

LOCATION MAP SCALE IN MILES



LEG	END:
വ	UTILITY POLE
$\ominus$	GUY WIRE ANCH
¢	UTILITY POLE
	MAILBOX
$\oplus$	CATCH BASIN
W	WATER VALVE
*රි	WATER SHUTOFF
ЪС	HYDRANT
*	FLOODLIGHT
	ELECTRIC METE
,	CON. SHRUB
$\bigcirc$	DEC. SHRUB

ND:		
UTILITY POLE	— GAS —	GAS LINE
GUY WIRE ANCHOR		SEWER
UTILITY POLE W/LIGHT	W	WATER
MAILBOX	OE	OVERHEAD UTILITY WIRES
CATCH BASIN	0	WOOD FENCE
WATER VALVE	-0000000-	STONE WALL
WATER SHUTOFF		CHAIN LINK FENCE
HYDRANT		EXISTING EDGE OF PAVEMENT
FLOODLIGHT		EDGE OF TRAVEL WAY
ELECTRIC METER	— D W — –	WETLANDS
CON. SHRUB	$\sim\sim\sim\sim$	TREELINE
DEC. SHRUB	·	APPROX. PROPERTY LINE
SIGNS	• • •	EDGE OF CHANNEL
MANHOLE (SEWER, TEL)		CUT SLOPE
TREES		FILL SLOPE
	— — ТОВ—	TOP OF BANK

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TOWN OF AMHERST HILLSBOROUGH COUNTY NEW HAMPSHIRE



PLANS OF THE PROPOSED REPLACEMENT OF THE MANCHESTER ROAD BRIDGE OVER BEAVER BROOK (BR. NO. 134/100) NHDOT PROJECT NO. 20242 JULY 2014

A RECORD DRAWINGS

I HEREBY CERTIFY THAT TO THE BEST OF MY KNOWLEDGE, BASED UPON CONSTRUCTION RECORDS, THAT ALL WORK SHOWN IN THESE DRAWINGS HAS BEEN COMPLETED AS SHOWN.

HOYLE, TANNER & ASSOCIATES, INC.

SIGNED Rester **MARCH 2016** 

**NOTE: WORDING CONTAINED HEREON** IS UNDERSTOOD TO BE PAST TENSE.

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40	PROJECT NOTES (1 OF 2)	ATE	2015 2016		lain th inated Tanne
40	SUMMARY OF QUANTITIES	۵	3/3/		I rem ssemi ther yle, -
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40	DRIVEWAY DETAILS				of se ed, r ronic
40	ROADWAY GENERAL PLAN				nent be us elect
40	ROADWAY PROFILE (1 OF 2)		NO		istrur not l ding the w
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40	TRAFFIC CONTROL PLAN PHASE 2	DES	N R		epare anne / mai
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40	SUBSTRUCTURE REINFORCING DETAILS				H H H H H
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40	TYPICAL BRIDGE SECTIONS			WN E	Ю. B Ш
40 40	SUPERSTRUCTURE PLAN AND DETAILS DECK REINFORCING PLAN AND DETAILS			DRA	SCAL CHK
40	PRESTRESSED DECK BEAM DETAILS (1 OF 4)			<u>l</u>	<u> </u>
40	PRESTRESSED DECK BEAM DETAILS (2 OF 4)				<u>.</u>
40	PRESTRESSED DECK BEAM DETAILS (3 OF 4) PRESTRESSED DECK BEAM DETAILS (4 OF 4)				227 58
40	BRIDGE RAIL LAYOUT PLAN		$\tilde{}$		10 11 11
40	T4 BRIDGE AND APPROACH RAIL (STEEL POST)			<b>`</b>	031 031 665 er.cc
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	<u>General notes</u>	
(1)	GENERAL NOTES SHALL APPLY TO ALL DRAWINGS PREPARED BY HOYLE, TANNER & ASSOCIATES (Hoyle, Tanner) and the proposed work they convey.	(1)
(2)	ALL WORK SHALL CONFORM TO ALL FEDERAL, STATE AND LOCAL CODES, REGULATIONS AND	(2)
(3)	THE GENERAL CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY	(4)
(4)	RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS AND COORDINATION OF OTHER TRADES.	
	CARE OF ADJACENT PROPERTIES DURING CONSTRUCTION AND COMPLIANCE WITH STATE AND FEDERAL REGULATIONS REGARDING SITE SAFETY SHALL SOLELY BE THE CONTRACTORS RESPONSIBILITY.	(5)
(5)	ALL DIMENSIONS, ELEVATIONS AND CONDITIONS MUST BE VERIFIED BY THE GENERAL CONTRACTOR OR RESPONSIBLE TRADES PRIOR TO COMMENCING WITH THE WORK, FABRICATION OR ORDERING MATERIALS, DO NOT SCALE DRAWINGS, USE DIMENSIONS SHOWN.	
(6)	ANY DISCREPANCIES BETWEEN THESE DRAWINGS AND EXISTING CONDITIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER IMMEDIATELY, BEFORE PROCEEDING WITH THE WORK.	(6)
(7)	THE INFORMATION SHOWN ON THESE PLANS CONCERNING THE TYPE AND LOCATION OF UNDERGROUND UTILITIES IS NOT GUARANTEED TO BE ACCURATE OR ALL-INCLUSIVE. THE CONTRACTOR IS RESPONSIBLE FOR MAKING DETERMINATIONS AS TO THE TYPE AND LOCATION OF UNDERGROUND UTILITIES AS MAY BE NECESSARY TO AVOID DAMAGE THERETO. ALL COSTS FOR DETERMINING UNDER GROUND UTILITY TYPES AND LOCATIONS SHALL BE SUBSIDIARY TO THE CONTRACT.	
(8)	ALL APPLICABLE UTILITY DEPARTMENTS AND COMPANIES SHALL BE NOTIFIED BEFORE EXCAVATION IS STARTED, UTILITIES WITHIN 50 FEET OF AN EXCAVATION SHALL BE MARKED IN THE FIELD,	(7)
(9)	HOYLE, TANNER WAIVES ANY AND ALL RESPONSIBILITY AND LIABILITY FOR PROBLEMS THAT ARISE Due to the failure of the contractor:	(
	st to follow these drawings and specifications and the design intent they convey.	(8)
	* TO NOTIFY HOYLE, TANNER OF ANY DISCREPANCIES, ERRORS, OMISSIONS OR CONFLICTS AND OBTAIN THEIR GUIDANCE TO RESOLVE.	
(10)	THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SHORING AND BRACING REQUIRED DURING CONSTRUCTION.	
(11)	THE CONTRACTOR SHOULD NOTE THAT THE NHDOT "STANDARD PLANS FOR ROAD AND BRIDGE	(1)
	CONSTRUCTION" ARE MADE A PART OF THIS PROJECT AND ALL APPLICABLE DETAILS, STANDARDS AND SPECIFICATIONS SHALL APPLY. THIS PROJECT SHALL INCLUDE, BUT IS NOT LIMITED TO, THE FOLLOWING STANDARD PLANS:	(2)
	CR-1 - GRANITE CURB DETAILS	(4)
	DL-1 – ROADSIDE DELINEATION DR-1 – GRATE AND FRAME DETAILS DR-2 – D.I. MANHOLE COVER AND PAVEMENT DEPRESSION DETAILS	(5)
	DR-4 – UNDERDRAIN FLUSHING BASIN AND POLYETHYLENE LINER DETAILS DR-5 – PRECAST REINFORCED CONCRETE C.B., D.I. AND M.H.	(6)
	<pre>GR-2 - BEAM GUARDRAIL STANDARD SECTION - STEEL POSTS AND HARDWARE DETAILS GR-3 - PREFERED PLATFORM FOR ENERGY ABSORBING GUARDRAIL TERMINAL (EAGRT) DM 1</pre>	(7)
	PM-1 - LATOUT DETAILS PM-2 - TOLERANCES FOR PAVEMENT MARKING LINES PM-9 - PAVEMENT MARKING AT MINOR INTERSECTIONS	
	PS-3 - ALUMINUM SHEET DETAILS FOR TUBING AND U-CHANNEL POSTS	(1)
(12)	THE CONTRACTOR SHALL BE RESPONSIBLE FOR IMPLEMENTING AN EROSION AND SEDIMENT CONTROL MEASURES IN ACCORDANCE WITH THE "NEW HAMPSHIRE STORMWATER MANUAL, VOLUME 3 EROSION AND SEDIMENT CONTROLS DURING CONSTRUCTION" BY THE NEW HAMPSHIRE DEPARTMENT OF ENVIRONMENTAL SERVICES.	
(13)	DESIGN SPEED: 25 MPH	(2)
	TOPOGRAPHIC SURVEY NOTES	
(1)	THE SURVEY FOR THIS PROJECT WAS COMPLETED BY:	
	597 NEW BOSTON ROAD BEDFORD, NH 03110 (603) 472-2265	(1)
	EARL J. SANDFORD, LICENSED LAND SURVEYOR NO. 700	(2)
(2)	THE SURVEY CONSISTED OF 1 SHEET TITLED: EXISTING CONDITIONS PLAN MANCHESTER ROAD OVER BEAVER BROOK	(3)
(3)	WETLAND RESOURCES WITHIN THE SURVEY AREA WERE DELINEATED BY: CHRISTOPHER K. DANFORTH, C.W.S. NO. 77	
(4)	THE SURVEY IS BASED UPON THE FOLLOWING DATUMS: Vertrical: NAVD 88	(4)
	HORIZONTAL: NEW HAMPSHIRE STATE PLANE COORDINATE SYSTEM	

	<u>design</u> <u>And</u>	LOADS, MATERIALS Specifications
DESIGN LOADING:	HL-93	
DESIGN METHOD:	LOAD AND RE	ESISTANCE FACTOR DESIGN (LRFD)
SPECIFICATIONS:	AASHTO LRFI NHDOT 2010	D BRIDGE DESIGN SPECIFICATIONS 6TH EDITION WITH 2013 INTERIMS STANDARD SPECIFICATIONS AS AMENDED
FOUNDATION DATA:	ABUTMENTS SPREAD WITH 2.81 RESIST	AND WINGS: D FOOTINGS ON STRUCTURAL FILL A NOMINAL BEARING CAPACITY OF TSF IN COMBINATION WITH A TANCE FACTOR OF 0.45.
REINFORCING STEEL:	AASHTO M31 EPOXY COATE OVERLA	(ASTM A 615) GRADE 60 Ed bars: ay, deck beams, brush curb, sidewalk and flared beam
CONCRETE:	4,000 PSI: OVERLA 3,000 PSI: FOOTINGS (A ITEM 5 WINGWALLS A ITEM 5 OVERLAY, BF ITEM 5	AY, BRUSH CURB AND SIDEWALK NGS, WINGWALLS AND ABUTMENTS ABUTMENTS, WINGWALLS): 520.213, CONCRETE CLASS B, FOOTINGS (ON SOIL) (F) AND ABUTMENTS: 520.12, CONCRETE CLASS A, ABOVE FOOTINGS (F) RUSH CURB AND SIDEWALK: 520.7, CONCRETE BRIDGE DECK (F)
PRESTRESSED Concrete:	DECK BEAMS f'c = ITEM 5 BUTTED	AND FLARED BEAM: 6,500 PSI (AT 28 DAYS) (f'ci = 5,200 PSI) 528.311, PRESTRESSED CONCRETE BRIDGE DECK, ) DECK BEAMS (F)
SEISMIC:	PEAK GROUNI SITE CLASS ZONE = 1	D ACCELERATION (PGA) = 0.09 = B
		<u>Hydraulic data</u>
DRAINAGE AREA:		8.9 SQUARE MILES
DESIGN FLOOD:		Q <sub>50</sub>
DESIGN VELOCITY:		5.6 FPS
DESIGN FLOOD ELEVA	TION:	242.7 FT
DESIGN FLOOD FLOW:		1,430 CFS
BRIDGE OPENING:		280 SF

BRIDGE WATERWAY OPENING BELOW THE DESIGN FLOOD ELEVATION: 260 SF

## MOBILIZATION AREA NOTES

PRIOR TO COMMENCEMENT OF CONSTRUCTION, THE CONTRACTOR SHALL LAYOUT LIMITS OF ALL EASEMENTS AND TOWN'S RIGHT-OF-WAY WITHIN THE PROJECT LIMITS. COST IS INCLUDED UNDER ITEM 692, MOBILIZATION, LAYOUT SHALL BE PERFORMED BY A LAND SURVEYOR LICENSED IN THE STATE OF NEW HAMPSHIRE. SEE SPECIFICATIONS FOR ADDITIONAL INFORMATION.

THE CONTRACTOR SHALL BE LIMITED TO MOBILIZATION WITHIN THE TOWN'S RIGHT-OF-WAY AND EASEMENT LIMITS SHOWN ON THESE PLANS, ADDITIONAL MOBILIZATION AREAS REQUIRED BY THE CONTRACTOR SHALL BE COORDINATED BY THE CONTRACTOR WITH THE AFFECTED PROPERTY OWNERS AND SHALL BE AT THE CONTRACTOR'S EXPENSE.

### EXISTING BRIDGE REMOVAL NOTES

ALL COSTS FOR REMOVAL OF THE ENTIRE THREE EXISTING  $9'-9''\times6'-7''$  structural plate pipe ARCHES SHALL BE SUBSIDIARY TO ITEMS 504.1 AND 207.3. NO PAYMENT SHALL BE MADE FOR THE VOLUME DISPLACED BY THE EXISTING PIPE ARCHES.

REMOVAL OF THE EXISTING STONE HEADWALLS AT THE INLET AND OUTLET OF THE CORRUGATED METAL PIPES SHALL BE PAID UNDER ITEM 504.2, ROCK BRIDGE EXCAVATION.

THE CONTRACTOR SHALL TAKE SPECIAL CARE TO ENSURE THAT NO DEBRIS FALLS INTO BEAVER BROOK DURING CONSTRUCTION OPERATIONS, THE ERECTION, MAINTENANCE AND REMOVAL OF TEMPORARY STRUCTURES OR OTHER METHODS TO PREVENT DEBRIS FROM FALLING INTO BEAVER BROOK, AND THE CONTRACTOR'S METHOD OF REMOVAL SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW AND APPROVAL. ALL COSTS SHALL BE SUBSIDIARY TO THE APPROPRIATE ITEM OF WORK BEING PERFORMED.

PLANS OF THE EXISTING BRIDGE STRUCTURE ARE NOT AVAILABLE.

- FOLLOWS:
  - ABUTMENT A: ITEM 503.201, COFFERDAMS (W/IN LIMITS SHOWN)
  - ABUTMENT B: ITEM 503.202, COFFERDAMS SHOWN) AND 503.302
- IMPACT PLAN.
- EXPENSE OF THE CONTRACTOR.

- APPROVED FOR THIS PROJECT.
- THE COFFERDAM STABILITY.

- - INSUFFICIENT SLOPE STABILITY AT 1.5H:1V.
- CONTRACT SPECIFICATIONS FOR DETAILS.

GENERAL CONSTRUCTION NOTES

- (1) SEE SHEET 21 FOR BRIDGE LAYOUT WORKING POINTS COORDINATES.

- (5) WATER LEVEL MAY VARY FROM THAT SHOWN.

COFFERDAM NOTES (1) ALL ITEMS COVERED UNDER SECTION 503 OF THE SPECIFICATIONS SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF NEW HAMPSHIRE. THE CONTRACTOR SHALL SUBMIT STAMPED WORKING DRAWINGS AND CALCULATIONS TO THE ENGINEER FOR REVIEW AND APPROVAL IN ACCORDANCE WITH THE CONTRACT SPECIFICATIONS.

