

**HYDRAULIC REPORT**  
**LONGMEADOW PARKWAY BRIDGE OVER FOX RIVER**  
**SECTION NUMBER 94-00215-01-ES**  
**STRUCTURE NUMBER 045-3024**  
**KANE COUNTY, ILLINOIS**

*Prepared For:*

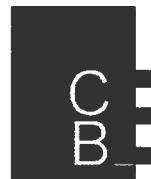
Kane County Division of Transportation  
41W011 Burlington Road  
St. Charles, IL 60175

*Prepared By:*

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CBBEL Project No. 99-236

March, 2005  
Revised April 2011  
Revised October 2012



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## **SECTION 1**

### **NARRATIVE**

## NARRATIVE

### General Project Description

The hydraulic report is for a proposed bridge carrying Longmeadow Parkway over the Fox River in Kane County, Illinois. Longmeadow Parkway will follow the alignment of Bolz Road east of the River. The proposed bridge will be located approximately 11,150 feet upstream of the Carpentersville Dam. The adjacent areas are generally rural, with surveyed residential structures located inside the Fox River floodplain. A general location drainage map is included.

The proposed bridge is an 8-span structure with an overall span length of 1307'-3", face to face of the abutments. The proposed low chord elevation of 757.22 feet is approximately 28 feet above the 50-year flood elevation of 729.21 feet. The bridge width will be approximately 85 feet, to accommodate the proposed 4-lane roadway, raised median, 6' sidewalk, and 10' bike path. Drawings of the proposed bridge can be found in Section 7.

### Field Observations

The Fox River is a single channel in the vicinity of the proposed crossing, extending upstream (north) and downstream (south) with a very slight meandering. The Fox River floodplain contains residential structures adjacent to the proposed crossing. The channel is very well defined throughout this reach and the Fox River generally flows through the west side of the floodplain, with no obvious siltation or scour having been observed. An aerial photo of the area near the proposed bridge is provided in Section 4.

### Stream Survey and Benchmark Correlation

A stream survey of the Fox River was performed by Engineering Enterprises, Inc. (EEI) in February, 2004. A benchmark correlation was made based on FEMA Reference Mark RM 70-3, which is on the NGVD 1929 datum. EEI survey datum is 0.16 feet higher than the NGVD 1929 datum. Information on RM 70-3 is provided in Section 5 on 2002 Flood Insurance Rate Map (FIRM) Panel Number 17089C0070F excerpt. The EEI datum correlation is provided at the beginning of Section 6.

Floodplain cross-sections were surveyed, ranging from approximately 1000 feet upstream to 1000 feet downstream of the proposed bridge to an elevation above the FIRM designated 100-year elevation. Survey cross section location information is provided in Section 6.

### Historical Flooding Observations

According to the USGS Hydrologic Investigations Atlas HA-253 (1967), Crystal Lake Quadrangle, the flood of record for the Fox River occurred in April, 1960. Figure 8 of HA-253 indicates that the flood stage in the area of the proposed bridge was approximately 726.4 feet based on the NGVD 1929 datum during this event. HA-253 Figure 8 flood profiles are provided in Section 4.

According to flow data from USGS Gage 05550000, Fox River at Algonquin, located approximately 0.9 miles upstream, the peak flow on April 6, 1960 was 6,610 cfs. Similar high flows of 6,720 cfs were subsequently observed on May 22, 2004, and August 25, 2007. Gage data are provided in Section 5. For comparison purposes, peak flows from the USGS Scientific Investigations Report 2004-5103 and from StreamStats were compared to the Flood Insurance Study (FIS) regulatory flows. For all analyzed flood events, the FIS regulatory flows are higher than the gage flows and StreamStats flows at the proposed bridge site.

The FIS regulatory 100-year flood flow is 10,095 cfs, and the 100-year flood elevation is approximately 729.77 feet based on the NGVD 1929 datum, according to Table 6 and Sheet 63P of the FIS for Kane County, Illinois and Incorporated Areas, dated December 20, 2002. The 2002 FIS was subsequently revised August 3, 2009. The only change to the Fox River information in this reach is the adjustment from the NGVD 1929 datum to the NAVD 1988 datum, accomplished by subtracting 0.2' from each NGVD 1929 elevation. FIS excerpts are provided in Section 5.

### **Sensitive Flood Receptors**

There are identified buildings and structures in the vicinity of the project site, which are below the existing 100-year flood elevation of the Fox River. The EEI Exhibit, "Location of Buildings and Structures with Potential Flood Damages of the Fox River" is provided in Section 6.

### **Hydraulic Analysis**

A hydraulic analysis of the Fox River was performed to develop flood elevations of the natural conditions and to determine the effect, if any, of the proposed bridge on water surface profiles. The basis for the hydraulic analysis was the hard copy of the regulatory HEC-2 hydraulic model of the Fox River, obtained from the Illinois State Water Survey (ISWS). This model was reportedly created circa 1979, is on the NGVD 1929 datum, and used a different cross section identification scheme than the 2002 FIS. The output received had been hand-annotated to reflect the FIS stationing identification, which indicates the distance in feet upstream from the mouth of the Fox River. The flow rates included in the regulatory HEC-2 model were verified as current and used in the analysis. The regulatory HEC-2 model hard copy is provided in Section 8.

Several hydraulic models were created for this analysis, as summarized in the following sections.

#### **Baseline Model**

The regulatory HEC-2 input for the Fox River was entered into HEC-RAS v. 4.1.0. The annotated FIS stationing was used, rather than the original HEC-2 stationing. All geometry, reach lengths, and flow rates for the study reach were copied from the regulatory HEC-2 model printout. The reach lengths were compared to the FIS

stationing and found to be reasonable. In the Baseline model and all subsequent models, the river stations are equal to the stream distance above the mouth of the Fox River, divided by 1000 (i.e. cross section DF at distance 428,504 feet above the mouth is Baseline model cross section 428.504).

The first page of the HEC-2 hardcopy, including the Manning's n-values, was missing. Therefore the Manning's n-values were determined by calibrating to the published output elevations. The Manning's n-values are approximately 0.05 for the channel and 0.10 for the overbank areas. These values represent a range of ground roughness corresponding to a major stream channel (top width > 100 feet) with regular section and brush on the banks, and to medium brush or heavy timber with little undergrowth in the overbank areas. The calibrated n-values are within the range published for the Fox River in FIS Table 5 that is provided in Section 5 of this report.

The HEC-RAS Baseline model was executed to confirm the results reflected in the FIRM Panel 70 of 410 for Kane County, Illinois and Incorporated areas, dated December 20, 2002, as well as the FIS Floodway Data on Table 6. Pertinent FIS excerpts are provided in Section 5. The Baseline model extends from FIS cross section DA located 8,955 feet downstream of the proposed bridge to cross section DF located 4,673 feet upstream. The starting water surface elevations were taken from the regulatory model at cross section DA.

The Baseline model was executed to ensure that the results of the regulatory HEC-2 model had been duplicated. At all cross sections, the HEC-RAS water surface profiles matches the regulatory HEC-2 output within 0.01 feet for all storm events. From this point, the HEC-RAS model was considered to be the Baseline regulatory model. Tables 1 through 4 illustrate the comparison between the regulatory HEC-2 model and the Baseline HEC-RAS model for the 10, 50, 100, and 500-year storm events.

#### Modified Existing/Natural Conditions Model

Modified Existing and Natural conditions are the same, because there is no existing bridge at the site. The Modified Existing/Natural model was developed by inserting the EEI surveyed stream cross-sections into the baseline regulatory model. The inserted stream cross-sections, ranging from approximately 1000 feet downstream to 1000 feet upstream, include Sections 3073, 2453, 2098, 2000, 1885, 1402, and 912, based on the alignment established by EEI (Section 3073 is EEI cross section 30+73.04). Because the EEI alignment runs from upstream to downstream, opposite normal hydraulic modeling conventions, the cross sections were renumbered to fit within the FIS numbering system.

The Manning's "n" values as determined from the regulatory model were slightly modified for the inserted surveyed sections, so that the Modified Existing/Natural model water surface elevations match the Baseline model water surface elevations within 0.03 feet at all cross section for all analyzed storm events. Tables 1 through 4 illustrate the comparison between the Baseline regulatory model and the Modified Existing/Natural Conditions model for the 10, 50, 100, and 500-year storm events.

### Proposed Conditions Model

The final model prepared for the hydraulic analysis was the Proposed Conditions model, which reflects the proposed Longmeadow Parkway Bridge. This model was used to demonstrate that the proposed bridge is sized properly and that there is less than a 0.1 foot raise in flood stages for all storm events up to and including the 100-year storm frequency due to the proposed bridge. Model cross sections 1 foot from the bridge faces were inserted, and the reach lengths were adjusted to reflect a new structure. The proposed bridge has 8 spans totaling 1,300 feet in length, a width of 85 feet, and low chord at 757.22 feet, based on the EEI survey datum.

As the Proposed model reflects, there is a negligible increase in flood stages due to the construction of the proposed Bolz Road/Longmeadow Parkway Bridge. Tables 1 through 4 illustrate the comparison between the Modified Existing/Natural model and the Proposed model for the 10, 50, 100, and 500-year storm events.

### Modeling Results

The results of the hydraulic analysis demonstrate that the applicable criteria have been met, specifically that the resulting flood stages do not exceed 0.1 foot rise over the Natural condition flood stages due to implementation of the proposed project for all storm events up to and including the 500-year storm frequency. Four flood frequencies were analyzed for each of the three models summarized above, including the 10-, 50-, 100-, and 500-year events. A summary of the results are included in tables on the following pages, and the input/output of the HEC-RAS hydraulic models are included in Section 8.

**TABLE 1**  
**Comparison of 10 Year Water Surface Elevations, NGVD 1929 Datum**

10 year	Cross Section			Water Surface Elevation (NGVD 1929)			
X-section	2002 FIS XS Designation	Source	Location	Regulatory HEC-2 Hardcopy	Baseline (Reg. HEC-2 recreated in HEC-RAS )	Modified Existing / Natural	Proposed
428.504	DF	ISWS FIS		729.49	729.49	729.48	729.48
426.207	—	ISWS FIS		728.31	728.31	728.30	728.30
424.800		EEI Section	Approx. 1000' U/S	—	—	727.77	727.77
424.600		EEI Section	Approx. 500' U/S	—	—	727.71	727.70
424.528	DE	ISWS FIS		727.69	727.69	727.66	727.66
423.900		EEI Section	Approx. 100' U/S	—	—	727.53	727.53
423.850		CBBEL	Prop. U/S Face +1'	—	—	—	727.51
423.800		EEI Section	Prop. Centerline	—	—	727.49	—
423.750		CBBEL	Prop. D/S Exit +1'	—	—	—	727.47
423.700		EEI Section	Approx. 100' D/S	—	—	727.45	727.45
423.300		EEI Section	Approx. 500' D/S	—	—	727.28	727.28
422.600		EEI Section	Approx. 1000' D/S	—	—	727.02	727.02
422.521	DD	ISWS FIS		726.91	726.91	726.91	726.91
420.346	DC	ISWS FIS		726.16	726.16	726.16	726.16
417.841	DB	ISWS FIS		725.15	725.15	725.15	725.15
414.015	DA	ISWS FIS		723.86	723.86	723.86	723.86

**TABLE 2**  
**Comparison of 50 Year Water Surface Elevations, NGVD 1929 Datum**

50 year	Cross Section			Water Surface Elevation (NGVD 1929)			
X-section	2002 FIS XS Designation	Source	Location	Regulatory HEC-2 Hardcopy	Baseline (Reg. HEC-2 recreated in HEC-RAS )	Modified Existing / Natural	Proposed
428.504	DF	ISWS FIS		731.11	731.11	731.11	731.10
426.207	—	ISWS FIS		729.92	729.92	729.91	729.91
424.800		EEI Section	Approx. 1000' U/S	—	—	729.35	729.35
424.600		EEI Section	Approx. 500' U/S	—	—	729.27	729.27
424.528	DE	ISWS FIS		729.23	729.23	729.22	729.22
423.900		EEI Section	Approx. 100' U/S	—	—	729.07	729.07
423.850		CBBEL	Prop. U/S Face +1'	—	—	—	729.06
423.800		EEI Section	Prop. Centerline	—	—	729.03	—
423.750		CBBEL	Prop. D/S Exit +1'	—	—	—	729.02
423.700		EEI Section	Approx. 100' D/S	—	—	728.99	728.99
423.300		EEI Section	Approx. 500' D/S	—	—	728.79	728.79
422.600		EEI Section	Approx. 1000' D/S	—	—	728.51	728.51
422.521	DD	ISWS FIS		728.38	728.39	728.39	728.39
420.346	DC	ISWS FIS		727.55	727.55	727.55	727.55
417.841	DB	ISWS FIS		726.37	726.37	726.37	726.37
414.015	DA	ISWS FIS		724.72	724.72	724.72	724.72

**TABLE 3**  
**Comparison of 100 Year Water Surface Elevations, NGVD 1929 Datum**

100 year	Cross Section			Water Surface Elevation (NGVD 1929)				
	2002 FIS XS Designation	Source	Location	2002 FIS Floodway Data Table 6	Regulatory HEC-2 Hardcopy	Baseline (Reg. HEC-2 recreated in HEC-RAS )	Modified Existing / Natural	Proposed
428.504	DF	ISWS FIS		732.0	732.02	732.02	732.02	732.02
426.207	—	ISWS FIS			730.82	730.82	730.82	730.82
424.800		EEI Section	Approx. 1000' U/S		—	—	730.25	730.26
424.600		EEI Section	Approx. 500' U/S		—	—	730.16	730.16
424.528	DE	ISWS FIS		730.1	730.11	730.11	730.11	730.11
423.900		EEI Section	Approx. 100' U/S		—	—	729.95	729.95
423.850		CBBEL	Prop. U/S Face +1'		—	—	—	729.94
423.800		EEI Section	Prop. Centerline		—	—	729.91	—
423.750		CBBEL	Prop. D/S Exit +1'		—	—	—	729.90
423.700		EEI Section	Approx. 100' D/S		—	—	729.87	729.87
423.300		EEI Section	Approx. 500' D/S		—	—	729.65	729.65
422.600		EEI Section	Approx. 1000' D/S		—	—	729.35	729.35
422.521	DD	ISWS FIS		729.2	729.23	729.23	729.23	729.23
420.346	DC	ISWS FIS		728.4	728.35	728.35	728.35	728.35
417.841	DB	ISWS FIS		727.1	727.09	727.09	707.09	727.09
414.015	DA	ISWS FIS		725.3	725.25	725.25	725.25	725.25

**TABLE 4**  
**Comparison of 500 Year Water Surface Elevations, NGVD 1929 Datum**

500 year	Cross Section			Water Surface Elevation (NGVD 1929)			
	2002 FIS XS Designation	Source	Location	Regulatory HEC-2 Hardcopy	Baseline (Reg. HEC-2 recreated in HEC-RAS )	Modified Existing / Natural	Proposed
428.504	DF	ISWS FIS		733.13	733.13	733.13	733.13
426.207	—	ISWS FIS		731.92	731.92	731.93	731.93
424.800		EEI Section	Approx. 1000' U/S	—	—	731.35	731.36
424.600		EEI Section	Approx. 500' U/S	—	—	731.25	731.25
424.528	DE	ISWS FIS		731.17	731.17	731.19	731.20
423.900		EEI Section	Approx. 100' U/S	—	—	731.02	731.03
423.850		CBBEL	Prop. U/S Face +1'	—	—	—	731.02
423.800		EEI Section	Prop. Centerline	—	—	730.99	—
423.750		CBBEL	Prop. D/S Exit +1'	—	—	—	730.96
423.700		EEI Section	Approx. 100' D/S	—	—	730.94	730.94
423.300		EEI Section	Approx. 500' D/S	—	—	730.69	730.69
422.600		EEI Section	Approx. 1000' D/S	—	—	730.38	730.38
422.521	DD	ISWS FIS		730.25	730.26	730.26	730.26
420.346	DC	ISWS FIS		729.32	729.32	729.32	729.32
417.841	DB	ISWS FIS		727.97	727.97	727.97	727.97
414.015	DA	ISWS FIS		725.92	725.92	725.92	725.95

## **IDNR-OWR Floodway Permitting Requirements**

A permit is required from the Illinois Department of Natural Resources (IDNR) – Office of Water Resources (OWR) for the work proposed within the floodway of the Fox River. The work includes fill in the floodplain and floodway due to construction of the Longmeadow Parkway bridge piers. The hydraulic modeling demonstrates that flood stages are not increased by more than 0.01 feet at any location for all storm events up to and including the 500-year storm frequency.

It is estimated that construction of the proposed bridge piers will require floodplain fill volume of 88 cubic yards between the ground elevation and the 10-year flood elevation. All of this fill is assumed to be in the floodway. Floodplain fill volume of 144 cubic yards is anticipated between the 10-year flood elevation and the 100-year flood elevation. All of this fill is also assumed to be in the floodway. IDNR-OWR requires compensatory storage for floodway fill at a 1:1 ratio. The Kane County Stormwater Management Ordinance §410(a) requires “compensatory storage volume...shall be at least equal to the regulatory floodplain flood storage volume displaced multiplied by 1.5.” Therefore, a minimum compensatory storage volume of 348 cubic yards shall be provided at the appropriate elevations between the ground elevation and the 100-year flood elevation to satisfy the IDNR-OWR floodway permit and the Kane County Ordinance requirements. A Permit Summary Form and fill calculations are included in Section 9.

If temporary construction features are necessary, these must also be permitted.

## **IDNR-OWR Individual Public Waters Permit Requirements**

A permit is required from the IDNR-OWR for work proposed in Public Waters, which includes the channel of the Fox River. The proposed piers span most of the channel, providing approximately 290 feet of horizontal clearance. The proposed bridge also provides over 25 feet of vertical clearance above the design high water level. These clearances exceed the requirements of the Part 720.10 Rules Establishing Horizontal and Vertical Clearances for Bridges over the Fox River. A copy of the Part 720.10 Rules, requiring that “the minimum horizontal clearance for bridges hereafter constructed over the Fox River between Algonquin and the Illinois-Wisconsin State Line shall be 100 feet and the minimum vertical clearance for such bridges shall be 15 feet above the normal pool level,” is included in Section 7.

If temporary construction features are necessary, these must also be permitted.

## **Scour Analysis**

The Scout Critical Evaluation Coding Report Form is provided in Section 10, along with scour calculations for the proposed structure. The calculations show that the maximum total scour depth is 6.62 feet for the 500-year storm event. As a result, the

piles will be constructed at the appropriate depth and structure protection will be implemented.

## Conclusion and Design Recommendations

The results from the hydraulic analysis are summarized in the Waterway Information Table located in Section 3. The results indicate that the proposed bridge construction will not adversely impact the hydraulics of the Fox River. The proposed bridge will meet the IDOT drainage criteria that the low chord will have a minimum of 2 feet above the 50-year natural water elevation and is not below the all time recorded high water elevation, and that the proposed roadway edge of pavement will be at least 3 feet above the 50-year created headwater elevation.

Additionally the IDNR-OWR requirements will be met, including compensatory storage for fill in the floodway and maintaining a created head of no greater than 0.1 foot for all storm events up to and including the 100-year storm frequency. Adequate horizontal and vertical clearances are provided in accordance with IDNR-OWR Rules. The Kane County Stormwater Ordinance requirement of providing 1.5:1 compensatory storage in the floodplain will also be satisfied.

An IDNR-OWR permit is required because the proposed construction will work at the identified Fox River floodway and because the Fox River is designated as a Public Body of Water.

IAD/BWL/jmc/mk  
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## **SECTION 2**

**PRELIMINARY BRIDGE DESIGN AND HYDRAULIC REPORT  
(BLR 10210)**

Municipality Unincorporated  
 County Kane  
 Road District KDOT  
 Other Agency IDOT  
 Project Longmeadow Parkway  
 Section 94-00215-01 ES



### Preliminary Bridge Design and Hydraulic Report

Route Longmeadow Parkway  
 Stream Fox River  
 Ex. St. No. n/a  
 Pr. St. No. 045-3024  
 Prepared by Ilene Dailey, PE, CFM  
 Agency/Firm Christopher B. Burke Eng L  
 Date 10/08/2012

Funding Type:  HBP  STU  STR  Enhancement  
 TBP  MFT  Non-MFT  Other ( \_\_\_\_\_)

Sufficiency Rating n/a Existing clear span length n/a

Functionally Obsolete  Yes  No  
 Structurally Deficient  Yes  No

#### Construction Information

Proposed Letting Date 09/01/2016

Shop Plan Review by  Local Agency  Consultant  State  
 Fabrication Inspection by  Local Agency  Consultant  State

#### Approach Roadway Information

Surface Type: Existing n/a Proposed Concrete

Surface Width: Existing n/a Proposed 60'

Shldr to Shldr Width: Existing n/a Proposed 64'

Elevation of Low Point: Existing n/a Proposed 764.35

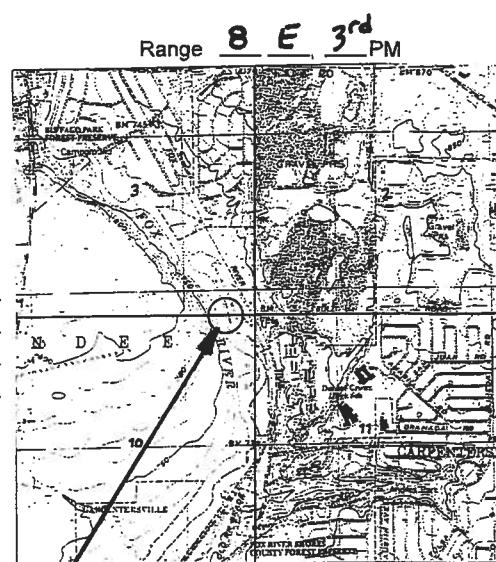
Proposed Side Slopes 3:1

Roadway Functional Classification Minor Arterial

DHV 2050 Current ADT n/a Design Year ADT 26,000

% Trucks 4% Design Speed 50 mph

3R Design Guidelines Used  Yes  No



Locate bridge accurately above

#### Proposed Structure Information

Type of Structure Proposed  Bridge  Culvert  "Standard Plans" Bridge  Pedestrian/Bicycle

Vehicle Design Loading HL-93 Pedestrian/Bicycle Design Loading 75 psf

Superstructure Type Steel Plate Girder and Prestressed Concrete Bulb-T

Structure Length Back to Back Abutments 1307'-3" Span Length 120'-200'-300'-200'-120'-120'-120'-120'

Clear Roadway Width 64' Rail Type 42" Type F Crash Tested Rail Required  Yes  No

Wearing Surface Type n/a Wearing Surface Thickness n/a

Deicing Agents Used  Yes  No

Embankment Slope Under Bridge 2:1 Proposed Skew Angle 15 degr Forward on.  Rt.  Lt.

Pier Type solid wall Abutment Type Pile Supported Stub

Proposed Pile Type Steel H

Borings By Wang Engineering, Inc. Expected Submittal Date for Borings \_\_\_\_\_

#### Hydraulic Data

Exist. Br. Cr. El. n/a @ Sta. n/a Prop. Br. Cr. El. 764.53 @ Sta. 2218+65.40

Exist. Low Beam Elev. n/a Proposed Low Beam Elev. 757.22

Exist. Freeboard n/a Proposed Freeboard 34.62 Streambed Elev. 715.16

Drainage Area Appr. 1415 sq. mi. Crossing Location  Rural  Urban

Crossing Located within a Mapped National Flood Insurance Program Area  Yes  No (Map No. 0070F)

Crossing Located within a Northeast Region (District #1) FEMA Mapped Floodway  Yes  No

Crossing Located over designated "Public Bodies of Water"  Yes  No

#### Design Flood Data

Design Flood Frequency 50-yr Design Discharge 8345 cfs Design High Water Elev. 729.21

Exist. Br. Opening n/a Exist. Over-the-Road n/a

Prop. Br. Opening 3990 sft. Prop. Over-the-Road 0 cfs

#### 100 Year Flood Data

100 Year Discharge 10095 cfs 100 Year High Water Elev. 730.09

Exist. Br. Opening n/a Exist. Over-the-Road n/a Exist. Created Head n/a

Prop. Br. Opening 4688 sft. Prop. Over-the-Road 0 cfs Prop. Created Head 0.00'

If proposed structure and over-the-road area will not carry entire flow, state kind and area of additional waterway  
n/a

Type of Streambed soil gravelly sand/silt clay loam Will drift or ice permit pier in channel?  Yes  No  
Has scour occurred at or near existing structure?  Yes  No; If yes, reason for scour n/a

Comments on hydraulic adequacy of existing structure n/a

Has the existing structure been the cause of demonstrable flood damage to adjacent property?  Yes  No  
If yes, describe damage n/a

Comments on the hydraulic adequacy of upstream and downstream structures and their comparable relationship to the proposed structure No other bridges are located near the proposed structure. The Carpentersville dam, located approximately 11,150 feet downstream, affects WSELs for this reach of the Fox River.

Will houses, places of business or valuable property be affected by backwater from the proposed bridge?  Yes  No  
If yes, describe property and effect of backwater There are buildings located upstream of the proposed bridge with doorways and/or window wells below the 100-year elevation (730.09) of the Fox River. The proposed bridge does not raise water surface elevations more than 0.01' for all flood events up to and including the 500-yr.

Is any channel excavation beyond that required to construct the substructure required in the channel?  Yes  No  
If yes, describe extent of channel excavation

Will a channel realignment be required?  Yes  No (If yes, attach Channel Change Sketch)  
Are stream flow data (gaging station or flood study) available for the stream at or near the proposed site?  Yes  No  
(If yes, attach an analysis of the stream flow data)

Provide information regarding high water from other streams, reservoirs, flood control projects, proposed channel changes, strip mine areas or other controls affecting the hydraulic or hydrologic properties of the crossing site The hydraulic effects of the Carpentersville dam are included in the FEMA modeling used as the basis of the proposed bridge analysis.

### Scour Analysis

Was a HEC-18 scour analysis performed?  Yes  No

Were all substructure units being utilized evaluated to consider the effect of anticipated scour?  Yes  No

Will scour protection or corrective actions be required?  Yes  No

If yes, describe protection or corrective actions. Pier foundations within the floodplain will be on piles at the appropriate elevations per the SGR.

### Attachments (Check those items below that are included.)

- Reproduction of applicable portion of USGS quadrangle showing locations of proposed bridge and properties affected by backwater caused by the proposed structure
- Cross sections as required by WSPRO including floodplain above high water elevation
- Streambed profile
- Profile of existing and proposed roadway across floodplain
- Hydraulic calculations
- Joint Application Form for construction permit submittals (Joint Form NCR-426)
- Waterway sketch
- Channel change sketch
- Applicable certification(s)
- Boring data
- Scour analysis/evaluation
- Other Permit Summary for Floodway Construction in Northeast Illinois

## **SECTION 3**

### **WATERWAY INFORMATION TABLE**



## Waterway Information Table EEI Datum

Route: Longmeadow Parkway  
Section: 94-00212-01-ES  
County: Kane  
Date: 01-27-11

Existing SN: n/a  
Proposed SN: 045-3024  
Waterway: Fox River  
Prepared by: Ilene A. Dailey, PE, CFM

Drainage Area = Approx. 1415 sq. mi.				Proposed Overtopping Elev.				Existing Overtopping Elev. = n/a				at Sta. 2219+41			
Flood	Freq. Yr.	Q Ft <sup>3</sup> /s	Opening - ft <sup>2</sup>	Natural H.W.E.	Existing	Proposed	Head - ft.	Existing	Proposed	Headwater Elevation	at Sta.	n/a	n/a	n/a	n/a
Design	10	5775	n/a	2853	727.67	n/a	0.00	n/a	0.00	727.67					
Base	50	8345	n/a	3990	729.21	n/a	0.00	n/a	0.00	729.21					
Overtop Existing	100	10095	n/a	4688	730.09	n/a	0.00	n/a	0.00	730.09					
Overtop Proposed	n/a														
Max. Calc.	500	12525	n/a	5643	731.17	n/a	0.01	n/a	0.01	731.18					

10 YEAR VELOCITY THROUGH EXISTING BRIDGE = n/a ft/s

ALL-TIME H.W.E. & DATE: 726.4 (NGVD 1929) in April 1960

**Scope of Work:** Construct new 8-span bridge to carry proposed Longmeadow Parkway over the Fox River.  
Note: All WIT elevations based on EEI datum = FEMA Datum (NGVD 1929) + 0.16'.

### EXISTING STRUCTURE

TYPE: n/a  
LENGTH: n/a  
# SPANS: n/a  
LOW BEAM: n/a  
SKEW: n/a  
LOW E.O.P.: n/a

### PROPOSED STRUCTURE

TYPE: Steel Plate Girder and Prestressed Concrete Bulb-T  
LENGTH: 1307'-3"  
# SPANS: 120'-200'-300'-200'-120'-120'-120'-120'  
LOW BEAM: 757.22  
SKEW: 15 degrees  
LOW E.O.P.: 763.89

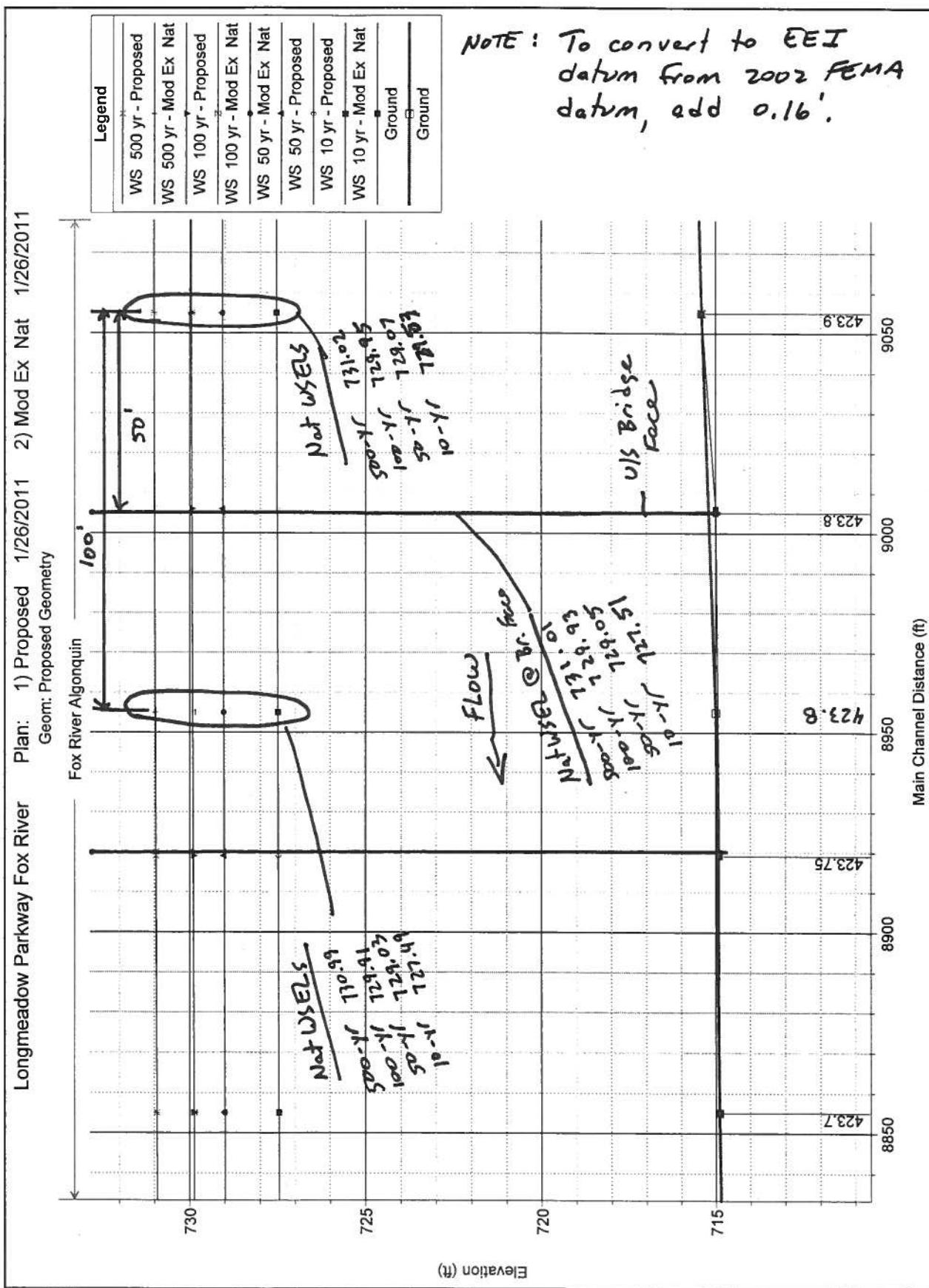
NOTE: PROPOSED STRUCTURE DETAILS ARE PRELIMINARY; SUBJECT TO REFINEMENT IN TSL STAGE.

*WIT Supporting Data*

HEC-RAS Locations: User Defined

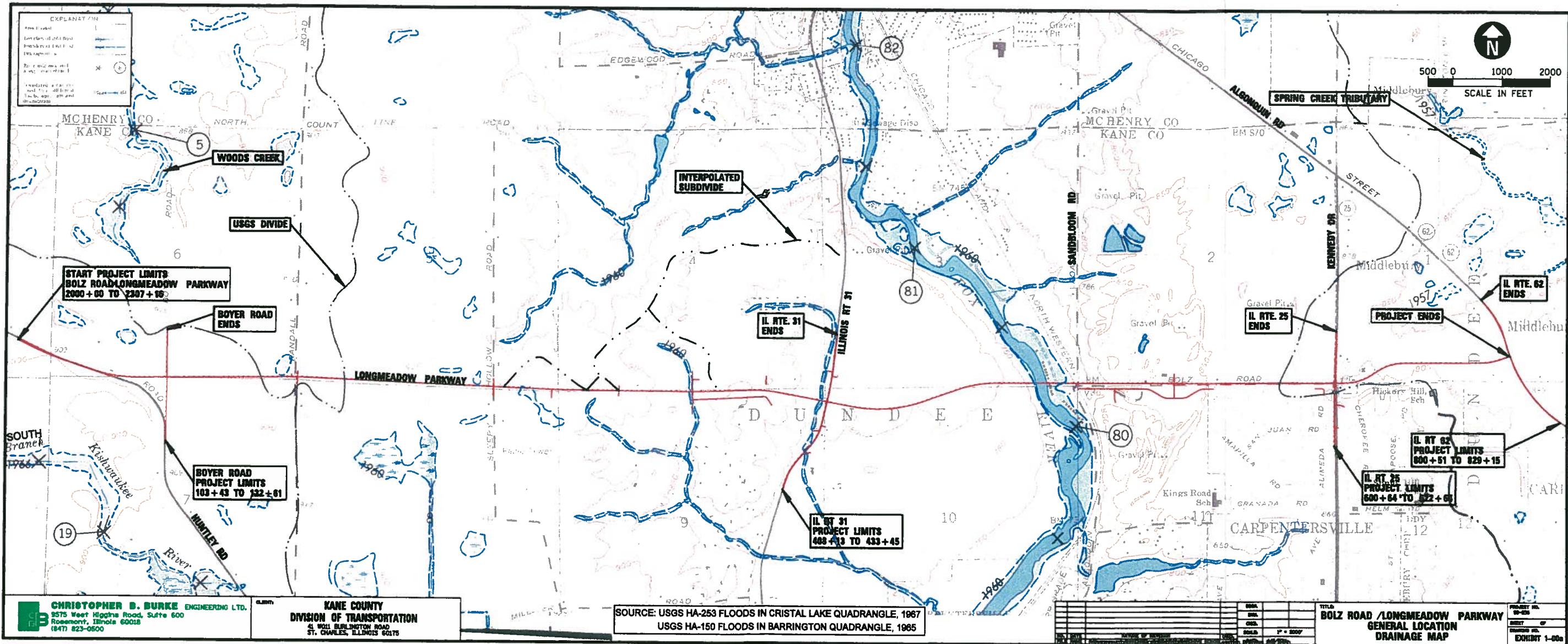
River	Reach	River Sta	Profile	Plan	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Criti (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Ch
Fox River	Algonquin	424.528	10 yr	Mod Ex Nat	5775.00	716.60	727.66	727.73	0.000272	2.10	3250.66	782.36	0.12	
Fox River	Algonquin	424.528	10 yr	Proposed	5775.00	716.60	727.66	727.72	0.000272	2.10	3248.52	781.70	0.12	
Fox River	Algonquin	424.528	50 yr	Mod Ex Nat	8345.00	716.60	729.22	729.31	0.000299	2.45	4580.96	888.50	0.13	
Fox River	Algonquin	424.528	50 yr	Proposed	8345.00	716.60	729.22	729.31	0.000299	2.45	4580.20	888.46	0.13	
Fox River	Algonquin	424.528	100 yr	Mod Ex Nat	10085.00	716.60	730.11	730.21	0.000313	2.64	5386.25	924.34	0.14	
Fox River	Algonquin	424.528	100 yr	Proposed	10085.00	716.60	730.11	730.21	0.000313	2.64	5387.38	924.46	0.14	
Fox River	Algonquin	424.528	500 yr	Mod Ex Nat	12525.00	716.60	731.19	731.30	0.000328	2.88	6425.32	983.89	0.14	
Fox River	Algonquin	424.528	500 yr	Proposed	12525.00	716.60	731.20	731.31	0.000328	2.87	6430.39	984.08	0.14	
Fox River	Algonquin	423.9	10 yr	Mod Ex Nat	5775.00	715.40	727.53	727.61	0.000359	2.36	2864.94	623.18	0.14	
Fox River	Algonquin	423.9	10 yr	Proposed	5775.00	715.40	727.53	727.61	0.000359	2.36	2863.15	622.98	0.14	
Fox River	Algonquin	423.9	50 yr	Mod Ex Nat	8345.00	715.40	729.07	729.18	0.000425	2.75	3940.32	725.39	0.15	
Fox River	Algonquin	423.9	50 yr	Proposed	8345.00	715.40	729.07	729.18	0.000425	2.75	3939.75	725.37	0.15	
Fox River	Algonquin	423.9	100 yr	Mod Ex Nat	10085.00	715.40	729.95	730.08	0.000444	2.97	4584.19	739.47	0.16	
Fox River	Algonquin	423.9	100 yr	Proposed	10085.00	715.40	729.95	730.08	0.000444	2.97	4585.09	739.49	0.16	
Fox River	Algonquin	423.9	500 yr	Mod Ex Nat	12525.00	715.40	731.02	731.16	0.000468	3.25	5384.36	756.60	0.16	
Fox River	Algonquin	423.9	500 yr	Proposed	12525.00	715.40	731.03	731.17	0.000468	3.24	5386.10	756.68	0.16	
Fox River	Algonquin	423.85	10 yr	Proposed	5775.00	715.00	727.51	727.59	0.000474	2.35	2876.53	671.96	0.15	
Fox River	Algonquin	423.85	50 yr	Proposed	8345.00	715.00	729.06	729.16	0.000472	2.66	4044.34	775.76	0.16	
Fox River	Algonquin	423.85	100 yr	Proposed	10085.00	715.00	729.94	730.05	0.000474	2.83	4762.04	902.82	0.16	
Fox River	Algonquin	423.85	500 yr	Proposed	12525.00	715.00	731.02	731.14	0.000477	3.04	5744.41	920.22	0.16	
Fox River	Algonquin	423.8	10 yr	Mod Ex Nat	5775.00	715.00	727.49	727.57	0.000478	2.35	2862.84	671.11	0.15	
Fox River	Algonquin	423.8	50 yr	Mod Ex Nat	8345.00	715.00	729.03	729.14	0.000476	2.66	4026.77	775.63	0.16	
Fox River	Algonquin	423.8	100 yr	Mod Ex Nat	10085.00	715.00	729.91	730.03	0.000482	3.06	5718.89	920.02	0.16	
Fox River	Algonquin	423.75	10 yr	Proposed	5775.00	714.90	727.47	727.55	0.000349	2.30	3146.64	631.97	0.13	
Fox River	Algonquin	423.75	50 yr	Proposed	8345.00	714.90	729.02	729.12	0.000384	2.68	4285.09	768.53	0.14	
Fox River	Algonquin	423.75	100 yr	Proposed	10085.00	714.90	729.90	730.01	0.000402	2.89	4963.83	776.06	0.15	
Fox River	Algonquin	423.75	500 yr	Proposed	12525.00	714.90	730.96	731.09	0.000426	3.16	5797.19	785.20	0.16	
Fox River	Algonquin	423.7	10 yr	Mod Ex Nat	5775.00	714.90	727.45	727.53	0.000352	2.31	3132.77	630.79	0.13	
Fox River	Algonquin	423.7	10 yr	Proposed	5775.00	714.90	727.45	727.53	0.000352	2.31	3132.77	630.79	0.13	
Fox River	Algonquin	423.7	50 yr	Mod Ex Nat	8345.00	714.90	728.99	729.09	0.000387	2.69	4266.73	768.32	0.15	
Fox River	Algonquin	423.7	100 yr	Mod Ex Nat	10085.00	714.90	728.99	729.09	0.000387	2.69	4266.73	768.32	0.15	
Fox River	Algonquin	423.7	100 yr	Proposed	10085.00	714.90	729.87	729.98	0.000406	2.90	4944.42	775.84	0.15	
Fox River	Algonquin	423.7	500 yr	Mod Ex Nat	12525.00	714.90	729.87	729.98	0.000406	2.90	4944.42	775.84	0.15	
Fox River	Algonquin	423.7	500 yr	Proposed	12525.00	714.90	730.94	731.07	0.000430	3.17	5776.79	784.98	0.16	
Fox River	Algonquin	423.7	500 yr	Proposed	12525.00	714.90	730.94	731.07	0.000430	3.17	5776.79	784.98	0.16	

*Note: To convert to EET datum, add 0.16'.  
From FEMA datum, add 0.16'.*



## **SECTION 4**

**GENERAL LOCATION DRAINAGE MAP (HA-253 and HA-150)  
USGS PROFILES OF FLOODS ON FOX RIVER (HA-253)  
AERIAL PHOTO**



USGS Hydrologic Investigations Atlas, HA-253 (1967)

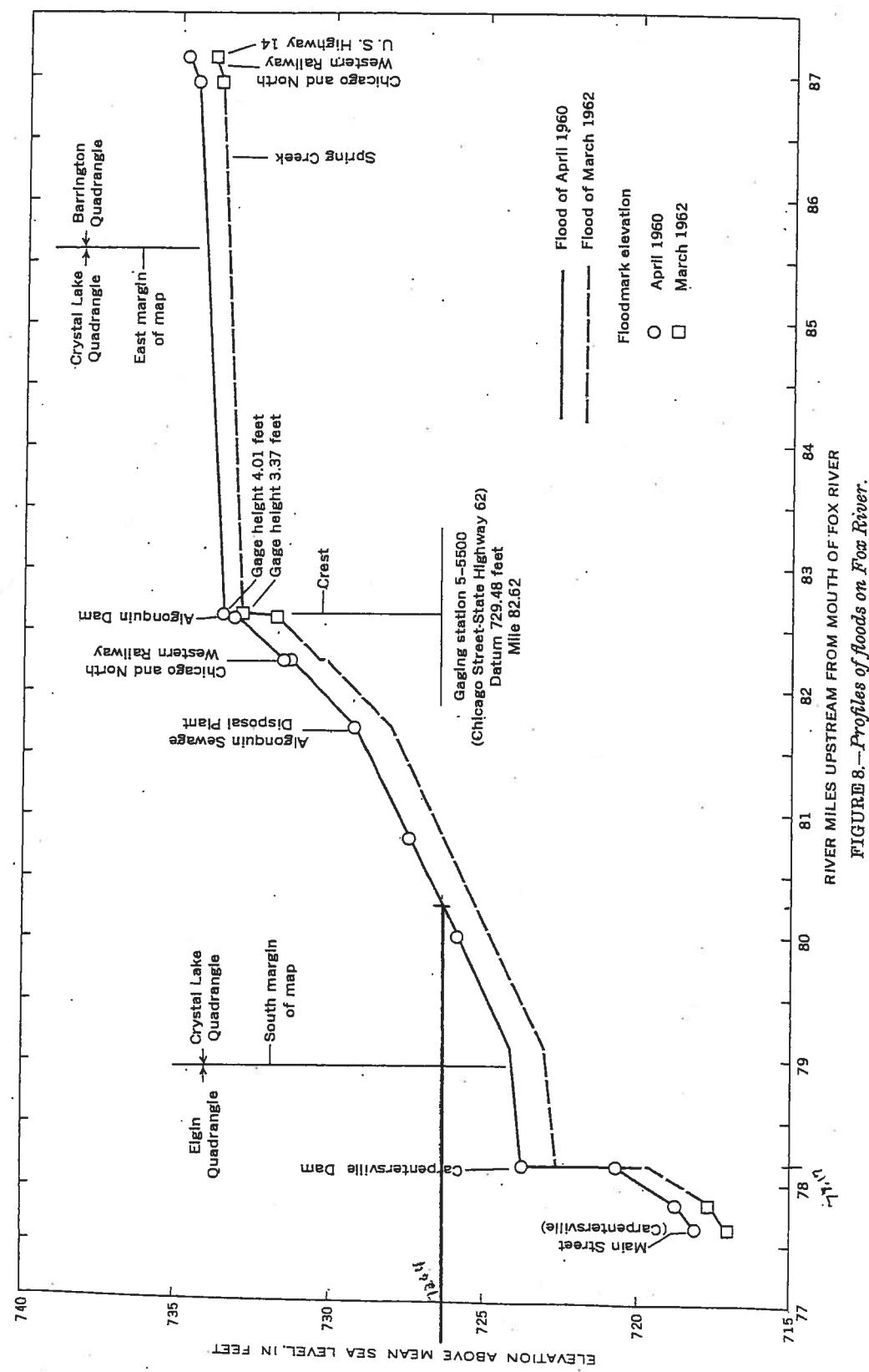


FIGURE 8.—Profiles of floods on Fox River.

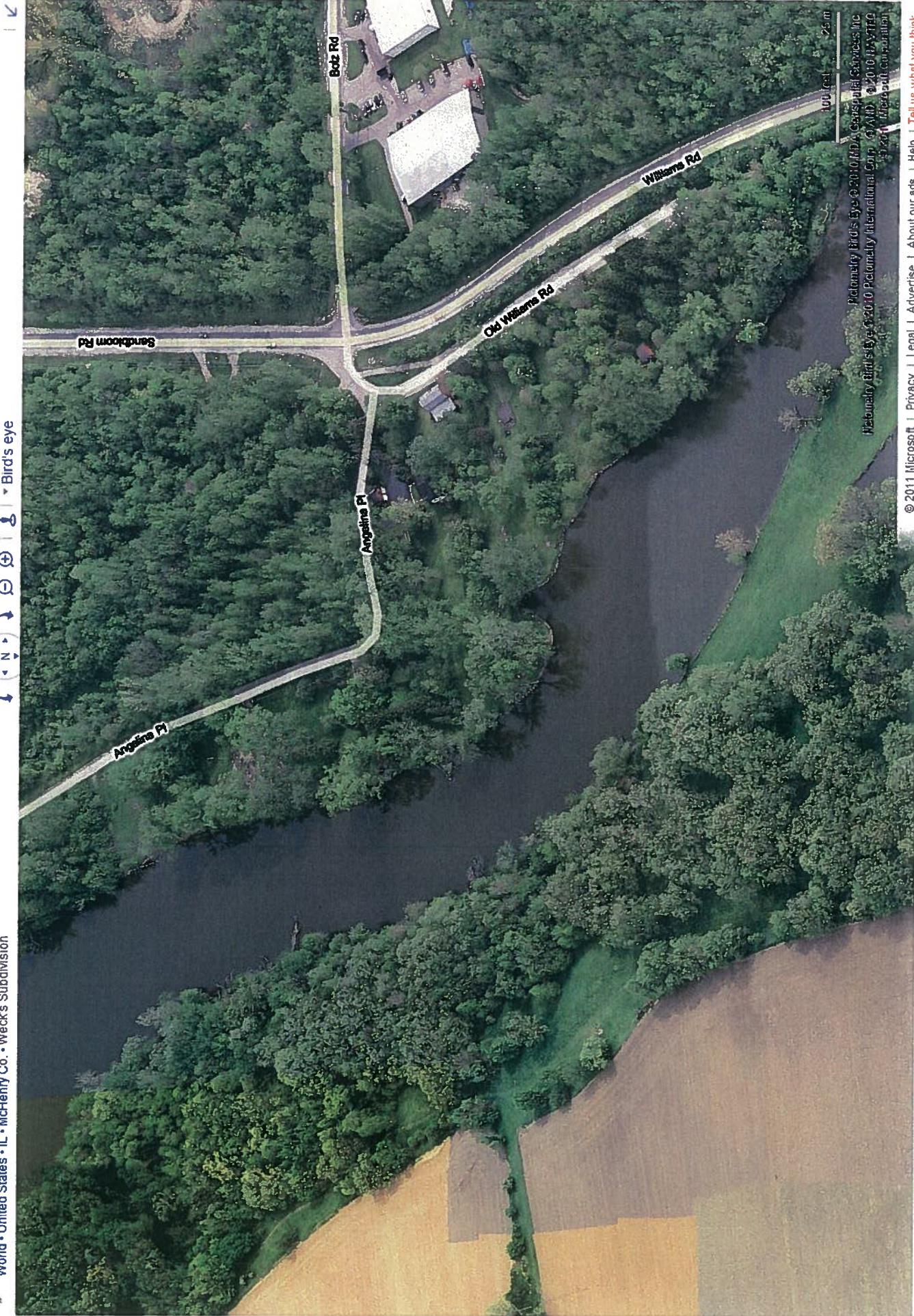


Maps

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[17] 17th March 2011

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## **SECTION 5**

**2002 FEMA FLOOD INSURANCE STUDY DATA AND RATE MAP  
2009 FEMA FLOOD INSURANCE STUDY DATA AND  
RATE MAP  
USGS GAGE AND STREAMSTATS DATA**

# **2002 FEMA FLOOD INSURANCE STUDY DATA AND RATE MAP**

---

Christopher B. Burke Engineering, Ltd.

Hydraulic Report  
Longmeadow Parkway over the  
Fox River

# FLOOD INSURANCE STUDY

VOLUME 1 OF 3



## KANE COUNTY, ILLINOIS AND INCORPORATED AREAS

Kane County



COMMUNITY NAME	COMMUNITY NUMBER
BATAVIA, CITY OF	170321
BIG ROCK, VILLAGE OF	171081
CARPENTERSVILLE, VILLAGE OF	170322
EAST DUNDEE, VILLAGE OF	170323
ELBURN, VILLAGE OF	171026
ELGIN, CITY OF	170087
GENEVA, CITY OF	170325
GILBERTS, VILLAGE OF	170326
HAMPSHIRE, VILLAGE OF	170327
HOFFMAN ESTATES, VILLAGE OF	170107
KANE COUNTY (UNINCORPORATED AREAS)	170896
LILY LAKE, VILLAGE OF	171023
MAPLE PARK, VILLAGE OF	171018
MONTGOMERY, VILLAGE OF	170328
NORTH AURORA, VILLAGE OF	170329
PINGREE GROVE, VILLAGE OF	171078
SLEEPY HOLLOW, VILLAGE OF	170331
SOUTH ELGIN, VILLAGE OF	170332
ST. CHARLES, CITY OF	170330
SUGAR GROVE, VILLAGE OF	170333
DIRL, VILLAGE OF	171024
WEST DUNDEE, VILLAGE OF	170335

DECEMBER 20, 2002



Federal Emergency Management Agency

FLOOD INSURANCE STUDY NUMBER  
17089CV001A

TABLE 4 - SUMMARY OF DISCHARGES - continued

<u>FLOODING SOURCE AND LOCATION</u>	<u>DRAINAGE AREA (sq. miles)</u>	<u>PEAK DISCHARGES (cfs)</u>			
		<u>10-YEAR</u>	<u>50-YEAR</u>	<u>100-YEAR</u>	<u>500-YEAR</u>
<b>FOX RIVER</b>					
Upstream of U.S. 30 and downstream of Ashland Avenue	1,710	12,100	17,050	18,700	24,100
Approximately 1.2 miles downstream of North Avenue	1,705	12,100	17,000	18,600	24,100
At Aurora Dam	1,705	5,950	8,400	9,180	11,900
At North Aurora	1,680	8,565	12,770	14,350	18,760
At confluence of Mill Creek	1,670	8,565	12,770	14,350	18,760
Approximately 319,757 feet from mouth	1,649	7,535	11,225	12,250	16,875
At River Station	294,500	1,629	8,500	12,500	13,500
At Geneva Dam	1,580	7,535	11,225	12,250	16,875
Approximately 356,400 feet from mouth	1,568	7,535	11,225	12,250	16,875
Approximately 359,964 feet from mouth	1,556	6,870	9,965	11,350	14,680
Just upstream of confluence of Norton Creek	1,540	7,535	11,225	12,250	16,875
At U.S. Route 20	1,532	6,870	9,965	11,305	14,680
At Lawrence Avenue	1,509	5,910	8,950	10,540	13,475
Approximately 8,400 feet upstream of confluence of Jelkes Creek	1,446	5,910	8,950	10,540	13,475
At Carpentersville Dam	1,425	5,775	8,345	10,095	12,525
At Algonquin approximately 428,541 feet from mouth	1,403	5,480	7,990	9,690	11,800
Approximately 7,000 feet downstream of upstream county boundary	1,390	5,775	8,345	10,095	12,525
<b>FOX RIVER EAST CHANNEL</b>					
At Aurora Dam	1,705	6,150	8,600	9,420	12,200

TABLE 5 - MANNING'S "n" VALUES - continued

<u>Stream</u>	<u>Channel "n"</u>	<u>Overbank "n"</u>
Bowes Creek	0.035-0.072	0.065-0.100
Bowes Creek Tributary	0.045	0.070
Brewster Creek	0.060	0.140
Carpenter Creek	0.10	0.12
Eakin Creek	0.05	0.06
Ferson Creek	0.030-0.08	0.070-0.100
Fitchie Creek	0.035-0.104	0.070-0.100
Four Winds Way Creek	0.10	0.12
Fox River	0.025-0.10	0.035-0.10
Fox River East Channel	0.025-0.040	0.060-0.070
Fox River Tributary (East Branch)	0.040	0.050
Fox River Tributary	0.020-0.060	0.050-0.080
Geneva Creek	0.040-0.085	0.04-0.10
Hampshire Creek	0.046-0.150	0.035-0.145
Hampshire Creek Tributary No. 1	0.050-0.120	0.050
Hampshire Creek Tributary No. 2	0.035-0.110	0.040-0.090
Hampshire Creek Tributary No. 3	0.065-0.075	0.050
Hampshire Creek Tributary No. 4	0.075-0.080	*
Hampshire Creek Tributary	0.030-0.100	0.030-0.080
Indian Creek	0.030-0.055	0.040-0.150
Indian Creek Tributary B	0.050-0.070	0.070-0.095
Jelkes Creek	0.035-0.070	0.050-0.090
Mahoney Creek	0.100	0.120
Main Street Ditch	0.07	0.08
McKee Road Tributary	0.035-0.090	0.050-0.100
Mill Creek	0.020-0.100	0.040-0.140
Mill Creek Tributary No. 2	0.060	0.080-0.100
North Arm Brewster Creek	0.080	0.120
Norton Creek	0.045-0.15	0.05-0.150
Norton Creek Tributary	0.030-0.055	0.045-0.055
Otter Creek	0.035-0.075	0.070-0.090
Otter Creek Tributary	0.035-0.055	0.070-0.085
Pingree Creek	0.055-0.065	0.05-0.08
Poplar Creek	0.015-0.040	0.060-0.080
Sandy Creek	0.04-0.09	0.07-0.12
Selmarten Creek	0.030-0.060	0.070-0.100
Sleepy Creek	0.05-0.100	0.10
South Tributary	0.050-0.060	0.075-0.150
Stoney Creek	0.030-0.072	0.060-0.110
Tyler Creek	0.045-0.07	0.05-0.1
Waubansee Creek	0.035-0.055	0.050-0.070
Welch Creek	0.045-0.090	0.050-0.090
Welch Creek Tributary No. 1	0.045-0.090	0.050-0.090

\*Data not available

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NGVD)			
CROSS SECTION	DISTANCE <sup>1</sup>	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Fox River (continued)								
CL	379,621	456	5,782	1.8	713.5	713.5	713.5	0.0
CM	381,396	720	6,117	1.7	713.7	713.7	713.7	0.0
CN	384,653	492	2,928	3.6	714.3	714.3	714.3	0.0
CO	387,061	517	5,643	1.9	715.1	715.1	715.2	0.1
CP	389,516	652	6,004	1.8	715.4	715.4	715.4	0.0
CQ	391,116	644	7,038	1.5	715.6	715.6	715.6	0.0
CR	392,769	576	5,783	1.8	715.7	715.7	715.8	0.1
CS	395,700	619	5,758	1.8	716.0	716.0	716.1	0.1
CT	397,400	1,101	7,398	1.4	716.3	716.3	716.4	0.1
CU	402,518	469	2,669	3.9	718.0	718.0	718.1	0.1
CV	404,511	281	2,865	3.7	719.8	719.8	719.9	0.1
CW	406,439	381	3,508	3.0	720.2	720.2	720.3	0.1
CX	407,468	374	3,841	2.7	720.6	720.6	720.7	0.1
CY	409,944	227	2,352	4.5	721.4	721.4	721.5	0.1
CZ	410,874	708	5,994	1.8	722.3	722.3	722.4	0.1
DA	414,015	575	4,181	2.4	725.3	725.3	725.4	0.1
DB	417,841	558	4,139	2.4	727.1	727.1	727.2	0.1
DC	420,346	650	4,101	2.5	728.4	728.4	728.5	0.1
DD	422,521	643	4,035	2.5	729.2	729.2	729.3	0.1
DE	424,528	787	5,117	2.0	730.1	730.1	730.2	0.1
DF	428,504	878	5,043	2.0	732.0	732.0	732.1	0.1

STUDY REACH

<sup>1</sup>Feet above mouth

TABLE 6

KANE COUNTY, IL  
AND INCORPORATED AREAS

FLOODWAY DATA 2002 FIS

FOX RIVER

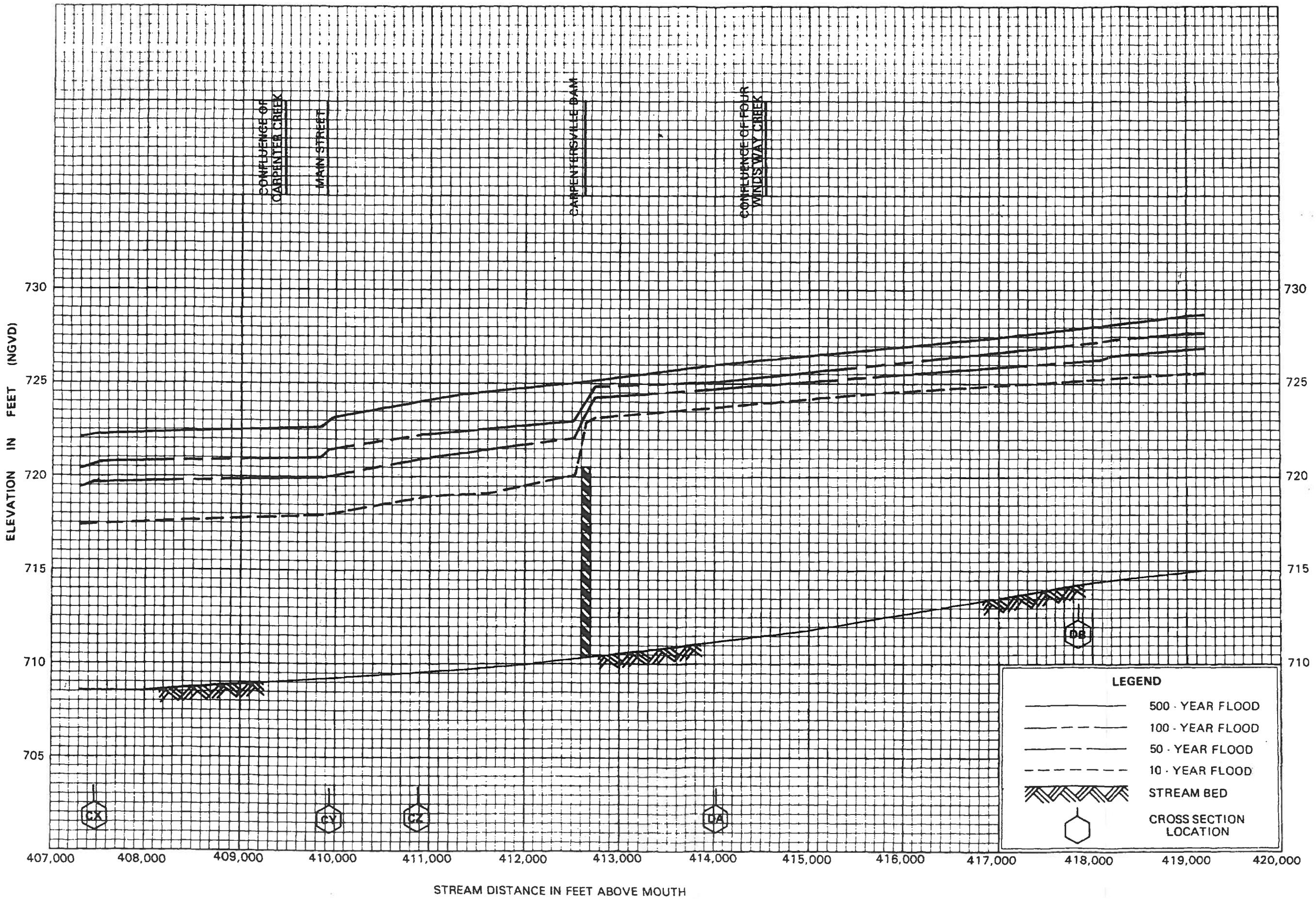
FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL  
AND INCORPORATED AREAS

FOX RIVER

FLOOD PROFILES 2002 FIS

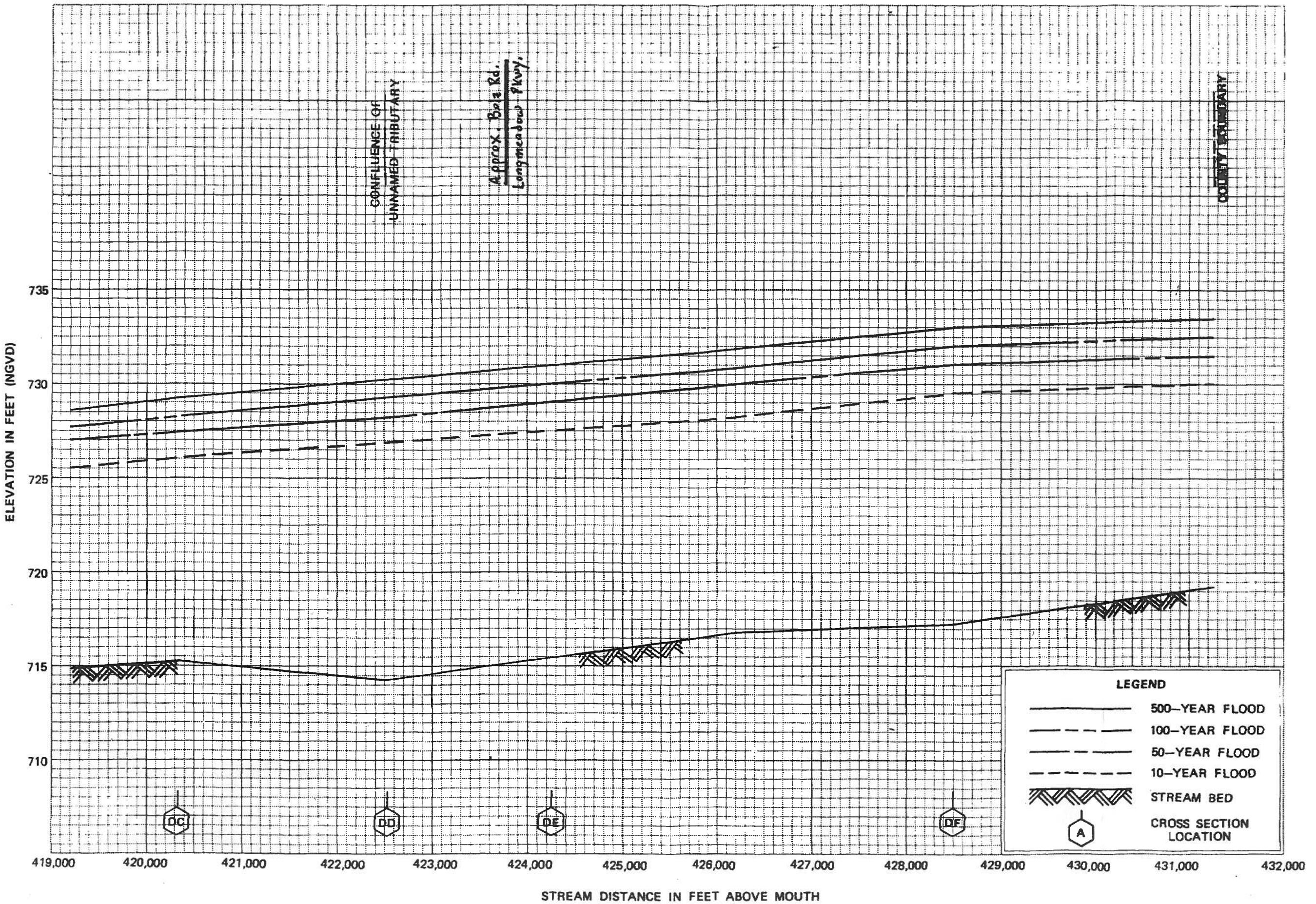
FEDERAL EMERGENCY MANAGEMENT AGENCY



## FOX RIVER

### FLOOD PROFILES 2002 FIS

FEDERAL EMERGENCY MANAGEMENT AGENCY  
KANE COUNTY, IL  
AND INCORPORATED AREAS



## ELEVATION REFERENCE MARKS

### REFERENCE MARK ELEVATION IN FT. (NGVD)<sup>1</sup>

RM 70-1 734.52 U.S. Coast and Geodetic Survey standard disk located approximately 87 feet east-southeast of intersection of Boltz Road and Algonquin Road, approximately 32 feet northeast of Chicago and North Western Railroad.

