



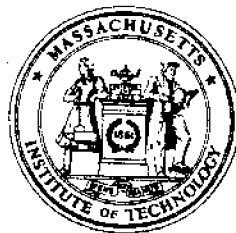
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**PREDICTION OF UNSTEADY SALINITY INTRUSION
IN ESTUARIES: MATHEMATICAL MODEL AND
USER'S MANUAL**

by

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Report No. MITSG 72-21
November 30, 1972

FOREWORD

This report is intended as a documentation of the computer program and as a user's manual for the implementation of the mathematical model for the prediction of unsteady salinity intrusion in estuaries. The details of the model development and verification are contained in the following report:

"A Mathematical Model for the Prediction of Unsteady Salinity Intrusion in Estuaries" by M. Llewellyn Thatcher and Donald R. F. Harleman, Ralph M. Parsons Laboratory for Water Resources and Hydrodynamics, Technical Report No. 144, February 1972 [also published by M.I.T. Sea Grant Project Office as Report No. MITSG 72-7 (index No. 72-307-Ccb)].

The unsteady salinity intrusion model is one-dimensional, although varying degrees of stratification are accounted for in the assumed longitudinal dispersion relationship.

This model is a component in two additional studies, one of which is concerned with the two-dimensional aspects of salinity intrusion (i.e. vertical salinity and velocity distributions in estuaries):

"Mathematical Simulation of Tidal Time-Averages of Salinity and Velocity Profiles in Estuaries" by John S. Fisher, John D. Ditmars and Arthur T. Ippen, Ralph M. Parsons Laboratory for Water Resources and Hydrodynamics, Technical Report No. 151, July 1972 [also published by M.I.T. Sea Grant Project Office as Report No. MITSG 72-11 (index No. 72-311-Ccb)]

The second study is concerned with the development of a model for predicting the transient longitudinal distribution of water quality parameters in estuaries:

"Numerical Model for the Prediction of Transient Water Quality in Estuary Networks" by James E. Dailey and Donald R. F. Harleman, Ralph M. Parsons Laboratory for Water Resources and Hydrodynamics, Technical Report No. 158, October 1972 [also published by M.I.T. Sea Grant Project Office as Report No. MITSG 72-15 (index No. 72-315-Ccb)]

This user's manual therefore assists in the implementation of the analytical and numerical models listed above.

ACKNOWLEDGEMENT

Primary support for this study came from the Office of Sea Grant, National Oceanic and Atmospheric Administration, U. S. Department of Commerce, Coherent Area Project Grant GH-88 and 2-35150, under the Estuary Modeling Program underway at the Ralph M. Parsons Laboratory for Water Resources and Hydrodynamics of the Department of Civil Engineering. Partial support was also provided by a grant from the Henry L. and Grace Doherty Charitable Foundation, Inc. This program is under the administrative and technical supervision of Professor Arthur T. Ippen and Professor Donald R. F. Harleman (DSR 72602 and DSR 73479). The purpose of the Estuary Modeling Program is to develop analytical and numerical techniques for modeling the behavior of estuaries and coastal embayments in order to extend the basic understanding of estuarine dynamics and the ability to make predictions relating to the estuarine environment.

The authors wish to express their appreciation to Mr. David Najarian, Research Assistant, for his valuable assistance throughout the preparation of this manual.

Table of Contents

	<u>Page</u>
I. <u>Introduction and Review</u>	5
1.1 Introduction	5
1.2 Review of the Mathematical Model	5
II. <u>Principal Steps in the Application of the Numerical Model</u>	11
2.1 Initial Decisions in Reducing Estuary Geometry to One-Dimensional Parameters	11
2.2 General Principles of Schematization	13
2.3 Establishing the Datum	13
2.4 Schematization to a Trapezoidal Cross-Section Without Storage	15
2.5 Schematization Including Storage and Conveyance (or Core) Areas	17
2.6 Verification of Tidal Hydraulics	19
2.6.1 Choice of Δt , Based on Stability Requirements	19
2.6.2 Channel Roughness as the Variable Parameter in Tidal Hydraulics Verification	19
2.7 Treatment of Entrance Salinity for Different Entrance Configurations	20
2.8 Initial Conditions	21
III. <u>Objective and Scope of the Computer Program</u>	22
IV. <u>Structure of the Program</u>	24
V. <u>User Control Through Input Data</u>	28
5.1 Introduction	28
5.2 Input Description and Format	28
VI. <u>Test Case and Example</u>	35
6.1 Introduction	35
6.2 Input Preparation and Results	35
VII. <u>Possible Error Messages</u>	57
VIII. <u>Implementation on Different Computer Systems</u>	59
8.1 Memory Size	59
8.2 FORTRAN IV Restrictions	59
8.3 Hints for Modifying the Program	59

	<u>Page</u>
References	60
List of Figures	61
Appendix I. Program Listing	I-1
Appendix II. Brief Description of Formats Used in Input	II-1
Appendix III. A Plotting Program Using a Calcomp Plotter	III-1

I. Introduction and Review

1.1 Introduction

This is a user's manual for the computer program implementing the transient salinity intrusion model developed by Thatcher and Harleman(1). The purpose of this manual is to enable researchers and engineers to use the computer program in an effective manner without requiring that they have other than a fundamental understanding of how digital computers accept input data, execute logical and numerical instructions, and present calculations in readable form. Although a knowledge of FORTRAN programming is helpful, it is not considered necessary for the prospective user of this program. However, to avoid unnecessary computing expense, it is recommended that a user not familiar with computer applications seek the assistance of someone who is experienced when it comes to the actual execution of the computer program.

1.2 Review of the Mathematical Model

Reference 1 describes the development of a predictive, one-dimensional mathematical model for longitudinal salinity distributions in estuaries under transient conditions of fresh water inflow and ocean tidal elevations. The governing equations for the tidal motion are:

the continuity equation,

$$b \frac{\partial h}{\partial t} + \frac{\partial Q}{\partial x} - q = 0 \quad (1-1)$$

and the longitudinal momentum equation,

$$\frac{\partial Q}{\partial t} + u \frac{\partial Q}{\partial x} + Q \frac{\partial u}{\partial x} + g \frac{\partial h}{\partial x} A + g \frac{Ad}{\rho} \frac{\partial \rho}{\partial x} + g \frac{Q|Q|}{AC^2 R_h} = 0 \quad (1-2)$$

Determination of the salinity distribution requires the conservation of salt, or salt balance equation,

$$\frac{\partial (A_{\text{Total}} s)}{\partial t} + \frac{\partial (Qs)}{\partial x} = \frac{\partial}{\partial x} \left[EA_{\text{Total}} \frac{\partial s}{\partial x} \right] \quad (1-3)$$

and an equation of state relating salinity and density,

$$\rho = 0.75 s + 1000 \quad (1-4)$$

where

- b = the total width of the channel [b_{Total} in Reference 1]
- h = the depth of water from surface to a horizontal datum
- Q = cross-sectional discharge
- q = lateral inflow per unit length
- u = average cross-sectional velocity of the conveyance area
- g = acceleration of gravity
- A = cross-sectional area of conveyance area [A_{core} of Reference 1]
- A_{Total} = total area of cross section, including storage
- d_c = depth of the centroid of conveyance area
- ρ = fluid density
- C = Chezy coefficient
- R_h = hydraulic radius
- E = longitudinal dispersion coefficient
- s = salinity representative of a cross-section (parts per thousand)

The boundary conditions required to solve these equations are:

- (1) the specification of tidal elevation at the ocean entrance as a function of time
- (2) the specification of fresh water inflow at the upstream boundary and of tributary inflows as functions of time
- (3) the specification of zero salt flux across the upstream boundary and
- (4) the specification of conditions on salinity at the downstream entrance of the estuary.

Information on water surface elevations at the ocean entrance is usually available from tide tables, and it is assumed that the user has fresh water inflow hydrographs. Conditions on salinity at the downstream boundary require

only a specification of ocean salinity, s_o or a maximum salinity. This salinity specification constitutes the boundary condition during the flood (incoming) flow. During ebb (outgoing) flow, the ocean boundary salinity is not specified, as the solution is continued in terms of a convective transport across the downstream boundary. When the ebb flow ceases and flood begins, the salinity at the downstream boundary will return to its maximum value s_o . When the downstream boundary coincides with the ocean entrance, the time of return of salinity to its maximum value will be relatively rapid due to longshore currents which sweep away much of the diluted water. It is assumed that this return is linear in time, and for the ocean entrance good results have been obtained using 5% of a tidal period as the time span over which the salinity returns to its maximum.

Figure 1.1 shows that this time span can be specified as considerably longer than 5% in order to account for the cases in which the downstream boundary is not at the ocean entrance.

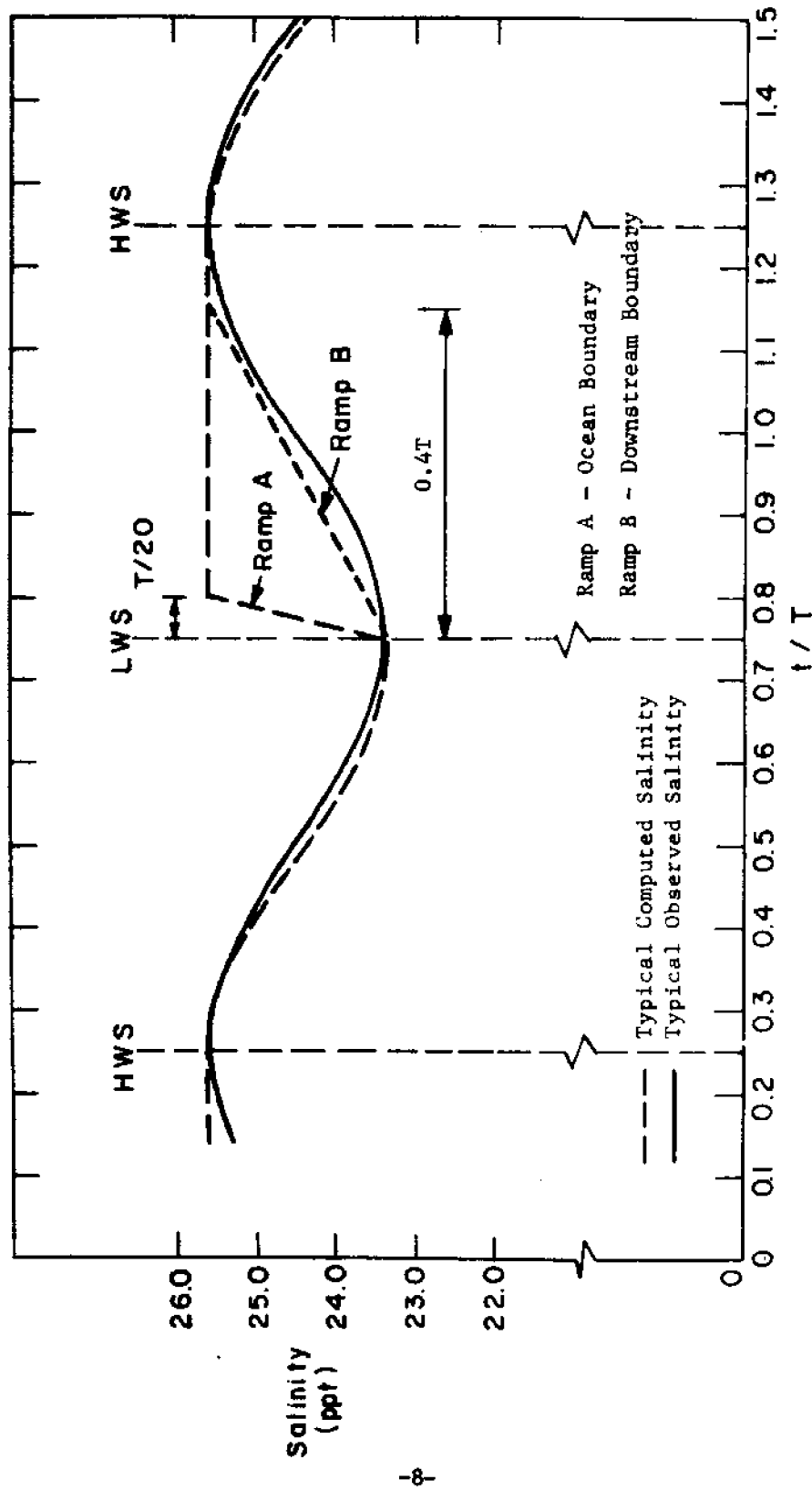
The longitudinal dispersion coefficient is related to the local non-dimensional salinity gradient, $\frac{\partial s/s_o}{\partial x/L}$, (where L = length of estuary) in the salinity intrusion region. The dispersion coefficient for the entire estuary is

$$E(x,t) = K \left| \frac{\partial s/s_o}{\partial x/L} \right| + 77 n u R_h^{5/6} \quad (1-5)$$

where K is a constant of proportionality related to the degree of stratification as measured by gross estuarine parameters. The second term represents the dispersion in the fresh water region where n is the Manning resistance coefficient, u is the cross-sectional average velocity and R_h is the hydraulic radius.

Reference 1 has shown that a correlation exists between $K/u_o L$ and an estuary number E_D . This permits the determination of a value for K for each tidal period based on the previous tidal period's calculated value of the tidal prism, P_T , the densimetric Froude number, F_D , the fresh water inflow, Q_f , and the tidal period, T . These are the gross parameters which define the estuary number

$$E_D = \frac{P_T F_D^2}{Q_f T} \quad (1-6)$$



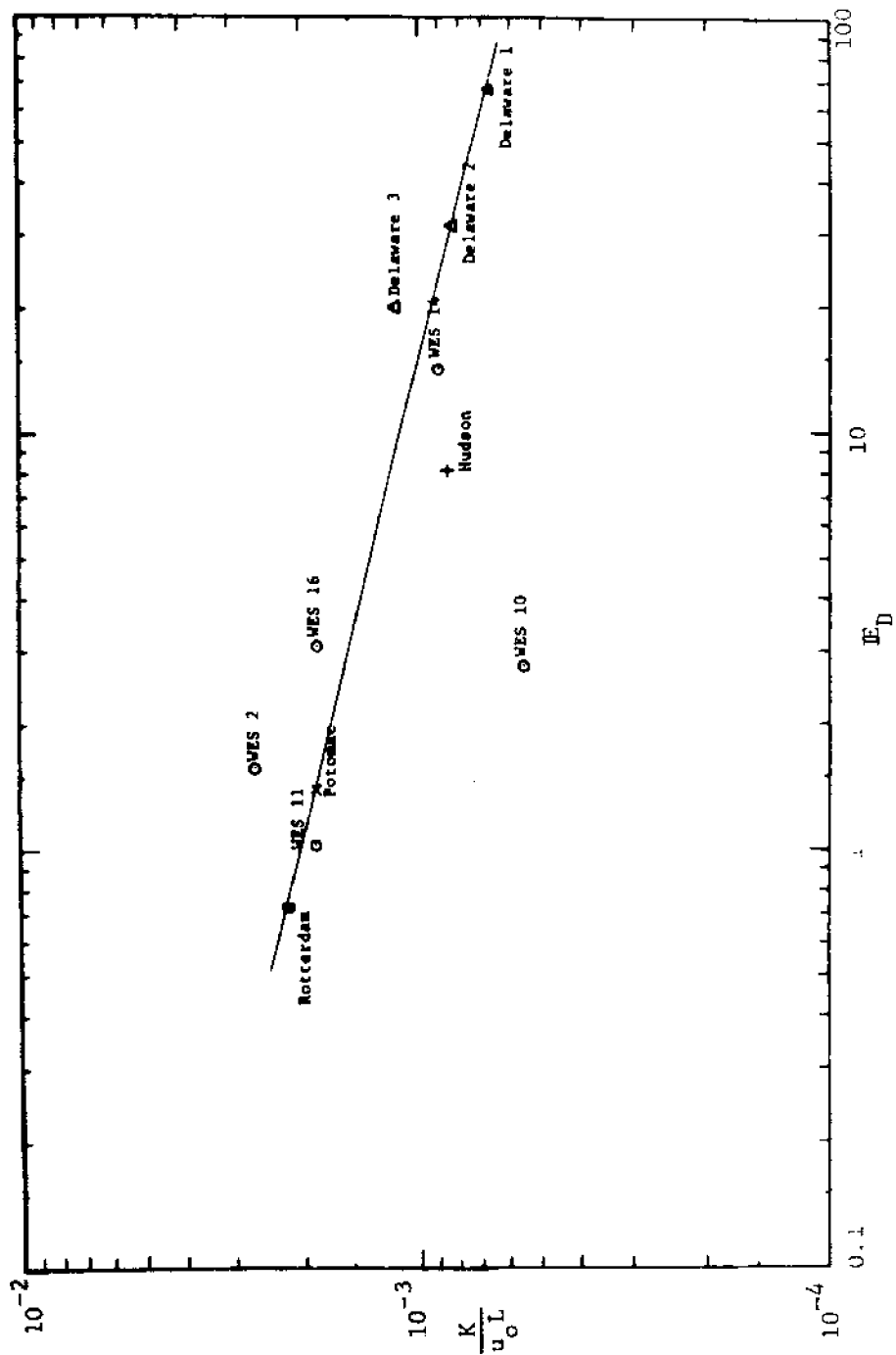
Representation of Entrance Boundary Condition on Salinity for Estuary

Figure 1.1

The correlation between $K/u_o L$ and E_D , developed using steady-state salinity distribution data, covers a wide range of stratification conditions. This relationship is shown in Figure 1.2.

Calculations of salinity, water surface elevation and discharge are made by solving Equations 1-1, 1-2, 1-3 and 1-4 in finite difference form. The numerical model consists of two fundamental components of calculation. The first component solves for the water surface elevations and discharges and is a modification of the numerical model of Harleman and Lee(2). The second component is the finite difference solution of the salt balance Equation 1-3. Details of the finite-difference techniques used are described in Reference 1.

The solution requires initial conditions and advances in time in accordance with the values of the time-varying boundary conditions of tidal elevation at the entrance and fresh water inflows. The solution determines surface elevations, discharges and salinities as functions of longitudinal distance and time for any specified number of tidal periods. Details pertaining to the practical application of the transient salinity intrusion numerical model are presented in the following sections of this report.



Correlation of Dispersion Parameter to Degree of Stratification

Figure 1.2

II. Principal Steps in the Application of the Numerical Model

2.1 Initial Decisions in Reducing Estuary Geometry to One-Dimensional Parameters

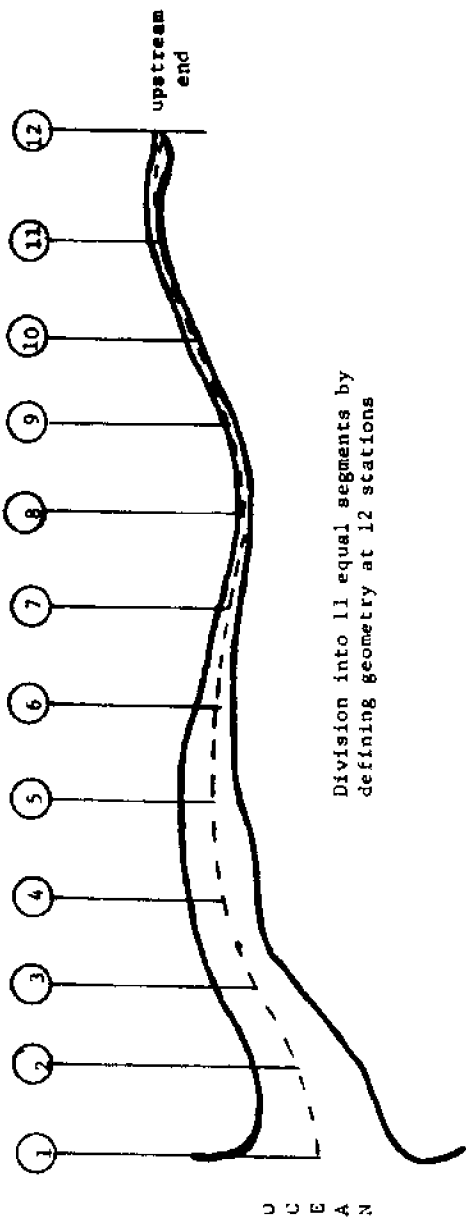
The geometric configuration of the estuary must be studied to determine the appropriate extent or domain of the estuary, and the degree of detail necessary in terms of discretization. The longitudinal extent of the estuary is the first determination to be made. The upstream boundary is usually taken at the head of tide. This can be a physical closed-end such as a dam, lock, waterfall or rapids. If no physical barrier exists, the estuary is of the open-end type, and the upstream boundary is set at a point far enough upstream so as to be unaffected by tidal action.

Ideally, the downstream boundary should coincide with the ocean entrance to the estuary. The case studies presented in Ref. (1) consider three different types of estuaries from the standpoint of downstream boundary location:

1. Delaware. The downstream boundary is at the ocean entrance (Cape Henlopen-Cape May).
2. Potomac. The downstream boundary is at the junction with Chesapeake Bay.
3. Hudson. The downstream boundary was chosen to be at the Battery. Below the Battery the estuary is no longer one-dimensional because of the connection to Long Island Sound through the East River.

The salinity boundary condition for the Delaware and Potomac were assumed to follow "ramp A" of Figure 1.1 with the value of s_0 being determined by salinity conditions in the Atlantic and in Chesapeake Bay respectively. In the case of the Hudson the salinity variation at the Battery was assumed to be more nearly sinusoidal as shown by "ramp B" of Figure 1.1.

Once the limits of the estuary have been defined, the next decision is that of determining the segment length, Δx , for discretization of the physical characteristics. This interval of discretization, Δx , must be small enough to be representative of the principal variation in geometry. Case studies made in Ref. (1) have shown that segment lengths of 2 to 3 miles generally provide sufficient detail. The computer model requires that the total number of stations be an even number. This is the result of a staggered finite-difference scheme in which water surface elevations and salinities are calculated at odd numbered stations and discharges at even numbered stations. Figure 2.1 illustrates



Station Number

1	2	3	4	5	6	7	8	9	10	11	12
	Q		Q		Q		Q		Q		Q
S		S		S		S		S		S	

S - water surface elevation and S - salinity; determined at odd numbered stations
Q - discharge; determined at even numbered stations

Segmentation of the Estuary

Figure 2.1

the division of a simplified estuary into 12 stations (11 segments).

After the identification of longitudinal stations has been accomplished the schematization of the natural geometry into basic parameters of depth and area is performed. Two basic types of schematization are available in this model and the user must choose that which best suits his particular study.

The first type, Figure 2.2a, is that of a trapezoidal cross-section. This type of schematization is especially useful in cases where it is important to represent the change in estuary width with change in depth. It assumes that the entire trapezoidal area participates in the longitudinal tidal flow and consequently it does not provide for portions of the estuary which act as storage. It is noted that schematization to a rectangular cross-section is a special case of the trapezoidal schematization.

The second type, Figure 2.2b, represents the area of the discrete interval by two rectangular areas; one for storage, the other for conveyance. This schematization simplifies the cross-section in terms of geometry, and allows the user to specially treat the cases of embayments and other portions of the estuaries which do not act directly or significantly in terms of the longitudinal tidal flow.

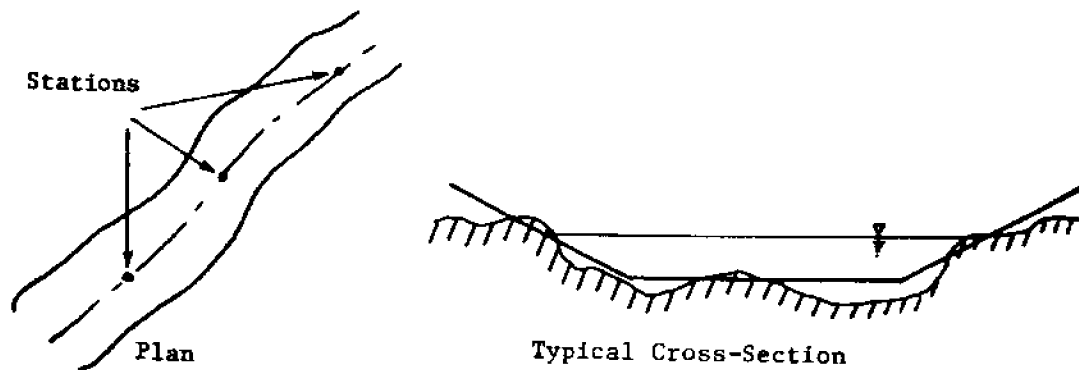
After the user has decided which of the types of schematization to use he can proceed to the schematization process itself. This process will depend on the particular case, the time available and sources of data. The following sections are intended to serve as an example of the schematization process.

2.2 General Principles of Schematization

The numerical model employs discrete quantities. At each station such as those indicated in Figure 2.2 the user must determine basic parameters which permit the calculation of a conveyance cross-sectional area, A_{core} , and, if so chosen, a storage area, A_{storage} . These areas are to be representative of the estuary in the region of the station being considered. More precisely, they represent an average for the segment centered on that station. The cross-section at the station itself may not be representative of the entire segment.

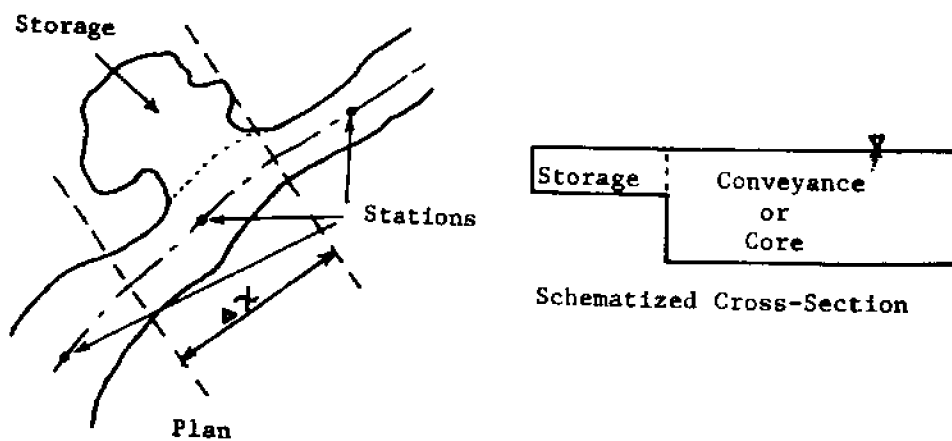
2.3 Establishing the Datum

In order to correctly represent the water surface the data defining each cross-section must be related to a common horizontal datum. Often this presents



Trapezoidal Schematization

Figure 2.2a



Storage and Conveyance (or Core) Area Schematization

Figure 2.2b

a problem because hydrographic charts are intended for navigation and not for numerical modeling. Chart depths are relative to some water surface such as local mean low water, local mean river level, or other local water surface which is not necessarily horizontal.

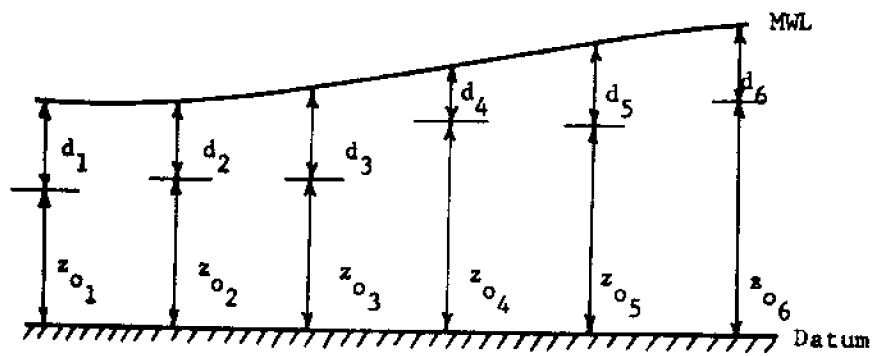
In order to accurately define the estuarine geometry the bottom elevations of the schematized cross-sections must be specified relative to a common horizontal datum. This entails obtaining information which defines the local datums. Such information is usually available from the organization that issued the charts. (For example, the National Ocean Survey of the U.S. Department of Commerce.)