(2) COFFERDAMS SHALL BE REQUIRED FOR THE REMOVAL OF THE EXISTING PIPE ARCHES AND AT EACH ABUTMENT CONSTRUCTION LOCATION TO CONTROL THE RIVER INFLOW AND ADEQUATELY DEWATER THE FOOTING EXCAVATION AND TO CONSTRUCT ABUTMENTS, WINGWALLS AND STONE FILLS. STEEL SHEETING MAY BE REQUIRED DEPENDING ON THE AVERAGE WATER LEVEL CONDITIONS AT THE TIME OF CONSTRUCTION AND THE CONTRACTOR'S METHOD OF DEWATERING. ALL COSTS FOR MATERIALS, INSTALLATION, MAINTENANCE AND REMOVAL SHALL BE INCLUDED IN THE APPROPRIATE 503. PAY ITEM, ALL WORK REQUIRED TO MAINTAIN A DEWATERED CONDITION SHALL BE INCLUDED IN THE APPROPRIATE 503. PAY ITEM. COFFERDAM LOCATIONS AND PAY ITEMS ARE SUMMARIZED AS

ITEM 503.301, COFFERDAMS WITH SHEETING LEFT-IN-PLACE

ITEM 503.302, COFFERDAMS WITH SHEETING LEFT-IN-PLACE (W/IN LIMITS

(3) COFFERDAM LIMITS SHOWN ON THESE PLANS ARE APPROXIMATE, CONTRACTOR IS RESPONSIBLE FOR THE LAYOUT OF THE COFFERDAMS UNLESS THE LOCATIONS SHOWN ARE DENOTED AS ITEM 503,301 AND 503.302 COFFERDAMS WITH SHEETING LEFT-IN-PLACE, WHICH SHALL BE CONSTRUCTED IN THE LOCATION SHOWN, COFFERDAMS REQUIRED TO CONSTRUCT THE PROJECT SHALL REMAIN WITHIN THE PERMANENT SITE FOOTPRINT AND TEMPORARY IMPACT AREAS DENOTED ON THE WETLANDS

(4) CONTROL OF WATER WITHIN THE COFFERDAMS SHALL BE CONDUCTED IN SUCH A MANNER AS TO PREVENT DISTURBANCE OF THE BEARING SOIL. PUMPING AREAS SHALL BE LOCATED OUTSIDE THE FOOTING SUPPORT LIMITS AND PROPERLY FILTERED TO PREVENT THE PUMPING OF FINES.

(5) ANY FOUNDATION SOIL WEAKENED AS A RESULT OF INSUFFICIENT CARE TAKEN IN MAINTAINING A DEWATERED CONDITION SHALL BE REMOVED AND REPLACED WITH STRUCTURAL FILL AT THE

(6) THE CONTRACTOR SHALL BE REQUIRED TO POUR SUBSTRUCTURE CONCRETE IN THE DRY.

(7) DEWATERING SHALL BE CONTINUOUS UNTIL SUBSTRUCTURES ARE BACKFILLED TO THE ELEVATIONS OF THE SURROUNDING WATER TABLE, UNLESS OTHERWISE DIRECTED.

(8) ALL MEANS AND METHODS ASSOCIATED WITH HANDLING WATER DURING CONSTRUCTION OF FOUNDATIONS SHALL BE LOCATED WITHIN THE LIMITS OF WORK SHOWN ON THE WETLANDS PERMIT

(9) THE COFFERDAM DESIGN SHALL ACCOUNT FOR THE EFFECTS OF UNBALANCED EARTH PRESSURE ON

(10) IT SHOULD BE NOTED THAT IN SOME LOCATIONS PRE-EXCAVATION OF COBBLES AND BOULDERS MAY BE REQUIRED TO PLACE STEEL SHEETING. DURING EXCAVATION THE CONTRACTOR SHALL DISTURB THE AREA AS LITTLE AS POSSIBLE AND USE NECESSARY PRECAUTIONS TO MINIMIZE THE IMPACTS TO THE RIVER. ALL COSTS INCLUDED IN THE APPROPRIATE 503. PAY ITEM.

(11) COFFERDAMS LOCATED WITHIN THE DEFLECTION DISTANCE OF THE TRAFFIC BARRIER SHALL BE DESIGNED TO WITHSTAND THE APPROPRIATE TRAFFIC BARRIER COLLISION LOAD AS SPECIFIED IN THE CURRENT AASHTO GUIDELINES. THE COFFERDAM SHALL EXTEND UP TO A HEIGHT THAT IS EQUAL TO OR HIGHER THAN THE TOP OF THE ADJACENT TRAFFIC BARRIER.

(12) EXCAVATION BACKSLOPES BELOW IN-SERVICE ROADWAYS THAT ARE USED IN COMBINATION WITH, OR IN-PLACE-OF, A COFFERDAM SHALL MEET THE FOLLOWING CRITERIA. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND MAINTENANCE OF ALL EXCAVATED BACKSLOPES.

A) THE EXCAVATION BACKSLOPES SHALL BE NO STEEPER THAN 1.5H:1V. A FLATTER BACKSLOPE SHALL BE USED IF THE CONTRACTOR'S CALCULATIONS INDICATE

B) FOR CASES WHERE EXISTING GUARDRAIL IS USED FOR TRAFFIC BARRIER ABOVE THE EXCAVATION, THE CREST OF EXCAVATED BACKSLOPES SHALL BE OFFSET A MINIMUM OF 8 FEET FROM FACE OF EXISTING GUARDRAIL AND THE EXCAVATED BACKSLOPES SHALL BE MAINTAINED IN THEIR ORIGINAL CONFIGURATION.

C) FOR CASES WHERE CONCRETE TRAFFIC BARRIERS ARE USED IN PLACE OF EXISTING GUARDRAIL, THE CREST OF EXCAVATED BACKSLOPES SHALL BE OFFSET A MINIMUM OF 2 FEET FROM THE OUTSIDE EDGE OF THE CONCRETE BARRIER.

(13) ABBUTTER HOME INSPECTIONS WILL BE REQUIRED PRIOR TO ALL SHEET PILING OPERATIONS, SEE

(2) THE EXISTING BRIDGE HAS ZERO LIVE LOAD CAPACITY AND IS CLOSED. TRAFFIC SHALL CONTINUE TO BE DETOURED VIA MANCHESETER ROAD AND MACK HILL ROAD. MANCHESTER ROAD AND MACK HILL ROAD TRAFFIC WILL BE REDUCED TO ONE LANE DURING CONSTRUCTION (SEE TRAFFIC CONTROL PLANS), LIMITED CLOSURES OF THESE ROADS, IF REQUIRED TO PERFORM CERTAIN WORK, MUST BE APPROVED BY THE TOWN TWO WEEKS PRIOR TO COMMENCING THE WORK, ACCESS TO DRIVES WITHIN THE PROJECT LIMITS SHALL BE MAINTAINED DURING CONSTRUCTION.

(3) DIMENSIONS, ANGLES, BEARINGS AND ELEVATIONS SHOWN ON THESE CONTRACT PLANS HAVE BEEN OBTAINED FROM LIMITED FIELD INVESTIGATIONS AND SURVEY, AND MAY NOT ACCURATELY REFLECT ACTUAL FIELD CONDITIONS. ACCORDINGLY, THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAKING FIELD MEASUREMENTS OF ALL EXISTING STRUCTURE COMPONENTS IMPACTED BY THE PROPOSED WORK TO ASSURE CONSISTENCY WITH THE PROPOSED MODIFICATIONS. ANY DISCREPANCIES IN DIMENSIONS, CHARACTER OR EXTENT OF THE EXISTING FEATURES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER BEFORE ADVANCING THE WORK.

(4) THE CONTRACTOR SHALL PROTECT ALL EXISTING UTILITIES FROM DAMAGE DURING CONSTRUCTION.



	<u>prestressed deck beam notes</u>		
(1)	THE CONCRETE COMPRESSIVE STRENGTH OF THE PRECAST DECK BEAM UNITS AND FLARED BEAM SHALL BE 5200 PSI AT RELEASE AND 6500 PSI AT 28 DAYS.	(1)	OVERHEAD AND TAKE
(2)	PRESTRESSING STANDS SHALL BE 0.6-IN. DIA. UNCOATED SEVEN-WIRE STRAND CONFORMING TO AASHTO M203-05 (ASTM A416) GRADE 270 LOW RELAXATION. ALL STRANDS SHALL BE PRE-TENSIONED TO 44 KIPS PER STRAND (75% INITIAL PULL).		SHALL COC MOBILIZAT DISCONNEC COORDINAT
(3)	POST-TENSIONING STRANDS SHALL BE 0.6-IN. DIA. SEVEN-WIRE STRAND CONFORMING TO AASHTO M203 (ASTM A416) GRADE 270 LOW RELAXATION. POST-TENSIONING STRANDS SHALL BE COMPLETELY COATED WITH A CORROSION PREVENTATIVE COATING SUCH AS FLO-GARD, AS MANUFACTURED BY INSTEEL INDUSTRIES, INC., SANDERSON, FL., OR POLYSTRAND, AS MANUFACTURED BY LANG TENDONS, INC., TOUGHKENAMON, PA. OR AN APPROVED EQUAL. IF THE FLO-GARD COATING SYSTEM IS SUPPLIED, GROUT SHALL BE EXCLUDED FROM THE LATERAL POST-TENSIONING DUCTS DURING GROUTING OF THE SHEAR KEYS BETWEEN THE BEAMS. THE CONTRACTOR'S PROPOSED METHOD FOR EXCLUDING GROUT FROM THE POST-TENSIONING DUCTS SHALL BE SUBMITTED WITH THE SHOP DRAWINGS. POST-TENSIONING ANCHORAGE SYSTEM SHALL BE MONO-STRAND CORROSION PROTECTION SYSTEM AS MANUFACTURED BY HAYES INDUSTRIES, INC., HOUSTON, TEXAS, OR APPROVED EQUAL.		MISCELLAN INCLUDED
(4)	TRANSVERSE POST-TENSIONING OF THE PRECAST DECK BEAMS SHALL BE PERFORMED IN ACCORDANCE WITH SECTION 528 OF NHDOT'S STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION.		
(5)	ALL REINFORCING STEEL FOR THE SUPERSTRUCTURE SHALL CONFORM TO AASHTO M31 (ASTM A615) GRADE 60 AND SHALL BE EPOXY COATED.		
(6)	THE PRECAST DECK BEAM REINFORCING STEEL SHALL HAVE A MINIMUM CLEAR COVER OF $1^{1}$ /2 $^{\prime\prime}$ UNLESS OTHERWISE NOTED.	(2)	UNDERGROL F AMILIAR
(7)	THE COST OF PRESTRESSING STRANDS, POST-TENSIONING STRANDS AND ANCHORAGES, AND REINFORCING STEEL CAST INTO THE PRECAST DECK BEAM UNITS SHALL BE PAID UNDER ITEM 528.311. ALL OTHER STEEL IN THE SUPERSTRUCTURE SHALL BE PAID UNDER ITEM 544.31.		
(8)	LIFTING DEVICES SHALL BE WITHIN 24" OF EACH END OF THE PRECAST DECK BEAM UNITS, COST SHALL BE PAID UNDER ITEM 528,311,		
(9)	1 -IN. DIA. DRAINS SHALL BE PROVIDED AT THE LOW END OF ALL DECK BEAM VOIDS.		
(10	) THE DECK BEAM SHEAR KEYS SHALL BE BLAST CLEANED PRIOR TO SHIPPING.	(3)	ТИЕ Е <b>у</b> ісі
(11	) THE TOP SURFACE OF THE DECK BEAMS SHALL BE RAKED TRANSVERSELY TO A $^{1}\prime_{4}^{\prime\prime}$ AMPLITUDE.	())	WATER WOR
(12	) DRILLING INTO THE DECK BEAMS SHALL NOT BE ALLOWED.		
(13	) CONTRACTOR SHALL SUBMIT TWO (2) SETS OF APPROVED SHOP DRAWINGS ON PERMANENT, ARCHIVAL QUALITY 22"×34" DOUBLE-MATTE MYLAR.	(1)	ITEM 535.
(14	) THE EXTERIOR FACES OF THE PRESTRESSED DECK BEAMS SHALL BE COATED WITH COLORED CONCRETE SEALANT TO THE LIMITS SHOWN ON THE TYPICAL BRIDGE SECTIONS SHEET, THE COST OF WHICH SHALL BE PAID FOR WITH ITEM 535.1. THE SEALANT COLOR WILL BE SELECTED BY THE OWNER.		BOTH ABUT REPELLENT OTHER EXF SELECTED
	<u>ELASTOMERIC BEARING NOTES</u>	(2)	ITEM 538.
(1)	BEARING ASSEMBLIES, INCLUDING ELASTOMERIC BEARING LAYERS AND INTERNAL STEEL PLATES SHALL BE PAI AS ELASTOMERIC BEARING ASSEMBLIES (F), ITEM 548.21.	D (3)	PLACED ON
(2)	DESIGN LOADS (SERVICE I LOADS - DESIGN METHOD A): Maximum dead load 13.1 kips		FOOTINGS. BE SUBSIE
	MAXIMUM SUPERIMPOSED DEAD LOAD 2.7 KIPS MAXIMUM LIVE LOAD 13.6 KIPS	(4)	ITEM 585.
	DESIGN MOVEMENT COMPRESSIVE DEFLECTION $0.039$ IN TOTAL MOVEMENT ( $\Delta T = 80^{\circ}F$ ) $0.12$ IN	(5)	ALL FOOTI (F). ALL
	DESIGN MOVEMENT (SHEAR DEFORMATION) 0.24 IN	(6)	ARIITMENITS
(3)	ELASTOMERIC BEARING PADS SHALL BE VIRGIN RUBBER, HARDNESS (SHORE "A" DUROMETER) OF 60, GRADE 4. BUT WITH A SHEAR MODULUS RANGE 130 PSI TO 200 PSI.	(7)	CONSTRUCT
(4)	STEEL REINFORCING FOR ELASTOMERIC BEARING PADS SHALL CONFORM TO SECTION 548.2.3.		(CONTRACT
(5)	BEARINGS SHALL BE INSTALLED AT TEMPERATURES BETWEEN 20°F AND 70°F. INSTALLATION TEMPERATURES OUTSIDE THIS RANGE WILL REQUIRE ADJUSTMENT AT NO ADDITIONAL COST TO THE OWNER.	(8)	RE INFORCE RE INFORCI
(6)	FOLLOWING THE MANUFACTURE OF ELASTOMERIC BEARINGS AND VERIFICATION OF THE INTERNAL STEEL	(9)	SHEAR KEN
	SPACE FILLED WITH SILICONE CAULKING.	(10)	ALL EXPOS
	PAVING NOTES	(11)	EXPOSED F
(1)	ALL PAVING OPERATIONS SHALL BE PERFORMED BY A SUBCONTRACTOR THAT IS LISTED ON THE NHDOT PREQUALIFIED CONTRACTORS LIST IN THE CATEGORY OF PAVING.		48"×16" F MIAMISBUF
(2)	THE BITUMINOUS MIXTURE SHALL BE THOROUGHLY UNIFORMLY COMPACTED BY ROLLING. THE INITIAL		(TEL: 1 8 IN ITEM 5
	ROLLING SHALL BE DONE WITH A STATIC OR VIBRATORT STEEL-DROM ROLLER. INTERMEDIATE ROLLING SHALL BE DONE BY A PNEUMATIC-TIRED ROLLER. FINAL ROLLING SHALL BE DONE WITH A STATIC-DRUM ROLLER. THE MINIMUM WEIGHT OF STATIC ROLLER SHALL BE 8 TONS.	(12)	THE FORM
(3)	SUBMIT PAVEMENT MIX DESIGN TO ENGINEER FOR APPROVAL PRIOR TO PAVING, SEE SECTION 401 OF THE NHDOT STANDARD SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS,		
(4)	THE GRADE OF ASPHALT CEMENT SHALL BE PG64-28.		