RM 70-2 733.78 Metal rivet in south end of west stone culvert headwall of Chicago and North Western Railroad located approximately 350 feet north of intersection of Algonquin Road and Chicago and North Western Railroad.

RM 70-3 777.91 60d nail driven into northwest side of power pole located at southeast corner of Lake Shore Drive and Algonquin Road.

<sup>1</sup>National Geodetic Vertical Datum of 1929

APPROXIMATE SCALE  
1000 FEET  
0  
1000

## NATIONAL FLOOD INSURANCE PROGRAM

# FIRM FLOOD INSURANCE RATE MAP

KANE COUNTY,  
ILLINOIS  
AND INCORPORATED AREAS

### PANEL 70 OF 410

(SEE MAP INDEX FOR PANELS NOT PRINTED)

CONTAINS:  
COMMUNITY NUMBER PANEL SUFFIX  
CARPENTERSVILLE, VILLAGE OF 770322 0070 F  
KANE COUNTY 770886 0070

Notice to User: The MAP NUMBER shown below should be used when placing map orders; the COMMUNITY NUMBER shown above should be used on insurance applications for the subject community.

**MAP NUMBER**  
**17089C0070F**

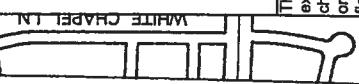
**EFFECTIVE DATE:**  
**DECEMBER 20, 2002**

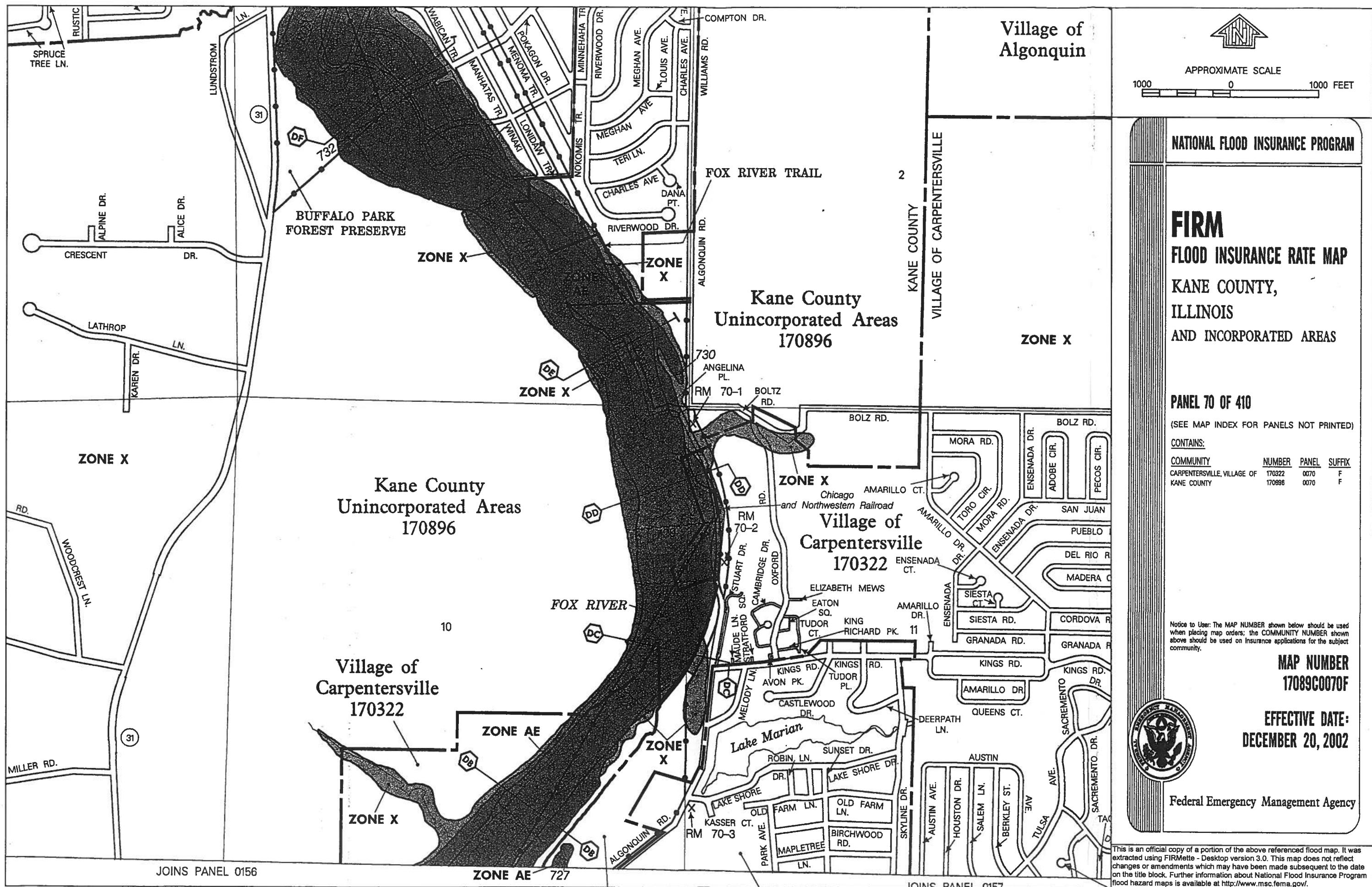


Federal Emergency Management Agency

This is an official copy of a portion of the above referenced flood map. It was extracted using FIRMatte - Desktop version 3.0. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. Further information about National Flood Insurance Program flood hazard maps is available at <http://www.msfc.fema.gov>.

JOINS PANEL 0065





This is an official copy of a portion of the above referenced flood map. It was extracted using FIRMette - Desktop version 3.0. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. Further information about National Flood Insurance Program flood hazard maps is available at <http://www.msfc.fema.gov>.

# **2009 FEMA FLOOD INSURANCE STUDY DATA AND RATE MAP**

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Christopher B. Burke Engineering, Ltd.

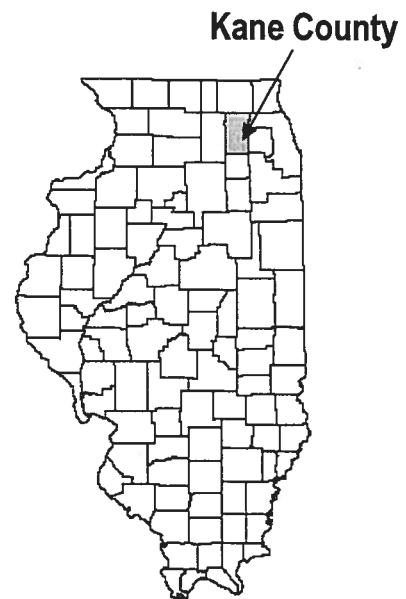
Hydraulic Report  
Longmeadow Parkway over the  
Fox River

# FLOOD INSURANCE STUDY



## KANE COUNTY, ILLINOIS AND INCORPORATED AREAS

Volume 1 of 3



COMMUNITY NAME	COMMUNITY NUMBER	COMMUNITY NAME	COMMUNITY NUMBER
ALGONQUIN, VILLAGE OF	170474	* KANEVILLE, VILLAGE OF	171396
AURORA, CITY OF	170320	LILY LAKE, VILLAGE OF	171023
BARRINGTON HILLS, VILLAGE OF	170058	MAPLE PARK, VILLAGE OF	171018
BARTLETT, VILLAGE OF	170059	MONTGOMERY, VILLAGE OF	170328
BATAVIA, CITY OF	170321	NORTH AURORA, VILLAGE OF	170329
BIG ROCK, VILLAGE OF	171081	PINGREE GROVE, VILLAGE OF	171078
* BURLINGTON, VILLAGE OF	171077	SLEEPY HOLLOW, VILLAGE OF	170331
CAMPTON HILLS, VILLAGE OF	171396	SOUTH ELGIN, VILLAGE OF	170332
CARPENTERSVILLE, VILLAGE OF	170322	ST. CHARLES, CITY OF	170330
EAST DUNDEE, VILLAGE OF	170323	SUGAR GROVE, VILLAGE OF	170333
ELBURN, VILLAGE OF	171026	VIRGIL, VILLAGE OF	171024
ELGIN, CITY OF	170087	WAYNE, VILLAGE OF	170865
GENEVA, CITY OF	170325	WEST DUNDEE, VILLAGE OF	170335
GILBERTS, VILLAGE OF	170326		
HAMPSHIRE, VILLAGE OF	170327		
* HOFFMAN ESTATES, VILLAGE OF	170107		
HUNTLEY, VILLAGE OF	170480		
KANE COUNTY (UNINCORPORATED AREAS)	170896		

\* NO SPECIAL FLOOD HAZARD AREAS IDENTIFIED IN  
KANE COUNTY



Revised August 3, 2009

Federal Emergency Management Agency

FLOOD INSURANCE STUDY NUMBER

17089CV001C

to the present. Historic floods and the corresponding river stages are shown in Table 7, "Historical Flood Data."

**Table 7 – Historical Flood Data**  
**Fox River at Algonquin, Illinois - USGS Gage Number 05550000**

Datum of gage is 729.48 feet NGVD 1929

<u>Date</u>	<u>Peak Streamflow (cfs)</u>	<u>River Stage (feet)</u>
5/22/2004	6,720	3.09
4/2/1979	6,610	4.00
10/3/1987	6,170	3.99
4/23/1993	6,150	3.75
4/1/1916	5,850	4.50
7/5/1938	5,630	4.37
3/16/1929	5,450	4.42
6/17/2000	5,080	3.42
5/23/1996	4,570	3.43

Flooding and damages in the Blackberry Creek watershed area have increased over the past two decades. Major flood damage in the Blackberry Creek watershed area resulted during the storms of July 1983, July 1996, and February 1997. The storm of July 17-18, 1996 caused damage to over 1,000 homes and over \$13 million in damages (Reference 44).

In Batavia, the primary cause of flooding in the Mahoney Creek basin is usually a combination of snowmelt and rainfall. The approximately 2.39 square mile Mahoney Creek watershed is located entirely within Batavia's planning boundaries. Because of extensive urban development and steep overland slopes in the watershed, storm water runoff moves quickly toward the Mahoney Creek Tributary and rapidly through the stream channel itself. Consequently, high intensity, short duration storms tend to produce higher flood flows (Reference 45).

In Hampshire, the primary cause of flooding in the Hampshire Creek basin is usually a combination of snowmelt and rainfall events. Known flood events on Hampshire Creek South occurred in 1960, 1968, 1972, and 1979. Information on historical floods in the area was obtained from a gaging station on Hampshire Creek from the Village of Hampshire.

In Maple Park, the principal flooding problems are caused by the overflow of Union Ditch No. 2 in the southern section of the village.

In Montgomery, severe flooding occurs along the Fox River and the Waubonsee Creek near the Parkview Estates area. The principal causes of the Waubonsee Creek flooding include the channel's inadequate hydraulic capacity and a severe flow restriction caused by a railroad crossing. The flood of August 26, 1972,

**Table 8 - Summary of Discharges (Continued)**

<i>Flooding Source and Location</i>	<i>Drainage Area (square miles)</i>	<i>Peak Discharges (cubic feet per second)</i>			
		<i>10-Percent- Annual-Chance</i>	<i>2-Percent- Annual-Chance</i>	<i>1-Percent- Annual-Chance</i>	<i>0.2-Percent- Annual-Chance</i>
<b>FOX RIVER (Continued)</b>					
Approximately 356,400 feet from mouth	1,568	7,535	11,225	12,250	16,875
Approximately 359,964 feet from mouth	1,556	6,870	9,965	11,350	14,680
Just upstream of confluence of Norton Creek	1,540	7,535	11,225	12,250	16,875
At U.S. Route 20	1,532	6,870	9,965	11,305	14,680
At Lawrence Avenue	1,509	5,910	8,950	10,540	13,475
Approximately 8,400 feet upstream of confluence of Jelkes Creek	1,446	5,910	8,950	10,540	13,475
At Carpentersville Dam	1,425	5,775	8,345	10,095	12,525
At Algonquin approximately 428,541 feet from mouth	1,403	5,480	7,990	9,690	11,800
Approximately 7,000 feet downstream of upstream county boundary	1,390	5,775	8,345	10,095	12,525
<b>FOX RIVER EAST CHANNEL</b>					
At Aurora Dam	1,705	6,150	8,600	9,420	12,200
<b>FOX RIVER TRIBUTARY</b>					
Upstream of confluence with Fox River	1.9	134	282	360	510
<b>FOX RIVER TRIBUTARY (EAST BRANCH)</b>					
Upstream of confluence with Fox River Tributary	0.3	25	56	75	105
<b>GENEVA CREEK</b>					
Just downstream of the Chicago and North Western railroad yard	1.2	323	521	539	689
At South Street	1.1	305	466	568	784
<b>HAMPSHIRE CREEK</b>					
Approximately 80 feet downstream of confluence of Hampshire Creek South	5.8	745	*	1,406	*

\*Data not available

The water-surface elevations for Mill Creek were determined by the slope/area method and a rating curve from a cross section located 2,714 feet downstream of Kaneville Road.

Starting water-surface elevations were calculated using corresponding flood elevations on the main stem, flood profiles from previous studies by the State of Illinois for Brewster Creek, Ferson Creek, Hampshire Creek South, Mahoney Creek, McKee Road Tributary, Mill Creek, and North Arm Brewster Creek, and rating curves (Reference 4, 72, 73).

The water-surface elevations on the Fox River, the Fox River East Channel, and Waubonsee Creek were computed using the USACE HEC-2 step-backwater program (Reference 69). Cross sections and structural data for the Fox River and the Fox River East Channel were provided by the IDOT-DWR from a 1960 field survey (Reference 74, 75). Cross sections and structural data for Waubonsee Creek were obtained from the Illinois State Water Survey (Reference 72). Cross sections for the backwater analyses were located at close intervals above and below bridges and culverts in order to compute the significant backwater effects from these structures.

The only serious backwater effect due to bridge constriction is on Waubonsee Creek. Backwater effects from Montgomery Dam have become a problem according to residents in the area. This situation was also studied.

In the unincorporated areas of Kane, starting water-surface elevations on the Fox River, the Fox River East Channel, and Waubonsee Creek were based on the slope/area method. Starting elevations for the 10-, 2-, 1-, and 0.2-percent-annual-chance floods for the Fox River, in other communities, were based upon the discharge recurrence interval rating curves at Carpentersville Dam, Elgin Dam, Geneva Dam, and South Elgin Dam (Reference 76).

In Montgomery, water-surface profiles for Fox River Tributary and Fox River Tributary (East Branch) were determined for the 10-, 2-, 1-, and 0.2-percent-annual-chance floods by use of the USACE HEC-2 computer program. Starting water-surface elevations were determined with either known high-water marks, by assuming critical depth, or by the slope/area method.

Water-surface elevation for floods of the selected recurrence intervals of Four Winds Way Creek and Carpenter Creek were computed through use of the USACE HEC-2 step-backwater computer program (Reference 69). This program relates stream geometry, characteristics, and discharge to stream elevation. Flood profiles were drawn showing computed water-surface elevations to an accuracy of 0.5 foot for floods of selected recurrence intervals.

Starting water-surface elevations for Carpenter Creek and Four Winds Way Creek were determined using normal depth analysis. Flood elevations can often be increased by ice jams during spring thaws or by debris clogging bridges.

Water-surface elevations of floods of the selected recurrence intervals of Geneva Creek were computed through use of the USACE HEC-2 backwater computer program (Reference 69). The starting water-surface elevation for Geneva Creek

### 3.3 Vertical Datum

All FISs and FIRMs are referenced to a specific vertical datum. The vertical datum provides a starting point against which flood, ground, and structure elevations can be referenced and compared. Until recently, the standard vertical datum in use for newly created or revised FISs and FIRMs was the National Geodetic Vertical Datum of 1929 (NGVD 29). With the finalization of the North American Vertical Datum of 1988 (NAVD 88), many FIS and FIRMs are being prepared using NAVD 88 as the referenced vertical datum.

All flood elevations shown in this FIS and on the FIRM are referenced to NAVD 88. Structure and ground elevations in the community must, therefore, be referenced to NAVD 88. It is important to note that adjacent counties may be referenced to NGVD 29. This may result in differences in base flood elevations (BFEs) across the county boundary.

Effective information for this FIS was converted from NGVD 29 to NAVD 88 based on data presented in Figure 1 and Table 11a. Computations show an average conversion factor of -0.206 feet (NGVD 29 – 0.206 = NAVD 88) for the county. The Single Conversion Factor (countywide) method was applied uniformly across the county, except as noted below, and used to prepare the Summary of Stillwater Elevations Table, Floodway Data Tables, Flood Profiles, and FIRMs.

The Multiple Conversion Factors (stream-by-stream) method was implemented for a stream when a detailed study reach was located in two or more counties (multi-county stream) and the countywide conversion factor for each county differed by more than one-tenth of a foot. For the stream-by-stream method, the stream is assigned an average conversion factor based on the conversion factors computed at three points along the stream. These results are shown in Table 11b.

For more information on NAVD 88, see *Guidelines and Specifications for Flood Hazard Mapping Partners Appendix B: Guidance for Converting to the North American Vertical Datum of 1988* (Reference 85) available at [http://www.fema.gov/plan/prevent/fhm/dl\\_cgs.shtm](http://www.fema.gov/plan/prevent/fhm/dl_cgs.shtm) or contact the Vertical Network Branch, National Geodetic Survey, Coast and Geodetic Survey, National Oceanic and Atmospheric Administration, Rockville, Maryland 20910 (Internet address <http://www.ngs.noaa.gov>).

Temporary vertical monuments are often established during the preparation of a flood hazard analysis for the purpose of establishing local vertical control. Although these monuments are not shown on the FIRM, they may be found in the Technical Support Data Notebook associated with the FIS report and FIRM for this county. Interested individuals may contact FEMA to access these data.

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Fox River (Continued)								
CY (D4)	410,874 <sup>14</sup>	708	5,994	1.8	722.1	722.2	722.2	0.1
CZ (D4)	414,015 <sup>14</sup>	575	4,181	2.4	725.1	725.2	725.2	0.1
DA (D5)	417,841 <sup>14</sup>	558	4,139	2.4	726.9	727.0	727.0	0.1
DB (Pc)	420,346 <sup>14</sup>	650	4,101	2.5	728.2	728.3	728.3	0.1
DC (D5)	422,521 <sup>14</sup>	643	4,035	2.5	729.0	729.1	729.1	0.1
DD (Pc)	424,528 <sup>14</sup>	787	5,117	2.0	729.9	729.9	730.0	0.1
DE (D5)	428,504 <sup>14</sup>	878	5,043	2.0	731.8	731.9	731.9	0.1
<i>(1002 Feet)</i>								
Fox River East Channel								
A	253,290 <sup>1</sup>	144	1,335	6.3	624.8	624.8	624.8	0.0
B	254,170 <sup>1</sup>	156	1,471	5.7	626.1	626.2	626.2	0.1
C	254,360 <sup>1</sup>	160	1,261	6.7	626.3	626.4	626.4	0.1
D	257,060 <sup>1</sup>	164	1,815	5.2	629.1	629.2	629.2	0.1
E	257,060 <sup>1</sup>	162	1,725	5.5	629.3	629.4	629.4	0.1
F	257,060 <sup>1</sup>	171	1,580	6.0	629.4	629.5	629.5	0.1
G	257,060 <sup>1</sup>	171	1,680	5.9	629.6	629.7	629.7	0.1
H	257,060 <sup>1</sup>	235	2,716	3.5	634.8	634.8	634.8	0.0
Fox River Tributary								
A	2,300 <sup>16</sup>	250	223	1.61	640.6	640.6	640.7	0.1

<sup>1</sup>Feet above confluence with Fox River

<sup>14</sup>Feet above mouth at Illinois River

<sup>16</sup>Feet above confluence with Fox River Tributary (East Branch)

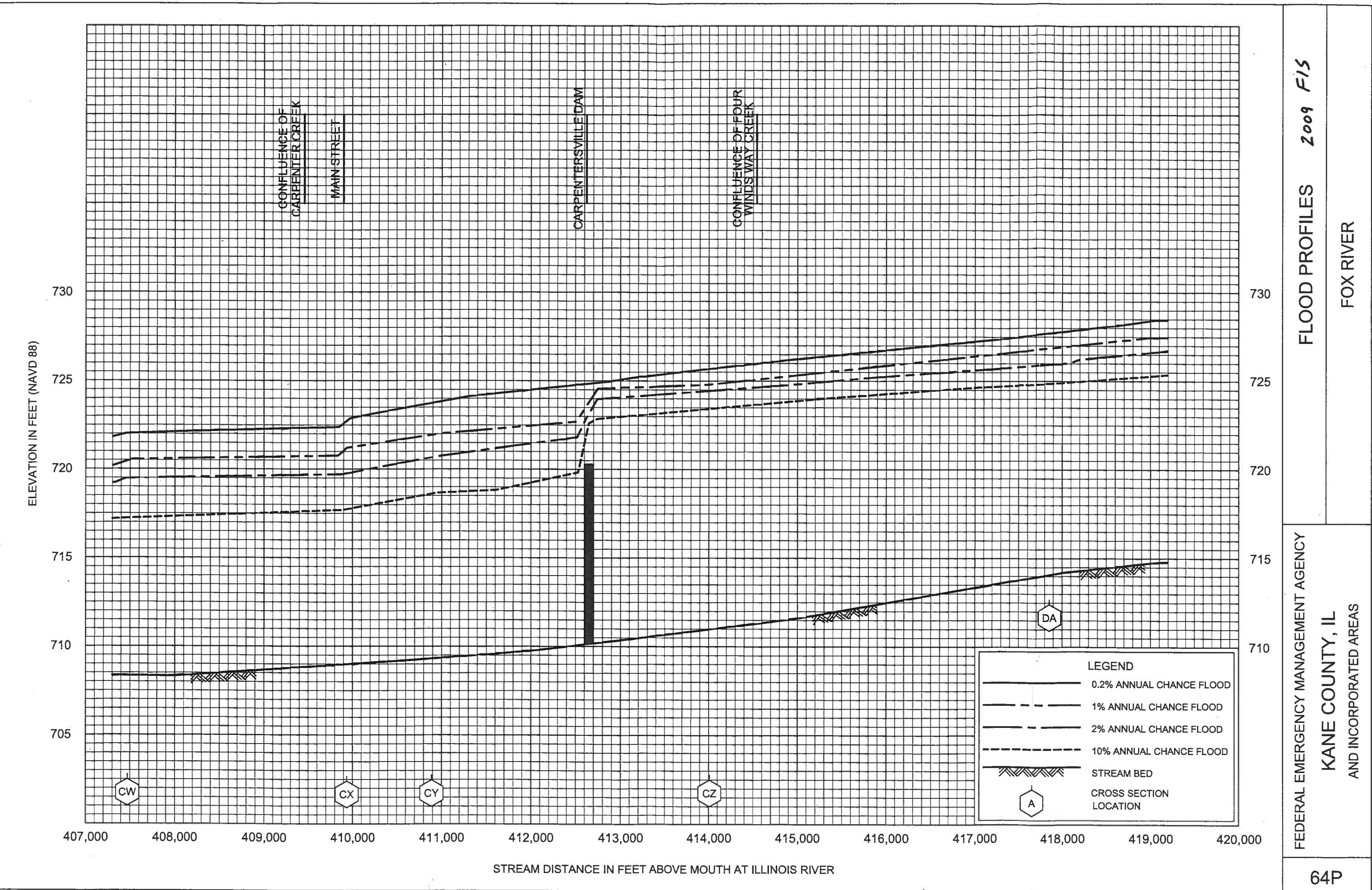
#### FEDERAL EMERGENCY MANAGEMENT AGENCY

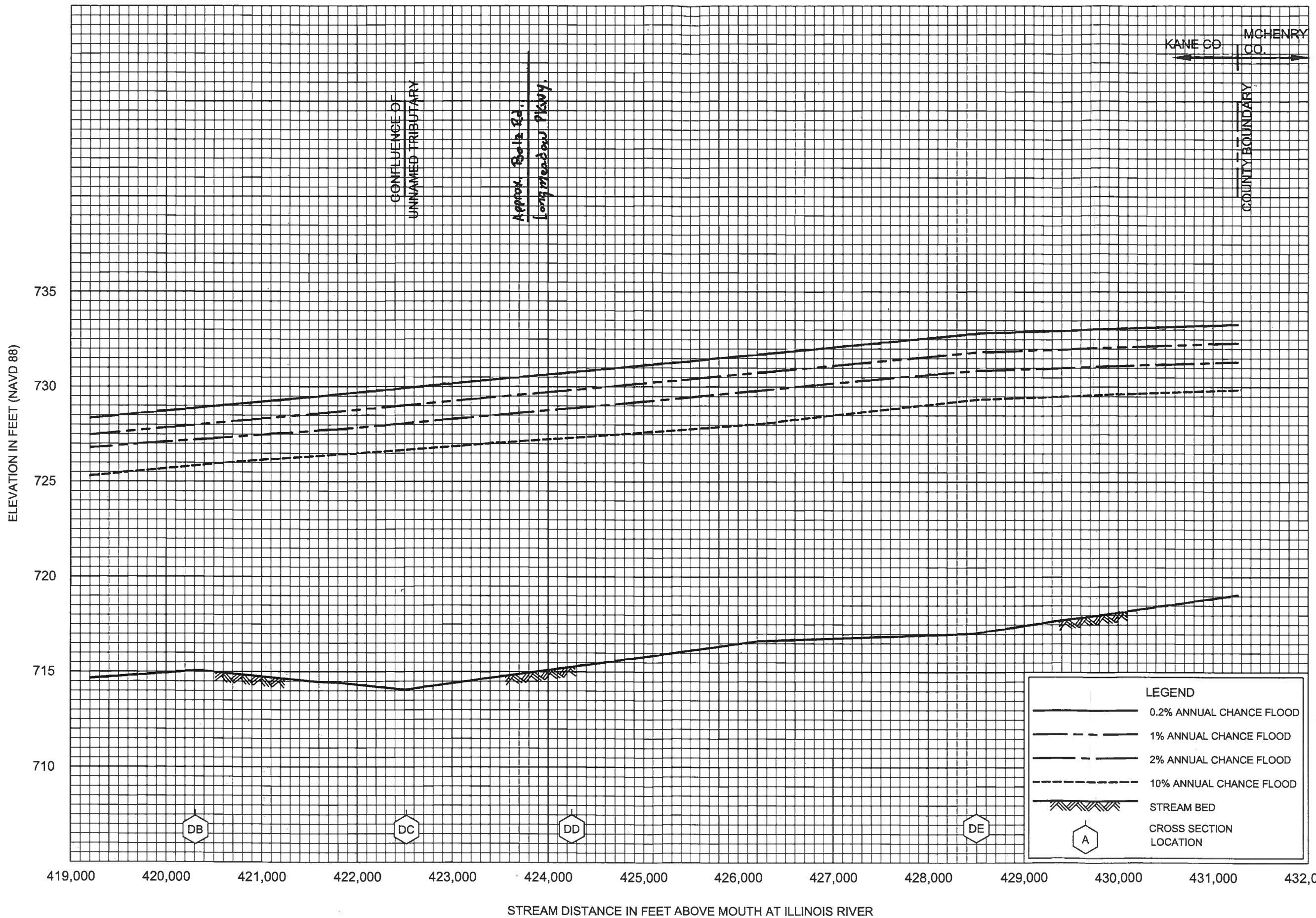
#### FLOODWAY DATA

#### KANE COUNTY, IL AND INCORPORATED AREAS

#### FOX RIVER - FOX RIVER EAST CHANNEL - FOX RIVER TRIBUTARY

TABLE 12





FEDERAL EMERGENCY MANAGEMENT AGENCY  
KANE COUNTY, IL  
AND INCORPORATED AREAS

FLOOD PROFILES 2009 FIS  
FOX RIVER



**MAP SCALE 1" = 1000'**

A scale bar indicating distances of 500, 0, 1000, and 2000 feet. The bar is marked at 500, 1000, and 2000, with 0 in the center. The word "FEET" is written below the bar.

ETEI

PANEL 0070H

**FIRM  
FLOOD INSURANCE RATE MAP  
KANE COUNTY,  
ILLINOIS  
AND INCORPORATED AREAS**

PANEL 70 OF 410

(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

10

# NATIONAL FLOOD INSURANCE PROGRAM

**Notice to User:** The Map Number shown below should be used when placing map orders; the Community Number shown above should be used on insurance applications for the subject community.

**MAP NUMBER  
17089C0070H**

**MAP REVISED  
AUGUST 3, 2009**



Federal Emergency Management Agency

This is an official copy of a portion of the above referenced flood map. It was extracted using FIRMette - Desktop version 3.0. This map does not reflect changes or amendments which may have been made subsequent to

# **USGS GAGE AND STREAMSTATS DATA**

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Christopher B. Burke Engineering, Ltd.

Hydraulic Report  
Longmeadow Parkway over the  
Fox River

## Water-Data Report 2009

**05550000 FOX RIVER AT ALGONQUIN, IL**

Upper Illinois Basin  
Upper Fox Subbasin

LOCATION.--Lat 42°09'59", long 88°17'25" referenced to North American Datum of 1927, in SW ¼ NE ¼ NW ¼ sec.34, T.43 N., R.8 E., McHenry County, IL, Hydrologic Unit 07120006, on right bank 20 ft upstream from bridge on State Highway 62 (Algonquin Road) in Algonquin, 140 ft upstream from Algonquin Dam, and at mile 81.6.

DRAINAGE AREA.--1,403 mi<sup>2</sup>.

**SURFACE-WATER RECORDS****PERIOD OF RECORD.—****SURFACE-WATER DISCHARGE AND STAGE**

DISCHARGE: October 1915 to September 2009. Monthly discharge only for some periods, published in WSP 1308.

STAGE: Water years 1994 to current year.

**BIOLOGICAL**

ALGAE: Water years 1989-90.

**REVISED RECORDS.--WSP 1175: 1916. WDR IL-75-1: Drainage area.**

GAGE.--Water-stage recorder, phone telemeter, and concrete dam with adjustable gate. Datum of gage is 729.48 ft above NGVD of 1929 (729.29 ft NAVD 1988). Prior to Oct. 20, 1933, non-recording gage at site 20 ft downstream at same datum.

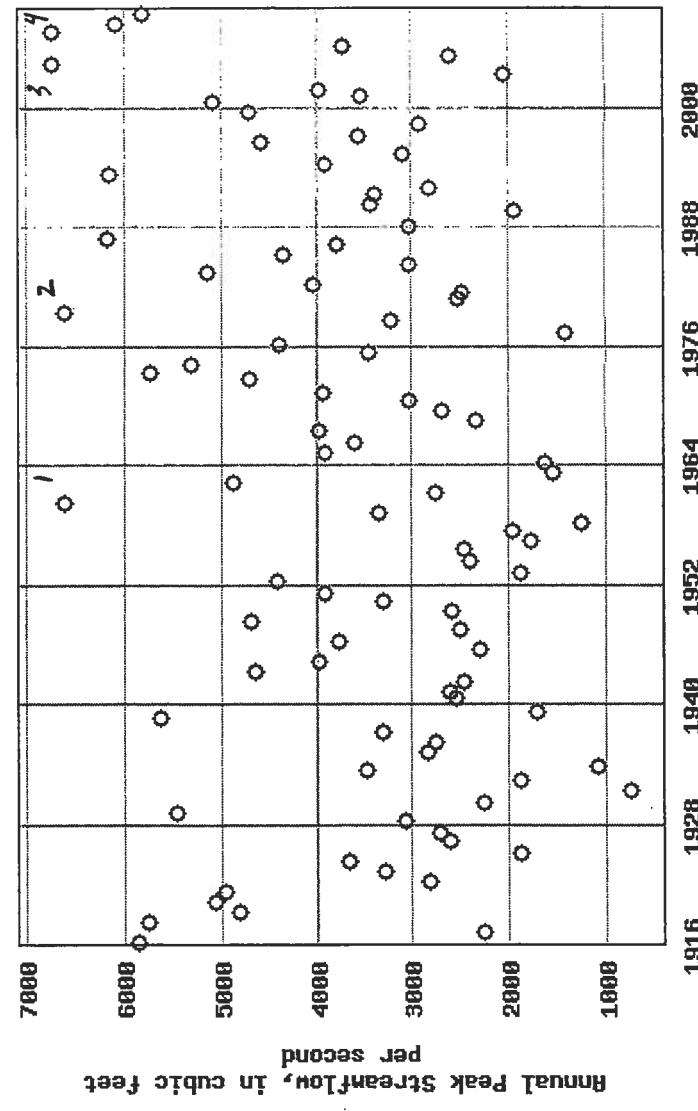
REMARKS.--Records fair except for estimated daily discharges, which are poor. Flow regulated by gate in Algonquin Dam, Stratton Lock and Dam (previously known as McHenry Dam) 16 mi upstream from station, and occasionally affected by wind action. Adjustable gate installed in 2002.

**EXTREMES FOR PERIOD OF RECORD.—**

**SURFACE-WATER DISCHARGE AND STAGE:** Maximum discharge, 6,720 ft<sup>3</sup>/s, May 22, 2004 and Aug. 25, 2007, gage heights 3.09 ft and 3.50 ft respectively, due to regulation; maximum gage height, 4.50 ft, April 1, 1916; from graph based on gage readings; no flow, Nov. 26, 1952, Nov. 20, 1953, Oct. 25, 1956 and Sept. 9, 14, 1958, result of windstorm.



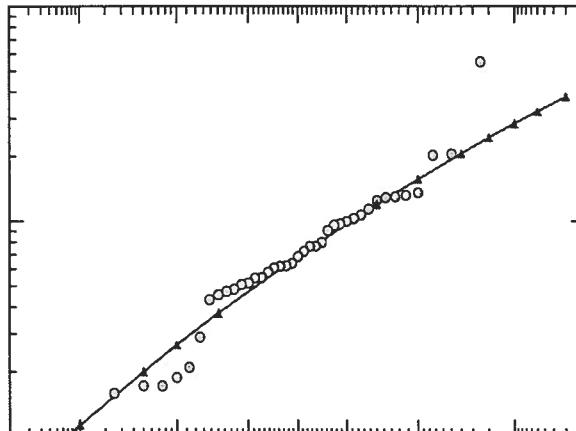
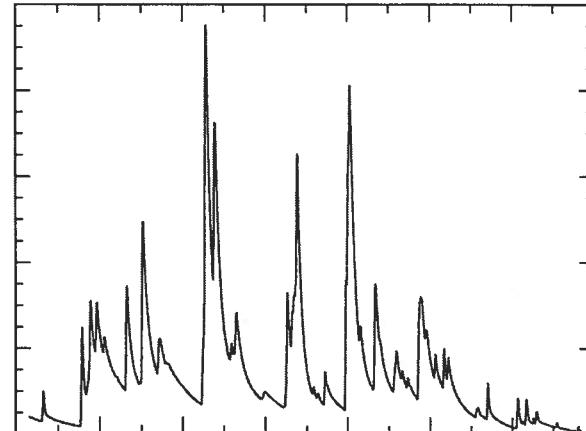
## USGS 05550000 FOX RIVER AT ALGONQUIN, IL



1. April 6, 1940 6,610 cfs
2. April 2, 1979 6,610 cfs
3. May 22, 2004 6,720 cfs
4. Aug. 25, 2007 6,720 cfs

In cooperation with the Illinois Department of Natural Resources, Offices of Water Resources, Realty and Environmental Planning—Conservation 2000 Program, and Resource Conservation; and with the Illinois Department of Transportation

## Estimating Flood-Peak Discharge Magnitudes and Frequencies for Rural Streams in Illinois



Scientific Investigations Report 2004-5103

**Table 1.** Flood-peak discharges for recurrence intervals,  $T$ , of 2, 5, 10, 25, 50, 100, and 500 years estimated from the annual maximum series at streamflow-gaging stations in Illinois and adjacent States.

[ $T$ , recurrence interval in years,  $Q_T$ , instantaneous peak-flood discharge, in cubic feet per second, for a given  $T$  of 2-, 5-, 10-, 25-, 50-, 100-, and 500-year flood. Three estimates are listed for each station: the values in the top row are  $Q_T$  from at-site frequency curves; values in the middle row are  $Q_T$  from regional regression equations; values in the bottom row are  $Q_T$  obtained by weighting the at-site and regional regression frequency curves; NA, not assigned; dashes (--) given in any  $Q_T$  row indicates that the corresponding frequency curves are not computed. Station noted by an asterisk (\*) have anomalous characteristics and are omitted from the regional analysis]

Station number (figs. 2A and 2B)	Station Name	Hydrologic Region	Flood Quantiles of Selected Recurrence Interval						
			$Q_2$	$Q_5$	$Q_{10}$	$Q_{25}$	$Q_{50}$	$Q_{100}$	$Q_{500}$
03336100	Big Four Ditch Tributary near Paxton, Ill.	3	115 132 117	183 251 192	228 342 245	284 467 315	324 565 368	363 667 420	450 913 542
03336500	Bluegrass Creek at Potomac, Ill.	3	1,840 1,310 1,780	2,870 2,350 2,810	3,580 3,130 3,520	4,490 4,160 4,440	5,180 4,970 5,150	5,870 5,780 5,870	7,500 7,750 7,570
03336645	Middle Fork Vermilion River above Oakwood, Ill.	3	6,500 5,560 6,380	9,470 9,510 9,470	11,400 12,300 11,500	13,800 16,000 14,200	15,500 18,800 16,100	17,200 21,700 18,100	21,000 28,300 22,600
03336900	Salt Fork near St. Joseph, Ill.	3	2,530 2,580 2,530	3,810 4,560 3,880	4,740 6,010 4,870	5,990 7,940 6,220	6,970 9,420 7,280	8,000 10,900 8,390	10,600 14,600 11,200
03337000	Boneyard Creek at Urbana, Ill.	NA	533 --- ---	705 --- ---	815 --- ---	951 --- ---	1,050 --- ---	1,150 --- ---	1,370 --- ---
03337500	Saline Branch at Urbana, Ill.	3	1,300 1,070 1,280	2,110 1,840 2,100	2,680 2,380 2,650	3,410 3,090 3,380	3,970 3,630 3,930	4,530 4,170 4,480	5,850 5,440 5,790
01	Salt Fork near Homer, Ill.	3	3,780 4,140 3,800	6,120 7,120 6,200	7,880 9,250 8,000	10,330 12,000 10,500	12,300 14,200 12,500	14,400 16,300 14,600	19,800 21,400 20,100
03338100	Salt Fork Trib near Catlin, Ill.	3	189 188 189	375 354 371	515 481 509	703 655 693	847 792 836	991 934 980	1,330 1,280 1,320
03338500	Vermilion River near Catlin, Ill.	3	8,570 9,600 8,700	15,000 16,400 15,200	20,300 21,200 20,400	27,900 27,600 27,900	34,500 32,400 34,000	41,700 37,300 40,600	61,400 48,900 58,100
03338780	North Fork Vermilion River near Bismarck, Ill.	3	8,010 3,900 6,900	13,300 6,780 11,300	17,200 8,870 14,300	22,500 11,600 18,200	26,600 13,700 21,200	30,900 15,800 24,300	41,500 20,900 32,000
03338800	N F Vermilion River Tributary near Danville, Ill.	3	292 199 274	539 387 505	737 535 685	1,030 740 941	1,270 904 1,150	1,530 1,080 1,380	2,220 1,500 1,960
03339000	Vermilion River near Danville, Ill.	3	14,200 12,200 14,200	22,200 20,800 22,100	27,600 27,000 27,600	34,600 35,100 34,700	39,900 41,300 40,000	45,200 47,500 45,300	57,600 62,400 58,000

Table 1. Flood-peak discharges for recurrence intervals, T, of 2, 5, 10, 25, 50, 100, and 500 years estimated from the annual maximum series

Station number (figs. 2A and 2B)	Station Name	Hydrologic Region	Flood Quantiles of Selected Recurrence Interval						
			$Q_2$	$Q_5$	$Q_{10}$	$Q_{25}$	$Q_{50}$	$Q_{100}$	$Q_{500}$
05549850	Flint Creek near Fox River Grove, Ill.	2	275 507 285	380 794 401	449 987 481	535 1,220 584	599 1,390 660	663 1,560 736	812 1,920 911
05549900	Fox River Tributary near Cary, Ill.	2		11 18 12	25 32 26	37 44 38	56 58 56	72 69 71	90 81 88
05550000	<u>Fox River at Algonquin, Ill.</u>	NA		3,270 ---	4,520 ---	5,280 ---	6,190 ---	6,820 ---	7,420 ---
05550300	Tyler Creek at Elgin, Ill.	2	342 773 370	457 1,270 514	531 1,610 618	622 2,040 754	688 2,350 855	753 2,650 955	902 3,340 1,180
05550430	East Branch Poplar Creek near Palatine, Ill.	NA		87 ---	125 ---	151 ---	184 ---	210 ---	235 ---
05550450	Poplar Creek near Ontarioville, Ill.	NA		211 ---	290 ---	340 ---	398 ---	440 ---	480 ---
05550470	Poplar Creek Trib near Bartlett, Ill.	2	162 155 161	240 257 242	295 327 299	365 414 373	419 477 429	474 539 486	606 678 620
05550500	Poplar Creek at Elgin, Ill.	NA		431 ---	643 ---	781 ---	951 ---	1,070 ---	1,190 ---
05551030	Brewster Creek at Valley View, Ill.	2	236 308 242	411 498 419	535 627 546	695 787 709	815 903 830	933 1,020 949	1,210 1,270 1,220
05551050	Norton Creek near Wayne, Ill.	2	97 192 103	201 311 210	295 392 305	443 492 450	576 565 574	730 635 713	1,180 791 1,090
05551060	Norton Creek near St. Charles, Ill.	2	123 292 133	216 478 235	290 604 319	397 762 439	486 876 537	584 988 641	845 1,240 908
05551200	Ferson Creek near St. Charles, Ill.	2	929 1,120 938	1,530 1,850 1,550	1,910 2,370 1,940	2,370 3,010 2,420	2,680 3,490 2,750	2,970 3,960 3,070	3,560 5,030 3,730
05551520	Indian Creek near North Aurora, Ill.	2	133 92 129	217 148 208	275 186 262	350 232 329	406 264 379	463 296 429	595 365 543
05551530	Indian Creek at Aurora, Ill.	NA		525 ---	659 ---	733 ---	814 ---	866 ---	913 ---

## Fox River at Longmeadow Parkway

### Explanation

- IDOT Structures
- △ GlobalWatershedPoint
- ◆ Site0BSPPoint
- ◆ BasinLengthPoint
- LongestFlowPath
- BasinLengthL
- Stream Gages**
  - △ Gaging Station, Continuous
  - LowFlowPartial record
  - ▲ Peak Flow Partial record
  - △ Peak and LowFlow, Partial record
  - ▲ Miscellaneous Record
  - ▲ Unknown
- Stream Grid
- GlobalWatershed
- ExcludePoly
- hubby



STREAM STATS

4/1/2011 1:12:09 PM



Illinois StreamStats

## Streamstats Ungaged Site Report

Date: Fri Apr 1 2011 13:06:31 Mountain Daylight Time

Site Location: Illinois

NAD27 Latitude: 42.1395 (42 08 22)

NAD27 Longitude: -88.2801 (-88 16 48)

NAD83 Latitude: 42.1396 (42 08 22)

NAD83 Longitude: -88.2802 (-88 16 49)

Drainage Area: 1436.03 mi<sup>2</sup>

Peak Flow Basin Characteristics				
100% Region 2 AMS (1440 mi <sup>2</sup> )				
Parameter	Value	Regression Equation Valid Range		
		Min	Max	
Drainage Area (square miles)	1440	0.03	9554	
Stream Slope 10 and 85 Method (feet per mi )	1.030	0.81	317	
Percent Open Water AND Herb Wetland (percent)	7.419	0	8	

Peak Flow Streamflow Statistics					
Statistic	Flow (ft <sup>3</sup> /s)	Prediction Error (percent)	Equivalent years of record	90-Percent Prediction Interval	
				Minimum	Maximum
PK2	3360	40	2.6	1760	6440
PK5	4760	41	3.1	2480	9150
PK10	5610	5175 *	42	2860	11000
PK25	6580	45	4.6	3220	13400
PK50	7280	8345 *	47	3460	15300
PK100	7860	10, 045 *	49	3610	17100
PK500	9220	12, 525 *	55	3900	21800

\* FIS Regulatory Flows

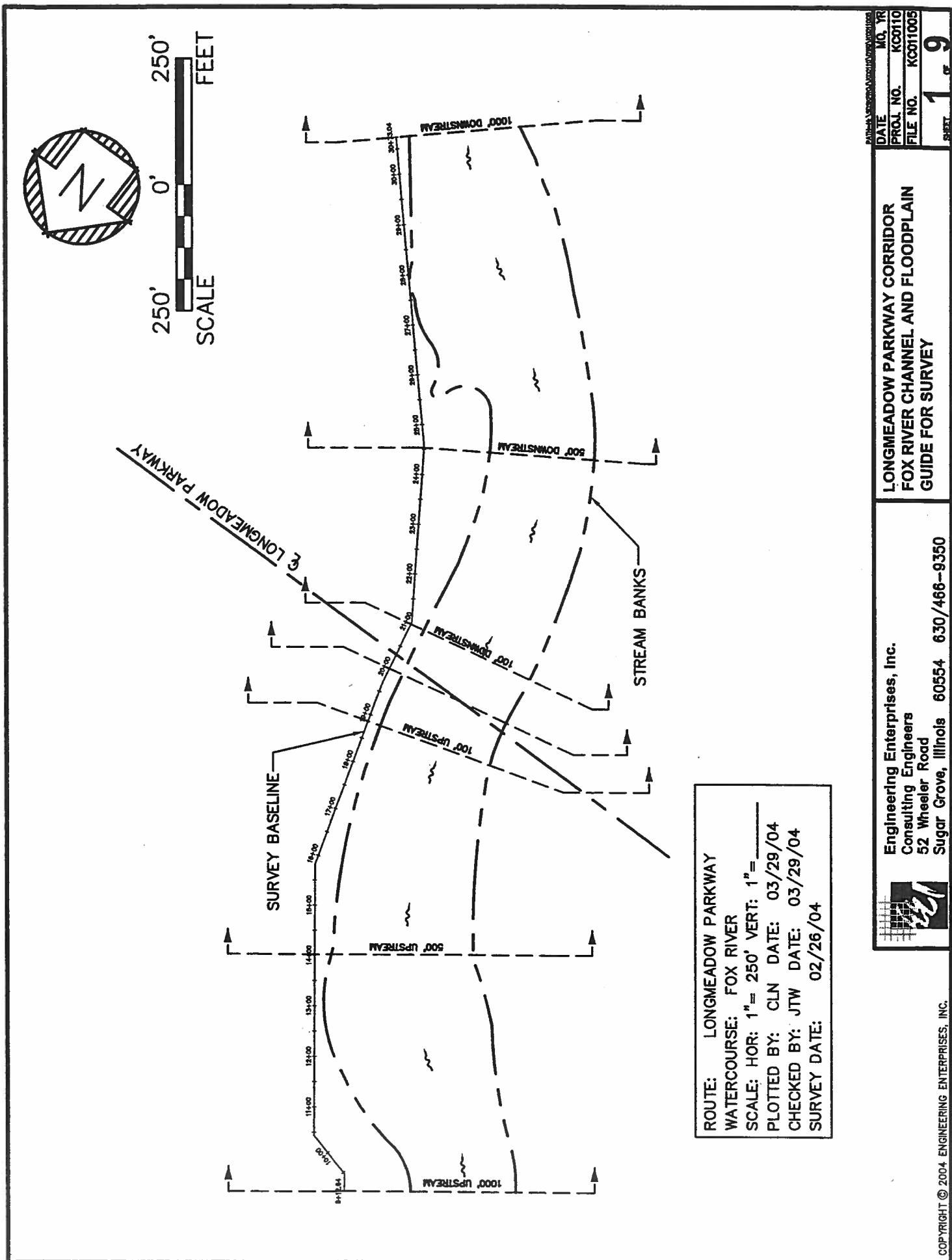
## **SECTION 6**

**DATUM CORRELATION  
STREAMBED PLAN AND PROFILE  
STREAM CROSS-SECTIONS  
DAMAGEABLE BUILDINGS AND STRUCTURES IN FLOODPLAIN**

# Data Correlation

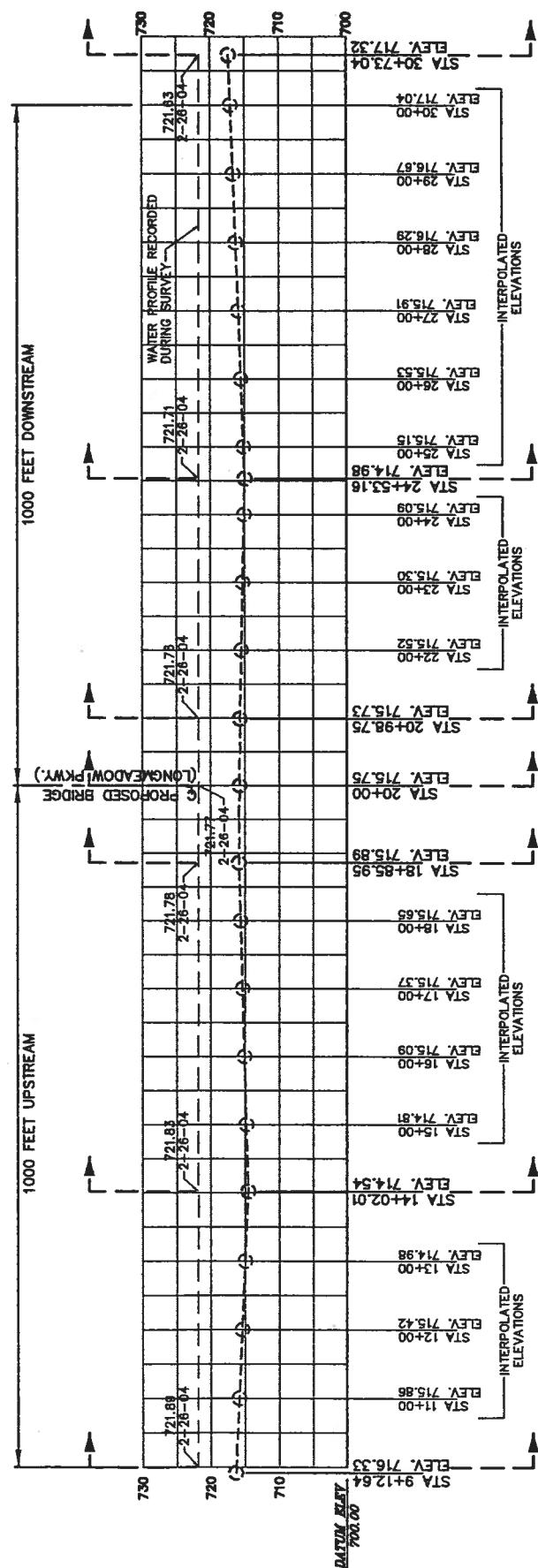
+ H2	-	Elev	Segm C# 1032	R# 116 E 51 T 0 R #
2.45	735.31	6.92	732.33	
0.95	733.28	7.22	726.06	
7.22	733.83	8.12	730.71	
5.62	736.12	1.03	732.59	
10.75	750.81	2.70	730.06	
14.58	764.11	2.69	739.59	
11.57	766.10	1.19	764.61	
14.55	779.46	1.82	772.48	12.2 S PRC
1.29	776.36	14.48	764.61	16 d nail
2.89	782.72	8.33	759.74	
0.08	759.82	14.36	745.46	
0.39	745.47	8.67	737.18	
3.91	741.69	9.06	732.05	
4.66	736.69	8.29	726.46	
3.08	731.46	3.68	727.80	
10.02	732.89	3.57	734.30	
5.32	736.62	2.76	736.91	
close to pt 1032				
TB - RC new 2.20 - 04				
Vert: 0.01 Closure = 0.06				
loop闭合 1032 + RM 103 (Firm)				

LST Topographic Survey than Firm datum  
is adopted. 1/2 miles  
based on RM 103  
LST @ 100' higher than Firm datum