Figure 2.3 shows how the common horizontal datum is used. Each station is schematized with reference to the mean water level (MWL). The depth from MWL to the bottom of the schematized cross-section is d_j . The location of the common horizontal datum is specified by the user, and the changes in local datums are included by defining z_{o_j} so that $d_j + z_{o_j}$ measures the distance from the common horizontal datum to the local MWL. The changes in local datums are illustrated in Figure 2.3 by the fact that the MWL varies with respect to the horizontal.

If no data is available on the local datums it is possible to assume that the local datums are all the same. This means that the MWL is assumed to be horizontal from one end of the estuary to the other. The effect of such an assumption is the introduction of an error in the depth of the schematized cross-sections equal to the difference between the assumed local MWL and the actual local MWL.

2.4 Schematization to a Trapezoidal Cross-Section Without Storage

The cross-section which typifies each reach is considered to be centered on the station. The trapezoid which most closely represents the cross-section of the reach should be found. This can be achieved by a variety of techniques



Sketch of Common Datum

Figure 2.3

ranging from estimation by visual inspection to mathematical fitting techniques employing chart soundings. In terms of required input data to the computer program, the trapezoid selected must be defined by the following four parameters as shown in Figure 2.4a:

1. bottom width, b_o
2. depth, d
3. horizontal distance per unit vertical distance, SS
4. distance from common datum to the bottom of the channel, z_o

The depth is measured to mean water level (MWL).

2.5 Schematization Including Storage and Conveyance (or Core) Areas

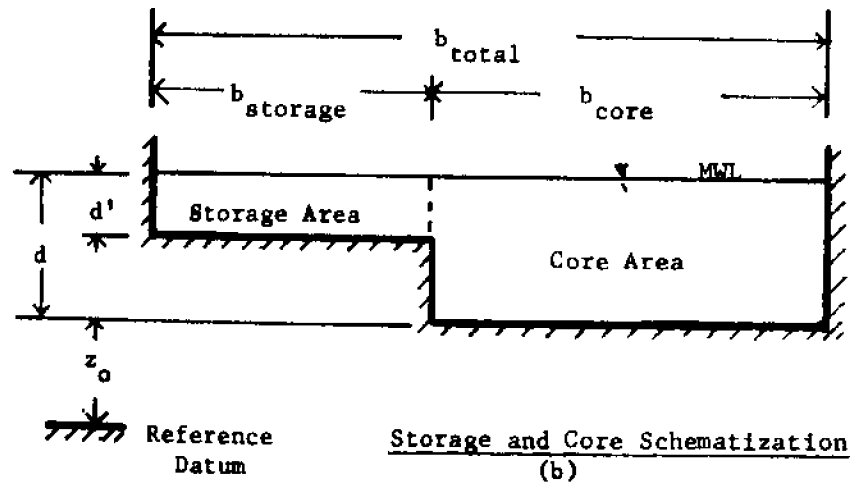
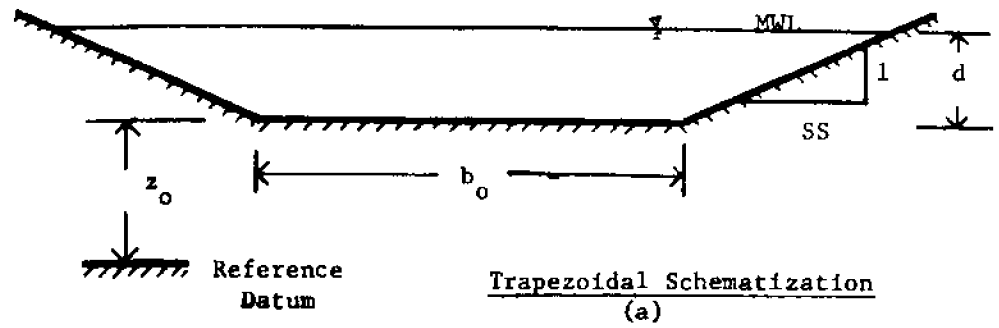
The plan view of Figure 2.2b shows a typical estuary reach containing an embayment. The water in the embayment does not participate in the longitudinal tidal transport, however it fills and empties with the change of water surface elevation. The embayment acts as storage, and in estuaries where the surface area of such embayments is a significant percentage of the total surface area, the schematization should represent the storage action.

The schematization employed is based on the determination of a conveyance or core area which is defined in terms of a width b_{core} and a depth d . This requires that the user determine an average cross-sectional area and width for the segment. This area is then represented in rectangular form by dividing it by the average width to obtain the depth, d .

Figure 2.4b shows how this core area is joined to a storage area. To define the storage area it is necessary to define a depth of the storage area, d' . This depth, multiplied by the surface area of storage, $A_{storage}^s$, yields a volume of storage $V_{storage}$. To obtain an equivalent cross-sectional storage area $A_{storage}$, the volume of storage is divided by the segment length, Δx . Further division of this cross-sectional storage area by the storage depth, d' gives the equivalent width, $b_{storage}$, of the schematized rectangular cross-section. These relationships are:

$$V_{storage} = A_{storage}^s d'$$

$$A_{storage} = \frac{V_{storage}}{\Delta x}$$



Basic Schematized Cross-Sections

Figure 2.4

$$b_{\text{storage}} = \frac{A_{\text{storage}}}{d'} = \frac{A_{\text{storage}}^s}{\Delta x} \quad (2-1)$$

The final relationship shows how the schematization process spreads out the surface area over the reach length Δx .

The data required by the computer program for each section are:

1. the core width, b_{core}
2. the core depth, d
3. the total width, $b_{\text{total}} = b_{\text{core}} + b_{\text{storage}}$
4. the storage depth, d'
5. the distance from the common datum to the bottom of the schematized section, z_0

Again, it must be remembered that the depth is with respect to mean water level.

2.6 Verification of Tidal Hydraulics

2.6.1 Choice of Δt , Based on Stability Requirements

The explicit finite-difference solution of the continuity and the momentum equations (1-1 and 1-2) imposes a limit on the size of time increment, Δt . This requirement can be approximated by

$$\Delta t \leq \frac{\Delta x}{u+c} \quad (2-2)$$

where u is the average cross-sectional velocity and c is the wave speed \sqrt{gh} at the same location. Equation 2-2 can be used to estimate the Δt corresponding to a particular Δx by estimating u and c . For example, if the maximum depth of an estuary is 40 ft. and the expected tidal velocity will not be greater than 2 feet per second, Equation 2-2 limits Δt to a maximum of about $\frac{\Delta x}{38}$ seconds. For $\Delta x = 10,000$ feet, $\Delta t \leq 263$ seconds, and for a tidal period of 12.4 hours, this would require 170 time intervals per tidal cycle.

2.6.2 Channel Roughness as the Variable Parameter in Tidal Hydraulics Verification

Tide table data on tidal range and phase is utilized to insure that the numerical model correctly represents the advective characteristics of the estuary. The tidal verification process is executed by varying the channel roughness (Manning's n) so as to achieve the best fit with available data. Often the data is presented in terms of local tidal range and phase lags for a particular tidal

range at the ocean, or downstream boundary. The tidal verification runs are made by assuming typical fresh water inflows and holding these constant for each tidal cycle of calculation. The time varying surface elevation at the ocean is repeated for each tidal cycle. Such a repetition of boundary conditions defines a quasi-steady state condition. Initial conditions of surface elevation and discharge can be approximated (or set equal to zero) and any reasonable approximation of the longitudinal salinity distribution can be used for an initial condition on salinity. The numerical model is then run and it has been found that about 5 to 8 tidal periods of calculation will result in tidal elevations and discharges which are essentially the same from one period to the next. This procedure can be applied for different variations in channel roughness until the resulting convergent surface elevations give a satisfactory verification of the tidal data. The unsteady salinity intrusion study can then be continued using this distribution of channel roughness. It is noted that the tidal hydraulics are not very sensitive to small changes in the salinity distribution and this is why the above procedure can be successfully executed using only an approximate salinity distribution.

Typical values of Manning's n found in Ref. (1) are:

Delaware $n = 0.017$ to 0.033 (ave. = 0.024)

Potomac $n = 0.018$

Hudson $n = 0.015$

2.7 Treatment of Entrance Salinity for Different Entrance Configurations

When the estuary being studied can be schematized as far downstream as the ocean, the boundary treatment requires the specification of the ocean salinity, s_o , and the portion of the tidal period during which the salinity returns from the value at low water slack (LWS) to the maximum value, s_o . For such cases it has been found that a time equal to $0.05T$ for the return portion gives reasonable results. A well defined ocean boundary permits the prediction of salinity distributions starting with initial conditions requiring only the specification of the ocean salinity, s_o .

In cases where the downstream boundary cannot be taken at the ocean, but must be located farther upstream, the assumption that longshore currents will sweep away most of the diluted ebb flow is not as valid as it was for the ocean

boundary. The user can predict longitudinal salinity distributions under such conditions by specifying the maximum salinity at the downstream boundary and also by specifying a reasonable return time during which the salinity will return from its low water slack value to the maximum value. This return time will be between $0.05T$ and $0.4T$. Ref. (1) has shown that for the Hudson a satisfactory representation of the downstream boundary at the Battery was achieved using $0.4T$. The observed depth averaged salinity variation at the Battery was approximately sinusoidal. Figure 1.1 illustrates how the different specifications of return time affect the representation of the temporal variation of salinity at the downstream boundary.

2.8 Initial Conditions

An initial longitudinal salinity distribution must be specified to begin the calculation. In addition initial values of surface elevations and discharges are required. As discussed in Section 2.6.2, the effect of arbitrary initial conditions of surface elevation and discharge is negligible after 5 to 8 tidal cycles of calculation, and this characteristic can be used to generate realistic initial conditions of surface elevations and discharges for the unsteady salinity intrusion study.

Coast and Geodetic Tide Tables can be used to specify the maximum and minimum tidal elevations at the downstream boundary. By assuming an approximate salinity distribution, the user can back-up eight tidal cycles from the beginning of the study period and initiate a calculation using initial values of elevation and discharges equal to zero. The tidal surface elevations and the discharges produced after eight cycles are appropriate initial conditions for the unsteady study. The user can combine the elevations and discharges at the end of the introductory eight cycles of calculation with the initial salinity distribution and thus obtain a complete set of initial conditions for the unsteady salinity intrusion study.

III. Objective and Scope of the Computer Program

The objective of the computer program is to calculate water surface elevations, salinities and discharges as functions of time and of longitudinal distance along the estuary. This calculation is an implementation of the finite difference mathematical model developed in Ref. 1.

The scope of the program is defined in terms of three areas of user control. These areas of control are (1) specification of estuarine geometry and roughness, (2) specification of time-varying boundary conditions and of initial conditions, and (3) selection of the frequency and content of the output from the computer.

In the specification of geometry the user describes the estuary cross-sections in terms of either a rectangular or trapezoidal channel, or in terms of a rectangular channel with a storage area (Sections 2.2, 2.3, 2.4 and 2.5). Simplified descriptions can be used for the special cases of a uniform rectangular section and for a rectangular section whose width varies in an exponential manner. The channel roughness is described in terms of Manning's n and wind shear effects can be specified.

The boundary conditions are: tidal surface elevations at the downstream boundary, fresh water inflows at the head and at other locations along the estuary, and the maximum salinity, s_0 , at the downstream boundary. If the tidal surface elevations at the downstream boundary are repeating from tidal cycle to tidal cycle, then the specification of this boundary condition can be made in terms of harmonic components or in tabular form. For the situation in which tidal variations are expressed in terms of a series of high and low water elevations (as from data found in tide tables), the user can specify the time series and the program will fit a cosine curve to the specified values of high and low water elevations. The specification of fresh water inflows and the maximum entrance salinity s_0 is made for each tidal period of calculation.

The initial conditions of surface elevation, discharge, and salinity can be specified in a convenient form; either from the result of a previous calculation stored as a "data set", or in punch card form.

The computer output can be printed or stored on a sequential data device such as a magnetic tape. The program provides the user with the ability

to specify the frequency of printing the surface elevations, salinities and discharges. It is also possible to print out high water slack salinities and to punch the final calculated values into cards so that these values can become the initial conditions of a subsequent calculation. The storing of calculated values of elevations, salinities and discharges on a sequential data device is convenient for subsequent plotting of these results by a plotting program such as that included in Appendix III. It is also possible to store the calculated values of the longitudinal dispersion coefficient, E , on a sequential data device.

IV. Structure of the Program

The program consists of two principal multipurpose routines and several single purpose routines.

The first principal multipurpose routine is the main-line program, hereafter referred to as MAIN. MAIN executes the input routines TIDIN and OSIN and proceeds, cycle by cycle, to read in the time varying boundary specifications and execute the basic calculation routine, MARCH. When the calculation is completed MAIN outputs the final values of surface elevation, discharge and salinity.

The second principal multipurpose routine is MARCH. MARCH performs the basic calculation in time steps of $2\Delta t$, solving first the continuity equation, then the momentum equation, and finally the salt balance equation. In the solution of the salt balance equation, three routines are executed: (1) TDISP, which calculates the dispersion coefficient; (2) TOCNB, which performs the entrance or ocean boundary calculation; and (3) TRIDG, which solves the resulting tri-diagonal simultaneous equations.

The output is produced by executing routine SPEW.

At the end of each tidal cycle MARCH calculates the total mass of salt in the estuary. If requested by the user, the time of high water slack is found and the corresponding high water slack salinities are printed. Finally, the value of the dispersion parameter K is determined by executing routine ESNOD which determines the estuary number $\frac{P_{TF} D^2}{Q_f T}$ and determines K in terms of the specified $K/u_o L$ correlation with the estuary number.

The single purpose routines executed ("called") by MAIN and by MARCH are:

TIDIN: An input routine for geometry, roughness, wind effects and entrance tidal variation.

OSIN: An input routine for specification of parameters related to the solution of the salt balance equation and for the specification of output frequencies and options.

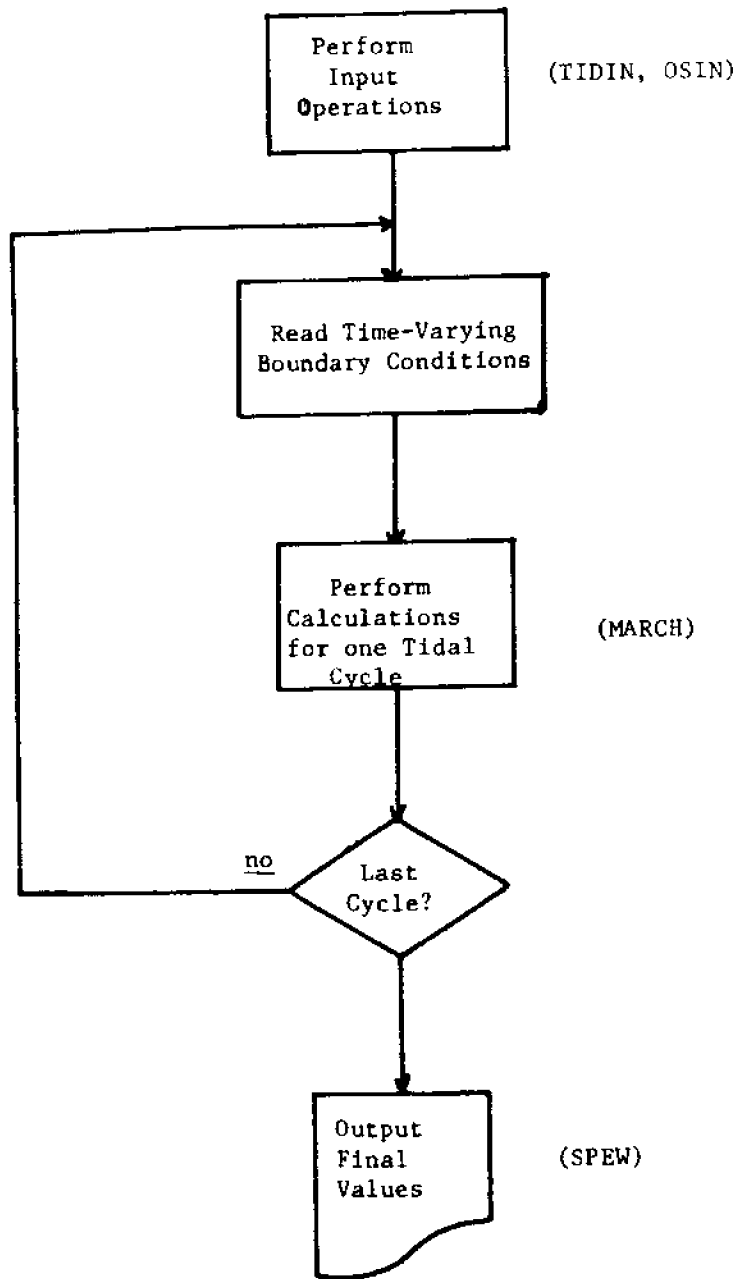
TDISP: A calculation of the dispersion coefficient from the equation

$$E = K \left| \frac{\partial S}{\partial x} \right| + n E_T \quad (4-1)$$

where $n = 1$, unless otherwise specified by the user.

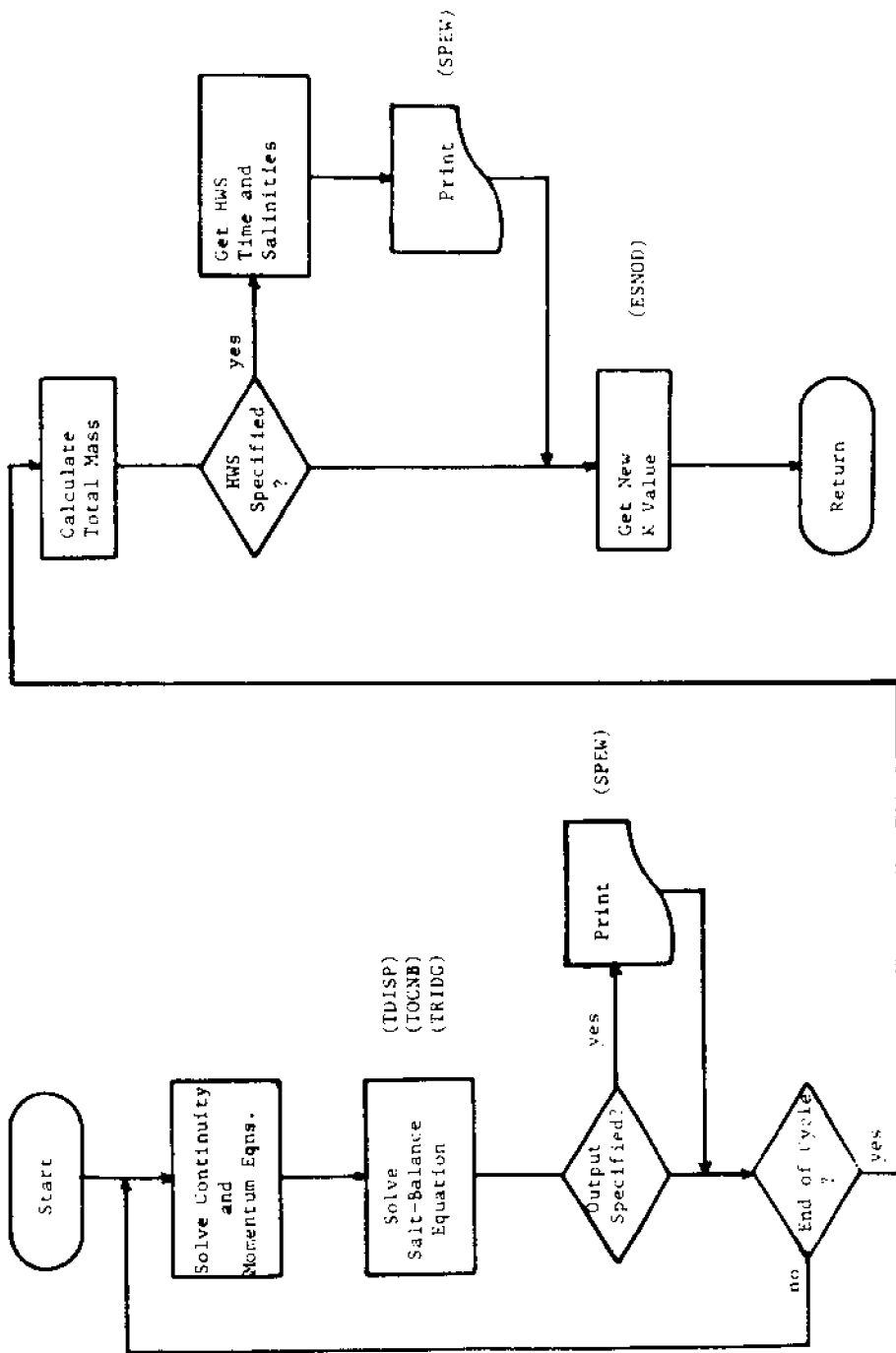
- TOCNB: This routine supplies the finite-difference equation for the downstream element of the estuary (entrance boundary treatment).
- TRIDG: A solver of simultaneous equations in tri-diagonal form.
- ESNOD: Calculates the tidal prism, P_T , the densimetric Froude number, the estuary number $\frac{P_T F_D^2}{Q_f T}$ and then uses the correlation of $\frac{K}{u_o L}$ vs. $\frac{P_T F_D^2}{Q_f T}$ to determine the value of dispersion parameter K to be used in the next tidal cycle of calculation.
- SPEW: An output routine for the surface elevation, discharge, and salinities.
- ERRL: An error handling routine to assist the user in preparing input decks and to understand what went wrong.
- SEQTST: A sequence checking routine.

Figure 4.1 shows the basic structure of the program MAIN and Figure 4.2 shows the basic structure of subroutine MARCH. In both figures the single purpose routines used at the corresponding step are indicated in parenthesis.



Structural Diagram of Routine MAIN

Figure 4.1



Structural Diagram of Subroutine MARCH

Figure 4.2

V. User Control Through Input Data

5.1 Introduction

This section presents the requirements and format for control of the computer program. There are six subsections, each one defining input requirements. The input is in the form of eighty column unit record cards and the perforation of these cards is according to specified FORTRAN formats. The card is divided according to the format instructions which specify the columns that correspond to each piece of input data. These columns constitute the "field" in the card for each item of data. A detailed explanation of FORTRAN formats can be found in many texts, the specific reference for this program is the IBM System/360, "FORTRAN IV Language", manual Form C28-6515, obtainable from local IBM branch offices (Ref. 3).

For the user who has had little experience with FORTRAN programming, a brief description of the format codes used in this section is included with explanation in Appendix II.

5.2 Input Description and Format

(Note: Punch decimal point in all "F" format fields.)

Section 1. Description of Schematized Estuary

<u>Card No.</u>		<u>Format</u>
A1	Descriptive Information for a Heading	20A4
A2	Descriptive Information for a Heading	"
A3	Field 1 (right justified) Number of Sections (even)	I5
	Field 2 Total Length in Feet	F10.d
A4	Field 1 (right justified) Number of Time Intervals Per Cycle (even)	I5
	Field 2 Tidal Period (cycle) in Seconds	F10.d
A5	Field 1, Type of Schematization, Number 1,2,3,4 or 5 in Column 5 for:	I5
	1: Rectangular Cross-Section	
	2: Irregular Cross-Section (storage)	
	3: Trapezoidal Cross-Section	
	4: Exponentially Varying Width, Rectangular Cross-Section	
	5: Constant Rectangular Cross-Section	
	Field 2, for Closed End Estuary Blank or 0, for Open End Case = 1	I5

For each case:

Case 1, Rectangular Cross-Section

For each section a card:

<u>Card No.</u>		<u>Format</u>
A6(1)	1: Sequence Number ⁽¹⁾ (right justified)	I5,5x
	2: Depth, d	F10.d
	3: Distance from Datum to Bottom of Section, z _o	F10.d
	4: Width at Section, b = b _{core}	F10.d

Case 2, Irregular Cross Section (storage)

For each section a card:

A6(2)	Field 1 Sequence Number ⁽¹⁾ (right justified)	I5,5x
etc.	Field 2 Depth, d _{core}	F10.d
	Field 3 Distance from Datum to Bottom of Section, z _o	"
	Field 4 Total Width, b _{total}	"
	Field 5 Core Width, b _{core}	"
	Field 6 Storage Depth, d'	"

Case 3, Trapezoidal Cross-Section

For each section a card:

A6(3)	Field 1 Sequence Number ⁽¹⁾ (right justified)	I5,5x
etc.	Field 2 Depth, d	F10.d
	Field 3 Distance from Datum to Bottom of Section, z _o	"
	Field 4 Side Slope, Horizontal Distance Corresponding to a Unit Vertical Distance	"
	Field 5 Bottom Width, b _o	"

Case 4, Exponentially Varying Width, Rectangular Cross-Section

Width is assumed to vary as

$$b(j) = b(1) \exp(-\delta x)$$

A6(4)	Field 1 Argument, δ	E10.d
	Field 2 Width at Entrance, b(1)	F10.d
	then for each station a card:	
A7(4)	Field 1 Sequence Number ⁽¹⁾ (right justified)	I5,5x
etc.	Field 2 Depth, d	F10.d
	Field 3 Distance from Datum to Bottom of Section, z _o	F10.d

(1) Note: Sequencing need not start at the number 1, but must be ascending with an increment of unity (i.e. j, j + 1, j + 2, . . .)

<u>Card No.</u>		<u>Format</u>
	<u>Case 5, Constant Rectangular Cross-Section</u>	
A6(5)	Field 1 Blank	10x
	Field 2 Depth, d	F10.d
	Field 3 Distance from Datum to Bottom of Section, z_0	F10.d
	Field 4 Width, $b = b_{\text{core}}$	F10.d
	<u>Section 2. Channel Roughness and Wind Effects</u>	
B1	<u>Case 1</u> (Enter "1" in Column 5) Constant Roughness and Constant Wind Effect	
	<u>Case 2</u> (Enter "2" in Column 5) Variation in Roughness, Wind, or both	I5
	If Case 1 is specified only one card is necessary; for Case 2 a card for each section must be punched.	
B2	Field 1 Sequence Number	I5, 5x
etc.	Field 2 Manning's "n" in ft-sec. units	F10.d, 10
	Field 3 Wind Speed ft/sec, V_j	F10.d
	Field 4 Angle of Wind in Radians Relative to Axis of Channel (blowing upstream is 0 angle)	F10.d
	<u>Section 3. Surface Elevation at Entrance</u>	
C1	Type of Variation: Number 1, 2, or 3 in Column 5	
	<u>Case 1</u> Cosine Fit to HW, LW Series	
	<u>Case 2</u> Repeating, Harmonically Defined in Terms of Amplitudes and Periods of each Component	
	<u>Case 3</u> Repeating, Defined by a Table of Values	I5
C2	Distance MWL above MSL at Entrance	F10.d
	<u>Case 1</u> Cosine Fit to HW, LW Series	
C3(1)	Field 1 Initial HW with Respect to MWL at the Entrance	F10.d
	<u>Case 2</u> Repeating, Harmonically Determined	
C3(2)	Field 1 Number of Harmonics	I5
C4(2)	Field 1 Amplitude in Feet, Field 2 Period in Seconds ⁽¹⁾	2F10.d
	<u>Case 3</u> Repeating Tabular	
C3(3)	Card A4 specified the number of time intervals per cycle and the specification was an even number. A series of elevations of length equal to 1/2 the number of time intervals is required. Thus every other interval is defined. The tabular information will be in five fields per card.	5F10.d

⁽¹⁾ Note: If only one harmonic, the period is taken as that of card A4.

