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# Verification Study of a Bathystrophic Storm Surge Model

by  
George Pararas-Carayannis

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Louisiana; (d) Hurricane Camille at Biloxi, Mississippi; and (e) Hurricane Carol at Narragansett Pier, Rhode Island. Comparisons were made with theoretical results for several hypothetical storm surge problems for which analytical solutions could be obtained. Although reasonable empirical solutions were obtained by combining values of initial rise and of coefficients of bottom friction and wind stress, the significance, variation and interdependence of these parameters could not be determined adequately because of limited historical data. Extrapolation of empirically derived wind stress and bottom friction relationships, as determined from lower windspeeds, to extreme probable maximum conditions associated with the synthetic hurricanes, could not be conclusively verified. Because of the complexity of the problem, data limitations, and the variability of different factors entering the calibration process, correlation for all historical hurricanes at all traverses was difficult to obtain. However, overall evaluation indicates that when the assumptions used in developing the bathystrophic storm surge model are satisfied, the model reproduces theoretical solutions with an acceptable degree of accuracy.



## PREFACE

This report is published to assist coastal engineers in the computation of design storm surge resulting from the combined meteorologic, oceanic, and astronomic effects coincident with the arrival of a hurricane at the coast for use in the planning and design of protective coastal works. The work was carried out under the coastal construction research program of the U.S. Army Coastal Engineering Research Center (CERC).

It was undertaken as a joint effort by CERC with support from the U.S. Atomic Energy Commission (AEC) under contract AT(49-1) 3318 to verify a CERC numerical model of hurricane surge prediction with data of historical hurricanes.

Construction of nuclear powerplants and superports and other large coastal installations have necessitated the development of conservative criteria in obtaining estimates of potential storm surges. Prediction of hurricane surge elevations is based primarily on the use of analytic and numerical models, since it is difficult to study these events in real time or with the use of physical models. The earliest and simplest of the numerical models for hurricane-induced storm surge, in which the most essential two-dimensional characteristics of the hurricane and the response of the sea are considered, is called the bathystrophic storm surge model. This model has been widely used because of its simplicity and relatively modest computer requirements, and is applied to several historical storms in this report to show its strengths and weaknesses. A computer program for simulating the hurricane wind and pressure fields from a few parameters, and for computing the bathystrophic storm surge is included. A variable astronomical tide may be used. The basic computer program was developed by Mr. B. R. Bodine of CERC. Changes were made to permit the use of wind data from both actual and simulated hurricanes.

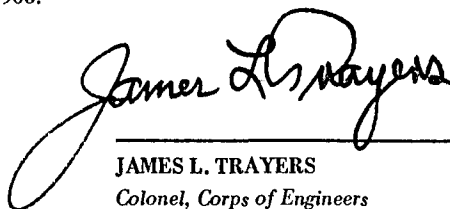
Historical hurricane data for the calibration and verification of the model were developed by cooperative efforts with the AEC staff and the consultant firm of Nunn, Snyder and Associates; particularly, the preparation of wind-field data reductions for computer use for Hurricanes Carla and Audrey, and suggestions which were incorporated in this version of the model.

This report was prepared by Mr. George Pararas-Carayannis, Oceanographer, Design Branch, under the direct supervision of Mr. R. A. Jachowski, Chief, Design Branch, Engineering Development Division. Mr. H. D. McClung was responsible for organizing computer program documentation and making computer program revisions. Mr. J. C. Alquist assisted with programming revisions. The report was particularly benefited by comments and suggestions of Messrs. L. G. Hulman, E. Hawkins and W. S. Bevins, AEC, and from discussions with Messrs. B. R. Bodine and P. N. Stoa, and Drs. D. L. Harris, and B. E. Herchenroder, CERC.

A more powerful, fully two-dimensional, hurricane storm surge prediction model is used operationally by the National Weather Service, NOAA. Two versions of this model, called SPLASH I and SPLASH II, are available from the National Technical Information Service (NTIS), Springfield, Virginia 22151. The computer tape is listed as Number COM-75-101-80/AS and the Users Guide as COM-75-101-81/AS. Although the SPLASH models are considerably more complex and require more computer facilities than the bathystrophic models, they require fewer assumptions, and are valid for a wider range of considerations. They are undergoing continual development to increase the accuracy of the predictions.

Comments on this publication are invited.

Approved for publication in accordance with Public Law 166, 79<sup>th</sup> Congress, approved 31 July 1945, as supplemented by Public Law 172, 88<sup>th</sup> Congress, approved 7 November 1963.



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JAMES L. TRAYERS  
Colonel, Corps of Engineers  
Commander and Director

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## LIST OF SYMBOLS

A	kinematic form of wind stress
B	kinematic form of wind stress
$C_1$	dimensional constant
$C_2$	dimensional constant
$C_3$	dimensional constant
D	depth of water at edge of Continental Shelf
d	depth of water on the Continental Shelf (below SWL)
f	Coriolis parameter in radians per hour ( $f = 2\omega \sin \theta$ )
g	gravitational acceleration
in. hg.	inches of mercury
k	dimensionless surface stress coefficient
K	bottom friction coefficient (empirical constant)
$K_1$	constant part of the wind-stress coefficient
$K_2$	constant multiplier of the velocity-dependent part of wind-stress coefficient
L	length of disturbance
$M_{xx}$	momentum transport quantity
$M_{xy}$	momentum transport quantity
$M_{yy}$	momentum transport quantity
mb	millibar
p	precipitation rate (depth/time)
$p_n$	peripheral pressure (inches of mercury)
$p_o$	central pressure (inches of mercury)
R	radius of maximum winds (nautical miles)
r	distance from storm center to points on traverse
$R_E$	radius of the earth
S	setup (cumulative water elevation)
$S_A$	setup due to astronomical tide
$S_e$	initial setup of the water level at the time the storm surge computations are started
$S_L$	local setup or setdown due to such local effects as inland runoff or the coastal hydrography

## LIST OF SYMBOLS—Continued

$S_{\Delta p}$	atmospheric pressure setup
$S_T$	total setup at shore
$S_w$	wave setup at shore due to breaking waves
$S_x$	x-component of storm setup
$S_y$	y-component of storm setup
$T$	shear stress at boundary
$t$	time
$U$	x-component of volume transport per unit width
$u$	x-component of current velocity
$U_{max}$	maximum gradient windspeed (miles per hour)
$U_R$	maximum windspeed 30 feet above the water (miles per hour)
$V$	y-component of volume transport per unit width
$v$	y-component of current velocity
$V_F$	forward speed (miles per hour or knots)
$W$	windspeed
$W_c$	critical windspeed taken as 16 miles per hour
$W_o$	wind velocity at a given location along the "prime vector"
$W_x$	x-component of windspeed
$W_y$	y-component of windspeed
$X_n$	distance of each point of the traverse from the shore intercept of the traverse
$\Delta_x$	discrete increment of distance along x-axis
$\Delta_y$	discrete increment of distance along y-axis
$a$	azimuth of traverse
$\beta$	azimuth of line referenced by $r$
$\gamma$	dimensionless resistance coefficient
$\delta$	computed angle between the traverse and $r$
$\epsilon$	angle between "prime vector" and the radius from storm center to any point on the traverse
$\zeta$	astronomical tide potential in head of water
$\theta$	angle of wind direction measured counterclockwise from the x-axis

### LIST OF SYMBOLS—Continued

$\xi$	atmospheric pressure deficit in head of water
$\rho$	mass density of water
$\tau_{bx}$	x-component of bottom stress
$\tau_{by}$	y-component of bottom stress
$\tau_s$	resultant of x- and y-components of surface wind stress
$\tau_{sx}$	x-component of surface wind stress
$\tau_{sy}$	y-component of surface wind stress
$\phi$	geographical latitude in degrees
$\omega$	angular velocity of earth equal to $2\pi/24$ radians per hour ( $7.29 \times 10^5$ radian/second)



# VERIFICATION STUDY OF A BATHYSTROPHIC STORM SURGE MODEL

by

*George Pararas-Carayannis*

## I. INTRODUCTION

### 1. General.

Hurricanes are severe tropical cyclones with winds spiraling inward toward a center or eye of low pressure at speeds which may reach more than 150 miles per hour (130 knots). Although usually erratic and unpredictable, hurricanes generally follow a westerly to northwesterly path toward the gulf or Atlantic coasts causing abnormal water level fluctuations. These water level fluctuations are called *storm surges* and are caused by an atmospheric pressure field and wind stress on the water surface, accompanying moving storm systems. Specific factors which can combine to produce extreme water fluctuations at a coast during the passage of a storm include: storm intensity, size, path, and duration over water; atmospheric pressure variation; speed of translation; winds and rainfall; bathymetry of the offshore region; astronomical tides; initial water level rise; surface waves and associated wave setup and runup.

Hurricane surge is an oceanographic phenomenon and constitutes a greater hazard to lives and coastal property than hurricane winds. Hurricane surges have been estimated to account for 75 to 90 percent of all deaths resulting from a hurricane. Surge inundation is also responsible for extensive damage to coastal property. Since 1900, hurricane damages to coastal property have averaged more than \$50 million per year (Perdikis, 1967). The storm that hit Galveston in 1900 resulted in 6,000 deaths and the almost complete destruction of a large part of the city. Hurricane Camille, which struck the Mississippi coast in 1969, killed 262 persons and caused damages of nearly \$1 billion (U.S. Army Engineer District, Mobile, 1970).

The development of an improved hurricane warning system has reduced the loss of life by recent large storms. Hurricane Carla, which struck the Texas coast in 1961, killed 32 persons. Hurricane Betsy hit near New Orleans in 1965, and was responsible for 81 deaths. However, the damage to property continued to rise because of the continuing development of the coastal zone. The damage from Hurricane Carla was more than \$400 million (U.S. Army Engineer District, Galveston, 1962). Damages from Hurricane Betsy were estimated to be \$372 million (Perdikis, 1967).

Increasing pressure for large coastal installations, such as powerplants, and superport terminals, and the increasing residential development of the coastal zones, have emphasized the need for more accurate estimates of potential storm surge hazards.

The prediction of storm surge resulting from the combined meteorologic, oceanic and astronomic effects coincident with the arrival of a hurricane at the coast is a very important problem, but rather a difficult one to solve. The present capability for the prediction of hurricane surge is based primarily on the use of analytic and numerical models, none of which have been adequately verified. Furthermore, the interactions between tide and storm surge in shallow waters are not well understood. Nonetheless, a consideration of water level fluctuation due to the passage of a hurricane is essential for the planning and design of coastal engineering works.

## 2. Design Storms.

The Corps of Engineers is interested in knowing the extent and elevation of flooding induced by hurricane surges for various reaches along the gulf and Atlantic coasts and in developing guidelines for determining engineering design criteria.

Hurricane wind and pressure field, and surge hydrograph data are not available for most of the gulf and Atlantic coasts. Therefore, design analysis for coastal structures requires that a uniform and conservative approach be taken based on statistical probabilities of hurricane occurrence. Based on this approach, and considering the degree of desired protection, the U.S. Army, Corps of Engineers established specific storm characteristics for use in the design of coastal structures (Graham and Nunn, 1959; U.S. Weather Bureau, HUR 7-97, 1968). The storms that are used for establishing design criteria are termed *hypothetical hurricanes*, or *hypohurricanes*.

Two hypothetical storm terms, dependent on geographical location, have been established for practical application in the analysis of the design of coastal structures. These are the *standard project hurricane* (SPH), and the *probable maximum hurricane* (PMH). The meteorological considerations for these hurricanes are based on central pressure index (CPI), peripheral pressure ( $p_n$ ), radius to maximum winds (R), maximum gradient windspeed ( $U_{max}$ ), maximum windspeed ( $U_R$ ), and speed of translation ( $V_F$ ). Generally, synthetic hurricanes are classified by speed of translation and length of the radius from the eye of the hurricane to the region of maximum winds. Specifically, synthetic hurricanes are classified as of large, medium, or small radius, each with a high, moderate, or slow speed of translation. The choice of which design hurricane to use for a coastal structure depends on economics and desired degree of protection.

Graham and Nunn (1959), from an analysis of historic hurricanes, developed criteria for a design storm where the CPI has an occurrence probability of once every 100 years. This is the storm termed *standard project hurricane* (SPH). It is defined as a *synthetic hurricane* intended to represent the most severe combination of hurricane parameters that are reasonably characteristic of a specified region, excluding rare combinations. The probability of the SPH occurrence would be less than the CPI probability as determined by Graham and Nunn (1959), because other parameters have also been selected from historical maximum values.

The maximum gradient windspeed ( $U_{max}$ ) and maximum windspeed ( $U_R$ ) in the zone of maximum winds in miles per hour of the SPH can be determined by the following equations:

$$U_{max} = K(p_n - p_o)^{1/2} - R(0.575 f), \quad (1)$$

$$U_R = 0.865 U_{max} + 0.5 V_F, \quad (2)$$

where  $K$  is taken as 73 for the SPH;  $p_n$  and  $p_o$  are the peripheral and central pressures in inches of mercury, respectively;  $R$  is the radius of maximum winds in nautical miles;  $f$  is the Coriolis parameter (bathystrophic effect) in radians per hour; and  $V_F$  is the forward speed in miles per hour.  $U_R$  is the maximum windspeed 30 feet above the water in miles per hour.

Construction of nuclear powerplants in the coastal zone required the use by the Atomic Energy Commission (AEC) (function now assigned to the Nuclear Regulatory Commission) of an extreme hurricane termed the *probable maximum hurricane* (PMH). The PMH was selected by AEC to ensure that there would be no likelihood that the safety aspects of powerplants would be compromised during severe hurricane surges (including wind-generated surface waves). The AEC concluded that adequate safety against flooding would be provided if the safety-related facilities of powerplants were designed to withstand a flood protection level, based on the PMH surge. In a report by the U.S. Weather Bureau (HUR 7-97, 1968), and in the *Shore Protection Manual* (U.S. Army, Corps of Engineers, Coastal Engineering Research Center, 1973) the PMH is defined as "a hypothetical hurricane having that combination of characteristics which will make the most severe storm that is reasonably possible in the region involved, if the hurricane should approach the point under study along a critical path and at an optimum rate of movement."

This application of PMH parameters defined in HUR 7-97, is considered proper as such a hurricane is associated with conditions reasonably possible. The wind field development for the PMH as it relates to radius of maximum winds, windspeed ( $U_R$  and  $U_{max}$ ), and forward speed of the storm, is described in HUR 7-97. One of the differences between the SPH and PMH parameters is that the peripheral pressure,  $p_n$ , for the SPH is taken as a constant value of 29.92 inches of mercury while,  $p_n$ , for the PMH varies as a function of latitude. In addition, the factor  $K$  is taken as a constant for the SPH and is considered a function of the latitude for the PMH. The CPI for the PMH was not assigned a particular probability of occurrence, but was taken as only an extreme event because of the relatively small amount of data available. Storms, other than the SPH, with different CPI probabilities of occurrence have been used to obtain hurricanes of various intensities for the design of coastal structures based on economics and the desired degree of protection.

### 3. Numerical Models.

Freeman, Baer, and Jung (1957) introduced a quasi-two-dimensional model by approximating the setup due to the alongshore transport of water and the bathystrophic flow, in addition to the setup normal to a coast.

The Bathystrophic Storm Tide Theory, developed by Freeman, Baer, and Jung, has been used by some investigators in predicting storm surge elevations. Bretschneider and Collins (1963) used the theory to predict the open-coast hurricane surge at Corpus Christi, Texas and vicinity. Marinos and Woodward (1968) modified the model used by Bretschneider and Collins to compute surge elevations for several locations along the Texas coast.

A quasi-two-dimensional mathematical model developed by Bodine (1971) has been used by the Coastal Engineering Research Center (CERC) to determine storm surge design water level elevations for coastal structures. This model is also based on theoretical approximations of the governing hydrodynamic equations, proposed by Freeman, Baer and Jung (1957); it calculates storm surge based on parameters of both actual and synthetic hurricanes. This numerical model was used in the verification study presented in this report.

### 4. Objective Scope and Significance of Present Investigation.

In CERC's bathystrophic storm surge model the basic equations describe in simplified form the water motions associated with nearly horizontal flows, as those of the hurricane surge, and provide a reasonable estimate of the surge. The mathematics, geometry and basic assumptions of the numerical method, have been presented by Bodine (1971) and are summarized later in this report. Because of the simplicity of this numerical model and the number of assumptions on which it is based, further verification was necessary. The objective of the present investigation was to verify the model by comparing computed storm surge levels with those observed or recorded along the open coasts during the passage of actual storms.

Review of available hurricane data revealed that few significant hurricanes of record were sufficiently documented to provide a valid basis for analysis and verification. The following hurricanes and traverse lines (computational lines) along both the gulf and the Atlantic coasts were selected for verification: (a) Hurricane of 3 October 1949: Traverses at Galveston and Freeport, Texas, (b) Hurricane Carla, 1961: Traverses at Galveston and Freeport, Texas; (c) Hurricane Audrey, 1957: Traverse at Eugene Island, Louisiana; (d) Hurricane Camille, 1969: Traverse at Biloxi, Mississippi; and (e) Hurricane Carol, 1954: Traverse at Narragansett Pier, Rhode Island.

Wind stress and bottom friction coefficients are significant in the numerical model and affect the computation of surge. Of these two coefficients, wind stress has the more



pronounced effect on surge elevation, and according to Van Dorn (1953), appears to vary with the windspeed. Initial rise and astronomical tide can also have a pronounced effect on final surge elevation. Proper determination of these parameters from tide gage recordings is necessary. No calibration of these parameters can be made. Therefore, the goal of the present investigation was limited in ascertaining which coefficients of wind stress and bottom friction when used in conjunction with the bathystrophic model of hurricane surge prediction, initial rise and astronomical tide, would yield surge levels comparable to those of historic hurricanes.

## 5. Data Limitations.

In engineering design, the normal practice is to use a hypothetical hurricane as the design storm to predict storm surge levels for various coastal locations. Such predictions are usually determined by mathematical models, because of the difficulty in simulating the complex surge generating processes with physical hydraulic models. In a physical model, it is difficult if not impossible, to duplicate the complex wind pattern of a hurricane, or the Coriolis effects. However, reasonable estimates of water level fluctuations at a coast due to hurricanes can be obtained in a numerical model by taking into account the physical laws governing the interactions between wind, water, land, and differences in atmospheric pressures. However, comparison and verification of the model are not simple due to the lack of sufficient historic hurricane wind field and surge hydrographic data. Only limited historic information is available for the gulf and Atlantic coasts.

*a. Hurricane Data.* Sufficient hurricane wind data is a prerequisite for reliable storm surge calculations. However, wind information on weather charts of most historic hurricanes is sporadic and is given for infrequent time intervals; therefore, the data cannot always permit the reduction of the wind field along a computational line (traverse).

*b. Hydrographic Data.* Reliable tide gage recordings of the hurricane surge are important in the verification of a numerical model. Most tide gages are located in sheltered areas and do not record nor represent the peak storm surge at the open coast. The few tide stations located on the open coast are widely separated and often are not near paths of hurricanes. Therefore, most tide gage data used in the calculations of surge on the open coast were from tide gages located some distance away. To avoid errors, corrections must be applied to account for phase differences in astronomical tides between the tide gage station and the point on the open coast where surge is computed. Similarly, corrections should be applied to the recorded hydrographs because of differences in the water level datum of tide gages, e.g., some of the recorded or observed surge data associated with hurricanes are reported in the literature relative to mean sea level (MSL). Other tide gage data are given relative to mean low water (MLW).

When tide gage records are not available, peak storm surge elevations on the open coast are often obtained visually from debris marks, which generally include wave-induced setup

and runup. Then, it is impossible to determine accurately how much of the water level elevation is from surge and how much is from wave action. The observed levels in these cases are higher than those of the surge alone.

*c. Initial Water Levels.* Water levels along the coast have been observed for long periods of time before the arrival of a hurricane, and are different than the predicted astronomical tide. This difference in water level between observed and predicted tide appears to be independent of the storm or the astronomical forces, and is referred to as the *initial water level*. Harris (1963) has shown that such water level deviations can be as much as 2 feet above mean water level (MLW) preceding the approach of a hurricane. Others have attributed this initial rise to a "forerunner" of the storm, while Harris maintains that it is due to short-period anomalies in the mean sea level not related to the hurricane, and that predicted astronomical tides do not take this phenomenon into account. In some instances this initial rise is believed to be a significant factor in the total water level observed on the coast during the passage of storm systems and therefore cannot be ignored. This additional component is particularly important when determining the total water level for a synthetic hurricane as the PMH.

The initial water rise used as input to the numerical calculation of storm surge is usually the average difference between predicted astronomical and observed tides at a tide gage station at or closest to the shore intercept (open coast) of the traverse prior to the influencing effects of hurricane winds and pressures. In the numerical calculations, this value is treated as a constant and is added to the total water level. This may be an over-simplification, since the cause for such rise or its exact magnitude during the passage of the storm is unknown. Whatever the cause, however, the initial water level should be considered when evaluating the open-coast storm surge. This value is determined before the hurricane reaches the edge of the Continental Shelf or at a time when winds on the shore are less than 15 miles per hour. No available definition exists for identifying or measuring a value of initial rise for historic hurricanes. Because of the significance of the assumption of initial rise in hurricane verification, specific definition of initial rise for use in hurricane verification studies may be appropriate. Therefore, for the purpose of calibration, initial rise can be defined as "the average water level variation above the predicted astronomical tide at a station during the 2 days preceding the occurrence of a 15- to 20-mile per hour isovel of a hurricane advancing across the Continental Shelf."

Although there are numerous assumptions and data limitations, verification of the numerical computation of storm surge is possible with the data of a few recent hurricanes. In this study, the data were taken from a few well documented hurricanes. An important aspect of any future verification study will be the increased accuracy of basic data, the collection of additional historical data on storm surge wave action along coastlines, and data on the extent of coastal flooding.

## II. THE BATHYSTROPHIC STORM SURGE MODEL

The numerical model described here is based on the Bathystrophic Storm Tide Theory and is used to estimate the rise of water on the open coast taking into account the combined effects of direct onshore and alongshore wind-stress components on the surface of the water, and the effects of the Coriolis force (bathystrophic effect), and the different pressure effects. This model can be described as a quasi-one-dimensional numerical scheme, which is a steady-state integration of the wind stresses of the hurricane winds on the surface of the water from the edge of the Continental Shelf to the shore, taking into consideration some of the effects of bottom friction and the alongshore flow caused by the earth's rotation. The bathystrophic contribution to hurricane surge can be explained as follows: In the northern hemisphere hurricane winds approaching the coast have a counterclockwise motion. Because of the Coriolis effect, the flow of water induced by the cyclonic winds will deflect to the right, causing a rise in the water level. The bathystrophic storm tide, therefore, is important in producing maximum surge even when winds blow parallel to the coast. Coastal morphology may also affect the extent of rise of water. However, in this model the surge is calculated only along a single traverse line at a time over the Continental Shelf for a straight open-ocean coast.

The model uses the onshore and alongshore wind-stress components of a moving wind field over the Continental Shelf, and a frictional component of bottom stress. The nonlinear storm surge is computed at selected points along the traverse by integrating numerically the one-dimensional hydrodynamic equations of motion and continuity.

The computed surge is a composite of water elevation obtained from components of the astronomical tide, the atmospheric pressure, the initial rise, the rises due to wind and bottom friction stresses, and wave setup.

### 1. Basic Assumptions, Conditions, and Limitations.

The bathystrophic theory on which this model is based, represents an approximation to the complete storm-generation process. Therefore, the model is limited by a number of initial conditions and assumptions. In most instances, the bathystrophic approximation appears to give a reasonable estimate of the open-coast surge; however, at times the surge estimate could be in error by a factor of 2 or more. The basic equations which govern the generation of storm surge will not be derived here, but to understand the bathystrophic approximation and its limitations, it is important to emphasize the assumptions, initial conditions, and the hydrodynamic processes neglected in development.

The following initial conditions are placed on the basic equations which govern storm surge generation (Bodine, 1971):

a. The hurricane creates a disturbance on the ocean surface of such horizontal dimensions so that  $L \gg D$  and  $L \ll R_E$ , where  $L$  is the length of the disturbance;  $D$  is

the depth of the water (at the edge of the Continental Shelf), and  $R_E$  is the radius of the earth. It is also assumed that:

(1) The space derivatives of the current velocity and acceleration can be neglected. Thus, the vertical pressure gradient is hydrostatic, and vertical accelerations are negligible.

(2) The curvature of the earth can be neglected and a flat earth approximation can be used.

b. The acceleration due to the earth's rotation is a constant.

c. Water density  $\rho$  is a constant, and internal forces due to viscosity can be neglected.

d. The seabed is fixed, impermeable, and forms the lower boundary.

e. Surface storm waves are linearly superimposed on storm surge. A basic assumption of the model is that the surge involves only horizontal fluid motions. In respect to wave theory, such horizontal fluid motions are often referred to as *long waves*. The water motion associated with the propagation of long waves is in a continuous state of gradual change.

## 2. Hydrodynamic Equations.

Integrations of the primary hydrodynamic equations describing the storm surge problem have been shown by Haurwitz (1951), Welander (1961), Fortak (1962), Platzman (1963), Reid (1964), and Harris (1967). These derivations show the actual approximations involved. In the bathystrophic model by Bodine (1971), the basic equations are the simplified and vertically integrated equations of continuity expressing conservation of mass and motion, according to Newton's second law. The equations were taken directly from Bodine in integrated form for the purpose of illustrating the principal approximations of the bathystrophic model.

The governing two-dimensional hydrodynamic equations in a volume-transport form, appropriate for tropical or extratropical storm surge problems on the open coast and in enclosed or semienclosed basins, are as follows:

$$\frac{\partial U}{\partial t} + \underbrace{\frac{\partial M_{xx}}{\partial x} + \frac{\partial M_{xy}}{\partial y}}_{\text{Advection of Momentum}} = \underbrace{+ fV}_{\text{Coriolis}} - \underbrace{gD \frac{\partial S}{\partial x}}_{\text{Surface Slope}} + \underbrace{gD \frac{\partial \xi}{\partial x}}_{\text{Inverse Barometer}} + \underbrace{gD \frac{\partial \zeta}{\partial x}}_{\text{Astro. Tide Potential}} + \underbrace{\frac{\tau_{sx}}{\rho}}_{\text{Wind Stress}} - \underbrace{\frac{\tau_{bx}}{\rho}}_{\text{Bottom Stress}} + \underbrace{W_x P}_{\text{Rainfall Rate}} \quad (3)$$

$$\frac{\partial V}{\partial t} + \underbrace{\frac{\partial M_{yy}}{\partial y} + \frac{\partial M_{xy}}{\partial x}}_{\text{Advection of Momentum}} = -fU - \underbrace{gD \frac{\partial S}{\partial y}}_{\text{Surface Slope}} + \underbrace{gD \frac{\partial \xi}{\partial y}}_{\text{Inverse Barometer}} + \underbrace{gD \frac{\partial \zeta}{\partial y}}_{\text{Astro. Tide Potential}} + \underbrace{\frac{\tau_{sy}}{\rho}}_{\text{Wind Stress}} - \underbrace{\frac{\tau_{by}}{\rho}}_{\text{Bottom Stress}} + \underbrace{W_y P}_{\text{Rainfall Rate}} \quad (4)$$

$$\frac{\partial S}{\partial t} + \frac{\partial U}{\partial x} + \frac{\partial V}{\partial y} = P. \quad (5)$$

where  $M_{xx}$ ,  $M_{yy}$ ,  $M_{xy}$ ,  $U$ , and  $V$  are given by:

$$M_{xx} = \int_{-d}^S u^2 dz; \quad M_{yy} = \int_{-d}^S v^2 dz; \quad M_{xy} = \int_{-d}^S uv dz,$$

$$U = \int_{-d}^S u dz; \quad V = \int_{-d}^S v dz .$$

where,

$U, V$  = x and y components, respectively, of volume transport per unit width,

$t$  = time,

$M_{xx}^{\circ}, M_{yy}^{\circ}, M_{xy}^{\circ}$  = momentum transport quantities,

$f = 2\omega \sin \phi$  = Coriolis parameter,

$\omega$  = angular velocity of earth =  $2\pi/24$  radians per hour  
( $7.29 \times 10^{-5}$  radians per second),

$\phi$  = geographical latitude,

$\tau_{sx}, \tau_{sy}$  = x and y components of surface wind stress,

$\tau_{bx}, \tau_{by}$  = x and y components of bottom stress,

$\rho$  = mass density of water,

$W_x, W_y$  = x and y components of windspeed,

$\xi$  = atmospheric pressure deficit in head of water,

$\zeta$  = astronomical tide potential in head of water,

$u, v$  = x and y components, respectively, of current velocity,

$p$  = precipitation rate (depth/time),

$S$  = setup of water surface above SWL,

$g$  = gravitational acceleration,

$D$  = depth of water at edge of Continental Shelf,

$d$  = depth of water on the Continental Shelf,

and

$\theta$  = angle of wind measured counterclockwise from the x-axis.

Equations (3) and (4) are the equations of motion, while equation (5) is the continuity relation for a fluid of constant density. These basic equations provide a complete description of nearly all horizontal water motions resulting from hurricane surge. An exact solution of these equations would be very desirable for solving the surge problem; however, it is difficult to obtain. The model described here obtains only a useful approximation by ignoring some terms in the basic equations. Accurate solutions can only be acquired by retaining the full two-dimensional characteristics of the surge problem.

### 3. The Bathystrophic Model Approximation.

Application of the Bathystrophic Storm Tide Theory of Freeman, Baer, and Jung (1957) to the hydrodynamic equations of motion and continuity for the solution of the surge problem requires a number of assumptions. According to Bodine (1971) these assumptions imply that: (a) there is no volume transport normal to the shore, (b) the onshore wind setup responds instantaneously to the onshore wind stress, (c) advection of momentum (field acceleration) is negligible, (d) the alongshore sea surface height is uniform, and (e) precipitation can be neglected. When applied to the terms of the equations of motion and continuity (1), (2), and (3), according to Bodine these assumptions have the following physical significance:

$$\frac{\partial U}{\partial t}, fU, \frac{\partial U}{\partial x}, \frac{\tau_{bx}}{\rho} \rightarrow 0, \text{ (no onshore water volume transport),}$$

$$M_{xx}, M_{yy}, M_{xy} \rightarrow 0, \text{ (the advection of momentum can be neglected),}$$

$$\frac{\partial S}{\partial y}, \frac{\partial V}{\partial y} \rightarrow 0, \text{ (the alongshore sea surface and current are uniform),}$$

$$P \rightarrow 0, \text{ (the precipitation can be neglected),}$$

$$\frac{\partial \xi}{\partial x}, \frac{\partial \xi}{\partial y} \rightarrow \text{(The barometric effects are neglected in this approximation, but are accounted for elsewhere. These effects are discussed later.)}$$

$$\frac{\partial \zeta}{\partial x}, \frac{\partial \zeta}{\partial y} \rightarrow \text{(The astronomical tide effects are neglected in this approximation but are accounted for in the final estimate of the surge on the coast. These effects are discussed later.)}$$



Based on these assumptions, the motion equations (3) and (4), reduce to the following approximations:

$$gD \frac{\partial S}{\partial x} = fV + \frac{\tau_{sx}}{\rho} , \quad (6)$$

$$\frac{\partial V}{\partial t} = \frac{\tau_{sy} - \tau_{by}}{\rho} . \quad (7)$$

The mass-continuity equation (5) is disregarded in the bathystrophic approximation because the assumptions make it unnecessary for a unique solution, and of no interest, for each term has been set equal to zero. The reduced equations (6) and (7) are now quasi-two-dimensional, since their solution can be obtained only along a single axis, the x-axis; however, the y-axis bathystrophic component of transport is retained and can be accounted for. The forces and responses for the bathystrophic approximation are shown in Figure 1. The weakness of the bathystrophic surge model lies on the numerous approximations outlined here. A reduction in the number of these approximations and the solution of the hydrodynamic equations in a more complete form may result in better estimates of storm surges.

#### 4. Wind and Bottom Frictional Stresses.

Equations (6) and (7) include the x- and y-components of wind stress on the surface of the water,  $\tau_{sx}$  and  $\tau_{sy}$ , and the y component of bottom stress due to the water motion,  $\tau_{by}$ . These are frictional stresses which need to be quantified for the solution of the surge problem. Formulas derived from theoretical solutions and physical experiments have been introduced which provide reasonable values of frictional forces at the bottom and water surface boundaries. However, these stress values have not been verified for the entire spectrum of conditions encountered in nature. Furthermore, the surface wind stresses and the bottom stresses must be "coupled" for the numerical computation of hurricane surge described in this report. Although surface and bottom stresses can be obtained individually from empirical experiments, the combined interaction of these stress factors does not obey a linear relationship. Friction models which take into account the interaction of surface wind and bottom friction stresses have been proposed by Reid (1957), Platzman (1963), and Jelesnianski (1967, 1970). These proposals, however, fail to describe the interactions of stresses in shallow water near the shore. Similarly, extrapolation of the surface wind stress relationship, as determined from lower windspeeds, to extreme wind conditions may not be realistic, as there may be interaction of other unknown or unmeasurable variables. Nonetheless, reasonable estimates of boundary stresses can be obtained if simplified stress laws are used, and the influence of vertical velocity distribution is neglected. Assuming horizontally uniform flows of wind and water at the water surface and seabed boundaries,

NOTE: Various scales have been distorted to give a clearer pictorial representation.

LEGEND:

- SWL = Stillwater Level  
 $S_T$  = Total Setup at Shore  
 $S$  = Setup  
 $d$  = depth below SWL  
 $D$  = Total depth  
 $\tau_{sx}, \tau_{sy}$  = x,y components of wind stress  
 $V$  = y-component of water transport per unit width (of x)  
 $W$  = Windspeed  
 $W_x, W_y$  = x,y components of windspeed  
 $\tau_{by}$  = y-component of bottom stress  
 $f$  = Coriolis parameter  
 $\rho$  = density of water  
 $g$  = gravity  
 $t$  = time

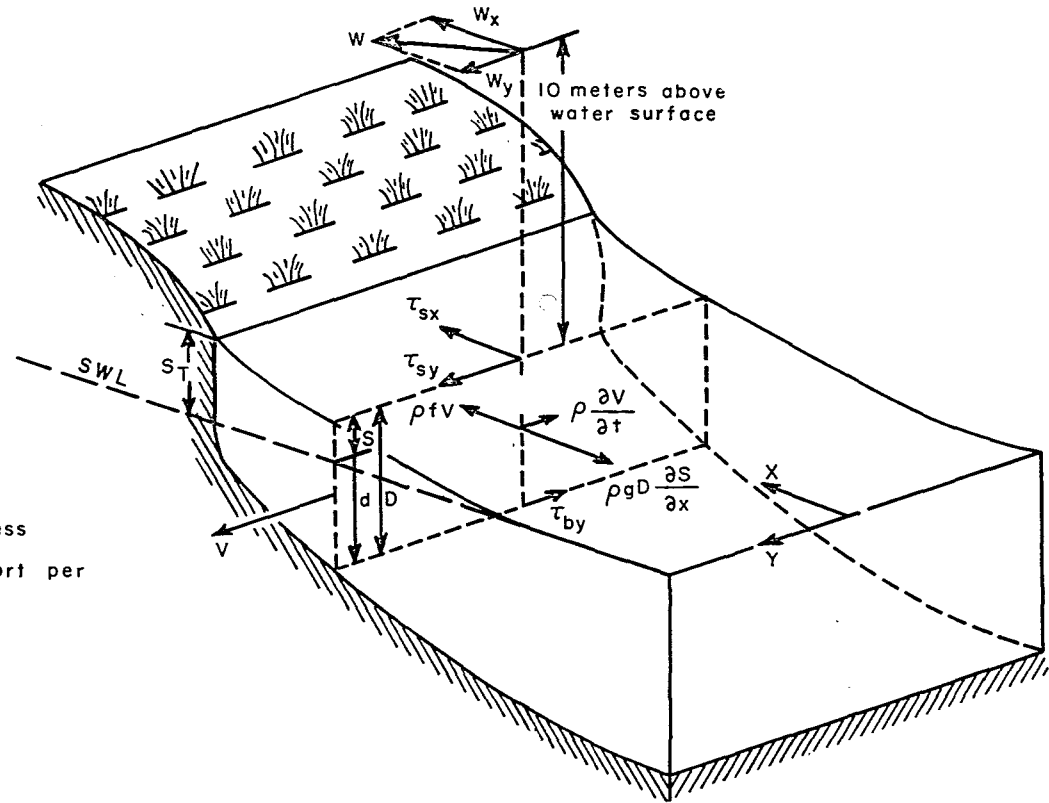


Figure 1. Schematic of forces and responses for bathystrophic approximation (U. S. Army, Corps of Engineers, Coastal Engineering Research Center, 1973).

respectively, similar formulas can be adopted for representing the wind and bottom friction stresses. Considering these flows, the mean shear stress at the boundary,  $\tau$  relates to the mean velocity gradient  $\bar{v}$  as follows:

$$\tau = \gamma\rho\bar{v}^2,$$

where  $\rho$  is the water density, and  $\gamma$  is a dimensionless resistance coefficient. To retain the proper sign consistent with the coordinate system used, the above equation can be written as follows:

$$\tau = \gamma\rho\bar{v}|\bar{v}|. \quad (8)$$

Accordingly, the shear stress at the bottom ( $\tau_{by}$ ) divided by water density, consistent with the stress term required for equation (7), is:

$$\frac{\tau_{by}}{\rho} = Kv|v|, \quad (9)$$

where the bed friction coefficient  $K$ , and the  $y$ -component of the water velocity,  $v$ , replace  $\bar{v}$  and  $\bar{v}$  respectively, in equation (8). The bed friction coefficient  $K$ , as presented here, is dimensionless in accordance to the Prandtl-von Karman Boundary-Layer Theory (Prandtl, 1935; von Karman, 1930). In the bathystrophic model used in this study (Bodine, 1971), the Prandtl-von Karman Boundary-Layer Theory was chosen because of the simplicity in computation. In transport form, equation (9) is given by Bodine as:

$$\frac{\tau_{by}}{\rho} = \frac{KV|V|}{D^2} \text{ (bottom shear stress)}. \quad (10)$$

For typical seabed conditions, the bottom friction coefficient  $K$  has been assigned values ranging from  $2 \times 10^{-3}$  and  $5 \times 10^{-3}$ .  $K$  is related to the coefficient of Chezy  $C$  and the Darcy-Weisbach friction factor  $f_f$  as follows:

$$K = \frac{g}{C^2} = \frac{f_f}{2}.$$

The wind-induced water surface stress, in accordance with equation (8), is given by:

$$\tau_s = \rho kW^2 = \rho kW|W|, \quad (11)$$

where  $W$  is the wind velocity as given at standard anemometer level (30 feet above the wave surface, based on 10-minute averages), and  $k$  is a dimensionless surface friction coefficient. The square of the wind velocity is given as an absolute term  $W|W|$  rather

than  $W^2$  to retain the proper sign consistent with the coordinate system used. According to a relationship worked out by Van Dorn (1953), the wind-stress coefficient is a function of the windspeed given by:

$$k = K_1 + K_2 \left(1 - \frac{W_c}{W}\right)^2 \text{ for } W \geq W_c, \quad (12)$$

where the constants  $K_1$  and  $K_2$  were derived empirically by Van Dorn to be  $1.1 \times 10^{-6}$  and  $2.5 \times 10^{-6}$  respectively, and  $W_c$  is a critical windspeed taken as 14 knots (about 16 miles per hour). When  $W \leq W_c$ , equation (12), reduces to:

$$k = K_1. \quad (13)$$

On the basis of equation (11), the x- and y-components of wind shear stress can be written as:

$$\frac{\tau_{sx}}{\rho} = kW^2 \cos \theta$$

(wind shear stress) (14)

$$\frac{\tau_{sy}}{\rho} = kW^2 \sin \theta,$$

where  $\theta$  is the angle between the x-axis and the local wind vector.

Equations (11) through (14) can now be introduced into the reduced equations of motion of the bathystrophic approximation; equations (6) and (7), can now be written as follows:

$$\frac{\partial S}{\partial x} = \frac{1}{gD} [fV + kW^2 \cos \theta], \quad (15)$$

$$\frac{\partial V}{\partial t} = kW^2 \sin \theta - \frac{KV^2}{D^2} \quad (16)$$

These equations are simplified forms of the hydrodynamic equations which are applied to the estimation of storm surge. The solution of these equations by numerical integration is given later in this report.

### 5. Numerical Solution of the Surge Problem.

A discussion of the numerical scheme for computing bathystrophic storm surge is given by Bodine (1971). The basic concepts are detailed here to show the geometry of the finite-difference numerical method and solution of the numerical equations of setup and flux.

Equations (15) and (16) giving setup and flux, respectively, can be solved in finite increments of time and space, assuming the functional relationships of the bathystrophic

equations as continuous over the entire computing interval. The two-dimensional finite step method of the numerical scheme is illustrated in Figures 2 and 3 for discrete increments of space,  $(\Delta x)_i$  and time  $(\Delta t)_n$  along a single Cartesian axis, the computational plane represented by the traverse. For convenience, the seabed slope AB is shown to be uniform (Fig. 3). Point C represents position of the edge of the Continental Shelf or the most seaward point along the traverse, above point A. The line CB represents the equilibrium condition of the sea surface before being affected by the approaching hurricane winds. The line CD represents the sea surface altered by the cumulative effects of the winds, astronomical tide, initial rise, pressure differences and storm waves. A detailed explanation of the variables contributing to storm surge on the shore is discussed later. The cumulative water elevation (S) associated with the hurricane is DB, while point B represents the shore intercept of the traverse.

For convenience, a boundary condition is placed on the model at point B where the shoreline is represented by a vertical wall and surge is calculated at a one-half step increment in front of this wall. The model treats the final water elevation as a static condition, and the shoreline as a vertical impermeable boundary. However, in reality, the shore is a sloping surface and dynamic processes and momentum forces associated with water transport and storm waves may result in greater surge elevation along the coast.

An initial assumption of the numerical system is that, at the beginning of the calculation, when  $t = t_o$ , the system is in an equilibrium state, with a uniform water surface, and no currents. This implies that the water flux,  $V$ , is zero at  $t = t_o$  and that  $S$  has a constant value for the system. Although in reality the system does not exist in a state of complete equilibrium, it is a reasonable assumption for the calculation. Later, this assumption is of little consequence, since the response of the system reflects only the effects of the input-forcing functions.

The discrete position  $x$  along the traverse line at any time level,  $t$ , is defined as:

$$x = x_o - \sum_{i=1}^{IM} (\Delta x)_i , \quad (17)$$

and the time level  $t$  is given by:

$$t = t_o + \sum_{n=1}^{NM} (\Delta t)_n , \quad (18)$$

where  $x_o$  as shown in Figure 3, is the distance from the shore intercept of the traverse to the most seaward point of the traverse. The summation of  $\Delta x$  is for all  $i$  intervals up to and including IM, so that at the shore  $x = x_o$ . Although  $x$ , according to the coordinate system shown here, would be negative, changing the sign of  $x$  to a positive value does not

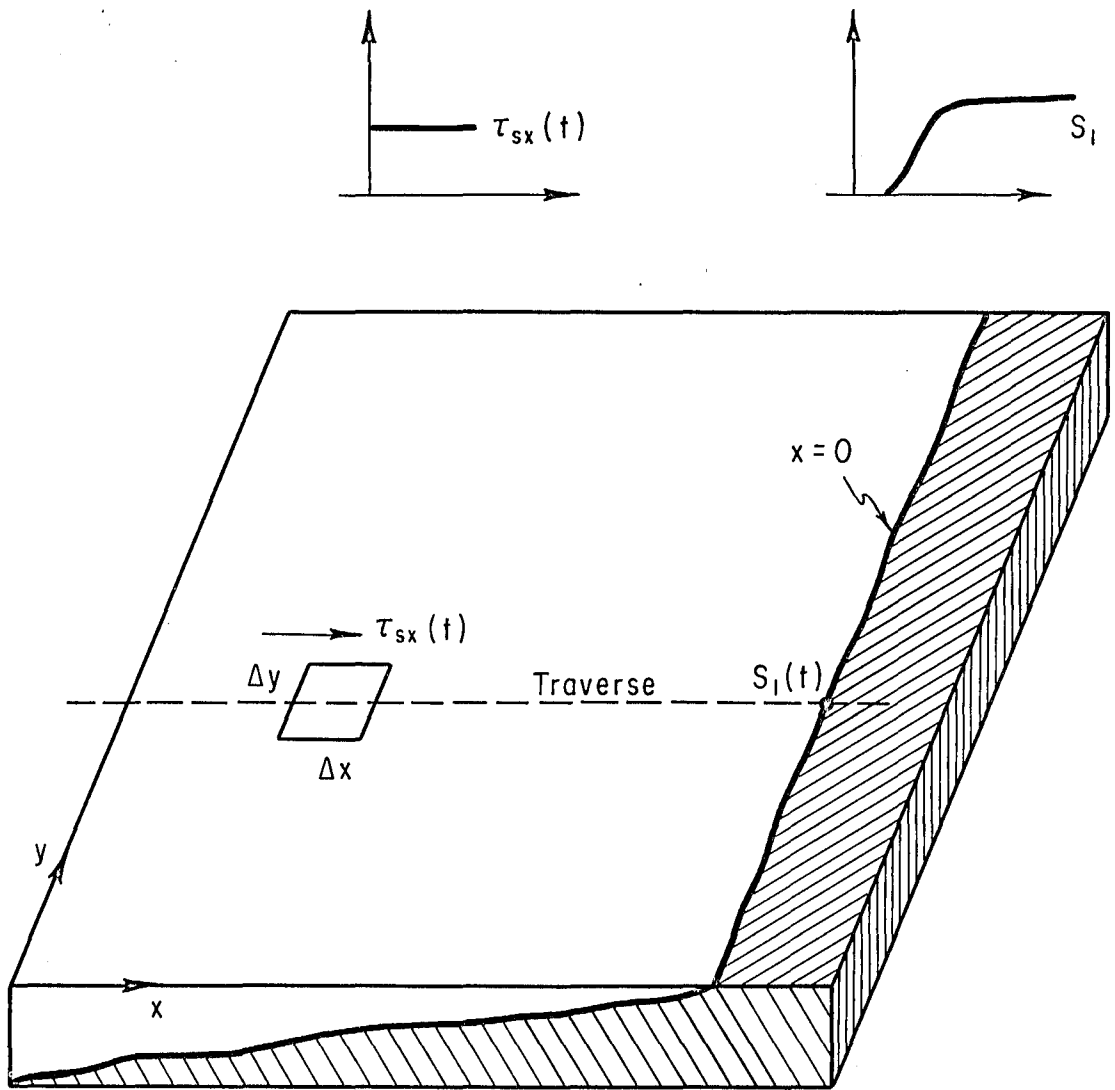


Figure 2. The response of sea level  $S_1 \Delta x \Delta y$  due to a step function in the wind stress  $\tau_{sx}$  acting over a unit area  $\Delta x \Delta y$ .



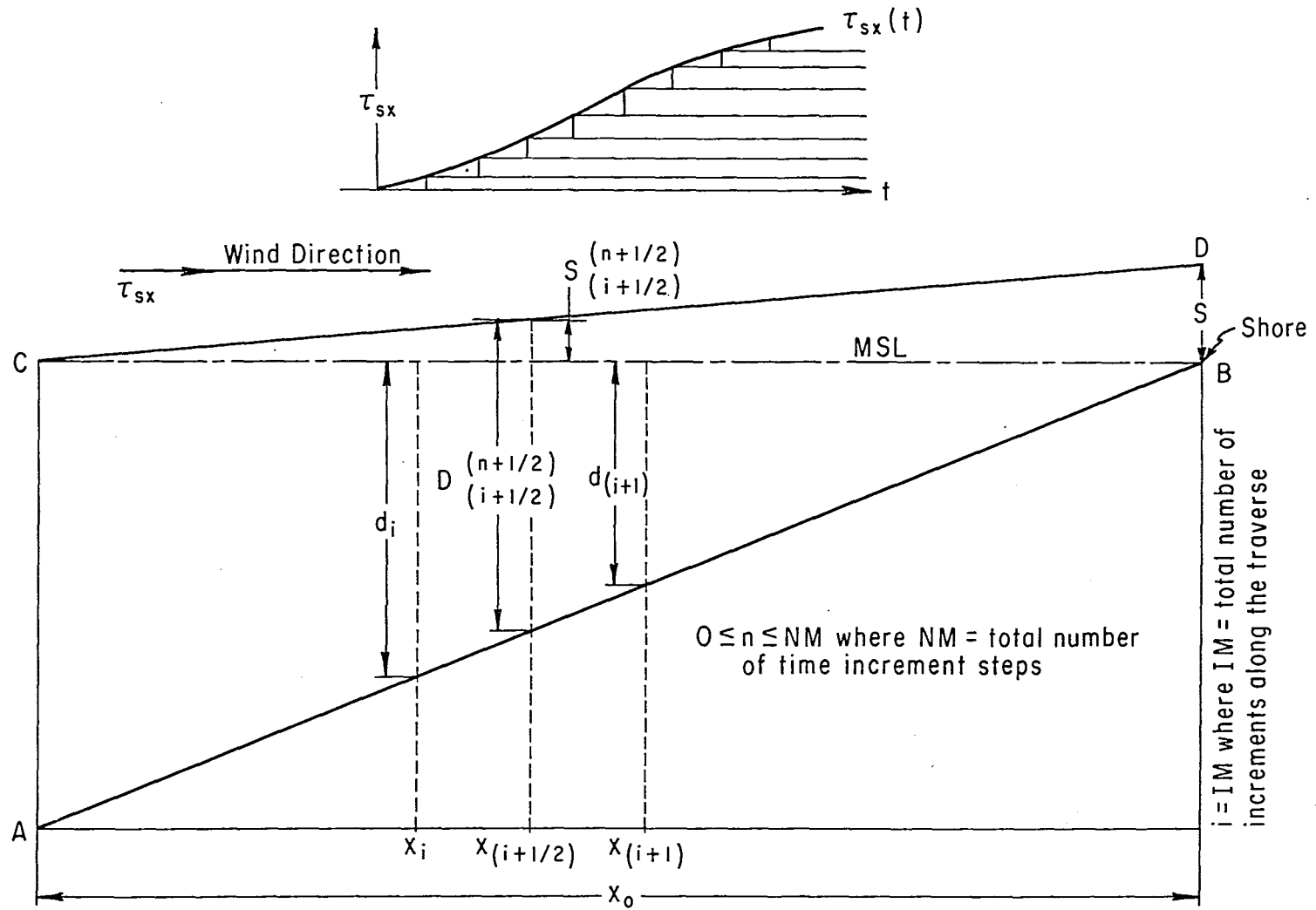


Figure 3. Numerical solution by summation of step functions.

change the computational method. The time scale at the beginning of the calculation at the seaward point of the computational line is  $t_0$  and the time level is the summation of  $\Delta t$  for all  $n$ 's up to, and including a specified value  $n = NM$ .

From the chosen geometry of the present numerical scheme, the values of wind stress  $\tau_s$ , seabed depths below the undisturbed level  $d$ , and the Coriolis effects are supplied at all discrete positions of  $i$ . The setup  $S$ , the total water depth at each increment, and the value of volume transport  $V$ , are evaluated at intervals,  $i + \frac{1}{2}$ .

The total water depth ( $D_{i+\frac{1}{2}}^{n+\frac{1}{2}}$ ) midway between two time levels  $n$  and  $n + 1$  is centered between the points  $x_i$  and  $x_{i+1}$ , and the cumulative water depth is given by:

$$D_{i+\frac{1}{2}}^{n+\frac{1}{2}} = \frac{d_i + d_{i+1}}{2} + S_e + \frac{S_A^n + S_A^{n+1}}{2} + (S_x + S_y)_{i+\frac{1}{2}}^n + \frac{1}{4} \left( [(S_{\Delta p})_i + (S_{\Delta p})_{i+1}]^n + [(S_{\Delta p})_i + (S_{\Delta p})_{i+1}]^{n+1} \right), \quad (19)$$

where

$S_e$  = initial rise in the water level at the time the storm surge computations are started,

$S_A$  = setup due to astronomical tide,

$S_{\Delta p}$  = atmospheric pressure setup in feet, given by:

$$S_{\Delta p} = 1.14 (p_n - p_o) (1 - e^{-R/r}),$$

which is an approximate relationship when pressure is expressed in inches of mercury and where  $p_n$  is the pressure at the periphery of the storm, and  $r$  is the radial distance from the storm center to the computation point on the traverse line and  $S_x$  and  $S_y$  are the components of the storm setup given by:

$$S_x = \sum_{j=1} (\Delta S_x)_j, \quad (20)$$

$$S_y = \sum_{j=1} (\Delta S_y)_j. \quad (21)$$

The physical significance of equations (20) and (21) is that total wind setup for any discrete position along the traverse is the setup in that reach superimposed cumulatively on the setups in all reaches seaward.

For a new time level,  $n + 1$ , the total water depth is given by:

$$D_{i+\frac{1}{2}}^{n+1} = \frac{d_i + d_{i+1}}{2} + S_e + S_A^{n+1} + (S_x + S_y)_{i+\frac{1}{2}}^n + \frac{1}{2} \left[ (S_{\Delta p})_i + (S_{\Delta p})_{i+1} \right]^{n+1}. \quad (22)$$

From Figure 3 and equations (19) and (22) it is apparent that a small error is introduced each time  $D$  is calculated because the term  $(S_x + S_y)$  is taken at the previous time level rather than at time  $(n + \frac{1}{2})$  for equation (19) and  $(n + 1)$  for equation (22). The reason is that the correct values are not known for these time intervals; therefore, an approximation is made. This error, however, is minimized by using small increments of time and space in the calculations.

The differential hydrodynamic equations (15) and (16) can now be solved by numerical integration. Equation (15) gives the cumulative setup resulting from onshore and alongshore effects. The onshore, wind-induced component, according to Bodine (1971) can be separated from the alongshore bathystrophic component and equation (15) can be written in its equivalent forms as follows:

$$\frac{\partial S_x}{\partial x} = \frac{kW^2 \cos \theta}{gD}, \quad (23)$$

$$\frac{\partial S_y}{\partial x} = \frac{fV}{gD}, \quad (24)$$

where the total setup along the x-axis is the sum of the two, given by

$$\frac{\partial S}{\partial x} = \frac{\partial S_x}{\partial x} + \frac{\partial S_y}{\partial x}.$$

Numerical integration of equations (23) and (24) will give the following numerical analogs (Bodine, 1971):

$$(\Delta S_x)_{i+\frac{1}{2}}^{n+1} = \frac{\Delta x}{2gD_{i+\frac{1}{2}}^{n+1}} (A_i + A_{i+1})^{n+1}, \quad (25)$$

$$(\Delta S_y)_{i+\frac{1}{2}}^{n+1} = \frac{\Delta x}{2gD_{i+\frac{1}{2}}^{n+1}} (f_i + f_{i+1}, V_{i+\frac{1}{2}}^{n+1}), \quad (26)$$

where  $A$  is a Kinematic form of wind stress given by:

$$A = kW^2 \cos \theta.$$

Similarly, the alongshore differential equation (16) of water flux can be resolved by numerical integration as:

$$\frac{\partial V}{\partial t} = kW^2 \sin \theta - \frac{KV^2}{D^2},$$

or

$$\frac{\partial V}{\partial t} = kW^2 \sin \theta - KV|V|D^{-2}.$$

However,

$$\frac{\partial V}{\partial t} \simeq \frac{V_{i+\frac{1}{2}}^{n+1} - V_{i+\frac{1}{2}}^n}{\Delta t}. \quad (27)$$

The first term of equation (16),  $kW^2 \sin \theta$  can be made equal to  $B$ , which is also a Kinematic form of the wind stress, so that,

$$B = kW^2 \sin \theta \simeq \frac{1}{4} \left[ (B_i + B_{i+1})^n + (B_i + B_{i+1})^{n+1} \right]. \quad (28)$$

The second term of equation (16) can be approximated as follows:

$$KV|V|D^{-2} \simeq KV_{i+\frac{1}{2}}^{n+1} \left| V_{i+\frac{1}{2}}^n \right| (D^{-2})_{i+\frac{1}{2}}^{n+\frac{1}{2}}. \quad (29)$$

Substituting equations (27), (28), and (29) into equation (16), yields:

$$\begin{aligned} \frac{V_{i+\frac{1}{2}}^{n+1} - V_{i+\frac{1}{2}}^n}{\Delta t} &= \frac{1}{4} \left[ (B_i + B_{i+1})^n + (B_i + B_{i+1})^{n+1} \right] \\ &\quad - KV_{i+\frac{1}{2}}^{n+1} \left| V_{i+\frac{1}{2}}^n \right| (D^{-2})_{i+\frac{1}{2}}^{n+\frac{1}{2}}. \end{aligned} \quad (30)$$

Multiplying equation (30) by  $\Delta t$  and transposing the term  $V_{i+\frac{1}{2}}^n$  yields:

$$\begin{aligned} V_{i+\frac{1}{2}}^{n+1} &= \frac{1}{4} \left[ (B_i + B_{i+1})^n + (B_i + B_{i+1})^{n+1} \right] \Delta t \\ &\quad - KV_{i+\frac{1}{2}}^{n+1} \left| V_{i+\frac{1}{2}}^n \right| (D^{-2})_{i+\frac{1}{2}}^{n+\frac{1}{2}} \Delta t + V_{i+\frac{1}{2}}^n, \end{aligned} \quad (31)$$

transposing the term

$$KV_{i+\frac{1}{2}}^{n+1} \left| V_{i+\frac{1}{2}}^n \right| (D^{-2})_{i+\frac{1}{2}}^{n+\frac{1}{2}} \Delta t,$$

yields:

$$\begin{aligned} V_{i+\frac{1}{2}}^{n+1} + KV_{i+\frac{1}{2}}^{n+1} \left| V_{i+\frac{1}{2}}^n \right| (D^{-2})_{i+\frac{1}{2}}^{n+\frac{1}{2}} \Delta t \\ = V_{i+\frac{1}{2}}^n + \frac{1}{4} \left[ (B_i + B_{i+1})^n + (B_i + B_{i+1})^{n+1} \right] \Delta t. \end{aligned} \quad (32)$$

Factoring out the term  $V_{i+\frac{1}{2}}^{n+1}$  and dividing equation (32) by the term

$$1 + K \left| V_{i+\frac{1}{2}}^n \right| (D^{-2})_{i+\frac{1}{2}}^{n+\frac{1}{2}} \Delta t,$$

becomes:

$$V_{i+\frac{1}{2}}^{n+1} = \frac{\left(\frac{1}{4}\right) \left[ (B_i + B_{i+1})^n + (B_i + B_{i+1})^{n+1} \right] \Delta t + V_{i+\frac{1}{2}}^n}{1 + K \left| V_{i+\frac{1}{2}}^n \right| \Delta t (D^{-2})_{i+\frac{1}{2}}^{n+\frac{1}{2}}}, \quad (33)$$

which is the numerical analog of equation (16).

In the numerical analogs of the bathystrophic equations, nonuniform spacing  $\Delta x$  and time,  $\Delta t$ , steps can be taken. This permits coarse spacing,  $\Delta x$ , where the seabed is relatively flat, and fine spacing near the shore where the bed slope changes rapidly. Similarly, nonuniform time steps,  $\Delta t$ , permit more frequent storm-surge computations during the period when rapid water level changes are anticipated.

Based on this numerical scheme and logic, calculations of surge are started at the seaward boundary to the shore-intercept through all prescribed spatial positions on the traverse at the initial time level. The process is repeated for each prescribed time level, and continued for the entire temporal range. For each discrete position along the traverse line, the flux  $V$ , at each new time level, is evaluated based on the flux  $V$  at the previous time level. Similarly the stress term  $B$ , and depth  $D$ , are evaluated as the average values in the incremental domain of  $(x + \Delta x)$  and  $(t + \Delta t)$ . Determination of  $V$  at each new time level can be made with equation (33), then using this value, determination of the  $x$ - and  $y$ -components of setup,  $\Delta S_x$  and  $\Delta S_y$ , can be made with equation (25) and (26), to obtain the total setups  $S_x$  and  $S_y$ .

As mentioned earlier, the total water level rise on the shore will be the summation of a number of components from the meteorological storm plus those unrelated to the storm. The total setup is given by:

$$S_T = S_x + S_y + S_{\Delta p} + S_e + S_A + S_W + S_L. \quad (34)$$

Definitions of most components have been given. Some other components are related to the storm but some unrelated terms are provided as input to the surge computation. In equation (34),  $S_w$  is the wave setup at the shore due to breaking waves given (U.S. Army, Corps of Engineers, Coastal Engineering Research Center, 1973):

$$S_w = 0.19 \left[ 1 - 2.82 \left( \frac{H_b}{gT^2} \right)^{1/2} \right] H_b, \quad (35)$$

where  $H_b$  is the height of the breaking wave,  $g$  is the gravitational acceleration, and  $T$  is the wave period. The local setup or setdown  $S_L$ , is the deviation of the water surface from the computed water level due to such local effects as inland runoff inside the coastal barrier or the coastal hydrography. This component can only be estimated from full consideration of the influences of topography and hydrography not considered in the numerical computations. A schematic representation of the different setup components contributing to storm surge on the shore is shown in Figure 4.

Calculation of volume transport  $V$ , is based on repeated computations using the same formula, and can result in round-off errors, as each computed value will influence the values which remain to be determined. To ensure that the value of  $V$  does not exceed the maximum possible value, Bodine (1971) derived the following relationship. In an incremental form, equation (16) can be written as:

$$\Delta V = kW^2 \sin \theta \Delta t - KV^2 D^{-2} \Delta t,$$

or

$$KV^2 D^{-2} \Delta t = kW^2 \sin \theta \Delta t - \Delta V.$$

For small

$$\Delta V, KV^2 D^{-2} \leq kW^2 \sin \theta.$$

Thus, the y-component of volume transport becomes

$$V \leq \sqrt{\frac{D^2 kW^2 \sin \theta}{K}}.$$

At the new time level, the above equation can be written in its numerical analog form, as:

$$V_{i+1/2}^{n+1} \leq \sqrt{\frac{|(B_i + B_{i+1})^{n+1}| (D^2)_{i+1/2}^{n+1}}{2K}}. \quad (36)$$

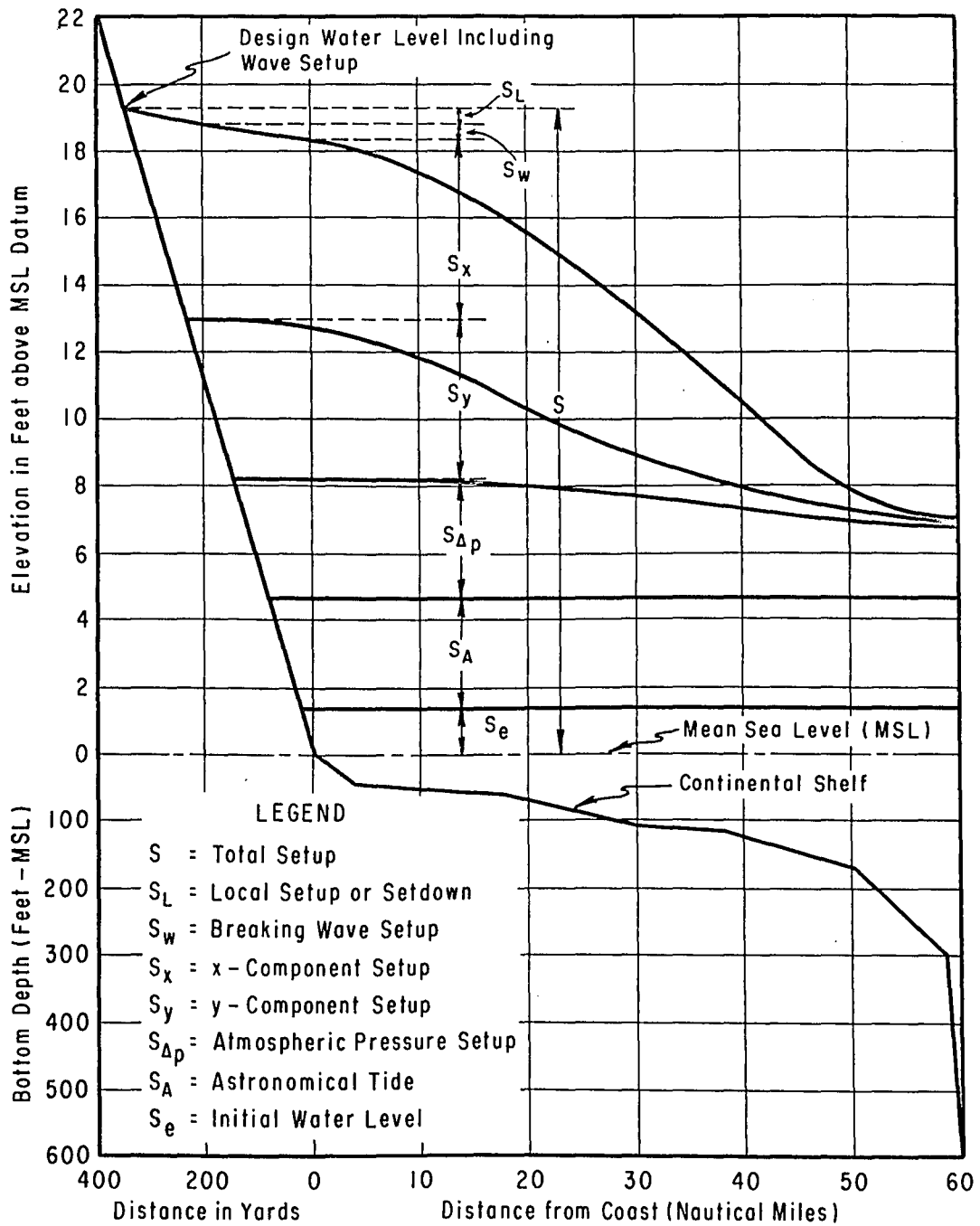


Figure 4. Various setup components over the Continental Shelf contributing to storm surge on the shore (Bodine, 1971).

According to this equation the absolute value of the flux must never exceed the term on the right-hand side. This relation is used as a check, and if this value is exceeded, the flux at the new time level, as an estimate, can be set equal to the value given by the right-hand side of equation (36). The integrated equations (25), (26), (33) and (36) can be used for the numerical solution of the surge problem. However, because of the inconsistency of units in the different terms used in these equations, it is desirable to absorb the invariant coefficients of these terms and substitute with constants. Such substitution reduces the possibility of errors and the equations are easier to use in the program. For example, weather charts usually present the wind data in knots. Distances taken from hydrographic maps are given in nautical miles, and depths in fathoms or feet. To eliminate conversion of units, Table 1 gives the dimensions of the variables used in the numerical scheme in four systems of units and the corresponding value of the constants for each system. The first column of units is given in the metric system while the other three are given in mixed units of the English system.

Table 1. Systems of Units for Storm Surge Computations

Parameters	Units and Constant Values			
	Metric	Mixed English		
$\Delta x$	km	nm	nm	mi
$\Delta S_x, \Delta S_y$	m	ft	ft	ft
g	m/sec <sup>2</sup>	ft/sec <sup>2</sup>	ft/sec <sup>2</sup>	ft/sec <sup>2</sup>
D	m	ft	ft	ft
A, B	(km/hr) <sup>2</sup>	(nm/hr) <sup>2</sup>	(mi/hr) <sup>2</sup>	(mi/hr) <sup>2</sup>
V	km <sup>2</sup> /hr	nm <sup>2</sup> /hr	mi <sup>2</sup> /hr	mi <sup>2</sup> /hr
f	hr <sup>-1</sup>	hr <sup>-1</sup>	hr <sup>-1</sup>	hr <sup>-1</sup>
$\Delta t$	hr	hr	hr	hr
C <sub>1</sub>	3.94	269	203	176
C <sub>2</sub>	2.06	141	106	92
C <sub>3</sub>	(1,000) <sup>2</sup>	(6,080) <sup>2</sup>	(5,280) <sup>2</sup>	(5,280) <sup>2</sup>

(U.S. Army, Corps of Engineers, Coastal Engineering Research Center, 1973).



Equations (25), (26), (33), and (36) can now be given in a more simplified computational form, as follows:

$$(\Delta S_x)_{i+\frac{1}{2}}^{n+1} = \frac{C_1 \Delta x}{D_{i+\frac{1}{2}}^{n+1}} (A_i + A_{i+1})^{n+1}, \quad (37)$$

$$(\Delta S_y)_{i+\frac{1}{2}}^{n+1} = \frac{C_2 \Delta x}{D_{i+\frac{1}{2}}^{n+1}} [(\sin \phi)_i + (\sin \phi)_{i+1}] V_{i+\frac{1}{2}}^{n+1}, \quad (38)$$

$$V_{i+\frac{1}{2}}^{n+1} = \frac{\left(\frac{1}{4}\right) [(B_i + B_{i+1})^n + (B_i + B_{i+1})^{n+1}] (\Delta t) + V_{i+\frac{1}{2}}^n}{1 + C_3 |V_{i+\frac{1}{2}}^n| \Delta t K (D^{-2})_{i+\frac{1}{2}}^{n+1}}, \quad (39)$$

$$V_{i+\frac{1}{2}}^{n+1} \leq \sqrt{\frac{|(B_i + B_{i+1})^{n+1}| (D^2)_{i+\frac{1}{2}}^{n+1}}{2C_3 K}} \quad (40)$$

The values of the dimensional constants  $C_1$ ,  $C_2$ , and  $C_3$  in equations (37), (38), (39), and (40) will depend on the system of units used in performing the computations. In the present model, the English system of units is used, so  $C_1 = 203$ ,  $C_2 = 106$ , and  $C_3 = (5280)^2$ .

### III. HURRICANE AND HYDROGRAPHIC DATA

Verification of the bathystrophic model consisted of modeling surge response to the Continental Shelf at six locations for five historical hurricanes. Historical hurricanes and traverses used were: (a) Hurricane of 1949 at Galveston and Freeport, Texas; (b) Hurricane Carla at Galveston and Freeport, Texas; (c) Hurricane Audrey at Eugene Island, Louisiana; (d) Hurricane Camille at Biloxi, Mississippi; and (e) Hurricane Carol at Narragansett Pier, Rhode Island. The data describing these hurricanes, the resulting surges, and the traverses used in the verification, are presented in this section.

#### 1. Hurricane of 3 October 1949.

*a. Hurricane Data.* This hurricane formed in the Bay of Campeche, Mexico, from a tropical depression which was observed 2 or 3 days before 1 October, over Yucatan, Honduras, and Guatemala. During the night of 30 September and 1 October, the center of the depression moved into the Gulf of Mexico near Carmen, Mexico and increased to hurricane intensity by 1045 hours Central Standard Time (CST), 2 October. The hurricane moved from Yucatan almost directly northward across the gulf and its center crossed the Texas coast 22 miles southwest of Freeport during the nights of 3 and 4 October. The storm passed Houston during the early morning of 4 October. Winds were estimated at 135 miles per hour, 5 miles west of Freeport (Zoch, 1949). The storm's center moved across

southeastern Texas, northwestern Louisiana, eastern Arkansas, southeastern Missouri, western Illinois, southeastern Wisconsin, and northern lower Michigan. The intense winds and rainfall associated with this storm caused heavy damage to crops and property in Texas, Louisiana and Arkansas (Seamon, 1949).

Surface wind fields and pressure fields for this hurricane were obtained from the U.S. Weather Bureau (HUR 7-37, 1957). The report provides hourly wind fields from 1800 hours CST, 3 October to 0500 hours CST, 4 October 1949 (Fig. 5). Physical parameters generally describing the hurricanes are given in Table 2.

Table 2. Hurricane Parameters.

Parameter	Hurricane of 1949	Carla	Camille	Audrey	Carol
Central pressure (inches of mercury)	28.45	27.64	26.73	27.95	28.69
Peripheral pressure (inches of mercury)	29.95	29.92	29.92	29.70	29.92
Radius to maximum wind (nautical miles)	15.0	46.0	14.0	19.0	25.0
Translation speed <sup>1</sup> (knots)	11.0	3.0	13.0	13.0	33.3
Maximum gradient wind (miles per hour)	88.0	100.0	125.0	95.0	95.0

1. Average value

*b. Hydrographic Data.* The Hurricane of 3 October 1949 generated the following surges along the Texas coast: 11 feet at Velasco, 8 feet at Matagorda, 9 feet at Anahuac, and 11.4 feet at Harrisburg in the Houston Ship Channel (Zoch, 1949) (Figs. 6 and 7).

Hydrographs of surges associated with this hurricane, and used in this study, were recorded by the Brazos River and Galveston tide gages. The Brazos River station is located 1.2 nautical miles inland from the Gulf of Mexico, near Freeport where the Brazos River and the Intracoastal Waterway intersect. The Galveston gage used is located at Pier 21, Galveston Channel.

The method of hydrographic data reduction used for this study is given in Section IV.

## 2. Hurricane Carla.

*a. Hurricane Data.* Hurricane Carla has been described as one of the largest, most intense, and destructive hurricanes ever to strike the gulf coast. This hurricane evolved from a weak perturbation in the Intertropical Convergence Zone in the eastern Caribbean and was

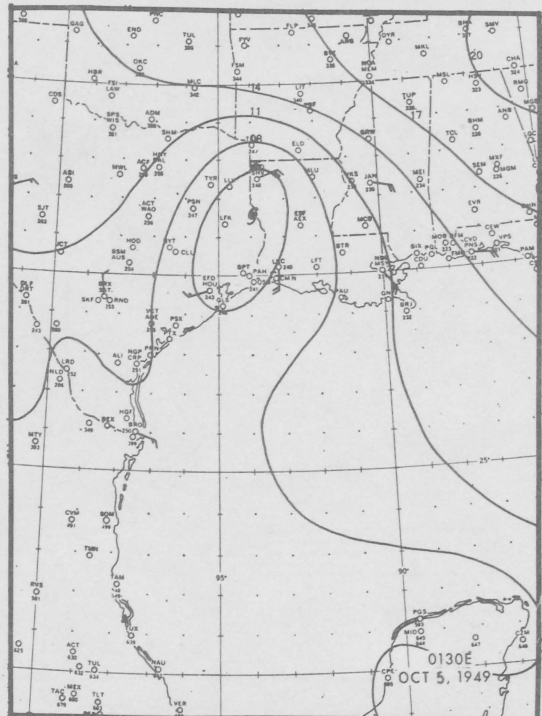
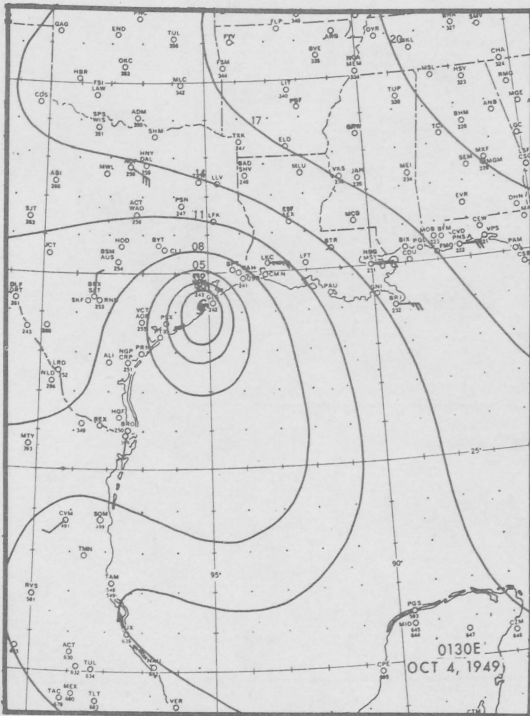
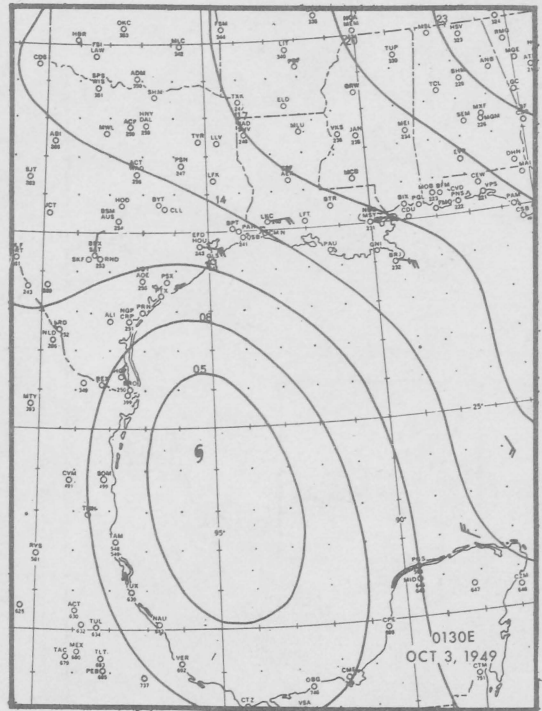
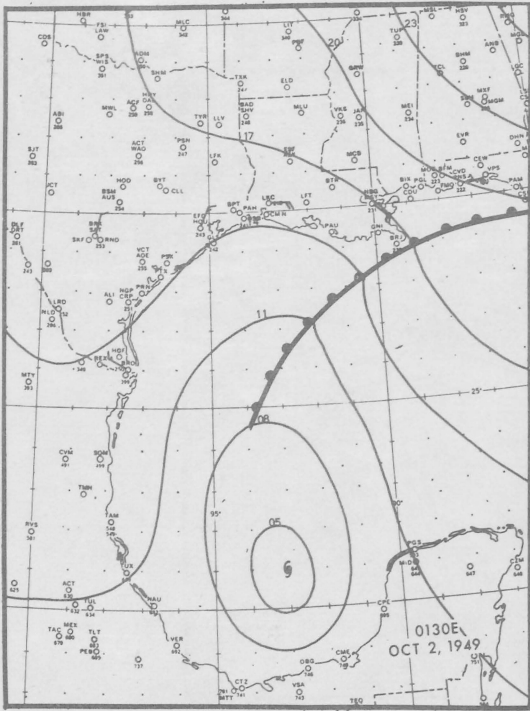


Figure 5. Hurricane of 3-4 October 1949. Synoptic charts (Harris, 1963).

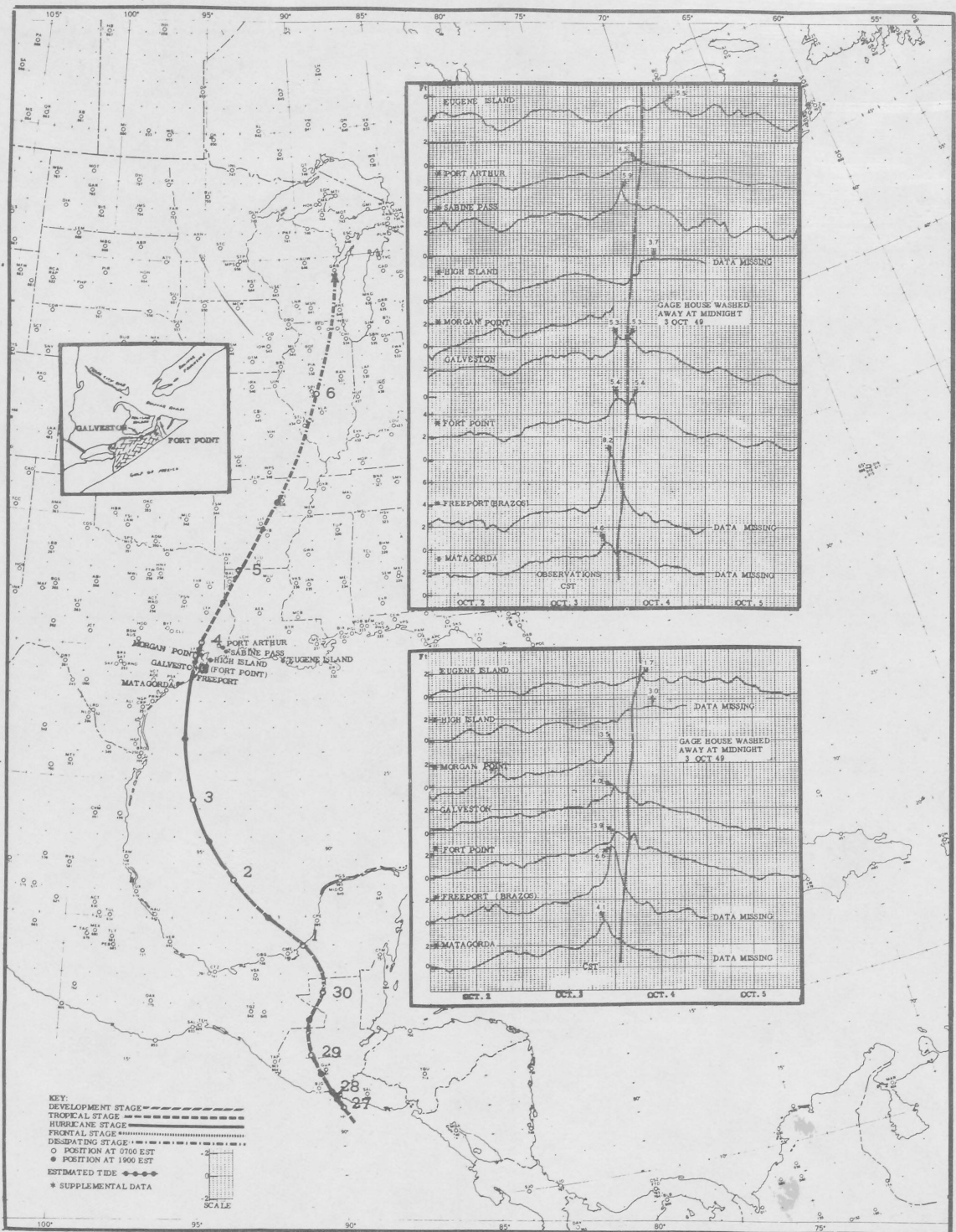


Figure 6. Hurricane of 3-4 October 1949. Storm surge (lower panel) and tide observations chart (hourly values only). Insert map for Galveston, Texas (Harris, 1963).

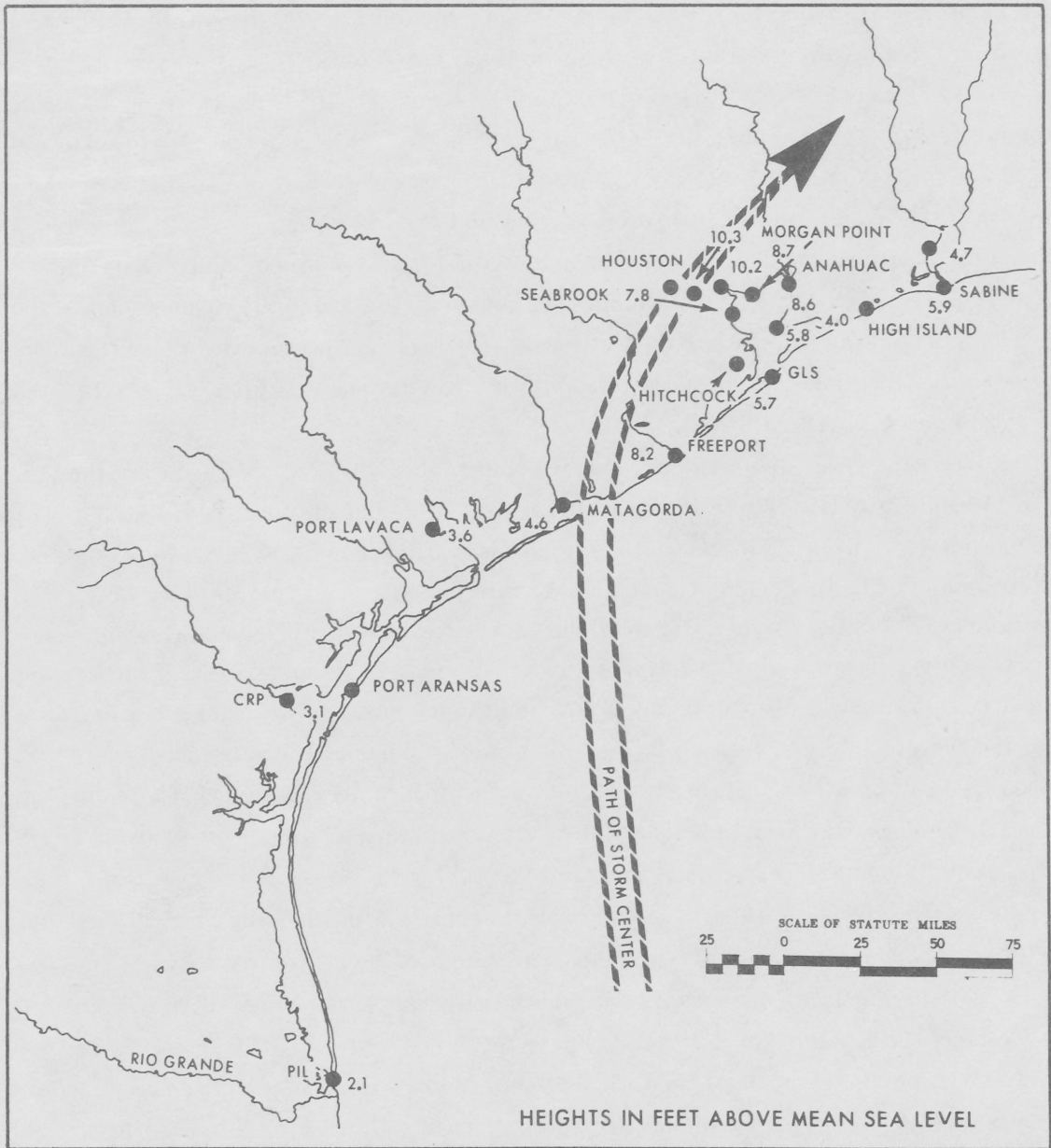


Figure 7. Hurricane of 3-4 October 1949. High water marks in Texas (Harris, 1963).

observed as early as 1 September 1961. By 0600 hours Central Standard Time (CST), 3 September, this disturbance intensified and developed a closed circulation. By 0600 hours CST, 4 September, the circulation increased to depression intensity with winds ranging from 32 to 38 miles per hour. By 1900 hours CST, of the same day, Carla was identified as a storm and an advisory was issued by the Miami Hurricane Center.

During the next several days, Carla continued to intensify gradually reaching hurricane force on the morning of the 6th. By the afternoon of the 11th, Carla's center moved inland over the Port O'Connor--Port Lavaca area on the central Texas coast. Hurricane force winds were reported from Corpus Christi to Galveston and hurricane gusts were felt along almost the entire length of the Texas coast. Maximum gusts of 175 miles per hour were estimated at Port Lavaca, Texas. A gust of 153 miles per hour was observed on the anemometer of the Bauer Dredging Company before the instrument failed. The lowest reported pressure at Port Lavaca was 27.61 inches of mercury (935 millibars) and it remained at that value from 1545 to 1735 hours CST (Dunn, 1962).

Surface wind fields and pressure fields for Hurricane Carla were obtained from the U.S. Weather Bureau (HUR 7-76A, 1964). In the report, wind and pressure fields were given at 6-hour intervals from 1200 hours Greenwich Mean Time (GMT), 9 September, through 1200 hours GMT, 10 September, and 3-hour intervals thereafter to 1500 hours GMT, 12 September. Synoptic weather charts of this storm are shown in Figures 8 and 9. These charts contain lines of equal windspeed (isotachs) and lines of equal angle of incurvature (isoangles). The angle of incurvature at any point is the angle between the wind direction and the tangent to a circle drawn through that point and concentric with the storm center.

*b. Hydrographic Data.* Storm surge associated with Hurricane Carla, began affecting the upper Texas coast on 8 September, until the center of the storm moved slowly inland 3 days later. Along the entire Texas coast, the storm surge began to develop when the winds were parallel to the coast at a distance of 100 miles, reaching a peak when the winds at the coast were from the north and actually had a slight offshore component (Harris, 1963).

Because of the storm's slow forward movement, water levels remained within about 1 foot of their peak values for nearly 24 hours. Surge levels were over 12 feet above mean sea level (MSL) on the open coast, and 20 feet above MSL within bays and estuaries. Storm surge and high water marks on the Texas coast during Hurricane Carla have been well documented (Cooperman and Sumner, 1961; Cry, 1961; Dunn, 1962). Reported surges were 16.6 feet above MSL at Port Lavaca, 14.5 feet above MSL at Port O'Connor, 15.2 feet above MSL at Matagorda, and 14.8 feet above MSL on the upper Houston Ship Channel. A high water line varying from 15.7 to 22 feet above MSL, established from debris near the head of Lavaca Bay, probably included the undetermined effects of wave setup and runup (Dunn, 1962).