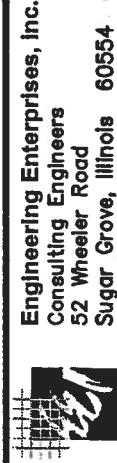


# STREAMBED PROFILE

(LOOKING EAST)



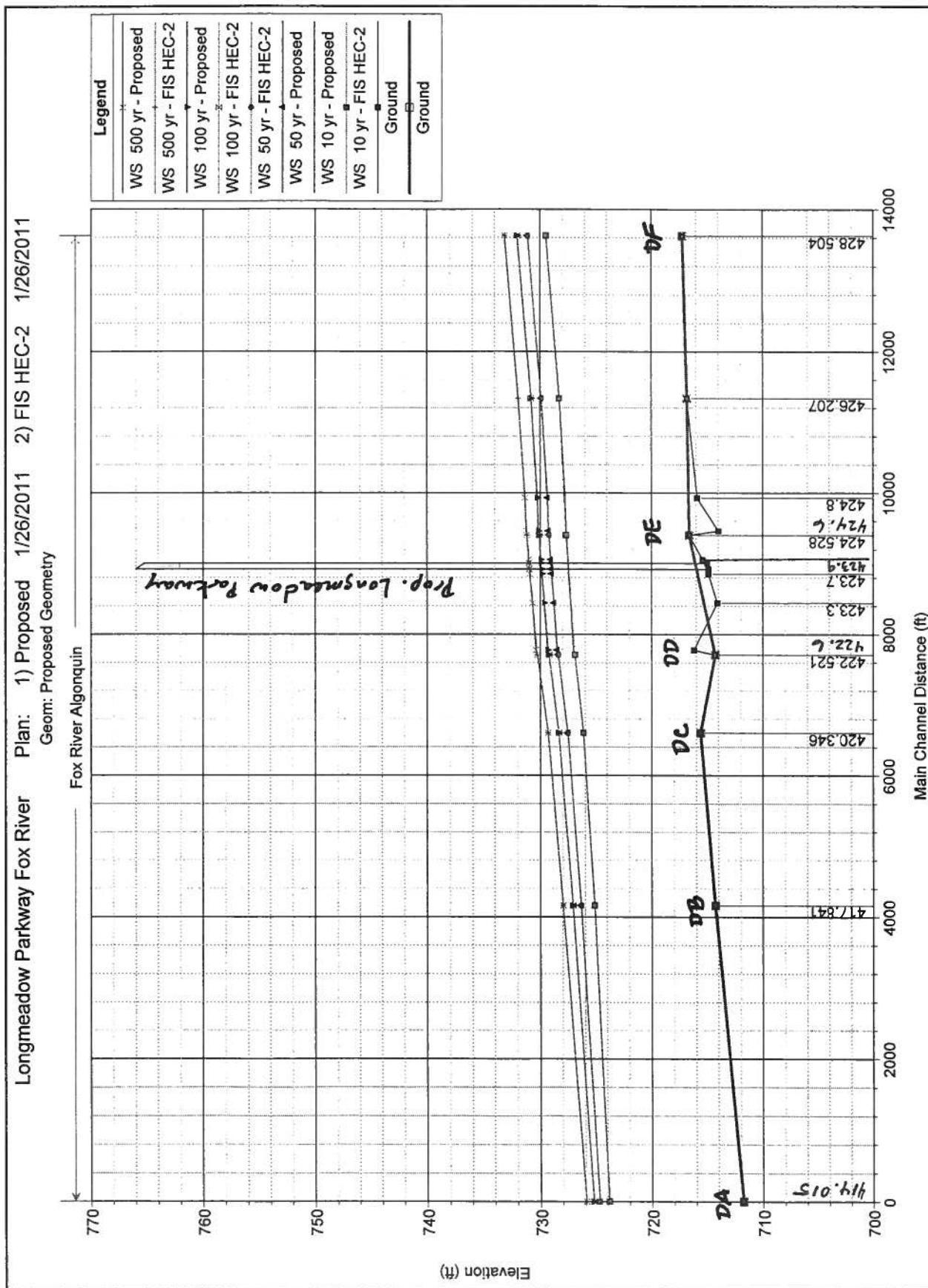
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WATERCOURSE: FOX RIVER  
SCALE: HOR: 1" = 250' VERT: 1" = 25'  
PLOTTED BY: CLN DATE: 03/29/04  
CHECKED BY: JTW DATE: 03/29/04  
SURVEY DATE: 02/26/04

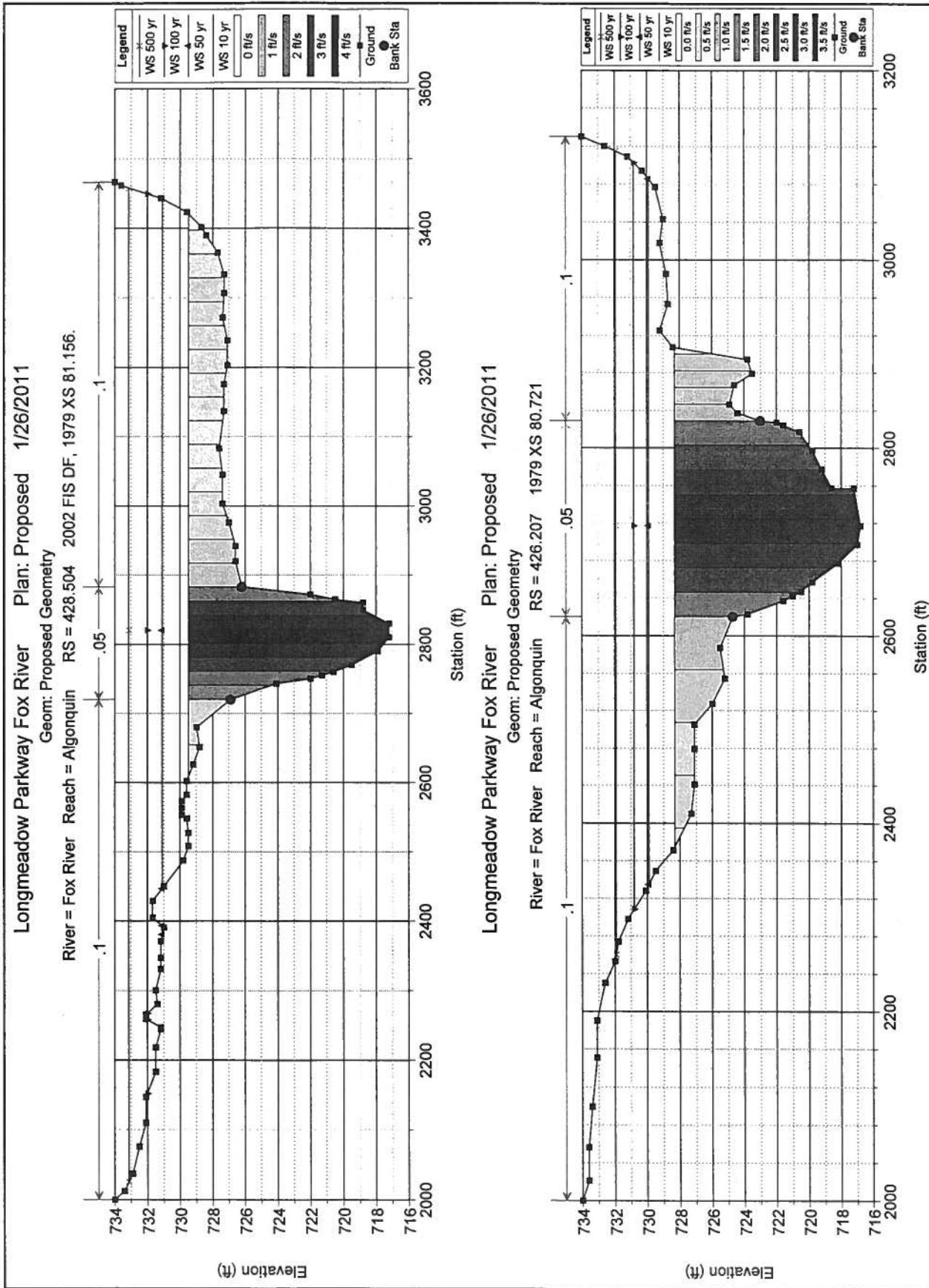


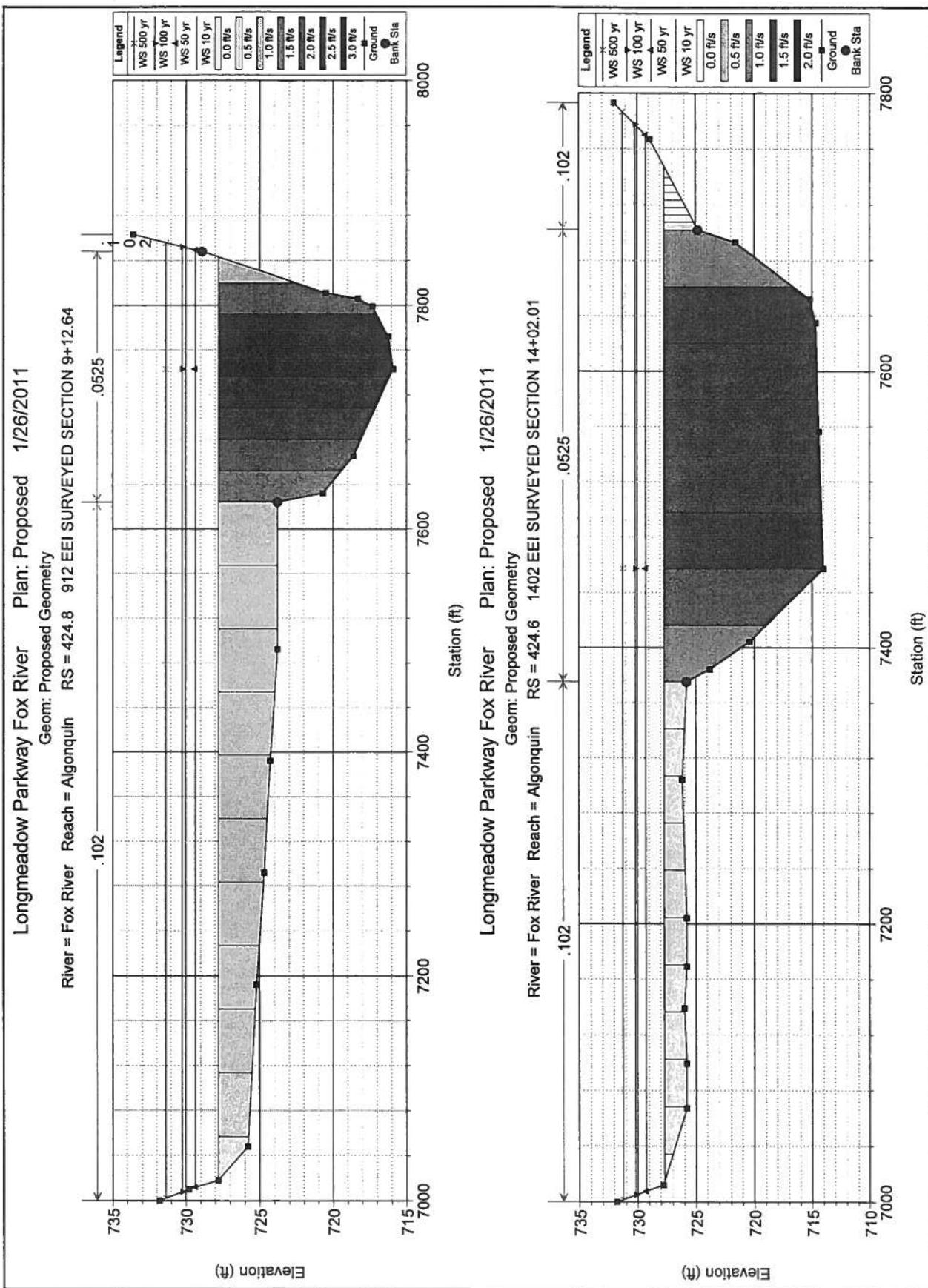
DATE: 03/29/04  
NO. YR: 2004  
PROJ. NO.: KCN105  
FILE NO.: KCN1005  
**2 of 9**

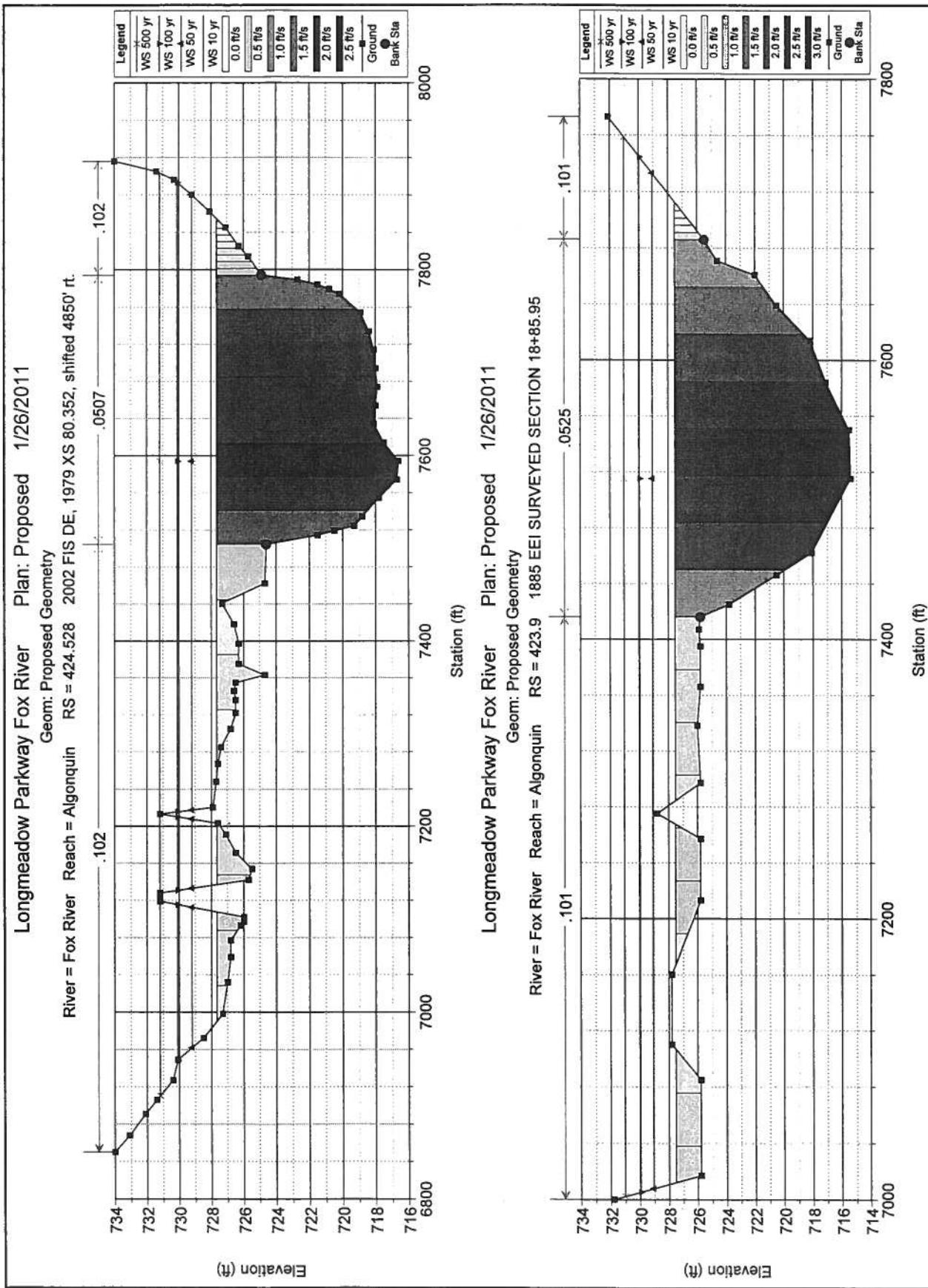
LONGMEADOW PARKWAY CORRIDOR  
FOX RIVER CHANNEL AND FLOODPLAIN  
STREAMBED PROFILE @ CENTERLINE  
OF CHANNEL

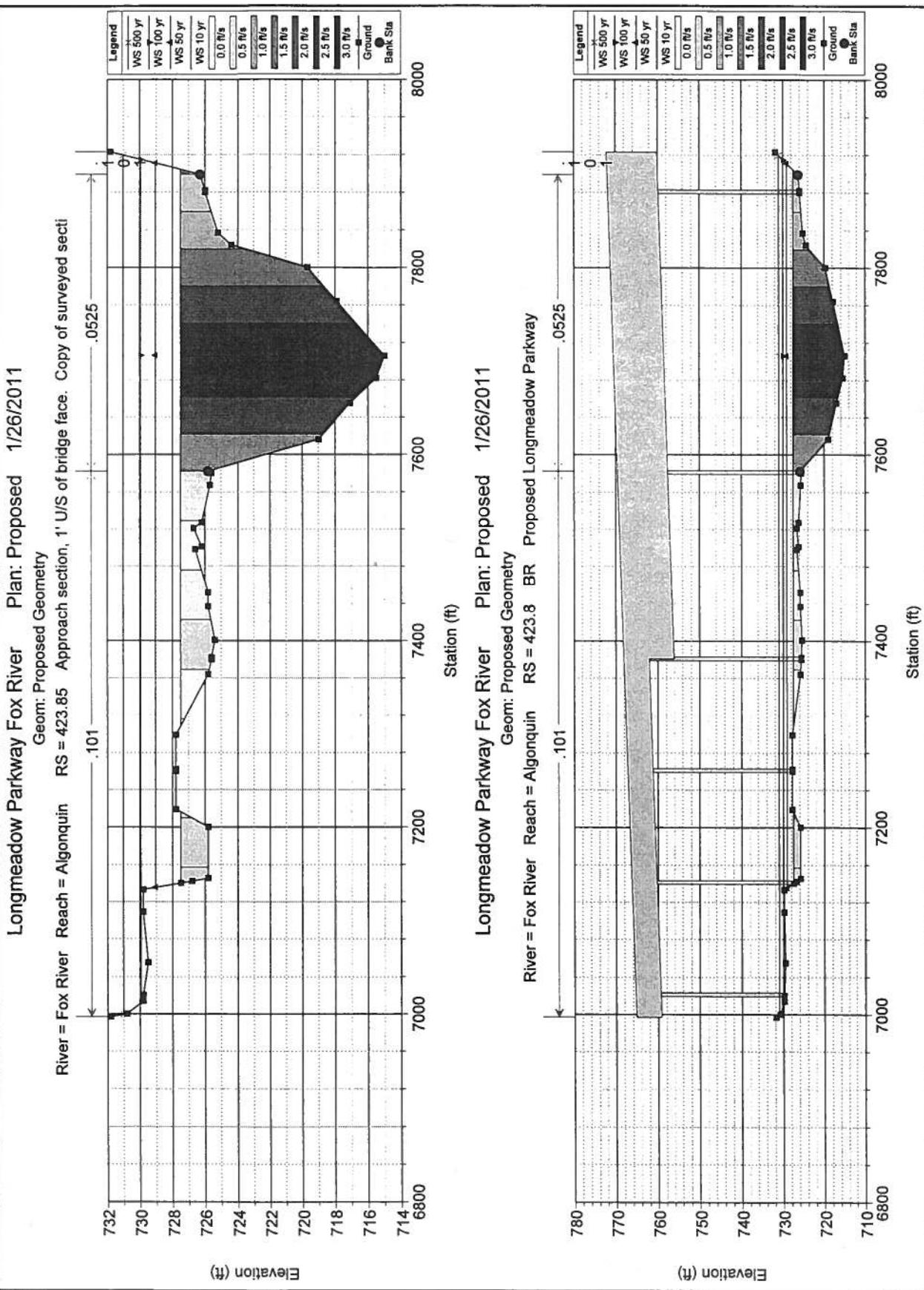
# FOX RIVER PROFILE

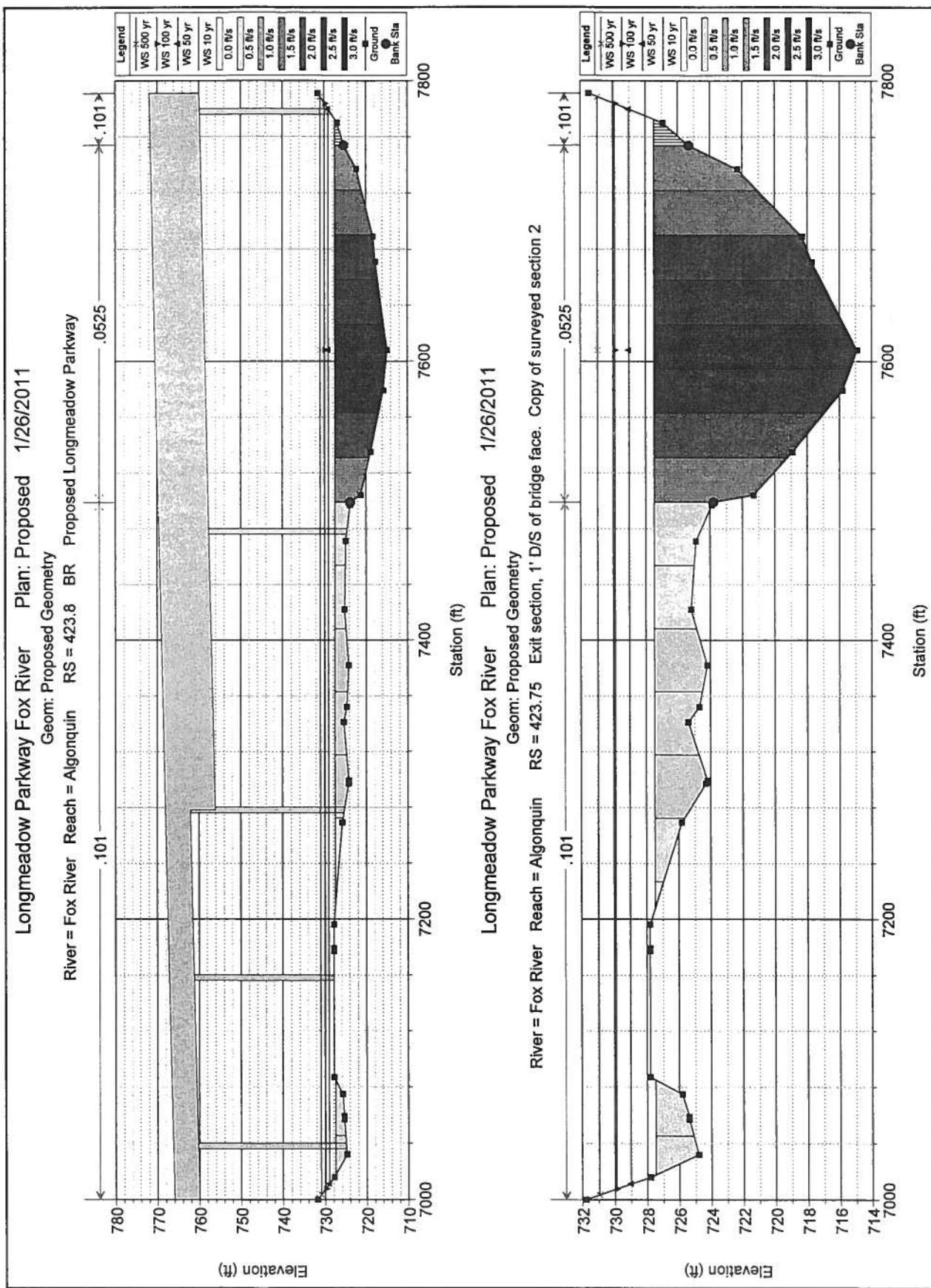


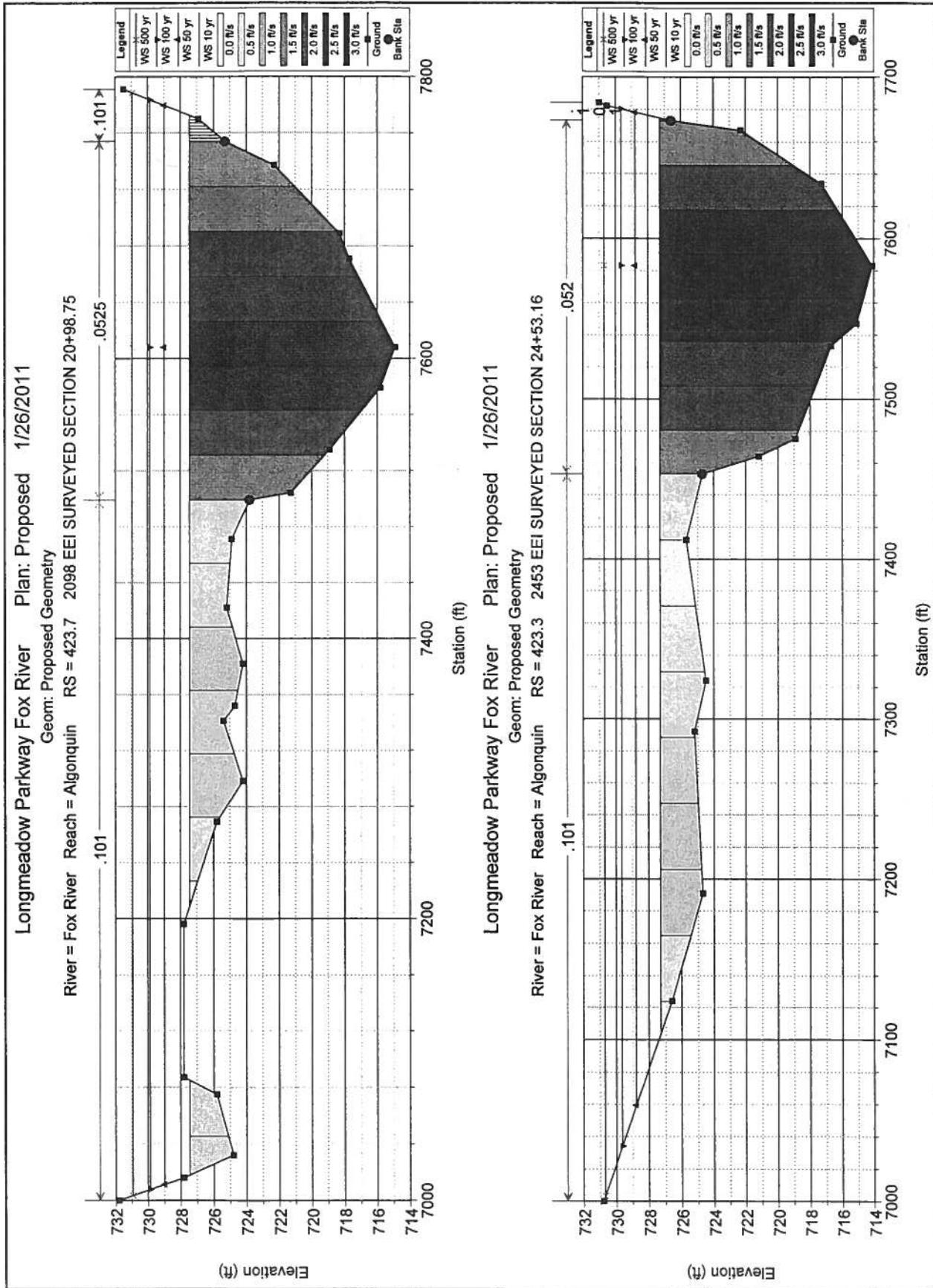


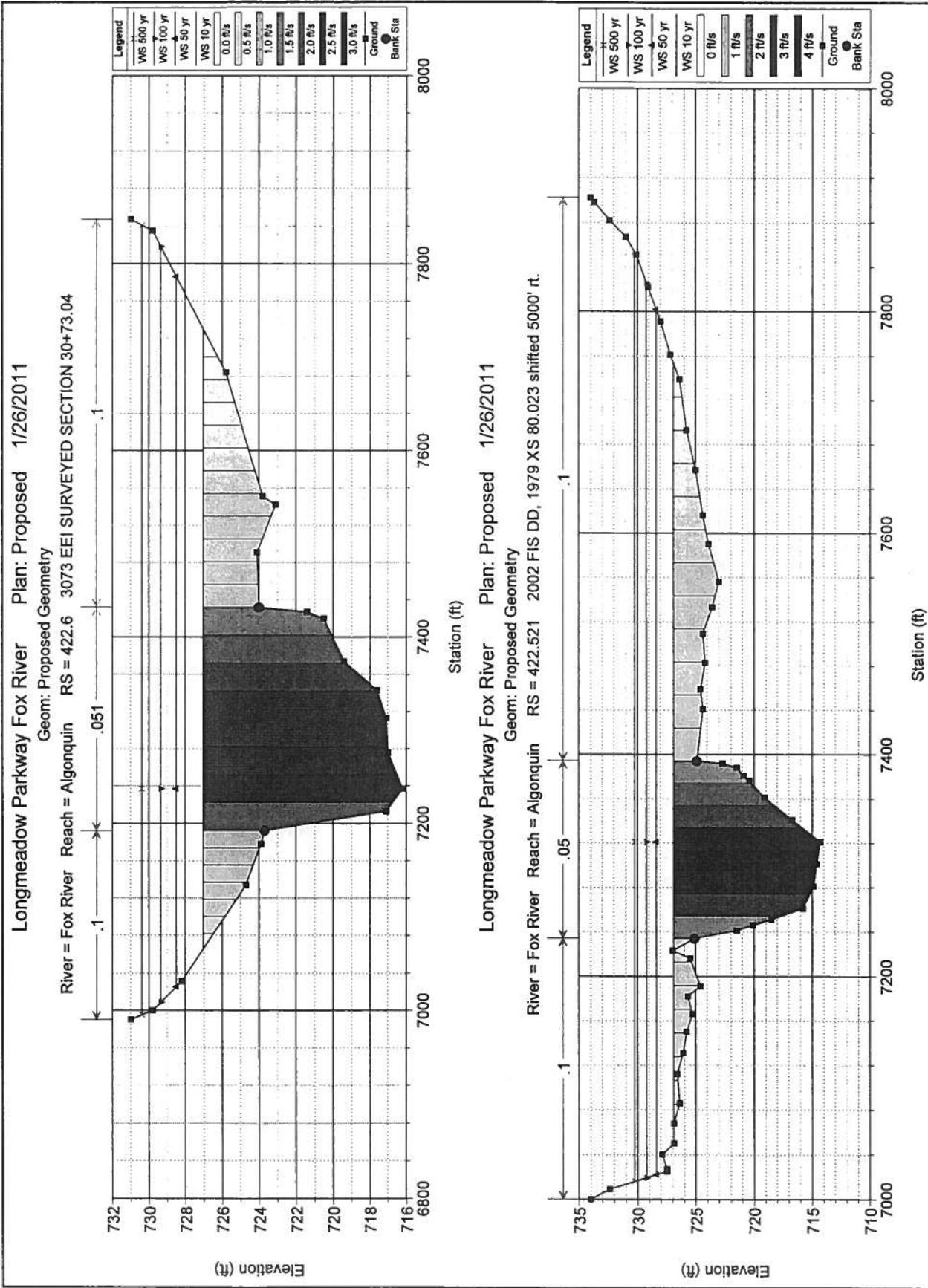


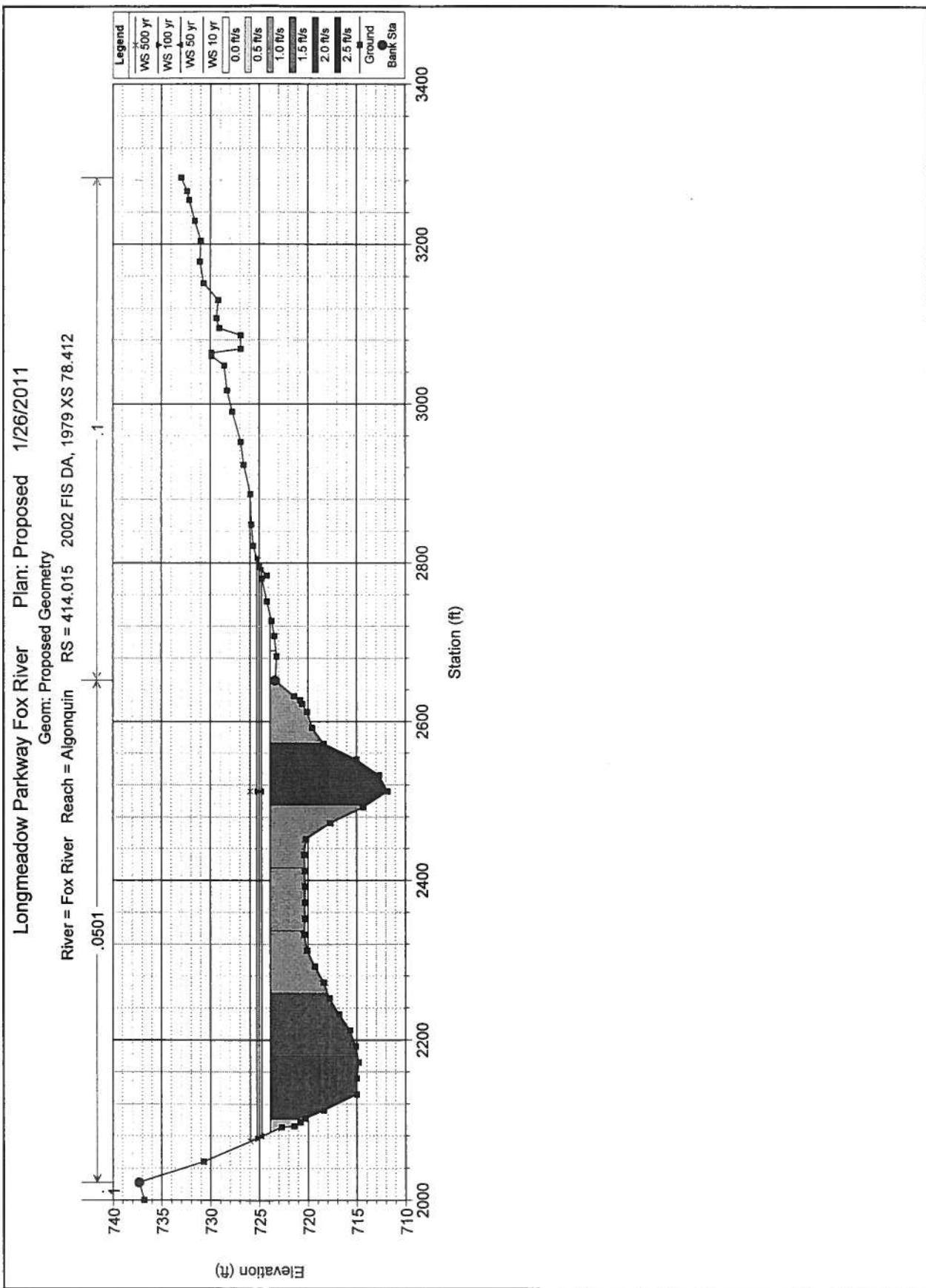














- DAMAGEABLE ELEVATION AND LOCATION  
(DOORWAY, WINDOW WELL, ETC.)
- BUILDINGS / STRUCTURES  
(HOUSE, GARAGE, BUSINESS, ETC.)
- BUILDINGS / STRUCTURES  
NO LONGER EXISTING

ROUTE: LONGMEADOW PARKWAY  
 WATERCOURSE: FOX RIVER  
 SCALE: HOR: 1" = 100' VERT: 1" = N/A  
 PLOTTED BY: CLN DATE: 04/26/04  
 CHECKED BY: JTW, DATE: 04/26/04  
 SURVEY DATE: 03/29/04

LOCATION OF BUILDINGS & STRUCTURES WITH POTENTIAL FLOOD DAMAGES						
Building or Structure #	Description	Address	Damageable Location	Elevation	Station Along Survey Baseline	Offset Distance From Survey Baseline
1	Frame One Story Name: Vicki Zordin	18N058 Angelina Pl.	Vents into crawl space West side of house	728.39	19+50	25'L
2	Frame One Story Name: unknown	none found	Window sill into crawl space	728.01	20+12	25'L
3	Frame Shed	same parcel	doorway into shed	725.06	20+42	17'R
4	Frame Shed Name: same parcel	18N058 Sandbloom Rd.	Garage door East side	728.52	21+71	524'L
5	Frame Two Story Name: Robby Kuczak	18N058 Sandbloom Rd.	Vent into crawl space Near NW corner Bldg	728.80	22+10	520'L
6	Frame Garage	18N058 Sandbloom Rd.	Garage door East side	728.71	22+40	518'L
7	Frame Garage	18N988 Williams	Garage door South side	727.99	24+08	260'L
8	Brick & Frame Two Story Name: Mike Zarback	18N988 Williams	window sill into basement West side of house	728.70	25+43	198'L
9	Barn	18N988 Williams	Doorway at NW corner	730.83	26+38	277'L
10	Frame Shed	18N824 Williams	Doorway on South side	728.81	27+77	43'L
11	Frame One Story	18N814 Williams	Vents on North side Bldg	728.27	28+41	63'L
12	Frame Shed	18N814 Williams	Doorway into shed East side	728.25	28+43	19'L
13	Metal Shed Name: Frank Traje	18N908 Williams	ground at shed	728.29	28+75	44'L
14	Metal Shed	18N908 Williams	ground at shed	728.14	28+69	23'L
15	Frame One Story	18N908 Williams	Doorway @ SW Bldg corner 1st floor, house on crawl	728.80	29+03	96'L
16	Frame Outhouse like Bldg	18N908 Williams	Doorway on West side conc.	727.83	29+68	116'L
17	Frame One Story	18N908 Williams	Vents on North side into cra	727.00	29+79	61'L
18	Frame One Story	18N908 Williams	Doorway on Southside slab	728.33	30+11	61'L
19	Frame Outhouse like Bldg	18N908 Williams	Doorway on West side conc.	727.29	30+45	101'L
20	Metal Shed	18N908 Williams	Doorway on North side conc.	728.06	30+22	22'L

NOTE: A PORTION OF THE FOLLOWING INFORMATION WAS PROVIDED IN PART BY THE KANE COUNTY GIS DEPT. UNAUTHORIZED USE OF THIS INFORMATION BY PARTIES OTHER THAN THOSE AUTHORIZED BY MUNICIPALITIES WITHIN KANE COUNTY IS PROHIBITED.

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**Engineering Enterprises, Inc.**  
 Consulting Engineers  
 52 Wheeler Road  
 Sugar Grove, Illinois 60554 630/466-9350

**LONGMEADOW PARKWAY**  
**CORRIDOR**

**LOCATION OF BUILDINGS & STRUCTURES  
WITH POTENTIAL FLOOD DAMAGES OF  
THE FOX RIVER**

DATE: APRIL 2004  
 PROJ. NO. KC0110  
 FILE NO. KC011006  
 SHEET 1 OF 1

## **SECTION 7**

**PROPOSED BRIDGE TS&L  
PROPOSED PARKWAY PLANS AND PROFILES  
RULES ESTABLISHING CLEARANCES FOR BRIDGES  
OVER THE FOX RIVER**

# **PROPOSED BRIDGE TS&L**

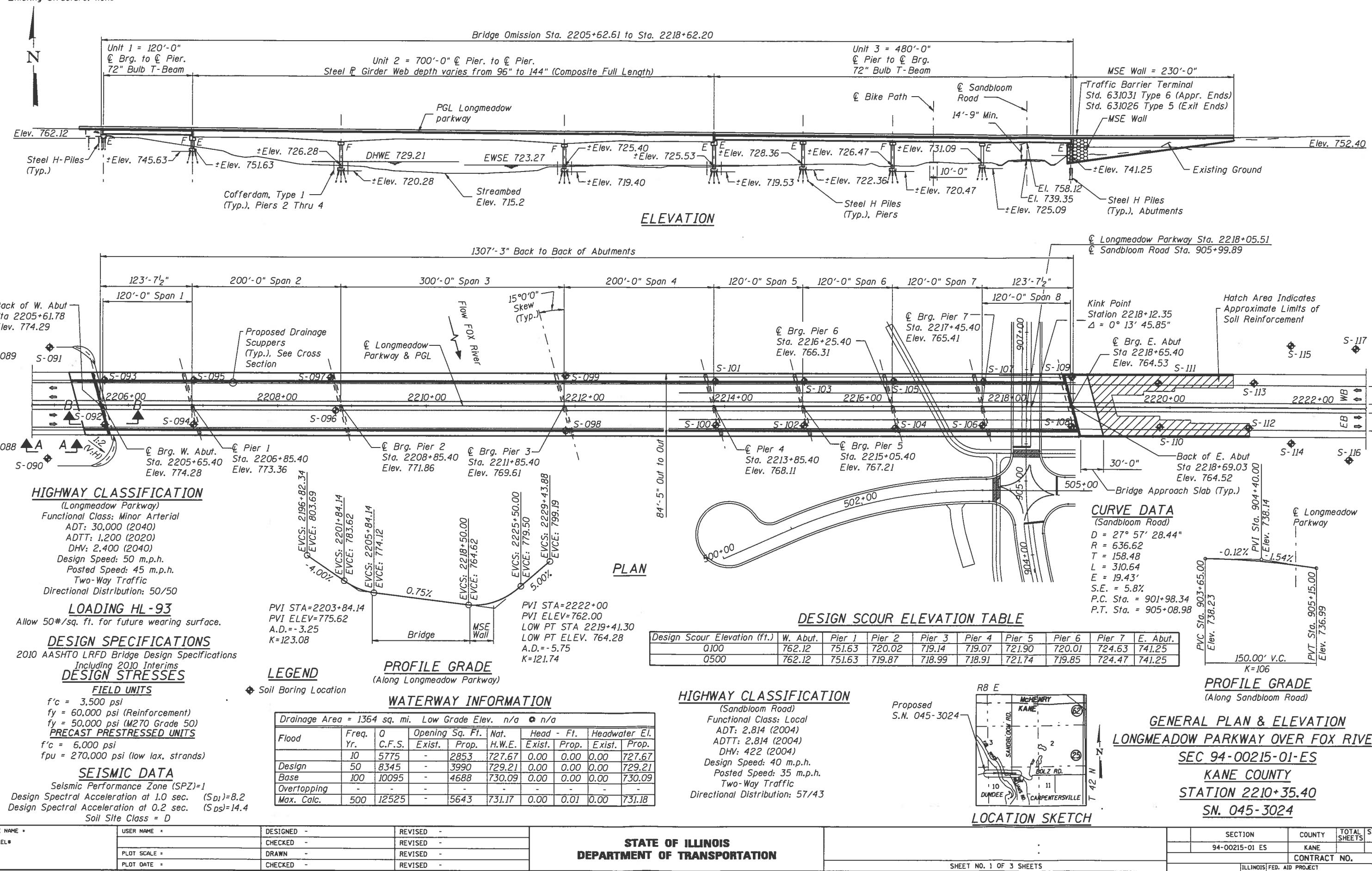
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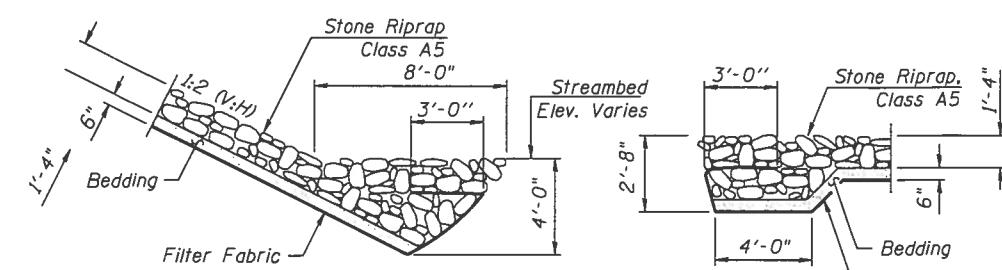
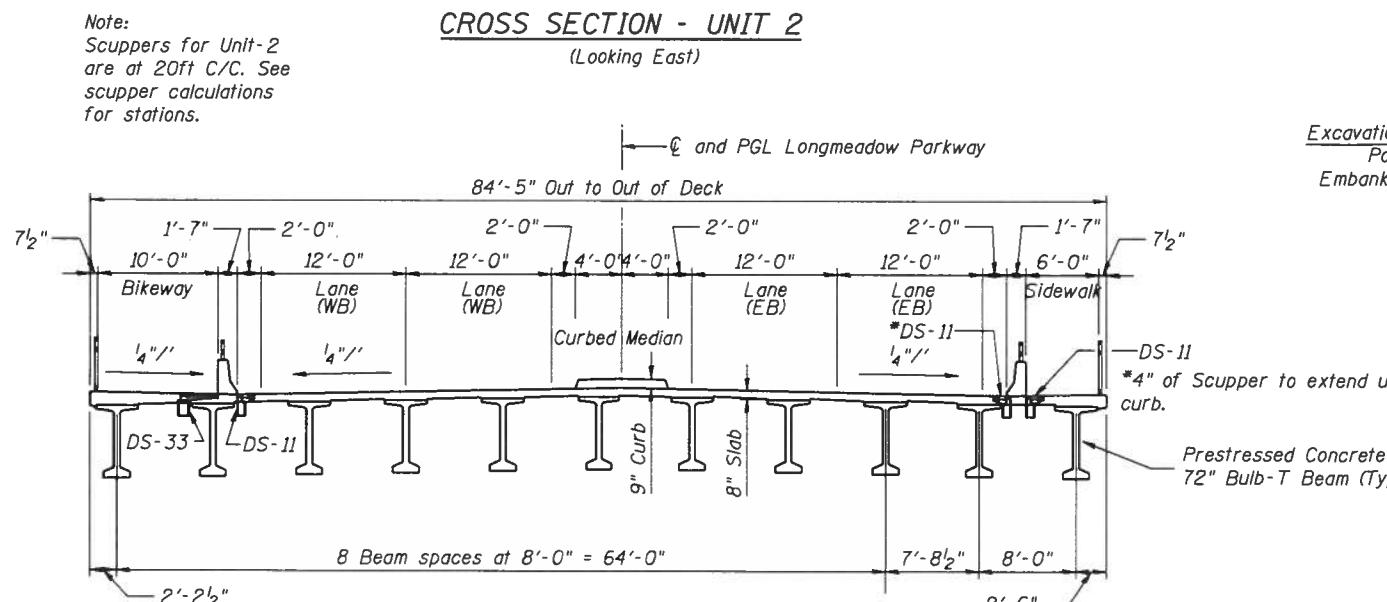
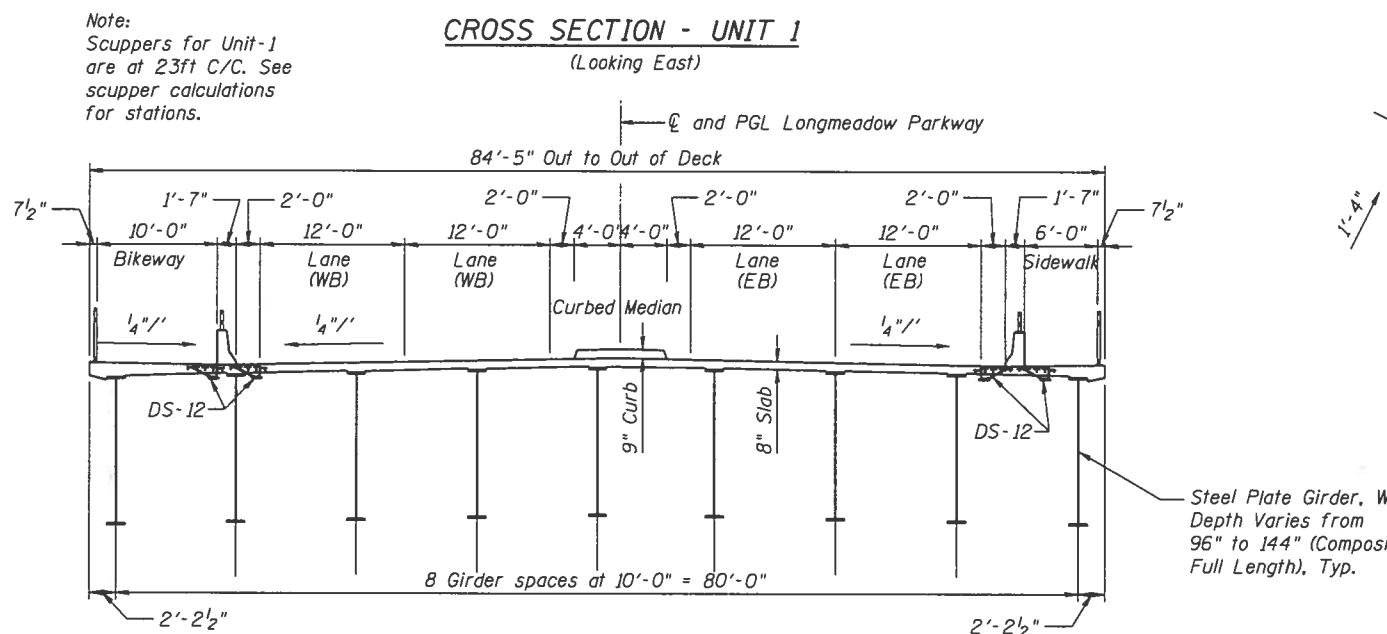
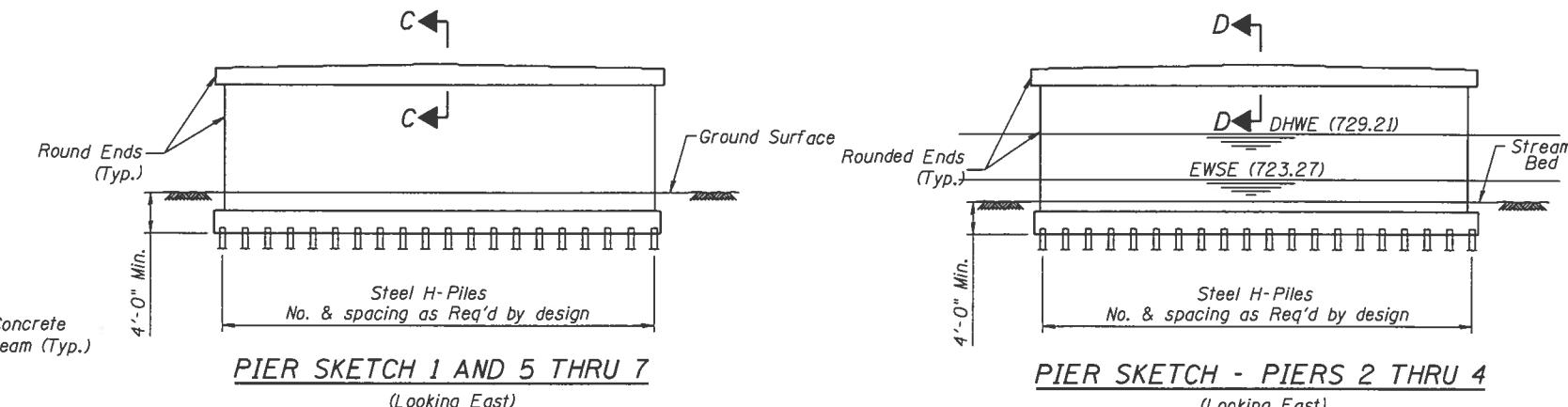
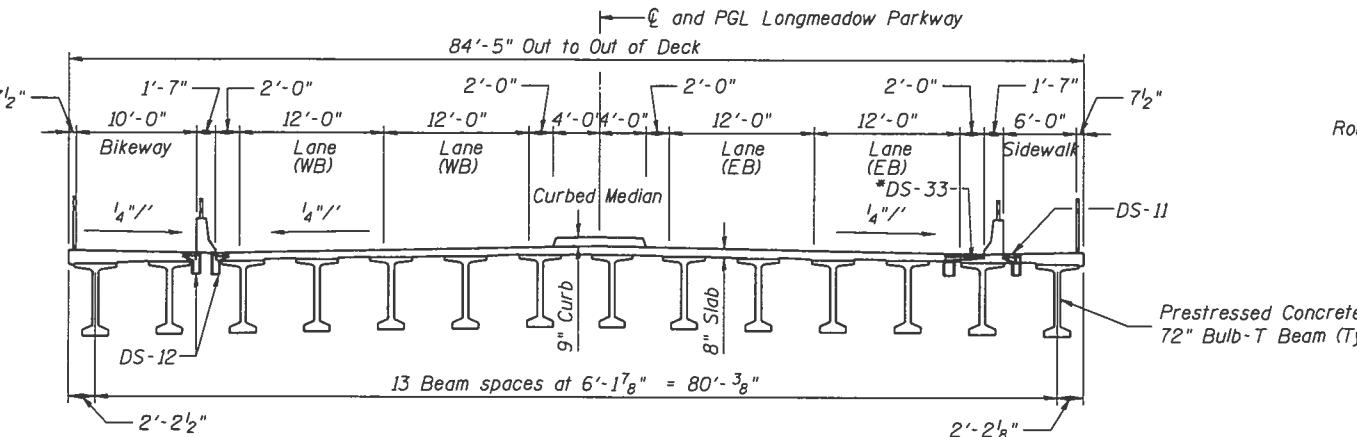
Christopher B. Burke Engineering, Ltd.