Card No.		<u>Forma</u>
	<u>Section 4. Definitions Concerning the Salt Balance Equation</u>	
D1	Field 1 Initial Value of the Dispersion Parameter, K	F10.
	Field 2 Multiplying Factor n in Equation 4-1, see below. If the field is set to zero or left blank n is automatically set to 1. Right justify to Column 15. ($E = K \left \frac{\partial \theta}{\partial x} \right + n E_T$)	I5
D2	Field 1 Intercept of $K/u_0 L$ vs. IE_D relation; if blank or zero, intercept set at 0.00215, (this corresponds to Figure 1.2)	F10.d
	Field 2 Maximum limit of intrusion length for the calculation. If left blank or set to zero the total estuary length is taken. (This can be used to save computing time in cases where the maximum intrusion is well known and is significantly less than the total estuary length.)	F10.d
	Field 3 Fraction (0-0.5) of the tidal period during which the entrance salinity returns linearly to s_0 from its value at LWS. (Usually 0.05 if entrance is at the ocean but not more than 0.5.)	F10.d
	Field 4 Fraction (0-0.5) of the tidal period during which the entrance salinity remains at s_0 after HWS (usually 0.0).	F10.d
D3	Source of Initial Conditions: Punch 1, 2 or 3 in Column 5 to define one of the following cases. Case 1: Sequential dataset containing a complete tidal period of elevations, η , salinities, s , and discharges Q . Case 2: Elevations, η , and discharges Q , from a sequential dataset as in Case 1, but salinities s from cards. Case 3: Elevations, η , salinities, s , and discharges, Q , all from cards.	I5
	For cases 1 and 2: The sequential dataset is assumed to have a FORTRAN dataset reference number of 3 unless the user wishes to override this by specifying another number in Columns 6-10, right justified. It is assumed that the dataset is organized in record lengths of "total number of sections /2" and that there are "total number of time intervals /2" sets of three	I5

<u>Card No.</u>		<u>Format</u>
	records, the records being in the sequence η , s , Q . (All elevations are assumed to be relative to MSL.) The last set of η , s , and Q will be taken as the initial conditions.	
	<u>For Case 2:</u>	
D4(2)	Initial salinities, s , at each odd section starting at the entrance, 5 per card	5F10.d
	<u>For Case 3:</u>	
D4(3)	Initial elevations, η at each odd station starting at entrance, 5 per card	5F10.d
	Initial salinities s , at each odd station starting at the entrance, 5 per card, the first value beginning a new card	5F10.d
	Initial discharges Q , at each even station, starting at the seaward-most station, 5 per card, the first value beginning a new card	5F10.d
	<u>Section 5. Output Selection</u>	
	<u>Options</u>	
E1	Four options are taken by punching keywords into any of the first four fields of 5 columns, always beginning on the first of the 5 columns.	
	<u>Option 1:</u> keyword = KEEP. This keyword will cause the program to store the calculated results of each time step on a sequential dataset, thereby enabling the user to keep the entire result for plotting or further processing. The unit number of this dataset, will be 15 unless the user specifies another number in Columns 26-30, right justified.	
	<u>Option 2:</u> keyword = HWS. This keyword will cause the calculation of High Water Slack salinities and these will be printed at the end of each tidal cycle.	
	<u>Option 3:</u> keyword = FC. This keyword will cause the program to punch out the final values of elevation, salinity and discharge in the format which permits these cards to be used as the initial conditions for a subsequent calculation. Elevations are relative to MSL.	

Card No.

Format

Option 4: keyword = E. This keyword causes the program to output the dispersion coefficient calculated at each time step to a sequential dataset. The unit number of this dataset will be 11 unless the user specifies another number in Columns 31-35 right justified.

4(A4,1x),
215

Frequency of Basic Output

Frequency is measured in time increments between outputting a calculated variable, η , s , or Q . "0" would be not at all, "2" would be at the end of each calculation, "m" would be at the beginning of each tidal cycle and every m intervals thereafter. (Each calculation is of $2\Delta t$, therefore m must be even.)

E2 Surface elevation specification: H is punched in Column 1 and the frequency is punched, right justified, in Columns 6-10.

(A1,4x,I5)

E3 Salinity specification: S is punched in Column 1 and the frequency is punched, right justified, in Columns 6-10

(A1,4x,I5)

E4 Discharge specification: Q is punched in Column 1 and the frequency is punched, right justified, in Columns 6-10.

Note: Cards E2, E3 and E4 can be in any order relative to each other.

Section 6. Input of Time Varying Boundary Parameters at each Tidal Cycle

F1 Specification of Transient Fresh Water Input Locations

Field 1: Number of Transient Fresh Water Inputs

Field 2: The number of these which will be summed to compute the Q_f used in calculating the estuary number.

Field 3,4,5,6, etc.: The section numbers of the inflows.

These refer to an inflow between the section designated and the adjacent seaward section. The order given in fields 3,4,5,6 . . . is the order to be followed in the following specification cards for each tidal cycle. Those inflows to be summed into Q_f as specified in field 2 must be given first.

1615

F2 Number of tidal cycles of calculation in Columns 1-5, right justified

I5

Card No.

Format

F3 Time varying boundary parameters for each cycle.

etc. Field 1 Number of the Tidal Cycle

15,5x

Field 2 If surface elevation at the entrance is time varying (case 1), the LW or minimum elevation.

F10.d

Field 3 If surface elevation at the entrance is time varying (case 1), the HW or maximum elevation

F10.d

Field 4 The maximum salinity at the entrance during the cycle, s_0

F10.d

Fields 5,6,7,8,etc.

These fields are for specification of the fresh water inflows. In the case of an open end estuary field 5 contains the end discharge and 6,7,8, etc. the lateral inflows. Lateral inflows must be given in the order specified in fields 3,4,5,6, etc. of card F1. If more than four values are specified a second card can be included with fields of F10.d There will be a card similar to this F3 for each tidal cycle of calculation

4F10.

End of Input to Program.

VI. Test Case and Example

6.1 Introduction

A test case is developed in order to further document the use of the computer program and to furnish the user with a basis of comparison. The user should be able to reproduce the results of the test case by using the input decks described.

The test case is a study of the Potomac Estuary (Figure 6.1) with the geometry described by Tables 6.1, 6.2 and 6.3. (Reference 1 also discusses this case.) A step-by-step development of the input deck will be followed in accordance with Section 5.2

6.2 Input Preparation and Results (see Table 6.5 for listing)

Section 1

Cards A1 and A2 contain descriptive information and are as shown.

Card A3: (Field 1) number of sections is taken as 40, (Field 2) the total length is 603,768 ft.

Card A4: (Field 1) number of time intervals per cycle taken as 120, (Field 2) the tidal period is 44,640 seconds.

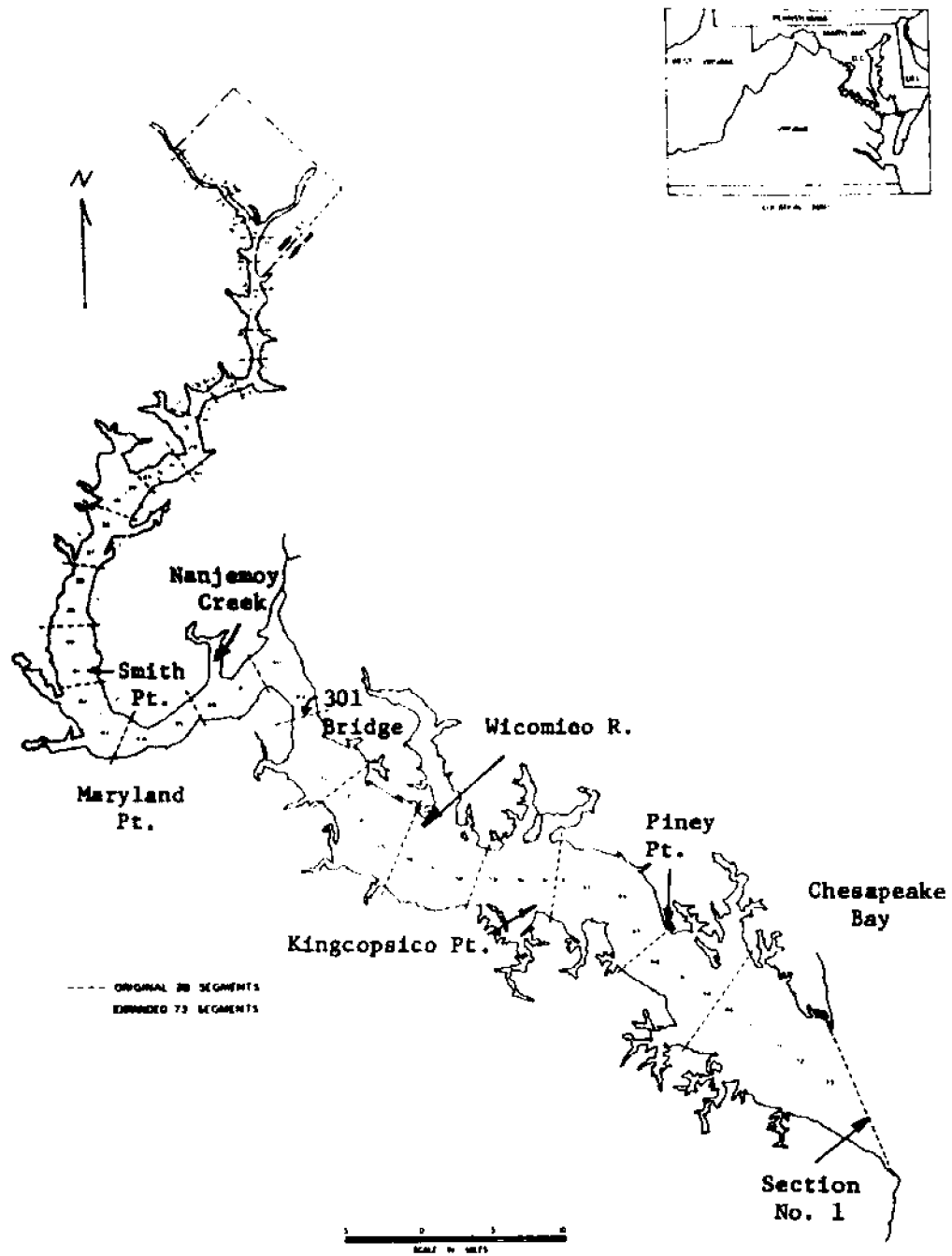
Card A5: The type of schematization for this case is that of the irregular cross-section with provisions for storage. This is type number 2. Field 2 is left blank for closed end case.

Cards A6(2), A7(2), A45(2): These cards describe the forty cross-sections and are those shown in Table 6.3. (Note that the reference datum is 40 ft. below MWL at the entrance, and that as a horizontal water surface was assumed: $d + z_o = 40$ for all stations.)

Section 2

Card B1: As Manning's n is constant for the entire estuary and no wind effects are considered this is case number 1.

Card B2: (Field 1) no sequence number is necessary as only one card is required for case 1. (Field 2) Manning's n is taken at 0.018. (Field 3) wind speed is 0. (The field can be left blank.) (Field 4) no wind angle, leave blank.



POTOMAC ESTUARY
 (from Data Source Reference 1)
 Figure 6.1

Segment Geometry of Potomac Estuary
 Excluding Embayments (Mean Water Data)
 (from Jaworski and Clark, Data Source Reference 1)

Segment Number	Length in ft (from Chain Bridge)	Average Width in ft	Average Depth in ft
1	14,890	559	24.7
2	10,665	1,302	20.0
3	9,187	2,092	10.8
4	9,504	2,677	10.5
5	8,396	2,911	13.2
6	11,404	2,708	13.2
7	13,992	3,739	12.2
8	11,300	4,227	13.2
9	13,516	3,386	20.0
10	10,085	5,695	13.2
11	13,570	4,118	18.5
12	24,129	6,086	17.0
13	15,312	8,053	15.5
14	14,732	12,368	12.1
15	22,387	8,732	20.5
16	21,859	10,799	17.9
17	22,123	16,950	13.7
18	25,291	15,475	14.2
19	28,354	8,856	20.3
20	24,816	13,186	15.3
21	27,614	10,371	22.3
22	32,103	17,406	20.7
23	33,739	24,757	18.8
24	31,152	30,397	20.2
25	28,934	20,830	18.35
26	42,135	27,043	25.0
27	31,416	26,846	33.0
28	51,163	44,342	27.4

Table 6.1

Embayment Data for Potomac Estuary (Mean Water Data)
 (from Jaworski and Clark, Data Source Reference 1)

Name	Average Depth ft	Volume ft ³ x 10 ⁸	Location (Miles below Chain Bridge)
Columbia Island Channel	6.40	0.16	4.65 - 5.76
Tidal Basin	10.40	0.46	5.81
Washington Channel	24.45	1.98	7.60 - 8.20
Anacostia River	15.45	5.56	7.60 - 8.20
Four Mile (Hunter Pt.)	12.45	0.79	8.79 - 9.70
Oxon Creek (Upper)	9.40	1.28	10.55 - 12.13
Oxon Creek (Lower)	9.35	1.71	12.13 - 13.57
Hunting Creek	3.35	0.71	12.13 - 13.50
Broad Creek	4.30	0.70	14.90 - 15.92
Piscataway Creek	4.20	1.53	18.11 - 18.63
Little Hunting Creek	3.10	0.14	19.90 - 20.33
Dogue Creek	4.05	0.72	21.85 - 22.80
Gunston Creek	5.00	3.27	24.02 - 25.42
Pomonkey Creek	3.95	0.35	26.73 - 27.10
Belmont Bay	4.80	3.33	31.45 - 34.09
Occoquan Bay	5.80	8.63	31.45 - 34.09
Powells Creek	2.80	0.54	34.79 - 35.92
Mattawoman Creek	8.80	6.56	34.13 - 35.60
Quantico Creek	2.70	0.79	38.10 - 38.55
Chicamuxen Creek	3.70	0.84	36.91 - 37.75
Chopawamsic Creek	2.67	0.36	40.75
Mallows Bay	4.65	0.21	41.64 - 42.44
Aquia Creek	4.60	4.65	46.89 - 48.40
Potomac Creek	3.58	2.76	49.20 - 49.70
Nanjemoy Creek	3.55	4.44	58.18 - 59.20
Port Tobacco River	6.75	11.06	62.00 - 63.80

Table 6.2
 (to be continued)

Name	Average Depth ft	Volume ft ³ x 10 ⁸	Location (Miles below Chain Bridge)
Upper Machodoc Creek	5.80	4.16	69.45 - 71.32
Rosier Creek	3.80	0.55	72.60 - 73.27
Cuckold Creek	2.80	0.47	72.00 - 72.21
Monroe Creek	3.80	0.70	75.90
Mattox Creek	5.80	3.60	75.98 - 77.32
Popes Creek	1.85	0.23	79.15
Wicomico River	9.92	38.62	80.52 - 82.85
St. Clement Bay	9.90	15.25	86.05 - 88.35
Breton Bay	9.90	13.40	89.36 - 90.20
Nomini Bay	6.80	8.57	87.26 - 89.48
Lower Machodoc Creek	7.85	7.27	91.15 - 93.38
Herring Creek	4.80	0.88	96.10
St. Georges Creek	5.75	4.45	102.96 - 104.35
St. Mary's River	11.75	33.51	102.96 - 104.35
Yeocomico River	6.63	9.39	103.80 - 104.65
Smith Creek	7.75	3.77	105.15 - 106.65
Coan River	6.60	6.63	107.20 - 109.00
Hull Creek	6.60	1.34	113.00

Table 6.2
(continued)

	Section no.	d feet	z ₀ feet	b _{total} feet	b _{core} feet	d' feet
Chesapeake Bay	1	30.00	10.00	59911.	57500.	6.60
	2	27.87	12.13	49325.	49325.	0.00
	3	27.65	12.35	49184.	41695.	6.60
	4	30.46	9.54	46673.	34032.	8.72
	5	32.97	7.03	48740.	29043.	10.43
	6	31.57	8.43	26016.	26016.	0.00
	7	27.63	12.37	28213.	27029.	4.80
	8	23.99	16.01	29123.	26386.	7.85
	9	20.49	19.50	33839.	22519.	9.37
	10	18.34	21.66	36988.	20844.	8.71
	11	19.10	20.90	27865.	25918.	9.90
	12	20.21	19.79	54428.	30468.	9.92
	13	19.49	20.51	30109.	28637.	9.02
	14	18.90	21.20	29914.	24840.	5.47
	15	19.45	20.55	23500.	21527.	3.34
	16	20.56	19.44	22586.	17953.	5.80
	17	21.96	18.04	13752.	13752.	0.00
	18	22.43	17.57	14927.	10458.	6.75
	19	18.15	21.35	18008.	13893.	6.75
	20	15.43	24.57	20975.	12896.	3.55
	21	19.31	20.69	9519.	9519.	0.00
	22	19.15	20.85	10146.	10146.	0.00
	23	14.79	25.21	19852.	14793.	3.60
	24	13.32	26.67	23450.	17007.	4.60
	25	14.19	25.82	16272.	16272.	0.00
	26	17.42	22.58	12503.	11419.	3.40
	27	20.45	19.55	12179.	8905.	3.22
	28	17.95	22.05	17187.	9924.	7.80
	29	12.23	27.77	23908.	11878.	5.52
	30	15.97	24.03	7456.	7456.	0.00
	31	16.94	23.06	8221.	6160.	4.76
	32	18.99	21.01	7849.	4383.	4.79
	33	13.19	26.81	6155.	5702.	3.51
	34	19.12	20.89	5751.	3398.	4.20
	35	12.20	27.80	5535.	4103.	5.92
	36	12.94	27.06	5311.	2909.	8.30
	37	12.71	27.29	6001.	2892.	12.30
	38	10.47	29.53	2577.	2150.	9.37
	39	23.03	16.97	1005.	1005.	0.00
	Chain Bridge	40	20.00	20.00	110.	110.

Potomac Estuary
Schematized Geometry after Including Embayments

Table 6.3

Section 3

Card C1: The type of water surface variation at the entrance will be that typically defined by tide tables, a HW and LW series. This is case 1.

Card C2: The distance that MWL is above MSL at the entrance is assumed to be 0.0 ft.

Card C3(1): The initial HW value with respect to MWL at the entrance is 0.52 ft. (from Table 6.4).

Section 4

Card D1: (Field 1) The initial value of the dispersion parameter K is taken as $600 \text{ ft}^2/\text{sec}$. (Field 2) To use the dispersion relation $E = K \left| \frac{\partial \eta}{\partial x} \right| + 3E_T$ the multiplying factor $n = 3$ must be punched in column 15.

Card D2: (Field 1) The $K/u_o L$ vs. IE_D relations will be used with the standard intercept of 0.00215, therefore the field is left blank.

(Field 2) It is assumed that reliable information exists which permits the specification of a limit for the salinity intrusion (and thus calculations) at a point 540,000 ft. above the entrance. (Without such justification the field would be left blank.)

(Field 3) The conditions at the entrance to the Potomac have been assumed to be those of a longshore current and thus the salinity is assumed to return to its maximum value in 0.05 of a tidal cycle after LWS. (Field 4) The salinity is not restricted to remain at its maximum value after HWS, therefore the field is either left blank or set to 0.

Card D3: Sequential datasets will not be used in this test case so as to accommodate those users without such facilities. Therefore the initial conditions will be all from cards - case 3.

Cards D4(3), D5(3), etc.: These are the initial conditions. Note that they are specified every other station due to the staggered mesh used in the finite difference calculations. Elevations and salinities are defined on the odd numbered sections starting at the entrance, whereas discharge is defined on the even numbered stations.

Maximum and Minimum Tidal Elevations
at the Entrance to the Potomac
(Relative to MWL at the Entrance)

Table 6.4

Tidal Period	LW	HW	Tidal Period	LW	HW
0		2.20	16	-2.00	2.80
1	-1.30	2.80	17	-2.60	2.90
2	-1.40	2.60	18	-1.80	2.60
3	-1.30	2.10	19	-2.30	2.00
4	-1.70	2.70	20	-1.70	2.60
5	-1.70	2.10	21	-2.20	2.10
6	-1.70	3.00	22	-1.50	2.60
7	-1.80	2.50	23	-2.20	2.30
8	-1.80	3.00	24	-1.60	2.50
9	-2.20	2.10	25	-2.10	2.50
10	-2.20	3.10	26	-1.80	2.30
11	-2.40	2.00	27	-2.40	2.70
12	-2.20	3.20	28	-2.70	2.10
13	-2.50	2.20	29	-2.50	3.00
14	-2.00	3.20	30	-2.20	2.30
15	-2.50	2.00	31	-2.60	2.10

For 40 stations this means 20 values, 5 per card for each variable or four cards per variable.

Section 5

Card E1: The output will not be put on a sequential data set so option 1 will not be taken. High water slack salinities will be requested, therefore the letters HWS will be punched in columns 1-3. As continuation after this run might be of interest, the third option of punching the final values will be taken. The letters FC are thus punched in columns 6-7. It is not desired to place the time and spatially varying dispersion coefficients on a sequential data set so option 4 will not be taken.

Card E2, E3, E4: For 120 time intervals per tidal cycle and a possible output every other interval due to the staggered finite difference mesh one could obtain 60 outputs per tidal cycle. 10 each cycle are to be specified which requires an output interval of 12. In this case all three variables will be assigned the same frequency. E2, E3 and E4 will contain the letters H, S, and Q in column one and 12 in columns 9 and 10.

Section 6

Card F1: (Field 1) Only one transient input is assumed. (Field 2) Again one input is involved. (Field 3) The input is at the head of the estuary and so it is identified as entering between section 39 and 40 by designating section 40.

Card F2: Only two tidal cycles are specified as this is a test case.

Cards F3, F4: (Field 1) cycle no. (Field 2,3) The LW, HW from Table 6.4. (Field 4) The maximum salinity at the entrance which is taken to be 16.5 ppt for both cycles. (Field 5) The fresh water discharge for these cycles which are 2730 and 2626 cfs.

The resulting input listing for this test case is in Table 6.5 and the output for two tidal cycle's calculation is in Table 6.6.

Output Listing from Test Case

Table 6.6

 * N.I.T. SALINITY INTRUSION PROGRAM, CW-3-DIMENSIONAL SCHEMATIZATION *

SECTION 1: DESCRIPTION OF THE SCHEMATIZED ESTUARY

***** TEST CASE *****
 IDENTIFICATION OF INPUT-OUTPUT RELATIONSHIPS
 GEOMETRIC DATA *** CLOSURE ESTUARY ***

NUMBER OF STATIONS = 40	ESTUARY LENGTH = 60376.0 FT.	PERIOD = 4444.0 SECONDS					
NUMBER OF TIME INCREMENTS PER PERIOD = 120	CASE 2: IRREGULAR CROSS-SECTION (STORAGE)						
MILES	C	D	B	BIC(MEI)	D*	SECT	SECTION
0.0	30.0000	10.0000	5811.4	97499.5	0.0	0.	0.
2.93205	27.1700	12.1300	4924.7	49324.7	0.0	1461.	2
5.86410	24.6400	12.3500	4813.0	41494.5	0.0	3062.	3
8.79615	22.9400	9.5000	4672.0	34032.1	0.0	4646.	4
11.72820	21.5600	7.0000	4273.0	28422.5	10.0	6192.	5
14.66025	20.5800	6.4300	2821.5	26015.6	0.0	7406.	6
17.59230	19.6000	17.3000	2821.5	27620.7	0.0	7406.	7
20.52434	18.6200	18.0000	2912.4	22388.3	7.0	10836.	8
23.45639	17.6400	19.5000	5381.9	22310.9	0.0	12836.	9
26.38844	16.6600	21.6500	3658.4	20844.4	0.0	18431.	10
29.32050	15.6800	21.6500	2786.4	25518.1	0.0	17038.	11
32.25255	14.7000	24.2600	5442.6	30667.3	0.0	18431.	12
35.18460	13.7200	27.0000	3010.9	26676.8	0.0	18431.	13
38.11665	12.7400	29.0000	2900.1	24840.1	0.0	20125.	14
41.04870	11.7600	31.0000	2350.1	21827.0	0.0	21672.	15
43.98074	10.7800	33.0000	2284.8	17932.6	0.0	23221.	16
46.91279	9.8000	35.0000	13751.9	13751.9	0.0	24770.	17
49.84485	8.8200	37.0000	14926.6	10457.8	0.0	26318.	18
52.77690	7.8400	39.0000	1808.0	11892.8	0.0	27866.	19
55.70895	6.8600	41.0000	20979.3	12946.3	0.0	29414.	20
58.64100	5.8800	43.0000	5115.3	9375.7	0.0	30962.	21
61.57305	4.9000	45.0000	10146.0	10146.0	0.0	32510.	22
64.50510	3.9200	47.0000	14952.1	14952.1	0.0	34058.	23
67.43714	2.9400	49.0000	23450.0	14952.1	0.0	34058.	24
70.36919	1.9600	51.0000	16271.5	17078.5	0.0	37156.	25
73.30124	0.9800	53.0000	12702.5	16413.4	0.0	38703.	26
76.23329	0.0000	55.0000	12178.4	16004.4	0.0	40251.	27
79.16534	0.0000	57.0000	17187.3	16004.4	0.0	41799.	28
82.09739	0.0000	59.0000	23908.2	17973.5	0.0	43347.	29
85.02944	0.0000	61.0000	7455.6	16451.8	0.0	44895.	30
87.96149	0.0000	63.0000	7455.6	16451.8	0.0	46443.	31
90.89354	0.0000	65.0000	7455.6	16451.8	0.0	47991.	32
93.82559	0.0000	67.0000	4158.0	1702.0	0.0	49539.	33
96.75764	0.0000	69.0000	5150.8	3957.7	0.0	51087.	34
99.68969	0.0000	71.0000	5426.7	4103.4	0.0	52635.	35
102.62174	0.0000	73.0000	5311.2	2902.8	0.0	54183.	36
105.55379	0.0000	75.0000	6601.0	2491.6	0.0	55731.	37
108.48584	0.0000	77.0000	2871.3	2149.6	0.0	57279.	38
111.41789	0.0000	79.0000	1094.2	1605.2	0.0	58827.	39
114.34994	0.0000	81.0000	110.1	110.1	0.0	60376.	40

NOTE: D* DEEPER THAN D FOR SECTION 17

Table 6.6 (cont'd)

SECTION 2 CHANNEL ROUGHNESS AND WIND EFFECTS
CASE 1 CONSTANT ROUGHNESS AND CONSTANT WIND

NO	WIND	MILES	MANCC	FO	V	PMI	FEET	SECTION
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	1
2.53205	0.0180000	0.0	0.0	0.0	0.0	15481.0	15481.0	2
5.86610	0.0180000	0.0	0.0	0.0	0.0	30962.0	30962.0	3
8.79615	0.0180000	0.0	0.0	0.0	0.0	46443.0	46443.0	4
11.72820	0.0180000	0.0	0.0	0.0	0.0	61924.0	61924.0	5
14.66025	0.0180000	0.0	0.0	0.0	0.0	77405.0	77405.0	6
17.59230	0.0180000	0.0	0.0	0.0	0.0	92886.0	92886.0	7
20.52434	0.0180000	0.0	0.0	0.0	0.0	108367.0	108367.0	8
23.45639	0.0180000	0.0	0.0	0.0	0.0	123848.0	123848.0	9
26.38844	0.0180000	0.0	0.0	0.0	0.0	139329.0	139329.0	10
29.32050	0.0180000	0.0	0.0	0.0	0.0	154810.0	154810.0	11
32.25255	0.0180000	0.0	0.0	0.0	0.0	170291.0	170291.0	12
35.18460	0.0180000	0.0	0.0	0.0	0.0	185772.0	185772.0	13
38.11665	0.0180000	0.0	0.0	0.0	0.0	201253.0	201253.0	14
41.04870	0.0180000	0.0	0.0	0.0	0.0	216734.0	216734.0	15
43.98075	0.0180000	0.0	0.0	0.0	0.0	232215.0	232215.0	16
46.91280	0.0180000	0.0	0.0	0.0	0.0	247696.0	247696.0	17
49.84485	0.0180000	0.0	0.0	0.0	0.0	263177.0	263177.0	18
52.77690	0.0180000	0.0	0.0	0.0	0.0	278658.0	278658.0	19
55.70895	0.0180000	0.0	0.0	0.0	0.0	294139.0	294139.0	20
58.64100	0.0180000	0.0	0.0	0.0	0.0	309620.0	309620.0	21
61.57305	0.0180000	0.0	0.0	0.0	0.0	325101.0	325101.0	22
64.50510	0.0180000	0.0	0.0	0.0	0.0	340582.0	340582.0	23
67.43715	0.0180000	0.0	0.0	0.0	0.0	356063.0	356063.0	24
70.36920	0.0180000	0.0	0.0	0.0	0.0	371544.0	371544.0	25
73.30125	0.0180000	0.0	0.0	0.0	0.0	387025.0	387025.0	26
76.23330	0.0180000	0.0	0.0	0.0	0.0	402506.0	402506.0	27
79.16535	0.0180000	0.0	0.0	0.0	0.0	417987.0	417987.0	28
82.09740	0.0180000	0.0	0.0	0.0	0.0	433468.0	433468.0	29
85.02945	0.0180000	0.0	0.0	0.0	0.0	448949.0	448949.0	30
87.96150	0.0180000	0.0	0.0	0.0	0.0	464430.0	464430.0	31
90.89355	0.0180000	0.0	0.0	0.0	0.0	479911.0	479911.0	32
93.82560	0.0180000	0.0	0.0	0.0	0.0	495392.0	495392.0	33
96.75765	0.0180000	0.0	0.0	0.0	0.0	510873.0	510873.0	34
99.68970	0.0180000	0.0	0.0	0.0	0.0	526354.0	526354.0	35
102.62175	0.0180000	0.0	0.0	0.0	0.0	541835.0	541835.0	36
105.55380	0.0180000	0.0	0.0	0.0	0.0	557316.0	557316.0	37
108.48585	0.0180000	0.0	0.0	0.0	0.0	572797.0	572797.0	38
111.41790	0.0180000	0.0	0.0	0.0	0.0	588278.0	588278.0	39
114.35000	0.0180000	0.0	0.0	0.0	0.0	603759.0	603759.0	40

SECTION 3 SURFACE ELEVATION AT THE ENTRANCE

MWL 0.0 FT ABOVE MSL AT OCEAN
MSL 40.00 FT ABOVE THE HORIZONTAL TATHM
CASE 1, COSINE FIT TO MWL SERIES
FIRST MW VALUE RELATIVE TO MWL = C.52

DT = 372.70 SEC, DX = 15481.2 FT,
DELTA X / DELTA T = 41.62 FT./SEC.