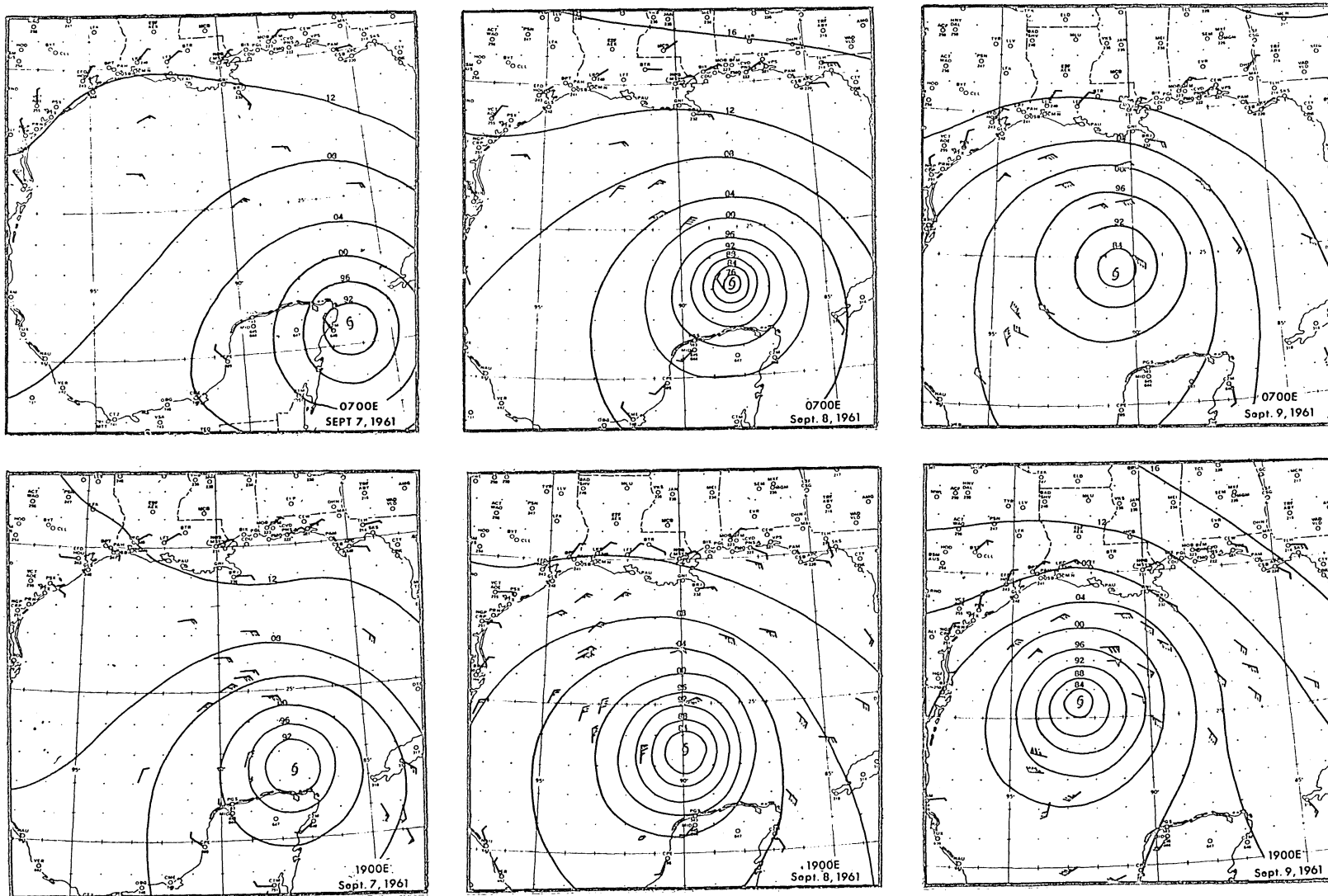


Figure 8. Hurricane Carla, 7-12 September 1961. Synoptic charts, 7-9 September (Harris, 1963).

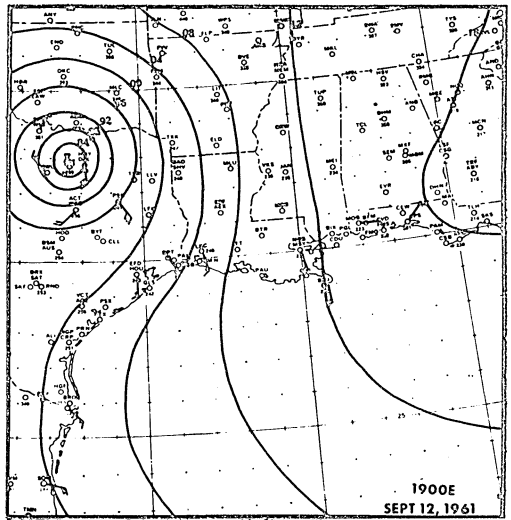
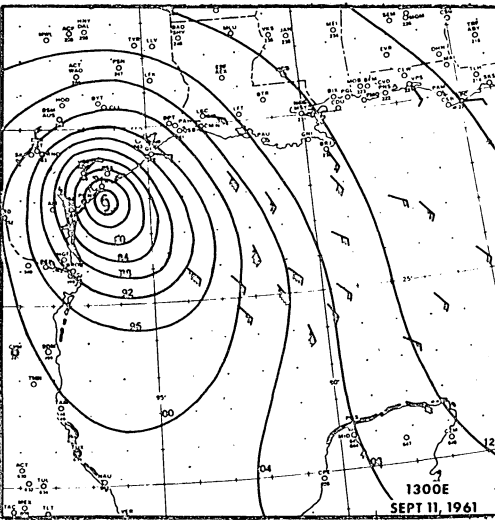
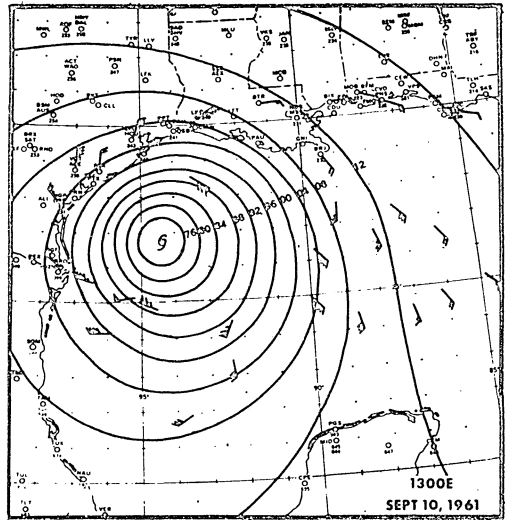
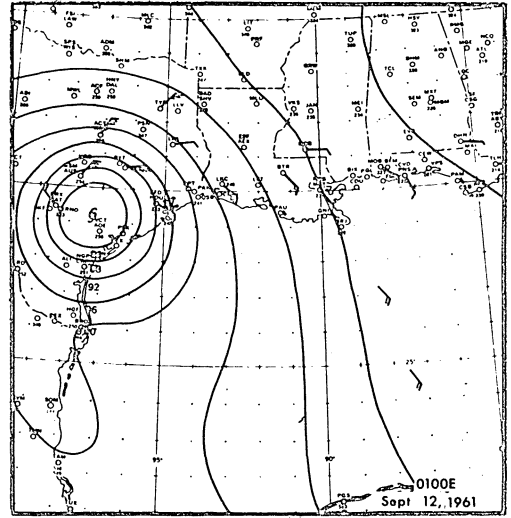
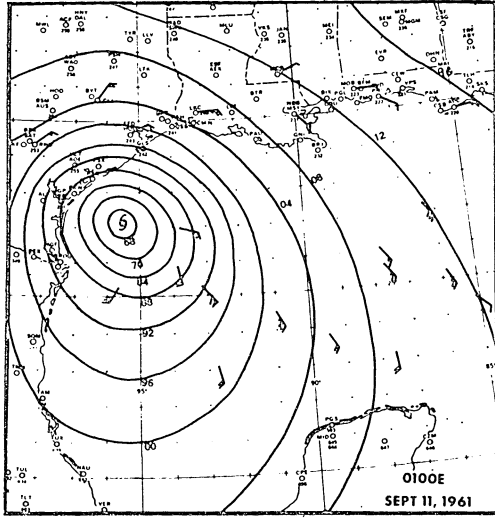
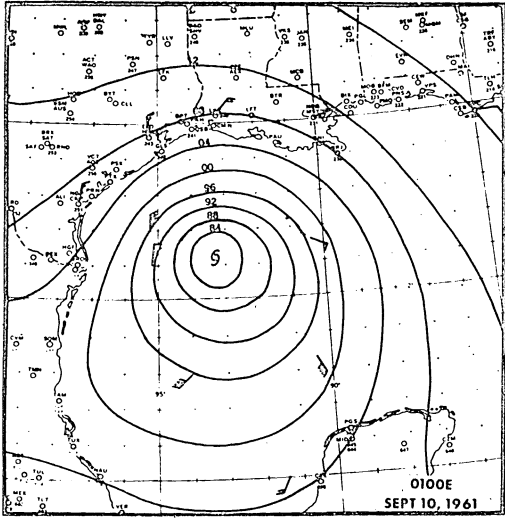


Figure 9. Hurricane Carla, 7-12 September 1961. Synoptic charts, 10-12 September (Harris, 1963).



Surge hydrograph data was taken from the U.S. Army Engineer District, Galveston (1962). Observed and recorded storm surges along the Texas coast and the predicted astronomical tide at Galveston are shown in Figures 10 and 11. Peak surge values along the Texas and Louisiana coasts are given in Figures 12 and 13.

### 3. Hurricane Audrey.

*a. Hurricane Data.* Hurricane Audrey developed from a tropical depression over the Bay of Campeche in southwestern Gulf of Mexico on 25 June 1954. The position of the tropical depression at that time was 22.5°N., 93°W., or about 380 miles southeast of Brownsville, Texas. Highest winds were estimated at 35 to 40 miles per hour.

By 1200 hours CST, 25 June, the tropical depression had reached hurricane strength with winds near the center increasing in speed from 75 to over 100 miles per hour. By 0600 hours CST on the 26th, Audrey was moving northward at about 10 miles per hour, and had reached a position 250 miles southeast of Brownsville. The hurricane's translational speed gradually increased until it reached 15 miles per hour early on the 27th. By 0700 hours CST, 27 June, the storm was centered near the Texas-Louisiana coast south of Port Arthur, Texas. Highest reported winds on the coast at that time were 75 miles per hour and surge elevation had reached 7 feet above MSL (Ross and Blum, 1957).

At 0930 hours CST on the 27th, a report from Orange, Texas stated that after experiencing winds of over 100 miles per hour the town was now experiencing the dead calm associated with the eye of the storm and was expecting the return of strong winds. Lake Charles, Louisiana, as the storm passed just to the west, reported maximum winds of 105 miles per hour. On land the storm rapidly increased forward momentum and continued in a northeast direction. On the morning of 28 June the storm was centered over west-central Tennessee and had lost its hurricane-strength winds. The storm, however, developed several tornadoes in Mississippi, Louisiana and Alabama. The following day when Audrey was expected to assume extratropical characteristics, it merged with a polar front which had formed in the vicinity of Chicago, Illinois on 28 June. The merge of these storms was complete and the new center was about 140 miles north of Buffalo, New York, in southwestern Quebec, Canada. At this time the storm had reached maximum intensity as an extratropical storm with a central pressure of 28.76 inches of mercury (974 millibars). Winds of 95 to 100 miles per hour were reported at Jamestown, New York (U.S. Weather Bureau, 1957). It then became difficult to differentiate between the contribution made by the remnants of Audrey and that of the polar trough. Meteorological discussions of the storm are given by Ross and Blum (1957), and by Moore (1957). A detailed analysis of the wind field as this storm crossed the coastline was made by Graham and Hudson (1960); additional historical data can be obtained from Sumner (1957a, 1957b). Other accounts of the storm have been presented in Klein (1957) and Morgan, Nichols, and Wright (1958).

Surface wind field data and central pressures describing Hurricane Audrey were obtained from Hydrometeorological Reports, HUR 7-51A, 7-57, and 7-57A (U.S. Weather Bureau,

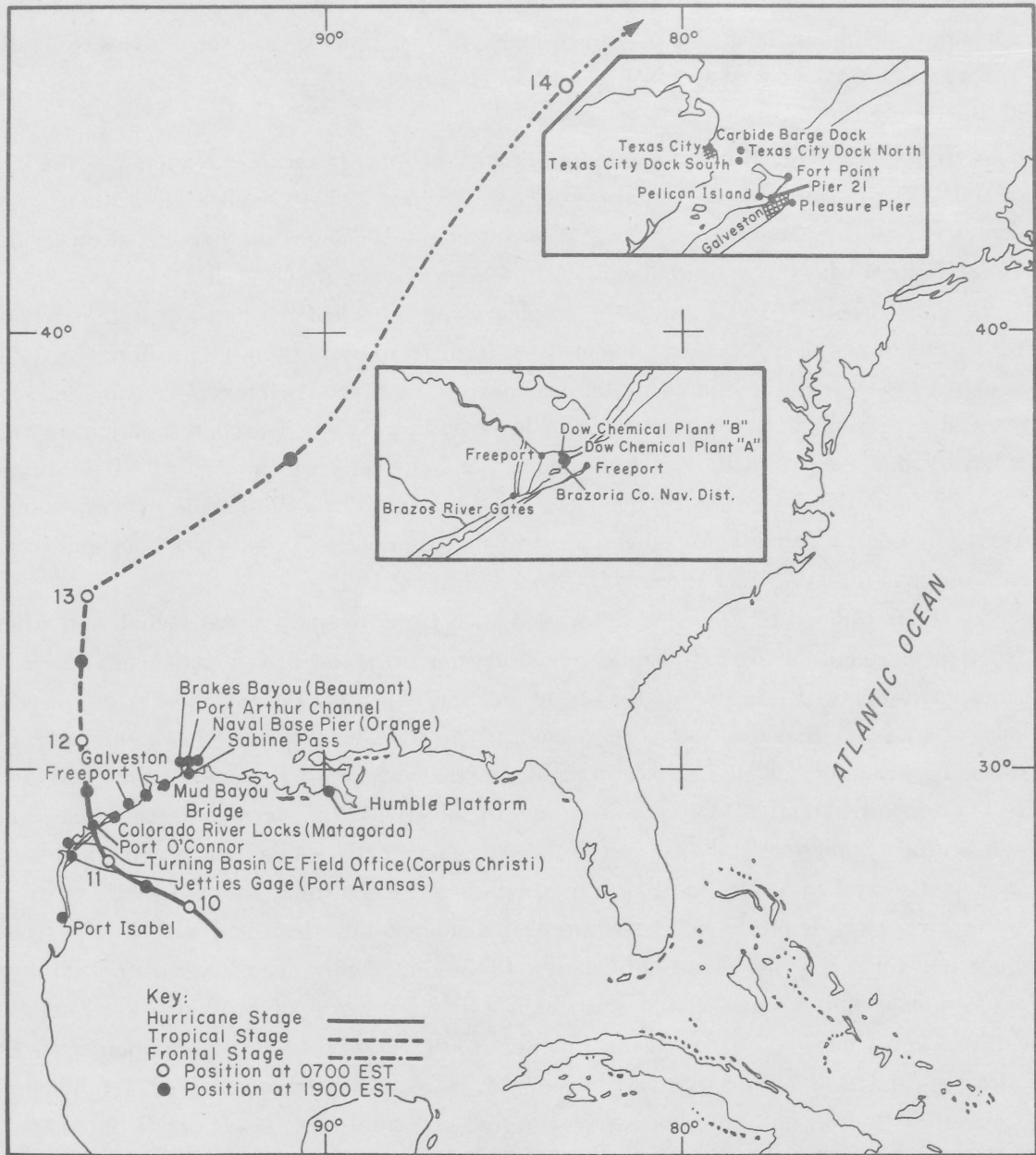
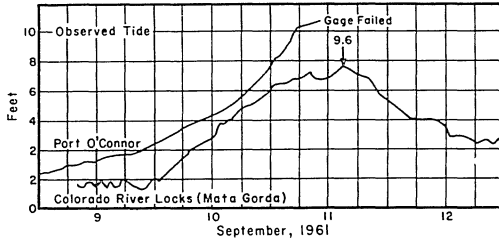
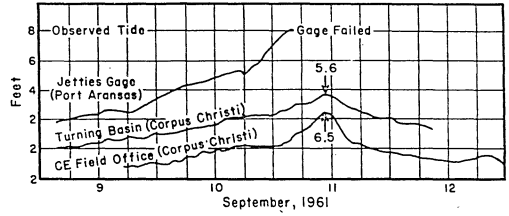
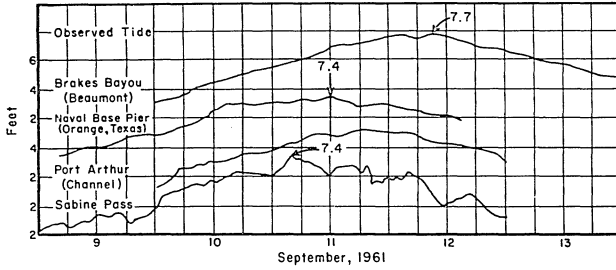
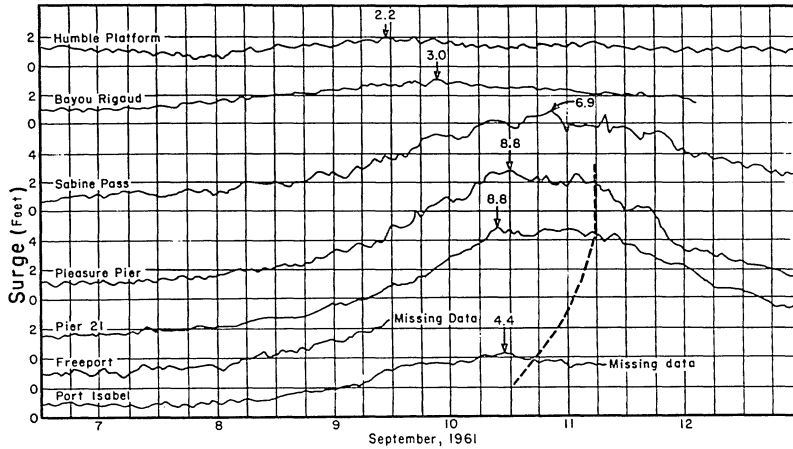
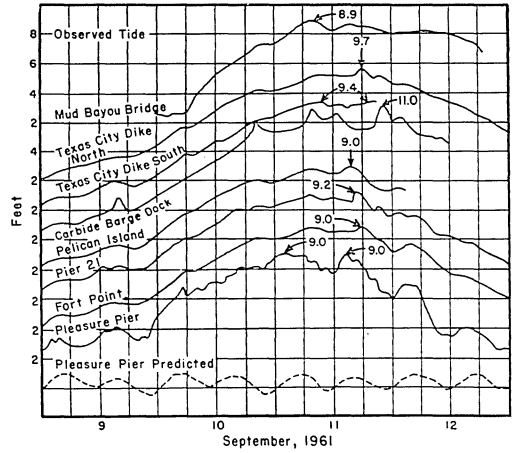
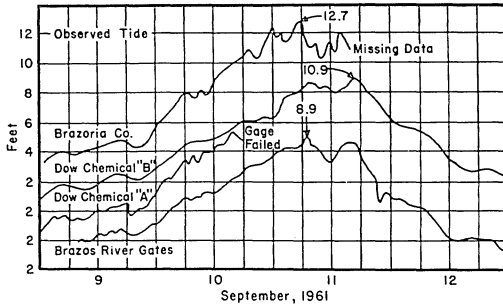


Figure 10. Hurricane Carla, 7-12 September 1961. Insert maps for Freeport and Galveston, Texas (Harris, 1963).



Feet  
+2  
0  
-2  
Scale



LEGEND  
Time Eye of Hurricane  
Nearest to Station -----

Figure 11. Hurricane Carla, 7-12 September 1961. Storm surge and observed tide chart (Harris, 1963).

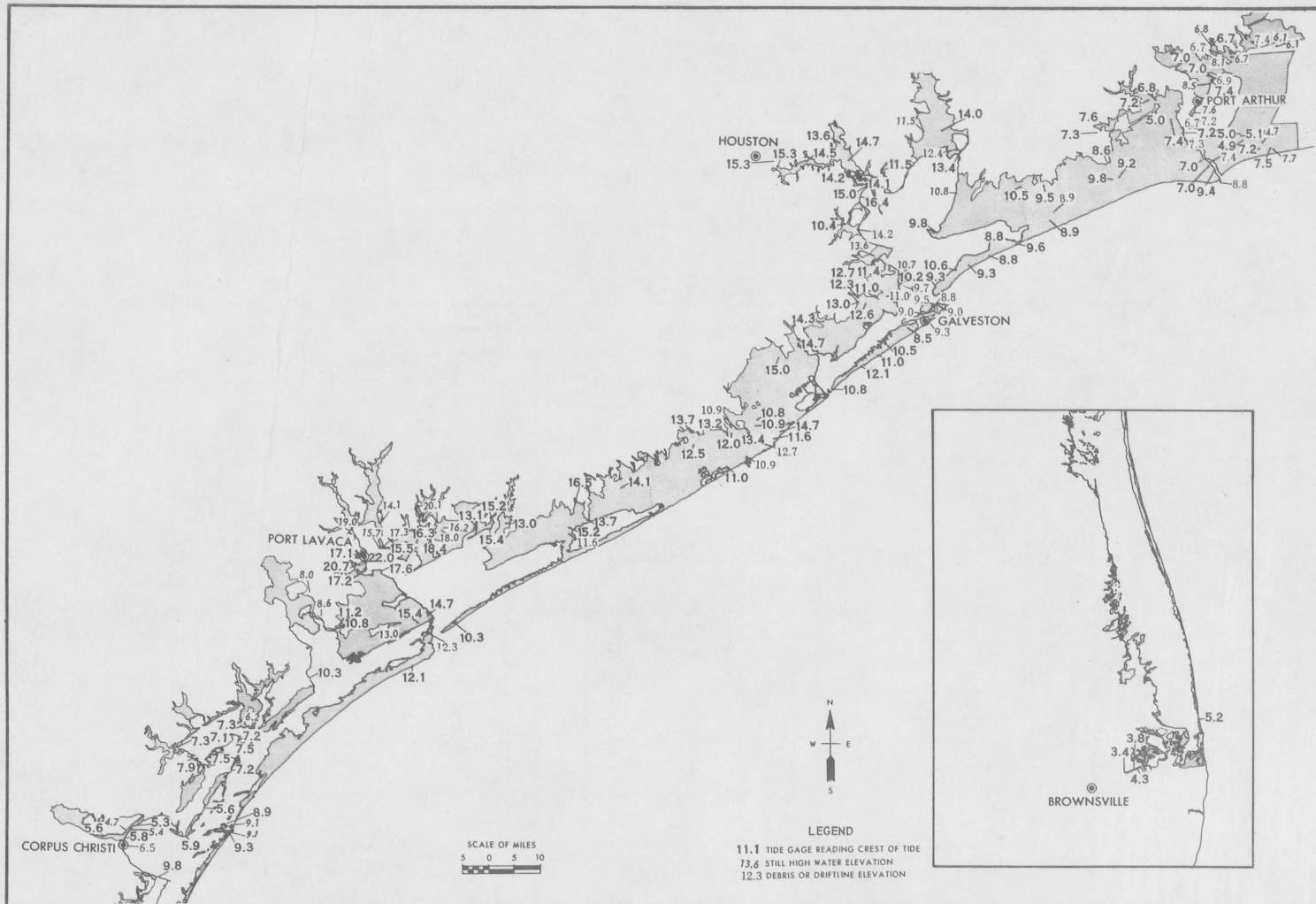


Figure 12. Hurricane Carla, 7-12 September 1961. High water mark chart for Texas. Shaded area indicates the extent of flooding (Harris, 1963).

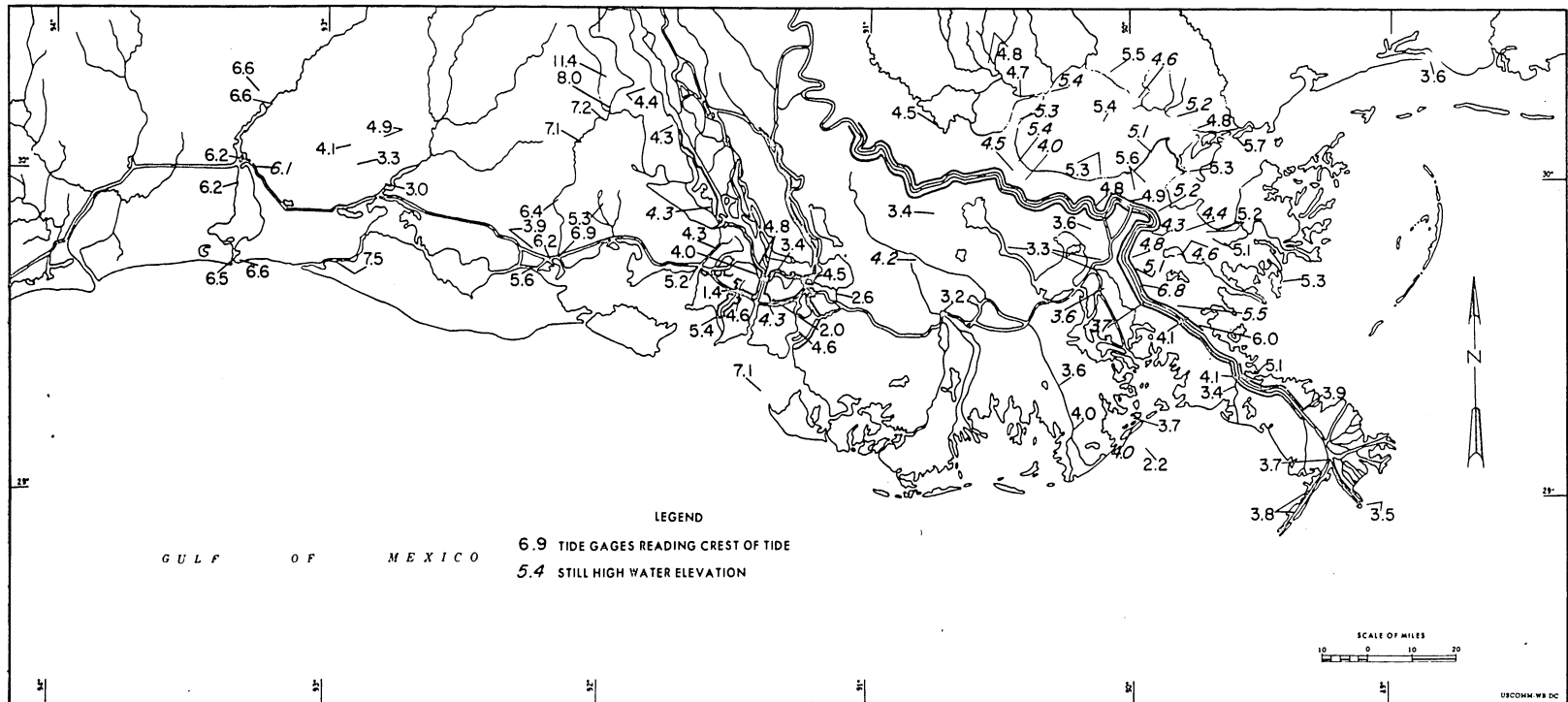


Figure 13. Hurricane Carla, 7-12 September 1961. High water mark chart for southern Louisiana (Harris, 1963).

1964). In these reports, wind fields are given in 2-hour intervals from 0000 to 0600 hours CST and 1200 to 2400 hours CST, and at 1-hour intervals from 0600 to 1200 hour CST. The charts contain windspeeds shown by isotachs and directions shown by arrows. Physical parameters describing Hurricane Audrey are given in Table 2; synoptic weather charts are shown in Figure 14, and Figure 15 shows the hurricane track.

*b. Hydrographic Data.* Most damage by Hurricane Audrey along the gulf coast resulted from high surges. The normal diurnal tidal range for this area is 1 to 2 feet. The coast of Louisiana where the hurricane struck is near sea level, and in places is below sea level for considerable distances inland. Therefore, the area affected by high water was extensive, and low lying regions were inundated as far as 10 to 20 miles inland. The limits of flooding along the coast have been documented by the U.S. Army Engineer District, New Orleans.

Hurricane surges of 6 feet or more above MSL extended from Galveston, Texas to a point 330 miles eastward. These have been documented by Harris (1958, 1963). The observed storm water levels of about 30 recording stations effected by Hurricane Audrey are shown in Figures 16 and 17 (Harris, 1963). These water level records were obtained from tide gage records by plotting hourly values of water elevation, and reduced to a common scale for showing peak values where available. Maximum surge observations along the Texas and Louisiana coasts are shown in Figures 18 and 19. At Galveston, observed water levels reached a peak of 6.2 feet above MSL at 0430 hours CST on 27 June (Fig. 20).

The storm center's landfall point was about 23 miles to the west of Cameron, Louisiana. The highest surge occurred 40 miles to the east of the storm's landfall point, about 3 miles east of Creole. No records of surge for that area were made. Although the tide gage near Cameron, Louisiana was destroyed by the storm, a part of the record was reconstructed from a U.S. Coast Guard Station log on Monkey Island. At that station, the water began to rise at the rate of 1.5 feet per hour by 0400 hours CST, 27 June, reaching a peak elevation of 10.6 feet above MSL between 1000 and 1100 hours CST on the 27th. Storm waves between 4 and 5 feet high with a few peaks possibly reaching 8 to 10 feet, were superimposed on the surge at Cameron. High water remained in the area until 1400 hours CST, then began to recede when winds shifted to an offshore direction (Ross and Blum, 1957). At the Eugene Island tide station, the recording pen of the tide gage went off scale shortly after 0600 hours CST, came back on scale briefly between 0700 and 0800 hours CST, and went off scale again after 0800 hours CST. The recording pen returned to the recording position shortly after 1000 hours CST inferring a peak surge elevation of 7.3 feet MSL. This may be in error by a few tenths of a foot. The Eugene Island gage record is shown in Figure 21.

#### 4. Hurricane Camille.

*a. Hurricane Data.* Hurricane Camille originated off the coast of Africa on 5 August 1969, and was recognized as a tropical disturbance on the 9th, about 480 miles east of the northern Leeward Islands. By 14 August, near the island of Grand Cayman in the Caribbean,

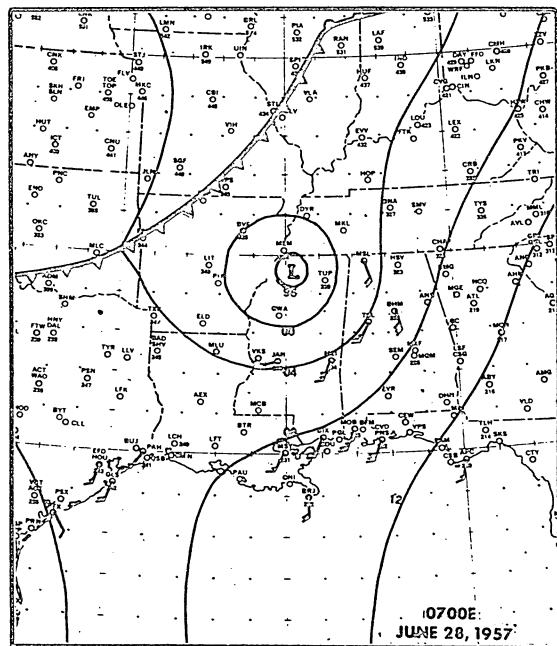
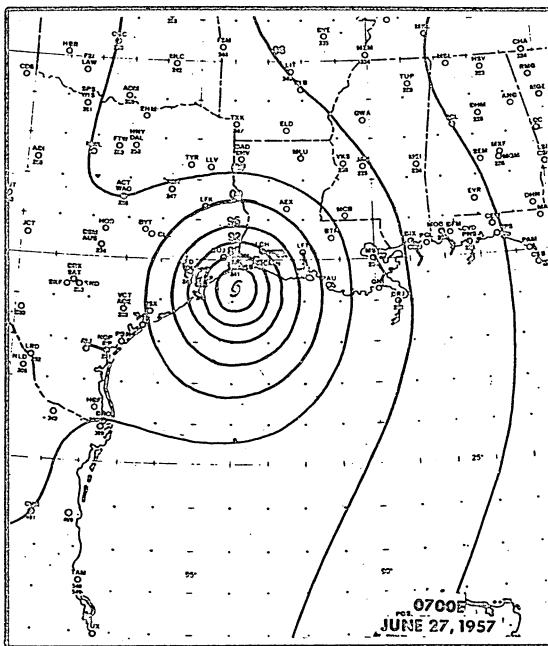
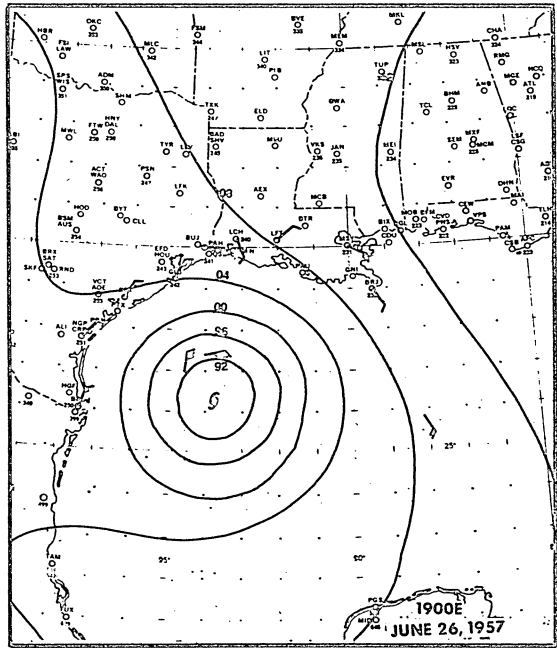
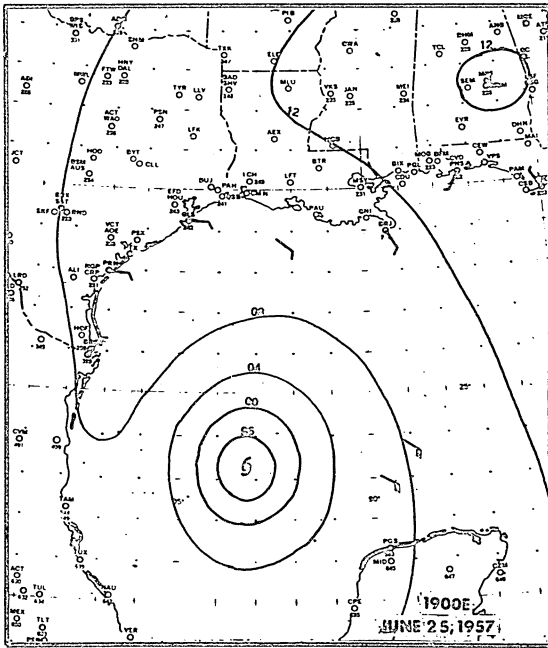


Figure 14. Hurricane Audrey, 26-27 June 1957. Synoptic charts (Harris, 1963).



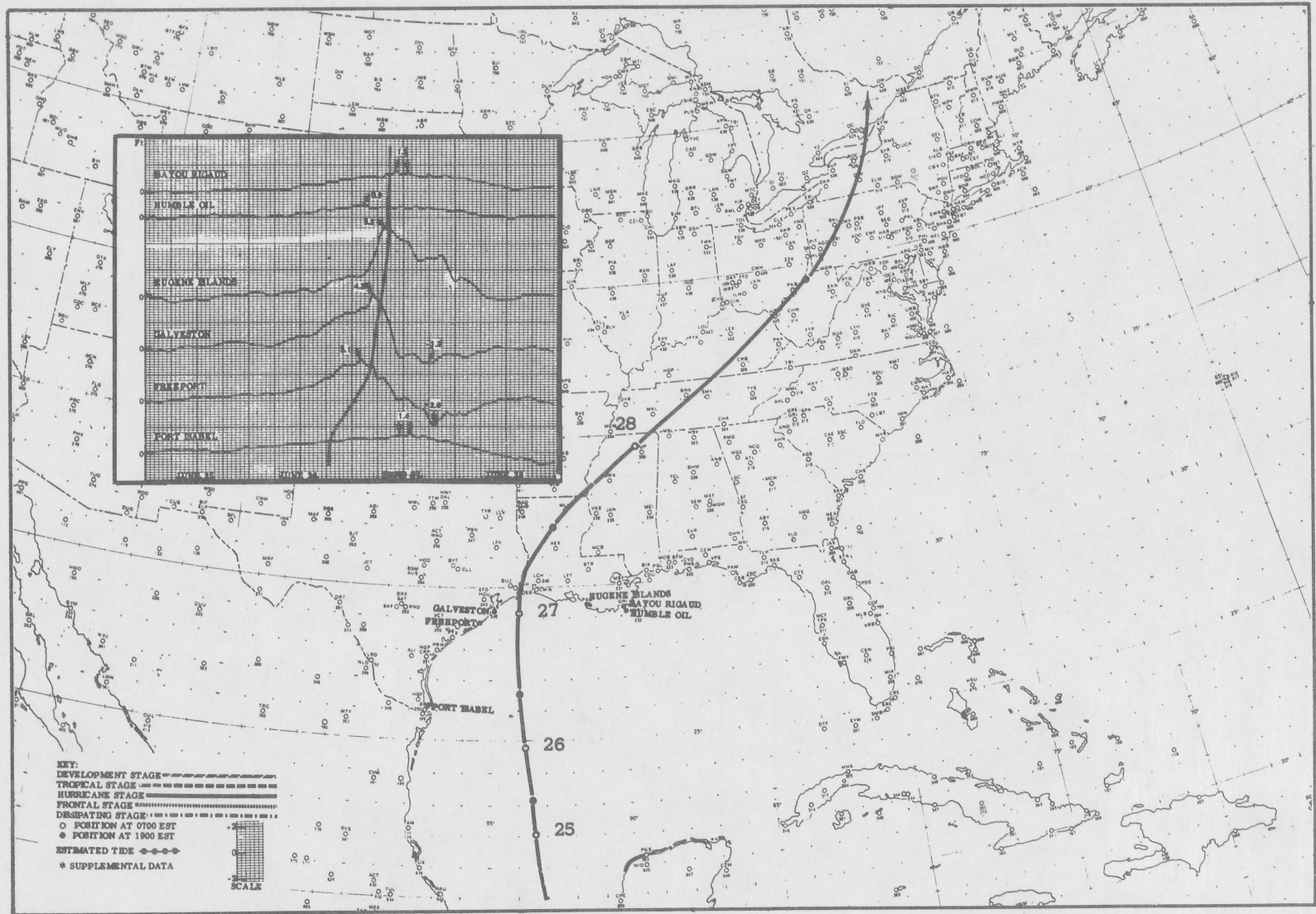


Figure 15. Hurricane Audrey, 26-27 June 1957. Storm surge chart (Harris, 1963).



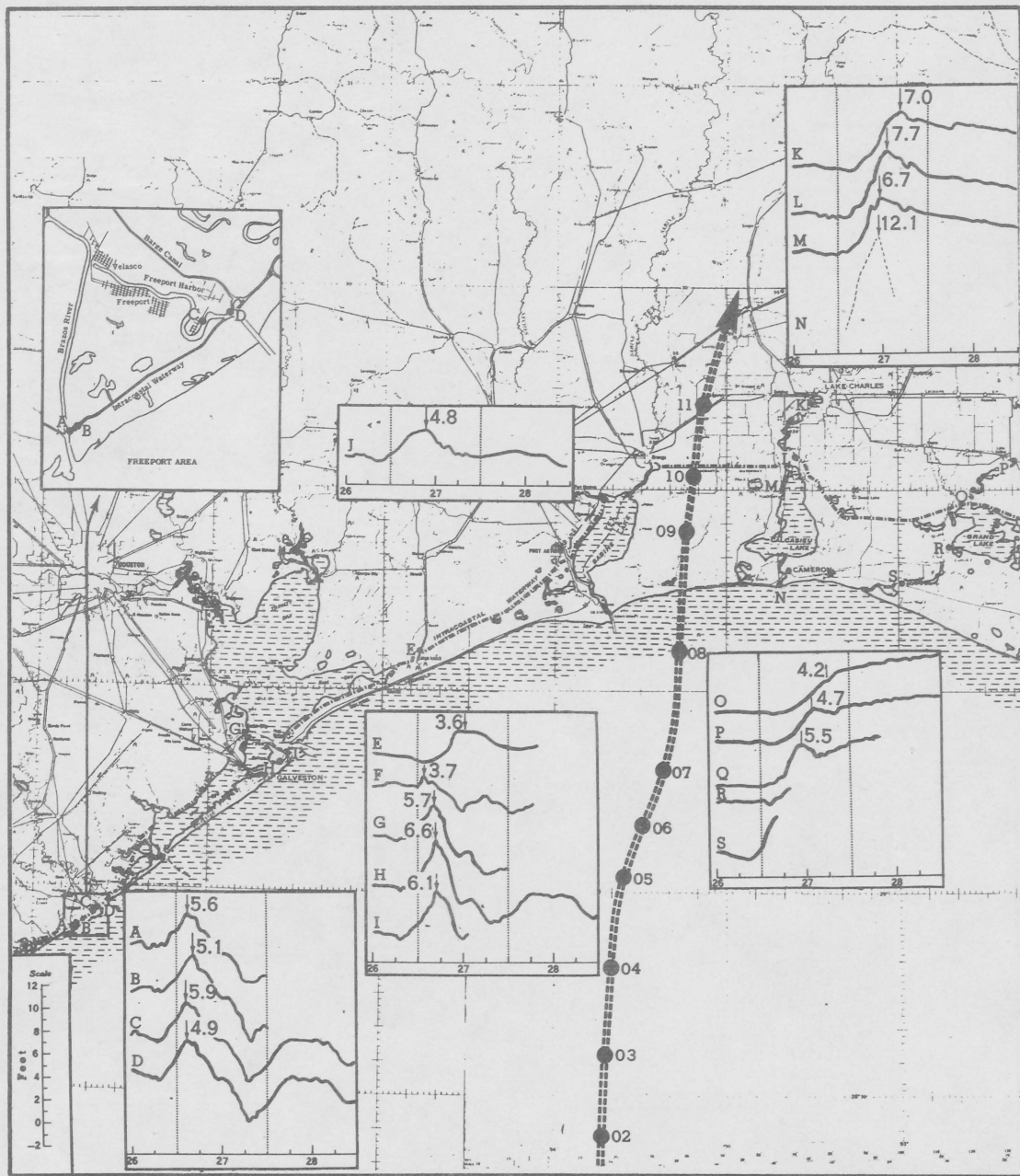


Figure 16. Hurricane Audrey, 26-27 June 1957. Observed tides, Texas and western Louisiana. Insert map of Freeport, Texas (Harris, 1963).

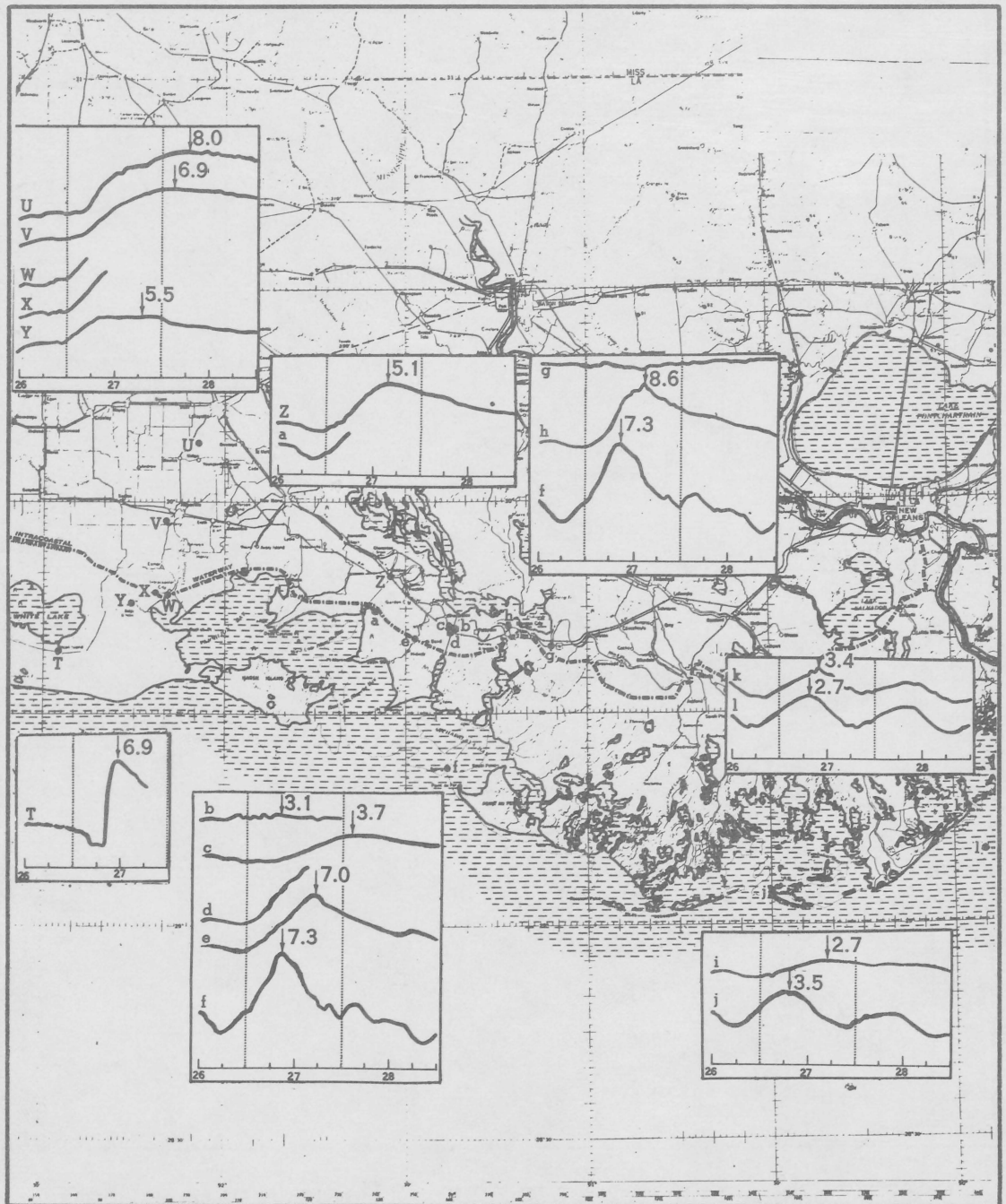


Figure 17. Hurricane Audrey, 26-27 June 1957. Observed tides, eastern Louisiana (Harris 1963).

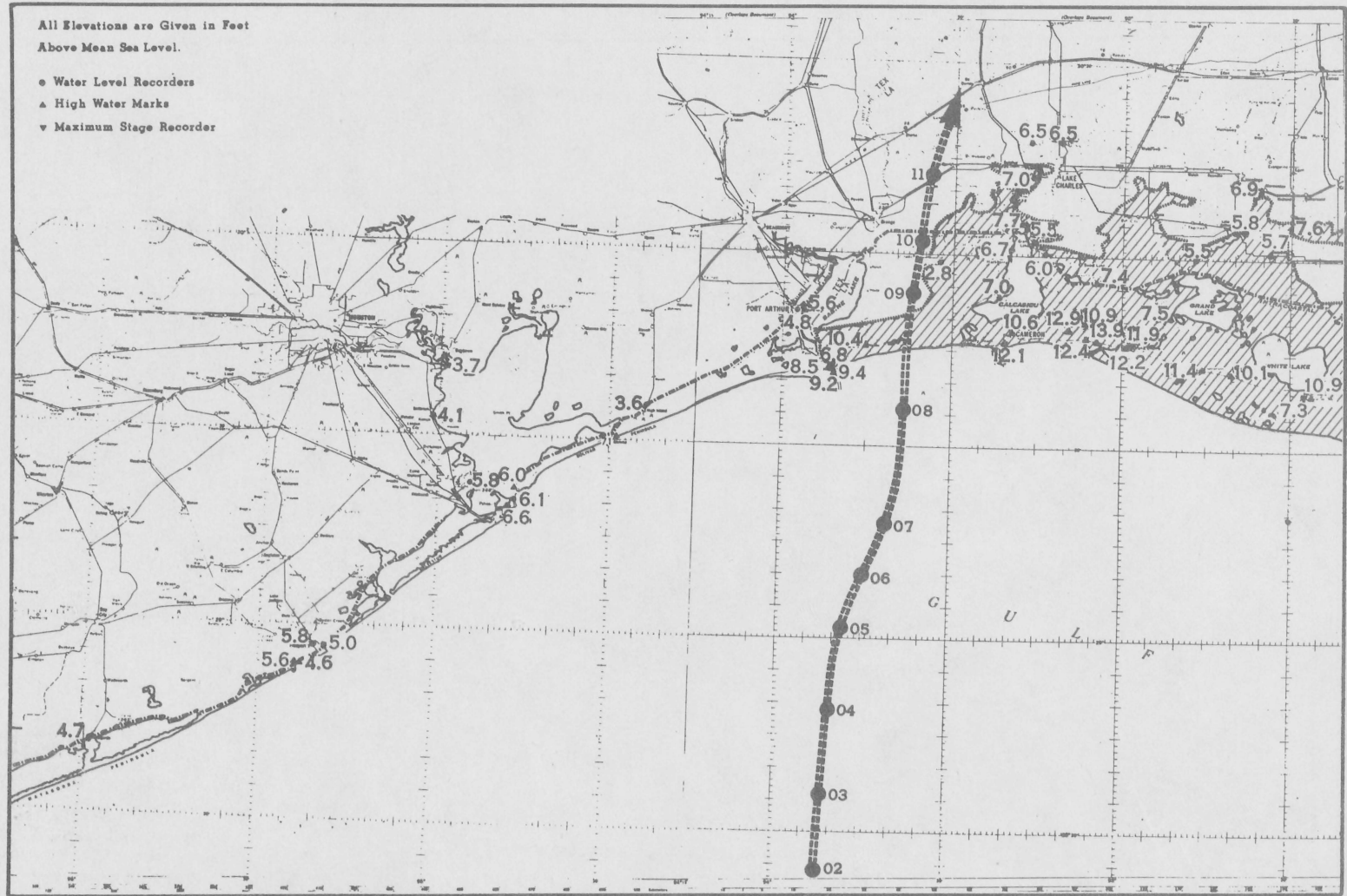


Figure 18. Hurricane Audrey, 26-27 June 1957. High water mark chart for Texas and western Louisiana. Hatched area gives limit of inundation in Louisiana (Harris, 1963).



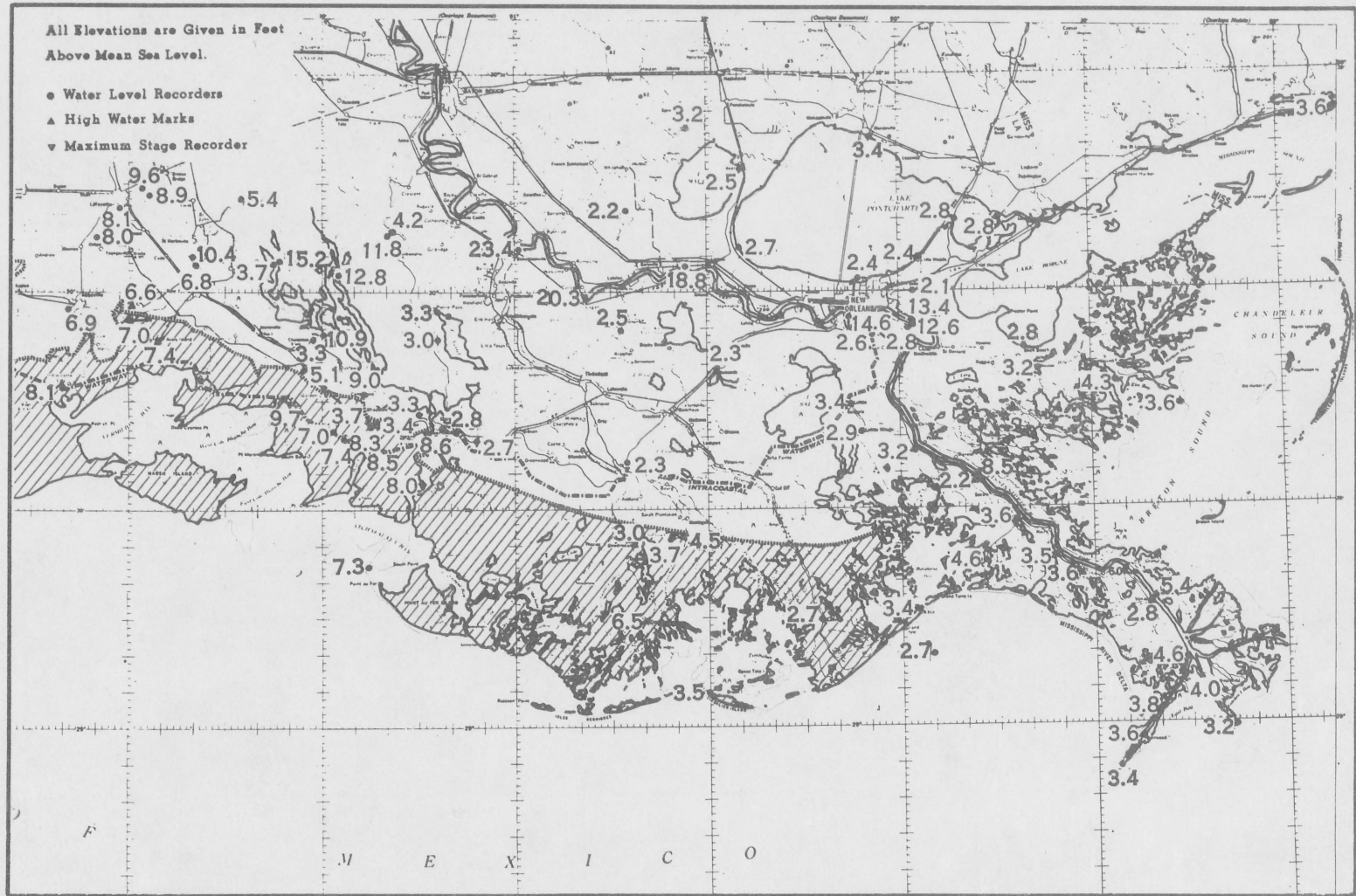


Figure 19. Hurricane Audrey, 26-27 June 1957. High water mark chart, eastern Louisiana. Hatched area gives limit of inundation in Louisiana (Harris, 1963).



Figure 20. Hurricane Audrey, 26-27 June 1957. Observed tide records for Galveston, Texas, and U.S. Coast and Geodetic Survey tide station (Harris, 1963).

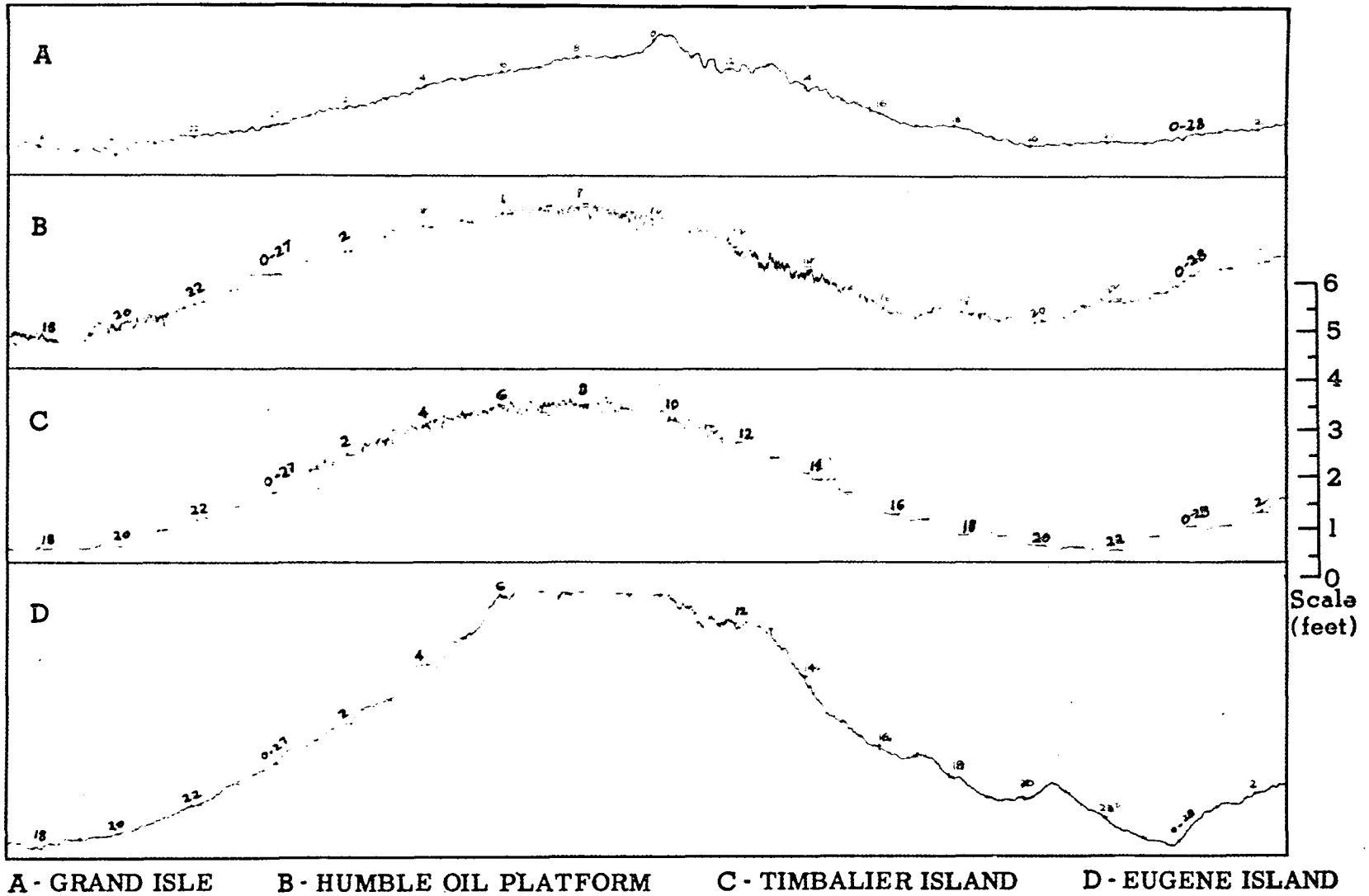


Figure 21. Hurricane Audrey, 26-27 June 1957. Observed tide records at U.S. Coast and Geodetic Survey tide stations in Louisiana (Harris, 1963).

the storm had developed a 29.50 inches of mercury (999 millibars) pressure center and surface winds of 55 miles per hour. The disturbance was christened Tropical Storm Camille.

The storm moved northwestward at 9 miles per hour and its central pressure dropped to 29.26 inches of mercury (991 millibars) late on the 14th. By 15 August, Camille was located 60 miles southeast of Cape San Antonio, Cuba, and had developed into a full hurricane with 28.47 inches of mercury (964 millibars) central pressure and winds of 115 miles per hour (De Angelis, 1969). The storm moved into the Gulf of Mexico on the 16th and changed to a north-northwestward direction with a translational speed of 14 miles per hour. Its central pressure had dropped to 26.72 inches of mercury (905 millibars). Hurricane watches were set up from Biloxi, Mississippi, to St. Marks, Florida. On 17 August, Camille was 250 miles south of Mobile, Alabama with surface winds near the center estimated at more than 201.5 miles per hour. Hurricane warnings were issued for the Mississippi coastal areas extending westward to New Orleans.

The center of Hurricane Camille made landfall at Clermont Harbor, Waveland, and Bay St. Louis, Louisiana at about 2330 hours CDT on the 17th. There were no records of winds near the center, but estimates ranged up to 201.5 miles per hour. At the west end of the Bay St. Louis Bridge a pressure of 26.85 inches of mercury (909.3 millibars) was reported. At Boothville, Louisiana, winds of 107 miles per hour were recorded before a power failure; at Pilottown, Louisiana, the S.S. *Cristobal* estimated winds at 160 miles per hour. Winds at Keesler Air Force Base, Biloxi, Mississippi were measured at 81 miles per hour with gusts to 129 miles per hour. At Ingalls Shipyard, Pascagoula, Mississippi the highest sustained wind reached 81 miles per hour while a local radio station reported 104 miles per hour winds before power failure.

Hurricane-force winds were concentrated close to the storm's center as it moved inland. These winds extended from east of New Orleans to Pascagoula, while gusts of hurricane force winds extended along the coast from New Orleans to west of Mobile Bay.

Once over land, Hurricane Camille weakened considerably moving northward through Mississippi passing close to Columbia, Prentis, Jackson, Canton, and Greenwood. At Jackson, winds gusted to 67 miles per hour as the storm center passed 10 miles west of the city. The storm was only identifiable as a depression by the time it reached the northern Mississippi border and was tracked northward to southern Virginia.

Torrential rainfall caused devastating flash floods and landslides along the eastern slopes of the Blue Ridge Mountains and record flooding along the James River.

Surface wind fields and pressure fields of Hurricane Camille were obtained from the Hydrometeorological Reports HUR 7-113, and 7-113A (Environmental Science Services Administration, 1969 and 1970). In the latter report wind fields are given at 6-hour intervals from 0000 hours through 1800 hours GMT, 17 August, then at 3-hour intervals through 0500 hours GMT, 18 August, and then at 6-hour intervals until 0000 hours GMT, 19 August. The track for Hurricane Camille is given in Figure 22; Figures 23, 24, and 25 are the synoptic weather charts used in this study.

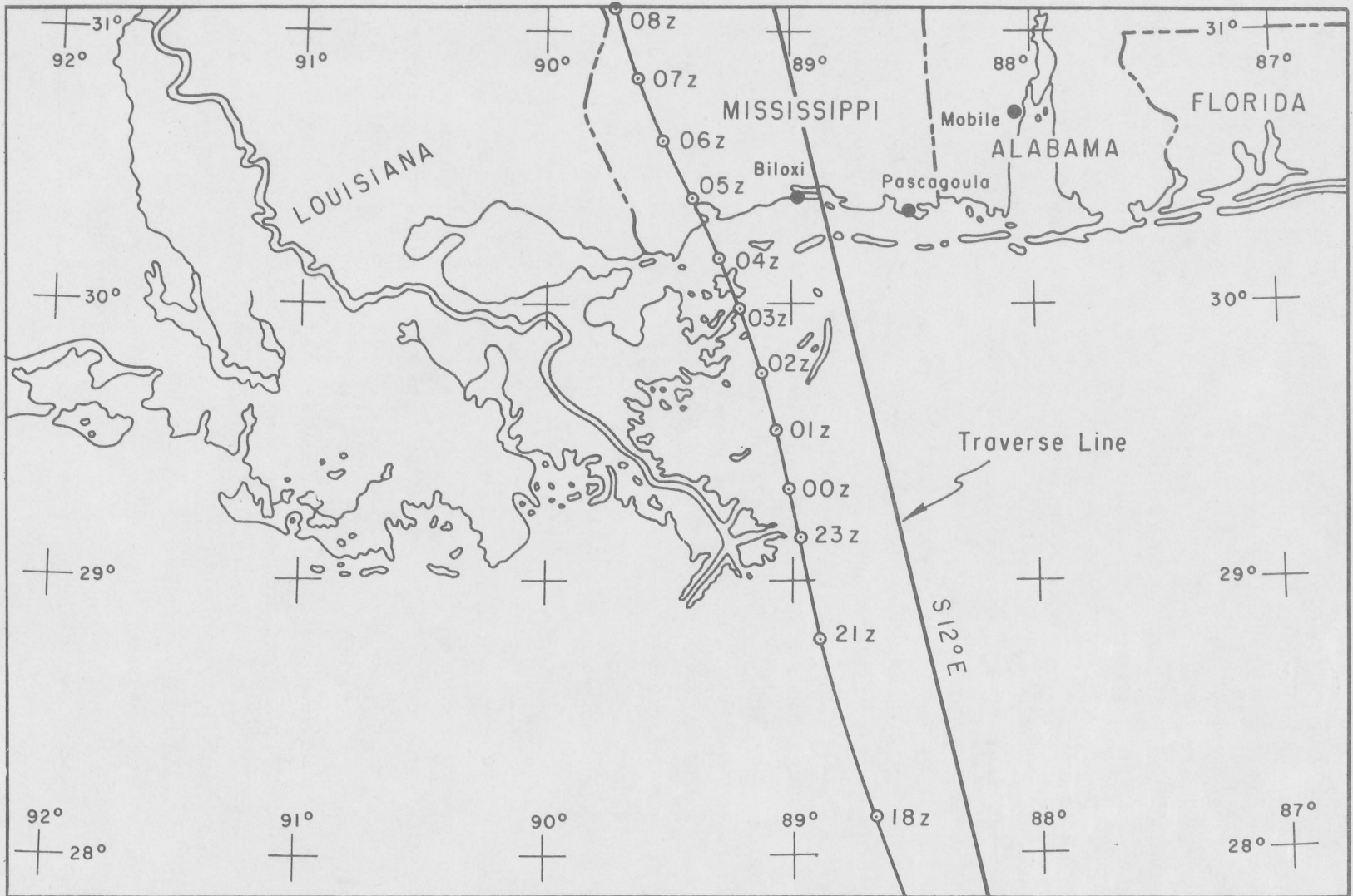
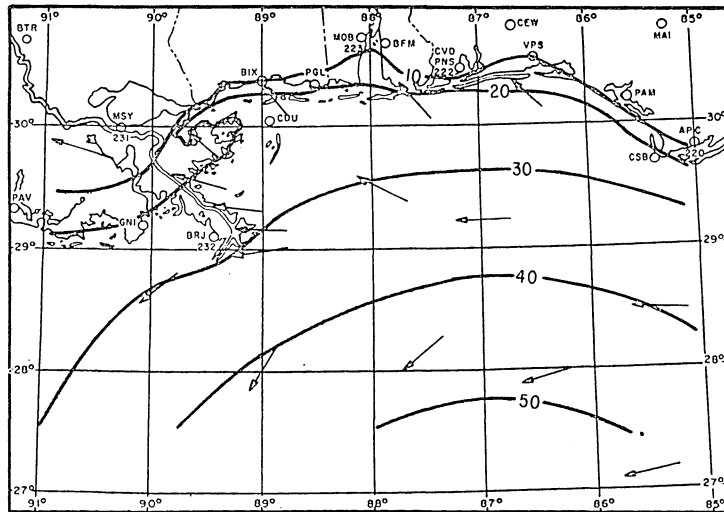
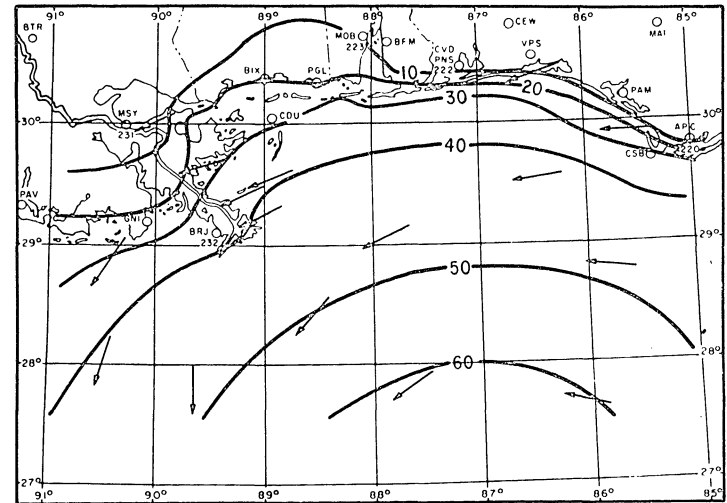


Figure 22. Track for Hurricane Camille, August 1969 (Environmental Science Services Administration; HUR 7-113A, 1970).

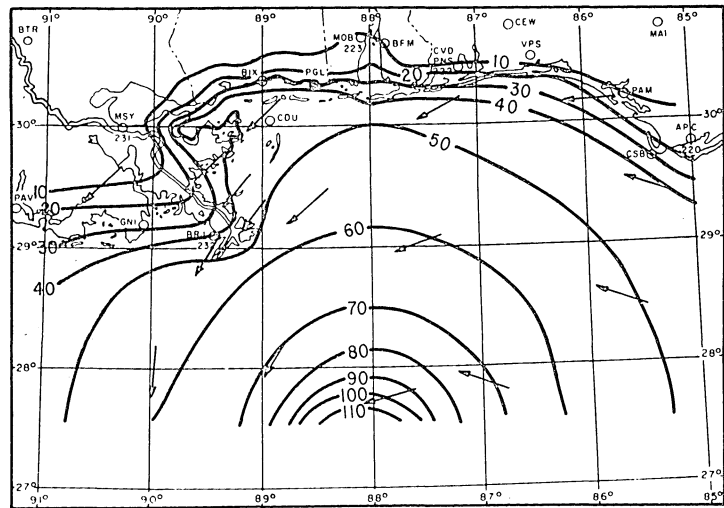




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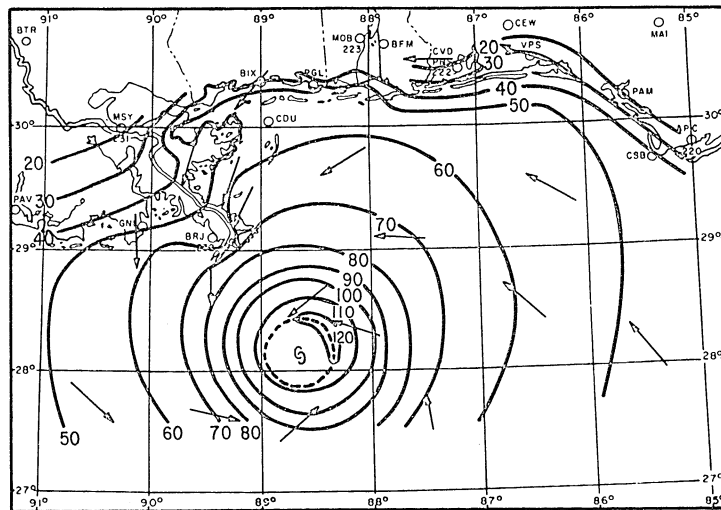


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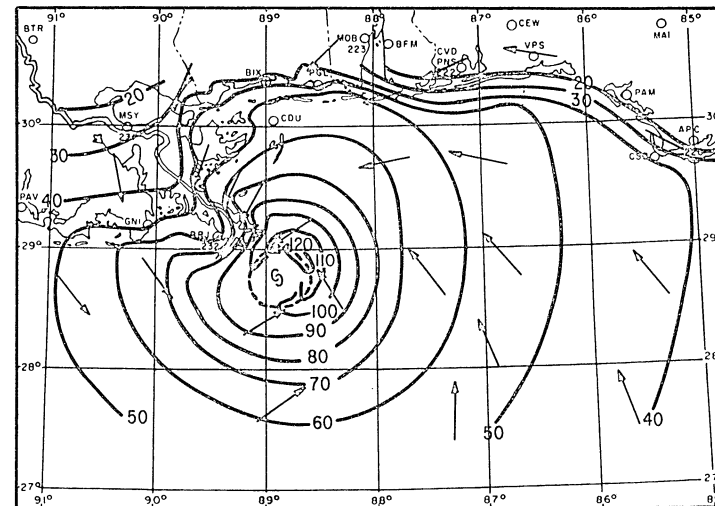


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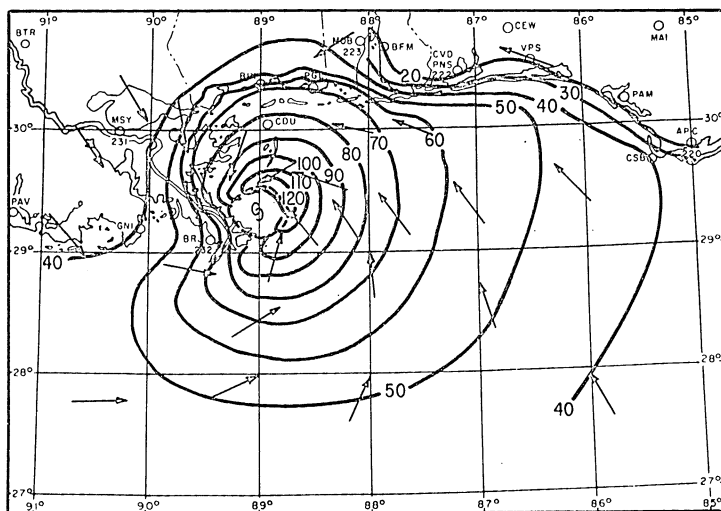
Figure 23. 30-foot surface isovels (knots), Hurricane Camille (Environmental Science Services Administration; HUR 7-113A, 1970).



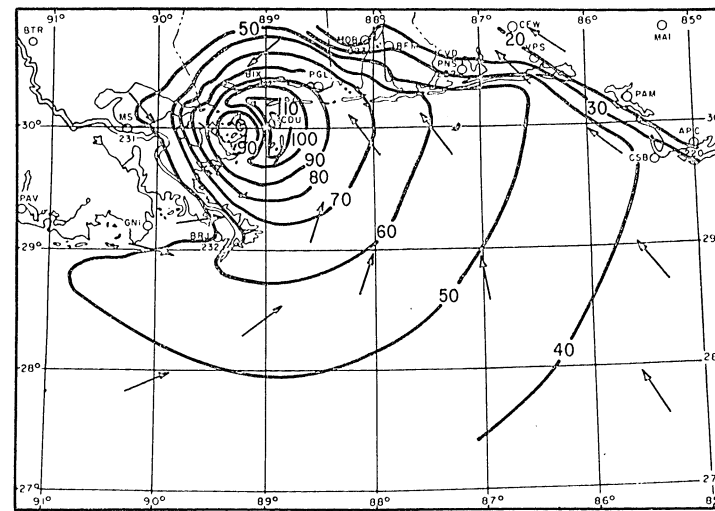
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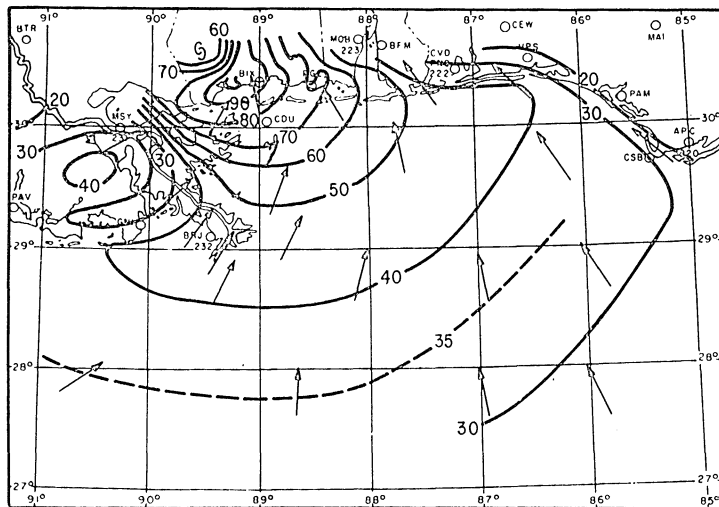


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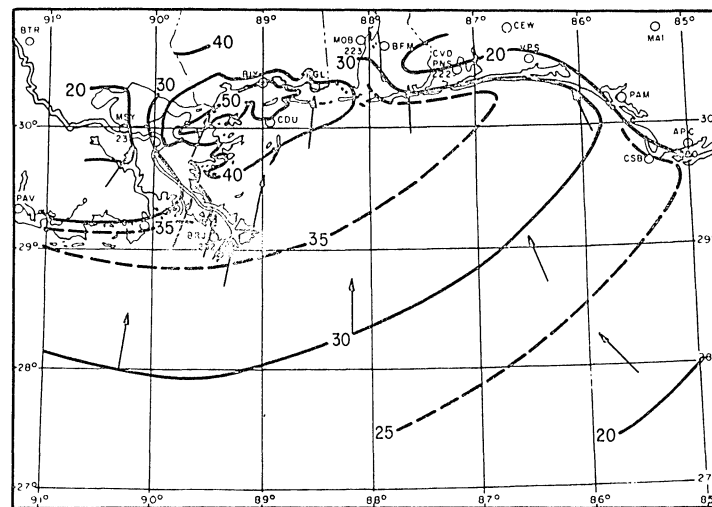


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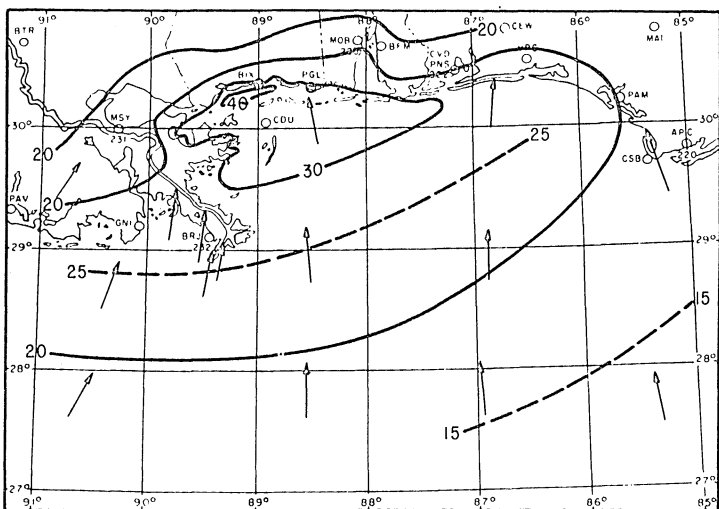
Figure 24. 30-foot surface isovels (knots), Hurricane Camille (Environmental Science Services Administration; HUR 7-113A, 1970).



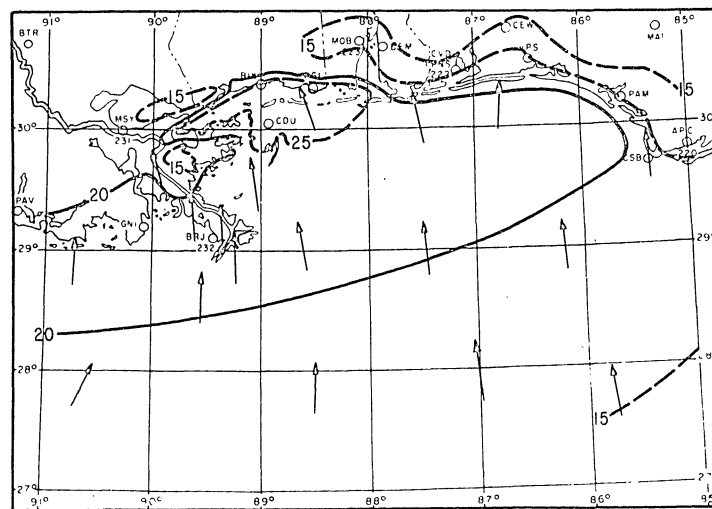
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I 1200 GMT - 18 Aug 69



J 1800 GMT - 18 Aug 69



K 0000 GMT - 19 Aug 69

Figure 25. 30-foot surface isovels (knots), Hurricane Camille (Environmental Science Services Administration; HUR 7-113A, 1970).

*b. Hydrographic Data.* The storm surge generated by Camille flooded coastal regions from lower Plaquemines Parish in Louisiana, to Perido Pass, Alabama. Maximum storm surge of 24.6 feet above MSL was measured in the Pass Christian-Long Beach, Mississippi, area. In St. Louis Bay, maximum surge was 18 feet above MSL. At Back Bay of Biloxi, Mississippi, the surge elevation reached about 15 feet above MSL (U.S. Army Engineer District, Mobile, 1970), and near 10 feet above MSL as far east as the Mississippi-Alabama border.

Along the Louisiana coast, the storm surge was 15 feet above MSL at Boothville, and 9 feet above MSL near the mouth of the Mississippi, at Garden Island. At Alluvial City surge height reached 7.97 feet above MSL, at Chef Menteur Pass 11.06 feet, at Shell Beach 11.06 feet, and at Rigolets 9 feet. Detailed maps of areas flooded and surge measurements of Hurricane Camille are presented by Wilson and Hudson (1969). Additional surge elevations along the Louisiana and Mississippi coasts are shown in Figure 26.

Surge data along the coast of Louisiana, Mississippi and Alabama, were obtained from the U.S. Army Engineer District, Mobile Report (1970). The surge elevation of 19.5 feet above MSL used for the calibration is based on a visual observation of high water mark at Biloxi, Mississippi (Fig. 27). This value may include runup due to wave action. The reduction of tide data is discussed in Section IV.

## 5. Hurricane Carol.

*a. Hurricane Data.* Hurricane Carol formed near the northeastern Bahama Islands on 26 and 27 August 1954. Then, it moved northward to a position near  $30^{\circ}\text{N.}$ ,  $76^{\circ}\text{W.}$ , and became nearly stationary. For the next three days the storm drifted very slowly and by 30 August had moved to about  $32.5^{\circ}\text{N.}$ ,  $77.5^{\circ}\text{W.}$  Then, it began accelerating in a north-northeast direction passing near Cape Hatteras at about 2100 or 2200 hours EST, 30 August (Fig. 28). Highest winds were estimated to vary from 75 to 125 miles per hour.

During the morning of the 31st, Carol moved very rapidly in a north-northeast direction from Cape Hatteras, crossed over the eastern end of Long Island and entered the New England area at the south shore of Rhode Island. From there, Carol crossed Rhode Island, eastern Massachusetts and along the Maine-New Hampshire border into Canada through the St. Lawrence Valley. The storm left 60 dead and over \$460 million damage to property and crops in the North Atlantic States (Davis, 1954).

Carol struck the Rhode Island area in full strength at about 1030 hours EST, 31 August, with sustained winds up to 90 miles per hour and gusts up to 105 miles per hour. Highest winds were at Block Island, Rhode Island, where 130 miles per hour was measured in gusts.

In the vicinity of Narragansett Bay, at Point Judith, Rhode Island, maximum hurricane winds with velocities of 73 to 82 miles per hour were observed about 1100 hours EST. At the entrance of Narragansett Bay near Brenton Point winds with velocities from 73 to 82 miles per hour were observed between 1100 and 1200 hours EST.

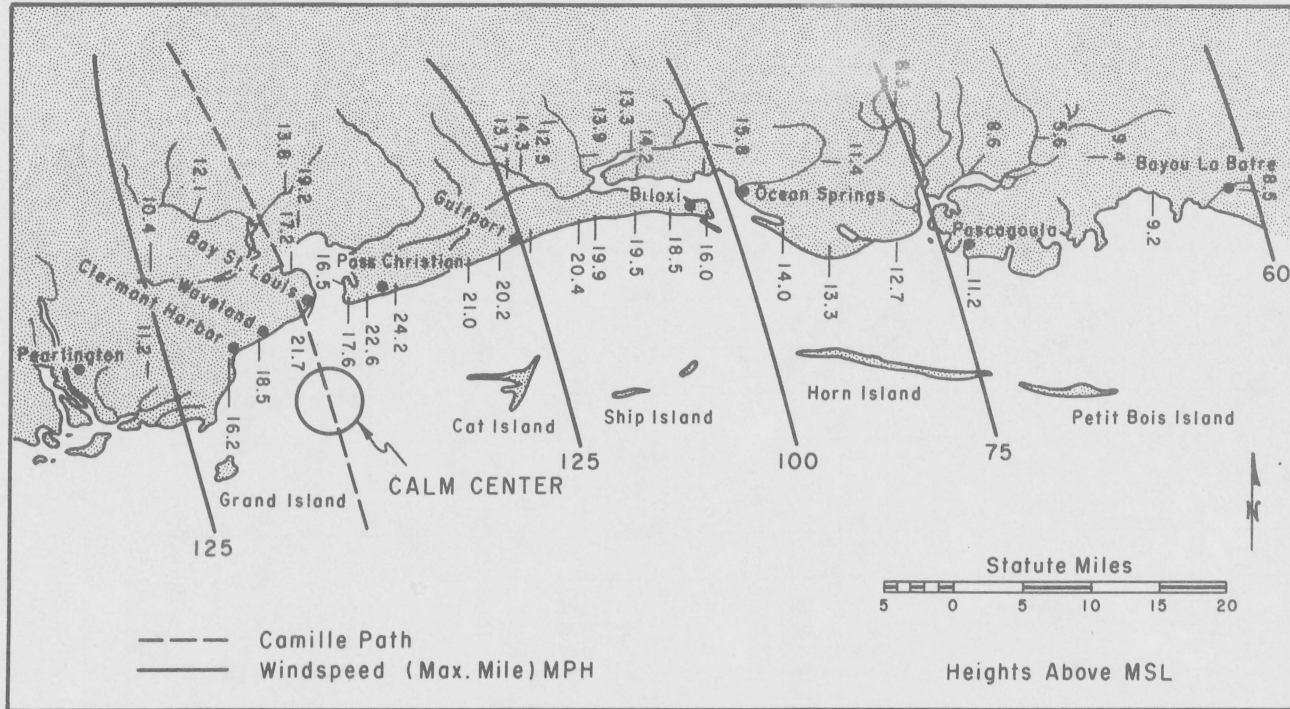


Figure 26. Hurricane Camille track, windspeeds and resulting surges along the Mississippi coast.

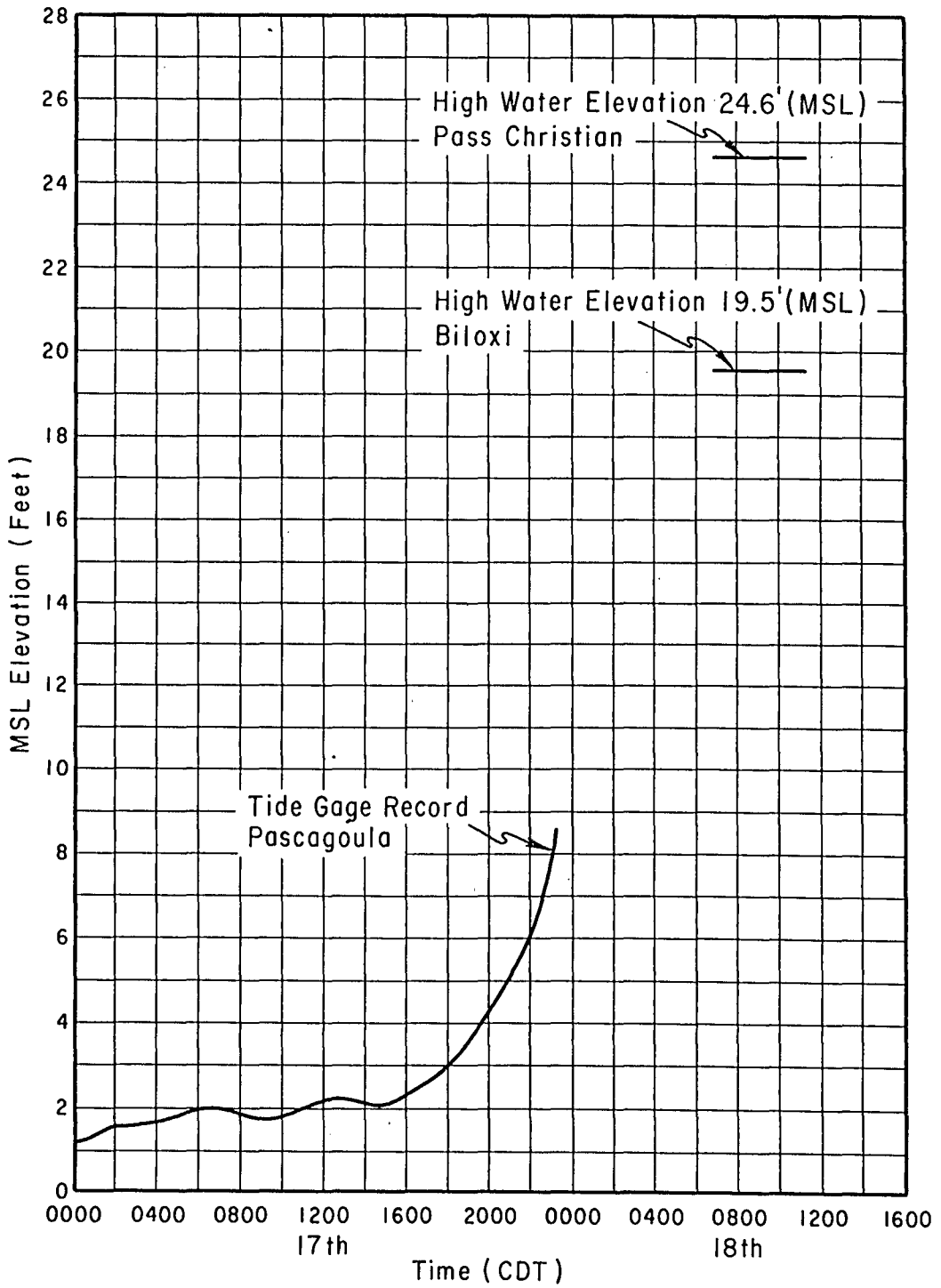


Figure 27. Hurricane Camille, August 1969. Storm surge chart.