Hydraulic Report  
Longmeadow Parkway over the  
Fox River

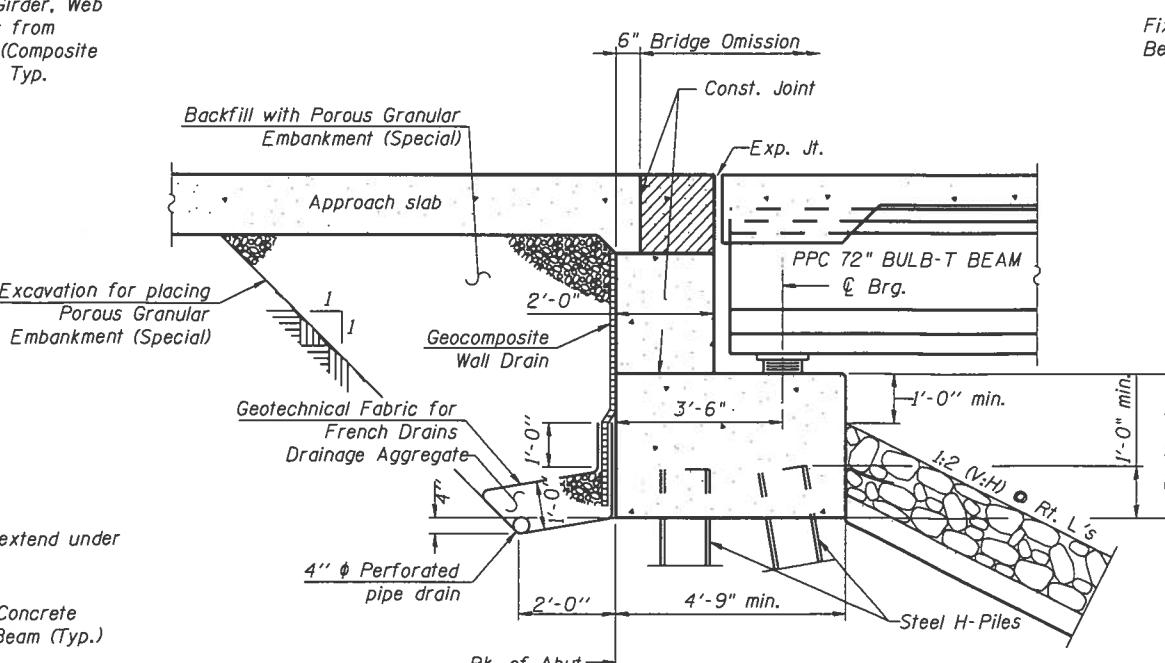
Bench Mark: Chis. " □ " S.E. COR. CONC. SLAB.  
The INT. of ILL. 31 and Miller Rd. go N. 0.9 MI ± to Man  
Elev. 806.34

*Existing Structure: None*

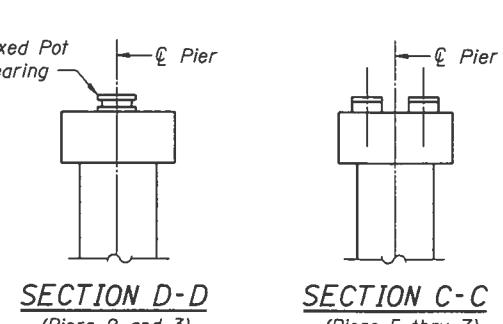
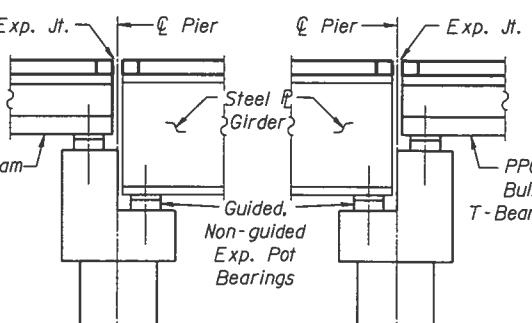




SECTION A-A



(Horiz. dim. @ Rt. L's)



SECTIONS & DETAILS 1 OF 2  
LONGMEADOW PARKWAY OVER FOX RIVER

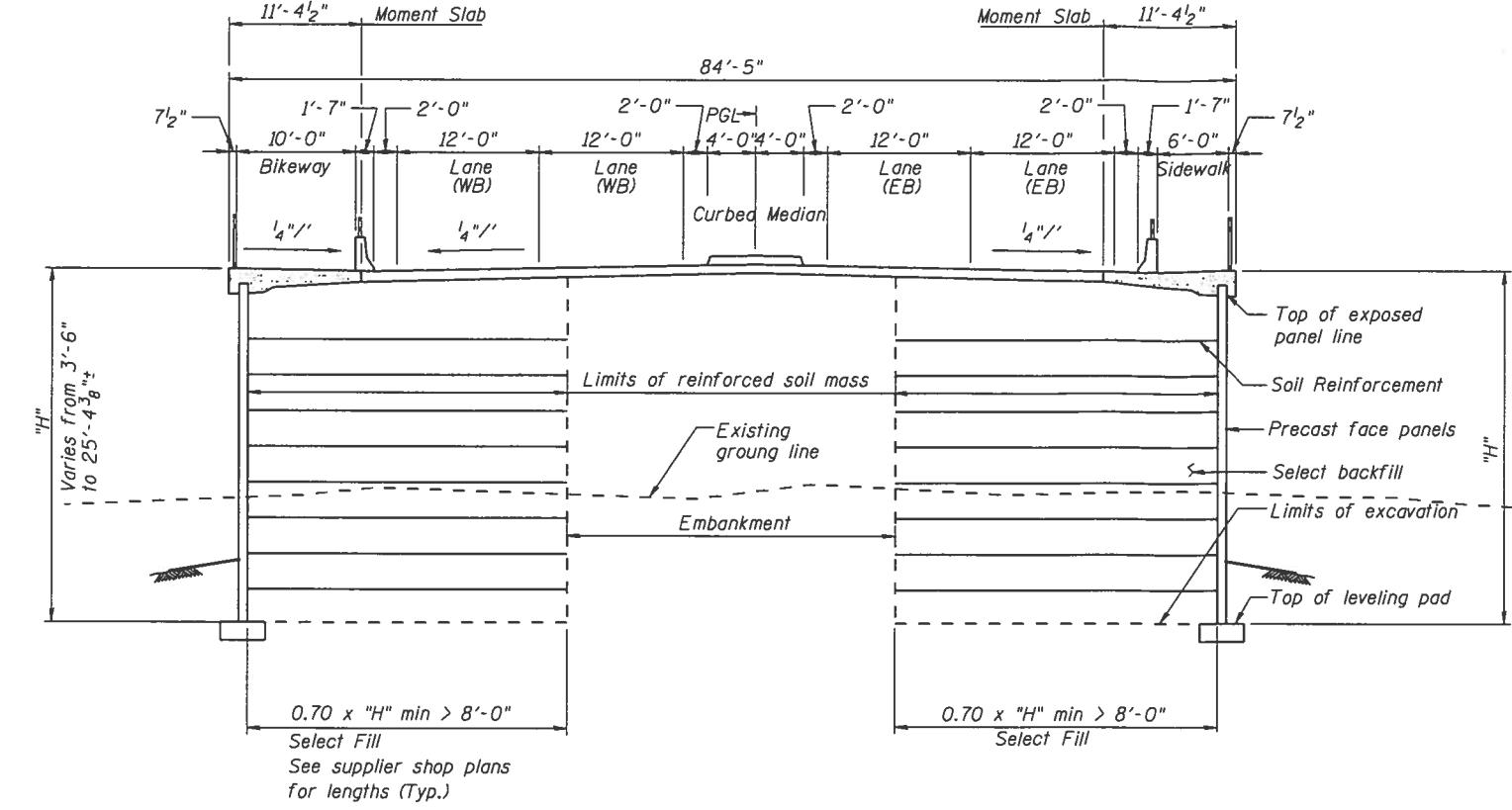
SEC 94-00215-01-ES

KANE COUNTY

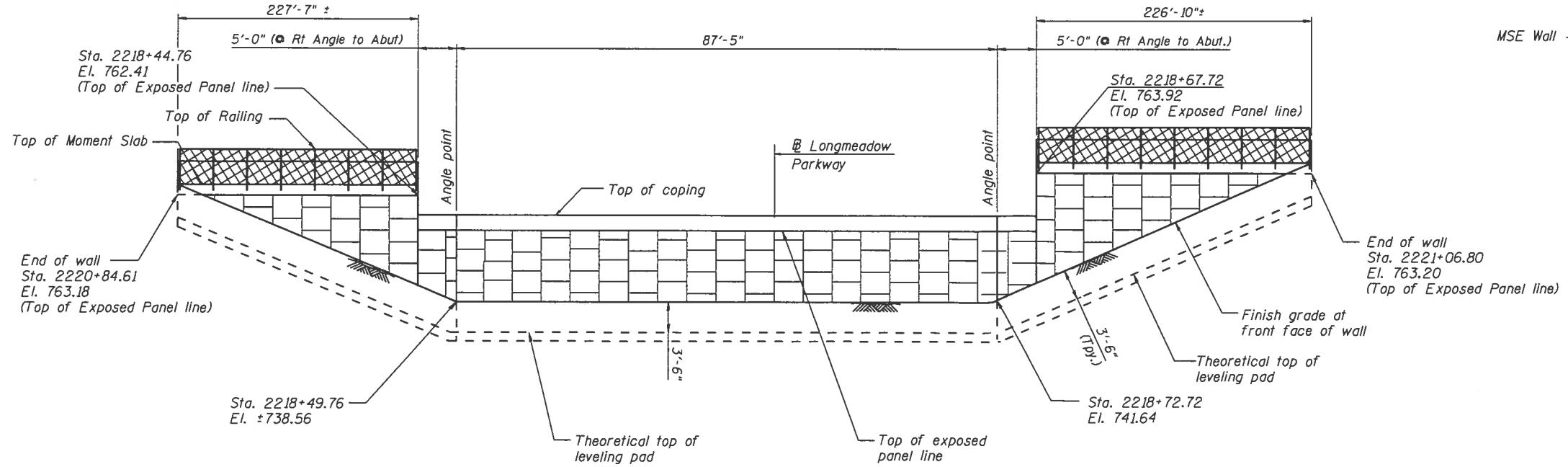
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SN. 045-3024

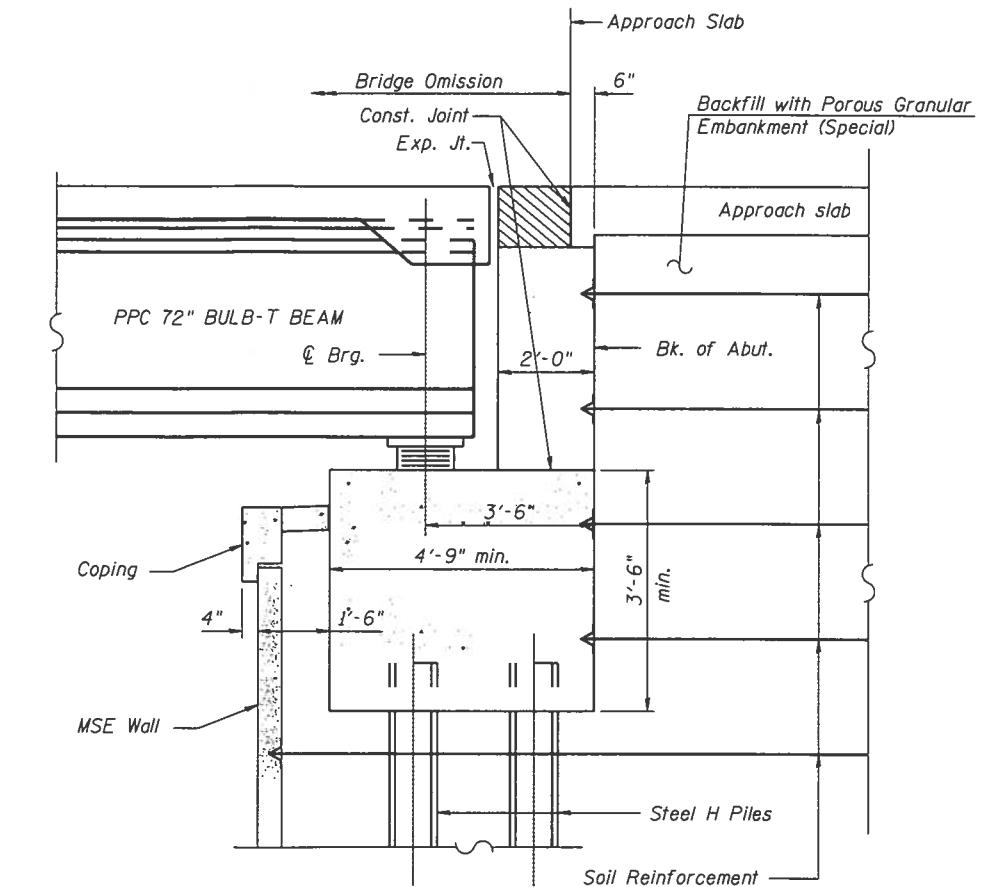
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	CHECKED -	REVISED -		
PLOT SCALE = 1:16	DRAWN -	REVISED -		
PLOT DATE = 9/10/2012	CHECKED -	REVISED -		



EAST ABUTMENT SECTION  
(Looking East)



EAST ABUTMENT WALL  
(Developed elevation along front face of wall)



SECTION THRU PILE SUPPORTED STUB EAST ABUTMENT  
(Horiz. dim. @ Rt. L's)

SECTIONS & DETAILS 2 OF 2  
LONGMEADOW PARKWAY OVER FOX RIVER

SEC 94-00215-01-ES

KANE COUNTY

STATION 2210+35.40

SN. 045-3024

Ilene Dailey

PRELIM. PIER SHAPE

**From:** Chin Wang [cwang@cbbel.com]  
**Sent:** Wednesday, February 09, 2011 1:20 PM  
**To:** 'Nathaniel K. Utz'  
**Cc:** mantas@cbbel.com; idailey@cbbel.com  
**Subject:** RE: Pier Dimensions Longmeadow parkway Preliminary TSL drawing

# DIMENSIONS

Nathan:

We will recalculate the pier scour depth based on rounded ends.

**Chinliang R. Wang, PE**

*Vice President*

*Head, Drainage Department*

**Christopher B. Burke Engineering, Ltd.**

9575 W. Higgins Road, Suite 600 Rosemont, IL 60018

Phone: (847) 823-0500 Fax: (847) 823-0520

E-Mail: [cwang@cbbel.com](mailto:cwang@cbbel.com)

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---

**From:** Nathaniel K. Utz [mailto:NUTz@Maiengr.com]

**Sent:** Wednesday, February 09, 2011 1:15 PM

**To:** Nathaniel K. Utz; cwang@cbbel.com

**Cc:** mantas@cbbel.com

**Subject:** RE: Pier Dimensions Longmeadow parkway Preliminary TSL drawing

Chin:

All piers will have rounded ends. The revised TSL showing the scour elevations will be forwarded shortly.

Nathan

---

**From:** Nathaniel K. Utz

**Sent:** Wednesday, February 09, 2011 9:44 AM

**To:** 'cwang@cbbel.com'

**Cc:** Nathaniel K. Utz

**Subject:** Pier Dimensions Longmeadow parkway Preliminary TSL drawing

Chin:

It's my understanding from our structural people that the dimensions below should be sufficient for hydraulic studies. IDOT no longer requires all these dimensions on the TS&L, as they used to 10 years ago. I will follow up with a phone call today to close the loop on this issue.

Nathan

---

**From:** Baig Mirza

**Sent:** Wednesday, February 09, 2011 9:40 AM

**To:** Nathaniel K. Utz

**Subject:** FW: Longmeadow parkway Preliminary TSL drawing

These are preliminary dimensions. They are good enough for hydraulic studies and SGR.

---

**From:** Baig Mirza  
**Sent:** Friday, January 14, 2011 12:10 PM  
**To:** Nathaniel K. Utz; 'cwang@cbbel.com'  
**Cc:** Gerald Koylass  
**Subject:** RE: Longmeadow parkway Preliminary TSL drawing

Nathan/C Wang

Pier shapes/types are already shown on the drawings, Abutment shapes are already shown on drawings. Elevations at Top of deck and at bottom of footing at abutment locations at the centerline of bridge are already shown on drawings.

Elevations at Top of deck and stream bed elevations at pier locations at the centerline of bridge are already shown on drawings.

At this point where we have only preliminary studies done. The following minimum dimensions can suitable be assumed.

- 1) pier widths as 4'-0", pier cap widths as 8'-0", pier footing widths as 9'-0", pier footing depths as 4'-0", depth of bottom of pier footing to top of stream bed as 7'-0".
- 2) Abutment widths as 7'-0" and abutment depths as 4'-6"

I am including Wang's draft SGR for your information only. For any questions please give me a call.



**Mirza G Baig ME, PE.**  
**Associate**  
**Structural Engineer**  
130 E Randolph Street, Suite 1000  
Chicago, IL 60601

(312)946-7144 Direct (312)946-8600 Main (312)946-7199 Fax

[www.mbaig@maiengr.com](mailto:www.mbaig@maiengr.com)

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---

**From:** Nathaniel K. Utz  
**Sent:** Friday, January 14, 2011 11:05 AM  
**To:** Baig Mirza; Gerald Koylass  
**Subject:** FW: Longmeadow parkway Preliminary TSL drawing

---

**From:** Chin Wang [mailto:cwang@cbbel.com]  
**Sent:** Friday, January 14, 2011 10:25 AM  
**To:** Nathaniel K. Utz  
**Cc:** mantas@cbbel.com  
**Subject:** RE: Longmeadow parkway Preliminary TSL drawing

Nathan:

To determine the scour depth, we need data such as pier shape/type, dimensions and elevations, abutment shape, dimensions and elevation, structure boring logs.

**Chinliang R. Wang, PE**  
*Vice President*  
*Head, Drainage Department*  
**Christopher B. Burke Engineering, Ltd.**  
9575 W. Higgins Road, Suite 600 Rosemont, IL 60018  
Phone: (847) 823-0500 Fax: (847) 823-0520  
E-Mail: [cwang@cbbel.com](mailto:cwang@cbbel.com)

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---

**From:** Nathaniel K. Utz [mailto:NUtz@Maiengr.com]  
**Sent:** Thursday, January 13, 2011 12:14 PM  
**To:** cwang@cbbel.com  
**Cc:** mantas@cbbel.com; Nathaniel K. Utz  
**Subject:** FW: Longmeadow parkway Preliminary TSL drawing

Chin, let me know if you need anything more for the bridge. These are DRAFT.

Nathan

---

**From:** Baig Mirza  
**Sent:** Thursday, January 13, 2011 11:58 AM  
**To:** Nathaniel K. Utz  
**Cc:** Gerald Koylass  
**Subject:** Longmeadow parkway Preliminary TSL drawing

Nathan

These are the preliminary TSL drawings for Longmeadow parkway.  
Please forward these drawings for hydraulic studies and scupper calculations.  
In our drawings we have shown some of the scuppers that will fit in  
Our beam spacing. These are only for study purposes.

Mirza

# **PROPOSED PARKWAY PLANS AND PROFILES**

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Christopher B. Burke Engineering, Ltd.

Hydraulic Report  
Longmeadow Parkway over the  
Fox River

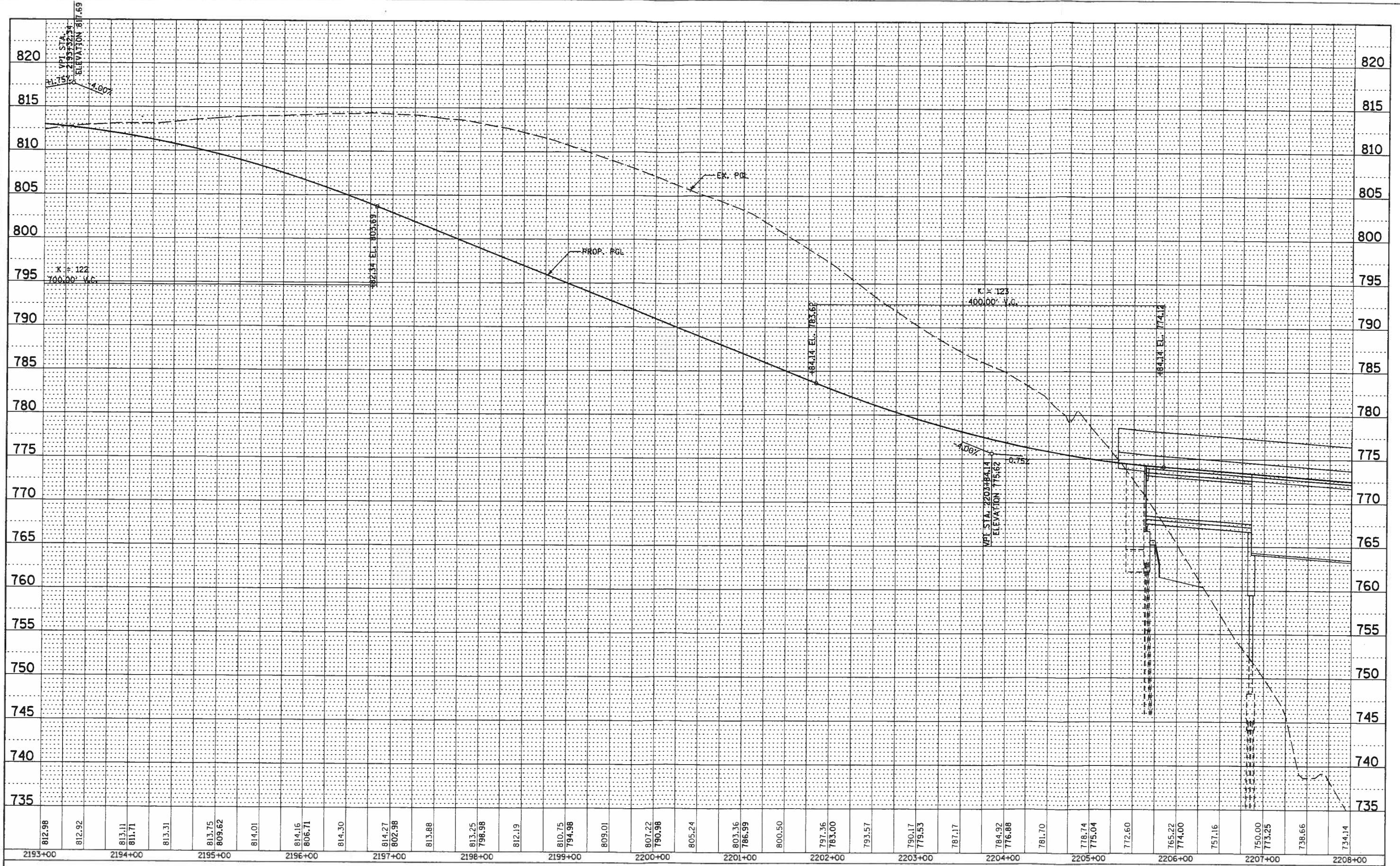


A horizontal graphic scale with tick marks at 0, 50, 100, and 150. Below the scale, the text '( IN FEET )' is centered.

			KANE COUNTY HIGHWAY DEPARTMENT
			LONGMEADOW PARKWAY / BOLZ ROAD
			FOX RIVER BRIDGE
			PLAN
NSL	DATE	REVISION	BY
 McDonough Associates Inc. Engineers/Architects			DESIGNED BY:
			CHECKED BY:
			SCALE: 1" = 50'

PLAN	SURVEYED	BY	DATE
SUBMITTED	PILOTED	GRADE CHECKED	NOTE BOOK NO. _____
PILOTED	ALL NOTED	STRUCTURE ROTATNS. CHTD	FILE NAME _____

PROFILE	SUBMITTED	BY	DATE
PLOITED	PILOTED	GRADES CHECKED	NOTE BOOK NO. _____
PILOTED	ALL NOTED	STRUCTURE ROTATNS. CHTD	FILE NAME _____



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DRAWN - REVISED -  
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STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

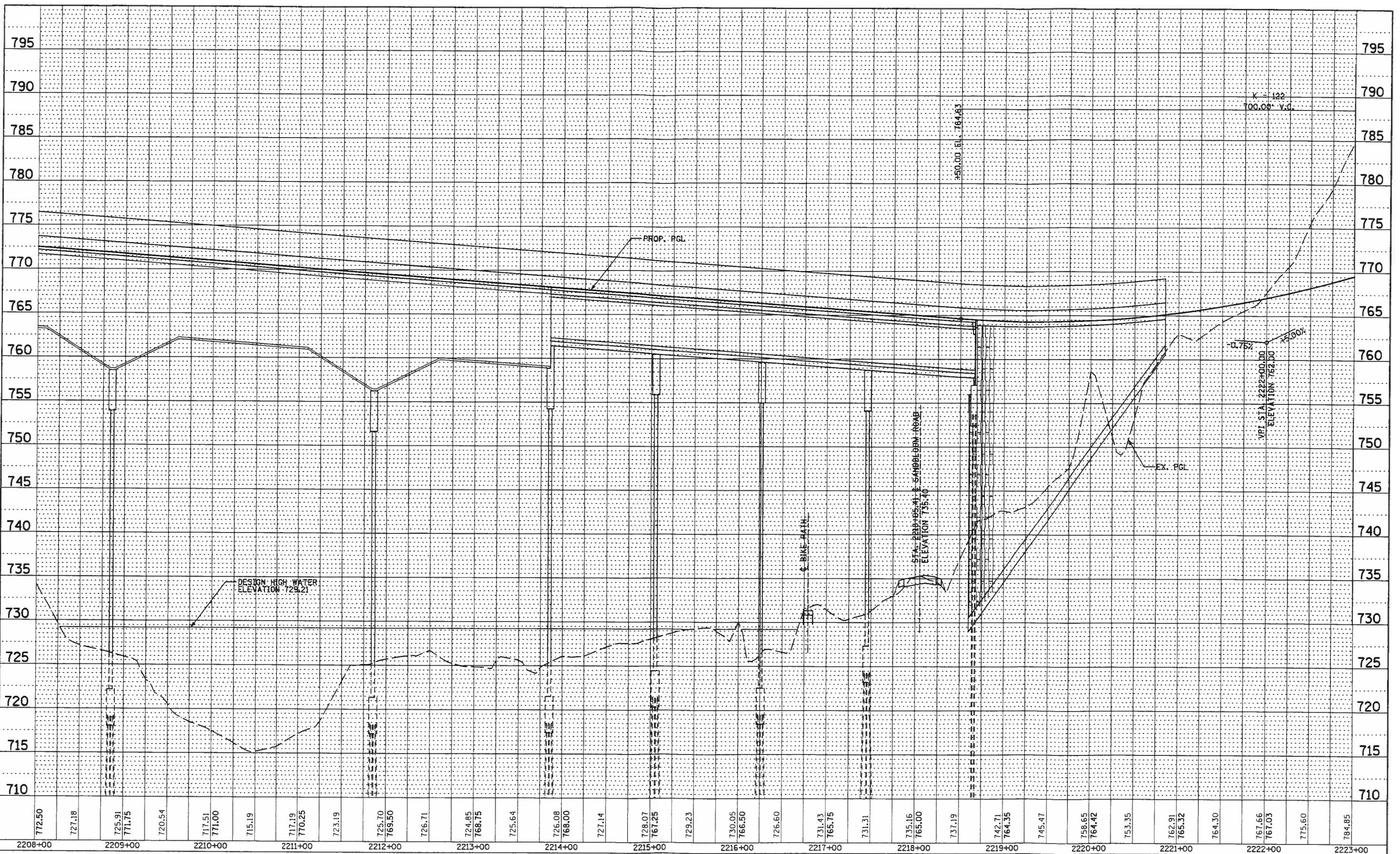
PROFILE - LONGMEADOW PARKWAY

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<b>PG-08</b>				CONTRACT NO. 63073

SCALE: SHEET NO. OF SHEETS STA. TO STA. ILLINOIS FED. AID PROJECT

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PLOTTED	PRINTED		
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CHECKED -
REVISED -
DATE - 3/27/09
REVISED -

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

PROFILE - LONGMEADOW PARKWAY

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	Sheet No.
361	06-00214-10-BR	KANE	219	
PG-09				CONTRACT NO. 63073
ILLINOIS FED. AID PROJECT				

SCALE: SHEET NO. OF SHEETS STA. TO STA.

# **RULES ESTABLISHING CLEARANCES FOR BRIDGES OVER THE FOX RIVER**

---

Christopher B. Burke Engineering, Ltd.

Hydraulic Report  
Longmeadow Parkway over the  
Fox River

JUL 01 1991

Certificate of Amended Part

The Illinois Department of Transportation certifies that the attached hereto is a true and correct copy of:

Heading of Part: Rules Establishing Horizontal and Vertical Clearances for Bridges Over the Fox River

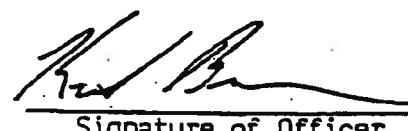
Code Citation: 92 Ill. Adm. Code 720

Sections Involved: 720.10

which was duly adopted by this agency on the 7th day of June, 1991.

Statutory Authority:

Illinois Revised Statutes      19  
                                        Chapter      52 et seq.  
                                        Paragraphs



Signature of Officer

Secretary  
Title of Officer

FILED  
STATE LIBRARY  
ADM. CODE UNIT

JUN 10 1991

IN THE OFFICE OF  
SECRETARY OF STATE

92 ILLINOIS ADMINISTRATIVE CODECHAPTER I, §720SUBCHAPTER i

TITLE 92: TRANSPORTATION  
CHAPTER 1: DEPARTMENT OF TRANSPORTATION  
SUBCHAPTER i: WATER RESOURCES

PART 720  
RULES ESTABLISHING HORIZONTAL AND VERTICAL  
CLEARANCES FOR BRIDGES OVER THE FOX RIVER

## Section

## 720.10 Authority and Clearances

AUTHORITY: Implementing and authorized by "An Act in relation to the regulation of the rivers, lakes and streams of the State of Illinois" (Ill. Rev. Stat. 1989, ch. 19, pars. 52 et seq.)

SOURCE: Filed March 4, 1958; codified at 6 Ill. Reg. 14689; amended at 15 Ill. Reg. 9068, effective June 10, 1991.

AMENDED

JUN 10 1991

## 92 ILLINOIS ADMINISTRATIVE CODE

## CHAPTER I, §720.10

## SUBCHAPTER 1

**Section 720.10 Authority and Clearances**

The Department of Transportation, acting under authority conferred upon it by "AN ACT in relation to the regulation of rivers, lakes and streams of the State of Illinois", approved June 10, 1911, effective July 1, 1911, as amended (Ill. Rev. Stat. 1989, ch. 19, pars. 52 et seq.), does hereby declare and order, pursuant to an investigation and hearing concerning the adequacy of horizontal and vertical bridge clearance of a new bridge proposed to be constructed by the Department's Division of Highways to replace the existing five arch structure known as Burton's Bridge in Section 19, Township 44 North, Range 9 East of the Third Principal Meridian, McHenry County, Illinois, that the minimum horizontal clearance for bridges hereafter constructed over the Fox River between Algonquin and the southern (downstream) right-of-way limit of route 173 shall be 100 feet and minimum vertical clearance for such bridges shall be 15 feet, above normal pool level.

(SOURCE: Amended at 15 Ill. Reg. \_\_\_\_\_, effective \_\_\_\_\_)

**AMENDED****JUN 10 1991**

### Vertical Clearances - Fox River Bridges

Pursuant to the request of the Commission for the Development of the Fox River at its meeting in St. Charles on February 24, 1958, there is attached a tabulation of existing bridges over the Fox River, beginning at its mouth at Ottawa and progressing in upstream order, and showing the vertical clearances afforded by each structure under existing conditions.

The clearances noted are measured from the water surface at that stage which closely approximates the normal flow of the river, in its present condition, during the boating season. The clearance data for those bridges upstream from Yorkville is considered more accurate than that for those structures between Ottawa and Yorkville for the reason that detailed flow studies have not yet been made in the latter reach.

It will also be noted that, in many cases, two sets of clearance figures are given. This arises from the fact that the low steel elevation of the main span of some of the bridges is not constant across the span, due to the fact that some are arch structures and others are built on a grade determined by local conditions. Hence the available vertical clearance varies with the limits stated, depending upon one's transverse location along the span in question.

No attempt has been made, in this report, to show vertical clearances above high water. Such data has been omitted, since this would require a concensus both as to whether clearances should be given above ordinary, mean, or extreme record

high water or for the high water to be expected from some particular frequency of flood and as to whether the particular high water chosen should be that representing all-year conditions or during the boating season only.

In this general connection, the Commission is advised that, by order of the Department of Public Works and Buildings dated April 20, 1953, "the minimum horizontal clearance for bridges hereafter constructed over the Fox River between Algonquin and the Illinois-Wisconsin State Line shall be 100 feet and the minimum vertical clearance for such bridges shall be 15 feet, above normal pool level".

## **SECTION 8**

**HYDRAULIC ANALYSIS (HEC-RAS)  
COMPACT DISK**

# **REGULATORY HEC-2 MODEL HARDCOPY**

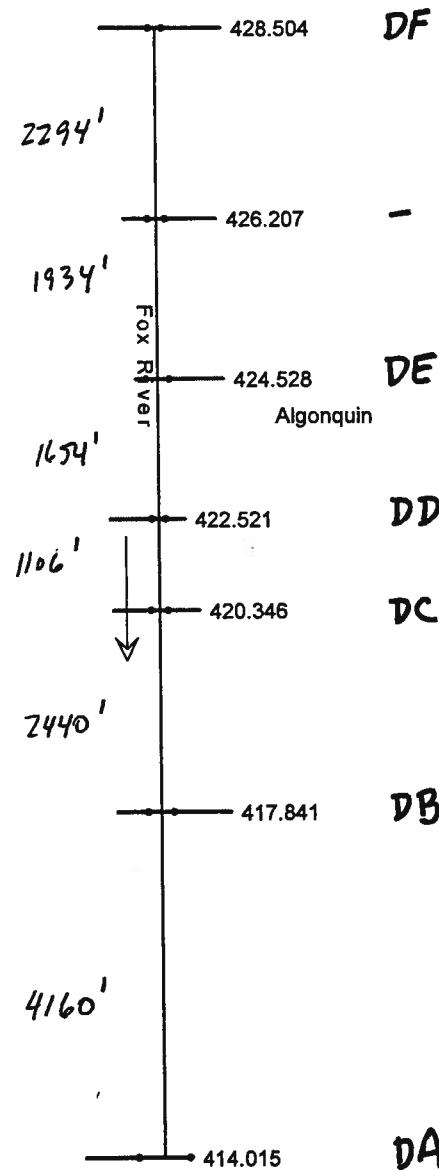
---

Christopher B. Burke Engineering, Ltd.

Hydraulic Report  
Longmeadow Parkway over the  
Fox River

# FIS Regulatory Model

HEC-2 (NGVD29)



Saylor, Bill

**From:** Saylor, Bill  
**Sent:** Wednesday, February 18, 2004 1:31 PM  
**To:** 'Chinliang R. Wang'  
**Subject:** RE: Fox R HEC-2 Carpentersville dam to Algonquin dam



FRP1012.zip

Our administration office received your payment.

Attached is a WinXP zipped text file that includes your reach of interest. This is the data I believe to include 1993 revisions in certain parts. (Note: the filedate is not the data vintage.) Note also that the X1 labeling scheme changes three or four times in this file!

If you have trouble extracting the attachment, let me know and I can resend by pasting the text straight into email.

I will mail the c. 1979 run printout today.

Thanks,  
-Bill Saylor

William Saylor  
Associate Supportive Scientist  
Watershed Science Section

Illinois State Water Survey  
2204 Griffith Drive  
Champaign, IL 61820-7495 MC-674

Surface Water & Floodplain  
Information

(217) 333-0447  
(217) 333-2304 (fax)

MESSAGING (1<sup>st</sup> ): (we begin FRIO test file)

~~W/addrress~~ ( - 15/15/15 )  
103 revised version

3.  $\text{DA} - \text{DE}$ , but see [the related discussion](#) ([here](#))

DE  
1942

See next page



SECNO	XLCH	ELTRD	ELLC	ELVIN	G	CSEL	CRMS	EG	10KAS	VCH	AREA	.01K
47240	78.151	0.	0.	714.70	5775.00	722.94	0.	723.32	105.21	4.63	1183.17	.561.02
DAM	78.151	0.	0.	714.70	8345.00	723.46	0.	724.01	121.24	5.88	1427.18	.75.72
	78.151	0.	0.	714.70	10095.00	723.80	0.	724.45	122.51	6.54	1593.40	.890.50
	78.151	0.	0.	714.70	12525.00	724.22	0.	725.00	134.96	7.14	1838.46	1078.13
DA	78.412	110.00	0.	716.60	5775.00	723.51	0.	723.54	2.93	1.52	3802.25	3371.96
	78.412	110.00	0.	716.60	8345.00	724.24	0.	724.30	3.95	1.92	4378.08	4203.65
	78.412	110.00	0.	716.60	10095.00	724.71	0.	724.78	4.47	2.15	4748.97	4774.18
	78.412	110.00	0.	716.60	12525.00	725.30	0.	725.39	5.11	2.43	5238.96	5541.78
DB	78.412	1186.00	0.	711.10	5775.00	723.86	0.	723.91	3.22	1.13	3371.56	3217.21
	78.412	1186.00	0.	711.10	8345.00	724.72	0.	724.79	4.27	2.17	3954.44	4030.15
	78.412	1186.00	0.	711.10	10095.00	725.25	0.	725.34	4.83	2.42	4536.27	4591.55
	78.412	1186.00	0.	711.10	12525.00	725.92	0.	726.03	5.52	2.14	4839.29	5331.34
DC	79.137	4160.00	0.	714.30	5775.00	725.15	0.	725.22	3.04	2.13	3031.77	3312.78
	79.137	4160.00	0.	714.30	8345.00	726.37	0.	726.47	3.79	2.60	3903.71	4286.26
	79.137	4160.00	0.	714.30	10095.00	727.09	0.	727.21	4.16	2.86	4539.26	4946.60
	79.137	4160.00	0.	714.30	12525.00	727.97	0.	728.11	4.55	3.16	5492.39	5869.09
DD	79.611	2400.00	0.	715.60	5775.00	726.16	0.	726.29	6.40	3.08	2660.01	2228.40
	79.611	2400.00	0.	715.60	8345.00	727.55	0.	727.71	6.73	3.49	3698.57	3224.64
	79.611	2400.00	0.	715.60	10095.00	728.35	0.	728.52	6.90	3.73	4326.79	3844.58
	79.611	2400.00	0.	715.60	12525.00	729.32	0.	729.51	7.16	4.03	5110.95	4688.21
DE	80.023	1106.00	0.	714.30	5775.00	726.91	0.	727.07	6.46	3.36	2503.40	2271.42
	80.023	1106.00	0.	714.30	8345.00	728.38	0.	728.57	6.87	3.81	3598.46	3183.91
	80.023	1106.00	0.	714.30	10095.00	729.23	0.	729.43	7.04	4.05	4267.24	3805.44
	80.023	1106.00	0.	714.30	12525.00	730.25	0.	730.46	7.25	4.34	5107.27	4650.79
DF	80.352	1694.00	0.	716.60	5775.00	727.69	0.	727.75	2.61	2.10	3273.70	.3573.26
	80.352	1694.00	0.	716.60	8345.00	729.23	0.	729.32	2.89	2.45	4592.88	.4906.84
	80.352	1694.00	0.	716.60	10095.00	730.11	0.	730.21	3.04	2.64	5387.07	.5193.37
	80.352	1694.00	0.	716.60	12525.00	731.17	0.	731.28	3.21	2.88	6405.71	.6990.90
DG	80.721	1934.00	0.	716.80	5775.00	728.31	0.	728.42	4.44	2.74	2676.71	.2741.60
	80.721	1934.00	0.	716.80	8345.00	729.32	0.	730.06	4.81	3.18	3724.29	.3804.37
	80.721	1934.00	0.	716.80	10095.00	730.92	0.	730.98	4.98	3.41	4424.86	.4524.97
	80.721	1934.00	0.	716.80	12525.00	731.92	0.	732.09	5.17	3.68	5332.22	.5510.56
DF	81.156	2294.00	0.	717.20	5775.00	729.49	0.	729.64	6.34	3.28	2774.40	.2293.23
	81.156	2294.00	0.	717.20	8345.00	731.11	0.	731.26	5.87	3.52	4312.76	.3443.41
	81.156	2294.00	0.	717.20	10095.00	732.02	0.	732.17	5.64	3.63	5382.93	.4252.27
	81.156	2294.00	0.	717.20	12525.00	733.13	0.	733.27	5.29	3.74	6915.81	.5445.41

N6-D29

## TEST RUN 10 YEAR FREQL

## SUMMARY PRINTOUT TABLE 150

SECNC	YC	CWSEL	DIFKSP	DIFKSX	DIFKWS	TOPKID	XLCN
78.151	5775.00	722.94	0.52	0.	0.	458.01	-0.
78.151	8345.00	723.46	0.	0.	0.	482.65	-0.
78.151	10095.00	723.80	.34	0.	0.	496.22	-0.
78.151	12525.00	724.22	.42	0.	0.	627.58	-0.
78.172	5775.00	723.51	0.	.57	0.	736.89	110.00
78.172	8345.00	724.24	.74	.76	0.	792.50	110.00
78.172	10095.00	724.71	.47	.91	0.	812.4	110.00
78.172	12525.00	725.30	.59	1.08	0.	866.79	110.00
78.412	5775.00	723.86	0.	.36	0.	650.26	1186.00
78.412	8345.00	724.72	.86	.48	0.	711.15	1186.00
78.412	10095.00	725.25	.53	.54	0.	728.94	1186.00
78.412	12525.00	725.92	.67	.62	0.	813.67	1186.00
79.137	5775.00	725.15	0.	1.29	0.	633.55	4160.00
79.137	8345.00	726.37	1.22	1.65	0.	806.83	4160.00
79.137	10095.00	727.09	.72	1.84	0.	999.2	4160.00
79.137	12525.00	727.97	.88	2.05	0.	1143.43	4160.00
79.611	5775.00	726.16	0.	1.01	0.	714.76	2440.00
79.611	8345.00	727.55	1.39	1.18	0.	772.47	2440.00
79.611	10095.00	728.35	.60	1.26	0.	796.16	2440.00
79.611	12525.00	729.32	.97	1.35	0.	836.08	2440.00
80.023	5775.00	726.91	0.	.75	0.	702.17	1106.00
80.023	8345.00	728.38	1.48	.83	0.	779.74	1106.00
80.023	10095.00	729.23	.84	.88	0.	806.76	1106.00
80.023	12525.00	730.25	1.02	.93	0.	837.03	1106.00
80.352	5775.00	727.69	0.	.78	0.	788.92	1694.00
80.352	8345.00	729.23	1.55	.85	0.	888.97	1694.00
80.352	10095.00	730.11	.88	.88	0.	924.29	1694.00
80.352	12525.00	731.17	1.06	.92	0.	983.02	1694.00
80.721	5775.00	728.31	0.	.63	0.	532.74	1934.00
80.721	8345.00	729.92	1.60	.69	0.	751.58	1934.00
80.721	10095.00	730.82	.91	.71	0.	794.49	1934.00
80.721	12525.00	731.92	1.10	.75	0.	853.24	1934.00
81.156	5775.00	729.49	0.	1.18	0.	812.07	2294.00
81.156	8345.00	731.11	1.62	1.19	0.	1008.73	2294.00
81.156	10095.00	732.02	.91	1.20	0.	1288.40	2294.00
81.156	12525.00	733.13	1.11	1.21	0.	1431.86	2294.00

SUMMARY OF ERRORS

\*\*\*\*\*  
HEC2 RELEASE DATE: NOV 76 UPDATED JULY 1979  
PART NUMBER = 01-02-03  
MODIFICATION = 150-51-52-53  
\*\*\*\*\*

NOTE: ASTERISK (\*) AT LEFT OF CROSS=SECTION NUMBER INDICATES MESSAGE IN SUMMARY OF ERRORS LIST

TEST RUN 10 YEAR FREOU

SUMMARY PRINTOUT TABLE T10

SECNC	CHSEL	DIFFNG	EG	TOPNID	GLOB	GROB	PERRNC	STENCL	STCHRL	STCHR	STENCRL
78.151	723.80	0.00	724.45	496.02	.3180.22	.6830.79	83.98	0.00	0.00	2381.00	2766.00
78.151	723.90	.010	720.57	4356.24	.2856.02	.7236.98	.00	.05	.2219.10	2381.00	2766.00
78.172	720.71	0.00	724.78	812.59	0.00	10079.03	15.97	0.00	.070	2171.00	2917.00
78.172	724.33	.12	721.90	743.13	0.00	10095.00	0.00	.00	2171.00	2171.00	2917.00
78.412	725.35	0.00	728.34	728.99	0.00	10000.65	94.35	0.00	0.00	2022.00	2652.00
78.412	725.35	.10	725.44	575.27	0.00	10045.00	0.00	.01	.2022.00	.2022.00	.2652.00
79.137	727.49	0.00	727.21	999.62	365.05	4336.35	393.40	0.00	.070	2684.00	3000.00
79.137	727.16	.08	727.30	557.07	289.37	9487.88	317.75	.02	.2571.14	.2684.00	3000.00
79.611	728.35	0.00	728.52	796.46	889.46	7893.06	1312.48	0.00	0.00	2386.00	2517.00
79.611	728.04	.09	728.62	649.08	817.11	8021.69	1256.21	.02	.2195.11	.2366.00	.2517.00
80.023	729.71	0.00	729.53	806.56	942.18	7680.12	1860.70	0.00	0.00	.2394.00	.2394.00
80.023	729.71	.09	729.52	642.89	465.56	7791.65	1831.79	.02	.2087.99	.2234.00	.2394.00
80.392	730.11	0.00	730.21	924.29	1051.28	8872.73	170.99	0.00	0.00	2654.00	2904.00
80.392	730.20	.06	730.30	767.76	1105.95	9005.33	85.72	.02	.2162.46	.2654.00	.2904.00
80.721	730.92	0.00	730.98	794.49	995.64	8379.39	719.98	0.00	0.00	.2620.00	.2620.00
80.721	730.92	.09	731.07	581.75	938.40	8501.59	655.01	.02	.2400.60	.2400.60	.2620.00
81.156	732.02	0.00	732.17	1288.10	556.03	7008.85	2530.12	0.00	0.00	2720.00	2833.00
81.156	732.02	.10	732.27	878.09	878.03	7113.60	2503.37	.02	.2507.24	.2720.00	2833.00

X  
 TEST RUN 10 YEAR FREQ  
 SUMMARY PRINTOUT TABLE 150

SENG	XICH	FLTRD	ELLC	ELVIN	O	CASEL	CRKS	EG	10K&S	VOL	AREA	.01K
76.151	0.00	0.00	0.00	714.70	10095.00	723.80	0.00	724.45	128.51	6.54	1593.40	890.50
76.151	0.00	0.00	0.00	714.70	10095.00	723.90	0.00	724.57	128.21	6.69	1544.04	891.56
78.172	110.00	0.00	0.00	716.60	10095.00	724.71	0.00	724.78	4.97	2.15	4748.97	4774.18
78.172	110.00	0.00	0.00	716.60	10095.00	724.81	0.00	724.90	4.92	2.11	4786.86	4955.46
78.412	1166.00	0.00	0.00	7111.80	10095.00	725.25	0.00	725.34	4.83	2.42	4336.27	4591.55
78.412	1166.00	0.00	0.00	7111.80	10095.00	725.35	0.00	725.44	4.74	2.41	41880.64	4636.16
79.137	4160.00	0.00	0.00	714.30	10095.00	727.09	0.00	727.21	4.16	2.86	4539.26	4947.60
79.137	4160.00	0.00	0.00	714.30	10095.00	727.16	0.00	727.30	4.19	2.89	4138.92	4934.64
79.611	2400.00	0.00	0.00	715.60	10095.00	728.35	0.00	728.52	6.90	3.73	4326.79	3842.58
79.611	2400.00	0.00	0.00	715.60	10095.00	728.44	0.00	728.62	6.93	3.76	4101.46	3834.63
80.023	1106.00	0.00	0.00	714.30	10095.00	729.23	0.00	729.43	7.04	4.05	4267.24	3805.94
80.023	1106.00	0.00	0.00	714.30	10095.00	729.31	0.00	729.52	7.08	4.08	4034.70	3795.04
80.352	1694.00	0.00	0.00	716.60	10095.00	730.11	0.00	730.21	3.04	2.64	5387.07	5793.37
80.352	1694.00	0.00	0.00	716.60	10095.00	730.20	0.00	730.30	3.04	2.66	5317.05	5786.04
80.721	1934.00	0.00	0.00	716.80	10095.00	730.32	0.00	730.38	4.98	3.01	4424.86	4524.97
80.721	1934.00	0.00	0.00	716.80	10095.00	730.92	0.00	731.07	4.99	3.03	4143.25	4519.25
81.156	2294.00	0.00	0.00	717.20	10095.00	732.02	0.00	732.17	5.64	3.63	5382.93	4892.27
81.156	2294.00	0.00	0.00	717.20	10095.00	732.12	0.00	732.27	5.64	3.66	5043.28	4209.63

TEST RUN 10 YEAR FREQ  
SUMMARY PRINTOUT TABLE 150

SECDNO	G	CWSEL	DIFNSP	DIFNSX	DIFKNS	TOPKID	XLCN
78,151	10095.00	723.80	0.00	0.00	0.00	496.02	0.00
78,151	10095.00	723.90	.10	.00	.10	436.24	0.00
78,172	10095.00	724.71	0.00	.91	0.00	812.54	110.00
78,172	10095.00	724.83	.12	.93	.12	743.13	110.00
78,412	10095.00	725.25	0.00	.54	0.00	728.99	1186.00
78,412	10095.00	725.35	.10	.52	.10	575.27	1186.00
79,137	10095.00	727.09	0.00	1.84	0.00	999.62	4160.00
79,137	10095.00	727.16	.08	1.83	.08	557.87	4160.00
79,611	10095.00	728.35	0.00	1.26	0.00	796.46	2440.00
79,611	10095.00	728.44	.09	1.26	.09	649.88	2440.00
80,023	10095.00	729.23	0.00	.88	0.00	804.36	1106.00
80,023	10095.00	729.31	.09	.87	.09	642.89	1106.00
80,352	10095.00	730.11	0.00	.88	0.00	924.29	1694.00
80,352	10095.00	730.20	.09	.89	.09	767.76	1694.00
80,721	10095.00	730.82	0.01	.71	0.00	794.49	1934.00
80,721	10095.00	730.92	.09	.71	.09	581.75	1934.00
81,156	10095.00	731.02	0.00	1.20	0.00	1288.40	2294.00
81,156	10095.00	731.12	.10	1.20	.10	878.49	2294.00

SUMMARY OF ERRORS

FLOODWAY DATA, TEST RCH 10 YEAR FREQU.  
PROFILE NO. 2

STATION W.C.M.	FLOODWAY		WATER SURFACE ELEVATION	
	SECTION	MEAN	WITH WITCHCUT	DIFFERENCE
(FT)	AREA	VELOCITY	FLOODWAY	FLOODWAY
78' 151	644.	1544.	6.5	723.9
78' 172	743.	4787.	2.1	724.8
78' 4120	575.	4161.	2.4	725.3
79' 117E	531.	4139.	2.4	727.2
79' 611F	650.	4101.	2.5	728.5
80' 021G	641.	4035.	2.5	729.3
80' 352A	787.	5117.	2.0	730.2
80' 721	582.	4143.	2.4	730.9
81' 156B	878.	5043.	2.0	732.1
				DF

## **BASELINE HEC-RAS MODEL**

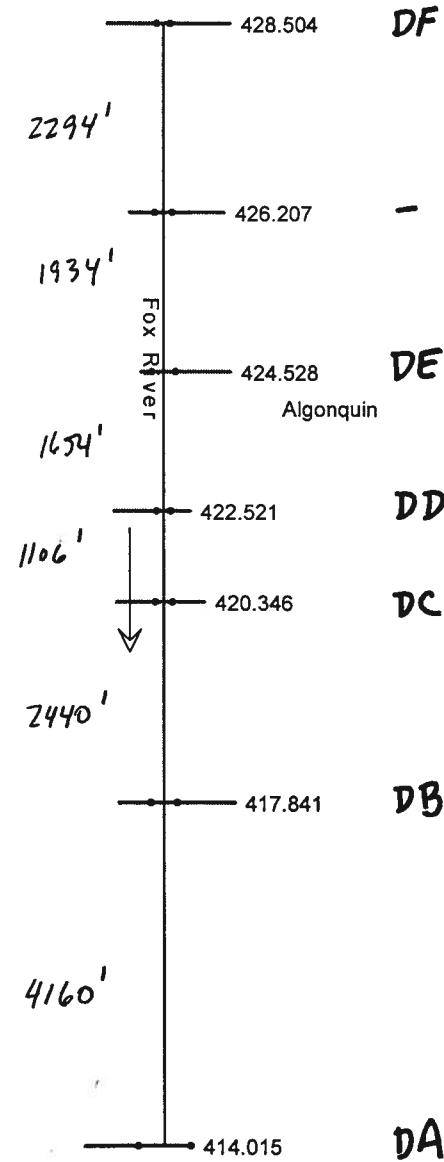
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Christopher B. Burke Engineering, Ltd.

Hydraulic Report  
Longmeadow Parkway over the  
Fox River

Baseline  
FIS Regulatory Model

from HEC-2 (NGVD29)



one of the XS's are Geo-Referenced (- Geo-Ref user entered XS - Geo-Ref interpolated XS - Non Geo-Ref user entered XS - Non Geo-Ref interpolated XS)

*FIS Regulatory  
Model  
from HEC-2 (NGVD2*

HEC-RAS Version 4.1.0 Jan 2010  
U.S. Army Corps of Engineers  
Hydrologic Engineering Center  
609 Second Street  
Davis, California

X	X	XXXXXX	XXXX	XXXX	XX	XXXX
X	X	X	X X	X X	X X	X
X	X	X	X	X X	X X	X
XXXXXX	XXXX	X	XXX	XXXX	XXXXXX	XXXX
X	X	X	X	X X	X X	X
X	X	X	X X	X X	X X	X
X	X	XXXXXX	XXXX	X X	X X	XXXXXX

#### PROJECT DATA

Project Title: Longmeadow Parkway Fox River  
Project File : LongmeadowParkway.prj  
Run Date and Time: 1/26/2011 3:00:59 PM

Project in English units

#### Project Description:

Conversion of HEC-2 for analysis of proposed Longmeadow Parkway bridge over Fox, River. CBBEL 2011 PROPOSED BOLZ ROAD HYDRAULIC MODEL

#### REGULATORY MODEL

WAT10(FRE1003)  
FOX RIVER - EAST DUNDEE  
1 YR FLOOD

#### PLAN DATA

Plan Title: FIS Regulatory HEC-2  
Plan File : n:\kanecounty\99236\Drain\Model\HEC-RAS\LongmeadowParkway.p02

Geometry Title: FIS Regulatory Geometry  
Geometry File : n:\kanecounty\99236\Drain\Model\HEC-RAS\LongmeadowParkway.g02

Flow Title : FIS Regulatory Flows and Starting WSELs  
Flow File : n:\kanecounty\99236\Drain\Model\HEC-RAS\LongmeadowParkway.f02

#### Plan Description:

HEC-2 model circa 1979, hardcopy received from ISWS Feb. 2004. Truncated to include all sections from DA (78.151) to DF (81.156).

#### Plan Summary Information:

Number of:	Cross Sections	=	7	Multiple Openings	=	0
	Culverts	=	0	Inline Structures	=	0
	Bridges	=	0	Lateral Structures	=	0

#### Computational Information

Water surface calculation tolerance	=	0.01
Critical depth calculation tolerance	=	0.01
Maximum number of iterations	=	20
Maximum difference tolerance	=	0.3
Flow tolerance factor	=	0.001

#### Computation Options

Critical depth computed only where necessary
Conveyance Calculation Method: Between every coordinate point (HEC2 Style)
Friction Slope Method: Average Conveyance
Computational Flow Regime: Subcritical Flow

## CROSS SECTION

RIVER: Fox River  
 REACH: Algonquin RS: 422.521

## INPUT

Description: 2002 FIS DD, 1979 XS 80.023

Station	Elevation	Data	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
2000	734		2009	732.4	2025	727.5	2027	727.5	2040	727.9
2050	726.9		2068	726.9	2086	726.4	2112	726.6	2131	726.1
2150	725.8		2166	725.3	2182	725.7	2191	724.6	2216	725.5
2223	727		2234	725.1	2241	721.5	2246	720.1	2251	718.5
2261	715.8		2281	714.9	2301	714.6	2321	714.3	2341	716.7
2361	719.1		2376	720.4	2381	720.9	2388	721.5	2392	722.7
2394	724.9		2441	724.4	2459	724.6	2483	724.2	2509	724.4
2533	723.6		2556	723	2590	723.9	2616	724.4	2657	725
2693	725.8		2739	726.4	2761	727.2	2791	728	2822	729.1
2851	730.1		2867	731	2882	732.4	2898	733.7	2902	734

Manning's n Values	Sta	n Val	Sta	n Val	Sta	n Val
	2000	.1	2234	.05	2394	.1

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	2234	2394		2280	1106	1654		.1	.3

## CROSS SECTION

RIVER: Fox River  
 REACH: Algonquin RS: 420.346

## INPUT

Description: 2002 FIS DC, 1979 XS 79.611

Station	Elevation	Data	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
2000	733		2007	729	2021	729.2	2043	732	2068	732.8
2085	733.1		2111	732.8	2129	731.2	2143	728.9	2161	725.9
2176	724.9		2192	725.2	2213	725.2	2236	724.6	2263	724.4
2295	724		2320	724.1	2352	723.8	2386	723.3	2392	721.4
2397	720.5		2402	720	2412	719.1	2432	717.4	2452	716.1
2472	715.6		2492	715.7	2512	716.6	2532	716.9	2552	718.4
2562	719.2		2572	720.5	2577	720.7	2582	721.4	2585	722.2
2587	724		2622	723.7	2633	723.7	2665	723.5	2683	723.2
2708	723.2		2736	723.5	2776	724.1	2808	724.8	2850	725.7
2892	726.5		2934	727.9	2965	729.5	3000	731.5	3030	732.8
3044	733									

Manning's n Values	Sta	n Val	Sta	n Val	Sta	n Val
	2000	.1	2386	.0506	2587	.1

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	2386	2587		2454	2440	2400		.1	.3

CROSS SECTION

RIVER: Fox River  
REACH: Algonquin RS: 417.841

INPUT

Description: 2002 FIS DB, 1979 XS 79.137

Station	Elevation	Data	num=	63	Sta	Elev	Sta	Elev	Sta	Elev
2000	733	2014	731.7	2041	729.5	2079	728.7	2109	727.9	
2153	727.5	2189	726.2	2216	726.5	2240	724.9	2265	726.7	
2282	730.1	2286	730.3	2293	730	2302	728.9	2311	725.1	
2342	727.5	2367	726.7	2397	727	2423	727	2434	726.8	
2445	727.1	2467	727.3	2492	727	2525	726	2556	724.9	
2593	724.6	2635	723.3	2684	722.4	2685	721.4	2685	721.1	
2690	720.8	2695	720.4	2705	719.7	2725	718.4	2745	716.4	
2765	715.2	2785	714.4	2805	714.3	2825	714.6	2845	715.4	
2865	715.8	2885	716.2	2905	716.4	2925	716.2	2945	716.1	
2965	718.4	2985	720.4	2992	720.7	2999	721.4	3000	722.7	
3030	723.4	3047	723.3	3072	723.7	3102	724.4	3129	724.4	
3160	725	3188	725.3	3238	726.4	3274	727.5	3293	728.5	
3325	729.6	3360	731.8	3375	733					

Manning's n Values

Sta	n Val	Sta	n Val	Sta	n Val
2000	.1	2684	.05	3000	.1

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	2684	3000		3520	4160	3746	.1		.3

CROSS SECTION

RIVER: Fox River  
REACH: Algonquin RS: 414.015

INPUT

Description: 2002 FIS DA, 1979 XS 78.412

Station	Elevation	Data	num=	66	Sta	Elev	Sta	Elev	Sta	Elev
2000	736.8	2022	737.3	2048	730.7	2091	722.7	2092	721.4	
2097	720.8	2102	720.3	2112	718.4	2132	715	2152	715	
2172	714.8	2192	715.1	2212	715.7	2232	716.8	2252	717.8	
2272	718.4	2292	719.3	2312	720.1	2332	720.4	2352	720.3	
2372	720.3	2392	720.3	2412	720.3	2432	720.4	2452	720.2	
2472	717.7	2492	714.3	2512	711.8	2532	712.7	2552	715	
2572	718.4	2592	719.6	2612	720.1	2622	720.6	2627	720.8	
2632	721.4	2652	723.3	2682	723.2	2708	723.4	2727	723.7	
2751	724.2	2780	724.7	2784	724.2	2795	725	2821	725.6	
2848	725.8	2886	725.9	2923	726.6	2952	726.9	2990	727.8	
3017	728.3	3048	728.6	3060	729.9	3064	729.9	3069	726.9	
3086	726.9	3095	729.1	3107	729.4	3130	729.2	3151	730.7	
3178	731.1	3204	731	3229	731.6	3255	732.2	3266	732.4	
3283	733									

Manning's n Values

Sta	n Val	Sta	n Val	Sta	n Val
2000	.1	2202	.0501	2652	.1

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	2022	2652		0	0	0	.1		.3

SUMMARY OF MANNING'S N VALUES

River: Fox River

Reach	River Sta.	n1	n2	n3
Algonquin	428.504	.1	.05	.1
Algonquin	426.207	.1	.05	.1
Algonquin	424.528	.102	.0507	.102
Algonquin	422.521	.1	.05	.1
Algonquin	420.346	.1	.0506	.1
Algonquin	417.841	.1	.05	.1
Algonquin	414.015	.1	.0501	.1

SUMMARY OF REACH LENGTHS

River: Fox River

Reach	River Sta.	Left	Channel	Right
Algonquin	428.504	2094	2294	2134
Algonquin	426.207	1960	1934	1946
Algonquin	424.528	1746	1694	1666
Algonquin	422.521	2280	1106	1654
Algonquin	420.346	2454	2440	2400
Algonquin	417.841	3520	4160	3746
Algonquin	414.015	0	0	0

SUMMARY OF CONTRACTION AND EXPANSION COEFFICIENTS

River: Fox River

Reach	River Sta.	Contr.	Expan.
Algonquin	428.504	.1	.3
Algonquin	426.207	.1	.3
Algonquin	424.528	.1	.3
Algonquin	422.521	.1	.3
Algonquin	420.346	.1	.3
Algonquin	417.841	.1	.3
Algonquin	414.015	.1	.3

Errors Warnings and Notes for Plan : FIS HEC-2

Location:	River: Fox River Reach: Algonquin RS: 428.504 Profile: 10 yr
Warning:	The energy loss was greater than 1.0 ft (0.3 m). between the current and previous cross section. This may indicate the need for additional cross sections.
Location:	River: Fox River Reach: Algonquin RS: 428.504 Profile: 50 yr
Warning:	Divided flow computed for this cross-section.
Warning:	The energy loss was greater than 1.0 ft (0.3 m). between the current and previous cross section. This may indicate the need for additional cross sections.
Location:	River: Fox River Reach: Algonquin RS: 428.504 Profile: 100 yr
Warning:	Divided flow computed for this cross-section.
Warning:	The energy loss was greater than 1.0 ft (0.3 m). between the current and previous cross section. This may indicate the need for additional cross sections.
Location:	River: Fox River Reach: Algonquin RS: 428.504 Profile: 500 yr
Warning:	The energy loss was greater than 1.0 ft (0.3 m). between the current and previous cross section. This may indicate the need for additional cross sections.
Location:	River: Fox River Reach: Algonquin RS: 424.528 Profile: 10 yr
Warning:	Divided flow computed for this cross-section.
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7 or greater than 1.4. This may indicate the need for additional cross sections.
Location:	River: Fox River Reach: Algonquin RS: 424.528 Profile: 50 yr
Warning:	Divided flow computed for this cross-section.
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7 or greater than 1.4. This may indicate the need for additional cross sections.
Location:	River: Fox River Reach: Algonquin RS: 424.528 Profile: 100 yr
Warning:	Divided flow computed for this cross-section.
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7 or greater than 1.4. This may indicate the need for additional cross sections.
Location:	River: Fox River Reach: Algonquin RS: 424.528 Profile: 500 yr
Warning:	Divided flow computed for this cross-section.
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7 or greater than 1.4. This may indicate the need for additional cross sections.
Location:	River: Fox River Reach: Algonquin RS: 422.521 Profile: 10 yr
Warning:	Divided flow computed for this cross-section.
Location:	River: Fox River Reach: Algonquin RS: 420.346 Profile: 10 yr
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7 or greater than 1.4. This may indicate the need for additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m). between the current and previous cross section. This may indicate the need for additional cross sections.
Location:	River: Fox River Reach: Algonquin RS: 420.346 Profile: 50 yr
Warning:	The energy loss was greater than 1.0 ft (0.3 m). between the current and previous cross section. This may indicate the need for additional cross sections.
Location:	River: Fox River Reach: Algonquin RS: 420.346 Profile: 100 yr
Warning:	The energy loss was greater than 1.0 ft (0.3 m). between the current and previous cross section. This may indicate the need for additional cross sections.
Location:	River: Fox River Reach: Algonquin RS: 420.346 Profile: 500 yr
Warning:	Divided flow computed for this cross-section.
Warning:	The energy loss was greater than 1.0 ft (0.3 m). between the current and previous cross section. This may indicate the need for additional cross sections.
Location:	River: Fox River Reach: Algonquin RS: 417.841 Profile: 10 yr
Warning:	Divided flow computed for this cross-section.