SECTION	CELERITY (FT/SEC)	WITH RESPECT TO DEPTH ONLY	WAVE LENGTH (MILES)
1	21.08		262.77
2	29.56		253.27
3	25.64		252.27
4	31.22		266.78
5	32.58		275.47
6	31.88		269.56
7	29.93		252.18
8	27.75		234.98
9	25.65		217.16
10	24.30		205.46
11	24.80		209.67
12	25.61		215.68
13	25.05		211.80
14	24.60		208.02
15	25.03		211.59
16	25.73		217.54
17	26.55		224.82
18	26.87		227.21
19	24.17		204.39
20	22.25		188.45
21	24.94		210.82
22	24.83		209.94
23	21.82		184.50
24	20.72		175.16
25	21.37		180.66
26	23.68		200.24
27	25.66		216.95
28	24.04		203.26
29	19.84		167.78
30	22.68		191.72
31	23.36		197.46
32	24.73		239.06
33	20.61		174.24
34	24.81		209.78
35	19.82		167.57
36	20.41		172.58
37	20.23		171.04
38	18.36		155.24
39	27.23		230.23
40	25.38		214.55

Table 6.6 (cont'd)

SECTION 4, DEFINITIONS RELATIVE TO THE SALT BALANCE EQUATION, AND TO INITIAL SALINITIES

E=K(DS/DX)+3*(E/TAYLOR) K FOR THE NEXT TIDAL CYCLE IS 600.00

K VS. ED GIVEN BY K/(UZ*LI)=0.215E-02ED**(-1/4)

SALINITY EXCURSION LIMIT = 540000.00 FT

SALINITY RETURNS TO ZERO AFTER LWS IN 0.05 OF A TIDAL PERIOD
EXTENSION OF ZERO AFTER HWS FOR 0.0 OF A TICAL PERIOD

CASE 3, INITIAL CONDITIONS ALL FROM CARDS

TIDAL CYCLE	STATIONS	ELEV.	STATIONS	ELEV.	DAY	HOURS	DAY	HOURS	HOURS	INSTANTANEOUS SURFACE ELEVATION WITH RESPECT TO MSL, FT
1	3	0.520	5	0.562	7	5	9	0.0	11	13
	29	0.443	31	-0.511	35	33	37	0.658	39	0.603
										0.559
										0.499
										0.381
										0.292
										-0.070
										-0.192
										-0.325

TIDAL CYCLE	STATIONS	SALINITY	STATIONS	SALINITY	DAY	HOURS	DAY	HOURS	HOURS	INSTANTANEOUS SALINITIES IN PPT
1	3	16.890	5	16.460	7	5	9	0.0	11	13
	29	0.0	31	0.0	35	33	37	13.500	39	11.700
										11.100
										10.900
										7.780
										6.385
										1.527
										0.250

TIDAL CYCLE	STATIONS	DISCHARGE	STATIONS	DISCHARGE	DAY	HOURS	DAY	HOURS	HOURS	INSTANTANEOUS DISCHARGES IN CFS
1	2	338446.	4	337211.	6	4	8	0.103	10	12
	22	68725.	24	19291.	26	24	28	296609.	30	223690.
										174974.
										133874.
										109044.
										-12941.
										-24682.
										-4572.
										20
										1A
										109044.
										42771.
										1A
										40
										0.

SECTION 5, OUTPUT SELECTION

OPTION HWS SPECIFIED

OPTION FC SPECIFIED

BASIC OUTPUT	FREQUENCY
H	12
S	12
Q	12

CALCULATION RECIENS

TRANSIENT FRESH WATER INPUTS AT SEGMENTS 40
 TRANSIENT MIN-MAX OCEAN ELEVATIONS MET MM

CALCULATION FOR 2 CYCLES

CYCLE	1	MPIN	-0.47	MMAX	C-EP	SIFRC	14.57	QF5	AND STATIONS	ARE	2790-000	47			
TIDAL CYCLE	1	1,240	HOURS	DAY	0	1,240	HOURS	13	15	17	19	21	23	25	27
STATIONS	1	0.425	0.541	0.604	0.679	0.719	0.731	0.702	0.689	0.681	0.693	0.614	0.514	-7.379	
ELEV.	2	-0.221	-0.390	-0.546	-0.653	-0.745	-0.790								
ELEV.	3	0.482	0.541	0.604	0.679	0.719	0.731	0.702	0.689	0.681	0.693	0.614	0.514	-7.379	
TIDAL CYCLE	1	1,240	HOURS	DAY	0	1,240	HOURS	13	15	17	19	21	23	25	27
STATIONS	1	16.570	16.591	16.204	15.131	13.600	12.490	11.742	11.134	10.567	9.071	5.941	2.930	1.498	7.279
SALINITY	2	-0.022	0.001	-0.000	0.000	0.000	-0.000								
SALINITY	3	1.343	1.343	1.343	1.343	1.343	1.343	1.343	1.343	1.343	1.343	1.343	1.343	1.343	1.343
TIDAL CYCLE	1	1,343	HOURS	DAY	0	1,343	HOURS	12	14	16	18	20	22	24	26
STATIONS	1	13.220	13.220	13.220	13.220	13.220	13.220	13.220	13.220	13.220	13.220	13.220	13.220	13.220	13.220
DISCHARGE	2	16.790	16.790	16.790	16.790	16.790	16.790	16.790	16.790	16.790	16.790	16.790	16.790	16.790	16.790
DISCHARGE	3	1.603	1.603	1.603	1.603	1.603	1.603	1.603	1.603	1.603	1.603	1.603	1.603	1.603	1.603
TIDAL CYCLE	1	2,480	HOURS	DAY	0	2,480	HOURS	13	15	17	19	21	23	25	27
STATIONS	1	0.178	0.229	0.281	0.340	0.424	0.524	0.614	0.684	0.734	0.764	0.774	0.764	0.734	0.684
SALINITY	2	0.214	0.214	0.214	0.214	0.214	0.214	0.214	0.214	0.214	0.214	0.214	0.214	0.214	0.214
SALINITY	3	0.214	0.214	0.214	0.214	0.214	0.214	0.214	0.214	0.214	0.214	0.214	0.214	0.214	0.214
TIDAL CYCLE	1	2,480	HOURS	DAY	0	2,480	HOURS	13	15	17	19	21	23	25	27
STATIONS	1	16.577	16.603	16.179	15.339	13.610	12.522	11.750	11.177	10.683	9.441	5.904	3.797	1.490	0.375
SALINITY	2	-0.002	0.001	-0.000	0.000	0.000	-0.000								
SALINITY	3	1.343	1.343	1.343	1.343	1.343	1.343	1.343	1.343	1.343	1.343	1.343	1.343	1.343	1.343
TIDAL CYCLE	1	2,583	HOURS	DAY	0	2,583	HOURS	12	14	16	18	20	22	24	26
STATIONS	1	11.976	12.027	11.976	11.976	11.976	11.976	11.976	11.976	11.976	11.976	11.976	11.976	11.976	11.976
DISCHARGE	2	16.790	16.790	16.790	16.790	16.790	16.790	16.790	16.790	16.790	16.790	16.790	16.790	16.790	16.790
DISCHARGE	3	1.603	1.603	1.603	1.603	1.603	1.603	1.603	1.603	1.603	1.603	1.603	1.603	1.603	1.603
TIDAL CYCLE	1	3,720	HOURS	DAY	0	3,720	HOURS	13	15	17	19	21	23	25	27
STATIONS	1	-0.128	-0.104	-0.065	-0.012	0.076	0.180	0.236	0.307	0.366	0.450	0.493	0.547	0.610	7.421
SALINITY	2	0.503	0.455	0.406	0.381	0.293	0.218	0.161	0.114	0.076	0.046	0.024	0.012	0.006	0.003
SALINITY	3	1.343	1.343	1.343	1.343	1.343	1.343	1.343	1.343	1.343	1.343	1.343	1.343	1.343	1.343
TIDAL CYCLE	1	3,720	HOURS	DAY	0	3,720	HOURS	13	15	17	19	21	23	25	27
STATIONS	1	16.569	16.613	15.406	13.498	12.604	11.751	11.194	10.792	9.890	6.320	4.014	2.414	0.440	
SALINITY	2	0.004	-0.002	0.001	-0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
SALINITY	3	1.343	1.343	1.343	1.343	1.343	1.343	1.343	1.343	1.343	1.343	1.343	1.343	1.343	1.343

Table 6.6 (cont'd)

TIDAL CYCLE	1	3-823 HOURS	DAY 0	3-823 HOURS	INSTANTANEOUS DISCHARGES IN CFS	
STATIONS	2	-86704.	-280223.	-127071.	-127071.	18
DISCHARGE	24	26	26	26	31486.	131700.
STATIONS	22	167014.	180940.	101240.	82198.	43952.
DISCHARGE	170326.	167014.	180940.	101240.	82198.	43952.
TIDAL CYCLE	1	4-960 HOURS	DAY 0	4-960 HOURS	INSTANTANEOUS SURFACE ELEVATION WITH REFLECT TO WSL, FT	
STATIONS	1	3	3	3	15	10
DISCHARGE	-0.375	-0.372	-0.356	-0.280	-0.191	-0.002
ELEV.	29	31	33	37	39	21
STATIONS	29	31	33	37	39	21
ELEV.	0.764	0.745	0.833	1.038	1.060	0.154
TIDAL CYCLE	1	4-960 HOURS	DAY 0	4-960 HOURS	INSTANTANEOUS SALINITIES IN PPT	
STATIONS	1	3	3	3	13	10
SALINITY	16.954	16.607	16.054	15.370	13.422	11.715
STATIONS	24	31	33	35	37	21
SALINITY	0.014	-0.008	0.002	-0.001	0.000	0.000
TIDAL CYCLE	1	5-083 HOURS	DAY 0	5-083 HOURS	INSTANTANEOUS DISCHARGES IN CFS	
STATIONS	2	-514154.	-446900.	-303169.	-164031.	-88920.
DISCHARGE	22	74	76	28	34	34
STATIONS	5303.	78670.	89649.	87502.	53417.	36310.
DISCHARGE	1	3	3	3	3	3
STATIONS	1	3	3	3	3	3
ELEV.	-0.470	-0.507	-0.519	-0.549	-0.528	-0.459
ELEV.	20	31	33	35	37	39
STATIONS	20	31	33	35	37	39
ELEV.	0.743	0.908	1.073	1.194	1.304	1.337
TIDAL CYCLE	1	6-200 HOURS	DAY 0	6-200 HOURS	INSTANTANEOUS SALINITIES IN PPT	
STATIONS	1	3	3	3	13	10
SALINITY	16.957	16.599	16.002	14.200	13.303	12.492
STATIONS	24	31	33	35	37	21
SALINITY	0.014	-0.009	0.003	-0.001	0.000	0.000
TIDAL CYCLE	1	6-303 HOURS	DAY 0	6-303 HOURS	INSTANTANEOUS DISCHARGES IN CFS	
STATIONS	2	-274523.	-324067.	-311523.	-293468.	-269138.
DISCHARGE	24	24	24	24	24	24
STATIONS	22	148319.	148319.	148319.	148319.	148319.
DISCHARGE	1	3	3	3	3	3
STATIONS	1	3	3	3	3	3
ELEV.	-0.341	-0.400	-0.463	-0.469	-0.540	-0.552
ELEV.	29	31	33	35	37	39
STATIONS	29	31	33	35	37	39
ELEV.	0.857	0.647	0.816	0.984	1.129	1.172
TIDAL CYCLE	1	7-440 HOURS	DAY 0	7-440 HOURS	INSTANTANEOUS SURFACE ELEVATION WITH REFLECT TO WSL, FT	
STATIONS	1	3	3	3	13	10
SALINITY	16.953	16.588	15.995	15.100	13.240	12.459
STATIONS	24	31	33	35	37	21
SALINITY	0.012	-0.006	0.002	-0.000	0.000	0.000
TIDAL CYCLE	1	7-443 HOURS	DAY 0	7-443 HOURS	INSTANTANEOUS DISCHARGES IN CFS	
STATIONS	2	-187442.	-187442.	-187442.	-187442.	-187442.
DISCHARGE	24	24	24	24	24	24
STATIONS	22	148319.	148319.	148319.	148319.	148319.
DISCHARGE	1	3	3	3	3	3
STATIONS	1	3	3	3	3	3
ELEV.	-0.341	-0.400	-0.463	-0.469	-0.540	-0.552
ELEV.	29	31	33	35	37	39
STATIONS	29	31	33	35	37	39
ELEV.	0.857	0.647	0.816	0.984	1.129	1.172
TIDAL CYCLE	1	8-080 HOURS	DAY 0	8-080 HOURS	INSTANTANEOUS SURFACE ELEVATION WITH REFLECT TO WSL, FT	
STATIONS	1	3	3	3	13	10
SALINITY	16.954	16.599	16.002	14.200	13.303	12.492
STATIONS	24	31	33	35	37	21
SALINITY	0.012	-0.007	0.002	-0.000	0.000	0.000
TIDAL CYCLE	1	8-080 HOURS	DAY 0	8-080 HOURS	INSTANTANEOUS DISCHARGES IN CFS	
STATIONS	2	-187442.	-187442.	-187442.	-187442.	-187442.
DISCHARGE	24	24	24	24	24	24
STATIONS	22	148319.	148319.	148319.	148319.	148319.
DISCHARGE	1	3	3	3	3	3
STATIONS	1	3	3	3	3	3
ELEV.	-0.341	-0.400	-0.463	-0.469	-0.540	-0.552
ELEV.	29	31	33	35	37	39
STATIONS	29	31	33	35	37	39
ELEV.	0.857	0.647	0.816	0.984	1.129	1.172

Table 6.6 (cont'd)

TIDAL CYCLE 1		6.000 HOURS		DAY 0		9.680 HOURS		INSTANTANEOUS SALINITIES IN PPT		23		25		27	
STATIONS	1	3	5	7	9	11	13	15	17	19	21	23	25	27	29
SALINITY	16.970	16.973	16.047	15.082	13.278	12.432	11.599	11.143	10.810	9.179	8.068	7.916	1.099	5.688	
STATIONS	29	31	33	35	37										
SALINITY	0.002	-0.001	0.000	-0.000	-0.000	-0.000									
TIDAL CYCLE 1		8.783 HOURS		DAY 0		9.703 HOURS		INSTANTANEOUS DISCHARGES IN CFS		37		40		42	
STATIONS	2	4	6	8	10	12	14	16	18	20	22	24	26	28	30
DISCHARGE	412949.	272904.	122496.	28816.	-58183.	-124955.	-186820.	-239472.	-226146.	-239489.					
STATIONS	22	24	26	28	30	32	34	36	38	40					
DISCHARGE	-222442.	-196806.	-170161.	-148994.	-84909.	-77607.	-54899.	-46799.							
TIDAL CYCLE 1		9.920 HOURS		DAY 0		9.920 HOURS		INSTANTANEOUS SURFACE ELEVATION WITH RESPECT TO WSL, FT		23		25		27	
STATIONS	1	3	5	7	9	11	13	15	17	19	21	23	25	27	29
ELEV.	0.414	0.373	0.324	0.262	0.177	0.049	-1.028	-0.124	-0.212	-0.299	-0.363	-0.313	-0.128		
STATIONS	26	31	33	35	37										
ELEV.	-0.115	-0.108	-0.104	-0.111	-0.096										
TIDAL CYCLE 1		9.920 HOURS		DAY 0		9.920 HOURS		INSTANTANEOUS SALINITIES IN PPT		31		33		35	
STATIONS	1	3	5	7	9	11	13	15	17	19	21	23	25	27	29
SALINITY	16.970	16.959	16.129	15.036	13.407	12.372	11.617	11.124	10.787	9.468	8.068	7.916	1.099	5.688	
STATIONS	29	31	33	35	37										
SALINITY	-0.004	0.002	-0.001	0.000	-0.000	-0.000									
TIDAL CYCLE 1		10.023 HOURS		DAY 0		10.023 HOURS		INSTANTANEOUS DISCHARGES IN CFS		37		40		42	
STATIONS	2	4	6	8	10	12	14	16	18	20	22	24	26	28	30
DISCHARGE	684684.	548452.	309373.	157222.	157222.	66963.	-11872.	-103499.	-167819.	-276069.					
STATIONS	22	24	26	28	30	32	34	36	38	40					
DISCHARGE	-178079.	-186050.	-166276.	-123882.	-76684.	-58689.	-43186.	-27784.							
TIDAL CYCLE 1		11.160 HOURS		DAY 0		11.160 HOURS		INSTANTANEOUS SURFACE ELEVATION WITH RESPECT TO WSL, FT		23		25		27	
STATIONS	1	3	5	7	9	11	13	15	17	19	21	23	25	27	29
ELEV.	0.791	0.746	0.710	0.612	0.467	0.342	0.409	0.423	0.321	0.147	-0.064	-0.275	-0.204	-0.198	
STATIONS	29	31	33	35	37	39									
ELEV.	-0.375	-0.401	-0.423	-0.450	-0.487	-0.448									
TIDAL CYCLE 1		11.160 HOURS		DAY 0		11.160 HOURS		INSTANTANEOUS SALINITIES IN PPT		31		33		35	
STATIONS	1	3	5	7	9	11	13	15	17	19	21	23	25	27	29
SALINITY	16.970	16.941	16.226	15.068	13.608	12.372	11.688	11.140	10.768	9.419	8.019	7.916	1.099	5.688	
STATIONS	29	31	33	35	37										
SALINITY	-0.007	0.004	-0.002	0.001	-0.001	-0.000									
TIDAL CYCLE 1		11.243 HOURS		DAY 0		11.243 HOURS		INSTANTANEOUS DISCHARGES IN CFS		37		40		42	
STATIONS	2	4	6	8	10	12	14	16	18	20	22	24	26	28	30
DISCHARGE	768371.	659640.	493001.	474864.	1110070.	288221.	180847.	100477.	48942.	288490.					
STATIONS	22	24	26	28	30	32	34	36	38	40					
DISCHARGE	-35079.	-27037.	-90490.	-42973.	-40423.	-37444.	-28189.	-18421.							
TIDAL CYCLE 1		12.400 HOURS		DAY 0		12.400 HOURS		INSTANTANEOUS SURFACE ELEVATION WITH RESPECT TO WSL, FT		23		25		27	
STATIONS	1	3	5	7	9	11	13	15	17	19	21	23	25	27	29
ELEV.	0.880	0.828	0.877	1.012	1.022	0.846	0.920	0.867	0.772	0.609	0.484	0.373	-0.047	-0.160	
STATIONS	29	31	33	35	37	39									
ELEV.	-0.453	-0.547	-0.617	-0.687	-0.674	-0.671									
TIDAL CYCLE 1		12.400 HOURS		DAY 0		12.400 HOURS		INSTANTANEOUS SALINITIES IN PPT		31		33		35	
STATIONS	1	3	5	7	9	11	13	15	17	19	21	23	25	27	29
SALINITY	16.970	16.933	16.306	15.148	13.815	12.441	11.727	11.205	10.797	9.397	8.128	8.019	1.099	5.688	
STATIONS	29	31	33	35	37										
SALINITY	-0.004	0.005	-0.003	0.001	-0.001	-0.000									
TIDAL CYCLE 1		12.503 HOURS		DAY 0		12.503 HOURS		INSTANTANEOUS DISCHARGES IN CFS		37		40		42	
STATIONS	2	4	6	8	10	12	14	16	18	20	22	24	26	28	30
DISCHARGE	80796.	518950.	492437.	470213.	424119.	471916.	304817.	292924.	29182.	189227.					
STATIONS	22	24	26	28	30	32	34	36	38	40					
DISCHARGE	184841.	108056.	51829.	-13182.	-13120.	-13538.	-8712.	-5864.							
															Avg Mass = 0.103319 13

Table 6.6 (cont'd)

TIDAL CYCLE	1	0.0	HOURS	DAY	0	C.C.	HOURS	13	15	17	19	21	23	25	27
STATIONS	1	240	1.053	2.067	2.480	3	4	11	13	15	17	19	21	23	25
TIME HRS	1.240	1.053	2.067	2.480	2.893	3	4	11	13	15	17	19	21	23	25
ELEV.	16.570	16.793	16.189	15.389	13.586	12.593	11.743	11.181	10.843	9.748	8.854	8.271	7.907	6.997	6.081
STATIONS	29	31	33	35	37	39	41	43	45	47	49	51	53	55	57
TIME HRS	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200
SALINITY	0.019	-0.080	0.003	-0.001	C.000										
PRISM	0.103722E 11	CMP7													
DENSIMETRIC BRIDGE NO. #	0.186409														
DENSIMETRIC ESTUARY NO. #	2.701														
MAXIMUM DISCHARGE	0.9481	FT/SEC	AT	10.93	HRS	FROM	BEGINNING	OF	CYCLE						
SUM OF DISCHARGE	2730.000	CUM/FT/SEC													
K FOR THE NEXT CYCLE IS	578.67	SOFT/SEC													

CYCLE	Z	PP1M	-0.52	HMAX	0.42	SZERD	16.56	DP'S	AND	STATIONS	MP	2046.000	OF		
TIDAL CYCLE	2	1.240	HOURS	DAY	0	13.640	HOURS	13	15	17	19	21	23	25	27
STATIONS	1	240	1.053	2.067	2.480	2.893	3	4	11	13	15	17	19	21	23
TIME HRS	1.240	1.053	2.067	2.480	2.893	3	4	11	13	15	17	19	21	23	25
ELEV.	16.570	16.793	16.189	15.389	13.586	12.593	11.743	11.181	10.843	9.748	8.854	8.271	7.907	6.997	6.081
STATIONS	29	31	33	35	37	39	41	43	45	47	49	51	53	55	57
TIME HRS	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200
SALINITY	0.019	-0.080	0.003	-0.001	C.000										
PRISM	0.103722E 11	CMP7													
DENSIMETRIC BRIDGE NO. #	0.186409														
DENSIMETRIC ESTUARY NO. #	2.701														
MAXIMUM DISCHARGE	0.9481	FT/SEC	AT	10.93	HRS	FROM	BEGINNING	OF	CYCLE						
SUM OF DISCHARGE	2730.000	CUM/FT/SEC													
K FOR THE NEXT CYCLE IS	578.67	SOFT/SEC													

TIDAL CYCLE	2	1.240	HOURS	DAY	0	13.640	HOURS	13	15	17	19	21	23	25	27
STATIONS	1	240	1.053	2.067	2.480	2.893	3	4	11	13	15	17	19	21	23
TIME HRS	1.240	1.053	2.067	2.480	2.893	3	4	11	13	15	17	19	21	23	25
ELEV.	16.570	16.793	16.189	15.389	13.586	12.593	11.743	11.181	10.843	9.748	8.854	8.271	7.907	6.997	6.081
STATIONS	29	31	33	35	37	39	41	43	45	47	49	51	53	55	57
TIME HRS	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200
SALINITY	0.019	-0.080	0.003	-0.001	C.000										
PRISM	0.103722E 11	CMP7													
DENSIMETRIC BRIDGE NO. #	0.186409														
DENSIMETRIC ESTUARY NO. #	2.701														
MAXIMUM DISCHARGE	0.9481	FT/SEC	AT	10.93	HRS	FROM	BEGINNING	OF	CYCLE						
SUM OF DISCHARGE	2730.000	CUM/FT/SEC													
K FOR THE NEXT CYCLE IS	578.67	SOFT/SEC													