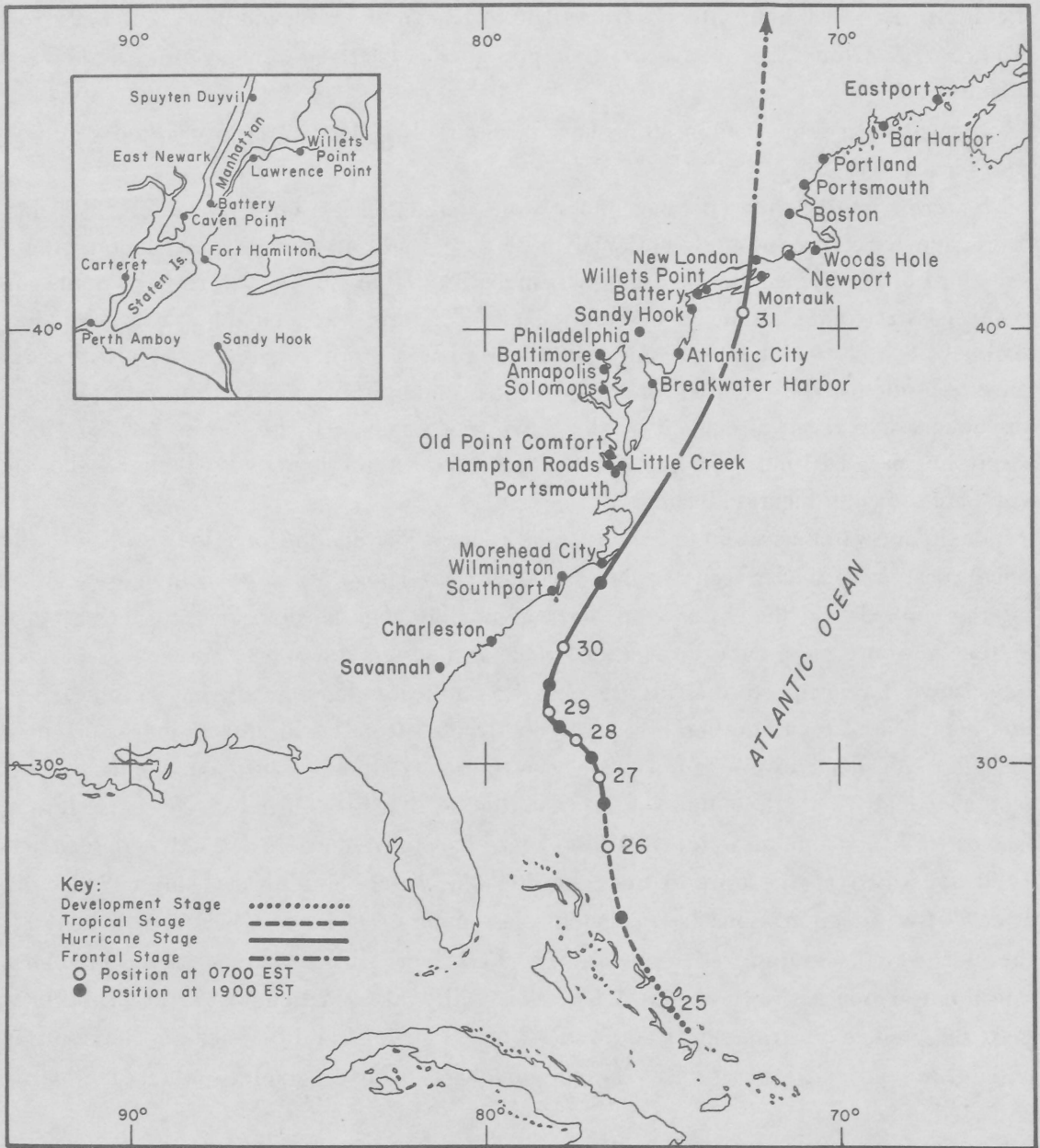


Figure 28. Storm track of Hurricane Carol, 30-31 August 1951. Insert map for New York Harbor (Harris, 1963).

Surface wind field and pressure fields for Hurricane Carol were obtained from the Hydrometeorological Report HUR 7-54 (U.S. Weather Bureau, 1959). In the report, wind fields are given at 3-hour intervals from 0130 to 1330 hours EST, and 2-hour intervals from 1330 to 1730 hours EST, 31 August. Synoptic weather charts of the hurricane are shown in Figure 29.

Historical accounts for this storm have been given by McGuire (1954), Rhodes (1954), and Davis (1954).

*b. Hydrographic Data.* Because of forward speed, intensity, and arrival during high tide, Hurricane Carol produced exceptionally high surges and destructive wave action. About one-third of Providence, Rhode Island, was under 8 to 10 feet of water for several hours and many coastal communities were severely damaged. Storm waves outside Narragansett Bay had heights over 40 feet approaching from the southeast. Storm surges associated with Carol have been discussed by Redfield and Miller (1957) and by Harris (1963). Surge data in these discussions have been assembled by the U.S. Army Engineer District, New York and U.S. Army Engineer Division, New England. Peak surge values and surge hydrographs for the east coast are shown in Figures 30 and 31.

Maximum water elevation observed at Narragansett Pier due to Hurricane Carol along the open coast outside Narragansett Bay was 12.8 feet above MSL. No complete recorded hydrograph exists at the entrance to Narragansett Bay, and the closest gage to the entrance is the Newport gage on Constellation Dock at Coasters Harbor Island, which became inoperative during the storm. This gage was 4.5 nautical miles from the bay entrance. The hydrograph used for the calibration was reconstructed from visual observations which may include runup due to wave action. Water elevation at Newport reached a maximum of 11.6 feet above MLW at 1230 hours EST (reported by the City Engineer, Newport, Rhode Island) while a maximum water elevation of 12.5 feet above the gage datum was observed at 1100 hours EST at the Newport tide gage site. This water elevation probably includes the effects of wave action. Considering that the gage datum was 1.4 feet below MLW in 1954, the net water elevation at Newport was 11.1 feet MLW. The maximum predicted astronomical tide at Newport was 4 feet above MLW at 0917 hours EST. At 1100 hours EST, the predicted astronomical tide was 3.2 feet. The 7.9 feet difference was the result of initial rise and hurricane surge. The methodology of hydrographic data reduction is discussed in Section IV.

## IV. DATA REDUCTION

### 1. Wind Data Reduction.

A prerequisite of accurate storm surge calculation is the availability of good wind field data along a selected computational traverse. Most wind field data of hurricanes are given in 3- and 6-hour weather charts; however, critical hurricane surge effects may be experienced at a coastline between time intervals for which wind field charts are available. Drastic changes in windspeed and direction can occur during these widely separated time intervals



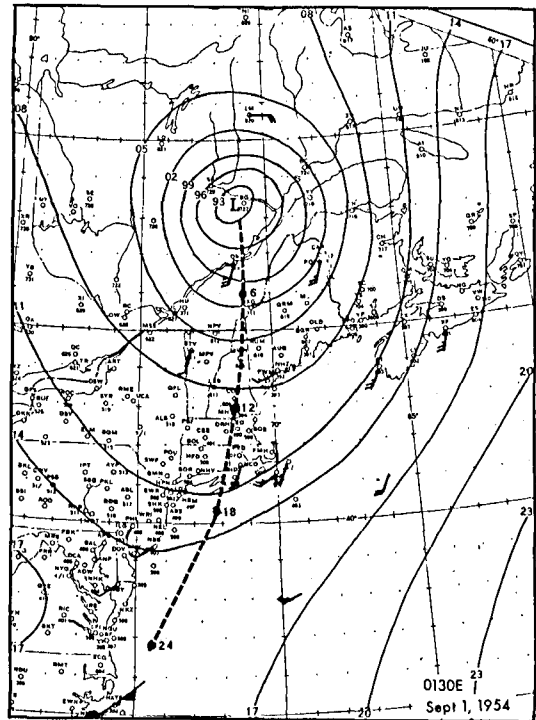
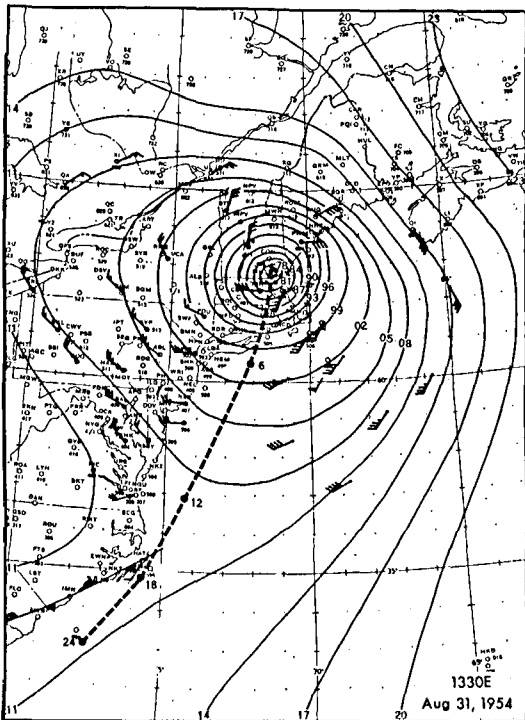
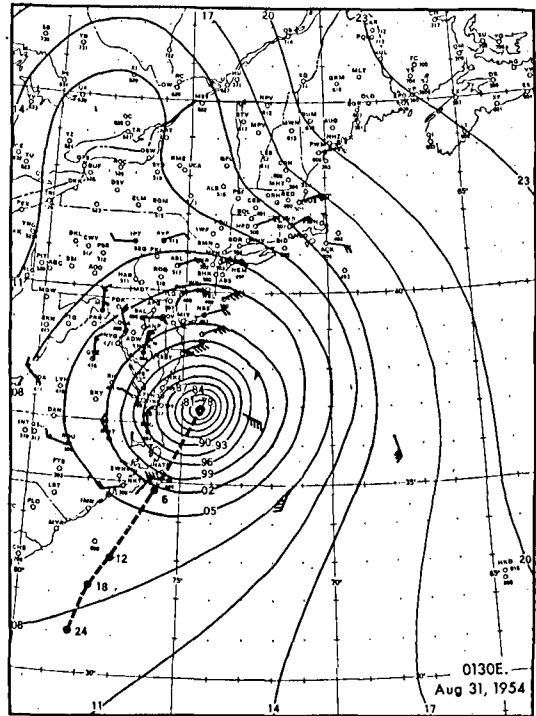
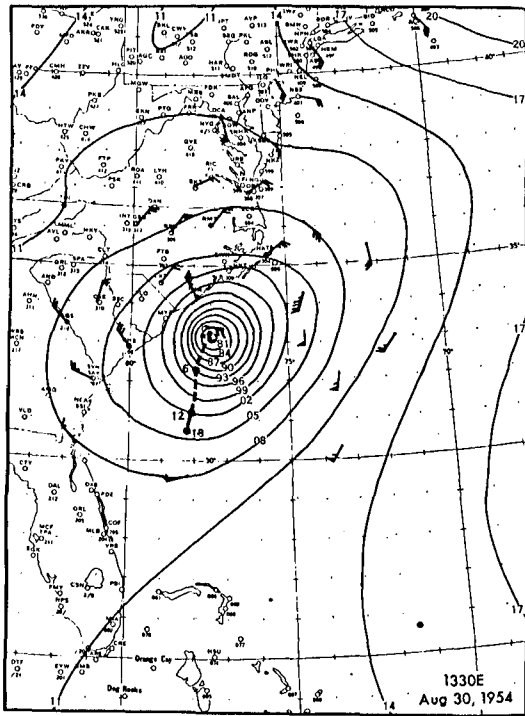


Figure 29. Hurricane Carol, 30-31 August 1954. Synoptic charts (Harris, 1963).

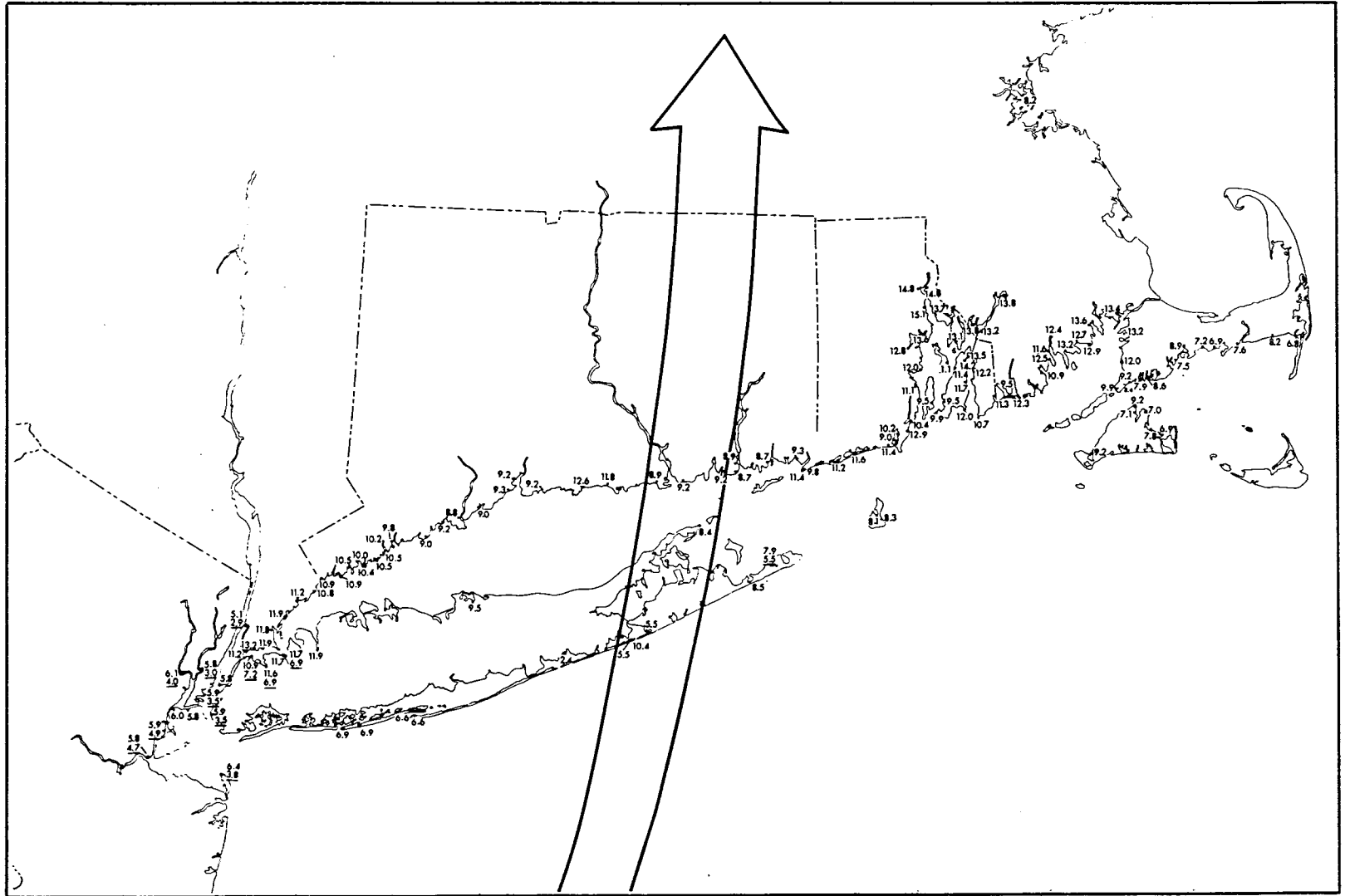


Figure 30. Hurricane Carol, 30-31 August 1954. High water marks in New York and New England (Harris, 1963).

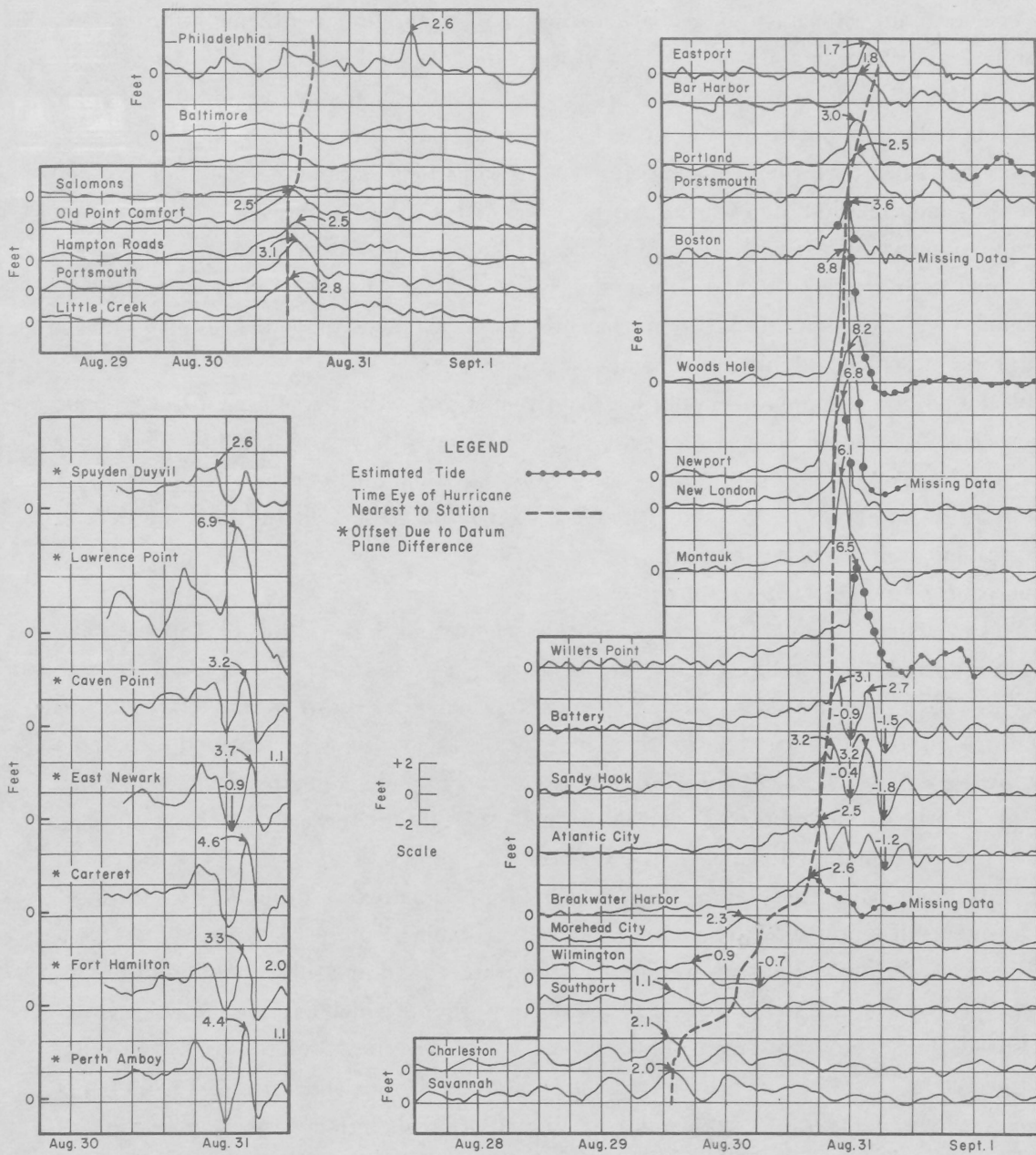


Figure 31. Hurricane Carol, 30-31 August 1951. Storm surge chart (Harris, 1963).

which may render storm surge calculations invalid. Therefore, frequent wind information is desirable, at least along traverses where storm surge calculations are made. An accurate interpolation procedure must be used to generate additional temporal wind information. This data reduction can be done graphically or by computer. However, both schemes have limitations. When a storm is moving slowly, the wind field data may be voluminous and graphical interpolation time consuming. When the storm is moving rapidly, linear interpolation may give inaccurate wind field data, especially in situations where abrupt wind direction reversals occur, and when the hurricane center crosses the coastline. In interpolating, abrupt wind direction changes often require a subjective evaluation and interpretation, or very sophisticated computer programs.

In this study, a graphical, nonlinear, data reduction method was used to interpolate windspeeds,  $W$ , and wind vector directions,  $\theta$ , for time intervals of 1 hour or less along each traverse. A computer subroutine was introduced to obtain the distances of the storm center,  $r$ , to points on the traverse. The methodology used for reduction of wind data of historical hurricanes is discussed below.

*a. Windspeeds and Wind Vector Directions.*

(1) A computational line (traverse) was selected and drawn on a chart of the same scale as the wind charts in the same geographical area (Fig. 32).

(2) Each traverse was plotted, or a transparency showing the traverse was superimposed, on all appropriate weather charts of the same scale which covered the chosen time period for which calculations of surge were to be made, for a particular hurricane. Weather charts with contours of windspeeds in knots (30-foot surface isotachs or isovels), and vectors of wind directions, were used in this study.

(3) Points along the traverse were selected for calculations at different time stages of the storm. Wind vector directions and speeds were obtained from each weather chart for a time interval starting at the seaward point of the traverse and at a subsequent point on the traverse. This was done at the earliest phase of the storm for which data was available if the windspeeds at the seaward point exceeded the critical value of 14 knots. Windspeeds,  $W$ , were obtained by interpolation between isovels and converted from knots to miles per hour (1 knot = 1.15 miles per hour). Wind direction angles,  $\theta$ , (the angles between traverse line and wind direction) were obtained by averaging two or more wind vectors on either side of the traverse and these measured counterclockwise from the traverse line from  $0^\circ$  to  $360^\circ$ . Also, the initial distance values,  $r_{LM}$ , of the storm center to the traverse shore-intercept were obtained in nautical miles for each position of the storm given on the weather charts (Fig. 32). Subsequent values of distances  $r$  to all other points on the traverse were developed internally by the program, according to the method given in Section VI. This data reduction procedure was repeated for each time step in the development of the storm (using a 6-, 3-, or 1-hour weather chart). The process of interpolation was subjective, and attention was given in developing the data in a reasonable fashion.

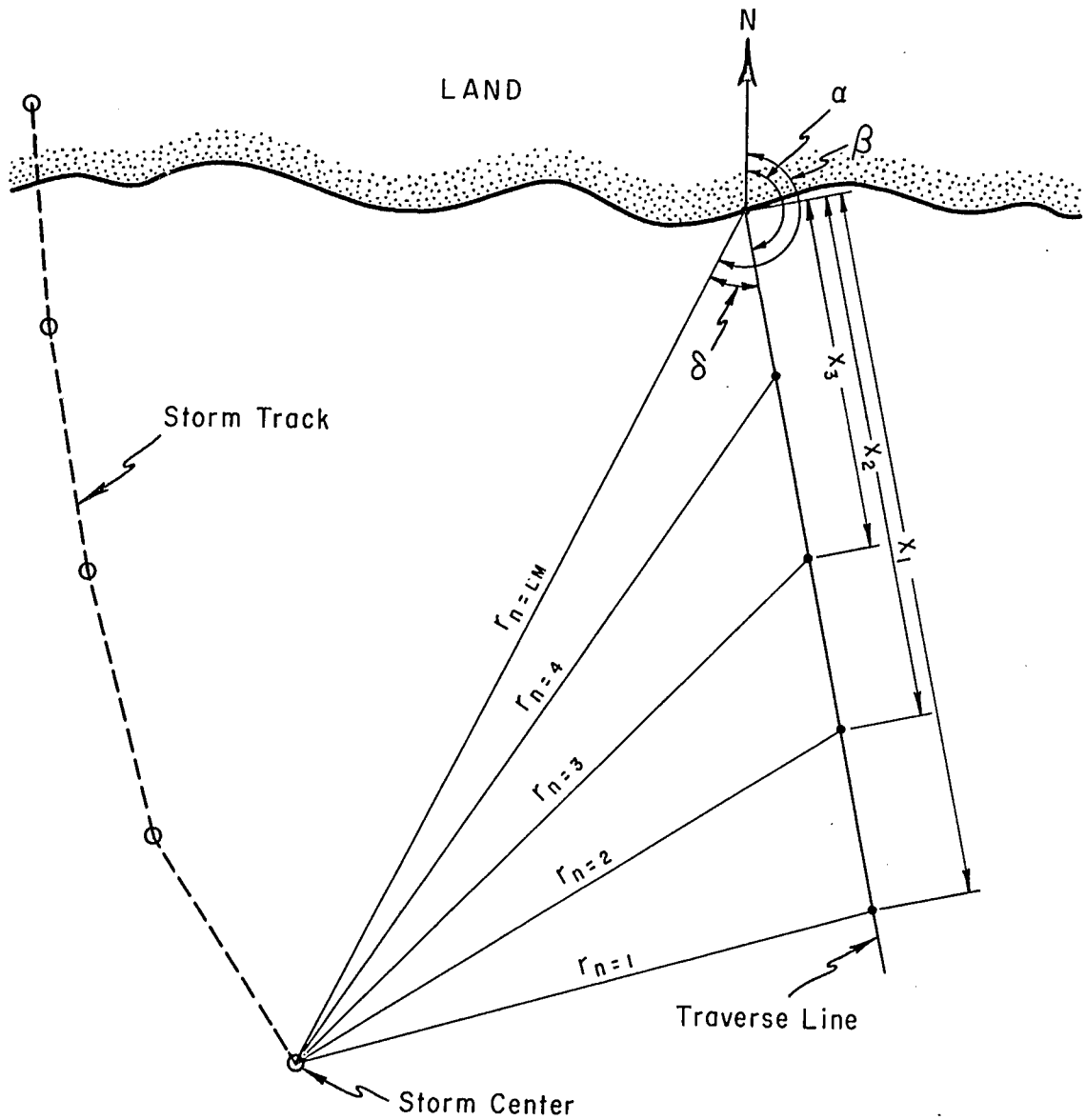


Figure 32. Schematic of nonlinear graphical interpolation of wind data and distance of the storm center to points on the traverse.

(4) If the data obtained by this procedure were from hourly wind charts, it was tabulated and then punched directly on computer cards without further reduction. This form provided an input for the numerical surge calculations. An example of the input data format is shown in Appendix A.

(5) If data were obtained from 3- or 6-hour weather charts, then graphs were prepared of: windspeed changes with time for points on the traverse, wind direction changes with time for points on the traverse, wind direction changes along the traverse for each 3- or 6-hour interval, and changes with time in the distance of the hurricane center with each point on the traverse. Examples of graphs used for reduction of the 3- and 6-hour wind data are given in Figures 33 through 36. Hourly wind directions and speeds were obtained from the graphs by nonlinear interpolation for all points along each traverse, and for each hurricane. In addition,  $r$  values were obtained. Finally, all values were tabulated, punched on cards, and used as input for surge calculations.

*b. Storm Center Distance,  $r$ , to Traverse.* Values of distances  $r$  between points along the traverse and the moving storm center were calculated internally by a routine added to the computer program. Given the azimuth  $\alpha$  of the traverse, the initial distance,  $r_{LM}$ , between the storm center and shore-intercept of the traverse, and azimuth  $\beta$  of this line, subsequent values of  $r$  for each point on the traverse were calculated by the program for each time interval. From geometry shown in Figure 32, the value of  $r_n$  was determined each time using the law of cosines. Accordingly:

$$r_n = \sqrt{X_n^2 + r_{LM}^2 - 2X_n r_{LM} \cos \delta} \quad (41)$$

where

$X_n$  = the distance of each point of the traverse from the shore intercept of that traverse,

and

$\delta$  = the computed angle between the traverse and  $r_{LM}$ .

## 2. Hydrographic Data Reduction.

Tide gage hydrographs of historic hurricane surges are important in the calibration and verification of a numerical model. The importance of such data was discussed in Section I. For the purpose of this study, corrections and adjustments were applied to the hydrographic data to account for tide gage locations, tide phases, water level datums of tide gages, and initial rise preceding the arrival of hurricanes.

*a. Tide Gage Locations.* In the discussion of data limitations, it was pointed out that most tide gage stations are situated in protected areas of the coast and do not record storm

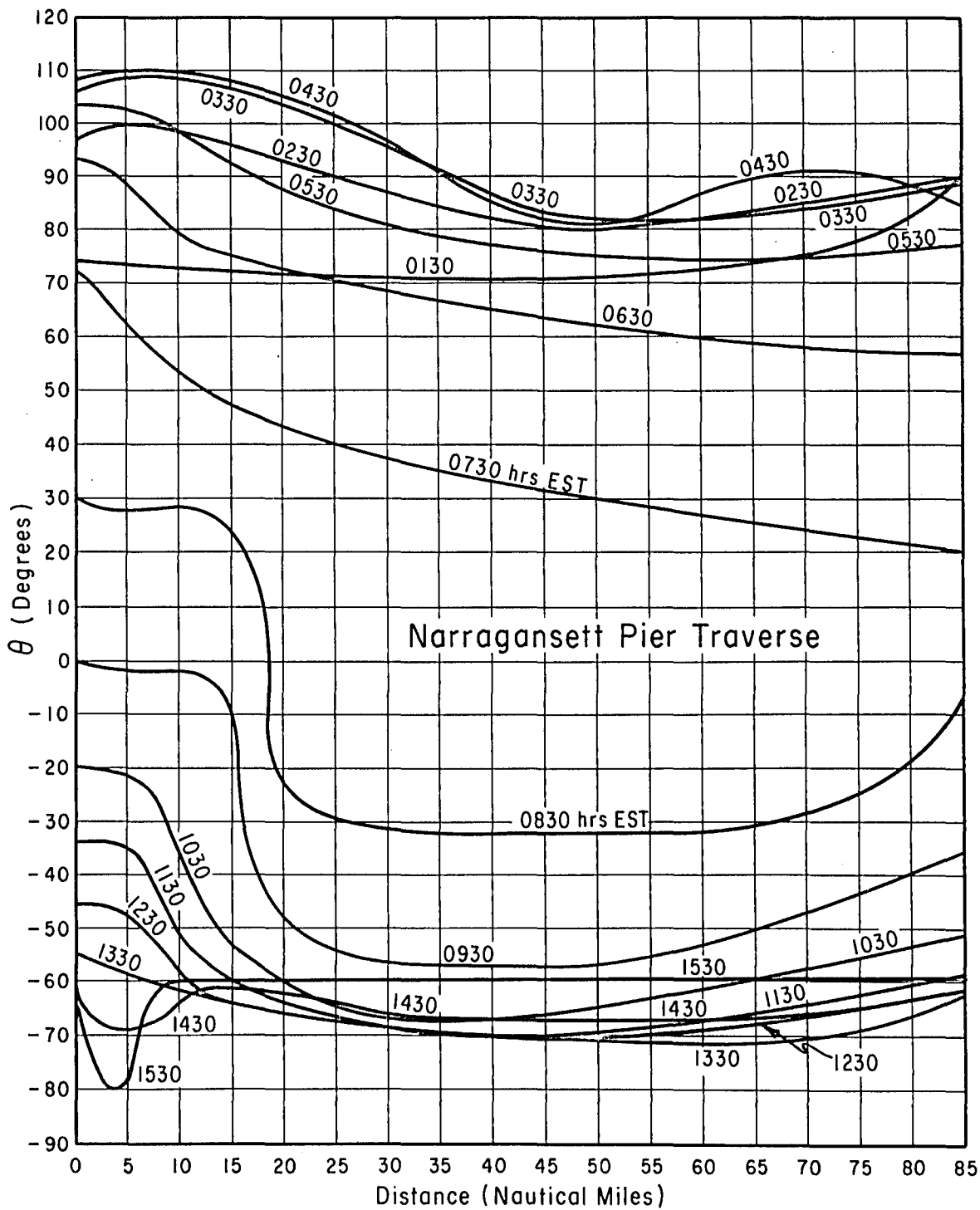


Figure 33. Hurricane Carol. Narragansett Pier Traverse. Theta ( $\theta$ ) versus distance.

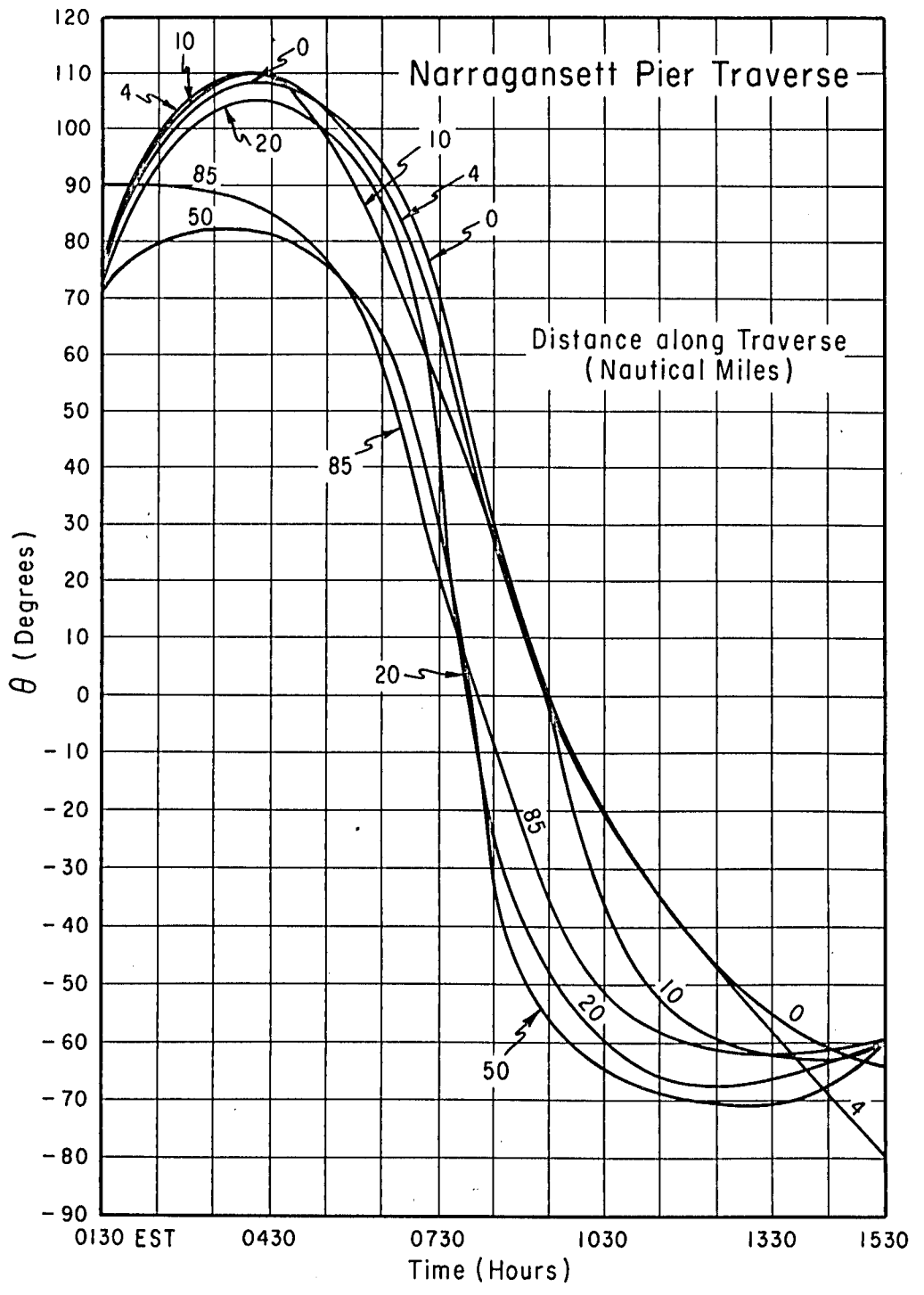


Figure 34. Hurricane Carol. Narragansett Pier Traverse. Theta ( $\theta$ ) versus time.



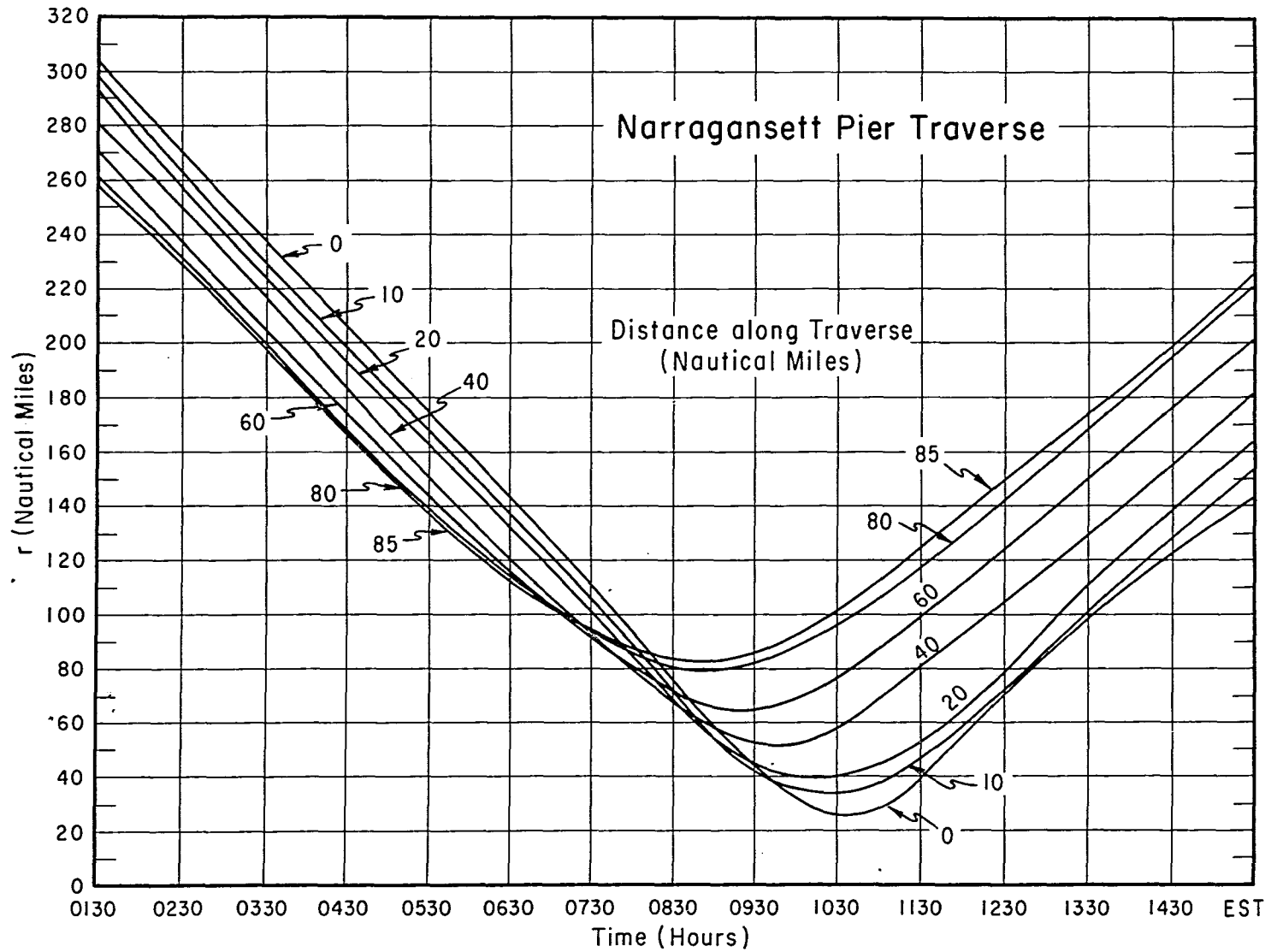


Figure 35. Hurricane Carol. Radius ( $r$ ) versus time.

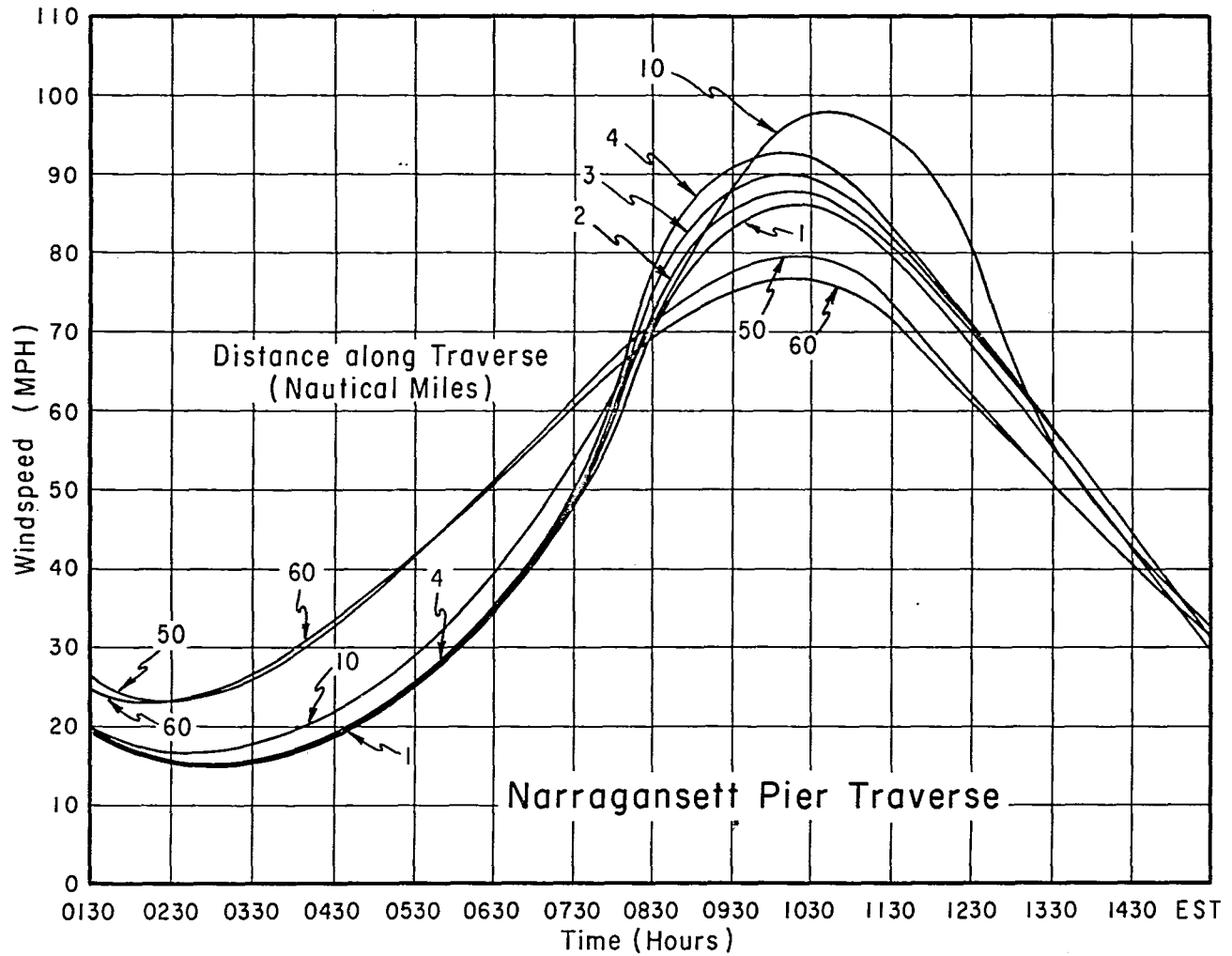


Figure 36. Hurricane Carol. Windspeed versus time.

surge at the open coast. The few tide gage stations located on the open coast are widely separated and often are not near paths of hurricanes. Some tide gage data used in the computation of surge on the open coast can only be estimated from tide gages located a distance away. The following tide gages were used for each hurricane:

(1) Hurricane of 1949. For the Freeport traverse the nearest tidal station which recorded the surge of this hurricane, was at Brazos River Gates. Brazos River Gates is located inland of the Gulf of Mexico at the intersection of the Brazos River and the Intracoastal Waterway (Fig. 37). For the Galveston traverse the nearest tidal station was located at Galveston's Pier 21, adjacent to Galveston Channel. This station provided the best recorded data.

(2) Hurricane Carla. For the Freeport and Galveston traverses, the nearest tidal stations recording surge from Carla were the Brazos River Gates and the Pleasure Pier gages, respectively.

(3) Hurricane Audrey. Eugene Island, Louisiana was selected as the traverse for calculation of surge from Audrey because this island is located on the seaward side of Atchafalaya Bay and represents an open coast situation. The Eugene Island tide gage station operated by the National Ocean Survey only partially recorded this event.

(4) Hurricane Camille. No complete surge hydrograph exists. The Biloxi, Mississippi, traverse used for computation of surge did not have a tide gage. The peak surge for this location (19.5 feet above MSL) is based on a visual observation. The nearest tide gage station from which a partial record of this event could be obtained, was at Pascagoula, Mississippi.

(5) Hurricane Carol. The Narragansett Pier, selected as the traverse in computing surge from Carol, represents an open-coast location which experienced maximum surge. The nearest tide gage station was at Coasters Harbor Island, Newport, Rhode Island (Fig. 38). Corrections were applied to the records from this gage, accounting for phase differences in the astronomical tide values used in the computation, e.g., the maximum water level due to Carol at the Newport gage (11.61 feet above MLW) occurred at 1100 hours EST. The high tide of 4 feet above MLW was predicted to occur at 0917 hours EST, for that location. Therefore, at the time of maximum surge at Newport, the predicted tide elevation was 3.2 feet above MLW, the difference (observed minus predicted) in tide being 7.9 feet. The tide tables were consulted for Narragansett Pier; however, no corrections were applied for tidal time and height differences between this station and Newport as these differences were relatively insignificant.

When tide gage records were not available, peak storm surge elevations on the open coast were obtained from visual observations of debris marks, which generally include the effects of wave-induced setup and runup. However, it was not possible to determine accurately how much of the water level elevation was due to surge and to wave action. The observed levels



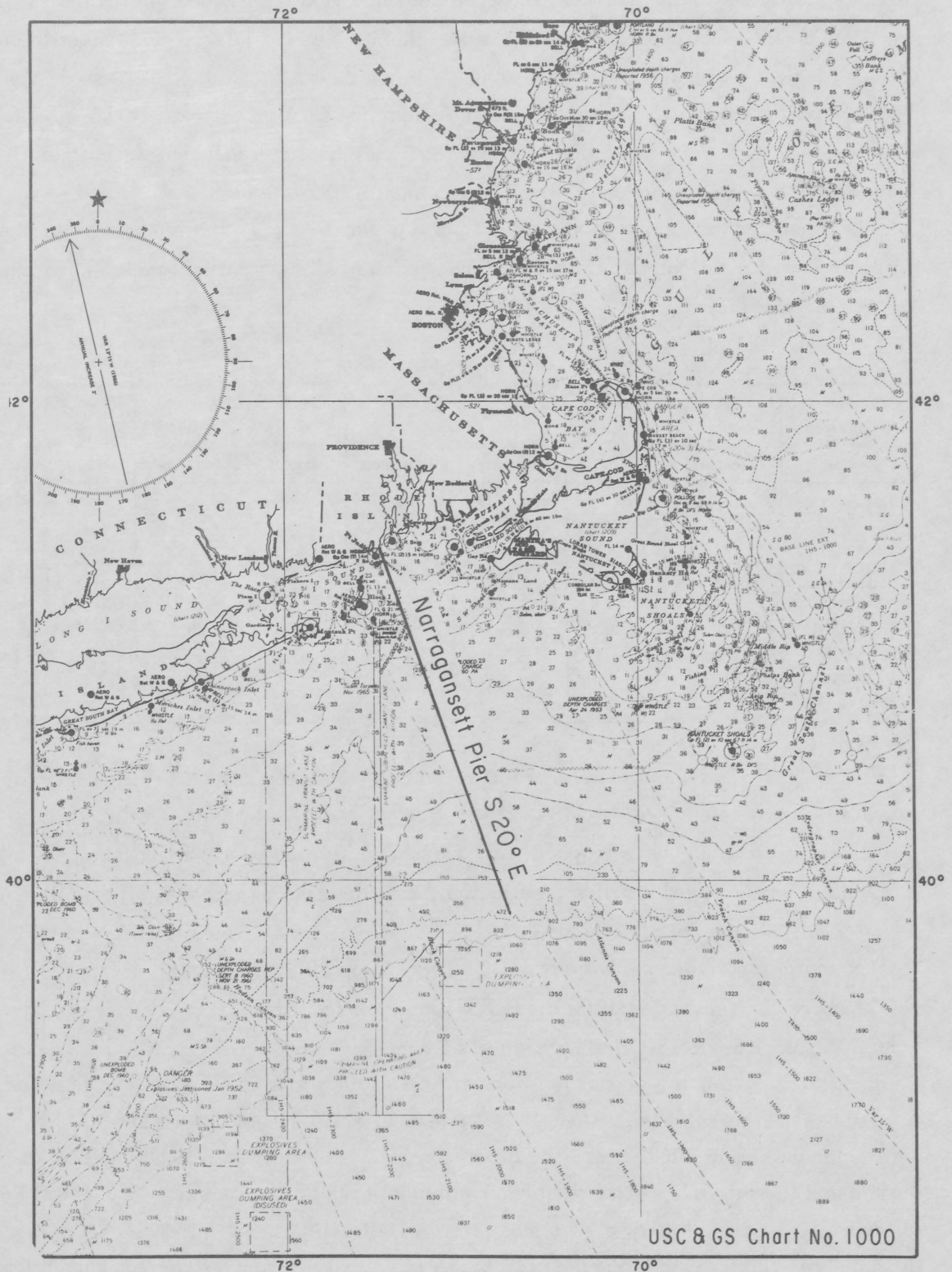


Figure 38. The Narragansett Pier traverse.

in these cases may be higher than those of the surge alone, e.g., for Camille, the surge elevation of 19.5 feet above MSL used for the calibration was based on a visual observation of high water marks at Biloxi, Mississippi. Without a better record, the observed surge hydrograph for Carol at Newport, used in the calibration for the Narragansett Pier traverse, was reconstructed from visual observations which may have also included wave runup. In both instances there was no logical way of correcting for the wave runup.

The predicted astronomical tide elevations for the tide gage station nearest to the traverses used for the computation at the time maximum surges occurred for each hurricane is given in Table 3.

Table 3. Hydrographic Data.

Parameter (feet MLW)	Hurricane of 1949 Texas		Carla Texas		Camille Mississippi	Audrey Louisiana	Carol Rhode Island
	Freeport	Galveston	Freeport	Galveston	Biloxi	Eugene Island	Narragansett Pier
Tide Station	Brazos River	Pier 21 Galveston Channel	Brazos River	Pleasure Pier	Pascagoula		Coaster Island
Initial rise	2.00	2.00	2.50	1.90	1.20	1.00	1.20
Astronomical tide at time of maximum observed surge	1.42	1.02	1.19	1.10	1.60	0.81	3.2

*b. Sea Level Datum Corrections.* Some recorded or observed surge data associated with hurricanes were referenced to MSL. Other tide gage data referred to MLW, or SLD. Therefore, corrections were applied for differences in the water level datum. Also, corrections were applied for tide gage datums, e.g., the gage datum at Newport, Rhode Island (Constellation Dock at Coasters Harbor Island) was 1.4 feet below MLW; therefore, the maximum gage value was corrected to 11.1 feet MLW. A correction was also applied for the phase of the tide and at 1100 hours EST the predicted astronomical tide should have been 3.2 feet. The surge for that location was 7.9 feet.

Predicted tides for the various locations at or near the traverses used in the calibration were obtained from tide tables which are referenced to MLW. The tide tables give maximum and minimum heights of tides and the times of respective occurrences. Height values were plotted and smooth curves fitted to obtain continuous values of astronomical tides (Figs. 39 through 71). These tide curves represent projected astronomical tides used for estimating the initial rise and for computing the surge hydrographs. All reported water level values refer to MLW datum.

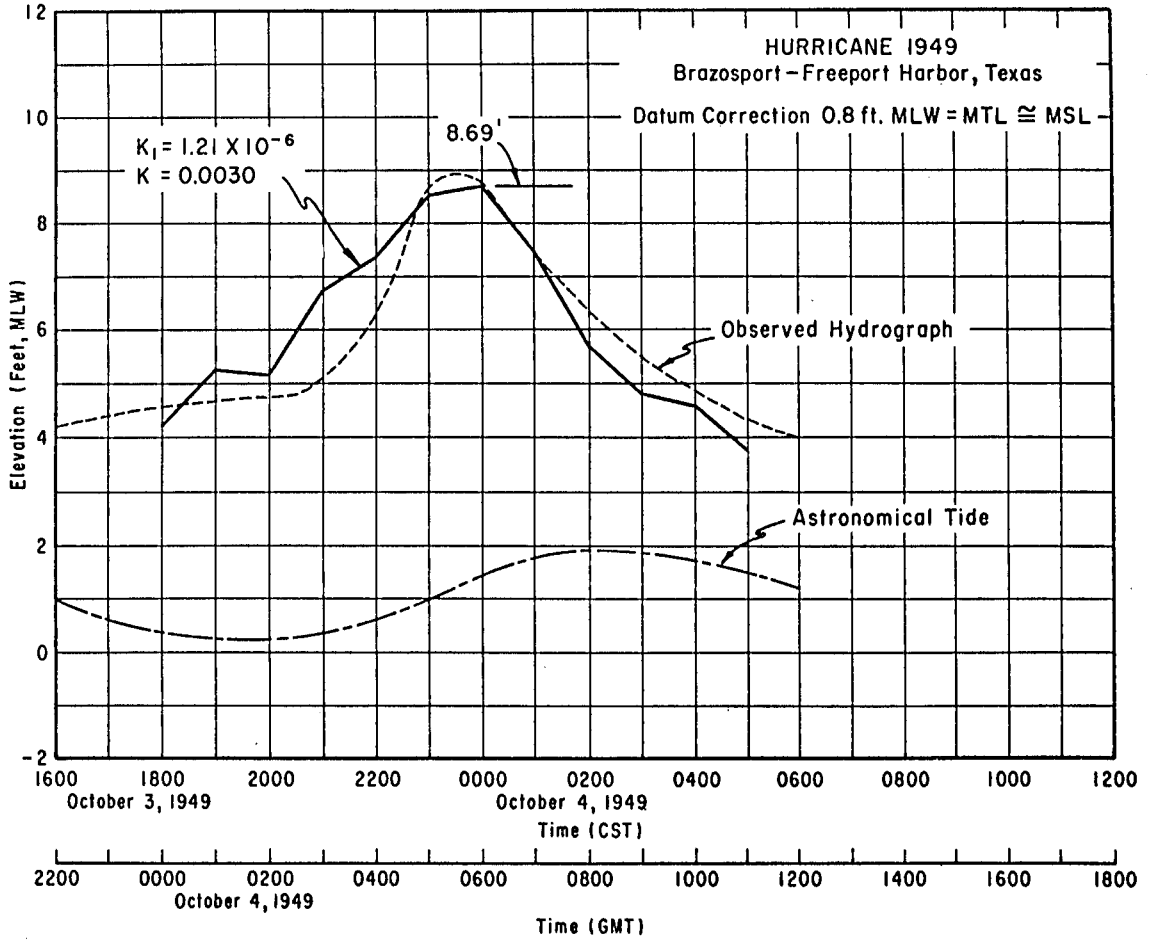


Figure 39. Observed and computed surge hydrographs for the Hurricane of 1949 at Freeport, Texas,  $K_1 = 1.21 \times 10^{-6}$ ,  $K_2 = 2.75 \times 10^{-6}$ ,  $K = 0.0030$ .

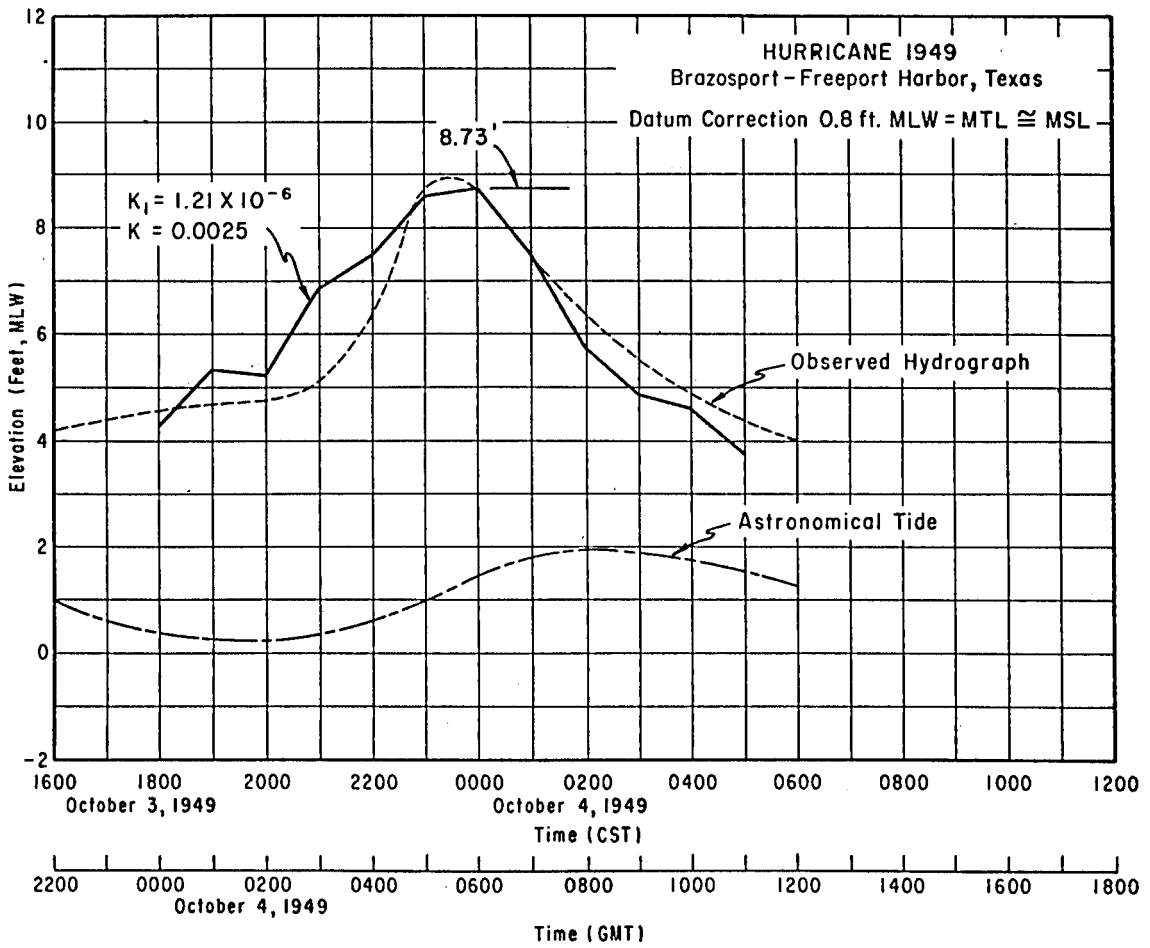


Figure 40. Observed and computed surge hydrographs for the Hurricane of 1949 at Freeport, Texas,  $K_1 = 1.21 \times 10^{-6}$ ,  $K_2 = 2.75 \times 10^{-6}$ ,  $K = 0.0025$ .



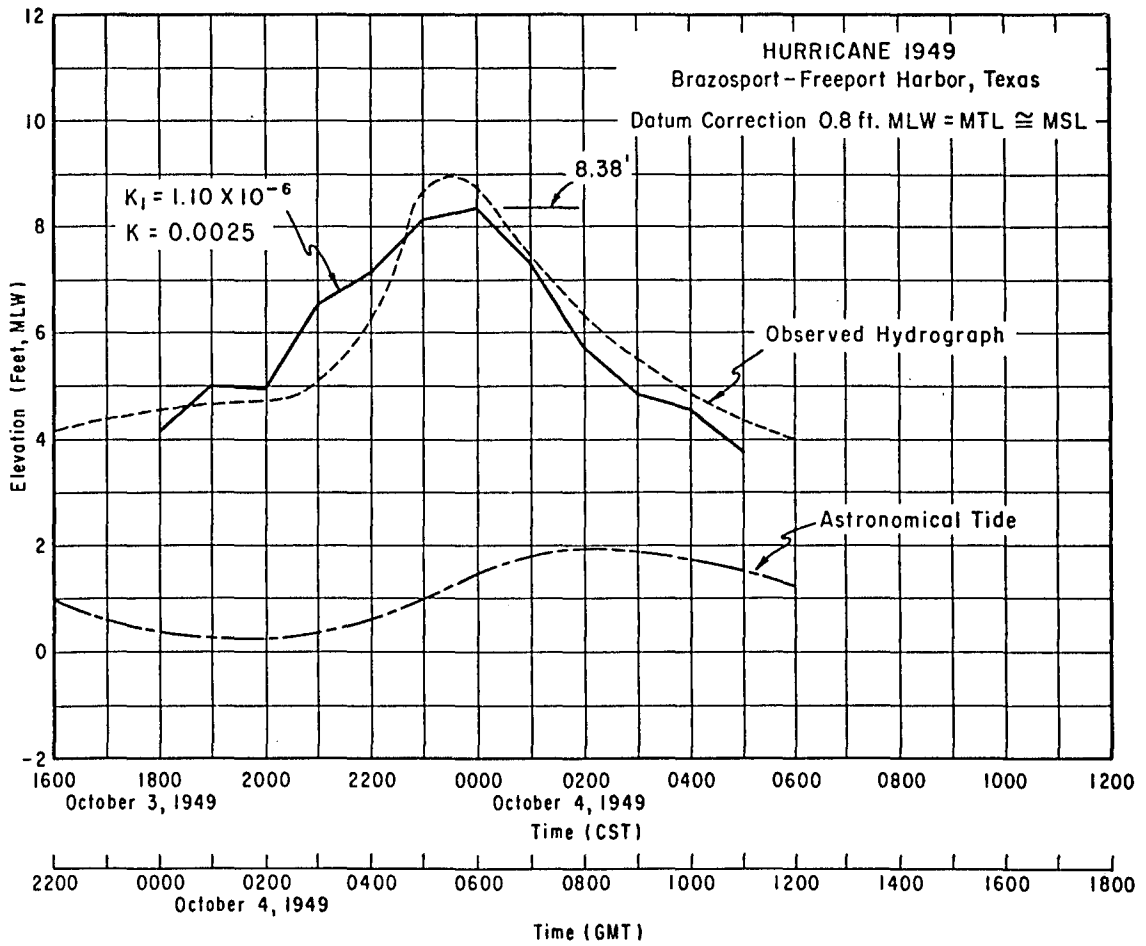


Figure 41. Observed and computed surge hydrographs for the Hurricane of 1949 at Freeport, Texas,  $K_1 = 1.10 \times 10^{-6}$ ,  $K_2 = 2.50 \times 10^{-6}$ ,  $K = 0.0025$ .

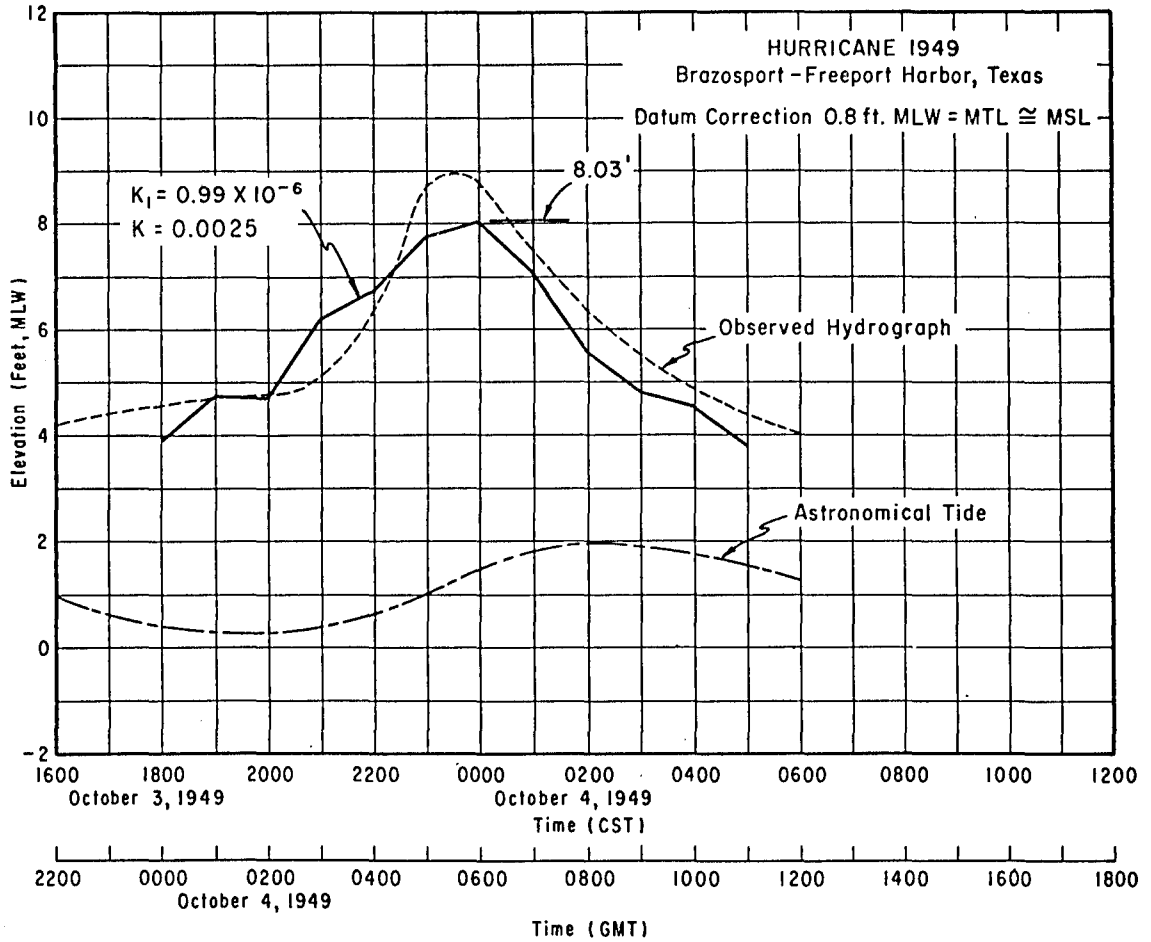


Figure 42. Observed and computed surge hydrographs for the Hurricane of 1949 at Freeport, Texas,  $K_1 = 0.99 \times 10^{-6}$ ,  $K_2 = 2.25 \times 10^{-6}$ ,  $K = 0.0025$ .

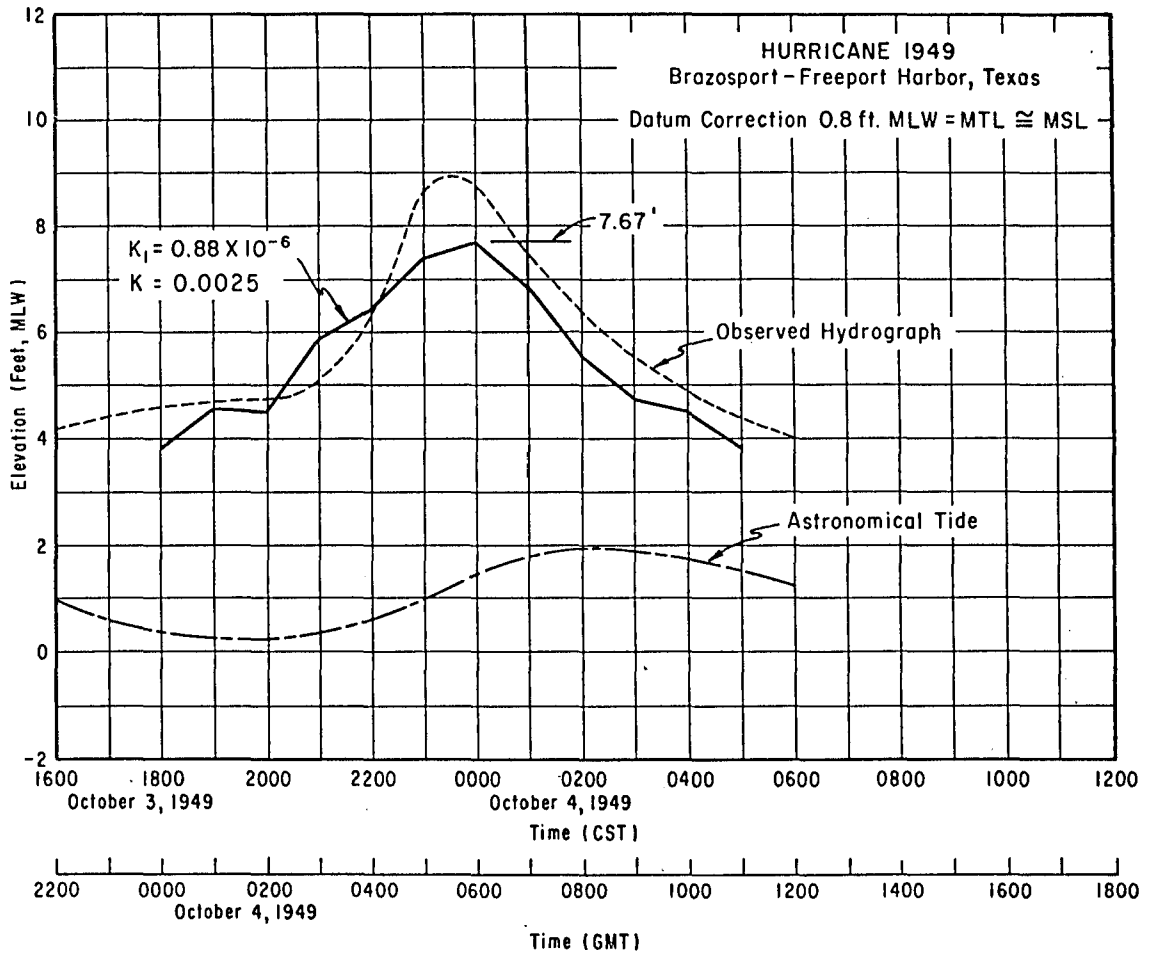


Figure 43. Observed and computed surge hydrographs for the Hurricane of 1949 at Freeport, Texas,  $K_1 = 0.88 \times 10^{-6}$ ,  $K_2 = 2.00 \times 10^{-6}$ ,  $K = 0.0025$ .

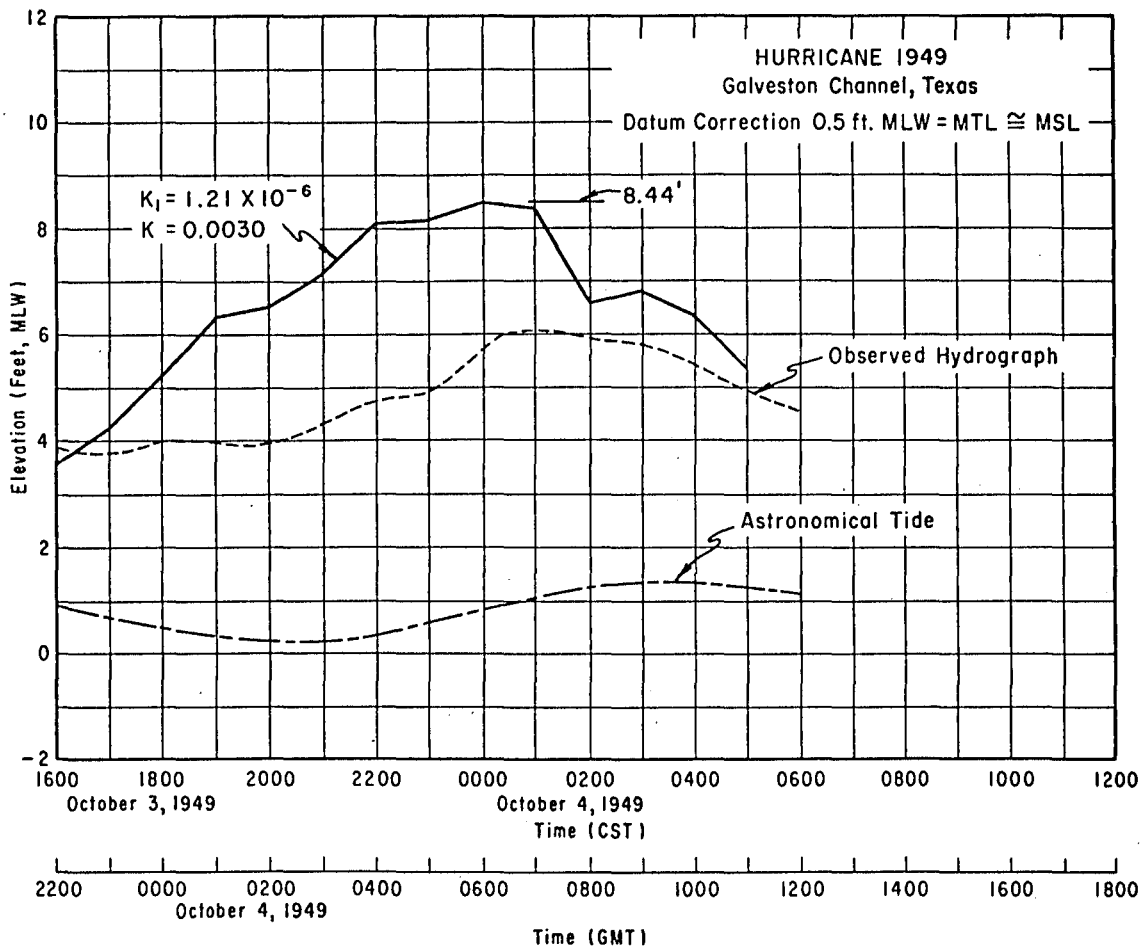


Figure 44. Observed and computed surge hydrographs for the Hurricane of 1949 at Galveston, Texas,  $K_1 = 1.21 \times 10^{-6}$ ,  $K_2 = 2.75 \times 10^{-6}$ ,  $K = 0.0030$ .

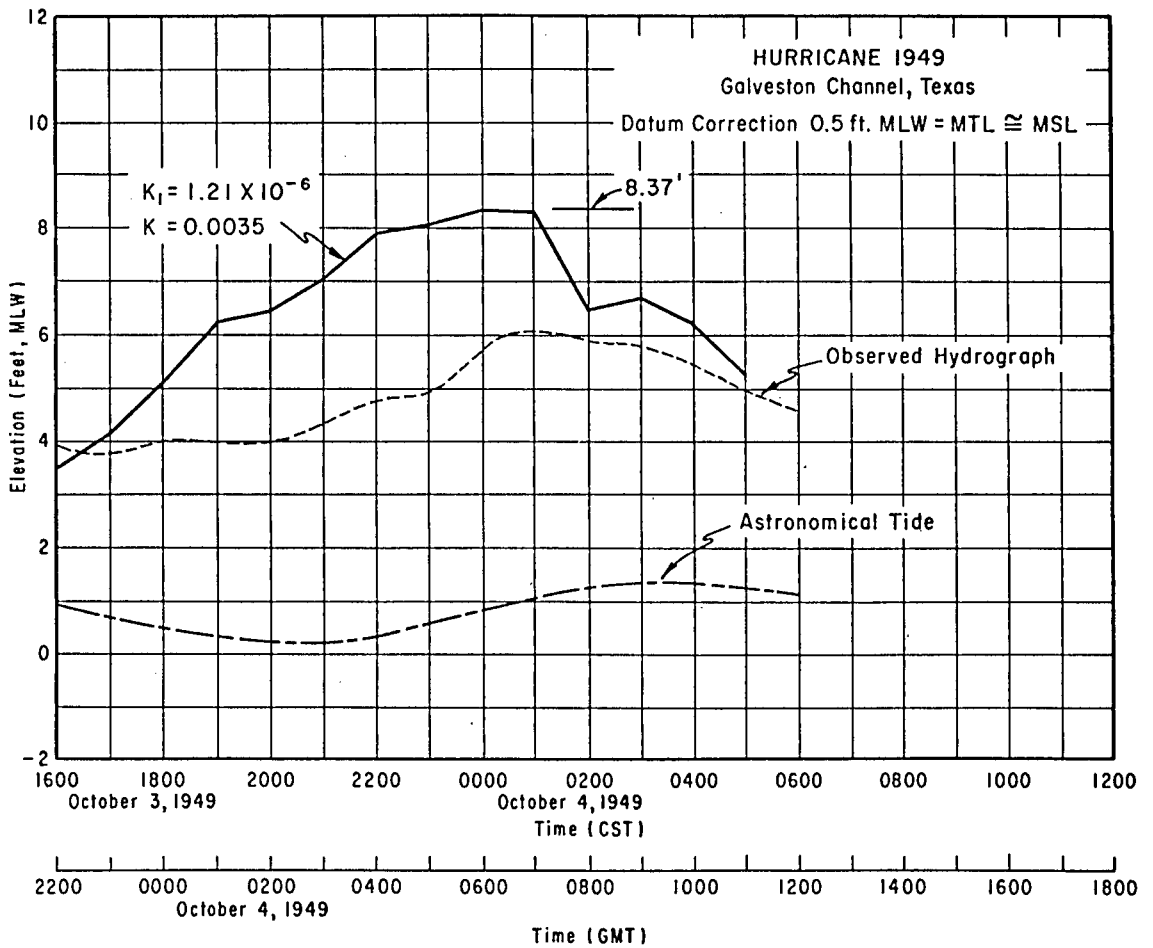


Figure 45. Observed and computed surge hydrographs for the Hurricane of 1949 at Galveston, Texas,  $K_1 = 1.21 \times 10^{-6}$ ,  $K_2 = 2.75 \times 10^{-6}$ ,  $K = 0.0035$ .

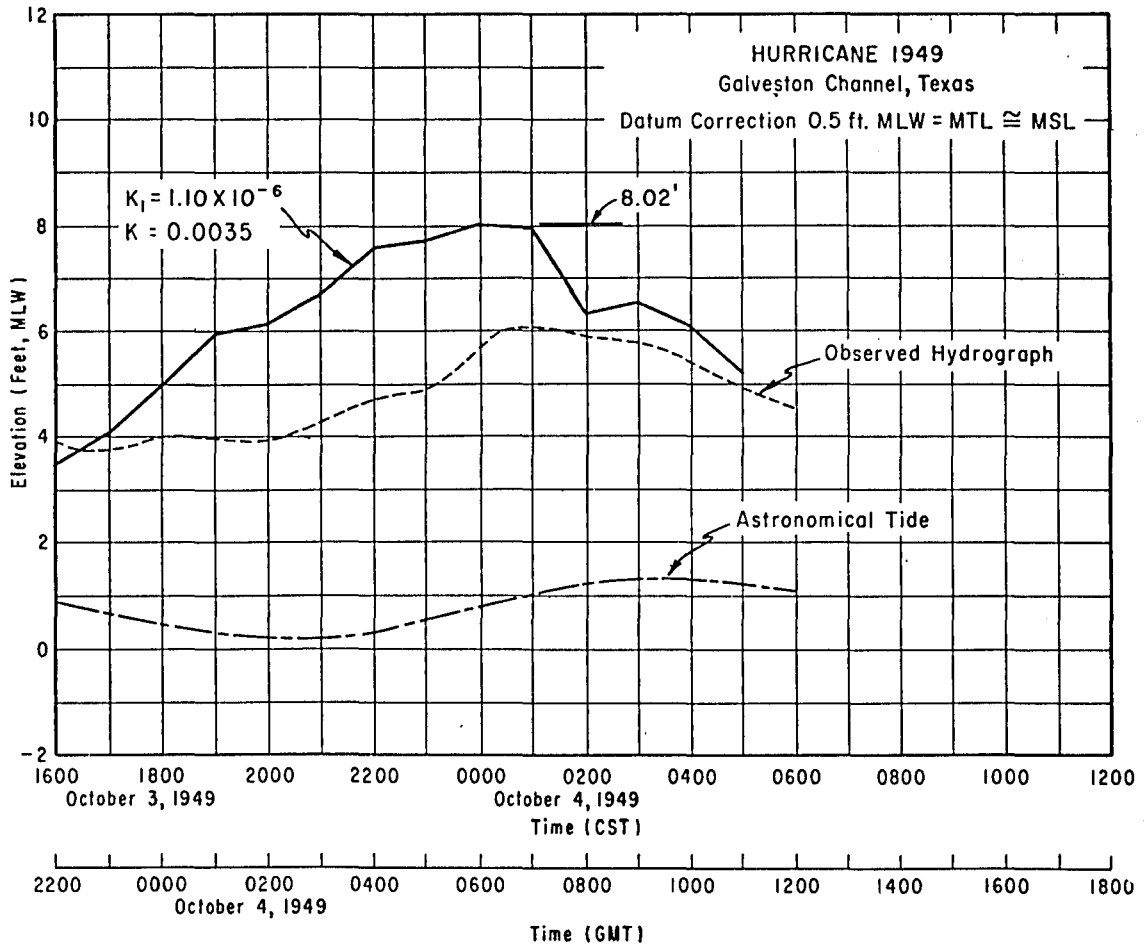


Figure 46. Observed and computed surge hydrographs for the Hurricane of 1949 at Galveston, Texas,  $K_1 = 1.10 \times 10^{-6}$ ,  $K_2 = 2.50 \times 10^{-6}$ ,  $K = 0.0035$ .

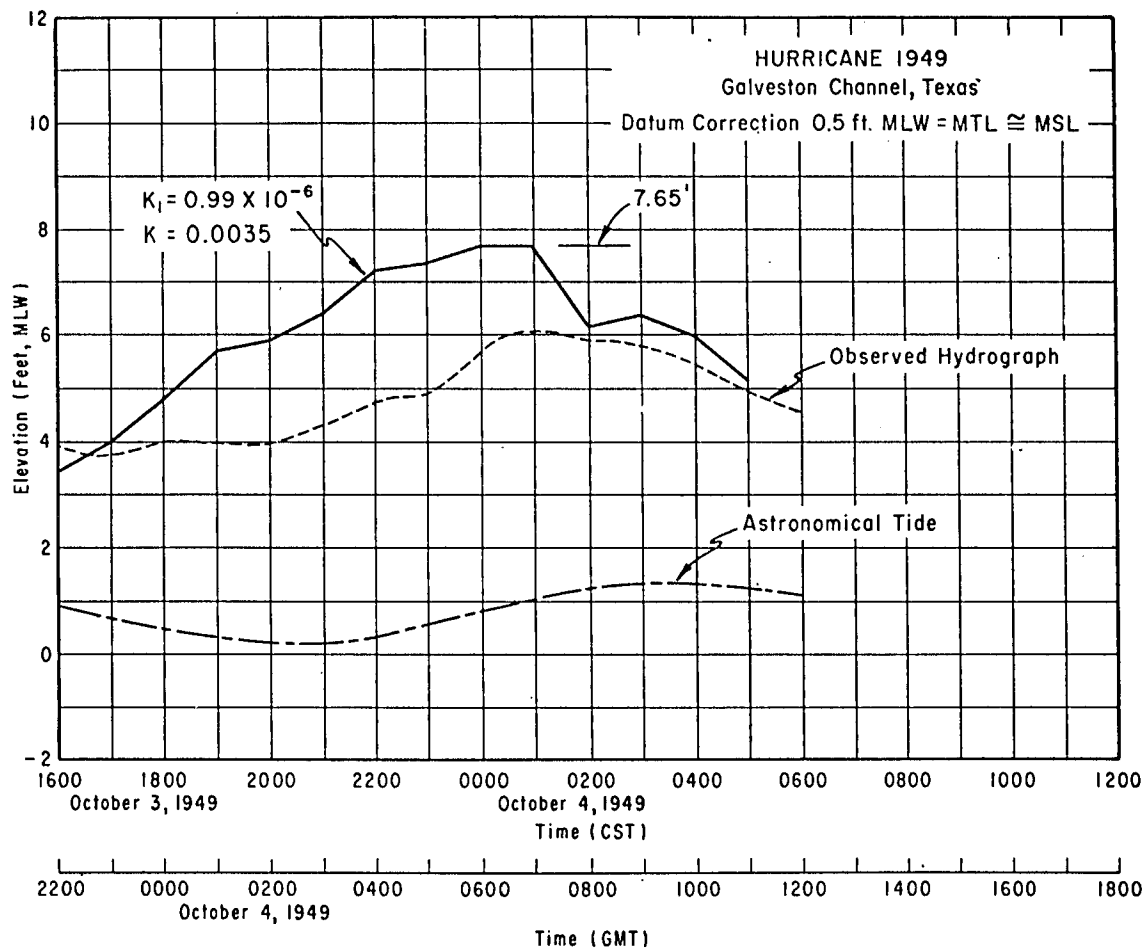


Figure 47. Observed and computed surge hydrographs for the Hurricane of 1949 at Galveston, Texas,  $K_1 = 0.99 \times 10^{-6}$ ,  $K_2 = 2.25 \times 10^{-6}$ ,  $K = 0.0035$ .

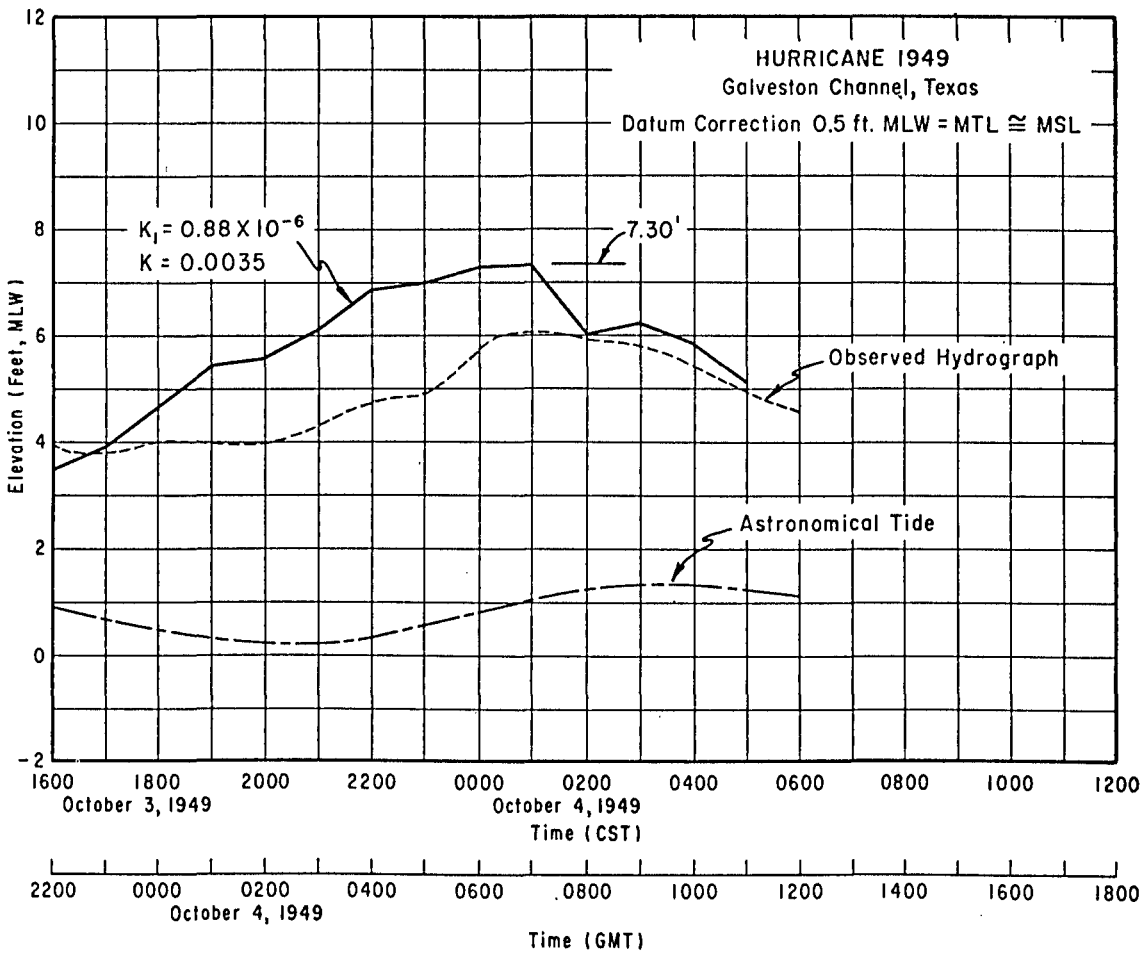


Figure 48. Observed and computed surge hydrographs for the Hurricane of 1949 at Galveston, Texas,  $K_1 = 0.88 \times 10^{-6}$ ,  $K_2 = 2.00 \times 10^{-6}$ ,  $K = 0.0035$ .