## HEC-RAS Plan: FIS HEC-2 River: Fox River Reach: Algonquin

Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Algonquin	428.504	10 yr	5775.00	717.20	729.49		729.63	0.000636	3.29	2769.96	811.61	0.19
Algonquin	428.504	50 yr	8345.00	717.20	731.11		731.26	0.000588	3.52	4312.07	1008.62	0.19
Algonquin	428.504	100 yr	10095.00	717.20	732.02		732.17	0.000563	3.63	5387.33	1288.75	0.19
Algonquin	428.504	500 yr	12525.00	717.20	733.13		733.27	0.000530	3.74	6911.69	1431.69	0.18
Algonquin	426.207	10 yr	5775.00	716.80	728.31		728.42	0.000444	2.74	2676.57	532.73	0.16
Algonquin	426.207	50 yr	8345.00	716.80	729.92		730.06	0.000482	3.18	3720.93	751.57	0.17
Algonquin	426.207	100 yr	10095.00	716.80	730.82		730.98	0.000498	3.41	4421.26	794.47	0.18
Algonquin	426.207	500 yr	12525.00	716.80	731.92		732.09	0.000517	3.68	5321.28	853.41	0.18
Algonquin	424.528	10 yr	5775.00	716.60	727.69		727.75	0.000269	2.10	3270.41	788.32	0.12
Algonquin	424.528	50 yr	8345.00	716.60	729.23		729.32	0.000298	2.45	4590.02	888.90	0.13
Algonquin	424.528	100 yr	10095.00	716.60	730.11		730.20	0.000313	2.64	5383.83	924.08	0.14
Algonquin	424.528	500 yr	12525.00	716.60	731.17		731.28	0.000331	2.88	6404.75	983.04	0.14
Algonquin	422.521	10 yr	5775.00	714.30	726.91		727.07	0.000648	3.36	2498.92	702.21	0.19
Algonquin	422.521	50 yr	8345.00	714.30	728.39		728.57	0.000688	3.81	3595.58	779.82	0.20
Algonquin	422.521	100 yr	10095.00	714.30	729.23		729.43	0.000704	4.05	4264.41	806.45	0.21
Algonquin	422.521	500 yr	12525.00	714.30	730.26		730.47	0.000725	4.34	5107.75	837.77	0.21
Algonquin	420.346	10 yr	5775.00	715.60	726.16		726.29	0.000655	3.07	2657.55	714.55	0.19
Algonquin	420.346	50 yr	8345.00	715.60	727.55		727.71	0.000687	3.49	3698.36	772.46	0.20
Algonquin	420.346	100 yr	10095.00	715.60	728.35		728.52	0.000704	3.72	4326.32	796.45	0.20
Algonquin	420.346	500 yr	12525.00	715.60	729.32		729.51	0.000727	4.01	5114.60	836.64	0.21
Algonquin	417.841	10 yr	5775.00	714.30	725.15		725.22	0.000304	2.13	3030.74	633.28	0.13
Algonquin	417.841	50 yr	8345.00	714.30	726.37		726.47	0.000379	2.60	3903.71	806.83	0.15
Algonquin	417.841	100 yr	10095.00	714.30	727.09		727.21	0.000416	2.86	4539.05	999.57	0.16
Algonquin	417.841	500 yr	12525.00	714.30	727.97		728.11	0.000455	3.16	5495.27	1143.89	0.17
Algonquin	414.015	10 yr	5775.00	711.80	723.86		717.77	0.000325	1.73	3366.13	649.91	0.13
Algonquin	414.015	50 yr	8345.00	711.80	724.72		718.64	0.000430	2.17	3949.06	711.01	0.15
Algonquin	414.015	100 yr	10095.00	711.80	725.25		719.13	0.000488	2.43	4329.53	728.54	0.16
Algonquin	414.015	500 yr	12525.00	711.80	725.92		719.73	0.000555	2.74	4834.96	813.36	0.17

## FLOW DATA

Flow Title: FIS Regulatory Flows and Starting WSELS  
Flow File : n:\kanecounty\99236\Drain\Model\HEC-RAS\LongmeadowParkway.f02

### Flow Data (cfs)

River	Reach	RS	10 yr	50 yr	100 yr	500 yr
Fox River	Algonquin	428.504	5775	8345	10095	12525

## Boundary Conditions

River	Reach	Profile	Upstream	Downstream
Fox River	Algonquin	10 yr		Known WS = 723.86
Fox River	Algonquin	50 yr		Known WS = 724.72
Fox River	Algonquin	100 yr		Known WS = 725.25
Fox River	Algonquin	500 yr		Known WS = 725.92

## GEOMETRY DATA

Geometry Title: FIS Regulatory Geometry  
Geometry File : n:\kanecounty\99236\Drain\Model\HEC-RAS\LongmeadowParkway.g02

### CROSS SECTION

RIVER: Fox River  
REACH: Algonquin RS: 428.504

#### INPUT

Description: 2002 FIS DF, 1979 XS 81.156.

Station	Elevation	Data	num=	67
2000	734	2012	733.4	2037
2147	732.1	2183	731.5	2218
2259	732.1	2265	732.1	2280
2347	731.2	2371	731.2	2391
2450	731	2487	729.8	2508
2553	729.9	2563	729.9	2573
2626	729.2	2651	728.8	2680
2750	722	2755	721.3	2760
2810	717.2	2830	717.2	2850
2872	722	2883	726.2	2920
3003	727.4	3045	727.4	3083
3203	727.1	3239	727.1	3272
3365	727.7	3390	728.4	3402
3461	733.6	3466	734	

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
2000	.1	2720	.05	2883	.1

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.

Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
2720	2883	2094	2294	2134		.1		.3

## CROSS SECTION

RIVER: Fox River  
REACH: Algonquin

RS: 426.207

## INPUT

Description: 1979 XS 80.721

Station	Elevation	Data	num=	53
2000	734	2021	733.6	2056
2190	733.1	2230	732.6	2253
2328	730.1	2349	729.5	2371
2479	727.1	2505	727.1	2527
2620	724.7	2623	723.8	2637
2657	719.8	2677	718.2	2697
2757	718.6	2777	719.2	2797
2827	722	2829	723	2837
2879	723.5	2894	723.8	2907
2985	728.8	3018	729.2	3043
3109	731.2	3120	732.6	3130
				734

## Manning's n Values

Sta	n Val	Sta	n Val	Sta	n Val
2000	.1	2620	.05	2829	.1

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	2620	2829		1960	1934	1946		.1	.3

## CROSS SECTION

RIVER: Fox River  
REACH: Algonquin

RS: 424.528

## INPUT

Description: 2002 FIS DE, 1979 XS 80.352

Station	Elevation	Data	num=	68
2000	734	2018	733.1	2041
2099	730.1	2122	728.5	2148
2227	726.8	2243	726.2	2247
2273	731.2	2278	731.2	2292
2341	727.1	2353	727.6	2363
2417	727.6	2435	727.4	2455
2496	726.6	2505	726.5	2513
2568	726.6	2590	727.3	2591
2664	721.5	2669	720.5	2674
2724	716.7	2744	716.6	2764
2824	717.9	2844	718	2864
2924	720.2	2929	720.8	2934
2964	725.7	2975	726.3	2995
3046	730.3	3055	731.4	3066
				734

## Manning's n Values

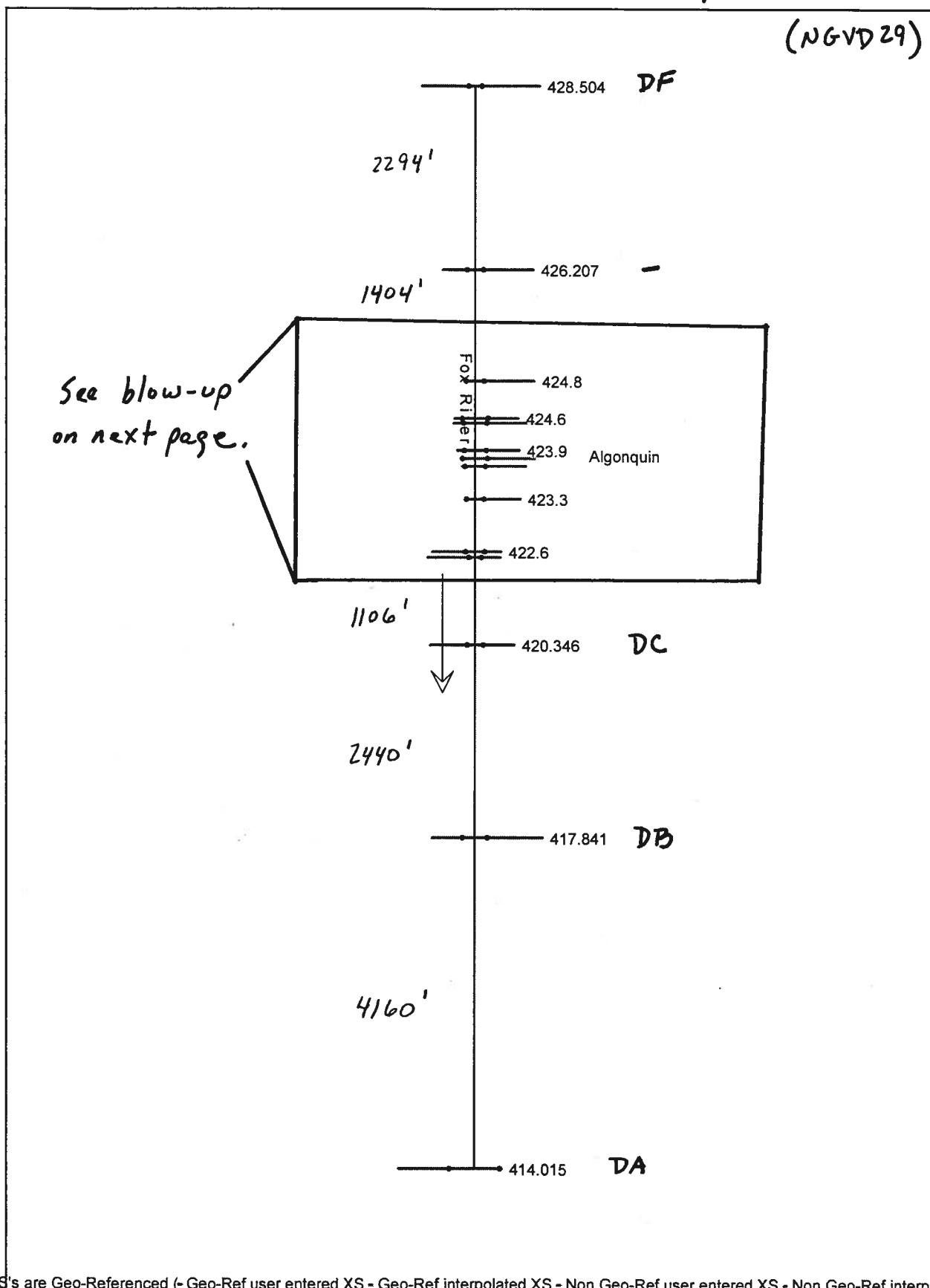
Sta	n Val	Sta	n Val	Sta	n Val
2000	.102	2654	.0507	2944	.102

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	2654	2944		1746	1694	1666		.1	.3

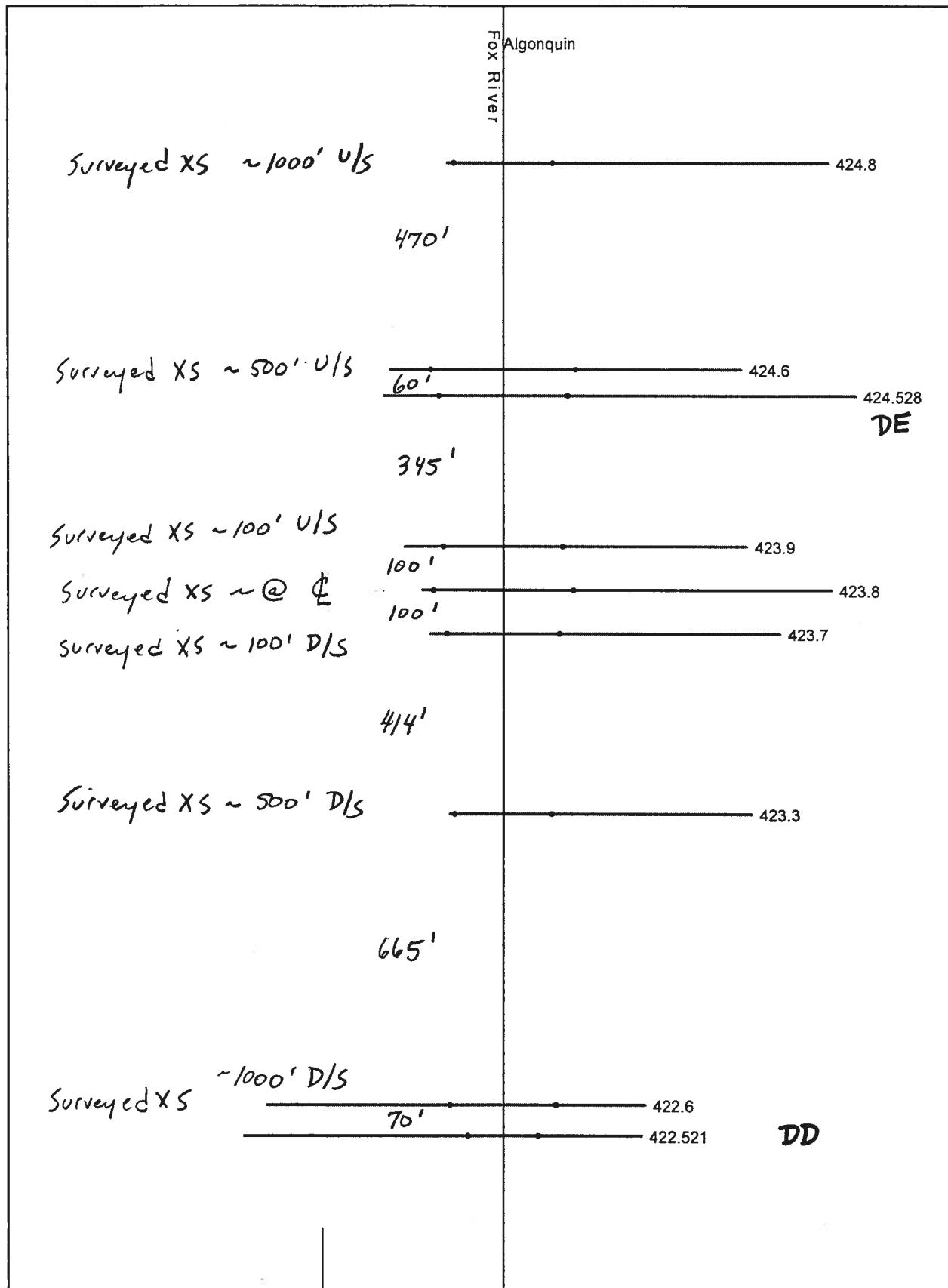
## **MODIFIED EXISTING/NATURAL CONDITIONS MODEL**

Mod. Ex. / Nat. Model

(NGVD 29)



# Mod. Ex. / Nat. Model



None of the XS's are Geo-Referenced (- Geo-Ref user entered XS - Geo-Ref interpolated XS - Non Geo-Ref user entered XS - Non Geo-Ref interpolated XS)

# Mod. Ex. / Nat. Model

(NGVD 29)

HEC-RAS Version 4.1.0 Jan 2010  
U.S. Army Corps of Engineers  
Hydrologic Engineering Center  
609 Second Street  
Davis, California

X	X	XXXXXX	XXXX	XXXX	XX	XXXX
X	X	X	X X	X X	X X	X
X	X	X	X	X X	X X	X
XXXXXX	XXXX	X	XXX	XXXX	XXXXXX	XXXX
X	X	X	X	X X	X X	X
X	X	X	X X	X X	X X	X
X	X	XXXXXX	XXXX	X X	X X	XXXXX

## PROJECT DATA

Project Title: Longmeadow Parkway Fox River  
Project File : LongmeadowParkway.prj  
Run Date and Time: 1/26/2011 3:00:59 PM

Project in English units

### Project Description:

Conversion of HEC-2 for analysis of proposed Longmeadow Parkway bridge over Fox River. CBBEL 2011 PROPOSED BOLZ ROAD HYDRAULIC MODEL  
NATURAL / EXISTING MODEL

WAT10(FRE1003)  
FOX RIVER -  
EAST DUNDEE  
1 YR FLOOD

## PLAN DATA

Plan Title: Mod Ex / Natural  
Plan File : n:\kanecounty\99236\Drain\Model\HEC-RAS\LongmeadowParkway.p03

Geometry Title: Mod Ex / Nat Geometry  
Geometry File : n:\kanecounty\99236\Drain\Model\HEC-RAS\LongmeadowParkway.g01

Flow Title : FIS Regulatory Flows and Starting WSELS  
Flow File : n:\kanecounty\99236\Drain\Model\HEC-RAS\LongmeadowParkway.f02

### Plan Description:

EEI surveyed XS inserted into HEC-2 model circa 1979, hardcopy received from ISWS Feb. 2004. Truncated to include all sections from DA (78.151) to DF (81.156).

### Plan Summary Information:

Number of: Cross Sections =	14	Multiple Openings =	0
Culverts =	0	Inline Structures =	0
Bridges =	0	Lateral Structures =	0

### Computational Information

Water surface calculation tolerance =	0.01
Critical depth calculation tolerance =	0.01
Maximum number of iterations =	20
Maximum difference tolerance =	0.3
Flow tolerance factor =	0.001

### Computation Options

Critical depth computed only where necessary
Conveyance Calculation Method: Between every coordinate point (HEC2 Style)
Friction Slope Method: Average Conveyance
Computational Flow Regime: Subcritical Flow

## FLOW DATA

Flow Title: FIS Regulatory Flows and Starting WSELS  
Flow File : n:\kanecounty\99236\Drain\Model\HEC-RAS\LongmeadowParkway.f02

### Flow Data (cfs)

River	Reach	RS	10 yr	50 yr	100 yr	500 yr
Fox River	Algonquin	428.504	5775	8345	10095	12525

### Boundary Conditions

River	Reach	Profile	Upstream	Downstream
Fox River	Algonquin	10 yr		Known WS = 723.86
Fox River	Algonquin	50 yr		Known WS = 724.72
Fox River	Algonquin	100 yr		Known WS = 725.25
Fox River	Algonquin	500 yr		Known WS = 725.92

## GEOMETRY DATA

Geometry Title: Mod Ex / Nat Geometry  
Geometry File : n:\kanecounty\99236\Drain\Model\HEC-RAS\LongmeadowParkway.g01

### CROSS SECTION

RIVER: Fox River  
REACH: Algonquin                    RS: 428.504

#### INPUT

Description: 2002 FIS DF, 1979 XS 81.156.

Station	Elevation	Data num=	67	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
2000	734	2012	733.4	2037	732.9	2076	732.5	2110	732.1		
2147	732.1	2183	731.5	2218	731.5	2244	731.2	2247	731.2		
2259	732.1	2265	732.1	2280	731.4	2300	731.5	2331	731.2		
2347	731.2	2371	731.2	2391	731	2405	731.7	2429	731.7		
2450	731	2487	729.8	2508	729.5	2527	729.5	2548	729.6		
2553	729.9	2563	729.9	2573	729.9	2582	729.6	2602	729.6		
2626	729.2	2651	728.8	2680	729	2720	726.9	2743	724.1		
2750	722	2755	721.3	2760	720.6	2770	719.5	2790	717.9		
2810	717.2	2830	717.2	2850	718.8	2860	718.8	2865	720.5		
2872	722	2883	726.2	2920	726.6	2942	726.6	2976	727		
3003	727.4	3045	727.4	3083	727.6	3137	727.3	3176	727.3		
3203	727.1	3239	727.1	3272	727.4	3307	727.3	3334	727.3		
3365	727.7	3390	728.4	3402	728.7	3423	729.6	3443	731.2		
3461	733.6	3466	734								

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
2000	.1	2720	.05	2883	.1

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.

Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
2720	2883	2094	2294	2134		.1		.3

Manning's n Values			num=			3		
Sta	n	Val	Sta	n	Val	Sta	n	Val
6997	.101		7582	.0525		7899	.101	

Bank	Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
		7582	7899		100	100	100		.1	.3

## CROSS SECTION

RIVER: Fox River  
REACH: Algonquin RS: 423.7

## INPUT

Description: 2098 EEI SURVEYED SECTION 20+98.75

**Station Elevation Data      num=      24**

Sta	Elev								
7000	731.8	7016	727.8	7032	724.8	7075	725.8	7087	727.8
7196	727.8	7269	725.8	7298	724.2	7341	725.4	7352	724.7
7382	724.2	7422	725.2	7471	724.9	7499	723.8	7504	721.3
7535	718.9	7579	715.8	7608	714.9	7671	717.7	7689	718.3
7737	722.3	7754	725.3	7770	726.9	7791	731.5		

```

Manning's n Values          num=      3
      Sta   n Val      Sta   n Val      Sta   n Val
      7000   .101    7499   .0525    7754   .101

```

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 7499 7754 426 414 410 .1 .3

## CROSS SECTION

RIVER: Fox River  
REACH: Algonquin RS: 423.3

## INPUT

Description: 2453 EEI SURVEYED SECTION 24+53.16

Station Elevation Data      num=

Sta	Elev								
7000	730.8	7124	726.6	7191	724.7	7292	725.2	7324	724.5
7412	725.7	7453	724.7	7464	721.2	7475	718.9	7533	716.7
7547	715.1	7583	714.1	7634	717.3	7667	722.3	7673	726.6
7682	730.5	7684	731						

Manning's n Values			num=			3		
Sta	n	Val	Sta	n	Val	Sta	n	Val
7000	.101	7453	.052	7673	.101			

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	7453	7673		690	665	650	.1	.3	

## CROSS SECTION

RIVER: Fox River  
REACH: Algonquin RS: 422.6

## INPUT

Description: 3073 EEI SURVEYED SECTION 30+73.04

ISWS REGULATORY MODEL PRIOR TO

#### THIS POINT

Manning's n Values num= 3  
 Sta n Val Sta n Val Sta n Val  
 6990 .1 7192 .051 7432 .1

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 7192 7432 75 70 68 .1 .3

CROSS SECTION

RIVER: Fox River  
 REACH: Algonquin RS: 422.521

INPUT  
 Description: 2002 FIS DD, 1979 XS 80.023 shifted 5000' rt.  
 Station Elevation Data num= 50  

Sta	Elev								
7000	734	7009	732.4	7025	727.5	7027	727.5	7040	727.9
7050	726.9	7068	726.9	7086	726.4	7112	726.6	7131	726.1
7150	725.8	7166	725.3	7182	725.7	7191	724.6	7216	725.5
7223	727	7234	725.1	7241	721.5	7246	720.1	7251	718.5
7261	715.8	7281	714.9	7301	714.6	7321	714.3	7341	716.7
7361	719.1	7376	720.4	7381	720.9	7388	721.5	7392	722.7
7394	724.9	7441	724.4	7459	724.6	7483	724.2	7509	724.4
7533	723.6	7556	723	7590	723.9	7616	724.4	7657	725
7693	725.8	7739	726.4	7761	727.2	7791	728	7822	729.1
7851	730.1	7867	731	7882	732.4	7898	733.7	7902	734

Manning's n Values num= 3  
 Sta n Val Sta n Val Sta n Val  
 7000 .1 7234 .05 7394 .1

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 7234 7394 2280 1106 1654 .1 .3

CROSS SECTION

RIVER: Fox River  
 REACH: Algonquin RS: 420.346

INPUT  
 Description: 2002 FIS DC, 1979 XS 79.611  
 Station Elevation Data num= 51  

Sta	Elev								
2000	733	2007	729	2021	729.2	2043	732	2068	732.8
2085	733.1	2111	732.8	2129	731.2	2143	728.9	2161	725.9
2176	724.9	2192	725.2	2213	725.2	2236	724.6	2263	724.4
2295	724	2320	724.1	2352	723.8	2386	723.3	2392	721.4
2397	720.5	2402	720	2412	719.1	2432	717.4	2452	716.1
2472	715.6	2492	715.7	2512	716.6	2532	716.9	2552	718.4
2562	719.2	2572	720.5	2577	720.7	2582	721.4	2585	722.2
2587	724	2622	723.7	2633	723.7	2665	723.5	2683	723.2
2708	723.2	2736	723.5	2776	724.1	2808	724.8	2850	725.7
2892	726.5	2934	727.9	2965	729.5	3000	731.5	3030	732.8
3044	733								

Manning's n Values num= 3  
 Sta n Val Sta n Val Sta n Val  
 2000 .1 2386 .0506 2587 .1

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 2386 2587 2454 2440 2400 .1 .3

## HEC-RAS Plan: Mod Ex Nat River: Fox River Reach: Algonquin

Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Algonquin	428.504	10 yr	5775.00	717.20	729.48		729.63	0.000638	3.29	2764.66	811.07	0.19
Algonquin	428.504	50 yr	8345.00	717.20	731.11		731.25	0.000589	3.52	4305.54	1007.57	0.19
Algonquin	428.504	100 yr	10095.00	717.20	732.02		732.17	0.000564	3.63	5383.40	1288.44	0.19
Algonquin	428.504	500 yr	12525.00	717.20	733.13		733.27	0.000529	3.74	6919.99	1432.03	0.18
Algonquin	426.207	10 yr	5775.00	716.80	728.30		728.41	0.000446	2.75	2670.30	532.28	0.16
Algonquin	426.207	50 yr	8345.00	716.80	729.91		730.05	0.000484	3.18	3712.67	750.95	0.17
Algonquin	426.207	100 yr	10095.00	716.80	730.82		730.97	0.000499	3.41	4417.33	794.25	0.18
Algonquin	426.207	500 yr	12525.00	716.80	731.93		732.10	0.000515	3.68	5329.20	854.46	0.18
Algonquin	424.8	10 yr	5775.00	715.90	727.77		727.84	0.000359	2.33	3858.08	824.55	0.14
Algonquin	424.8	50 yr	8345.00	715.90	729.35		729.43	0.000371	2.60	5176.69	837.67	0.14
Algonquin	424.8	100 yr	10095.00	715.90	730.25		730.34	0.000378	2.77	5933.84	844.60	0.14
Algonquin	424.8	500 yr	12525.00	715.90	731.35		731.45	0.000391	2.99	6684.19	853.57	0.15
Algonquin	424.6	10 yr	5775.00	714.00	727.71		727.74	0.000114	1.52	4372.44	733.44	0.08
Algonquin	424.6	50 yr	8345.00	714.00	729.27		729.32	0.000144	1.86	5547.92	762.54	0.09
Algonquin	424.6	100 yr	10095.00	714.00	730.16		730.22	0.000161	2.06	6231.29	772.67	0.10
Algonquin	424.6	500 yr	12525.00	714.00	731.25		731.32	0.000184	2.31	7074.87	785.01	0.11
Algonquin	424.528	10 yr	5775.00	716.60	727.66		727.73	0.000272	2.10	3250.66	782.36	0.12
Algonquin	424.528	50 yr	8345.00	716.60	729.22		729.31	0.000299	2.45	4580.96	888.50	0.13
Algonquin	424.528	100 yr	10095.00	716.60	730.11		730.21	0.000313	2.64	5386.25	924.34	0.14
Algonquin	424.528	500 yr	12525.00	716.60	731.19		731.30	0.000328	2.88	6425.82	983.89	0.14
Algonquin	423.9	10 yr	5775.00	715.40	727.53		727.61	0.000389	2.36	2864.94	623.18	0.14
Algonquin	423.9	50 yr	8345.00	715.40	729.07		729.18	0.000425	2.75	3940.32	725.39	0.15
Algonquin	423.9	100 yr	10095.00	715.40	729.95		730.08	0.000444	2.97	4584.19	739.47	0.16
Algonquin	423.9	500 yr	12525.00	715.40	731.02		731.16	0.000469	3.25	5384.36	756.60	0.16
Algonquin	423.8	10 yr	5775.00	715.00	727.49		727.57	0.000478	2.35	2862.84	671.11	0.15
Algonquin	423.8	50 yr	8345.00	715.00	729.03		729.14	0.000476	2.66	4026.77	775.63	0.16
Algonquin	423.8	100 yr	10095.00	715.00	729.91		730.03	0.000478	2.84	4740.65	902.38	0.16
Algonquin	423.8	500 yr	12525.00	715.00	730.99		731.11	0.000482	3.06	5718.89	920.02	0.16
Algonquin	423.7	10 yr	5775.00	714.90	727.45		727.53	0.000352	2.31	3132.77	630.79	0.13
Algonquin	423.7	50 yr	8345.00	714.90	728.99		729.09	0.000387	2.69	4266.73	768.32	0.15
Algonquin	423.7	100 yr	10095.00	714.90	729.87		729.98	0.000406	2.90	4944.42	775.84	0.15
Algonquin	423.7	500 yr	12525.00	714.90	730.94		731.07	0.000430	3.17	5776.79	784.98	0.16

## HEC-RAS Plan: Mod Ex Nat River: Fox River Reach: Algonquin (Continued)

Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Algonquin	423.3	10 yr	5775.00	714.10	727.28	727.37	0.000383	2.53	2853.89	570.64	0.14	
Algonquin	423.3	50 yr	8345.00	714.10	728.79	728.92	0.000444	3.00	3752.99	618.77	0.16	
Algonquin	423.3	100 yr	10095.00	714.10	729.65	729.80	0.000478	3.27	4296.38	646.11	0.17	
Algonquin	423.3	500 yr	12525.00	714.10	730.69	730.87	0.000522	3.61	4987.51	679.64	0.18	
Algonquin	422.6	10 yr	5775.00	716.20	727.02	727.11	0.000407	2.49	3056.90	664.44	0.15	
Algonquin	422.6	50 yr	8345.00	716.20	728.51	728.62	0.000445	2.89	4116.26	761.05	0.16	
Algonquin	422.6	100 yr	10095.00	716.20	729.35	729.48	0.000466	3.11	4779.42	809.30	0.16	
Algonquin	422.6	500 yr	12525.00	716.20	730.38	730.52	0.000490	3.39	5633.65	845.57	0.17	
Algonquin	422.521	10 yr	5775.00	714.30	726.91	727.07	0.000648	3.36	2498.92	702.21	0.19	
Algonquin	422.521	50 yr	8345.00	714.30	728.39	728.57	0.000688	3.81	3595.58	779.82	0.20	
Algonquin	422.521	100 yr	10095.00	714.30	729.23	729.43	0.000704	4.05	4264.41	806.45	0.21	
Algonquin	422.521	500 yr	12525.00	714.30	730.26	730.47	0.000725	4.34	5107.75	837.77	0.21	
Algonquin	420.346	10 yr	5775.00	715.60	726.16	726.29	0.000655	3.07	2657.55	714.55	0.19	
Algonquin	420.346	50 yr	8345.00	715.60	727.55	727.71	0.000687	3.49	3698.36	772.46	0.20	
Algonquin	420.346	100 yr	10095.00	715.60	728.35	728.52	0.000704	3.72	4326.32	796.45	0.20	
Algonquin	420.346	500 yr	12525.00	715.60	729.32	729.51	0.000727	4.01	5114.60	836.64	0.21	
Algonquin	417.841	10 yr	5775.00	714.30	725.15	725.22	0.000304	2.13	3030.74	633.28	0.13	
Algonquin	417.841	50 yr	8345.00	714.30	726.37	726.47	0.000379	2.60	3903.71	806.83	0.15	
Algonquin	417.841	100 yr	10095.00	714.30	727.09	727.21	0.000416	2.86	4539.05	999.57	0.16	
Algonquin	417.841	500 yr	12525.00	714.30	727.97	728.11	0.000455	3.16	5495.27	1143.89	0.17	
Algonquin	414.015	10 yr	5775.00	711.80	723.86	723.91	0.000325	1.73	3366.13	649.91	0.13	
Algonquin	414.015	50 yr	8345.00	711.80	724.72	724.79	0.000430	2.17	3949.06	711.01	0.15	
Algonquin	414.015	100 yr	10095.00	711.80	725.25	725.34	0.000488	2.43	4329.53	728.54	0.16	
Algonquin	414.015	500 yr	12525.00	711.80	725.92	719.73	0.000555	2.74	4834.96	813.36	0.17	

CROSS SECTION

RIVER: Fox River  
REACH: Algonquin RS: 424.528

INPUT

Description: 2002 FIS DE, 1979 XS 80.352, shifted 4850' rt.

Station	Elevation	Data	num=	68
6850	734	6868	733.1	6891
6949	730.1	6972	728.5	6998
7077	726.8	7093	726.2	7097
7123	731.2	7128	731.2	7142
7191	727.1	7203	727.6	7213
7267	727.6	7285	727.4	7305
7346	726.6	7355	726.5	7363
7418	726.6	7440	727.3	7441
7514	721.5	7519	720.5	7524
7574	716.7	7594	716.6	7614
7674	717.9	7694	718	7714
7774	720.2	7779	720.8	7784
7814	725.7	7825	726.3	7845
7896	730.3	7905	731.4	7916
				734

Manning's n Values

Sta	n Val	Sta	n Val	Sta	n Val
6850	.102	7504	.0507	7794	.102

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	7504	7794		355	345	338		.1	.3

CROSS SECTION

RIVER: Fox River  
REACH: Algonquin RS: 423.9

INPUT

Description: 1885 EEI SURVEYED SECTION 18+85.95

Station	Elevation	Data	num=	26
7000	731.8	7017	725.8	7085
7213	725.8	7257	725.8	7275
7366	725.8	7395	725.8	7407
7446	720.5	7462	718.1	7515
7614	718.2	7639	720.5	7661
7773	732.1			722
				7671
				724.6
				7686
				725.5

Manning's n Values

Sta	n Val	Sta	n Val	Sta	n Val
7000	.101	7416	.0525	7686	.101

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	7416	7686		100	100	100		.1	.3

CROSS SECTION

RIVER: Fox River  
REACH: Algonquin RS: 423.8

INPUT

Description: 2000 EEI PROPOSED BOLZ ROAD CENTERLINE SURVEYED SECTION 20+00

Station	Elevation	Data	num=	30
6997	731.8	7000	730.8	7014
7133	729.8	7145	725.8	7200
7364	725.8	7401	725.4	7437
7501	726.2	7521	726.7	7527
7616	719	7655	717.1	7682
7800	719.7	7824	724.4	7837
				725.2
				7899
				726.3
				7923
				731.8

CROSS SECTION

RIVER: Fox River  
REACH: Algonquin RS: 426.207

INPUT

Description: 1979 XS 80.721

Station	Elevation	Data	num=	53
2000	734	2021	733.6	2056
2190	733.1	2230	732.6	2253
2328	730.1	2349	729.5	2371
2479	727.1	2505	727.1	2527
2620	724.7	2623	723.8	2637
2657	719.8	2677	718.2	2697
2757	718.6	2777	719.2	2797
2827	722	2829	723	2837
2879	723.5	2894	723.8	2907
2985	728.8	3018	729.2	3043
3109	731.2	3120	732.6	3130

Manning's n Values	Sta	n Val	Sta	n Val	Sta	n Val	num=	3
	2000	.1	2620	.05	2829	.1		

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	2620	2829		1430	1404	1416		.1	.3

CROSS SECTION

RIVER: Fox River  
REACH: Algonquin RS: 424.8

INPUT

Description: 912 EEI SURVEYED SECTION 9+12.64

Station	Elevation	Data	num=	18
7000	731.8	7010	729.8	7018
7292	724.7	7392	724.3	7492
7665	718.6	7743	715.9	7772
7811	720.5	7848	728.9	7863

Manning's n Values	Sta	n Val	Sta	n Val	Sta	n Val	num=	3
	7000	.102	7624	.0525	7848	.102		

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	7624	7848		470	470	470		.1	.3

CROSS SECTION

RIVER: Fox River  
REACH: Algonquin RS: 424.6

INPUT

Description: 1402 EEI SURVEYED SECTION 14+02.01

Station	Elevation	Data	num=	19
7000	731.8	7012	727.8	7067
7169	725.8	7204	725.8	7304
7404	720.4	7457	714	7556
7693	721.6	7702	724.8	7767

Manning's n Values	Sta	n Val	Sta	n Val	Sta	n Val	num=	3
	7000	.102	7375	.0525	7702	.102		

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	7375	7702		60	60	60		.1	.3

SUMMARY OF MANNING'S N VALUES

River: Fox River

Reach	River Sta.	n1	n2	n3
Algonquin	428.504	.1	.05	.1
Algonquin	426.207	.1	.05	.1
Algonquin	424.8	.102	.0525	.102
Algonquin	424.6	.102	.0525	.102
Algonquin	424.528	.102	.0507	.102
Algonquin	423.9	.101	.0525	.101
Algonquin	423.8	.101	.0525	.101
Algonquin	423.7	.101	.0525	.101
Algonquin	423.3	.101	.052	.101
Algonquin	422.6	.1	.051	.1
Algonquin	422.521	.1	.05	.1
Algonquin	420.346	.1	.0506	.1
Algonquin	417.841	.1	.05	.1
Algonquin	414.015	.1	.0501	.1

SUMMARY OF REACH LENGTHS

River: Fox River

Reach	River Sta.	Left	Channel	Right
Algonquin	428.504	2094	2294	2134
Algonquin	426.207	1430	1404	1416
Algonquin	424.8	470	470	470
Algonquin	424.6	60	60	60
Algonquin	424.528	355	345	338
Algonquin	423.9	100	100	100
Algonquin	423.8	100	100	100
Algonquin	423.7	426	414	410
Algonquin	423.3	690	665	650
Algonquin	422.6	75	70	68
Algonquin	422.521	2280	1106	1654
Algonquin	420.346	2454	2440	2400
Algonquin	417.841	3520	4160	3746
Algonquin	414.015	0	0	0

SUMMARY OF CONTRACTION AND EXPANSION COEFFICIENTS

River: Fox River

Reach	River Sta.	Contr.	Expan.
Algonquin	428.504	.1	.3
Algonquin	426.207	.1	.3
Algonquin	424.8	.1	.3
Algonquin	424.6	.1	.3
Algonquin	424.528	.1	.3
Algonquin	423.9	.1	.3
Algonquin	423.8	.1	.3
Algonquin	423.7	.1	.3
Algonquin	423.3	.1	.3
Algonquin	422.6	.1	.3
Algonquin	422.521	.1	.3
Algonquin	420.346	.1	.3
Algonquin	417.841	.1	.3
Algonquin	414.015	.1	.3

## CROSS SECTION

RIVER: Fox River  
REACH: Algonquin

RS: 417.841

## INPUT

Description: 2002 FIS DB, 1979 XS 79.137

Station	Elevation	Data	num=	63
2000	733	2014	731.7	2041
2153	727.5	2189	726.2	2216
2282	730.1	2286	730.3	2293
2342	727.5	2367	726.7	2397
2445	727.1	2467	727.3	2492
2593	724.6	2635	723.3	2684
2690	720.8	2695	720.4	2705
2765	715.2	2785	714.4	2805
2865	715.8	2885	716.2	2905
2965	718.4	2985	720.4	2992
3030	723.4	3047	723.3	3072
3160	725	3188	725.3	3238
3325	729.6	3360	731.8	3375
				733

Manning's n Values

num= 3

Sta	n Val	Sta	n Val	Sta	n Val
2000	.1	2684	.05	3000	.1

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	2684	3000		3520	4160	3746		.1	.3

## CROSS SECTION

RIVER: Fox River  
REACH: Algonquin

RS: 414.015

## INPUT

Description: 2002 FIS DA, 1979 XS 78.412

Station	Elevation	Data	num=	66
2000	736.8	2022	737.3	2048
2097	720.8	2102	720.3	2112
2172	714.8	2192	715.1	2212
2272	718.4	2292	719.3	2312
2372	720.3	2392	720.3	2412
2472	717.7	2492	714.3	2512
2572	718.4	2592	719.6	2612
2632	721.4	2652	723.3	2682
2751	724.2	2780	724.7	2784
2848	725.8	2886	725.9	2923
3017	728.3	3048	728.6	3060
3086	726.9	3095	729.1	3107
3178	731.1	3204	731	3229
3283	733			731.6
				3255
				732.2
				3266
				732.4

Manning's n Values

num= 3

Sta	n Val	Sta	n Val	Sta	n Val
2000	.1	2022	.0501	2652	.1

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	2022	2652		0	0	0		.1	.3

## **PROPOSED CONDITIONS MODEL**

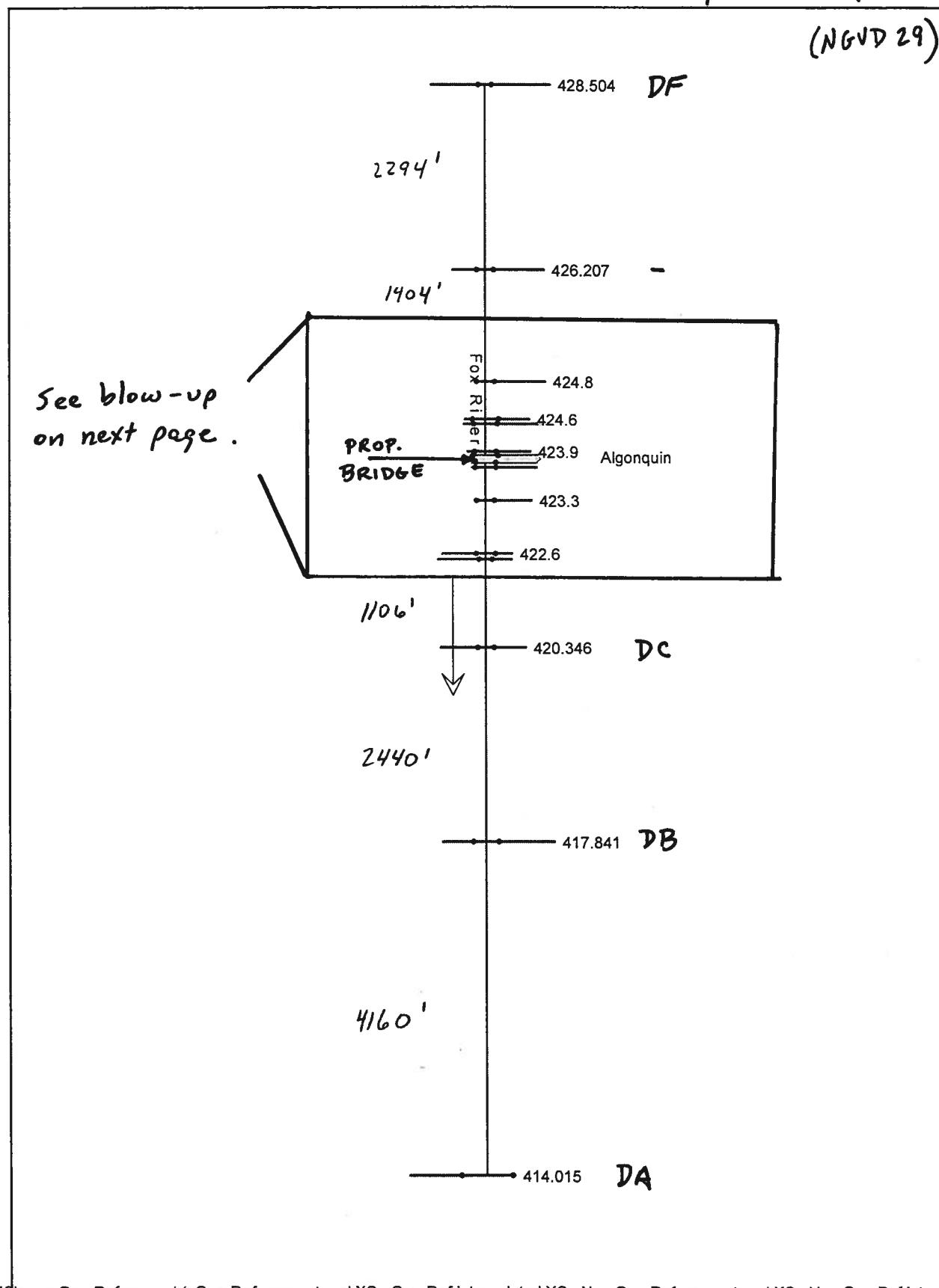
---

Christopher B. Burke Engineering, Ltd.

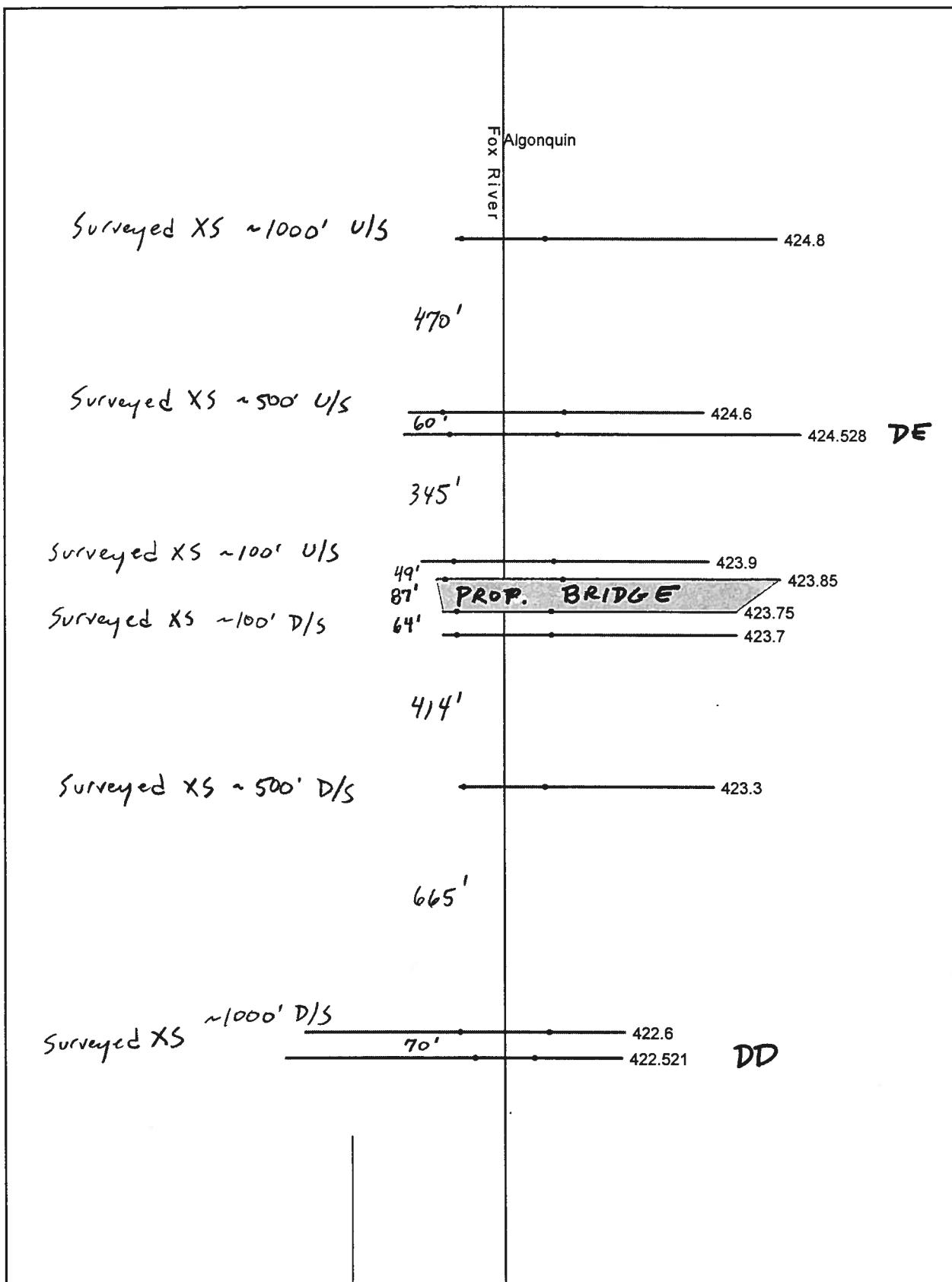
Hydraulic Report  
Longmeadow Parkway over the  
Fox River

Prop. Model

(NGVD 29)



# PROP. MODEL



None of the XS's are Geo-Referenced (~ Geo-Ref user entered XS - Geo-Ref interpolated XS - Non Geo-Ref user entered XS - Non Geo-Ref interpolated XS)

*Prop. Model*  
*(NGVD 29)*

HEC-RAS Version 4.1.0 Jan 2010  
U.S. Army Corps of Engineers  
Hydrologic Engineering Center  
609 Second Street  
Davis, California

X	X	XXXXXX	XXXX	XXXX	XX	XXXX
X	X	X	X X	X X	X X	X
X	X	X	X	X X	X X	X
XXXXXX	XXXX	X	XXX	XXXX	XXXXXX	XXXX
X	X	X	X	X X	X X	X
X	X	X	X X	X X	X X	X
X	X	XXXXXX	XXXX	X X	X X	XXXXX

PROJECT DATA

Project Title: Longmeadow Parkway Fox River  
Project File : LongmeadowParkway.prj  
Run Date and Time: 1/26/2011 3:17:30 PM

Project in English units

Project Description:

Conversion of HEC-2 for analysis of proposed Longmeadow Parkway bridge over Fox River. CBBEL 2011 PROPOSED BOLZ ROAD HYDRAULIC MODEL

PROPOSED MODEL

WAT10(FRE1003)  
FOX RIVER -  
EAST DUNDEE  
1 YR FLOOD

PLAN DATA

Plan Title: Proposed  
Plan File : n:\kanecounty\99236\Drain\Model\HEC-RAS\LongmeadowParkway.p01

Geometry Title: Proposed Geometry  
Geometry File : n:\kanecounty\99236\Drain\Model\HEC-RAS\LongmeadowParkway.g04

Flow Title : FIS Regulatory Flows and Starting WSELS  
Flow File : n:\kanecounty\99236\Drain\Model\HEC-RAS\LongmeadowParkway.f02

Plan Description:

Proposed bridge from 2005 HR. On EEI surveyed XS inserted into HEC-2 model circa 1979, hardcopy received from ISWS Feb. 2004. Truncated to include all sections from DA (78.151) to DF (81.156).

Plan Summary Information:

Number of:	Cross Sections	=	15	Multiple Openings	=	0
	Culverts	=	0	Inline Structures	=	0
	Bridges	=	1	Lateral Structures	=	0

Computational Information

Water surface calculation tolerance	=	0.01
Critical depth calculation tolerance	=	0.01
Maximum number of iterations	=	20
Maximum difference tolerance	=	0.3
Flow tolerance factor	=	0.001

Computation Options

Critical depth computed only where necessary
Conveyance Calculation Method: Between every coordinate point (HEC2 Style)
Friction Slope Method: Average Conveyance
Computational Flow Regime: Subcritical Flow

## FLOW DATA

Flow Title: FIS Regulatory Flows and Starting WSELS  
Flow File : n:\kanecounty\99236\Drain\Model\HEC-RAS\LongmeadowParkway.f02

Flow Data (cfs)

River	Reach	RS	10 yr	50 yr	100 yr	500 yr
Fox River	Algonquin	428.504	5775	8345	10095	12525

## Boundary Conditions

River	Reach	Profile	Upstream	Downstream
Fox River	Algonquin	10 yr		Known WS = 723.86
Fox River	Algonquin	50 yr		Known WS = 724.72
Fox River	Algonquin	100 yr		Known WS = 725.25
Fox River	Algonquin	500 yr		Known WS = 725.92

## GEOMETRY DATA

Geometry Title: Proposed Geometry  
Geometry File : n:\kanecounty\99236\Drain\Model\HEC-RAS\LongmeadowParkway.g04

### CROSS SECTION

RIVER: Fox River  
REACH: Algonquin

RS: 428.504

#### INPUT

Description: 2002 FIS DF, 1979 XS 81.156.

Station	Elevation	Data	num=	67
2000	734	2012	Elev	733.4
2147	732.1	2183	Elev	731.5
2259	732.1	2265	Elev	732.1
2347	731.2	2371	Elev	731.2
2450	731	2487	Elev	729.8
2553	729.9	2563	Elev	729.9
2626	729.2	2651	Elev	728.8
2750	722	2755	Elev	721.3
2810	717.2	2830	Elev	717.2
2872	722	2883	Elev	726.2
3003	727.4	3045	Elev	727.4
3203	727.1	3239	Elev	727.1
3365	727.7	3390	Elev	728.4
3461	733.6	3466	Elev	734
2076	732.5	2037	Elev	732.9
2244	731.2	2183	Elev	731.5
2300	731.5	2280	Elev	731.4
2405	731.7	2371	Elev	731.2
2527	729.5	2487	Elev	729.5
2582	729.6	2563	Elev	729.9
2720	726.9	2651	Elev	728.8
2770	719.5	2755	Elev	721.3
2860	718.8	2830	Elev	717.2
2942	726.6	2883	Elev	726.2
3137	727.3	3045	Elev	727.4
3307	727.3	3239	Elev	727.1
3423	729.6	3390	Elev	728.4
2110	732.1	2094	Elev	732.9
2247	731.2	2294	Elev	731.5
2331	731.2	2134	Elev	731.4
2429	731.7		Elev	
2548	729.6		Elev	
2602	729.6		Elev	
2743	724.1		Elev	
2790	717.9		Elev	
2865	720.5		Elev	
2976	727		Elev	
3176	727.3		Elev	
3334	727.3		Elev	
3443	731.2		Elev	

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
2000	.1	2720	.05	2883	.1

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
2720 2883 2094 2294 2134 .1 .3

## CROSS SECTION

RIVER: Fox River  
REACH: Algonquin

RS: 426.207

## INPUT

Description: 1979 XS 80.721

Station	Elevation	Data	num=	53	Sta	Elev	Sta	Elev	Sta	Elev
2000	734	2021	733.6	2056	733.6	2099	733.4	2151	733.1	
2190	733.1	2230	732.6	2253	732	2274	731.8	2298	731.2	
2328	730.1	2349	729.5	2371	728.4	2410	727.3	2441	727.1	
2479	727.1	2505	727.1	2527	726	2554	725.2	2587	725.5	
2620	724.7	2623	723.8	2637	721.6	2642	721	2647	720.5	
2657	719.8	2677	718.2	2697	717	2717	716.8	2757	717.2	
2757	718.6	2777	719.2	2797	719.8	2817	720.6	2824	721.6	
2827	722	2829	723	2837	724.4	2846	724.9	2867	724.6	
2879	723.5	2894	723.8	2907	728.4	2925	729.2	2953	728.7	
2985	728.8	3018	729.2	3043	729	3077	729.5	3094	730.3	
3109	731.2	3120	732.6	3130	734					

Manning's n Values

num=

3

Sta	n Val	Sta	n Val	Sta	n Val
2000	.1	2620	.05	2829	.1

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	2620	2829		1430	1404	1416	.1		.3

## CROSS SECTION

RIVER: Fox River  
REACH: Algonquin

RS: 424.8

## INPUT

Description: 912 EEI SURVEYED SECTION 9+12.64

Station	Elevation	Data	num=	18	Sta	Elev	Sta	Elev	Sta	Elev
7000	731.8	7010	729.8	7018	727.8	7048	725.8	7192	725.2	
7292	724.7	7392	724.3	7492	723.8	7624	723.8	7632	720.7	
7665	718.6	7743	715.9	7772	716.2	7799	717.3	7806	718.3	
7811	720.5	7848	728.9	7863	733.6					

Manning's n Values

num=

3

Sta	n Val	Sta	n Val	Sta	n Val
7000	.102	7624	.0525	7848	.102

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	7624	7848		470	470	470	.1		.3

## CROSS SECTION

RIVER: Fox River  
REACH: Algonquin

RS: 424.6

## INPUT

Description: 1402 EEI SURVEYED SECTION 14+02.01

Station	Elevation	Data	num=	19	Sta	Elev	Sta	Elev	Sta	Elev
7000	731.8	7012	727.8	7067	725.8	7099	725.8	7139	726	
7169	725.8	7204	725.8	7304	726.2	7375	725.8	7384	723.8	
7404	720.4	7457	714	7556	714.4	7635	714.7	7652	715.2	
7693	721.6	7702	724.8	7767	728.9	7793	732			

Manning's n Values

num=

3

Sta	n Val	Sta	n Val	Sta	n Val
7000	.102	7375	.0525	7702	.102

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	7375	7702		60	60	60	.1		.3

CROSS SECTION

RIVER: Fox River  
REACH: Algonquin

RS: 424.528

INPUT

Description: 2002 FIS DE, 1979 XS 80.352, shifted 4850' rt.

Station	Elevation								
6850	734	6868	733.1	6891	732.1	6906	731.4	6927	730.4
6949	730.1	6972	728.5	6998	727.3	7032	727	7059	726.8
7077	726.8	7093	726.2	7097	726	7102	726	7119	731.2
7123	731.2	7128	731.2	7142	725.7	7154	725.5	7171	726.5
7191	727.1	7203	727.6	7213	731.2	7220	727.9	7248	727.7
7267	727.6	7285	727.4	7305	726.8	7322	726.5	7336	726.5
7346	726.6	7355	726.5	7363	724.7	7375	726.3	7397	726.3
7418	726.6	7440	727.3	7441	727.3	7462	724.7	7504	724.6
7514	721.5	7519	720.5	7524	719.3	7534	718.8	7554	717.8
7574	716.7	7594	716.6	7614	717.5	7634	718.1	7654	718
7674	717.9	7694	718	7714	718.1	7734	718.4	7754	718.9
7774	720.2	7779	720.8	7784	721.5	7789	722.7	7794	724.9
7814	725.7	7825	726.3	7845	727.1	7862	728.1	7880	729.2
7896	730.3	7905	731.4	7916	734				

Manning's n Values

Station	n Val	Station	n Val	Station	n Val
6850	.102	7504	.0507	7794	.102

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	7504	7794		355	345	338		.1	.3

CROSS SECTION

RIVER: Fox River  
REACH: Algonquin

RS: 423.9

INPUT

Description: 1885 EEI SURVEYED SECTION 18+85.95

Station	Elevation								
7000	731.8	7017	725.8	7085	725.8	7110	727.8	7160	727.8
7213	725.8	7257	725.8	7275	728.8	7297	725.8	7338	726
7366	725.8	7395	725.8	7407	725.9	7416	725.8	7425	723.8
7446	720.5	7462	718.1	7515	715.4	7550	715.5	7584	717.1
7614	718.2	7639	720.5	7661	722	7671	724.6	7686	725.5
7773	732.1								

Manning's n Values

Station	n Val	Station	n Val	Station	n Val
7000	.101	7416	.0525	7686	.101

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	7416	7686		79	49	34		.1	.3

CROSS SECTION

RIVER: Fox River  
REACH: Algonquin

RS: 423.85

INPUT

Description: Approach section, 1' U/S of bridge face. Copy of surveyed section 2000. This is a REPEATED section.

