0066	0.00
0.0099	0.33
0.0133	0.63
0.0166	0.73
0.0200	0.62
0.0233	1.22
0.0266	1.15
0.0300	1.10
0.0333	1.05
0.0366	1.02
0.0400	0.95
0.0433	0.91
0.0466	0.90
0.0500	0.65
0.0533	0.66
0.0566	0.74
0.0600	0.76
0.0633	0.75
0.0666	0.72
0.0700	0.69
0.0733	0.66
0.0766	0.66
0.0800	0.64
0.0833	0.62
0.0866	0.61
0.0900	0.59
0.0933	0.57
0.0966	0.59
0.1000	0.45
0.1033	0.44
0.1066	0.36
0.1100	0.34
0.1133	0.30
0.1166	0.27
1.0000	0.24
1.0633	0.21
1.1667	0.19
1.2500	0.16
1.3333	0.16
1.4166	0.14
1.5000	0.13
1.5833	0.12
1.6667	0.11
1.7500	0.10
1.8333	0.09

2.0000	0.07
2.5000	0.05
3.0000	0.03
3.5000	0.02
4.0000	0.02
4.5000	0.01
5.0000	0.01
5.5000	0.01
6.0000	0.01
6.5000	0.01
7.0000	0.01
7.5000	0.01
8.0000	0.01
8.5000	0.01
9.0000	0.01
9.5000	0.01
10.0000	0.01
12.0000	0.01
14.0000	0.01
16.0000	0.01

END



SE10006  
 Environmental Logger  
 06/14 14:17

Units 00000 Tests 7

INPUT 1: Level (F)

Reference 0.00  
 Scale factor 10.00  
 Offset 0.00

Start 1 06/06 20:07

Elapsed Time	Value
0.0000	- 2.27
0.0033	- 2.06
0.0066	- 1.96
0.0099	- 0.06
0.0133	- 0.79
0.0166	- 1.46
0.0200	- 1.66
0.0233	- 1.57
0.0266	- 1.51
0.0300	- 1.46
0.0333	- 1.41
0.0500	- 1.26
0.0666	- 1.11
0.0833	- 1.02
0.1000	- 0.92
0.1166	- 0.84
0.1333	- 0.76
0.1500	- 0.69
0.1666	- 0.63
0.1833	- 0.57
0.2000	- 0.52
0.2166	- 0.47
0.2333	- 0.43
0.2500	- 0.38
0.2666	- 0.35
0.2833	- 0.32
0.3000	- 0.29
0.3166	- 0.26
0.3333	- 0.24
0.4167	- 0.15
0.5000	- 0.11
0.5833	- 0.06
0.6667	- 0.05
0.7500	- 0.05
0.8333	- 0.04
0.9167	- 0.03
1.0000	- 0.02
1.0833	- 0.02
1.1667	- 0.01
1.2500	- 0.01
1.3333	- 0.01
1.4166	- 0.00
1.5000	- 0.00
1.5833	- 0.00
1.6667	- 0.00
1.7500	- 0.00
1.8333	- 0.00

*K23A Rising Head*

SE10006  
Environmental Logger  
06/14 14:16

Units 00000 Tests 7

INPUT 1: Level (F)

Reference 0.00  
Scale factor 10.00  
Offset 0.00

Step 0 06/06 19:51

Elapsed Time	Value
0.0000	- 0.51
0.0033	- 1.97
0.0066	- 1.99
0.0099	- 0.25
0.0133	- 0.92
0.0166	- 1.22
0.0200	- 1.77
0.0233	- 1.61
0.0266	- 1.54
0.0300	- 1.46
0.0333	- 1.44
0.0366	- 1.26
0.0400	- 1.14
0.0433	- 1.04
0.0466	- 0.94
0.0500	- 0.85
0.0533	- 0.79
0.0566	- 0.71
0.0600	- 0.85
0.0633	- 0.59
0.0666	- 0.54
0.0700	- 0.46
0.0733	- 0.44
0.0766	- 0.46
0.0800	- 0.36
0.0833	- 0.32
0.0866	- 0.30
0.0900	- 0.27
0.0933	- 0.25
0.0966	- 0.16
0.1000	- 0.12
0.1033	- 0.06
0.1066	- 0.67
0.1100	- 0.65
0.1133	- 0.64
0.1166	- 0.63
1.0000	- 0.63
1.0633	- 0.62
1.1667	- 0.62
1.2500	- 0.62
1.3333	- 0.61
1.4166	- 0.61
1.5000	- 0.61
1.5833	- 0.61
1.6667	- 0.61
1.7500	- 0.61
1.8333	- 0.61

*1138*

*K23A Rising Head*

2.0000	-	0.01
2.5000	-	0.01
3.0000	-	0.01
3.5000	-	0.01
4.0000	-	0.01
4.5000	-	0.01
5.0000	-	0.01
5.5000	-	0.01
6.0000	-	0.01
6.5000	-	0.01
7.0000	-	0.01
7.5000	-	0.01
8.0000	-	0.01
8.5000	-	0.01
9.0000	-	0.01
9.5000	-	0.01
10.0000	-	0.01
12.0000	-	0.01
14.0000	-	0.02
16.0000	-	0.01

END

SE10005  
Environmental Logger  
06/14 14:13

Units 00000 Tests 5

INPUT 1: Level (F)

Reference 0.00  
Scale Factor 10.00  
Offset 0.00

*K24 Falling Head*

Steps 0 06/06 16:43

Elapsed Time	Value
0.0000	- 1.64
0.0033	- 3.86
0.0066	- 4.27
0.0099	- 6.74
0.0133	- 1.86
0.0166	- 2.13
0.0200	- 2.97
0.0233	- 3.16
0.0266	- 3.13
0.0300	- 3.14
0.0333	- 3.13
0.0500	- 3.07
0.0666	- 3.05
0.0833	- 3.03
0.1000	- 3.02
0.1166	- 3.02
0.1333	- 3.02
0.1500	- 3.01
0.1666	- 3.00
0.1833	- 3.00
0.2000	- 3.00
0.2166	- 2.99
0.2333	- 2.99
0.2500	- 2.99
0.2666	- 2.98
0.2833	- 2.98
0.3000	- 2.98
0.3166	- 2.97
0.3333	- 2.97
0.4167	- 2.96
0.5000	- 2.96
0.5633	- 2.95
0.6667	- 2.93
0.7500	- 2.94
0.8333	- 2.93
0.9167	- 2.93
1.0000	- 2.93
1.0633	- 2.92
1.1667	- 2.92
1.2500	- 2.92
1.3333	- 2.91
1.4166	- 2.91
1.5000	- 2.91
1.5633	- 2.91
1.6667	- 2.90
1.7500	- 2.90
1.8333	- 2.89

2.0000	-	2.93
2.5000	-	2.68
3.0000	-	2.27
3.5000	-	2.66
4.0000	-	2.65
4.5000	-	2.64
5.0000	-	2.63
5.5000	-	2.62
6.0000	-	2.61
6.5000	-	2.60
7.0000	-	2.79
7.5000	-	2.78
8.0000	-	2.77
8.5000	-	2.76
9.0000	-	2.75
9.5000	-	2.74
10.0000	-	2.73
12.0000	-	2.70
14.0000	-	2.66
16.0000	-	2.63
18.0000	-	2.59
20.0000	-	2.56
22.0000	-	2.52
24.0000	-	2.49
26.0000	-	2.48
28.0000	-	2.39
30.0000	-	2.37
32.0000	-	2.34
34.0000	-	2.32
36.0000	-	2.29
38.0000	-	0.63
40.0000	-	0.57
42.0000	-	0.53
44.0000	-	0.48
46.0000	-	0.36
48.0000	-	0.29

END

SEI0006  
Environmental Logger  
05/14 14:06

UNITY 00000 Tests 4

INPUT 1: Level (F)

REFERENCE 0.00  
Scale factor 10.04  
Offset 0.00

Steps 0 05/06 15:16

*KIBA Falling Head*

Elapsed Time	Value
0.0000	0.01
0.0033	0.01
0.0066	0.02
0.0099	0.01
0.0133	0.00
0.0166	7.50
0.0200	6.64
0.0233	7.69
0.0266	2.47
0.0300	4.51
0.0333	3.97
0.0366	2.67
0.0400	2.52
0.0433	2.91
0.0466	2.64
0.0500	3.02
0.0533	3.06
0.0566	2.52
0.0600	2.94
0.0633	2.93
0.0666	2.92
0.0700	2.94
0.0733	2.93
0.0766	2.93
0.0800	2.93
0.0833	2.93
0.0866	2.93
0.0900	2.93
0.0933	2.93
0.0966	2.93
0.1000	2.92
0.1033	2.92
0.1066	2.91
0.1100	2.91
0.1133	2.90
0.1166	2.90
0.1200	2.90
0.1233	2.89
0.1266	2.89
0.1300	2.88
0.1333	2.88
0.1366	2.87
0.1400	2.87
0.1433	2.86
0.1466	2.86
0.1500	2.86
0.1533	2.86
0.1566	2.86
0.1600	2.85
0.1633	2.85
0.1666	2.85
0.1700	2.85
0.1733	2.85
0.1766	2.85
0.1800	2.85

2.0000	2.63
2.5000	2.61
3.0000	2.70
3.5000	2.75
4.0000	2.73
4.5000	2.70
5.0000	2.67
5.5000	2.65
6.0000	2.62
6.5000	2.60
7.0000	2.57
7.5000	2.55
8.0000	2.52
8.5000	2.50
9.0000	2.47
9.5000	2.45
10.0000	2.43
12.0000	2.33
14.0000	2.24
16.0000	2.14
18.0000	2.05
20.0000	1.96
22.0000	1.88
24.0000	1.80
26.0000	1.72
28.0000	1.65
30.0000	1.58
32.0000	1.52
34.0000	1.45
36.0000	1.38
38.0000	1.33
40.0000	1.26