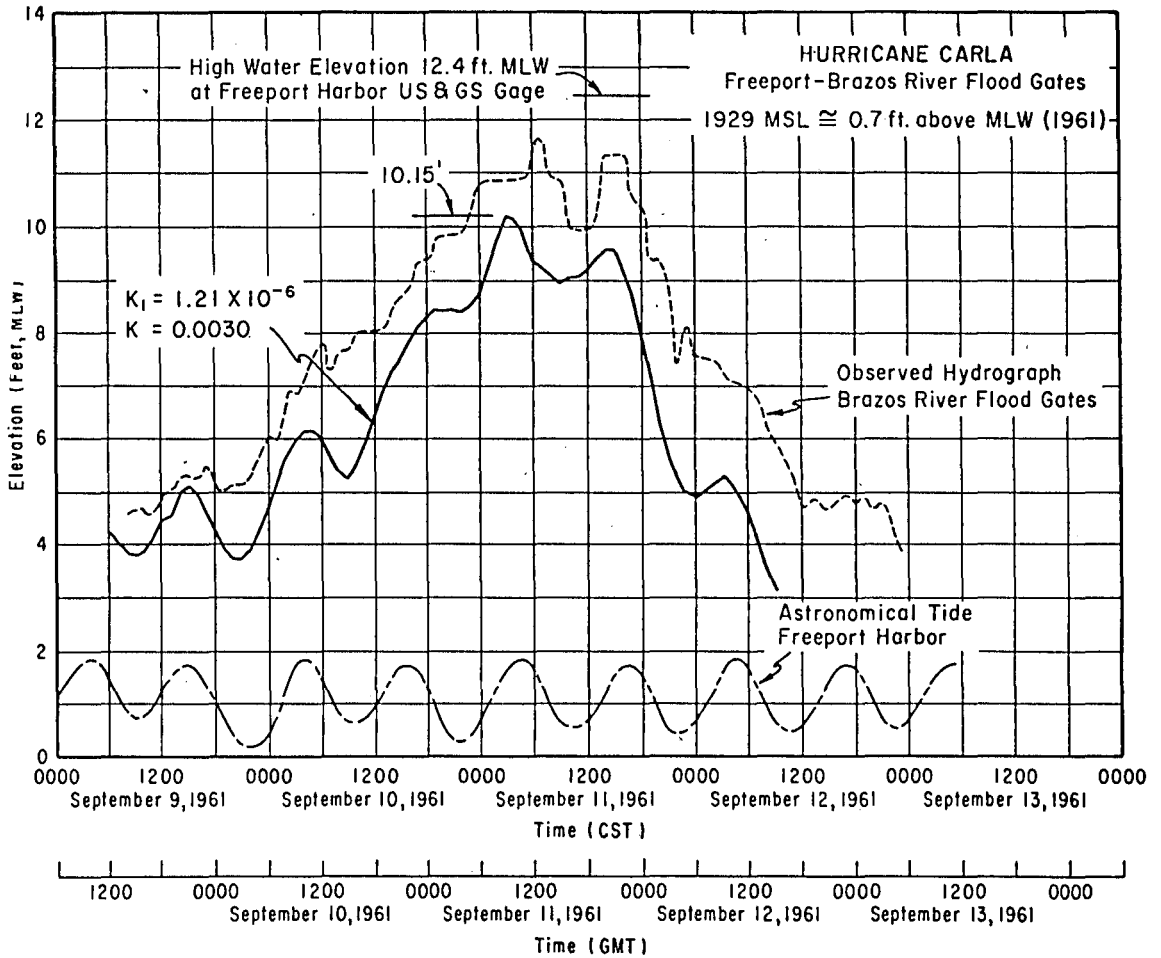


Figure 49. Observed and computed surge hydrographs for Hurricane Carla at Freeport, Texas,  $K_1 = 1.21 \times 10^{-6}$ ,  $K_2 = 2.75 \times 10^{-6}$ ,  $K = 0.0030$ .

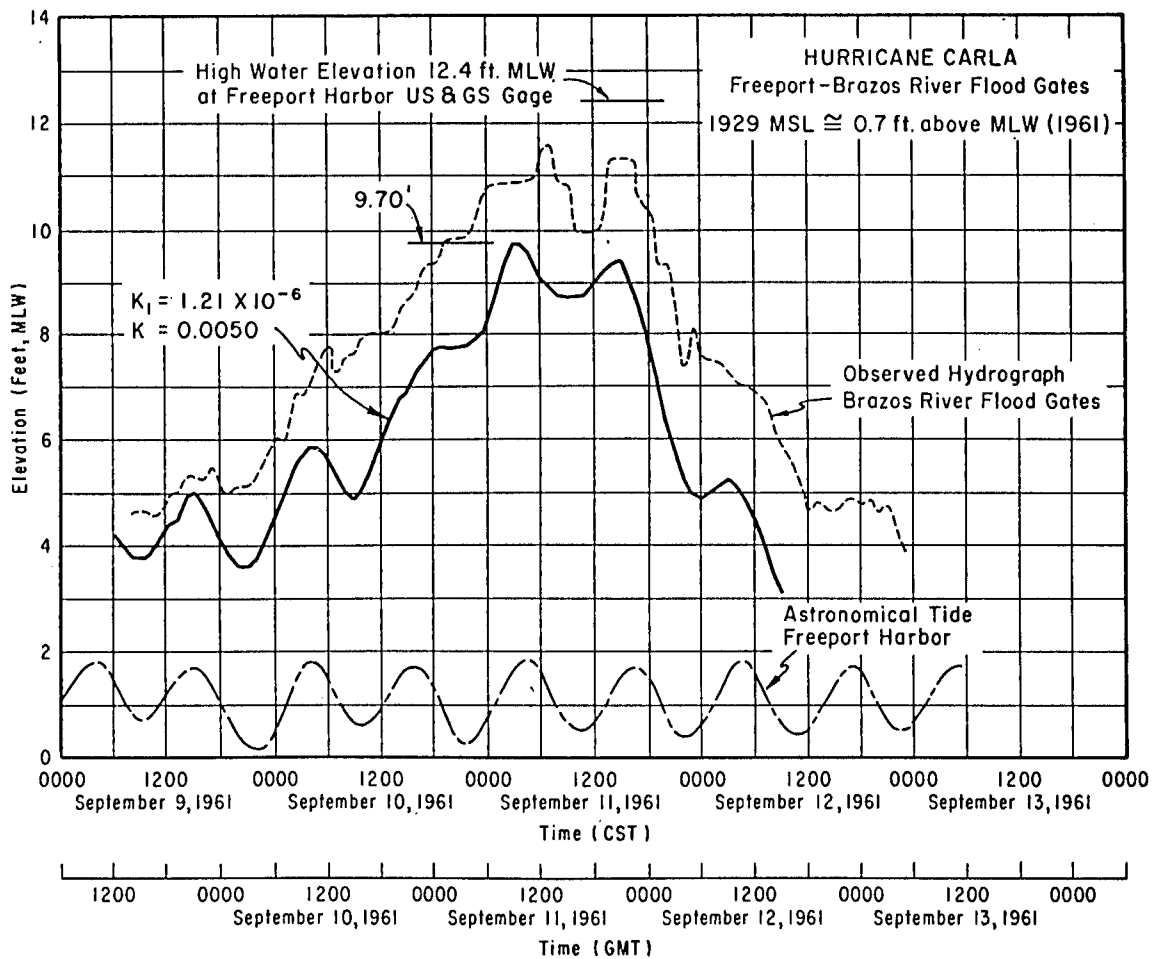


Figure 50. Observed and computed surge hydrographs for Hurricane Carla at Freeport, Texas,  $K_1 = 1.21 \times 10^{-6}$ ,  $K_2 = 2.75 \times 10^{-6}$ ,  $K = 0.0050$ .

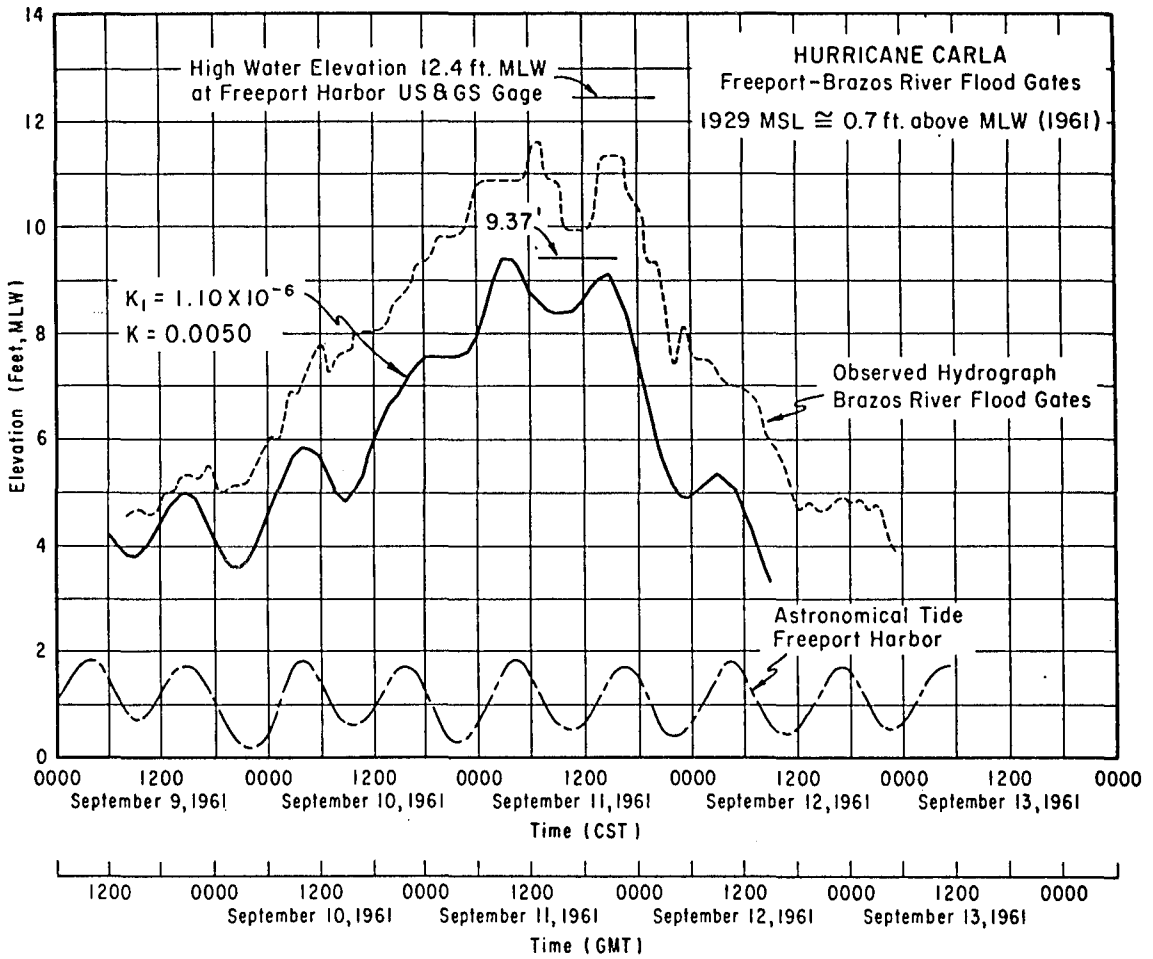


Figure 51. Observed and computed surge hydrographs for Hurricane Carla at Freeport, Texas,  $K_1 = 1.10 \times 10^{-6}$ ,  $K_2 = 2.50 \times 10^{-6}$ ,  $K = 0.0050$ .

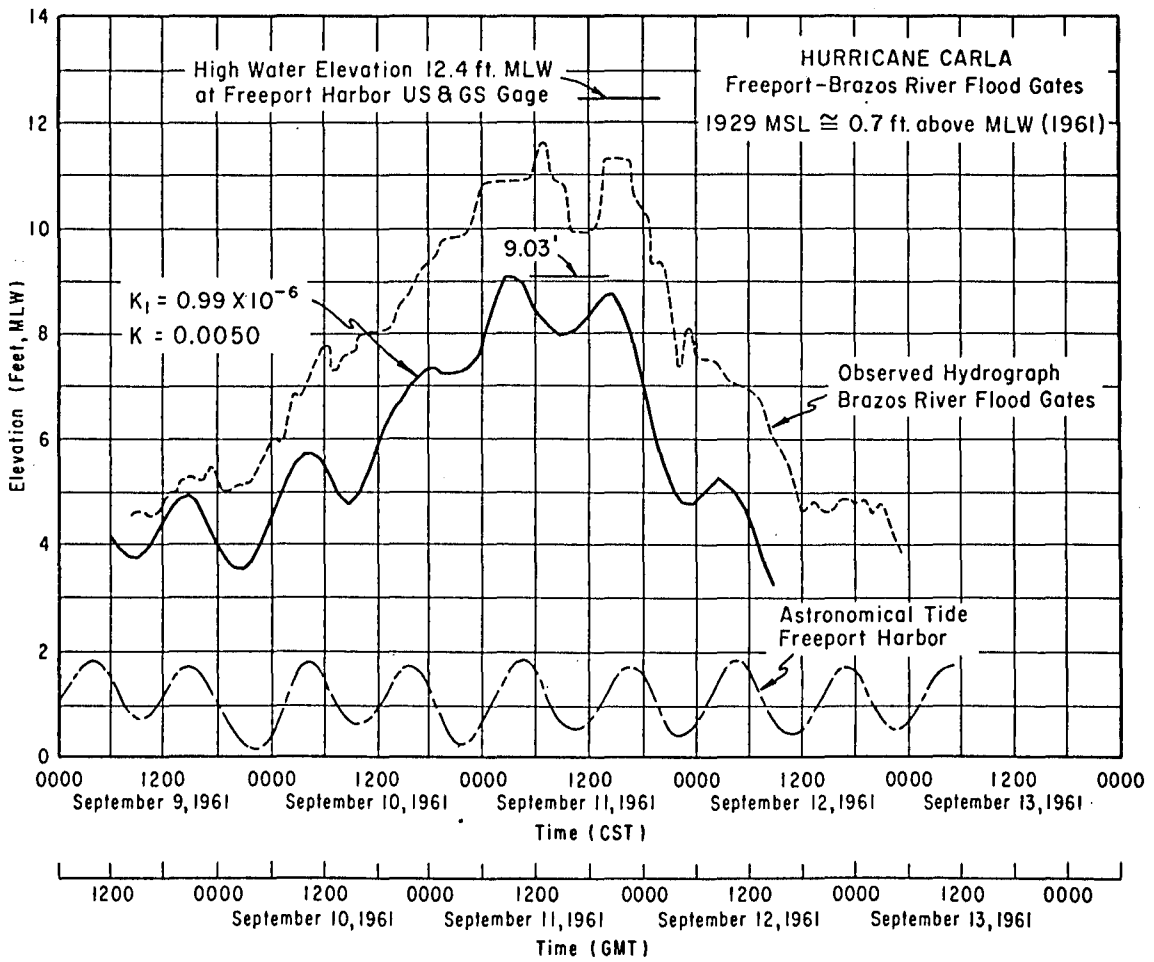


Figure 52. Observed and computed surge hydrographs for Hurricane Carla at Freeport, Texas,  $K_1 = 0.99 \times 10^{-6}$ ,  $K_2 = 2.25 \times 10^{-6}$ ,  $K = 0.0050$ .

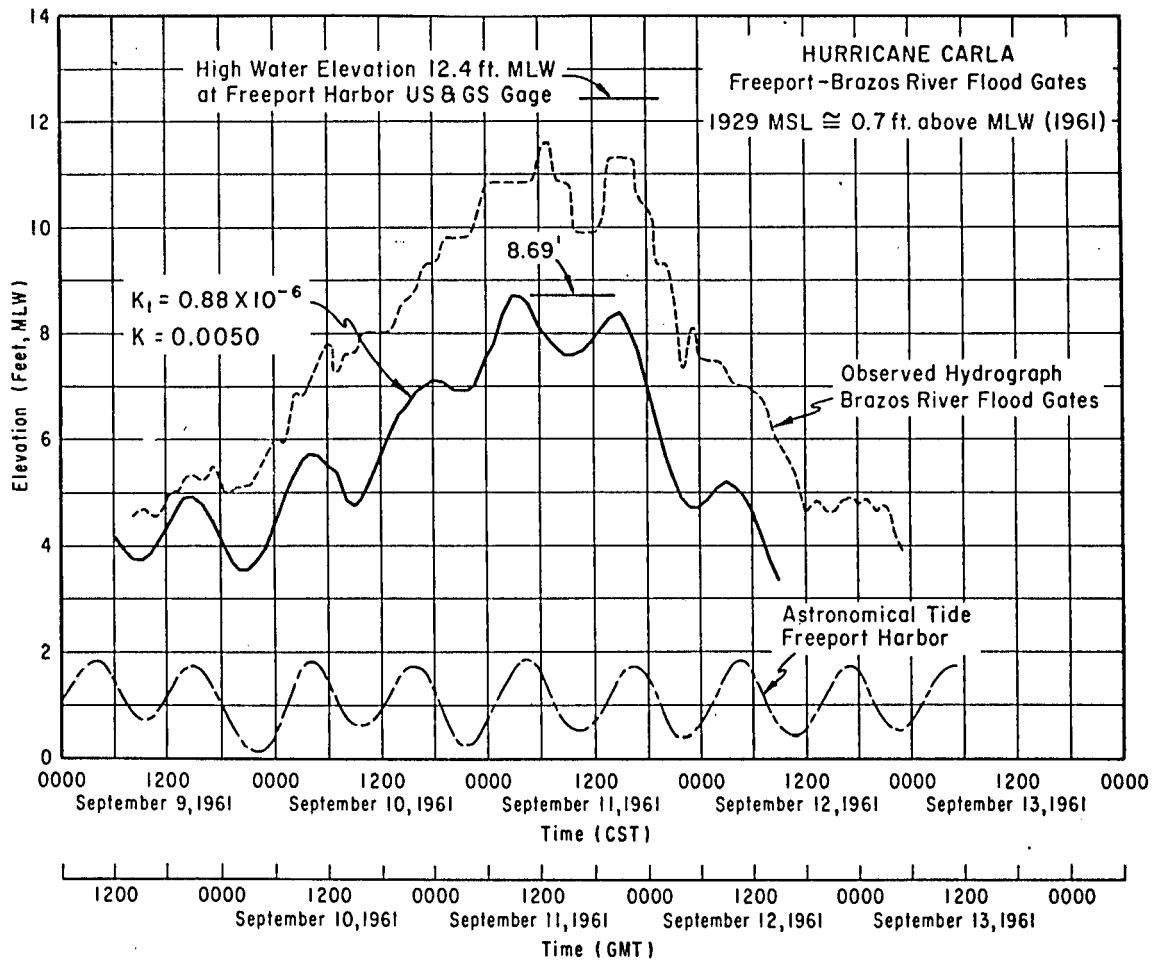


Figure 53. Observed and computed surge hydrographs for Hurricane Carla at Freeport, Texas,  $K_1 = 0.88 \times 10^{-6}$ ,  $K_2 = 2.00 \times 10^{-6}$ ,  $K = 0.0050$ .

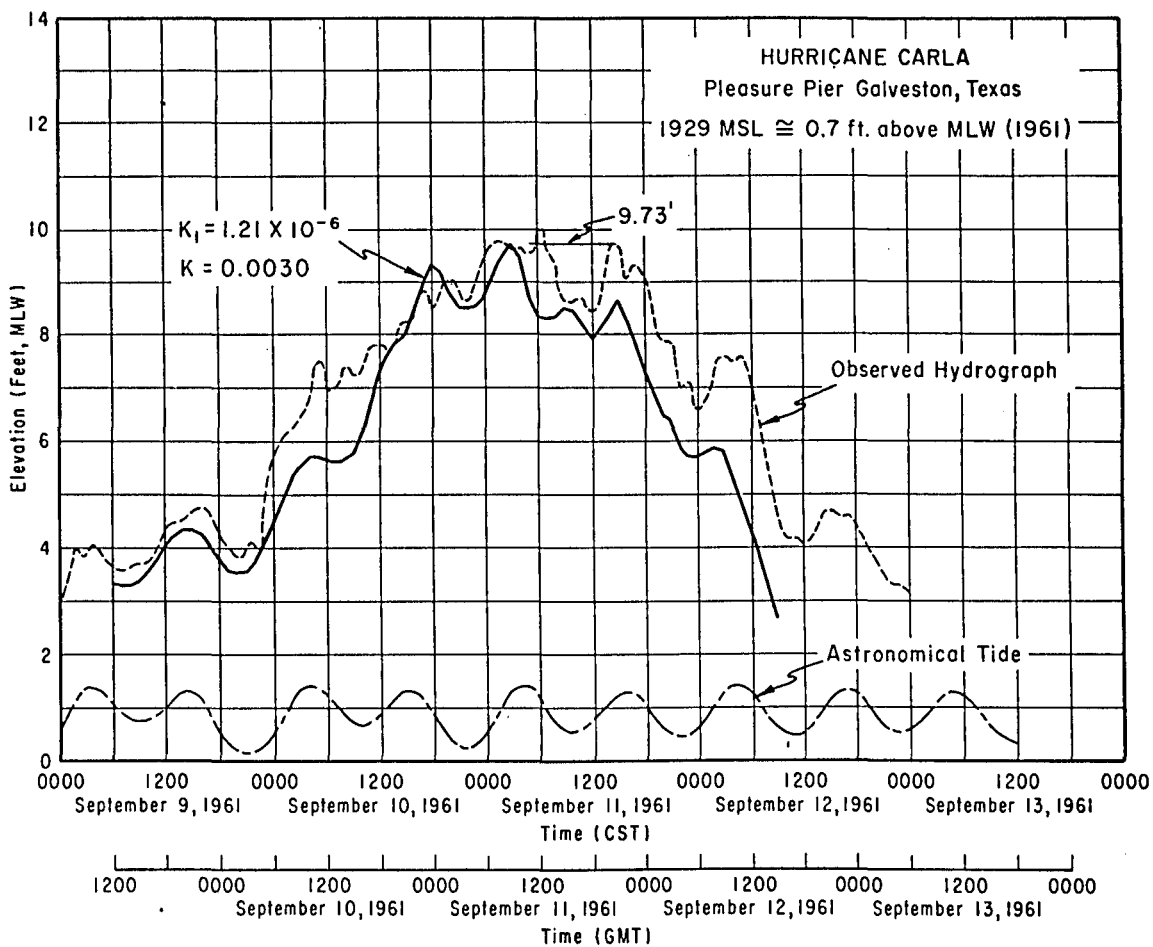
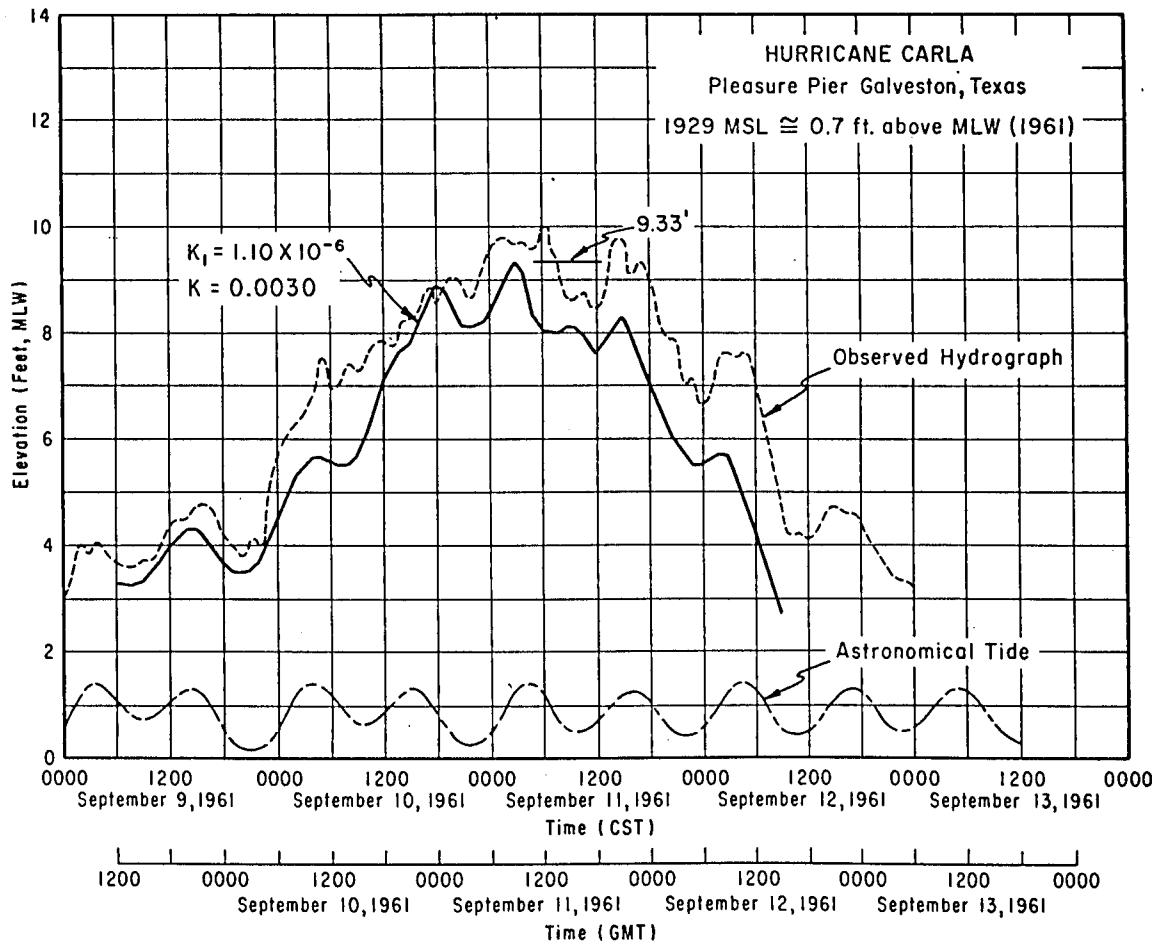


Figure 54. Observed and computed surge hydrographs for Hurricane Carla at Galveston, Texas,  $K_1 = 1.21 \times 10^{-6}$ ,  $K_2 = 2.75 \times 10^{-6}$ ,  $K = 0.0030$ .



**Figure 55.** Observed and computed surge hydrographs for Hurricane Carla at Galveston, Texas,  $K_1 = 1.10 \times 10^{-6}$ ,  $K_2 = 2.50 \times 10^{-6}$ ,  $K = 0.0030$ .

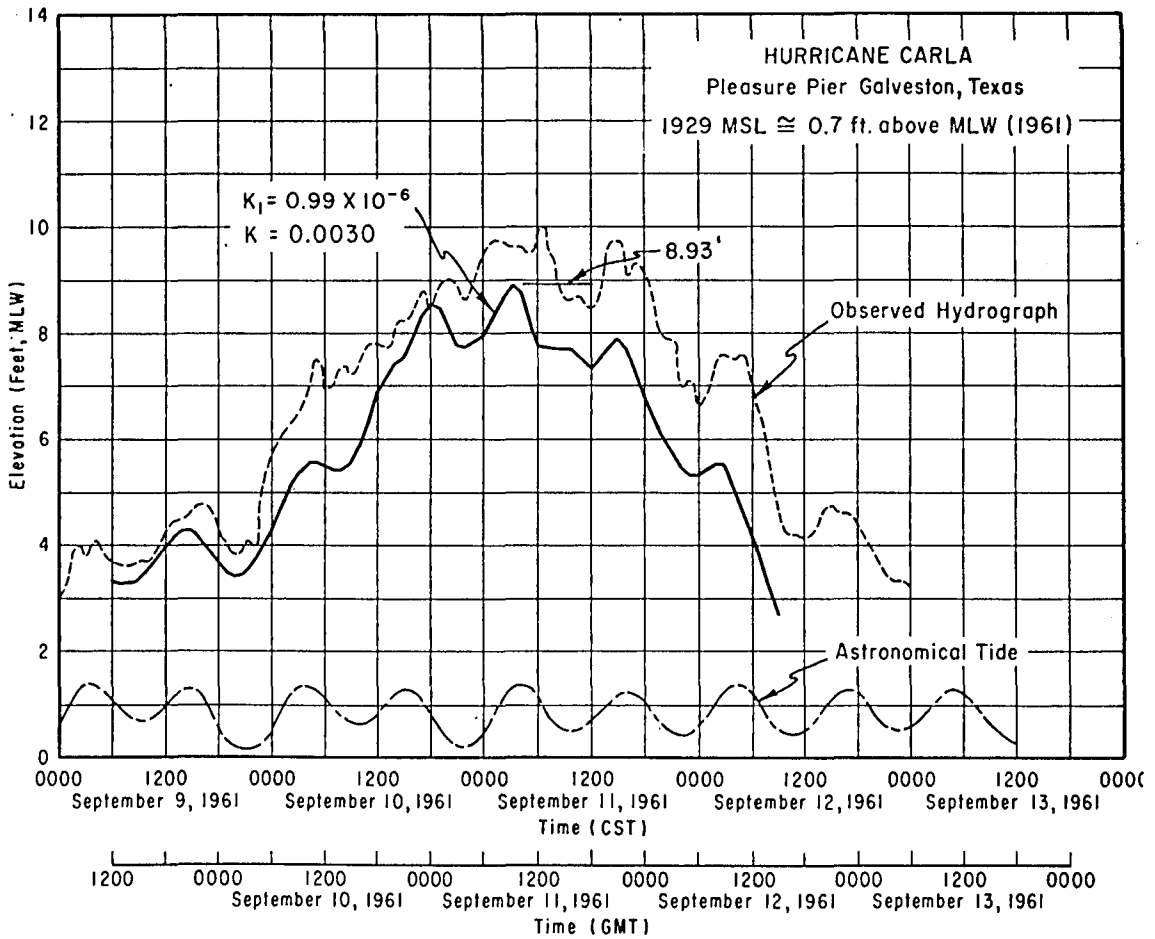
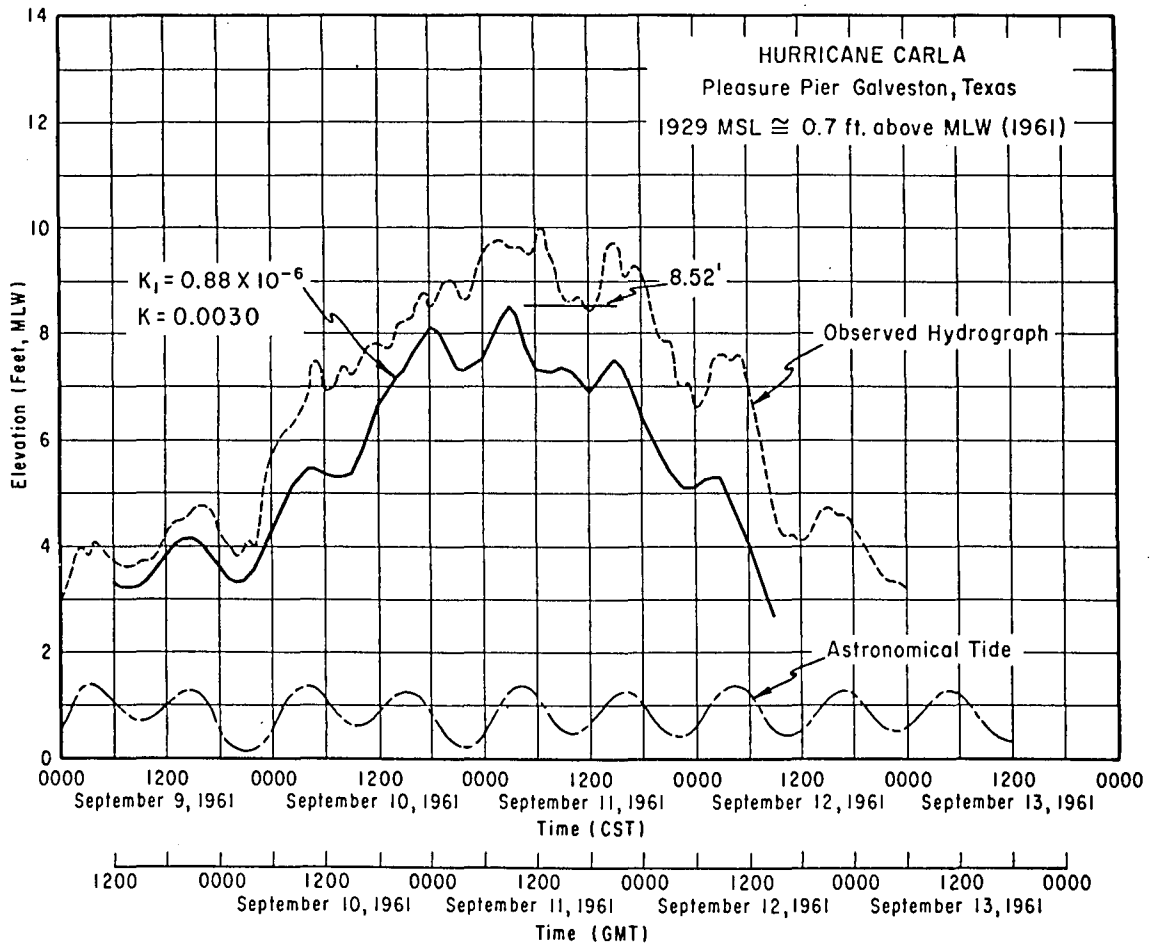


Figure 56. Observed and computed surge hydrographs for Hurricane Carla at Galveston, Texas,  $K_1 = 0.99 \times 10^{-6}$ ,  $K_2 = 2.25 \times 10^{-6}$ ,  $K = 0.0030$ .





**Figure 57.** Observed and computed surge hydrographs for Hurricane Carla at Galveston, Texas,  $K_1 = 0.88 \times 10^{-6}$ ,  $K_2 = 2.00 \times 10^{-6}$ ,  $K = 0.0030$ .

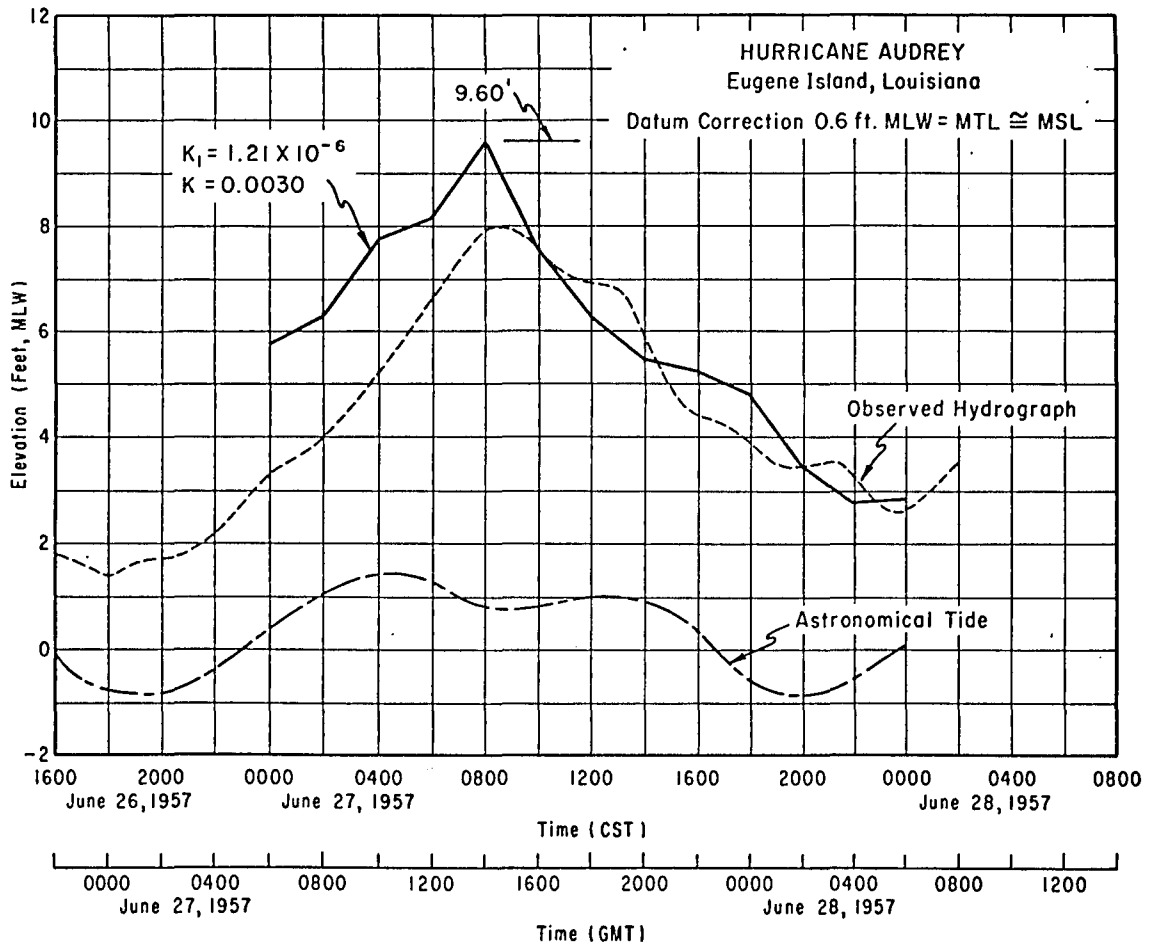


Figure 58. Observed and computed surge hydrographs for Hurricane Audrey at Eugene Island, Louisiana,  $K_1 = 1.21 \times 10^{-6}$ ,  $K_2 = 2.75 \times 10^{-6}$ ,  $K = 0.0030$ .

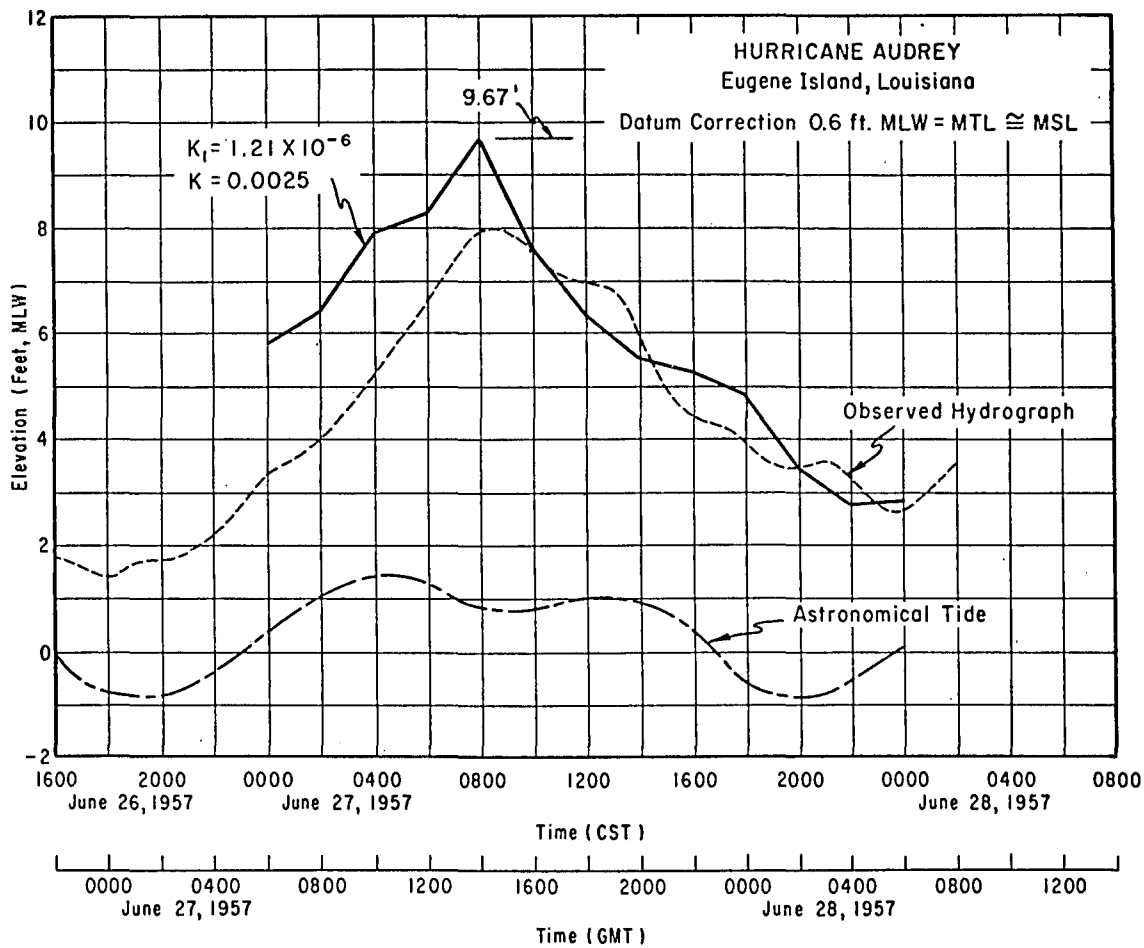
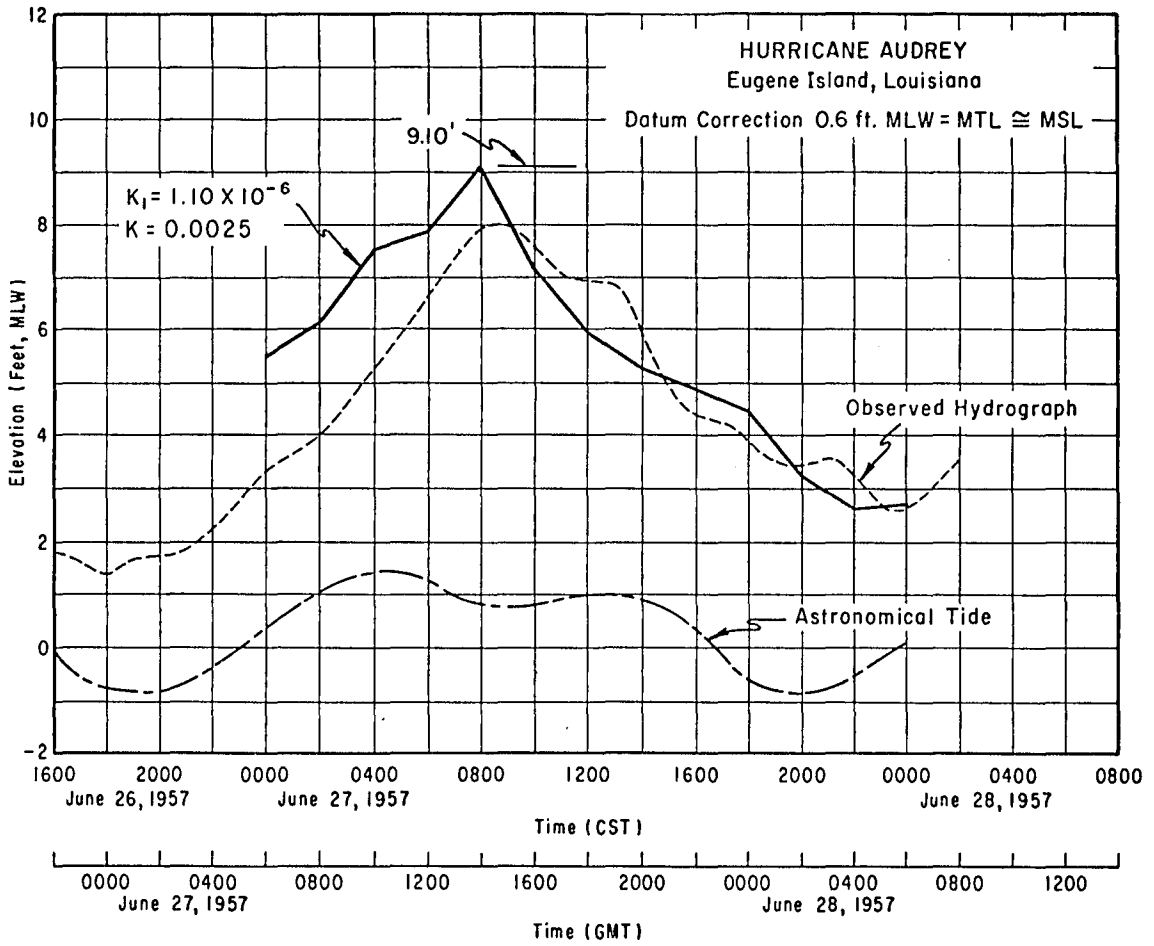
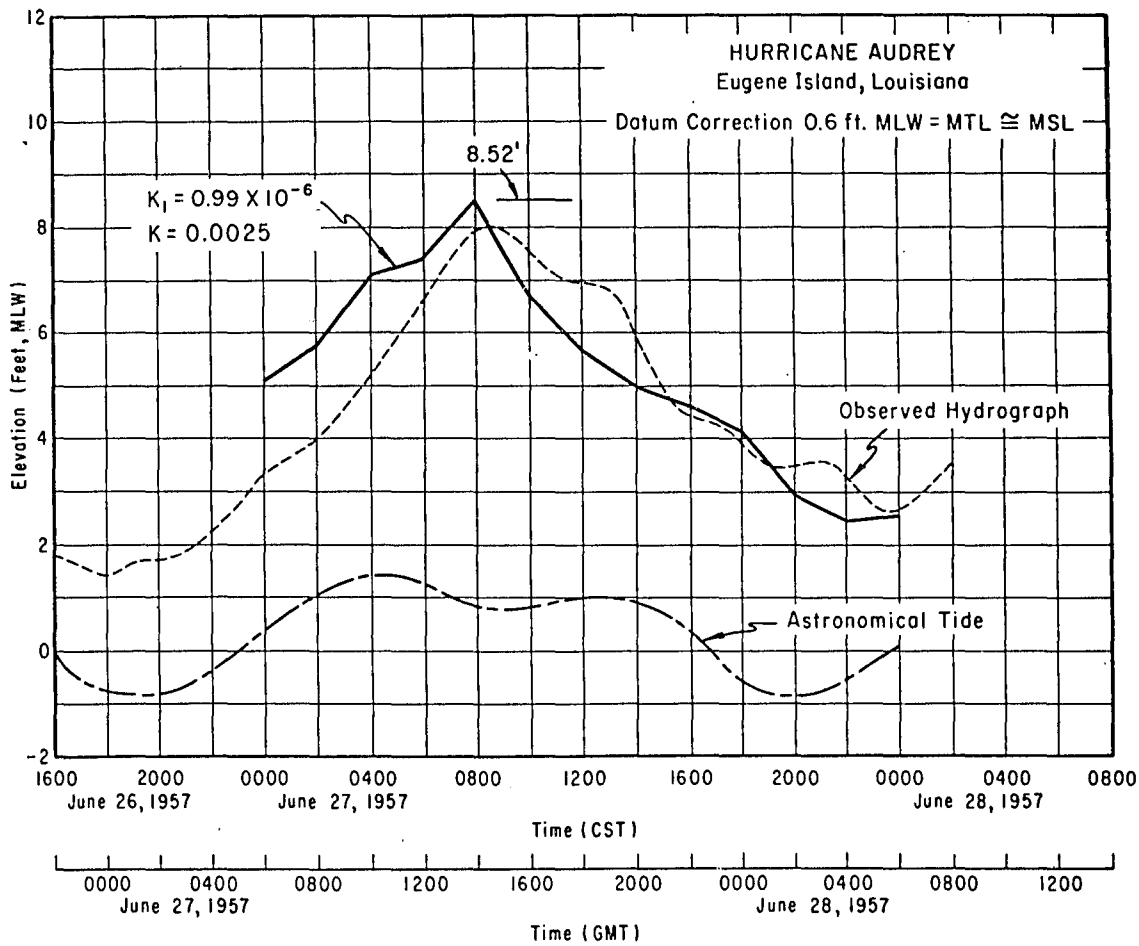


Figure 59. Observed and computed surge hydrographs for Hurricane Audrey at Eugene Island, Louisiana,  $K_1 = 1.21 \times 10^{-6}$ ,  $K_2 = 2.75 \times 10^{-6}$ ,  $K = 0.0025$ .



**Figure 60.** Observed and computed surge hydrographs for Hurricane Audrey at Eugene Island, Louisiana,  $K_1 = 1.10 \times 10^{-6}$ ,  $K_2 = 2.50 \times 10^{-6}$ ,  $K = 0.0025$ .



**Figure 61.** Observed and computed surge hydrographs for Hurricane Audrey at Eugene Island, Louisiana,  $K_1 = 0.99 \times 10^{-6}$ ,  $K_2 = 2.25 \times 10^{-6}$ ,  $K = 0.0025$ .

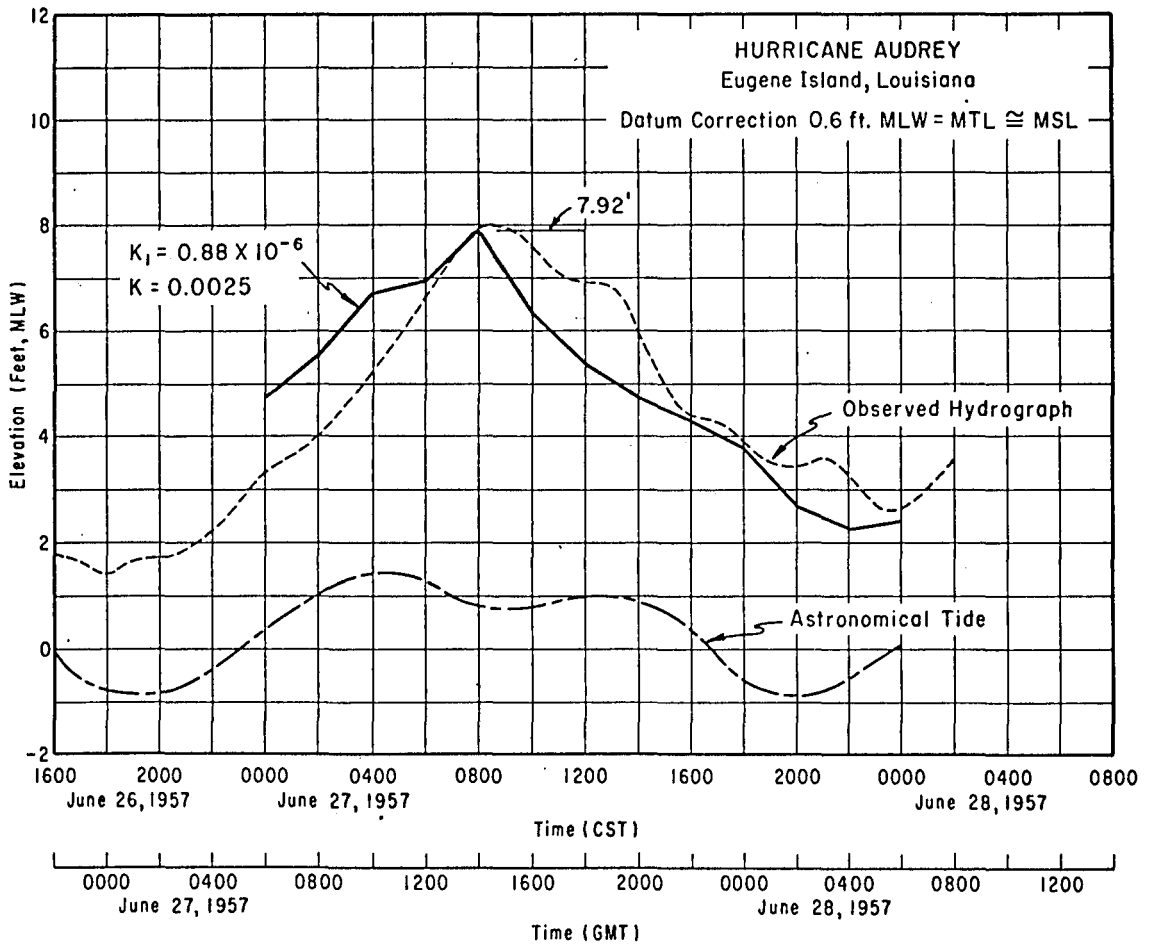


Figure 62. Observed and computed surge hydrographs for Hurricane Audrey at Eugene Island, Louisiana,  $K_1 = 0.88 \times 10^{-6}$ ,  $K_2 = 2.00 \times 10^{-6}$ ,  $K = 0.0025$ .

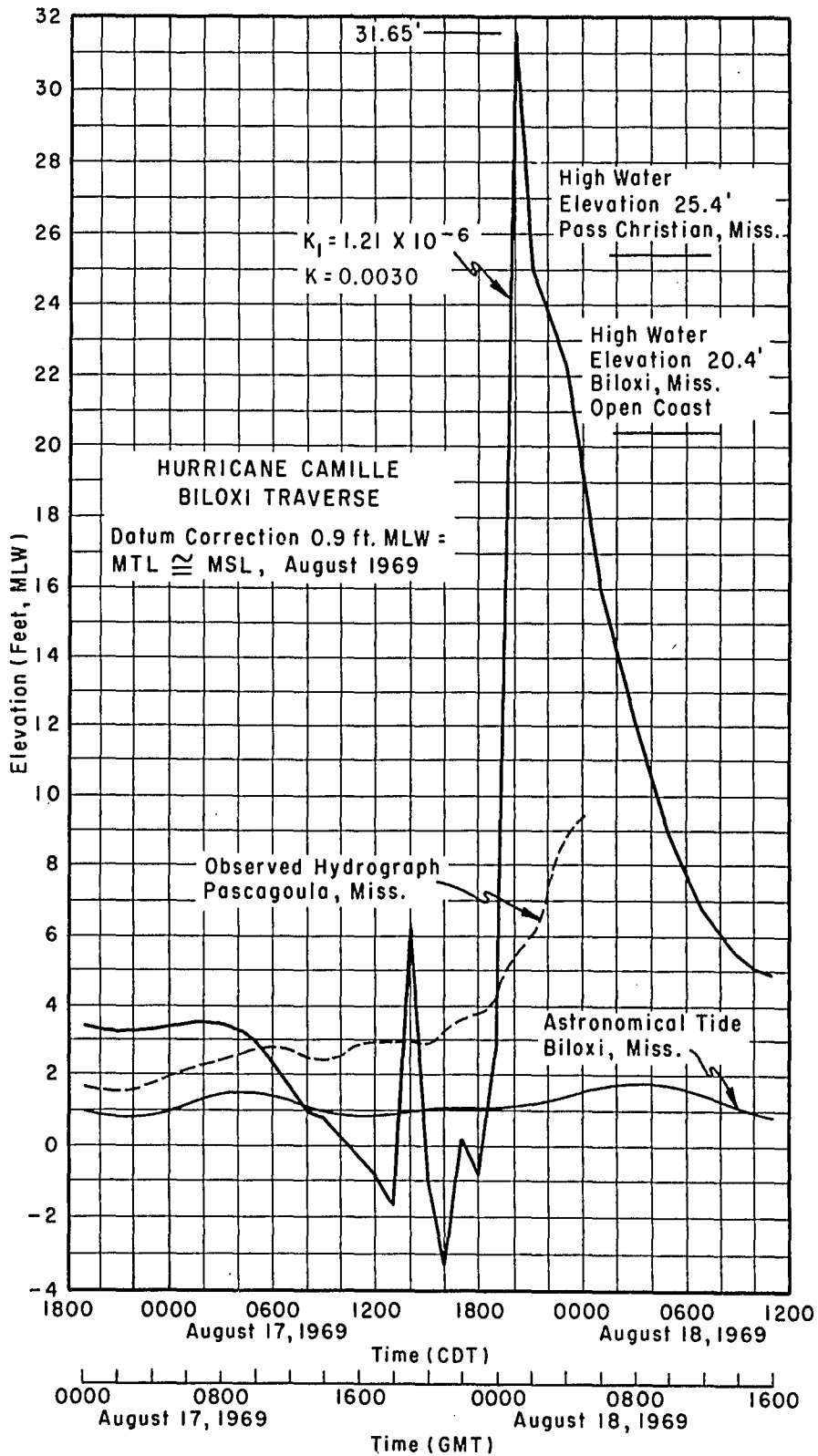


Figure 63. Observed and computed surge hydrographs for Hurricane Camille at Biloxi, Mississippi,  $K_1 = 1.21 \times 10^{-6}$ ,  $K_2 = 2.75 \times 10^{-6}$ ,  $K = 0.0030$ .

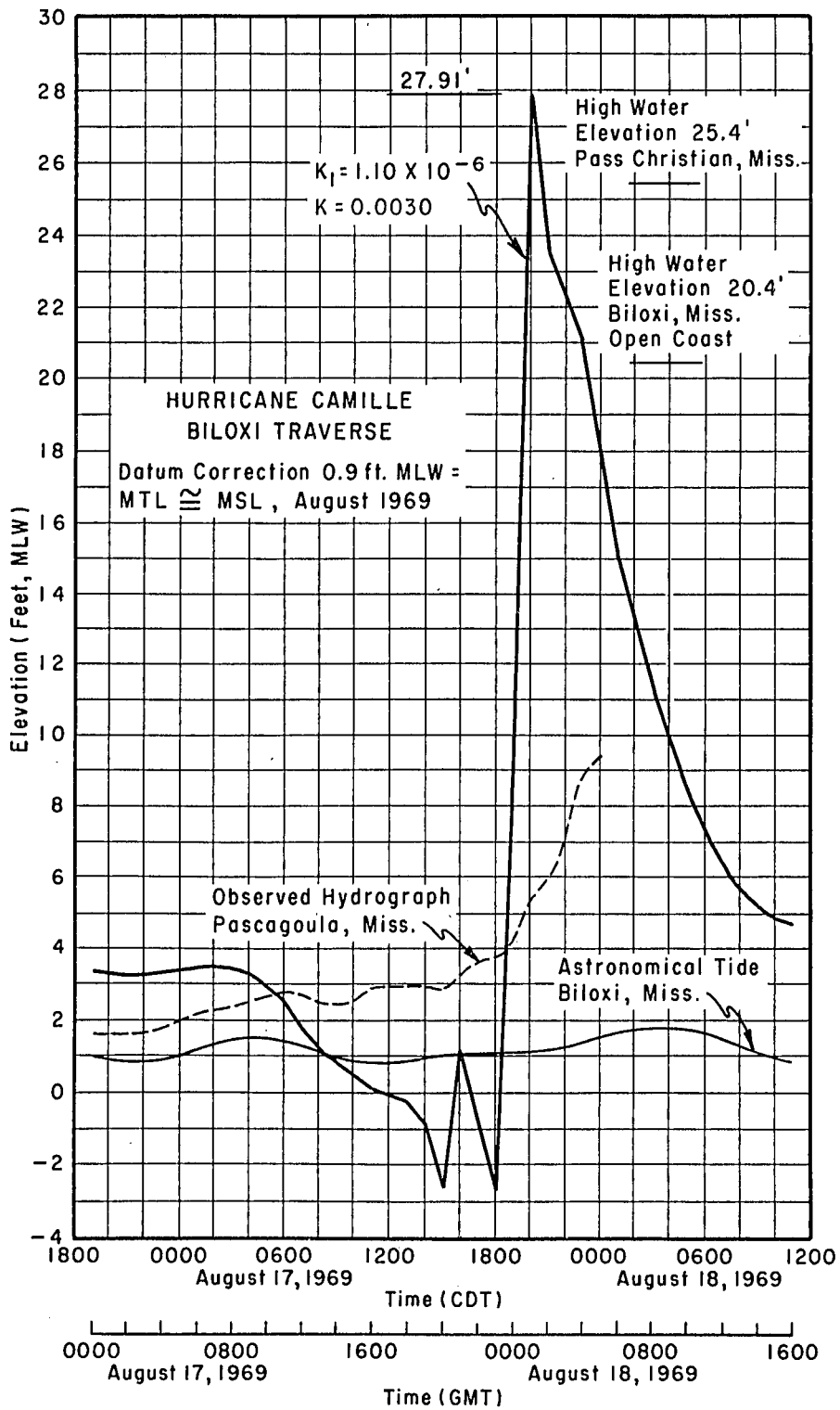


Figure 64. Observed and computed surge hydrographs for Hurricane Camille at Biloxi, Mississippi,  $K_1 = 1.10 \times 10^{-6}$ ,  $K_2 = 2.50 \times 10^{-6}$ ,  $K = 0.0030$ .



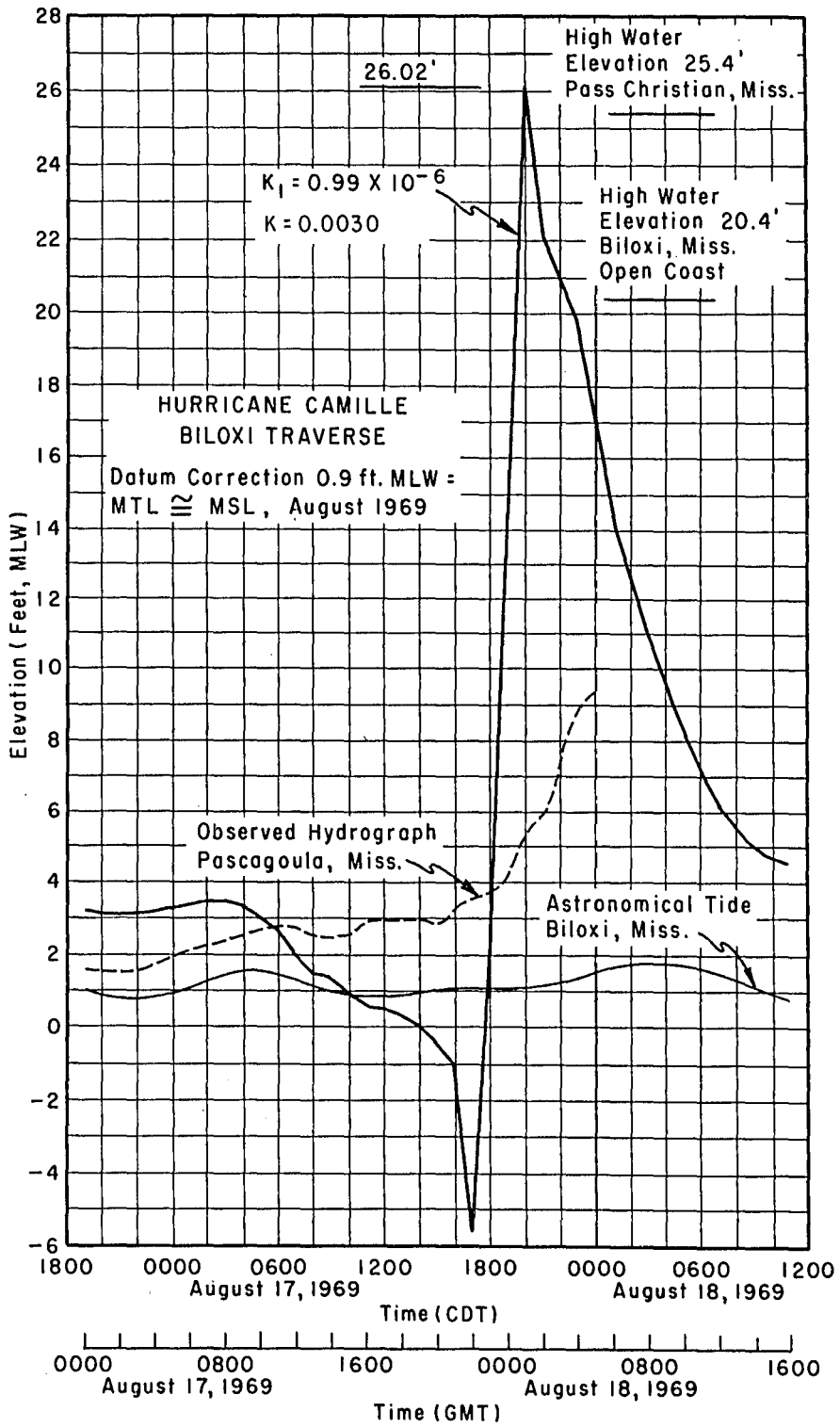


Figure 65. Observed and computed surge hydrographs for Hurricane Camille at Biloxi, Mississippi,  $K_1 = 0.99 \times 10^{-6}$ ,  $K_2 = 2.25 \times 10^{-6}$ ,  $K = 0.0030$ .

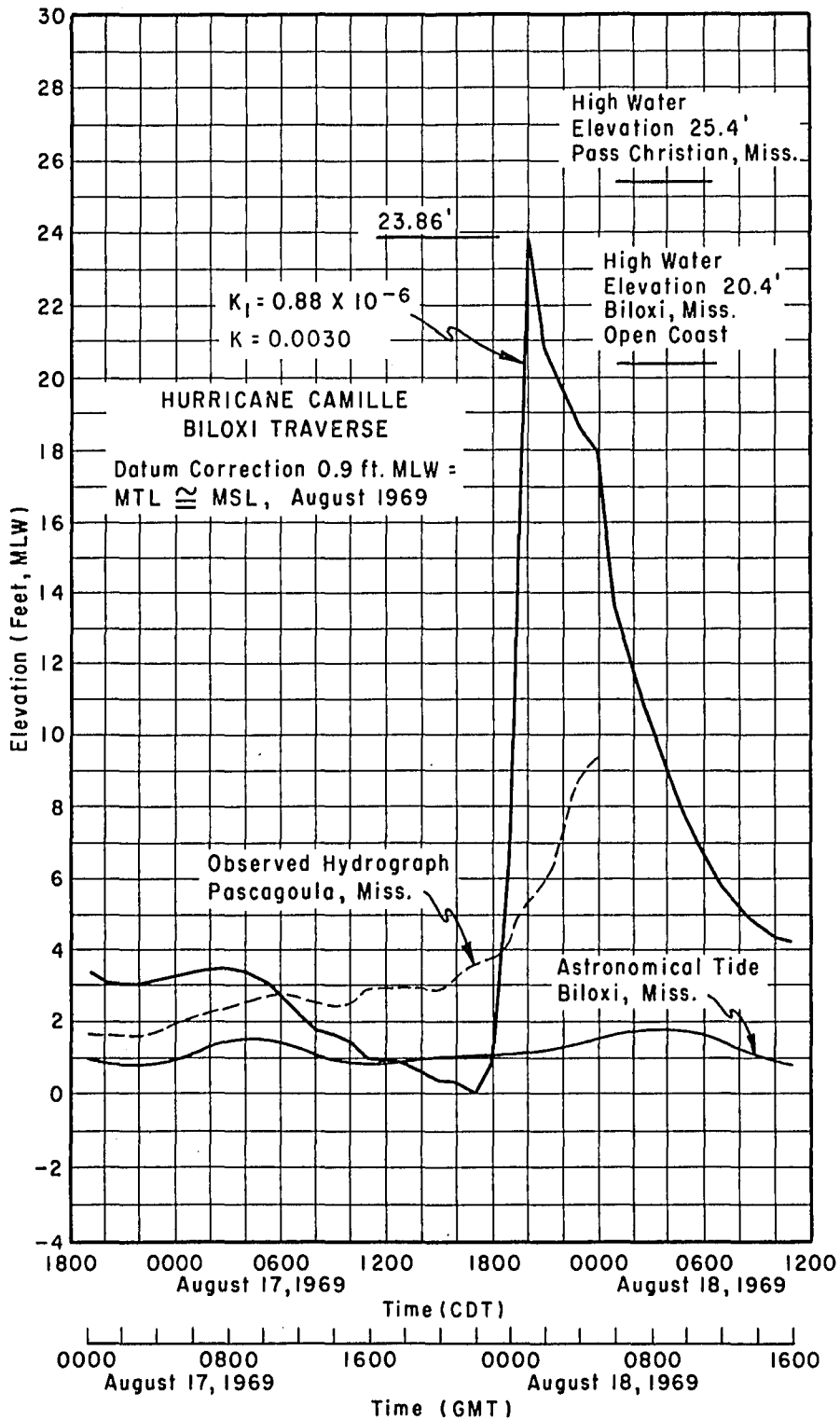


Figure 66. Observed and computed surge hydrographs for Hurricane Camille at Biloxi, Mississippi,  $K_1 = 0.88 \times 10^{-6}$ ,  $K_2 = 2.00 \times 10^{-6}$ ,  $K = 0.0030$ .

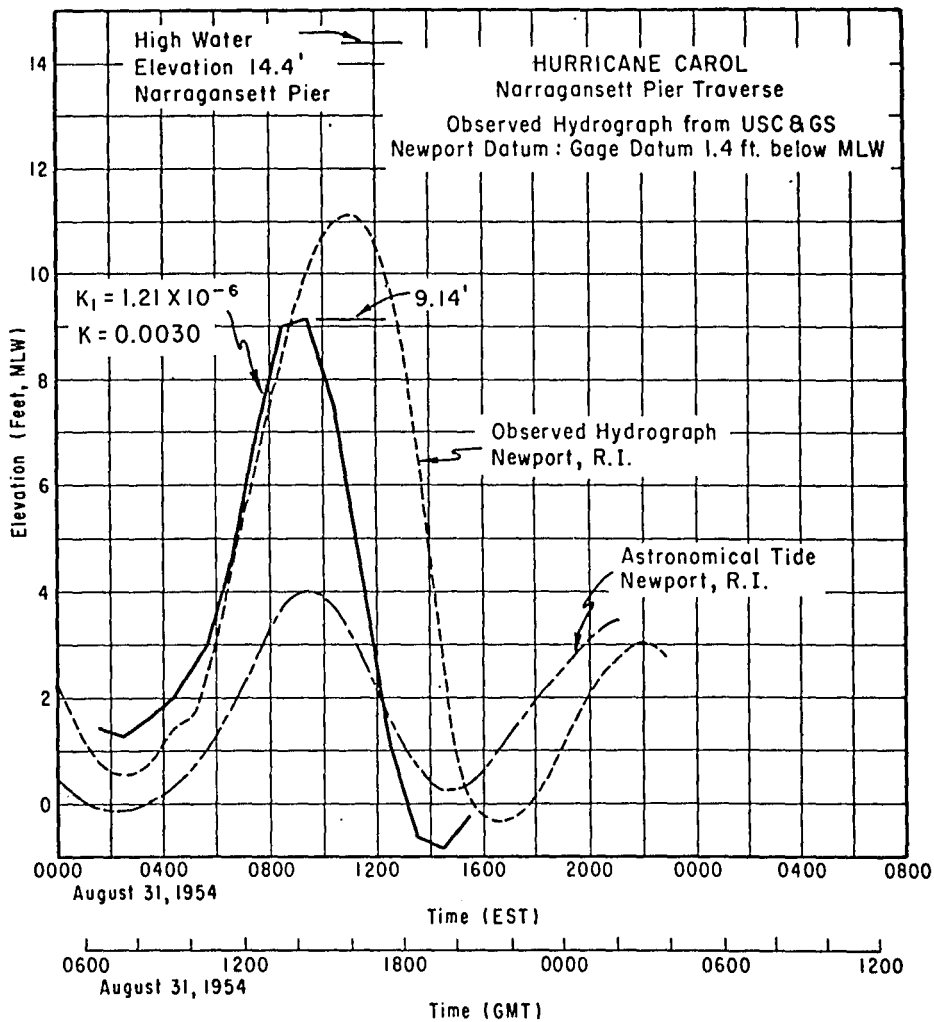


Figure 67. Reconstructed and computed surge hydrographs for Hurricane Carol at Narragansett Pier, Rhode Island,  $K_1 = 1.21 \times 10^{-6}$ ,  $K_2 = 2.75 \times 10^{-6}$ ,  $K = 0.0030$ .

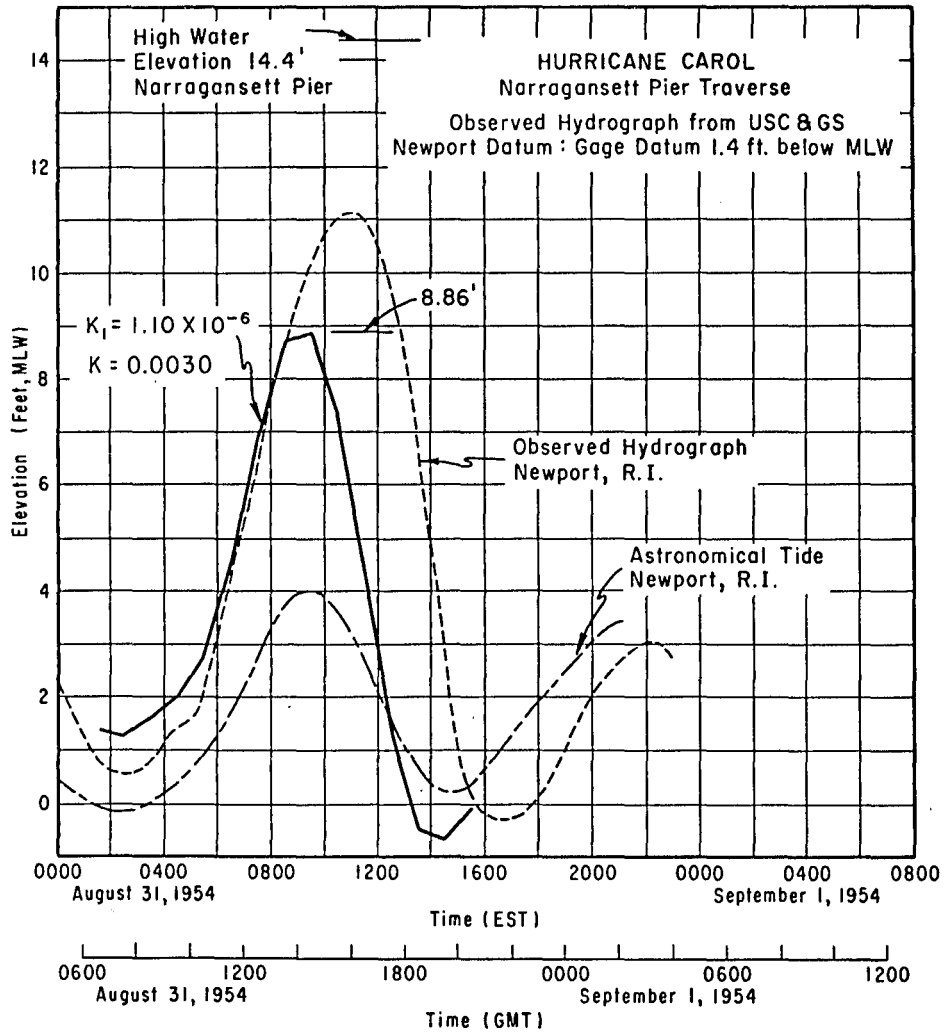
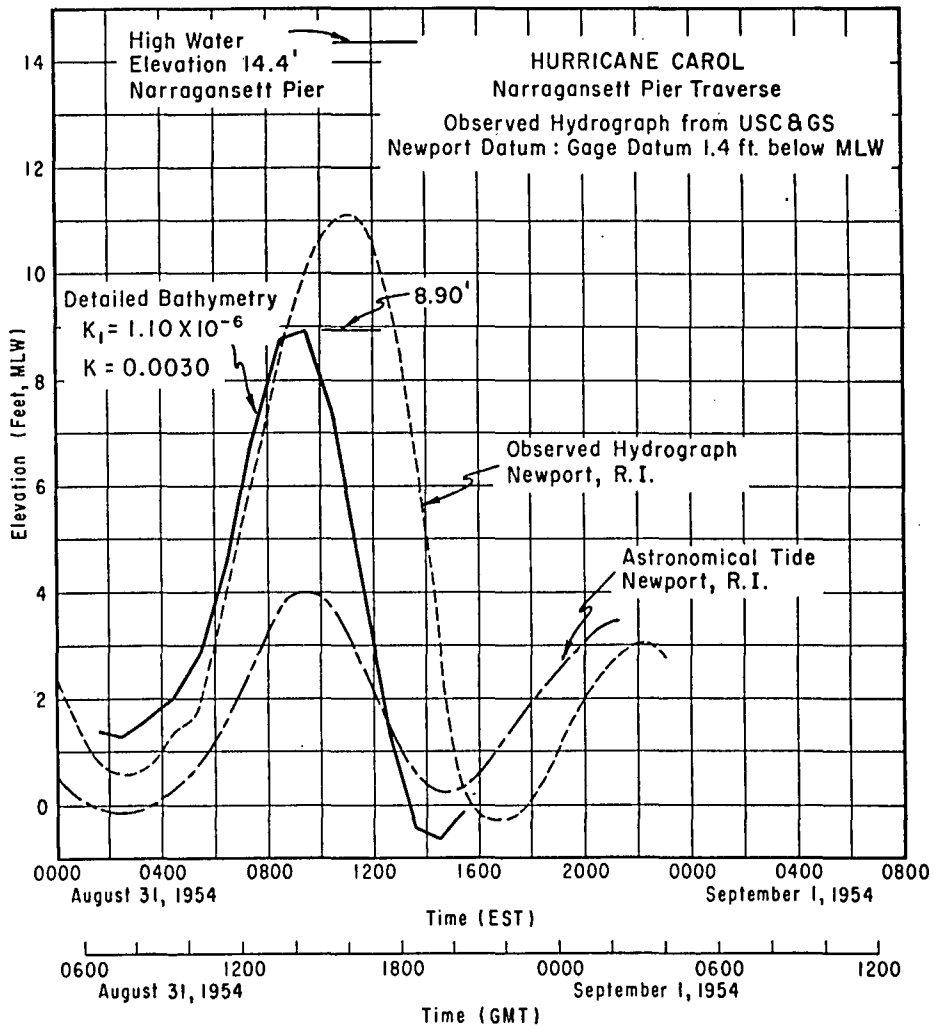


Figure 68. Reconstructed and computed surge hydrographs for Hurricane Carol at Narragansett Pier, Rhode Island,  $K_1 = 1.10 \times 10^{-6}$ ,  $K_2 = 2.50 \times 10^{-6}$ ,  $K = 0.0030$ .



**Figure 69.** Reconstructed and computed surge hydrographs for Hurricane Carol at Narragansett Pier, Rhode Island,  $K_1 = 1.10 \times 10^{-6}$ ,  $K_2 = 2.50 \times 10^{-6}$ ,  $K = 0.0030$ .

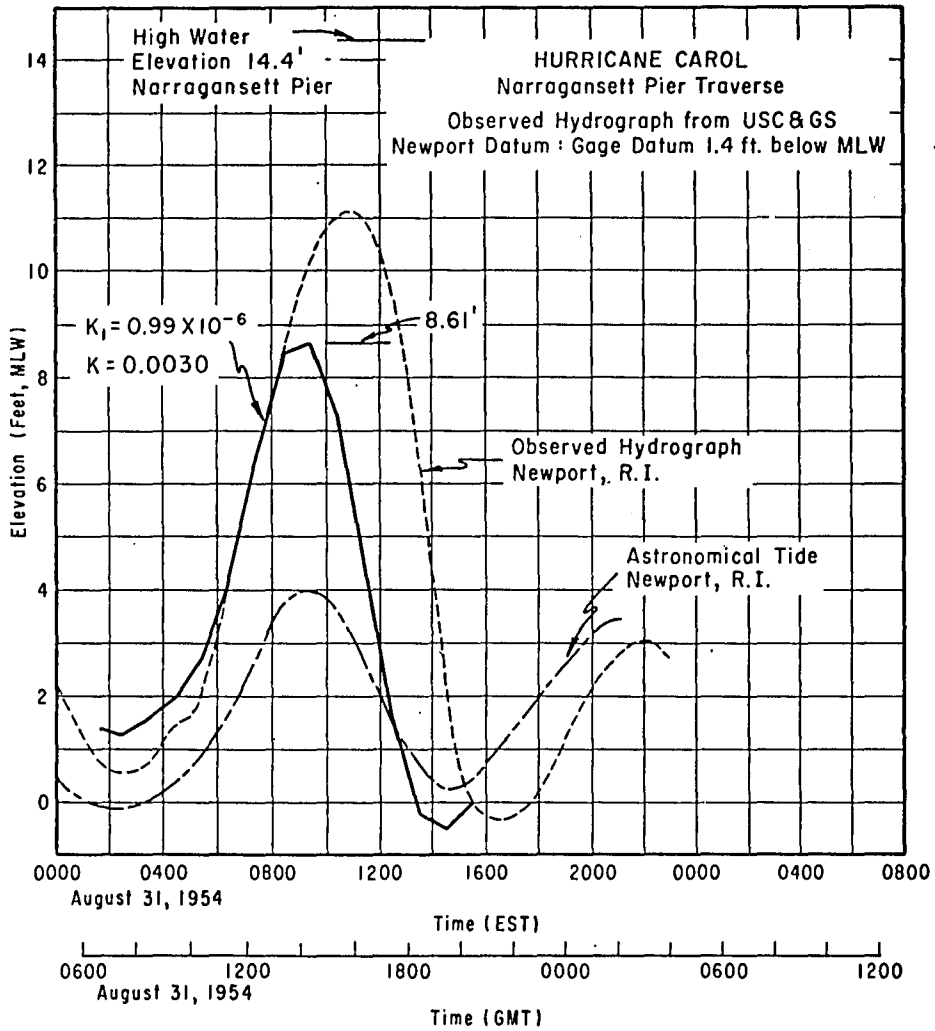


Figure 70. Reconstructed and computed surge hydrographs for Hurricane Carol at Narragansett Pier, Rhode Island,  $K_1 = 0.99 \times 10^{-6}$ ,  $K_2 = 2.25 \times 10^{-6}$ ,  $K = 0.0030$ .

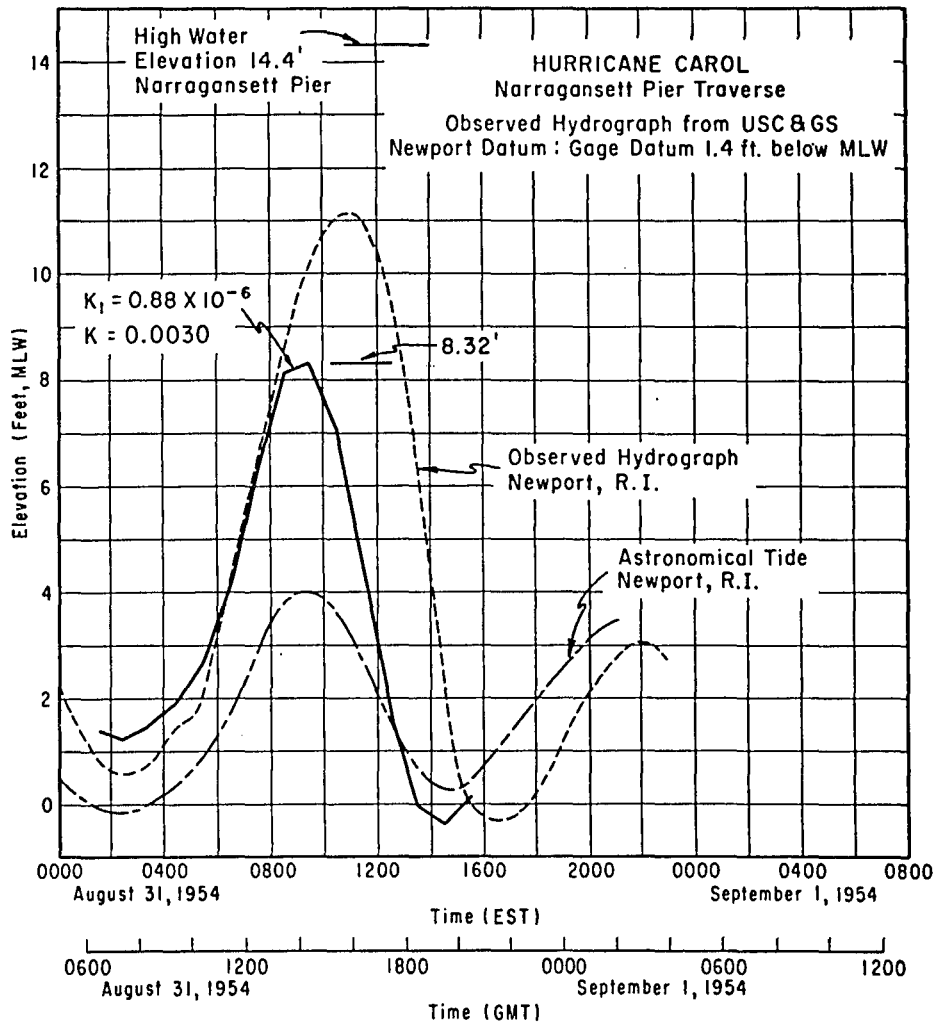


Figure 71. Reconstructed and computed surge hydrographs for Hurricane Carol at Narragansett Pier, Rhode Island,  $K_1 = 0.88 \times 10^{-6}$ ,  $K_2 = 2.00 \times 10^{-6}$ ,  $K = 0.0030$ .

c. *Initial Rise.* Other hydrographic data of major importance in the calculation of storm surge with a numerical model involves the initial rise in water level preceding the arrival of a hurricane. At such times, water levels along the coast have been observed to be different than the predicted astronomical tide. This difference in water levels between observed and predicted tides appears to be in many cases, independent of the storm or the astronomical forces. The significance of initial rise was discussed in Section I. Examination of prestorm tide gage records (from which initial rise is obtained) indicates that a considerable range of values may be selected. Harris (1963) has shown that such water level deviations can be as much as 2 feet.

The initial rise used as input to the numerical calculation of storm surge is usually the average difference between predicted astronomical and observed tides at a tide gage station at or closest to the shore-intercept (open coast) of the traverse before the influencing effects of hurricane winds and pressures. In the numerical calculations, this value should be treated as a constant and added to the total water level. This may be an oversimplification, since the cause for such rise or its exact magnitude during the passage of the storm is not known with certainty. For the purpose of calibration, initial rise was defined in Section I as "the average water level variation above the predicted astronomical tide at a station during the 2 days preceding the occurrence of a 15 to 20 miles per hour isovel of a hurricane advancing across the Continental Shelf." Based on this definition, the initial rise in water levels used in the calculation of surges at Galveston, Freeport, Eugene Island, Biloxi and Narragansett Pier for the selected hurricanes, were estimated using the predicted tides shown in Figures 39 through 71, taken from the tide tables and the observed or recorded hydrographs of water levels at the nearest tide gage stations. Because of these limitations, the initial rises in water levels used in this study may represent rough approximations.

### 3. Computational Traverses.

Computational traverses, used for surge calculation of each selected hurricane, were chosen on the basis of their proximity to hurricane tracks, to recording tide gages on relatively open coasts, and to points on the coast where maximum, well documented observations of historic surge water levels were available. The selected traverses were also situated, in each case, to the right of the hurricane tracks, to accommodate a limitation of the bathystrophic numerical model for surge computation (preferably at a distance of about 1 to 3 times the radius to maximum winds away from the path of the hurricane center, in order to intercept the maximum effect of the hurricane).

For the Hurricane of 3 October 1949 and Hurricane Carla, traverses were selected at Galveston and Freeport, Texas, for which tide gage records and good historical accounts of surges were available. A traverse for Hurricane Audrey was selected at Eugene Island, Louisiana, a relatively open-coast location which experienced maximum surges and where a semicomplete tide gage record and visual measurement of peak surge existed. For Hurricane Camille, a traverse at Biloxi, Mississippi was selected; this location was on the open coast



where large surge water levels were observed and documented and was nearer the tide gage at Pascagoula, Mississippi, where some tidal information could be obtained. A better traverse may have been Pass Christian, Mississippi, where maximum surge for this hurricane occurred; however, it was not selected because of the unusual coastal configuration of the region and the presence of offshore islands.

A traverse was selected at Narragansett Pier, Rhode Island for Hurricane Carol, the only hurricane chosen on the open Atlantic coast which experienced maximum surge. Although the data for Narragansett Pier were based on a peak value which may have included some storm wave setup and runup, and the nearest tide gage along the coast was at Newport, Rhode Island, in a semisheltered area, this was the only location unobstructed by offshore islands and where the open-coast condition could best be met.