Station	Elevation								
6997	731.8	7000	730.8	7014	729.8	7018	729.8	7020	729.8
7055	729.5	7109	729.8	7133	729.8	7140	727.5	7142	726.8
7145	725.8	7200	725.8	7219	727.8	7260	727.8	7262	727.8
7299	727.8	7364	725.8	7380	725.6	7382	725.6	7401	725.4
7437	725.8	7452	725.8	7498	726.6	7501	726.2	7521	726.7

Manning's n	Values	num=	3					
Sta	n	Val	Sta	n	Val	Sta	n	Val
6997	.101	7582	.0525	7899	.101			

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.

BRIDGE

RIVER: Fox River  
REACH: Algonquin RS: 423.8

## INPUT

Description: Proposed Longmeadow Parkway

Distance from Upstream XS = 1

Deck/Roadway Width = 85

Weir Coefficient = 2.0

### stream Deck/R

### Upstream Bridge Cross Section Data

Station Elevation Data num= 41

Manning's n	Values	num=	3					
Sta	n	Val	Sta	n	Val	Sta	n	Val
6997	.101	7582	.0525	7899	.101			

Bank Sta: Left      Right      Coeff Contr.      Expan.

#### Downstream Deck/Roadway Coordinates

num= 13

### Downstream Bridge Cross Section Data

Downstream Bridge Cross Section Data

Manning's n Values num= 3  
Sta n Val Sta n Val Sta n Val  
7000 .101 7499 .0525 7754 .101

Bank Sta: Left Right Coeff Contr. Expan.  
7499 7754 .3 .5

Upstream Embankment side slope = 0 horiz. to 1.0 vertical  
Downstream Embankment side slope = 0 horiz. to 1.0 vertical  
Maximum allowable submergence for weir flow = .98  
Elevation at which weir flow begins =  
Energy head used in spillway design =  
Spillway height used in design =  
Weir crest shape = Broad Crested

Number of Piers = 7

Pier Data  
Pier Station Upstream= 7021 Downstream= 8098  
Upstream num= 2  
Width Elev Width Elev  
4 729 4 768.28  
Downstream num= 2  
Width Elev Width Elev  
4 729 4 768.28

Pier Data  
Pier Station Upstream= 7141 Downstream= 7038  
Upstream num= 2  
Width Elev Width Elev  
4 725 4 768.28  
Downstream num= 2  
Width Elev Width Elev  
4 725 4 768.28

Pier Data  
Pier Station Upstream= 7261 Downstream= 7158  
Upstream num= 2  
Width Elev Width Elev  
4 727 4 768.28  
Downstream num= 2  
Width Elev Width Elev  
4 727 4 768.28

Pier Data  
Pier Station Upstream= 7381 Downstream= 7278  
Upstream num= 2  
Width Elev Width Elev  
4 724 4 768.28  
Downstream num= 2  
Width Elev Width Elev  
4 724 4 768.28

Pier Data  
Pier Station Upstream= 7581 Downstream= 7478  
Upstream num= 2  
Width Elev Width Elev  
4 724 4 768.28  
Downstream num= 2  
Width Elev Width Elev  
4 724 4 768.28

Pier Data  
Pier Station Upstream= 7881 Downstream= 7778  
Upstream num= 2  
Width Elev Width Elev  
4 725 4 768.28  
Downstream num= 2  
Width Elev Width Elev  
4 725 4 768.28

Pier Data  
Pier Station Upstream= 8081 Downstream= 7978  
Upstream num= 2  
Width Elev Width Elev  
4 750 4 768.28  
Downstream num= 2  
Width Elev Width Elev  
4 750 4 768.28

Number of Bridge Coefficient Sets = 1

Low Flow Methods and Data

Energy  
Yarnell KVal = 1.25  
Selected Low Flow Methods = Highest Energy Answer

High Flow Method  
Energy Only

Additional Bridge Parameters

Add Friction component to Momentum  
Do not add Weight component to Momentum  
Class B flow critical depth computations use critical depth  
inside the bridge at the upstream end  
Criteria to check for pressure flow = Upstream energy grade line

CROSS SECTION

RIVER: Fox River  
REACH: Algonquin RS: 423.75

INPUT

Description: Exit section, 1' D/S of bridge face. Copy of surveyed section  
2098. This is a REPEATED section.

Station	Elevation	Data num= 31	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
7000	731.8		7016	727.8	7032	724.8	7057	725.4	7059	725.4
7075	725.8		7087	727.8	7177	727.8	7179	727.8	7196	727.8
7269	725.8		7297	724.3	7298	724.2	7299	724.2	7341	725.4
7352	724.7		7382	724.2	7422	725.2	7471	724.9	7497	723.9
7499	723.8		7504	721.3	7535	718.9	7579	715.8	7608	714.9
7671	717.7		7689	718.3	7737	722.3	7754	725.3	7770	726.9
7791	731.5									

Manning's n Values num= 3	Sta	n Val	Sta	n Val	Sta	n Val
	7000	.101	7499	.0525	7754	.101

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff Contr.	Expan.
	7499	7754		34	64	79	.3	.5

CROSS SECTION

RIVER: Fox River  
REACH: Algonquin RS: 423.7

INPUT

Description: 2098 EEI SURVEYED SECTION 20+98.75

Station	Elevation	Data num= 24	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
7000	731.8		7016	727.8	7032	724.8	7075	725.8	7087	727.8
7196	727.8		7269	725.8	7298	724.2	7341	725.4	7352	724.7
7382	724.2		7422	725.2	7471	724.9	7499	723.8	7504	721.3
7535	718.9		7579	715.8	7608	714.9	7671	717.7	7689	718.3
7737	722.3		7754	725.3	7770	726.9	7791	731.5		

Manning's n Values num= 3	Sta	n Val	Sta	n Val	Sta	n Val
	7000	.101	7499	.0525	7754	.101

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 7499 7754 426 414 410 .1 .3

CROSS SECTION

RIVER: Fox River  
 REACH: Algonquin RS: 423.3

INPUT

Description: 2453 EEI SURVEYED SECTION 24+53.16

Station Elevation Data num= 17

Sta	Elev								
7000	730.8	7124	726.6	7191	724.7	7292	725.2	7324	724.5
7412	725.7	7453	724.7	7464	721.2	7475	718.9	7533	716.7
7547	715.1	7583	714.1	7634	717.3	7667	722.3	7673	726.6
7682	730.5	7684	731						

Manning's n Values

Sta	n Val	Sta	n Val	Sta	n Val
7000	.101	7453	.052	7673	.101

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 7453 7673 690 665 650 .1 .3

CROSS SECTION

RIVER: Fox River  
 REACH: Algonquin RS: 422.6

INPUT

Description: 3073 EEI SURVEYED SECTION 30+73.04

ISWS REGULATORY MODEL PRIOR TO

THIS POINT

Station Elevation Data num= 21

Sta	Elev								
6990	731	7000	729.8	7031	728.2	7134	724.7	7178	723.9
7192	723.7	7213	717.1	7237	716.2	7276	717	7313	717.1
7343	717.6	7374	719.4	7420	720.5	7427	721.4	7432	724
7491	724.1	7542	723.1	7551	723.8	7684	725.8	7835	729.8
7847	731								

Manning's n Values

Sta	n Val	Sta	n Val	Sta	n Val
6990	.1	7192	.051	7432	.1

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 7192 7432 75 70 68 .1 .3

CROSS SECTION

RIVER: Fox River  
 REACH: Algonquin RS: 422.521

INPUT

Description: 2002 FIS DD, 1979 XS 80.023 shifted 5000' rt.

Station Elevation Data num= 50

Sta	Elev								
7000	734	7009	732.4	7025	727.5	7027	727.5	7040	727.9
7050	726.9	7068	726.9	7086	726.4	7112	726.6	7131	726.1
7150	725.8	7166	725.3	7182	725.7	7191	724.6	7216	725.5
7223	727	7234	725.1	7241	721.5	7246	720.1	7251	718.5
7261	715.8	7281	714.9	7301	714.6	7321	714.3	7341	716.7
7361	719.1	7376	720.4	7381	720.9	7388	721.5	7392	722.7
7394	724.9	7441	724.4	7459	724.6	7483	724.2	7509	724.4
7533	723.6	7556	723	7590	723.9	7616	724.4	7657	725
7693	725.8	7739	726.4	7761	727.2	7791	728	7822	729.1
7851	730.1	7867	731	7882	732.4	7898	733.7	7902	734

Manning's n Values num= 3  
 Sta n Val Sta n Val Sta n Val  
 7000 .1 7234 .05 7394 .1

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 7234 7394 2280 1106 1654 .1 .3

CROSS SECTION

RIVER: Fox River  
 REACH: Algonquin RS: 420.346

INPUT  
 Description: 2002 FIS DC, 1979 XS 79.611  
 Station Elevation Data num= 51  

Sta	Elev								
2000	733	2007	729	2021	729.2	2043	732	2068	732.8
2085	733.1	2111	732.8	2129	731.2	2143	728.9	2161	725.9
2176	724.9	2192	725.2	2213	725.2	2236	724.6	2263	724.4
2295	724	2320	724.1	2352	723.8	2386	723.3	2392	721.4
2397	720.5	2402	720	2412	719.1	2432	717.4	2452	716.1
2472	715.6	2492	715.7	2512	716.6	2532	716.9	2552	718.4
2562	719.2	2572	720.5	2577	720.7	2582	721.4	2585	722.2
2587	724	2622	723.7	2633	723.7	2665	723.5	2683	723.2
2708	723.2	2736	723.5	2776	724.1	2808	724.8	2850	725.7
2892	726.5	2934	727.9	2965	729.5	3000	731.5	3030	732.8
3044	733								

Manning's n Values num= 3  
 Sta n Val Sta n Val Sta n Val  
 2000 .1 2386 .0506 2587 .1

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 2386 2587 2454 2440 2400 .1 .3

CROSS SECTION

RIVER: Fox River  
 REACH: Algonquin RS: 417.841

INPUT  
 Description: 2002 FIS DB, 1979 XS 79.137  
 Station Elevation Data num= 63  

Sta	Elev								
2000	733	2014	731.7	2041	729.5	2079	728.7	2109	727.9
2153	727.5	2189	726.2	2216	726.5	2240	724.9	2265	726.7
2282	730.1	2286	730.3	2293	730	2302	728.9	2311	725.1
2342	727.5	2367	726.7	2397	727	2423	727	2434	726.8
2445	727.1	2467	727.3	2492	727	2525	726	2556	724.9
2593	724.6	2635	723.3	2684	722.4	2685	721.4	2685	721.1
2690	720.8	2695	720.4	2705	719.7	2725	718.4	2745	716.4
2765	715.2	2785	714.4	2805	714.3	2825	714.6	2845	715.4
2865	715.8	2885	716.2	2905	716.4	2925	716.2	2945	716.1
2965	718.4	2985	720.4	2992	720.7	2999	721.4	3000	722.7
3030	723.4	3047	723.3	3072	723.7	3102	724.4	3129	724.4
3160	725	3188	725.3	3238	726.4	3274	727.5	3293	728.5
3325	729.6	3360	731.8	3375	733				

Manning's n Values num= 3  
 Sta n Val Sta n Val Sta n Val  
 2000 .1 2684 .05 3000 .1

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 2684 3000 3520 4160 3746 .1 .3

## CROSS SECTION

RIVER: Fox River  
REACH: Algonquin

RS: 414.015

## INPUT

Description: 2002 FIS DA, 1979 XS 78.412

Station	Elevation	Data num=	66	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
2000	736.8	2022		737.3	2048	730.7	2091	722.7	2092	721.4	
2097	720.8	2102		720.3	2112	718.4	2132	715	2152	715	
2172	714.8	2192		715.1	2212	715.7	2232	716.8	2252	717.8	
2272	718.4	2292		719.3	2312	720.1	2332	720.4	2352	720.3	
2372	720.3	2392		720.3	2412	720.3	2432	720.4	2452	720.2	
2472	717.7	2492		714.3	2512	711.8	2532	712.7	2552	715	
2572	718.4	2592		719.6	2612	720.1	2622	720.6	2627	720.8	
2632	721.4	2652		723.3	2682	723.2	2708	723.4	2727	723.7	
2751	724.2	2780		724.7	2784	724.2	2795	725	2821	725.6	
2848	725.8	2886		725.9	2923	726.6	2952	726.9	2990	727.8	
3017	728.3	3048		728.6	3060	729.9	3064	729.9	3069	726.9	
3086	726.9	3095		729.1	3107	729.4	3130	729.2	3151	730.7	
3178	731.1	3204		731	3229	731.6	3255	732.2	3266	732.4	
3283	733										

## Manning's n Values

Sta	n Val	Sta	n Val	Sta	n Val
2000	.1	2022	.0501	2652	.1

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	2022	2652		0	0	0	.1		.3

## SUMMARY OF MANNING'S N VALUES

River:Fox River

Reach	River Sta.	n1	n2	n3
Algonquin	428.504	.1	.05	.1
Algonquin	426.207	.1	.05	.1
Algonquin	424.8	.102	.0525	.102
Algonquin	424.6	.102	.0525	.102
Algonquin	424.528	.102	.0507	.102
Algonquin	423.9	.101	.0525	.101
Algonquin	423.85	.101	.0525	.101
Algonquin	423.8 Bridge			
Algonquin	423.75	.101	.0525	.101
Algonquin	423.7	.101	.0525	.101
Algonquin	423.3	.101	.052	.101
Algonquin	422.6	.1	.051	.1
Algonquin	422.521	.1	.05	.1
Algonquin	420.346	.1	.0506	.1
Algonquin	417.841	.1	.05	.1
Algonquin	414.015	.1	.0501	.1

## SUMMARY OF REACH LENGTHS

River: Fox River

Reach	River Sta.	Left	Channel	Right
Algonquin	428.504	2094	2294	2134
Algonquin	426.207	1430	1404	1416
Algonquin	424.8	470	470	470
Algonquin	424.6	60	60	60
Algonquin	424.528	355	345	338
Algonquin	423.9	79	49	34
Algonquin	423.85	87	87	87
Algonquin	423.8	Bridge		
Algonquin	423.75	34	64	79
Algonquin	423.7	426	414	410
Algonquin	423.3	690	665	650
Algonquin	422.6	75	70	68
Algonquin	422.521	2280	1106	1654
Algonquin	420.346	2454	2440	2400
Algonquin	417.841	3520	4160	3746
Algonquin	414.015	0	0	0

## SUMMARY OF CONTRACTION AND EXPANSION COEFFICIENTS

River: Fox River

Reach	River Sta.	Contr.	Expan.
Algonquin	428.504	.1	.3
Algonquin	426.207	.1	.3
Algonquin	424.8	.1	.3
Algonquin	424.6	.1	.3
Algonquin	424.528	.1	.3
Algonquin	423.9	.1	.3
Algonquin	423.85	.3	.5
Algonquin	423.8 Bridge		
Algonquin	423.75	.3	.5
Algonquin	423.7	.1	.3
Algonquin	423.3	.1	.3
Algonquin	422.6	.1	.3
Algonquin	422.521	.1	.3
Algonquin	420.346	.1	.3
Algonquin	417.841	.1	.3
Algonquin	414.015	.1	.3

## HEC-RAS Plan: Proposed River: Fox River Reach: Algonquin

Reach	River Sta	Profile	Q Total (cfs)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Algonquin	428.504	10 yr	5775.00	717.20	729.48	729.63	0.000639	3.29	2763.72	810.97	0.19
Algonquin	428.504	50 yr	8345.00	717.20	731.10	731.25	0.000590	3.52	4305.23	1007.52	0.19
Algonquin	428.504	100 yr	10095.00	717.20	732.02	732.17	0.000564	3.63	5384.11	1288.50	0.19
Algonquin	428.504	500 yr	12525.00	717.20	733.13	733.28	0.000528	3.73	6923.14	1432.15	0.18
Algonquin	426.207	10 yr	5775.00	716.80	728.30	728.41	0.000447	2.75	2669.20	532.20	0.16
Algonquin	426.207	50 yr	8345.00	716.80	729.91	730.04	0.000484	3.18	3712.26	750.92	0.17
Algonquin	426.207	100 yr	10095.00	716.80	730.82	730.97	0.000499	3.41	4418.06	794.29	0.18
Algonquin	426.207	500 yr	12525.00	716.80	731.93	732.10	0.000514	3.68	5332.28	854.87	0.18
Algonquin	424.8	10 yr	5775.00	715.90	727.77	727.83	0.000359	2.33	3855.92	824.50	0.14
Algonquin	424.8	50 yr	8345.00	715.90	729.35	729.43	0.000371	2.60	5176.08	837.66	0.14
Algonquin	424.8	100 yr	10095.00	715.90	730.26	730.34	0.000378	2.77	5934.87	844.61	0.14
Algonquin	424.8	500 yr	12525.00	715.90	731.36	731.45	0.000390	2.99	6867.99	853.61	0.15
Algonquin	424.6	10 yr	5775.00	714.00	727.70	727.74	0.000114	1.52	4370.42	733.32	0.08
Algonquin	424.6	50 yr	8345.00	714.00	729.27	729.32	0.000144	1.86	5547.27	762.53	0.09
Algonquin	424.6	100 yr	10095.00	714.00	730.16	730.22	0.000161	2.06	6232.18	772.69	0.10
Algonquin	424.6	500 yr	12525.00	714.00	731.25	731.32	0.000183	2.31	7078.46	785.06	0.11
Algonquin	424.528	10 yr	5775.00	716.60	727.66	727.72	0.000272	2.10	3248.52	781.70	0.12
Algonquin	424.528	50 yr	8345.00	716.60	729.22	729.31	0.000299	2.45	4580.20	888.46	0.13
Algonquin	424.528	100 yr	10095.00	716.60	730.11	730.21	0.000313	2.64	5387.38	924.46	0.14
Algonquin	424.528	500 yr	12525.00	716.60	731.20	731.31	0.000328	2.87	6430.39	984.08	0.14
Algonquin	423.9	10 yr	5775.00	715.40	727.53	727.61	0.000389	2.36	2863.15	622.98	0.14
Algonquin	423.9	50 yr	8345.00	715.40	729.07	729.18	0.000425	2.75	3939.75	725.37	0.15
Algonquin	423.9	100 yr	10095.00	715.40	729.95	730.08	0.000444	2.97	4585.09	739.49	0.16
Algonquin	423.9	500 yr	12525.00	715.40	731.03	731.17	0.000468	3.24	5388.10	756.68	0.16
Algonquin	423.85	10 yr	5775.00	715.00	727.51	720.22	0.000474	2.35	2876.53	671.96	0.15
Algonquin	423.85	50 yr	8345.00	715.00	729.06	721.10	0.000472	2.66	4044.34	775.76	0.16
Algonquin	423.85	100 yr	10095.00	715.00	729.94	730.05	0.000474	2.83	4762.04	902.82	0.16
Algonquin	423.85	500 yr	12525.00	715.00	731.02	731.14	0.000477	3.04	5744.41	920.22	0.16
Algonquin	423.8	Bridge									
Algonquin	423.75	10 yr	5775.00	714.90	727.47	727.55	0.000349	2.30	3146.64	631.97	0.13
Algonquin	423.75	50 yr	8345.00	714.90	729.02	729.12	0.000384	2.68	4285.09	768.53	0.14

## HEC-RAS Plan: Proposed River: Fox River Reach: Algonquin (Continued)

Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Cnt W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Algonquin	423.75	100 yr	10095.00	714.90	729.90		730.01	0.000402	2.89	4963.63	776.06	0.15
Algonquin	423.75	500 yr	12525.00	714.90	730.96		731.09	0.000426	3.16	5797.19	785.20	0.16
Algonquin	423.7	10 yr	5775.00	714.90	727.45		727.53	0.000352	2.31	3132.77	630.79	0.13
Algonquin	423.7	50 yr	8345.00	714.90	728.99		729.09	0.000387	2.68	4266.73	768.32	0.15
Algonquin	423.7	100 yr	10095.00	714.90	729.87		729.98	0.000406	2.90	4944.42	775.84	0.15
Algonquin	423.7	500 yr	12525.00	714.90	730.94		731.07	0.000430	3.17	5776.79	784.98	0.16
Algonquin	423.3	10 yr	5775.00	714.10	727.28		727.37	0.000353	2.53	2853.89	570.64	0.14
Algonquin	423.3	50 yr	8345.00	714.10	728.79		728.92	0.000444	3.00	3752.99	618.77	0.16
Algonquin	423.3	100 yr	10095.00	714.10	729.65		729.80	0.000478	3.27	4296.38	646.11	0.17
Algonquin	423.3	500 yr	12525.00	714.10	730.69		730.87	0.000522	3.61	4987.51	679.64	0.18
Algonquin	422.6	10 yr	5775.00	716.20	727.02		727.11	0.000407	2.49	3056.90	664.44	0.15
Algonquin	422.6	50 yr	8345.00	716.20	728.51		728.62	0.000445	2.89	4116.26	761.05	0.16
Algonquin	422.6	100 yr	10095.00	716.20	729.35		729.48	0.000466	3.11	4779.42	809.30	0.16
Algonquin	422.6	500 yr	12525.00	716.20	730.38		730.52	0.000490	3.39	5633.65	845.57	0.17
Algonquin	422.521	10 yr	5775.00	714.30	726.91		727.07	0.000648	3.36	2498.92	702.21	0.19
Algonquin	422.521	50 yr	8345.00	714.30	728.39		728.57	0.000688	3.81	3595.58	779.82	0.20
Algonquin	422.521	100 yr	10095.00	714.30	729.23		729.43	0.000704	4.05	4264.41	806.45	0.21
Algonquin	422.521	500 yr	12525.00	714.30	730.26		730.47	0.000725	4.34	5107.75	837.77	0.21
Algonquin	420.346	10 yr	5775.00	715.60	726.16		726.29	0.000655	3.07	2657.55	714.55	0.19
Algonquin	420.346	50 yr	8345.00	715.60	727.55		727.71	0.000687	3.49	3698.36	772.46	0.20
Algonquin	420.346	100 yr	10095.00	715.60	728.35		728.52	0.000704	3.72	4326.32	796.45	0.20
Algonquin	420.346	500 yr	12525.00	715.60	729.32		729.51	0.000727	4.01	5114.60	836.64	0.21
Algonquin	417.841	10 yr	5775.00	714.30	725.15		725.22	0.000304	2.13	3030.74	633.28	0.13
Algonquin	417.841	50 yr	8345.00	714.30	726.37		726.47	0.000379	2.60	3903.71	806.83	0.15
Algonquin	417.841	100 yr	10095.00	714.30	727.09		727.21	0.000416	2.86	4539.05	999.57	0.16
Algonquin	417.841	500 yr	12525.00	714.30	727.97		728.11	0.000455	3.16	5495.27	1143.89	0.17
Algonquin	414.015	10 yr	5775.00	711.80	723.86		723.91	0.000325	1.73	3366.13	649.91	0.13
Algonquin	414.015	50 yr	8345.00	711.80	724.72		724.79	0.000430	2.17	3949.06	711.01	0.15
Algonquin	414.015	100 yr	10095.00	711.80	725.25		725.34	0.000488	2.43	4329.53	728.54	0.16
Algonquin	414.015	500 yr	12525.00	711.80	725.92		726.03	0.000555	2.74	4834.96	813.36	0.17

## **SECTION 9**

### **IDNR-OWR PERMIT SUMMARY FILL AND COMPENSATORY STORAGE CALCULATIONS**

**Illinois Department of Transportation  
Permit Summary for Floodway Construction in Northeast Illinois**

Applicant Agency: *Kane County Division of Transportation*

Route: *Longmeadow Parkway*

Section: *94-00215-01-ES*

SN: *045-3024*

County: *Kane*

Stream: *Fox River*

General Description (bridge length, bridge width, number of spans, abutment type, proposed scope of work within floodway, etc.):

Existing Facility: *No Existing Bridge.*

Proposed Improvement: *The proposed Longmeadow Parkway Bridge crossing the Fox River will be constructed with 8 spans totaling 1300', the largest single span being 300' over the main river channel. The structure will have 7 piers, with none being located in the main channel. The bridge will be approximately 85' wide. The profile of the proposed bridge and the approach roadway will provide approximately 28' of clearance and 35' of freeboard, based on the 50-year storm event.*

1. Is the proposed work classified as repairs such as deck replacement,  Yes  No pavement resurfacing, or the armoring or filling of a scour hole?
2. Does the proposed work only consist of modifications to the existing structure which will occur above the regulatory 100-year flood profile?  Yes  No

Note: If the answer to question 1 or 2 is yes, no permit is required and questions 3 through 12 may be omitted.

3. Does the proposed work below the regulatory 100-year flood profile consist of widening of the existing structure by 12 feet or less?  Yes  No

Note: If yes, Regional Permit No. 2 applies and questions 4 through 9 may be omitted.

4. Is the proposed improvement, including the approach roadway, more restrictive to normal and flood flows than the existing structure?  Yes  No
5. Is a Channel Modification proposed?  Yes  No
6. Are there any buildings or structures located upstream in the 100-year floodplain within the influence of the structure backwater?  Yes  No

- 6a. If no, does the backwater of the proposed improvement exceed the backwater of the existing structure by more than 0.1 foot?  Yes  No
- 6b. If yes, does the proposed backwater exceed the natural high water elevation by more than 0.1 foot?  Yes  No
7. Are transitions required for this project?  Yes  No
8. Is the flood profile at the project site impacted by backwater from a downstream receiving stream?  Yes  No
9. Is backwater from a downstream structure affecting the flood profile at the project site?  Yes  No
- 9a. Was the existing downstream structure used in the analysis for determining flood profile at the project site?  Yes  No
- 9b. Is the downstream structure scheduled for improvement in the next 5 years? (Attach documentation)  Yes  No
- 9c. Was the proposed downstream improvement used in the analysis?  Yes  No
10. Is a floodway map change required due to the proposed project?  Yes  No
11. Will fill or material be placed in the floodway due to the proposed work?  Yes  No
- 11a. If yes, is compensatory storage provided at the project location? (Attach a copy of completed Attachment A)  Yes  No
- 11b. If the answer to 11a is no, is compensatory storage provided at another location? If yes, give location and attach a copy of completed Attachment A.  Yes  No
- 11c. Has compensatory storage relief been granted? (Attach Documentation)  Yes  No
12. Coordination based on Memorandum of Agreement has occurred with Agency(ies) (Attach documentation):

All engineering analysis have been performed by me or under my direct supervision.

Signature: Helen A Dailey

IL/P.E. #: 062-047420

Date: 04/04/2011

P.E. Expiration Date: 11/30/2011

---

FOR DEPARTMENTAL USE ONLY

Is a permit required for this project?

Yes  No

If yes, specify type of permit: Floodway, Regional 1, Regional 2  
(Circle One.)

**Permit Summary**  
**(Attachment A - Compensatory Storage)**

Part of Permit Summary for Floodway Construction in Northeast Illinois:

Phase I (Preliminary)

Phase II (Final)

Applicant Agency: *Kane County Division of Transportation*

Route: *Longmeadow Parkway*

Section: *94-00215-01-ES*

SN: *045-3024*

County: *Kane*

Stream: *Fox River*

Provide the following information for Item 11:

a. Flood Water Elevations (Natural):      100-year 730.09      10-year 727.67  
    Normal 721.77

b.\*\* Determine the amount of fill or material being placed in the floodway:

1. Between the 100-year and 10-year flood elevation    144 cu. yds.
2. Between the 10-year and normal water elevation    88 cu. yds.

c.\*\* Determine the volume being provided to compensate for above item b:  
(i.e. from structures removal, excavation, etc.)

1. Between the 100-year and 10-year flood elevation    216 cu. yds.
2. Between the 10-year and normal water elevation    132 cu. yds.

d. Mark on the exhibits the location and amount of compensatory storage to be excavated. Also show the location of floodway and floodplain boundaries.  
(Include a set of plans and cross sections)

Attach copy of calculations and Exhibit(s) reflecting the above finding.

*\*\*Only proposed piers to generate fill in the floodway. Amount of fill determined from TS&L drawings. Detailed grading of compensatory storage area(s) to be determined in Phase II.*

All engineering analysis have been performed by me or under my direct supervision.

Signature: Marc A Dailey

IL/P.E. #: 062-047420

Date: 04/04/2011

P.E. Expiration Date: 11/30/2011

**Project:** KDOT 99-236 Longmeadow Parkway  
**Calc. By:** IAD  
**Date:** 4/4/2011

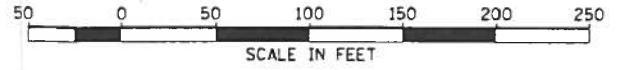
## Pier Fill Calculations Fox River, Kane County, IL

El 100 (BFE) = 730.09 (EEI Datum - see Note 4)  
El 10 = 727.67 (EEI Datum - see Note 4)

Pier Width = 4 ft.  
Pier Length = 85 ft.

Pier No.	Ground El. (EEI Datum)	Fill Volume (cft)		
		0-100-yr	0-10-yr	10-100-yr
1	> BFE	n/a	n/a	n/a
2	726.28	1295	473	823
3	725.40	1595	772	823
4	725.53	1550	728	823
5	728.36	588	0	588
6	726.47	1231	408	823
7	> BFE	n/a	n/a	n/a
TOTAL		6259	2380	3879

- NOTES:
1. Both abutments are located outside the floodplain limits, above the BFE.
  2. IDNR-OWR requires 1:1 compensatory volume for fill in the floodway. Kane County requires 1.5:1 compensatory volume for fill in the floodplain.
  3. In the area of the proposed crossing, the Fox River floodway is generally concurrent with the floodplain. Therefore, to be conservative, calculate FW fill for all piers that are shown in the FP by survey elevation.
  4. EEI Datum = NGVD29 (per FEMA RM 70-3) + 0.16'. See Datum Correlation in Section 6.

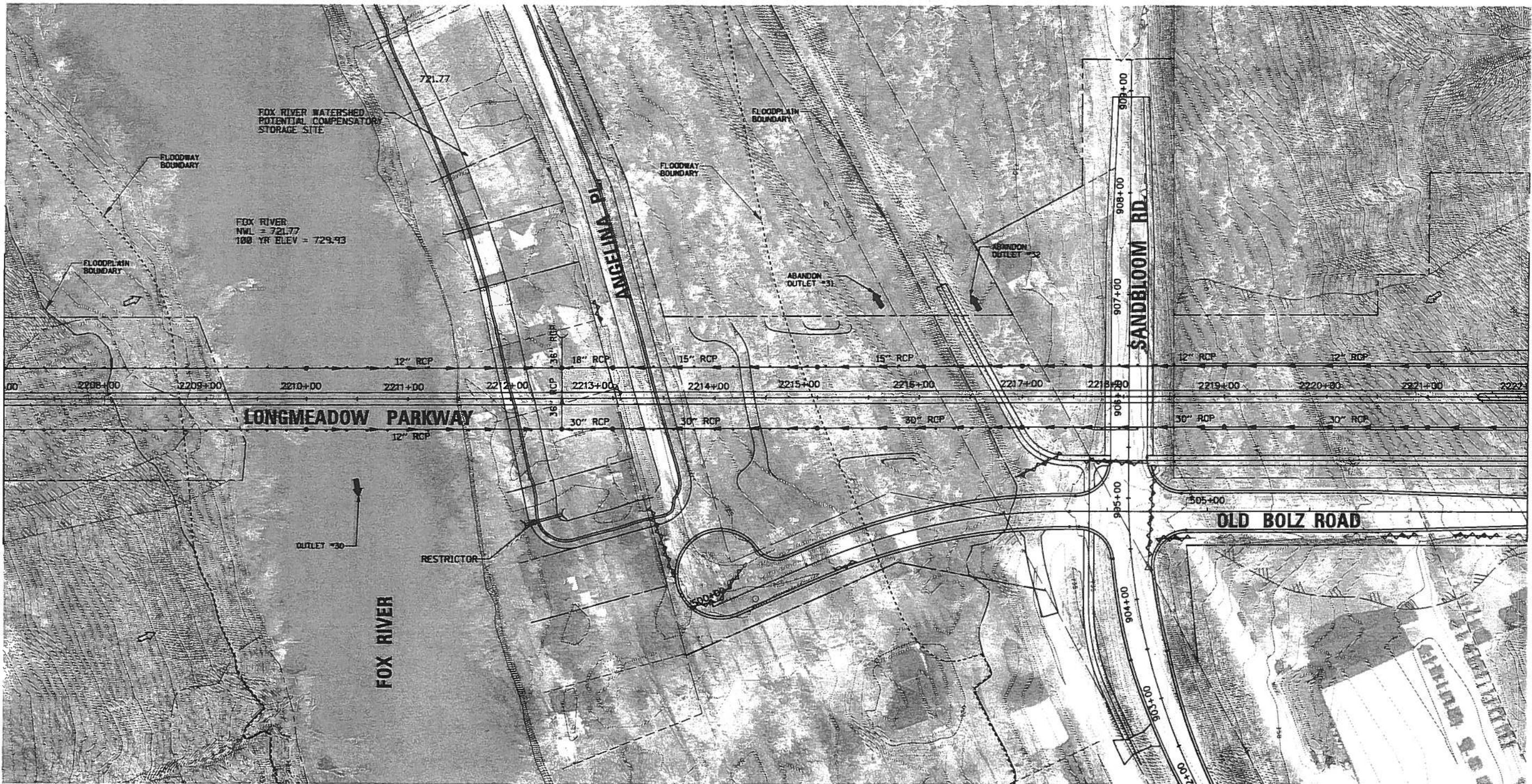


**SCALE IN FEET**

N

MATCH LINE STATION 2207 + 00

33



NOTE: INLET SPACING AND LATERAL PIPES ARE FOR PICTORIAL ILLUSTRATION ONLY.

LEGEND:	
BOUNDARY LINES/SYMBOLS	EXISTING
REFERENCE LINE/CENTERLINE AND STATIONING	1430+00
RIGHT OF WAY LINE	— — — — —
DRAINAGE DIVIDE (HYDROLOGIC ATLAS)	— — — — —
INTERPRETED DRAINAGE AREA TO OUTLET/SUB-DIVIDE	\\\\\\ \\\\\\
FLOODPLAIN BOUNDARY	— — — — —
FLOODWAY BOUNDARY	- - - - -
SWALE	→
PROP. DITCH	~ ~ ~
OUTLET	→
SHEET FLOW	→
CHANNEL	== == ==
CUL VERT. SIZE - TYPE	2 x 2 BOX
BRIDGE LOCATION - BRIDGE NO.	— — — — —
LOCAL STORM SEWER	L
PROP. STORM SEWER	— — — — —
CATCH BASIN	●
HEADWALL/ENDWALL	— — — — —
MANHOLE	○
SEWER REMOVAL	~~~~~



**KANE COUNTY  
DIVISION OF TRANSPORTATION**

END. DATE	NATURE OF REVISION	CHKD.	MODEL:	DEFULAT
FILE NAME	McKonecounty 99236 Vrchn VPLN_99236_Role 15.SHT	CHKD.	mgoldenber9	103'

**LONGMEADOW PARKWAY  
PROPOSED DRAINAGE PLAN  
STA. 2207 + 00 TO STA. 2222 + 00**

PROJ. NO. 99236  
DATE:  
SHEET OF 86  
DRAWING NO.  
**DPLN15**

## **SECTION 10**

### **SCOUR ANALYSIS**

## **Section 3 - Inspection**

Illinois Department of Transportation

### **Scour Critical Evaluation Coding Report**

This "form" should be used by agencies for reporting coding recommendations for Scour Critical Evaluation (ISIS Item 113) and other associated ISIS Items.

Structure Number: 045 - 3024

Report Action:

- New Evaluation  
 Re-Evaluation  
 Error Revision

Refer to the IDOT Structure Information and Procedure Manual for information related to the coding of ISIS Item 113, 113A, 113B, 113C and 113D.

Item 113      Scour Critical Evaluation: 5 (valid codes: 0, 1, 2, 3, 4, 5, 6, 7, 8, or 9)

Item 113A    Scour Critical Evaluation Date: 08 / 29 / 12 (mm / dd / yr)

Item 113B    Scour Critical Evaluation Method: A (valid codes: A, B, C, D)

Item 113C    Scour Critical Evaluation By: Irene Dailey, P.E. (20 characters max.)

Item 113D    Scour Critical Remarks: (3 lines, 79 characters max. each line)

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If Item 113 is coded "4" or "5", and the structure was evaluated using BSAP without the completion of a Scour Evaluation Study, indicate below whether or not periodic "inspection for scour" is required:

Yes  No

If Yes, a Special Feature Inspection should be established. In the space provide below, provide information establishing the interval for inspections and describing the feature(s) requiring inspection (ISIS Items 92C and 92C1 respectively) and submit form BBS-SFI-1.

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## Design Scour Elevation Tables, EEI Datum \*

<b>500-yr. Design Scour Elevation (ft.) →</b>	W. Abut.	Pier 1	Pier 2	Pier 3	Pier 4	Pier 5	Pier 6	Pier 7	E. Abut.
<b>762.12</b>	<b>751.63</b>	<b>719.87</b>	<b>718.99</b>		<b>721.74</b>	<b>719.85</b>	<b>724.47</b>	<b>741.25</b>	
Ground Elev. per MAI TS&L 08/13/2012	762.12	751.63	726.28	725.40	725.53	728.36	726.47	731.09	741.25
500-yr. Scour Depth ** per HEC-RAS	n/a	n/a	6.41	6.41	6.62	6.62	6.62	6.62	n/a
500-yr BFE				731.17					
Thalweg				715.2					

<b>100-yr. Design Scour Elevation (ft.) →</b>	W. Abut.	Pier 1	Pier 2	Pier 3	Pier 4	Pier 5	Pier 6	Pier 7	E. Abut.
<b>762.12</b>	<b>751.63</b>	<b>720.02</b>	<b>719.14</b>	<b>719.07</b>		<b>721.90</b>	<b>720.01</b>	<b>724.63</b>	<b>741.25</b>
Ground Elev. per MAI TS&L 08/13/2012	762.12	751.63	726.28	725.40	725.53	728.36	726.47	731.09	741.25
500-yr. Scour Depth ** per HEC-RAS	n/a	n/a	6.26	6.26	6.46	6.46	6.46	6.46	n/a
100-yr BFE				730.09					
Thalweg				715.2					

### NOTES:

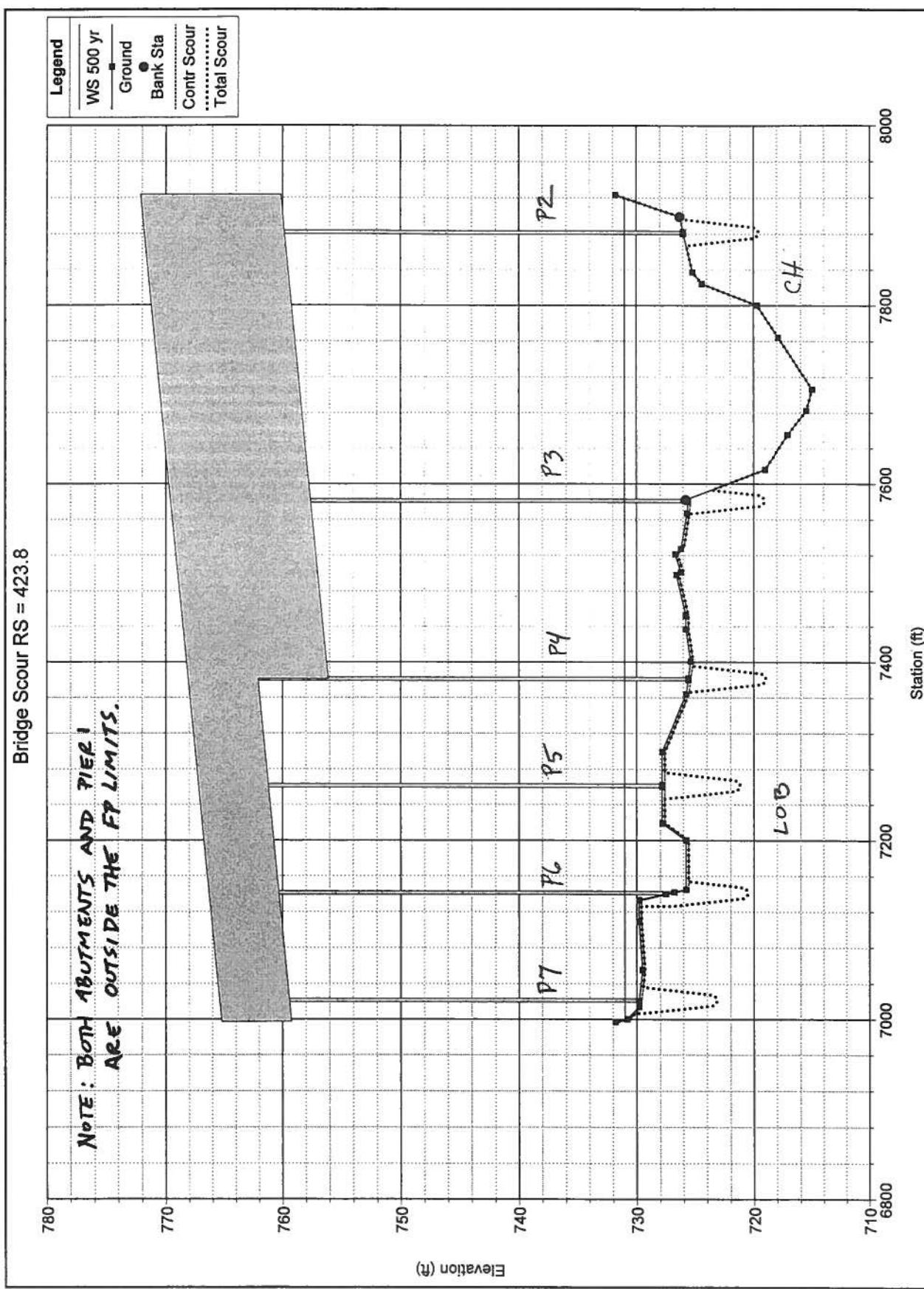
1. \* EEI Datum = FEMA Datum (NGVD29) + 0.16'
2. Both Abutments and Pier 1 are outside of the Fox River Floodplain.

\*\*Per IDOT Drainage Manual, Chapter 10, Section 10-501-02, Entering Pier Scour Data into HEC-RAS, Page 51 of 53:

"The CSU equation is the default. ... The user has the option to use the maximum velocity and depth in the main channel, or the local velocity and depth at each pier for the calculation of the pier scour. However, using the maximum velocity and depth is recommended in order to account for the potential of the main channel thalweg to migrate back and forth within the bridge opening. The migration of the main channel thalweg could cause the maximum potential scour to occur at any one of the bridge piers."

[REDACTED] = revised per Ground Elev. on MAI TSL received 08/13/2012.

500-YR SCOUR



500-yr Scour  
CSU Eqn.

**Contraction Scour**

	Left	Channel	Right
<b>Input Data</b>			
Average Depth (ft):	4.54	12.25	2.76
Approach Velocity (ft/s):	0.89	3.24	0.63
Br Average Depth (ft):	3.80	11.07	2.36
BR Opening Flow (cfs):	1851.35	10645.51	28.14
BR Top WD (ft):	563.63	312.00	20.55
Grain Size D50 (mm):	.02	.02	.02
Approach Flow (cfs):	1667.24	10731.76	126.00
Approach Top WD (ft):	413.81	270.00	72.86
K1 Coefficient:	0.690	0.690	0.690
<b>Results</b>			
Scour Depth Ys (ft):	0.21	0.00	0.00
Critical Velocity (ft/s):	0.58	0.69	0.54
Equation:	Live	Live	Live

**Pier Scour**

All piers have the same scour depth		
<b>Input Data</b>		
Pier Shape:	Round nose	
Pier Width (ft):	4.00	
Grain Size D50 (mm):	0.02000	clay loam
Depth Upstream (ft):	15.29	
Velocity Upstream (ft/s):	3.57	
K1 Nose Shape:	1.00	
Pier Angle:	0.00	
Pier Length (ft):	85.00	
K2 Angle Coef:	1.00	
K3 Bed Cond Coef:	1.10	
Grain Size D90 (mm):	0.30000	
K4 Armouring Coef:	1.00	
<b>Results</b>		
Scour Depth Ys (ft):	6.41	
Froude #:	0.16	
Equation:	CSU equation	

**Combined Scour Depths**

Pier Scour + Contraction Scour (ft):	Left Bank: 6.62
	Channel: 6.41
	Right Bank: 6.41

500-yr Scour  
CSU Eqn.

Contraction Scour

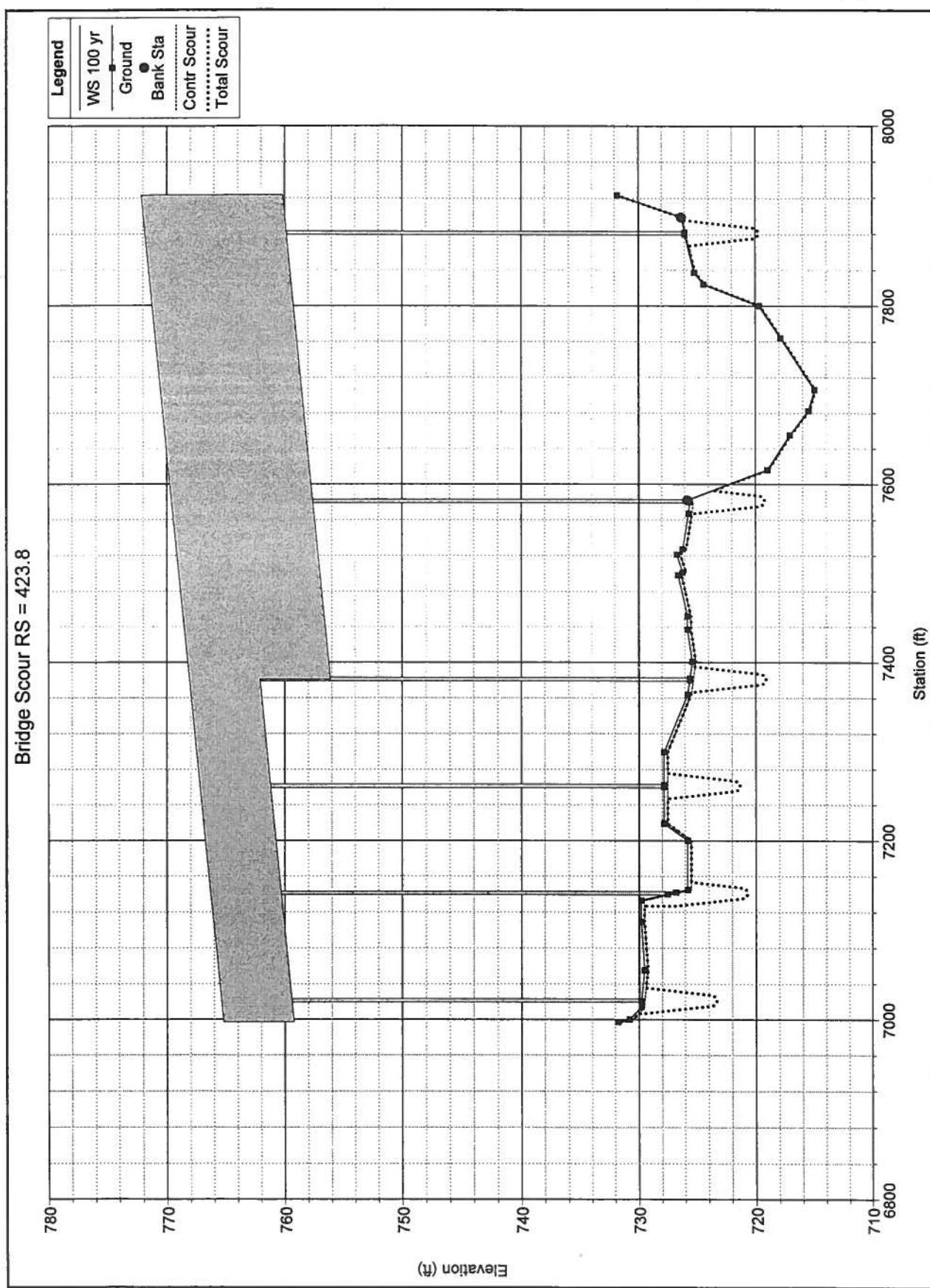
	Left	Channel	Right
Input Data			
Average Depth (ft):	4.54	12.25	2.76
Approach Velocity (ft/s):	0.89	3.24	0.63
Br Average Depth (ft):	3.80	11.07	2.36
BR Opening Flow (cfs):	1851.35	10645.51	28.14
BR Top WD (ft):	563.63	312.00	20.55
Grain Size D50 (mm):	.5	.5	.5
Approach Flow (cfs):	1667.24	10731.76	126.00
Approach Top WD (ft):	413.81	270.00	72.86
K1 Coefficient:	0.690	0.690	0.690
Results			
Scour Depth Ys (ft):	0.00	0.00	0.00
Critical Velocity (ft/s):	1.70	2.01	1.56
Equation:	Clear	Live	Clear

Pier Scour

All piers have the same scour depth

Input Data	Pier Shape:	Round nose	gravelly sand (for sensitivity analysis)
	Pier Width (ft):	4.00	
	Grain Size D50 (mm):	0.50000	
	Depth Upstream (ft):	15.29	
	Velocity Upstream (ft/s):	3.57	
	K1 Nose Shape:	1.00	
	Pier Angle:	0.00	
	Pier Length (ft):	85.00	
	K2 Angle Coef:	1.00	
	K3 Bed Cond Coef:	1.10	
	Grain Size D90 (mm):	10.00000	
	K4 Armouring Coef:	1.00	
Results	Scour Depth Ys (ft):	6.41	
	Froude #:	0.16	
	Equation:	CSU equation	

100 - yr SCOUR



100 - YR SCOUR  
CSU Egn.

**Contraction Scour**

	Left	Channel	Right
<b>Input Data</b>			
Average Depth (ft):	3.50	11.18	2.23
Approach Velocity (ft/s):	0.73	2.97	0.53
Br Average Depth (ft):	2.80	9.99	1.82
BR Opening Flow (cfs):	1153.24	8927.84	13.93
BR Top WD (ft):	550.88	312.00	15.86
Grain Size D50 (mm):	.02	.02	.02
Approach Flow (cfs):	1052.59	8973.39	69.02
Approach Top WD (ft):	410.77	270.00	58.71
K1 Coefficient:	0.690	0.690	0.690
<b>Results</b>			
Scour Depth Ys (ft):	0.29	0.08	0.00
Critical Velocity (ft/s):	0.56	0.68	0.52
Equation:	Live	Live	Live

**Pier Scour**

All piers have the same scour depth		
<b>Input Data</b>		
Pier Shape:	Round nose	
Pier Width (ft):	4.00	
Grain Size D50 (mm):	0.02000	
Depth Upstream (ft):	14.21	
Velocity Upstream (ft/s):	3.34	clay loam
K1 Nose Shape:	1.00	
Pier Angle:	0.00	
Pier Length (ft):	85.00	
K2 Angle Coef:	1.00	
K3 Bed Cond Coef:	1.10	
Grain Size D90 (mm):	0.30000	
K4 Armouring Coef:	1.00	
<b>Results</b>		
Scour Depth Ys (ft):	6.17	
Froude #:	0.16	
Equation:	CSU equation	

**Combined Scour Depths**

Pier Scour + Contraction Scour (ft):	Left Bank: 6.46
	Channel: 6.26
	Right Bank: 6.17

Table 4.2. Soil Properties and Summary Statistics for Samples used in Scour Prediction

Sample	AASHTO	Soil Classification	IDOT	Unconfined Compressive Strength						Index	$D_{50}$ mm	Particle Size	Percent Sand	Percent Silt	Percent Clay
				Wet Density	Percent Moisture Content	(tons/ft <sup>2</sup> )	Liquid Limit	Plastic Limit							
1-1 Soil 1	A-7-5(29)	Silty Clay	104.2	45.0	0.27	59	34	25	0.0041	0.5	18.2	54.5	20.9		
1-4 Soil 1	A-6 (02)	Sand Loam	133.3	19.8	2.39	28	17	11	0.0148	30.4	16.3	46.8	32.1		
1-4 Soil 2	A-4 (2)	Silt Loam	142.0	17.3	0.19	21	16	5	0.0100	0.8	32.4	29.7	16.4	Simulat Clay	10 am
1-4 Soil 3	A-4 (3)	Clay Loam	135.3	15.1	3.52	23	15	8	0.0217	13.9	32.9	36.2	15.9		
1-4 Soil 4	A-4 (3)	Silty Clay-Loam	139.4	16.7	1.78	22	15	7	0.0149	6.4	37.7	41.2	16.5		
1-6 Soil 1	A-7-6(20)	Clay	112.7	35.7	0.18	51	27	24	0.0084	4.8	15.0	61.5	23.5		
1-7 Soil 1	A-6 (02)	Sand Loam	130.9	21.1	0.72	33	21	12	0.1296	21.5	14.7	65.0	20.3		
1-7 Soil 2	A-4 (2)	Loam	119.8	23.4	0.60	27	18	9	0.0679	15.0	31.0	38.8	22.4		
3-25 Soil 1	A-4 (0)	Loam	140.3	10.4	1.20	17	13	4	0.0591	4.6	30.7	42.1	20.9		
4-5 Soil 1	A-6 (09)	Silty-Clay Loam	118.8	29.2	0.21	32	20	12	0.0148	0.0	12.0	65.6	21.5		
4-5 Soil 2	A-4 (8)	Silty-Clay Loam	123.2	27.0	0.66	30	20	10	0.0204	0.0	31.3	46.1	18.1		
5-17 Soil 1	A-4 (1)	Clay-Loam	143.0	11.7	5.47	21	14	7	0.0213	7.8	31.4	48.7	18.7		
5-20 Soil 1	A-4 (3)	Clay-Loam	140.1	13.4	3.53	24	15	9	0.0274	6.3	32.0	42.7	18.7		
6-22 Soil 1	A-4 (8)	Silty-Clay Loam	119.5	20.2	0.97	33	22	11	0.0167	0.9	4.0	66.9	29.0		
7-1 Soil 1	A-4 (4)	Loam	118.6	23.6	0.21	27	17	10	0.0331	4.5	6.3	68.2	25.4		
7-1 Soil 2	A-4 (3)	Loam	119.0	23.1	0.25	25	17	8	0.0304	1.2	29.1	49.8	21.1		
7-18 Soil 1	A-4 (1)	Loam (Till)	143.1	9.8	7.53	19	13	6	0.0345	6.6	6.2	68.8	25.0		
8-3 Soil 1	A-6 (17)	Silty-Clay Loam	119.7	23.5	0.81	38	21	17	0.0095	0.1	4.8	70.2	25.0		
8-50 Soil 1	A-6 (10)	Silty Clay-Loam	121.1	24.1	0.51	30	18	12	0.0105	0.1	18.2	54.5	20.9		
9-1 Soil 1	A-6 (06)	Clay Loam	125.5	24.9	0.18	28	16	12	0.0314	0.0	16.3	46.8	32.1		
9-2 Soil 1	A-6 (12)	Silty Clay-Loam	118.0	23.7	0.47	33	19	14	0.0173	0.0	32.4	29.7	16.4		
9-2 Soil 2	A-6 (14)	Silty Clay-Loam	118.5	21.3	0.49	34	19	15	0.0171	0.0	32.9	36.2	15.9		
<b>Maximum</b>			143.1	45.0	7.5	59	34	25	0.1296	30.4	37.7	75.2	39.6		
<b>Minimum</b>			104.2	9.8	0.2	17	13	4	0.0041	0.0	4.0	29.7	15.8		
<b>Median</b>			122.2	22.2	0.6	28	17	11	0.0189	2.9	18.1	49.3	21.0		
<b>Average</b>			126.6	21.8	1.5	29	18	11	0.0280	5.7	19.9	52.3	22.2		
<b>Standard Deviation</b>			11.2	8.1	1.9	9.9	4.8	5.3	0.0275	8.0	11.5	13.5	5.7		

Various Soil d<sub>50</sub>

Various Soil  $d_{50}$  &  $d_{95}$

Table 2. Particle Size Characteristics of Bank Material Samples

Sample number	$d_{50}$ (mm)	$d_{95}$ (mm)	$\sigma$	$U$	Remarks	Sample number	$d_{50}$ (mm)	$d_{95}$ (mm)	$\sigma$	$U$	Remarks
Reach 1, river mile 24.4						Reach 15, river mile 180.0					
116 0.013	0.13				Clayey silt	53 0.26	5.0		4.48	40.0	Fine-to-coarse sand
115 0.014	0.065				Clayey silt	52 0.19	0.38		10.26	80.0	Silty fine-to-medium sand
Reach 2, river mile 38.4						51 0.017	0.24				Clayey silt
111 0.021	0.19				Silt	Reach 16, river miles 204.0-204.5					
Reach 3, river mile 60.2						47 0.005	0.27				Clayey silt
107 0.04	0.175		5.88		Sandy silt	46 0.0033	0.20				Clayey silt
105 0.063	0.19		4.74	30.40	Sandy silt	Reach 17, river mile 213.0					
Reach 4, river mile 82.1						44 0.17	0.26		1.11	2.25	Fine sand
100 0.012	0.20				Clayey silt	43 0.042	0.23		12.40		Sandy silt
99 0.15	0.24		1.59	2.83	Fine sand	Reach 18, river mile 227.5					
98 0.17	0.32		4.60	23.75	Fine-to-medium sand	39 0.29	0.94		2.56	34.0	Fine-to-coarse sand
Reach 5, river miles 101.0 to 102.0						38 0.08	0.27		11.65	105.0	Silty fine sand
124 0.018	0.51				$\approx$ clayey loam	37 0.12	0.27		10.19	80.0	Silty fine sand
123 0.017	0.26				Sandy clayey silt	36 0.011	0.13				Clayey silt
122 0.014	0.27				Sandy clayey silt	Reach 18, river mile 228.5					
Reach 6, river mile 104.0						28 0.024	0.24		12.77		Sandy silt
92 0.01	0.30				Clayey silt	27 0.23	0.40		1.57	3.0	Fine-to-medium sand
91 0.0084	0.065				Clayey silt	26 0.12	0.35		11.08	62.96	Silty fine-to-medium sand
90 0.0034	0.042				Silty clay	Reach 19, river mile 229.0					
Reach 7, river mile 113.0						32 0.27	0.45		4.56	25.45	Fine-to-medium sand
89 0.016	0.17				Silt	31 0.06	0.24		11.58		Sandy silt
88 0.027	0.20				Silt $\approx$ gravelly sand	30 0.07	0.28		10.46		Fine-to-medium sand
Reach 8, river mile 116.5						29 0.20	0.39		1.29	1.4	Fine sand
85 0.52	10.0		6.23	5.0	Fine-to-coarse sand	Reach 20, river mile 228.9					
84 0.27	0.44		1.75	3.29	Fine sand	35 0.08	8.0		25.0		Sandy silt
83 0.008	0.19				Silty clay	34 0.29	0.57		1.67	3.16	Medium-to-fine sand
Reach 9, river mile 121.4						33 0.39	0.53		1.44	2.15	Medium-to-fine sand
80 0.75	13.0		5.14	4.31	Fine-to-coarse sand	Reach 21, river mile 235.6					
79 2.40	36.0		7.07	16.07	Fine-to-coarse sand and gravel	24B 0.23	1.10		29.82		Silty fine-to-coarse sand
Reach 10, river mile 126.0						24A 0.18	0.5		20.94		Silty fine-to-coarse sand
77 0.019	1.0				Silt	23 0.40	15.0		1.80	1.96	Fine-to-coarse sand
76 0.0115	0.24				Silt	Reach 22, river mile 262.0					
75 0.034	0.25				Silt	18 0.02	0.18				Little clay and fine sand
Reach 11, river mile 134.0						17 0.24	0.47		1.42	1.63	Fine-to-medium sand
73 0.24	0.55		1.50	1.80	Medium-to-fine sand	Reach 23, river mile 267.9					
72 0.23	0.70		1.87	3.43	Medium-to-fine sand	15 0.35	7.0		4.83	4.50	Fine-to-coarse sand
71 0.0074	0.075				Clayey silt	14 2.0			30.13	427.27	Fine-to-coarse sand
Reach 12, river mile 142.5						13 0.075	0.38				Silty fine-to-medium sand
68 0.035	0.12		2.63		Mottled gray silt	Reach 24, river mile 276.8					
67 0.0073	0.14				Clayey silt	9 20.0	67.0		1667.92		Fine-to-coarse gravel
66 0.013	0.49				Clayey silt	7,8 14.0	103.0		6.52	28.57	Sandy fine-to-coarse gravel
Reach 13, river mile 150.0											
64 0.0073	0.26				Clayey silt						
63 0.17	0.42		15.14	115.0	Silty fine-to-coarse sand						
62 0.032	0.40		17.83		Sandy silt						
Reach 14, river mile 154.0											
60 0.14	0.24		2.98	15.0	Fine-to-medium sand						
59 0.04	0.20		8.04		Sandy silt						
58 0.05	0.15		6.10		Sandy silt						