END

SE10006  
Environmental Logger  
06/14 14:06

Units 00000 Tests 3

INPUT 1: Level (F)

Reference 0.00  
Scale factor 10.00  
Offset 0.00

Step 1 06/06 14:24

K19A

Elapsed Time	Value
0.0000	- 9.06
0.0033	- 10.68
0.0066	- 11.38
0.0099	- 11.50
0.0133	- 11.25
0.0166	- 11.20
0.0200	- 11.16
0.0233	- 11.12
0.0266	- 11.07
0.0300	- 11.02
0.0333	- 10.98
0.0500	- 10.79
0.0666	- 10.62
0.0833	- 10.47
0.1000	- 10.33
0.1166	- 10.20
0.1333	- 10.06
0.1500	- 9.95
0.1666	- 9.90
0.1833	- 9.82
0.2000	- 9.75
0.2166	- 9.70
0.2333	- 9.65
0.2500	- 9.62
0.2666	- 9.59
0.2833	- 9.56
0.3000	- 9.54
0.3166	- 9.52
0.3333	- 9.51
0.4167	- 9.47
0.5000	- 9.46
0.5833	- 9.45
0.6667	- 9.45
0.7500	- 9.45
0.8333	- 9.45
0.9167	- 9.45
1.0000	- 9.45
1.0833	- 9.44
1.1667	- 9.44
1.2500	- 9.45
1.3333	- 9.45
1.4166	- 9.45
1.5000	- 9.45
1.5833	- 9.45
1.6667	- 9.45
1.7500	- 9.45
1.8333	- 9.45

? See Strip Charts



ENG

2.0000

- 9.45



SE10008  
Environmental Logger  
06/14 14:59

Units 00000 Tests 3

INPUT 1: Level (F)

Reference 0.00  
Scale factor 10.04  
Offset 0.00

Steps 0 06/06 14:31

K19A

Elapsed Time	Value
0.0000	- 13.52
0.0033	- 10.73
0.0066	- 11.55
0.0099	- 11.40
0.0133	- 11.31
0.0166	- 11.27
0.0200	- 11.23
0.0233	- 11.18
0.0266	- 11.14
0.0300	- 11.10
0.0333	- 11.06
0.0366	- 10.97
0.0400	- 10.72
0.0433	- 10.55
0.0466	- 10.41
0.0500	- 10.28
0.0533	- 10.16
0.0566	- 10.06
0.0600	- 9.97
0.0633	- 9.83
0.0666	- 9.62
0.0700	- 9.47
0.0733	- 9.32
0.0766	- 9.16
0.0800	- 9.00
0.0833	- 8.83
0.0866	- 8.66
0.0900	- 8.49
0.0933	- 8.32
0.0966	- 8.15
0.1000	- 7.98
0.1033	- 7.81
0.1066	- 7.64
0.1100	- 7.47
0.1133	- 7.30
0.1166	- 7.13
0.1200	- 6.96
0.1233	- 6.79
0.1266	- 6.62
0.1300	- 6.45
0.1333	- 6.28
0.1366	- 6.11
0.1400	- 5.94
0.1433	- 5.77
0.1466	- 5.60
0.1500	- 5.43
0.1533	- 5.26
0.1566	- 5.09
0.1600	- 4.92
0.1633	- 4.75
0.1666	- 4.58
0.1700	- 4.41
0.1733	- 4.24
0.1766	- 4.07
0.1800	- 3.90
0.1833	- 3.73
0.1866	- 3.56
0.1900	- 3.39
0.1933	- 3.22
0.1966	- 3.05
0.2000	- 2.88
0.2033	- 2.71
0.2066	- 2.54
0.2100	- 2.37
0.2133	- 2.20
0.2166	- 2.03
0.2200	- 1.86
0.2233	- 1.69
0.2266	- 1.52
0.2300	- 1.35
0.2333	- 1.18
0.2366	- 1.01
0.2400	- 0.84
0.2433	- 0.67
0.2466	- 0.50
0.2500	- 0.33
0.2533	- 0.16
0.2566	- 0.00
0.2600	- 0.17
0.2633	- 0.34
0.2666	- 0.51
0.2700	- 0.68
0.2733	- 0.85
0.2766	- 1.02
0.2800	- 1.19
0.2833	- 1.36
0.2866	- 1.53
0.2900	- 1.70
0.2933	- 1.87
0.2966	- 2.04
0.3000	- 2.21
0.3033	- 2.38
0.3066	- 2.55
0.3100	- 2.72
0.3133	- 2.89
0.3166	- 3.06
0.3200	- 3.23
0.3233	- 3.40
0.3266	- 3.57
0.3300	- 3.74
0.3333	- 3.91
0.3366	- 4.08
0.3400	- 4.25
0.3433	- 4.42
0.3466	- 4.59
0.3500	- 4.76
0.3533	- 4.93
0.3566	- 5.10
0.3600	- 5.27
0.3633	- 5.44
0.3666	- 5.61
0.3700	- 5.78
0.3733	- 5.95
0.3766	- 6.12
0.3800	- 6.29
0.3833	- 6.46
0.3866	- 6.63
0.3900	- 6.80
0.3933	- 6.97
0.3966	- 7.14
0.4000	- 7.31
0.4033	- 7.48
0.4066	- 7.65
0.4100	- 7.82
0.4133	- 7.99
0.4166	- 8.16
0.4200	- 8.33
0.4233	- 8.50
0.4266	- 8.67
0.4300	- 8.84
0.4333	- 9.01
0.4366	- 9.18
0.4400	- 9.35
0.4433	- 9.52
0.4466	- 9.69
0.4500	- 9.86
0.4533	- 10.03
0.4566	- 10.20
0.4600	- 10.37
0.4633	- 10.54
0.4666	- 10.71
0.4700	- 10.88
0.4733	- 11.05
0.4766	- 11.22
0.4800	- 11.39
0.4833	- 11.56
0.4866	- 11.73
0.4900	- 11.90
0.4933	- 12.07
0.4966	- 12.24
0.5000	- 12.41

? see strip charts

2.0000	-	9.45
2.5000	-	9.45
3.0000	-	9.45
3.5000	-	9.45
4.0000	-	9.45
4.5000	-	9.45
5.0000	-	9.45
5.5000	-	9.46
6.0000	-	9.45
6.5000	-	9.45
7.0000	-	9.45
7.5000	-	9.45
8.0000	-	9.45
8.5000	-	9.45
9.0000	-	9.45
9.5000	-	9.45
10.0000	-	9.45
12.0000	-	9.46

END

SE10006  
Environmental Logger  
06/14 13:59

Units 00000 Tests 6

INPUT 1: Level (F)

Reference 0.00  
Scale factor 10.00  
Offset 0.00

Step 1 06/06 16:32

Elapsed Time	Value
0.0000	4.95
0.0033	4.95
0.0066	2.54
0.0099	0.67
0.0133	2.16
0.0166	2.76
0.0200	2.74
0.0233	2.66
0.0266	2.60
0.0300	2.52
0.0333	2.44
0.0500	2.05
0.0666	1.77
0.0833	1.42
0.1000	1.17
0.1166	0.95
0.1333	0.76
0.1500	0.62
0.1666	0.50
0.1833	0.39
0.2000	0.31
0.2166	0.23
0.2333	0.19
0.2500	0.14
0.2666	0.11
0.2833	0.08
0.3000	0.06
0.3166	0.05
0.3333	0.04
0.4167	0.01
0.5000	0.01
0.5833	0.01
0.6667	0.00
0.7500	0.00
0.8333	0.00
0.9167	0.00
1.0000	0.00
1.0833	0.00
1.1667	0.00
1.2500	0.00
1.3333	0.00
1.4166	0.00
1.5000	0.00
1.5833	0.00
1.6667	0.00
1.7500	0.00
1.8333	0.00

*R21 Rising Head*

2.0000  
2.5000  
3.0000  
3.5000  
4.0000

0.00  
0.00  
0.00  
0.00  
0.00

END

SE10008  
Environmental Logger  
06/14 13:45

Units 00000 Test 1

INPUT 1: Level (F)

Reference 0.00  
Scale factor 10.00  
Offset 0.00

Steps: 06/06 11:55

*20 c Rising Head*

Elapsed Time	Value
0.0000	- 0.96
0.0033	- 1.63
0.0066	- 1.99
0.0099	- 1.99
0.0133	- 1.95
0.0166	- 1.67
0.0200	- 1.63
0.0233	- 1.60
0.0266	- 1.77
0.0300	- 1.75
0.0333	- 1.72
0.0500	- 1.62
0.0666	- 1.53
0.0833	- 1.47
0.1000	- 1.40
0.1166	- 1.34
0.1333	- 1.26
0.1500	- 1.23
0.1666	- 1.18
0.1833	- 1.13
0.2000	- 1.06
0.2166	- 1.04
0.2333	- 0.99
0.2500	- 0.95
0.2666	- 0.91
0.2833	- 0.86
0.3000	- 0.84
0.3166	- 0.81
0.3333	- 0.77
0.4167	- 0.63
0.5000	- 0.52
0.5833	- 0.44
0.6667	- 0.36
0.7500	- 0.33
0.8333	- 0.30
0.9167	- 0.27
1.0000	- 0.25
1.0833	- 0.23
1.1667	- 0.22
1.2500	- 0.21
1.3333	- 0.21
1.4166	- 0.20
1.5000	- 0.20
1.5833	- 0.20
1.6667	- 0.20
1.7500	- 0.20
1.8333	- 0.20

2.0000	-	0.20
2.5000	-	0.20
3.0000	-	0.20
3.5000	-	0.20
4.0000	-	0.20

END

SE10006  
Environmental Logger  
06/14 13:53

Units 00000 Tests 6

INPUT 1: Level (F)