## UTILITY COORDINATION

UTILITIES ARE PRESENT WITHIN THE PROJECT SITE, THE CONTRACTOR SHALL BE FAMILIAR NECESSARY PRECAUTIONS WITH THESE UTILITIES DURING CONSTRUCTION, CONTRACTOR ORDINATE TEMPORARY RELOCATIONS (IF REQUIRED); SHIELDING NECESSARY FOR EQUIPMENT TION (SUCH AS CRANE TO INSTALL THE NEW SUPERSTRUCTURE) AND TEMPORARY CTION OF POWER WITH THE UTILITY OWNERS IF REQUIRED. ALL COST FOR THIS TION SHALL BE INCLUDED IN ITEM 692, MOBILIZATION, ALL COSTS ASSOCIATED WITH NEOUS TREE TRIMMING & CLEARING FOR TEMPORARY UTILITY RELOCATIONS SHALL BE IN ITEM 201.1, CLEARING AND GRUBBING (F), AND REQUIRE ABUTTER PERMISSION.

OVERHEAD UTILITY OWNER INFORMATION: PSNH (ELECTRIC) CONTACT: RAYNA ACAMPORA PHONE: (603) 673-0108 EXT, 5555805 EMAIL: RAYNA.ACAMPORA@NU.COM

FAIRPOINT COMMUNICATIONS (TELEPHONE) CONTACT: YEW CHAI PHONE: (603) 645-2705 EMAIL: YCHAI@FAIRPOINT.COM

COMCAST (CABLE TELVISION) CONTACT: BARRY SULLIVAN PHONE: (617) 279-6460 EMAIL: BARRY\_SULLIVAN@CABLE.COMCAST.COM

UND UTILITIES ARE PRESENT WITHIN THE PROJECT SITE. THE CONTRACTOR SHALL BE AND TAKE NECESSARY PRECAUTIONS WITH THESE UTILITIES DURING CONSTRUCTION.

UNDERGROUND UTILITY OWNER INFORMATION: PENNICHUCK WATER WORKS, INC. (WATER) CONTACT: PETE TEDDER PHONE: (603) 882-5191

FAIRPOINT COMMUNICATIONS (TELEPHONE) CONTACT: YEW CHAI PHONE: (603) 645-2705 EMAIL: YCHAI@FAIRPOINT.COM

TING WATERLINE LOCATION UPSTREAM OF THE BRIDGE WAS PROVIDED BY PENNICHUCK RKS, INC. AND IS APPROXIMATE. A TEMPORARY BYPASS OF THIS LINE WILL BE REQUIRED ONSTRUCTION AND PAID UNDER ITEM 611.99 (SEE SPECIFICATIONS).

## ABUTMENT AND WINGWALL NOTES /2

.1, COLORED CONCRETE SEALANT, SHALL BE APPLIED TO EXPOSED VERTICAL SURFACES OF TMENTS AND ALL WINGWALLS TO 1'-O" BELOW THE FILL LINES. ITEM 534.3, WATER (SILANE/SILOXANE) SHALL BE APPLIED TO THE EXPOSED CONCRETE BRIDGE SEATS AND ANY POSED CONCRETE SURFACES NOT COATED WITH ITEM 535.1. THE SEALANT COLOR WILL BE BY THE OWNER.

.2, BARRIER MEMBRANE, PEEL AND STICK-VERTICAL SURFACES (F), 2'-O" WIDE, SHALL BE VER THE ABUTMENT AND WINGWALL EXPANSION JOINT, 1'-O" EACH SIDE OF THE JOINT.

SHALL BE PLACED SYMETRICALLY 10'-0" APART AND CENTERED AT 12" ABOVE THE TOP OF WEEPERS SHALL BE 4" DIA. AND SLOPED TO DRAIN WITH A 12:1 SLOPE. ALL COSTS SHALL DIARY TO ITEM 520.12.

.21, STONE FILL, CLASS B (BRIDGE), SHALL BE 2'-O" THICK, UNLESS OTHERWISE NOTED.

ING CONCRETE SHALL BE PAID AS ITEM 520.213, CONCRETE CLASS B, FOOTINGS (ON SOIL) CONCRETE IN THE ABUTMENT AND WINGWALL STEMS SHALL BE PAID AS ITEM 520.12, CLASS A, ABOVE FOOTINGS (F).

SHALL BE BACKFILLED TO THE LEVEL OF THE BRIDGE SEAT ELEVATION PRIOR TO TING THE SUPERSTRUCTURE.

AND WINGWALL REINFORCEMENT SHALL BE PAID AS ITEM 544.3, REINFORCING STEEL TOR DETAILED).

EMENT DENOTED (E) SHALL BE EPOXY COATED AND SHALL BE PAID UNDER ITEM 544.31, ING STEEL, EPOXY COATED (CONTRACTOR DETAILED).

YS SHALL BE 3" HIGH BY ONE-THIRD THE WIDTH OF THE WALL, CENTERED.

SED EDGES OF CONCRETE SHALL BE CHAMFERED  ${}^{3}{}_{\prime 4}{}^{\prime \prime}$  .

FACES OF THE ABUTMENTS, WINGWALLS AND EXTERIOR FACE OF THE CONCRETE CURB AND SHALL HAVE A ROCK FACE BLOCK FORM LINER PATTERN. THE FORM LINER SHALL BE SYMONS ROCK FACE BLOCK P/C F70636 AS MANUFACTURED BY DAYTON SUPERIOR, 1125 BYERS ROAD, RG, OH 45342 (937) 866-0711 <del>or running bond ashlar no. 329 Multicast as</del>

JRED BY GREENSTREAK, 3400 TREE COURT INDUSTRIAL BOULEVARD, ST. LOUIS, MO 63233 <del>800 325 9504) or an approved equal,</del> the cost of the form liner shall be included 520.12.

LINER FINISH SHALL EXTEND TO A MINIMUM 12" BELOW THE LOWEST FINISHED GRADE AT THE WALL.

- OUTSIDE THE PROPOSED EDGE OF FOOTINGS.
- OR SPLIT TO PROVIDE A LEVEL BEARING SURFACE.
- BUCKET TO PREVENT EXCESS DISTURBANCE TO THE EXISTING SUBGRADE.
- THE ENGINEER.
- CONTRACTOR'S EXPENSE.

- MATERIAL. ALL COSTS SHALL BE INCLUDED IN ITEM 544.3 OR 544.31.
- (4) REINFORCING LEGEND: ALT = ALTERNATE CLR = CLEAR FS = FAR SIDE
  - MID = MIDDLESECT = SECTION
- TYPE I AS RECOMMENDED IN THE APPROVED PLAN.
- INVASIVE SPECIES PLANTS HANDBOOK.

- ITEM 503.101, WATER DIVERSION STRUCTURES.
- IMPACT AREAS SHOWN ON THE WETLANDS IMPACT PLAN.
- PRIOR TO START OF THE WORK.



SHEET 3 OF 40

ITEM	ITEM DESCRIPTION	Qua	ntity	]
NO		Unit	Amount	_
201.1	REMOVING SMALL TREES	EA	-2-	3
201.881	INVASIVE SPECIES CONTROL TYPE I	SY	<del>-100-</del>	200
202.41	REMOVAL OF EXISTING PIPE 0-24" DIAMETER		5	-
202.5	REMOVAL OF CATCH BASINS, DROP INLETS, AND MANHOLES REMOVAL OF GUARDRAIL		410	_
203.1	COMMON EXCAVATION	CY	1080	-
203.5555	GUARDRAIL 25'EAGRT PLATFORM	<u> </u>	1	_
203.55551	EMBANKMENT-IN-PLACE (F)	CY	120	_
206.19	COMMON STRUCTURE EXCAVATION EXPLORATORY	CY	20	
207.3		CY	525	_
304.2	GRANULAR BACKFILL (BRIDGE) (F) GRAVEL (F)	CY CY	565	-
304.21	GRAVEL (STREAMBED)	CY	-70-	42
304.3		CY	370	_
<u> </u>	HOT BITUMINOUS PAVEMENT MACHINE METHOD		10 <u>-422</u>	397.830
403.12	HOT BITUMINOUS PAVEMENT, HAND METHOD	TON	10	
403.6		LF	<del>-1250-</del>	1046
403.911	HOT BITUMINOUS BRIDGE PAVEMENT, 1" BASE COURSE (F)	TON	8 15	_
417	COLD PLANING BITUMINOUS SURFACES	SY	-100-	65
503.101	WATER DIVERSION STRUCTURES	U	1	-
503.201	COFFERDAMS	<u> </u>	1	-
503.301	COFFERDAMS WITH SHEETING LEFT-IN-PLACE	U	1	_
503.302	COFFERDAMS WITH SHEETING LEFT-IN-PLACE	U	4	1.5
504.1		CY	950	207
504.2 508	STRUCTURAL FILL	CY CY	<del>-00</del> 75	203
520.1	CONCRETE CLASS A	CY	2	1
520.12	CONCRETE CLASS A, ABOVE FOOTINGS (F)	CY	147	-
520.213 520.7	CONCRETE CLASS B, FOOTINGS (ON SOIL) (F)	CY CY	121	-
528.311	PRESTRESSED CONCRETE BRIDGE DECK, BUTTED DECK BEAMS (F)	SF	1688	-
534.3	WATER REPELLENT (SILANE/ SILOXANE)	GAL	6	-
535.1	COLORED CONCRETE SEALANT	GAL	15	_
538.5	BARRIER MEMBRANE, PEEL AND STICK - VERTICAL SURFACES (F) BARRIER MEMBRANE, HEAT WELDED (F)	SY SY	150	_
541.4	PVC WATERSTOPS, NH TYPE 4 (F)	LF	55	-
544.1	REINFORCING STEEL (ROADWAY)	LB	16	04 075
544.3 544.31	REINFORCING STEEL (CONTRACTOR DETAILED) REINFORCING STEEL EPOXY COATED (CONTRACTOR DETAILED)	LB LB	- <del>18800-</del> -6100-	7.716
548.21	ELASTOMERIC BEARING ASSEMBLIES (F)	EA	42	
562.1	SILICONE JOINT SEALANT (F)	LF	87	
563.24	BRIDGE APPROACH RAIL T4 (STEEL POSTS)		90 4	
585.21	STONE FILL, CLASS B (BRIDGE)	CY	275	-
585.3	STONE FILL, CLASS C	CY	4	-
592.1 593./11		SF SV	72	-
603.00315	15" R.C. PIPE, 3000D	LF	400	_
603.00318	18" R.C. PIPE, 3000D	LF	64	-
604.0007		EA	1	_
606.1255	BEAM GUARDRAIL (TERM, UNIT TYPE EAGRT 25 FT) (STEEL POST)	U	4	-
606.18001	31" W-BEAM GR W/8" BLOCKOUTS (STEEL POSTS)	LF	<del>- 87.5</del>	123.5
606.417	PORTABLE CONCRETE BARRIER FOR TRAFFIC CONTROL	LF	<del>-220-</del>	96
606.9522	2" BIT IMINOUS SIDEWALK (F)	U SY	2	_
609.01	STRAIGHT GRANITE CURB	LF	250	
609.3	STRAIGHT GRANITE CURB (BRIDGE)	LF	90	
611.90001	ADJUSTING WATER GATES AND SHUTOFFS SET BY OTHERS	EA	<del>3</del> 1	15
615.03	TRAFFIC SIGN TYPE C (F)	SF	34.25	-
615.034	RELOCATING TRAFFIC SIGN, TYPE C	U	1	]
615.06	IRAFFIC SIGN TYPE CC (F)	SF	8.67	-
616.171	PORTABLE TEMPORARY TRAFFIC SIGNALS, TRAILER MOUNTED	U	1	1
618.61	UNIFORMED OFFICERS WITH VEHICLE	\$	4	0.077
618.7		HR	<del>1250 1</del>	247
621.2	RETROREFLECTIVE BEAM GUARDRAIL DELINEATOR	EA	<u>-15-</u>	3
622.1	STEEL WITNESS MARKERS	EA	2	]
622.52		EA		4
632.0104	RETROREFLECTIVE PAINT PAVE, MARKING, 4" LINF		<u>-4300</u>	1,070
632.0124	RETROREFLECTIVE PAINT PAVE. MARKING, 24" LINE	LF	-50-	24
632.3118	RETROREFLECT. THERMOPLAS. PAVE. MARKING, 18" LINE		40	40
632.911 632.912	OBLITERATE PAVE. WARKING LINE, 12" WIDE & UNDER OBLITERATE PAVE, MARKING LINE, OVER 12" WIDE	<u> </u>	<del>- 250</del> - <del>25</del> -	24
641	LOAM	CY	130	]
645.3		TON	<del>125</del>	10
645.41 645.43	TEMPORARY SLOPE STABILIZATION TYPE A	SY SY	200	-
<u>645</u> .51	HAY BALES FOR TEMPORARY EROSION CONTROL	EA	50	1
645.52	RYEGRASS FOR TEMPORARY EROSION CONTROL	LB	-11	20
645.531 645.7	SILT FENCE STORM WATER POLLITION PREVENTION PLAN	LF	870 1	-
645.71	MONITORING SWPPP AND EROSION AND SEDIMENT CONTROLS	EA	- <u>-25-</u>	26
646.31	TURF ESTABLISHMENT WITH MULCH AND TACKIFIERS	SY	1230	]
670.101	TEMPORARY LIGHTING	U	<del>1</del>	0
697.11	INVASIVE SPECIES CONTROL AND MANAGEMENT PLAN	U	<u> </u>	0.38
699	MISCELLANEOUS TEMPORARY EROSION AND SEDIMENT CONTROL	\$	1	]

SUMMARY	OF	QUANTITIES	2

	CHANGE ORDER NO.	1	
		QUANTITY	
I TEM NO.	DESCRIPTION	UNIT	AMOUNT
1	BURRIED UTILITIES	LS	1
2	AL TERNATE CONCRETE STAIN	LS	1
3	6" ELASTOMERIC EXP. JOINT	LF	60

	CHANGE ORDER NO.2	2		
		QUANTITY		
ITEM NU.	DESCRIPTION	UNI T	AMOUNT	
4	ADDITIONAL DRAINAGE	<i>LS</i>	1	
5	AL TERNATE COMPACTION	CY	520	

	CHANGE ORDER NO.3		
TTEM NO		QUANTITY	
ITEM NU.	DESCRIPTION	UNI T	AMOUNT
6	ALTERNATE CONCRETE STAIN (REVISED)	<i>LS</i>	1
7	STAIN MOCK-UP	L <i>S</i>	1

REV. DESCRIPTION DRW. CHKD. DATE 21 0 H RU	Z     RECORD COPY REVISIONS     TAG     JCR     3/2016     3 = 1 < 2      3     <		ARCH 2 CONT CONT CONT CONT	This document is prepared as an instrument of service and shall remain the service are service and shall remain the service are service and shall remain the service are service are service and shall remain the service are	property of Hoyle, Tanner. It may not be used, reproduced, disseminated	or transferred in any manner, including electronically, for any other $\vec{x} = \vec{x}$ in $\vec{y}$ purpose than this project, without the written permission of Hoyle, Tanner. $\vec{x} = \vec{x}$	
N10C		WLD	JBM	E		AS SHOWN	
		DESIGN BY:	DRAWN BY:			SCALE:	
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			OK	SUMMARY OF QUANTITIES			
TOWN OF AMHEDCT		AMHERSI, NEW HAMPSHIRE	MANCHESTER ROAD OVER BEAVER BRO		SUMMARY OF OUANTITIES		
		E i o AMHERSI, NEW HAMPSHIKE	T I G B MANCHESTER ROAD OVER BEAVER BRO	9191 1910 9101 NO.	Image: Summary of Ouantities	т ГТ S З	