TIDAL CYCLE	2	1.240	HOURS	DAY	0	13.640	HOURS	13	15	17	19	21	23	25	27
STATIONS	1	240	1.053	2.067	2.480	2.893	3	4	11	13	15	17	19	21	23
TIME HRS	1.240	1.053	2.067	2.480	2.893	3	4	11	13	15	17	19	21	23	25
ELEV.	16.570	16.793	16.189	15.389	13.586	12.593	11.743	11.181	10.843	9.748	8.854	8.271	7.907	6.997	6.081
STATIONS	29	31	33	35	37	39	41	43	45	47	49	51	53	55	57
TIME HRS	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200
SALINITY	0.019	-0.080	0.003	-0.001	C.000										
PRISM	0.103722E 11	CMP7													
DENSIMETRIC BRIDGE NO. #	0.186409														
DENSIMETRIC ESTUARY NO. #	2.701														
MAXIMUM DISCHARGE	0.9481	FT/SEC	AT	10.93	HRS	FROM	BEGINNING	OF	CYCLE						
SUM OF DISCHARGE	2730.000	CUM/FT/SEC													
K FOR THE NEXT CYCLE IS	578.67	SOFT/SEC													

TIDAL CYCLE	2	1.240	HOURS	DAY	0	13.640	HOURS	13	15	17	19	21	23	25	27
STATIONS	1	240	1.053	2.067	2.480	2.893	3	4	11	13	15	17	19	21	23
TIME HRS	1.240	1.053	2.067	2.480	2.893	3	4	11	13	15	17	19	21	23	25
ELEV.	16.570	16.793	16.189	15.389	13.586	12.593	11.743	11.181	10.843	9.748	8.854	8.271	7.907	6.997	6.081
STATIONS	29	31	33	35	37	39	41	43	45	47	49	51	53	55	57
TIME HRS	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200	6.200
SALINITY	0.019	-0.080	0.003	-0.001	C.000										
PRISM	0.103722E 11	CMP7													
DENSIMETRIC BRIDGE NO. #	0.186409														
DENSIMETRIC ESTUARY NO. #	2.701														
MAXIMUM DISCHARGE	0.9481	FT/SEC	AT	10.93	HRS	FROM	BEGINNING	OF	CYCLE						
SUM OF DISCHARGE	2730.000	CUM/FT/SEC													
K FOR THE NEXT CYCLE IS	578.67	SOFT/SEC													

Table 6.6 (cont'd)

TIDAL CYCLE	2	3,720 HOURS	DAY 0	10,120 HOURS	INSTANTANEOUS SALINITIES IN PPT	23	24	25	26
STATIONS	1	16,578	16,201	15,407	12,749	11,784	11,580	10,892	9,078
SALINITY	20	31	33	37	37	37	37	37	37
SALINITY	C.C.C	-0.005	0.002	-0.002	-0.000				
TIDAL CYCLE	2	3,823 HOURS	DAY 0	10,223 HOURS	INSTANTANEOUS DISCHARGES IN CFS	23	24	25	26
STATIONS	1	16,578	16,201	15,407	12,749	11,784	11,580	10,892	9,078
DISCHARGE	2	-688261.	-448261.	-187919.	-103717.	-187198.	-200279.	-200279.	-200279.
STATIONS	24	24	24	24	24	24	24	24	24
DISCHARGE	2	208786.	203555.	188180.	167784.	119341.	92072.	69379.	43635.
TIDAL CYCLE	2	4,060 HOURS	DAY 0	11,360 HOURS	INSTANTANEOUS SURFACE ELEVATION WITH RESPECT TO MSL, FT	23	24	25	26
STATIONS	1	16,578	16,201	15,407	12,749	11,784	11,580	10,892	9,078
ELEV.	1	-0.384	-0.373	-0.343	-0.294	-0.212	-0.052	0.111	0.273
STATIONS	24	31	32	35	37	37	37	37	37
ELEV.	1.103	1.163	1.242	1.304	1.415	1.444			
TIDAL CYCLE	7	4,960 HOURS	DAY 0	11,960 HOURS	INSTANTANEOUS SALINITIES IN PPT	23	24	25	26
STATIONS	1	16,578	16,201	15,407	12,749	11,784	11,580	10,892	9,078
SALINITY	20	31	33	37	37	37	37	37	37
SALINITY	0.079	-0.016	0.004	-0.001	0.000				
TIDAL CYCLE	2	5,063 HOURS	DAY 0	11,963 HOURS	INSTANTANEOUS DISCHARGES IN CFS	23	24	25	26
STATIONS	1	16,578	16,201	15,407	12,749	11,784	11,580	10,892	9,078
DISCHARGE	2	-774741.	-574120.	-451041.	-340287.	-246666.	-180093.	-88150.	-34027.
STATIONS	24	24	24	24	24	24	24	24	24
DISCHARGE	2	61204.	94739.	124590.	104294.	32449.	36491.	19441.	1420.
TIDAL CYCLE	2	6,200 HOURS	DAY 0	11,800 HOURS	INSTANTANEOUS SURFACE ELEVATION WITH RESPECT TO MSL, FT	23	24	25	26
STATIONS	1	16,578	16,201	15,407	12,749	11,784	11,580	10,892	9,078
ELEV.	1	-0.220	-0.372	-0.409	-0.423	-0.392	-0.310	-0.203	-0.004
STATIONS	20	31	31	37	37	37	37	37	37
ELEV.	1.145	1.332	1.492	1.623	1.743	1.739			
TIDAL CYCLE	2	6,200 HOURS	DAY 0	11,800 HOURS	INSTANTANEOUS DISCHARGES IN CFS	23	24	25	26
STATIONS	1	16,578	16,201	15,407	12,749	11,784	11,580	10,892	9,078
SALINITY	20	31	33	37	37	37	37	37	37
SALINITY	0.040	-0.019	0.005	-0.001	0.000				
TIDAL CYCLE	2	6,303 HOURS	DAY 0	11,703 HOURS	INSTANTANEOUS DISCHARGES IN CFS	23	24	25	26
STATIONS	1	16,578	16,201	15,407	12,749	11,784	11,580	10,892	9,078
DISCHARGE	2	-503759.	-458814.	-427031.	-402449.	-345241.	-276466.	-223010.	-194089.
STATIONS	24	24	24	24	24	24	24	24	24
DISCHARGE	2	142736.	-114437.	-63753.	-35802.	-13116.	34	230.	322.
TIDAL CYCLE	2	7,460 HOURS	DAY 0	11,860 HOURS	INSTANTANEOUS SURFACE ELEVATION WITH RESPECT TO MSL, FT	23	24	25	26
STATIONS	1	16,578	16,201	15,407	12,749	11,784	11,580	10,892	9,078
ELEV.	1	-0.421	-0.482	-0.542	-0.597	-0.672	-0.608	-0.450	-0.275
STATIONS	20	31	33	35	37	37	37	37	37
ELEV.	0.757	0.990	1.202	1.374	1.566	1.621			
TIDAL CYCLE	2	7,460 HOURS	DAY 0	11,860 HOURS	INSTANTANEOUS SALINITIES IN PPT	23	24	25	26
STATIONS	1	16,578	16,201	15,407	12,749	11,784	11,580	10,892	9,078
SALINITY	20	31	33	35	37	37	37	37	37
SALINITY	0.077	-0.013	0.004	-0.001	0.000				
TIDAL CYCLE	2	7,943 HOURS	DAY 0	11,543 HOURS	INSTANTANEOUS DISCHARGES IN CFS	23	24	25	26
STATIONS	1	16,578	16,201	15,407	12,749	11,784	11,580	10,892	9,078
DISCHARGE	2	-104370.	-117963.	-247449.	-328982.	-316345.	-327864.	-307052.	-280876.
STATIONS	24	24	24	24	24	24	24	24	24
DISCHARGE	2	293501.	-222783.	-208493.	-170911.	-107113.	-47887.	-28812.	-6810.

Table 6.6 (cont'd)

TIDAL CYCLE 2		0,680 HOURS		DAY 0		21,680 HOURS		INSTANTANEOUS SURFACE ELEVATION WITH RESPECT TO MSL, FT	
STATIONS	1	2	3	4	5	6	7	8	9
ELEV.	-0.161	-0.207	-0.257	-0.311	-0.370	-0.479	-0.525	-0.547	-0.527
STATIONS	20	21	22	23	24	25	26	27	28
ELEV.	0.356	0.482	0.599	0.713	0.910	1.129	1.328	1.487	1.599
TIDAL CYCLE 2		0,680 HOURS		DAY 0		21,680 HOURS		INSTANTANEOUS SALINITIES IN PPT	
STATIONS	1	2	3	4	5	6	7	8	9
SALINITY	16.940	16.549	16.040	15.015	13.764	12.448	11.298	10.355	9.510
STATIONS	20	21	22	23	24	25	26	27	28
SALINITY	0.664	-0.005	0.002	-0.000					
TIDAL CYCLE 2		0,783 HOURS		DAY 0		21,183 HOURS		INSTANTANEOUS DISCHARGES IN CFS	
DISCHARGE	23381.	121931.	6933.	-80930.	-147859.	-200811.	-231856.	-242904.	-248821.
STATIONS	20	21	22	23	24	25	26	27	28
DISCHARGE	-274200.	-237407.	-202419.	-175041.	-147859.	-121853.	-92146.	-60147.	-3114.
TIDAL CYCLE 2		0,920 HOURS		DAY 0		22,320 HOURS		INSTANTANEOUS SURFACE ELEVATION WITH RESPECT TO MSL, FT	
STATIONS	1	2	3	4	5	6	7	8	9
ELEV.	0.161	0.129	0.091	0.041	-0.006	-0.110	-0.186	-0.284	-0.384
STATIONS	20	21	22	23	24	25	26	27	28
ELF.	0.040	0.070	0.095	0.104	0.171	0.183			
TIDAL CYCLE 2		0,920 HOURS		DAY 0		22,320 HOURS		INSTANTANEOUS SALINITIES IN PPT	
STATIONS	1	2	3	4	5	6	7	8	9
SALINITY	16.560	16.413	16.098	15.073	13.438	12.422	11.576	10.812	10.011
STATIONS	20	21	22	23	24	25	26	27	28
SALINITY	-0.000	-0.001	0.000	-0.000					
TIDAL CYCLE 2		10,023 HOURS		DAY 0		22,423 HOURS		INSTANTANEOUS DISCHARGES IN CFS	
DISCHARGE	45104.	946200.	322355.	163888.	374355.	510787.	-108228.	-208024.	-224989.
STATIONS	20	21	22	23	24	25	26	27	28
DISCHARGE	-227104.	-206499.	-178544.	-152216.	-127158.	-84393.	-50794.	-32765.	-8932.
TIDAL CYCLE 2		11,180 HOURS		DAY 0		23,580 HOURS		INSTANTANEOUS SURFACE ELEVATION WITH RESPECT TO MSL, FT	
STATIONS	1	2	3	4	5	6	7	8	9
ELEV.	0.421	0.417	0.409	0.395	0.364	0.299	0.242	0.193	0.104
STATIONS	20	21	22	23	24	25	26	27	28
ELEV.	-0.295	-0.299	-0.306	-0.324	-0.348	-0.387			
TIDAL CYCLE 2		11,180 HOURS		DAY 0		23,580 HOURS		INSTANTANEOUS SALINITIES IN PPT	
STATIONS	1	2	3	4	5	6	7	8	9
SALINITY	16.760	16.522	16.171	14.977	13.764	12.408	11.610	10.824	10.044
STATIONS	20	21	22	23	24	25	26	27	28
SALINITY	-0.004	0.001	-0.001	0.000	-0.000				
TIDAL CYCLE 2		11,263 HOURS		DAY 0		23,683 HOURS		INSTANTANEOUS DISCHARGES IN CFS	
DISCHARGE	97112.	248771.	348015.	30941.	24474.	18412.	8177.	3448.	-10409.
STATIONS	20	21	22	23	24	25	26	27	28
DISCHARGE	-121481.	-133072.	-125775.	-110751.	-86414.	-57908.	-37840.	-18941.	-4914.
TIDAL CYCLE 2		12,400 HOURS		DAY 1		0,400 HOURS		INSTANTANEOUS SURFACE ELEVATION WITH RESPECT TO MSL, FT	
STATIONS	1	2	3	4	5	6	7	8	9
ELEV.	0.470	0.557	0.595	0.631	0.650	0.623	0.404	0.449	0.377
STATIONS	20	21	22	23	24	25	26	27	28
ELEV.	-0.477	-0.538	-0.585	-0.611	-0.607	-0.595			
TIDAL CYCLE 2		12,400 HOURS		DAY 1		0,400 HOURS		INSTANTANEOUS SALINITIES IN PPT	
STATIONS	1	2	3	4	5	6	7	8	9
SALINITY	16.560	16.517	16.230	15.078	13.703	12.444	11.652	10.918	10.244
STATIONS	20	21	22	23	24	25	26	27	28
SALINITY	-0.007	0.003	-0.001	0.001	-0.001				

Table 6.6 (cont'd)

TIDAL CYCLE STATIONS	2	12.503 HOURS	DAY 6	1	0.903 HOURS	INSTANTANEOUS DISCHARGES IN CFS	20
DISCHARGE STATIONS	22	342601.	355858.	336673.	315037.	277675.	111027.
DISCHARGE	64635.	13949.	-19194.	-19194.	-31512.	-31570.	-27338.
							AVE MASS = 0.304719F 13

TIDAL CYCLE STATIONS	2	0.0 HOURS	DAY 7	0	12.400 HOURS	HIGH WATER SLACK SALINITIES IN PPT	23
TIME HRS	1	1.647	2.273	2.480	3.100	3.307	3.927
SALINITY STATIONS	29	16.560	16.550	16.294	12.915	12.756	11.777
TIME HRS	31	31	32	35	27	27	27
SALINITY	0.64C	-0.018	0.005	-0.001	0.000	5.373	5.373

PRISM= 0.7327736F 10 CUFT
 DENSMETRIC PRODUCE NO.= 0.173325
 DENSMETRIC ESTLBYR NO.= 1.878
 VMAX= 0.57E7 FT/SEC AT 4.75 HRS FROM BEGINNING OF CYCLE
 SUM CF QF'S= 2626.000 CU.FT/SEC
 K FOR THE NEXT CYCLE IS 041.66 SQFT/SEC

TIDAL CYCLE STATIONS	2	0.0 HOURS	DAY 7	0	12.400 HOURS	INSTANTANEOUS SURFACE ELEVATION WITH RESPECT TO WSL, FT	25
ELEV. STATIONS	29	0.52C	0.557	0.631	0.650	0.623	0.604
ELEV.	-0.472	-0.536	-0.585	-0.611	-0.607	-0.599	-0.599

TIDAL CYCLE STATIONS	2	0.0 HOURS	DAY 7	0	12.400 HOURS	INSTANTANEOUS SALINITIES IN PPT	25
SALINITY STATIONS	29	16.560	16.517	16.230	13.703	12.444	11.652
SALINITY	-0.003	0.003	-0.001	0.001	-0.000	9.918	7.926

TIDAL CYCLE STATIONS	2	0.103 HOURS	DAY 6	0	12.403 HOURS	INSTANTANEOUS DISCHARGES IN CFS	27
DISCHARGE STATIONS	22	362601.	355858.	336673.	315037.	277675.	111027.
DISCHARGE	64635.	13949.	-19194.	-19194.	-31512.	-31570.	-27338.
							AVE MASS = 0.304719F 13

END OF ALL CALCULATIONS

VII. Possible Error Messages

There are a variety of error messages which may result from an inadvertent omission, a badly punched card or a misunderstanding in the preparation of input data. The program has built-in checks on certain logic conditions, and when these conditions occur a message is printed out to aid the user in correcting the condition. The following messages are produced by the computer program. All but the last are preceeded by

"* * * SALINITY INTRUSION PROGRAM ERROR * * *"

- 1) NUMBER OF SECTIONS MUST BE EVEN - This refers to card A3, field 1.
- 2) NUMBER OF TIME INTERVALS MUST BE EVEN - This refers to card A4, field 1
- 3) CASE NOT 1 THRU 5 - This refers to card A5.
- 4) NEGATIVE INTRUSION LENGTH IN INPUT - This refers to card D2, field 2.
- 5) COLUMN 1 DOES NOT CONTAIN H, Q, OR S - in cards E2, E3 or E4
the first column is not correctly punched.
- 6) SEQUENCING ERROR IN INPUT STREAM - In the input of geometry, Manning's n or wind data, the first sequence field shows that the cards are not sequentially identified. The sequence number of each card must be 1 greater than the preceding card.
- 7) INTERVAL FOR OUTPUT MUST BE EVEN - This refers to the interval for outputting n (H), S, or Q as specified in cards E2, E3 or E4.
- 8) WATER SURFACE BELOW THE BOTTOM WHICH INDICATES A PROBABLE NUMERICAL INSTABILITY. THE REMEDY MAY WELL BE A SMALLER DELTA-T. This message speaks for itself and results from a probable instability. A check of the stability criteria and of the calculated wave speeds printed by the program may be of use to the user in specifying a smaller Δt .
- 9) THE WATER SURFACE IN THE STORAGE AREA HAS FALLEN BELOW THE BOTTOM. THIS IS NOT NECESSARILY AN INSTABILITY, BUT IS PROBABLY A PROBLEM IN SCHEMATIZATION - This message indicates that a deeper depth of storage area is necessary to continue the calculations. Often the depth of storage area is calculated by dividing a total volume by a planar or surface area. This may result in a depth which is less than the expected lowering of the water surface during tidal action. The program is not designed to account for schematizations which contain bottom sections which become dry. To correct for this

situation it is recommended that the volume of storage be maintained, but that a deeper depth of storage be taken with a correspondingly narrower storage width.

10) CARDS NOT IN SEQUENCE, EXIT TAKEN AFTER LAST CYCLE'S CALCULATION

- The cards not in sequence are F3, F4, . . . etc. which specify the time varying boundary conditions from tidal cycle to tidal cycle. It should be possible to restart the calculation from the last cycle's calculation if the user so desires.

VIII. Implementation on Different Computer Systems

8.1 Memory Size

This program has been implemented on IBM 360/65 and 370/155 computers running in a MVT/OS environment. The G-level FORTRAN compiler has been used for compilation of the FORTRAN program. The memory requirements corresponding to the program as listed in this report are about 120 K bytes. This permits discretizations of up to 200 sections by 449 time intervals.

Should the user find it necessary to reduce the memory used by the program he may do so by two different procedures. First of all he can overlay the subroutines TIDIN and OSIN which are required for input operations only. Secondly he can reduce the dimensioned variables used in the program, if such a reduction is permitted by the size of his particular mesh of Δx and Δt .

8.2 FORTRAN IV Restrictions

Some FORTRAN IV compilers are not always compatible with others. Possible sources of incompatibility in this program's FORTRAN IV language are the use of mixed-mode expressions, T format codes and literals enclosed in apostrophes. If the user has available to him a computer whose FORTRAN IV language does not accept one or more of these conventions some minor reprogramming will be necessary.

8.3 Hints for Modifying the Program

It may be desirable for some users to modify or extend the program in accordance with their specific needs. This section will give some details of the program's construction which should prove helpful to those users.

The program's COMMON area is based on that contained in the program of Harleman and Lee, 1969 (Ref. 2). This was modified to form the Tidal Dynamics part of this model. The modification consists of including a density gradient term, and also eliminating the storage of the elevations and discharges at every time step of a tidal cycle. This eliminated a huge matrix which was always only one half full due to the use of subscripts which represented the full staggered Δx , Δt mesh. Harleman and Lee's input routines have been slightly modified and form the subroutine TIDIN. The use of subscripting which includes each cross-section, rather than every other one still exists, and most arrays such as H, S, and Q are only one half full as a result.

The computations proceed from time step to time step in the following manner. First the Tidal Dynamics section is executed which produces elevation,

η , and discharge, Q , distributions at time $t + \Delta t$ and $t + 2\Delta t$ respectively. Then the salinity, or Salt Balance Section, is executed using the η 's, at $t + \Delta t$ and $t - \Delta t$ and the Q 's at time t . The salinity distribution calculated is at time $t + \Delta t$. The new η , S , and Q distributions then become the old distributions for the next time step's calculation. The time subscripting is unfortunately a bit complicated in this calculation which is accomplished in subroutine MARCH. This subroutine uses subscripts I, IH, or IQ for the old time step and K, KH, or KQ for the new time step. These subscripts can be either 1 or 2. At the end of a time step when the new values of η , S , and Q become the old values for the next time step. This transposition is accomplished by simply setting the new value of subscripts I, IH or IQ equal to the old value of subscript K, KH or KQ.

References

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3. "FORTRAN IV Language", IBM System/360, Manual Form C28-6515, International Business Machines.
4. "CALCOMP PLOTTER SUBROUTINES FOR THE IBM 360", Applications Program Series AP-59, Information Processing Service Center, M.I.T., January 31, 1972.
5. "Programming CALCOMP PEN PLOTTERS", California Computer Products, Inc., Anaheim, California 92801.

Data Source References

1. Jaworski, N.A. and Clark, L.J., "Physical Data Potomac River Tidal System Including Mathematical Model Segmentation", Technical Report No. 43, Chesapeake Technical Support Laboratory, EPA (former FWQA), about 1970.

List of Figures and Tables

<u>Figure</u>		<u>Page</u>
1.1	Representation of Entrance Boundary Condition on Salinity	8
1.2	Correlation of Dispersion Parameter to Degree of Stratification	10
2.1	Segmentation of the Estuary	12
2.2	Typical Schematizations	14
2.3	Sketch of Common Datum	16
2.4	Basic Schematized Cross-Sections	18
4.1	Structural Diagram of Routine MAIN	26
4.2	Structural Diagram of Subroutine MARCH	27
6.1	Potomac Estuary	36
III.1	Symbol Codes for M.I.T. Calcomp Subroutines	III-3
III.2	Typical Plot	III-4
 <u>Table</u>		
6.1	Segment Geometry of Potomac Estuary Excluding Embayments	37
6.2	Embayment Data for Potomac Estuary	38
6.3	Potomac Estuary, Schematized Geometry, After Including Embayments	40
6.4	Maximum and Minimum Tidal Elevations at the Entrance to the Potomac	42
6.5	Input Listing for Test Case	44
6.6	Output Listing from Test Case	46
III.1	Listing of Plotting Program and Test Case	III-5

Appendix I. Program Listing

```

1  MAIN LINE FOR TRANSIENT SIMULATION
2  INTEGER CASE
3  REAL FQ(200), R(200), SS(200), D(200), S(200), R(200), MANCO(200)
4  REAL ZZ(200), R(200), C(200), Q4(200), W(200)
5  REAL BS(200), E(200), H(2,200), S(2,200), Q(2,200)
6  REAL HDA(900), Q78(900)
7  DIMENSION DUM(100), ZD(200), ADUM(100)
8  DIMENSION MFQ(10), PF(10)
9  DIMENSION ETA(3), FW(4), LFW(4)
10 COMMON KO, KIN, KKK, JJJ
11 COMMON NDS, CASE, DELT, DELX, NMAX, NMAXH, JMAX, JMAXH, JJTEM, JMAXO
12 COMMON OI, O2, O3, EH, EZ, FXL4, SZERO, FS
13 COMMON MH, MS, ISW1, ISW2, ISW3, ISW4, ISW5, ISW6, ISW7
14 COMMON H, Q, HDA, O1B, F1, B8, D, SS, R, ZZ, R, MANCO, C, O4, W, RS, S, E
15 COMMON /SW/ ISW10, ISW11
16 COMMON /T/ MMM, QF, IPE, MNN
17 COMMON /EXL14/ JLIM, J2LIM, MOUT, LOUT
18 COMMON /SL/ SLD, ZO
19 COMMON /OUTP/ LEW, NDI, ADUM, NC
20 DATA DUM /100*0./
21 DATA PI /3.141593/, KP /7/
22
23 C FILE 'MMM' HOLDS THE SEQUENTIAL OUTPUT OF 'NC' CYCLES
24 C
25 KO=6
26 KIN=5
27 LLL=3
28 CALL TIOIN
29 CALL OSIN
30 CALL TDISP
31 NMAXQ=NMAX+1
32 NMD2=(NMAX-1)/2
33 NF=JMAX/2
34 ST=PI*2./(NMAX-1)
35
36 C NEW IS NO. OF VARYING FRFSH WATER INPUTS.
37 C NCOF IS THE NO. OF THESE WHICH ARE TO BE SUMMED INTO 'QF',
38 C

```

```

C      THE FRESH WATER FLOW USED TO CALCULATE THE ESTUARY NUMBER.
C      THESE VALUES WILL BE TAKEN IN SEQUENCE.
C      THE LEW(J),J=1,NFW ARE THE STATION LOCATIONS.
      READ(KIN,1112) NFW,NCDF,(LEW(J),J=1,NFW)
      FORMAT(16I5)
      WRITE(K,720) (LEW(J),J=1,NFW)
      FORMAT(/,' TRANSIENT FRESH WATER INPUTS AT SEGMENTS',4I5)
      IF(ISW .EQ. 1) WRITE(KD,1428)
      FORMAT(' TRANSIENT MIN-MAX OCFAN ELEVATIONS WRT MWL')
      READ(KIN,1112) NC
      WRITE(KD,4285) NC
      FORMAT('O',I5,' CALCULATION FOR',I4,' CYCLES')
C
C      BEGIN MASTER LOOP BY TIDAL CYCLE
C      ETA'S ARE WRT MWL AT THE OCEAN
      ETA(1)=HDA(1)
      DO 1111 LEW=1,NC      GOTO 5
      IF(NOS .EQ. 0)
      READ(KIN,721)KCK,ETA(2),ETA(3),SZERO,FEND,(FW(J),J=1,NFW)
      FORMAT(I5,5X,7F10.0,(RF10.0))
      SET RIVER END DISCHARGE FOR OPEN END CASE
      DO 7 J=2,NMAXJ,2
      QDR(J) = -FEND
      GOTO 6
      READ(KIN,721) KCK,ETA(2),ETA(3),SZERO,(FW(J),J=1,NFW)
      WRITE(KD,2857)
      FORMAT(/,'O',I20(' '*))
      IF(ISW .EQ. 2)      GOTO 722
      WRITE(K,1113) KCK,ETA(2),ETA(3),SZERO,(FW(J),LEW(J),J=1,NFW)
      FORMAT(/,')CYCLE',I4,5X,'HMIN=',F7.2,3X,'HMAX=',F7.2,4X,'SZERO=',
      1      F7.2,5X,'QDR'S AND STATIONS ARE',4(F10.3,I4))
      GOTO 723
      WRITE(KD,1133) KCK,SZERO,(FW(J),LEW(J),J=1,NFW)
      FORMAT(/,')CYCLE',I4,4X,'SZERO=',F7.2,5X,'QDR'S AND STATIONS ARE',
      1      4(F10.3,I4))
C

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MAIN 37
MAIN 38
MAIN 39
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MAIN 43
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 MAIN 106
 MAIN 107
 MAIN 108

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723 WRITE(KO,1114) EZ
1114 FORMAT('16, *K=',F10.2)
IF(NOS .NE. 0) WRITE(KO,724) FEND
724 FORMAT(' ',T31,'UPSTREAM BOUNDARY DISCHARGE=',F10.2)
C
TEST SEQUENCE
IF(KCK .EQ. LEW) GOTO 1117
WRITE(KO,207)
207 FORMAT(' CARDS NOT IN SEQUENCE, EXIT TAKEN AFTER LAST CYCLE',5')
1
CALCULATION')
GOTO 1118
1117 DO 1115 J=1,NEW
MYJO=LFW(J)
1115 FQ(MYJO)=F4(J)
C
CALC. 'OF' FOR ESTUARY NUMBR.
OF=0.
DO 1116 J=1,NCQF
MYJO=LFW(J)
OF=OF+FQ(MYJO)
IF(NOS .NE. 0) OF=OF+FEND
C
ISWO TO CALCULATE TRANSIENT TIDAL ELEVATIONS
GOTO(20,25),ISWO
RANG1=(ETA(1)-ETA(2))/2.
RANG2=(ETA(3)-ETA(2))/2.
IF((RANG1 .LT. 0) .OR. (RANG2 .LT. 0)) GOTO 22
GOTO 23
22 WRITE(KO,221) LEW
221 FORMAT(' WARNING EQUAL PLEVATIONS IN CYCLE',I5)
23 DO 30 I=1,NMAXH,2
ARG=ST*(I-1)
IF(ARG .GT. PI) GOTO 21
C
GOING DOWN
HRA(I)=ETA(1)-RANG1*(1.-COS(ARG))
GOTO 30
C
GOING UP
  