Reference 0.00  
Scale factor 10.00  
Offset 0.00

*R21 Falling Head*

Step 0 06/06 16:19

Elapsed Time	Value
0.0000	0.00
0.0033	0.62
0.0066	3.15
0.0099	4.57
0.0133	4.46
0.0166	2.12
0.0200	3.05
0.0233	2.40
0.0266	0.95
0.0300	1.51
0.0333	2.44
0.0500	1.92
0.0555	1.56
0.0633	1.34
0.1000	1.11
0.1166	0.91
0.1333	0.75
0.1500	0.61
0.1666	0.50
0.1833	0.40
0.2000	0.32
0.2166	0.25
0.2333	0.20
0.2500	0.16
0.2666	0.12
0.2833	0.10
0.3000	0.08
0.3166	0.06
0.3333	0.05
0.4167	0.03
0.5000	0.02
0.5633	0.02
0.6667	0.02
0.7500	0.02
0.8333	0.02
0.9167	0.02
1.0000	0.01
1.0833	0.01
1.1667	0.01
1.2500	0.01
1.3333	0.01
1.4166	0.01
1.5000	0.01
1.5833	0.01
1.6667	0.01
1.7500	0.01
1.8333	0.01



2.0000	0.01
2.5000	0.01
3.0000	0.02
3.5000	0.02
4.0000	0.02
4.5000	0.02
5.0000	0.02
5.5000	0.02
6.0000	0.02
6.5000	0.02
7.0000	0.02
7.5000	0.02
8.0000	0.02
8.5000	0.02
9.0000	0.02
9.5000	0.02
10.0000	0.02
12.0000	0.02

END

SE10006  
Environmental Logger  
06/14 13:43

Units 00000 Tests 1

INPUT 1: Level (F)

Reference 0.00  
Scale factor 10.00  
Offset 0.00

*20 C Rising Head*

Step 0 06/06 11:40

Elapsed Time	Value
0.0000	- 2.77
0.0033	- 2.30
0.0066	- 0.52
0.0099	- 2.11
0.0133	- 1.56
0.0166	- 1.67
0.0200	- 1.76
0.0233	- 1.76
0.0266	- 1.73
0.0300	- 1.70
0.0333	- 1.66
0.0366	- 1.52
0.0400	- 1.42
0.0433	- 1.33
0.0466	- 1.25
0.0500	- 1.18
0.0533	- 1.12
0.0566	- 1.26
0.0600	- 1.00
0.0633	- 0.95
0.0666	- 0.90
0.0700	- 0.66
0.0733	- 0.62
0.0766	- 0.77
0.0800	- 0.74
0.0833	- 0.70
0.0866	- 0.56
0.0900	- 0.63
0.0933	- 0.60
0.0966	- 0.47
0.1000	- 0.36
0.1033	- 0.31
0.1066	- 0.26
0.1100	- 0.23
0.1133	- 0.21
0.1166	- 0.19
1.0000	- 0.16
1.0033	- 0.16
1.0066	- 0.17
1.0100	- 0.16
1.0133	- 0.17
1.0166	- 0.17
1.0200	- 0.17
1.0233	- 0.17
1.0266	- 0.17
1.0300	- 0.17
1.0333	- 0.17
1.0366	- 0.17
1.0400	- 0.17
1.0433	- 0.17
1.0466	- 0.17
1.0500	- 0.17
1.0533	- 0.17
1.0566	- 0.17
1.0600	- 0.17
1.0633	- 0.17
1.0666	- 0.17
1.0700	- 0.17
1.0733	- 0.17
1.0766	- 0.17
1.0800	- 0.17
1.0833	- 0.17
1.0866	- 0.17
1.0900	- 0.17
1.0933	- 0.17
1.0966	- 0.17
1.1000	- 0.17
1.1033	- 0.17
1.1066	- 0.17
1.1100	- 0.17
1.1133	- 0.17
1.1166	- 0.17
1.1200	- 0.17
1.1233	- 0.17
1.1266	- 0.17
1.1300	- 0.17
1.1333	- 0.17
1.1366	- 0.17
1.1400	- 0.17
1.1433	- 0.17
1.1466	- 0.17
1.1500	- 0.17
1.1533	- 0.17
1.1566	- 0.17
1.1600	- 0.17
1.1633	- 0.17
1.1666	- 0.17
1.1700	- 0.17
1.1733	- 0.17
1.1766	- 0.17
1.1800	- 0.17
1.1833	- 0.17
1.1866	- 0.17
1.1900	- 0.17
1.1933	- 0.17
1.1966	- 0.17
1.2000	- 0.17
1.2033	- 0.17
1.2066	- 0.17
1.2100	- 0.17
1.2133	- 0.17
1.2166	- 0.17
1.2200	- 0.17
1.2233	- 0.17
1.2266	- 0.17
1.2300	- 0.17
1.2333	- 0.17
1.2366	- 0.17
1.2400	- 0.17
1.2433	- 0.17
1.2466	- 0.17
1.2500	- 0.17
1.2533	- 0.17
1.2566	- 0.17
1.2600	- 0.17
1.2633	- 0.17
1.2666	- 0.17
1.2700	- 0.17
1.2733	- 0.17
1.2766	- 0.17
1.2800	- 0.17
1.2833	- 0.17
1.2866	- 0.17
1.2900	- 0.17
1.2933	- 0.17
1.2966	- 0.17
1.3000	- 0.17
1.3033	- 0.17
1.3066	- 0.17
1.3100	- 0.17
1.3133	- 0.17
1.3166	- 0.17
1.3200	- 0.17
1.3233	- 0.17
1.3266	- 0.17
1.3300	- 0.17
1.3333	- 0.17
1.3366	- 0.17
1.3400	- 0.17
1.3433	- 0.17
1.3466	- 0.17
1.3500	- 0.17
1.3533	- 0.17
1.3566	- 0.17
1.3600	- 0.17
1.3633	- 0.17
1.3666	- 0.17
1.3700	- 0.17
1.3733	- 0.17
1.3766	- 0.17
1.3800	- 0.17
1.3833	- 0.17
1.3866	- 0.17
1.3900	- 0.17
1.3933	- 0.17
1.3966	- 0.17
1.4000	- 0.17
1.4033	- 0.17
1.4066	- 0.17
1.4100	- 0.17
1.4133	- 0.17
1.4166	- 0.17
1.4200	- 0.17
1.4233	- 0.17
1.4266	- 0.17
1.4300	- 0.17
1.4333	- 0.17
1.4366	- 0.17
1.4400	- 0.17
1.4433	- 0.17
1.4466	- 0.17
1.4500	- 0.17
1.4533	- 0.17
1.4566	- 0.17
1.4600	- 0.17
1.4633	- 0.17
1.4666	- 0.17
1.4700	- 0.17
1.4733	- 0.17
1.4766	- 0.17
1.4800	- 0.17
1.4833	- 0.17
1.4866	- 0.17
1.4900	- 0.17
1.4933	- 0.17
1.4966	- 0.17
1.5000	- 0.17
1.5033	- 0.17
1.5066	- 0.17
1.5100	- 0.17
1.5133	- 0.17
1.5166	- 0.17
1.5200	- 0.17
1.5233	- 0.17
1.5266	- 0.17
1.5300	- 0.17
1.5333	- 0.17
1.5366	- 0.17
1.5400	- 0.17
1.5433	- 0.17
1.5466	- 0.17
1.5500	- 0.17
1.5533	- 0.17
1.5566	- 0.17
1.5600	- 0.17
1.5633	- 0.17
1.5666	- 0.17
1.5700	- 0.17
1.5733	- 0.17
1.5766	- 0.17
1.5800	- 0.17
1.5833	- 0.17
1.5866	- 0.17
1.5900	- 0.17
1.5933	- 0.17
1.5966	- 0.17
1.6000	- 0.17
1.6033	- 0.17
1.6066	- 0.17
1.6100	- 0.17
1.6133	- 0.17
1.6166	- 0.17
1.6200	- 0.17
1.6233	- 0.17
1.6266	- 0.17
1.6300	- 0.17
1.6333	- 0.17
1.6366	- 0.17
1.6400	- 0.17
1.6433	- 0.17
1.6466	- 0.17
1.6500	- 0.17
1.6533	- 0.17
1.6566	- 0.17
1.6600	- 0.17
1.6633	- 0.17
1.6666	- 0.17
1.6700	- 0.17
1.6733	- 0.17
1.6766	- 0.17
1.6800	- 0.17
1.6833	- 0.17
1.6866	- 0.17
1.6900	- 0.17
1.6933	- 0.17
1.6966	- 0.17
1.7000	- 0.17
1.7033	- 0.17
1.7066	- 0.17
1.7100	- 0.17
1.7133	- 0.17
1.7166	- 0.17
1.7200	- 0.17
1.7233	- 0.17
1.7266	- 0.17
1.7300	- 0.17
1.7333	- 0.17
1.7366	- 0.17
1.7400	- 0.17
1.7433	- 0.17
1.7466	- 0.17
1.7500	- 0.17
1.7533	- 0.17
1.7566	- 0.17
1.7600	- 0.17
1.7633	- 0.17
1.7666	- 0.17
1.7700	- 0.17
1.7733	- 0.17
1.7766	- 0.17
1.7800	- 0.17
1.7833	- 0.17
1.7866	- 0.17
1.7900	- 0.17
1.7933	- 0.17
1.7966	- 0.17
1.8000	- 0.17
1.8033	- 0.17
1.8066	- 0.17
1.8100	- 0.17
1.8133	- 0.17
1.8166	- 0.17
1.8200	- 0.17
1.8233	- 0.17
1.8266	- 0.17
1.8300	- 0.17
1.8333	- 0.17
1.8366	- 0.17
1.8400	- 0.17
1.8433	- 0.17
1.8466	- 0.17
1.8500	- 0.17
1.8533	- 0.17
1.8566	- 0.17
1.8600	- 0.17
1.8633	- 0.17
1.8666	- 0.17
1.8700	- 0.17
1.8733	- 0.17
1.8766	- 0.17
1.8800	- 0.17
1.8833	- 0.17
1.8866	- 0.17
1.8900	- 0.17
1.8933	- 0.17
1.8966	- 0.17
1.9000	- 0.17
1.9033	- 0.17
1.9066	- 0.17
1.9100	- 0.17
1.9133	- 0.17
1.9166	- 0.17
1.9200	- 0.17
1.9233	- 0.17
1.9266	- 0.17
1.9300	- 0.17
1.9333	- 0.17
1.9366	- 0.17
1.9400	- 0.17
1.9433	- 0.17
1.9466	- 0.17
1.9500	- 0.17
1.9533	- 0.17
1.9566	- 0.17
1.9600	- 0.17
1.9633	- 0.17
1.9666	- 0.17
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1.9733	- 0.17
1.9766	- 0.17
1.9800	- 0.17
1.9833	- 0.17
1.9866	- 0.17
1.9900	- 0.17
1.9933	- 0.17
1.9966	- 0.17
2.0000	- 0.17