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		TABLE	
NUMBER	ELEVATION	STATION & OFFSET	DESCRIPTION
TBM#1	249.17	102+90 RT 21'	NAIL WITH SSE CAP
TBM#2	245.99	100+63 LT 22'	ARROW BOLT ON HYDRANT
TBM#3	249.35	201+05 LT 15'	NAIL WITH SSE CAP
			•

3/24/2016 10:39:17 / 10:419101

	HORIZONTAL CONTROL TABLE									
NUMBER	NORTHING	EASTING	ELEVATION	STATION & OFFSET*	DESCRIPTION					
CP#1	133686.4349	995847.2279	246.72	102+91 RT 11′	MAGNETIC NAIL					
CP#2	133411.5247	995787.4582	245.32	100+09 RT 11'	MAGNETIC NAIL					
	* INF	ORMATION PROVI	DED FOR GE	NERAL LOCATION ONLY						

				ΤA	BLE OF	PROP	ERTY ACQUIS	JITION	
	PROPERTY OWNER	TOTAL AREA	AREA OF	REMA	INDER				
TAN NU		L AC		LT	RT		PERN	/ANENT	
		AC	AC	AC	AC	SF	TYPE	SF	Т
2	ANDREA MANIGLIA, TRUSTEE								
3	SCOTT W. O'CONNELL & SUSAN JACOBS O'CONNELL								
4	ROLLAND RESIDENTIAL TRUST								
5	TOWN OF AMHERST					3846	SLOPE		

			PIS	UF	KE V
	TEN	/PORARY	ACCE	ESS	NO
SF	DESCRIPTION	EXPIRES (DURATION)	LT	RΤ	
1300	CONSTRUCTION	COMPLETION OF PROJECT			
607	CONSTRUCTION	COMPLETION OF PROJECT			
457	CONSTRUCTION	COMPLETION OF PROJECT			
1035	CONSTRUCTION	COMPLETION OF PROJECT			



# Northern Test Boring, Inc. Boring Log

APPROX GROL EL = 246.13

Client: HTA

Location: Amherst, NH

BOTTOM OF AB EL = 231.00

Core Ground Water Observ 8.7'	Core	Sample SS	5	asing IW	C H				e	Тур
Start Date: Fin		1 3/8"		1"	2					Size
11/28/11 11	11/28/11							$\frac{Vt}{2a^{11}}$	mer V	Harr
		30	<u>.</u>		10			an	imer i	нап
Stratum Descri		Depth		low	mple B Counts	Sa	Sample Depth	Rec	Pen	No.
6.5" Pavement Brown Fine-Medium Sand and Grave	6.5" Pave Brown Fi		18	19	30	25	0'-2'	11"	24"	S-1
	2 <u>1</u>	5'	2	3	2	2	5'-7'	12"	24"	S-2
Brown Fine-Coarse Sand Trace Silt	Brown Fi									
APPROX GROU		10'	19	27	30	27	10'-12'	8"	24"	S-3
Brown Sandy Gravel Trace Silt	Brown Sa									
		15'	14	14	27	30	15'-17'	11"	24"	S-4
_	,;;	10	14		27	50	15 -17		21	5-1
Brown Fine-Medium Sand Some Gra	Brown Fi							(		
Bedrock Surface @ 20.4'	Bedrock S	20*		<u></u>		50/ 5"	20'-22'	5"	5" 5'	S-5 R-1
White/Red Coarse Granite	White/Re				-		25.4'			
R-1 RQD = 78%	R-1 RQE	25'			-		25.4'-	5'	5'	R-2
		-			-		30.4'			
R-2 RQD = $92\%$	R-2 RQE				-					
Bottom of Exploration @ 30.4'	Bottom of	30'								
				5	2				5	

ion sh Date: 28/11 Trace Silt ) WATER LEVEL and Silt Boring #: B-1 Sheet: 1 of 1

Project Name: Manchester Road over Beaver Brook

Driller: Michael Nadeau

# Northern Test Boring, Inc. Boring Log

Clie	nt: H	ГА							Project N	an
Loc	ation:	Amh	erst, NH						Driller: N	1ic
Tyn	e				C	asing IW		Sample	Core	0
Size Han	e mer '	Wt.			2	1"		1 3/8" 140	_	S 1
Han	nmer l	Fall			**		N.M.	30"		
No.	Pen	Rec	Sample Depth	Sai	nple B Counts	low	10	Depth		
S-1	24"	12"	0'-2'	4	4	8	10		Brown Fi	ne-
S-2	24"	18"	5'-7'	5	6	5	10	5'	-	
S-3	24"	15"	10'-12'	12	12	12	10	10'	Brown C	oar
S-4	24"	14"	15'-17'	17	15	12	11	15'		
S-5	24"	20"	20'-22'	12	14	14	19	20'	Brown Fi	ne-
R-1	5'	4.7'	24'-29'		1 1 1			25*	Bedrock	Sur
R-2	5'	4.6'	29'-34'		1 1 1			, , ,	White/Re R-1 RQI	:d C ) =
					1			30'	R-2 RQI	) =
									Bottom o	fE

ne: Manchester Road over Beaver Brook

hael Nadeau

Ground Water Observation 8.3' tart Date: Finish Date: 1/28/11

11/28/11

Stratum Description

Medium Sand and Gravel Trace Silt

\_\_\_\_\_APPROX GROUND WATER LEVEL

se Sand Gravel Trace Silt

Medium Sand Some Gravel and Silt

face @ 24.0'

Coarse Granite

63%

94%

xploration @ 34.0'

Boring #: B-2 Sheet: 1 of 1

I HEREB BEST OF CONSTRU WORK SH BEEN CO	Y CERTI MY KNO CTION F OWN IN MPLETED	IFY THA DWLEDGE RECORDS THESE D AS SH	T TO TH • BASEL • THAT DRAWING OWN•	HE D UPON ALL GS HAS
HOYLE, SIGNED -		& ASSO	CIATES	
DRW. CHKD. BY. CHKD. BY. CHKD. BY. CHKD. BY. CHKD. BY. CHKD. BY. CHKD. BY. CHKD. BY. CHKD. Strategy (1997)	RSTOOD		int of service and shall remain the red used, reproduced, disseminated	ectronically, for any other
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			150 Dow Str Tel (603) (	Webpi
TOWN OF AMHERST	AMHERST, NEW HAMPSHIRE	MANCHESTER ROAD OVER BEAVER BROOK	BORING LOGS	
PROJECT FILE NAM MODEL N	NO.: 1E: A IAME: SHE	91 AB919 9191 ET N	19101 9101B 01BC O.	OR2 DR2
SHE	ET	<b>7</b> C	)F 4(	o





![](_page_8_Figure_8.jpeg)

![](_page_9_Figure_0.jpeg)

		STAT	ION	
	A	B	$\bigcirc$	D
FER ROAD (SOUTH)	100+ <del>00</del> 08	100+10	100+20	100+30
L ROAD	103+72	103+60	103+50	103+40
FER ROAD (EAST)	202+28	202+20	202+10	202+00

			<u>. 101010</u>	<u> </u>	<u>IECTION DE</u>	<u>I AILO</u>	
2			NOT TO	SCALE			
	CULVEF	RT OUT	LET ER	OSION	PROTECTION		
DRAIN NOTE #	OUTLET STATION	W <sub>1</sub> (ft)	W <sub>2</sub> (ft)	L <sub>a</sub> (ft)	THICKNESS (ft)	STONE CLASS	VOLUME (CY)
		11'	11'	8'			3.25
1	102+33.99, LT. 24.28	<del>7.0</del>	<del>-7.0</del> -	<del>-5.0</del> -	1.00	CLASS C	<del>-1.3-</del>
							3.25
			ITEM	585.3	CLASS C	SUB-TOTAL	<del>-1.3-</del>

	WORK	SHOWN I	I RECOR	GE, BASE DS, THAT E DRAWIN SHOWN.	ALL GS HAS
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	DATE	9/2016		ll remain the seminated	other oyle, Tanner
	<pre>CHKD. BY 0. BY 0. BY 0.</pre>			and shal uced, dis	or any o on of Hc
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<u>Z</u> <u>KNESS</u>				© Copyright 150 Dow Street, Mar Tal /603) 660-5551	Webpage: www.h
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XNESS Y)			MANCHESTER ROAD OVER BEAVER BROOK	© Copyright 150 Dow Street, Mar MISCFI I ANFOLIS DFTAILS Tal /602) 660-5551	
				O 010 06 00 00 00 00 00 00 00 00 00 00 00 00	

![](_page_10_Figure_0.jpeg)

# MANCHESTER ROAD (SOUTH)/ MACK HILL ROAD 🖄

![](_page_11_Figure_1.jpeg)

DESCRIPTION BY DATE DESCRIPTION BY DATE DESCRIPTION	RECORD COPY REVISIONS     TAG     JCR     3/2016     I <thi< th=""> <thi< th=""></thi<></thi<>	BY CERT MY KN JCTION HOWN IN DMPLETE TANNEF MORDING ERSTOOD	ARCH 2 CONT CON	HAT T B GE, F DRAM SOCIA CALL AND	try of Hoyle, Tanner. It may not be used, reproduced, disseminated	nsferred in any manner, including electronically, for any other $33 \pm 33$ s at $133 \pm 33$ s than this project, without the written permission of Hoyle, Tanner.
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				© Copyright 2014 150 Dow Street Mancher 150 Dow Street Mancher		

![](_page_12_Figure_0.jpeg)

# MANCHESTER ROAD (EAST)

	NOTE: IS UND NOTE: IS UND NOTE:	WORD IN WORD IN WORD IN	NUMLEDG RECORD N THESE ED AS S R & ASS AR & ASS MARCH 24 G CONTA D TO BE	ed as an instrument of service and shall remain the transformed to service and	t, without the written permission of Hoyle, Tanner.
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	JULY 2014	ESIGN BY: SBH	RAWN BY: JFMS	HKD. BY: CED	CALE: AS SHOWN
				150 Dow Street, Man	Webpage: www
SCALE: 20' HORIZ. = 4' VERT.	TOWN OF AMHERST	AMHERST, NEW HAMPSHIRE	MANCHESTER ROAD OVER BEAVER BROOK	ROADWAY PROFILE (2 OF 2)	

I HEREBY CERTIFY THAT TO THE

1 ''= 1 ''=

![](_page_13_Figure_1.jpeg)

CUF	RBIN			
			609.01	
	MARK NO.	RADIUS	STRAIGHT GRANITE CURB	COMMENTS
		FT	LF	
LT. 13.33	C-1		2.00	4" TO 5" REVEAL
LT. 13.33	C-2		21.03	7" REVEAL (5" TO 7" OVER FIRST 6.25')
LT. 13.33	C-3	236.50	9.46	7" REVEAL
LT. 13.33	C-4	236.50	30.85	7" REVEAL (7" TO 5" OVER LAST 6.25')
LT. 14.25	C-5		4.82	5" TO 4" REVEAL
RT. 12.00	C-6	568.65	11.97	7" TO 0" TIP DOWN AT SOUTH END
RT. 12.21	C-7		48.86	7" REVEAL
RT. 13.94	C-8	35.00	30.00	7" REVEAL
RT. 12.00	C-9		48.55	7" REVEAL
RT. 12.00	C-10	532.00	30.99	7" TO 0" TIP DOWN AT EAST END
			238.53	
			11.47	
			250.00	

TRAFFIC CONTROL SEQUENCING 🔨 ∕2∖ PHASE 1A (NOT SHOWN) W20-1a-(350' FROM W20-4) - MAINTAIN BRIDGE CLOSED DETOUR SIGNAGE. ADJUST AS REQUIRED. W20-4 & W13-1P(20)-- UTILIZING DAILY LANE SHIFTS AND 1-WAY ALTERNATING TRAFFIC (350′ FROM W3-3) PATTERN, CONSTRUCT STA. 102+15 TO 103+72 LT & RT AND STA. 200+00 TO 202+28 LT TO BINDER GRADE. RESTORE 2-WAY TRAFFIC W3-3-NIGHTLY. (350' FROM R10-6) - INSTALL TEMPORARY WATERLINE BYPASS - CONSTRUCT DRAINAGE NOTE T1, 1, PORTIONS OF 2, AND EMBANKMENT WITHIN WORK ZONE. CONST. TEMP. IMPACT ATTENUATION 🕂 - CONSTRUCT TEMPORARY WIDENING STA. 200+12 TO 200+95, LT (SEE DEVICE (NON-REDIRECTIVE), TEST LEVEL 2 (ITEM 606,9522) DETAIL). -STA: 102+94-- CONSTRUCT TEMPORARY TRAFFIC SIGNAL AND LIGHTING. LT. 16.09 (FACE OF PCB) - CONSTRUCT PHASE 1B BARRIER, AS SHOWN, SHIFT TRAFFIC INTO PHASE 1B LANE USE. ∕2∖ STA. 102+66-<u>Phase 1B</u> LT. 12.22 (FACE OF PCB) - CONSTRUCT PHASE 2, 3, & 4 PORTION OF BRIDGE CONSTRUCTION -ITEM 670.101 --SEQUENCING (FOOTINGS, ABUTMENTS, & WINGWALLS)(SEE SHEET 19). TEMPORARY LIGHTING <del>₩1-8L</del> - CONSTRUCT STA. 200+00 TO 202+28, RT TO BINDER GRADE. <del>₩1-8R</del> CONSTRUCT REMAINING PORTIONS OF DRAINAGE NOTE 2 AND 3. STA. 102+55-- CONSTRUCT PHASE 2 BARRIER LAYOUT. SHIFT TRAFFIC INTO PHASE 2 LANE USE. W1-8L - REMOVE TEMPORARY TRAFFIC SIGNAL AND LIGHTING. W1-8R RELOCATED POLE (BY OTHERS 🚱 🖉 🖉 - REMOVE TEMPORARY WIDENING AND CONSTRUCT SLOPE WORK STA. 200+12 TO 200+95, LT. UTILITY POLE -TO BE RELOCATED 2 SEE NOTE ON SHEET 25A 60}  $\odot_{an}$ 2 BEGIN CONSTRUCTION STA. 100+08 10.54 )OS+ POS+

3/24/2016 10:39:40 / k:\919101

![](_page_14_Figure_1.jpeg)

## TRAFFIC CONTROL NOTES

1. TRAFFIC CONTROL DEVICES SHALL CONFORM TO SECTION 619 OF THE NHDOT STANDARD SPECIFICATIONS, AND THE 2009 MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (MUTCD), AS PUBLISHED BY THE U.S. DEPARTMENT OF TRANSPORTATION (USDOT) AND ADOPTED BY THE COMMISSIONER OF THE NEW HAMPSHIRE DEPARTMENT OF TRANSPORTATION, SIGNS SHALL ALSO CONFORM TO USDOT STANDARD HIGHWAY SIGNS AND NHDOT CONSTRUCTION SIGN STANDARDS.

2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR SUPPLYING, ERECTING AND MAINTAINING PERMANENT CONSTRUCTION SIGNS AND WARNING DEVICES AS LISTED ON THE PLANS, AND SHALL ALSO BE RESPONSIBLE FOR SUPPLYING, ERECTING AND MAINTAINING ALL OPERATIONAL SIGNS AND WARNING DEVICES FOR THE PLANNED METHODS OF OPERATION IN CONFORMANCE WITH THE MUTCD.

3. THE CONTRACTOR SHALL MARK ALL HAZARDS WITHIN THE LIMITS OF THE PROJECT AND CONNECTING ROADS WITH WELL MAINTAINED SIGNS AND WARNING DEVICES. ALL SIGNS AND WARNING DEVICES SHALL BE MOVED, SUPPLEMENTED, CHANGED, OR REMOVED DURING THE PROGRESS OF THE CONSTRUCTION AS NEEDED.