```

```

21 HDA(11)=ETA(3)-RANG2*(1.-COS(ARG))
30 CONTINUE
C ETA(1)=ETA(3)
25 CALL MARCH
1111 CONTINUE
1118 IF(IISW1.EQ.1) GOTO 1119
END FILE MMM
1119 REWIND MMM
IF(IISW4.EQ.1) GOTO 1120
END FILE NNN
REWIND NNN
1120 WRITE(KO,2857)
C
C OUTPUT FINAL TIME STEPS H,S,&Q
C REFER H TO MSL
182 DO 182 J=1,JMAXH,2
H(LOUT,J)=H(LOUT,J)-SLO+ZD(J)+D(J)
CALL SPEW(1,1)
CALL SPEW(2,1)
CALL SPEW(3,1)
GOTO (1200,1201),IISW3
1201 WRITE(KP,1202) (H(LOUT,J),J=1,JMAXH,2)
1202 FORMAT(5F10.5)
WRITE(KP,1203) (S(LOUT,J),J=1,JMAXH,2)
1203 FORMAT(5F10.4)
WRITE(KP,1204) (Q(LOUT,J),J=2,JMAX,2)
1204 FORMAT(5F10.4)
1200 WRITE(KO,1205)
1205 FORMAT(/,'OEND OF ALL CALCULATIONS',/,,'1')
CALL EXIT
END

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MAIN 109
MAIN 110
MAIN 111
MAIN 112
MAIN 113
MAIN 114
MAIN 115
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MAIN 117
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MAIN 138
MAIN 139
MAIN 140

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C          SECTION 1, DESCRIPTION OF SCHEMATIZED ESTUARY
C          WRITE(K0,200)
200        FORMAT('1',T16,37('* '),/,T16,'**',T18,'**',/,T16,'**',T20,'M.I.T. ',
1          'SALINITY INTRUSION PROGRAM, ONE-DIMENSIONAL ',
2          'SCHEMATIZATION',T8,'*',/,T16,'**',T89,'**',/,T16,37('* '),
3          '///',OSECTION 1, DESCRIPTION OF THE SCHEMATIZED ESTUARY',
4          '///)
          DO 1010 I=1,2
          READ(KIN,1020) WDIUM
1020        FORMAT(20A4)
1010        WRITE(KOUT,1021)WDUM
1021        FORMAT(1X,20A4)
C
          READ(KIN,1050) JMAX,CHLEN,NMAX,TCOM
1050        FORMAT(15,F10.0)
C          NOS =0 FOR CLOSED END, = ANYTHING ELSE FOR OPEN END
          READ(KIN,1055) CASE,NOS
1055        FORMAT(2I5)
          IF(NOS .EQ. 0) GOTO 300
          WRITE(K0,3001)
3001        FORMAT('OGEOMETRIC DATA',5X,'*** OPEN END ESTJARY ***')
          GOTO 302
          WRITE(K0,3002)
300        WRITE('OGEOMETRIC DATA',5X,'*** CLOSED END ESTUARY ***')
3002        FORMAT('OGEOMETRIC DATA',5X,CHLEN,NMAX,TCOM)
302        WRITE(K0,1090) JMAX,CHLEN,NMAX,TCOM
1090        FORMAT(/,0 NUMBER OF STATIONS =',I4,5X,
1          'ESTUARY LENGTH=',F10.0,' FT.',/,0 NUMBER OF TIME ',
2          'INCREMENTS PER PERIOD=',I4,5X,'PERIOD=',F7.0,' SECONDS',
          IF(2*(JMAX/2) .NE. JMAX) CALL ERRL(1)
          IF(2*(NMAX/2) .NE. NMAX) CALL ERRL(2)
          NMAX=NMAX+1
          JMAXH=JMAX-1
          JMAXQ=JMAX
          NMAXH=NMAX
          NMAXQ=NMAX+1

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TIDI 37
TIDI 38
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TIDI 40
TIDI 41
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TIDI 43
TIDI 44
TIDI 45
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TIDI 70
TIDI 71
TIDI 72

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NEQ=NMAXQ-2
JUTEM = JMAXQ-2
DELT=TCOM/(NMAX-1)
DELX=CHLEN/(JMAX-1)
C   SFLECT SCHEMATIZATION CASE          CALL EPRL(3)
C   IF((CASE .LT. 1) .OR. (CASE .GT. 5))
C   INPUT SCHEMATIZED GFCMETRY
C   GO TO (540,550,550,565,565),CASE
C
C   CASE 1  RECTANGULAR CROSS-SECTION
540 WRITE(KOUT,1110)
    WRITE(KOUT,1120)
1110 FORMAT('O CASE 1,  RECTANGULAR CROSS-SECTION')
1120 FORMAT('O MILES',8X,'O',11X,'ZO',14X,'R',7X,'B(CORF)',6X,
1      'FEET',3X,'SECTION')
    DO 1 J=1,JMAX
    READ(KIN,1130) ISEI,D(J),ZO(J),B(J)
1130 FORMAT('I5,5X,5F10.0)
    DFEEF=(J-1)*DELX
    DIST=DFEEF/5280.
    BS(J)=B(J)
    WRITE(KOUT,1140) DIST,D(J),ZO(J),B(J),BS(J),DFEEF,J
1140 FORMAT ('F10.5,2(2X,F10.5),2(2X,F10.2),2X,F10.0,1A)
    CALL SEQST(J,ISEI)
1 CONTINUE
    GO TO 570
C
C   CASE 2  IRREGULAR CROSS-SECTION, (STORAGE)
550 WRITE(KOUT,1150)
    WRITE(KOUT,1160)
1150 FORMAT('O CASE 2,  IRREGULAR CROSS-SECTION (STORAGE)')
1160 FORMAT ('O MILES',8X,'O',11X,'ZO',14X,'R',7X,'B(CORF)',8X,
1      'D',8X,'FEET',3X,'SECTION')
    DO 2 J=1,JMAX
    READ(KIN,1130) ISEI,D(J),ZO(J),B(J),BS(J),DP(J)
    DFEEF=(J-1)*DELX

```

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T101 73
T101 74
T101 75
T101 76
T101 77
T101 78
T101 79
T101 80
T101 81
T101 82
T101 83
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T101 106
T101 107
T101 108

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DIST=DFEET/5280.
WRITE(KOUT,1180) DIST,D(J),ZD(J),R(J),BS(J),DP(J),DFFFT,J
1180 FORMAT (F10.5,2(2X,F10.5),3(2X,F10.1),2X,F10.0,19)
CALL SECTST(J,ISEQ)
2 CONTINUE
C
C CALCULATE CORE AREA IF STORAGE TO BE CONSIDERED
DO 30 J=1,JMAX
COREA(J)=RS(J)*D(J)-DP(J)
IF (COREA(J).LT.0) WRITE(KO,31)J
31 FORMAT('O NOTE, O' DEPPER THAN O FOR SECTION',15)
30 CONTINUE
GO TO 570
C
C CASE 3 TRAPEZOIDAL CROSS-SECTION
560 WRITE(KOUT,1190)
1190 FORMAT('O CASE 3, TRAPEZOIDAL CROSS-SECTION')
WRITE(KOUT,1200)
1200 FORMAT ('O MILES',RX,1HD,11X,2H70,13X,2HBB,7X,2HSS,11X,2HZZ,
1 8X,'FEET',3X,'SECTION')
DO 3 J=1,JMAX
READ(KIN,1130) ISEQ,ZO(J),SS(J),BR(J)
ZZ(J)=1.+SS(J)**2
ZZ(J)=SQRT(ZZ(J))
DFEET=(J-1)*DELX
DIST=DFEET/5280.
WRITE(KOUT,1230) DIST,D(J),ZO(J),BR(J),SS(J),ZZ(J),DFEET,J
1230 FORMAT (F10.5,2(2X,F10.5),2X,F10.1,2(2X,F10.5),F10.0,18)
CALL SECTST(J,ISEQ)
3 CONTINUE
GO TO 570
C
C CASE 4 EXPONENTIALLY VARYING WIDTH, RECTANGULAR CROSS-SECTION
565 READ(KIN,1231) KR,BO
1231 FORMAT(2F10.0)
WRITE(KOUT,1232) KB

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TIDI 109
TIDI 110
TIDI 111
TIDI 112
TIDI 113
TIDI 114
TIDI 115
TIDI 116
TIDI 117
TIDI 118
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TIDI 120
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TIDI 137
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TIDI 139
TIDI 140
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TIDI 142
TIDI 143
TIDI 144

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1232 1 FORMAT(10, CASE 4, EXPONENTIALLY VARYING WIDTH, RECTANGULAR,
1 WRITE(KOUT,1120)
DO 9 J=1,JMAX
READ(KIN,1130) ISEQ,D(J),ZC(J)
XI=KR*(J-1)*DELX
B(J)=80*EXP(-XI)
BS(J)=B(J)
DFEET=(J-1)*DELX
DIST=DFEET/5280.
WRITE(KOUT,1140) DIST,D(J),ZC(J),BS(J),DFEET,J
CALL SEQTST(J,ISEQ)
9 CONTINUE
GOTO 570

C
C CASE 5, CONSTANT RECTANGULAR CROSS-SECTION
WRITE(KO,1112)
FORMAT(10,CASE 5, CONSTANT RECTANGULAR CROSS-SECTION)
WRITE(KO,1120)
READ(KIN,1130) ISEQ,D(1),ZC(1),B(1)
DO 567 J=1,JMAX
DFEET=(J-1)*DELX
DIST=DFEET/5280.
D(J)=D(1)
ZC(J)=ZC(1)
B(J)=B(1)
BS(J)=B(1)
WRITE(KO,1140) DIST,D(J),ZC(J),B(J),BS(J),DFEET,J
END OF GEOMETRIC DATA INPUT
567 C
C SECTION 2 CHANNEL ROUGHNESS AND WIND EFFECTS
C
C WRITE(KO,1260)
FORMAT(10,1,T10,SECTION 2 CHANNEL ROUGHNESS AND WIND EFFECTS,
1)
READ(KIN,1050) ROUGH
GOTO(1294,1292),ROUGH

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T101 145
T101 146
T101 147
T101 148
T101 149
T101 150
T101 151
T101 152
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T101 171
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T101 174
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T101 177
T101 178
T101 179
T101 180

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C          CONSTANT ROUGHNESS AND WIND
C          NOTE THAT ALL LATERAL INFLOWS ARE SET TO ZERO AT THIS POINT
1294 READ(KIN,1300) ISEQ,MANCO(1),V(1),PHI(1)
      DO 1296 J=2,JMAX
      MANCO(J)=MANCO(1)
      FQ(J-1)=0.
      V(J)=V(1)
1296   PHI(J)=PHI(1)
      FQ(JMAX)=0.
      WRITE(KO,1310)
1310   FORMAT('O CASE 1 CONSTANT ROUGHNESS AND CONSTANT WIND')
      GOTO 630
C
C          VARIABLE ROUGHNESS OR VARIABLE WIND
1292   DO 4 J=1,JMAX
1300   READ(KIN,1300) ISEQ,MANCO(J),V(J),PHI(J)
      FORMAT('5,5X,F10.7,10X,2F10.7')
      CALL SEQTST(J,ISEQ)
      FQ(J)=0.
      WRITE(KOUT,1320)
1320   FORMAT('O CASE 2 EITHER ROUGHNESS OR WIND EFFECTS VARY')
C
C          TEST FOR NO WIND
      WEND=1
      DO 5 J=1,JMAX
      IF(V(J).NE.0) WEND=2
      CONTINUE
      GOTO(640,650),WEND
C          NO WIND EFFECT
      WRITE(KOUT,1350)
1350   FORMAT('O NO WIND')
      DO 6 J=2,JMAX,2
      W(J)=0.
      CONTINUE
      GOTO 661

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TIDI 181
TIDI 182
TIDI 183
TIDI 184
TIDI 185
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TTDI 217
 TTDI 218
 TTDI 219
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 TTDI 250
 TTDI 251
 TTDI 252

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C      WIND BEING CONSIDERED
    650 WRITE(KOUT,1360)
    1360 FORMAT('O WIND EFFECT INCLUDED')
    DO 7 J=2,JMAX,2
    VCOS = V(J)*COS(PHI(J))
    VCNS = VCOS*ABS(VCOS)
    W(J)= BETA*ARDEN*VCOS/WAUMT
  7 CONTINUE
    WRITE(KO,1380) WAUMT,BETA,ARDEN
    1380 FORMAT(' DENSITY OF WATER=',F6.2,' BETA=',F9.4,' AIR DENSITY=',
    1 F10.4)
  C      OUTPUT
    661 WRITE(KOUT,1390)
    1390 FORMAT ('O MILES',5X,SHMANCO,11X,2HF0,7X,1HV,10X,3HPHI,
    1 12X,'FEET SECTION')
    DO 8 J=1,JMAX
    DFEEF=(J-1)*DELX
    DIST=DFEEF/5280.
    WRITE(KOUT,1400) DIST,MANCC(J),FO(J),V(J),PHI(J),DFEEF,J
    1400 FORMAT (F10.5,2X,F10.7,2X,F10.3,2(2X,F10.7),2X,F10.0,15)
  8 CONTINUE
  C      SECTION 3 SURFACE ELEVATION AT THE ENTRANCE
  C      ISWO=2
    WRITE(KOUT,1405)
    1405 FORMAT('O SECTION 3, SURFACE ELEVATION AT THE ENTRANCE',F)
    READ(KIN,141) CLASA,DSL0
    1410 FORMAT(I5/F10.0)
    SLD=20*(1+D(1))-DSL0
    WRITE(KO,1406) DSL0,SLD
    1406 FORMAT(' MWL',F5.2,' FT ABOVE MSL AT OCEAN',/,
    1 MSL',F7.2,' FT ABOVE THE HORIZONTAL DATUM')
  C      GOTO(665,680,670),CLASA
  C      CASE 1, COSINE FIT TO HW,LW SERIES
  C      PUT FIRST HW IN VARIABLE HOA(1)
  
```

```

665 READ(KIN,1231) HDA(1)
WRITE(KO,1407) HDA(1)
1407 FOPMAT('O CASE 1, COSINE FIT TO HW-LW SERIES',/,
1 ' FIRST HW VALUE RELATIVE TO WXL =',F10.2,/)
ISWO=1
GOTO 745
C
C CASE 2, ELEVATIONS REPEATING AND HARMONIC
680 READ(KIN,1050) LMAX
WRITE(KO,1457)
1450 FORMAT('O CASE 2, ELEVATIONS AT ENTRANCE REPEAT AND APE HARMONIC',)
DO 10 L=1,LMAX
READ(KIN,1231) AMP(L),T(L)
IF(LMAX.EQ.1) T(L)=TCOM
WRITE(KO,1441) AMP(L),T(L)
1441 FORMAT(5X,'A=',F10.4,5X,'T=',F10.2)
OMEGA(L)=2.*PI/T(L)
10 CONTINUE
DO 11 N=1,NMAXH*2
HDA(N)=0.
DO 11 L=1,LMAX
THETA(L)=OMEGA(L)*(N-1)*DELT
HDA(N)=HDA(N)+AMP(L)*COS(THETA(L))
11 CONTINUE
GO TO 745
C
C CASE 3, REPEATING ELEVATIONS FROM TABLE
670 WRITE(KOUT,1430)
1430 FOPMAT('O CASE3, REPEATING ELEVATIONS FROM TABLE',)
NDUM=NMAXH-2
READ(KIN,1420) (HDA(N), N=1,NDUM*2)
1420 FOPMAT (5F10.5)
WRITE(KO,1408) (HDA(N),N=1,NDUM*2)
1408 FOPMAT(5X,5F10.5)
HDA(NMAXH)=HDA(1)
C BOUNDARY CONDITION AT OCFAN A DEFINED
TIDI 253
TIDI 254
TIDI 255
TIDI 256
TIDI 257
TIDI 258
TIDI 259
TIDI 260
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 TIDI 323
 TIDI 324

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C
C
C   SET UP BOUNDARY CONDITIONS AT RIVER END
745  DO 19 N=2,NMAXQ,2
    QOR(N)=0.
19  CONTINUE
C   INITIAL CONDITIONS DEFINED
D1=DELT/DELX
D2=.5/(DELT*3)
D3=.5/DELX
DTEM=1./D1
TDH=DELT/3600.
DO 21 J=2,JJTEM,2
D4(J)=D3*(D(J-1)-D(J+1))+Z0(J-1)-Z0(J+1))
21  CONTINUE
1500 WRITE(KOUT,1300) DELT,DELX
    FORMAT(//,' DT=',F10.2,' SEC.   DX=',F10.1,' FT.')
```

```

1510 WRITE(KOUT,1510) DTEM
    FORMAT(15H DELTAX/DELTAI=,F8.2, 9H FT./SEC.)
C   COMPUTATION OF CELERITY WITH RESPECT TO MEAN DEPTH
C   SCCEL DEFINTES THE CELERITY WITH RESPECT TO THE DEPTH ONLY
1520 WRITE(KOUT,1520)
    FORMAT (36H0CELERITY WITH RESPECT TO DEPTH ONLY)
1530 WRITE(KOUT,1530)
    FORMAT(T11,'CELERITY   WAVE LENGTH',/, ' SECTION (FT/SEC)',
1      T23,'(MILES)')
```

```

1540 IWARN=0
    DO 22 J=1,JMAX
      SCFL(J)=(G*D(J))**.5
      WALEN(J)=SCFL(J)*TCOM/5280.
      WRITE(KOUT,1540) J,SCFL(J),WALEN(J)
      FORMAT(1X,I7,F10.2,2X,F10.2)
      IF(SCFL(J) .GT. DTEM) IWARN=1
22  CONTINUE
1550 IFC(IWARN .EQ. 1) WRITE(KOUT,1550)
    FORMAT('O*** WARNING, CELERITY GREATER THAN DX/DT IN SOME',
```

1 RETURN • SECTIONS. INSTABILITY C/ULO RESULT.1
END

TIME 325
TIME 326
TIME 327

CSIN 1
 CSIN 2
 CSIN 3
 OSIN 4
 OSIN 5
 CSIN 6
 CSIN 7
 OSIN 8
 CSIN 9
 CSIN 10
 OSIN 11
 CSIN 12
 OSIN 13
 OSIN 14
 OSIN 15
 OSIN 16
 OSIN 17
 OSIN 18
 OSIN 19
 OSIN 20
 CSIN 21
 CSIN 22
 OSIN 23
 CSIN 24
 OSIN 25
 CSIN 26
 CSIN 27
 OSIN 28
 CSIN 29
 OSIN 30
 CSIN 31
 CSIN 32
 OSIN 33
 OSIN 34
 OSIN 35
 OSIN 36

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SUBROUTINE OSIN
INTEGER CASE,ODS1,ODS2
REAL FC(200),BB(200),C(200),SS(200),B(200),MANCO(200)
REAL ZZ(200),R(200),C(200),D4(200),W(200)
REAL BS(200),E(200),H(2,200),S(2,200),Q(2,200)
REAL HOA(900),QCB(900)
DIMENSION OPT(4),OKEY(4),KVAR(3)
DIMENSION DUM(100),ZC(200)
COMMON KO,KIN,KKK,JJJ
COMMON NOS,CASE,DELT,DELX,NMAX,NMAXH,JMAX,JMAXH,JJTEM,JMAXQ
COMMON D1,D2,D3,EH,EZ,EXLP,SZERC,ES
COMMON MH,MS,ISW0,ISW1,ISW2,ISW3,ISW4,ISW5,ISW6,ISW7
COMMON H,Q,HQA,QCB,FC,RB,C,SS,B,ZZ,R,MANCO,C,D4,M,BS,S,E
COMMON /CEPT/ XNCPT
COMMON /T/ MMM,QF,IPF,IPE,NNN,IETM
COMMON /SL/ SLD,ZO
DATA CKEY/'KEEP',HMS',FC ',E ',KVAR/'H','S','Q'/'
DATA INEPT/0/
ISW1=1
ISW2=1
ISW3=1
ISW4=1
ISW5=-1
ISW6=-1
ISW7=-1
IETM=1
C SECTION 4, DEFINITIONS RELATIVE TO THE SALT BALANCE EQN.
C AND TO INITIAL SALINITIES
C WRITE(KC,200)
C FORMAT(/,'1 SECTION 4, DEFINITIONS RELATIVE TO THE SALT BALANCE
200 EQUATION, AND TO INITIAL SALINITIES',//)
C NMD2=(NMAX-1)/2
C NF=JMAX/2
C I11=5
C MMM=15
  
```

```

NNN=11
LLL=3
C
199 READ(KIN,199) EZ, CDS2
   FORMAT(F10.0,15)
   IF(CDS2 .NE. 0) IETM=ODS2
100 READ(KIN,100) XNCPT,EXLM,PF,PE
   FORMAT(5F10.0)
201 WRITE(KC,201) IETM,EZ
   FORMAT(/,,' E=K(DS/DX)+',I2,'*E(TAYLCR)',5X,' K FOR THE NEXT ',
   ' TIDAL CYCLE IS',F10.2,/)
1 IF(XNCPT .EQ. 0.) XNCPT=2.15E-3
   WRITE(KC,205) XNCPT
205 FORMAT('OK VS. ED GIVEN BY K/(UZ*L)=' ,E10.3,'ED**(-1/4)',/)
51 IF(EXLM) 51,1,2
   CALL ERRLL(4)
1 WRITE(KC,202)
202 FORMAT(/,,' SALINITY EXCURSION NOT LIMITED',/)
   GOTC 3
2 WRITE(KC,203) EXLM
203 FORMAT(/,,' SALINITY EXCURSION LIMIT =',F10.2,' FT',/)
3 IPE=PF*AMD2+0.5
   IPE=PE*AMD2+0.5
204 WRITE(KC,204) PF,PE,IPE
   FORMAT(' SALINITY RETURNS TO ZERO AFTER LWS IN',F5.2,
1 ' OF A TIDAL PERIOD',/, ' EXTENSION OF ZERO AFTER HWS FOR',
2 ' F5.2, OF A TIDAL PERIOD',T1CC,2I8)
C
C DEFINITION OF THE SOURCE OF INITIAL CONDITIONS
C ALL ELEVATIONS IN INITIAL CONDITIONS ARE WITH RESPECT TO MSL
101 READ(KIN,101) ICASE,ODS1
   FORMAT(2I5)
   IF(CDS1 .NE. 0) LLL=CDS1
C
C DEFINE INITIAL CONDITIONS
C GOTO(19,195,18),ICASE
OSIN 37
CSIN 38
CSIN 39
OSIN 40
CSIN 41
OSIN 42
OSIN 43
OSIN 44
OSIN 45
CSIN 46
CSIN 47
OSIN 48
CSIN 49
OSIN 50
OSIN 51
OSIN 52
OSIN 53
CSIN 54
OSIN 55
OSIN 56
OSIN 57
OSIN 58
CSIN 59
CSIN 60
OSIN 61
OSIN 62
CSIN 63
OSIN 64
CSIN 65
CSIN 66
CSIN 67
CSIN 68
OSIN 69
OSIN 70
CSIN 71
CSIN 72

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C
C
C
C
18
217
180
C
C
19
216
C
C
195
218
20
520
C
181
C
CSIN 73
CSIN 74
CSIN 75
CSIN 76
OSIN 77
CSIN 78
CSIN 79
CSIN 80
OSIN 81
CSIN 82
OSIN 83
OSIN 84
OSIN 85
CSIN 86
CSIN 87
OSIN 88
CSIN 89
OSIN 90
CSIN 91
CSIN 92
OSIN 93
OSIN 94
CSIN 95
OSIN 96
CSIN 97
OSIN 98
CSIN 99
CSIN 100
CSIN 101
CSIN 102
CSIN 103
OSIN 104
CSIN 105
OSIN 106
CSIN 107
OSIN 108

CASE 3, INPUT ALL I.C. FROM CARDS
H INPUT WRT MSL, THEN REFERED TO NWL AT THE OCEAN
ALL STARTING VALUES FROM UNIT III
WRITE(KC,217)
FORMAT('OCASE 3, INITIAL CONDITIONS ALL FROM CARDS')
READ(III,100) (H(1,J),J=1,JMAXH,2)
CALL SPEW(1,4)
DO 180 J=1,JMAXH,2
H(1,J)=H(1,J)+SLD-ZC(J)-D(J)
REAC(III,100) (S(1,J),J=1,JMAXH,2)
READ(III,100) (C(1,J),J=2,JMAXQ,2)
GOTC 17

CASE1, ALL I.C. FROM DATASET
WRITE(KC,216)
FORMAT('OCASE 1, ALL INITIAL CONDITIONS FROM DATASET')
GOTO 20

CASE 2, ELEVATIONS AND DISCHARGES FROM DATASET,
SALINITY FROM CARDS
WRITE(KC,218)
FCRMT('OCASE 2, INITIAL ELEVATIONS AND DISCHARGES FROM DATASET',
, SALINITIES FROM CARDS')
1 READ(KIN,100) (S(1,J),J=1,JMAXH,2)
NTNL=NMD2-1
DO 520 I=1,NTNL
READ(LLL) (DUM(J),J=1,NF)
READ(LLL) (DUM(J),J=1,NF)
READ(LLL) (DUM(J),J=1,NF)
SET UP H,S AND Q CN LST READ
READ(LLL) (F(1,J),J=1,JMAXH,2)
CALL SPEW(1,4)
DO 181 J=1,JMAXH,2
H(1,J)=H(1,J)+SLD-ZC(J)-D(J)
DECIDE WHETHER OR NOT TO USE S(INIT) FROM CARDS