2.6000	-	0.17
2.5000	-	0.17
3.6000	-	0.17
3.5000	-	0.16
4.6000	-	0.16
4.5000	-	0.16
5.6000	-	0.16
5.5000	-	0.19
6.6000	-	0.19
6.5000	-	0.16
7.6000	-	0.16
7.5000	-	0.16
8.6000	-	0.16
8.5000	-	0.16
9.6000	-	0.19
9.5000	-	0.19
10.6000	-	0.16
12.6000	-	0.19
14.6000	-	0.22

END

SE10006  
 Environmental Loader  
 06/12 12:12

Unit# 00000 Test# 0

INPUT 1: Level (F)

Reference: 0.00  
 Scale factor: 10.00  
 Offset: 0.00

*K20A Falling Head*

Step# 1 06/06 11:05

Elapsed Time	Value
0.0000	2.34
0.0033	1.97
0.0066	1.55
0.0099	1.29
0.0132	1.05
0.0165	0.87
0.0198	0.75
0.0231	0.65
0.0264	0.58
0.0297	0.52
0.0330	0.47
0.0363	0.43
0.0396	0.40
0.0429	0.37
0.0462	0.35
0.0495	0.33
0.0528	0.31
0.0561	0.29
0.0594	0.28
0.0627	0.27
0.0660	0.26
0.0693	0.25
0.0726	0.24
0.0759	0.23
0.0792	0.22
0.0825	0.21
0.0858	0.20
0.0891	0.19
0.0924	0.18
0.0957	0.17
0.0990	0.16
0.1023	0.15
0.1056	0.14
0.1089	0.13
0.1122	0.12
0.1155	0.11
0.1188	0.10
0.1221	0.09
0.1254	0.08
0.1287	0.07
0.1320	0.06
0.1353	0.05
0.1386	0.04
0.1419	0.03
0.1452	0.02
0.1485	0.01
0.1518	0.00
0.1551	0.00
0.1584	0.00
0.1617	0.00
0.1650	0.00
0.1683	0.00
0.1716	0.00
0.1749	0.00
0.1782	0.00
0.1815	0.00
0.1848	0.00
0.1881	0.00
0.1914	0.00
0.1947	0.00
0.1980	0.00
0.2013	0.00
0.2046	0.00
0.2079	0.00
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0.2310	0.00
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0.2640	0.00
0.2673	0.00
0.2706	0.00
0.2739	0.00
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0.2805	0.00
0.2838	0.00
0.2871	0.00
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0.2970	0.00
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0.3036	0.00
0.3069	0.00
0.3102	0.00
0.3135	0.00
0.3168	0.00
0.3201	0.00
0.3234	0.00
0.3267	0.00
0.3300	0.00
0.3333	0.00
0.3366	0.00
0.3399	0.00
0.3432	0.00
0.3465	0.00
0.3498	0.00
0.3531	0.00
0.3564	0.00
0.3597	0.00
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0.3762	0.00
0.3795	0.00
0.3828	0.00
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0.3894	0.00
0.3927	0.00
0.3960	0.00
0.3993	0.00
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0.4092	0.00
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0.4158	0.00
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0.4587	0.00
0.4620	0.00
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0.4719	0.00
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0.4785	0.00
0.4818	0.00
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0.4884	0.00
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0.5115	0.00
0.5148	0.00
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0.5445	0.00
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0.5511	0.00
0.5544	0.00
0.5577	0.00
0.5610	0.00
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0.5775	0.00
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0.6402	0.00
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0.6468	0.00
0.6501	0.00
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0.6600	0.00
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0.6732	0.00
0.6765	0.00
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0.7095	0.00
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0.7161	0.00
0.7194	0.00
0.7227	0.00
0.7260	0.00
0.7293	0.00
0.7326	0.00
0.7359	0.00
0.7392	0.00
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0.8085	0.00
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0.8844	0.00
0.8877	0.00
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0.8976	0.00
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0.9042	0.00
0.9075	0.00
0.9108	0.00
0.9141	0.00
0.9174	0.00
0.9207	0.00
0.9240	0.00
0.9273	0.00
0.9306	0.00
0.9339	0.00
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0.9405	0.00
0.9438	0.00
0.9471	0.00
0.9504	0.00
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0.9636	0.00
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0.9867	0.00
0.9900	0.00
0.9933	0.00
0.9966	0.00
0.9999	0.00

2.0000	-	0.20
2.5000	-	0.16
3.0000	-	0.11
3.5000	-	0.06
4.0000	-	0.04
4.5000	-	0.02
5.0000	-	0.01
5.5000	-	0.00
6.0000	-	0.00
6.5000	-	0.00
7.0000	-	0.00
7.5000	-	0.00
8.0000	-	0.01
8.5000	-	0.01
9.0000	-	0.00
9.5000	-	0.00
10.0000	-	0.01

END

SE:0000  
Environmental Logoff  
05/14 13:23

Unit: 00000 Text: 0

INPUT 1: Level (F)

Reference 0.00  
Scale Factor 10.00  
Offset 0.00

Step 0 05:00 10:00

*K20A Felling Head*

Elapsed Time Value

Elapsed Time	Value
0.0000	1.140
0.0010	1.140
0.0020	1.140
0.0030	1.140
0.0040	1.140
0.0050	1.140
0.0060	1.140
0.0070	1.140
0.0080	1.140
0.0090	1.140
0.0100	1.140
0.0110	1.140
0.0120	1.140
0.0130	1.140
0.0140	1.140
0.0150	1.140
0.0160	1.140
0.0170	1.140
0.0180	1.140
0.0190	1.140
0.0200	1.140
0.0210	1.140
0.0220	1.140
0.0230	1.140
0.0240	1.140
0.0250	1.140
0.0260	1.140
0.0270	1.140
0.0280	1.140
0.0290	1.140
0.0300	1.140
0.0310	1.140
0.0320	1.140
0.0330	1.140
0.0340	1.140
0.0350	1.140
0.0360	1.140
0.0370	1.140
0.0380	1.140
0.0390	1.140
0.0400	1.140
0.0410	1.140
0.0420	1.140
0.0430	1.140
0.0440	1.140
0.0450	1.140
0.0460	1.140
0.0470	1.140
0.0480	1.140
0.0490	1.140
0.0500	1.140
0.0510	1.140
0.0520	1.140
0.0530	1.140
0.0540	1.140
0.0550	1.140
0.0560	1.140
0.0570	1.140
0.0580	1.140
0.0590	1.140
0.0600	1.140
0.0610	1.140
0.0620	1.140
0.0630	1.140
0.0640	1.140
0.0650	1.140
0.0660	1.140
0.0670	1.140
0.0680	1.140
0.0690	1.140
0.0700	1.140
0.0710	1.140
0.0720	1.140
0.0730	1.140
0.0740	1.140
0.0750	1.140
0.0760	1.140
0.0770	1.140
0.0780	1.140
0.0790	1.140
0.0800	1.140
0.0810	1.140
0.0820	1.140
0.0830	1.140
0.0840	1.140
0.0850	1.140
0.0860	1.140
0.0870	1.140
0.0880	1.140
0.0890	1.140
0.0900	1.140
0.0910	1.140
0.0920	1.140
0.0930	1.140
0.0940	1.140
0.0950	1.140
0.0960	1.140
0.0970	1.140
0.0980	1.140
0.0990	1.140
0.1000	1.140

11 00000000  
10 00000000  
09 00000000  
08 00000000  
07 00000000  
06 00000000  
05 00000000  
04 00000000  
03 00000000  
02 00000000  
01 00000000  
00 00000000

END

00 00000000  
01 00000000  
02 00000000  
03 00000000  
04 00000000  
05 00000000  
06 00000000  
07 00000000  
08 00000000  
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10 00000000  
11 00000000

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KELLOGG-DEERING WELL FIELD  
ZELL BUILDING EVALUATION  
NORWALK, CT

PREPARED FOR:

MURTHA, CULLINA, RICHTER AND PINNEY

DECEMBER 4, 1987

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**Dames & Moore**  
ONE BLUE HILL PLAZA, SUITE 530, PEARL RIVER, NY 10965



NET 000



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Figure 1 - Site Location Map

Figure 2 - Zell Building Schematic

Figure 3 - Engineering Drawing of Building Drainage

### Tables

Table 1 - Results of Records Review



# DAMES & MOORE

ONE BLUE HILL PLAZA, STE. 500 PEARL RIVER, NEW YORK 10663-5000 (914) 734-1200

December 4, 1987

Mr. Mark R. Sussman  
Murtha, Cullina, Richter and Pinney  
City Place  
P.O. Box 3157  
Hartford, CT 06103

Re: Final Report  
Pitney Bowes (Zell Building)  
Kellogg-Deering Wellfield

Dear Mr. Sussman:

Dames & Moore has been retained by Murtha, Cullina, Richter and Pinney to collect and evaluate data relating to Pitney Bowes Zell Building operations. As you know, Pitney Bowes was recently named as a PRP for the Kellogg-Deering well field contamination in Norwalk, CT by Region I - USEPA.