4. TRAFFIC CONTROL DEVICES SHALL BE REMOVED, AND SIGNS SHALL BE COVERED OR REMOVED, WHEN THEY NO LONGER APPLY TO THE EXISTING CONDITIONS.

5. PLYWOOD SUBSTRATE FOR CONSTRUCTION SIGNS SHALL CONFORM TO SECTION 619. AND FLAT ALUMINUM SHEETS SHALL CONFORM TO SECTION 615 OF THE NHDOT STANDARD SPECIFICATIONS.

6. DETOURS INVOLVING THE ROUTING OF TRAFFIC OVER ROADS OUTSIDE THE LIMITS OF THE PROJECT SHALL BE MARKED AND MAINTAINED BY THE CONTRACTOR (UNLESS OTHERWISE NOTED). THE CONTRACTOR SHALL BE REQUIRED TO ERECT AND MAINTAIN ANY REQUIRED SIGNS AND WARNING DEVICES AT THE BEGINNING AND END OF THE WORK AND AT INTERSECTING ROADWAYS, THE LOCATION AND POSITION OF THESE SIGNS AND WARNING DEVICES SHALL BE AS APPROVED BY THE ENGINEER. THE CONTRACTOR MAY ALSO BE REQUIRED TO UNCOVER, COVER AND OTHERWISE MAINTAIN DETOUR SIGNS SUPPLIED BY OTHERS.

7. WORK ON THE PROJECT, OR ANY SEPARATE ACTIVITY THEREIN, SHALL NOT START UNTIL ALL THE REQUIRED SIGNS AND WARNING DEVICES ARE INSTALLED AND APPROVED BY THE ENGINEER.

8. SIGN LOCATIONS SHOWN ON THESE STANDARDS ARE RECOMMENDED AND MAY BE ADJUSTED AS DETERMINED BY THE ENGINEER, TYPICAL LAYOUTS SHOWN ARE NOT TO SCALE,

9. THE CONTRACTOR SHALL BE REQUIRED TO PROVIDE THE ENGINEER WITH CERTIFICATION THAT ALL THE SIGNS AND WARNING DEVICES USED ON THE PROJECT MEET THE SPECIFICATIONS.

10. THE USE OF CONSTRUCTION SIGNS AND WARNING DEVICES NOT SHOWN ON THESE STANDARDS OR MUTCD, UNLESS APPROVED BY THE ENGINEER, SHALL BE PROHIBITED.

11. ALL COSTS FOR TRAFFIC CONTROL DEVICES, INCLUDING PLACEMENT, RELOCATION AND REMOVAL OF SIGNS SHALL BE INCLUDED IN ITEM 619.1, MAINTENANCE OF TRAFFIC.

12. THE CONTRACTOR SHALL MAINTAIN SAFE, CONTINUOUS ACCESS TO ALL PROPERTIES ADJACENT TO THE PROJECT LOCATION.

![](_page_15_Figure_13.jpeg)

(CONSTRUCTION AND REMOVAL OF TEMPORARY SIGNAL PAID UNDER ITEM 616.171) (CONSTRUCTION AND REMOVAL OF TEMPORARY LIGHTING PAID UNDER ITEM 670.101)

	CONST. 6' X 30' DETECTION ZONE (CONTRACTOR'S OPTION)
2	STA. 102+89.1, LT. 14.1' CONST. TEMPORARY PORTABLE TRAFFIC SIGNAL SYSTEM INSTALL TRAFFIC SIGNAL HEADS AS SHOWN

![](_page_15_Figure_16.jpeg)

![](_page_15_Figure_17.jpeg)

SIGNAL PI	HASII	NG
	Φ1	Ф2
TIMING IN SECONDS	-	-
INITIAL INTERVAL	10	10
VEHICLE EXTENSION	5	5
MAX. I	20	20
MAX. II	—	_
YELLOW	4	4
ALL RED	20	20
RECALL	OFF	SOFT
DETECTOR MEMORY	L	L
FLASH	RED	RED

2	CONST	TRUCTION SIGNS AND	WARNI	NG DE	VICE	S ( )	ITEM 61	9.1)
	TYPE	DESCRIPTION	SIZE W×H	SQ. FT.	NO REQ.	TOTAL AREA	POST	COLOR
	G20-2a	END ROAD WORK	36″×18″	4.5	2	9	1 POST PER SIGN	B/O
	R10-6	STOP HERE ON RED	24″ × 36″	6	2	12	1 POST PER SIGN	B∕W
	W1-8		18" × 24"	3	8	24	5' MOUNTING HEIGHT 55' SPACING (TYP.)	B/Y
	W3-3	B	30" × 30"	6.25	2	12.5	1 POST PER SIGN	B/O
	W13-1P	<b>20</b> M.P.H.	18″ × 18″	2.25	2	4.5	MOUNT WITH W20-4	B/O
	W20-1a	ROAD WORK AHEAD	36″ × 36″	9	2	18	1 POST PER SIGN	B/O
	W20-4	ONE LANE ROAD 700 FT	36″ × 36″	9	2	18	1 POST PER SIGN	B/O

## SIGNAL HEADS

ALL LENSES ARE 12" LED WITH 5" LOUVERED BACK PLATE

![](_page_15_Picture_24.jpeg)

STA. 201+59.7, LT. 15.1' CONST. TEMPORARY PORTABLE TRAFFIC SIGNAL SYSTEM INSTALL TRAFFIC SIGNAL HEADS AS SHOWN

CONST. 6' X 30' DETECTION ZONE (CONTRACTOR'S OPTION)

## AM, PM PEAK TRAFFIC VOLUMES

						<u>LE(</u>	Gend	
		$\bigcirc$		AM	(	PM)	PEAK	HOUR
107		]A[						
Ù	CK	Ц Ц						
61	MA							
Ţ								
Y		-	-107	(161	)			
	MAN	СНЕ	STER	ROA	D	(EA	ST)	

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			DESIGN B			CHKD. BY	SCALE:	
				ASSC	© Copy	150 Dow Street, Tel (603) 669-5	Webpage:	
	TOWN OF AMHERST AMHERST, NEW HAMPSHIRE MANCHESTER ROAD OVER BEAVER BROOK TRAFFIC CONTROL NOTES							
P F	rro. Tile 100	Ject Nam Del N	NO.: E: AME: SH	AB9 91 EET	91 19 91 N	19101 101T( 01TC 0.	CPR1 P01	
	ļ	SHF	= ET	<b>L</b> 16		) )F 40	,	

## TRAFFIC CONTROL SEQUENCING (CONT.)

<u>PHASE 2</u>

- CONSTRUCT BRIDGE SUPERSTRUCTURE & DECK.
- CONSTRUCT ROADWAY APPROACHES TO BINDER GRADE STA. 100+00 TO 101+45 AND 101+90 TO 102+15.
- CONSTRUCT PERMANENT GUARDRAIL, CURBING, SIDEWALK, AND SLOPE WORK WITHIN PROJECT LIMITS.

## <u>Phase 3 (not shown)</u>

- SHIFT TRAFFIC INTO PERMANENT LANE USE.
- CONSTRUCT WEARING COURSE AND FINAL PAVEMENT MARKINGS FOR ENTIRE PROJECT.
- REMOVE CONSTRUCTION SIGNAGE. CONSTRUCT PERMANENT SIGNAGE.
- PERFORM FINAL RESTORATION.

![](_page_16_Figure_11.jpeg)

![](_page_17_Figure_0.jpeg)

I HEREBY CERTIFY THAT TO THE BEST OF MY KNOWLEDGE, BASED UPON CONSTRUCTION RECORDS, THAT ALL WORK SHOWN IN THESE DRAWINGS HAS BEEN COMPLETED AS SHOWN. HOYLE, TANNER & ASSOCIATES, INC Riply SIGNED \_\_ MARCH 2016 NOTE: WORDING CONTAINED HEREON IS UNDERSTOOD TO BE PAST TENSE. ≿| ິິິ | ¥≍ 8 ment of be used, j electror B CONSTRUCTION an instru may not including MACK HILL ROAD  $\sum_{i=1}^{n} \frac{1}{i}$ it as TO JONES ROAD je re t This prof 2014 wLD ES ЪВМ JUNE POC STA. 102+13.35 -충 R В MANCHESTER RD/MACK HILL RD BE = POT STA. 200+00.00 - MANCHESTER RI D  $\widetilde{\phantom{a}}$ 2 OVERHEAD UTILITY LAYOUT SHOWN IS BASED ON FINAL CONSTRUCTION, PROPOSED J LAYOUT FROM CONTRACT DOCUMENTS NOT SHOWN FOR A **ש** מ CLARITY. •-----14'-6″ ∖relocate monument <del>10′-0″</del> OFF EP ITEM 622,52 <u>Ó</u> EXIST OVERHEAD WIRES TO 📈 BE RELOCATED (BY OTHERS L'and TOWN OF AMHERST AMHERST, NEW HAMPSHIRE GENERAL PLAN AND ELEVATION CHESTER ROAD OVER BEAVER PROJECT NO .: 919101 FILE NAME: AB919101GPE MODEL NAME: 919101GPE SHEET NO. 18 2 SEE SHEET 27A FOR WATER MAIN LAYOUT SCALE:  ${}^{3}_{32}'' = 1' - 0''$ 2 0 4 8 12 16 SHEET 18 OF 40

![](_page_18_Figure_0.jpeg)

TOWN OF AMHERST       TOWN OF AMHERST       UNE 2014       Intel 2014         AMHERST, NEW HAMPSHIRE       AMHERST, NEW HAMPSHIRE       Design BY:       WID         MANCHESTER ROAD OVER BEAVER BROOK       © copyright 2014 Hoyle, Tamer & Associates, Inc.       Design BY:       WID         SITE PLAN       STE PLAN       CHO. BY:       STE:       STE:       STE:       STE:	IOWN OF AMHERST         TOWN OF AMHERST         TOWN OF AMHERST         TOWN OF AMHERST         AMHERST, NEW HAMPSHIRE         AMHERST, NEW HAMPSHIRE         AMHERST, NEW HAMPSHIRE         MANCHESTER ROAD OVER BEAVER BROOK         IDINE 2014         IDINE 2014         IDINE 2014         IDINE 2014         MANCHESTER ROAD OVER BEAVER BROOK         IDINE 2014 LOS, INC.         IDINE 2014 Hove, Tanne & Associates, Inc.         IDINE 2014 Hove, Tanne & Associates, Inc.         ISTE PLAN         SCALE: AS SHOWN by Paramet.com         SCALE: AS SHOWN	EV.       DESCRIPTION       DRW. CHKD.       DATE       Isolation         Record copy revisions       TAG       JCR       3/2016       Isolation	BY CER F MY KI UCTION HOWN II TANNEL WORDING ERSTOOL	NOWLEDO RECORD N THESE ED AS S R & ASS MARCH 2 G CONTA D TO BE	nis document is prepared as an instrument of service and shall remain the tage of tage	transferred in any manner, including electronically, for any other हिंदी हिंही हैं है urpose than this project, without the written permission of Hoyle, Tanner. हिंही है है
TOWN OF AMHERST       TOWN OF AMHERST       JUNE         AMHERST, NEW HAMPSHIRE       AMHERST, NEW HAMPSHIRE       JUNE         AMHERST, NEW HAMPSHIRE       EMANCHART       JUNE         MANCHESTER ROAD OVER BEAVER BROOK       © COPYIGHT 2014 Hoyle, Tamer & Associates, Inc.       JUNE         SITE PLAN       SITE PLAN       JUNE       JUNE         SITE PLAN       Webpage: www.hoyletanner.com       SCALE:       /	TOWN OF AMHERST       TOWN OF AMHERST         TOWN OF AMHERST       AMHERST, NEW HAMPSHIRE         AMHERST, NEW HAMPSHIRE       AMHERST, NEW HAMPSHIRE         MANCHESTER ROAD OVER BEAVER BROOK       ESIGN BY:         MANCHESTER ROAD OVER BEAVER BROOK       © copyright 2014 Holy, Tamer & Associate, Inc.         LEI NAME       © copyright 2014 Holy, Tamer & Associate, Inc.         Iste PLAN       Iste Banchester, NH 03101-1227         CHO, BY:       Tel (603) 669-4168         Webbage: www.hoyletanner.com       SCALE:	2014 R	MLD	JBM	ES ES	
TOWN OF AMHERST AMHERST, NEW HAMPSHIRE MANCHESTER ROAD OVER BEAVER BROOK SITE PLAN SITE PLAN SITE PLAN SITE PLAN AMONG AMHERST Manchester, New Hamber, New Hoyle Flaner & Associates, Inc. 150 Dow Street, Manchester, NH 03101-1227 Tel (603) 669-5555 · Fax (603) 669-4168 Webpage: www.hoyletanner.com	TOWN OF AMHERST TOWN OF AMHERST AMHERST, NEW HAMPSHIRE MANCHESTER NOAD OVER BEAVER BROOK MANCHESTER ROAD OVER BEAVER BROOK SITE PLAN SITE PLAN Mebage: www.hoyletanner.com Mebage: www.hoyletanner.com SHEET NO:	JUNE	DESIGN BY:	DRAWN BY:	CHKD. BY:	SCALE: A
TOWN OF AMHERST AMHERST, NEW HAMPSHIRE MANCHESTER ROAD OVER BEAVER BROOK SITE PLAN	TOWN OF AMHERST AMHERST, NEW HAMPSHIRE AMHERST, NEW HAMPSHIRE BANER BROOK FILE NAME: 919101SITE PLAN SHEET NO.	net di		sociales, II	© соругдит 2014 поуке, тапиет & Ass Treet, Manchester, NH 03101 660-5555 · Fax (603) 660-2	age: www.hoyletanner.com
i I	PROJECT NO.: 919101 FILE NAME: AB919101SITE MODEL NAME:919101SITEPLAN SHEET NO.				C COPYRIGHT ZU14 HOVIE, TAIMET & ASS 150 Dow Street, Manchester, NH 03101 Tal (603) 669-5555 · Fax (603) 669-4	Webpage: www.hoyletanner.com

![](_page_19_Figure_0.jpeg)

![](_page_20_Figure_0.jpeg)

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![](_page_21_Figure_0.jpeg)

![](_page_21_Figure_3.jpeg)

![](_page_22_Figure_0.jpeg)

3/24/2016 10:39:54 AN k:\919101\c

![](_page_23_Figure_0.jpeg)

![](_page_23_Figure_1.jpeg)

![](_page_23_Figure_5.jpeg)

\1'-O" (MIN) STRUCTURAL FILL (CONFORMING TO ITEM 508.) (COST INCLUDED IN ITEM 592.1)

16′-0″

 $\overline{\langle \langle \rangle}$ 

233.06 (AVG)

GRANULAR BACKFILL BEYOND SUBSIDIARY TO ITEM 503.202

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			AMHERST, NEW HAMPSHIRE ■ E E E E E E E E E E E E E E E E E E	MANCHESTER ROAD OVER BEAVER BROOK	AUUSWI IWYUUM 919101 9101W 02WIM NO. <b>1</b>	/ing2

![](_page_24_Figure_0.jpeg)

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![](_page_25_Figure_0.jpeg)

## TYPICAL WINGWALL SECTION A

SCALE:  $\frac{3}{8}'' = 1' - 0''$ \* SEE SITE PLAN FOR LOCATION.

<u>Notes</u>

- (1) NORTHEAST WINGWALL SHOWN, OTHERS SIMILAR.
- (2) DIMENSION DENOTED \* INDICATES LOCATION TO BE DETERMINED BY CONTRACTOR.
- (3) SEE SITE PLAN FOR APPROXIMATE COFFERDAM AND WATER DIVERSION STRUCTURE LAYOUT.