The traverses at Galveston, Freeport, Eugene Island, Biloxi and Narragansett Pier are given in Figures 37 and 38. The bathymetric profiles for all traverses and the coordinates of the shore-intercepts and orientation of the traverses used in the computations are summarized in Table 4. The bearings of the traverses were established by a perpendicular orientation to the bathymetric contours between the tide gage stations on the open coast (shore-intercept) or nearby points and the 600-foot depth contour on the Continental Shelf. The bathymetry for the profiles was taken from detailed nautical charts.

## V. MODEL VERIFICATION

### 1. Calibration.

The ability of the numerical model to reproduce historical hurricane surges or to adequately calculate hypothetical hurricane surges depends primarily on proper selection of values of specific parameters such as wind stress and bottom friction. Selection of specific calibration coefficients and establishing their relationship to each other is important to verification of the model. Both wind and bottom friction (stress) coefficients can be calibrated for use in the hydrodynamic equations of the surge estimate. However, it should be emphasized that because hurricane surge is a complex phenomenon, the wind stress and bottom friction coefficients obtained through calibration actually represent model calibration coefficients which include more than the physical effects of wind stress and bottom friction. Therefore, these model coefficients are considered to be representative of all the remaining unknowns contributing to storm surge.

In reconstituting the historical surge data, difficulties were encountered with establishing values of initial rise which varied with location of each particular hurricane. Since it was difficult to calibrate for such an initial rise, an attempt was made to standardize the method of measurement and to assign a consistent value in each case, independent of wind stress and bottom friction. This was added to the final estimate of surge. A definition of initial rise and method of measurement is discussed in Sections I and IV. For the calibration, raw data describing historical hurricanes were corrected, reduced, digitized, and used with the model to calculate surge hydrographs and then compared to the observed or recorded surges of these hurricanes. Methods of data reduction were discussed in Section IV.

Table 4. Data Used for Calibration.

Texas		Mississippi		Rhode Island		Louisiana			
Freeport	Galveston	Biloxi		Narragansett Pier		Eugene Island			
Coordinates of Shore Intercept									
28°55.5'N	29°15.4'N	30°23'N		41°25.65'N		29°21'N			
95°17.25'W	94°48.75'W	88°52'W		71°27.35'W		91°22.5'W			
Orientation of Traverse									
S 28°E		S 25°E		S 12°E		S 20°E		S 12.5°W	
DFS	DIF	DFS	DIF	DFS	DIF	DFS	DIF	DFS	DIF
0.0	0	0.0	0	0.0	0	0.0	0	0.0	0
0.05	1	0.1	4	1.0	3	0.1	15	1.0	5
0.1	5	0.2	6	2.0	9	0.2	25	2.0	10
0.4	10	0.4	12	3.0	9	0.3	30	3.0	12
0.5	20	0.5	16	5.0	12	0.4	40	5.0	15
1.0	30	1.0	18	10.0	13	0.5	45	10.0	15
2.0	32	2.0	29	15.0	35	0.6	47	15.0	18
3.0	37	3.0	34	20.0	36	0.7	49	20.0	20
4.0	40	4.0	40	30.0	40	0.8	55	30.0	50
5.0	47	5.0	45	40.0	52	0.9	57	40.0	60
10.0	66	10.0	57	50.0	90	1.0	60	50.0	140
15.0	78	15.0	60	60.0	160	2.0	80	60.0	200
20.0	90	20.0	54	70.0	235	3.0	90	70.0	260
30.0	114	30.0	72	77.0	600	4.0	103	80.0	320
40.0	132	40.0	95	---	---	5.0	107	90.0	600
50.0	168	50.0	123	---	---	10.0	120	---	---
60.0	240	60.0	150	---	---	15.0	120	---	---
70.0	570	70.0	180	---	---	20.0	150	---	---
70.9	600	80.0	230	---	---	30.0	160	---	---
---	---	90.0	348	---	---	40.0	192	---	---
---	---	92.5	600	---	---	50.0	215	---	---
---	---	---	---	---	---	60.0	270	---	---
---	---	---	---	---	---	70.0	370	---	---
---	---	---	---	---	---	80.0	450	---	---
---	---	---	---	---	---	85.0	600	---	---

DFS = Distance From Shore (nautical miles)

DIF = Depth In Feet (mean low water)

Calibration was achieved by varying the values of the relative wind stress,  $k$ , and bottom friction,  $K$ , coefficients in the numerical equations used for surge computation. These coefficients are interdependent (coupled) in the basic equations (15) and (16) of the model, and were adjusted so that computed surges reproduced, as well as possible, the observed or recorded storm surges of historic hurricanes.

Emphasis was placed on calibrating the constants in Van Dorn's (1953) wind-stress relationship, which has the following forms in the numerical model:

$$k = K_1 + K_2 \left[ 1 - \left( \frac{W_c}{W} \right)^2 \right] \text{ for } W \geq W_c, \quad (12)$$

and

$$k = K_1 \text{ when } W \leq W_c, \quad (13)$$

where,

- $k$  = wind-stress coefficient,
- $K_1$  = the constant part of the wind-stress coefficient,
- $K_2$  = the constant multiplier of the velocity-dependent part of the wind-stress coefficient,
- $W$  = windspeed in miles per hour,
- $W_c$  = critical windspeed taken about 16 miles per hour (14 knots).

In equations (12) and (13) the constants  $K_1$  and  $K_2$  account for pressure, density, precipitation, temperature and other factors of a hurricane system which are not easily measured. These factors effect the wind stress and the model as a whole and can only be obtained empirically.

In verifying the bathystrophic model, the wind-stress constants were varied until reasonable agreement of the peak values for the calculated and observed or recorded surge hydrographs were obtained at each traverse for each hurricane. Similarly, the value of the bottom friction coefficient was varied for each pair of constants in the wind-stress relationship to determine the effects of bottom friction on the overall surge computation. This procedure was continued until the entire range of values for  $K_1$ ,  $K_2$  and bottom friction,  $K$ , were analyzed, and an optimal condition of *best fit* was obtained between observed and computed surge hydrographs. *Best fit* can be best described as a condition of reasonable agreement between observed or recorded and computed surge hydrographs in shape, height, and time of peak surge values. Because of complexities in the shapes of both computed and observed or recorded surge hydrographs, no attempts were made to perform statistical correlation studies of the entire hydrographs. In each case, best fit was obtained

when maximum computed surge reached a value slightly greater than the observed maximum surge value. To ensure that the maximum computed surge was reasonable and conservative, a 5-percent limit was placed on the difference with the peak observed surge.

The results of calibration with historical hurricanes are shown in Figures 39 through 71. In these figures, computed surge hydrographs of each hurricane at each traverse are shown for different values of wind-stress constants and bottom friction. Observed or recorded surge hydrographs and predicted astronomical tides are also shown. Computed surge hydrographs incorporate the varying predicted tides and values of initial rise. Computer outputs of surge estimates for each hurricane are summarized in Appendix B. Initial conditions used and the results of calibration obtained for historical hurricanes are summarized in Table 5. Only peak values of surge are given in the table.

## 2. Evaluation of Calibration.

Data reliability from an evaluation of the results obtained through calibration of each hurricane follows:

a. *Hurricane of 1949.* The computed surge hydrograph of the Hurricane of 1949 at Galveston showed poor correlation to the recorded surge hydrograph; this may be due to lack of wind data for this traverse. A fair correlation was obtained for the Freeport traverse.

b. *Hurricane Carla.* Good results were obtained using Hurricane Carla data for the Galveston traverse which provided most of the desired conditions for an open-coast station. Good results were obtained for the Freeport traverse.

c. *Hurricane Audrey.* The computed surge hydrograph of Hurricane Audrey for the Eugene Island traverse showed a good degree of correlation to the observed surge hydrograph.

d. *Hurricane Camille.* Inadequate correlation between the computed hurricane surge hydrograph and the observed high water elevation at the Biloxi, Mississippi, traverse for Hurricane Camille, is attributed to insufficient wind-field information and the irregularity of the coastline. Furthermore, Camille's wind-field charts were available for only 6-hour intervals and the highest surge occurred sometime between charts. The interpolated wind field for that period probably does not accurately represent actual wind conditions which produced the maximum surge. Similarly, there is no hydrograph at the point of the coast where the maximum surge occurred. The value provided is an observed elevation extrapolated from debris and high water marks and may include the effects of wave-induced runup. Because of the Mississippi River Delta and the offshore islands, and lack of sufficient wind field and hydrograph data, Biloxi was a poor site for numerical calculations of surge.

e. *Hurricane Carol.* Poor correlation was obtained for computed and reconstructed surge hydrographs of the Narragansett Pier traverse using data of Hurricane Carol. There was one limitation with the available wind data—the maximum surge occurred between the 3-hour time interval for which wind data were available, and since Carol crossed the coast at

Table 5. Effect of Wind Stress and Bottom Friction on Calculated Surge Values of Historic Hurricanes

Hurricane Traverse	Wind Equation	Stress Constants	Bottom Friction	Initial Rise	Surge Peak Height		Percent Difference
	$K_1$	$K_2$	K		Calculated	Observed	
October 1949 Freeport, Texas	$1.21 \times 10^{-6}$	$2.75 \times 10^{-6}$	0.0030	2.00	8.69	8.9	- 2.4
	$1.21 \times 10^{-6}$	$2.75 \times 10^{-6}$	0.0025	2.00	8.83	8.9	- 1.9
	$1.10 \times 10^{-6}$	$2.50 \times 10^{-6}$	0.0025	2.00	8.38	8.9	- 5.8
	$0.99 \times 10^{-6}$	$2.25 \times 10^{-6}$	0.0025	2.00	8.03	8.9	- 9.8
	$0.88 \times 10^{-6}$	$2.00 \times 10^{-6}$	0.0025	2.00	7.67	8.9	-13.8
October 1949 Galveston, Texas	$1.21 \times 10^{-6}$	$2.75 \times 10^{-6}$	0.0030	2.00	8.44	6.0	40.7
	$1.21 \times 10^{-6}$	$2.75 \times 10^{-6}$	0.0035	2.00	8.37	6.0	39.5
	$1.10 \times 10^{-6}$	$2.50 \times 10^{-6}$	0.0035	2.00	8.02	6.0	33.7
	$0.99 \times 10^{-6}$	$2.25 \times 10^{-6}$	0.0035	2.00	7.65	6.0	27.5
	$0.88 \times 10^{-6}$	$2.00 \times 10^{-6}$	0.0035	2.00	7.30	6.0	21.7
Carla Freeport, Texas	$1.21 \times 10^{-6}$	$2.75 \times 10^{-6}$	0.0030	2.50	10.15	11.6	-12.5
	$1.21 \times 10^{-6}$	$2.75 \times 10^{-6}$	0.0050	2.50	9.70	11.6	-16.4
	$1.10 \times 10^{-6}$	$2.50 \times 10^{-6}$	0.0050	2.50	9.37	11.6	-19.2
	$0.99 \times 10^{-6}$	$2.25 \times 10^{-6}$	0.0050	2.50	9.03	11.6	-22.2
	$0.88 \times 10^{-6}$	$2.00 \times 10^{-6}$	0.0050	2.50	8.69	11.6	-25.1
Carla Galveston, Texas	$1.21 \times 10^{-6}$	$2.75 \times 10^{-6}$	0.0030	1.90	9.73	10.0	- 2.7
	$1.10 \times 10^{-6}$	$2.50 \times 10^{-6}$	0.0030	1.90	9.33	10.0	- 6.7
	$0.99 \times 10^{-6}$	$2.25 \times 10^{-6}$	0.0030	1.90	8.93	10.0	-10.7
	$0.88 \times 10^{-6}$	$2.00 \times 10^{-6}$	0.0030	1.90	8.52	10.0	-14.8
Audrey Eugene Island, Louisiana	$1.21 \times 10^{-6}$	$2.75 \times 10^{-6}$	0.0030	1.00	9.60	7.9	21.5
	$1.21 \times 10^{-6}$	$2.75 \times 10^{-6}$	0.0025	1.00	9.67	7.9	22.4
	$1.10 \times 10^{-6}$	$2.50 \times 10^{-6}$	0.0025	1.00	9.10	7.9	15.2
	$0.99 \times 10^{-6}$	$2.25 \times 10^{-6}$	0.0025	1.00	8.52	7.9	7.8
	$0.88 \times 10^{-6}$	$2.00 \times 10^{-6}$	0.0025	1.00	7.92	7.9	0.3
Camille Biloxi, Mississippi	$1.21 \times 10^{-6}$	$2.75 \times 10^{-6}$	0.0030	1.20	31.65	20.4	55.1
	$1.10 \times 10^{-6}$	$2.50 \times 10^{-6}$	0.0030	1.20	27.91	20.4	36.8
	$0.99 \times 10^{-6}$	$2.25 \times 10^{-6}$	0.0030	1.20	26.02	20.4	27.5
	$0.88 \times 10^{-6}$	$2.00 \times 10^{-6}$	0.0030	1.20	23.86	20.4	17.0
Carol Narragansett Pier, Rhode Island	$1.21 \times 10^{-6}$	$2.75 \times 10^{-6}$	0.0030	1.20	9.14	11.1 <sup>1</sup>	-17.7
	$1.10 \times 10^{-6}$	$2.50 \times 10^{-6}$	0.0030	1.20	8.86	11.1	-20.2
	$1.10 \times 10^{-6}$	$2.50 \times 10^{-6}$	0.0030	1.20	8.90 <sup>2</sup>	11.1	-19.8
	$0.99 \times 10^{-6}$	$2.25 \times 10^{-6}$	0.0030	1.20	8.61	11.1	-22.4
	$0.88 \times 10^{-6}$	$2.00 \times 10^{-6}$	0.0030	1.20	8.32	11.1	-25.0

1. Newport peak value.

2. Detail bathymetry.

a high translational velocity, the interpolated wind values used for the calculation may be underestimates of the actual windspeeds. Therefore, the calculated surge may be less than what it would have been calculated if the actual wind field was more accurately described. The maximum observed surge runup obtained for this site may include some storm-wave setup and runup with the surge. However, both the predicted tide and the observed surge hydrograph were obtained for the Coasters Island tide gage at Newport, Rhode Island, a site inside Narragansett Bay. The observed hydrograph at Newport may reflect frictional and restricted entrance effects not representative of the open-coast surge. In addition, the hydrograph was not a gage recording, as the gage became inoperative during the storm, but the hydrograph is a composite of visual observations and a survey of high water marks.

Examination of computed hydrographs for historic hurricanes in Figures 39 through 71, and of the summary of peak surges and calibration variables given in Table 5, show that best results were obtained when the  $K_1$  and  $K_2$  coefficients in the wind-stress relationship were  $1.21 \times 10^{-6}$  and  $2.75 \times 10^{-6}$  respectively, and when the bottom friction ( $K$ ) was given values ranging from 0.0025 to 0.003. Based on these results the wind-stress equation was given the following numerical form:

$$k = \left[ 1.21 + 2.75 \left( 1 - \frac{16}{W} \right)^2 \right] 10^{-6} \quad (42)$$

This combination of wind stress and bottom friction coefficients gave optimum results with the bathystrophic numerical model. Of the two coefficients, the wind-stress coefficient was more important and had the most pronounced effect on the final surge computation. Large changes in the value of bottom friction coefficient did not appreciably change the final estimate of surge, while small changes in wind-stress coefficient produced large changes in the surge estimate. The values for  $K_1$  and  $K_2$ , in the wind-stress relationship obtained through the calibration procedure are not appreciably different from those used previously with this model, e.g., Reid and Bodine (1968) used values of  $K_1 = 1.1 \times 10^{-6}$  and  $K_2 = 2.5 \times 10^{-6}$  in the wind-stress equation used in their study of Galveston Bay. The form of equation (42) is empirically derived for low windspeeds. It is extrapolated here for use at the high windspeeds of hurricanes and indicates a nonlinear relation between windspeed and wind stress. This may be due to decapitation of the wave tops at higher windspeeds, increased turbulence, or some other unknown or unmeasurable factors.

To check on the relationship of the wind stress and windspeed using the coefficients given above, the following test was applied. Using the parameters of the probable maximum hurricane (PMH), ( $p_o = 26.7$  inches of mercury,  $p_n = 31.25$  inches of mercury,  $R = 24$  nautical miles,  $V_t = 20$  knots, and  $U_{max} = 149.8$  miles per hour) windspeeds and pressure were calculated along the hurricane's prime vector for increasing integer distance intervals of the ratio,  $r/R$ . Utilizing these data, the wind-stress equation was solved and values of wind-stress coefficient were tabulated (Table 6). A plot was prepared to show the

Table 6. Change in the Wind-Stress Coefficient with Windspeed and Pressure.

r/R	Windspeed, W (miles per hour)	Atmospheric Pressure, p (inches of mercury)	Wind-Stress Coefficient, k
1	149.9	28.4	$3.40 \times 10^{-6}$
2	137.9	29.5	$3.36 \times 10^{-6}$
3	100.0	30.0	$3.15 \times 10^{-6}$
4	86.6	30.2	$3.04 \times 10^{-6}$
5	76.4	30.4	$2.93 \times 10^{-6}$
6	68.2	30.6	$2.82 \times 10^{-6}$
7	60.6	30.6	$2.70 \times 10^{-6}$
8	53.7	30.7	$2.57 \times 10^{-6}$
9	48.0	30.8	$2.43 \times 10^{-6}$
10	43.0	30.8	$2.29 \times 10^{-6}$
12	34.8	30.9	$2.01 \times 10^{-6}$
14	28.9	30.9	$1.76 \times 10^{-6}$
16	24.3	31.0	$1.53 \times 10^{-6}$
18	20.5	31.0	$1.34 \times 10^{-6}$
20	16.9	31.0	$1.22 \times 10^{-6}$
22	13.8	31.0	$1.21 \times 10^{-6}$

Wind-Stress Equation:  $k = 1.21 \times 10^{-6} + 2.75 \times 10^{-6} \left(1 - \frac{16}{W}\right)^2$

Atmospheric pressure Equation:  $p = p_o + p_n = p_o) e^{-R/r}$

change of the wind-stress coefficients with increasing windspeeds (Fig. 72). As shown in the figure, at higher windspeeds the wind-stress coefficient increases but at a slower rate than at lower speeds.

## VI. DESIGN STORM WIND FIELD REDUCTION

The meteorological considerations leading to development of wind field data for synthetic hurricanes such as the SPH and the PMH, have been discussed by Myers (1954), Graham and Nunn (1959) and U.S. Weather Bureau (HUR 7-97, 1968). Definitions of design storms have been given previously in this report. The development of model winds and pressure fields of hurricanes along the lines presented by Myers is summarized in the *Shore Protection Manual* (U.S. Army, Corps of Engineers, Coastal Engineering Research Center, 1973).

The numerical model used in this study is capable of developing internally the wind velocity field of a synthetic hurricane, and of using this data for surge calculations. The methodology of wind data reduction and pressure field development for synthetic hurricanes are included as an example. The hurricane pressure field can be obtained by the following equation:

$$\frac{p - p_o}{p_n - p_o} = e^{-R/r}, \quad (43)$$

where,

- $p$  = pressure anywhere in the storm system,
- $p_o$  = pressure at center of storm,
- $p_n$  = pressure at the periphery of the storm,
- $r$  = the radial distance from the storm center to the computation point on the traverse line,

and

$R$  = radius to maximum winds.

Equation (43) is the model's radial profile of sea level pressure developed empirically from hurricane data in Schloemer (1954). Another form of equation (43) used in this model is:

$$p_n - p = (p_n - p_o) (1 - e^{-R/r}). \quad (44)$$

The same equation can be used for estimating the pressure setup,  $S_{\Delta p}$ , contribution to the surge with the following equation:

$$S_{\Delta p} = 1.14 (p_n - p_o) (1 - e^{-R/r}), \quad (45)$$



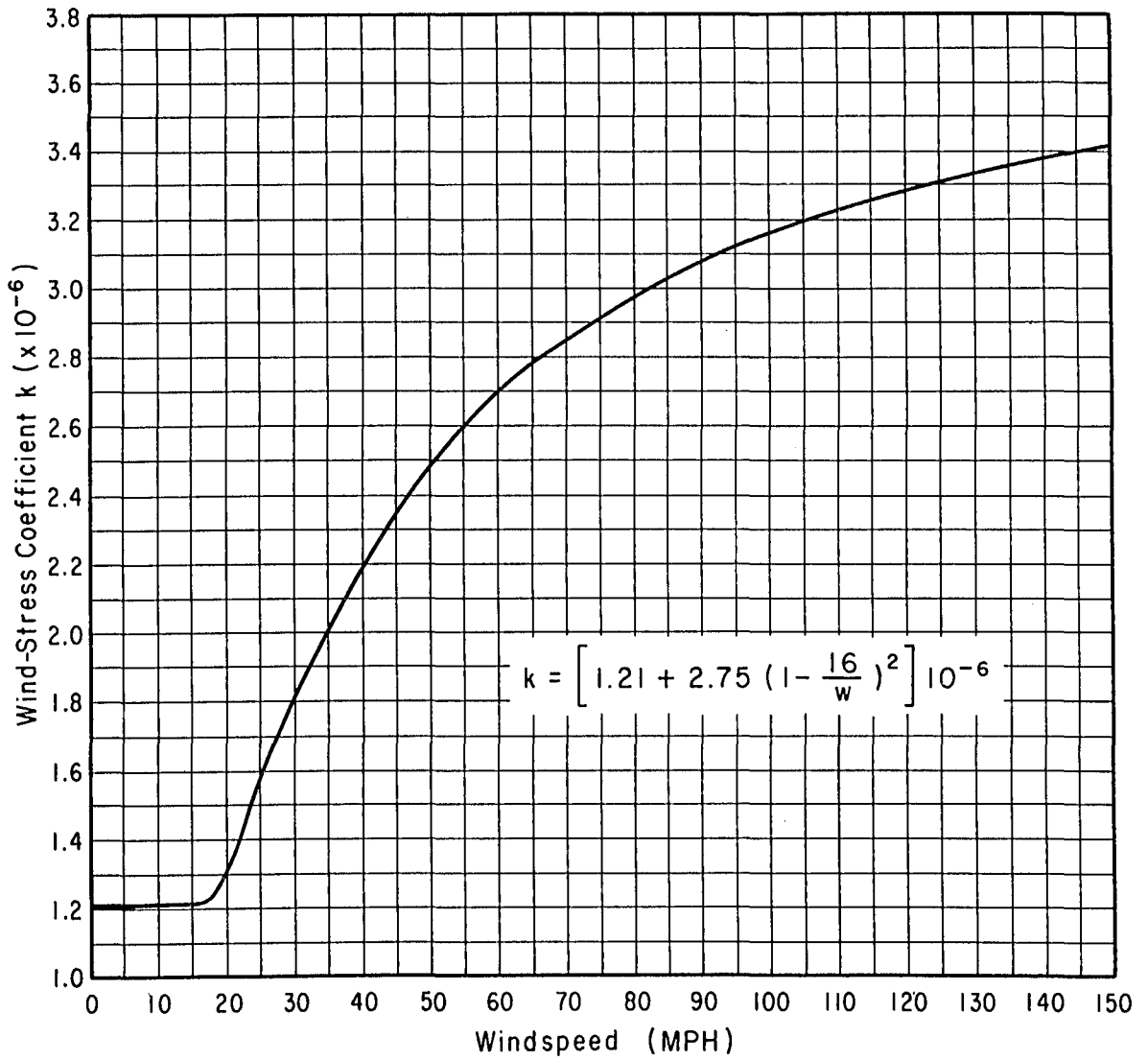


Figure 72. Relationship of wind-stress coefficient to windspeed along the prime vector of PMH.

where 1.14 is a conversion constant changing pressure in inches of mercury to feet of setup, accounting also for density effects.

The maximum gradient windspeed ( $U_{max}$ ), the maximum wind ( $U_R$ ) in the belt of maximum winds, and the windspeed ( $U$ ) anywhere along a computational traverse, can be determined by the following equations:

$$U_{max} = K(p_n - p_o)^{1/2} - R(0.576 f), \quad (46)$$

$$U_R = 0.865 U_{max} + 0.576 V_F, \quad (47)$$

$$U = \left( \frac{U_o}{U_R} \right) U_R - 0.576 V_F (1 - \cos \epsilon), \quad (48)$$

where

$U_{max}$  = maximum gradient windspeed in miles per hour for a symmetrical, synthetic hurricane,

$U_R$  = maximum windspeed at the radius to maximum winds ( $R$ ) given in miles per hour, 30 feet above the water,

$K$  = empirical constant; units are miles per hour per inches of mercury<sup>1/2</sup> (a function of latitude for the PMH),

$p_n$  = asymptotic pressure (inches of mercury),

$p_o$  = central pressure (inches of mercury),

$p$  = pressure at a given location (inches of mercury),

$R$  = radius to maximum winds (nautical miles),

$f$  = Coriolis parameter =  $2\omega \sin \phi$ ,

$\phi$  = north latitude (degrees),

$\omega$  = 0.2625 radians per hour,

$V_F$  = forward speed of the storm (knots),

$U_o$  = wind velocity at a given location along the "prime vector." This is a function of  $r$  and  $R$  (see U.S. Weather Bureau, HUR 7-97, 1968),

$\epsilon$  = the angle between the prime vector and the radius to station location,

0.576 = the dimensionless product of 0.5, a fundamental factor in equations 46, 47, and 48, and the conversion from nautical to statute miles (1 nautical mile = 1.1515 statute mile),

and

0.865 = dimensionless empirical reduction factor from  $U_{max}$  to  $U_R$  for all zones except zone B where a value of 0.885 is used.

Figure 73 shows hurricane zones for the Atlantic and gulf coasts.

Equations (46) and (47) relate the maximum gradient and sustained winds to the hurricane pressure field, the Coriolis force, and the storm's translational velocity or forward speed. The wind velocity field for any design (hypothetical) hurricane can be computed and graphically constructed by solving the equations given above. However, for both the PMH and SPH, the values of gradient windspeed,  $U_{max}$ , and the maximum sustained wind,  $U_R$ , are given by definition, so for these hurricanes, only equation (48) is solved internally to develop the wind field along the traverse. The geometry of a synthetic hurricane system for which the wind field is calculated is shown in Figure 74. At any time, the hurricane wind field can be depicted as near-symmetrical and characterized by a storm center at point O and an approximate zero-windspeed isovel. The computational traverse is placed by definition through the radius of maximum winds and parallel to the storm track indicated by the line ABC (Fig. 74). The storm's track is indicated by line OD'. The hurricane's *prime vector*, indicated by the line OBE, forms an angle of  $115^\circ$  with the track and traverse. The wind field of only a stationary hurricane is symmetrical on both sides of this line. The wind field of a moving hurricane is distorted by the forward velocity of the storm. The shore-intercept of the traverse indicated by point A, is placed initially at the zero-wind isovel, at zero time in the computation. At this time, the theoretical wind direction at point A forms a deflection angle of  $25^\circ$  with a line normal to the radius through the point. This is the deflection angle  $\gamma$  shown in Figure 74 and at this point has a maximum value. From geometry shown in the figure it becomes apparent that the wind vector  $\theta$ , the angle formed between the traverse and the wind vector, is given by:

$$\theta = \epsilon - 25^\circ + \gamma, \quad (49)$$

where  $\gamma$  = the angle of wind vector deflection, will vary as follows:

$$0 \leq \frac{r}{R} < 1, \gamma \text{ will vary from } 0^\circ \text{ to } 10^\circ,$$

$$1 \leq \frac{r}{R} < 1.2, \gamma \text{ will vary from } 10^\circ \text{ to } 25^\circ,$$

$$1.2 \leq \frac{r}{R}, \text{ the } \gamma = 25^\circ,$$

and  $\epsilon$  = the angle between the prime vector and the line drawn from the storm center to any point on the traverse.

The storm is not stationary, and as it moves towards the coast, the windspeeds may increase and wind vectors will change direction for the shore-intercept point and all other points on the traverse. However, windspeeds are adjusted to account for frictional effects of

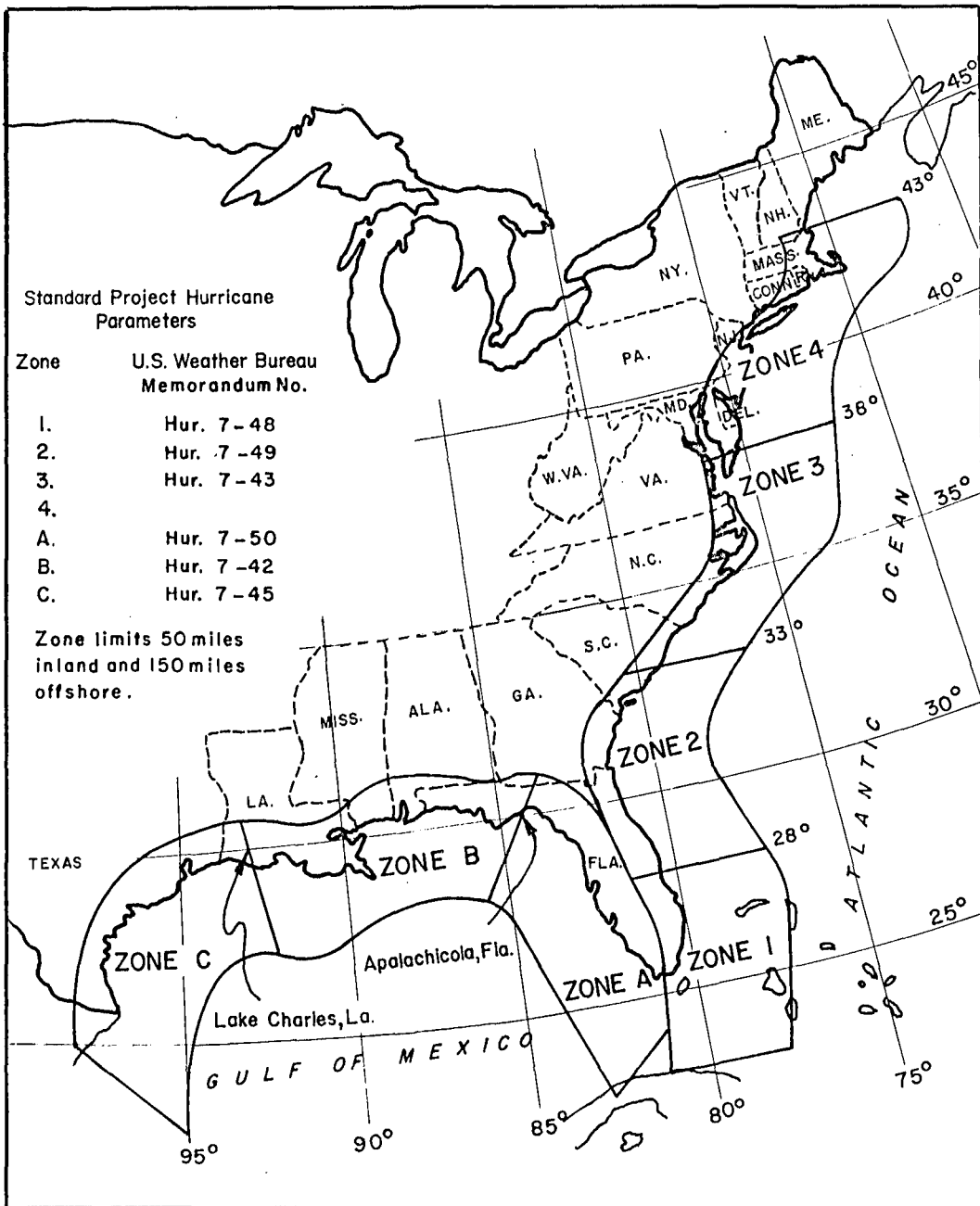


Figure 73. Atlantic and gulf coasts—hurricane zones (U.S. Army, Corps of Engineers, Coastal Engineering Research Center, 1973).

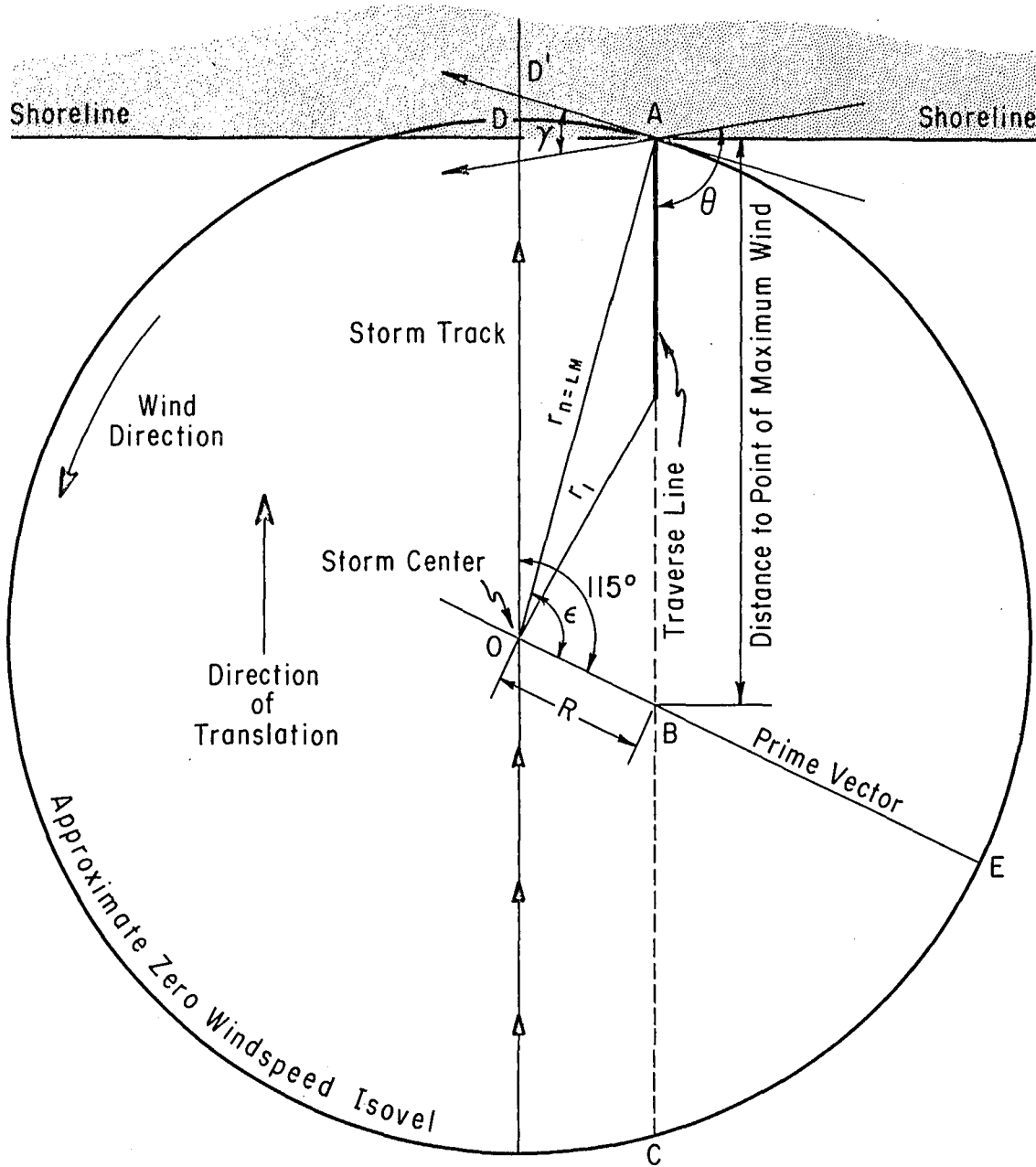


Figure 74. Wind field of a synthetic hurricane at time,  $t = 0$ .

land within 2 nautical miles of the shore; windspeeds are multiplied by a filling factor which varies linearly from 1.00 at 2 nautical miles to 0.89 at the shore. The maximum windspeed is experienced at the shore-intercept of the traverse when point B along the prime vector has moved to position A (Fig. 74). Similarly, the direction of winds along the traverse change as the hurricane moves shoreward. Following passage of the hurricane, the winds decrease, and wind vectors reverse direction.

Windspeeds and directions are obtained for the traverse and extension of the traverse, BC. Considering the geometry of the system and the equations given earlier, the model develops the complete wind field of a synthetic hurricane. Characteristics of a hypothetical hurricane developed internally by the computer program are shown graphically in Figure 75.

The following input parameters are assigned to the hypothetical hurricane: radius of maximum winds, forward speed of the storm, asymptotic pressure, central pressure, maximum windspeed at R, latitude of traverse line at the shore, initial rise, astronomical tide (constant), hurricane zone, and bottom profile data. With these parameters, the program computes the wind profile along the prime vector as a function of the radius of maximum wind and then the wind profile and wind vector angle (wind field) along the traverse line as a function of forward speed. The windspeed is also corrected for filling within 2 nautical miles of shore. The internal computer calculations are in accordance with procedures described in HUR 7-79.

## VII. SUMMARY AND CONCLUSIONS

The prediction of storm surge resulting from the combined meteorologic, oceanic, and astronomic effects coincident with the arrival of a hurricane at the coast is important in planning and the design of coastal structures. Increasing requirements for large coastal installations, such as nuclear powerplants and superport terminals, have required conservative criteria in obtaining estimates of potential storm surges. The present capability for prediction of hurricane surge is based primarily on the use of analytic and numerical models, none of which have been adequately verified. This study was undertaken for the purpose of further verifying a numerical model of hurricane surge prediction using data of historical hurricanes at selected traverses of the Gulf of Mexico and the Atlantic Coast. The numerical model used in this report is based on the *Bathystrophic Storm Tide Theory* and used to estimate the rise of water on the open coast by taking into account the combined effects of direct onshore and alongshore wind-stress components on the water surface and the effects of the Coriolis force to integrate numerically the one-dimensional hydrodynamic equations of motion and continuity.

Verification was achieved empirically in the model by varying the values of coefficients in the wind stress relationship and bottom friction. Surge hydrographs were calculated and compared with the observed or recorded surge hydrographs of the following hurricanes and traverses: (a) Hurricane of 1949 at Galveston and Freeport, Texas; (b) Hurricane Carla at

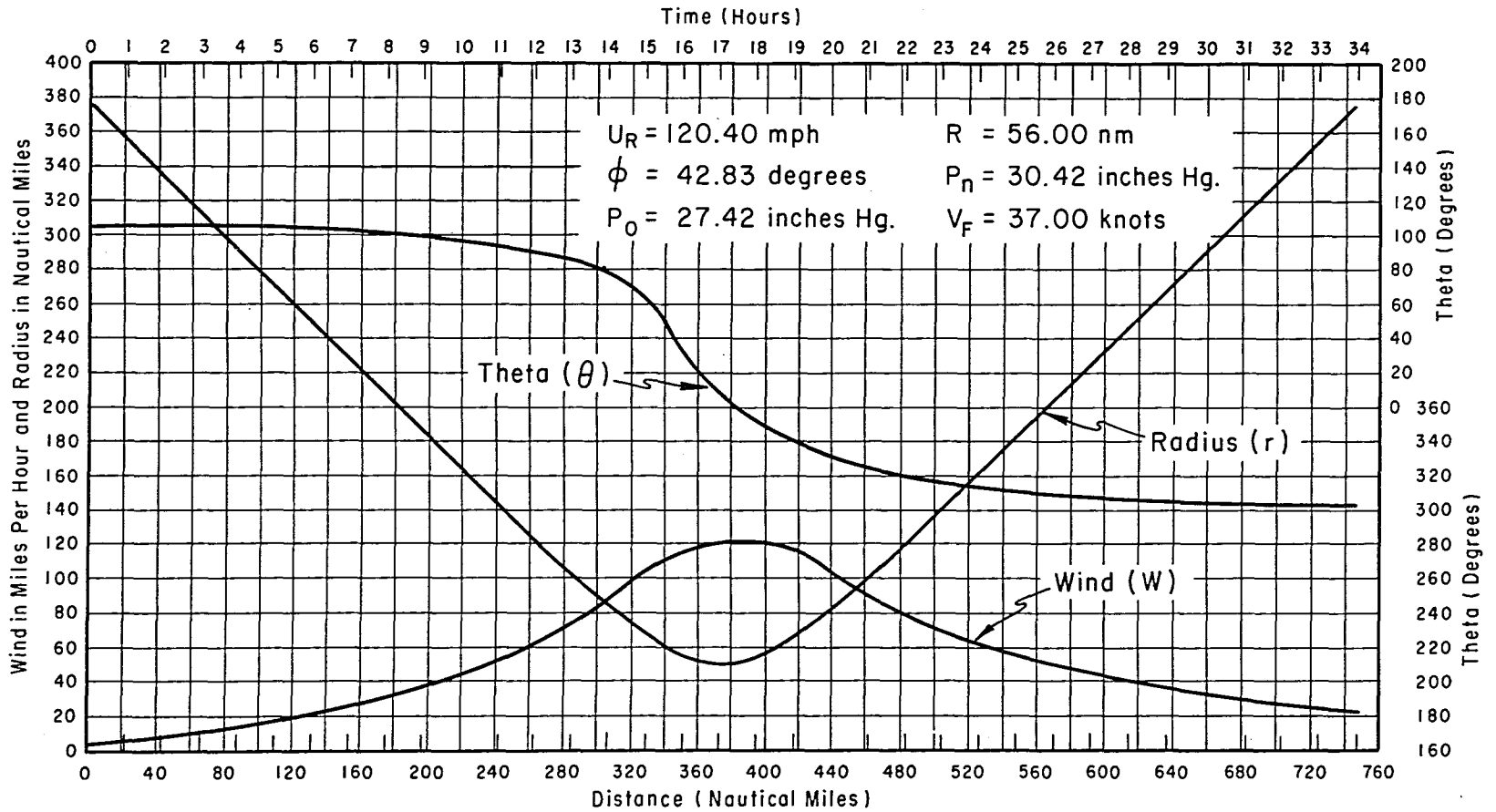


Figure 75. Hypohurricane characteristics (PMH).

Galveston and Freeport, Texas; (c) Hurricane Audrey at Eugene Island, Louisiana; (d) Hurricane Camille at Biloxi, Mississippi; and (e) Hurricane Carol at Narragansett Pier, Rhode Island.

The calibration of the coefficients and verification of the numerical model was somewhat limited by the quantity and quality of the historical hurricane data available. Only the Galveston traverse for Hurricane Carla and the Eugene Island traverse for Hurricane Audrey were satisfactory open-coast locations. Of the two traverses, the Galveston traverse provided the best data for calibration. The computed hydrograph for the Galveston traverse satisfied the *best-fit* condition, while a fair degree of correlation was obtained for the Freeport traverse with data from Hurricane Carla and the Hurricane of 1949. Poor correlation at other traverses with the other hurricanes is attributed to lack of sufficient data and to a deficiency of the numerical model. Where both hurricane wind and hydrographic data were of fair or good quality, and where the condition of open coast was met (Hurricane Carla at Galveston), the model reproduced the recorded historical surges satisfactorily.

The best results were obtained when the coefficients  $K_1$  and  $K_2$  in the wind stress relationship were set equal to  $1.21 \times 10^{-6}$ , and  $2.75 \times 10^{-6}$  respectively, when bottom friction ranged from 0.0025 to 0.003, and when the wind-stress equation was given the following numerical form:

$$K = \left[ 1.21 + 2.75 \left( 1 - \frac{16}{W} \right)^2 \right] 10^{-6} .$$

These results indicate that the numerical model can be used for estimating realistically the values of surges from both actual and synthetic hurricanes. However, certain parameters used in the calculations, such as initial rise, wind stress, and bottom friction, and their interrelationships cannot be adequately determined at the present time. The reasons are the following:

a. No standardized method exists for measuring the values of initial rises associated with historic hurricanes from tide gage records. Such initial rises appear to occur independently of hurricanes.

b. It is generally believed that bottom friction as used in the hurricane surge computations, is a function of depth and offshore bottom topography. No specific method exists for determining a value of bottom friction coefficient for use in the model which is not dependent on other factors.

c. Bottom friction and wind stress are combined in the numerical computation of hurricane surge. However, accurate determination of wind stress is not presently possible. The magnitude of wind-stress coefficient has been determined empirically with actual storms having windspeeds of less than 90 miles per hour. Extrapolation of the wind stress relationship, as determined from lower windspeeds, to extreme probable maximum



conditions may be introducing an error. There is a basis for suggesting that the wind stress magnitude at high windspeeds may be lower than that obtained by extrapolation. Decapitation of the wave tops at high speeds may be the mechanism responsible for lower wind stress values. Correlation of the hurricanes at all traverses was difficult to obtain because of the complexity of the problem, data limitations, the variability and the unknown relationship between wind and bottom stress coefficients, of the factors entering the calibration process. Reliable results obtained with a part of the historical data, support the conclusions that the quasi-two-dimensional bathystrophic storm surge model can be reasonably applied to synthetic hurricane problems; also, the combined values of wind stress and bottom friction coefficients of the bathystrophic numerical model which were obtained through calibration are conservative. However, the results of this study indicated a combination of wind stress and bottom friction coefficients which should be used with the model. Because of the coupled nature of wind stress and bottom friction coefficients, the results of the study did not conclusively determine individual values of these parameters which can be extrapolated to PMH conditions at locations where no historical records exist. Therefore, it is recommended that the most conservative estimates of each parameter be used in the model for computing surges of synthetic hurricanes required for the design of coastal structures; and when additional data becomes available on future hurricanes, further testing and verification of the model be undertaken.

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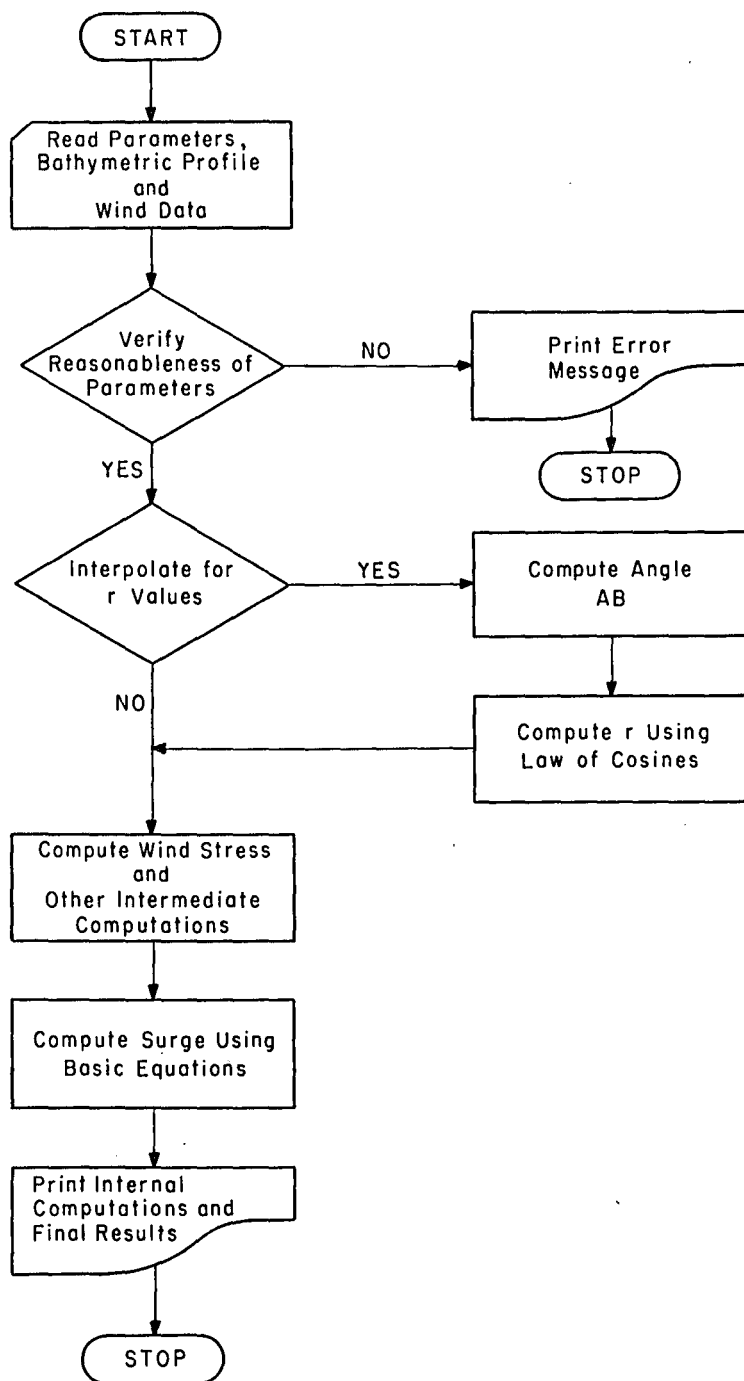
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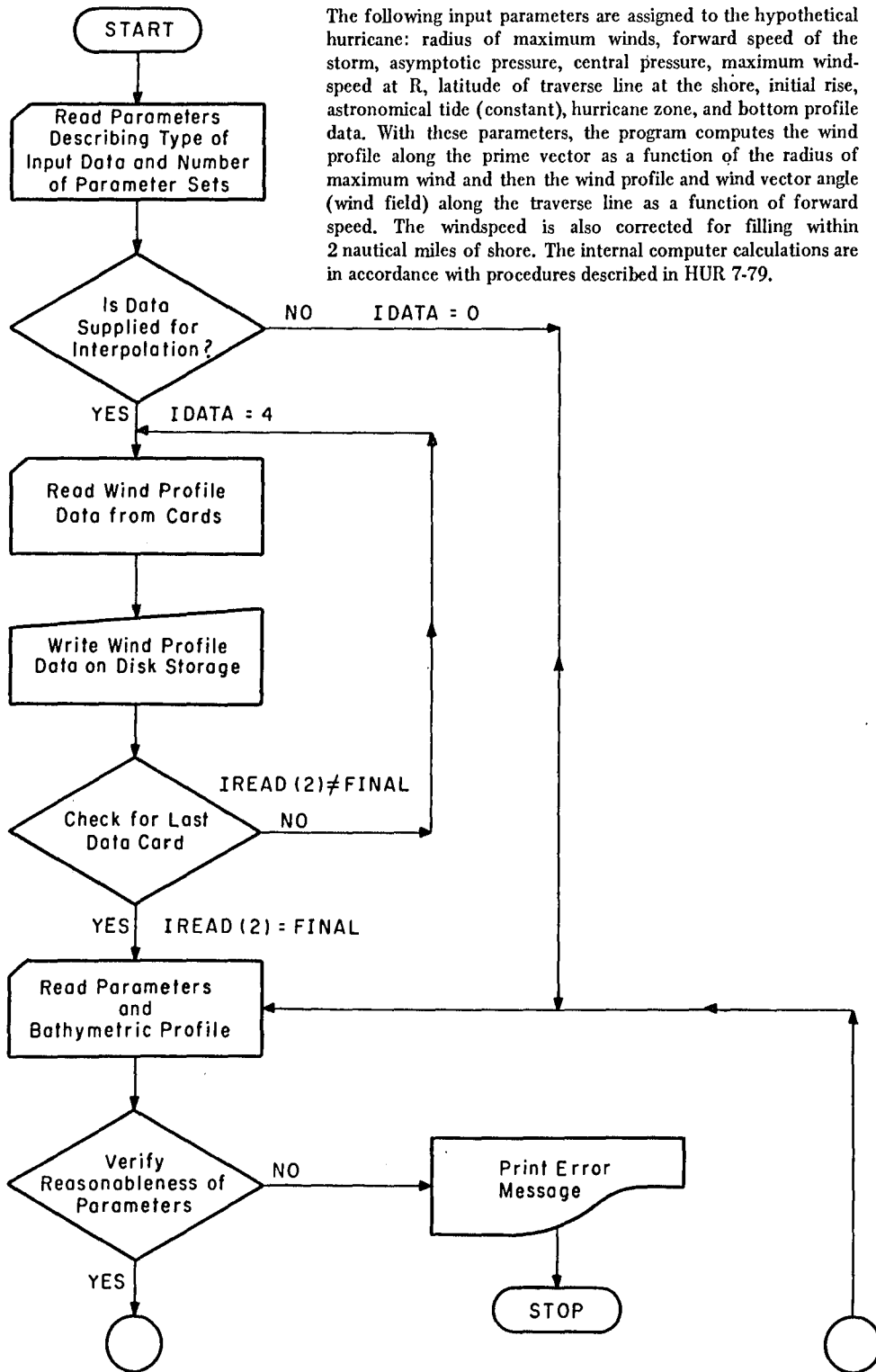
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APPENDIX A. COMPUTER PROGRAM DOCUMENTATION  
APPENDIX A-1. FLOW CHARTS FOR ACTUAL HURRICANES

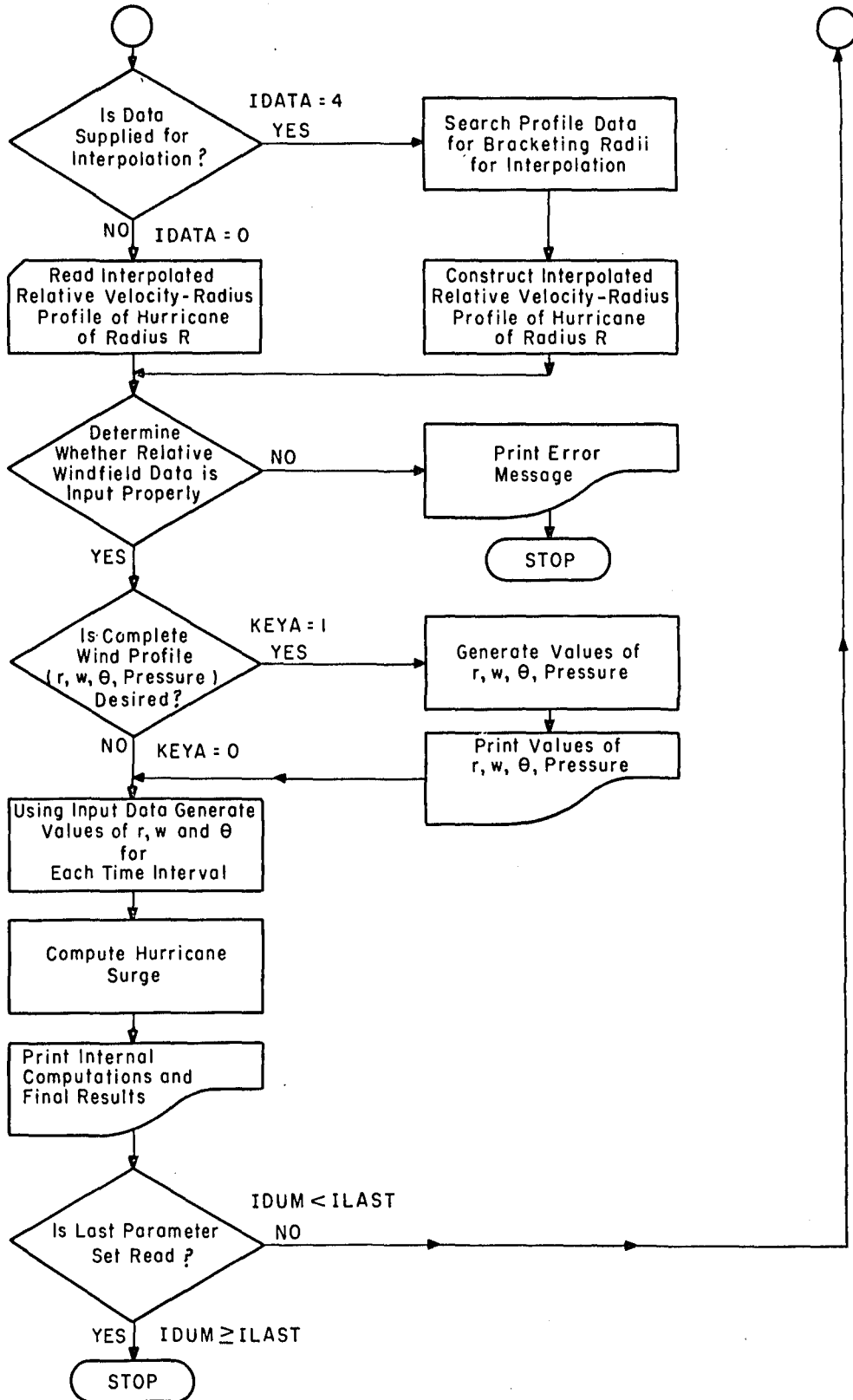


# APPENDIX A-1. FLOW CHARTS FOR HYPOTHETICAL HURRICANES



The following input parameters are assigned to the hypothetical hurricane: radius of maximum winds, forward speed of the storm, asymptotic pressure, central pressure, maximum wind-speed at R, latitude of traverse line at the shore, initial rise, astronomical tide (constant), hurricane zone, and bottom profile data. With these parameters, the program computes the wind profile along the prime vector as a function of the radius of maximum wind and then the wind profile and wind vector angle (wind field) along the traverse line as a function of forward speed. The windspeed is also corrected for filling within 2 nautical miles of shore. The internal computer calculations are in accordance with procedures described in HUR 7-79.





## APPENDIX A-2. PROGRAM LISTINGS FOR ACTUAL HURRICANES

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PROGRAM BURGEA(INPUT,OUTPUT,TAPE5=INPUT,TAPE6=OUTPUT,TAPE39,TAPE38)SGA00100
2)SGA00150
C***** ADPO FILE IDENTIFICATION *****SGA00200
C 752X6R17G0 (BURGEA) COMPUTES STORM BURGE BY BATHYSTROPHIC METHOD SGA00300
C (ACTUAL HURRICANES) SGA00400
C SGA00500
C *****SGA00600
C QUASI-TWO-DIMENSIONAL OPEN COAST STORM BURGE PROGRAM SGA00700
C SGA00800
C SGA00900
C SGA01000
C NOTATION USED A
C AB = CALCULATED ANGLE BETWEEN THE TRAVERSE AND INITIAL STORM SGA01100
C CENTER LINE (RS(1)) SGA01200
C AMOHR = VARIABLE USED TO ESTABLISH STORM TIME. SGA01300
C ALPHA = AZIMUTH OF THE TRAVERSE (DEGREES) SGA01400
C AN = INTERMEDIATE VALUE OF WIND STRESS AND SETUP FOR X-COMPONENT. SGA01500
C BEAR = BEARING OF TRAVERSE IN DEGREES, DOES NOT ENTER COMPUTATIONS. SGA01600
C BETA0 = AZIMUTH OF THE STORM CENTER LINE (RS(1)) (DEGREES) SGA01700
C BFF = BOTTOM FRICTION FACTOR. SGA01800
C BN = INTERMEDIATE VALUE OF WIND STRESS AND SETUP FOR Y-COMPONENT. SGA01900
C BP = AN ARRAY USED TO STORE SUCCESSIVE VALUES OF BN. SGA02000
C BY = ABSOLUTE VALUE OF BN. SGA02100
C C1,C2,C3 = DIMENSION CORRECTION FACTORS. SGA02200
C D = DEPTH BELOW MEAN WATER LEVEL IN FEET. SGA02300
C DASQ = TOTAL WATER DEPTH SQUARED AT EACH TIME LEVEL. SGA02400
C DAVG = AVERAGE BOTTOM DEPTH AT EACH TIME LEVEL. SGA02500
C DELT = TIME INCREMENTS IN HOURS. SGA02600
C DELSXN = INCREMENTAL X-COMP SETUP IN FEET. SGA02700
C DELSYN = INCREMENTAL Y-COMP SETUP IN FEET. SGA02800
C DEN = INTERMEDIATE COMPUTATION FOR DENOMINATOR OF FLUX EQUATION SGA02900
C AT EACH TIME LEVEL SGA03000
C DIST = DISTANCE FROM COAST IN N.M. SGA03100
C DIST(1) AND DEPTH(1) ARE FARTHEST POINTS FROM SHORE WITH SGA03200
C SUCCESSIVE VALUES TAKEN SHORFWARD OF THESE POINTS SGA03300
C DTH = TOTAL WATER DEPTH AT EACH TIME LEVEL (FEET). SGA03400
C DTN = INTERMEDIATE COMPUTATION OF WATER DEPTH SGA03500
C DTS = INTERMEDIATE COMPUTATION OF TOTAL WATER DEPTH (DTH) SGA03600
C ICHECK = SENTINEL USED TO INDICATE ERRORS. SGA03700
C ID = IDENTIFIES TYPE OF DATA ON CARD SGA03800
C IOMITD = PRINT OPTION PARAMETER (DESCRIBED IN DETAIL WHEN REFERENCED) SGA03900
C IOMIT = PRINT OPTION PARAMETER (DESCRIBED IN DETAIL WHEN REFERENCED) SGA04000
C ISKIP = PRINT OPTION SGA04100
C ITYPE = THE FIRST 3 LETTERS OF THE HURRICANE NAME. SGA04200
C LM = MAX. NO. OF SHELF REACHES (LIMIT OF 50) SGA04300
C NM = MAX. NO. OF TIME INCREMENTS (LIMIT OF 50) SGA04400
C NN = CARD NUMBER READ FROM CARD SGA04500
C PHI = GEOGRAPHICAL LATITUDE IN DEGREES GIVEN BY PHII, SGA04600
C PHII = GEOGRAPHICAL LATITUDE IN DEGREES SUPPLIED AS INPUT SGA04700
C PI = 3.14159 SGA04800
C PN = PERIPHERAL PRESSURE IN INCHES OF HG. SGA04900
C PD = CENTRAL PRESSURE INCHES OF HG. SGA05000
C R = RADIUS OF MAX. WINDS IN N.M. SGA05100
C RS = RADII MEASURES FROM STORM CENTER TO POINT ON TRAVERSE IN NM. SGA05200
C RS(1) = DISTANCE (N.M.) FROM STORM CENTER TO SHORE INTERSECTION OF SGA05300
C THE TRAVERSE SGA05400
C SA = SETUP DUE TO ASTRONOMICAL FORCES IN FEET. SGA05500
C SAP = OLD VALUE OF ASTRONOMICAL TIDE SGA05600
C SE = INITIAL SETUP IN FEET. SGA05700
C SINPHI = SINE OF GEOGRAPHICAL LATITUDE SGA05800
C SP = SETUP DUE TO PRESSURE IN FEET. SGA05900
C SPN = PRESSURE SETUP AT NEW TIME LEVEL. SGA06000
C SPP = PRESSURE SETUP AT PREVIOUS TIME LEVEL. SGA06100
C SPRES = SUCCESSIVE VALUES OF PRESSURE SETUP AT EACH TIME LEVEL SGA06200
C ST = SUCCESSIVE VALUES OF TOTAL SURGE ALONG TRAVERSE LINE SGA06300
C STOT = VALUE OF TOTAL SURGE AT SHORE FOR EACH TIME LEVEL. SGA06400
C SUMSX = SUCCESSIVE VALUES OF X-COMPONENT OF SURGE ALONG TRAVERSE SGA06500
C LINE SGA06600
C SUMSY = SUCCESSIVE VALUES OF Y-COMPONENT OF SURGE ALONG TRAVERSE SGA06700
C LINE SGA06800
C SW = SUCCESSIVE VALUES OF SUMSX + SUMSY ALONG TRAVERSE LINE SGA06900
C SWW = SUCCESSIVE VALUES OF SW FOR EACH TIME LEVEL AT SHORE SGA07000
C SX = TOTAL X-COMP SETUP AT SHORE IN FEET. SGA07100
C SXP = SUCCESSIVE VALUES OF SUMSX SGA07200
C SY = TOTAL Y-COMP SETUP AT SHORE IN FEET. SGA07300
C SYP = SUCCESSIVE VALUES OF SUMSY SGA07400
C T = ANGLE MEASURED FROM THE POSITIVE X-AXIS COUNTER CLOCKWISE TO SGA07500
C THE WIND VECTOR (THETA). SGA07600
C VP = FORWARD SPEED OF STORM IN KNOTS. SGA07700
C VN, VNN, VNTST, VP AND VS REFER TO Y-COMPONENT OF VOLUME TRANSPORT SGA07800

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80 C VN = SUCESSIVE VALUES OF FLUX ALONG TRAVERSE LINE SGA07900
C VNN = ABSOLUTE VALUE OF FLUX SGA08000
C VNTEST = CHECK ON VALUE OF FLUX SGA08100
C VP = FLUX AT PREVIOUS TIME LEVEL SGA08200
C VS = Y*FLUX (FT*FT/SFC) SGA08300
85 C W = WIND SPEED IN MPH. SGA08400
C WK = VAN DORN'S WIND STRESS FACTOR. SGA08500
C WKCOR = CORRECTION FACTOR FOR WIND STRESS COEFFICIENT. SGA08600
C WSO = WIND (W) SQUARED SGA08700
C WWX = W * W * COS(THETA) IN MPH SQ. SGA08800
C WWY = W * W * SIN(THETA) IN MPH SQ. SGA08900
C WX = MAXIMUM WIND SPEED (MPH). SGA09000
90 C DIMENSION SP(50),WWX(50),WWY(50),WK(50),D(50),DIST(50),RS(50),
2 W(50),A(20),SA(200),DELT(200),PHI(50),Y(50),DAVG(50),DELT(50),
3 SX(200),SY(200),SXX(200),SPRES(200),STOT(200),SINPHI(50),SXP(100) SGA09300
95 C 4,SYP(100),SPP(100),VP(100),BP(100)
DIMENSION VX(2000),REAR(2) SGA09400
DATA C1,C2,C3 /203.,106.,5280./ SGA09500
C READ STORM IDENTIFICATION TITLE. SGA09600
100 C READ (5,10) (A(I),I=1,20) SGA09700
10 C FORMAT (20A4) SGA09800
C READ PARAMETERS AND CHECK IDENTIFICATION DATA. SGA09900
C SGA10000
C***NOTE. IDENTIFICATION DATA IS CHECKED ON EACH DATA CARD TO SGA10100
C AVOID MISREADING INPUT DATA. SGA10200
105 C SGA10300
C SGA10400
20 READ (5,20) NM,LM,ISKIP,IOMIT,IOMITO,ID SGA10500
C FORMAT (5I5,50X,A5) SGA10600
C FLAG IS SET SGA10700
ICHECK=0 SGA10800
110 C CHECK FOR EXCEEDED DIMENSIONS. SGA10900
IF (NM=200) 30,30,40 SGA11000
30 IF (LM=50) 90,90,40 SGA11100
C SGA11200
C ERROR MESSAGE 01 WRITTEN SGA11300
115 C 40 WRITE (6,730) SGA11400
ERROR=2H01 SGA11500
ICHECK=1 SGA11600
C 50 WRITE (6,50) SGA11700
FORMAT (50X,13HERROR MESSAGE,///) SGA11800
120 C 60 WRITE (6,60) ERROR SGA11900
FORMAT (1X,6HERROR ,A2,/) SGA12000
WRITE (6,70) SGA12100
70 C FORMAT (/,3X,88H SEE FRORR TABLE IN CFRC TM TITLE VERIFICATION STUSGA12200
2DY OF A BATHYSTROPIC STORM SURGE MODEL/) SGA12300
125 C WRITE (6,80) NM,LM SGA12400
80 C FORMAT (1H ,68HMAX, NO. OF TIME INCREMENTS = 200 MAX, NO. OF SSGA12500
SHELF REACHES = 50,/,60H THE VALUE OF TIME INCREMENTS OR SHELF REASGA12600
SHELS WAS TOO LARGE,/,41H YOUR VALUE FOR TIME INCREMENTS(NM) WAS =SGA12700
4,15,/,39H YOUR VALUP FOR SHELF REACHES(LM) WAS =,15) SGA12800
130 C WRITE (6,730) SGA12900
C SGA13000
90 C IF (ID,EQ,5HPAR 1) GO TO 110 SGA13100
C SGA13200
C ERROR MESSAGE 02 WRITTEN SGA13300
135 C IDA=1 SGA13400
WRITE (6,730) SGA13500
ERROR=2H02 SGA13600
C 50 WRITE (6,50) SGA13700
WRITE (6,60) ERROR SGA13800
140 C 60 WRITE (6,70) SGA13900
WRITE (6,100) ID,IDA SGA14000
100 C FORMAT (23H ID IN ERROR * ID GIVEN,1X,A5,/,18H ID SHOULD BE PAR ,ISGA14100
21) SGA14200
WRITE (6,730) SGA14300
145 C SGA14400
C ICHECK=1 SGA14500
110 C LMM=LM=1 SGA14600
PI=3.14159 SGA14700
150 C 120 READ (5,120) PHI,I,TYPE,BEAR,ID SGA14800
FORMAT (10X,F10,2,27X,A3,2A5,15X,A5) SGA14900
IF (ID,EQ,5HPAR 2) GO TO 130 SGA15000
C SGA15100
C ERROR MESSAGE 02 WRITTEN SGA15200
155 C IDA=2 SGA15300
WRITE (6,730) SGA15400
ERROR=2H02 SGA15500
C 50 WRITE (6,50) SGA15600
WRITE (6,60) ERROR SGA15700
WRITE (6,70) SGA15800

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160	WRITE (6,100) ID,IDA WRITE (6,730)	SGA15900 SGA16000
	C	SGA16100
	ICHECK=1	SGA16200
130	DO 140 I=1,LM	SGA16300
165	140 PHI(I)=PHII	SGA16400
	READ (5,150) R,PO,PN,VF,SE,WKCDR,ID	SGA16500
150	FORMAT (5F7,2,21X,F7,2,12X,A5) IF (ID,EQ,5HPAR 3) GO TO 160	SGA16600 SGA16700
	C	SGA16800
170	C ERROR MESSAGE 02 WRITTEN	SGA16900
	IDA=3	SGA17000
	WRITE (6,730)	SGA17100
	ERROR=2H02	SGA17200
	WRITE (6,50)	SGA17300
175	WRITE (6,60) ERROR	SGA17400
	WRITE (6,70)	SGA17500
	WRITE (6,100) ID,IDA	SGA17600
	WRITE (6,730)	SGA17700
	C	SGA17800
180	ICHECK=1	SGA17900
	C READ MAXIMUM WIND VELOCITY AND BOTTOM FRICTION FACTOR.	SGA18000
160	READ (5,170) WX,BFF,ID	SGA18100
170	FORMAT (2F7,1,61X,A5) IF (ID,EQ,5HPAR 4) GO TO 180	SGA18200 SGA18300
	C	SGA18400
185	C ERROR MESSAGE 02 WRITTEN	SGA18500
	IDA=4	SGA18600
	WRITE (6,730)	SGA18700
	ERROR=2H02	SGA18800
190	WRITE (6,50)	SGA18900
	WRITE (6,60) ERROR	SGA19000
	WRITE (6,70)	SGA19100
	WRITE (6,100) ID,IDA	SGA19200
	WRITE (6,730)	SGA19300
195	C	SGA19400
	ICHECK=1	SGA19500
	C READ DISTANCE VALUES AND CHECK IDENTIFICATION DATA.	SGA19600
180	NC=1	SGA19700
	IB=1	SGA19800
200	190 IE=IB+5	SGA19900
	IF (IE,GE,LM) IF=LH	SGA20000
	READ (5,240) ID,NN,(DIST(L),L=IB,IE)	SGA20100
	IF (ID,EQ,6HDIST ,AND,NN,EQ,NC) GO TO 210	SGA20200
	C	SGA20300
205	C ERROR MESSAGE 03 WRITTEN	SGA20400
	WRITE (6,730)	SGA20500
	ERROR=2H03	SGA20600
	WRITE (6,50)	SGA20700
	WRITE (6,60) ERROR	SGA20800
210	WRITE (6,70)	SGA20900
	WRITE (6,200) ID,NN	SGA21000
200	FORMAT (23H ID IN ERROR * ID GIVEN,1X,A6,/,20H ID SHOULD BE DIST 2,12HCARD NUMBER ,I5)	SGA21100 SGA21200
	WRITE (6,730)	SGA21300
215	C	SGA21400
	ICHECK=1	SGA21500
210	IF (IE,GE,LM) GO TO 220	SGA21600
	IB=IB+6	SGA21700
	NC=NC+1	SGA21800
220	GO TO 190	SGA21900
	C READ DEPTH VALUES AND CHECK IDENTIFICATION DATA.	SGA22000
220	IR=1	SGA22100
	NC=1	SGA22200
	IE=IR+5	SGA22300
225	IF (IF,GE,LM) IF=LH	SGA22400
	READ (5,240) ID,NN,(D(L),L=IB,IF)	SGA22500
240	FORMAT (A6,4X,I5,5X,6F10,2) IF (ID,EQ,6HDEPTH ,AND,NN,EQ,NC) GO TO 260	SGA22600 SGA22700
	C	SGA22800
230	C ERROR MESSAGE 04 WRITTEN	SGA22900
	WRITE (6,730)	SGA23000
	ERROR=2H04	SGA23100
	WRITE (6,50)	SGA23200
	WRITE (6,60) ERROR	SGA23300
235	WRITE (6,70)	SGA23400
	WRITE (6,250) ID,NN	SGA23500
250	FORMAT (23H ID IN ERROR * ID GIVEN,1X,A6,/,20H ID SHOULD BE DEPTH 2,12HCARD NUMBER ,I5)	SGA23600 SGA23700
	WRITE (6,730)	SGA23800

240	C		SGA23900
		ICHECK=1	SGA24000
260		IF (IF,GE,LM) GO TO 270	SGA24100
		IB=IB+6	SGA24200
		NC=NC+1	SGA24300
245		GO TO 230	SGA24400
	C		SGA24500
	C		SGA24600
270		REWIND 38	SGA24700
	C	INITIALIZE SENTINEL ITPP AND INDEX VARIABLE NTP	SGA24800
250		ITPP=0	SGA24900
		NTP=LM+1	SGA25000
		DO 500 N=1,NM	SGA25100
	C	READ TIME INCREMENT, ASTRO TIDE, AZIMUTH TRAVERSE, AND AZIMUTH RS(1)	SGA25200
		READ (5,320) IMUR,NN,LX,DELT(N),SA(N),ALPHA,BETA0	SGA25300
255		WHEN ISKIP = 1 OR 2 ORIGINAL WIND FIELD INPUT DATA ARE NOT PRINTED	SGA25400
		IF (ISKIP,EQ,1,OR,ISKIP,EQ,2) GO TO 300	SGA25500
	C	PRINT HEADING, TIME INCREMENT, ASTRO TIDE, AZIMUTH TRAVERSE, AND	SGA25600
	C	AZIMUTH RS(1)	SGA25700
		WRITE (6,280)	SGA25800
260	280	FORMAT (/,16X,7HDELT(N),4X,5HSA(N),4X,5HALPHA,5X,5HBETA0,/)	SGA25900
		WRITE (6,290) IMUR,NN,LX,DELT(N),SA(N),ALPHA,BETA0	SGA26000
	290	FORMAT (1X,A3,I2,I5,4F10,1)	SGA26100
	C	CHECK IDENTIFICATION,	SGA26200
	300	IF (IMUR,EQ,ITYPE,AND,NN,EQ,N) GO TO 310	SGA26300
265			SGA26400
	C	IDENTIFICATION ERROR MESSAGE 05 WRITTEN	SGA26500
		WRITE (6,730)	SGA26600
		ERRROR=2H05	SGA26700
		WRITE (6,50)	SGA26800
270		WRITE (6,60) FRROR	SGA26900
		WRITE (6,70)	SGA27000
		WRITE (6,330) IMUR,NN,LX	SGA27100
		WRITE (6,340) ITYPE,N	SGA27200
		WRITE (6,730)	SGA27300
275			SGA27400
	C		SGA27500
		ICHECK=1	SGA27600
	C		SGA27700
	C		SGA27800
280	310	DO 520 L=1,LM	SGA27900
	C	READ RADII, MAGNITUDES, AND ANGLES FOR WIND FOR SUBSEQUENT TIMES,	SGA28000
		READ (5,320) IMHR,NN,LX,RS(L),W(L),T(L)	SGA28100
	320	FORMAT (A3,I2,I5,4F10,1)	SGA28200
	C	CHECK IDENTIFICATION DATA,	SGA28300
		IF (IMHR,EQ,ITYPE,AND,NN,EQ,N,AND,LX,EQ,L) GO TO 350	SGA28400
285			SGA28500
	C		SGA28600
	C	IDENTIFICATION ERROR MESSAGE 05 WRITTEN	SGA28700
		WRITE (6,730)	SGA28800
		ERRROR=2H05	SGA28900
290		WRITE (6,50)	SGA29000
		WRITE (6,60) FRROR	SGA29100
		WRITE (6,70)	SGA29200
		WRITE (6,330) IMHR,NN,LX,RS(L),W(L),T(L)	SGA29300
	330	FORMAT (25X,18HTHIS CARD IN ERROR,2X,A3,I2,I5,3F10,1)	SGA29400
295		WRITE (6,340) ITYPE,N,L	SGA29500
	340	FORMAT (1X,42HTHSE VALUES SHOULD AGREE WITH THOSE ABOVE,2X,A3,I2,2I5)	SGA29600
		WRITE (6,730)	SGA29700
	C		SGA29800
300		ICHECK=1	SGA29900
	C	CHECK IS MADE TO SEE IF ERRORS WERE ENCOUNTERED IN INPUT DATA	SGA30000
		IF (ICHECK,EQ,1) GO TO 1030	SGA30100
	C		SGA30200
	350	IF (RS(L),GT,.00001) GO TO 510	SGA30300
305			SGA30400
	C		SGA30500
	C	THE FOLLOWING STEPS PERFORM INTERPOLATION OF RS	SGA30600
		WHEN INTERPOLATION PROCEDURE FOR RS IS USED, WIND DATA (RS,W,T)	SGA30700
		MUST BE SUPPLIED IN AN ORDER BEGINNING WITH SHOREWARD MOST POINT	SGA30800
		AND ENDING WITH SEAWARD MOST POINT FOR EACH TIME PERIOD.	SGA30900
310			SGA31000
	C		SGA31100
	C	TO COMPUTE RS GIVEN AZIMUTHS IN DEGREES OF THE TRAVERSE	SGA31200
		AND AZIMUTH IN DEGREES OF THE LINE CONNECTING THE STORM CENTER	SGA31300
		TO THE SHORE INTERSECTION OF THE TRAVERSE	SGA31400
315		AB= CALCULATED ANGLE BETWEEN THE TRAVERSE AND INITIAL STORM	SGA31500
		CENTER LINE (RS(1))	SGA31600
		ALPHA = AZIMUTH OF THE TRAVERSE (DEGREES)	SGA31700
		RS(1) = DISTANCE (N,M.) FROM STORM CENTER TO SHORE INTERSECTION	SGA31800
		OF THE TRAVERSE	

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320 C BETA0 = AZIMUTH OF THE STORM CENTER LINE (RS(1)) (DEGREES) SGA31900
C ITPP=1 SGA32000
C SGA32100
C SGA32200
C CHECK IS MADE TO SEE WHAT QUADRANT ANGLES ALPHA AND BETA0 ARE SGA32300
C LOCATED. ANGLE AR IS CALCULATED AND CONVERTED TO RADIANS. SGA32400
325 C THIS CONVERSION IS DONE BECAUSE THE ARGUMENTS OF TRIG FUNCTIONS SGA32500
C FOR COMPUTER CALCULATION MUST BE IN RADIANS. SGA32600
C SGA32700
C SGA32800
330 C IF (BETA0.LT.0.OR.ALPHA.LT.0.) GO TO 360 SGA32900
C IF (BETA0.GT.360.OR.ALPHA.GT.360.) GO TO 360 SGA33000
C GO TO 380 SGA33100
C SGA33200
C ERROR MESSAGE 06 WRITTEN SGA33300
335 360 WRITE (6,730) SGA33400
C ERROR=2406 SGA33500
C WRITE (6,60) ERROR SGA33600
C WRITE (6,70) SGA33700
C WRITE (6,370) ALPHA,BETA0 SGA33800
340 370 FORMAT (//,1X,89THE AZIMUTHS ALPHA AND BETA0 CANNOT BE GREATER TH SGA33900
C 2AN 360 DEGREES OR BE LESS THAN 0 DEGREES,/,30X,17HALPHA GIVEN WAS SGA34000
C 3=X.F10.1,2X,17HBETA0 GIVEN WAS =F10.1) SGA34100
C WRITE (6,730) SGA34200
C SGA34300
345 GO TO 1030 SGA34400
380 IF (ALPHA.GT.180.) GO TO 420 SGA34500
C IF (BETA0.GT.90.) GO TO 400 SGA34600
390 AB=(BETA0-ALPHA+180.)*PI/180. SGA34700
C GO TO 480 SGA34800
350 400 IF (BETA0.GT.ALPHA) GO TO 410 SGA34900
C GO TO 390 SGA35000
410 AB=(ALPHA-BETA0+180.)*PI/180. SGA35100
C GO TO 480 SGA35200
420 IF (BETA0.GT.ALPHA) GO TO 440 SGA35300
355 IF (BETA0.GT.90.) GO TO 430 SGA35400
C IF ((ALPHA-180.)GT.BETA0) GO TO 470 SGA35500
C AB=(BETA0-ALPHA-180.)*PI/180. SGA35600
C GO TO 480 SGA35700
430 IF (BETA0.LT.180.) GO TO 390 SGA35800
360 IF (BETA0.LT.270.) GO TO 390 SGA35900
440 IF (BETA0.GT.270.) GO TO 450 SGA36000
C IF (BETA0.LT.90.) GO TO 460 SGA36100
C GO TO 410 SGA36200
450 BETA0=BETA0-180. SGA36300
365 AB=(ALPHA+BETA0-180.)*PI/180. SGA36400
C GO TO 480 SGA36500
460 AB=(ALPHA-BETA0+180.)*PI/180. SGA36600
C GO TO 480 SGA36700
470 AB=(ALPHA-BETA0-180.)*PI/180. SGA36800
370 C THE EQUATION AT STATEMENT 480 IS OF THE BASIC FORM SGA36900
C C=(A**2+B**2-2*A*B*CCOS(CC))**,.5 SGA37000
C SGA37100
C LAW OF COSINES SGA37200
C SGA37300
375 480 RS(1)=(DIST(NTP=L)**2.+RS(1)**2.-2.*DIST(NTP=L)*RS(1)*COS(AB))**.5 SGA37400
C WHEN ISKIP = 1 OR 2 COMPUTATIONAL AND RELATED INTERMEDIATE INPUT SGA37500
C DATA ARE NOT PRINTED SGA37600
C IF (ISKIP.EQ.1.OR.ISKIP.EQ.2) GO TO 510 SGA37700
C FUD=COS(AB) SGA37800
380 IF (L.EQ.2) WRITE (6,490) SGA37900
490 FORMAT (/,11X,1HL,5X,4HDIST,5X,5HRS(1),6X,3HFUD,4X,5HRS(L),/) SGA38000
C WRITE (6,500) NTP,L,DIST(NTP=L),RS(1),FUD,RS(L) SGA38100
500 FORMAT (2X,2I5,4F10,3) SGA38200
385 510 T(L)=PI*I(L)/180. SGA38300
520 CONTINUE SGA38400
C SGA38500
C IF (ITPP.EQ.0) GO TO 550 SGA38600
C SGA38700
390 C RE-ORDERS DATA SO MOST SEAWARD POINT IS FIRST AND MOST LANDWARD SGA38800
C POINT IS LAST. SGA38900
C SGA39000
C DO 530 L=1,LM SGA39100
C VX(L)=RS(L) SGA39200
C VX(500+L)=W(L) SGA39300
395 VX(1000+L)=T(L) SGA39400
530 CONTINUE SGA39500
C SGA39600
C DO 540 L=1,LM SGA39700
C RS(L)=VX(NTP=L) SGA39800

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400          W(L)=VX(500+NTP=L)
              T(L)=VX(1000+NTP=L)
540          CONTINUE
C
C          WHEN ISKIP = 2 WIND FIELD DATA IN FINAL FORM ARE NOT PRINTED
405          550 IF (ISKIP.EQ.2) GO TO 580
              DO 570 I=1,LM
                  LX=L
560          IF (L.EQ.1) WRITE (6,560)
              FORMAT (/,'1X,21HACTUAL INPUT OF RS(L),5X,4HW(L),6X,4HT(L),/')
410          WRITE (6,290) IHUR,NN,LX,RS(L),W(L),T(L)
570          CONTINUE
C
580          WRITE (38) (W(L),RS(L),T(L),L=1,LM)
C
415          590 CONTINUE
C
C
C          REWIND 38
              REWIND 39
420          DO 630 N=1,NM
C
C          READ (38) (W(L),RS(L),T(L),L=1,LM)
C
C          DO 610 L=1,LM
              SP(L)=1.14*(PN+PO)*(1.0+EXP(-R/RS(L)))
425          WSG=W(L)*W(L)
              WWX(L)=WSG*COS(T(L))
              WWY(L)=WSG*9JN(T(L))
C          VAN DORN'S WIND STRESS COEFFICIENT IS DETERMINED.
430          IF (W(L).LE.16.0) GO TO 600
              WK(L)=0.0000011+0.0000025*(1.0-16.0/W(L))**2
              GO TO 610
600          WK(L)=0.0000011
435          610 CONTINUE
              DO 620 LL=1,LMM
                  WRITE (39) WWX(LL+1),WWX(LL),WWY(LL+1),WWY(LL),SA(N),SP(LL+1),SP(L
                    2L),WK(LL)
620          CONTINUE
630          CONTINUE
440          WRITE (6,640)
C          THE BASIC INFORMATION OF THE STORM PROBLEM IS WRITTEN OUT.
640          FORMAT (1H1,30X,52HQUASI-TWO-DIMENSIONAL OPEN COAST STORM SURGE PRSGA44100
              20GRAM///)
445          WRITE (6,10) (A(I),I=1,20)
              WRITE (6,450)
650          FORMAT (54X,10HINPUT DATA,/,54X,10H-----,/,/)
              WRITE (6,660) ITYPE
660          FORMAT (/,29X,46HFIRST THREE LETTERS OF HURRICANE NAME (ITYPE) ,A3SGA44700
              2,/,75X,3H---//)
450          WRITE (6,670) PO,PN,R,WX,VF,SE,BFF,WKCOR
670          FORMAT (6X,24HCENTRAL PRESSURE (PO) = ,F5,2.8H IN. HG.,17X,27HPERISGA45000
              2PHERAL PRESSURE (PN) = ,F5,2.8H IN. HG.,/,1X,29HRAIUS TO MAXIMUM SGA45100
              3WIND (R) = ,F4,1.5H N.M.,.28X,20HMAXIMUM WIND (WX) = ,F5,1.7H MI/HRSGA45200
              4,/,5X,25HTRANSLATION SPEED (VF) = ,F4,1.6H KNOTS,27X,20HINITIAL SGA45300
455          5RISE (SF) = ,F4,2.5H FEET,/,1X,31HROTTON FRICTION FACTOR (BFF) = SGA45400
              6,/,F6,4.9X,40HWIND STRESS CORRECTION FACTOR (WKCOR) = ,F4,2,/)
              WRITE (6,680) PHII,RLAR
680          FORMAT (1X,29HGeOGRAPHICAL LATITUDE(PHII) = ,F10,2.21X,26HTRAVERSE SGA45700
              2BEARING (BFAR) = ,2A5,///)
460          WRITE (6,690)
690          FORMAT (40X,41HROTTON PROFILE OVER THE CONTINENTAL SHELF,/,46X,13SGA46000
              2HDISTANCE FROM,AX,5HDEPTH,/,47X,12HSHORE (N.M.),.8X,6H(FEET))
              DO 710 L=1,LM
                  WRITE (6,700) DIST(L),D(L)
465          700          FORMAT (49X,F6,2.9X,F5,1)
710          CONTINUE
              WRITE (6,720) ISKIP,IOMIT,IOMITD
470          720          FORMAT (/,10X,24HINPUT AND OUTPUT CONTROL,/,10X,24(1H-),/,1X,7HISGA46700
              2SKIP = ,I5,67H 0 = ALL WIND FIELD DATA BEFORE AND AFTER INTERPOLATSGA46800
              3ION IS PRINTED,/,15X,57H1 = WIND FIELD DATA USED FOR INTERPOLATIONSGA46900
              4 IS NOT PRINTED,/,15X,35H2 = ALL WIND FIFLD DATA NOT PRINTED,/,1X,78GA47000
              5HOMIT = ,I5,3X,21H0 = DETAILED PRINTING,/,1X,8HOMITD = ,I5,2X,26HISGA47100
              6 = SKIP DETAILED PRINTING,/)
              REWIND 39
475          TII=0.0
              IF (IOMITD.EQ.1) GO TO 790
C          IF IOMITD NOT EQUAL 1, INPUT COMPUTATIONAL DATA PRINTED OUT.
              DO 780 N=1,NM
                  WRITE (6,730)