Enclosed are two (2) copies of our report describing the results of our work at this facility. In summary we find that:

- Pitney Bowes operations at the Zell Building location could not have caused or contributed to TCE contamination at the Kellogg Deering well field; and
- Data were not present in either Region I - USEPA files or CT-DEP files that indicated Pitney Bowes caused or contributed to the problem. Pitney Bowes identification as a PRP was solely based on the speculative comment of HRP Associates in a report prepared for the ELINCO Corp., an adjoining property owner which is also a PRP.

We are prepared to fully support the above conclusions in discussions with Region I-USEPA. Let me know how you wish to proceed.

Sincerely,  
Dames & Moore

Robert P. Blauvelt  
Associate

cc: Mr. John T. Schmidt  
Assistant General Counsel  
Pitney Bowes

## 1.0 INTRODUCTION

The Kellogg-Deering well field is a public water supply ground water system located in southwestern Fairfield County, along the western bank of the Norwalk River in Norwalk, Connecticut (Figure 1). It is owned and operated by the Norwalk First Taxing District and provides between 15 percent and 35 percent of Norwalk's water supply.

In 1975, trichloroethene (TCE), a chlorinated solvent, was discovered in the groundwater of the Kellogg-Deering well field. Based on findings from a study conducted by the Center for Disease Control, the Kellogg-Deering well field became a federal Superfund site (#316) and was placed on the National Priorities List (NPL) in September of 1983.

A Remedial Investigation/Feasibility Study (RI/FS) for the site was completed by an EPA contractor in April, 1986. The RI concluded that the source (s) of TCE was to the east of the well field. The FS evaluated five remedial alternatives including no action (with continued monitoring) as well as several different types of treatment technologies (e.g., air stripping, activated carbon).

In September, 1986 the U.S. Environmental Protection Agency (USEPA) issued a Record of Decision (ROD) selecting air stripping as the cost-effective remedy for the TCE contamination. The air-stripping system became operational at the well field at the end of June, 1987. The cost for installation of the system was estimated to be approximately \$400,000, exclusive of RI/FS and enforcement action costs.

### 1.1 Project Background

Pitney Bowes recently was notified by Region I of the USEPA that a building leased by Pitney Bowes at 276 Main Street in Norwalk (the Zell Building) may have contributed to the well field contamination and that Pitney Bowes was now considered a Potentially Responsible Party (PRP) as described in Section 122 of the Superfund Amendments and Reauthorization Act of 1986.

This notification was based on several speculative comments made in an consulting engineers report (prepared by HRP Associates of New Britain, CT) for the Elinco Corporation, a company located at 272 Main Street, next door to the Pitney Bowes leased building. The Elinco Corporation has a well documented history of TCE releases and also has been named by Region I of the USEPA as a PRP for the Kellogg-Deering site.

#### 1.2 Objectives of the Report

In December, 1986 Dames & Moore was retained by Murtha, Cullina, Richter and Pinney to characterize the types of activities taking place at 276 Main Street and evaluate the potential for the release of TCE and TCE related chemicals from the facility.

Dames & Moore completed project activities in mid-June, 1987. This work had been performed in accordance with the Dames & Moore letter proposal of December 17, 1986 as modified at a meeting with Pitney Bowes corporate staff and outside counsel on April 3, 1987.

This report presents findings and conclusions developed during the course of that work and includes the results of:

- Interviews with key Pitney Bowes technical staff responsible for maintenance and building operations at 276 Main Street between the late 1960's and late 1970's.
- Review of Pitney Bowes internal records and memoranda relating to the purchase and handling of chemicals at 276 Main Street.
- Evaluation of existing structural information and the results of a site reconnaissance of the facility.

Conclusions are presented in Section 4.0 of the Report.

## 2.0 ZELL BUILDING CHARACTERISTICS

This section of the report provides information on the physiographic setting of the Zell Building, its usage by Pitney Bowes and the past and current waste handling practices utilized at the building.

### 2.1 Physiographic Setting

The Pitney Bowes leased building at 276 Main Street (the Zell Building) in Norwalk, Connecticut is located in an area dominated by light industrial and commercial establishments. The Zell Building actually consists of two buildings (Figure 2) joined by a common lobby and surrounded by a paved parking area with associated loading docks.

To the east of the Zell Building complex is an elevated bedrock ridge upon which a small housing development has been built. To the south is the manufacturing operation of the Elinco Corporation. Along the Zell Building's northern border is a small undeveloped wooded area and to the west (across Main Street) is a row of small commercial (retail) businesses. The Kellogg-Deering well field is located approximately one half mile to the west of the Zell Building.

Topographically, the site can be divided into two parts: a lower level which contains the buildings and associated service areas and an upper level, approximately 15 feet to 20 feet higher, present along the eastern half of the property which contains most of the parking areas. In both the lower and upper levels, the land slopes west and southwest towards Main Street. Bedrock at the property is close to the ground surface (approximately 5 to 10 feet below grade) and is overlain by glacial till. Till is a heterogeneous mixture of boulders, cobbles, gravel, sand, silt, and clay. Groundwater in the bedrock underlying the Zell Building is assumed to follow regional flow patterns and be moving westward towards the Norwalk River.

### 2.2 Building Usage

The information presented in this sub-section has been developed by interviews with Pitney Bowes technical staff familiar with operation/history of the Zell Building.

The Zell 1 Building (Figure 2) was built approximately in 1945 by the Zell Products Corporation and was used in the production of metal cosmetic cases and handbag frames. Sometime in 1955, the Zell 2 Building was added apparently to expand production and warehouse capabilities. Zell 1 operated until approximately 1968-1969 and Zell 2 operated until about 1973 or 1974.

The Zell 1 Building contained a depressed floor area in which the Zell Products Corporation conducted its metal plating operation. Open earth trenches were used apparently by the Zell Products Corporation to drain spillage from this operation to an underground conduit which led eventually to the sanitary sewer system. Information is not available about the Zell 2 Building operation.

In 1969, Pitney Bowes leased the Zell 1 Building as a warehouse for the storage of machine parts, plastics and plastics regrindings. Before occupying the building, the depressed floor area and associated trenches were filled with gravel and cement block and capped with concrete. This was done to provide a uniform, level floor. No solvents were stored or used during this phase of building occupancy by Pitney Bowes. No production operations were carried out in this building. This building was used solely as a warehouse for the storage of machine parts, plastics and plastics regrindings.

In 1977, (two years after the discovery of TCE in the Kellogg-Deering well field ground water) the Zell Building was renovated into a combination of office space, plastic storage warehouse and small (5 bench) research and development laboratory.

In 1974 Pitney Bowes occupied the Zell 2 Building with the intention of housing a parts assembly line. However, this assembly line was subsequently moved to Pitney Bowes' Danbury, CT operations and in 1975 the current model shop and engineering offices were installed in the building. Also in 1975, Pitney Bowes constructed a lobby between the two buildings and paved the lobby area. The Zell 2 Building currently acts as the main receiving point for Pitney Bowes' Norwalk operations, a cardboard storage warehouse, a model (prototype) shop and as engineering offices.

### 2.3 Chemical/Waste Handling Practices

Based on Dames & Moore interviews and the review of internal Pitney Bowes records, only one type of operation involved the use, handling and disposal of chemicals (including solvents) at the Zell Building complex. This was the small research and development laboratory present in the Zell 1 Building.

The research and development laboratory was moved into the Zell 1 Building in 1977, two years after the presence of TCE was detected in the ground water from the Kellogg-Deering well field. Laboratory facilities consist of 4 wet chemistry benches and 1 bench for physical testing. Approximately 6 to 8 employees are assigned to research work in this lab. The laboratory is dedicated to ink research, the testing and development of photo polymers, thermal printing and paper coating products.

Although a wide variety of chemicals have been used in the laboratory, the quantities and handling practices for these chemicals would not be expected to lead to their release into the ground water.

Chemicals used in the laboratory research at the Zell Building have been stored since 1977 in a locked storage room. A flood drain present in this storage room was sealed during Pitney Bowes renovation of the building in 1977 and was still sealed at the time of a Dames & Moore site visit in mid-March, 1987. Typical quantities used in the laboratory range from 1 liter bottles to 20 liter bottles for liquids and between 5 kg and 10 kg packages for solids. Although the types of chemicals have changed over the years as research efforts are redirected, the typical quantities used have remained the same.

In order to demonstrate the consistency of chemical quantities utilized at the Zell Building, a review of Pitney Bowes purchasing records for the Zell Building between 1979 and 1984 was made. During the review of approximately 50,000 purchase orders, those indicating chemicals delivered to the Zell Building were segregated and copied. Table 1 summarizes the data gathered from the review. From this table it can be seen that:

- For the seven year period between 1978 and 1984, approximately 90 gallons of several types of chlorinated solvents were purchased.\* This is an average usage rate of approximately 13 gallons a year or about 1 gal a month.
- Typical quantities purchased ranged from 1 pint to 1 gallon. Occasionally a 5 gallon or 10 gallon container was purchased.
- Of these 90 gallons, approximately 56 gallons (one-1 gallon bottle purchased in 1983 and one-55 gallon drum purchased in 1984) were TCE.