![](_page_25_Figure_7.jpeg)

![](_page_25_Figure_10.jpeg)

![](_page_25_Figure_11.jpeg)

ITEM 538.2, BARRIER MEMBRANE, PEEL AND STICK-VERTICAL SURFACES (F) (2'-O")

![](_page_25_Figure_19.jpeg)

![](_page_25_Figure_21.jpeg)

NOTE 2 AT THE REQUEST OF THE CONTRACTOR, THE WINGWALLS WERE NOT BATTERED.

![](_page_26_Figure_0.jpeg)

![](_page_26_Figure_1.jpeg)

![](_page_26_Figure_2.jpeg)

![](_page_26_Figure_6.jpeg)

top layer

SCALE:  $1_{4''} = 1' - 0''$ 

ABUTMENT FOOTING —

*, − #*6 @ 12″

*─*#5 @ 12″

—#5 @ 12″

BEST OF MY KNOWLEDGE, BASED UPON CONSTRUCTION RECORDS, THAT ALL WORK SHOWN IN THESE DRAWINGS HAS BEEN COMPLETED AS SHOWN. HOYLE, TANNER & ASSOCIATES, INC SIGNED NOTE: WORDING CONTAINED HEREON IS UNDERSTOOD TO BE PAST TENSE. PERPENDICULAR WINGWALL ≧ Š BARS (----) ON TOP OF REINFORCING MAT ~⊱<mark>∂</mark> , | PERPENDICULAR ABUTMENT BARS ( 🛊 ) ON TOP OF REINFORCING MAT 8 REV. FOOTING REINFORCING - SOUTHWEST CORNER DETAIL MLD 2014 JUNE PERPENDICULAR WINGWALL BARS (◀►) ON BOTTOM OF REINFORCING MAT PERPENDICULAR ABUTMENT BARS ( ) ON BOTTOM OF REINFORCING MAT Ó TOWN OF AMHERST AMHERST, NEW HAMPSHIRE PROJECT NO.: 919101 FILE NAME: AB919101Dt110 MODEL NAME: 915302DTL10

SCALE: 1/4" = 1'-0" 4 1 0 2 6

SCALE:  $\frac{1}{2}'' = 1' - 0''$ 1 0 1 2

SHEET 26 OF 40

SHEET NO.

26

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SUBSTRUCTURE REINFORCING DETAILS

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MARCH 2016

## RETAINING WALL NOTES

- STABILIZED EARTH RETAINING WALL, FOR FURTHER INFORMATION.

- AND ALL NECESSARY INCIDENTALS, COMPLETE AND IN PLACE.
- COURSE.
- THE WINGWALL FORMLINER AS CLOSELY AS POSSIBLE.
- WALL TO 1'-O" BELOW FILL LINES.
- SHALL BE ADDRESSED IN THE CONTRACTOR SUPPLIED SHOP DRAWINGS.

![](_page_27_Figure_12.jpeg)

![](_page_27_Figure_13.jpeg)

- (5) INSTALL TEMP CAP AND BLOW OFF

- (10) EXCAVATE ON NORTH SIDE OF BRIDGE
- (11) CUT & CAP EXISTING AC WATER MAIN AS SHOWN

THIS WORK.

![](_page_27_Figure_38.jpeg)

![](_page_27_Figure_39.jpeg)

CONTACT PENNICHUCK WATERWORKS, INC. FOR ADDITIONAL WATERMAIN INFORMATION NEW WATER MAIN AT BEGIN CONSTRUCTION STA. 100+08, LT 0.65' N=133413.4081 E=995775.8455 APPROX EXIST R.O.W XX STA. 100+90.92, STA. 100+18.08, RT. 1.75' LT. 1.20' STA. 100+51.35. LT. 1.07' APPROX EXIST R.O.W. X/

![](_page_28_Figure_2.jpeg)

SHEET 27A OF 40

![](_page_29_Figure_0.jpeg)

VARIES 38'-4" (MIN) 44'-6'4" (MAX) E CONST CHORD B CONST CHORD B CONSTRUCTION & PGL VARIES 12'-2'2"* (MIN) 20'-63g"* (MAX) 0" (MIN) TRAVEL LANE 10 <sup>5</sup> g"* (MAX) ITEM 538.5, BARRIER MEMBRANE, HEAT WELDED (F) ITEM 503.2, GRANITE DRIDGE DECK (5" MIN THICK) ITEM 609.3, GRANITE CURB (BRIDGE) (TYP) 3.0%*	
	VARIES 2″(MIN) 6 <sup>7</sup> / <sub>8</sub> ″(MAX
$\frac{1}{4 \text{ CONSTRUCTION } 2(2) - 336 - 18 + 2(3)^{-0^{\circ}} - 38' - 0^{\circ}}$ $\frac{1}{4 + 28 + 314}, \text{ PRISTRUSTIC CONCETT BRIDEL DECK,}$ ECK REAKS (F) MADE COMPTSITE WITH CONCETTE DECK $\frac{\text{VARIES}}{\text{SCALE: } \sqrt{2^{\circ}} - 1' - 0^{\circ}}$ $\frac{\text{VARIES}}{35' - 4'' (M'N)}$ $\frac{1}{44' - 6^{1}4'' (NAX)}$ $\frac{10^{2} - 2^{1} - 2^{1} + 328}{10^{2} - 2^{1} - 2^{1} + 328}$ $\frac{10^{2} - 2^{1} - 2^{1} + 328}{10^{2} - 2^{1} - 2^{1} + 328}$ $\frac{10^{2} - 2^{1} - 2^{1} + 328}{10^{2} - 2^{1} - 2^{1} + 328}$ $\frac{10^{2} - 2^{1} - 2^{1} + 328}{10^{2} - 2^{1} - 2^{1} + 328}$ $\frac{10^{2} - 2^{1} - 2^{1} + 328}{10^{2} - 2^{1} - 2^{1} + 328}$ $\frac{10^{2} - 2^{1} - 2^{1} + 328}{10^{2} - 2^{1} - 2^{1} + 328}$ $\frac{10^{2} - 2^{1} - 2^{1} + 328}{10^{2} - 2^{1} - 2^{1} + 328}$ $\frac{10^{2} - 2^{1} - 2^{1} + 328}{10^{2} - 2^{1} - 2^{1} + 328}$ $\frac{10^{2} - 2^{1} - 2^{1} + 328}{10^{2} - 2^{1} - 2^{1} + 328}$ $\frac{10^{2} - 2^{1} - 2^{1} + 328}{10^{2} - 2^{1} - 2^{1} + 328}$ $\frac{10^{2} - 2^{1} - 2^{1} + 328}{10^{2} - 2^{1} - 2^{1} + 328}$ $\frac{10^{2} - 2^{1} - 2^{1} + 328}{10^{2} - 2^{1} - 2^{1} + 328}$ $\frac{10^{2} - 2^{1} - 2^{1} + 328}{10^{2} - 2^{1} - 2^{1} + 328}$ $\frac{10^{2} - 2^{1} - 2^{1} + 328}{10^{2} - 2^{1} - 2^{1} + 328}$ $\frac{10^{2} - 2^{1} - 2^{1} + 328}{10^{2} - 2^{1} + 328}$ $\frac{10^{2} - 2^{1} - 2^{1} + 328}{10^{2} - 2^{1} - 2^{1} + 328}$ $\frac{10^{2} - 2^{1} - 2^{1} + 328}{10^{2} - 2^{1} - 2^{1} + 328}$ $\frac{10^{2} - 2^{1} - 2^{1} + 328}{10^{2} - 2^{1} - 2^{1} + 328}$ $\frac{10^{2} - 2^{1} - 2^{1} + 328}{10^{2} - 2^{1} - 2^{1} + 328}$ $\frac{10^{2} - 2^{1} - 2^{1} + 328}{10^{2} - 2^{1} + 328}{10^{2} - 2^{1} + 328}$ $\frac{10^{2} - 2^{1} - 2^{1} + 328}{10^{2} - 2^{1} + $	5'- SIDEW FAC
(8) S48-18 UNITS @ 4'-0" & (2) S36-18 @ 3'-0" = 38'-0" ITEM 528.311, PRESTRESSED CONCRETE BRIDGE DECK, BUTTED DECK BEAMS (F) MADE COMPOSITE WITH CONCRETE DECK	
TYPICAL SECTION WITH FLARED BEAM AS SCALE: 1/2" = 1'-0"	NOTES (1) prec dept fabr 528. (2) dime

![](_page_29_Figure_3.jpeg)

![](_page_30_Figure_0.jpeg)

![](_page_31_Figure_0.jpeg)

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![](_page_32_Figure_0.jpeg)

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![](_page_32_Figure_2.jpeg)

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A DENOTES DEBONDED STRAND 5'-0" FROM END OF BEAM	DDr			919101
⊞ DENOTES DEBONDED STRAND 7'-0" FROM END OF BEAM     NOTE	FIL	E NAME:	 AB9 ≞ q10	19101Dtl4
NUIL (1) ALL REINFORCING STEEL TO BE EPOXY COATED AND IS INCLUDED IN ITEM 528.311.		S	HEET	NO.
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![](_page_33_Figure_0.jpeg)

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![](_page_33_Figure_2.jpeg)

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				ROAD OVER BEAVER BROOK	DECK BEAM DETAILS (2 OF 4)
SEE PRESTRESSED DECK BEAM SHEETS FOR FABRICATION DETAILS	IC F			MANCHESTER	PRESTRESSED
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<ul> <li>+ DENUIES STRAIGHT STRAND</li> <li>⊕ DENOTES DEBONDED STRAND 3'-O" FROM END OF BEAM</li> <li>▲ DENOTES DEBONDED STRAND 5'-O" FROM END OF BEAM</li> <li>⊞ DENOTES DEBONDED STRAND 7'-O" FROM END OF BEAM</li> <li>NOTE</li> <li>(1) ALL REINFORCING STEEL TO BE EPOXY COATED AND IS INCLUDED IN ITEM 528.311.</li> <li>SCALE: 1<sup>1</sup>/<sub>2</sub>" = 1'-O"</li> </ul>	PRC FILE MOI	DJECT I E NAMI DEL NA	NO.: :: ME: SHI	9 AB91 919 ET N	19101 9101Dtl8 101Dtls8 IO.

![](_page_34_Figure_0.jpeg)

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![](_page_38_Figure_0.jpeg)

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![](_page_39_Figure_1.jpeg)

![](_page_39_Figure_2.jpeg)

![](_page_39_Figure_3.jpeg)

![](_page_39_Figure_4.jpeg)

![](_page_39_Figure_5.jpeg)

![](_page_39_Figure_6.jpeg)

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SEE MANCHESTER ROAD

200 SERIES CROSS SECTIONS

![](_page_39_Figure_7.jpeg)

![](_page_39_Figure_8.jpeg)

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![](_page_41_Figure_0.jpeg)

![](_page_41_Figure_1.jpeg)

![](_page_41_Figure_2.jpeg)

![](_page_41_Figure_3.jpeg)

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I I	Hove Tanc			150 Dow Street, Manchester, NH 03101 Tel (603) 669-5555 · Fax (603) 669-4	Webpage: www.hoyletanner.com

![](_page_42_Figure_0.jpeg)

![](_page_42_Figure_1.jpeg)

![](_page_42_Figure_2.jpeg)

<b>TRESSED CONCRETE BEAMS</b>	PROJECT #21514				
NEIL H. DANIELS, INC.	BRIDGE #134/100				
NCHESTER ROAD BRIDGE	SHOP DRAWINGS BASED ON CONTRACT	SHEET			
AMHERST, NH	DRAWINGS DATED: 11/3/2014				

![](_page_43_Figure_0.jpeg)

VARIES 38'-4" (MIN.) 44'-6¼" (MAX.) VARIES 12'--2½" (MIN.) 20'--6⅔" (MAX.) TRAVEL LANE € CONSTRUCTION CHORD--BARRIER MEMBRANE HEAT WELDED (BY OTHERS) -BRIDGE DECK (5" MIN. THICK) (BY OTHERS) ½"\_\_\_ JOINT 2'-1112" JOINT 7 BEAMS @ 3'-11½" & (6) ½" JOINTS = 27'-11½"  $\frac{\text{SECTION } D-D}{\text{SCALE: } 1/2'' = 1'-0''}$ J\_\_\_\_\_ CHECKED APPROVED PREST CONCRE JNI  $\overline{F}$ RLE 02/12/15 02/12/15 JIJ 02/12/15 PRODUCTS, Inc. CUSTOMER 173 CHURCH STREET JOB MAN Eriksson 813.989.3317 TAMPA, FL YALESVILLE, CT 06492 LOCATION TEL: (203)-269-3119 technologies LRFD.COM FAX: (203)-265-4941 © 2015 ERIKSSON TECHNOLOGIES, INC. ALL RIGHTS RESERVED

![](_page_43_Figure_2.jpeg)

		and the second secon			
RESSED CONCRETE BEAMS	PROJECT #21514				
NEIL H. DANIELS, INC.	BRIDGE #134/	100			
NCHESTER ROAD BRIDGE	SHOP DRAWINGS BASED ON CONTRACT	SHEET			
AMHERST, NH	DRAWINGS DATED: 11/3/2014	-2			
	4	1 · · · · · · · · · · · · · · · · · · ·			

![](_page_44_Figure_0.jpeg)

![](_page_45_Figure_0.jpeg)

![](_page_45_Figure_1.jpeg)

![](_page_46_Figure_0.jpeg)

![](_page_46_Figure_1.jpeg)

![](_page_46_Figure_2.jpeg)

![](_page_46_Figure_3.jpeg)

![](_page_47_Figure_0.jpeg)

![](_page_48_Figure_0.jpeg)

![](_page_49_Figure_0.jpeg)