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480      730  FORMAT (///,1X,119H*****SGA47900
          2*****SGA48000
          3*****)
          TI=TI1
          TII=TI+DELT(N)
485      WRITE (6,740) TI,TII
          740  FORMAT (1X,11H1HTIME (T) = ,F6.2,6X,16HTIME (T+DELT) = ,F6.2,/)
          WRITE (6,750)
          750  FORMAT (10X,8HWWX(I+1),7X,6HWWX(I),7X,8HWWY(I+1),8X,6HWWY(I),5X,11H
          PHASTRO, TIDE,4X,7HSP(I+1),8X,5HSP(I),9X,11H
490      WIND STRESS,/,67X,6H(FEESGA48800
          3T),9X,4H(FT),10X,4H(FT),6X,9HPARAMETER,/)
          DO 770 L=1,LMM
          READ (39) WWX(LL+1),WWX(LL),WWY(LL+1),WWY(LL),SA(N),SP(LL+1),SP(LL)
          2),WK(LL)
          WRITE (6,760) WWX(LL+1),WWX(LL),WWY(LL+1),WWY(LL),SA(N),SP(LL+1),SSGA49300
          2P(LL),WK(LL)
495      760  FORMAT (10X,F7.1,8X,F7.1,7X,F7.1,8X,F7.1,7X,F4.2,10X,F4.2,10X,F4.2,2SGA49500
          2,5X,F10.4)
          770  CONTINUE
          780  CONTINUE
500      C DUMMY CONTINUE
          790  CONTINUE
          REWIND 39
          WRITE (6,800)
          IF (IOMIT.EQ.1) GO TO 820
505      800  FORMAT (1H1)
          WRITE (6,810)
          810  FORMAT (54X,11HOUTPUT DATA,/,54X,11H----- - - - - -,/)
          WRITE (6,660) ITYPE
          WRITE (6,670) PD,PN,R,WX,VF,SE,BFF,WKCOR
510      WRITE (6,680) PHII,REAR
          C DUMMY CONTINUE
          820  CONTINUE
          DO 830 L=1,LMM
          C INCREMENTAL Y-VALUES AND AVG. DEPTHS ARE COMPUTED.
          DFLX(L)=DIST(L)-DIST(L+1)
          DAVG(L)=(D(L)+D(L+1))/2.0
515      830  CONTINUE
          DO 840 L=1,LM
          PHI(L)=(PHI(L)*PI)/180.0
          C THE SINE OF THE LATITUDE IS EVALUATED.
          SINPHI(L)=SIN(PHI(L))
520      840  CONTINUE
          TII=0.0
          DO 970 N=1,NM
          IF (IOMIT.EQ.1) GO TO 850
          WRITE (6,800)
525      C DUMMY CONTINUE
          850  CONTINUE
          SUMSX=0.0
          SUMSY=0.0
          IF (IOMIT.EQ.1) GO TO 870
          TI=TI1
          TII=TI+DELT(N)
          WRITE (6,740) TI,TII
          WRITE (6,860)
535      860  FORMAT (1X,5HDIST,3X,5HDEPTH,5X,6H AVG,3X,9HPRES,RISE,2X,10HASTSGA53500
          PRO,TIDE,3X,7HINITIAL,5X,6HY=FLUX,5X,7HONSHORE,4X,10HALONGSHORE,2X,SGA53600
          310MTOTAL WIND,3X,5HTOTAL,/,1X,4H(NM),3X,8H(FT,MLW),2X,8H(FT,MLW),4SGA53700
          4X,4H(FT),7X,4H(FT),6X,10HLEVEL(FT),1X,11H(FT*FT/SEC),1X,10HSETUP (SGA53800
          5FT),2X,10HSETUP (FT),2X,10HSETUP (FT),2X,10HSURGE (FT),/)
          870  CONTINUE
          DO 950 L=1,LMM
          C STORM SURGE COMPUTATIONS BEGIN.
          READ (39) WWX(L+1),WWX(L),WWY(L+1),WWY(L),SA(N),SP(L+1),SP(L),WK(L)
          2)
          AN=WK(L)*(WWX(L)+WWX(L+1))*WKCOR
          BN=WK(L)*(WWY(L)+WWY(L+1))*0.5*WKCOR
          SPN=(SP(L)+SP(L+1))/2.0
          IF (N.NE.1) GO TO 880
          C PROBLEM IS INIALIZED FOR FIRST TIME LEVEL.
          SXP(L)=0.0
          SYP(L)=0.0
          SPP(L)=SPN
          VP(L)=0.0
          HP(L)=BN
          SAP=SA(N)
          880  DTS=DAVG(L)+8F+SXP(L)+SYP(L)
          DTN=DTS+SA(N)+SPN

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560      DTM=DTS+((SA(N)+SAP)/2.0)+((SPP(L)+SPN)/2.0)          SGA55900
      DASQ=(G3/DTH)**2                                          SGA56000
      DEN=BFF*ARS(VP(L))*DEL(TN)*DASQ                          SGA56100
      VN=((BN+BP(L))*DEL(TN)*0.5+VP(L))/(1.0+DEN)             SGA56200
      BY=ABS(BN)                                                SGA56300
565      VNTEST=SQRT(BY/(BFF*DASQ))                             SGA56400
      VNN=ABS(VN)                                              SGA56500
      C A CHECK IS MADE TO INSURE THAT THE ALONGSHORE DOES NOT EXCEED THE
      C   LIMITING VALUE.
      IF (VNTEST.GT.VNN) GO TO 910
570      IF (VN) 890,900,900                                     SGA56600
      890  VN=VNTEST                                           SGA57000
      GO TO 910                                               SGA57100
      900  VN=VNTEST                                           SGA57200
      910  DELSYN=(C2*DELX(L)*((SINPHI(L)+SINPHI(L+1))*VN)/DTN SGA57300
      DELSXN=(C1*DELX(L)*AN)/DTN                               SGA57400
      SUMSX=SUMSX+DELSXN                                        SGA57500
      SUMSY=SUMSY+DELSYN                                        SGA57600
      VS=7744.0*VN                                             SGA57700
      ST=SA(N)+SE+SUMSX+SUMSY+SPN                              SGA57800
      SW=SUMSX+SUMSY                                           SGA57900
580      IF (IOMIT.EQ.1) GO TO 940                               SGA58000
      C IF IOMIT NOT EQUAL 1, OUTPUT RESULTS ARE PRINTED OUT IN DETAIL FOR
      C   EACH TIME LEVEL.
      WRITE (6,920) DIST(I),D(L)                                SGA58100
585      920  FORMAT (1X,F5.1,5X,F5.1)                            SGA58200
      WRITE (6,930) DAVG(L),8PN,SA(N),SE,VS,SUMSX,SUMSY,SW,ST SGA58300
      930  FORMAT (21X,F5.1,5X,F4.2,8X,F4.2,7X,F4.2,8X,F7.1,5X,F6.3,6X,F7.3,6 SGA58400
      2X,F6.3,5X,F5.2)
      940  SXP(L)=SUMSX                                          SGA58500
590      SYP(L)=SUMSY                                           SGA58600
      SPP(L)=SPN                                                SGA58700
      VP(L)=VN                                                  SGA58800
      BP(L)=BN                                                  SGA58900
      950  CONTINUE                                             SGA59000
595      IF (IOMIT.EQ.1) GO TO 960                               SGA59100
      WRITE (6,920) DIST(LM),D(LM)                             SGA59200
      C NEW VALUES ARE SET EQUAL TO PREVIOUS VALUES.
      960  SX(N)=SUMSX                                          SGA59300
      SY(N)=SUMSY                                              SGA59400
600      SW(N)=SW                                               SGA59500
      SPRES(N)=SPN                                             SGA59600
      STOT(N)=ST                                               SGA59700
      SAP=SA(N)                                                SGA59800
      970  CONTINUE                                             SGA59900
605      K=1                                                    SGA60000
      C DUMMY CONTINUE
      980  CONTINUE                                             SGA60100
      WRITE (6,800)                                             SGA60200
      WRITE (6,990)                                             SGA60300
610      C THE RESULTS ARE SUMMARIZED FOR ALL TIME LEVELS.
      990  FORMAT (3X,41HSUMMARY OF OPEN COAST STORM SURGE PROBLEM,///) SGA60400
      WRITE (6,10) (A(I),I=1,20)                               SGA60500
      WRITE (6,660) ITYPE                                       SGA60600
      WRITE (6,670) PD,PN,R,WX,VF,SE,BFF,WKCOR                SGA60700
      WRITE (6,680) PHI,REAR                                     SGA60800
      WRITE (6,1000)                                           SGA60900
615      1000  FORMAT (///,2X,4HTIME,6X,5HSETUP,5X,5HSETUP,4X,9HTOT, WIND,3X,9H SGA61000
      2ST, TIDE,3X,13HINITIAL WATER,4X,8HPRESSURE,3X,11HTOTAL WATER,/,1X, SGA61100
      37H(HOURS),3X,7HX=COMP,/,3X,7HY=COMP,/,4X,5HSETUP,7X,5HLEVEL,9X,5HLEV SGA61200
620      4EL,10X,5HSETUP,7X,5HLEVEL,/,12X,5H(FT.),5X,5H(FT.),5X,5H(FT.),6X,9 SGA61300
      5H(FT., MLW),6X,5H(FT.),10X,9H(FT.),6X,9H(FT., MLW),//) SGA61400
      C
      AHDUR=0.0                                                SGA61500
      DO 1020 N=1,NM                                           SGA61600
      AHDUR=AHDUR+DEL(TN)                                       SGA61700
625      WRITE (6,1010) AHDUR,SX(N),SY(N),SW(N),SA(N),SE,SPRES(N),STOT(N) SGA61800
      1010  FORMAT (2X,F6.2,5X,F6.2,4X,F6.2,4X,F6.2,6X,F6.2,8X,F6.2,9X,F6.2,7 SGA61900
      2X,F6.2)
      1020  CONTINUE                                           SGA62000
630      IF (K.EQ.3) GO TO 1040                                  SGA62100
      K=K+1                                                     SGA62200
      GO TO 980                                                 SGA62300
      C IF STOP APPEARS IN DAYFILE AN ERROR WAS ENCOUNTERED
      1030  STOP                                               SGA62400
635      C IF STOP 00000 APPEARS IN DAYFILE PROGRAM TERMINATED PROPERLY SGA62500
      C AND OUTPUT WAS GENERATED.
      1040  STOP 00000                                         SGA62600
      END                                                       SGA62700
      SGA62800
      SGA62900
      SGA63000
      SGA63100
      SGA63200
      SGA63300
      SGA63400
      SGA63500
      SGA63600
      SGA63700

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## APPENDIX A-2. PROGRAM LISTINGS FOR HYPOTHETICAL HURRICANES

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PROGRAM SURGE(INPUT,OUTPUT,TAPE5=INPUT,TAPE6=OUTPUT,TAPE39,TAPE4) SRG00100
C 752X6R1590 (SURGE) COMPUTES STORM SURGE BY BATHYSTROPHIC METHOD SRG00200
C QUASI-TWO-DIMENSIONAL OPEN COAST STORM SURGE PROGRAM SRG00300
C SRG00400
5 C ***** SRG00500
C **(SURGE FOR HYPOTHETICAL HURRICANE)** SRG00600
C ***** SRG00700
C SRG00800
C *****REVISIONS***** SRG00900
C SRG01000
10 C ALL REVISIONS SHOULD BE LISTED BELOW BY ADDING A COMMENT CARD SRG01100
C SIMILAR TO THE FIRST REVISION (0), SRG01200
C 0 IN COLUMN 80 INDICATES INITIAL REVISION, SRG01300
C A TO Z IN COLUMN 80 INDICATES SUBSEQUENT REVISIONS, SRG01400
15 C REVISION DATE NAME OF INDIVIDUAL RESPONSIBLE FOR REVISION SRG01500
C SRG01600
C 0 4-29-74 HAROLD D. MCCLUNG SRG01700
C A SRG01800
20 C B SRG01900
C C SRG02000
C D SRG02100
C E SRG02200
C F SRG02300
25 C G SRG02400
C SRG02500
C SRG02600
C NOTATION USED IN ALPHABETICAL ORDER< SRG02700
C AMOUR * VARIABLE USED TO ESTABLISH STORM TIME, SRG02800
C AN * INTERMEDIATE VALUE OF WIND STRESS AND SETUP FOR X-COMPONENT, SRG02900
30 C BEAR * BEARING OF TRAVERSE IN DEGREES, DOES NOT ENTER COMPUTATIONS, SRG03000
C BFF * BOTTOM FRICTION FACTOR, SRG03100
C AN * INTERMEDIATE VALUE OF WIND STRESS AND SETUP FOR Y-COMPONENT, SRG03200
C BP * AN ARRAY USED TO STORE SUCCESSIVE VALUES OF BN, SRG03300
C BY * ABSOLUTE VALUE OF BN, SRG03400
35 C C1,C2,C3 * DIMENSION CORRECTION FACTORS, SRG03500
C D * DEPTH BELOW MEAN WATER LEVEL IN FEET, SRG03600
C DASQ * INTAL WATER DEPTH SQUARED AT EACH TIME LEVEL, SRG03700
C DAVG * AVERAGE BOTTOM DEPTH AT EACH TIME LEVEL, SRG03800
C DELSXN * INCREMENTAL X-COMP SETUP IN FEET, SRG03900
40 C DELSYN * INCREMENTAL Y-COMP SETUP IN FEET, SRG04000
C DELT * TIME INCREMENTS IN HOURS, SRG04100
C DELTT * TIME INCREMENT USED FOR COMPUTATIONS, SRG04200
C DELX * DIFFERENCE BETWEEN TRAVERSE DISTANCES AT EACH TIME LEVEL, SRG04300
C DEN * INTERMEDIATE COMPUTATION FOR DENOMINATOR OF FLUX EQUATION SRG04400
45 C AT EACH TIME LEVEL SRG04500
C DIST * DISTANCE FROM COAST IN N.M, SRG04600
C DIST(1) AND DEPTH(1) ARE FARTHEST POINTS FROM SHORE WITH SRG04700
C SUCCESSIVE VALUES TAKEN SHOREWARD OF THESE POINTS SRG04800
C DISTO * INITIAL DISTANCE ALONG TRAVERSE LINE TO POINT OF MAXIMUM SRG04900
50 C WIND VELOCITY, SRG05000
C DTM * TOTAL WATER DEPTH AT EACH TIME LEVEL (FEET), SRG05100
C DTN * INTERMEDIATE COMPUTATION OF WATER DEPTH SRG05200
C DTS * INTERMEDIATE COMPUTATION OF TOTAL WATER DEPTH (DTM) SRG05300
C E * PARAMETER USED TO CHECK CONVERGENCE SRG05400
55 C EE * PARAMETER USED TO CHECK CONVERGENCE SRG05500
C ICHECK * SENTINEL USED TO INDICATE ERRORS, SRG05600
C ID * IDENTIFIES TYPE OF DATA ON CARD, SRG05700
C IDATA * IDENTIFIES TYPE OF WIND DATA SUPPLIED AS INPUT (DESCRIBED SRG05800
C IN DETAIL WHEN REFERENCED) SRG05900
60 C IDUM * DUMMY VARIABLE NAME SRG06000
C ILAST * INDICATES NUMBER OF DIFFERENT PARAMETER SETS TO BE READ SRG06100
C IOMIT * PRINT OPTION PARAMETER (DESCRIBED IN DETAIL WHEN REFERENCED) SRG06200
C IOMITD * PRINT OPTION PARAMETER (DESCRIBED IN DETAIL WHEN REFERENCED) SRG06300
C IREAD * USED TO READ ALL DIGITIZED WIND DATA ONTO DISK STORAGE SRG06400
65 C ITYPE * THE TYPE OF HURRICANE MODEL, (PMH OR SPH) SRG06500
C IYX * INTERPOLATED WIND DATA GENERATED IN SUBROUTINE PROFIL SRG06600
C IZONE * HURRICANE ZONE SRG06700
C KEY * PRINT OPTION FOR SUBROUTINE HRWDPF (DESCRIBED IN DETAIL WHEN SRG06800
C REFERENCED) SRG06900
70 C KEVA * OPTION WHICH DETERMINES WHETHER SUBROUTINE ZERWIND IS TO BE SRG07000
C CALLED (DESCRIBED IN DETAIL WHEN REFERENCED) SRG07100
C LIST * PRINT OPTION FOR SUBROUTINE PROFIL (DESCRIBED IN DETAIL WHEN SRG07200
C REFERENCED) SRG07300
75 C LM * MAX. NO. OF SHELF REACHES, SRG07400
C NM * MAX. NO. OF TIME INCREMENTS, SRG07500
C NN * CARD NUMBER READ FROM CARD SRG07600
C NPTS * THE NUMBER OF POINTS ALONG THE PMH PROFILE SRG07700
C PHI * GEOGRAPHICAL LATITUDE IN DEGREES GIVEN BY PHII, SRG07800
C PHII * GEOGRAPHICAL LATITUDE IN DEGREES SUPPLIED AS INPUT SRG07900

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80 C PH1IA = LATITUDE LIMIT FOR HURRICANE ZONE SRG08000
C PH1IB = LATITUDE LIMIT FOR HURRICANE ZONE SRG08100
C PI = 3.14159 SRG08200
C PN = PERIPHERAL PRESSURE IN INCHES OF HG. SRG08300
C PO = CENTRAL PRESSURE INCHES OF HG. SRG08400
85 C R = RADIUS OF MAX. WINDS IN N.M. SRG08500
C RK = RADIUS TO MAXIMUM WIND OF WIND FIELD DATA SUPPLIED AS INPUT SRG08600
C RS = RADII MEASURES FROM STORM CENTER TO POINT ON TRAVERSE IN NM. SRG08700
C SA = SETUP DUE TO ASTRONOMICAL FORCES IN FEET. SRG08800
C SAA = VALUE OF ASTRONOMICAL TIDE SUPPLIED AS INPUT SRG08900
90 C SAP = OLD VALUE OF ASTRONOMICAL TIDE SRG09000
C SE = INITIAL SETUP IN FEET. SRG09100
C SINPHI = SINE OF GEOGRAPHICAL LATITUDE SRG09200
C SP = SETUP DUE TO PRESSURE IN FEET. SRG09300
C SPN = PRESSURE SETUP AT NEW TIME LEVEL. SRG09400
C SPP = PRESSURE SETUP AT PREVIOUS TIME LEVEL. SRG09500
95 C SPRES = SUCCESSIVE VALUES OF PRESSURE SETUP AT EACH TIME LEVEL SRG09600
C ST = SUCCESSIVE VALUES OF TOTAL SURGE ALONG TRAVERSE LINE SRG09700
C STOT = VALUE OF TOTAL SURGE AT SHORE FOR EACH TIME LEVEL. SRG09800
C SUMSX = SUCCESSIVE VALUES OF X-COMPONENT OF SURGE ALONG TRAVERSE SRG09900
100 C SUMSY = SUCCESSIVE VALUES OF Y-COMPONENT OF SURGE ALONG TRAVERSE SRG10000
C LINE SRG10100
C SW = SUCCESSIVE VALUES OF SUMSX + SUMSY ALONG TRAVERSE LINE SRG10200
C SWW = SUCCESSIVE VALUES OF SW FOR EACH TIME LEVEL AT SHORE SRG10300
105 C SX = TOTAL X-COMP SETUP AT SHORE IN FEET. SRG10400
C SXP = SUCCESSIVE VALUES OF SUMSX SRG10500
C SY = TOTAL Y-COMP SETUP AT SHORE IN FEET. SRG10600
C SYP = SUCCESSIVE VALUES OF SUMSY SRG10700
C T = ANGLE MEASURED FROM THE POSITIVE X-AXIS COUNTER CLOCKWISE TO SRG10800
110 C THE WIND VECTOR, (THETA). SRG10900
C VF = FORWARD SPEED OF STORM IN KNOTS. SRG11000
C VN, VNN, VNTST, VP AND VS REFER TO Y-COMPONENT OF VOLUME TRANSPORT SRG11100
C VN = SUCCESSIVE VALUES OF FLUX ALONG TRAVERSE LINE SRG11200
C VNN = ABSOLUTE VALUE OF FLUX SRG11300
115 C VNTST = CHECK ON VALUE OF FLUX SRG11400
C VP = FLUX AT PREVIOUS TIME LEVEL SRG11500
C VS = Y-FLUX (MT*FT/SEC) SRG11600
C VX = THE VALUES OF RELATIVE WIND VELOCITY ALONG THE SRG11700
C AXIS OF SYMMETRY OF THE HURRICANE. SRG11800
120 C AXIS OF SYMMETRY IS LINE THROUGH STORM CENTER AND RADIUS TO SRG11900
C MAX WINDS DRAWEN 115 DEGREES FROM DIRECTION OF FORWARD MOTION SRG12000
C W = WIND SPEED IN MPH. SRG12100
C WIND = ADJUSTED WIND SPEED NEAR SHORE DUE TO INTERFERENCE WITH SRG12200
C LAND MASS SRG12300
125 C WK = VAN DORN'S WIND STRESS FACTOR. SRG12400
C WRCOR = CORRECTION FACTOR FOR WIND STRESS COEFFICIENT WHICH SRG12500
C ADJUSTS STRESS FACTORS UPWARD BY 10 PERCENT. THIS ADJUSTMENT SRG12600
C PROVIDES FOR MORE ACCURATE ESTIMATE OF ACTUAL SURGE. SRG12700
C WWSQ = WIND SQUARED SRG12800
130 C WWX = * * W * COS(THETA) IN MPH SQ. SRG12900
C WWY = * * W * SIN(THETA) IN MPH SQ. SRG13000
C VARIABLES NOT DEFINED IN THIS ROUTINE WILL BE DEFINED IN ANOTHER SRG13100
C ROUTINE. SRG13200
135 C DIMENSION SP(50),WWX(50),WWY(50),WK(50),D(50),DIST(50),RS(1500), SRG13300
C 2 *(1500),A(20),SA(200),DELT(200),PHI(50),T(1500),DAVG(50),BEAR(2),SRG13400
C 3 DELX(50),SX(200),SY(200),SWW(200),SPRES(200),STOT(200),VX(1600),SRG13500
C 4 SINPHI(50),SXP(100),SYP(100),SPP(100),VP(100),BP(100),VX(1600) SRG13600
C DIMENSION IREAD(16) SRG13700
140 C COMMON /WIND/ W,T,DELT,DIST,VX,RS SRG13800
C COMMON /PAR/ LM,VF,WX,R,DIST0,XP SRG13900
C COMMON /PTS/ NPTS SRG14000
C COMMON /PFF/ KEY SRG14100
C COMMON /ATM/ PN,PO SRG14200
C COMMON /TES/ HOMB SRG14300
145 C NPTS IS THE NUMBER OF POINTS ALONG THE PMH PROFILE COMMENCING AT SRG14400
C RHO/R = .8, TERMINATING AT RHO/R = 30, SPACED AT INCREMENTS OF .02, SRG14500
C WARNING. DO NOT INCREASE NPTS ABOVE 1600 WITHOUT INCREASING THE SRG14600
C DIMENSION OF VX ACCORDINGLY. OR SOMETHING WILL BE CLOBBEDED. SRG14700
150 C DATA NPTS,E /1461,000005/ SRG14800
C DATA C1,C2,C3 /203,106,5280,/ SRG14900
C DATA WRCOR /1.10/ SRG15000
C READ LOOP PARAMETER AND DATA TYPE PARAMETER SRG15100
C READ (5,10) ILAST,IDATA SRG15200
10 C FORMAT (2I5) SRG15300
155 C INITIALIZE COUNTER IDUM SRG15400
C IDUM=1 SRG15500
C CHECK TO SEE IF DATA SUPPLIED IS TO BE INTERPOLATED. SRG15600
C IF IDATA = 0 THESE STEPS ARE SKIPPED. SRG15700
C IF (IDATA.EQ.0) GO TO 40 SRG15800
SRG15900

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160	C	DATA IS READ IN AND WRITTEN OUT ON DISK REFERENCED BY IDATA	SRG16000
	C	(NORMALLY IDATA = 4 WHEN DATA IS TO BE INTERPOLATED)	SRG16100
	20	READ (5,30) IREAD	SRG16200
	30	FORMAT (16A5)	SRG16300
		WRITE (IDATA,30) IREAD	SRG16400
165		IF (IREAD(2),EQ,5HFINAL) GO TO 40	SRG16500
		GO TO 20	SRG16600
	40	WRITE (6,50) IDUM	SRG16700
	50	FORMAT (1M1,38(1M*),//,1X,36MTHIS IS THE BEGINNING OF RUN NUMBER ,	SRG16800
		22,/,1X,38(1M*),//)	SRG16900
170	C		SRG17000
	C	READ STORM IDENTIFICATION TITLE,	SRG17100
	C		SRG17200
		READ (5,60) (A(I),I=1,20)	SRG17300
	60	FORMAT (20A4)	SRG17400
175	C		SRG17500
	C	READ PARAMETERS AND CHECK IDENTIFICATION DATA,	SRG17600
	C		SRG17700
	C	NOTE, IDENTIFICATION DATA IS CHECKED ON EACH DATA CARD TO	SRG17800
	C	AVOID MISREADING INPUT DATA,	SRG17900
180	C		SRG18000
		READ (5,70) NM,LM,IOMIT,IOMITD,KEY,KEYA,LIST,LD	SRG18100
	70	FORMAT (215,5X,515,35X,A5)	SRG18200
	C		SRG18300
	C	FLAG IS SET	SRG18400
185	C		SRG18500
	C	ICHECK=0	SRG18600
	C		SRG18700
	C	CHECK FOR EXCEEDED DIMENSIONS,	SRG18800
	C		SRG18900
190		IF (NM=200) 80,80,90	SRG19000
	80	IF (LM=50) 130,130,90	SRG19100
	90	WRITE (6,690)	SRG19200
		ICHECK=1	SRG19300
	C		SRG19400
195	C	WRITE ERROR MESSAGE	SRG19500
	C		SRG19600
		WRITE (6,100)	SRG19700
	100	FORMAT (50X,13HERROR MESSAGE,///)	SRG19800
		WRITE (6,110) NM,LM	SRG19900
200	110	FORMAT (1H,6BHMAX, NO. OF TIME INCREMENTS = 200 MAX. NO. OF	SRG20000
		2SHELF REACHES = 50,///,60H THE VALUE OF TIME INCREMENTS OR SHELF REASR	SRG20100
		3CHES WAS TOO LARGE,///,4H YOUR VALUE FOR TIME INCREMENTS(NM) WAS =	SRG20200
		4,15,///,39H YOUR VALUE FOR SHELF REACHES(LM) WAS =,15,///,1X,8HERROR	SRG20300
		501)	SRG20400
205		WRITE (6,120)	SRG20500
	120	FORMAT (3X,8BH SEE ERROR TABLE IN CERC TM TITLE VERIFICATION STUDY	SRG20600
		2 OF A BATHYSTROPIC STORM SURGE MODEL)	SRG20700
		WRITE (6,690)	SRG20800
	C		SRG20900
210	C	CHECK FOR CARD ID	SRG21000
	C		SRG21100
	130	IF (ID,EQ,5HPAR 1) GO TO 150	SRG21200
		IDA=1	SRG21300
	C		SRG21400
215	C	WRITE ERROR MESSAGE	SRG21500
	C		SRG21600
		WRITE (6,690)	SRG21700
		WRITE (6,100)	SRG21800
		WRITE (6,140) ID,IDA	SRG21900
220	140	FORMAT (23H ID IN ERROR * ID GIVEN,1X,A5,/,18H ID SHOULD BE PAR ,I	SRG22000
		21,/,1X,8HERROR 02)	SRG22100
		WRITE (6,120)	SRG22200
		WRITE (6,690)	SRG22300
		ICHECK=1	SRG22400
225	C		SRG22500
	150	LM=LM-1	SRG22600
		PI=3,10159	SRG22700
		READ (5,160) DELTT,PHII,SA,IZONE,ITYPE,BEAR,LD	SRG22800
	160	FORMAT (3F10,2,8X,A2,7X,A3,2A5,15X,A5)	SRG22900
230	C		SRG23000
	C	VERIFY ZONE AND LATITUDE SPECIFICATIONS	SRG23100
	C		SRG23200
		IF (IZONE,NE,2H 1) GO TO 170	SRG23300
235		IF (PHII,GE,25,00,AND,PHII,LE,28,00) GO TO 250	SRG23400
		PHIIA=25,00	SRG23500
		PHIIB=28,00	SRG23600
		GO TO 230	SRG23700
	170	IF (IZONE,NE,2H 2) GO TO 180	SRG23800
		IF (PHII,GE,28,00,AND,PHII,LE,33,00) GO TO 250	SRG23900

240	PHIIA=2A,00	SRG24000
	PHIIB=33,00	SRG24100
	GO TO 230	SRG24200
180	IF (IZONE,NE,2H 3) GO TO 190	SRG24300
	IF (PHII,GE,33,00,AND,PHII,LE,38,00) GO TO 250	SRG24400
245	PHIIA=33,00	SRG24500
	PHIIB=38,00	SRG24600
	GO TO 230	SRG24700
190	IF (IZONE,NE,2H 4) GO TO 200	SRG24800
	IF (PHII,GE,38,00,AND,PHII,LE,43,00) GO TO 250	SRG24900
250	PHIIA=3A,00	SRG25000
	PHIIB=43,00	SRG25100
	GO TO 230	SRG25200
200	IF (IZONE,NE,2H A,AND,IZONE,NE,2H B,AND,IZONE,NE,2H C) GO TO 210	SRG25300
	IF (PHII,GE,25,00,AND,PHII,LE,31,00) GO TO 250	SRG25400
255	PHIIA=25,00	SRG25500
	PHIIB=31,00	SRG25600
	GO TO 230	SRG25700
210	ICHECK=1	SRG25800
	C	SRG25900
260	C WRITE ERROR MESSAGE	SRG26000
	C	SRG26100
	WRITE (6,690)	SRG26200
	WRITE (6,100)	SRG26300
	WRITE (6,220) IZONE	SRG26400
265	220 FORMAT (60H ZONE REQUESTED NOT VALID, VALID ZONES ARE A,B,C,1,2,3	SRG26500
	2AND 4,/,20H YOU REQUESTED ZONE ,A2,/,1X,8HERROR 03)	SRG26600
	WRITE (6,120)	SRG26700
	WRITE (6,690)	SRG26800
	GO TO 250	SRG26900
270	240 ICHECK=1	SRG27000
	C	SRG27100
	C WRITE ERROR MESSAGE	SRG27200
	C	SRG27300
	WRITE (6,690)	SRG27400
275	WRITE (6,100)	SRG27500
	WRITE (6,240) IZONE,PHIIA,PHIIB,PHII	SRG27600
	240 FORMAT (26H LATITUDE LIMITS FOR ZONE ,A2,4H ARE,1X,F5,2,3H TO,1X,F8	SRG27700
	25,2,8H DEGREES,/,24H YOU REQUESTED LATITUDE ,F6,2,/,1X,8HERROR 03	SRG27800
	34)	SRG27900
280	WRITE (6,120)	SRG28000
	WRITE (6,690)	SRG28100
	C	SRG28200
	C CHECK FOR CARD ID	SRG28300
	C	SRG28400
285	250 IF (ID,EQ,5HPAR 2) GO TO 260	SRG28500
	IDA=2	SRG28600
	C	SRG28700
	C WRITE ERROR MESSAGE	SRG28800
	C	SRG28900
290	WRITE (6,690)	SRG29000
	WRITE (6,100)	SRG29100
	WRITE (6,140) ID,IDA	SRG29200
	WRITE (6,120)	SRG29300
	WRITE (6,690)	SRG29400
295	ICHECK=1	SRG29500
	260 DO 270 I=1,LM	SRG29600
	270 PHI(I)=PHII	SRG29700
	DO 280 I=1,NM	SRG29800
	SA(I)=SAA	SRG29900
300	280 DELT(I)=DEFLT	SRG30000
	READ (5,290) R,PD,PN,VF,SE,ID	SRG30100
	290 FORMAT (5F7,2,40X,A4)	SRG30200
	C	SRG30300
	C CHECK FOR CARD ID	SRG30400
	C	SRG30500
305	IF (ID,EQ,5HPAR 3) GO TO 300	SRG30600
	IDA=3	SRG30700
	C	SRG30800
	C WRITE ERROR MESSAGE	SRG30900
	C	SRG31000
310	WRITE (6,690)	SRG31100
	WRITE (6,100)	SRG31200
	WRITE (6,140) ID,IDA	SRG31300
	WRITE (6,120)	SRG31400
315	WRITE (6,690)	SRG31500
	ICHECK=1	SRG31600
	C	SRG31700
	C READ MAXIMUM WIND VELOCITY AND BOTTOM FRICTION FACTOR,	SRG31800
	C	SRG31900

320	300	READ (5,310) WX,BFF, ID	SRG32000
	310	FORMAT (2F7,1+61X+A5)	SRG32100
	C		SRG32200
	C	CHECK FOR CARD ID	SRG32300
	C		SRG32400
325		IF (ID,EQ,5HPAR 4) GO TO 320	SRG32500
		IDA=4	SRG32600
	C		SRG32700
	C	WRITE ERROR MESSAGE	SRG32800
	C		SRG32900
330		WRITE (6,690)	SRG33000
		WRITE (6,100)	SRG33100
		WRITE (6,140) ID,IDA	SRG33200
		WRITE (6,120)	SRG33300
		WRITE (6,690)	SRG33400
335		ICHECK=1	SRG33500
	C		SRG33600
	C	READ DISTANCE VALUES AND CHECK IDENTIFICATION DATA.	SRG33700
	C		SRG33800
	C		SRG33900
340	320	NC=1	SRG34000
		IB=1	SRG34100
	330	IE=IB+5	SRG34200
	C		SRG34300
	C	LOOP LENGTH IS CORRECTED	SRG34400
	C		SRG34500
345		IF (IE,GE,LM) IE=LM	SRG34600
		READ (5,380) ID,NN,(DIST(L),L=IB,IE)	SRG34700
	C		SRG34800
	C	CHECK FOR CARD ID AND CARD NUMBER	SRG34900
	C		SRG35000
350		IF (ID,EQ,6HDIST ,AND,NN,EQ,NC) GO TO 350	SRG35100
	C		SRG35200
	C	WRITE ERROR MESSAGE	SRG35300
	C		SRG35400
355		WRITE (6,690)	SRG35500
		WRITE (6,100)	SRG35600
		WRITE (6,340) ID,NN	SRG35700
	340	FORMAT (23H ID IN ERROR * ID GIVEN,1X,A6,/,20H ID SHOULD BE DIST	SRG35800
		2,12HCARD NUMBER ,15,/,1X,8HERROR 05)	SRG35900
		WRITE (6,120)	SRG36000
360		WRITE (6,690)	SRG36100
		ICHECK=1	SRG36200
	C		SRG36300
	C	CHECK FOR LAST SHELF REACH DISTANCE	SRG36400
	C		SRG36500
365	350	IF (IE,GE,LM) GO TO 360	SRG36600
		IB=IB+6	SRG36700
		NC=NC+1	SRG36800
		GO TO 330	SRG36900
	C		SRG37000
370		READ DEPTH VALUES AND CHECK IDENTIFICATION DATA.	SRG37100
	C		SRG37200
	360	IB=1	SRG37300
		NC=1	SRG37400
	370	IE=IB+5	SRG37500
375	C		SRG37600
	C	LOOP LENGTH IS CORRECTED	SRG37700
	C		SRG37800
		IF (IE,GE,LM) IE=LM	SRG37900
		READ (5,380) ID,NN,(D(L),L=IB,IE)	SRG38000
380	380	FORMAT (A6,4X,15,5X,6F10,2)	SRG38100
	C		SRG38200
	C	CHECK FOR CARD ID AND CARD NUMBER	SRG38300
	C		SRG38400
		IF (ID,EQ,6HDEPTH ,AND,NN,EQ,NC) GO TO 400	SRG38500
385	C		SRG38600
	C	WRITE ERROR MESSAGE	SRG38700
	C		SRG38800
		WRITE (6,690)	SRG38900
		WRITE (6,100)	SRG39000
		WRITE (6,390) ID,NN	SRG39100
	390	FORMAT (23H ID IN ERROR * ID GIVEN,1X,A6,/,20H ID SHOULD BE DEPTH	SRG39200
		2,12HCARD NUMBER ,15,/,1X,8HERROR 06)	SRG39300
		WRITE (6,120)	SRG39400
		WRITE (6,690)	SRG39500
395		ICHECK=1	SRG39600
	C		SRG39700
	C	CHECK FOR LAST SHELF REACH DEPTH	SRG39800
	C		SRG39900
	400	IF (IE,GE,LM) GO TO 410	

400		IH=IB+6	SRG40000
		NC=NC+1	SRG40100
		GO TO 370	SRG40200
	C		SRG40300
	C	READ RELATIVE WIND VELOCITY ALONG AXIS OF SYMMETRY AND CHECK	SRG40400
405	C	IDENTIFICATION DATA.	SRG40500
	C	CHECK IS MADE TO SEE IF ERRORS WERE ENCOUNTERED IN INPUT DATA	SRG40600
	C		SRG40700
410		IF (ICHECK, EQ, 1) GO TO 1020	SRG40800
		IB=1	SRG40900
410		NC=1	SRG41000
		IF=IB+11	SRG41100
	C		SRG41200
	C	LOOP LENGTH IS CORRECTED	SRG41300
	C		SRG41400
415		IF (IE, GE, NPTS) IE=NPTS	SRG41500
	C		SRG41600
	C	CHECK IS MADE TO SEE HOW PROFILE DATA IS SUPPLIED.	SRG41700
	C	IF IDATA = 0 DATA SUPPLIED IS IN INTERPOLATED FORM. SUBROUTINE	SRG41800
	C	PROFIL IS NOT CALLED.	SRG41900
420	C	IF IDATA = 4 DATA SUPPLIED IS A SET OF BRACKETING RADII FOR	SRG42000
	C	HURRICANE ZONE OF INTEREST OR IS COMPLETE SET OF PROFIL DATA	SRG42100
	C	(DATA FOR ALL ZONES AND RADII). WHEN COMPLETE SET OF PROFIL	SRG42200
	C	DATA IS SUPPLIED A SEARCH IS MADE FOR BRACKETING RADII FOR	SRG42300
	C	REQUESTED ZONE.	SRG42400
425	C	WHEN IDATA = 4 INTERPOLATION IS DONE INTERNALLY BY SPECIFYING	SRG42500
	C	ZONE, HURRICANE TYPE, AND RADIUS	SRG42600
	C	TO MAX. WIND,	SRG42700
	C		SRG42800
		IF (IDATA, EQ, 4) GO TO 440	SRG42900
430		READ (5, 430) ID1, ID2, ID3, RR, NN, (VX(I), I=IB, IE)	SRG43000
	430	FORMAT (A4, A2, I, A3, F5.1, I5, I2F5.3)	SRG43100
		GO TO 460	SRG43200
	440	CALL PROFIL (IZONE, ITYPE, R, IVX, IDATA, LIST)	SRG43300
	C		SRG43400
435	C	CHECK IS MADE FOR ERROR FROM SUBROUTINE PROFIL	SRG43500
	C		SRG43600
		IF (HOMB, EQ, 5HERROR) GO TO 660	SRG43700
	C		SRG43800
	C	DATA THAT WAS ROUNDED OFF IN SUBROUTINE PROFIL IS CONVERTED TO	SRG43900
440	C	REAL NUMBERS	SRG44000
	C		SRG44100
		DO 450 I=1, NPTS	SRG44200
		VX(I)=IVX(I)/1000.	SRG44300
	450	CONTINUE	SRG44400
		GO TO 500	SRG44500
445		EE=ABS(RR=R)	SRG44600
	C		SRG44700
	C	CHECK AGREEMENT OF HURRICANE IDENTIFICATION TO SEE IF IT AGREES	SRG44800
	C	WITH DATA IDENTIFICATION	SRG44900
450	C		SRG45000
		IF (ID1, EQ, 4HZONE, AND, ID2, EQ, IZONE, AND, ID3, EQ, ITYPE, AND, EE, LT, E, ANSRG45100	SRG45200
		2D, NN, EQ, NC) GO TO 490	SRG45300
		ICHECK=1	SRG45400
455	C		SRG45500
	C	WRITE ERROR MESSAGE	SRG45600
		WRITE (6, 690)	SRG45700
		WRITE (6, 470)	SRG45800
470		FORMAT (40X, 38MRELATIVE WIND FIELD DATA CARD IN ERROR, /)	SRG45900
460		WRITE (6, 480) ID1, ID2, IZONE, ID3, ITYPE, RR, NN	SRG46000
	480	FORMAT (1X, 34M IZONE I SHOULD = YOUR VALUE OF [(, A4, 2M [(, 4X, 2M [(, A5, 2M [(, 22, 12M [(, 5M HURRICANE TYPE [(, A3, 12M [(, 5M RADIUS TO MAX. WIND =, F5.1, 19M CHECK CARD NUMBER, I5, /, 1X, 8M ER	SRG46100
		4ROR 07)	SRG46200
465		WRITE (6, 120)	SRG46300
			SRG46400
	C		SRG46500
	C	CHECK SENTINEL TO SEE IF ERRORS WERE ENCOUNTERED.	SRG46600
	C		SRG46700
		IF (ICHECK, EQ, 1) GO TO 1020	SRG46800
470	C		SRG46900
	C	CHECK FOR LAST WIND PROFILE POINT	SRG47000
	C		SRG47100
	C		SRG47200
490		IF (IL, EQ, NPTS) GO TO 500	SRG47300
		IH=IH+12	SRG47400
475		NC=NC+1	SRG47500
		GO TO 420	SRG47600
	C		SRG47700
	C		SRG47800
	C	COMPUTE MIN. DIST. FROM TRAVERSE LINE TO HUR. CENTER.	SRG47900

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480 C COMPUTE INITIAL LOCATION ON TRAVERSE LINE, SRG48000
500 CALL STRPT SRG48100
C SRG48200
C SRG48300
C DO 570 N=1,NM SRG48400
485 C SRG48500
C IF KEYA = 1 SUBROUTINE ZERWND IS CALLED, IF KEYA = 0 SUBROUTINE SRG48600
C ZERWND IS NOT CALLED. SRG48700
C SRG48800
C IF (KEYA.EQ.1.AND.N.EQ.1) CALL ZERWND SRG48900
490 CALL HRWDPF (N) SRG49000
C SRG49100
C THE STORMS POSITION RELATIVE TO THE SHORELINE IS DETERMINED, SRG49200
C SRG49300
C DO 550 L=1,LM SRG49400
495 C SRG49500
C SETUP DUE TO THE ATMOSPHERIC PRESSURE DIFFERENTIAL IS EVALUATED, SRG49600
C SRG49700
C SP(L)=1.14*(PN=PO)*(1.0+EXP(*R/RS(L))) SRG49800
C IF (DIST(L)=2.0) 510,510,520 SRG49900
500 C SRG50000
C THE WIND SPEED NEAR SHORE DUE TO THE STORMS INTERFERENCE WITH THE SRG50100
C LAND MASS, SRG50200
C SRG50300
C 510 WIND=W(L)*.89 SRG50400
C GO TO 530 SRG50500
505 C SRG50600
C 520 WIND=W(L) SRG50700
C 530 WSD=WIND*WIND SRG50800
C WXX(L)=WSD*COS(T(L)) SRG50900
C WYY(L)=WSD*SIN(T(L)) SRG51000
510 C IF (W(L),LE,16.0) GO TO 540 SRG51100
C SRG51200
C VAN DORN'S WIND STRESS COEFFICIENT IS DETERMINED. SRG51300
C SRG51400
C WK(L)=0.0000011+0.0000025*(1.0+16.0/W(L))**2 SRG51500
515 C GO TO 550 SRG51600
C 540 WK(L)=0.0000011 SRG51700
C 550 CONTINUE SRG51800
C WRITE (39) DELT(N) SRG51900
C DO 560 LL=1,LM SRG52000
520 C *RITE (39) WXX(LL+1),WXX(LL),WYY(LL+1),WYY(LL),SA(N),SP(LL+1),SP(L) SRG52100
C 2L)WK(LL) SRG52200
C 560 CONTINUE SRG52300
C 570 CONTINUE SRG52400
525 C THE BASIC INFORMATION OF THE STORM PROBLEM IS WRITTEN OUT. SRG52500
C SRG52600
C WRITE (6,580) SRG52700
580 C FORMAT (1M1,30X,52HQUASI=TWO-DIMENSIONAL OPEN COAST STORM SURGE PRSRG52800
C 20GRAM///) SRG52900
530 C WRITE (6,60) (A(I),I=1,20) SRG53000
C WRITE (6,590) SRG53100
590 C FORMAT (54X,10HINPUT DATA,/,54X,10H----- ///) SRG53200
C WRITE (6,600) ITYPE,PHII,IZONE SRG53300
600 C FORMAT (/,36X,32HTYPE OF HURRICANE MODEL (ITYPE) ,A3,/,68X,3H---,/,SRG53400
535 C 2/,1X,30HGEOGRAPHICAL LATITUDE (PHII) =,F10,2,8H DEGREE8,15X,22HHRSRG53500
C 3RICANE ZONE (IZONE),A2,/) SRG53600
C WRITE (6,610) PN,PN,R,WX,VF,SE,BEAR,BFF SRG53700
610 C FORMAT (6X,24HCENTRAL PRESSURE (PO) = ,F5,2,8H IN, HG,017X,27HPERISRG53800
C 2PHERAL PRESSURE (PN) = ,F5,2,8H IN, HG,/,1X,29HRADIUS TO MAXIMUM SRG53900
540 C 3WIND (R) = ,F4,1,5H N,M,/,28X,20HMAXIMUM WIND (WX) = ,F5,1,7H MI/HRSRG54000
C 4,/,5X,25HTRANSLATION SPEED (VF) = ,F4,1,6H KNOTS,27X,20HINITIAL SRG54100
C 5RISE (SE) = ,F4,2,5H FEET,/,4X,26HTRAVERSE BEARING (BEAR) = ,2A5,SRG54200
C 616X,31HBDOTTOM FRICTION FACTOR (BFF) = ,F6,4) SRG54300
C WRITE (6,670) SRG54400
545 C WRITE (6,620) SRG54500
620 C FORMAT (1X,90HSURGE TRAVERSE IS PARALLEL TO STORM TRACK AND 115 DESR54600
C 2GRES FROM THE LINE OF STRONGEST WINDS//) SRG54700
C WRITE (6,630) SRG54800
630 C FORMAT (40X,41HBDOTTOM PROFILE OVER THE CONTINENTAL SHELF,/,46X,13SRG54900
550 C 2HDISTANCE FROM,8X,5HDEPTH,/,47X,12HSHORE (N,M),8X,6H(FEET)) SRG55000
C DO 640 L=1,LM SRG55100
C WRITE (6,650) DIST(L),D(L) SRG55200
640 C CONTINUE SRG55300
650 C FORMAT (49X,F6,2,9X,F5,1) SRG55400
555 C 660 WRITE (6,670) IDATA,IOMIT,IOMITD,KEY SRG55500
C 670 C FORMAT (/,10X,24HINPUT AND OUTPUT CONTROL,/,10X,24(1H=),/,1X,7HISR55600
C 2DATA =15,50H 0 = WIND FIELD DATA INPUT BY CARDS IN FINAL FORM,/,SRG55700
C 315X,57H0 = WIND FIELD DATA INPUT BY CARDS AND TRANSFERED TO DISK,/,SRG55800
C 4,1X,7H1OMIT =15,3X,21H0 = DETAILED PRINTING,/,1X,8H1OMITD =15,2XSRG55900

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560      5,20M1 = SKIP DETAILED PRINTING,///,3X,6MKEY =,15,09M 0 = WIND DATSRG56000
        6A(PROFILE DISTANCE, RADIUS, WIND, AND THETA) NOT PRINTED,/,16X,63M1SRG56100
        7 = WIND DATA(PROFILE DISTANCE, RADIUS, WIND, AND THETA) PRINTED,///)SRG56200
        ,WRITE (6,680) LIST,KEYA,DELT SRG56300
680      FORMAT (3X,6HLIST =,15,42M 0 = DATA GENERATED IN PROFIL NOT PRINTSRG56400
        2ED,/,16X,36M1 = DATA GENERATED IN PROFIL PRINTED,///,3X,6MKEYA =,15SRG56500
        3,34M 0 = SUBROUTINE ZERWIND NOT CALLED,/,16X,31M1 = SUBROUTINE ZERSRG56600
        4WIND IS CALLED,///,1X,6MDELT =,F10,2,1X,33MHOUR INITIAL COMPUTATIONSRG56700
        5N INTERVAL) SRG56800
        C SRG56900
        C CHECK IS MADE FOR ERROR FROM SUBROUTINE PROFIL SRG57000
        C SRG57100
          IF (80MR,EQ,5HERKOR) GO TO 1020 SRG57200
          REWIND 39 SRG57300
          TII=0,0 SRG57400
575      C SRG57500
          IF (IDMITD,EQ,1) GO TO 750 SRG57600
        C IF IDMITD NOT EQUAL 1, INPUT COMPUTATIONAL DATA PRINTED OUT, SRG57700
        C SRG57800
          DO 740 N=1,NM SRG57900
          WRITE (6,690) SRG58000
690      FORMAT (///,1X,119M*****SRG58100
        2*****SRG58200
        3*****) SRG58300
          READ (39) DELT(N) SRG58400
585      TII=TII SRG58500
          TII=TI+DELT(N) SRG58600
          WRITE (6,700) TI,TII SRG58700
700      FORMAT (1X,/,1X11MTIME (T) =,F6,2,6X,16HTIME (T+DELT) =,F6,2,///) SRG58800
          WRITE (6,710) SRG58900
590      710      FORMAT (10X,6MWWW(I+1),7X,6MWWW(I),7X,6MWWW(I+1),8X,6MWWW(I),5X,11SRG59000
        2HASTRO, TIDE,4X,7HSP(I+1),8X,5HSP(I),5X11HWIND STRESS,/,67X,6H(FEESRG59100
        3T),9X,4H(FT),10X,4H(FT),6X,9HPARAMETER,///) SRG59200
          DO 750 LL=1,LMM SRG59300
          READ (39) WWW(LL+1),WWW(LL),WWW(LL+1),WWW(LL),6A(N),SP(LL+1),SP(LL)SRG59400
595      2),WK(LL) SRG59500
          WRITE (6,720) WWW(LL+1),WWW(LL),WWW(LL+1),WWW(LL),3A(N),SP(LL+1),3SRG59600
        2P(LL),WK(LL) SRG59700
720      FORMAT (10X,F7,1,8X,F7,1,7X,F7,1,8X,F7,1,7X,F4,2,10X,F4,2,10X,F4,2SRG59800
        2,5X,F10,8) SRG59900
600      730      CONTINUE SRG60000
        740      CONTINUE SRG60100
        C DUMMY CONTINUE SRG60200
        750      CONTINUE SRG60300
          REWIND 39 SRG60400
605      WRITE (6,760) SRG60500
        C SRG60600
        C CHECK TO SEE IF INPUT COMPUTATIONAL DATA IS PRINTED SRG60700
          IF (IDMIT,EQ,1) GO TO 780 SRG60800
        C SRG60900
610      760      FORMAT (1M1) SRG61000
          WRITE (6,770) SRG61100
770      FORMAT (54X,11HOUTPUT DATA,/,54X,11M-----)SRG61200
          WRITE (6,600) ITYPE,PHI,IZONE SRG61300
          WRITE (6,610) PD,PN,R,WX,VF,8E,BEAR,BFF SRG61400
615      780      CONTINUE SRG61500
          DO 790 L=1,LMM SRG61600
        C SRG61700
        C INCREMENTAL X-VALUES AND AVG. DEPTHS ARE COMPUTED, SRG61800
        C SRG61900
620      DELX(L)=DIST(L)-DIBT(L+1) SRG62000
          DAVG(L)=(D(L)+D(L+1))/2.0 SRG62100
790      CONTINUE SRG62200
          DO 800 L=1,LM SRG62300
          PHI(L)=(PHI(L)*PI)/180.0 SRG62400
625      C SRG62500
        C THE SINE OF THE LATITUDE IS EVALUATED, SRG62600
        C SRG62700
          SINPHI(L)=SIN(PHI(L)) SRG62800
630      800      CONTINUE SRG62900
          TII=0,0 SRG63000
          DO 930 N=1,NM SRG63100
        C SRG63200
        C CHECK TO SEE IF INPUT COMPUTATIONAL DATA IS PRINTED SRG63300
        C SRG63400
635      IF (IDMIT,EQ,1) GO TO 810 SRG63500
          WRITE (6,760) SRG63600
        C DUMMY CONTINUE SRG63700
        810      CONTINUE SRG63800
          READ (39) DELT(N) SRG63900

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640          SUMSX=0.0                      SRG64000
          SUMSY=0.0                      SRG64100
C
C CHECK TO SEE IF INPUT COMPUTATIONAL DATA IS PRINTED SRG64200
C IF (IOMIT,EQ,1) GO TO 630              SRG64300
645      C                                  SRG64400
          C                                  SRG64500
          TI=TII                          SRG64600
          TII=T1+DELT(N)                  SRG64700
          WRITE (6,700) TI,TII            SRG64800
          WRITE (6,820)                    SRG64900
650      820  FORMAT (1X,SHDIST,,3X,SHDEPTH,5X,6HD AVG,,3X,9MPRES,RISE,2X,10MAST SRG65000
          2RD,TIDE,3X,7HINITIAL,5X,6HY+PLUX,5X,7HONSHORE,4X,10HALONGSHORE,2X, SRG65100
          310HTOTAL WIND,3X,5HTOTAL,/,1X,4H(NM),3X,8H(FT,MLW),2X,8H(FT,MLW),4 SRG65200
          4X,4H(FT),7X,4H(FT),6X,10HLEVEL(FT),1X,11H(FT*FT/SEC),1X,10HSETUP (SRG65300
          5FT),2X,10HSETUP (FT),2X,10HSETUP (FT),2X,10HSETUP (FT),//) SRG65400
655      C DUMMY CONTINUE                    SRG65500
          850  CONTINUE                      SRG65600
          DO 910 L=1,LMM                    SRG65700
          C                                  SRG65800
          C                                  SRG65900
660      C STORM SURGE COMPUTATIONS BEGIN, SRG66000
          C                                  SRG66100
          C                                  SRG66200
          C                                  SRG66300
          READ (39) WWW(L+1),WWW(L),WWW(L+1),WWW(L),SA(N),SP(L+1),SP(L),WK(L SRG66400
          2)                                  SRG66500
          AN=WK(L)*(WWW(L)+WWW(L+1))*WKCOR SRG66600
          BN=WK(L)*(WWW(L)+WWW(L+1))*0.5*WKCOR SRG66700
          SPN=(SP(L)+SP(L+1))/2.0           SRG66800
          IF (N,NE,1) GO TO 840             SRG66900
670      C PROBLEM IS INIALIZED FOR FIRST TIME LEVEL. SRG67000
          C                                  SRG67100
          C                                  SRG67200
          SXP(L)=0.0                        SRG67300
          SYP(L)=0.0                        SRG67400
          SPP(L)=SPN                        SRG67500
675      VP(L)=0.0                          SRG67600
          BP(L)=BN                          SRG67700
          SAP=SA(N)                         SRG67800
          840  DTS=DAVG(L)+SE+SXP(L)+SYP(L) SRG67900
          DTH=DTS+SA(N)+SPN                 SRG68000
          DTH=DTH+((SA(N)+SAP)/2.0)+((SPP(L)+SPN)/2.0) SRG68100
          DASQ=(C3/DTH)**2                 SRG68200
          DEN=BFF*ABS(VP(L))*DELT(N)+DASQ  SRG68300
          VN=((BN+BP(L))*DELT(N)+0.5*VP(L))/(1.0+DEN) SRG68400
          BY=ABS(BN)                        SRG68500
          VNTEST=BURT(BY)/(BFF+DASQ)       SRG68600
          VNN=ABS(VN)                      SRG68700
          C                                  SRG68800
          C A CHECK IS MADE TO INSURE THAT THE ALONGSHORE DOES NOT EXCEED THE SRG68900
          C LIMITING VALUE.                 SRG69000
690      C                                  SRG69100
          IF (VNTEST,GT,VNN) GO TO 870     SRG69200
          IF (VN) 850,860,860              SRG69300
          850  VN=VNTEST                    SRG69400
          GO TO 870                         SRG69500
          860  VN=VNTEST                    SRG69600
          870  DELSYN=(C2*DELX(L)*(SINPHI(L)+SINPHI(L+1))*VN)/DTH SRG69700
          DELSXN=(C1*DELX(L)*AN)/DTH      SRG69800
          SUMSX=SUMSX+DELSXN                SRG69900
          SUMSY=SUMSY+DELSYN                SRG70000
          V8=7744.0*VN                      SRG70100
          ST=SA(N)+SE+SUMSX+SUMSY+SPN     SRG70200
          S=SUMSX+SUMSY                     SRG70300
          C                                  SRG70400
          IF (IOMIT,EQ,1) GO TO 900       SRG70500
705      C IF IOMIT NOT EQUAL 1, OUTPUT RESULTS ARE PRINTED OUT IN DETAIL FOR SRG70600
          C EACH TIME LEVEL.               SRG70700
          C                                  SRG70800
          WRITE (6,880) DIST(L),D(L)      SRG70900
          880  FORMAT (1X,F5.1,5X,F5.1)    SRG71000
          WRITE (6,890) DAVG(L),SPN,SA(N),BE,VS,SUMSX,SUMSY,SW,ST SRG71100
          890  FORMAT (21X,F5.1,5X,F4.2,8X,F4.2,7X,F4.2,8X,F7.1,5X,F6.3,6X,F7.3,6 SRG71200
          2X,F6.3,5X,F5.2)                SRG71300
          900  SXP(L)=SUMSX                 SRG71400
          SYP(L)=SUMSY                     SRG71500
          SPP(L)=SPN                       SRG71600
          VP(L)=VN                          SRG71700
          BP(L)=BN                          SRG71800
          910  CONTINUE                      SRG71900
          C

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720      C CHECK TO SEE IF OUTPUT RESULTS ARE TO BE PRINTED
          IF (IUMIT,EQ,1) GO TO 920
      C
          WRITE (6,880) DIST(LM),D(LM)
725      C NEW VALUES ARE SET EQUAL TO PREVIOUS VALUES,
      C
920      SX(N)=SUMSX
          SY(N)=SUMSY
          SWW(N)=SW
730      SPRES(N)=SPN
          STOT(N)=ST
          SAP=SA(N)
930      CONTINUE
          K=1
735      C DUMMY CONTINUE
940      CONTINUE
          WRITE (6,760)
          WRITE (6,950)
      C
740      C THE RESULTS ARE SUMMARIZED FOR ALL TIME LEVELS.
      C
950      FORMAT (38X,41MSUMMARY OF OPEN COAST STORM SURGE PROBLEM,///)
          WRITE (6,960)
960      FORMAT (40X,38MPROJECT ELEMENT 151001 AEC STORM SURGE,///)
745      WRITE (6,60) (A(I),I=1,20)
          WRITE (6,600) ITYPE,PHIL,IZONE
          WRITE (6,610) PD,PN,R,X,VF,SE,BEAR,BFF
          WRITE (6,970)
970      FORMAT (/,20X,74HWIND STRESS EQUATION WK(L) = 0.00000121+ 0.000005
750      2275* (1.0 + 16.0/W(L))**2,/)
          WRITE (6,620)
          WRITE (6,980)
980      FORMAT (,///,2X,4HTIME,6X,5HSETUP,5X,5HSETUP,4X,9HTOT, WIND,3X,9HASR
755      2ST, TIDE,3X,13HINITIAL WATER,4X,8HPRESSURE,3X,11HTOTAL WATER,7X,14SR
          3HTIDE CORRECTED/,1X,7M(HOURS),3X,7HX=COMP,,3X,7HY=COMP,,4X,5HSETUPSRG75500
          4,7X,5HLEVEL,9X,5HLEVEL,10X,5HSETUP,7X,5HLEVEL,10X,17HTOTAL WATER LSRG75600
          SEVEL/,12X,5H(FT.),5X,5H(FT.),5X,5H(FT.),6X,9H(FT., MLW),6X,5H(FT.),SRG75700
          610X,5H(FT.),6X,9H(FT., MLW),11X,9H(FT., MLW)///)
          SRG75800
          AHOUR=0,0
          DO 1000 N=1,NM
          AHOUR=AHOUR+DELT(N)
          WRITE (6,990) AHOUR,SX(N),SY(N),SWW(N),SA(N),SE,SPRES(N),STOT(N)
990      FORMAT (2X,F6,2,5X,F6,2,4X,F6,2,4X,F6,2,6X,F6,2,8X,F6,2,9X,F6,2,7SHG76300
765      2X,F6,2,15X,1H,)
          CONTINUE
          IF (K.EQ.3) GO TO 1010
          K=K+1
          GO TO 940
      C DUMMY CONTINUE
770      1010 CONTINUE
          IF (IDUM,GE,ILAST) STOP 00000
          HE=IND 39
          IDUM=IDUM+1
          GO TO 40
775      1020 STOP
          END

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SUBROUTINE HRWDPF (M)
C
C *****
C
C THIS ROUTINE COMPUTES WIND VELOCITY MAGNITUDE AND DIRECTION ALONG A
C TRAVERSE LINE WHICH PASSES THRU A PROBABLE MAXIMUM HURRICANE AT THE
C POINT WHERE WIND VELOCITY IS GREATEST, THIS TRAVERSE IS 115,
C DEG,COUNTERCLOCKWISE TO THE LINE OF SYMMETRY OF THE P,M,H. ISOVELS.
C
C VARIABLES NOT DEFINED IN THIS ROUTINE WILL BE DEFINED IN ANOTHER
C ROUTINE
C
C THE MAGNITUDE OF THE WIND VELOCITY, W(L), IS ASSUMED TO BE GIVEN
C BY THE FORMULA
C
C  $W(L) = WX * VXX(RHO/R) = .5 * (1. + COS(THETA)) * VF * 1.151$ 
C
C *HERE
C RS(L) AND THETA ARE POLAR COORDINATES OF DISTANCE AND ANGLE RESPECTIVELY,
C (THE INITIAL RAY OF THE COORDINATE SYSTEM IS ALONG THE ISOVEL AXIS OF SYMMETRY
C AND PASSES THRU THE MAXIMUM VELOCITY POINT, THE ORIGIN IS AT THE HURRICANE
C CENTER,)
C AND WHERE VXX IS THE RELATIVE WIND VELOCITY DISTRIBUTION ALONG THE POLAR
C COORD, INITIAL RAY, R IS THE DISTANCE FROM THE ORIGIN TO THE MAXIMUM
C VELOCITY POINT, WX IS THE MAXIMUM WIND VELOCITY, VF IS THE VELOCITY AT
C WHICH THE ENTIRE HURRICANE IS MOVING, (DISTANCES ARE IN NAUTICAL MILES,
C VF IS IN KNOTS, W(L) AND WX ARE IN M,P,H,)
C
C THE VARIABLES THETA AND THETAD ARE USED FOR CLARITY TO ILLUSTRATE
C CHANGES IN THE VALUE OF TH, THESE VARIABLES DO NOT APPEAR IN THE
C ROUTINE, TH IS USED TO REPRESENT THETA AND THETAD, TH IS ORIGINALLY
C THETA IN RADIANS WHICH IS THE POLAR COORDINATE OF LOCATION FOR RS(L),
C TH IS CONVERTED TO THETAD (ANGLE WHICH INCLUDES DEFLECTION),
C THEN TH IS CONVERTED TO THETAD = 25, DEGREES, THE SUBTRACTION OF 25,
C DEGREES WAS A CORRECTION FOR GEOMETRY, THE WIND DIRECTION, THETAD,
C IS GIVEN BY THE FOLLOWING FORMULAS
C
C  $THETAD = THETA + 25$ , IF RS(L)/R LT 1.2
C  $THETAD = THETA + 15 * (RS(L) - R) / (1.2 * R - R) + 10$ , IF 1. LT RS(L)/R LT 1.2
C  $THETAD = THETA + 10 * RS(L) / R$ , IF RS(L)/R LT 1.
C
C (ANGLES ARE IN DEGREES, COUNTERCLOCKWISE IS POS,)
C
C OTHER FORTRAN VARIABLES ARE AS FOLLOWS,
C DEL = DIFFERENCE BETWEEN POINTS ON THE TRAVERSE.
C
C 45 DISTO = INITIAL DISTANCE ALONG TRAVERSE LINE TO POINT OF MAXIMUM
C WIND VELOCITY.
C HR = TIME STEP
C RAD2 = NUMBER OF DEGREES EQUAL TO ONE RADIAN (CONVERSION FACTOR)
C
C X AND Y ARE CARTESIAN COORDINATES CORRESPONDING TO THE POLAR
C COORDINATES, XX(S) ARE RHO/R RATIOS ALONG THE INITIAL RAY,
C VXS ARE DIGITIZED WIND SPEEDS CORRESPONDING TO THE XX(S),
C
C 50 DX AND DY ARE INCREMENTS OF X AND Y ALONG THE VELOCITY PROFILE.
C
C AL IS A DISTANCE ALONG THE VELOCITY PROFILE AT WHICH COMPUTATIONS
C BEGIN,
C
C 55 XP IS THE MIN, DISTANCE FROM THE HURRICANE CENTER TO THE TRAVERSE
C LINE.
C
C
C DIMENSION W(1500),T(1500),DELT(200),DIST(50),RS(1500),VX(1600)
C COMMON /FFF/ KEY
C COMMON /WND/ W,T,DELT,DIST,VX,RS
C COMMON /PAR/ LM,VF,WX,R,DISTO,XP
C
C SN AND CS ARE SIN(115) AND COS(115), RAD1 IS 25 DEG, IN RADIANS,
C
C 65 DATA SN,CS,RAD1,RAD2,PI /,906311,42262,436332,37,29577867,
C 2 3,1415927/
C
C COMPUTE PROFILE PARAMETERS,
C COMPUTE TIME ELAPSED,
C
C 70 HR=0,
C MX=M-1
C IF (MX.LE.0) GO TO 20
C DO 10 L=1,MX
C 10 HR=HR+DELT(L)
C 75 20 AL=DISTO-DIST(1)*HR*VF
C IF (KEY.EQ.1) WRITE (6,30) M,HR
C 30 FORMAT (15H TIME INCREMENT,13,5X,F5.1,6H HOURS)
C COMPUTE INITIAL X AND Y,
C X=R*AL*CS

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80      Y=AL*SN                                HRW08000
C
C      IF (KEY,EQ,0) GO TO 70                   HRW08100
C      PRINT PARAMETERS,                       HRW08200
C      WRITE (6,40) R,VF,WX,XP                HRW08300
85      40  FORMAT (1X,5X,3HR =,F6,2,9H N, MILES,4X,4HVF =,F6,2,6H KNOTS,4X,5HHRW08500
C      2 WX =,F7,2,7H M.P.H./1X,12HX DISTANCE =,F5,2,9H N, MILES) HRW08600
C      WRITE (6,50)                            HRW08700
50      FORMAT (//)                            HRW08800
C      PRINT TITLE,                            HRW08900
90      WRITE (6,60)                            HRW09000
60      FORMAT (1X,6X,13HDISTANCE FROM,6X,6HRADIAL,10X,4HWIND,5X,24HANGLE HRW09100
C      2FROM TRAVERSE LINE/1X,8X,10HCOAST LINE,6X,8HDISTANCE,7X,8HVELOCITYHRW09200
C      3,4X,21HCOUNTERCLOCKWISE POS./1X,9X,8HN, MILES,7X,8HN, MILEB,9X,6HHRW09300
C      4,P.H.,5X,9H(DEGREES))                HRW09400
95      WRITE (6,50)                            HRW09500
C      REPEITIVE COMPUTATION OF WIND VELOCITY AND DIRECTION VERSUS X AND Y,HRW09600
70      DO 90 L=1,LM                           HRW09700
C      RS(L)=SQRT(X**2+Y**2)                   HRW09800
C      CALL QUAD (X,Y,TH)                     HRW09900
100     C TH = ANGLE FROM LINE OF SYMMETRY TO RADIUS VECTOR (RS(L)) HRW10000
C      CALL VEL (RS(L),VXX)                   HRW10100
C      W(L)=WX*VXX*.5*(1.+COS(TH))*VF*1.151 HRW10200
C      CALL DEFL (TH,RS(L))                   HRW10300
C      TH = WIND ANGLE WITH CORRECTION FOR DEFLECTION HRW10400
105     C T(L) = WIND ANGLE - 29 DEGREES FOR GEOMETRY HRW10500
C      T(L)=TH-RAD1                            HRW10600
C      CONVERT WIND VELOCITY ANGLE TO DEG, FROM 115 DEG, LINE, HRW10700
C      TH=RAD2*T(L)                           HRW10800
C
C      IF KEY = 1, WIND VELOCITY PRINTOUT IS INCLUDED, IF KEY = 0, WIND HRW10900
C      VELOCITY PRINTOUT IS SUPRESSED,       HRW11000
C      HRW11100
C      HRW11200
C      IF (KEY,EQ,1) WRITE (6,80) DIST(L),RS(L),W(L),TH HRW11300
80      FORMAT (1X,4F15,2)                    HRW11400
115     IF (L,GE,LM) GO TO 90                  HRW11500
C      DEL=DIST(L)-DIST(L+1)                 HRW11600
C      X=X-DEL*CS                             HRW11700
C      Y=Y-DEL*SN                             HRW11800
90      CONTINUE                              HRW11900
120     IF (KEY,EQ,0) RETURN                   HRW12000
C      WRITE (6,100)                          HRW12100
100     FORMAT (//1X,49HX AXIS IS ALONG HURRICANE ISDVEL AXIS OF SYMMETRY)HRW12200
C      WRITE (6,110)                          HRW12300
110     FORMAT (1H;)                          HRW12400
125     RETURN                                 HRW12500
C      END                                    HRW12600

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C          SUBROUTINE QUAD (X,Y,TH)                                QUD00100
C          *****                                              QUD00200
C          *****                                              QUD00300
5  C          THIS ROUTINE COMPUTES ARCTANGENTS AND DETERMINES THE APPROPRIATE
C          QUADRANT.                                             QUD00400
C          *****                                              QUD00500
C          *****                                              QUD00600
C          *****                                              QUD00700
10 C          VARIABLES NOT DEFINED IN THIS ROUTINE WILL BE DEFINED IN ANOTHER
C          ROUTINE                                             QUD00800
C          *****                                              QUD00900
C          *****                                              QUD01000
C          *****                                              QUD01100
C          *****                                              QUD01200
C          *****                                              QUD01300
15 C          DATA PI /3,1415927/                                QUD01400
C          ERROR IS GIVEN AN INITIAL VALUE,                     QUD01500
C          DATA ERROR /2H00/                                    QUD01600
C          TEST FOR + OR - PI/2                                  QUD01700
C          IF (ABS(X).GT.,00001) GO TO 40                       QUD01800
C          IF (ABS(Y).LT.,00001) ERROR=2H08                    QUD01900
20 C          IF (ERROR,EQ,2H08) GO TO 10                        QUD02000
C          GO TO 40                                             QUD02100
C          PRINT ERROR MESSAGE                                  QUD02200
10 C          WRITE (6,20) ERROR                                 QUD02300
20 C          FORMAT (/1X,38H***** ERROR FROM SUBROUTINE QUAD *****,//1X,6HERR QUD02400
25 C          2OR ,A2)                                           QUD02500
C          WRITE (6,30)                                         QUD02600
30 C          FORMAT (3X,88H SEE ERROR TABLE IN CERC TH TITLE VERIFICATION STUDY QUD02700
C          2 OF A BATHYSTROPIC STORM SURGE MODEL)              QUD02800
C          STOP                                                QUD02900
35 C          TEST FOR QUADRANT,                                  QUD03000
40 C          IF (X.LT,0.,AND,Y.GT,0.) GO TO 60                QUD03100
C          IF (X.EQ,0.,AND,Y.GT,0.) GO TO 70                   QUD03200
C          IF (X.GT,0.,AND,Y.GE,0.) GO TO 80                   QUD03300
C          IF (X.GT,0.,AND,Y.LT,0.) GO TO 90                   QUD03400
35 C          ERROR=2H10                                         QUD03500
C          PRINT ERROR MESSAGE                                  QUD03600
C          WRITE (6,20) ERROR                                    QUD03700
C          WRITE (6,30)                                         QUD03800
40 C          WRITE (6,50) X,Y                                   QUD03900
50 C          FORMAT (1X,2F10,3)                                QUD04000
C          STOP                                                QUD04100
C          *****                                              QUD04200
C          *****                                              QUD04300
45 C          2ND QUADRANT,                                       QUD04400
60 C          TH=PI-ATAN(-Y/X)                                   QUD04500
C          GO TO 100                                            QUD04600
70 C          TH=PI/2.                                           QUD04700
C          GO TO 100                                            QUD04800
C          *****                                              QUD04900
50 C          1ST QUADRANT,                                       QUD05000
80 C          TH=ATAN(Y/X)                                       QUD05100
C          GO TO 100                                            QUD05200
C          *****                                              QUD05300
C          *****                                              QUD05400
90 C          4TH QUADRANT,                                       QUD05500
C          TH=-ATAN(-Y/X)                                       QUD05600
C          TEST FOR REASONABLENESS,                             QUD05700
C          THE VALUE OF THETA (TH) SHOULD BE BETWEEN 115, DEG, AND
55 C          =65, DEG, ALONG THE WIND VELOCITY PROFILE.        QUD05800
100 C          IF (TH.LT,(115./180.)*PI,AND,TH.GT,(65./180.)*PI) RETURN
C          ERROR=2H11                                           QUD05900
C          PRINT ERROR MESSAGE                                  QUD06000
C          WRITE (6,20) ERROR                                    QUD06100
60 C          WRITE (6,30)                                         QUD06200
C          WRITE (6,50) TH                                       QUD06300
C          STOP                                                QUD06400
C          END                                                  QUD06500

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      SUBROUTINE DEFL (TH,RHO)
C
C *****
5  THIS ROUTINE COMPUTES THE WIND DIRECTION AS A FUNCTION OF THETA
C   AND RHO/R.  THE FORMULAS ARE EXPLAINED ELSEWHERE.
C
C   VARIABLES NOT DEFINED IN THIS ROUTINE WILL BE DEFINED IN ANOTHER
C   ROUTINE
10  RHO = RADIAL DISTANCE FOR PRESENT POINT ON TRAVERSE
C     COMMON /PAR/ LM,VF,WX,R,DISTO,XP
C     DATA PI /3.1415927/
15  IF (RHO.GE.1.2*R) GO TO 10
C     IF (RHO.LT.R) GO TO 20
C     TH=TH*(10./180.)*PI+(15./180.)*PI*(RHO=R)/(1.2*R)
C     RETURN
10  TH=TH*(25./180.)*PI
C     RETURN
20  TH=TH*(RHO/R)*(10./180.)*PI
C     RETURN
C     END
DEF00100
DEF00200
DEF00300
DEF00400
DEF00500
DEF00600
DEF00700
DEF00800
DEF00900
DEF01000
DEF01100
DEF01200
DEF01300
DEF01400
DEF01500
DEF01600
DEF01700
DEF01800
DEF01900
DEF02000
DEF02100
DEF02200
DEF02300

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      SUBROUTINE VEL (RHO,VXX)
C
C *****
5  THIS ROUTINE MAKES AN INTERPOLATION OF WIND VELOCITY ALONG THE
C   P,M,H. AXIS OF SYMMETRY.
C
C   VARIABLES NOT DEFINED IN THIS ROUTINE WILL BE DEFINED IN ANOTHER
C   ROUTINE
10  RHO = RADIAL DISTANCE FOR PRESENT POINT ON TRAVERSE
C     DIMENSION W(1500),T(1500),DELT(200),DIST(50),RS(1500),VX(1600)
C     COMMON /WND/ W,T,DELT,DIST,VX,RS
15  COMMON /PAR/ LM,VF,WX,R,DISTO,XP
C     COMMON /PTS/ NPTS
C
C   XX AND XI ARE DISTANCES ALONG THE P,M,H. AXIS OF SYMMETRY,
C   XI=RHO/R
20  CHECK RHO/R FOR REASONABLENESS.
C     IF (XI.GT..84.AND.XI.LT.29.1) GO TO 50
C   PRINT ERROR MESSAGE
10  WRITE (6,20)
20  FORMAT (/,'1X,37H***** ERROR FROM SUBROUTINE VEL *****//,'1X,8HROV
25  2R 12)
C   WRITE (6,30)
30  FORMAT (3X,88H SEE ERROR TABLE IN CERC TM TITLE VERIFICATION STUDY
C     2 OF A BATHYSTROPIC STORM SURGE MODEL)
C   WRITE (6,40),XI
40  FORMAT (1X,4HXI =,F10.3,19H UNREASONABLE INPUT)
30  VXX=-999.
C     STOP
C   SEARCH FOR APPROPRIATE VX POINTS,
50  DO 60 I=1,NPTS
C     XX=.78+.02*I
35  IF (XI.GT,XX) GO TO 60
C     M=I
C     GO TO 70
60  CONTINUE
C     GO TO 10
40  C
70  XM=50.*(VX(M)-VX(M-1))
C     VXX=XM*(XI-XX)+VX(M-1)
C     RETURN
C     END
VEL00100
VEL00200
VEL00300
VEL00400
VEL00500
VEL00600
VEL00700
VEL00800
VEL00900
VEL01000
VEL01100
VEL01200
VEL01300
VEL01400
VEL01500
VEL01600
VEL01700
VEL01800
VEL01900
VEL02000
VEL02100
VEL02200
VEL02300
VEL02400
VEL02500
VEL02600
VEL02700
VEL02800
VEL02900
VEL03000
VEL03100
VEL03200
VEL03300
VEL03400
VEL03500
VEL03600
VEL03700
VEL03800
VEL03900
VEL04000
VEL04100
VEL04200
VEL04300
VEL04400

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SUBROUTINE STRPT          STR00100
C                          STR00200
C *****                STR00300
C                          STR00400
5 C THIS SUBROUTINE COMPUTES THE RADIAL DISTANCE AT WHICH WIND VELOCITY STR00500
C FOR A P.M.H. EQUALS 0. THIS IS USED TO COMPUTE THE STARTING POINT STR00600
C ON THE TRAVERSE LINE.   STR00700
C                          STR00800
C VARIABLES NOT DEFINED IN THIS ROUTINE WILL BE DEFINED IN ANOTHER STR00900
10 C ROUTINE                STR01000
C                          STR01100
C DISTO = INITIAL DISTANCE ALONG TRAVERSE LINE TO POINT OF MAXIMUM STR01200
C WIND VELOCITY.          STR01300
C RHO = RADIAL DISTANCE AT WHICH WIND VELOCITY EQUALS ZERO        STR01400
15 C YPP = DISTANCE ALONG TRAVERSE FROM POINT OF MAXIMUM WIND TO POINT STR01500
C WHERE XP INTERSECTS TRAVERSE LINE. STR01600
C DIMENSION W(1500),T(1500),DELT(200),DIST(50),RB(1500),VX(1600) STR01700
C COMMON /PAR/ LM,VF,WX,R,DISTO,XP STR01800
C COMMON /PTS/ NPTS STR01900
20 C COMMON /WIND/ W,T,DELT,DIST,VX,RS STR02000
C SN AND CS ARE SIN(115) AND COS(115). STR02100
C DATA SN,CS /,90631,42262/ STR02200
C                          STR02300
C                          STR02400
25 C                          STR02500
C                          STR02600
C DO TO I=1,NPTS STR02700
C IF (VX(I).GT,VXZ) GO TO 70 STR02800
C COMPUTE RADIAL DISTANCE AT WHICH WIND VELOCITY EQUALS 0. STR02900
30 C RHO=.76+I*.02 STR03000
C CHECK TO AVOID DIVISION BY ZERO. STR03100
C EE=ABS(VX(I)-VX(I-1)) STR03200
C IF (EE.GT,.00001) GO TO 50 STR03300
C EE=ABS(VXZ-VX(I-1)) STR03400
35 C IF (EE.LT,.00001) GO TO 40 STR03500
C PRINT ERROR MESSAGE STR03600
10 C WRITE (6,10) STR03700
C FORMAT (/,'IX,40H***** ERROR FROM SUBROUTINE STRPT *****//,'IX,8H) STR03800
C 2RROR 13) STR03900
40 C WRITE (6,20) STR04000
C 20 FORMAT (3X,88H SEE ERROR TABLE IN CERC TM TITLE VERIFICATION STUDY STR04100
C 2 OF A BATHYMETROPIC STORM SURGE MODEL) STR04200
30 C WRITE (6,30) VX(I-1),VX(I),VXZ,I STR04300
C FORMAT (1X,25HTILT YOU DIVIDED BY ZERO,/'F10,5,I5) STR04400
45 C STOP STR04500
C                          STR04600
40 C XH=.5 STR04700
C GO TO 60 STR04800
50 C XH=(VXZ-VX(I-1))/(VX(I)-VX(I-1)) STR04900
C MAKE FINAL INTERPOLATION. STR05000
50 C RHO=RHO+XH*.02 STR05100
C RHO IS INITIAL DIST. FROM P.M.H. CENTER. STR05200
C RHO=RHO*R STR05300
C GO TO 80 STR05400
70 C CONTINUE STR05500
55 C                          STR05600
80 C YPP=R*CS STR05700
C XP IS THE HORIZONTAL DISTANCE FROM THE P.M.H. CENTER TO THE STR05800
C TRAVERSE LINE. STR05900
60 C XP=R*SN STR06000
C COMPUTE INITIAL POINT ALONG TRAVERSE LINE. STR06100
C DISTO=YPP+SQRT(RHO**2-XP**2) STR06200
90 C WRITE (6,90) RHO,DISTO STR06300
C FORMAT (/,'IX,61HRADIAL DISTANCE AT WHICH WIND VELOCITY FOR A P.M.H. STR06400
65 C 2 EQUALS 0.,'F9,1/1H ,73HINITIAL DISTANCE ALONG TRAVERSE LINE FROM PSTR06500
C 3DINT OF MAX. WIND VEL. EQUALS ,F9,1) STR06600
C WRITE (6,100) STR06700
100 C FORMAT (1H057HRADIAL DISTANCE MEANS DISTANCE FROM CENTER OF HURRICSTR06800
C 2ANE, ) STR06900
70 C WRITE (6,110) STR07000
110 C FORMAT (1X,65HDISTANCE FROM COASTLINE MEANS DISTANCE PARALLEL TO TSTR07100
C 2RAVERSE LINE, ) STR07200
C WRITE (6,120) STR07300
120 C FORMAT (1H1) STR07400
C RETURN STR07500
75 C END