```

```

4550 GOTO(4560,4550),ICASE
      READ(LLL)
      GOTO(4570)
      (DUM(J),J=1,NF)
4560 READ(LLL)
      (S(1,J),J=1,JMAXH,2)
4570 READ(LLL)
      (Q(1,J),J=2,JMAXQ,2)
C
C      PLACE INTO S(2,J) FOR RPO CALC EXCEEDING J LIM
17 DO 4580 J=1,JMAXH,2
4580 S(2,J)=S(1,J)
      CALL SPEM(2,4)
      CALL SPEW(3,4)
C
C      SECTION 5. OUTPUT SELECTION
C      OPTICNS
      WRITE(KC,210)
      FORMAT('0',/, 'SECTION 5, OUTPUT SELECTION')
210 REAC(KIN,120)(OPT(II),I=1,4),ODS1,ODS2
      FORMAT(4(A4,1X),2I5)
      DO 3001 I=1,4
      DO 3001 II=1,4
      IF(OPT(II).NE.CKEY(II)) GOTO 3001
      WRITE(KO,270) OKEY(II)
      FORMAT('OPTICN ',A4,' SPECIFIED')
      INOPT=INOPT+1
      IF(II.EQ.1) ISW1=2
      IF(II.EQ.2) ISW2=2
      IF(II.EQ.3) ISW3=2
      IF(II.EQ.4) ISW4=2
      CONTINUE
3001 IF(INOPT.EQ.0) WRITE(KO,271)
      FORMAT('0 NO OPTIONS SPECIFIED')
      GET CATASET NO.S FOR 'KEEP' AND 'E' OPTIONS
      IF((ISW1.EQ.2).AND.(OCS1.NE.0)) MMM=ODS1
      IF((ISW4.EQ.2).AND.(OCS2.NE.3)) NNN=ODS2
      GET FREQUENCY OF BASIC OUTPUT
      WRITE(KC,272)

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CSIN 109
OSIN 110
CSIN 111
OSIN 112
CSIN 113
CSIN 114
OSIN 115
OSIN 116
CSIN 117
CSIN 118
CSIN 119
OSIN 120
OSIN 121
CSIN 122
OSIN 123
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CSIN 127
OSIN 128
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CSIN 132
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CSIN 138
OSIN 139
CSIN 140
OSIN 141
CSIN 142
CSIN 143
OSIN 144

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272  FORMAT('O BASIC OUTPUT',T20,'FREQUENCY')
DO 275 I=1,3
READ(KIN,121) KEY,NFREQ
FORMAT(A1,4X,I5)
DO 276 II=1,3
IF(KEY.NE.KVAR(II)) GOTO 276
IF(II.EQ.1) ISW5=NFREQ
IF(II.EQ.2) ISW6=NFREQ
IF(II.EQ.3) ISW7=NFREQ
GOTO 277
276  CCNTINUE
CALL ERRLL(5)
277  WRITE(KC,273) KEY,NFREQ
273  FORMAT(7X,A1,T19,I5)
IF((NFREQ/2)*2.NE.NFREQ) CALL ERRLL(7)
275  CONTINUE
C
C      CHECK FOR NON-FATAL ERRCR ACCUMULATION
CALL ERRLL(8)
RETURN
END
OSIN 145
CSIN 146
OSIN 147
CSIN 148
OSIN 149
OSIN 150
CSIN 151
CSIN 152
CSIN 153
CSIN 154
OSIN 155
OSIN 156
OSIN 157
OSIN 158
OSIN 159
CSIN 160
CSIN 161
OSIN 162
OSIN 163
OSIN 164
OSIN 165

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1 MARC
 2 MARC
 3 MARC
 4 MARC
 5 MARC
 6 MARC
 7 MARC
 8 MARC
 9 MARC
 10 MARC
 11 MARC
 12 MARC
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 35 MARC
 36 MARC

```

SUBROUTINE MARC
INTEGER CASE, I, NRS(10)
REAL FC(200), FB(200), C(200), SS(200), B(200), MANCO(200)
REAL ZZ(200), K(200), C(200), D4(200), W(200)
REAL BS(200), E(200), H(200), S(200), C(200)
REAL HDA(900), QUB(900), DP(200), COREA(200)
LOGICAL LV(100), KEY(100), LP, LDP
DIMENSION HKH(200), Z(200), WP(200), ZD(200), RHO(200), ZO(200)
DIMENSION CU(100), CB(100), CL(100), F(100)
DIMENSION A(2,200), HAS(100), HH(450), CQ(450)
COMMON K3, KIN, KKK, JJJ
COMMON NOS, CASE, DELT, DELX, NMAX, NMAXF, JMAX, JMAXH, JJTEM, JMAXQ
COMMON CL, D2, D3, EH, EZ, EXL, SZERC, ES
COMMON MH, MS, ISW3, ISW1, ISW2, ISW3, ISW4, ISW5, ISW6, ISW7
COMMON F, Q, HDA, QUB, FJ, BB, C, SS, B, ZZ, R, MANCO, C, D4, H, BS, S, E
COMMON /SW/ ISW1, ISW11
COMMON /T/ MM, CF, IPF, IPE, NNN
COMMON /EXLIM/ JUL14, J2LIM, K, I
COMMON /STOR/ DP, COREA, LDP
COMMON /AREAS/ A
COMMON /JUTP/ LE, IV, N, HKS, NDUMP, IHWS
COMMON /SL/ SL0, ZO
EQUIVALENCE (I, IH, I1), (K, KF, KQ), (NREC, JM2)
DATA THETA/, .333333/
DATA INIT/, 1/
DATA RHGS/, .75/, P/, .1666667/, G/, .32, .2/

INITIALIZATION
GOTO(1,2), INIT
NSEG=JMAX
INIT=2
GG=1., -THETA
NREC=NSEG/2
JM2M1=JM2-1
NMD2=(NMAX-1)/2

```

C
 C
 C
 1

MARC 37
MARC 38
MARC 39
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MARC 69
MARC 70
MARC 71
MARC 72

```

DX2=DELX*2.
XL=(JMAX-1)*DELX
DT2=DELT*2.
DTX=DELT/DELX
F(JM2+1)=0.
CT2=2.*THETA*DELX/DELT
CG4=4.*(1.-THETA)*DELX/DELT
CC4=2.*(1.-THETA*.5)/DTX
CF4=2.*(THETA*.5)/DTX
JLMM1=JLIM-1
J2LIM=2*JLIM-1

C
C   IH=I+1 IS THE PREVIOUS TIME STEP
C   KH=K+1 IS THE NEXT TIME STEP
C
C   IH=1
C   KH=2
C   NORMAL ENTRY
C   AVERAGE MASS BALANCE FOR ITERATION
2   TSUM=C.
C   FOR HIGH WATER SLACK CALCS
C   DO 13 J=1,JLIM
C   IF(Q(IH,2*J)) 11,11,12
C   LV(J)=.FALSE.
C   GOTO 13
C   LV(J)=.TRUE.
C   KEY(J)=.FALSE.
C
C   SET COUNTERS FOR SELECTIVE OUTPUT
C   KHCT=1+IS*5
C   KSCT=1+IS*5
C   KQCT=1+IS*7
C
C   MASTER LOOP TIME
C   DO 23 NN=3,NMAXH,2
C   H(IH,1)=HDA(NN-2)
C   H(KH,1)=HDA(NN)

```

11
12

```

Q(IQ,JMAXQ)=QJ3(NJ-1)
Q(KQ,JMAXQ)=QJ6(NN+1)
C
C CONTINUITY EQUATION
DO 24 J=3,JMAXH+2
FRUIS = FC(J)+F+(J+1)
GO TO (770,770,760,770,770),CASE
760 B(J)= BB(J)+ 2.*(D(J)+H(I+J))*SS(J)
770 H(KH,J)=H(I+J)-(Q(I,J+1)-FRDIS)*D1/B(J)
24 CONTINUE
C GET RHO
DO 8020 J=1,JMAXH+2
RHO(J)=RHOS*(1,J)+1.000.
C
C MOMENTUM EQUATION
DO 25 J=2,JJTEM+2
JJTEM =JMAXQ-2 IN THIS CASE
GO TO (790,790,780,790,790),CASE
780 HTEM=D(J)+.5*(H(KH,J-1)+H(KH,J+1))
B(J)= BB(J)+2.*HTEM*SS(J)
BS(J)= BB(J)+HTEM*SS(J)
CALC. DIST. SURFACE TO CENTROID
ZD(J)=HTEM*(2.*BS(J)+B(J))/6./BS(J)
Z(J)=BS(J)*HTEM
WP(J)= BB(J)+2.*HTEM*ZZ(J)
GO TO 8000
790 Z(J)=BS(J)*J(J)+.5*(H(KH,J+1)+H(KH,J-1))
WP(J)=2.*D(J)+BS(J)+1*(KH,J+1)+H(KH,J-1)
CALC. DIST. SURFACE TO CENTROID
ZD(J)=(Z(J)+BS(J))*J.5
R(J)=Z(J)/WP(J)
C TEST FOR NEGATIVE DEPTH AS AN EVIDENCE OF INSTABILITY
IF(Z(J) .LT. J) CALL ERR1(9)
C(J)=1.486*(R(J)*P)/MANCC(J)
E1=02/Z(J)
E2=.5/(C(J)**2)*Z(J)**2)*R(J)

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MARC 73
MARC 74
MARC 75
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MARC 100
MARC 101
MARC 102
MARC 103
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MARC 105
MARC 106
MARC 107
MARC 108
MARC 109

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E3=2.*B(J) /G*(Z(J)**2))
E4=.25*(H(KH,J+1) +H(KH,J-1))-H(IH,J+1)-H(IH,J-1))/DELT
FRDIS=(FQ(J)+FQ(J+1))/DX2
E5=FRDIS/(G*(Z(J)**2))
D6= ZD(J)/DELX*(RHJ(J-1)-RHC(J+1))/(RHO(J-1)+RHO(J+1))
Q(KQ,J)=(Q(IQ,J)*(E1-ABS(Q(IQ,J)))*E2+E3*E4-E5)+D3*(H(KH,J-1)-
1H(KH,J+1))+W(J)/R(J)+D4(J)+D6)/(E1+E2*ABS(Q(IQ,J)))
25 CONTINUE
C
C AREA CALCULATION
DO 10 J=1,J2LIM,2
GOTO (3001,3005,3011,3001),CASE
DO 6 IT=1,2
HTEM=D(J)+H(IT,J)
A(IT,J)= (BB(J)+HTEM*SS(J))*HTEM
GOTO 10
3 IF(B(J) .EQ. BS(J)) GOTO 3001
DO 4 IT=1,2
DPPN=DP(J)+H(IT,J)
IF(DPPN .LT. J) CALL ERKL(10)
A(IT,J)=B(J)*DPPN+DAREA(J)
GOTO 10
3001 DO 3002 IT=1,2
3002 A(IT,J)=BS(J)*((J)+H(IT,J))
10 CONTINUE
C CALCULATE DISPERSION COEFFICIENTS
CALL TDISP
C OCEAN BOUNDY CONDITION
CALL TOCN3(CJ,CJ,F,(NN-1)/2,NMD2)
C
C CALCULATE COEFFICIENTS
DO 7 M=2,JL4M1
M2=2*M-1
M2M2=M2-2
M2P2=M2+2
A2M=(A(Z, Y2)+A(1,M2))/2.
MARC 109
MARC 110
MARC 111
MARC 112
MARC 113
MARC 114
MARC 115
MARC 116
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MARC 120
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MARC 143
MARC 144

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MARC 181
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MARC 214
MARC 215
MARC 216

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851 IF(ISW6 .EQ. J) GOTO 852
IF(IN .NE. KSCT) GOTO 852
CALL SPEM(2,2)
KSCT=KSCT+ISW6
852 IF(ISW7 .EQ. J) GOTO 86
IF(IN .NE. KQCT) GOTO 86
CALL SPEM(3,2)
KQCT=KQCT+ISW7
C
C MASS BALANCE
SUM=J.
DO 31 J=3,J2LIM,2
SUM=SUM+S(K,J)*I(K,J)
SUM=SUM+.5*S(K,I)+A(K,1)
TSUM=TSUM+SUM
C
C TEST TO SEE IF JUPUT TO A SEQUENTIAL DATASET IS SPECIFIED
IF(ISW1 .EQ. 1) GOTO 4000
OUTPUT TIME STEP RESULTS TO FILE 'MMM'
DO 400 J=1,JMAXF,2
HKH(J)=F(K,J)-SLD+Z0(J)+D(J)
WRITE(MMM) (HKH(J),J=1,JMAXF,2)
WRITE(MMM) (S(K,J),J=1,JMAXH,2)
WRITE(MMM) (J(K,J),J=2,JMAX,2)
C
C TEST TO SEE IF HAS OUTPUT HAS BEEN SPECIFIED
IF(ISW2 .EQ. 1) GOTO 4001
HIGH WATER SLACK CALCULATIONS
DO 40 J=1,JLIM
IF(KEY(J)) GOTO *J
SIGN OF NEW 'Q' TJ LP
IF(Q(KH,2*J)) 41,41,42
LP=.FALSE.
GOTO 43
42 LP=.TRUE.
C TEST FOR HIGH WATER SLACK BY ZERC CROSSING OF Q, + TO -

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MARC 217
 MARC 218
 MARC 219
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 MARC 250
 MARC 251
 MARC 252

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43 IF(LV(J).AND.(INDI.LPI).GOTC 44
C   NOT HWS, SET UP FOR NEXT TRY
LV(J)=LP
GOTO 40
C   FCUNC THE HWS
44 IHWS(J)=NN
KEY(J)=.TRUE.
HWS(J)=S(KH,2*(J-1)
CENTINUE
40
C
C   SAVE H AND W VALUES AT THE OCEAN FOR PRISM CALCS
4001 ISUB=(NN-1)/2
HH(ISUB)=0.5*(H(KH,1)+H(KH,3)) + D(2)
QQ(ISUB)=Q(KH,2)
C   END OF MAIN LOOP
C   SWAP BEFORE AND AFTER INDICES.
ITTP=IH
IH=KH
KH=ITTP
23 CONTINUE
C
C   TSUM=TSUM*DX2/NMDZ
WRITE(KC,503) TSUM
FORMAT(I10,'AVE MASS =',F16.6)
503
C
IF(ISW2.EQ.1) GOTO 46
C   OUTPUT HWS
OC 405 J=1,JLIM
C   IF HWS UNDEFINED, FILL WITH 999
IF(KEY(J)) GOTO 435
IHWS(J)=999
HWS(J)=999.
405 CCNTINUE
CALL SPEW(4,3)
C
C   CALL TJ PRISM ROUTINE
  
```


46 CALL ESND(JT2,NMDZ,IREC,D(2),B(2),MANCO(2),QF,EEZ,HH,CQ,SZERO,XL)MARC 253
EZ=EEZ MARC 254
RETURN MARC 255
END MARC 256


```

DO 10 J=1,J2LIM,2
IF(J-1) 16,16,17
C1 BY CC CONTINUITY
H2OLD=0.5*(H(M,1)+H(M,3))
H2NEW=0.5*(H(K,1)+H(K,3))
Q1=Q(M,2)+FAC4*(B(1)*(H(K,1)-H(M,1))+B(2)*(H2NEW-H2OLD))
QODD=ABS(Q1)
GOTC 18
NORMAL ... INTERIOR PCINTS
QODD=C.5*ABS(Q(M,J-1)+Q(M,J+1))
HTEM=D(J)+0.5*(H(1,J)+H(2,J))
GOTO(13,13,14,13,13),CASE
TRAPEZOIDAL SECTION
AR=C.5*(A(1,J)+A(2,J))
R(J)=AR/(BB(J)+2.*HTEM*ZZ(J))
GOTO 15
OTHER SECTIONS
AR=BS(J)*HTEM
R(J)=AR/(BS(J)+HTEM+HTEM)
V=QODD/AR
DZERC(J)=77.*MANCO(J)*(R(J)**0.8333333)*V*ETM
CONTINUE
C
C GET EXK ON THE EVEN INTERVALS
DO 11 J=1,J2M2,2
TEST FOR SEAWARD POINT NEG.
SD=S(M,J)
IF(SD) 9,9,8
EXK(J)=C.
GOTO 11
EXK(J)=XK*ABS(SE-S(M,J+2))
CONTINUE
INTERPOLATE TO EVEN INTERVALS
DO 12 J=1,J2M2,2
E(J+1)=EXK(J)+C.5*(DZERO(J)+DZERO(J+2))

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TDIS 37
TDIS 38
TDIS 39
TDIS 40
TDIS 41
TDIS 42
TDIS 43
TDIS 44
TDIS 45
TDIS 46
TDIS 47
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TDIS 63
TDIS 64
TDIS 65
TDIS 66
TDIS 67
TDIS 68
TDIS 69
TDIS 70
TDIS 71
TDIS 72

```

```
20 E(1)=EXK(1)+DZERO(1)  
30 GOTC(30,20),ISM4  
    WRITE(NMN ) E(1),(E(J-1),J=3,J2L[M,2])  
    RETURN  
    END
```

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TDIS 73  
TDIS 74  
TDIS 75  
TDIS 76  
TDIS 77
```

TOCN 1
 TOCN 2
 TOCN 3
 TOCN 4
 TOCN 5
 TOCN 6
 TOCN 7
 TOCN 8
 TOCN 9
 TOCN 10
 TOCN 11
 TOCN 12
 TOCN 13
 TOCN 14
 TOCN 15
 TOCN 16
 TOCN 17
 TOCN 18
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 TOCN 27
 TOCN 28
 TOCN 29
 TOCN 30
 TOCN 31
 TOCN 32
 TOCN 33
 TOCN 34
 TOCN 35
 TOCN 36

```

SUBROUTINE      TOCNB (CD,CU,F,L,NMD2)
INTEGER CASE
REAL FC(200), BB(200), D(200), SS(200), B(200), MANCC(200)
REAL ZZ(200), R(200), C(200), D4(200), W(200)
REAL BS(200), E(200), H(2,200), S(2,200), Q(2,200)
REAL HOA(900), QOB(900)
DIMENSION CD(1), CU(1), F(1)
DIMENSION A(2,200)
COMMON KO, KIN, KKK, JJJ
COMMON NDS, CASE, DELT, DELX, NMAX, JMAX, JMAXH, JJTEM, JMAXQ
COMMON CI, D2, D3, EH, EZ, EXLM, SZERG, ES
COMMON MH, MS, ISW0, ISW1, ISW2, ISW3, ISW4, ISW5, ISW6, ISW7
COMMON H, Q, HOA, QOB, FQ, BB, C, SS, B, ZZ, R, MANCO, C, D4, W, BS, S, E
COMMON /T/ MMM, CF, IPF, IPE
COMMON /AREAS/ A, Q1
COMMON /EXLIM/ JDI, JD2, K, M
DATA INIT/1/
DATA THETA/0.333333/

      C      INITIALIZE SIGN OF PREVIOUS DISCHARGE, SMIN, COUNTER OF
      C      INCREMENTS SINCE FLOOD BEGAN
      C      GOTO(1,2), INIT
      C      IF THE FIRST CYCLE BEGINS ON FLOOD SMIN MUST BE
      C      ARBITRARILY DEFINED. IT IS SET TO ZERO FOR FIRST CYCLE ONLY
      C
      C      QLST=1.
      C      SMIN=SZERO
      C      NMAXM=NMAX-1
      C      NFLD=NMD2/2
      C      NEBB=0
      C      DXCT=DELX/DELT
      C      CC4=2.*(1.-THETA*0.5)*DXCT
      C      CF4=2.*(THETA*0.5)*DXDT
      C      INIT=2
      C
      C      NORMAL ENTRY
      C      IF(QLST*Q(M,2)) 10,20,20
  
```

C	IC	CHANGE OF SIGN	TOCN 37
C	11	IF(Q(M,2)) 11,12,12	TOCN 38
		TO EBB	TOCN 39
		NEBB=0	TOCN 40
		GOTO 21	TOCN 41
		TC FLOOD	TOCN 42
		NFLD=1	TOCN 43
		SMIN =S(M,1)	TOCN 44
		GOTO 22	TOCN 45
		NO SIGN CHANGE	TOCN 46
		IF(Q(M,2)) 21,22,22	TOCN 47
		FLOOD	TOCN 48
		IF(NFLD-IPF) 23,24,24	TOCN 49
		WITHIN RANGE	TOCN 50
		SX=SMIN+(SZERO-SMIN)*NFLD/FLOAT(IPF)	TOCN 51
		NFLD=NFLD+1	TOCN 52
		GOTO 77	TOCN 53
		OUT OF RANGE	TOCN 54
		SX=SZERC	TOCN 55
		GOTO 77	TOCN 56
		EBB	TOCN 57
		IF(NEBB-IPE) 25,26,26	TOCN 58
		WITHIN RANGE	TOCN 59
		NEBB=NEBB+1	TOCN 60
		SX=SZERC	TOCN 61
		GOTO 77	TOCN 62
		OUT OF RANGE *** SPECIAL ROUTINE ***	TOCN 63
		Q1 AND AREAS FROM COMMON	TOCN 64
		A18=(A(2,1)+A(1,1))*0.5	TOCN 65
		A38=(A(2,3)+A(1,3))*0.5	TOCN 66
		E8=0.5/DELX*(E(1)*A18-E(2))*(A18+A38)*0.5	TOCN 67
		CD(1)=CC4*A(K,1)-Q1-Q1+Q(M,2)-EB	TOCN 68
		CU(1)=CF4*A(K,3)+Q(M,2)+EB	TOCN 69
		F(1)=S(M,1)*(CC4*A(M,1)+Q1+Q1-Q(M,2)+EB)+	TOCN 70
			TOCN 71
			TOCN 72

```

1      S(M,3)*(CF4*A(M,3)-Q(M,2)-EB)
      GOTC 80
      CALCULATE COEFFICIENTS
      F(1)=SX
      CD(1)=1.0
      CU(1)=0.
      HOUSEKEEP
      QLST=C(M,2)
      RETURN
      END
C
77
C
EC

```

```

TOCN 73
TOCN 74
TOCN 75
TOCN 76
TOCN 77
TOCN 78
TOCN 79
TOCN 80
TOCN 81
TOCN 82

```

C	SUBROUTINE TRIDG (N,A,B,C,D)	1	TRID
C	ALGORITHM 24 OF ACM... BY B. LEAVENWORTH	2	TRID
	TRANSLATED TO FORTRAN IV BY M.L. THATCHER	3	TRID
	REAL W, A(100),B(100),C(100),D(100)	4	TRID
	INTEGER J,N,NMI,I	5	TRID
	D(1)=C(1)/B(1)	6	TRID
	W=B(1)	7	TRID
	NMI=N-1	8	TRID
	DO 1 J=1,NMI	9	TRID
	B(J)=C(J)/W	10	TRID
	W= B(J+1)-A(J)*B(J)	11	TRID
1	D(J+1)=(D(J+1)-A(J)*D(J))/W	12	TRID
	DO 2 J=1,NMI	13	TRID
	I=N-J	14	TRID
2	D(I)=D(I)-B(I)*C(I+1)	15	TRID
	RETURN	16	TRID
	END	17	TRID


```

SUBROUTINE ESNOD(OT,NT,NX,C,B,XN,FRESH,EEZ,H,Q,S,XL)
  1 ESNO
  2 ESNO
  3 ESNO
  4 ESNO
  5 ESNO
  6 ESNO
  7 NESNO
  8 ESNO
  9 ESNO
 10 ESNO
 11 ESNO
 12 ESNO
 13 ESNO
 14 ESNO
 15 ESNO
 16 ESNO
 17 ESNO
 18 ESNO
 19 ESNO
 20 ESNO
 21 ESNO
 22 ESNO
 23 ESNO
 24 ESNO
 25 ESNO
 26 ESNO
 27 ESNO
 28 ESNO
 29 ESNO
 30 ESNO
 31 ESNO
 32 ESNO
 33 ESNO
 34 ESNO
 35 ESNO
 36 ESNO

  THIS IS A MODIFICATION OF SUBROUTINE 'PRISM'
  WHICH CONVERTS THE ESTUARY NC. TC A DENSIMETRIC ONE AND
  EVALUATES THE GROSS DISPERSION PARAMETER 'K' IN TERMS
  OF (U*L)
  CALCULATION OF TIDAL PRISM, MAX. VELOCITY, DENSIMETRIC FROUDE
  Q(F) AND THE ESTUARY NUMBER(DENSIMETRIC)
    Q(450),H(450),V(450)
  REAL
  COMMON KO
  COMMON /CEPT/ XNCPT
  CGNST FOR CONVERTING SALINITY TO DENSITY DEFINED BELOW
  DATA TCCNS/0.75/
  VOLB(Q1,Q2)=Q2*Q2*DT*0.5/(Q2-Q1)
  VOLN(Q1,Q2)=0.5*(Q1+Q2)*DT
  VOLL(Q1,Q2)= Q1*Q1*DT*0.5/(Q1-Q2)

  PT=0.
  DRQ=TCONS*S/1000.
  I=1
  IF(Q(I)) 1,2,2
  STARTS GOING NEGATIVE
  I=I+1
  IF(Q(I) .LT. 0.) GOTO 1
  PT=VOLB(Q(I-1),Q(I))
  I=I+1
  IF(I .GT. NT) GOTO 30
  IF(Q(I) .LT. 0.) GOTO 12
  PT=PT+VOLN(Q(I-1),Q(I))
  GOTO 11
  PT=PT+VOLL(Q(I-1),Q(I))
  GOTO 30
  STARTS GOING POSITIVE
  I=I+1
  IF(Q(I) .LT. 0.) GOTO 2C
  PT=PT+VCLN(Q(I-1),Q(I))

```

20	GOTC 2	ESNO 37
21	PT=PT+VOLL(Q(I-1),Q(I))	ESNO 38
	I=I+1	ESNO 39
	IF(I .GT. NT) GOTO 30	ESNO 40
	IF(Q(I) .LT. 0.) GOTO 21	ESNO 41
22	PT=PT+VOLB(Q(I-1),Q(I))	ESNO 42
	I=I+1	ESNO 43
	PT=PT+VCLN(Q(I-1),Q(I))	ESNO 44
	IF(I .LT. NT) GOTO 22	ESNO 45
C		ESNO 46
C	FIND VMAX	ESNO 47
30	NTM1=NT-1	ESNO 48
	DO 40 I=1,NTM1	ESNO 49
40	V(I)=Q(I)/B/(H(I)+H(I+1))*2.	ESNO 50
	V(NT)=Q(NT)/B/(H(NT)+H(1))*2.	ESNO 51
	VMAX=0.	ESNO 52
	DO 41 I=1,NT	ESNO 53
	IF(ABS(V(I)) .LT. VMAX) GOTO 41	ESNO 54
	IH=I	ESNO 55
	VMAX=ABS(V(I))	ESNO 56
41	CONTINUE	ESNO 57
	TIME=IH*DT/3600.	ESNO 58
200	WRITE(KD,200) PT,VMAX,TIME	ESNO 59
	FORMAT('OPRISM=',E16.7,' CUFT',16X,'VMAX=',F10.4,' FT/SEC',3X,	ESNO 60
	'AT',F7.2,' HRS FROM BEGINNING OF CYCLE')	ESNO 61
	1 IF(IH .EQ. NT) H(NT+1)=H(I)	ESNO 62
	RH=B*(H(IH)+H(IH+1))*0.5/(B+H(IH)+H(IH+1))	ESNO 63
	FR=VMAX/SQRT(32.2*0)	ESNO 64
	FRDS=FR/SQRT(DRC)	ESNO 65
202	WRITE(KC,202) FRDS,FRESH	ESNO 66
	FORMAT(' DENSIMETRIC FROUDE NO.=',F10.6,10X,	ESNO 67
	'SUM OF QF'S=',F10.3,' CU.FT/SEC')	ESNO 68
	1 T=NT*DT	ESNO 69
	ESNC= PT*FR*FR/FRESH/T/DRO	ESNO 70
C	CORRELATION FOR K/(UL)=F(ESNC)	ESNO 71
	EEZ=VMAX*XL*XNCPT *(ESNC**(-.25))	ESNO 72