				TO BE C	AST W/ BM-05	
	MARK	(: BM-0	5 QTY: 1	TOTAL WT (BM-05 W/ SECONDARY POUR 17.7 T	VOL (SECONDARY POUR ONLY):	1.78 cy
	CONC	RETE ST	RENGTH	28 DAY: 6,500 PSI STR	RESS TRANSFER:	5,200 PS
				MATERIAL LIST		
	ITEM	MARK		DESCRIPTION		QTY
	1	530	#5 BENT	PERIOR #5 D370 TYPE 2 THREADED T	BAR, EPOXY COATED	6
	3	532	#5 BENT	BAR, EPOXY COATED		2
	4	533	#5 BENT	BAR, EPOXY COATED	n fan ferfan werd werd af fer an fer an fer an fer af fer yn fer yn fer yn fer an fer an fer an fer an fer an f	2
	5	534	#5 BENT	BAR, EPOXY COATED		2
	6	535	#5 BENT	BAR, EPOXY COATED		3
	/	537	#5 BENT	BAR, EPOXY COATED	างแก่นแนะเหมาะเหมาะ เอาสุขาร เอาสุขาร เอาสุขาร เอาสุขาร เอาสุขาร เอาสุขาร เอาสุขาร เอาสุขาร เอาสุขาร	
	9	538	#5 BENT	BAR, EPOXY COATED	******	ONI
	10	539	#5 BENT	BAR, EPOXY COATED		ONE
	11	540	#5 BENT	BAR, EPOXY COATED	a a se a company a se a company a se a company de la c La company de la company de	2
1'-6%"	12	541	#5 BENT	BAR, EPOXY COATED		
	13	543	#5 BENT	BAR, EPOXY COATED		11
$\bigcap$	15	050106	#5 STR.	BAR, EPOXY COATED	annag an an an an an an fant time y canton af allting of faller miljinterig	2
	16	050204	<b>#</b> 5 STR.	BAR, EPOXY COATED		2
	17	050300	#5 STR.	BAR, EPOXY COATED		2
	18	050404	#5 STR.	BAR, EPOXY COATED	en jasteka kanan er sijar trepenan	2
(3) 0.6"ø 270 KSI	19	050505	#5 STR	BAR, EPOXY COATED		3
STRANDS	20	050801	#5 STR.	BAR, EPOXY COATED		2
-4°	22	050911	#5 STR.	BAR, EPOXY COATED		2
	23	051203	#5 STR.	BAR, EPOXY COATED	· · · · · · · · · · · · · · · · · · ·	2
	24	050010	DAYTON SU	PERIOR #5 D370 TYPE 1 THREADED E	DAR, EPOXY COATED	2
	25	050100	DAYTON SU	PERIOR #5 D370 TYPE 1 THREADED E	BAR, EPOXY COATED	2
5½" 8"	20	050105	DATION SU	PERIOR #5 D370 TYPE 1 THREADED E	BAR, EPOXY COATED	2
INFORCEMENT NOT SHOWN FOR CLARITY)	28	050107	DAYTON SU	PERIOR #5 D370 TYPE 1 THREADED E	BAR, EPOXY COATED	2
$\frac{\text{SECTION } D-D}{\text{SCALE: 12"} = 1'-0"}$	29	050110	DAYTON SU	PERIOR #5 D370 TYPE 1 THREADED E	MAR, EPOXY COATED	2
	30	050200	DAYTON SU	PERIOR #5 D370 TYPE 1 THREADED E	BAR, EPOXY COATED	2
	31	050203	DAYTON SU	PERIOR #5 D370 TYPE 1 THREADED E	BAR, EPOXY COATED	2
1'—6 <b>%</b> "	32	050205	DAYTON SU	PERIOR #5 D370 TYPE 1 THREADED E	BAR, EPOXY COATED	2
	34	050200	DATION SU	PERIOR $\#5$ 0.370 TYPE 1 THREADED E	AR, EPOXY COATED	2
	35	050305	DAYTON SU	PERIOR #5 D370 TYPE 1 THREADED E	MAR, EPOXY COATED	2
<u>}</u>	36	050308	DAYTON SU	PERIOR #5 D370 TYPE 1 THREADED E	BAR, EPOXY COATED	2
	37	050400	DAYTON SU	PERIOR #5 D370 TYPE 1 THREADED E	BAR, EPOXY COATED	2
	38	050405	DAYTON SU	PERIOR #5 D370 TYPE 1 THREADED E	AR, EPOXY COATED	2
	40	050411	DATION SU	PERIOR #5 D370 TYPE 1 THREADED E	BAR, EPOXY COATED	4
FOR SHIPPING &	41	050502	DAYTON SU	PERIOR #5 D370 TYPE 1 THREADED E	BAR, EPOXY COATED	2
storage (each end) ELEVATION	42		TRIPLE 0	.6"ø STRAND LIFTING LOOPS	ny mantana yanya mana ka mantana manana ka kata kata ka manana mana ka kata ka k Manana manana ka	ONI
	43		3"ø X 3'	-0½" LONG P/T DUCTS		
: ¾" =1'-0"	44		74 X5 X5	PLAIE W/ 2/2" HOLE	SW -5 VIDS (ON)	
ESSED CONCRETE BEA	MS			PROJEC	T #21514	<u></u>
IFIL H DANIFLS INC	a - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -	had variance at take at the other that a sub		BRIDGE	#134/100	
				SHOP DRAWINGS		FT
HESTER HOAD BRIDGE				RACED ON CONT		
				DRALL UN CUNT		· C
				UNAWINGS DATED	•	()
				11/3/201/		$\mathcal{F}$

![](_page_49_Figure_2.jpeg)

		BENT	BARS, TI	ES, & S1	<b>IRRUPS</b>					EMBEDMENTS	
VARK #	BAR SIZE	QTY	LENGTH	WT/PC (LBS)	TOT. WT. (LBS)	REMARKS		MARK #	QTY	DESCRIPTION	REMARKS
401	<b>#</b> 4	481	5'-10"	3.9	1874	EPOXY COATED		EM-1	6	PL. ¾"X5"X5" W/ 2½"Ø HOLE	
402	<b>#</b> 4	481	6'-11"	4.6	2222	EPOXY COATED			6	SINGLE USE STRESSING CHUCK	FOR ERECTION
403	<b>#</b> 4	100	3'-11"	2.6	262	EPOXY COATED			2	0.6"\$ MONOSTRAND X 39'-0"	FOR ERECTION
404	#4	64	13'-10"	9.2	591	EPOXY COATED			ONE	0.6"ø MONOSTRAND X 42'-0"	FOR ERECTION
405	<b>#</b> 4	1047	2'-10"	1.9	1982	EPOXY COATED			36	DOUBLE 0.6"Ø STRAND LIFTING LOOPS	
406	<b>#</b> 4	182	4'-10"	3.2	588	EPOXY COATED	]  -		7	TRIPLE 0.6"¢ STRAND LIFTING LOOPS	
407	<b>#</b> 4	182	5'-11*	4.0	719	EPOXY COATED	]  -		4	3"¢ X 2'-7" LONG P/T DUCTS	
408	#4	16	12'-10"	8.6	137	EPOXY COATED			ONE	3"¢ X 3'-0" LONG P/T DUCTS	
501	<b>#</b> 5	87	5'-2"	5.4	469	EPOXY COATED			3	$3^{\circ} \phi \times 3^{\circ} - 7^{\circ}$ LONG P/T DUCTS	· · · ·
503	<b>#</b> 5	14	6'-2"	6.4	90	EPOXY COATED	_ ⊢		21	3"¢ X 3'-11%" LONG P/T DUCTS	
504	<b>#</b> 5	10	5'-0"	5.2	52	EPOXY COATED	↓		21		
505	<b>#</b> 5	140	6'-5"	6.7	937	EPOXY COATED	_ ⊢				
506	<b>#</b> 5	5	6'-4"	6.6	33	EPOXY COATED	4 –		88	1°Ø X 5½° NON-FERROUS VOID DRAIN	
507	#5 #5	ONE	6'-3"	6.5	7	EPOXY COATED	┥ ┝-		44	10"\$\overline{10} X 16'-0" LONG VOID	
508	#5	ONE	6-2 e' 0"	0.4	6			IN-1	46	DAYTON SUPERIOR D368 (TYPE 1) #5 X 2'-9" LONG, EPOXY	
510	#5		0-U 5'-11"	6.2	6			IN-2		DAYTON SUPERIOR (F-64) ½ # X6% LONG, SWL=5 KIPS, (GALV.)	
511	#5	ONE	5'_9"	6.0	6		$\frac{1}{1}$	530	6	DAYTON SUPERIOR #5 D370 TYPE 2 THREADED BAR, EPOXY COATED	
512	#5	ONE	5'-8"	5.0	6	EPOXY COATED		050010	2	DAYTON SUPERIOR #5 D370 TYPE 1 THREADED BAR, EPOXY COATED	
513	#5	ONE	5'-6"	5.7	6	FPOXY COATED	+	050100	2	DAYTON SUPERIOR #5 D370 TYPE 1 THREADED BAR, EPOXY COATED	
514	#5	ONE	5'-4"	5.6	6	EPOXY COATED		050103	2	DAYTON SUPERIOR #5 D370 TYPE 1 THREADED BAR, EPOXY COATED	
515	#5	ONE	5'-2"	5.4	5	EPOXY COATED		050105	2	DAYTON SUPERIOR #5 D370 TYPE 1 THREADED BAR, EPOXY COATED	
516	#5	ONE	5'0"	5.2	5	EPOXY COATED		050107	2	DAYTON SUPERIOR #5 D370 TYPE 1 THREADED BAR, EPOXY COATED	
517	#5	ONE	4'-10"	5.0	5	EPOXY COATED	1  -	050110	2	DAYTON SUPERIOR #5 D370 TYPE 1 THREADED BAR, EPOXY COATED	
518	<b>#</b> 5	ONE	4'-8"	4.9	5	EPOXY COATED		050200	2	DAYTON SUPERIOR #5 D.370 TYPE 1 THREADED BAR, EPOXY COATED	
519	<b>#</b> 5	ONE	4'-6"	4.7	5	EPOXY COATED		050200	2 2	DAYTON SUPERIOR #5 D370 TYPE 1 THREADED BAR EPOXY COATED	
520	<b>#</b> 5	3	6'-3 <b>"</b>	6.5	20	EPOXY COATED	].  -	050205		DAYTON SUBEDIOD #5 D370 TYDE 1 THREADED DAY, ELOXY COATED	
521	<b>#</b> 5	ONE	6'-0"	6.3	6	EPOXY COATED		050205	2		
522	<b>#</b> 5	ONE	5'-10"	6,1	6	EPOXY COATED		050208	2	DATION SUPERIOR #5 D370 TIPE 1 THREADED BAR, EPOXY COATED	
523	<b>#</b> 5	ONE	5'-7"	5.8	6	EPOXY COATED		050211	2	DATION SUPERIOR #5 D370 TYPE 1 THREADED BAR, EPOXY COATED	
524	<b>#</b> 5	ONE	5'-5"	5.6	6	EPOXY COATED		050305	2	DAYTON SUPERIOR #5 D370 TYPE 1 THREADED BAR, EPOXY COATED	<u></u>
531	<b>#</b> 5	2	4'-6"	4.7	9	EPOXY COATED		050308	2	DAYTON SUPERIOR #5 D370 TYPE 1 THREADED BAR, EPOXY COATED	
532	<b>#</b> 5	2	4'-10"	5.0	10	EPOXY COATED		050400	2	DAYTON SUPERIOR #5 D370 TYPE 1 THREADED BAR, EPOXY COATED	
533	#5 #5	2	5'-2"	5.4	11	EPOXY COATED		050403	2	DAYTON SUPERIOR #5 D370 TYPE 1 THREADED BAR, EPOXY COATED	
534	#5 #5	2	5 <sup>2</sup> -/ <sup>2</sup>	5.8	12	EPOXY COATED		050405	4	DAYTON SUPERIOR #5 D370 TYPE 1 THREADED BAR, EPOXY COATED	
535	#5 #5	J	0-1 6' 7"	0.0	19		-	050411	2	DAYTON SUPERIOR #5 D370 TYPE 1 THREADED BAR, EPOXY COATED	
537	#5	UNE.	0-/ 7'_2"	7.5	22			050502	2	DAYTON SUPERIOR #5 D370 TYPE 1 THREADED BAR, EPOXY COATED	
539	#5	ONE	8'-0"	83	8	EPOXY COATED	- - -		L		
540	#5	2	8'-5"	8.8	18	FPOXY COATED	4				
541	#5	30	2'-4"	2.4	73	EPOXY COATED	-				
542	#5	11	2'-10"	3.0	33	EPOXY COATED					
543	<b>#</b> 5	2	12'-10"	13.4	27	EPOXY COATED					
041700	#4	2	17'-0"	17.7	35	EPOXY COATED	1				
050106	<b>#</b> 5	2	1'-6"	1.6	3	EPOXY COATED					
050204	<b>#</b> 5	2	2'-4"	2.4	5	EPOXY COATED	]				
050300	<b>#</b> 5	2	3'-0"	3.1	6	EPOXY COATED	4				
050404	<b>#</b> 5	2	4'-4"	4.5	9	EPOXY COATED					
050505	<b>#</b> 5	3	5'-5"	5.6	17	EPOXY COATED					
050608	<b>#</b> 5	2	6'-8"	7.0	14	EPOXY COATED	4				
050801	<b>#</b> 5	2	8'-1"	8.4	17	EPOXY COATED	-				
•	#5	2	9'-11"	10.3	21	EPOXY COATED					
050911			- ··		<u>├</u> ┣		-				

 REVISIONS	[	DRAWN
A Rev. Note DATE 02/27/2015	JLB	02/12
		Er
		© 2015 Erik

![](_page_50_Figure_2.jpeg)

5'-3"			1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 -	enniylaraa (generaaraa aa aa aa	Na kata daga takat da ang ang ang ang ang ang ang ang ang an		
**							
			*	2'-7½"			
	T A	ø3" (TYP.)	03.	i an an an tha ship an the for			
	1'4"		1'-2½"			1'-2½"	
5'-3"			Ļ				
404	405			406			
LENGTH VARIES	THREADED	END		HREADED END			
Dayton superior #5 D370 type 1 threaded bar. Epoxy coated			530				
	DAYT	on superior	#5 D370 TYPE 2 THREA	ded bar, epoxy	COATED		
RESSED CONCRETE REAMS				OT #0	1514		
			BRIDGE	= #12/1	/100		
CHESTER ROAD BRIDGE		SHOP	DRAWINGS		SHEET		
AMHERST NH	[		) ON CON	TRACT		1	
		JNA WI 1	1/3/2014	_U.			
					l	· · ·	

![](_page_51_Figure_0.jpeg)

![](_page_51_Figure_1.jpeg)

![](_page_51_Figure_2.jpeg)

![](_page_51_Figure_3.jpeg)

![](_page_51_Figure_4.jpeg)

![](_page_51_Figure_6.jpeg)

![](_page_51_Figure_7.jpeg)

![](_page_51_Figure_8.jpeg)

![](_page_51_Figure_10.jpeg)

![](_page_51_Figure_11.jpeg)

RESSED CONCRETE BEAMS	PROJECT #21514					
NEIL H. DANIELS, INC.	BRIDGE #134/100					
NCHESTER ROAD BRIDGE	SHOP DRAWINGS SHEET					
AMHERST, NH	DRAWINGS DATED:					

![](_page_52_Picture_0.jpeg)

![](_page_52_Figure_1.jpeg)

DWG NO.