## **SECTION 11**

### **BORING DATA**





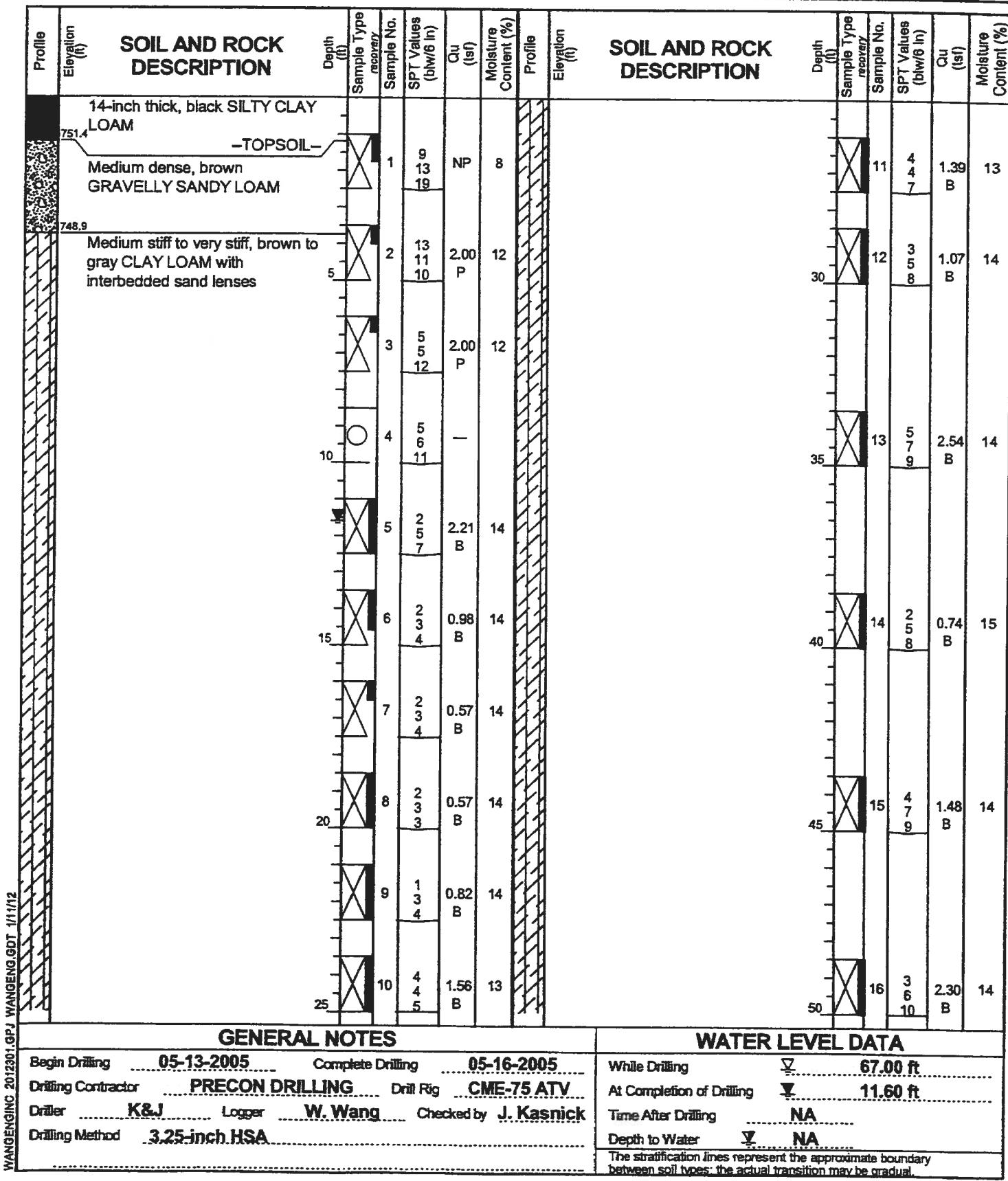
wangeng@wangeng.com  
1145 N Main Street  
Lombard, IL 60148  
Telephone: 630-953-9928  
Fax 630-953-9938

# BORING LOG S-094

WEI Job No.: 201-23-01  
Client McDonough Associates Inc.  
Project Longmeadow Parkway over Fox River, Section Co.  
Location

Datum: NGVD  
Elevation: 752.64 ft  
North: 1993516.01 ft  
East: 998358.59 ft  
Station: 2206+85.26  
Offset: 29.96R

Page 1 of 2





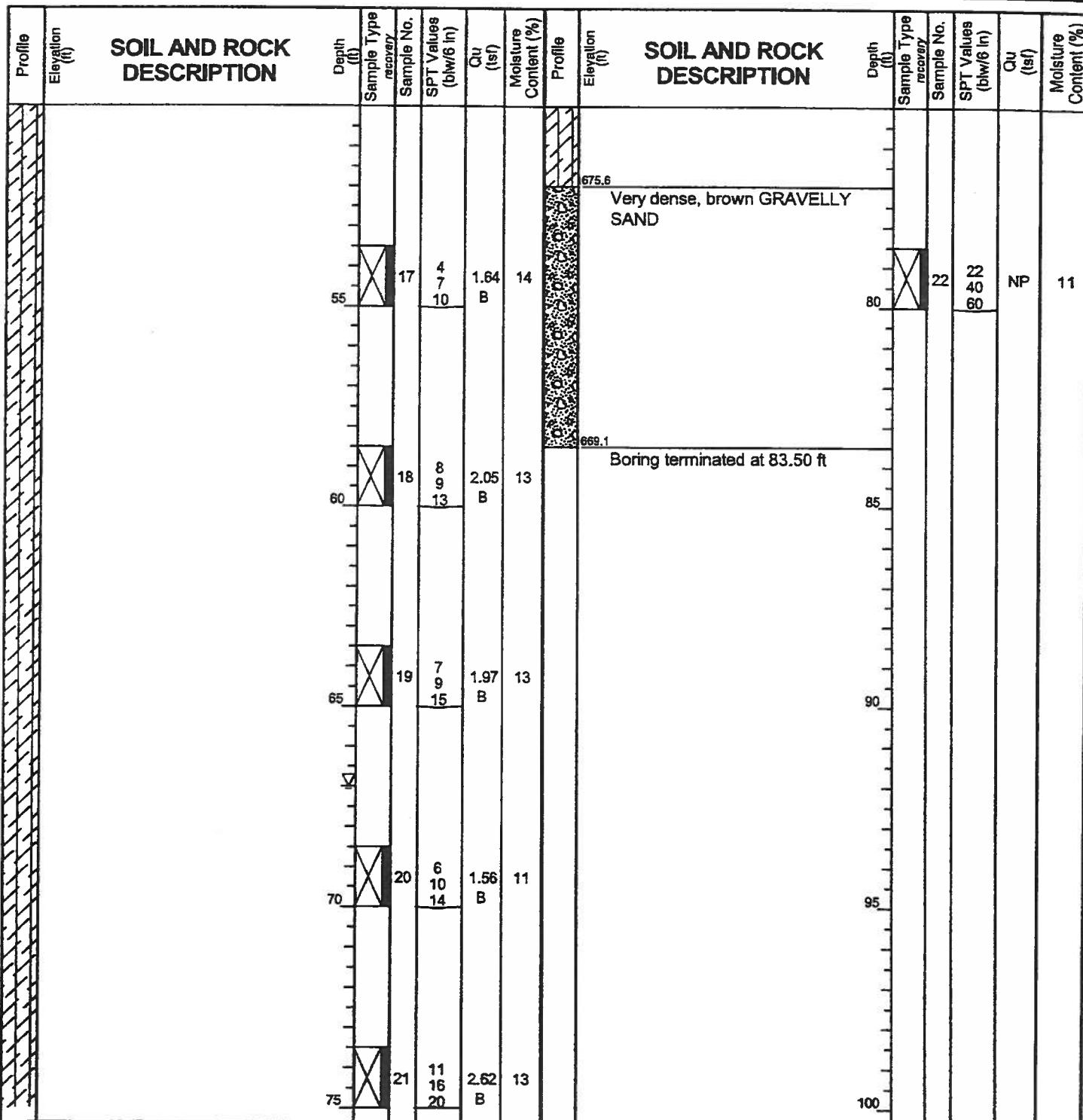
wangeng@wangeng.com  
1145 N Main Street  
Lombard, IL 60148  
Telephone: 630-953-9928  
Fax 630-953-9938

# BORING LOG S-094

WEI Job No.: 201-23-01  
Client McDonough Associates Inc.  
Project Longmeadow Parkway over Fox River, Section  
Location Co.

Page 2 of 2

Datum: NGVD  
Elevation: 752.64 ft  
North: 1993516.01 ft  
East: 998358.59 ft  
Station: 2206+85.26  
Offset: 29.96R



## GENERAL NOTES

Begin Drilling 05-13-2005 Complete Drilling 05-16-2005  
Drilling Contractor PRECON DRILLING Drill Rig CME-75 ATV  
Driller K&J Logger W. Wang Checked by J. Kasnick  
Drilling Method 3.25-inch HSA

## WATER LEVEL DATA

While Drilling ▽ 67.00 ft  
At Completion of Drilling ▽ 11.60 ft  
Time After Drilling NA  
Depth to Water ▽ NA

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



## **BORING LOG S-095**

Page 1 of 2

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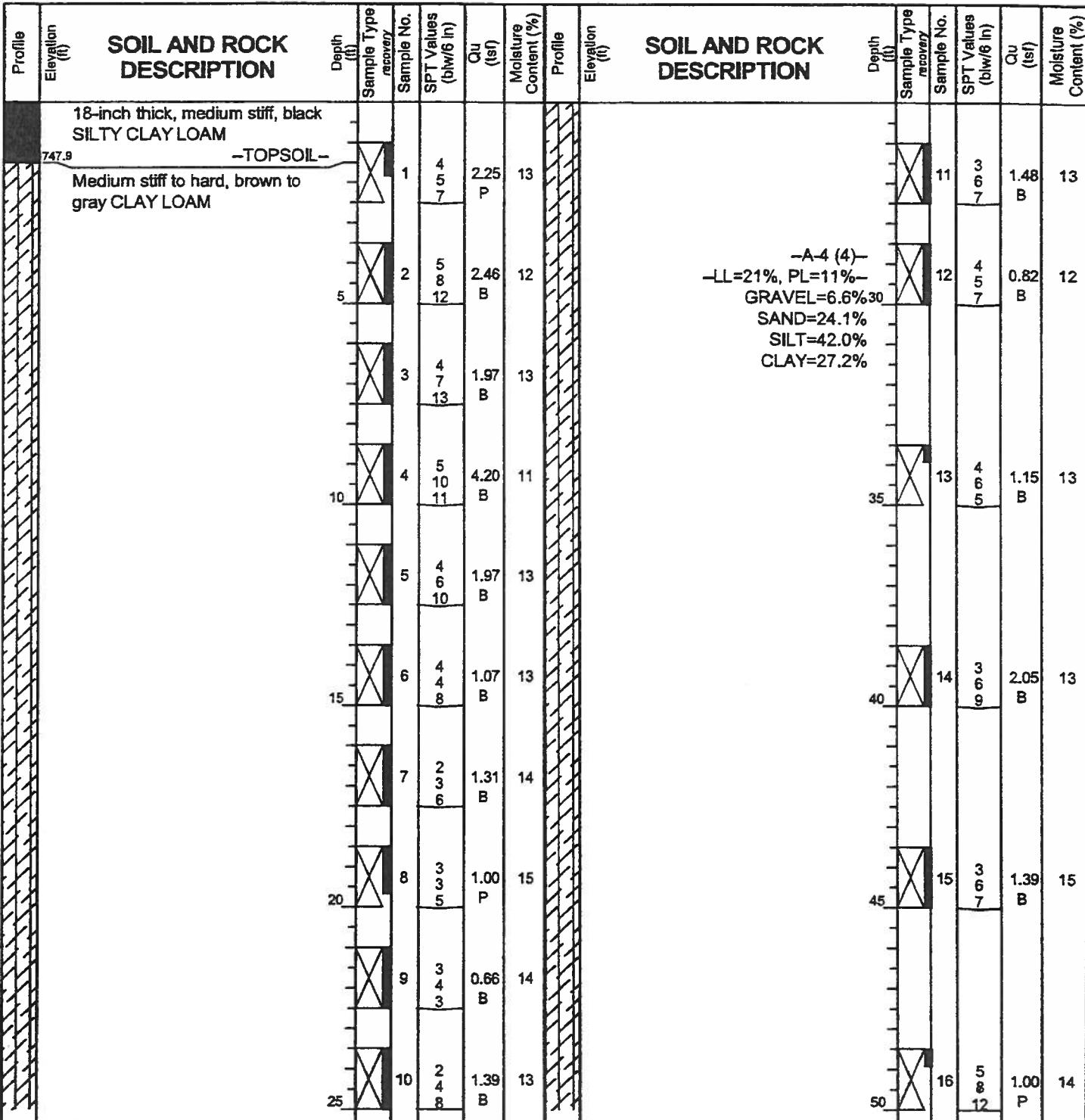
WEI Job No.: 201-23-01

Client ..... **McDonough Associates Inc.**

Project **Longmeadow Parkway over Fox River, Section**

Location ..... **Co.**

Datum: NGVD  
Elevation: 749.36 ft  
North: 1993576.04 ft  
East: 998358.63 ft  
Station: 2206+85.66  
Offset: 30.06L



## **GENERAL NOTES**

Begin Drilling 05-12-2005 Complete Drilling 05-13-2005  
Drilling Contractor PRECON DRILLING Drill Rig CME-75 ATV  
Driller K&J Logger W. Wang Checked by J. Kasnick  
Drilling Method 3.25-inch HSA

## **WATER LEVEL DATA**

While Drilling	<input checked="" type="checkbox"/>	DRY
At Completion of Drilling	<input checked="" type="checkbox"/>	DRY
Time After Drilling	<input type="checkbox"/>	NA
Depth to Water	<input checked="" type="checkbox"/>	NA

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



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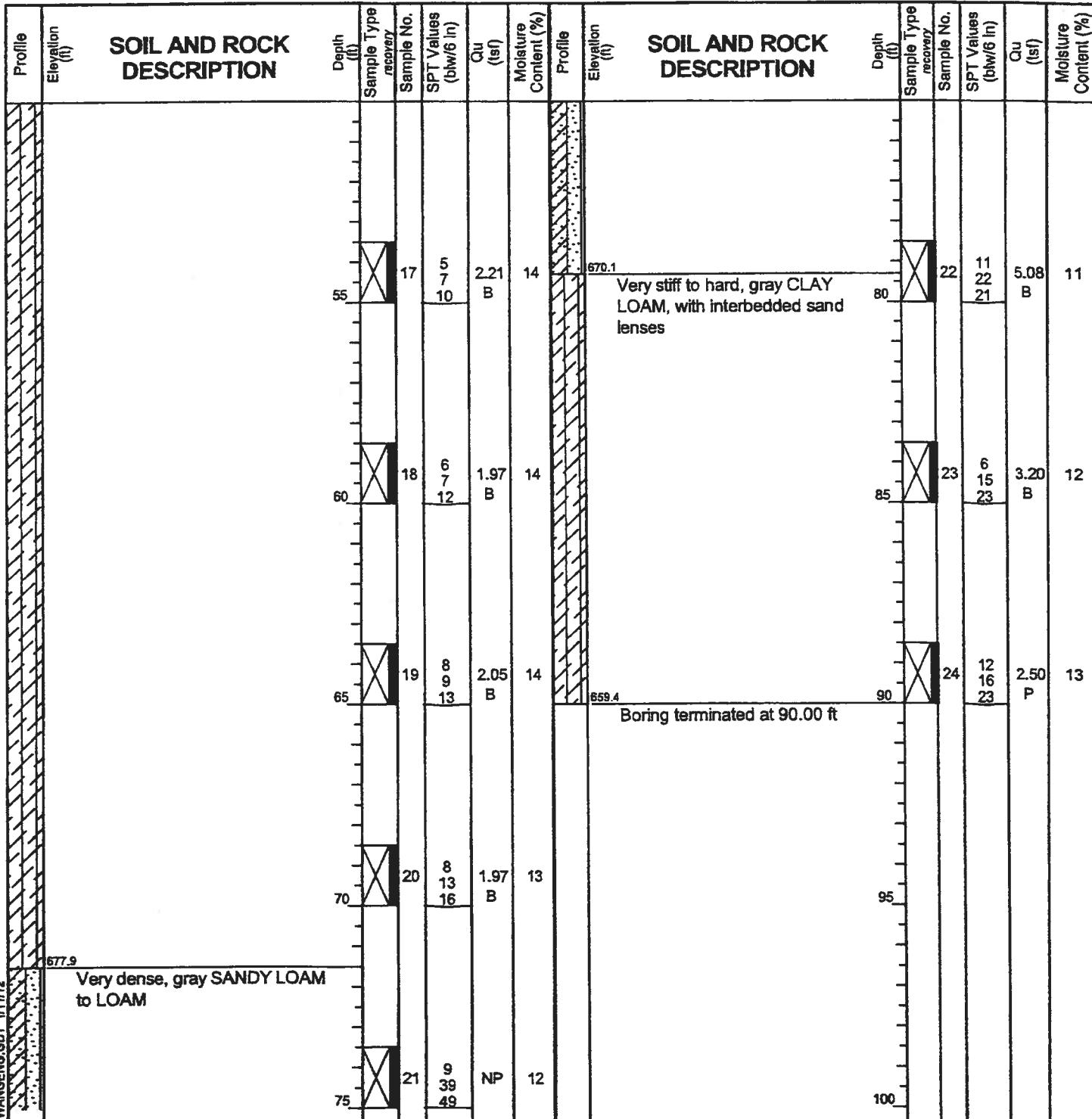
# BORING LOG S-095

WEI Job No.: 201-23-01

Client McDonough Associates Inc.  
Project Longmeadow Parkway over Fox River, Section  
Location Co.

Page 2 of 2

Datum: NGVD  
Elevation: 749.36 ft  
North: 1993576.04 ft  
East: 998358.63 ft  
Station: 2206+85.66  
Offset: 30.06L



## GENERAL NOTES

Begin Drilling 05-12-2005 Complete Drilling 05-13-2005  
Drilling Contractor PRECON DRILLING Drill Rig CME-75 ATV  
Driller K&J Logger W. Wang Checked by J. Kasnick  
Drilling Method 3.25-inch HSA

## WATER LEVEL DATA

While Drilling  DRY  
At Completion of Drilling  DRY  
Time After Drilling NA  
Depth to Water  NA

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



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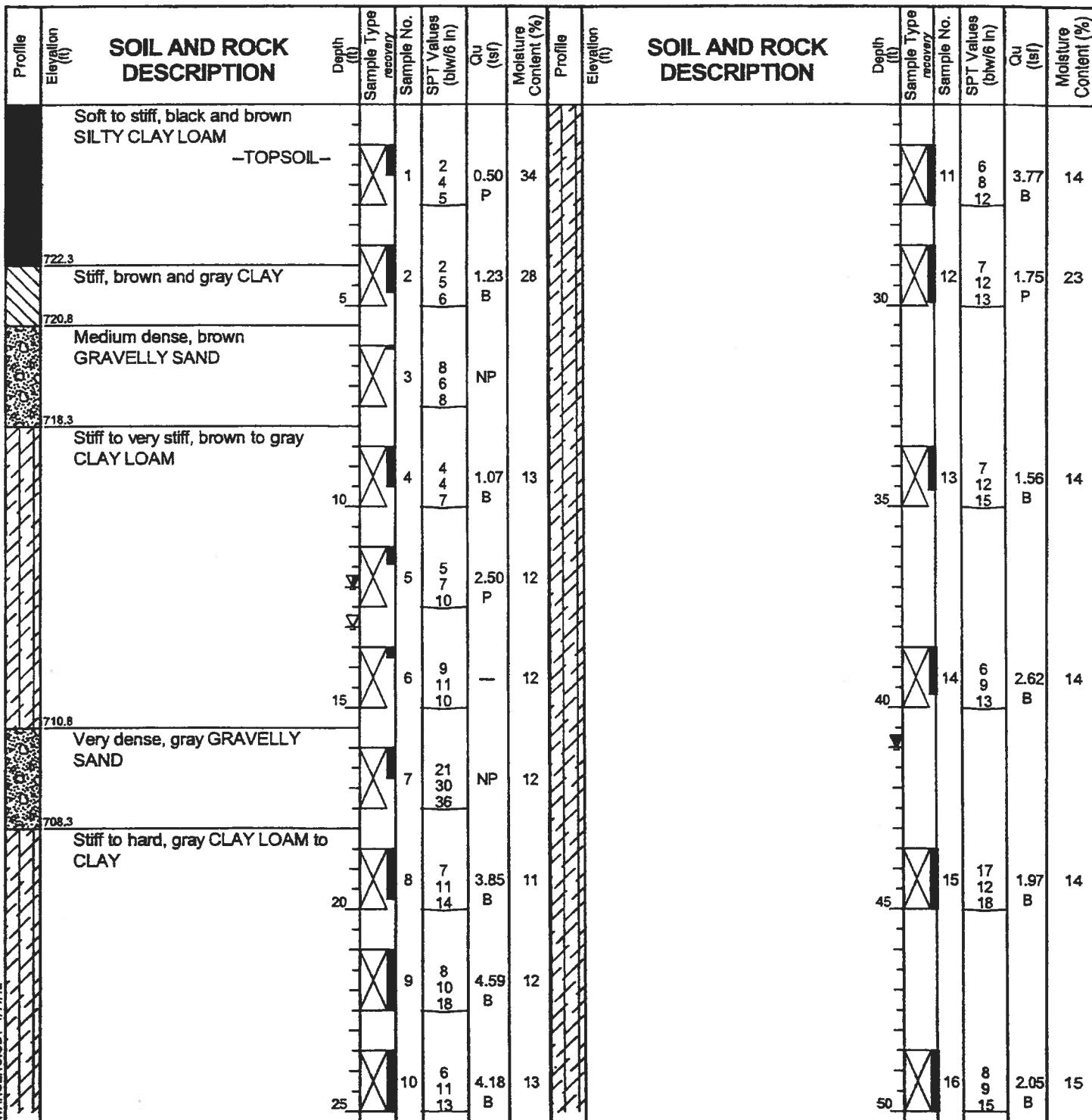
# BORING LOG S-096

WEI Job No.: 201-23-01

Client McDonough Associates Inc.  
Project Longmeadow Parkway over Fox River, Section  
Location Co.

Page 1 of 2

Datum: NGVD  
Elevation: 726.29 ft  
North: 1993536.94 ft  
East: 998549.60 ft  
Station: 2208+76.4  
Offset: 10.17R



## GENERAL NOTES

Begin Drilling 05-16-2005 Complete Drilling 05-17-2005  
Drilling Contractor PRECON DRILLING Drill Rig CME-75 ATV  
Driller K&J Logger T. Rickey Checked by J. Kasnick  
Drilling Method 3.25-inch HSA

## WATER LEVEL DATA

While Drilling	▽	13.00 ft
At Completion of Drilling	▽	41.00 ft
Time After Drilling	24 hrs hours	
Depth to Water	▽	12.00 ft

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



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# **BORING LOG S-096**

WEI Job No.: 201-23-01

Client McDonough Associates Inc.

Project Longmeadow Parkway over Fox River, Section

Location Co.

Page 2 of 2

WANGENGINEC 20112301.GPJ WANGENG.GDT 1/11/12

## **GENERAL NOTES**

Begin Drilling 05-16-2005 Complete Drilling 05-17-2005  
Drilling Contractor PRECON DRILLING Drill Rig CME-75 ATV  
Driller K&J Logger T. Rickey Checked by J. Kasnick  
Drilling Method 3.25-inch HSA

## **WATER LEVEL DATA**

While Drilling      **13.00 f**  
At Completion of Drilling      **41.00 f**  
Time After Drilling      **24 hrs hours**  
Depth to Water      **Y. 12.00 ft.**



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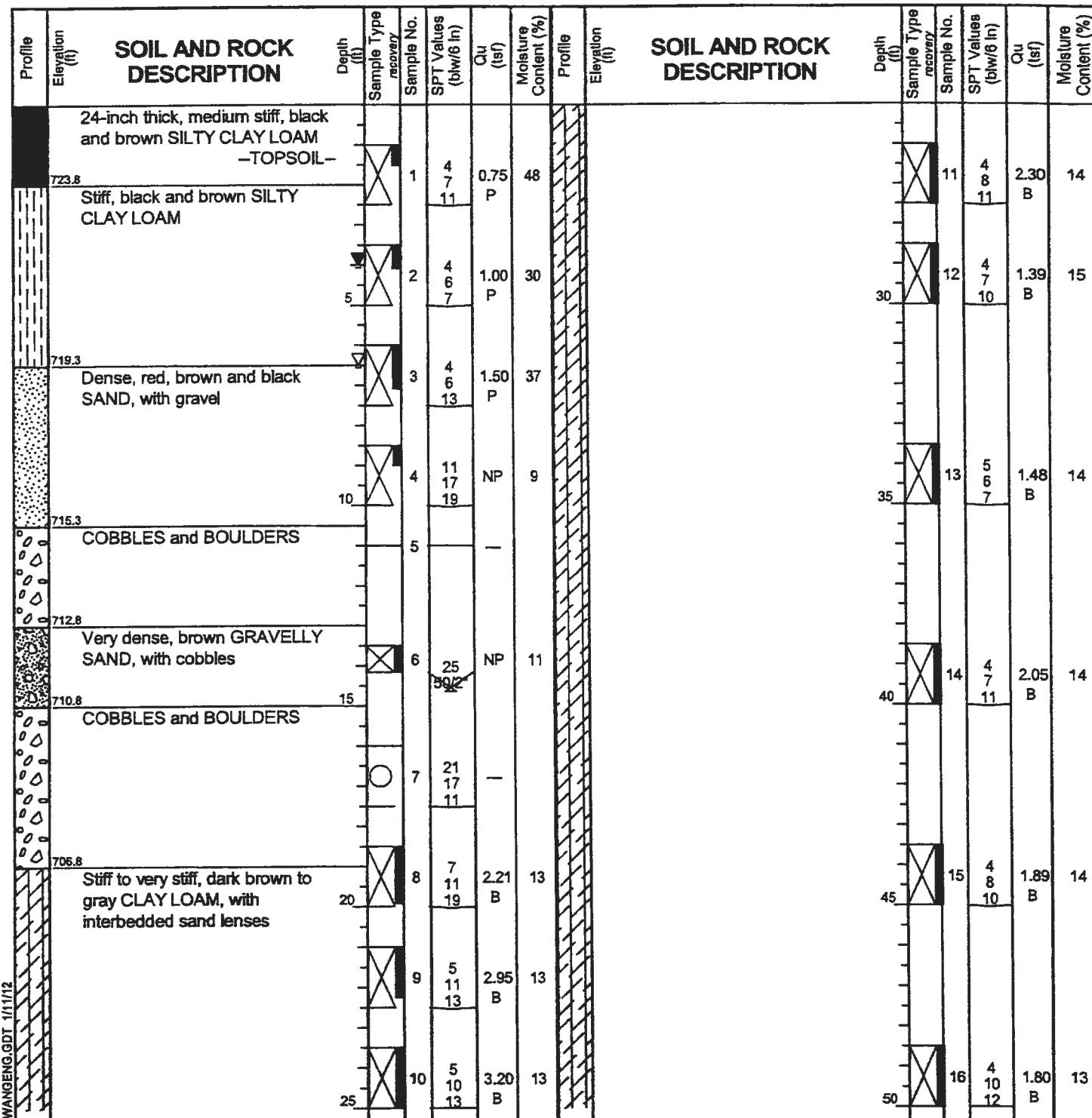
# BORING LOG S-097

WEI Job No.: 201-23-01

Client ..... McDonough Associates Inc.  
Project ..... Longmeadow Parkway over Fox River, Section Co.  
Location .....

Page 1 of 2

Datum: NGVD  
Elevation: 725.83 ft  
North: 1993581.20 ft  
East: 998542.73 ft  
Station: 2208+69.79  
Offset: 34.12L



## GENERAL NOTES

Begin Drilling 05-17-2005 Complete Drilling 05-18-2005  
Drilling Contractor PRECON DRILLING Drill Rig CME-75 ATV  
Driller K&J Logger T. Rickey Checked by J. Kasnick  
Drilling Method 3.25-inch HSA

## WATER LEVEL DATA

While Drilling ✓ 6.50 ft  
At Completion of Drilling ✓ 4.00 ft  
Time After Drilling NA  
Depth to Water ✓ NA  
The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



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## **BORING LOG S-097**

**WEI Job No.: 201-23-01**

Page 2 of 2

WANGENG INC 2012301.GPJ WANGENG.GDT 1/1/12

## **GENERAL NOTES**

Begin Drilling 05-17-2005

Complete Drilling 05-18-2005

Drilling Contractor PRECON DRILLING

Deeler K&J Looper T. Rickey Checked by J. Kasnick

- Method 3 25 inch HSA

## **WATER LEVEL DATA**

**While Drilling**  **6.50 ft**

At Completion of Drilling

Time After Drafting

Death in Water

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



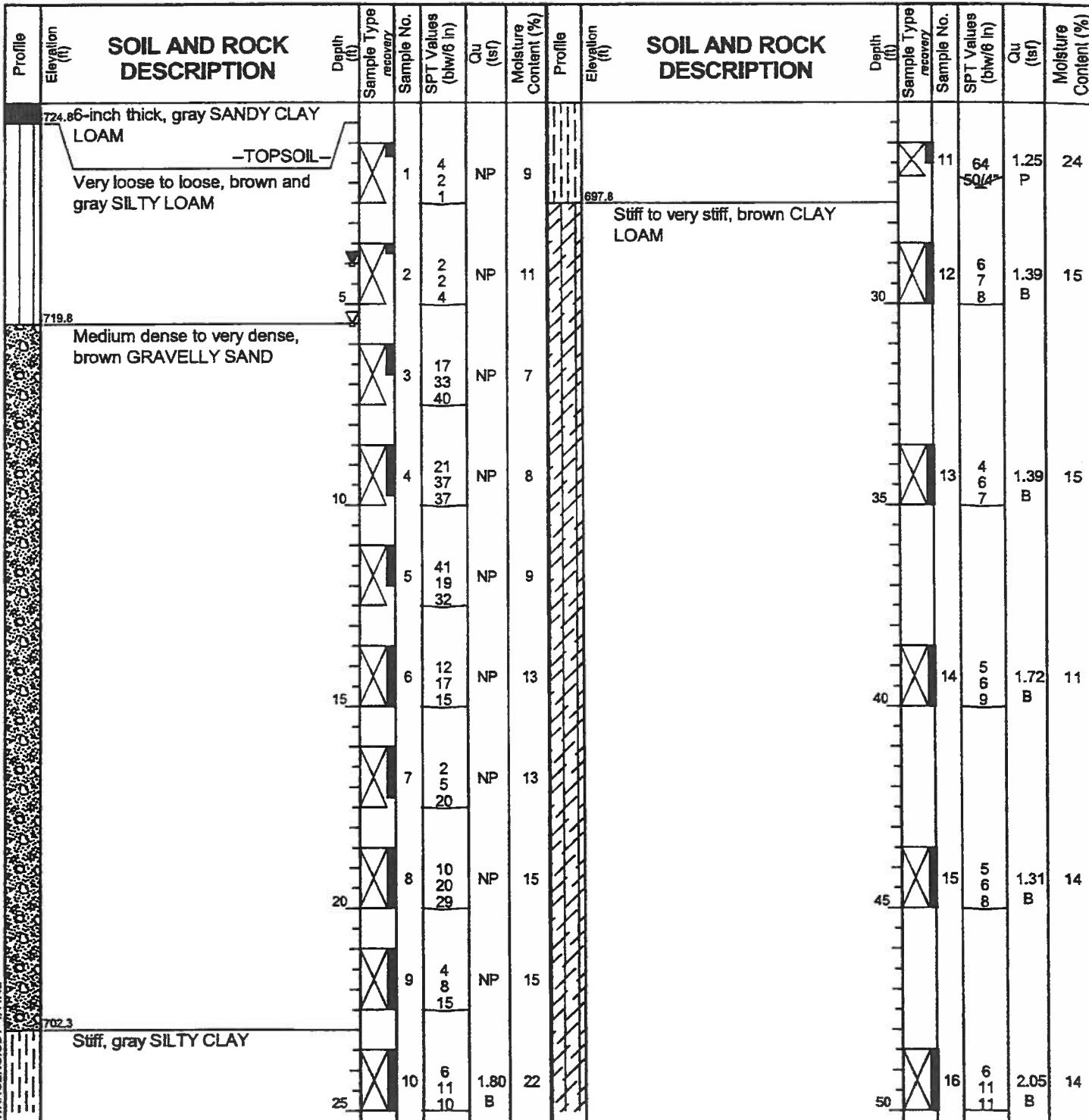
## **BORING LOG S-098**

Page 1 of 2

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WEI Job No.: 201-23-01  
Client McDonough Associates Inc.  
Project Longmeadow Parkway over Fox River, Section  
Location Co.

Datum: NGVD  
Elevation: 725.34 ft  
North: 1993511.85 ft  
East: 998858.63 ft  
Station: 2211+85.27  
Offset: 37.12R



## **GENERAL NOTES**

Begin Drilling 06-30-2005 Complete Drilling 06-30-2005  
Drilling Contractor PRECON DRILLING Drill Rig CME-75 ATV  
Driller K&J Logger K. Anderson Checked by B. Fugiel  
Drilling Method 3.25-inch HSA

# WATER LEVEL DATA

While Drilling		5.50 ft
At Completion of Drilling		4.00 ft
Time After Drilling	NA	
Depth to Water		NA

WANDENG INC 2012301 GP1 WANGENG QDT 1/11/12



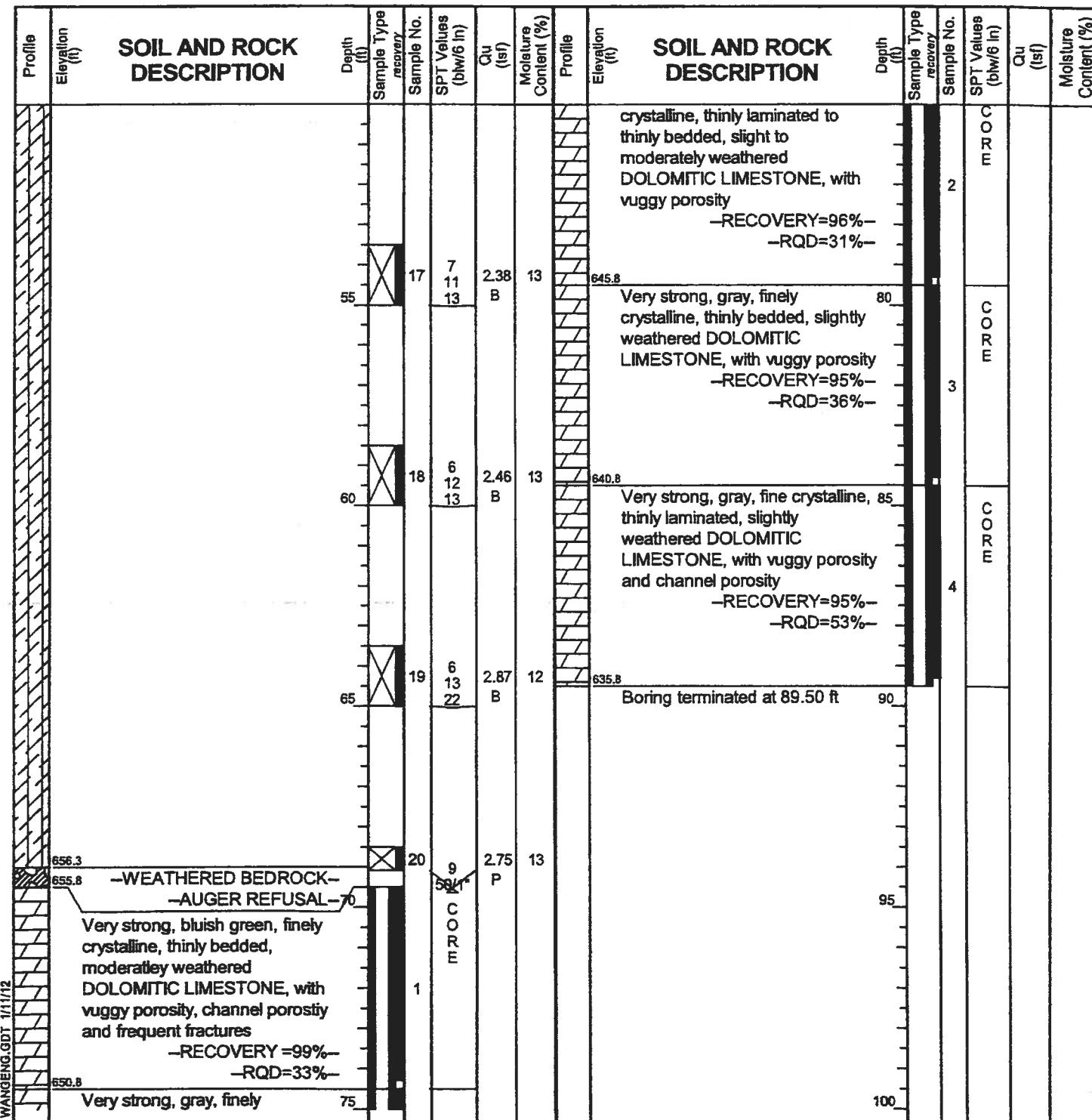
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# BORING LOG S-098

Page 2 of 2

WEI Job No.: 201-23-01  
Client McDonough Associates Inc.  
Project Longmeadow Parkway over Fox River, Section  
Location Co.

Datum: NGVD  
Elevation: 725.34 ft  
North: 1993511.85 ft  
East: 998858.63 ft  
Station: 2211+85.27  
Offset: 37.12R



The stratification lines represent the approximate boundary  
between soil types; the actual transition may be gradual.



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**BORING LOG S-099**

Page 1 of 2

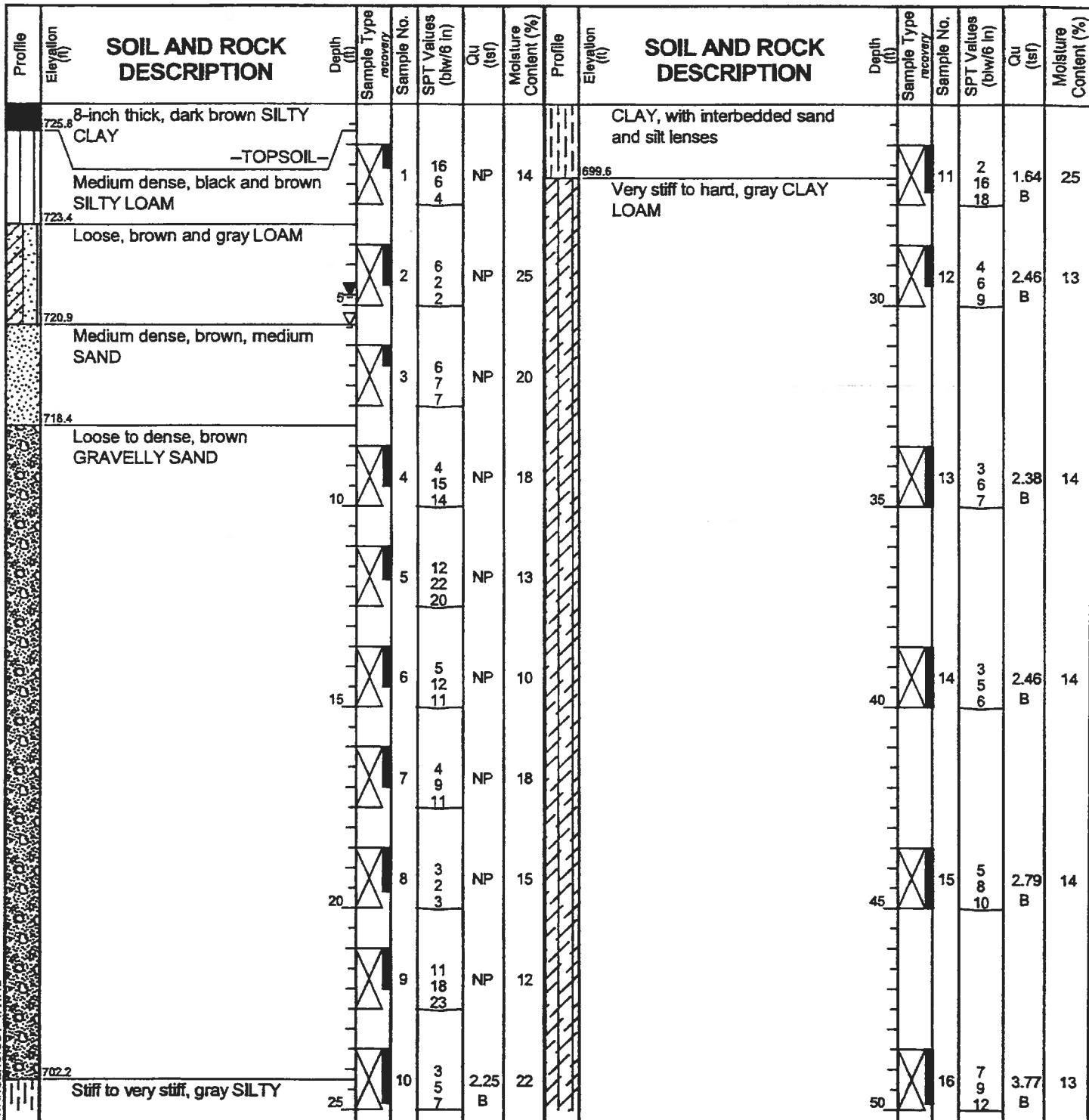
WEI Job No.: 201-23-01

McDonough Associates Inc.

Client .....  
Project .....  
Location .....

Datum: NGVD

Elevation: 726.44 ft  
North: 1993585.81 ft  
East: 998858.47 ft  
Station: 2211+85.55  
Offset: 36.83L





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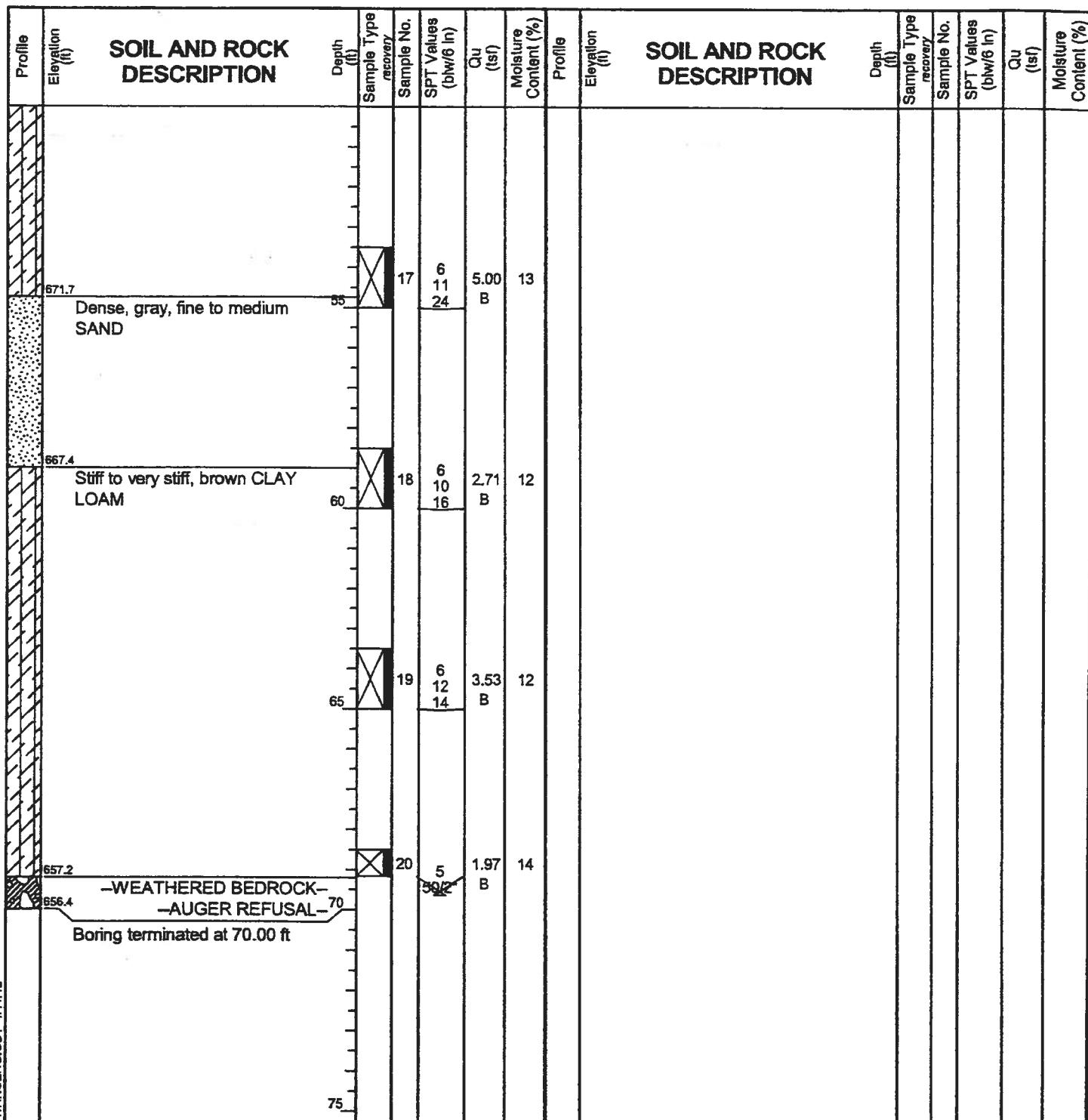
# BORING LOG S-099

WEI Job No.: 201-23-01

Client ..... McDonough Associates Inc.  
Project ..... Longmeadow Parkway over Fox River, Section Co.  
Location .....

Page 2 of 2

Datum: NGVD  
Elevation: 726.44 ft  
North: 1993585.81 ft  
East: 998858.47 ft  
Station: 2211+85.55  
Offset: 36.83L



## GENERAL NOTES

Begin Drilling 07-01-2005 Complete Drilling 07-01-2005  
Drilling Contractor PRECON DRILLING Drill Rig CME-75 ATV  
Driller S&J Logger J. Kasnick Checked by B. Fugiel  
Drilling Method 3.25-inch HSA

## WATER LEVEL DATA

While Drilling ▽ 5.50 ft  
At Completion of Drilling ▽ 4.75 ft  
Time After Drilling NA  
Depth to Water ▽ NA

The stratification lines represent the approximate boundary  
between soil types; the actual transition may be gradual.



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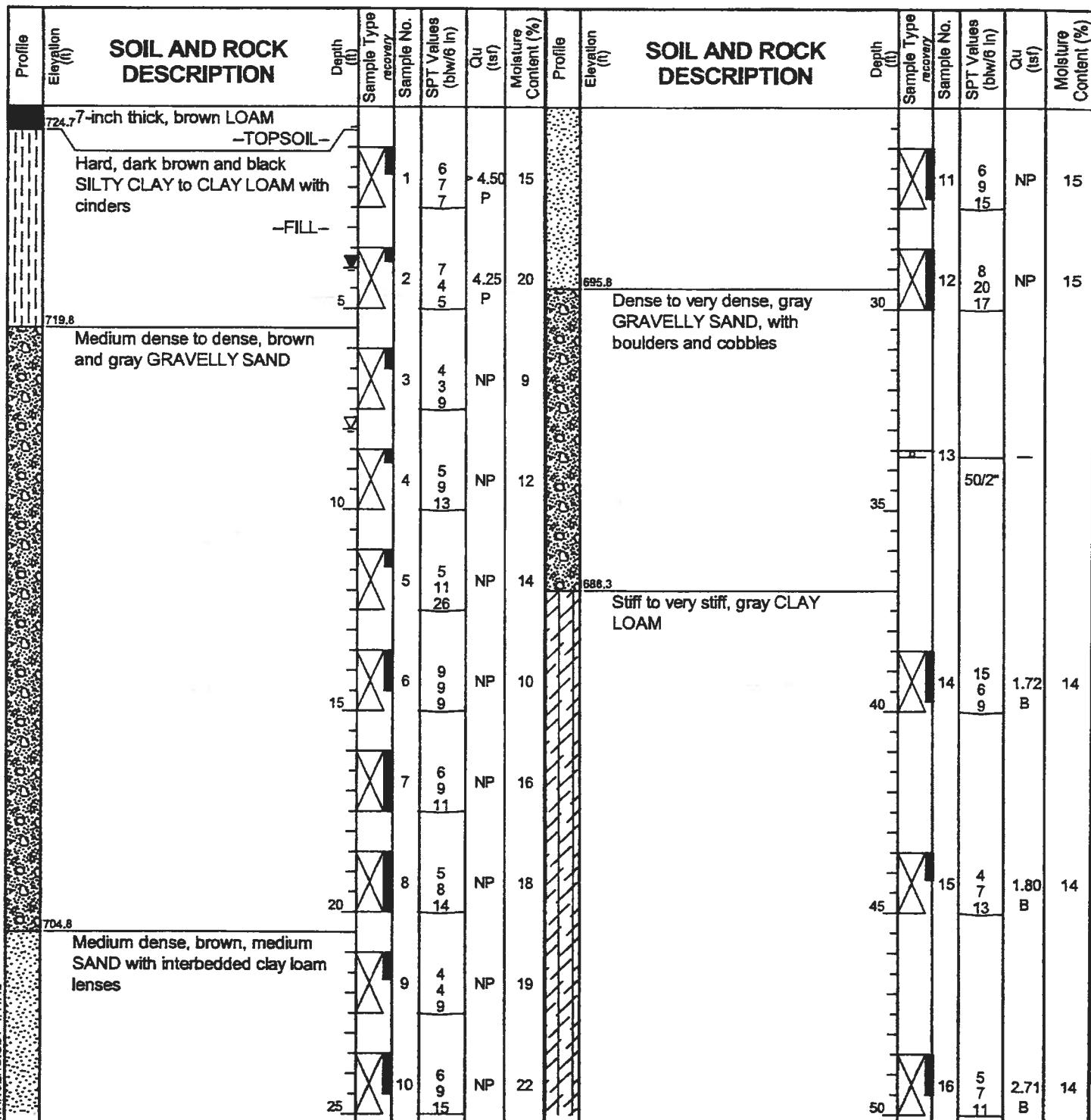
# BORING LOG S-100

WEI Job No.: 201-23-01

Client McDonough Associates Inc.  
Project Longmeadow Parkway over Fox River, Section  
Location Co.

Page: 1 of 2

Datum: NGVD  
Elevation: 725.32 ft  
North: 1993520.74 ft  
East: 999059.38 ft  
Station: 2213+86.07  
Offset: 29.44R



## GENERAL NOTES

Begin Drilling 07-05-2005 Complete Drilling 07-05-2005  
Drilling Contractor PRECON DRILLING Drill Rig CME-75 ATV  
Driller S&R Logger J. Kasnick Checked by B. Fugiel  
Drilling Method 3.25-inch HSA

## WATER LEVEL DATA

While Drilling 8.00 ft  
At Completion of Drilling 4.00 ft  
Time After Drilling NA  
Depth to Water NA



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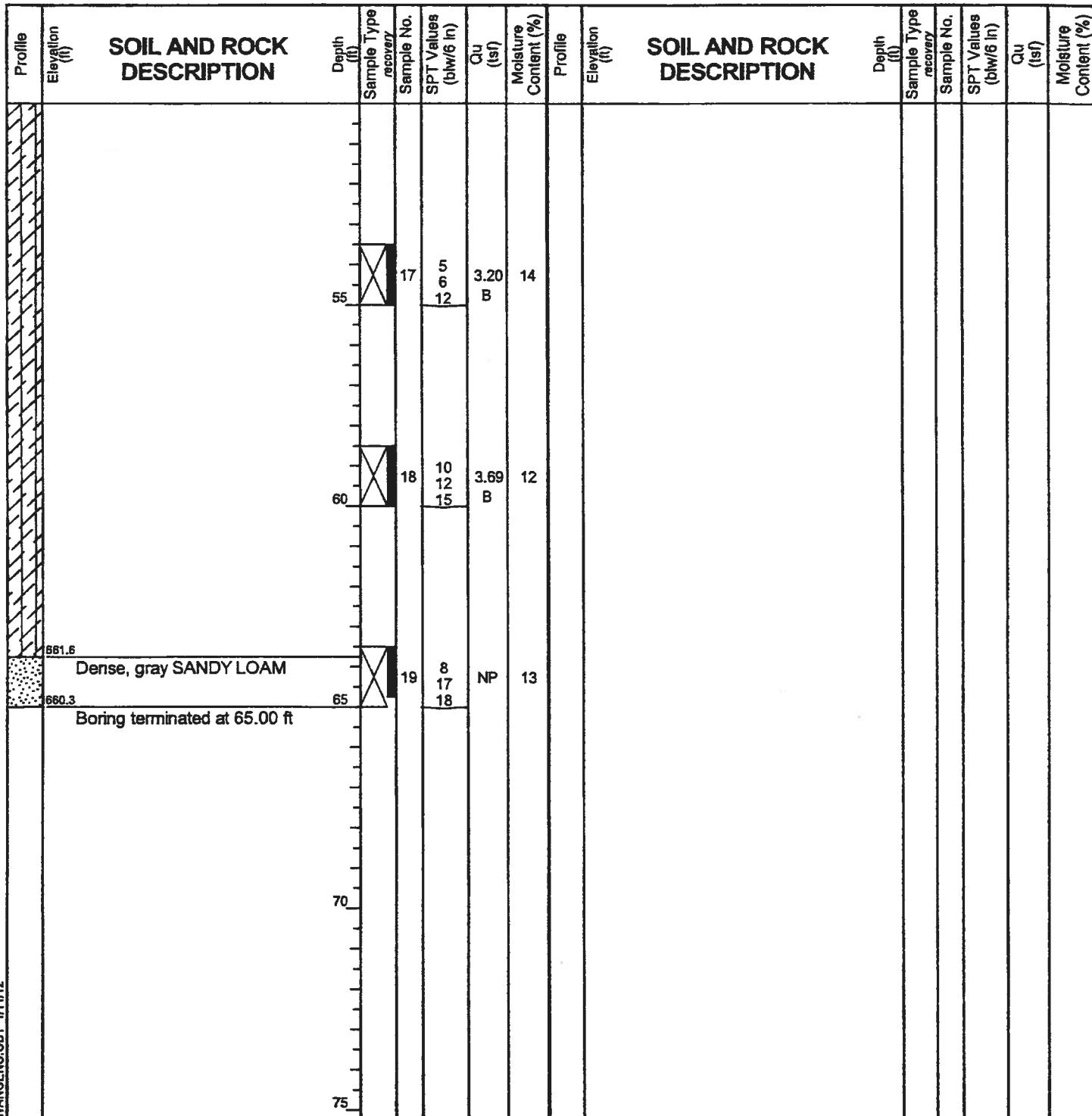
# BORING LOG S-100

WEI Job No.: 201-23-01

Client ..... McDonough Associates Inc.  
Project ..... Longmeadow Parkway over Fox River, Section Co.  
Location .....