ESND 73
ESND 74
ESND 75
ESND 76
ESND 77

```
203 WRITE(KC,203) ESND,EEZ  
    FORMAT(' DENSIMETRIC ESTUARY NO.=',F9.3,10X,'K FOR THE NEXT',  
1      ' CYCLE IS',F10.2,' SQFT/SEC')  
    RETURN  
    END
```

```

C
C
C
C
C
SUBROUTINE SPEW(KEY,IL)
ARGUMENT 'KEY' IS 1 FOR H, 2 FOR S, 3 FOR Q, AND 4 FOR HWS
ARGUMENT 'IL' IS 1 FOR FINAL CCNDITION OUTPUT
2 FOR NCAMAL OUTPUT
3 FOR HWS
4 FOR I.C.

INTEGER CASE
REAL FC(200), RR(200), D(200), SS(200), B(200), MANCC(200)
REAL ZZ(200), R(200), C(200), D4(200), W(200)
REAL BS(200), E(200), H(2,200), S(2,200), Q(2,200)
REAL HOA(900), QCB(900)
DIMENSION HWS(100), ZC(200), CUM(200), IHWS(100), KNO(200)
COMMON KO,KIN,KKK,JJJ
COMMON NDS,CASE,DELX,NMAX,NMAXH,JMAX,JMAXH,JJTEM,JMAXQ
COMMON D1,D2,D3,EH,EZ,EXL,SZERO,ES
COMMON MH,MS,ISW0,ISW1,ISW2,ISW3,ISW4,ISW5,ISW6,ISW7
COMMON H,Q,HOA,QCB,FC,BB,D,SS,B,ZZ,R,MANCO,C,D4,W,BS,S,E
COMMON /EXLIM/ JLIM,J2LIM,K,M
COMMON /SL/SLD,ZC
COMMON /OUTP/ NC,NN,HWS,NTCT,IHWS
DATA INIT/1/

C
C
C
C
C
NUMBER SEQUENTIAL ARRAY
GOTO(1000,2000),INIT
DO 1001 J=1,200
1001 KNO(J)=J
INIT=2
T=(NMAX-1)*DELT/3600.
TFAC=T/(NMAX-1)

C
C
C
C
C
SET UP THE 'FROM WHENCE' CCNDITIONS
GOTO(2001,2002,2003,2004),IL
2000 FROM MAIN, OUTPUT FINAL CCNDITIONS
2001 NP=1
IT=M
NTC=NTCT

```

```

SPEW 1
SPEW 2
SPEW 3
SPEW 4
SPEW 5
SPEW 6
SPEW 7
SPEW 8
SPEW 9
SPEW 10
SPEW 11
SPEW 12
SPEW 13
SPEW 14
SPEW 15
SPEW 16
SPEW 17
SPEW 18
SPEW 19
SPEW 20
SPEW 21
SPEW 22
SPEW 23
SPEW 24
SPEW 25
SPEW 26
SPEW 27
SPEW 28
SPEW 29
SPEW 30
SPEW 31
SPEW 32
SPEW 33
SPEW 34
SPEW 35
SPEW 36

```



```

12      GOTO 70
202     WRITE(KD,202) (KNC(J),J=NS,NF,2)
        FORMAT(' STATIONS',I5,13IE)
        IF(IL.EQ.2) GOTO 1212
203     WRITE(KC,203) (P(IT,J),J=NS,NF,2)
        FORMAT(' ELEV.',I,14F6.3)
        GOTO 80
1212    WRITE(KD,203) (DUM(J),J=NS,NF,2)
        GOTO 80
C      S- SALINITY
2       WRITE(KC,220)
220     FORMAT('+',T65,'INSTANTANEOUS SALINITIES IN PPT.')
        JM=J2LIN
        GOTO 70
21      WRITE(KD,202) (KNC(J),J=NS,NF,2)
207     WRITE(KC,207) (S(IT,J),J=NS,NF,2)
        FORMAT(' SALINITY',14F6.3)
        GOTO 80
C      Q- DISCHARGE
3       WRITE(KC,230)
230     FORMAT('+',T65,'INSTANTANEOUS DISCHARGES IN CFS')
        NS=2
        NF=20
23      IF(NF.LT.JMAX) GOTO 31
        ISTOP=2
        NF=JMAX
31      WRITE(KD,205) (KNC(J),J=NS,NF,2)
205     FORMAT(' STATIONS',2X,I5,9I10)
204     WRITE(KC,204) (Q(IT,J),J=NS,NF,2)
        FORMAT(' DISCHARGE ',10F10.0)
        IF(ISTOP.EQ.2) RETURN
        NS=NF+2
        NF=NS+18
        GOTO 33
C      S- HWS
4       WRITE(KC,240)

```

```

SPEM 73
SPEM 74
SPEM 75
SPEM 76
SPEM 77
SPEM 78
SPEM 79
SPEM 80
SPEM 81
SPEM 82
SPEM 83
SPEM 84
SPEM 85
SPEM 86
SPEM 87
SPEM 88
SPEM 89
SPEM 90
SPEM 91
SPEM 92
SPEM 93
SPEM 94
SPEM 95
SPEM 96
SPEM 97
SPEM 98
SPEM 99
SPEM 100
SPEM 101
SPEM 102
SPEM 103
SPEM 104
SPEM 105
SPEM 106
SPEM 107
SPEM 108

```

```

240 FORMAT('+',T65,'HIGH WATER SLACK SALINITIES IN PPT')
    JM=J2LIM
    C   GET TIME OF HWS IN HOURS FRCH BEGINNING OF CYCLE
    C   AND SET THE CODE IN CASE SCHE HWS NOT DEFINED
        ICODE=0
        DO 42 J=1,JLIM
          IF(IHWS(J) .NE. 999)   GOTC 45
          DUM(J)=599.
          HWS(J)=599.
        C   THIS WILL CAUSE 999. TO PRINT ON OUTPUT
        C   SET THE CODE
          ICODE=1
          GOTC 42
        45 DUM(J)=(IHWS(J)-1)*TFAC
        42 CONTINUE
        242 IF(ICODE .EQ. 1) WRITE(KO,242)
            FORMAT(' 999. INDICATES      THAT NO HWS WAS FOUND FOR THAT STATION')
            1 , )
            GOTC 70
        41 WRITE(KO,202) (KNC(J),J=NS,NF,2)
            C   AS HWS IS STORED CCMPACTLY, REFORMULATE INICES
            NS1=(NS+1)/2
            NF1=(NF+1)/2
            WRITE(KO,243) (DUM(J),J=NS1,NF1)
        243 FORMAT(' TIME HRS',I4F8.3)
            WRITE(KO,207) (HWS(J),J=NS1,NF1)
            GOTC 80
            C   GET INDICES
            C   NS=1
            C   NF=27
            C   75 IF(NF .LT. JM)   GOTC(12,21,55,41),KEY
            C   NF=JM
            C   ISTOP=2
            C   GOTC(12,21,55,41),KEY
            C   80 IF(ISTOP .EQ. 2) RETURN

```

```

SPEW 109
SPEW 110
SPEW 111
SPEW 112
SPEW 113
SPEW 114
SPEW 115
SPEW 116
SPEW 117
SPEW 118
SPEW 119
SPEW 120
SPEW 121
SPEW 122
SPEW 123
SPEW 124
SPEW 125
SPEW 126
SPEW 127
SPEW 128
SPEW 129
SPEW 130
SPEW 131
SPEW 132
SPEW 133
SPEW 134
SPEW 135
SPEW 136
SPEW 137
SPEW 138
SPEW 139
SPEW 140
SPEW 141
SPEW 142
SPEW 143
SPEW 144

```

```
SPEM 145  
SPEM 146  
SPEM 147  
SPEM 148  
SPEM 149  
SPEM 150  
SPEM 151
```

```
NS=NF+2  
NF=NS+26  
GOTO 75  
WRITE(KO,255)  
FORMAT(' PROG. ERR. IN SPEM')  
CALL EXIT  
END
```

```
55  
255
```



```

SUBROUTINE ERRL(KEY)
COMMON KD
C
C      THIS IS AN ERROR HANDLING ROUTINE, IT ISSUES MESSAGES TO HELP
C      THE USER IDENTIFY THE CAUSE OF INPUT AND OTHER ERRORS
DATA JINX/0/
IF(KEY .EQ. 8)      GOTO 8
WRITE(KC,200)
FORMAT('0 *** SALINITY INTRUSION PROGRAM ERROR ***')
IF((KEY .LT. 1) .OR. (KEY .GT. 10))  GOTO 60
GOTO(1,2,3,4,5,6,7,8,9,10),KEY
WRITE(KC,201)
FORMAT(' NUMBER OF SECTIONS MUST BE EVEN')
GOTC 500
WRITE(KC,202)
FORMAT(' NUMBER OF TIME INTERVALS MUST BE EVEN')
GOTC 500
WRITE(KC,203)
FORMAT(' CASE IS NOT 1 THRU 5')
GOTC 555
WRITE(KC,204)
FORMAT(' NEGATIVE INTRUSION LENGTH IN INPUT')
GOTO 555
WRITE(KC,205)
FORMAT(' COLUMN 1 DOES NOT CONTAIN H,Q OR S')
GOTC 500
WRITE(KC,206)
FORMAT(' SEQUENCING ERROR IN INPUT STREAM')
GOTO 555
WRITE(KC,207)
FORMAT(' INTERVAL FOR OUTPUT MUST BE EVEN',/)
C
C      CHECK FOR INPUT ERRORS
IF(JINX .NE. 0) GOTO 8C
WRITE(KC,208)
FORMAT('1 CALCULATION BEGINS',/)

```

```

ERRL 1
ERRL 2
ERRL 3
ERRL 4
ERRL 5
ERRL 6
ERRL 7
ERRL 8
ERRL 9
ERRL 10
ERRL 11
ERRL 12
ERRL 13
ERRL 14
ERRL 15
ERRL 16
ERRL 17
ERRL 18
ERRL 19
ERRL 20
ERRL 21
ERRL 22
ERRL 23
ERRL 24
ERRL 25
ERRL 26
ERRL 27
ERRL 28
ERRL 29
ERRL 30
ERRL 31
ERRL 32
ERRL 33
ERRL 34
ERRL 35
ERRL 36

```

```

8C      RETURN
2C80    WRITE(KO,2C80) JIAX
9        FORMAT('0',I3,' INPUT ERRCRS DETECTED, CALCULATION ABORTED',/)
209     CALL EXIT
        WRITE(KO,209)
        FORMAT(' WATER SURFACE BELOW THE BOTTOM WHICH INDICATES A ',
1         ' PROBABLE NUMERICAL INSTABILITY.',/, ' THE REMEDY MAY WELL ',
2         ' BE A SMALLER DELTA-T.')
        GOTO 555
10      WRITE(KC,210)
210     FORMAT(' THE WATER SURFACE IN THE STORAGE AREA HAS FALLEN BELOW ',
1         ' THE BOTTOM. THIS IS NOT NECESSARILY AN INSTABILITY.',/,
2         ' BUT IS PROBABLY A PROBLEM IN SCHEMATIZATION')
        GOTO 555
6C      WRITE(KC,298) KEY
298     FORMAT(' ERROR CODE UNRECOGNIZED, IT IS',I10)
555     GOTO 555
299     WRITE(KC,299)
        FORMAT('0 FATAL ERROR, EXIT TAKEN',/)
5C0     CALL EXIT
        JINX=JINX+1
        RETURN
        END
ERRL 37
ERRL 38
ERRL 39
ERRL 40
ERRL 41
ERRL 42
ERRL 43
ERRL 44
ERRL 45
ERRL 46
ERRL 47
ERRL 48
ERRL 49
ERRL 50
ERRL 51
ERRL 52
ERRL 53
ERRL 54
ERRL 55
ERRL 56
ERRL 57
ERRL 58
ERRL 59

```

```
C  
SUBROUTINE SEQST(J, ISEQ)  
  TEST SEQUENCING  
  ITST1=ISEQ-J  
  IF(J .EQ. 1) ITST=ITST1  
  IF(ITEST .NE. ITST1) CALL ERR1(6)  
  RETURN  
  END
```

```
SEQT  
SEQT  
SEQT  
SEQT  
SEQT  
SEQT  
SEQT  
1  
2  
3  
4  
5  
6  
7
```

Appendix II

Brief Description of Formats Used in Input

Fields are of three types: characters, integers and numbers with decimal points. A field is always given a width. Field specifications can be repeated n times by placing n in front of the field specification. For example 20A4 repeats a character field (A) of field width 4 a total of 20 times. This means effectively that all 80 columns (20×4) of the card are available for character information. The formats used here are:

- Aw - a character field of width "w"
- Iw - an integer field of width "w"
- Fw.d - a decimal number field of width "w" and with a decimal point and "d" digits after the decimal point
- nX - a spacing between fields of "n" columns

(There is one Ew.d format, but it will not be discussed.)

The integer specification contains the restraint that the number punched must be located all the way to the right of the field. This is called "right justified". For example the number 7 in a field of 15 must be punched in the 5th column. (The machine takes blank columns as zeros.)

The decimal specification requires a decimal point, but the location of the number within the field is completely up to the user as long as the decimal point is included.

As a final example the format 2F10.d, I5,5X, F10.d would require the first two fields to be decimal numbers of 10 columns each, then a 5 column field for an integer number, then skip 5 columns and finally a last decimal field of 10 columns.

Appendix III

A Plotting Program Using a CALCOMP Plotter

III.1 Introduction

As the results produced by the computer program are essentially a time series of elevations, η , salinities, S , and discharges Q at each station the presentation of results in graphical form is an obvious advantage. The CALCOMP plotter is especially well suited for such an application because it is widely available and because it is a drum plotter permitting an abscissa of great length.

The plotting program is written in FORTRAN IV and utilizes the CALCOMP subroutines described fully in Ref. 4.

The subroutines used are modifications of those described in Ref. 5, however the modifications are minor. Most data processing installations with CALCOMP plotter facilities have made some modifications to the basic subroutines and the user is advised to consult the programming staff of his particular installation for assistance in rendering the plotting program compatible.

III.2 Instructions for Use of the Plotting Program

The user must have specified the KEEP option in card E1 of the Salinity Intrusion Program, thereby generating a sequential data set such as a magnetic tape file which will serve as the source of data for the plotting program.

The user can plot any number of frames, each frame will consist of up to 8 curves of one variable, η , S or Q and any number of individual points corresponding to each curve.


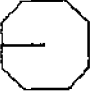
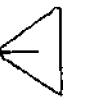

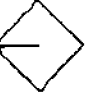
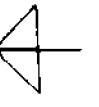


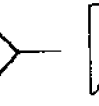
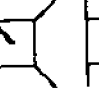
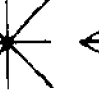





The input data and its formats are described as follows:

<u>Card</u>	<u>Description</u>	<u>Format</u>
1	The number of frames, right justified For each frame:	I5
A1	Field 1: (right justified) Number of Sections (even) Field 2: Total Length in Feet	I5 F10.d
A2	Field 1: (right justified) Number of Time Intervals per Cycle (even). Field 2: Tidal Period in Seconds	I5 F10.d
A3	Field 1: Number of Tidal Cycles Being Plotted. Field 2: Plot Length in Inches	I5 F10.d
A4	Field 1: Maximum Ordinate (η , S or Q)	F10.d

<u>Card</u>	<u>Description</u>	<u>Format</u>
	Field 2: Plot height in inches which corresponds to the maximum ordinate	F10.d
A5	Field 1: The letter H, S, or Q which determines the variable to be plotted (Elevation = H, Salinity = S, and Discharge = Q.)	A1
	Field 2: Number of Stations to be Plotted	I4
	Field 3: Place a number in Column 10 if points are to be plotted from card input. If left blank just the curves are plotted.	I5
A6	Fields 1 through 8: In each field starting from the left, punch the location of the station(s) to be plotted (feet from estuary entrance).	8F10.d
	If points are to be plotted as well as the curves, then supply points corresponding to <u>each</u> curve as follows:	
B1	Field 1: Number of Points of a Particular Symbol	I5
	Field 2: Symbol Code Described in Figure III.1.	I5
	For each point a card:	
C1,2 etc.	Field 1: Time of observation in days, assuming that the 1.0 days means 1200 hrs. from the beginning of the first tidal cycle	F10.d
	Field 2: Value of the observation	F10.d

An **example** of such a plot is given by the following listing which is of the FORTRAN program and input deck. The resulting plot is shown in Figure III.2.

SPECIAL SYMBOLS

-1		-2		-3		-4	+	-5		-6	
-7		-8		-9		-10		-11		-12	
-13		-14		-15		-16		-17		-18	

III-3

Figure III.1 Symbol Codes for M.I.T. Calcomp Subroutines

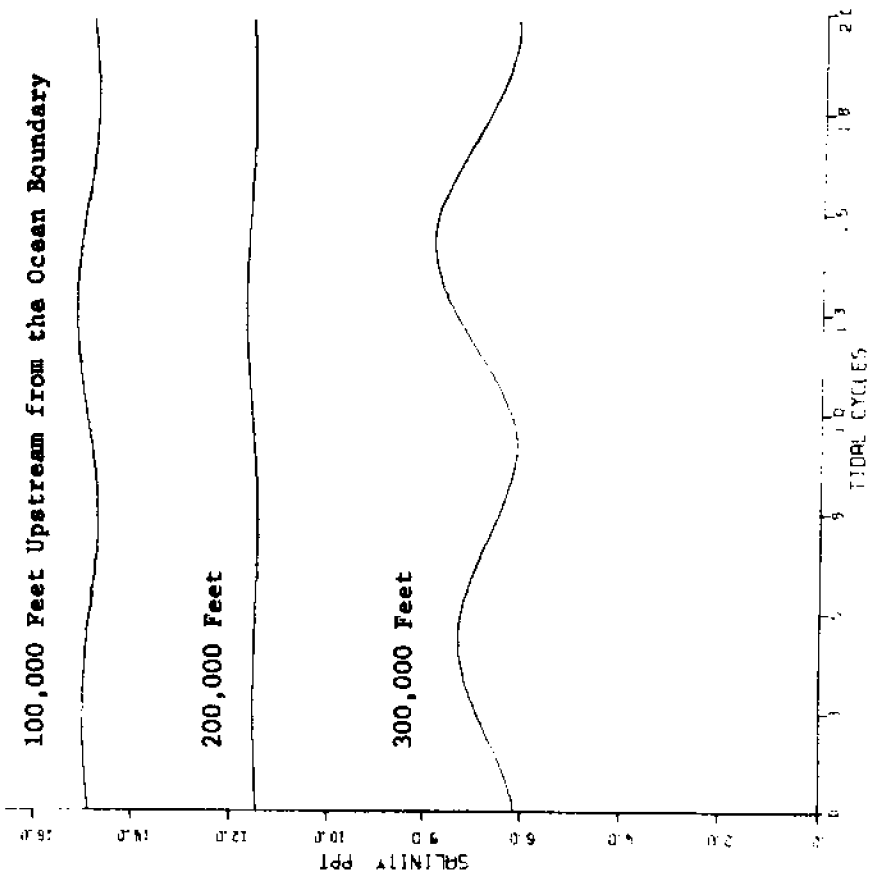


Figure III.2 Typical Plot

Table III.1

Listing of Plotting Program and Test Case

```

1 PLOT
2 PLOT
3 PLOT
4 PLOT
5 PLOT
6 PLOT
7 PLOT
8 PLOT
9 PLOT
10 PLOT
11 PLOT
12 PLOT
13 PLOT
14 PLOT
15 PLOT
16 PLOT
17 PLOT
18 PLOT
19 PLOT
20 PLOT
21 PLOT
22 PLOT
23 PLOT
24 PLOT
25 PLOT
26 PLOT
27 PLOT
28 PLOT
29 PLOT
30 PLOT
31 PLOT
32 PLOT
33 PLOT
34 PLOT
35 PLOT
36 PLOT

      A PLOT LENGTH OF ABOUT 25 INCHES AT 50 PTS./INCH IS DIMENSIONED
      IN THE MATRIX 'SAVE'
      REAL F(8),HSQ(100),Z(100),SAVE(8,1250),XC(1250)
      REAL ST(8),YLABEL(+),DLAB(3,4)
      INTEGER KID(3),NS(8)
      DIMENSION UGDS(100),VGDS(100)
      DATA DLAB/'ELEV','SALI','CISC','ATIC','NITY','HARG',
      'N F','PP','E C','T ','T ','FS ' /
      DATA KID/'H','S','R' /,KIN,KC,KU/5,6,1/
      SET PLOT DENSITY IN DATA STATEMENT, IN POINTS PER INCH
      DATA SDEN/50./
      DEFINE VARIABLE KPROG =3, FOR H,S,Q, OR =2 FOR H AND Q ONLY
      DATA KPROG/3/,ICUD/2/

      READ THE NUMBER OF FRAMES
      CALL NEWPLT('M8238','C91C','WHITE ','BLACK')
      REAC(KIN,10) NOPLT
      DO 55 IPLCT=1,NCPLT

      READ(KIN,100) NX2,ALT,NT2,Y,NTC,PL
      FORMAT(I5,F10.0)
      NX=NX2/2
      XX=2.*XLT/(NX2-1)
      IF(2#NX .EQ. NX2)
      CASE WITH ODD NUMBERED STATIONS AS UPSTREAM END BC BY ELEVATION
      I00D=1
      NX=NX+1
      NT=NT2/2
      IF(2#NT .NE. NT2) GOTO 552
      REAC(KIN,102) SCL,YL
      REAC(KIN,101) ID,NSTA,IPCINT
      FORMAT(A1,I4,I5)
      REAC(KIN,102) (ST(I),I=1,NSTA)
      FORMAT(8F10.0)

```

```

C
C      PRINT BACK IN INPUT
C      WRITE(KO,300) NTC,PL,YL
300  FORMAT('1',T7,'CALCULATED PLOTTING FCR',I5,' TIDAL CYCLES',/,
1      T3,'PLOT LENGTH =',F5.1,' INCHES HEIGHT =',F5.2,' INCHES')PLOT
301  WRITE(KC,301) NT2,NT2,SCL,XX
      FORMAT('0',I6,' TIME INCR. PER CYCLE',I6,' DISTANCE STATIONS',
1      /,I4,'MAX. JRJINATE =',F11.1,/,T4,
2      'DISTANCE BETWEEN STATIONS =',F8.0,' FEET',//
3      ,T6,'LOCATION',I19,'DISTANCE',/,T7,'NUMBER',T20,'(FEET)')
304  WRITE(KC,304) (DLAB(IN,J),J=1,4)
      FORMAT('0')PLOT OF ',4A4)
      DO 20 I=1,NSTA
302  WRITE(KC,302) I,ST(I)
      FORMAT(T9,I3,T15,F3.0)
C
C      FOR 'C' SHIFT ALIGNMENT DELTA-X/2
      IF(IN.NE.KPROG) GOTD 20
      ST(I)=ST(I)-XX/2.
20  CONTINUE
C
C      CALCULATE SAMPLING INTERVAL *IR*
      NOPTS=NTC*NT
      DEN=NOPTS/PL
      IR=1
      IF(DEN.GT.SDEN) IR=DEN/SDEN
PLOT 47
PLOT 48
PLOT 49
PLOT 50
PLOT 51
PLOT 52
PLOT 53
PLOT 54
PLOT 55
PLOT 56
PLOT 57
PLOT 58
PLOT 59
PLOT 60
PLOT 61
PLOT 62
PLOT 63
PLOT 64
PLOT 65
PLOT 66
PLOT 67
PLOT 68
PLOT 69
PLOT 70
PLOT 71
PLOT 72

```

```

303      WRITE(KC,303) IR
C        FORMAT('O SAMPLING EVERY',I4,' POINTS')
          FIND LAND SIDE JF INCR. FOR EACH STATION
DO 6 J=1,NSTA
DO 7 J=1,NX
IF(ST(I).EQ.0.) GOTO 8
IF(XX*(J-1).GE.ST(I)) GOTO 8
GOTO 7
NS(I)=J
IF(J.EQ.1) NS(I)=J+1
JM2=NS(I)-2
F(I)=(ST(I)-XX*JM2)/XX
GOTO 6
7 CONTINUE
WRITE(KC,700)
700     FORMAT('O STATION OUT OF RANGE')
CALL EXIT
CONTINUE
C
C      SET YMIN AND Y-LABEL
DO 3 I=1,4
YLAB(I)=DLAB(IN,I)
GOTO (4,5,4),IN
4 YMIN=-SCL
DY=2.*SCL/YL
GOTO 9
5 YMIN=C.
DY=SCL/YL
C      THE INCREMENT IS IN TICAL CYCLES
5 XINCR=1./NT*IR
DX=NTC/PL
XMAX=NTC
NDX=1
NDY=1
KXY=0
C

```

III-7

PLOT 73
PLOT 74
PLOT 75
PLOT 76
PLOT 77
PLOT 78
PLOT 79
PLOT 80
PLOT 81
PLOT 82
PLOT 83
PLOT 84
PLOT 85
PLOT 86
PLOT 87
PLOT 88
PLOT 89
PLOT 90
PLOT 91
PLOT 92
PLOT 93
PLOT 94
PLOT 95
PLOT 96
PLOT 97
PLOT 98
PLOT 99
PLOT 100
PLOT 101
PLOT 102
PLOT 103
PLOT 104
PLOT 105
PLOT 106
PLOT 107
PLOT 108


```

XG(ICUT)=X
IF( X .GT. XMAX)      GOTO 50
X=X+XINCR
SAVE(M, IOUT)=Y
IF(ARS(Y) .GT. SCL)  GOTO 5)
IC=IC+1
IF( IC .GT. IR)      IC=1
CONTINUE
CONTINUE
DO 12 M=1, NSTA
WRITE(KC, 20) J1, M, SF(M)
FORMAT(11STATION NJ., 15, 1 AT, F12.3, 1 FEET, //)
WRITE(KC, 21) J2) (KU(J), SAVE(M, J), J=1, IOUT)
FORMAT(F7.3, F12.3, F7.3, F12.3, F7.3, F12.3, F7.3, F12.3,
1 F7.3, F12.3)
DO 23 I=1, IOUT
CALL PLOT1(KC(I)/DX, (SAVE(M, I)-YMIN)/DY, 2)
IF(IPOINT .EQ. 0) GOTO 12
C
C      READ IN THE NO. OF POINTS AND PLOT CODE
C      IF CURVL HAS NO POINTS THEN ENTER ZERO (0)
C      READ(KIN, 103) NPT, NCJDE
103  FORMAT(2I5)
WRITE(KC, 221) T, NPT
221  FORMAT(//, 1 PERIOD=, FR. OF 1 SEC, 15, 1 POINTS)
IF( NPT .EQ. 0) GOTO 12
IF( NCJDE .EQ. 0) NCJDE =-12
DO 1002 J=1, NPT
READ(KIN, 102) X, Y
C
C      CONVERT THE DAY NUMBER TO TIDAL CYCLES, CENTERING THE
C      DAILY OBSERVATION AT 1200 HRS, AND ASSUMING THAT T=C
C      IS AT 0000 HRS OF THE FIRST DAY.
C      X=((XX-1.)+86400.)/32(2.)/T
WRITE(KC, 222) X, XX, Y
222  FORMAT(5X, F7.1, 2F7.2)

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PLOT 145
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PLOT 147
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 PLOT 197
 PLOT 198

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10 2    UGUS(J)=X/DX
      VGUS(J)=(Y-YMIN)/DY
      CALL GRAPH(JGDS,VGUS,-NPT,C,14,ACODE)
12      CALL PLOT1(J,2,3)
      REWIND KU
      CALL PLOT1(PL+3,2,3)
      WRITE(KC,2(1))
201     FORMAT(2, ' PLOTTED')
      CALL ENDPLOT
      GOTU 555
552     WRITE(KC,2552)
2552    FORMAT(1, 'NO. TIME INTERVALS NOT EVEN')
      GOTD 555
50      WRITE(KD,5014,1,Y,X
501     FORMAT(1, 'LOC',13,1, ' T INCR.',11,1, ' Y=',E16.6,1, ' X=',E16.6,1,
      1    ' X OR Y OUT OF RANGE, EXIT TAKEN')
555     CALL EXIT
      END

1      40 603768.
      120 4464.
2      8.
      18. 5.
3      10000. 21000. 30000.

```