S1.0

![](_page_53_Figure_0.jpeg)

![](_page_53_Figure_1.jpeg)

LOOKING AT TRAFFIC FACE OF RAILING FACING NORTHWEST FROM CENTERLINE OF ROAD

### BILL OF MATERIAL

4-BAR RAILING (BOTH SIDES COMBINED)

Qty	mk	Description	Spec.			
13		BRIDGE RAIL PED POST W6x25 (10-hole) 3'-5.375" OAH w/ 1 x 10 x 14 B.P.	A572 gr 50			
2		SPLICE TUBE FOR 8x4 RAIL TS 7x3x3/8 1'-8" LG w/ 2 WELDED NUTS	A500 gr B			
6		SPLICE TUBE FOR 4x4 RAIL TS 3x3x5/16 1'-8" LG w/ 2 WELDED NUTS	A500 gr B			
1	A1	rail tube HSS 8 x 4 x 5/16 x 19'-7.625" OAL	A500 gr B			
1	A2	rail tube HSS 8 x 4 x 5/16 x 24'-2.625" OAL	A500 gr B			
1	A3	rail tube HSS 8 x 4 x 3/8 x 17'-4.250" OAL (partial 30' radius)	A500 gr B			
1	A4	rail tube HSS 8 x 4 x 5/16 x 26'-8.375" OAL	A500 gr B			
3	A1	rail tube HSS 4 x 4 x 1/4 x 19'-7.625" OAL	A500 gr B			
3	A2	rail tube HSS 4 x 4 x 1/4 x 24'-2.625" OAL	A500 gr B			
3	A3	rail tube HSS 4 x 4 x 3/8 x 17'-4.250" OAL (partial 30' radius)	A500 gr B	Г		
3	A4	rail tube HSS 4 x 4 x 1/4 x 26'-8.375" OAL	A500 gr B		All steel bridge rail material to be Galv Duncan Galvanizing per NHDOT secti Top Coat Color = Dark Brown Fed Sto	/anized & Powder ion 708 1 #20062
				L		
13		BR NEOPRENE PAD 0.125 x 10.00 x 14.00 NETC 50 NH	neoprene			1
104		3/4" SLOTTED HEAD BOLT (GALV.) 0.750" DIA. X 6.000" LG. NO SHLDR	A325			
104		(3/4") HEAVY HEXNUT (GALV) 0.750" dia	A563 gr DH			
104		(3/4") PLAIN ROUND WASHER TYPE A (GALV) 0.750" DIA USS LARGE (HI-CARBON)	F844			
32		(5/8") HEX HEAD BOLT (GALV) 0.625" DIA x 1.750" LG.	A325			
32		(5/8") PLAIN ROUND WASHER (GALV) 0.625" USS LARGE	F844			
16		3/4" SPACER PIPE 0.750" DIA. (SCH 40) x 1/2" LG.	A53 gr B			
13		ANCHOR PLATE (GALV) PL .375 x 9.375 x 13.000 w/ 6.250 dia CTR HOLE	A36	[		
52		(1") THD. ANCHOR STUD (GALV) 1.000" DIA x 12" LONG full thread	A449			
156		(1") HEAVY HEX NUT (GALV) 1.000" DIA (hi-str)	A563 DH	0	Initial submittal	15123113
52		(1") PLAIN ROUND WASHER (GALV) 1.000" DIA SAE (SMALL) HI-STR.	F436	110.	REVISIONS	, baie

Dimensions measured at face of rail

Coated by

Approved Approved as Noted Revise and Resubmit Hoyle, Tanner

Checked by

Reviewed By: Matt Belden Daniels Construction 3/30/2015

Rejected (See     accompanying letter)	HIGHWAY SAFETY	CORP
t required) No Action Required	GLASTONBURY, CT	
Returned Without Review	<u> </u>	1
Project No. <u>919101</u>	ITEM 563.24 T4 BRIDGE RAILING ITEM 565.242 T4 BRIDGE RAILING APPROACH	
al conformance with the design concept of the nation given in the Contract Documents. for dimensions to be confirmed and correlated	MANCHESTER ROAD OVER BEAVER BROOK	
n that pertains solely to the fabrication process	TOWN OF AMHERST, NEW HAMPSHIRE	HSC JOB NO.
g all work in a safe and satisfactory manner. odify Contractor's duty to comply with Contract	GENERAL CONTRACTOR	2034
JCR Date 4/9/2015	SUB CONTRACTOR CWS FENCE	1 of 4
EGW Date 4/9/2015	DRAWN PAR CHECKER DATE 03-20-15 SCALE	NONE SIZE D
	i	

![](_page_54_Figure_0.jpeg)

![](_page_54_Figure_1.jpeg)

### SOUTHEAST RAILING ELEVATION

LOOKING AT TRAFFIC FACE OF RAILING FACING SOUTHEAST FROM CENTERLINE OF ROAD

Dimensions measured at face of rail	Approved Approved as Noted (Resubmission not required) Revise and Resubmit Revise and Revise and
	Concernment designates action taken)     Project No. 919101     ITEM 563.24 T4 BRIDGE RAILING     ITEM 565.242 T4 BRIDGE RAILING APPROACH     ITEM 565.242 T4 BRIDGE RAILING APPROACH     ITEM 565.242 T4 BRIDGE RAILING APPROACH     MANCHESTER ROAD OVER BEAVER BROOK     TOWN OF AMHERST, NEW HAMPSHIRE     Section 2007 Addition of the work of     all trades: and performing all work in a safe adaption of the work of     all trades: and performing all work in a safe adaption gradery manner.     CENTERAL CONTRACTOR
Duncan Galvanizing per NHDOT section 708 Top Coat Color = Dark Brown Fed Std #20062	This approval does not modify Contractor's duty to comply with Contract Documents.       Reviewed by

![](_page_55_Figure_0.jpeg)

### RAIL NOTES:

1. ITEMS 563.24 BRIDGE RAIL, SHALL INCLUDE POSTS, BASE PLATES, ANCHOR PLATES, ANCHOR RODS, PREFORMED PADS, RAIL ASSEMBLY BOLTS, NUTS, WASHERS, STUDS, STRUCTURAL TUBING, SPLICE BARS, PIPE SPACERS, ALL APPURTENANCES, AND GALVANIZING. IF PAINTING IS REQUIRED SEE SPECIAL PROVISIONS SEC 708

2. BRIDGE RAIL POSTS SHALL BE SET NORMAL (90 DEGREES) TO THE PROFILE GRADE, EXCEPT ON GRADES OVER 5% WHERE POSTS SHALL BE SET VERTICAL.

3 ENDS OF RAIL TUBE SECTIONS SHALL BE SAWED OR MILLED AND SHALL BE TRUE AND SMOOTH. ALL CUT EDGES OF ALL MATERIAL SHALL BE GROUND SMOOTH.

4. FACH PIECE OF RAIL TUBING SHALL BE ATTACHED TO A MINIMUM OF THREE (3) POSTS.

BOLT HOLES SHALL BE DRILLED OR PUNCHED. FLAME CUTTING MAY BE USED TO FINISH SLOTTED HOLES IF MECHANICALLY GUIDED

6. AT <u>INTERIOR</u> SPLICES, PIPE SPACERS SHALL BE USED ON ONLY <u>ONE</u> SIDE OF THE SPLICE TO ALLOW MOVEMENT ON THAT SIDE, THE TOP AND BOTTOM RAIL SHALL RECEIVE THE SAME TREATMENT, AT <u>END</u> SPLICES PIPE SPACERS SHALL BE USED ON <u>BOTH</u> SIDE OF THE SPLICE TO ALLOW MOVEMENT ON EACH SIDE

7. MILL OR SHOP TRANSVERSE WELDS SHALL NOT BE PERMITTED ON ANY RAIL ELEMENT. RAIL ELEMENTS USED ON CURVES SHALL USE 3/6" WALL TUBES AND SHALL BE SHOP FORMED TO THE REQUIRED CURVATURE.

8. NO PUNCHING, DRILLING, CUTTING OR WELDING SHALL BE PERMITTED AFTER GALVANIZING. DAMAGED AREAS OF GALVANIZING SHALL BE THOROUGHLY CLEANED, PRETREATED., AND PAINTED WITH TWO COATS OF ORGANIC ZINC-RICH GALVANIZING REPAIR PAINT. HAVING MIN. 94% ZINC BY WEIGHT. TO A THICKNESS EQUAL TO THE ORIGINAL COATING ACCORDING TO THE STANDARD SPECIFICATIONS AND ASTM A780.

9 NUTS FOR 1"& THREADED ANCHOR RODS CONNECTING THE BASE PLATE TO THE CONCRETE SHALL BE TIGHTENED TO A SNUG FIT AND GIVEN AN ADDITIONAL 1/8 TURN.

10. THREADS FOR ANCHOR RODS MAY BE ROLLED OR CUT. IF CUT THREADS ARE USED. BOLT DIA. SHALL NOT BE LESS THAN NOMINAL DIAMETER. IF ROLLED THREADS ARE USED, ROD DIA. SHALL NOT BE LESS THAN ROOT DIA. OF THREADS.

### MATERIAL NOTES:

11. STRUCTURAL TUBING SHALL CONFORM TO THE REQUIREMENTS OF ASTM A500, GRADE B, STRUCTURAL STEEL TUBING, RAIL TUBING SHALL MEET THE LONGITUDINAL CHARPY V-NOTCH REQUIREMENTS OF 15 FT/LBS AT O' F. FOR ASTM A500, GRADE B, THE TEST SAMPLES SHALL BE TAKEN AFTER FORMING THE TUBES. CHARPY V-NOTCH IS NOT REQUIRED FOR SPLICE TUBES.

12. RAIL POSTS AND BASE PLATES SHALL CONFORM TO THE REQUIREMENTS OF ASTM A572 GR 50, EXCEPT ANCHOR PLATES MAY BE ASTM A36.

13. 3/4"Ø ROUND HEAD BOLTS SHALL BE ASTM A325 OR A449. ALL OTHER BOLTS AND NUTS SHALL CONFORM TO ASTM A307 AND ASTM 563 GRADE A RESPECTIVELY OR BETTER. EXCEPT THAT ASTM A307 NUTS MAY BE USED ON THE BOTTOM OF ANCHOR ASSEMBLY. WASHERS SHALL BE HARDENED STEEL COMMERCIAL TYPE A PLAIN WIDE WASHERS AND SHALL MEET THE DIMENSIONAL REQUIREMENTS OF A.N.S.I. B18.22. ANCHOR RODS SHALL CONFORM TO ASTM A449.

14. ALL STEEL COMPONENTS SHALL BE GALVANIZED AFTER FABRICATION IN CONFORMANCE TO AASHTO M232 (ASTM A153) AND AASHTO M111 (ASTM A123). THE GALVANIZING KETTLE SHALL HAVE 0.05 TO 0.09 PERCENT NICKEL. GALVANIZED SURFACES SHALL HAVE A UNIFORM APPEARANCE AND GALVANIZED MATERIAL SHALL BE PROPERLY STORED.

15. PREFORMED BEARING PADS (1/8" THICK) SHALL CONFORM TO AASHTO M251.

![](_page_55_Figure_18.jpeg)

"L" 2'-1"

![](_page_56_Figure_0.jpeg)

ILL OF MATERIAL					
Description	Spec.				
POST #1 (10-hole) W6 x 25 (8-8" OAH)					
I POST #2 (10-hole) W6 x 25 (8'-8" OAH)					
POST #3 (10-hole) W6 x 25 (8-8" OAH)					
POST #4 (10-hole) W6 x 25 (8'-8" OAH)	A572 gr 50				
	A500 or B				
TUBE FOR 8x4 RAIL angled down TS 7x3x3/8 1'-8" LG w/ 2WELDED NUTS					
TUBE FOR 4x4 RAIL TS 3x 3x516 1'-8" LG w 2 WELDED NUTS					
TUBE UPPER RAIL angled down TS 3 x 3 x 5/16" - 20.000" LG. w/ 2 WELDED NUTS (7deg FLAREBACK)	A500 gr B				
TUBE FOR 8x4 RAIL angled down TS 7x3x3/8 1"-8" LG w/ 2 WELDED NUTS (7deg FLAREBACK)	A500 gr B				
TUBE FOR 4x4 RAIL TS 3x3x5/16 1'-8" LG w/ 2 WELDED NUTS (7deg FLAREBACK)	A500 gr B				
TUBE UPPER RAIL angled down TS 3 x 3 x 5/16" - 20.000" LG. w/ 2 WELDED NUTS (10deg FLAREBACK)	A500 gr B				
TUBE FOR 8x4 RAIL angled down TS 7x3x3/8 1"-8" LG w/ 2 WELDED NUTS (10deg FLAREBACK)	A500 gr B				
TUBE FOR 4x4 RAIL TS 3x3x5/16 17-8" LG w/ 2 WELDED NUTS (10deg FLAREBACK)	A500 gr B				
RÁIL TEIRE TS 4 x 4 x 1/4" x 9-4 000" ()ÁI	A500 Gr. B				
CRAIL TUBE TS 8 x 4 x 5/16" x 9-4.000" OAL	A500 Gr. B				
ALL TUBE TS 4 x 4 x 1/4" x 9-4000" OAL	A500 Gr. B				
PRAIL TUBE TS 4 x 4 x 1/4" x 8-11 625" OAI	A500 Gr. B				
HAND FLAREBACK TS 4 x 4 x 1/4 x 27-0 4375" OAL	A500 Gr. B				
AND FLAREBACK TS 4 x 4 x 1/4 x 27-0 4375° OAL	A500 Gr. B				
RAIL TUBE TS 4 x 4 x 3/8" x 9-4.000" OAL (SHOP CURVE 30"R)	A500 Gr. B				
CRAIL TUBE TS 8 x 4 x 3/8" x 9-4.000" OAL (SHOP CURVE 30'R)	A500 Gr. B				
RAIL TUBE TS 4 x 4 x 3/8" x 9"-4.000" OAL (SHOP CURVE 30"R)	A500 Gr. B				
RAIL TUBE TS 4x 4x 3/8" x 8-11.625" OAL (SHOP CURVE 30"R)	A500 Gr. B				
AND FLAREBACK TS 4 x 4 x 3/8 x 27-0.4375" OAL (SHOP CURVE 30'R)	A500 Gr. B				
CTION PLATE 3/8 x 1"-8" x 2-3" OAL w/ bottom clipped 3"	A36				
NP 3/16" x 4" x 8"	A36				
.₽ 3/16" x 4" x 4"	A36				
FFSET BLOCK TS 5x 4x 1/4 x 6"	A500 Gr. B				
POST W 06 x 008.5# x 07'00" 2 hls @ 7" / 2 hls @ 14.625" / 2 hls @ 27.313"	A36				
TION POST W06 x 008.5# x 0700" 2 hls @ 6.875" / 2 hls @ 11.375" / 2 hls @ 25.875"	A36				
I POST W06 x 008.5# x 0700" 2 hls @ 7.000" / 2 hls @ 23.500"	A36				
THYLENE BLOCK 4.0" x 8.0" x 21.5" ROUTED	POLY				
THYLEN BLOCK THRIE 4" x 8" x 18" ROUTED FOR TRANSITION POST	POLY				
THYLENE BLOCK 4.0"x 8.0" x 14.0" ROUTED	POLY				
IAL CONNECTOR 10 Ga. (with slotted holes)	M180 B2				
PANEL 12GA x 12'06.00" x 01'06.75" HDG	M180 A2				
PANEL 12GA x 12'06.00" x 01'06.75" HDG (SHOP CURVE 39'R CONVEX)	M180 A2				
RANSMON PANEL 10 Ga.	M180 B2				
OTTED HEAD BOLT (GALV.) 0.750" DIA. X 6.000" LG. NO SHLDR	A325				
X HEAD BOLT (GALV.) 0.625" DIA. X 1.750" LG.	A325				
LICE BOLT (GALV.)0.625" DIA. X 1.250" LG.	A307				
ACER PIPE 0.750" DIA. (SCH 40) x 1/2" LG.	A53 gr B				
AVY HEX NUT (GALV.) 0.750" DIA.	A563 DH				
WIDE WASHER(0.813"LD., 2.000" O.D., 5/32" THK)	F436				
. RECESSED NUT (GALV.) 0.625" DIA.	A563 Gr. A				
LAIN ROUND WASHER (GALV) 0.625" USS LARGE	F844				
WASHER (GALV.) 0.188" x 1.750" x 3.000" OAL	A36				
OST 0.625-11 x 09.500 HDG (5/8) A307	A307				
UND HEAD BOLT (GALV.) 0.750° DA. X 2.000° LG.	A307				
OST 0.625-11 x 06.000 GALV (5/8) A307	A307				

All steel bridge rail material to be Galvanized & Powder Coated by Duncan Galvanizing per NHDOT section 708 Top Coat Color = Dark Brown Fed Std #20062

Rejected (See		HIG	HWAY	SAFE	ΓY	C(	ЭR	Ρ
uired) No Action Required Submital Not Requested/ Returned Without Review		GLASTONBURY, CT 860-633-9445						
esignates action taken) Project No. <u>919101</u>	ITEM	ITEM 56 565.242	3.24 T4 B T4 BRIDGE	RIDGE RAILING RAILING APPRO	ACH	SCE		IED
nformance with the design concept of the given in the Contract Documents. imensions to be confirmed and correlated	мл	ANCHESTE	R ROAD OVE	R BEAVER BROO	к			
pertains solely to the fabrication process f construction; coordination of the work of work in a safe and satisfactory manner.	GENERAL C	TOWN OF	F AMHERST, N	IEW HAMPSHIRE		ныс лов на	。 <u>0</u> 3-	4
R DateDate	SUB CONTR	SUB CONTRACTOR CWS FENCE				sheet no. 4	of	4
W Date 4/9/2015	DRAWN	PAR	CHECKED	DATE 03-20-15	SCALE	NONE	SIZE	D