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          SUBROUTINE PROFIL (IZONE,ITYPE,RR,IRR,IDATA,LIST)
C
C *****
C
C FORMERLY PROGRAM PROFIL
C
C 7225BR12M0 (PROFIL) INTERPOLATION OF HURRIANE VELOCITY PROFILES.
C DIMENSION R(2),IRR(2000),VR2(2,750),XR2(2,750),VTMP(2,2000),XXA(2)
C 2,YYA(2),AX(2),IAX(16)
C DIMENSION ERR(9),DUM(16)
C COMMON /PTS/ NPTS
C COMMON /TES/ BOMB
C REWIND IDATA
C
C VARIABLES NOT DEFINED IN THIS ROUTINE WILL BE DEFINED IN ANOTHER
C ROUTINE
C
C THIS PROGRAM INTERPOLATES A PROFILE FOR A PMH WITH SIZE PARAMETER,
C RR, BETWEEN 2 DIGITIZED PMH PROFILES WITH SIZE PARAMETERS, R(1)
C AND R(2) AND PRINTS THE OUTPUT IF REQUESTED. THIS OUTPUT IS PART OF
C THE INPUT FOR PROGRAM SURGE.
C
C RELATIVE VELOCITY VALUES ARE INTERPOLATED ALONG THE PMH PROFILES
C WHICH WERE READ IN, AT INTERVALS OF .02. THESE VALUES ARE STORED
C IN THE VTMP ARRAY, THEN THE VALUES STORED IN VTMP ARE USED TO MAKE
C INTERPOLATIONS FOR THE CONSTRUCTION OF A PMH WITH A SIZE
C PARAMETER OF RR NAUTICAL MILES. THESE FINAL INTERPOLATIONS ARE
C STORED IN IRR.
C
C CAUTION. THE LOGIC OF THIS PROGRAM IS RATHER DELICATE IN
C PLACES, BEFORE MAKING ANY PROGRAM CHANGES IT WOULD
C BE WISE TO READ THE WARNINGS INDICATED BELOW.
C
C DEFINITION OF FORTRAN VARIABLES IS AS FOLLOWS.
C R IS THE DISTANCE FROM THE PMH CENTER TO THE POINT OF MAX WIND
C VELOCITY, H IS A PARAMETER FOR THE PMH PROFILES WHICH WERE
C DIGITIZED, RR IS THE DISTANCE FROM THE PMH CENTER TO THE POINT
C OF MAX WIND VELOCITY FOR THE PMH PROFILE BEING CONSTRUCTED,
C IRR IS THE FINAL INTERPOLATED VALUES OF THE PMH PROFILE BEING
C CONSTRUCTED, AS A FUNCTION OF RELATIVE DISTANCE, VR2 IS REL. VELOP
C CITY VALUES OF THE DIGITIZED PMH PROFILES, XR2 IS DIGITIZED
C VALUES OF RELATIVE DISTANCE CORRESPONDING TO THE VR2'S.
C VTMP IS INTERPOLATED VALUES OF RELATIVE VELOCITY ALONG THE DIGIT-
C IZED PROFILES. (XXA(1),YYA(1),I=1,2) ARE (X,Y) POINTS USED TO
C MAKE LINEAR INTERPOLATIONS.
C NPTS IS THE NUMBER OF POINTS BETWEEN X = .8 AND X = 30, SPACED AT
C INCREMENTS OF X EQUAL TO .02.
C IB AND IE ARE USED TO READ AND PRINT OUT DATA CARD BY
C CARD OR LINE BY LINE.
C ITYPE IS THE TYPE OF HURRICANE MODEL, (PMH OR SPH)
C
C NOTE. PMH MEANS PROBABLE MAXIMUM HURRICANE,
C
C DATA E,KX /,00001,5H,..../
C *WARNING, NPTS SHOULD ALWAYS BE LESS THAN 2000, OTHERWISE
C THE DIMENSIONS OF VTMP AND IRP WILL BE EXCEEDED AND
C SOMETHING WILL BE CLOBBEDED.
C
C DATA ERR /5HERR14,5HERR15,5HERR16,5HERR17,5HERR18,5HERR19,5HERR20,
C 2 5HERR21,5HERR22/
C
C ID IS GIVEN AN INITIAL VALUE . IF ID REMAINS = AAAAA THEN YOU
C KNOW IZONE WAS NOT A LEGAL VALUE FOR THIS PROGRAM
C ID=5HAAAAA
C IF (IZONE,EQ,2H A) ID=5H00001
C IF (IZONE,EQ,2H 1) ID=5H00001
C IF (IZONE,EQ,2H 2) ID=5H00001
C IF (IZONE,EQ,2H 4) ID=5H00001
C IF (IZONE,EQ,2H B) ID=5H00002
C IF (IZONE,EQ,2H C) ID=5H00002
C IF (IZONE,EQ,2H 3) ID=5H00003
C
C VERIFY THAT ZONE REQUESTED IS CORRECT
C IF (ID,EQ,5HAAAAA) GO TO 10
C GO TO 30
C WRITE ERROR MESSAGE
C BOMB=ERR(1)

```

80	WRITE (6,20) IZONE	PRO08000
20	FORMAT (1X,45HZONE REQUESTED IS NOT VALID FOR THIS PROGRAM,/,1X,5I	PRO08100
	2HVALID ZONES ARE A,B,C,1,2,3,4, YOU REQUESTED ZONE ,A2)	PRO08200
	GO TO 420	PRO08300
	C	PRO08400
85	C READ VELOCITY PROFILES FOR RHO/R FROM .8 TO 30,	PRO08500
	C INITIALIZE KK AND IFLAG	PRO08600
30	KK=1	PRO08700
	IFLAG=0	PRO08800
40	READ (IDATA,50) JID,R(KK)	PRO08900
90	50 FORMAT (5X,A5,F5.1,2X,A3)	PRO09000
	C CHECK FOR CORRECT ZONE	PRO09100
	BOMB=ERR(2)	PRO09200
	IF (JID,EQ,5HFINAL) GO TO 420	PRO09300
	BOMB=ERR(3)	PRO09400
95	IF (JID,NE, ID,AND,KK,EQ,2) GO TO 420	PRO09500
	IF (JID,NE, ID) GO TO 60	PRO09600
	GO TO 70	PRO09700
60	READ (IDATA,160) DUM	PRO09800
100	IF (DUM(1),EQ,5H.....) GO TO 30	PRO09900
	GO TO 60	PRO10000
70	BOMB=ERR(4)	PRO10100
	IF (R(KK),LT,2.0,OR,R(KK),GT,80.0) GO TO 80	PRO10200
	C CHECK FOR BRACKETING RADII	PRO10300
	IF (R(KK),LE,RR) GO TO 100	PRO10400
105	IF (R(KK),GT,RR) GO TO 110	PRO10500
80	WRITE (6,90) R(KK)	PRO10600
90	FORMAT (1X,39HINTERPOLATION RANGE FOR R = 2.0 TO 80.0,/,1X,17HYDU	PRO10700
	2REQUESTED R =/,1X,F5.1)	PRO10800
	GO TO 420	PRO10900
110	C	PRO11000
100	R(1)=R(KK)	PRO11100
	KK=1	PRO11200
	GO TO 120	PRO11300
110	BOMB=ERR(5)	PRO11400
115	IF (KK,EQ,1) GO TO 420	PRO11500
	C SET FLAG TO END READING	PRO11600
	IFLAG=1	PRO11700
	C	PRO11800
120	IB=1	PRO11900
130	IE=IB+7	PRO12000
	READ (IDATA,140) (XR2(KK,K),VR2(KK,K),K=IB,IE)	PRO12100
140	FORMAT (8(F5.2,F5.3))	PRO12200
	C TEST FOR LAST CARD,	PRO12300
	C ALL 9(S IN COLS 1=5 INDICATES END OF DATA,	PRO12400
125	EE=ABS(XR2(KK,IB)+999.99)	PRO12500
	IF (EE,LT,E) GO TO 150	PRO12600
	IB=IB+8	PRO12700
	GO TO 130	PRO12800
	C	PRO12900
130	C	PRO13000
	C CHECK TO MAKE CERTAIN THAT IE DOES NOT EXCEED 750,	PRO13100
	C WARNING, IF IE EXCEEDS 750, THE DIMENSIONS OF XR2 AND VR2	PRO13200
	C WILL BE EXCEEDED, AND SOMETHING WILL BE CLOBBERED,	PRO13300
135	150 BOMB=ERR(6)	PRO13400
	IF (IE,GT,750) GO TO 420	PRO13500
	C DECREASE IE BY 8 BECAUSE 1 EXTRA CARD WAS READ,	PRO13600
	IE=IE-8	PRO13700
	C READ PERIODS,	PRO13800
140	160 READ (IDATA,160) IAX	PRO13900
	FORMAT (16A5)	PRO14000
	C	PRO14100
	C COUNT EXACT NO OF DATA POINTS,	PRO14200
	DO 170 K=1,8	PRO14300
145	IF (ABS(XR2(KK,IE=K+1)),LT,E,AND,ABS(VR2(KK,IE=K+1)),LT,E) GO TO ;	PRO14400
	270	PRO14500
	C CHECK TO MAKE CERTAIN THAT LAST XR2 VALUE EXCEEDS 30,	PRO14600
	C WARNING, IF NO VALUE OF XR2 EXCEEDS 30, THE LOGIC IN THE DO	PRO14700
	C LOOP ENDING AT STATEMENT 220 IS BOOBY TRAPPED,	PRO14800
	C	PRO14900
150	BOMB=ERR(7)	PRO15000
	IF (XR2(KK,IE=K+1),LT,30.) GO TO 420	PRO15100
	MP=IE=K+1	PRO15200
	GO TO 180	PRO15300
170	CONTINUE	PRO15400
155	C IMPOSSIBLE,	PRO15500
	BOMB=ERR(8)	PRO15600
	GO TO 420	PRO15700
	C	PRO15800
180	DO 210 I=1,16	PRO15900

160	BOMB=ERR(9)	PRO16000
	IF (IAX(1),NE,KX) GO TO 190	PRO16100
	GO TO 210	PRO16200
190	WRITE (6,200) IAX	PRO16300
200	FORMAT (1X,16A5)	PRO16400
165	GO TO 420	PRO16500
	CONTINUE	PRO16600
	C	PRO16700
	C NOTE***** ONLY ONE SET OF DIGITIZED DATA IS USED WHEN R(KK) = RR	PRO16800
	C SET FLAG TO END READING ONLY IF R(KK) EQUALS RR	PRO16900
170	IF (R(KK),EQ,RR) IFLAG=1	PRO17000
	C CHECK FLAG TO END READING	PRO17100
	IF (IFLAG,EQ,1) GO TO 220	PRO17200
	KK=2	PRO17300
	GO TO 40	PRO17400
175	C	PRO17500
	C INPUT AND INTERNAL COMPUTATIONS ARE PRINTED OUT FOR DEBUGGING PURPOSES IF LIST = 1. IF LIST = 0 INPUT AND INTERNAL COMPUTATIONS	PRO17600
	C NOT PRINTED.	PRO17700
	C	PRO17800
180	C CHECK PRINT OPTION,	PRO17900
	220 IF (LIST,NE,1) GO TO 270	PRO18000
	DO 260 KK=1,2	PRO18100
	WRITE (6,230)	PRO18200
230	FORMAT (1H0,3HXR2//)	PRO18300
185	WRITE (6,240) (XR2(KK,L),L=1,MP)	PRO18400
	240 FORMAT (1X,16F7,3)	PRO18500
	WRITE (6,250)	PRO18600
	250 FORMAT (1H0,3HVR2//)	PRO18700
	WRITE (6,240) (VR2(KK,L),L=1,MP)	PRO18800
190	C SKIP OUT OF LOOP ONLY WHEN R(KK) = RR	PRO18900
	IF (R(KK),EQ,RR) GO TO 270	PRO19000
	260 CONTINUE	PRO19100
	C	PRO19200
	C	PRO19300
195	C CONSTRUCT INTERPOLATED RELATIVE VELOCITY PROFILE,	PRO19400
	C	PRO19500
	C	PRO19600
	C IN THE DO LOOP ENDING AT STATEMENT 250, VALUES OF RELATIVE VELOCITY	PRO19700
	C ARE INTERPOLATED STARTING AT X = .8 AND ENDING AT X = 30. BY	PRO19800
200	C INCREMENTS OF X, EQUAL TO .02.	PRO19900
	C	PRO20000
	270 DO 340 KK=1,2	PRO20100
	C	PRO20200
	C THIS DO LOOP REFINES SPACING BETWEEN DIGITIZED DATA POINTS	PRO20300
205	C	PRO20400
	X=.8	PRO20500
	IA=-1	PRO20600
	IFUD=0	PRO20700
	C SELECT DIGITIZED VALUES OF (X,Y) FOR INTERPOLATION. IA IS A	PRO20800
210	C COUNTER, WHEN IA = -1, THE SUBSCRIPT IA+2 (BELOW) EQUALS 1.	PRO20900
	C THE TEST VARIABLE, IFUD, IS USED BECAUSE THE INTERPOLATION	PRO21000
	C COEFFICIENTS MUST BE COMPUTED AT LEAST ONCE BEFORE USING THE	PRO21100
	C INTERPOLATION FORMULA.	PRO21200
	DO 310 I=1,NPTS	PRO21300
215	IF (X,LT,XR2(KK,IA+2),AND,IFUD,EQ,1) GO TO 300	PRO21400
	IFUD=1	PRO21500
	280 IA=IA+1	PRO21600
	IF (X,GT,XR2(KK,IA+2)) GO TO 280	PRO21700
	C	PRO21800
220	C X SHOULD BE BETWEEN XXA(1) AND XXA(2).	PRO21900
	C SUBSTITUTE APPROPRIATE (X+Y) VALUES FOR INTERPOLATION.	PRO22000
	DO 290 L=1,2	PRO22100
	XXA(L)=XR2(KK,IA+L)	PRO22200
290	YYA(L)=VR2(KK,IA+L)	PRO22300
225	CALL INTERP (XXA,YYA,AX)	PRO22400
300	VTMP(KK,I)=AX(1)*X+AX(2)	PRO22500
310	X=X+.02	PRO22600
	C CHECK PRINT OPTION,	PRO22700
	IF (LIST,NE,1) GO TO 330	PRO22800
230	WRITE (6,320)	PRO22900
	320 FORMAT (1H0,5HVTMP //)	PRO23000
	WRITE (6,240) (VTMP(KK,L),L=1,NPTS)	PRO23100
	C SKIP OUT OF LOOP ONLY WHEN R(KK) = RR	PRO23200
	330 IF (R(KK),EQ,RR) GO TO 350	PRO23300
235	340 CONTINUE	PRO23400
	C	PRO23500
	350 DO 380 I=1,NPTS	PRO23600
	C	PRO23700
	C THIS DO LOOP INTERPOLATES BETWEEN TWO SETS OF DIGITIZED DATA	PRO23800
		PRO23900

240	C AND STORES INTERPOLATED VALUES IN A NEW ARRAY (IRR).	PRO24000
	C	PRO24100
	C SKIP FOLLOWING LOOP ONLY WHEN R(KK) = RR	PRO24200
	IF (R(KK),EQ,RR) GO TO 370	PRO24300
	DO 360 KK=1,2	PRO24400
245	YYA(KK)=VTMP(KK,I)	PRO24500
	XXA(KK)=R(KK)	PRO24600
	360 CONTINUE	PRO24700
	CALL INTERP (XXA,YYA,AX)	PRO24800
	IRR(I)=(AX(1)*RR+AX(2))*1000,+.5	PRO24900
250	GO TO 380	PRO25000
	C EXECUTE THE FOLLOWING STATEMENT ONLY WHEN R(KK) = RR	PRO25100
	370 IRR(I)=VTMP(I,I)*1000,+.5	PRO25200
	380 CONTINUE	PRO25300
	C	PRO25400
255	C CHECK PRINT OPTION.	PRO25500
	IF (LIST,NE,1) GO TO 470	PRO25600
	WRITE (6,390) RR	PRO25700
	390 FORMAT (3HOR,PF5,1,///4H IRR,//)	PRO25800
	C	PRO25900
260	C PRINT FINAL INTERPOLATED VALUES OF RELATIVE VELOCITY.	PRO26000
	IB=1	PRO26100
	NC=1	PRO26200
	400 IE=IB+11	PRO26300
	IF (IE,GT,NPTS) IE=NPTS	PRO26400
265	WRITE (6,410) IZONE,ITYPE,RR,NC,(IRR(I),I=IB,IE)	PRO26500
	410 FORMAT (1X,4MZONE,A2,1X,A3,PF5,1,13I5)	PRO26600
	IF (IE,EQ,NPTS) GO TO 470	PRO26700
	IB=IB+12	PRO26800
	NC=NC+1	PRO26900
270	GO TO 400	PRO27000
	C ERROR MESSAGE IS WRITTEN	PRO27100
	420 WRITE (6,430)	PRO27200
	430 FORMAT (/,1X,40H***** ERROR FROM SUBROUTINE PROFIL *****)	PRO27300
	WRITE (6,440) BOMB	PRO27400
275	440 FORMAT (//,1X,A5)	PRO27500
	WRITE (6,450)	PRO27600
	450 FORMAT (3X,88H SEE ERROR TABLE IN CERC TM TITLE VERIFICATION STUDY	PRO27700
	2 OF A BATHYSTROPIC STORM SURGE MODEL)	PRO27800
	WRITE (6,460)	PRO27900
280	460 FORMAT (1X,86H*****)	PRO28000
	2*****	PRO28100
	C IF BOMB = ANY ERROR RETURN IS MADE TO MAIN PROGRAM SURGE WHERE THE	PRO28200
	C VALUES OF IDATA, KEY, KEYA, AND ETC. ARE WRITTEN BEFORE RUN	PRO28300
	C TERMINATION.	PRO28400
285	BOMB=5HERROR	PRO28500
	470 RETURN	PRO28600
	END	PRO28700

```

SUBROUTINE INTERP (XXA,YYA,AX)
C
C *****
C THIS SUBROUTINE MAKES LINEAR INTERPOLATIONS BETWEEN DIGITIZED (X,Y)
C DATA POINTS AND BETWEEN PROFILE CURVES OF DIFFERENT SIZE HURRICANES.
C VARIABLES NOT DEFINED IN THIS ROUTINE WILL BE DEFINED IN ANOTHER
C ROUTINE
C (XXA,YYA) IS ARE THE DATA POINTS, AX IS ARE THE INTERPOLATION COEF-
C ICIENTS,
C AX IS ARE DETERMINED BY USING THE STANDARD SLOPE INTERCEPT FORM
C AX(1) IS THE SLOPE, AX(2) IS THE INTERCEPT.
C
C DIMENSION XXA(2),YYA(2),AX(2)
C DATA E /,000005/
C
C CHECK TO SEE IF XXA POINTS ARE EVER INCREASING.
C IF (XXA(2)-XXA(1),GT,E) GO TO 40
C UNREASONABLE INPUT.
C PRINT ERROR MESSAGE
C WRITE (6,10)
C FORMAT (/,1X,40H***** ERROR FROM SUBROUTINE INTERP *****,//,1X,8HE
C 2RROR 23)
C WRITE (6,20)
C FORMAT (3X,88H SEE ERROR TABLE IN CERC TM TITLE VERIFICATION STUDY
C 2 OF A BATHYSTROPIC STORM SURGE MODEL)
C WRITE (6,30) XXA
C FORMAT (1X,2F10.3,41H TILT, X DATA SHOULD BE EVER INCREASING,)
C STOP
C
C NORMAL INPUT
C COMPUTE INTERPOLATION COEFFICIENTS.
C
C AX(1)=(YYA(2)-YYA(1))/(XXA(2)-XXA(1))
C AX(2)=YYA(1)-AX(1)*XXA(1)
C RETURN
C END

```

```

SUBROUTINE ZERWND
C
C *****
5 THIS ROUTINE COMPUTES WIND VELOCITY MAGNITUDE AND DIRECTION ALONG A
C TRAVERSE LINE WHICH PASSES THRU A PROBABLE MAXIMUM HURRICANE AT THE
C POINT WHERE WIND VELOCITY IS GREATEST, THIS TRAVERSE IS 115,
C DEG, COUNTERCLOCKWISE TO THE LINE OF SYMMETRY OF THE P,M,H, ISOVELS,
C
10 THIS ROUTINE DOES COMPUTATION FOR RS, W, AND TH ALONG A TRAVERSE
C DRAWN FROM THE ZERO WIND ISOVEL AT SHORE TO THE POINT WHERE
C THE TRAVERSE INTERSECTS WITH THE ZERO WIND ISOVEL AT THE TAIL
C OF THE STORM,
C
15 COMPUTATIONS ARE DONE SHOREWARD=SEAWARD FOR THIS ROUTINE
C
C *****
C NOTE
C NONE OF THE RESULTS FROM THIS ROUTINE ARE CARRIED TO THE MAIN
20 PROGRAM OR ANY OTHER ROUTINE,
C THE ONLY PURPOSE OF THIS ROUTINE IS TO SUPPLY A COMPLETE DESCRIPTION
C OF THE WIND FIELD USED IN COMPUTING SURGE,
C *****
25 VARIABLES NOT DEFINED IN THIS ROUTINE WILL BE DEFINED IN ANOTHER
C ROUTINE
C
C THE VARIABLES THETA AND THETA0 ARE USED FOR CLARITY TO ILLUSTRATE
C CHANGES IN THE VALUE OF TH, THESE VARIABLES DO NOT APPEAR IN
30 THE ROUTINE, TH IS USED TO REPRESENT THETA AND THETA0,
C TH IS ORIGINALLY THETA IN RADIANS WHICH IS THE POLAR COORDINATE
C OF LOCATION FOR RS(L), TH IS CONVERTED TO THETA0 (ANGLE WHICH
C INCLUDES DEFLECTION), THEN TH IS CONVERTED TO THETA = 25, DEGREES,
35 THE SUBTRACTION OF 25, DEGREES WAS A CORRECTION FOR GEOMETRY,
C THE WIND DIRECTION, THETA0, IS GIVEN BY THE FOLLOWING FORMULAS
C
C THETA0=THETA+25, IF RS(L)/R LT 1.2
C THETA0=THETA+15,*(RS(L)-R)/(1.2*R-R)+10, IF 1, LT RS(L)/R LT 1.2
C THETA0=THETA+10,*(RS(L)/R IF RS(L)/R LT 1.
40
C (DISTANCES ARE IN NAUTICAL MILES, V IS IN KNOTS, W(L) AND WX
C ARE IN M.P.H.)
C (ANGLES ARE IN DEGREES, COUNTERCLOCKWISE IS POS,)
C VARIABLES ARE AS FOLLOWS,
45 AL IS A DISTANCE ALONG THE VELOCITY PROFILE AT WHICH COMPUTATIONS
C BEGIN,
C DEL = DIFFERENCE BETWEEN DELA AND DELB,
C DELA = DISTANCE TO POINT ON TRAVERSE WHERE COMPUTATIONS TAKE PLACE
C DELB = DISTANCE TO POINT ON TRAVERSE 10 N.M. FARTHER FROM SHORE
50 DISTO = INITIAL DISTANCE ALONG TRAVERSE LINE TO POINT OF MAXIMUM
C WIND VELOCITY,
C PRESS = ATMOSPHERIC PRESSURE DIFFERENTIAL,
C R = DISTANCE FROM ORIGIN TO THE MAXIMUM WIND,
C RAD2 = THE NUMBER OF DEGREES IN ONE RADIAN (CONVERSION FACTOR)
C
55 RS = THE LENGTH OF THE RAY FROM THE STORM CENTER TO ANY POINT
C ON THE TRAVERSE,
C SN AND CB ARE SIN(115) AND COS(115), RAD1 IS 25 DEG, IN RADIANS,
C SNTF = SINF OF 25 DEGREES
C
C TH = THE ANGLE (DEGREES) BETWEEN 115 DEG, LINE AND THE RAY DRAWN
60 BETWEEN THE STORM CENTER AND ANY POINT ON THE TRAVERSE,
C TEST = VALUE USED TO TEST FOR END OF TRAVERSE DRAWN FROM ZERO WIND
C TO ZERO WIND,
C VF = SPEED OF TRANSLATION OF ENTIRE HURRICANE,
C W = MAGNITUDE OF WIND VELOCITY AT ANY POINT ALONG THE TRAVERSE LINE
65 WX = MAXIMUM WIND VELOCITY
C X AND Y = CARTESIAN COORDINATES OF EACH RAY RS, THE STORM
C CENTER IS THE ORIGIN TO WHICH X AND Y VALUES ARE REFERENCED,
C THE X- AXIS IS ALONG THE RADIUS TO MAX. WINDS (R),
C
C COMMON /PAH/ LM,VF,WX,R,DISTO,XP
70 COMMON /ATH/ Pn,PO
C DATA SN,CB,RAD1,RAD2 /,90631,42262,436332,57,29577867/
C DATA SNTF /,42262/
C
C TEST=2,*(DISTO-R*SNTF)
75 DELA=0,
C AL=DISTO
C WRITE (6,10)
10 FORMAT (18H TIME INCREMENT 1.5X,11H 0.0 HOURS)
C COMPUTE INITIAL X AND Y,

```

```

ZWD00100
ZWD00200
ZWD00300
ZWD00400
ZWD00500
ZWD00600
ZWD00700
ZWD00800
ZWD00900
ZWD01000
ZWD01100
ZWD01200
ZWD01300
ZWD01400
ZWD01500
ZWD01600
ZWD01700
ZWD01800
ZWD01900
ZWD02000
ZWD02100
ZWD02200
ZWD02300
ZWD02400
ZWD02500
ZWD02600
ZWD02700
ZWD02800
ZWD02900
ZWD03000
ZWD03100
ZWD03200
ZWD03300
ZWD03400
ZWD03500
ZWD03600
ZWD03700
ZWD03800
ZWD03900
ZWD04000
ZWD04100
ZWD04200
ZWD04300
ZWD04400
ZWD04500
ZWD04600
ZWD04700
ZWD04800
ZWD04900
ZWD05000
ZWD05100
ZWD05200
ZWD05300
ZWD05400
ZWD05500
ZWD05600
ZWD05700
ZWD05800
ZWD05900
ZWD06000
ZWD06100
ZWD06200
ZWD06300
ZWD06400
ZWD06500
ZWD06600
ZWD06700
ZWD06800
ZWD06900
ZWD07000
ZWD07100
ZWD07200
ZWD07300
ZWD07400
ZWD07500
ZWD07600
ZWD07700
ZWD07800
ZWD07900

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80      X=HAL*CS                      ZWD08000
      Y=AL*SN                        ZWD08100
C                                           ZWD08200
C PRINT PARAMETERS,                   ZWD08300
      WRITE (6,20) R,VF,WX,XP        ZWD08400
85      20  FORMAT (1X,5X,3HR =,F6,2,9H N, MILES,4X,4HV F =,F6,2,6H KNOTS,4X,5HZWD08500
      2  WX =,F7,2,7H M.P.H.,/1X,12HX DISTANCE =,F5,2,9H N, MILES) ZWD08600
      WRITE (6,30)                   ZWD08700
      30  FORMAT (//)                ZWD08800
C PRINT TITLE,                       ZWD08900
      90  WRITE (6,40)               ZWD09000
      40  FORMAT (27X,4H(RS),12X,3H(W),10X,4H(TH),19X,7H(PRESS)) ZWD09100
      WRITE (6,50)                   ZWD09200
      50  FORMAT (1X,6X,13HDISTANCE FROM,6X,6HRADIAL,10X,4HWIND,5X,24HANGLE ZWD09300
      2FROM TRAVERSE LINE,3X,8MPRESSURE/5X,16HZERO VELOCITY AT,4X,8HDISTAZWD09400
      3NCE,7X,8HVELOCITY,4X,21HCOUNTERCLOCKWISE POS.,6X,5HSETUP/4X,18HFROZWD09500
      4NT OF HURRICANE,3X,8HN, MILES,9X,6HM.P.H.,5X,9H(DEGREES),19X,3HFT,ZWD09600
      5/,10X,8HN, MILES)            ZWD09700
      WRITE (6,30)                   ZWD09800
C REPETITIVE COMPUTATION OF WIND VELOCITY AND DIRECTION VERBUS X AND Y,ZWD09900
      100 60  RS=SQRT(X**2+Y**2)      ZWD10000
      CALL QUAD (X,Y,TH)             ZWD10100
C TH = ANGLE FROM LINE OF SYMMETRY TO RADIUS VECTOR (R(L)) ZWD10200
      CALL VEL (RS,VXX)              ZWD10300
      ***X*VXX=.5*(1.-COS(TH))*VF*1.151 ZWD10400
      105  CALL DEFL (TH,RS)         ZWD10500
C TH = WIND ANGLE WITH CORRECTION FOR DEFLECTION ZWD10600
C SUBTRACT 25 DEGREES FOR GEOMETRY, ZWD10700
C CONVERT WIND VELOCITY ANGLE TO DEG, FROM 115 DEG, LINE, ZWD10800
      TH=RAD2*(TH+RAD1)              ZWD10900
      110  IF (TH.LE.0.) TH=TH+360.  ZWD11000
      PRESS=(PN-PO)*(1.0+EXP(-WR/RS)) ZWD11100
      WRITE (6,70) DELA,RS,W,TH,PRESS ZWD11200
      70  FORMAT (1X,4F15,2,10X,F15,2) ZWD11300
      IF (DELA.GE.TEST) GO TO 80     ZWD11400
      115  DELA=DELA+10.             ZWD11500
      IF (DELB.GE.TEST) DELB=TEST   ZWD11600
      DEL=DELB-DELA                 ZWD11700
      X=X+DEL*CS                    ZWD11800
      Y=Y+DEL*SN                    ZWD11900
      120  DELA=DELB                 ZWD12000
      GO TO 60                      ZWD12100
      80  WRITE (6,90)               ZWD12200
      90  FORMAT (//1X,49HX AXIS IS ALONG HURRICANE ISDEL AXIS OF SYMMETRY) ZWD12300
      WRITE (6,100)                  ZWD12400
      125  100  FORMAT (1H1)         ZWD12500
      RETURN                         ZWD12600
      END                            ZWD12700

```

**APPENDIX A-3. DEFINITION OF SYMBOLS AND COMPUTER PROGRAM  
VARIABLES FOR ACTUAL HURRICANES**

AB	Calculated angle between the traverse and initial storm centerline, RS (1).
AHOUR	Variable used to establish storm time.
ALPHA	Azimuth of the traverse (degrees).
AN	Intermediate value of wind stress and setup for x-component.
BEAR	Bearing of traverse in degrees. Not used for computations.
BETA 0	Azimuth of the storm centerline, RS (1) (degrees).
BFF	Bottom friction factor.
BN	Intermediate value of wind stress and setup for y-component.
BP	An array used to store successive values of BN.
BY	Absolute value of BN.
C1, C2, C3	Dimension correction factors.
D	Depth below mean water level in feet.
DASQ	Total water depth squared at each time level.
DAVG	Average bottom depth at each time level.
DELSXN	Incremental x-component setup in feet.
DELSYN	Incremental y-component setup in feet.
DELT	Time increments in hours.
DELX	Difference between traverse distances at each time level.
DEN	Intermediate computation for denominator of flux equation at each time level.
DIST	Distance from coast (nautical miles). DIST (1) and D (1) are farthest points from shore with successive values taken shoreward of these points.



DTH	Total water depth at each time level (feet).
DTN	Intermediate computation of water depth.
DTS	Intermediate computation of total water depth (DTH).
FUD	Cosine (AB).
IB, IE	Used to read and print out data; card-by-card or line-by-line.
ICHECK	Sentinel used to indicate errors.
ID	Identifies type of data on card.
IHUR	First three letters of hurricane punched on input data.
IOMIT	Print option parameter. If IOMIT equals 1, detailed printing is omitted. If IOMIT equals 0, detailed printing of type shown in example output for actual hurricane of Appendix A-7 is supplied.
IOMITD	Print option parameter. If IOMITD equals 1, detailed printing is omitted. If IOMITD equals 0, detailed printing of type shown in example output for actual hurricane of Appendix A-7 is supplied.
ISKIP	Print option parameter. If parameter equals 0, all wind data before and after interpolation is printed. If parameter equals 1, input wind-field data is omitted. If parameter equals 2, all wind-field data is omitted. See example output for actual hurricane in Appendix A-7.
ITYPE	First three letters of hurricane name.
ITPP	Sentinel used to verify interpolation of wind-field data.
LM	Number of shelf reaches for input supplied (maximum 50).
NM	Number of time increments for input supplied (maximum 200).
NN	Card number read from card.
NTP	Equals $LM + 1$ .
PHI	Geographical latitude in degrees of traverse (given by PHII).
PHII	Geographical latitude in degrees of traverse (supplied as input).

PI	3.14159
PN	Peripheral pressure (inches of HG).
PO	Central pressure (inches of HG).
R	Radius of maximum winds (nautical miles).
RS	Length of ray from storm center to any point on the traverse (nautical miles). Polar coordinate of distance.
SA	Setup due to astronomical forces (feet).
SAP	Old value of astronomical tide.
SE	Initial setup (feet).
SINPHI	Sine of geographical latitude.
SP	Setup due to pressure (feet).
SPN	Pressure setup at new time level.
SPRES	Successive values of pressure setup at each time level.
ST	Successive values of total surge along traverse line.
Storm Center	Origin to which x- and y-values are referenced.
STOT	Value of total surge at shore for each time level.
SUMSX	Successive values of x-component of surge along traverse line.
SUMSY	Successive values of y-component of surge along traverse line.
SW	Successive values of SUMSX + SUMSY along traverse line.
SWW	Successive values of SW for each time level at shore.
SX	Total x-component setup at shore (feet).
SXP	Successive values of SUMSX.
SY	Total y-component setup at shore (feet).

SYP	Successive values of SUMSY.
T	Angle measured from the positive x-axis counterclockwise to the wind vector. Polar coordinate of angle.
VF	Forward speed of storm (knots).
VN, VNN, VNTEST, VP, VS	y-component of volume transport.
VN	Successive values of flux along traverse line.
VNN	Absolute value of flux.
VNTEST	Check on value of flux.
VP	Flux at previous time level.
VS	Y-flux (ft × ft/sec).
W	Magnitude of wind velocity at any point along the traverse line in miles per hour.
WK	Van Dorn's wind-stress factor.
WKCOR	Empirical calibration factor for wind-stress coefficient.
WSQ	W squared.
WWX	$WSQ \times \cosine (T)$ in miles per hour squared.
WWY	$WSQ \times \text{sine } (T)$ in miles per hour squared.
WX	Maximum windspeed in miles per hour. Does not enter into computations for actual hurricane.
X-axis	Along the radius to maximum winds (R).

**APPENDIX A-3. DEFINITION OF SYMBOLS AND COMPUTER PROGRAM  
VARIABLES FOR HYPOTHETICAL HURRICANES**

AHOUR	Variable used to establish storm time.
AL	Distance along the velocity profile at which computations begin for each time step. Measured from point of maximum wind to most seaward point on traverse.
AN	Intermediate value of wind stress and setup for x-component.
AX	Interpolation coefficients determined by using the slope intercept form. AX (1) is the slope. AX (2) is the intercept.
BEAR	Bearing of traverse in degrees. Not used for computations.
BFF	Bottom friction factor.
BN	Intermediate value of wind stress and setup for y-component.
BP	An array used to store successive values of BN.
BY	Absolute value of BN.
C1, C2, C3	Dimension correction factors.
CS	Cosine 115°.
D	Depth below mean low water datum in feet.
DASQ	Total water depth squared at each time level.
DAVG	Average bottom depth at each time level.
DEL	Difference between points on the traverse.
DELA	Distance to point on traverse where computations take place.
DELB	Distance to point on traverse 10 nautical miles farther from shore than DELA.
DELSXN	Incremental x-component setup in feet.
DELSYN	Incremental y-component setup in feet.

DELT	Time increments in hours.
DELTT	Time increment used for computations.
DELX	Difference between traverse distances at each time level.
DEN	Intermediate computation for denominator of flux equation at each time level.
DIST	Distance from coast (nautical miles). DIST (1) and D (1) are farthest points from shore with successive values taken shoreward of these points.
DIST 0	Initial distance along traverse line to point of maximum wind velocity.
DTH	Total water depth at each time level (feet).
DTN	Intermediate computation of water depth.
DTS	Intermediate computation of total water depth (DTH).
E	Parameter used to check convergence.
EE	Parameter used to check convergence.
HR	Time step.
IB, IE	Used to read and print out data, card-by-card or line-by-line.
ICHECK	Sentinel used to indicate errors.
ID	Identifies type of data on card.
IDATA	Identifies type of wind data supplied as input. If IDATA equals 4 or 5, relative wind velocity data is supplied for interpolation. If IDATA equals 0, interpolated relative wind velocity data is supplied. See sample input sheet of Appendix A-6 for data format.
ILAST	Indicates number of different parameter sets to be read.
IOMIT	Print option parameter. If IOMIT equals 1, detailed printing is omitted. If IOMIT equals 0, detailed printing of type shown in example output for hypothetical hurricane of Appendix A-7 is supplied.

IOMITD	Print option parameter. If IOMITD equals 1, detailed printing is omitted. If IOMITD equals 0, detailed printing of type shown in example output for hypothetical hurricane of Appendix A-7 is supplied.
IREAD	Used to read all digitized wind data onto disk storage.
IRR	Final interpolated values of PMH profile being constructed as a function of relative distance.
ITYPE	Type of hurricane model (PMH or SPH).
IVX	Interpolated wind data generated in subroutine PROFIL.
IZONE	Hurricane zone.
KEY	Print option for subroutine HRWDPF. If KEY equals 0, detailed printing is omitted. If KEY equals 1, detailed printing of type shown in example output of this appendix is supplied.
KEYA	If KEYA equals 0, subroutine ZERWND is not called. If KEYA equals 1, subroutine ZERWND is called and printing of type shown in example output of Appendix A-7 is supplied.
LIST	Print option for subroutine PROFIL. If LIST equals 0, detailed printing is omitted. If LIST equals 1, detailed printing of type shown in example output of Appendix A-7 is supplied.
LM	Maximum number of 50 shelf reaches for input supplied.
NM	Maximum number of 200 time increments for input supplied.
NN	Card number read from card.
NPTS	Number of points along the PMH profile.
PHI	Geographical latitude in degrees of traverse (given by PHII).
PHII	Geographical latitude in degrees of traverse (supplied as input).
PHIIA	Latitude limit for hurricane zone.
PHIIB	Latitude limit for hurricane zone.
PI	3.14159.

PN	Peripheral pressure (inches of HG).
PO	Central pressure (inches of HG).
PRESS	Atmospheric pressure differential.
R	Radius to maximum wind in nautical miles (input parameter) or (subroutine PROFIL) radius to maximum wind for the PMH profiles which were digitized.
RAD 1	25° in Radians.
RAD 2	Number of degrees equal to one radian (conversion factor).
RHO	Each successive value of RS (L) along the traverse line or (subroutine STRTPT) initial distance from PMH center to shore intersection of traverse line.
RR	Radius to maximum winds of wind-field data supplied as input or (subroutine PROFIL) radius to maximum wind for the PMH profile being constructed.
RS	Length of the ray from the storm center to any point on the traverse (nautical miles). Polar coordinate of distance.
SA	Setup due to astronomical forces (feet).
SAA	Value of astronomical tide (input parameter in feet).
SAP	Old value of astronomical tide.
SE	Initial setup (feet).
SINPHI	Sine of geographical latitude.
SN	Sine 115°.
SNTF	Sine of 25°.
SP	Setup due to pressure (feet).
SPN	Pressure setup at new time level.
SPRES	Successive values of pressure setup at each time level.

ST	Successive values of total surge along traverse line.
STOT	Value of total surge at shore for each time level.
SUMSX	Successive values of x-component of surge along traverse line.
SUMSY	Successive values of y-component of surge along traverse line.
SW	Successive values of SUMSX + SUMSY along traverse line.
SWW	Successive values of SW for each time level at shore.
SX	Total x-component of setup at shore (feet).
SXP	Successive values of SUMSX.
SY	Total y-component of setup at shore (feet).
SYP	Successive values of SUMSY.
T	Angle measured from the positive x-axis counterclockwise to the wind vector. Polar coordinate of angle.
TEST	Value used to test for end of traverse which was drawn from zero to zero wind.
TH	Has three different values: (1) angle in radians between 115° line and the the ray drawn between the storm center and any point on the traverse similarly referred to as the polar coordinate of location for RS (L). (2) Wind angle with correction for deflection. (3) Wind angle less 25° correction for geometry.
VF	Forward speed of storm (knots).
VN, VNN, VNTEST, VP, VS	y-component of volume transport.
VN	Successive values of flux along traverse line.
VNN	Absolute value of flux.
VNTEST	Check on value of flux.



VP	Flux at previous time level.
VR2	Relative velocity values of digitized PMH profiles.
VS	Y-flux (ft + ft/sec).
VTMP	Interpolated values of relative velocity along each digitized profile.
VX	Values of digitized relative wind velocity along the axis of symmetry.
VXX	Relative wind velocity distribution along the polar coordinate initial ray RS (L).
W	Windspeed before adjustment due to interference with land mass. Magnitude of wind velocity at any point along the traverse line in miles per hour.
WIND	Adjusted windspeed near shore due to interference with land mass.
WK	Van Dorn's wind-stress factor.
WKCOR	Empirical calibration factor for wind-stress coefficient (constant).
WSQ	WIND squared.
WWX	$WSQ \times \cosine (T)$ in miles per hour squared.
WWY	$WSQ \times \text{sine } (T)$ in miles per hour squared.
WX	Maximum windspeed in miles per hour.
X	Along the PMH axis of symmetry through the storm center.
XP	Minimum distance from hurricane center to traverse line.
XR2	Digitized values of relative distance corresponding to VR2 .
XX, XI	Distances along the PMH axis of symmetry.
XXA, YYA	(x, y) points used to make linear interpolations.
X, Y	Cartesian coordinates corresponding to the polar coordinates RS (L) and T. Storm center is the origin to which x- and y-values are referenced. The x-axis is along the radius to maximum winds (R).

Y Perpendicular to X.

YPP Distance along traverse from point of maximum wind to point where XP intersects traverse line.

## APPENDIX A-4. INDEX OF VARIABLES FOR COMPUTER PROGRAMS

Index is to be used in locating and defining variables in the computer program. Variables are listed in alphabetical order with locations and definitions indicated by line number.

### ACTUAL HURRICANES

#### PROGRAM SURGE A

A	REFS	92	444	612	DEFINED	99		
AB	REFS	375	379	DEFINED	348	152	357	365
		367	369					
AMOUR	REFS	625	626	DEFINED	623	625		
ALPHA	REFS	261	330	331	319	346	348	350
		352	344	356	357	365	367	369
	DEFINED	294						
AN	REFS	575	DEFINED	547				
BEAR	REFS	96	457	510	615	DEFINED	149	
BETA0	REFS	261	330	331	339	347	348	350
		352	354	355	357	359	360	361
		362	364	365	367	369	DEFINED	254
BFF	REFS	450	509	562	565	614		364
	DEFINED	182						
BN	REFS	556	563	564	593	DEFINED	548	
BP	REFS	92	563	DEFINED	556	593		
BY	REFS	565	DEFINED	564				
C1	REFS	575	DEFINED	97				
C2	REFS	574	DEFINED	97				
C3	REFS	561	DEFINED	97				
D	REFS	92	464	2*516	584	596		
	DEFINED	226						
DAB0	REFS	562	565	DEFINED	561			
DAVG	REFS	92	558	586	DEFINED	516		
DELSXN	REFS	576	DEFINED	575				
DELSYN	REFS	577	DEFINED	574				
DELT	REFS	92	261	484	533	562	563	625
	DEFINED	294						
DELX	REFS	92	574	575	DEFINED	515		
DEN	REFS	563	DEFINED	562				
DIST	REFS	92	2*375	382	464	2*515	584	596
	DEFINED	202						
DTH	REFS	561	DEFINED	560				
DTN	REFS	574	575	DEFINED	559			
DT8	REFS	559	560	DEFINED	558			
ERROR	REFS	120	139	158	175	191	209	234
		270	291	337	DEFINED	116	137	173
		189	207	232	268	289	336	
FUD	REFS	382	DEFINED	379				
I	REFS	99	165	444	612	DEFINED	99	164
		444	612					
IB	REFS	200	202	218	224	226	243	
	DEFINED	199	218	222	243			
ICHECK	REFS	302	DEFINED	109	117	146	163	180
		196	216	241	276	300		
ID	REFS	132	141	151	160	168	177	184
		193	203	211	228	236	DEFINED	106
		166	182	202	226			
IDA	REFS	141	160	177	193	DEFINED	135	154
		171	187					
IK	REFS	201	202	217	225	226	242	
	DEFINED	200	201	224	225			
INUR	REFS	261	264	272	284	293	410	
	DEFINED	254	281					
IOMIT	REFS	467	504	525	531	581	595	
	DEFINED	106						
IOMITD	REFS	467	476	DEFINED	106			
ISKIP	REFS	2*256	2*378	485	467	DEFINED	106	
ITPP	REFS	387	DEFINED	250	322			
ITYPE	REFS	264	273	284	295	447	508	613

	DEFINED	149							
K	REFS	630	631	DEFINED	605	631			
L	REFS	202	226	3*201	284	3*293	295	304	
	3*375	380	3*382	2*384	2*393	2*394	2*395	2*399	
	2*400	2*401	407	408	3*410	3*413	3*422	2*425	
	2*426	2*427	2*428	430	2*431	433	2*464	3*515	
	3*516	2*519	2*521	7*545	3*547	3*548	2*549	592	
	553	554	555	556	3*558	560	562	2*563	
	3*574	575	2*584	586	589	590	591	592	
	593	DEFINED	202	226	279	392	398	406	
	413	422	424	463	513	518	543		
LL	REFS	7*436	7*492	7*494	DEFINED	435	491		
LM	REFS	112	125	147	164	2*201	217	2*225	
	242	251	279	392	398	406	413	422	
	424	463	518	2*596	DEFINED	106			
LMM	REFS	435	491	513	543	DEFINED	147		
LX	REFS	261	272	284	293	410			
	DEFINED	254	281	407					
N	REFS	2*254	2*261	264	273	284	293	436	
	484	492	494	533	545	550	557	559	
	560	562	563	579	586	598	599	600	
	601	602	603	625	6*626	DEFINED	252	420	
	478	524	624						
NC	REFS	203	219	228	244	DEFINED	198	219	
	223	244							
NM	REFS	111	125	252	420	478	524	624	
	DEFINED	106							
NN	REFS	203	211	228	236	261	264	272	
	244	293	410	DEFINED	202	226	254	281	
NTP	REFS	2*375	2*382	399	400	401			
	DEFINED	251							
PHI	REFS	92	519	521	DEFINED	165	519		
PHII	REFS	165	457	510	615	DEFINED	149		
PI	REFS	348	352	357	365	367	369	384	
	519	DEFINED	148						
PN	REFS	425	450	509	614	DEFINED	166		
PO	REFS	425	450	509	614	DEFINED	166		
R	REFS	425	450	509	614	DEFINED	166		
RS	REFS	92	293	304	2*375	2*382	393	410	
	413	425	DEFINED	281	375	399	422		
SA	REFS	92	261	436	494	557	559	560	
	579	586	603	626	DEFINED	254	492	545	
SAP	REFS	560	DEFINED	557	603				
SE	REFS	420	509	558	579	586	614	626	
	DEFINED	166							
SINPHI	REFS	92	2*574	DEFINED	521				
SP	REFS	92	2*436	2*494	2*549	DEFINED	425	2*492	
	2*545								
SPN	REFS	554	559	560	579	586	591	601	
	DEFINED	549							
SPP	REFS	92	560	DEFINED	554	591			
SPRES	REFS	92	626	DEFINED	601				
ST	REFS	586	602	DEFINED	579				
STOT	REFS	92	626	DEFINED	602				
SUMBX	REFS	576	579	580	586	589	598		
	DEFINED	529	576						
SUMBY	REFS	577	579	580	586	590	599		
	DEFINED	530	577						
SW	REFS	586	600	DEFINED	580				
SWW	REFS	92	626	DEFINED	600				
SX	REFS	92	626	DEFINED	598				
SXP	REFS	92	558	DEFINED	552	589			
SY	REFS	92	626	DEFINED	599				
SYP	REFS	92	558	DEFINED	553	590			
T	REFS	92	293	344	395	410	413	427	
	428	DEFINED	281	344	401	422			
TI	REFS	484	485	533	534	DEFINED	483	532	
III	REFS	483	485	532	534	DEFINED	475	484	
	523	533							
VP	REFS	450	509	614	DEFINED	166			
VN	REFS	566	570	574	578	592			
	DEFINED	563	571	573					
VNN	REFS	569	DEFINED	566					
VNTEBY	REFS	569	571	573	DEFINED	565			
VP	REFS	92	562	563	DEFINED	555	592		
VB	REFS	586	DEFINED	578					
VX	REFS	96	399	400	401	DEFINED	393	394	
	395								

W	REFS	92	293	394	410	413	2*426	430
	431	DEFINED	281	400	422			
WK	REFS	92	436	494	547	546		
	DEFINED	431	433	492	545			
WKCOR	REFB	450	509	547	548	614		
	DEFINED	166						
W80	REFS	427	428	DEFINED	426			
WHX	REFS	92	2*436	2*494	2*547	DEFINED	427	2*492
	2*545							
WHY	REFS	92	2*436	2*494	2*548	DEFINED	428	2*492
	2*545							
WX	REFB	450	509	614	DEFINED	182		

## APPENDIX A-4. INDEX OF VARIABLES FOR COMPUTER PROGRAMS FOR HYPOTHETICAL HURRICANES

VARIABLES		PROGRAM SURGE						
		REFS				DEFINED		
A	REFS	114	530	745	DEFINED	173		
AHOUR	REFS	761	762	DEFINED	759	761		
AN	REFS	697	DEFINED	665				
BEAR	REFS	134	537	614	747	DEFINED	228	
BFF	REFS	537	614	682	685	747		
	DEFINED	320						
BN	REFS	676	683	684	717	DEFINED	666	
BOMR	REFS	144	437	572				
BP	REFS	134	683	DEFINED	676	717		
BY	REFS	685	DEFINED	684				
C1	REFS	697	DEFINED	150				
C2	REFS	696	DEFINED	150				
C3	REFS	681	DEFINED	150				
D	REFS	134	552	621	708	723		
	DEFINED	379						
DASQ	REFS	682	685	DEFINED	681			
DAVO	REFS	134	678	710	DEFINED	621		
DELSXN	REFS	698	DEFINED	697				
DELSYN	REFS	699	DEFINED	696				
DELT	REFS	134	139	518	586	647	682	683
		761	DEFINED	300	584	639		
DELTT	REFS	300	563	DEFINED	228			
DELX	REFS	134	696	697	DEFINED	620		
DEN	REFS	683	DEFINED	682				
DIST	REFS	134	139	499	552	620	708	723
	DEFINED	346						
DISTO	REFS	140						
DTM	REFS	681	DEFINED	680				
DTN	REFS	696	697	DEFINED	679			
DTB	REFS	679	680	DEFINED	678			
E	REFS	451	DEFINED	149				
EE	REFS	451	DEFINED	446				
I	REFS	173	297	299	300	430	443	530
		745	DEFINED	173	296	298	430	442
		745						
IB	REFS	341	346	366	374	379	400	411
		430	474	DEFINED	340	366	400	409
		474						
ICHECK	REFS	408	469	DEFINED	186	193	224	258
		270	316	335	361	395	453	
ID	REFS	212	219	285	292	306	313	325
		332	356	384	390	DEFINED	181	228
		301	320	346	379			
IDA	REFS	219	292	313	332	DEFINED	213	286
		307	326					
IDATA	REFS	159	429	433	555	DEFINED	153	
	REFS	144						
IDUM	REFS	167	771	773	DEFINED	156	773	
ID1	REFS	451	460	DEFINED	430			
ID2	REFS	451	460	DEFINED	430			
ID3	REFS	451	460	DEFINED	430			
IE	REFS	345	346	365	378	379	399	415
		430	473	DEFINED	341	345	374	411
		415						
ILAST	REFS	771	DEFINED	153				
IDMIT	REFS	555	608	635	644	704	721	
	DEFINED	181						
IOMITD	REFS	555	576	DEFINED	181			
IREAD	REFS	138	164	165	DEFINED	162		
ITYPE	REFS	433	451	460	533	613	746	
	DEFINED	228						
IVX	REFS	134	433	443				
IZONE	REFS	233	238	243	248	253	264	276
		433	451	460	533	613	746	
	DEFINED	228						
K	REFS	766	767	DEFINED	734	767		
KEY	REFS	142	555	DEFINED	181			
KEYA	REFS	489	563	DEFINED	181			
L	REFS	346	379	498	499	504	506	508
		509	510	514	516	552	620	624
		628	663	665	666	667	672	674
		675	676	678	680	682	683	696
		708	710	713	714	715	716	717

VARIABLES

	DEFINED	346	379	494	551	616	623	657
LIBT	REFS	433	563	DEFINED	181			
LL	REFS	520	594	596	DEFINED	519	593	
LM	REFS	140	191	199	226	296	345	365
	REFS	378	494	551	623	723		
	DEFINED	181						
LMM	REFS	519	593	616	657	DEFINED	226	
N	REFS	489	490	518	520	584	586	594
	REFS	596	639	663	668	677	679	680
	REFS	682	683	701	710	727	729	730
	REFS	731	732	761	762	DEFINED	484	579
	REFS	760						
NC	REFS	350	367	384	401	451	475	
	DEFINED	339	367	373	401	410	475	
NM	REFS	190	199	298	484	579	631	760
	DEFINED	181						
NN	REFS	350	356	384	390	451	460	
	DEFINED	346	379	430				
NPTS	REFS	141	415	442	473	DEFINED	149	
PHI	REFS	134	624	628	DEFINED	297	624	
PHII	REFS	234	239	244	249	254	276	297
	REFS	533	613	746	DEFINED	228		
PHIIA	REFS	276	DEFINED	235	240	245	250	255
PHIIB	REFS	276	DEFINED	236	241	246	251	256
PI	REFS	624	DEFINED	227				
PN	REFS	143	498	537	614	747		
	DEFINED	301						
PD	REFS	143	498	537	614	747		
	DEFINED	301						
R	REFS	140	433	446	498	537	614	747
	DEFINED	301						
RR	REFS	446	460	DEFINED	430			
RS	REFS	134	139	498				
SA	REFS	134	520	596	677	679	680	701
	REFS	710	732	762	DEFINED	299	594	663
JAA	REFS	299	DEFINED	228				
JAP	REFS	680	DEFINED	677	732			
SE	REFS	537	614	678	701	710	747	762
	DEFINED	301						
SINPHI	REFS	134	696	DEFINED	628			
SP	REFS	134	520	596	667	DEFINED	498	594
	REFS	663						
SPN	REFS	674	679	680	701	710	713	730
	DEFINED	667						
SPP	REFS	134	680	DEFINED	674	715		
SPRES	REFS	134	762	DEFINED	730			
ST	REFS	710	731	DEFINED	701			
STOT	REFS	134	762	DEFINED	731			
SUMSX	REFS	698	701	702	710	713	727	
	DEFINED	640	698					
SUMSY	REFS	699	701	702	710	714	728	
	DEFINED	641	699					
SW	REFS	710	729	DEFINED	702			
SWW	REFS	134	762	DEFINED	729			
SX	REFS	134	762	DEFINED	727			
SXP	REFS	134	678	DEFINED	672	713		
SY	REFS	134	762	DEFINED	728			
SYP	REFS	134	678	DEFINED	673	714		
T	REFS	134	139	508	509			
TI	REFS	586	587	647	648	DEFINED	585	646
III	REFS	585	587	646	648	DEFINED	574	586
	REFS	647						
VF	REFS	140	537	614	747	DEFINED	301	
VN	REFS	686	692	696	700	716		
	DEFINED	683	693	695				
VNN	REFS	691	DEFINED	686				
VNTBST	REFS	691	693	695	DEFINED	685		
VP	REFS	134	682	685	DEFINED	675	716	
V8	REFS	710	DEFINED	700				
VX	REFS	134	139	DEFINED	430	443		
W	REFS	134	139	504	506	510	514	
WIND	REFS	507	DEFINED	504	506			
WK	REFS	134	520	596	663	666		
	DEFINED	514	516	594	663			
WKCOR	REFS	665	666	DEFINED	151			
WSO	REFS	508	509	DEFINED	507			
WXX	REFS	134	520	596	665	DEFINED	508	594
	REFS	663						
WXY	REFS	134	520	596	666	DEFINED	509	594
	REFS	663						
WX	REFS	140	537	614	747	DEFINED	320	
XP	REFS	140						

SUBROUTINE HRWDPF

VARIABLES	REFS	80	81	DEFINED	76			
AL	REFS	80	81	DEFINED	76			
CS	REFS	80	118	DEFINED	71			
DEL	REFS	118	119	DEFINED	117			
DELA	REFS	112	114	115	117	DEFINED	75	120
DELB	REFS	116	117	120	DEFINED	115	116	
DISTO	REFS	69	74	76				
LM	REFS	69						
PN	REFS	70	111					
PO	REFS	70	111					
PRESS	REFS	112	DEFINED	111				
R	REFS	69	74	80	84	111		
RAD1	REFS	109	DEFINED	71				
RAD2	REFS	109	DEFINED	71				
RS	REFS	103	105	111	112	DEFINED	100	
SN	REFS	81	119	DEFINED	71			
SNTF	REFS	74	DEFINED	72				
TEST	REFS	114	116	DEFINED	74			
TH	REFS	101	104	105	109	110	112	
	DEFINED	109	110					
VF	REFS	69	84	104				
VXX	REFS	103	104					
W	REFS	112	DEFINED	104				
WX	REFS	69	88	100				
X	REFS	100	101	118	DEFINED	80	118	
XP	REFS	69	84					
Y	REFS	100	101	119	DEFINED	81	119	

SUBROUTINE QUAD

VARIABLES	REFS	20	23	37	39	DEFINED	16	19
ERRDR	REFS	20	23	37	39	DEFINED	16	19
	REFS	33						
PI	REFS	44	46	56	DEFINED	14		
TH	REFS	56	61	DEFINED	1	44	46	49
	REFS	32						
X	REFS	18	31	32	33	34	39	44
	REFS	49	DEFINED	1				
Y	REFS	19	31	32	33	34	39	44
	REFS	49	DEFINED	1				

SUBROUTINE DEFL

VARIABLES	REFS	12			DEFINED	13		
DISTO	REFS	12						
LM	REFS	12						
PI	REFS	16	18	21	DEFINED	13		
R	REFS	12	14	15	16	21		
RHO	REFS	14	15	16	21	DEFINED	1	
TH	REFS	16	18	21	DEFINED	1	16	18
	REFS	21						
VF	REFS	12						
WX	REFS	12						
XP	REFS	12						



SUBROUTINE VEL

VARIABLES	REFS							
DELT	REFS	12		13				
DIST	REFS	12		13				
DISTO	REFS	14						
I	REFS	34		36	DEFINED		33	
LM	REFS	14						
M	REFS	41		42	DEFINED		36	
NPTS	REFS	15		33				
R	REFS	14		18				
RHO	REFS	18	DEFINED			1		
RB	REFS	12		13				
T	REFS	12		13				
VF	REFS	14						
VX	REFS	12		13		41		42
VXX	DEFINED	1		30		42		
W	REFS	12		13				
WX	REFS	14						
XI	REFS	20		28		35	42	DEFINED 18
XM	REFS	42	DEFINED			41		
XP	REFS	14						
XX	REFS	35		42	DEFINED		34	

SUBROUTINE STRIPT

VARIABLES	REFS							
CS	REFS	24		56	DEFINED		22	
DELT	REFS	17		20				
DIST	REFS	17		20				
DISTO	REFS	18		62	DEFINED		61	
EE	REFS	32		34	DEFINED		31	33
I	REFS	27		29		31	33	42 48
	DEFINED	26						
LM	REFS	18						
NPTS	REFS	19		26				
R	REFS	18		52		56	58	
RHO	REFS	50		52		61	62	DEFINED 29 50
		52						
RS	REFS	17		20				
SN	REFS	58	DEFINED			22		
T	REFS	17		20				
VF	REFS	18		24				
VX	REFS	17		20		27	31	33 42 48
VXZ	REFS	27		33		42	48	DEFINED 24
W	REFS	17		20				
WX	REFS	18		24				
XM	REFS	50	DEFINED			46	48	
XP	REFS	18		61	DEFINED		58	
YPP	REFS	61	DEFINED			56		

SUBROUTINE PROFIL

VARIABLES	REFS	225	226	248	249		
AX	REFS 8	225	226	248	249		
BOMB	REFS 12	274	DEFINED	79	92	94	101
	114	134	150	156	160	285	
DUM	REFS 10	99	DEFINED	98			
E	REFS 126	144	DEFINED	57			
EE	REFS 126	DEFINED	125				
ERR	REFS 10	79	92	94	101	114	134
	150	156	160	DEFINED	62		
I	REFS 161	226	245	249	252	265	
	DEFINED 159	214	237	265			
IA	REFS 215	217	218	223	224		
	DEFINED 207	217					
IAX	REFS 8	161	163	DEFINED	139		
IB	REFS 120	121	125	127	263	265	266
	DEFINED 119	127	261	268			
ID	REFS 76	95	96	DEFINED	67	68	69
	70	71	72	73	74		
IDATA	DEFINED 1	REFS	13	89	98	121	139
IE	REFS 121	135	137	144	151	152	264
	265	267	DEFINED	120	137	263	264
IFLAG	REFS 172	DEFINED	88	117	170		
IPUD	REFS 213	DEFINED	208	216			
IRR	REFS 8	265	DEFINED	1	249	252	
ITYPE	REFS 265	DEFINED	1				
IZONE	REFS 68	69	70	71	72	73	74
	80	265	DEFINED	1			
JID	REFS 93	95	96	DEFINED	89		
K	REFS 121	144	151	152	DEFINED	121	143
KK	REFS 89	95	102	104	105	106	111
	115	121	125	144	151	185	189
	191	215	218	223	224	226	234
	243	245	246	DEFINED	87	112	182
	202	244					
KX	REFS 161	DEFINED	57				
L	REFS 185	189	223	224	232		
	DEFINED 185	189	222	232			
LIST	REFS 181	229	256	DEFINED	1		
MP	REFS 185	189	DEFINED	152			
NC	REFS 265	269	DEFINED	262	269		
NPTS	REFS 11	214	232	237	264	267	
R	REFS 8	102	104	105	106	111	170
	191	234	243	246	DEFINED	89	111
RR	REFS 104	105	170	191	234	243	249
	257	265	DEFINED	1			
VR2	REFS 8	144	189	224	DEFINED	121	
VTMP	REFS 8	232	245	252	DEFINED	226	
X	REFS 215	218	226	227	DEFINED	206	227
XR2	REFS 8	125	144	151	185	215	218
	223	DEFINED	121				
XXA	REFS 8	225	248	DEFINED	223	246	
YYA	REFS 8	225	248	DEFINED	224	245	

SUBROUTINE INTERP

VARIABLES	REFS	16	37	DEFINED	1	36	37
AX	REFS 16	16	37	DEFINED	1	36	37
E	REFS 20	20	DEFINED	17			
XXA	REFS 16	16	20	29	36	37	
	DEFINED 1	1					
YYA	REFS 16	16	36	37	DEFINED	1	

SUBROUTINE ZERWND

VARIABLES									
AL	REFS	79	80	DEFINED	75				
CS	REFS	79	117	DEFINED	65				
DEL	REFS	117	118	DEFINED	114				
DFLT	REFS	60	62	74					
DIST	REFS	60	62	75	113	116			
DISTO	REFS	63	75						
HR	REFS	74	75	76	DEFINED	70	74		
KEY	REFS	61	76	82	113	120			
L	REFS	74	98	101	102	103	106	108	
		113	115	116	DEFINED	73	97		
LM	REFS	63	97	115					
M	REFS	71	76	DEFINED	1				
MX	REFS	72	73	DEFINED	71				
PI	DEFINED	65							
R	REFS	63	79	84					
RAD1	REFS	106	DEFINED	65					
RAD2	REFS	108	DEFINED	65					
RS	REFS	60	62	101	103	113			
	DEFINED	98							
SN	REFS	80	118	DEFINED	65				
T	REFS	60	62	108	DEFINED	106			
TH	REFS	99	102	103	106	113			
	DEFINED	108							
VF	REFS	63	75	84	102				
VX	REFS	60	62						
VXX	REFS	101	102						
W	REFS	60	62	113	DEFINED	102			
WX	REFS	63	84	102					
X	REFS	98	99	117	DEFINED	79	117		
XP	REFS	63	84						
Y	REFS	98	99	118	DEFINED	80	118		

## APPENDIX A-5. PROGRAM DECK STRUCTURE FOR ACTUAL HURRICANES

### ACTUAL HURRICANE DECK STRUCTURE

1. QJ590)                    MCCLUNG 752X6R1590                    162 PAGES                    JOB CARD
2. TASK(TN0072648,PWCERC1,TRTS)MCCLUNG AEC SURGE.                    JOB DEFINITION CARD
3. FTN(OPT=0,PL=10000)                    COMPILE USING FORTRAN COMPILER
4. REDUCE.                    REDUCE CORE SPACE MINIMUM VALUE
5. LGO.                    LOAD AND RUN PROGRAM
6. 7/8/9                    END OF CONTROL CARD DECK
7. MAIN PROGRAM DECK FOR SURGE
8. 7/8/9                    END OF PROGRAM DECK
9. DATA DECK
10. 7/8/9                    END OF DATA DECK
11. 6/7/8/9                    END OF JOB

## APPENDIX A-5. PROGRAM DECK STRUCTURE FOR HYPOTHETICAL HURRICANES

### HYPOTHETICAL HURRICANE DECK STRUCTURE

- |     |  |                                    |           |                     |
|-----|--|------------------------------------|-----------|---------------------|
| 1.  | QJ590)   | MCCLUNG 752X6R1590                 | 162 PAGES | JOB CARD            |
| 2.  | TASK(TNQQ72648,PWCERC1,TRTS)MCCLUNG AEC SURGE    |                                    |           | JOB DEFINITION CARD |
| 3.  | FTN(OPT=0,PL=10000)                              | COMPILE IN FORTRAN COMPILER        |           |                     |
| 4.  | REDUCE.  | REDUCE CORE SPACE TO MINIMUM VALUE |           |                     |
| 5.  | LGO.   | LOAD AND RUN PROGRAM               |           |                     |
| 6.  | 7/8/9  | END OF CONTROL CARD DECK           |           |                     |
| 7.  | MAIN PROGRAM DECK FOR SURGE                      |                                    |           |                     |
| 8.  | SUBROUTINE HRWDPF(M)                             |                                    |           |                     |
| 9.  | SUBROUTINE QUAD(X,Y,TH)                          |                                    |           |                     |
| 10. | SUBROUTINE DEFL(TH,RHO)                          |                                    |           |                     |
| 11. | SUBROUTINE VEL(RHO,VXX)                          |                                    |           |                     |
| 12. | SUBROUTINE STRPT                                 |                                    |           |                     |
| 13. | SUBROUTINE PROFIL(IZONE,ITYPE,RR,IRR,IDATA,LIST) |                                    |           |                     |
| 14. | SUBROUTINE INTERP(XXA,YYA,AX)                    |                                    |           |                     |
| 15. | SUBROUTINE ZERWNO                                |                                    |           |                     |
| 16. | 7/8/9  | END OF PROGRAM DECK                |           |                     |
| 17. | DATA DECK  |                                    |           |                     |
| 18. | 7/8/9  | END OF DATA DECK                   |           |                     |
| 19. | 6/7/8/9  | END OF JOB                         |           |                     |



1 2 3 4 5 6 7 8  
 1234567890123456789012345678901234567890123456789012345678901234567890

Storm Identification Title Card

1 2 20 40 60 80  
 1 CAMILLE VF=13.0 KNOTS (APPROX.) R=24. N. MILES (APPROX.)  
 2 NM LM ISKIP IOMIT IOMITD ID 76 80  
 3 43 14 0 0 0 PAR 1  
 3 PHII 15 20 ITYPE BEAR ID 76 80  
 33.00 CAM S 12.00 E PAR 2  
 4 R 7 9 PO PN 21 23 VF 28 30 SE WKCOR ID 76 80  
 24.00 27.63 29.92 13.00 1.20 58 63 PAR 3  
 1 WX 7 9 BFF ID 76 80  
 5 125.00 1.0000 PAR 4

Bottom profile distances from shore (values taken seaward - shoreward)

ID	4	23 DIST(L)	30	33 DIST(L)	40	43 DIST(L)	50	53 DIST(L)	60	63 DIST(L)	70	73 DIST(L)	80
6	DIST	1	77.	1	70.	1	60.	1	50.	1	40.	1	30.
7	DIST	2	20.	2	15.	2	10.	2	5.	2	3.	2	2.
8	DIST	3	7.	3	0.	3	.	3	.	3	.	3	.
9	DIST	4	.	4	.	4	.	4	.	4	.	4	.
10	DIST	5	.	5	.	5	.	5	.	5	.	5	.
11	DIST	6	.	6	.	6	.	6	.	6	.	6	.
12	DIST	7	.	7	.	7	.	7	.	7	.	7	.
13	DIST	8	.	8	.	8	.	8	.	8	.	8	.
14	DIST	9	.	9	.	9	.	9	.	9	.	9	.

Where L=1 to LM (maximum of 50)

Bottom profile depths (values taken seaward - shoreward)

ID	5	23 D(L)	30	33 D(L)	40	43 D(L)	50	53 D(L)	60	63 D(L)	70	73 D(L)	80
15	DEPTH	1	600.	1	235.	1	160.	1	90.	1	52.	1	40.
16	DEPTH	2	36.	2	35.	2	13.	2	12.	2	9.	2	9.
17	DEPTH	3	3.	3	0.	3	.	3	.	3	.	3	.
18	DEPTH	4	.	4	.	4	.	4	.	4	.	4	.
19	DEPTH	5	.	5	.	5	.	5	.	5	.	5	.
20	DEPTH	6	.	6	.	6	.	6	.	6	.	6	.
21	DEPTH	7	.	7	.	7	.	7	.	7	.	7	.
22	DEPTH	8	.	8	.	8	.	8	.	8	.	8	.
23	DEPTH	9	.	9	.	9	.	9	.	9	.	9	.

Where L=1 to LM (maximum of 50)

Wind field data where N=1 to NM ALPHA and BETA0 are supplied if interpolation for RS is required

24 IHUR NN LX DELT(N) SA(N) ALPHA BETA0  
 1 3 4 5 7 10 17 20 27 30 35 40 45 50  
 CAM 1 1.1 1.8 . . . .

Wind field data where L=1 to LM

25 IHUR NN LX RS(L) W(L) T(L)  
 1 3 4 5 7 10 15 20 25 30 35 40  
 CAM 1 1.1 274. 37. 86.

1 2 3 4 5 6 7 8  
 1234567890123456789012345678901234567890123456789012345678901234567890

SAMPLE

ACTUAL HURRICANE DATA DECK

Storm Identification Title									
1.	CAMILLE	VF=13.0	KNOTS (APPROX.)	R=24.	N. MILES (APPROX.)	HEADER CARD			
2.	NM	IM	ISKIP	ICMIT	ICMID	ID PAR 1			
	43	14	0.	0	0				
3.	PHII			ITYPE		BEAR		ID PAR 2	
	30.00			CAM S		12.00 E			
4.	R	PO	PN	VF	SE	WKCOR		ID PAR 3	
	14.00	27.63	29.92	13.00	1.20	1.10			
5.	WX	BFF		ID PAR 4					
	125.0	.0030							
6.	ID	NN	DIST(L)	DIST(L)	DIST(L)	DIST(L)	DIST(L)	DIST(L)	DIST(L)
	DIST	1	77.	70.	60.	50.	40.	30.	
7.	DIST	2	20.	15.	10.	5.	3.	2.	
8.	DIST	3	1.	0.					
9.	ID	NN	D(L)	D(L)	D(L)	D(L)	D(L)	D(L)	D(L)
	DEPTH	1	600.	235.	160.	90.	52.	40.	
10.	DEPTH	2	36.	35.	13.	12.	9.	9.	
11.	DEPTH	3	3.	0.					
12.	IHUR	NN	DELT(L)	SA(L)					
	CAM	1	1.	.8					
13.	IHUR	NN	LX	RS(L)	N(L)	T(L)	THESE CARDS GIVE DATA TAKEN FROM ISOVEL CHARTS		
	CAM	1	1	274.	37.	86.			

HURRICANE PARAMETER CARDS

THESE CARDS DEFINE BATHYMETRIC PROFILE





1 2 3 4 5 6 7 8  
 1|2|3|4|5|6|7|8|9|0|1|2|3|4|5|6|7|8|9|0|1|2|3|4|5|6|7|8|9|0|1|2|3|4|5|6|7|8|9|0|1|2|3|4|5|6|7|8|9|0|1|2|3|4|5|6|7|8|9|0

ILAST IDATA

1 1 5 10  
 1 1 4  
 2 If IDATA=4 pre-punched digitized wind field data decks are supplied for input  
 Storm Identification Title Card  
 2 20 40 60 80  
 3 SAMPLE STORM SURGE PROBLEM HAMPTON BEACH, N.H. HUR. H PMW 2-28-74 BY HDM  
 4 NM LM IOMIT IOMITD KEY KEYA LIST ID 76 80  
 4 3 5 9 10 20 25 30 35 40 PAR 1  
 4 4.4 4.9 2 2 1 1  
 5 DELTT PHII SAA IZONE ITYPE BEAR ID 76 80  
 5 5 10 15 20 25 30 40 48 50 52 60 PAR 2  
 5 . . . 5.3 4.2.83 111.2 4 PHM N 75.32 W  
 6 2 7 9 14 16 21 23 28 30 35 ID 76 80  
 6 56. 27.42 33.42 37. . . . .6 PAR 3  
 7 WX 7 9 14 BFF ID 76 80  
 7 1 2 3 4 1 0 2 5 PAR 4

Bottom profile distances from shore (values taken seaward - shoreward)

ID	NN	DIST (L)	DIST (L)	DIST (L)	DIST (L)	DIST (L)	DIST (L)
8	1	23	30	33	40	43	50
9	2	47.6	46.6	43.6	40.6	39.6	38.6
10	3	37.6	36.6	35.6	31.6	30.6	29.6
11	4	28.6	27.5	26.5	25.5	25.5	24.5
12	5	24.	23.5	22.5	21.5	20.5	20.
13	6	19.	16.	15.	14.	12.5	10.
14	7	8.5	5.	3.	2.	1.8	1.6
15	8	1.4	1.2	1.	.9	.8	.7
16	9	.6	.5	.4	.3	.2	.1

Where L = 1 to LM (maximum of 50)

Bottom profile depths (values taken seaward - shoreward)

ID	NN	D(L)	D(L)	D(L)	D(L)	D(L)	D(L)
17	1	23	30	33	40	43	50
18	2	600.	580.	456.	446.	410.	420.
19	3	432.	416.	402.	226.	230.	266.
20	4	324.	300.	190.	189.	196.	192.
21	5	186.	167.	165.	270.	276.	292.
22	6	397.	382.	365.	365.	317.	175.
23	7	249.	156.	69.	64.	60.	60.
24	8	49.	41.	33.	30.	28.	16.
25	9	15.	13.	11.	10.	9.	4.

Where L = 1 to LM (maximum of 50)

26 If IDATA = 0 pre-punched relative wind field data deck is supplied for input

1 2 3 4 5 6 7 8  
 1|2|3|4|5|6|7|8|9|0|1|2|3|4|5|6|7|8|9|0|1|2|3|4|5|6|7|8|9|0|1|2|3|4|5|6|7|8|9|0|1|2|3|4|5|6|7|8|9|0|1|2|3|4|5|6|7|8|9|0

HYPOTHETICAL HURRICANE DATA DECK

ILAST IDATA

1. 1 4
2. JID R(KK)  
000000001005000
3. XR2 VR2 XR2 VR2 XR2 VR2 XR2 VR2 XR2 VR2 XR2 VR2 XR2 VR2  
+0000+0000+0001+0000+0002+0001+0003+0002+0004+0002+0005+0004+0006+0006+0007+0006
4. +0008+0007+0009+0008+0010+0009+0011+0011+0012+0012+0013+0013+0014+0015+0015+0016
5. +0016+0016+0017 ETC.
6. 99999
7. IAX  
.....
8. JID R(KK)  
00000000100660
9. XR2 VR2 XR2 VR2 XR2 VR2 XR2 VR2 XR2 VR2 XR2 VR2 XR2 VR2  
+0000+0000+0001+0000+0002+0001+0003+0002+0004+0002+0005+0004+0006+0006+0007+0006
10. +0008+0007+0009+0008+0010+0009+0011+0011+0012+0012+0013+0013+0014+0015+0015+0016
11. +0016+0016+0017 ETC.
12. 99999
13. IAX  
.....
14. FINAL

When IDATA = 4 this type of data is supplied as input for interpolation; then wind field data is computed

15. SAMPLE STORM SURGE PROBLEM Storm Identification Title HAMPTON BEACH,N.H. HUR, H PMH 2-28-74 BY HDM HEADER CARD
16. NM LM IOMIT IOMITD KEY KEYA LIST ID PAR 1  
44 49 0 0 1 1 1
17. DELTT PHII SAA IZONE ITYPE BEAR ID PAR 2  
.50 42.83 11.2 4 PMH N 75.00 W
18. R PG PN VF SE ID PAR 3  
56. 27.82 30.42 37. .6
19. WX BFF ID PAR 4  
120.4 .0025

HURRICANE PARAMETER CARDS

20.	ID DIST	NN 1	DIST(L) 47.6	DIST(L) 46.6	DIST(L) 43.6	DIST(L) 40.6	DIST(L) 39.6	DIST(L) 38.6
21.	DIST	2	37.6	36.6	35.6	31.6	30.6	29.6
22.	DIST	3	28.6	27.	26.	25.5	25.	24.5
23.	DIST	4	24.	23.5	22.5	21.5	20.5	20.
24.	DIST	5	19.	16.	15.	14.	12.5	10.
25.	DIST	6	8.5	5.	3.	2.	1.8	1.6
26.	DIST	7	1.4	1.2	1.	.9	.8	.7
27.	DIST	8	.6	.5	.4	.3	.2	.1
28.	DIST	9	0.0					
29.	ID DEPTH	NN 1	D(L) 600.	D(L) 580.	D(L) 456.	D(L) 446.	D(L) 410.	D(L) 420.
30.	DEPTH	2	432.	416.	402.	226.	230.	266.
31.	DEPTH	3	324.	300.	190.	189.	196.	194.
32.	DEPTH	4	186.	167.	165.	278.	296.	294.
33.	DEPTH	5	397.	382.	365.	365.	317.	175.
34.	DEPTH	6	249.	156.	69.	64.	60.	60.
35.	DEPTH	7	49.	41.	33.	30.	28.	16.
36.	DEPTH	8	15.	13.	11.	10.	9.	4.
37.	DEPTH	9	0.0					

THESE CARDS DEFINE  
BATHYMETRIC PROFILE

HYPOTHETICAL HURRICANE DATA DECK

1.	ILAST IDATA								
	1	0							
2.	SAMPLE STORM SURGE PROBLEM		Storm Identification Title					HEADER CARD	
			HAMPTON BEACH, N.H.					HUR, H PMH 2-28-74 BY MDM	
3.	NM	LM	IOMIT	IOMITD	KEY	KEYA	LIST	ID	
	44	49	0	0	1	1	1	PAR 1	
4.	DELTT		PHII	SAA	IZONE	ITYPE	BEAR	ID	
	.50		42.83	11.2	4	PMH N	75.00 W	PAR 2	HURRICANE PARAMETER CARDS
5.	R	PO	PN	VF	SE	ID			
	56.	27.42	30.42	37.	.6	PAR 3			
6.	WX	BFF						ID	
	120.4	.0025						PAR 4	
7.	ID	NN	DIST(L)	DIST(L)	DIST(L)	DIST(L)	DIST(L)	DIST(L)	
	DIST	1	47.6	46.6	43.6	40.6	39.6	38.6	
8.	DIST	2	37.6	36.6	35.6	31.6	30.6	29.6	
9.	DIST	3	28.6	27.	26.	25.5	25.	24.5	
10.	DIST	4	24.	23.5	22.5	21.5	20.5	20.	
11.	DIST	5	19.	16.	15.	14.	12.5	10.	
12.	DIST	6	8.5	5.	3.	2.	1.8	1.6	
13.	DIST	7	1.4	1.2	1.	.9	.8	.7	
14.	DIST	8	.6	.5	.4	.3	.2	.1	THESE CARDS DEFINE BATHYMETRIC PROFILE.
15.	DIST	9	0.0						

16.	ID DEPTH	NN 1	D(L) 600.	D(L) 580.	D(L) 456.	D(L) 446.	D(L) 410.	D(L) 420.							
17.	DEPTH	2	432.	416.	402.	226.	230.	266.							
18.	DEPTH	2	432.	416.	402.	226.	230.	266.							
19.	DEPTH	3	324.	300.	190.	189.	196.	194.							
20.	DEPTH	4	186.	167.	165.	278.	296.	294.							
21.	DEPTH	5	397.	382.	365.	365.	317.	175.							
22.	DEPTH	6	249.	156.	69.	64.	60.	60.							
23.	DEPTH	7	49.	41.	33.	30.	28.	16.							
24.	DEPTH	8	15.	13.	11.	10.	9.	4.							
25.	DEPTH	9	0.0												
26.	ID1 ID2 ID3 ZONE 4 PMH	RR 56.0	NN 1	VX(I) 896	VX(I) 917	VX(I) 930	VX(I) 945	VX(I) 954	VX(I) 965	VX(I) 972	VX(I) 981	VX(I) 986	VX(I) 991	VX(I) 999	VX(I) 998
27.	ZONE 4 PMH	56.0	2	998	997	996	995	993	991	989	986	982	976	970	965
28.	ZONE 4 PMH	56.0	3	ETC.											

When IDATA = 0 this type  
of data is supplied as  
input for computing wind  
field data

APPENDIX A-7. OUTPUT LISTING EXAMPLES FOR ACTUAL HURRICANES\*

Output for ISKIP = 0

AUD	DELTA(N)	SA(N)	ALPHA	BETA0
AUD 1	1	2.0	.4	194.0
	L	DIST	RS(1)	FUD
				RS(L)
16	2	1,000	148,000	.695
16	3	2,000	148,000	.695
16	4	3,000	148,000	.695
16	5	5,000	148,000	.695
16	6	10,000	148,000	.695
16	7	15,000	148,000	.695
16	8	20,000	148,000	.695
16	9	30,000	148,000	.695
16	10	40,000	148,000	.695
16	11	50,000	148,000	.695
16	12	60,000	148,000	.695
16	13	70,000	148,000	.695
16	14	80,000	148,000	.695
16	15	90,000	148,000	.695

Output for ISKIP = 0 or 1

AUD	ACTUAL INPUT	OF RS(L)	W(L)	T(L)
AUD 1	1	107.2	30.0	1.1
AUD 1	2	108.9	33.4	1.1
AUD 1	3	111.4	35.3	1.1
AUD 1	4	114.7	36.7	1.1
AUD 1	5	118.8	38.7	1.1
AUD 1	6	123.6	41.0	1.1
AUD 1	7	129.0	42.2	1.1
AUD 1	8	134.9	43.3	1.0
AUD 1	9	138.0	45.7	1.0
AUD 1	10	141.2	48.1	.9
AUD 1	11	144.6	50.2	.8
AUD 1	12	145.9	52.7	.8
AUD 1	13	146.6	55.2	.7
AUD 1	14	147.3	57.6	.6
AUD 1	15	148.0	59.6	.5

AUD	DELTA(N)	SA(N)	ALPHA	BETA0
AUD 2	2	2.0	.1	194.0
	L	DIST	RS(1)	FUD
				RS(L)
16	2	1,000	140,000	.602
16	3	2,000	140,000	.602
16	4	3,000	140,000	.602
16	5	5,000	140,000	.602
16	6	10,000	140,000	.602
16	7	15,000	140,000	.602
16	8	20,000	140,000	.602
16	9	30,000	140,000	.602
16	10	40,000	140,000	.602
16	11	50,000	140,000	.602
16	12	60,000	140,000	.602
16	13	70,000	140,000	.602
16	14	80,000	140,000	.602
16	15	90,000	140,000	.602

AUD	ACTUAL INPUT	OF RS(L)	W(L)	T(L)
AUD 2	1	112.0	41.0	1.0
AUD 2	2	111.9	41.1	1.1
AUD 2	3	112.7	41.2	1.1
AUD 2	4	114.4	41.5	1.1
AUD 2	5	116.9	41.9	1.0
AUD 2	6	120.2	43.1	1.0
AUD 2	7	124.3	44.2	1.0
AUD 2	8	129.0	45.4	1.0
AUD 2	9	131.5	47.7	.9
AUD 2	10	134.2	50.0	.8
AUD 2	11	137.0	52.4	.8
AUD 2	12	138.2	54.7	.7
AUD 2	13	138.8	57.0	.6
AUD 2	14	139.4	59.2	.5
AUD 2	15	140.0	60.0	.4

\*See Appendix A-3 for definition of variables.





Output when control parameter IØMITD = 0

\*\*\*\*\*

TIME (T) = 0.00      TIME (T+DELT) = 2.00

WWX(I+1)	WWX(I)	WWY(I+1)	WWY(I)	ASTRO. TIDE (FEET)	SP(I+1) (FT)	SP(I) (FT)	WIND STRESS PARAMETER
471.5	380.4	1011.0	815.7	.40	.32	.32	.00000164
526.6	471.5	1129.3	1011.0	.40	.31	.32	.00000178
571.4	526.6	1219.7	1129.3	.40	.30	.31	.00000185
635.3	571.4	1356.3	1219.7	.40	.29	.30	.00000190
716.9	635.3	1510.9	1356.3	.40	.28	.29	.00000196
830.6	736.9	1575.3	1510.9	.40	.27	.28	.00000203
937.4	830.6	1623.7	1575.3	.40	.26	.27	.00000206
1173.9	937.4	1727.3	1623.7	.40	.26	.26	.00000209
1446.6	1173.9	1805.6	1727.3	.40	.25	.26	.00000216
1709.0	1446.6	1852.0	1805.6	.40	.25	.25	.00000221
2024.6	1709.0	1901.2	1852.0	.40	.24	.25	.00000226
2361.3	2024.6	1925.8	1901.2	.40	.24	.24	.00000231
2711.1	2361.3	1912.8	1925.8	.40	.24	.24	.00000236
3038.4	2711.1	1840.1	1912.5	.40	.24	.24	.00000240

\*\*\*\*\*

TIME (T) = 2.00      TIME (T+DELT) = 4.00

WWX(I+1)	WWX(I)	WWY(I+1)	WWY(I)	ASTRO. TIDE (FEET)	SP(I+1) (FT)	SP(I) (FT)	WIND STRESS PARAMETER
821.5	840.3	1476.0	1455.8	.10	.31	.31	.00000203
830.7	821.5	1480.3	1476.0	.10	.31	.31	.00000203
850.7	830.7	1497.5	1480.3	.10	.31	.31	.00000204
877.8	850.7	1520.4	1497.5	.10	.30	.31	.00000204
956.7	877.8	1592.3	1520.4	.10	.29	.30	.00000206
1049.7	956.7	1647.7	1592.3	.10	.28	.29	.00000209
1158.5	1049.7	1704.7	1647.7	.10	.27	.28	.00000212
1397.7	1158.5	1795.4	1704.7	.10	.27	.27	.00000215
1672.8	1397.7	1857.9	1795.4	.10	.26	.27	.00000220
1998.3	1672.8	1883.1	1857.9	.10	.26	.26	.00000226
2344.9	1998.3	1858.5	1883.1	.10	.26	.26	.00000231
2715.5	2344.9	1783.8	1858.5	.10	.26	.26	.00000235
3094.4	2715.5	1645.3	1783.8	.10	.25	.26	.00000239
3326.0	3094.4	1377.7	1645.3	.10	.25	.25	.00000243

\*\*\*\*\*



Output when control parameter IØMIT = 0

TIME (T) = 2.00      TIME (T+DELT) = 4.00

DIST. (NM)	DEPTH (FT,MLW)	D AVG. (FT,MLW)	PRES,RISE (FT)	ASTRO,TIDE (FT)	INITIAL LEVEL(FT)	Y=FLUX (FT*FT/SEC)	ONSHORE SETUP (FT)	ALONGSHORE SETUP (FT)	TOTAL WIND SETUP (FT)	TOTAL SURGE (FT)
90.0	600.0	460.0	.31	.10	1.00	63.6	.016	.016	.035	1.43
80.0	320.0	290.0	.31	.10	1.00	73.7	.042	.052	.094	1.50
70.0	260.0	230.0	.31	.10	1.00	80.3	.075	.099	.174	1.58
60.0	200.0	170.0	.30	.10	1.00	86.5	.121	.166	.287	1.69
50.0	140.0	100.0	.30	.10	1.00	91.4	.204	.287	.491	1.89
40.0	60.0	55.0	.29	.10	1.00	83.8	.368	.484	.852	2.24
30.0	50.0	35.0	.28	.10	1.00	66.0	.648	.721	1.369	2.75
20.0	20.0	19.0	.27	.10	1.00	40.5	.930	.846	1.776	3.15
15.0	18.0	16.5	.27	.10	1.00	37.9	1.314	.975	2.289	3.65
10.0	15.0	15.0	.26	.10	1.00	37.1	1.806	1.107	2.913	4.27
5.0	15.0	13.5	.26	.10	1.00	35.1	2.060	1.160	3.220	4.58
3.0	12.0	11.0	.26	.10	1.00	30.4	2.233	1.187	3.420	4.78
2.0	10.0	7.5	.25	.10	1.00	23.5	2.488	1.213	3.700	5.06
1.0	5.0	2.5	.25	.10	1.00	14.7	2.916	1.237	4.153	5.51
0.0	0.0									

# Standard Program Output

## SUMMARY OF OPEN COAST STORM SURGE PROBLEM

HURRICANE AUDREY VERIFICATION AT EUGENE ISLAND LA. JUN 1957 AEC 7/73

FIRST THREE LETTERS OF HURRICANE NAME (ITYPE) AUD

\*\*\*

CENTRAL PRESSURE (PO) = 27.95 IN. HG.	PERIPHERAL PRESSURE (PN) = 29.70 IN. HG.
RADIUS TO MAXIMUM WIND (R) = 19.0 N.M.	MAXIMUM WIND (WX) = 95.0 MI/HR.
TRANSLATION SPEED (VF) = 14.0 KNOTS	INITIAL RISE (SE) = 1.00 FEET
BOTTOM FRICTION FACTOR (BFF) = .0025	WIND STRESS CORRECTION FACTOR (=KCOR) = 1.10
GEOGRAPHICAL LATITUDE(PHII) = 29.30	TRAVERSE BEARING (BEAR) = S 12.5 W

TIME (HOURS)	SETUP X=COMP. (FT. MLW)	SETUP Y=COMP. (FT. MLW)	TOT. WIND SETUP (FT. MLW)	AST. TIDE LEVEL (FT. MLW)	INITIAL WATER LEVEL (FT. MLW)	PRESSURE SETUP (FT. MLW)	TOTAL WATER LEVEL (FT. MLW)
2.00	3.29	.99	4.28	-.40	1.00	.24	5.12
4.00	2.92	1.24	4.15	.10	1.00	.25	5.51
6.00	3.84	1.38	5.22	.52	1.00	.27	7.01
8.00	4.26	1.45	5.70	.32	1.00	.28	7.30
10.00	6.11	1.44	7.56	-.12	1.00	.30	8.73
12.00	4.77	.74	5.50	-.05	1.00	.29	6.74
14.00	3.50	.53	4.03	.08	1.00	.28	5.39
16.00	3.13	.25	3.37	-.18	1.00	.26	4.45
18.00	3.62	.10	3.71	-.80	1.00	.23	4.14
20.00	4.09	.14	4.23	-1.60	1.00	.21	3.84
22.00	3.21	-.04	3.18	-1.90	1.00	.20	2.47
24.00	2.29	-.08	2.21	-1.80	1.00	.18	1.99
26.00	1.81	-.18	1.62	-.60	1.00	.16	2.19

## APPENDIX A-7. OUTPUT LISTING EXAMPLES FOR HYPOTHETICAL HURRICANES\*

### Standard Program Output

\*\*\*\*\*  
THIS IS THE BEGINNING OF RUN NUMBER 1  
\*\*\*\*\*

Output when control parameter LIST = 1

XR2

0.000	.010	.020	.030	.040	.050	.060	.070	.080	.090	.100	.110	.120	.130	.140	.150
.160	.170	.180	.190	.200	.210	.220	.230	.240	.250	.260	.270	.280	.290	.300	.310
.320	.330	.340	.350	.360	.370	.380	.390	.400	.410	.420	.430	.440	.450	.460	.470
.480	.490	.500	.510	.520	.530	.540	.550	.560	.570	.580	.590	.600	.610	.620	.630
.640	.650	.660	.670	.680	.690	.700	.710	.720	.730	.740	.750	.760	.770	.780	.790
.800	.810	.820	.830	.840	.850	.860	.870	.880	.890	.900	.910	.920	.930	.940	.950
.960	.970	.980	.990	1.000	1.070	1.100	1.130	1.170	1.200	1.230	1.270	1.300	1.330	1.370	1.400
1.430	1.470	1.500	1.530	1.570	1.600	1.630	1.670	1.700	1.730	1.770	1.800	1.830	1.870	1.900	1.930
1.970	2.000	2.030	2.070	2.100	2.130	2.170	2.250	2.350	2.450	2.550	2.650	2.750	2.850	2.950	3.050
3.150	3.250	3.350	3.450	3.550	3.650	3.750	3.850	3.950	4.050	4.150	4.250	4.350	4.450	4.550	4.650
4.750	4.850	4.950	5.050	5.150	5.250	5.350	5.450	5.550	5.650	5.750	5.850	5.950	6.050	6.150	6.250
6.350	6.450	6.550	6.650	6.750	6.850	6.950	7.050	7.150	7.250	7.350	7.450	7.550	7.650	7.750	7.850
7.950	8.050	8.150	8.250	8.350	8.450	8.550	8.650	8.750	8.850	8.950	9.050	9.150	9.250	9.350	9.450
9.550	9.650	9.750	9.850	9.950	10.050	10.150	10.250	10.350	10.450	10.550	10.650	10.750	10.850	10.950	11.050
11.150	11.250	11.350	11.450	11.550	11.650	11.750	11.850	11.950	12.050	12.150	12.250	12.350	12.450	12.550	12.650
12.750	12.850	12.950	13.050	13.150	13.250	13.350	13.450	13.550	13.650	13.750	13.850	13.950	14.050	14.150	14.250
14.350	14.450	14.550	14.650	14.750	14.850	14.950	15.050	15.150	15.250	15.350	15.450				

VR2

0.000	0.000	.001	.002	.002	.004	.006	.006	.007	.008	.009	.011	.012	.013	.015	.016
.018	.018	.020	.023	.024	.027	.028	.031	.033	.036	.040	.043	.045	.048	.055	.058
.063	.066	.075	.080	.086	.091	.102	.108	.115	.122	.136	.143	.152	.161	.178	.188
.198	.207	.231	.243	.258	.271	.300	.315	.334	.353	.395	.418	.446	.471	.523	.547
.570	.594	.637	.658	.681	.702	.738	.760	.780	.794	.822	.834	.845	.857	.875	.885
.896	.904	.917	.924	.930	.934	.945	.950	.954	.959	.965	.970	.972	.976	.981	.984
.986	.989	.991	.992	.999	.997	.995	.992	.989	.984	.976	.966	.950	.938	.928	.916
.903	.892	.882	.872	.863	.851	.841	.830	.821	.809	.800	.791	.785	.772	.763	.756
.748	.739	.732	.725	.716	.710	.702	.684	.665	.645	.626	.612	.593	.579	.568	.555
.542	.531	.518	.508	.498	.490	.480	.469	.459	.445	.432	.425	.417	.409	.402	.402
.394	.386	.377	.371	.365	.358	.351	.346	.339	.333	.327	.321	.313	.307	.302	.295
.289	.284	.277	.274	.268	.264	.257	.251	.246	.241	.238	.230	.225	.220	.215	.210
.206	.200	.195	.191	.186	.181	.177	.175	.169	.165	.161	.158	.154	.151	.148	.144
.140	.137	.134	.131	.128	.125	.122	.120	.117	.115	.112	.110	.107	.104	.104	.102
.099	.096	.094	.092	.090	.089	.086	.083	.082	.081	.080	.078	.075	.073	.073	.071
.069	.067	.066	.065	.063	.063	.062	.060	.060	.059	.057	.056	.056	.054	.053	.053
.050	.049	.048	.048	.048	.048	.047	.045	.045	.043	.043	.043				

XR2

0.000	.010	.020	.030	.040	.050	.060	.070	.080	.090	.100	.110	.120	.130	.140	.150
.160	.170	.180	.190	.200	.210	.220	.230	.240	.250	.260	.270	.280	.290	.300	.310
.320	.330	.340	.350	.360	.370	.380	.390	.400	.410	.420	.430	.440	.450	.460	.470
.480	.490	.500	.510	.520	.530	.540	.550	.560	.570	.580	.590	.600	.610	.620	.630
.640	.650	.660	.670	.680	.690	.700	.710	.720	.730	.740	.750	.760	.770	.780	.790
.800	.810	.820	.830	.840	.850	.860	.870	.880	.890	.900	.910	.920	.930	.940	.950
.960	.970	.980	.990	1.000	1.030	1.070	1.100	1.130	1.170	1.200	1.230	1.270	1.300	1.330	1.370
1.400	1.430	1.470	1.500	1.530	1.570	1.600	1.630	1.670	1.700	1.730	1.770	1.800	1.830	1.870	1.900
1.930	1.970	2.000	2.030	2.060	2.140	2.240	2.340	2.440	2.540	2.640	2.740	2.840	2.940	3.040	3.140
3.240	3.340	3.440	3.540	3.640	3.740	3.840	3.940	4.040	4.140	4.240	4.340	4.440	4.540	4.640	4.740
4.840	4.940	5.040	5.140	5.240	5.340	5.440	5.540	5.640	5.740	5.840	5.940	6.040	6.140	6.240	6.340
6.440	6.540	6.640	6.740	6.840	6.940	7.040	7.140	7.240	7.340	7.440	7.540	7.640	7.740	7.840	7.940
8.040	8.140	8.240	8.340	8.440	8.540	8.640	8.740	8.840	8.940	9.040	9.140	9.240	9.340	9.440	9.540
9.640	9.740	9.840	9.940	10.040	10.140	10.240	10.340	10.440	10.540	10.640	10.740	10.840	10.940	11.040	11.140
11.240	11.340	11.440	11.540	11.640	11.740	11.840	11.940	12.040	12.140	12.240	12.340	12.440	12.540	12.640	12.740
12.840	12.940	13.040	13.140	13.240	13.340	13.440	13.540	13.640	13.740	13.840	13.940	14.040	14.140	14.240	14.340
14.440	14.540	14.640	14.740	14.840	14.940	15.040	15.140	15.240	15.340	15.440	15.540				

VR2

0.000	0.000	.001	.002	.002	.004	.006	.006	.007	.008	.009	.011	.012	.013	.015	.016
.018	.018	.020	.023	.024	.027	.028	.031	.033	.036	.040	.043	.045	.048	.055	.058
.063	.066	.075	.080	.086	.091	.102	.108	.115	.122	.136	.143	.152	.161	.178	.188
.198	.207	.231	.243	.258	.271	.300	.315	.334	.353	.395	.418	.446	.471	.523	.547
.570	.594	.637	.658	.681	.702	.738	.760	.780	.794	.822	.834	.845	.857	.875	.885
.896	.904	.917	.924	.930	.934	.945	.950	.954	.959	.965	.970	.972	.976	.981	.984
.986	.989	.991	.992	.999	.997	.995	.992	.989	.984	.976	.966	.950	.938	.928	.916
.903	.890	.881	.868	.855	.845	.833	.822	.813	.803	.791	.781	.769	.760	.750	.741
.734	.725	.715	.707	.701	.683	.660	.638	.620	.604	.584	.571	.558	.543	.530	.517
.505	.493	.483	.473	.462	.451	.441	.431	.420	.412	.402	.392	.381	.374	.365	.357
.347	.338	.331	.324	.316	.310	.303	.296	.289	.282	.274	.269	.264	.259	.253	.250
.244	.237	.229	.224	.218	.213	.207	.201	.196	.189	.184	.180	.174	.171	.167	.161
.151	.153	.147	.143	.138	.133	.129	.124	.120	.116	.112	.109	.106	.102	.098	.095
.091	.089	.086	.083	.079	.077	.075	.072	.069	.065	.062	.059	.058	.056	.054	.052
.050	.048	.045	.044	.042	.039	.038	.036	.035	.033	.032	.029	.028	.026	.024	.023
.021	.021	.020	.018	.017	.016	.015	.015	.013	.012	.011	.011	.010	.008	.008	.008
.007	.006	.005	.004	.003	.003	.002	.002	.001	.001	.000	.000				

\*See Appendix A-3 for definition of variables.





Output when control parameter LIST = 1

Re 56.0

IRR

ZONE 4 PHM 56.0	1	896	917	930	945	954	965	972	981	986	991	999	998
ZONE 4 PHM 56.0	2	998	997	996	995	993	991	989	986	982	976	970	965
ZONE 4 PHM 56.0	3	957	947	939	932	926	919	911	902	896	890	884	877
ZONE 4 PHM 56.0	4	869	863	859	852	844	837	831	826	821	814	806	800
ZONE 4 PHM 56.0	5	795	790	783	778	773	767	761	755	750	746	741	736
ZONE 4 PHM 56.0	6	730	725	721	717	712	707	703	699	694	690	685	681
ZONE 4 PHM 56.0	7	676	672	668	664	660	656	652	648	645	641	637	633
ZONE 4 PHM 56.0	8	630	626	623	619	615	612	609	606	602	599	596	593
ZONE 4 PHM 56.0	9	589	586	583	580	577	575	572	569	567	564	562	559
ZONE 4 PHM 56.0	10	557	554	552	549	546	544	541	539	536	533	531	529
ZONE 4 PHM 56.0	11	526	524	522	520	517	514	512	509	507	505	503	501
ZONE 4 PHM 56.0	12	499	497	495	493	491	489	487	485	484	482	480	478
ZONE 4 PHM 56.0	13	476	474	472	470	468	466	463	461	459	457	455	453
ZONE 4 PHM 56.0	14	451	449	447	445	444	442	440	438	437	435	433	432
ZONE 4 PHM 56.0	15	430	428	426	424	423	421	419	417	415	413	411	409
ZONE 4 PHM 56.0	16	408	406	404	402	401	399	398	396	395	393	392	390
ZONE 4 PHM 56.0	17	389	387	385	384	382	381	379	377	375	374	372	370
ZONE 4 PHM 56.0	18	368	367	365	363	361	360	359	358	356	355	354	353
ZONE 4 PHM 56.0	19	351	350	349	347	346	344	343	341	340	339	337	336
ZONE 4 PHM 56.0	20	335	334	332	331	330	329	328	326	325	323	322	321
ZONE 4 PHM 56.0	21	319	318	317	316	314	313	312	311	309	308	306	305
ZONE 4 PHM 56.0	22	304	303	301	300	298	297	296	295	294	292	291	290
ZONE 4 PHM 56.0	23	289	288	287	286	285	284	282	281	280	279	278	277
ZONE 4 PHM 56.0	24	276	275	274	273	271	270	269	268	267	265	264	262
ZONE 4 PHM 56.0	25	261	260	259	258	257	256	255	254	253	252	251	250
ZONE 4 PHM 56.0	26	249	248	247	246	245	243	242	241	240	238	237	236
ZONE 4 PHM 56.0	27	235	234	233	232	231	229	228	227	226	225	224	223
ZONE 4 PHM 56.0	28	223	222	221	220	219	217	216	215	213	212	211	210
ZONE 4 PHM 56.0	29	209	208	207	206	205	204	203	202	201	200	199	199
ZONE 4 PHM 56.0	30	198	197	196	195	194	193	192	191	190	189	188	187
ZONE 4 PHM 56.0	31	188	185	184	183	182	181	180	180	179	178	177	176
ZONE 4 PHM 56.0	32	175	174	173	172	171	170	169	168	167	166	165	164
ZONE 4 PHM 56.0	33	163	163	162	161	160	160	159	158	158	157	156	155
ZONE 4 PHM 56.0	34	154	153	152	151	150	150	149	148	148	147	146	144
ZONE 4 PHM 56.0	35	144	143	142	142	141	140	140	139	138	137	137	136
ZONE 4 PHM 56.0	36	136	135	134	134	133	132	132	131	130	129	129	128
ZONE 4 PHM 56.0	37	127	126	126	125	124	123	123	122	121	121	120	119
ZONE 4 PHM 56.0	38	119	118	118	117	117	116	116	115	114	114	113	113
ZONE 4 PHM 56.0	39	112	111	111	110	109	109	108	107	107	106	106	105
ZONE 4 PHM 56.0	40	105	104	104	104	103	103	102	102	101	100	100	99
ZONE 4 PHM 56.0	41	99	98	98	97	97	96	95	95	94	94	93	93
ZONE 4 PHM 56.0	42	92	92	91	90	90	89	89	88	88	87	87	87
ZONE 4 PHM 56.0	43	86	86	86	86	86	85	85	84	84	83	83	83
ZONE 4 PHM 56.0	44	82	82	81	81	80	79	79	78	78	78	77	77
ZONE 4 PHM 56.0	45	76	76	75	75	75	74	74	74	73	73	73	72
ZONE 4 PHM 56.0	46	72	72	71	71	70	70	69	69	68	68	67	67
ZONE 4 PHM 56.0	47	66	66	66	65	65	65	65	64	64	64	64	64
ZONE 4 PHM 56.0	48	63	63	63	62	62	62	62	61	61	60	60	59
ZONE 4 PHM 56.0	49	59	58	58	58	58	57	57	57	57	56	56	56
ZONE 4 PHM 56.0	50	55	55	54	54	54	53	53	53	52	52	51	51
ZONE 4 PHM 56.0	51	51	50	50	50	50	49	49	49	49	49	49	48
ZONE 4 PHM 56.0	52	48	48	47	47	47	46	46	46	46	46	46	46
ZONE 4 PHM 56.0	53	45	45	45	45	45	44	44	44	43	43	43	43
ZONE 4 PHM 56.0	54	43	43	43	43	42	42	42	42	41	41	41	40
ZONE 4 PHM 56.0	55	40	40	40	39	39	39	39	39	39	39	39	39
ZONE 4 PHM 56.0	56	38	38	38	37	37	37	36	36	36	36	36	36
ZONE 4 PHM 56.0	57	36	36	36	35	35	34	34	34	34	34	33	33
ZONE 4 PHM 56.0	58	33	33	33	32	32	32	32	32	32	32	32	32
ZONE 4 PHM 56.0	59	32	32	31	31	31	31	31	31	31	31	31	31
ZONE 4 PHM 56.0	60	30	30	30	29	29	29	29	29	29	29	29	28
ZONE 4 PHM 56.0	61	28	28	27	27	27	27	27	27	27	27	27	27
ZONE 4 PHM 56.0	62	27	27	27	27	27	27	27	27	26	26	26	26
ZONE 4 PHM 56.0	63	25	25	25	25	25	25	25	25	24	24	24	24
ZONE 4 PHM 56.0	64	24	24	24	24	24	24	24	24	24	24	24	24
ZONE 4 PHM 56.0	65	24	24	24	24	24	24	24	23	23	23	23	23
ZONE 4 PHM 56.0	66	23	23	23	22	22	22	22	22	22	22	22	22
ZONE 4 PHM 56.0	67	22	22	22	22	22	22	22	22	22	22	22	22
ZONE 4 PHM 56.0	68	22	22	22	22	22	22	22	22	21	21	21	21
ZONE 4 PHM 56.0	69	21	21	21	21	21	21	21	21	21	21	21	21
ZONE 4 PHM 56.0	70	21	21	21	21	21	21	20	20	20	20	20	20
ZONE 4 PHM 56.0	71	20	20	19	19	19	19	19	19	19	19	19	19
ZONE 4 PHM 56.0	72	19	19	19	19	19	19	19	19	19	19	19	19
ZONE 4 PHM 56.0	73	18	18	18	18	18	18	18	18	18	18	18	18
ZONE 4 PHM 56.0	74	17	17	17	17	17	17	17	17	17	17	17	17
ZONE 4 PHM 56.0	75	17	17	17	17	17	17	17	17	16	16	16	16
ZONE 4 PHM 56.0	76	16	16	16	16	16	16	16	16	16	16	16	16
ZONE 4 PHM 56.0	77	16	16	16	16	16	16	16	16	16	16	16	16
ZONE 4 PHM 56.0	78	16	16	16	16	16	16	16	16	16	16	16	16
ZONE 4 PHM 56.0	79	16	16	16	16	16	16	16	16	16	16	16	16



Output when control parameter LIST = 1

ZONE 4 PNH 56.0	80	13	13	13	13	13	13	13	13	13	13	12	12
ZONE 4 PNH 56.0	81	12	12	12	12	12	11	11	11	11	10	10	10
ZONE 4 PNH 56.0	82	10	10	10	10	10	10	10	10	10	10	9	9
ZONE 4 PNH 56.0	83	9	9	9	9	9	9	9	9	9	9	9	9
ZONE 4 PNH 56.0	84	9	9	9	9	9	9	9	9	9	9	9	9
ZONE 4 PNH 56.0	85	9	9	8	8	8	8	8	8	8	8	7	7
ZONE 4 PNH 56.0	86	7	7	7	7	7	7	7	7	7	7	7	7
ZONE 4 PNH 56.0	87	7	7	7	7	7	7	7	7	7	7	6	6
ZONE 4 PNH 56.0	88	6	6	6	6	6	6	6	6	6	6	5	5
ZONE 4 PNH 56.0	89	5	5	5	5	5	5	5	5	5	5	5	4
ZONE 4 PNH 56.0	90	4	5	5	5	5	5	5	5	5	4	4	4
ZONE 4 PNH 56.0	91	4	4	4	4	3	3	3	3	2	2	2	2
ZONE 4 PNH 56.0	92	2	2	2	2	2	2	2	2	2	2	2	2
ZONE 4 PNH 56.0	93	2	2	2	2	2	2	2	2	2	2	2	2
ZONE 4 PNH 56.0	94	2	2	2	2	2	2	2	2	2	2	1	1
ZONE 4 PNH 56.0	95	1	1	1	1	1	1	1	1	1	1	1	0
ZONE 4 PNH 56.0	96	0	0	0	0	0	0	0	0	0	0	0	0
ZONE 4 PNH 56.0	97	0	0	0	0	0	0	0	0	0	0	0	0
ZONE 4 PNH 56.0	98	0	0	0	0	0	0	0	0	0	0	0	0
ZONE 4 PNH 56.0	99	0	0	0	0	0	0	0	0	0	0	0	0
ZONE 4 PNH 56.0	100	0	0	0	0	0	0	0	0	0	0	0	0
ZONE 4 PNH 56.0	101	0	0	0	0	0	0	0	0	0	0	0	0
ZONE 4 PNH 56.0	102	0	0	0	0	0	0	0	0	0	0	0	0
ZONE 4 PNH 56.0	103	0	0	0	0	0	0	0	0	0	0	0	0
ZONE 4 PNH 56.0	104	0	0	0	0	0	0	0	0	0	0	0	0
ZONE 4 PNH 56.0	105	0	0	0	0	0	0	0	0	0	0	0	0
ZONE 4 PNH 56.0	106	0	0	0	0	0	0	0	0	0	0	0	0
ZONE 4 PNH 56.0	107	0	0	0	0	0	0	0	0	0	0	0	0
ZONE 4 PNH 56.0	108	0	0	0	0	0	0	0	0	0	0	0	0
ZONE 4 PNH 56.0	109	0	0	0	0	0	0	0	0	0	0	0	0
ZONE 4 PNH 56.0	110	0	0	0	0	0	0	0	0	0	0	0	0
ZONE 4 PNH 56.0	111	0	0	0	0	0	0	0	0	0	0	0	0
ZONE 4 PNH 56.0	112	0	0	0	0	0	0	0	0	0	0	0	0
ZONE 4 PNH 56.0	113	0	0	0	0	0	0	0	0	0	0	0	0
ZONE 4 PNH 56.0	114	0	0	0	0	0	0	0	0	0	0	0	0
ZONE 4 PNH 56.0	115	0	0	0	0	0	0	0	0	0	0	0	0
ZONE 4 PNH 56.0	116	0	0	0	0	0	0	0	0	0	0	0	0
ZONE 4 PNH 56.0	117	0	0	0	0	0	0	0	0	0	0	0	0
ZONE 4 PNH 56.0	118	0	0	0	0	0	0	0	0	0	0	0	0
ZONE 4 PNH 56.0	119	0	0	0	0	0	0	0	0	0	0	0	0
ZONE 4 PNH 56.0	120	0	0	0	0	0	0	0	0	0	0	0	0
ZONE 4 PNH 56.0	121	0	0	0	0	0	0	0	0	0	0	0	0
ZONE 4 PNH 56.0	122	0	0	0	0	0	0	0	0	0	0	0	0

Standard Program Output

RADIAL DISTANCE AT WHICH WIND VELOCITY FOR A P.M.H. EQUALS 0. 377.9  
 INITIAL DISTANCE ALONG TRAVERSE LINE FROM POINT OF MAX. WIND VEL. EQUALS 398.1  
 RADIAL DISTANCE MEANS DISTANCE FROM CENTER OF HURRICANE.  
 DISTANCE FROM COASTLINE MEANS DISTANCE PARALLEL TO TRAVERSE LINE.

# Output when control parameter KEYA = 1

TIME INCREMENT 1 0.0 HOURS  
 R = 56.00 N. MILES VF = 37.00 KNOTS WX = 120.00 M.P.H.  
 X DISTANCE = 50.75 N. MILES

DISTANCE FROM ZERO VELOCITY AT FRONT OF HURRICANE N. MILES	(RS) RADIAL DISTANCE N. MILES	(W) WIND VELOCITY M.P.H.	(TH) ANGLE FROM TRAVERSE LINE COUNTERCLOCKWISE POS. (DEGREES)	(PRESS) PRESSURE SETUP FT.
0.00	377.69	2.79	107.20	.41
10.00	367.98	3.93	107.07	.42
20.00	358.08	5.47	106.85	.43
30.00	348.18	6.58	106.62	.45
40.00	338.29	7.88	106.37	.46
50.00	328.41	9.47	106.11	.47
60.00	318.54	10.77	105.83	.48
70.00	308.67	12.30	105.54	.50
80.00	298.81	13.72	105.22	.51
90.00	288.96	15.38	104.86	.53
100.00	279.12	16.81	104.52	.55
110.00	269.29	18.72	104.14	.56
120.00	259.48	20.65	103.72	.58
130.00	249.68	22.48	103.27	.60
140.00	239.90	24.60	102.79	.62
150.00	230.13	26.69	102.26	.65
160.00	220.39	28.75	101.69	.67
170.00	210.67	31.19	101.06	.70
180.00	200.96	33.33	100.37	.73
190.00	191.32	35.78	99.62	.76
200.00	181.70	38.49	98.78	.80
210.00	172.12	41.41	97.85	.83
220.00	162.59	44.29	96.81	.87
230.00	153.12	47.63	95.64	.92
240.00	143.72	51.39	94.32	.97
250.00	134.42	55.79	92.82	1.02
260.00	125.21	60.49	91.09	1.08
270.00	116.14	65.55	89.09	1.15
280.00	107.20	70.97	86.75	1.15
290.00	98.50	77.24	84.00	1.22
300.00	90.12	84.21	80.72	1.30
310.00	82.05	91.27	76.79	1.39
320.00	74.25	98.78	72.02	1.48
330.00	67.48	106.07	68.22	1.59
340.00	61.35	109.56	51.34	1.68
350.00	56.34	112.55	36.19	1.89
360.00	52.77	112.38	25.33	1.96
370.00	50.05	112.90	14.13	2.00
380.00	51.05	114.70	2.89	2.09
390.00	53.08	117.54	352.46	1.96
400.00	56.81	120.30	344.36	1.88
410.00	61.96	119.64	342.98	1.78
420.00	68.19	117.40	343.10	1.68
430.00	75.25	111.18	337.42	1.57
440.00	82.89	104.91	332.76	1.47
450.00	91.00	98.25	328.90	1.38
460.00	99.46	92.15	325.68	1.29
470.00	108.10	86.21	322.98	1.21
480.00	117.10	81.17	320.66	1.14
490.00	126.19	76.26	318.72	1.08
500.00	135.41	71.99	317.01	1.02
510.00	144.73	67.87	315.53	.96
520.00	154.13	64.25	314.23	.91
530.00	163.61	61.25	313.07	.87
540.00	173.14	58.20	312.05	.83
550.00	182.73	55.64	311.13	.79
560.00	192.35	52.86	310.30	.76
570.00	202.01	50.69	309.55	.73
580.00	211.71	48.53	308.87	.70
590.00	221.43	46.02	308.25	.67
600.00	231.18	43.95	307.68	.65
610.00	240.94	41.99	307.16	.62
620.00	250.73	39.64	306.68	.60
630.00	260.53	38.22	306.23	.58
640.00	270.34	36.35	305.82	.56
650.00	280.17	34.38	305.44	.54
660.00	290.01	32.98	305.08	.53
670.00	299.86	31.33	304.74	.51
680.00	309.72	29.99	304.43	.50
690.00	319.59	28.42	304.14	.48
700.00	329.47	26.91	303.86	.47
710.00	339.35	25.53	303.60	.46
720.00	349.24	24.27	303.36	.44
730.00	359.14	23.01	303.12	.43
740.00	369.04	21.64	302.90	.42
748.93	377.89	20.63	302.72	.41

X AXIS IS ALONG HURRICANE ISOBEL AXIS OF SYMMETRY.

## Output when control parameter KEY = 1

TIME INCREMENT 1      0.0 HOURS  
 R = 56.00 N<sub>0</sub> MILES    VF = 37.00 KNOTS    WX = 120.40 M.P.H.  
 X DISTANCE = 50.75 N<sub>0</sub> MILES

DISTANCE FROM COAST LINE N <sub>0</sub> MILES	RADIAL DISTANCE N <sub>0</sub> MILES	WIND VELOCITY M.P.H.	ANGLE FROM TRAVERSE LINE COUNTERCLOCKWISE POS. (DEGREES)
47.60	330.78	9.05	106.17
46.60	331.77	8.74	106.20
45.60	334.74	8.39	106.28
40.60	337.70	7.92	106.36
39.60	338.69	7.81	106.38
38.60	339.68	7.69	106.41
37.60	340.67	7.58	106.43
36.60	341.66	7.46	106.46
35.60	342.65	7.35	106.48
31.60	344.60	6.95	106.58
30.60	347.59	6.65	106.60
29.60	348.58	6.54	106.63
28.60	349.57	6.42	106.65
27.00	351.15	6.24	106.69
26.00	352.14	6.12	106.71
25.50	352.64	6.07	106.73
25.00	353.13	6.01	106.74
24.50	353.63	5.95	106.75
24.00	354.12	5.90	106.76
23.50	354.62	5.84	106.77
22.50	355.61	5.72	106.79
21.50	356.60	5.61	106.82
20.50	357.59	5.50	106.84
20.00	358.08	5.47	106.85
19.00	359.07	5.20	106.87
16.00	362.04	4.86	106.94
15.00	363.03	4.85	106.96
14.00	364.02	4.51	106.99
12.50	365.51	4.42	107.02
10.00	367.98	3.93	107.07
8.50	369.47	3.76	107.10
5.00	372.94	3.36	107.18
3.00	374.92	3.14	107.22
2.00	375.91	3.02	107.24
1.80	376.11	3.00	107.24
1.60	376.30	2.98	107.25
1.40	376.50	2.96	107.25
1.20	376.70	2.93	107.26
1.00	376.90	2.91	107.26
.90	377.00	2.90	107.26
.80	377.10	2.88	107.27
.70	377.20	2.87	107.27
.60	377.30	2.86	107.27
.50	377.39	2.85	107.27
.40	377.49	2.84	107.27
.30	377.59	2.83	107.28
.20	377.69	2.82	107.28
.10	377.79	2.81	107.28
0.00	377.89	2.79	107.28

X AXIS IS ALONG HURRICANE ISOVEL AXIS OF SYMMETRY

## Output when control parameter KEY = 1

TIME INCREMENT 2                      .5 HOURS  
 R = 56.00 N. MILES                      VP = 37.00 KNOTS                      WX = 120.40 M.P.H.  
 X DISTANCE = 50.75 N. MILES

DISTANCE FROM COAST LINE N. MILES	RADIAL DISTANCE N. MILES	WIND VELOCITY M.P.H.	ANGLE FROM TRAVERSE LINE COUNTERCLOCKWISE POS. (DEGREES)
47.60	312.51	11.84	105.65
46.60	313.50	11.62	105.68
45.60	316.46	11.14	105.77
44.60	319.42	10.67	105.86
43.60	320.41	10.55	105.89
42.60	321.40	10.44	105.91
41.60	322.39	10.34	105.94
40.60	323.37	10.08	105.97
39.60	324.36	10.02	106.00
38.60	328.31	9.49	106.11
37.60	329.30	9.15	106.13
36.60	330.29	9.03	106.16
35.60	331.28	8.94	106.19
34.60	332.46	8.61	106.23
33.60	333.65	8.50	106.26
32.60	334.34	8.44	106.27
31.60	334.83	8.38	106.28
30.60	335.33	8.32	106.29
29.60	335.82	8.26	106.31
28.60	336.32	8.20	106.32
27.60	337.31	7.97	106.35
26.60	338.29	7.86	106.37
25.60	339.28	7.74	106.40
24.60	339.78	7.68	106.41
23.60	340.77	7.57	106.43
22.60	343.73	7.22	106.51
21.60	344.72	7.11	106.53
20.60	345.71	6.99	106.56
19.60	347.20	6.82	106.59
18.60	349.67	6.41	106.65
17.60	351.15	6.24	106.69
16.60	354.62	5.84	106.77
15.60	356.60	5.61	106.82
14.60	357.59	5.50	106.84
13.60	357.78	5.54	106.84
12.60	357.98	5.49	106.85
11.60	358.18	5.45	106.85
10.60	358.38	5.41	106.86
9.60	358.58	5.26	106.86
8.60	358.68	5.25	106.87
7.60	358.77	5.20	106.87
6.60	358.87	5.23	106.87
5.60	358.97	5.21	106.87
4.60	359.07	5.20	106.87
3.60	359.17	5.19	106.88
2.60	359.27	5.18	106.88
1.60	359.37	5.17	106.88
0.60	359.47	5.16	106.88
0.00	359.57	5.15	106.89

X AXIS IS ALONG HURRICANE ISOVEL AXIS OF SYMMETRY



Output when control parameter IOMITD = 0

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TIME (T) = 0.00      TIME (T+DELT) = .50

WMX(I+1)	WMX(I)	WMY(I+1)	WMY(I)	ASTRO. TIDE (FEET)	SP(I+1) (FT)	SP(I) (FT)	WIND STRESS PARAMETER
-21.3	-22.8	73.3	78.7	*.20	.53	.53	.00000110
-19.7	-21.3	67.6	73.3	*.20	.53	.53	.00000110
-17.2	-19.7	60.3	67.6	*.20	.52	.53	.00000110
-17.2	-17.2	58.5	60.3	*.20	.52	.52	.00000110
-16.7	-17.2	56.8	58.5	*.20	.52	.52	.00000110
-16.2	-16.7	55.1	56.8	*.20	.52	.52	.00000110
-15.8	-16.2	53.4	55.1	*.20	.52	.52	.00000110
-15.3	-15.8	51.8	53.4	*.20	.52	.52	.00000110
-13.8	-15.3	46.3	51.8	*.20	.51	.52	.00000110
-12.6	-13.8	42.4	46.3	*.20	.51	.51	.00000110
-12.2	-12.6	41.0	42.4	*.20	.51	.51	.00000110
-11.8	-12.2	39.5	41.0	*.20	.51	.51	.00000110
-11.2	-11.8	37.3	39.5	*.20	.50	.51	.00000110
-10.8	-11.2	35.9	37.3	*.20	.50	.50	.00000110
-10.6	-10.8	35.3	35.9	*.20	.50	.50	.00000110
-10.4	-10.6	34.6	35.3	*.20	.50	.50	.00000110
-10.2	-10.4	33.9	34.6	*.20	.50	.50	.00000110
-10.0	-10.2	33.3	33.9	*.20	.50	.50	.00000110
-9.8	-10.0	32.6	33.3	*.20	.50	.50	.00000110
-9.5	-9.8	31.0	32.6	*.20	.50	.50	.00000110
-9.1	-9.5	30.1	31.0	*.20	.50	.50	.00000110
-9.0	-9.1	29.8	30.1	*.20	.50	.50	.00000110
-8.7	-9.0	28.7	29.8	*.20	.50	.50	.00000110
-7.9	-8.7	25.9	28.7	*.20	.49	.50	.00000110
-6.9	-7.9	22.6	25.9	*.20	.49	.49	.00000110
-6.9	-6.9	22.5	22.6	*.20	.49	.49	.00000110
-5.9	-6.9	19.5	22.5	*.20	.49	.49	.00000110
-5.7	-5.9	18.7	19.5	*.20	.49	.49	.00000110
-4.5	-5.7	14.8	18.7	*.20	.48	.49	.00000110
-4.2	-4.5	13.5	14.8	*.20	.48	.48	.00000110
-3.3	-4.2	10.8	13.5	*.20	.48	.48	.00000110
-2.9	-3.3	9.4	10.8	*.20	.47	.48	.00000110
-2.1	-2.9	6.9	9.4	*.20	.47	.47	.00000110
-2.1	-2.1	6.8	6.9	*.20	.47	.47	.00000110
-2.1	-2.1	6.7	6.8	*.20	.47	.47	.00000110
-2.0	-2.1	6.6	6.7	*.20	.47	.47	.00000110
-2.0	-2.0	6.5	6.6	*.20	.47	.47	.00000110
-2.0	-2.0	6.4	6.5	*.20	.47	.47	.00000110
-2.0	-2.0	6.3	6.4	*.20	.47	.47	.00000110
-2.0	-2.0	6.3	6.3	*.20	.47	.47	.00000110
-1.9	-2.0	6.2	6.3	*.20	.47	.47	.00000110
-1.9	-1.9	6.2	6.2	*.20	.47	.47	.00000110
-1.9	-1.9	6.1	6.2	*.20	.47	.47	.00000110
-1.9	-1.9	6.1	6.1	*.20	.47	.47	.00000110
-1.9	-1.9	6.0	6.1	*.20	.47	.47	.00000110
-1.9	-1.9	6.0	6.0	*.20	.47	.47	.00000110
-1.9	-1.9	6.0	6.0	*.20	.47	.47	.00000110
-1.8	-1.9	5.9	6.0	*.20	.47	.47	.00000110

## Output when control parameter IOMITD = 0