From the purchase order records, it is apparent that chemicals were not used in bulk at this facility and that the type of chlorinated solvent usage needed to cause or contribute to the extensive ground water contamination present at the Kellogg-Deering well field was not being done during Pitney Bowes occupancy of the Zell Building. Chlorinated solvents purchased for use at the Zell Building were used up in laboratory experiments. Residual's amounts were either evaporated in hoods or drummed and shipped off-site for disposal.

The amounts of waste generated by the research and development laboratory prior to 1980 are not known. However, more recent waste generation rates would be representative of past activities, as the basic operations and scale of the laboratory has not changed. Approximately one 55 gallon drum of hazardous waste (which would include chlorinated solvents) is removed from the building every three months. Two 55 gallon drums of neutralized aqueous waste (acids) are disposed of every year. These low volumes of wastes are consistent with chemical usage described for the facility.

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\* It is chlorinated solvent (TCE) which is the principal contaminant in the Kellogg-Deering well field ground water.



### 3.0 PATHWAYS EVALUATION

As part of the Dames & Moore characterization of Zell Building activities, an evaluation was made as to potential pathways into the environment for chemicals that may have been released from the facility. Although the information presented in Section 2.0 clearly indicates that laboratory research and development activities would not have resulted in the handling and/or discharge to the environment of quantities of solvents to the ground water of the well field, the facility characterization would not be complete without this type of pathways evaluation.

Information presented in this section has been developed from a site reconnaissance survey, a review of facility engineering drawings and interviews with key Pitney Bowes Zell Building personnel.

#### 3.1 Floor Drains

Prior to Pitney Bowes occupancy of 276 Main Street, floor drains and trenches were used to divert spillage from both metal plating operations and from building bathrooms to the sanitary sewer system. Trenches were filled and sealed before Pitney Bowes occupied the building. After moving into the building, Pitney Bowes piped the building bathrooms to the sanitary sewer and sealed remaining flood drains at the surface. This was done shortly after building occupancy in 1969. Laboratory sink drainage is discharged to the sanitary sewer after passing through a neutralizer trap. Drainage from the building has been directly into the sanitary sewer systems since the early days of Pitney Bowes occupancy.

Figure 2 is a copy of an engineering drawing describing some revisions to plant utilities for the Zell 2 Building. This drawing also provides the following information on the Zell 1 Building drainage system:

- Roof and parking lot/pavement runoff is channeled to the storm sewer system (Location A on Figure 2).

- Subsurface tile drains (apparently used for building dewatering) also are connected to the storm sewer system (Location B on Figure 2).
- The Zell 1 Building is connected to the sanitary sewer system (Location C on Figure 2). This tends to confirm the presence of the previously described drainage patterns.

This drawing does not indicate the presence of floor drains or sumps which are not connected to the sanitary/storm sewer system.

### 3.2 Other Potential Release Points

The following paragraphs describe other pathways by which chemicals could enter the environment from the Zell Building facility.

Surface Runoff -Pitney Bowes receives and stores its chemicals in a fully enclosed warehouse area. According to several long-term employees no spillage of chemicals has occurred outside the facility.

A site reconnaissance by Dames & Moore has not indicated the presence of areas where spillage may have occurred (i.e. discolored or soft asphalt).

Spillage or breakage in the receiving area would not be environmentally significant because of the small quantities of chemicals typically used at the building (500 ml to 4 liter amounts).

Tanks- Prior to Pitney Bowes occupancy, Zell Products Corporation maintained 1 vertical and 2 horizontal outside above ground tanks. These were removed prior to Pitney Bowes paving of Zell Building property. Pitney Bowes currently has in use one 15K gallon tank for No. 4 fuel oil. No tanks, either above ground or below ground have been used by Pitney Bowes for the storage of disposal of wastes in this building since its occupancy in 1969.

Discharges to Ground-The Zell Building property was paved by Pitney Bowes in 1977. Prior to this paving, long-term Pitney Bowes employees can recall no discharge of waste or waste related materials to the ground at the site. Before paving of the parking lot, there was no Pitney Bowes laboratory in this building. Pitney Bowes operated a storage warehouse at this location.

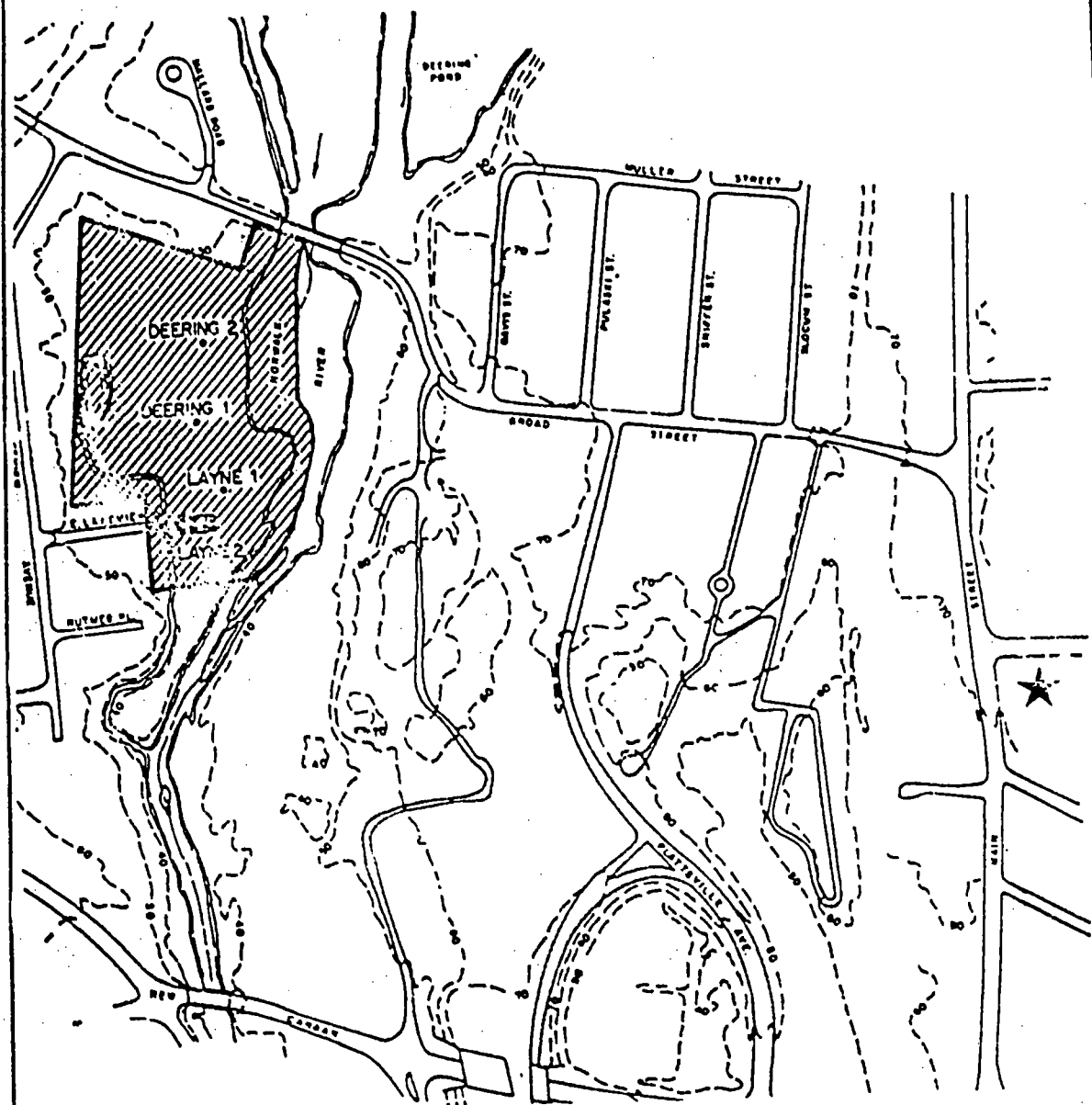
#### 4.0 CONCLUSIONS

It is Dames & Moore conclusion that based on the information presented in this Report, Pitney Bowes operations at 276 Main Street (Zell Building) could not have been responsible for, or contributed to the ground water contamination detected in the Kellogg-Deering well field.

This conclusion is supported by the following findings:

- Operations at the Zell Building since its occupancy by Pitney Bowes have been consistently those of general office and warehouse and a small research and development laboratory;
- The only Pitney Bowes operation at the Zell Building which utilized TCE was a small research and development laboratory established there in 1977, two years after TCE was detected in the Kellogg-Deering well field groundwater;
- Only small quantities of chlorinated solvents (approximately 1 gallon per month) and even smaller quantities of TCE (approximately 56 gallons in 7 years) were used by the Pitney Bowes laboratory in this building;
- Prior to operation of the laboratory, floor drains had been sealed. Laboratory drainage has always been to the sanitary sewer and residual amounts of chlorinated solvents not used up in the experiments were evaporated or drummed and sent off-site for disposal.