Page 2 of 2

Datum: NGVD  
Elevation: 725.32 ft  
North: 1993520.74 ft  
East: 999059.38 ft  
Station: 2213+86.07  
Offset: 29.44R



## GENERAL NOTES

Begin Drilling 07-05-2005 Complete Drilling 07-05-2005  
Drilling Contractor PRECON DRILLING Drill Rig CME-75 ATV  
Driller S&R Logger J. Kasnick Checked by B. Fugiel  
Drilling Method 3.25-inch HSA

## WATER LEVEL DATA

While Drilling 8.00 ft  
At Completion of Drilling 4.00 ft  
Time After Drilling NA  
Depth to Water NA

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



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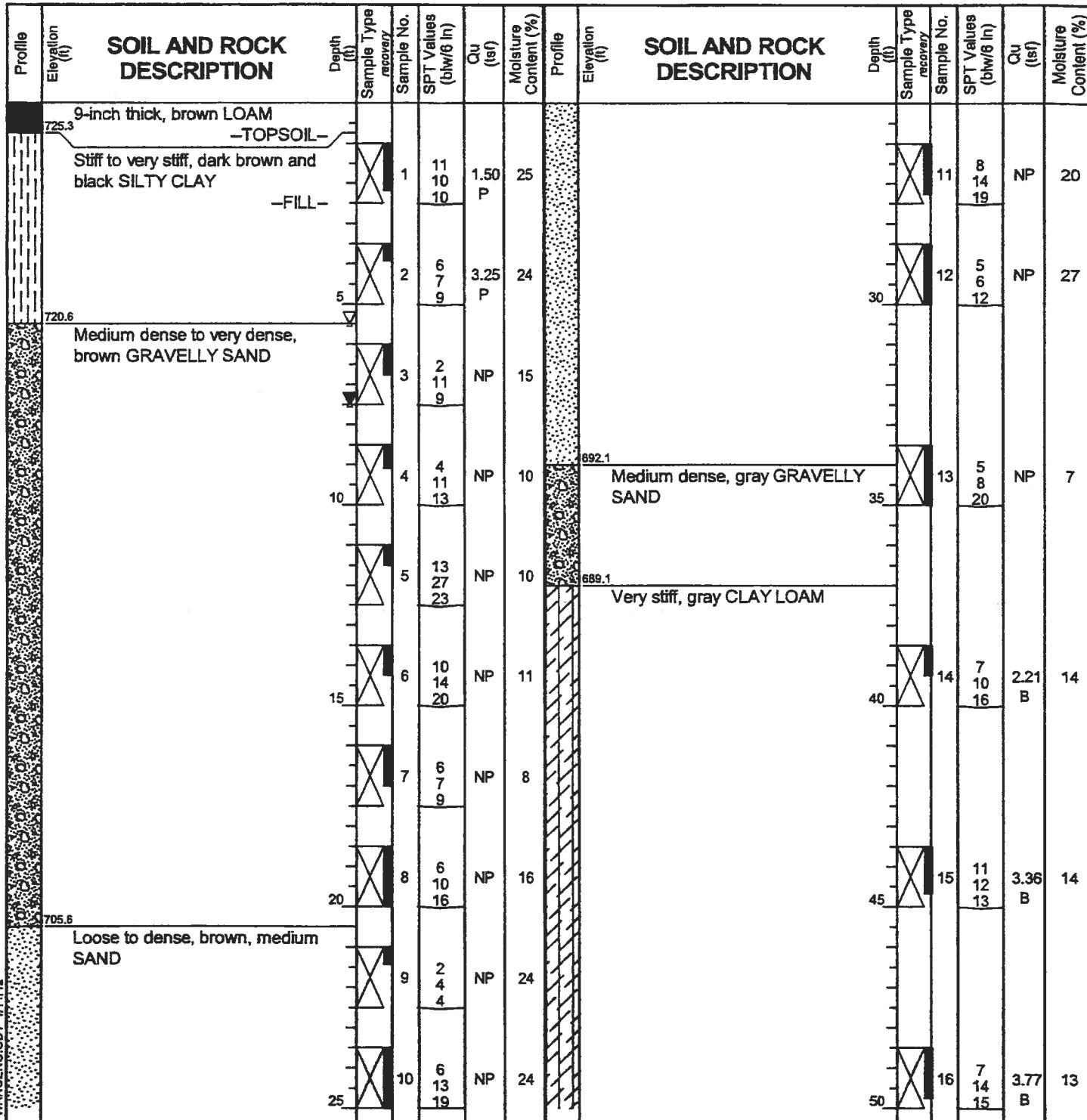
# BORING LOG S-101

WEI Job No.: 201-23-01

Client ..... McDonough Associates Inc.  
Project ..... Longmeadow Parkway over Fox River, Section Co.

Page 1 of 2

Datum: NGVD  
Elevation: 726.07 ft  
North: 1993579.98 ft  
East: 999058.47 ft  
Station: 2213+85.52  
Offset: 29.80L



## GENERAL NOTES

Begin Drilling 07-05-2005 Complete Drilling 07-05-2005  
Drilling Contractor PRECON DRILLING Drill Rig CME-75 ATV  
Driller K&J Logger J. Kasnick Checked by B. Fugiel  
Drilling Method 3.25-inch HSA

## WATER LEVEL DATA

While Drilling 5.50 ft  
At Completion of Drilling 7.50 ft  
Time After Drilling NA  
Depth to Water NA  
The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



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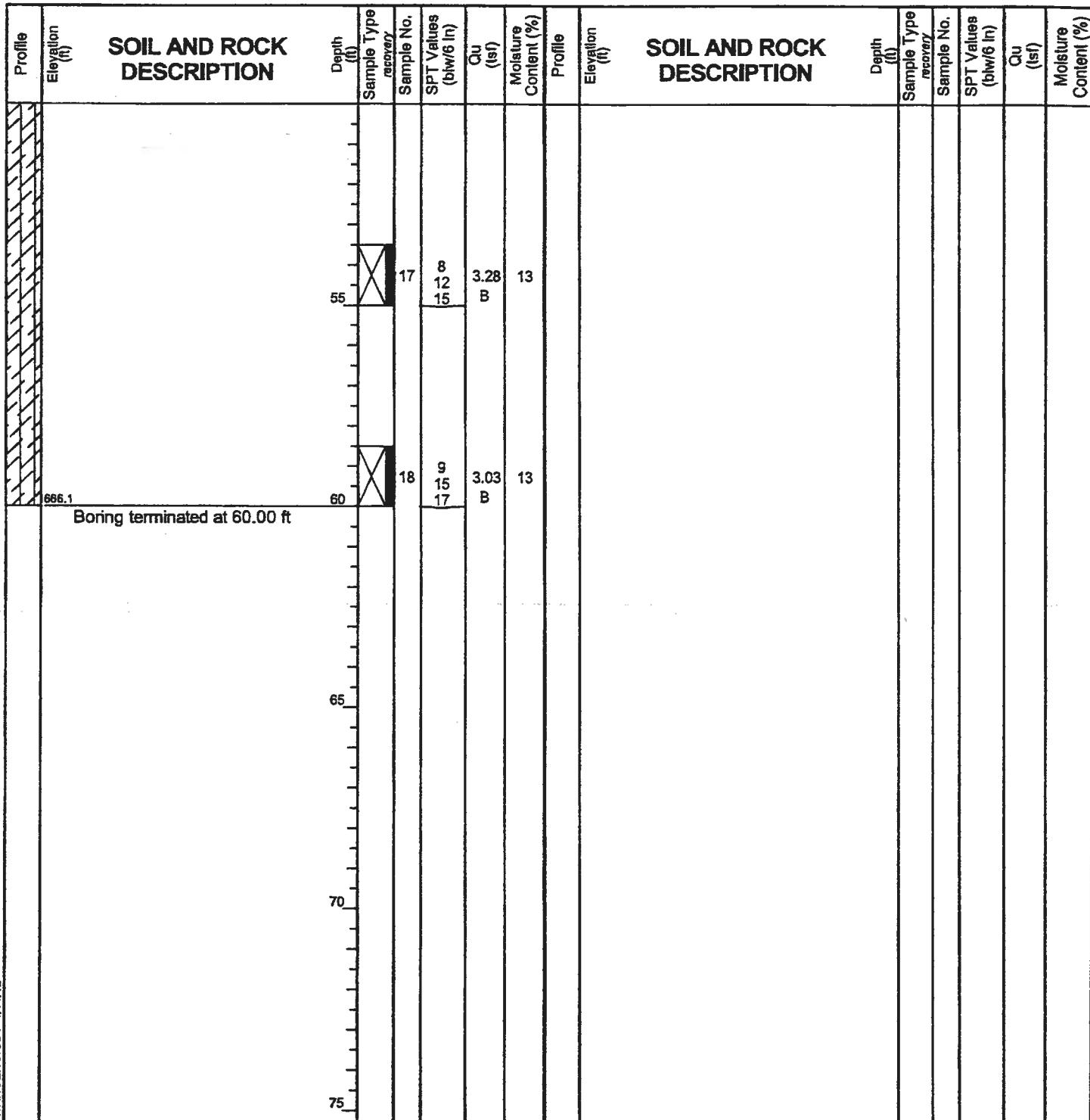
# BORING LOG S-101

WEI Job No.: 201-23-01

Client ..... McDonough Associates Inc.  
Project ..... Longmeadow Parkway over Fox River, Section Co.  
Location .....

Page 2 of 2

Datum: NGVD  
Elevation: 726.07 ft  
North: 1993579.98 ft  
East: 999058.47 ft  
Station: 2213+85.52  
Offset: 29.80L



## GENERAL NOTES

Begin Drilling 07-05-2005 Complete Drilling 07-05-2005  
Drilling Contractor PRECON DRILLING Drill Rig CME-75 ATV  
Driller K&J Logger J. Kasnick Checked by B. Fugiel  
Drilling Method 3.25-inch HSA

## WATER LEVEL DATA

While Drilling 5.50 ft  
At Completion of Drilling 7.50 ft  
Time After Drilling NA  
Depth to Water NA

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



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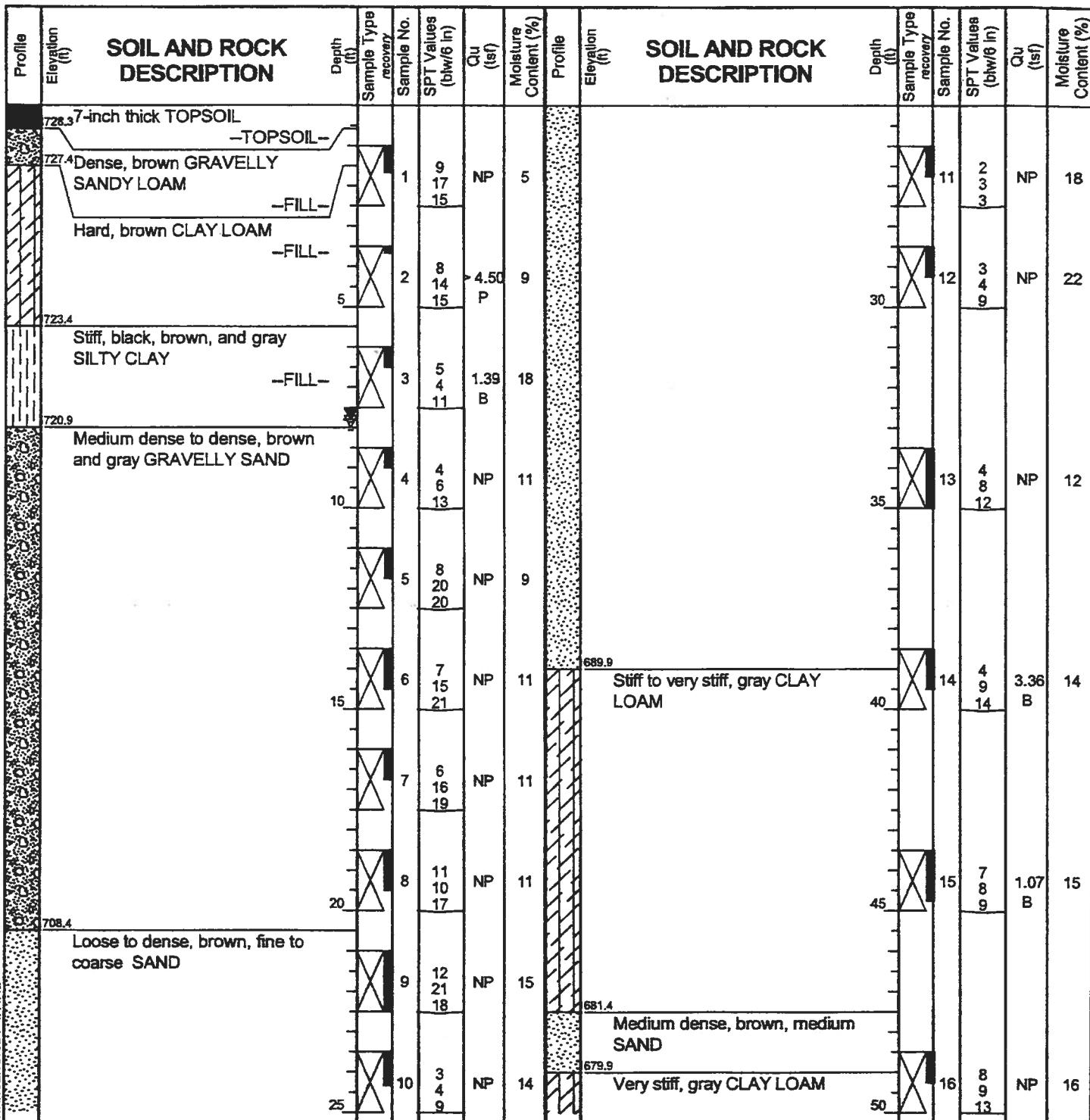
## BORING LOG S-102

WEI Job No.: 201-23-01

Client ..... McDonough Associates Inc.  
Project ..... Longmeadow Parkway over Fox River, Section Co.  
Location .....

Datum: NGVD  
Elevation: 728.90 ft  
North: 1993520.93 ft  
East: 999178.38 ft  
Station: 2215+05.07  
Offset: 29.97R

Page 1 of 2





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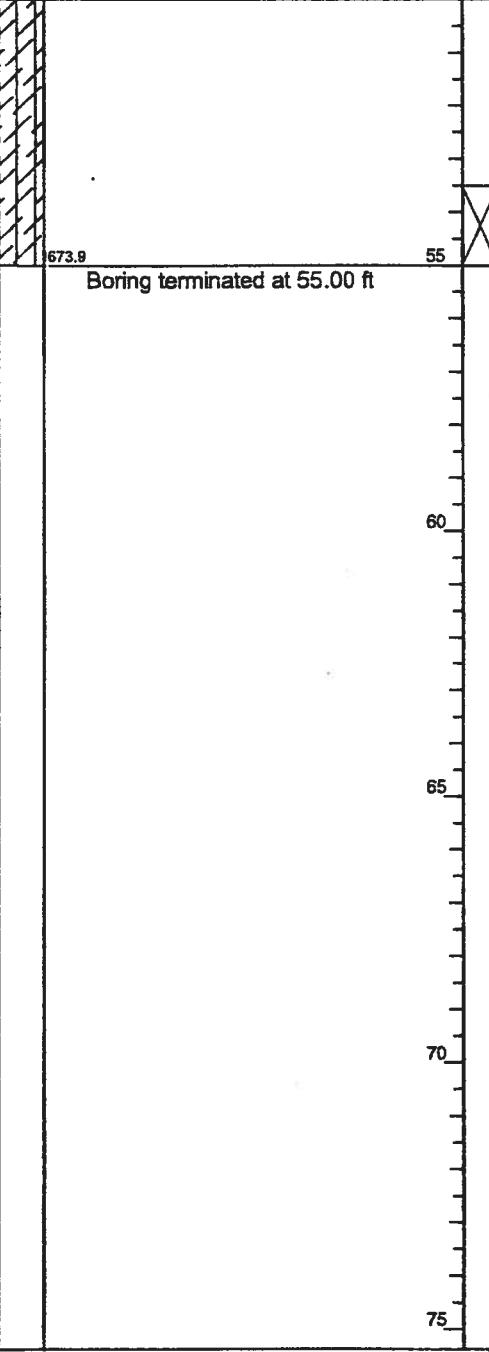
# BORING LOG S-102

WEI Job No.: 201-23-01

Client ..... McDonough Associates Inc.  
Project ..... Longmeadow Parkway over Fox River, Section Co.  
Location .....

Page 2 of 2

Datum: NGVD  
Elevation: 728.90 ft  
North: 1993520.93 ft  
East: 999178.38 ft  
Station: 2215+05.07  
Offset: 29.97R

Profile Ele (ft)	SOIL AND ROCK DESCRIPTION							Profile Ele (ft)	SOIL AND ROCK DESCRIPTION						
	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (bhw/in)	Qu (st)	Moisture Content (%)	Profile Ele (ft)	Depth (ft)	Sample Type recovery	Sample No.	SPT Values (bhw/in)	Qu (st)	Moisture Content (%)		
															
	673.8														
	Boring terminated at 55.00 ft														
	55														
	60														
	65														
	70														
	75														
GENERAL NOTES								WATER LEVEL DATA							
Begin Drilling	07-06-2005	Complete Drilling	07-06-2005					While Drilling	▽	8.00	ft				
Drilling Contractor	PRECON DRILLING	Drill Rig	CME-75 ATV					At Completion of Drilling	▽	7.80	ft				
Driller	S&J	Logger	J. Kasnick	Checked by	B. Fugiel			Time After Drilling	NA						
Drilling Method	3.25-inch HSA							Depth to Water	▽	NA					
								The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.							



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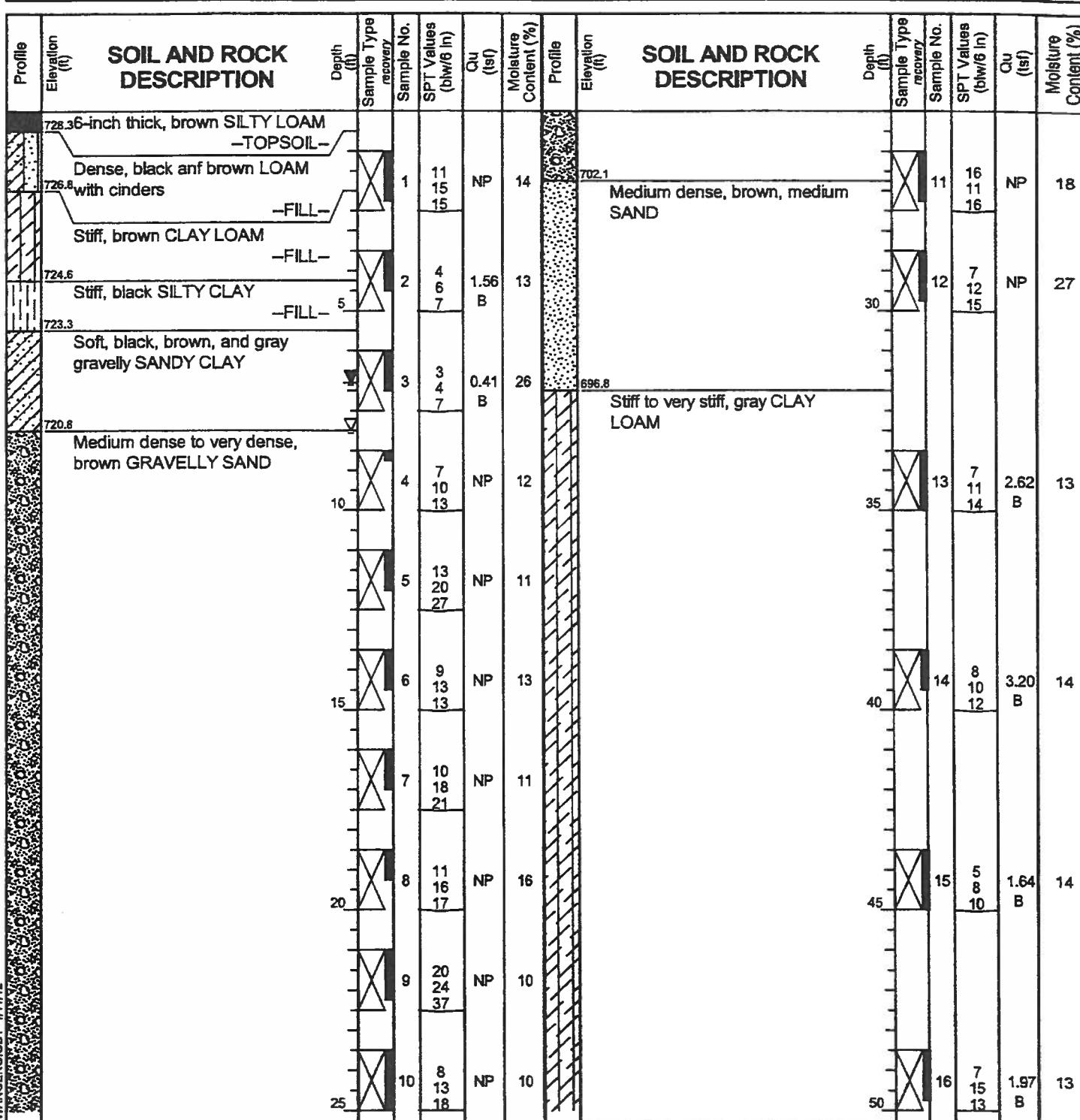
# BORING LOG S-103

WEI Job No.: 201-23-01

Client McDonough Associates Inc.  
Project Longmeadow Parkway over Fox River, Section  
Location Co.

Page 1 of 2

Datum: NGVD  
Elevation: 728.83 ft  
North: 1993580.85 ft  
East: 999178.46 ft  
Station: 2215+05.51  
Offset: 29.95L



## GENERAL NOTES

Begin Drilling 07-05-2005 Complete Drilling 07-06-2005  
Drilling Contractor PRECON DRILLING Drill Rig CME-75 ATV  
Driller K&J Logger J. Kasnick Checked by B. Fugiel  
Drilling Method 3.25-inch HSA

## WATER LEVEL DATA

While Drilling 8.00 ft  
At Completion of Drilling 6.80 ft  
Time After Drilling NA  
Depth to Water NA  
The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



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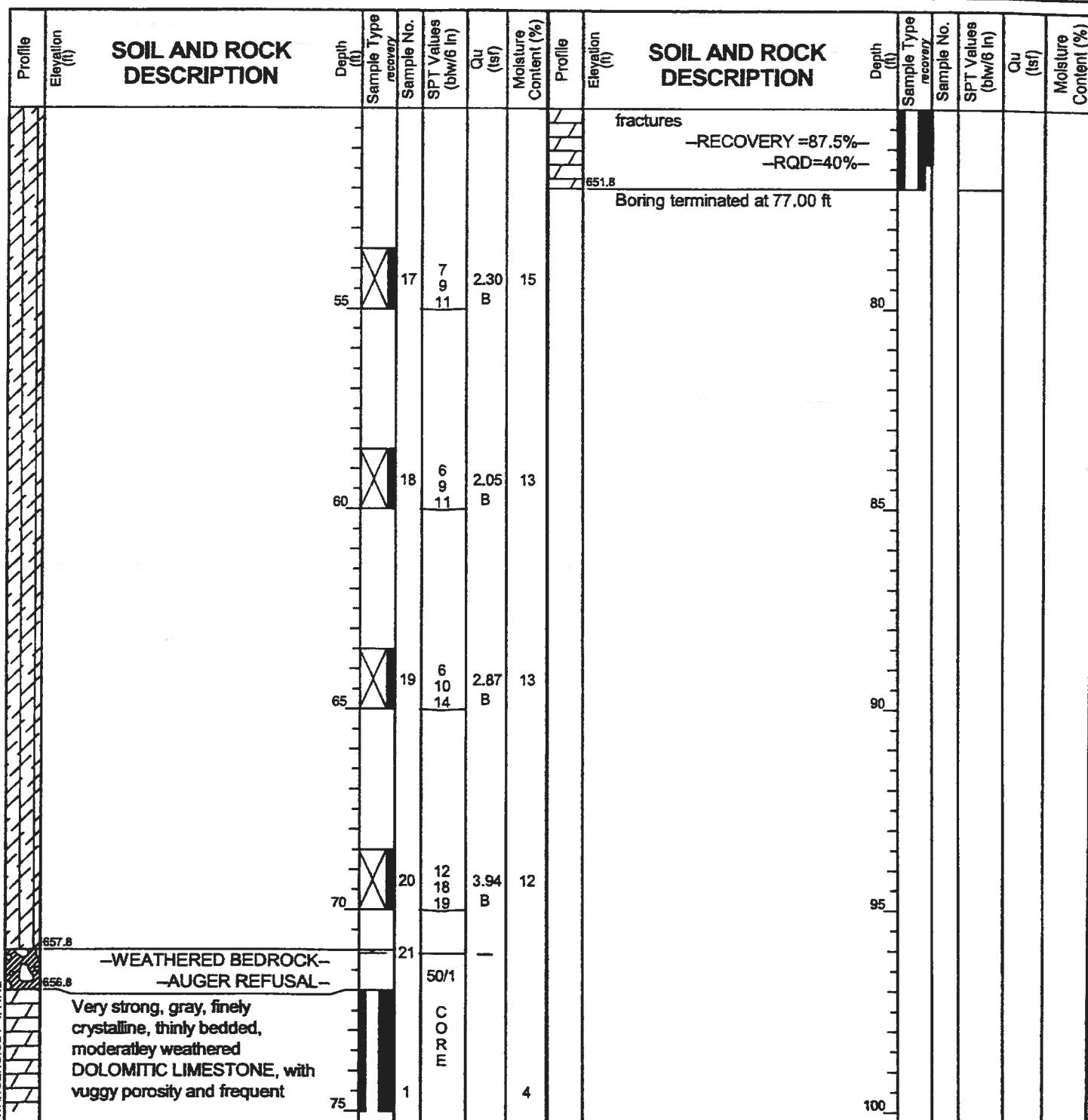
# BORING LOG S-103

WEI Job No.: 201-23-01

Client ..... McDonough Associates Inc.  
Project: Longmeadow Parkway over Fox River, Section Co.

Page 2 of 2

Datum: NGVD  
Elevation: 728.83 ft  
North: 1993580.85 ft  
East: 999178.46 ft  
Station: 2215+05.51  
Offset: 29.95L





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**BORING LOG S-104**

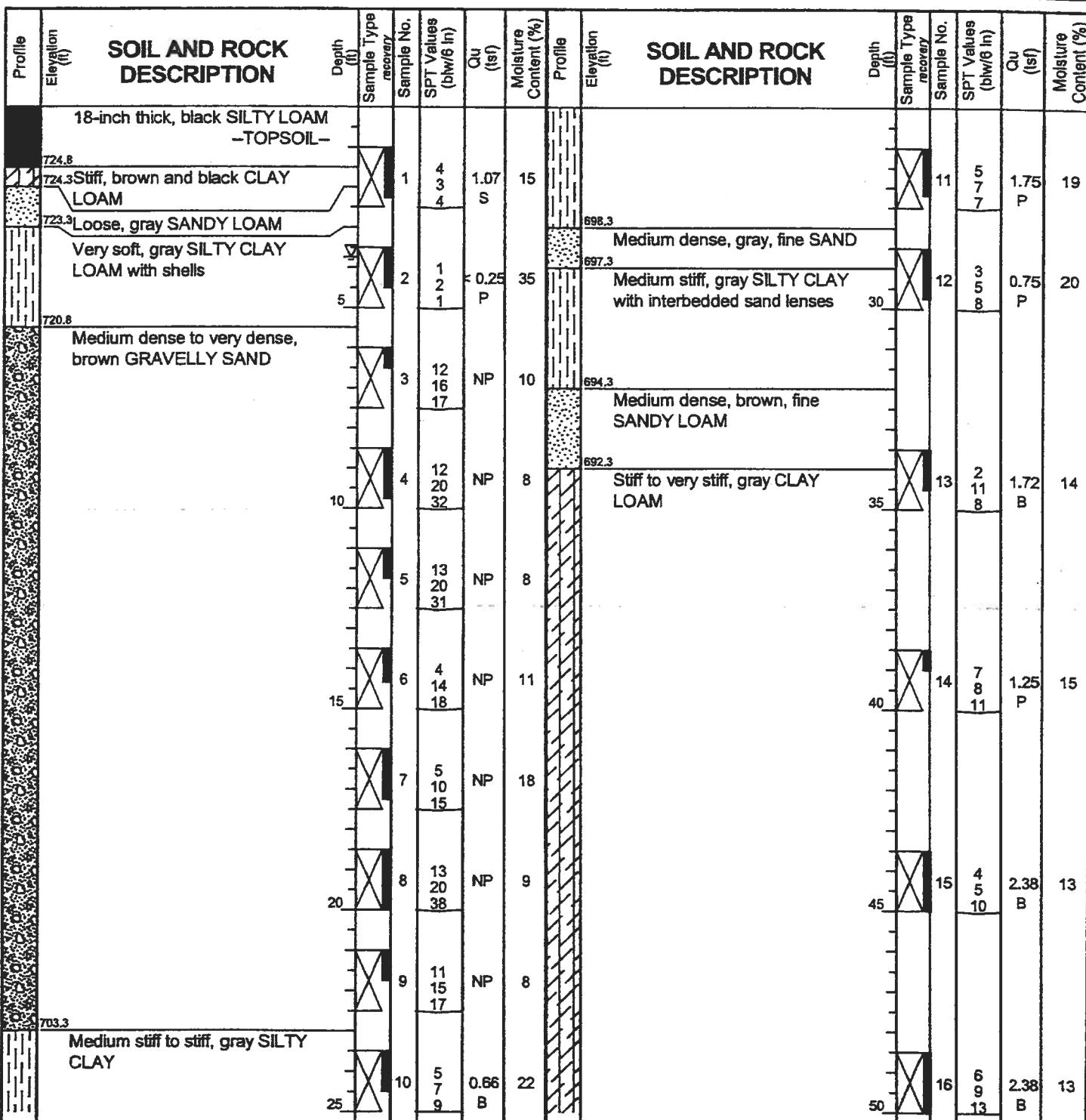
**WEI Job No.: 201-23-01**

**Client .....** **McDonough Associates Inc.**

**Project** **Longmeadow Parkway over Fox River, Section**

**Location** **Co.**

Page 1 of 2



## **GENERAL NOTES**

Begin Drilling 07-07-2005 Complete Drilling 07-08-2005  
Drilling Contractor PRECON DRILLING Drill Rig CME-75 ATV  
Driller S&J Logger J. Kasnick Checked by B. Fugiel  
Drilling Method 3.25-inch HSA

## **WATER LEVEL DATA**

While Drilling	<input checked="" type="checkbox"/>	3.75 ft
At Completion of Drilling	<input checked="" type="checkbox"/>	DRY
Time After Drilling	<input type="checkbox"/>	NA
Depth to Water	<input checked="" type="checkbox"/>	NA

The stratification lines represent the approximate boundary between soils from the actual test position and the



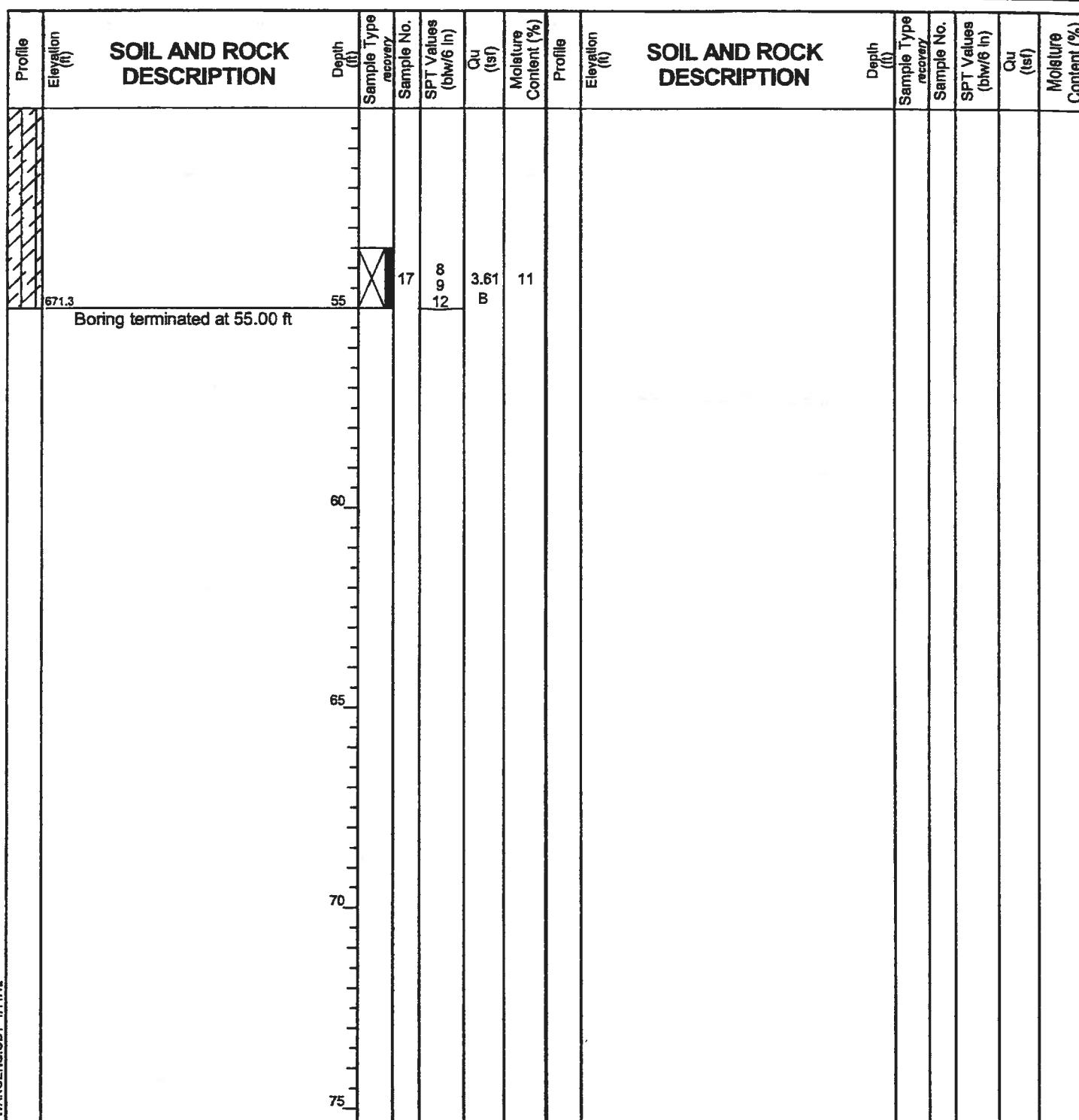
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# BORING LOG S-104

Page 2 of 2

WEI Job No.: 201-23-01  
Client McDonough Associates Inc.  
Project Longmeadow Parkway over Fox River, Section  
Location Co.

Datum: NGVD  
Elevation: 726.33 ft  
North: 1993518.51 ft  
East: 999300.82 ft  
Station: 2216+27.49  
Offset: 33.12R



## GENERAL NOTES

Begin Drilling 07-07-2005 Complete Drilling 07-08-2005  
Drilling Contractor PRECON DRILLING Drill Rig CME-75 ATV  
Driller S&J Logger J. Kasnick Checked by B. Fugiel  
Drilling Method 3.25-inch HSA

## WATER LEVEL DATA

While Drilling ▽ 3.75 ft  
At Completion of Drilling ▽ DRY  
Time After Drilling NA  
Depth to Water ▽ NA

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



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Fax 630-853-9938

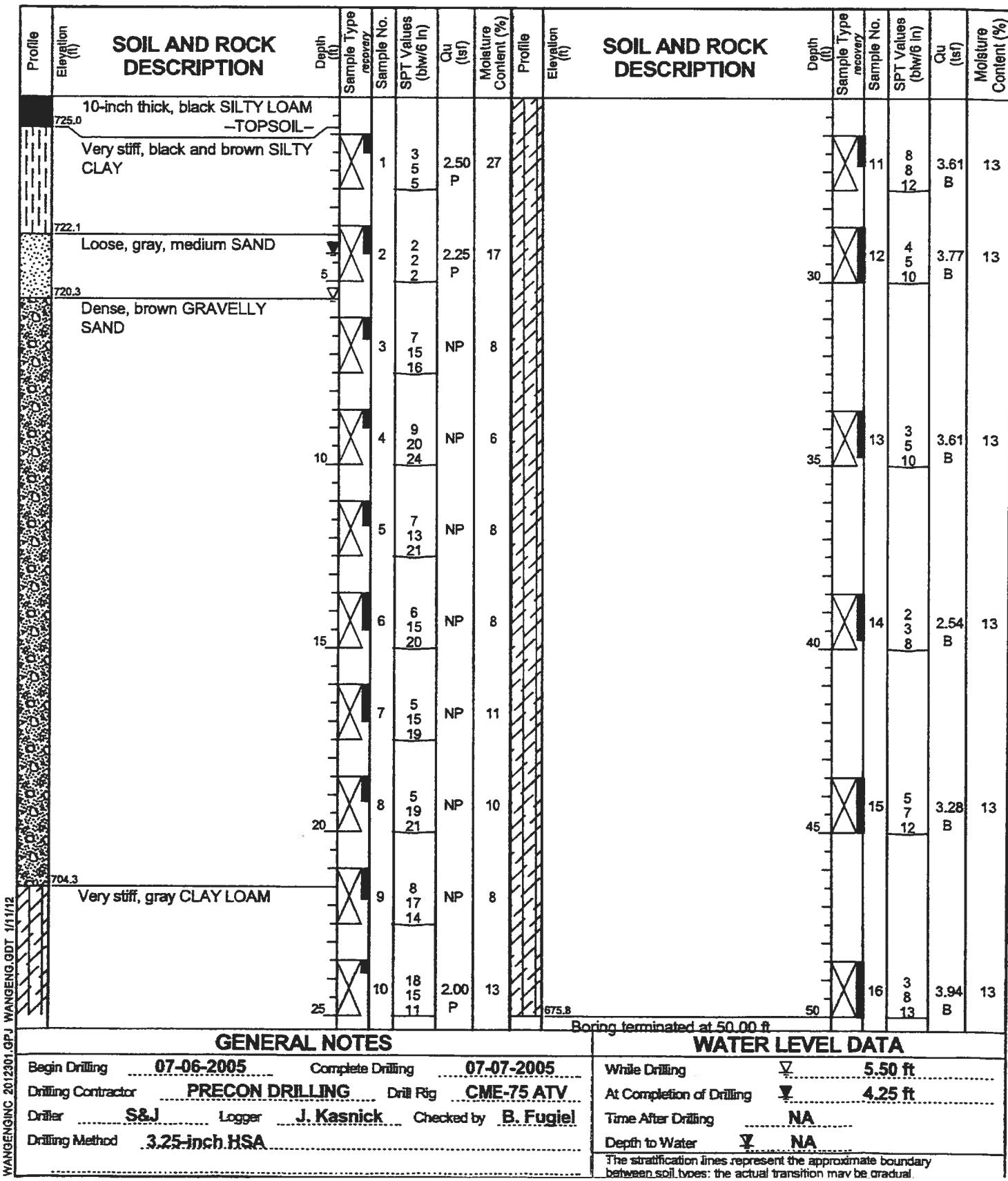
# BORING LOG S-105

WEI Job No.: 201-23-01

Client McDonough Associates Inc.  
Project Longmeadow Parkway over Fox River, Section  
Location Co.

Page 1 of 1

Datum: NGVD  
Elevation: 725.81 ft  
North: 1993581.79 ft  
East: 999298.29 ft  
Station: 2216+25.34  
Offset: 30.17L





## **BORING LOG S-106**

Page 1 of 1

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Telephone: 630-953-9928  
Fax 630-953-9938

**Client** ..... **McDonough Associates Inc.**  
**Project** ..... **Longmeadow Parkway over Fox River, Section**  
**Location** ..... **Co.**

Datum: NGVD  
Elevation: 731.32 ft  
North: 1993523.14 ft  
East: 999417.36 ft  
Station: 2217+44.06  
Offset: 29.19R

**SOIL AND ROCK DESCRIPTION**

Profile	Elevation (ft)	Depth (ft)	Type	Sample No.	SPT Values (blow/6 in)	Qu (lsf)	Moisture Content (%)	Profile	Elevation (ft)	Depth (ft)	Type	Sample No.	SPT Values (blow/6 in)	Qu (lsf)	Moisture Content (%)	
	731.04	0-4	-TOPSOIL-								-TOPSOIL-					
	728.3	4-11	Medium dense, brown and black SILTY LOAM -FILL-	1	6 5 5	NP	11		703.3	11-18	Stiff to hard, gray CLAY LOAM	11	21 12 8	NP	11	
	725.8	11-19	Very stiff, dark brown SILTY CLAY LOAM with cinders and brick -FILL-	2	5 7 6	3.50 P	19			30		12	6 7 12	2.79 B	14	
	723.3	19-27	Stiff, brown and gray SILTY CLAY with sand interbeds	3	6 8 9	1.64 B	16			35		13	6 8 12	4.67 B	13	
	713.3	27-35	Dense to very dense, brown GRAVELLY SAND, with cobbles	4	8 23 29	NP	8			40		14	6 9 14	2.30 B	15	
	710.8	35-45	Medium dense, brown SAND	5	27 27 31	NP	10			45		15	5 6 12	1.97 B	13	
		45-50	Medium dense to very dense, brown and gray GRAVELLY SAND	6	28 26 34	NP	8		681.3		16	8 10 14	3.20 B	14		
				7	14 16 20	NP	16									
				8	6 9 12	NP	20									
				9	26 49 25	NP	9									
				10	32 17 15	NP	11									

Boring terminated at 50.00 ft

## **GENERAL NOTES**

## **WATER LEVEL DATA**

Begin Drilling 07-07-2005

Complete Drilling 07-07-2005

While Drilling

**10.50 ft**

Drilling Contractor PRECON DRILLING

Drill Rig CME-75 ATV

**At Completion of Drilling**

**9.00 ft**

Driller K

Logger J. Kas

ick Checked by B. Fugie

### Time After Drilling

• • • • •

Drilling Method 3.25-inch HSA

**Depth to Water**

.....

Digitized by srujanika@gmail.com

## The stratification between soil types

estimate bounds may be cracked.

10. The following table shows the number of hours worked by each employee in a company.

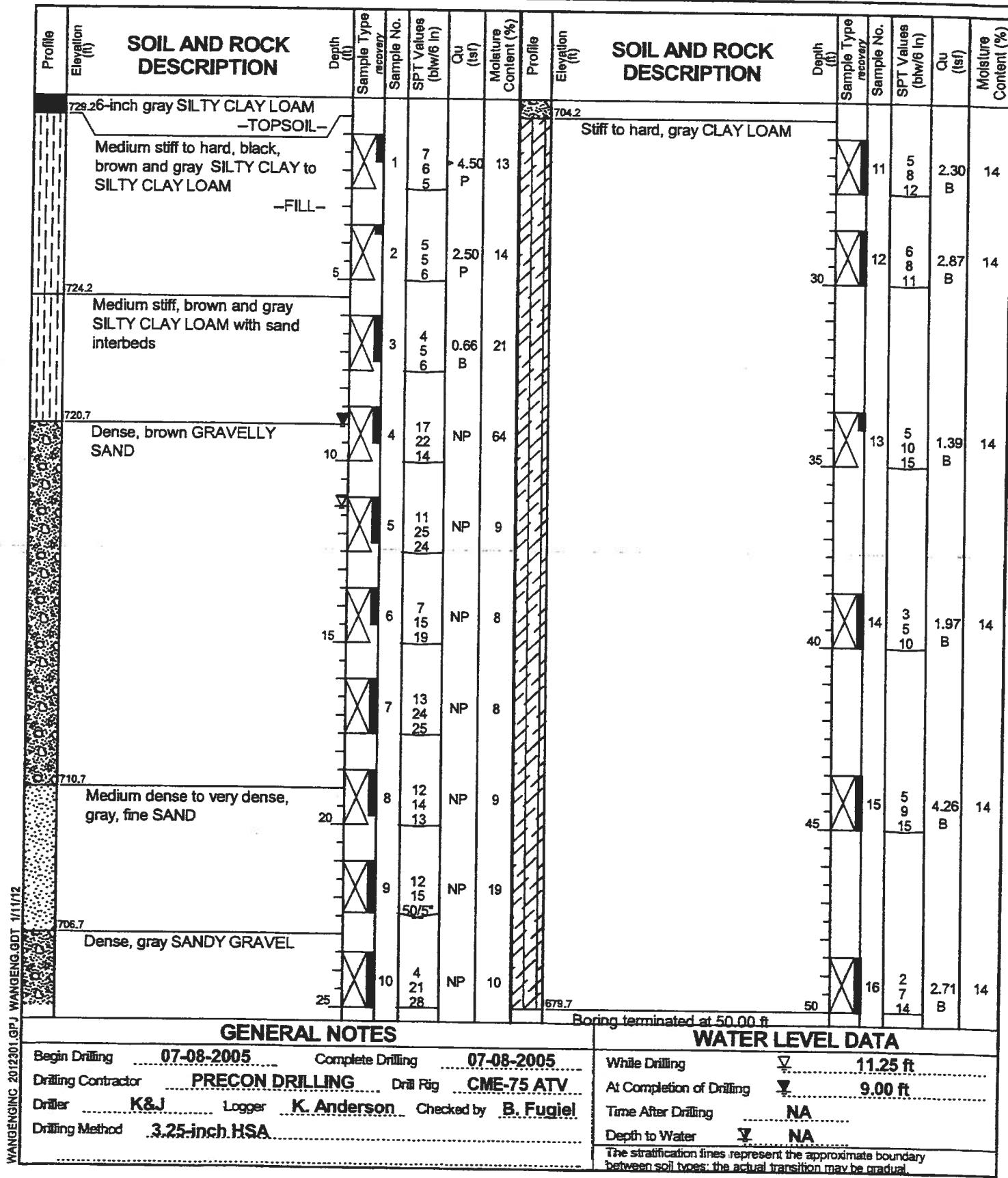


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# BORING LOG S-107

WEI Job No.: 201-23-01  
Client McDonough Associates Inc.  
Project Longmeadow Parkway over Fox River, Section  
Location Co.

Datum: NGVD  
Elevation: 729.66 ft  
North: 1993582.29 ft  
East: 999418.53 ft  
Station: 2217+45.58  
Offset: 29.94L





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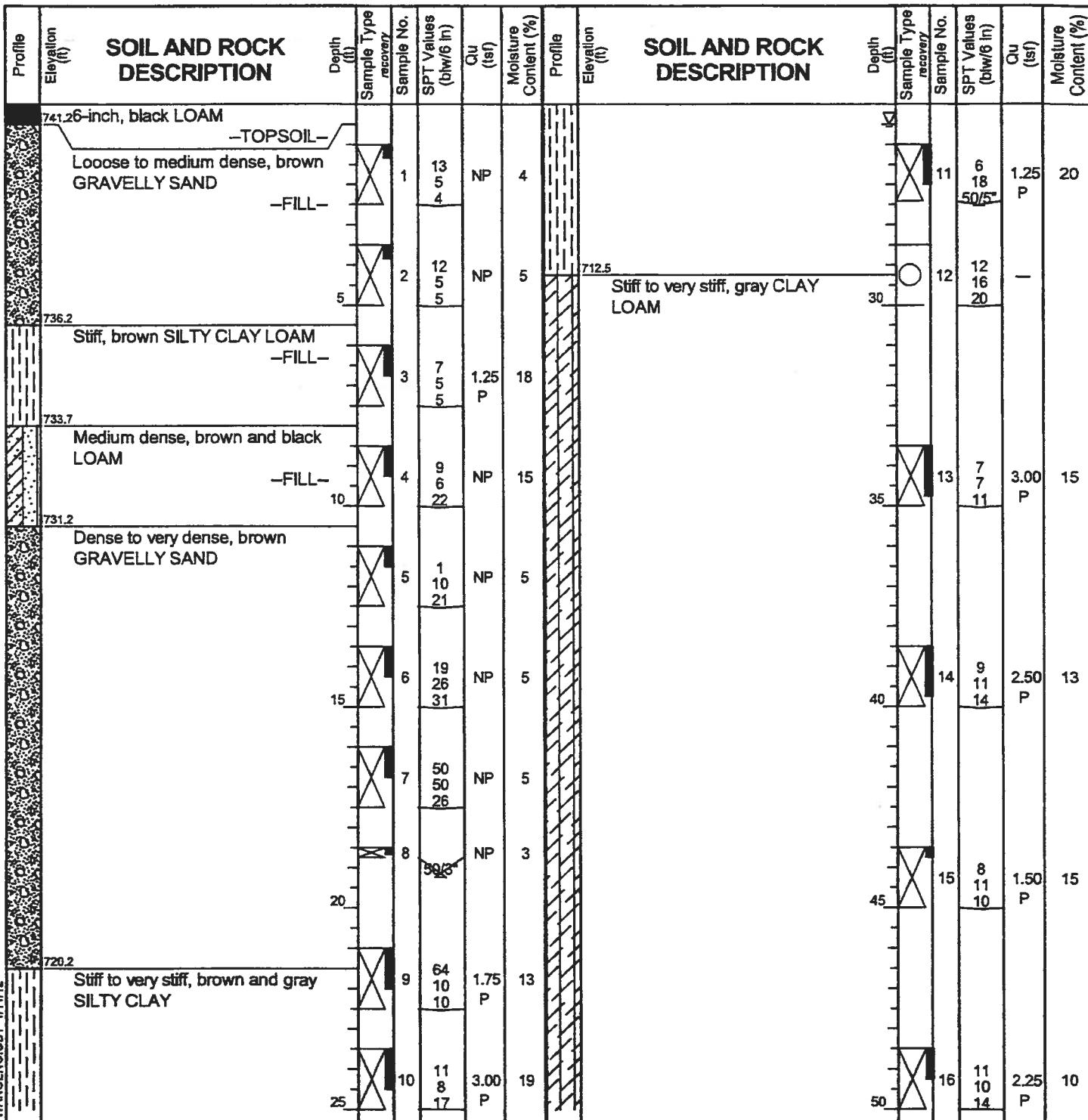
# BORING LOG S-108

WEI Job No.: 201-23-01

Client McDonough Associates Inc.  
Project Longmeadow Parkway over Fox River, Section  
Location Co.

Page 1 of 2

Datum: NGVD  
Elevation: 741.75 ft  
North: 1993522.46 ft  
East: 999538.07 ft  
Station: 2218+64.88  
Offset: 30.31R



## GENERAL NOTES

Begin Drilling 07-11-2005 Complete Drilling 07-11-2005  
 Drilling Contractor PRECON DRILLING Drill Rig CME-75 ATV  
 Driller J&L Logger K. Anderson Checked by B. Fugiel  
 Drilling Method 3.25-inch HSA

## WATER LEVEL DATA

While Drilling □ 25.50 ft

At Completion of Drilling □ DRY

Time After Drilling NA

Depth to Water □ NA

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



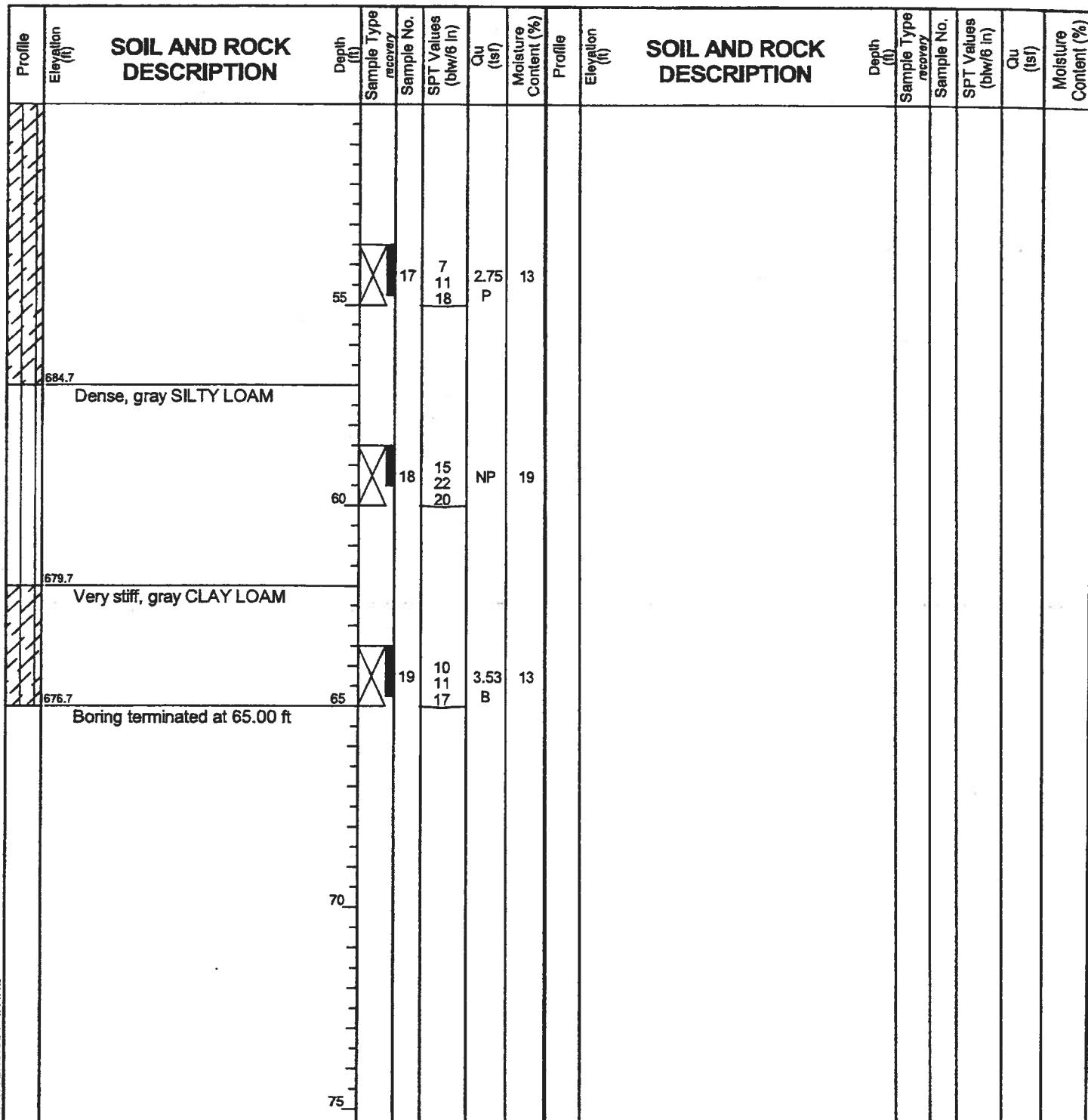
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# BORING LOG S-108

Page 2 of 2

WEI Job No.: 201-23-01  
Client McDonough Associates Inc.  
Project Longmeadow Parkway over Fox River, Section  
Location Co.

Datum: NGVD  
Elevation: 741.75 ft  
North: 1993522.46 ft  
East: 999538.07 ft  
Station: 2218+64.88  
Offset: 30.31R



## GENERAL NOTES

Begin Drilling 07-11-2005 Complete Drilling 07-11-2005  
Drilling Contractor PRECON DRILLING Drill Rig CME-75 ATV  
Driller J&L Logger K. Anderson Checked by B. Fugiel  
Drilling Method 3.25-inch HSA

## WATER LEVEL DATA

While Drilling □ 25.50 ft  
At Completion of Drilling □ DRY  
Time After Drilling NA  
Depth to Water □ NA

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



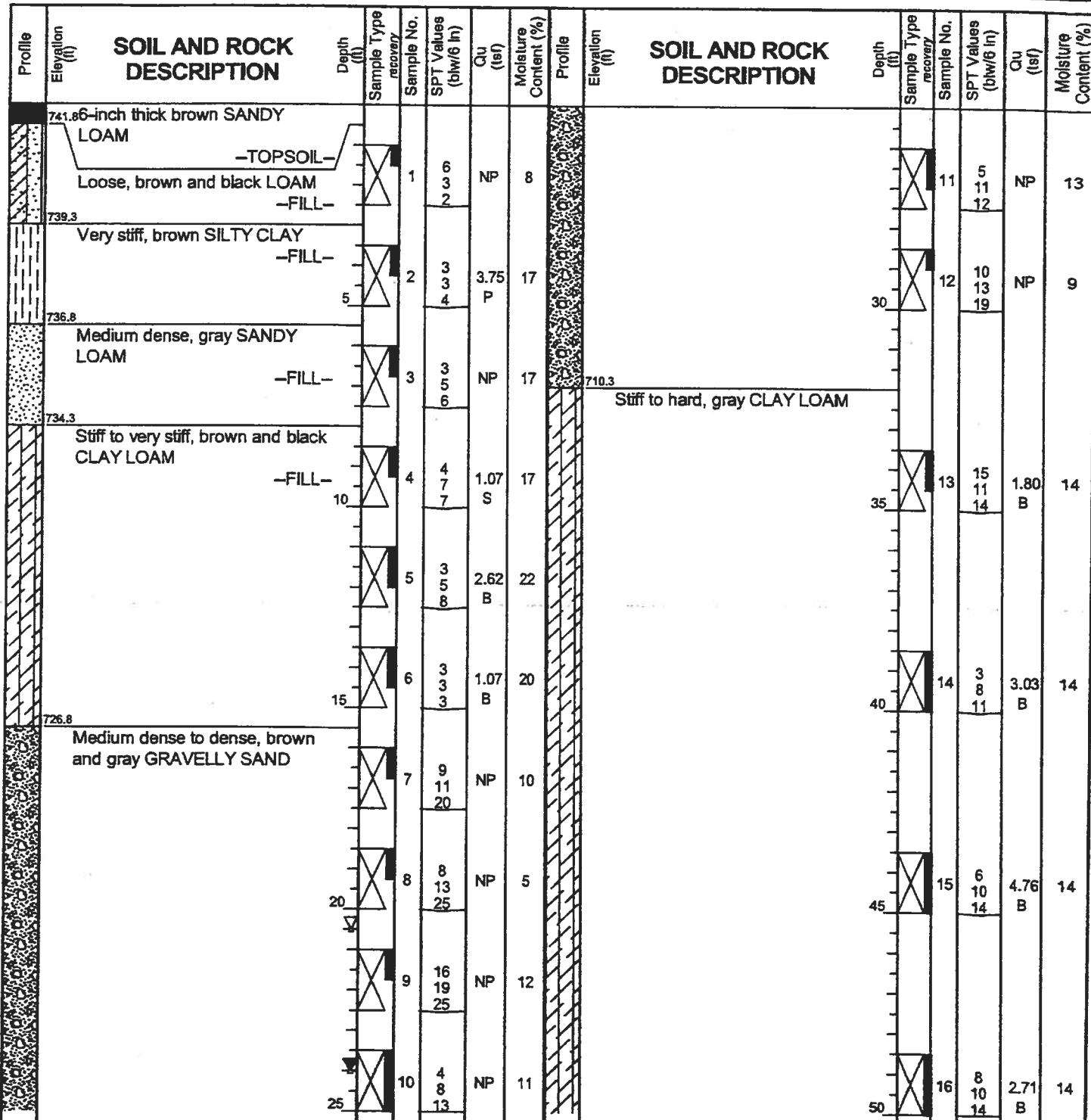
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# BORING LOG S-109

WEI Job No.: 201-23-01  
Client ..... McDonough Associates Inc.  
Project Longmeadow Parkway over Fox River, Section Co.  
Location .....

Datum: NGVD  
Elevation: 742.29 ft  
North: 1993589.19 ft  
East: 999538.22 ft  
Station: 2218+65.17  
Offset: 36.42L

Page 1 of 2



## GENERAL NOTES

Begin Drilling 07-08-2005 Complete Drilling 07-11-2005  
Drilling Contractor PRECON DRILLING Drill Rig CME-75 ATV  
Driller K&J Logger K. Anderson Checked by B. Fugiel  
Drilling Method 3.25-inch HSA

## WATER LEVEL DATA

While Drilling 20.50 ft  
At Completion of Drilling 24.00 ft  
Time After Drilling NA  
Depth to Water NA

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.



## **BORING LOG S-109**

Page 2 of 2

WEI Job No.: 201-23-01

McDonough Associates

Bolz Road

## **Kane County, Illinois**

Datum: NGVD  
Elevation: 742.29 ft  
North: 1993589.19 ft  
East: 999538.22 ft  
Station: 2218+65.17  
Offset: 36.42L

## **GENERAL NOTES**

## **WATER LEVEL DATA**

Begin Drilling	<b>07-08-2005</b>	Complete Drilling	<b>07-11-2005</b>	While Drilling	<b>20.50 ft</b>
Drilling Contractor	<b>PRECON DRILLING</b>	Drill Rig	<b>CME-75 ATV</b>	At Completion of Drilling	<b>24.00 ft</b>
Driller	<b>K&amp;J</b>	Logger	<b>K. Anderson</b>	Checked by	<b>B. Fugiel</b>
Drilling Method	<b>3.25-inch HSA</b>			Time After Drilling	<b>NA</b>
				Depth to Water	<b>NA</b>
The stratification lines represent the approximate boundary					

The stratification lines represent the approximate boundary between soil types; the actual transition may be gradual.

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