# SITE LOCATION MAP



KELLOGG-DEERING WELL FIELD

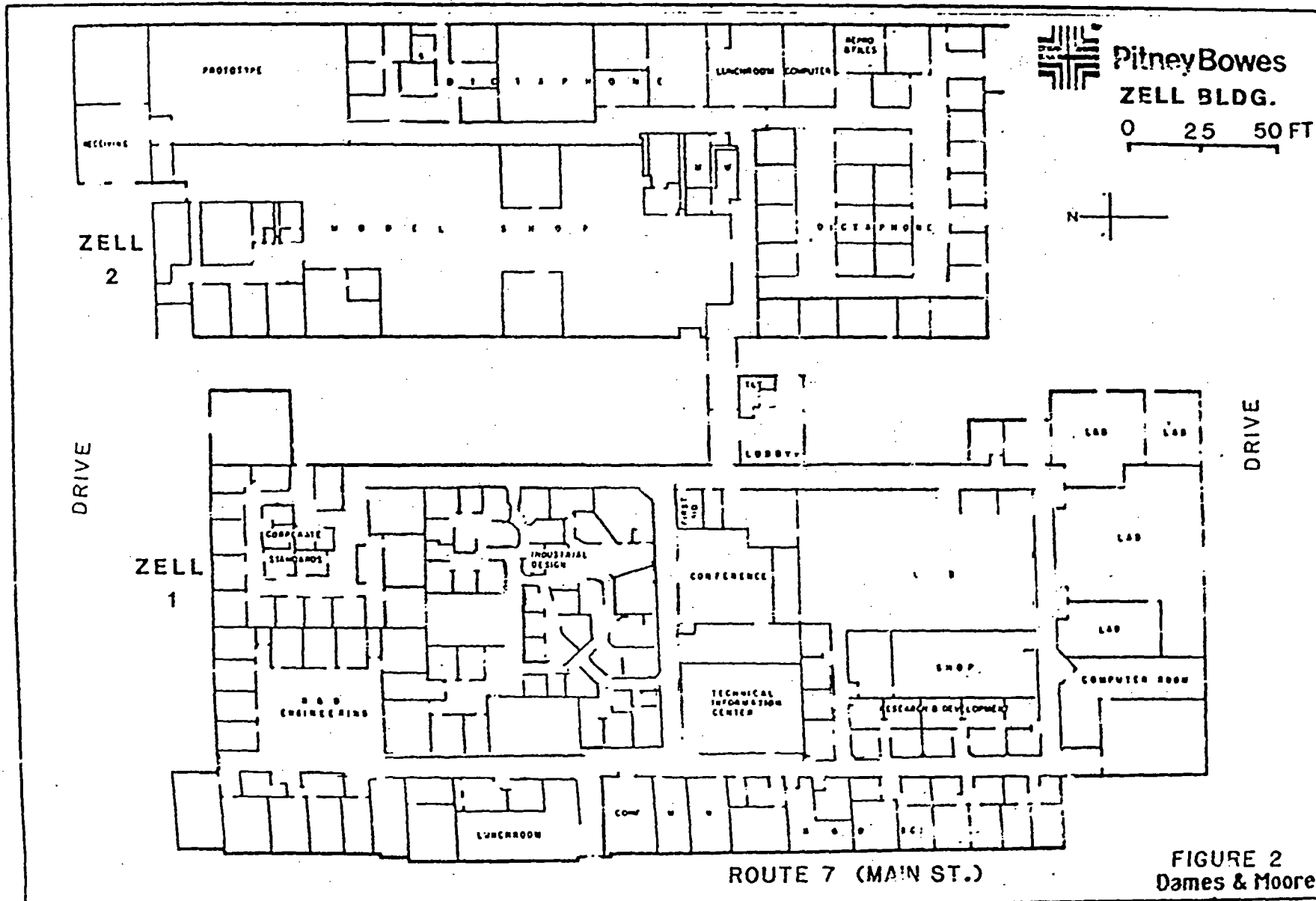


ZELL BUILDING

SCALE: 1" = 400'

Dames & Moore

FIGURE 1



**FIGURE 2**  
**Dames & Moore**

Table 1

	Quantity	Chemical Name	P. O. #	Date
1	12 qts	8790 micro cleaning solution	79224	3/6/78
2	55 gallon	Perolin	80847	3/21/78
3	2 gal	IPA	80259	3/22/78
4	100 lbs	Tri-7	83264	6/1/78
5	32 pts	Methyl Alcohol, anhydrous	48171	11/22/78
6	4 gal	Acetone AR ACS	12885	2/13/79
7	144 oz	Flux Remover MS-190	16723	4/26/79
8	12 oz	Contact RK-MU MS-24	18723	4/26/79
9	24 oz	Quick Freeze MS-240	18723	4/26/79
10	144 oz	Flux Remover MS-190HD	21755	8/1/79
11	5 gal	Clean-it Solvent	21755	8/6/79
12	288 oz	Flux Cleaner	25166	9/26/79
13	144 oz	Contact Cleaner	25166	9/26/79
14	55 gal	Ethylene Glycol	25247	10/8/79
15	288 oz	Flux Remover	26919	11/8/79
16	288 oz	Flux remover	27397	12/2/79
17	12 kg	Ethylenediamine	28810	1/4/80
18	100 lb	Marion M-50	28813	1/4/80
19	12 kg	N-Butylamine	28810	1/4/80
20	15 gal	Tetrahydrofuran	30958	1/29/80
21	3 pts	DC200 Fluid 100 cs	30990	1/31/80
22	30 gal	7200 Water Boiler Treatment	31208	2/5/80
23	3 kg	Triethylene Glycol	30988	2/6/80
24	25 lb	P-Diethylaminobenzaldehyde a	31275	2/12/80
25	1 kg	2346 Cyclohexylamine	31292	2/13/80
26	100 gm	FX 1300 Polyvinylpyrrolidinen	31531	2/20/80
27	100 gm	Polyvinylpyrrolidinene	31531	2/20/80
28	100 gm	FX 1301 Polyvinylpyrrolidinen	31531	2/20/80
29	5 gal	Methanol	23779	2/28/80
30	15 gal	tetrahydrofuran	32712	2/28/80
31	5 gal	Acetone	23779	2/28/80
32	55 gallons	Perotec 2116	32789	3/5/80
33	12 pts	Acetone	32793	3/10/80
34	500 gm	Methylsulfoxide	32793	3/10/80
35	10 gal	Methylene Chloride	50536	3/16/80 *
36	10 gal	Tetrahydrofuran	50536	3/16/80
37	5 gal	Methyl Ethyl Ketone	33680	3/19/80
38	2 liters	Carbon Disulfide	35170	4/9/80

\* Chlorinated Solvent

Table 1 (cont.)

	Quantity	Chemical Name	P. O. #	Date
39	1 gal	Hexane	35170	4/9/80
40	20 gal	Tetrahydrofuran	35169	4/9/80
41	1 pt	Benzaldehyde	35170	4/9/80
42	1 pt	Butyl Alcohol	35170	4/9/80
43	1 gal	Methanol	35170	4/9/80
44	24 kg	Ethylenediamine	370208	5/12/80
45	48 pts	Ethylene Di Amine	37252	5/27/80
46	25	Tetrahydrofuran	37253	5/28/80
47	100 gm	N-Methylmorpholine	55760	6/26/80
48	1 kg	MCB Ligolne	39512	7/2/80
49	4 pts	MCB Petroleum Ether	39512	7/2/80
50	5 gal	Methyl Ethyl Ketone	39496	7/2/80
51	1 gal	Acetonitrile	39582	7/14/80
52	1 gal	Tetrahydrofuran	39582	7/14/80
53	1 gm	Dimethylindoline	39605	7/14/80
54	1 gal	Methanol	39582	7/14/80
55	100 gm	Trifluoropropylsiloxane	39610	7/15/80
56	2 gal	Methyl Ethyl Ketone	56757	7/17/80
57	1 pt	Trichloroethylene	42717	8/1/80 *
58	1 pt	Reagent Alcohol	42717	8/1/80
59	1 kg	Thiodiethanol	40972	8/5/80
60	500 gm	Tetrahydrothiophene	40972	8/5/80
61	500 gm	Hydracrylonitrile	40972	8/5/80
62	4 gal	Methyl Alcohol, Anhydrous	42756	8/9/80
63	2 kg	Methylsulfoxide	41000	8/12/80
64	4 kg	Oxidiethanol	41000	8/12/80
65	1 gal	Toluene	41117	8/12/80
66	1 kg	Triethylene glycol	41000	8/12/80
67	1000 gm	Pyrrrolidinone	41117	8/12/80
68	1 gal	Methyl Ethyl Ketone	41117	8/12/80
69	2 gal	Acetonitrile	41238	8/22/80
70	250 gm	Hydroxypropionitrile	41234	8/22/80
71	500 gm	Thiodiethanol	41234	8/22/80
72	2 qts	Propanol-2	41238	8/22/80
73	20 gal	Tetrahydrofuran	41356	8/26/80
74	2 gm	Squarillum dye	41371	8/28/80
75	10 gm	Diethylthiocarbocyanine	41366	8/28/80
76	5 gm	Quinoline Hydrochloride	41371	8/28/80

\* Chlorinated Solvent



Table 1 (cont.)

Quantity	Chemical Name	P. O. #	Date
77 1 gal	Acetonitrile	43015	9/3/80
78 1 gal	Hexane	43015	9/3/80
79 55 gal	Methyl Ethyl Ketone (MEK)	41500	9/4/80
80 25 lb.	Diethylaminobenzaldehyde	41387	9/4/80
81 1 gal	Chloroform, unstabilized	43053	9/15/80 *
82 5 gm	Tetrocyanoquinodimethane	43038	9/15/80
83 1 gal	Chloroform, stabilized	43053	9/15/80 *
84 500 gm	Hexamethyldisilazane	43642	9/30/80
85 3 kg	Ethylene diamine	47705	9/30/80
86 4 qts	Tetrahydrofuran	43823	10/17/80
87 5 gal	Toluene	43860	10/17/80
88 20 gal	Tetrahydrofuran	43860	10/17/80
89 8 qts	Hexane	43823	10/17/80
90 4 qts	Propanol-2	43823	10/17/80
91 4 qts	Chloroform, stabilized	43823	10/17/80 *
92 5 gal	Methylene Chloride	43860	10/17/80 **
93 4 qts	Chloroform, unstabilized	43823	10/17/80 **
94 10 gal	Tetrahydrofuran	47766	11/10/80
95 4 gal	Ethyl alcohol	47799	11/10/80
96 1 gal	Ethyl Acetate	47799	11/10/80
97 14 lb.	Fluorinert	47845	11/19/80
98 14 lb.	Fluorud	47845	11/19/80
99 500 gm	Ethylhexylamine	48020	12/12/80
100 530 lb	Perotec #2116	48109	12/23/80
101 10 gm	Toluene	55769	6/2/81
102 500 gm	Mercaptoethanol	55769	6/2/81
103 200 gm	Taurine (amino acid)	55770	6/2/81
104 500 gm	Sulfanilic Acid	55769	6/2/81
105 200 gm	Perylenetetracarboxylic	55770	6/2/81
106 500 ml	Tetrahydrofuryl Alcohol	55769	6/2/81
107 288 oz	Flux Remover	55797	6/5/81
108 500 gm	Mesitylene	79106	6/14/81
109 4 liter	Acetone, Em grade	56575	6/16/81
110 4 gal	Activator-Versachem	56581	6/18/81
111 6 lb	TFE #12290-3	56609	6/22/81
112 288 oz	MSD 190HD	56644	6/29/81
113 5 gal	Methyl Ethyl Ketone	56677	7/7/81
114 5 gal	Acetone	56677	7/7/81

\* Chlorinated Solvent

Table 1 (cont.)

Quantity	Chemical Name	P. O. #	Date
115 5 gal	IPA	56677	7/7/81
116 15 gal	Tetrahydrofuran	56677	7/7/81
117 48 kg.	Toluene, ACS Reagent	56706	7/7/81
118 288 oz	Flux Remover	56681	7/7/81
119 50 kg	Dimercaptosuccinic Acid	56718	7/13/81
120 1 gal	Acetone	56757	7/17/81
121 1 gal	Toluene	56757	7/17/81
122 250 gm	Dimethylaminoethanol	5666	8/3/81
123 250 gm	Tetramethylguanidine	56862	8/3/81
124 250 gm	Diacetone Acrylamide	56939	8/13/81
125 500 gm	Hydroxyethyl Methacrylate	56939	8/13/81
126 4 kg	2,2'-Oxiethanol	56994	8/19/81
127 1000 gm	N-Methyldiethanolamine	56992	8/19/81
128 1 liter	Methyl Sulfoxide	56994	8/19/81
129 1000 gm	Nitrioltriethanol	56994	8/19/81
130 125 gm	Cobalt Chloride	56994	8/19/81
131 1500 ml	Tetrahydrofurfuryl Alcohol	56994	8/19/81
132 3 kg	Iminodiethanol	56994	8/19/81
133 1000 gm	Triethyleneglycol	56994	8/19/81
134 3 kg	Aminoethanol	56994	8/19/81
135 1 liter	Gauss Ferrofluid	59505	9/9/81
136 4 liters	C-574 Chloroform	59539	9/9/81 *
137 25 gm	D-85 S-Diphenyl Carbazide	59539	9/9/81
138 4 liter	A-18 Acetone	59539	9/9/81
139 1 liter	Gauss Ferrofluid	59505	9/9/81
140 120 oz	Nochromix Cleaning Solution	59585	9/15/81
141 144 oz	Inhibisol penetone	59622	9/15/81
142 20 gal	Tetrahydrofuran	59655	9/24/81
143 4 liters	Octacarbonyldicobalt	59851	10/15/81
144 10 gal	Methylene Chloride	59829	10/15/81 *
145 1 pt	Chloroform	59909	10/20/81
146 1 lb	Sodium Hydroxide	59909	10/20/81
147 1 lb.	Sodium Nitrate crystals	59909	10/20/81
148 1 lb.	Sodium Hydroxide	59909	10/20/81
149 500 gms	Thiodiethanol	64325	11/17/81
150 3 kg	Thiodiethanol	64491	12/17/81
151 500 gm	Dimethylamine Hydrochloride	64609	1/4/82
152 2 kg	Triethylamine	64609	1/4/82

\* Chlorinated Solvent

Table 1 (cont.)

Quantity	Chemical Name	P. O. #	Date
153 200 gm	Methacrylyl Chloride	64609	1/4/82
154 200 gm	Acryloyl Chloride	64609	1/4/82
155 2 liters	Nitric Acid	22304	1/20/82
156 2 liters	Hydrogen Fluoride CH 30301	22304	1/20/82
157 6 liters	Acetone	22402	2/2/82
158 6 liters	Sulfuric Acid	22402	2/2/82
159 55 gal	Ethylene glycol	22454	2/10/82
160 144 oz	Flux remover & cleaner	22714	3/9/82
161 12 oz	Aeroduster	22714	3/9/82
162 100 gm	Imidazolidinone	22797	3/17/82
163 10 gms	Methyl-2-Pyrrolidone ethanol	22797	3/17/82
164 20 gm	Methyl-2-Pyrrolidone 98.5%	22797	3/17/82
165 3 kg	Methyl-2-Pyrrolidone 98%	22797	3/17/82
166 20 liters	Compound NU-33, M-16	22880	3/26/82
167 20 liters	Compound NU-55-M-16	22880	3/26/82
168 15 lbs	H & H White Flux	23154	5/20/82
169 55 gal	Kerosene	22637	5/21/82
170 4 kg	Ethylene Glycol	79106	6/14/82
171 3 kg	Triethylene Glycol	79106	6/14/82
172 500 gm	Tetralin	79106	6/14/82
173 500 gm	Decalin	79106	6/14/82
174 3 kg	Tetraethylene Glycol	79106	6/14/82
175 2 liter	Formaldehyde	79145	6/22/82
176 5 gal	Trichlor 50/d	79145	6/22/82 *
177 5 gal	N-Hexane	79145	6/22/82
178 2 liter	Petrol Hydrocarbons	79145	6/22/82
179 100 gm	Hydroxyethyl Acetate	79149	7/2/82
180 1 kg	Hydroxypropyl Methacrylate	79149	7/2/82
181 500 gm	Hydroxyl-2-Butanone	79149	7/2/82
182 500 gm	Mesitylene	79149	7/2/82
183 2 gal	Bestine Solvent and Cleaner	79721	7/12/82
184 144 oz	Inhibisol Solvent Spray	79800	7/27/82
185 1 kg	Dimethylaminoethanol	79814	7/30/82
186 250 gm	Nitrilotriethanol	79814	7/30/82
187 3 kg	Oxydiethanol	79814	7/30/82
188 500 gm	Dimethylaminoethanol	79814	7/30/82
189 500 gm	L-miriodiethanol ?	79814	7/30/82
190 5 gal	Acetone	79964	8/24/82

\* Chlorinated Solvent

Table 1 (cont.)

Quantity	Chemical Name	P. O. #	Date
191 5 gal	Cal-clean	84059	9/14/82
192 4 gal	Cal-clean	84078	9/14/82
193 2 gal	Microposit 140 Remover	23239	10/1/82
194 4 liters	ISO Clear Acetone	23238	10/6/82
195 1 kg.	Vinylmethyldichlorosilane	84277	10/17/82
196 1 kg	Dimethyl Dichloro Silane	84277	10/17/82
197 750 gm	Trimethylchlorosilane	84277	10/17/82
198 1 kg	Tetrachlorosilane	84277	10/17/82
199 1.5 kg	Methyl dichloro Silane	84277	10/17/82
200 1 gal	IPA	84974	1/4/83
201 55 gal	acetone (wash)	10590	3/18/83
202 55 gal	IPA	10590	3/18/83
203 384 oz	Inhibisol safety solvent	12043	3/30/83
204 500 gm	2,2'-Thiodiethanol	89896	4/25/83
205 192 oz	Inhibisol spray penetone	10759	5/19/83
206 1 qt	Tetrahydrofuran AR	10790	5/26/83
207 1 gal	IPA AR	10790	5/26/83
208 2 pts	Dichloromethane spectrar (Met	10790	5/26/83 *
209 4 liters	Hexanes	10840	6/13/83
210 4 liters	Ethyl Acetate	10840	6/13/83
211 500 gm	Vinylmethyldichlorosilane	14127	7/6/83
212 1000 gm	Dimethyldichlorosilane	14127	7/6/83
213 500 gm	Dimethyldiethoxysilane	14161	7/19/83
214 1000 gm	Methyltriethoxysilane	14161	7/19/83
215 1000 gm	Methyltrimethoxysilane	14161	7/19/83
216 2 liters	Neutralizer 5	14172	7/21/83
217 48 oz	Freon TF (fluorocarbon)	14172	7/21/83
218 72 oz	Chlorothene NU	14172	7/21/83 *
219 1 liter	Dimethyl Formamide	14292	9/12/83
220 1 liter	Dichloromethane Spectral	14292	9/12/83 *
221 4 liters	IPA	14292	9/12/83
222 1 lb.	Synthesis of N-Methyl-p-tolui	14398	9/23/83
223 132 oz	Trichlorethane	14398	10/6/83 *
224 10 gal	Tetrahydrofuran	47755	11/6/83
225 1 gal	Fluorinert liquid	93673	11/28/83
226 1 gal	Sulfuric Acid	94243	1/20/84
227 10 gal	Acetone	94454	2/24/84
228 500 gm	Difluorotetrachloroethane	27252	3/19/84 *

\* Chlorinated Solvent

Table 1 (cont.)

Quantity	Chemical Name	P. D. #	Date
229 1 gallon	Triton X-100 surfactant	27384	7/30/84
230 50 lb. drum	CTA-639	27389	8/2/84
231 50 lb	ON-877	27389	8/2/84
232 50 kg	Polymist B-6	27383	8/2/84
233 100 lbs	RH-730 Aluminum Hydroxide	27366	8/2/84
234 50 lb	Vinol 107 Polyvinyl Alcohol	27382	8/2/84
235 50 lb	BIS-Phenol-A Polycarbonate	32812	9/7/84
236 200 gm	Octadecanamide	32814	9/10/84
237 25 gm	Merthiolate	32835	9/24/84
238 144 oz	Inhibisol solvent spray cleane	33814	10/2/84
239 55 gal	Trichloroethane	94189	10/26/84 *

\* Chlorinated Solvent