```

*****
TIME (T) = .50      TIME (T+DELT) = 1.00

      MXX(I+1)      MXX(I)      MMY(I+1)      MMY(I)      ASTRO. TIDE      SP(I+1)      SP(I)      WIND STRESS
                      (FEET)      (FT)      (FT)      PARAMETER

-36.5      -37.8      130.0      135.0      *.20      .56      .56      .00000110
-33.7      -36.5      119.4      130.0      *.20      .55      .56      .00000110
-31.1      -33.7      109.5      119.4      *.20      .55      .55      .00000110
-30.5      -31.1      107.1      109.5      *.20      .55      .55      .00000110
-29.9      -30.5      104.8      107.1      *.20      .55      .55      .00000110
-29.4      -29.9      102.8      104.8      *.20      .55      .55      .00000110
-28.0      -28.4      97.8      102.8      *.20      .54      .55      .00000110
-27.6      -28.0      98.4      97.8      *.20      .54      .54      .00000110
-25.0      -27.6      86.5      96.4      *.20      .54      .54      .00000110
-23.3      -25.0      80.4      86.5      *.20      .53      .54      .00000110
-22.7      -23.3      78.4      80.4      *.20      .53      .53      .00000110
-22.3      -22.7      76.8      78.4      *.20      .53      .53      .00000110
-20.7      -22.3      71.2      76.8      *.20      .53      .53      .00000110
-20.2      -20.7      69.3      71.2      *.20      .53      .53      .00000110
-19.9      -20.2      68.3      69.3      *.20      .53      .53      .00000110
-19.7      -19.9      67.4      68.3      *.20      .53      .53      .00000110
-19.4      -19.7      66.5      67.4      *.20      .53      .53      .00000110
-19.2      -19.4      65.6      66.5      *.20      .53      .53      .00000110
-19.3      -19.2      66.0      65.6      *.20      .52      .53      .00000110
-17.9      -19.3      61.0      66.0      *.20      .52      .53      .00000110
-17.4      -17.9      59.2      61.0      *.20      .52      .52      .00000110
-16.9      -17.4      57.5      59.2      *.20      .52      .52      .00000110
-16.7      -16.9      56.6      57.5      *.20      .52      .52      .00000110
-16.2      -16.7      54.9      56.6      *.20      .52      .52      .00000110
-14.8      -16.2      50.0      54.9      *.20      .51      .52      .00000110
-14.4      -14.8      48.4      50.0      *.20      .51      .51      .00000110
-13.9      -14.4      46.9      48.4      *.20      .51      .51      .00000110
-13.3      -13.9      44.6      46.9      *.20      .51      .51      .00000110
-11.8      -13.3      39.4      44.6      *.20      .50      .51      .00000110
-11.2      -11.8      37.3      39.4      *.20      .50      .51      .00000110
-9.8      -11.2      32.6      37.3      *.20      .50      .50      .00000110
-9.1      -9.8      30.1      32.6      *.20      .50      .50      .00000110
-7.1      -9.1      23.6      30.1      *.20      .50      .50      .00000110
-7.0      -7.1      23.2      23.6      *.20      .50      .50      .00000110
-6.9      -7.0      22.9      23.2      *.20      .50      .50      .00000110
-6.8      -6.9      22.5      22.9      *.20      .49      .50      .00000110
-6.7      -6.8      22.1      22.5      *.20      .49      .49      .00000110
-6.4      -6.7      21.0      22.1      *.20      .49      .49      .00000110
-6.3      -6.4      20.9      21.0      *.20      .49      .49      .00000110
-6.3      -6.3      20.9      20.9      *.20      .49      .49      .00000110
-6.3      -6.3      20.7      20.9      *.20      .49      .49      .00000110
-6.2      -6.3      20.6      20.7      *.20      .49      .49      .00000110
-6.2      -6.2      20.5      20.6      *.20      .49      .49      .00000110
-6.2      -6.2      20.4      20.5      *.20      .49      .49      .00000110
-6.1      -6.2      20.3      20.4      *.20      .49      .49      .00000110
-6.1      -6.1      20.2      20.3      *.20      .49      .49      .00000110
-6.1      -6.1      20.2      20.2      *.20      .49      .49      .00000110
-6.1      -6.1      20.1      20.2      *.20      .49      .49      .00000110
*****

```

## Output when control parameter IOMIT = 0

OUTPUT DATA  
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TYPE OF HURRICANE MODEL (ITYPE) PMH
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GEOGRAPHICAL LATITUDE (PHII) = 42.83 DEGREES      HURRICANE ZONE (IZONE) 4
CENTRAL PRESSURE (PD) = 27.42 IN. HG.             PERIPHERAL PRESSURE (PN) = 30.42 IN. HG.
RADIUS TO MAXIMUM WIND (R) = 56.0 N.M.            MAXIMUM WIND (MX) = 120.4 MI/HR.
TRANSLATION SPEED (VF) = 37.0 KNOTS               INITIAL RISE (SE) = 160 FEET
TRAVERSE BEARING (BEAR) = N 75.00 W              BOTTOM FRICTION FACTOR (BFF) = .0025

```

Output when control parameter IOMIT = 0

TIME (T) = 0.00		TIME (T+DELTA) = .50									
DIST. (NM)	DEPTH (FT,MLW)	D AVG. (FT,MLW)	PRES. RISE (FT)	ASTRO. TIDE (FT)	INITIAL LEVEL (FT)	Y-FLUX (FT/FT/SEC)	ONSHORE SETUP (FT)	ALONGSHORE SETUP (FT)	TOTAL SETUP (FT)	WIND (FT)	TOTAL SURGE (FT)
47.6	400.0	590.0	.53	*.20	.60	.4	-.000	.000	-.000		12.33
46.6	580.0	518.0	.53	*.20	.60	.3	-.000	.000	-.000		12.33
43.6	456.0	451.0	.52	*.20	.60	.3	-.000	.000	-.000		12.32
40.6	446.0	428.0	.52	*.20	.60	.3	-.000	.000	-.000		12.32
39.6	410.0	415.0	.52	*.20	.60	.3	-.000	.000	-.000		12.32
38.6	420.0	426.0	.52	*.20	.60	.3	-.000	.000	-.000		12.32
37.6	432.0	424.0	.52	*.20	.60	.3	-.000	.000	-.000		12.32
36.6	416.0	409.0	.52	*.20	.60	.2	-.000	.000	-.000		12.32
35.6	402.0	314.0	.51	*.20	.60	.2	-.000	.000	-.000		12.31
31.6	224.0	228.0	.51	*.20	.60	.2	-.000	.000	-.000		12.31
30.6	230.0	248.0	.51	*.20	.60	.2	-.000	.000	-.000		12.31
29.6	266.0	295.0	.51	*.20	.60	.2	-.000	.000	-.000		12.31
28.6	324.0	312.0	.51	*.20	.60	.2	-.000	.000	-.000		12.31
27.0	300.0	245.0	.50	*.20	.60	.2	-.000	.000	-.000		12.30
26.0	190.0	189.5	.50	*.20	.60	.2	-.000	.000	-.000		12.30
25.5	184.0	192.5	.50	*.20	.60	.2	-.000	.000	-.000		12.30
25.0	196.0	195.0	.50	*.20	.60	.2	-.000	.000	-.000		12.30
24.5	194.0	190.0	.50	*.20	.60	.2	-.000	.000	-.000		12.30
24.0	186.0	176.5	.50	*.20	.60	.2	-.000	.000	-.000		12.30
23.5	167.0	166.0	.50	*.20	.60	.1	-.001	.000	-.000		12.30
22.5	165.0	221.5	.50	*.20	.60	.1	-.001	.000	-.000		12.30
21.5	278.0	287.0	.50	*.20	.60	.1	-.001	.000	-.000		12.30
20.5	296.0	295.0	.50	*.20	.60	.1	-.001	.000	-.000		12.30
20.0	294.0	345.5	.49	*.20	.60	.1	-.001	.000	-.000		12.29
19.0	397.0	389.5	.49	*.20	.60	.1	-.001	.000	-.000		12.29
18.0	382.0	373.5	.49	*.20	.60	.1	-.001	.000	-.000		12.29
15.0	365.0	365.0	.49	*.20	.60	.1	-.001	.000	-.000		12.29
14.0	365.0	341.0	.49	*.20	.60	.1	-.001	.000	-.000		12.29
12.5	317.0	246.0	.48	*.20	.60	.1	-.001	.000	-.000		12.28
10.0	175.0	212.0	.48	*.20	.60	.1	-.001	.000	-.000		12.28
8.5	249.0	202.5	.48	*.20	.60	.1	-.001	.000	-.000		12.28
5.0	150.0	112.5	.48	*.20	.60	.0	-.001	.000	-.000		12.28
3.0	69.0	66.5	.47	*.20	.60	.0	-.001	.000	-.000		12.27
2.0	64.0	62.0	.47	*.20	.60	.0	-.001	.000	-.000		12.27
1.8	60.0	60.0	.47	*.20	.60	.0	-.001	.000	-.000		12.27
1.6	60.0	54.5	.47	*.20	.60	.0	-.001	.000	-.000		12.27
1.4	49.0	45.0	.47	*.20	.60	.0	-.001	.000	-.000		12.27
1.2	41.0	37.0	.47	*.20	.60	.0	-.001	.000	-.000		12.27
1.0	33.0	31.5	.47	*.20	.60	.0	-.001	.000	-.000		12.27
.9	30.0	29.0	.47	*.20	.60	.0	-.001	.000	-.000		12.27
.8	28.0	22.0	.47	*.20	.60	.0	-.001	.000	-.000		12.27
.7	16.0	15.5	.47	*.20	.60	.0	-.001	.000	-.000		12.27
.6	15.0	14.0	.47	*.20	.60	.0	-.001	.000	-.000		12.27
.5	13.0	12.0	.47	*.20	.60	.0	-.001	.000	-.000		12.27
.4	11.0	10.5	.47	*.20	.60	.0	-.001	.000	-.000		12.27
.3	10.0	9.5	.47	*.20	.60	.0	-.001	.000	-.000		12.27
.2	9.0	6.5	.47	*.20	.60	.0	-.001	.000	-.000		12.27
.1	4.0	2.0	.47	*.20	.60	.0	-.001	.000	-.000		12.27
0.0	0.0										



### Output when control parameter IOMIT = 0

TIME (T) = .50

TIME (T+DELT) = 1.00

DIST. (NM)	DEPTH (FT,MLW)	D AVG. (FT,MLW)	PRES. RISE (FT)	ASTRO. TIDE (FT)	INITIAL LEVEL (FT)	Y-FLUX (FT/SEC)	ONSHORE SETUP (FT)	ALONGSHORE SETUP (FT)	TOTAL SETUP (FT)	WIND (FT)	TOTAL BURGE (FT)
47.6	600.0	590.0	.56	*.20	.60	.8	-.000	.000	-.000	12.36	
46.6	580.0	518.0	.56	*.20	.60	.8	-.000	.000	-.000	12.36	
43.6	456.0	451.0	.55	*.20	.60	.7	-.000	.000	-.000	12.35	
40.6	406.0	428.0	.55	*.20	.60	.7	-.000	.000	-.000	12.35	
39.6	410.0	415.0	.55	*.20	.60	.7	-.000	.000	-.000	12.35	
38.6	420.0	426.0	.55	*.20	.60	.6	-.000	.000	-.000	12.35	
37.6	432.0	424.0	.54	*.20	.60	.6	-.000	.000	-.000	12.34	
36.6	416.0	409.0	.54	*.20	.60	.6	-.000	.000	-.000	12.34	
35.6	402.0	314.0	.54	*.20	.60	.6	-.001	.000	-.000	12.34	
31.6	226.0	228.0	.54	*.20	.60	.5	-.001	.000	-.000	12.34	
30.6	230.0	248.0	.53	*.20	.60	.5	-.001	.001	-.000	12.33	
29.6	266.0	295.0	.53	*.20	.60	.5	-.001	.001	-.000	12.33	
28.6	324.0	312.0	.53	*.20	.60	.4	-.001	.001	-.000	12.33	
27.0	300.0	245.0	.53	*.20	.60	.4	-.001	.001	-.000	12.33	
26.0	190.0	189.5	.53	*.20	.60	.4	-.001	.001	-.000	12.33	
25.5	189.0	192.5	.53	*.20	.60	.4	-.001	.001	-.000	12.33	
25.0	194.0	195.0	.53	*.20	.60	.4	-.001	.001	-.000	12.33	
24.5	194.0	190.0	.53	*.20	.60	.4	-.001	.001	-.000	12.33	
24.0	186.0	174.5	.52	*.20	.60	.4	-.001	.001	-.000	12.32	
23.5	167.0	166.0	.52	*.20	.60	.4	-.001	.001	-.000	12.32	
22.5	165.0	221.5	.52	*.20	.60	.4	-.001	.001	-.000	12.32	
21.5	278.0	287.0	.52	*.20	.60	.3	-.001	.001	-.000	12.32	
20.5	296.0	295.0	.52	*.20	.60	.3	-.001	.001	-.000	12.32	
20.0	294.0	345.5	.52	*.20	.60	.3	-.001	.001	-.000	12.32	
19.0	397.0	389.5	.52	*.20	.60	.3	-.001	.001	-.000	12.32	
18.0	382.0	373.5	.51	*.20	.60	.3	-.001	.001	-.000	12.31	
15.0	365.0	365.0	.51	*.20	.60	.3	-.001	.001	-.000	12.31	
14.0	365.0	341.0	.51	*.20	.60	.2	-.001	.001	-.000	12.31	
12.5	317.0	246.0	.51	*.20	.60	.2	-.001	.001	-.000	12.31	
10.0	175.0	212.0	.51	*.20	.60	.2	-.001	.001	-.000	12.30	
8.5	249.0	202.5	.50	*.20	.60	.2	-.001	.001	-.000	12.30	
5.0	156.0	112.5	.50	*.20	.60	.1	-.001	.001	-.000	12.30	
3.0	69.0	66.5	.50	*.20	.60	.1	-.001	.001	-.000	12.30	
2.0	64.0	62.0	.50	*.20	.60	.1	-.001	.001	-.000	12.30	
1.8	60.0	60.0	.50	*.20	.60	.1	-.001	.001	-.000	12.30	
1.6	60.0	54.5	.50	*.20	.60	.1	-.002	.001	-.000	12.29	
1.4	49.0	45.0	.49	*.20	.60	.1	-.002	.001	-.000	12.29	
1.2	41.0	37.0	.49	*.20	.60	.1	-.002	.001	-.000	12.29	
1.0	33.0	31.5	.49	*.20	.60	.1	-.002	.001	-.000	12.29	
.9	30.0	29.0	.49	*.20	.60	.1	-.002	.001	-.000	12.29	
.8	24.0	22.0	.49	*.20	.60	.1	-.002	.001	-.000	12.29	
.7	18.0	15.5	.49	*.20	.60	.1	-.002	.001	-.000	12.29	
.6	15.0	14.0	.49	*.20	.60	.1	-.002	.001	-.000	12.29	
.5	13.0	12.0	.49	*.20	.60	.1	-.002	.001	-.000	12.29	
.4	11.0	10.5	.49	*.20	.60	.1	-.002	.001	-.000	12.29	
.3	10.0	9.5	.49	*.20	.60	.1	-.002	.001	-.000	12.29	
.2	9.0	6.5	.49	*.20	.60	.1	-.002	.001	-.000	12.29	
.1	4.0	2.0	.49	*.20	.60	.1	-.002	.001	-.000	12.29	
0.0	0.0										

# Standard Program Output

## SUMMARY OF OPEN COAST STORM SURGE PROBLEM

### PROJECT ELEMENT

SAMPLE NUCLEAR POWER STATION HAMPTON BEACH, N.H., HUR. H PMH 2=26-74 BY HDM

TYPE OF HURRICANE MODEL (ITYPE) PMH

GEOGRAPHICAL LATITUDE (PHII) = 42.83 DEGREES

HURRICANE ZONE (IZONE) 4

CENTRAL PRESSURE (PO) = 27.42 IN. HG.

PERIPHERAL PRESSURE (PH) = 30.42 IN. HG.

RADIUS TO MAXIMUM WIND (R) = 56.0 N.M.

MAXIMUM WIND (WX) = 120.4 MI/HR.

TRANSLATION SPEED (VP) = 37.0 KNOTS

INITIAL TIDE (SE) = .60 FEET

TRAVERSE BEARING (BEAR) = S 12 E

BOTTOM FRICTION FACTOR (BFF) = .0025

WIND STRESS EQUATION  $WK(L) = 0.00000121 + 0.00000275 * (1.0 + 16.0/W(L))^{.42}$

SURGE TRAVERSE IS PARALLEL TO STORM TRACK AND 115 DEGREES FROM THE LINE OF STRONGEST WINDS

TIME (HOURS)	SETUP X-COMP. (FT.)	SETUP Y-COMP. (FT.)	TOT. WIND SETUP (FT.)	AST. TIDE LEVEL (FT. MLW)	INITIAL WATER LEVEL (FT.)	PRESSURE SETUP (FT.)	TOTAL WATER LEVEL (FT. MLW)	TIDE CORRECTED TOTAL WATER LEVEL (FT. MLW)
.50	.00	.00	.00	11.20	.60	.47	12.27	.
1.00	.00	.00	.00	11.20	.60	.49	12.29	.
1.50	.00	.00	.00	11.20	.60	.52	12.32	.
2.00	.00	.00	.00	11.20	.60	.54	12.34	.
2.50	.01	.01	.00	11.20	.60	.57	12.38	.
3.00	.01	.01	.00	11.20	.60	.61	12.41	.
3.50	.01	.02	.01	11.20	.60	.64	12.45	.
4.00	.02	.04	.02	11.20	.60	.69	12.50	.
4.50	.03	.06	.03	11.20	.60	.73	12.56	.
5.00	.03	.09	.05	11.20	.60	.79	12.64	.
5.50	.04	.13	.09	11.20	.60	.85	12.74	.
6.00	.04	.18	.14	11.20	.60	.92	12.86	.
6.50	.04	.27	.23	11.20	.60	1.01	13.03	.
7.00	.01	.38	.37	11.20	.60	1.11	13.28	.
7.50	.07	.54	.61	11.20	.60	1.23	13.63	.
8.00	.26	.75	1.02	11.20	.60	1.37	14.19	.
8.50	.68	1.04	1.72	11.20	.60	1.54	15.06	.
9.00	1.46	1.35	2.01	11.20	.60	1.74	16.36	.
9.50	2.39	1.61	4.00	11.20	.60	1.97	17.77	.
10.00	3.13	1.57	4.70	11.20	.60	2.17	18.67	.
10.50	3.46	1.29	4.75	11.20	.60	2.28	18.84	.
11.00	3.51	1.14	4.05	11.20	.60	2.24	18.69	.
11.50	3.15	.93	4.07	11.20	.60	2.07	17.94	.
12.00	2.89	.70	3.18	11.20	.60	1.85	16.83	.
12.50	1.81	.48	2.29	11.20	.60	1.63	15.73	.
13.00	1.32	.30	1.61	11.20	.60	1.45	14.86	.
13.50	.97	.15	1.12	11.20	.60	1.29	14.21	.
14.00	.73	.08	.77	11.20	.60	1.16	13.73	.
14.50	.56	.05	.51	11.20	.60	1.05	13.36	.
15.00	.44	.01	.32	11.20	.60	.96	13.08	.
15.50	.34	.01	.18	11.20	.60	.88	12.86	.
16.00	.27	.01	.07	11.20	.60	.82	12.68	.
16.50	.21	.01	.02	11.20	.60	.76	12.54	.
17.00	.16	.01	.08	11.20	.60	.71	12.43	.
17.50	.13	.01	.13	11.20	.60	.66	12.34	.
18.00	.10	.01	.16	11.20	.60	.62	12.27	.
18.50	.08	.01	.18	11.20	.60	.59	12.21	.
19.00	.06	.01	.19	11.20	.60	.56	12.17	.
19.50	.04	.01	.19	11.20	.60	.53	12.14	.
20.00	.03	.01	.19	11.20	.60	.50	12.11	.
20.50	.02	.01	.18	11.20	.60	.48	12.10	.
21.00	.02	.01	.17	11.20	.60	.46	12.09	.
21.50	.01	.01	.15	11.20	.60	.44	12.09	.
22.00	.01	.01	.14	11.20	.60	.42	12.09	.

## APPENDIX A-8. ERROR TABLES FOR ACTUAL AND HYPOTHETICAL HURRICANES

Error	Actual Hurricanes
01	The value of NM or LM has exceeded the maximum value allowable for the program.
02	The card ID in columns 76 through 80 is incorrect. Check 10 FORMAT in main program to make sure it reads: 10 FORMAT (8I5, 35X, A5).
03	ID in columns 1 through 6 is incorrect. It should be DIST starting in column 1.
04	ID in columns 1 through 6 is incorrect. It should be DEPTH starting in column 1.
05	Wind-field data card in error. The card should be of the form: three letters of hurricane name in columns 1 through 3; card set number in columns 4 and 5; and card number in columns 7 through 10. Three letters of hurricane name should agree with the value of ITYPE; card set number should agree with the value of N; and card number should agree with the value of L.
06	The values of ALPHA and BETA 0 read in where smaller than 0° or greater than 360°.

### Hypothetical Hurricanes

01	The value of NM or LM has exceeded the maximum value allowable for the program.
02	The card ID in columns 76 through 80 is incorrect. Check 10 FORMAT in main program to make sure it reads: 10 FORMAT (8I5, 35X, A5).
03	A hurricane zone (IZONE) other than A, B, C, 1, 2, 3, or 4 was requested.
04	A latitude (PHII) outside the limits of the zone was requested.
05	ID in columns 1 through 6 is incorrect. It should be DIST starting in column 1.
06	ID in columns 1 through 6 is incorrect. It should be DEPTH starting in column 1.
07	Relative wind-field data card in error. The card should be of the form: ZONE in columns 1 through 4; hurricane zone ID in column 6; hurricane type in columns 8 through 10; RR in columns 11 through 15; card number right justified in columns 16 through 20; and VX values follow in five column fields starting in column 21 and ending in column 80. The zone ID (ID2) on the card should agree with the zone (IZONE) requested. The hurricane type (ID3) on the card should agree with hurricane type (ITYPE) requested. The value of radius to maximum wind (RR) on the card should equal $\pm 0.000005$ the radius to maximum wind (R) requested. The card number on the card (NN) should equal the number of cards read (NC).

**Error****Hypothetical Hurricanes—Continued**

- 08 Both values of X and Y in subroutine QUAD are zero or nearly equal to zero. Check the values of DIST, VF, and R for validity.
- 09 None.
- 10 The values of X and Y in subroutine QUAD must fall in the first, second, or fourth quadrant. The values of X and Y were negative. Check the values of DIST, VF, and R for validity.
- 11 The value of TH in subroutine QUAD must fall between  $115^\circ$  and  $-65^\circ$ . The value computed was unreasonable.
- 12 The value of RHO/R in subroutine VEL is unreasonable. The value must be between 0.84 and 29.1. Check the value of R and RHO for validity.
- 13 The value of VX defined in subroutine STRTPT is not valid. Check the relative wind-field data given in the main program.
- 14 The zone identification defined in the main program does not match the zone identification in columns 6 through 10 of one of the header cards of the digitized (X,Y) data. Error from subroutine PROFIL.
- 15 Final digitized wind-field deck was read without matching zone ID. Check input data.
- 16 Second digitized wind-field deck was not found for zone requested. Check input data.
- 17 The value of R on one of the header cards of the digitized (X,Y) data is unreasonable. The value must be between 2.0 and 80.0. Error from subroutine PROFIL.
- 18 First deck of digitized wind-field data [R (KK)] has a larger radius than radius (RR) requested. First deck of digitized wind-field data must have a radius [R(KK)] less than or equal to radius (RR) requested.
- 19 The number of digitized (X,Y) points for one of the input hurricane profiles exceeds 750. The maximum number of points for each profile is 750. Error from subroutine PROFIL.
- 20 The last digitized X value for one of the input profiles is less than 30. The value must exceed 30. Error from subroutine PROFIL.

**Error****Hypothetical Hurricanes—Continued**

- 21 The impossible has occurred. Execution was terminated by dropping through the loop at statement 40. Check the program deck for correct statement sequence or for missing statements prior to statement 40 in subroutine PROFIL.
- 22 The end card following the digitized (X,Y) data for one of the input profiles does not contain periods in columns 1 through 80 as it should. Error from subroutine PROFIL.
- 23 The values of X are not in an increasing order; therefore, data must be supplied in increasing order. Error from subroutine INTERP.

**PRECAUTIONS**

1. For detailed output listing, run the program with only one parameter set at a time; otherwise run will be terminated for maximum number of lines of output.
2. When using digitized wind profile data deck, the last card of the digitized data deck should have the word FINAL punched in columns 6 through 10. See Appendix A-6., Input Coding Sheet for Hypothetical Hurricanes.
3. Other precautions and general information are given in Appendixes A-2 and A-3.
4. It is important to fully understand the precautions and to format the data properly before attempting to use this program.

## APPENDIX B. WIND FIELD AND COMPUTED HYDROGRAPHIC DATA OF HISTORIC HURRICANES

The wind field data is voluminous and for this reason it has not been included in this report. However it can be provided to potential users upon request. A computer summary of input and output computer data is given in this section. The computed hydrographs of storm surges have been depicted graphically in Figures 39 through 71.

### APPENDIX B-1. HURRICANE OF OCTOBER 1949, FREEPORT, TEXAS

#### SUMMARY OF OPEN COAST STORM SURGE PROBLEM

OCT 1949 HURRICANE VERIFICATION AT FREEPORT, TEXAS	AEC 6/73
CENTRAL PRESSURE = 28.45 IN. HG.	PERIPHERAL PRESSURE = 29.95 IN. HG.
RADIUS TO MAXIMUM WIND = 15.0 N.M.	MAXIMUM WIND = 88.0 MI/HR.
TRANSLATION SPEED = 11.0 KNOTS	INITIAL WISF = 2.00 FEET
BOTTOM FRICTION FACTOR = .0030	WIND STRESS CORRECTION FACTOR = 1.10
C1 = 203.0	C2 = 106.0
	C3 = 5280.0

TIME (HOURS)	SETUP X-COMP. (FT.)	SETUP Y-COMP. (FT.)	TOT. WIND SETUP (FT.)	AST. TIDE LFVEL (FT. MLW)	INITIAL WATER LEVEL (FT.)	PRESSURE SETUP (FT.)	TOTAL WATER LEVEL (FT. MLW)
1.00	1.12	.40	1.52	.37	2.00	.32	4.21
2.00	1.90	.71	2.61	.24	2.00	.38	5.23
3.00	1.52	.98	2.50	.22	2.00	.44	5.15
4.00	2.88	1.04	3.92	.35	2.00	.52	6.78
5.00	3.00	1.11	4.11	.60	2.00	.65	7.36
6.00	3.43	.81	4.64	.95	2.00	.89	8.48
7.00	3.40	.41	4.21	1.42	2.00	1.07	8.69
8.00	2.56	.08	2.64	1.80	2.00	1.02	7.47
9.00	1.27	-.30	.97	1.90	2.00	.82	5.69
10.00	.74	-.47	.26	1.89	2.00	.65	4.80
11.00	.45	-.51	.34	1.74	2.00	.53	4.60
12.00	.36	-.58	-.22	1.54	2.00	.43	3.75

SUMMARY OF OPEN COAST STORM SURGE PROBLEM

OCT 1949 HURRICANE VERIFICATION AT FREEPORT, TEXAS  
CENTRAL PRESSURE = 28.45 IN. HG.

AEC 6/73  
PERIPHERAL PRESSURE = 29.95 IN. HG.

RADIUS TO MAXIMUM WIND = 15.0 N.M.

MAXIMUM WIND = 88.0 MI/HR.

TRANSLATION SPEED = 11.0 KNOTS

INITIAL RISE = 2.00 FEET

BOTTOM FRICTION FACTOR = .0025

WIND STRESS CORRECTION FACTOR = 1.10

C1 = 203.0

C2 = 106.0

C3 = 5280.0

TIME (HOURS)	SETUP X-COMP. (FT.)	SETUP Y-COMP. (FT.)	TOT. WIND SETUP (FT.)	AST. TIDE LEVEL (FT. MLW)	INITIAL WATER LEVEL (FT.)	PRESSURE SETUP (FT.)	TOTAL WATER LEVEL (FT. MLW)
1.00	1.12	.40	1.52	.37	2.00	.32	4.21
2.00	1.90	.72	2.62	.24	2.00	.38	5.24
3.00	1.52	1.00	2.52	.22	2.00	.44	5.18
4.00	2.88	1.11	3.99	.35	2.00	.52	6.86
5.00	3.00	1.19	4.19	.60	2.00	.65	7.44
6.00	3.82	.88	4.71	.95	2.00	.89	8.55
7.00	3.79	.45	4.25	1.42	2.00	1.07	8.73
8.00	2.56	.13	2.68	1.80	2.00	1.02	7.51
9.00	1.27	-.27	1.00	1.90	2.00	.82	5.72
10.00	.74	-.45	.28	1.89	2.00	.65	4.82
11.00	.45	-.50	.34	1.74	2.00	.53	4.61
12.00	.36	-.56	-.22	1.54	2.00	.43	3.75

SUMMARY OF OPEN COAST STORM SURGE PROBLEM

OCT 1949 HURRICANE VERIFICATION AT FREEPORT, TEXAS  
CENTRAL PRESSURE = 28.45 IN. HG.

AEC 6/73  
PERIPHERAL PRESSURE = 29.95 IN. HG.

RADIUS TO MAXIMUM WIND = 15.0 N.M.

MAXIMUM WIND = 88.0 MI/HR.

TRANSLATION SPEED = 11.0 KNOTS

INITIAL RISE = 2.00 FEET

BOTTOM FRICTION FACTOR = .0025

WIND STRESS CORRECTION FACTOR = 1.00

C1 = 203.0

C2 = 106.0

C3 = 5280.0

TIME (HOURS)	SETUP X-COMP. (FT.)	SETUP Y-COMP. (FT.)	TOT. WIND SETUP (FT.)	AST. TIDE LEVEL (FT. MLW)	INITIAL WATER LEVEL (FT.)	PRESSURE SETUP (FT.)	TOTAL WATER LEVEL (FT. MLW)
1.00	1.02	.36	1.38	.37	2.00	.32	4.08
2.00	1.73	.66	2.39	.24	2.00	.38	5.01
3.00	1.38	.93	2.31	.22	2.00	.44	4.97
4.00	2.62	1.04	3.66	.35	2.00	.52	6.53
5.00	2.73	1.12	3.85	.60	2.00	.65	7.10
6.00	3.49	.84	4.33	.95	2.00	.89	8.17
7.00	3.46	.43	3.90	1.42	2.00	1.07	8.38
8.00	2.33	.13	2.47	1.80	2.00	1.02	7.29
9.00	1.16	-.23	.93	1.90	2.00	.82	5.65
10.00	.67	-.40	.27	1.89	2.00	.65	4.80
11.00	.77	-.46	.31	1.74	2.00	.53	4.58
12.00	.33	-.53	-.20	1.54	2.00	.43	3.77

SUMMARY OF OPEN COAST STORM SURGE PROBLEM

OCT 1949 HURRICANE VERIFICATION AT FREEPORT, TEXAS  
CENTRAL PRESSURE = 28.45 IN. HG.

AEC 6/73  
PERIPHERAL PRESSURE = 29.95 IN. HG.

RADIUS TO MAXIMUM WIND = 15.0 N.M.

MAXIMUM WIND = 88.0 MI/HR.

TRANSLATION SPEED = 11.0 KNOTS

INITIAL RISE = 2.00 FEET

BOTTOM FRICTION FACTOR = .0025

WIND STRESS CORRECTION FACTOR = .90

C1 = 203.0

C2 = 106.0

C3 = 5280.0

TIME (HOURS)	SETUP X-COMP. (FT.)	SETUP Y-COMP. (FT.)	TOT. WIND SETUP (FT.)	AST. TIDE LEVEL (FT. MLW)	INITIAL WATER LEVEL (FT.)	PRESSURE SETUP (FT.)	TOTAL WATER LEVEL (FT. MLW)
1.00	.92	.33	1.25	.37	2.00	.32	3.94
2.00	1.56	.60	2.16	.24	2.00	.38	4.78
3.00	1.25	.85	2.09	.22	2.00	.44	4.75
4.00	2.37	.97	3.33	.35	2.00	.52	6.20
5.00	2.47	1.04	3.51	.60	2.00	.65	6.76
6.00	3.16	.79	3.94	.95	2.00	.89	7.78
7.00	3.13	.41	3.54	1.42	2.00	1.07	8.03
8.00	2.11	.14	2.25	1.80	2.00	1.02	7.07
9.00	1.04	-.19	.86	1.90	2.00	.82	5.58
10.00	.60	-.35	.25	1.89	2.00	.65	4.79
11.00	.69	-.41	.29	1.74	2.00	.53	4.55
12.00	.29	-.47	-.18	1.54	2.00	.43	3.79

SUMMARY OF OPEN COAST STORM SURGE PROBLEM

OCT 1949 HURRICANE VERIFICATION AT FREEPORT, TEXAS  
CENTRAL PRESSURE = 28.45 IN. HG.

AEC 6/73  
PERIPHERAL PRESSURE = 29.95 IN. HG.

RADIUS TO MAXIMUM WIND = 15.0 N.M.

MAXIMUM WIND = 88.0 MI/HR.

TRANSLATION SPEED = 11.0 KNOTS

INITIAL RISE = 2.00 FEET

BOTTOM FRICTION FACTOR = .0025

WIND STRESS CORRECTION FACTOR = .80

C1 = 203.0

C2 = 106.0

C3 = 5280.0

TIME (HOURS)	SETUP X-COMP. (FT.)	SETUP Y-COMP. (FT.)	TOT. WIND SETUP (FT.)	AST. TIDE LEVEL (FT. MLW)	INITIAL WATER LEVEL (FT.)	PRESSURE SETUP (FT.)	TOTAL WATER LEVEL (FT. MLW)
1.00	.82	.29	1.11	.37	2.00	.32	3.80
2.00	1.39	.53	1.92	.24	2.00	.38	4.55
3.00	1.11	.77	1.88	.22	2.00	.44	4.53
4.00	2.11	.89	3.00	.35	2.00	.52	5.87
5.00	2.20	.97	3.17	.60	2.00	.65	6.42
6.00	2.82	.73	3.55	.95	2.00	.89	7.39
7.00	2.79	.39	3.18	1.42	2.00	1.07	7.67
8.00	1.88	.14	2.03	1.80	2.00	1.02	6.85
9.00	.93	-.15	.78	1.90	2.00	.82	5.50
10.00	.54	-.30	.24	1.89	2.00	.65	4.77
11.00	.62	-.36	.26	1.74	2.00	.53	4.53
12.00	.26	-.42	-.16	1.54	2.00	.43	3.82



# APPENDIX B-1. HURRICANE OF OCTOBER 1949, GALVESTON, TEXAS

## SUMMARY OF OPEN COAST STORM SURGE PROBLEM

OCT 1949 HURRICANE VERIFICATION AT GALVESTON, TEXAS  
CENTRAL PRESSURE = 28.45 IN. HG.

AEC 6/73  
PERIPHERAL PRESSURE = 29.95 IN. HG.

RADIUS TO MAXIMUM WIND = 46.0 N.M.

MAXIMUM WIND = 88.0 MI/HR.

TRANSLATION SPEED = 11.0 KNOTS

INITIAL RISE = 2.00 FEET

BOTTOM FRICTION FACTOR = .0030

WIND STRESS CORRECTION FACTOR = 1.10

C1 = 203.0

C2 = 106.0

C3 = 5280.0

TIME (HOURS)	SETUP X-COMP. (FT.)	SETUP Y-COMP. (FT.)	TOT. WIND SETUP (FT.)	AST. TIDE LEVEL (FT. MLW)	INITIAL WATER LEVEL (FT.)	PRESSURE SETUP (FT.)	TOTAL WATER LEVEL (FT. MLW)
1.00	-.37	.36	-.01	1.20	2.00	.25	3.43
2.00	-.26	.65	.39	.87	2.00	.28	3.54
3.00	.32	.91	1.23	.67	2.00	.32	4.22
4.00	.99	1.13	2.12	.46	2.00	.62	5.20
5.00	1.99	1.31	3.30	.30	2.00	.68	6.28
6.00	2.06	1.48	3.54	.22	2.00	.73	6.49
7.00	2.45	1.67	4.12	.20	2.00	.79	7.11
8.00	3.23	1.63	4.85	.33	2.00	.87	8.06
9.00	3.26	1.37	4.63	.52	2.00	.98	8.13
10.00	3.66	.92	4.59	.77	2.00	1.08	8.44
11.00	3.51	.66	4.17	1.02	2.00	1.17	8.36
12.00	1.66	.40	2.06	1.19	2.00	1.26	6.51
13.00	1.96	.22	2.19	1.29	2.00	1.30	6.77
14.00	1.68	.04	1.72	1.29	2.00	1.29	6.30
15.00	1.01	-.19	.83	1.23	2.00	1.22	5.28

SUMMARY OF OPEN COAST STORM SURGE PROBLEM

OCT 1949 HURRICANE VERIFICATION AT GALVESTON, TEXAS  
CENTRAL PRESSURE = 28.45 IN. HG.

AEC 6/73  
PERIPHERAL PRESSURE = 29.95 IN. HG.

RADIUS TO MAXIMUM WIND = 46.0 N.M.

MAXIMUM WIND = 88.0 MI/HR.

TRANSLATION SPEED = 11.0 KNOTS

INITIAL RISE = 2.00 FEET

BOTTOM FRICTION FACTOR = .0035

WIND STRESS CORRECTION FACTOR = 1.10

C1 = 203.0

C2 = 106.0

C3 = 5280.0

TIME (HOURS)	SETUP X-COMP. (FT.)	SETUP Y-COMP. (FT.)	TOT. WIND SETUP (FT.)	AST. TIDE LEVEL (FT. MLW)	INITIAL WATER LEVEL (FT.)	PRESSURE SETUP (FT.)	TOTAL WATER LEVEL (FT. MLW)
1.00	-.37	.35	-.02	1.20	2.00	.25	3.43
2.00	-.26	.65	.38	.87	2.00	.28	3.53
3.00	.32	.89	1.21	.67	2.00	.32	4.20
4.00	.99	1.10	2.08	.46	2.00	.62	5.17
5.00	1.99	1.26	3.26	.30	2.00	.68	6.24
6.00	2.06	1.42	3.48	.22	2.00	.73	6.43
7.00	2.45	1.60	4.05	.20	2.00	.79	7.04
8.00	3.23	1.52	4.75	.33	2.00	.87	7.95
9.00	3.27	1.27	4.54	.52	2.00	.98	8.04
10.00	3.67	.85	4.52	.77	2.00	1.08	8.37
11.00	3.52	.60	4.12	1.02	2.00	1.17	8.31
12.00	1.66	.36	2.01	1.19	2.00	1.26	6.47
13.00	1.96	.18	2.15	1.29	2.00	1.30	6.73
14.00	1.68	.00	1.68	1.29	2.00	1.29	6.26
15.00	1.01	-.22	.79	1.23	2.00	1.22	5.24

SUMMARY OF OPEN COAST STORM SURGE PROBLEM

OCT 1949 HURRICANE VERIFICATION AT GALVESTON, TEXAS  
CENTRAL PRESSURE = 28.45 IN. HG.

AEC 6/73  
PERIPHERAL PRESSURE = 29.95 IN. HG.

RADIUS TO MAXIMUM WIND = 46.0 N.M.

MAXIMUM WIND = 88.0 MI/HR.

TRANSLATION SPEED = 11.0 KNOTS

INITIAL RISE = 2.00 FEET

BOTTOM FRICTION FACTOR = .0035

WIND STRESS CORRECTION FACTOR = 1.00

C1 = 203.0

C2 = 106.0

C3 = 5280.0

TIME (HOURS)	SETUP X-COMP. (FT.)	SETUP Y-COMP. (FT.)	TOT. WIND SETUP (FT.)	AST. TIDE LEVEL (FT. MLW)	INITIAL WATER LEVEL (FT.)	PRESSURE SETUP (FT.)	TOTAL WATER LEVEL (FT. MLW)
1.00	-.34	.32	-.01	1.20	2.00	.25	3.43
2.00	-.24	.59	.35	.87	2.00	.28	3.50
3.00	.29	.82	1.11	.67	2.00	.32	4.10
4.00	.90	1.02	1.91	.46	2.00	.62	5.00
5.00	1.81	1.18	2.99	.30	2.00	.68	5.97
6.00	1.87	1.33	3.20	.22	2.00	.73	6.15
7.00	2.23	1.50	3.73	.20	2.00	.79	6.72
8.00	2.95	1.44	4.39	.33	2.00	.87	7.59
9.00	2.98	1.21	4.19	.52	2.00	.98	7.69
10.00	3.35	.81	4.16	.77	2.00	1.08	8.02
11.00	3.21	.56	3.79	1.02	2.00	1.17	7.98
12.00	1.51	.35	1.86	1.19	2.00	1.26	6.32
13.00	1.79	.19	1.98	1.29	2.00	1.30	6.56
14.00	1.53	.02	1.55	1.29	2.00	1.29	6.13
15.00	.92	-.18	.74	1.23	2.00	1.22	5.19

SUMMARY OF OPEN COAST STORM SURGE PROBLEM

OCT 1949 HURRICANE VERIFICATION AT GALVESTON, TEXAS  
CENTRAL PRESSURE = 28.45 IN. HG.

AEC 6/73  
PERIPHERAL PRESSURE = 29.95 IN. HG.

RADIUS TO MAXIMUM WIND = 46.0 N.M.

MAXIMUM WIND = 88.0 MI/HR.

TRANSLATION SPEED = 11.0 KNOTS

INITIAL RISE = 2.00 FEET

BOTTOM FRICTION FACTOR = .0035

WIND STRESS CORRECTION FACTOR = .90

C1 = 203.0

C2 = 106.0

C3 = 5280.0

TIME (HOURS)	SETUP X-COMP. (FT.)	SETUP Y-COMP. (FT.)	TOT. WIND SETUP (FT.)	AST. TIDE LEVEL (FT. MLW)	INITIAL WATER LEVEL (FT.)	PRESSURE SETUP (FT.)	TOTAL WATER LEVEL (FT. MLW)
1.00	-.30	.29	-.01	1.20	2.00	.25	3.44
2.00	-.21	.54	.32	.87	2.00	.28	3.47
3.00	.26	.75	1.01	.67	2.00	.32	4.00
4.00	.81	.93	1.74	.46	2.00	.62	4.82
5.00	1.63	1.08	2.72	.30	2.00	.68	5.70
6.00	1.69	1.23	2.92	.22	2.00	.73	5.87
7.00	2.02	1.39	3.41	.20	2.00	.79	6.40
8.00	2.66	1.36	4.02	.33	2.00	.87	7.22
9.00	2.69	1.15	3.84	.52	2.00	.98	7.34
10.00	3.03	.77	3.80	.77	2.00	1.08	7.65
11.00	2.90	.55	3.45	1.02	2.00	1.17	7.64
12.00	1.37	.34	1.71	1.19	2.00	1.26	6.16
13.00	1.62	.19	1.81	1.29	2.00	1.30	6.39
14.00	1.38	.04	1.42	1.29	2.00	1.29	6.00
15.00	.83	-.14	.69	1.23	2.00	1.22	5.14

SUMMARY OF OPEN COAST STORM SURGE PROBLEM

OCT 1949 HURRICANE VERIFICATION AT GALVESTON, TEXAS  
CENTRAL PRESSURE = 28.45 IN. HG.

AEC 6/73  
PERIPHERAL PRESSURE = 29.95 IN. HG.

RADIUS TO MAXIMUM WIND = 46.0 N.M.

MAXIMUM WIND = 88.0 MI/HR.

TRANSLATION SPEED = 11.0 KNOTS

INITIAL RISE = 2.00 FEET

BOTTOM FRICTION FACTOR = .0035

WIND STRESS CORRECTION FACTOR = .80

C1 = 203.0

C2 = 106.0

C3 = 5280.0

TIME (HOURS)	SETUP X-COMP. (FT.)	SETUP Y-COMP. (FT.)	TOT. WIND SETUP (FT.)	AST. TIDE LEVEL (FT. MLW)	INITIAL WATER LEVEL (FT.)	PRESSURE SETUP (FT.)	TOTAL WATER LEVEL (FT. MLW)
1.00	-.27	.26	-.01	1.20	2.00	.25	3.44
2.00	-.19	.48	.29	.87	2.00	.28	3.44
3.00	.23	.68	.91	.67	2.00	.32	3.90
4.00	.72	.85	1.56	.46	2.00	.62	4.65
5.00	1.46	.99	2.44	.30	2.00	.68	5.43
6.00	1.51	1.13	2.63	.22	2.00	.73	5.58
7.00	1.80	1.28	3.08	.20	2.00	.79	6.06
8.00	2.37	1.26	3.64	.33	2.00	.87	6.84
9.00	2.41	1.06	3.48	.52	2.00	.98	6.99
10.00	2.70	.73	3.43	.77	2.00	1.08	7.29
11.00	2.59	.52	3.11	1.02	2.00	1.17	7.30
12.00	1.22	.33	1.55	1.19	2.00	1.26	6.01
13.00	1.44	.20	1.64	1.29	2.00	1.30	6.22
14.00	1.23	.06	1.29	1.29	2.00	1.29	5.87
15.00	.74	-.11	.64	1.23	2.00	1.22	5.09

# APPENDIX B-2. HURRICANE CARLA, FREEPORT, TEXAS

## SUMMARY OF OPEN COAST STORM SURGE PROBLEM

HURRICANE CARLA VERIFICATION AT FREEPORT, TEXAS  
CENTRAL PRESSURE = 27.64 IN. HG.

SEP 1961 AEC 6/73  
PERIPHERAL PRESSURE = 29.92 IN. HG.

RADIUS TO MAXIMUM WIND = 46.0 N.M.

MAXIMUM WIND = 88.0 MI/HR.

TRANSLATION SPEED = 3.0 KNOTS

INITIAL RISE = 2.50 FEET

BOTTOM FRICTION FACTOR = .0030

WIND STRESS CORRECTION FACTOR = 1.10

C1 = 203.0

C2 = 106.0

C3 = 5280.0

TIME (HOURS)	SETUP X-COMP. (FT.)	SETUP Y-COMP. (FT.)	TOT. WIND SETUP (FT.)	AST. TIDE LEVEL (FT., MLW)	INITIAL WATER LEVEL (FT.)	PRESSURE SETUP (FT.)	TOTAL WATER LEVEL (FT., MLW)
1.00	-.04	.07	.04	1.36	2.50	.32	4.22
2.00	-.02	.14	.12	1.05	2.50	.33	4.00
3.00	-.01	.20	.20	.80	2.50	.34	3.83
4.00	.01	.26	.27	.70	2.50	.35	3.81
5.00	-.02	.32	.30	.77	2.50	.36	3.93
6.00	-.05	.37	.33	.97	2.50	.37	4.16
7.00	-.08	.42	.36	1.24	2.50	.38	4.47
8.00	-.08	.47	.39	1.51	2.50	.39	4.79
9.00	-.09	.53	.44	1.67	2.50	.40	5.00
10.00	-.10	.59	.48	1.69	2.50	.41	5.08
11.00	-.15	.65	.51	1.51	2.50	.42	4.98
12.00	-.20	.71	.51	1.18	2.50	.43	4.62
13.00	-.26	.78	.51	.79	2.50	.44	4.25
14.00	-.29	.85	.56	.42	2.50	.46	3.93
15.00	-.31	.92	.61	.17	2.50	.47	3.75
16.00	-.33	1.00	.67	.10	2.50	.48	3.75
17.00	-.36	1.09	.73	.21	2.50	.50	3.93
18.00	-.38	1.18	.80	.46	2.50	.48	4.24
19.00	-.40	1.28	.88	.80	2.50	.53	4.70
20.00	-.40	1.37	.98	1.16	2.50	.54	5.17
21.00	-.39	1.46	1.07	1.49	2.50	.55	5.61
22.00	-.39	1.55	1.16	1.72	2.50	.56	5.94
23.00	-.37	1.63	1.27	1.80	2.50	.57	6.13
24.00	-.34	1.71	1.37	1.70	2.50	.58	6.15
25.00	-.31	1.79	1.49	1.44	2.50	.59	6.01
26.00	-.38	1.87	1.49	1.10	2.50	.61	5.70
27.00	-.48	1.96	1.48	.79	2.50	.63	5.39
28.00	-.58	2.05	1.47	.62	2.50	.65	5.24
29.00	-.49	2.16	1.67	.63	2.50	.67	5.47
30.00	-.38	2.28	1.90	.78	2.50	.70	5.88
31.00	-.23	2.41	2.18	1.03	2.50	.72	6.43
32.00	-.22	2.53	2.31	1.32	2.50	.74	6.87
33.00	-.21	2.62	2.41	1.56	2.50	.77	7.24
34.00	-.21	2.69	2.49	1.69	2.50	.79	7.46
35.00	.03	2.78	2.81	1.67	2.50	.83	7.81
36.00	.36	2.90	3.25	1.45	2.50	.87	8.08
37.00	.72	3.03	3.76	1.10	2.50	.93	8.28
38.00	1.16	3.12	4.29	.71	2.50	.92	8.42
39.00	1.55	3.05	4.60	.38	2.50	.92	8.40
40.00	1.91	2.88	4.79	.21	2.50	.91	8.41
41.00	2.00	2.74	4.74	.24	2.50	.91	8.39
42.00	2.08	2.58	4.65	.41	2.50	.90	8.47
43.00	2.15	2.43	4.58	.70	2.50	.90	8.69
44.00	2.40	2.33	4.74	1.05	2.50	.95	9.23
45.00	2.67	2.22	4.89	1.39	2.50	.99	9.78
46.00	2.92	2.04	4.95	1.65	2.50	1.04	10.15
47.00	2.94	1.81	4.75	1.79	2.50	1.03	10.07
48.00	2.94	1.62	4.56	1.76	2.50	1.03	9.85
49.00	2.92	1.40	4.33	1.54	2.50	1.00	9.36
50.00	3.08	1.41	4.50	1.19	2.50	1.01	9.19
51.00	3.26	1.44	4.70	.81	2.50	1.02	9.04
52.00	3.41	1.48	4.89	.57	2.50	1.03	8.98
53.00	3.53	1.41	4.93	.50	2.50	1.07	9.00
54.00	3.59	1.18	4.77	.60	2.50	1.13	9.14
55.00	3.67	.98	4.65	.81	2.50	1.18	9.14
56.00	3.80	.75	4.55	1.09	2.50	1.22	9.35
57.00	3.85	.51	4.36	1.37	2.50	1.24	9.47
58.00	3.89	.26	4.14	1.59	2.50	1.28	9.51
59.00	3.61	.08	3.69	1.64	2.50	1.24	9.12
60.00	3.36	-.09	3.27	1.65	2.50	1.21	8.36
61.00	3.10	-.25	2.84	1.42	2.50	1.18	7.98
62.00	2.84	-.41	2.43	1.08	2.50	1.13	7.17
63.00	2.62	-.56	2.06	.73	2.50	1.08	6.37
64.00	2.39	-.69	1.70	.48	2.50	1.05	5.72
65.00	2.17	-.81	1.36	.40	2.50	1.00	5.26
66.00	1.96	-.91	1.04	.49	2.50	.96	4.99
67.00	1.76	-1.00	.76	.71	2.50	.92	4.89
68.00	1.70	-1.08	.62	1.00	2.50	.87	5.00
69.00	1.66	-1.16	.50	1.31	2.50	.84	5.15
70.00	1.59	-1.23	.36	1.59	2.50	.80	5.25
71.00	1.34	-1.29	.05	1.76	2.50	.75	5.04
72.00	1.10	-1.28	-.18	1.79	2.50	.72	4.83
73.00	.90	-1.21	-.32	1.64	2.50	.68	4.50
74.00	.71	-1.15	-.44	1.31	2.50	.66	4.04
75.00	.55	-1.10	-.55	.92	2.50	.64	3.51
76.00	.43	-1.03	-.60	.39	2.50	.62	3.11

SUMMARY OF OPEN COAST STORM SURGE PROBLEM

HURRICANE CARLA VERIFICATION AT FREEPORT, TEXAS  
CENTRAL PRESSURE = 27.64 IN. HG.

SEP 1961 AEC 6/73  
PERIPHERAL PRESSURE = 29.92 IN. HG.

RADIUS TO MAXIMUM WIND = 46.0 N.M.

MAXIMUM WIND = 88.0 MI/HR.

TRANSLATION SPEED = 3.0 KNOTS

INITIAL RISE = 2.50 FEET

BOTTOM FRICTION FACTOR = .0050

WIND STRESS CORRECTION FACTOR = 1.10

C1 = 203.0

C2 = 106.0

C3 = 520.0

TIME (HOURS)	SETUP X=COMP. (FT.)	SETUP Y=COMP. (FT.)	TOT. WIND SETUP (FT.)	AST. TIDE LFVEL (FT. MLW)	INITIAL WATER LEVEL (FT.)	PRESSURE SETUP (FT.)	TOTAL WATER LEVEL (FT. MLW)
1.00	-.04	.07	.04	1.36	2.50	.32	4.22
2.00	-.02	.14	.12	1.05	2.50	.33	3.99
3.00	-.01	.20	.19	.80	2.50	.34	3.83
4.00	.01	.25	.26	.70	2.50	.35	3.81
5.00	-.02	.30	.29	.77	2.50	.36	3.91
6.00	-.05	.35	.31	.97	2.50	.37	4.14
7.00	-.08	.40	.32	1.25	2.50	.38	4.45
8.00	-.08	.44	.35	1.51	2.50	.39	4.75
9.00	-.09	.48	.39	1.67	2.50	.40	4.96
10.00	-.10	.53	.43	1.69	2.50	.41	5.03
11.00	-.15	.58	.44	1.51	2.50	.42	4.87
12.00	-.20	.63	.43	1.18	2.50	.43	4.54
13.00	-.26	.68	.42	.79	2.50	.44	4.15
14.00	-.29	.74	.45	.42	2.50	.46	3.82
15.00	-.31	.79	.48	.17	2.50	.47	3.62
16.00	-.33	.86	.52	.10	2.50	.48	3.61
17.00	-.36	.92	.57	.21	2.50	.50	3.77
18.00	-.38	1.00	.62	.46	2.50	.48	4.06
19.00	-.40	1.07	.67	.80	2.50	.53	4.50
20.00	-.40	1.15	.75	1.16	2.50	.54	4.95
21.00	-.39	1.21	.82	1.49	2.50	.55	5.36
22.00	-.39	1.28	.89	1.72	2.50	.56	5.67
23.00	-.37	1.34	.97	1.80	2.50	.57	5.84
24.00	-.34	1.40	1.06	1.70	2.50	.58	5.83
25.00	-.31	1.46	1.15	1.44	2.50	.59	5.68
26.00	-.38	1.52	1.14	1.10	2.50	.61	5.35
27.00	-.48	1.58	1.10	.79	2.50	.63	5.02
28.00	-.59	1.65	1.07	.62	2.50	.65	4.84
29.00	-.49	1.74	1.25	.63	2.50	.67	5.06
30.00	-.38	1.84	1.46	.78	2.50	.70	5.43
31.00	-.23	1.94	1.71	1.03	2.50	.72	5.96
32.00	-.22	2.03	1.81	1.32	2.50	.74	6.37
33.00	-.21	2.09	1.88	1.56	2.50	.77	6.71
34.00	-.21	2.14	1.93	1.69	2.50	.79	6.91
35.00	-.03	2.20	2.23	1.67	2.50	.83	7.23
36.00	.35	2.30	2.65	1.45	2.50	.87	7.48
37.00	.72	2.41	3.13	1.10	2.50	.93	7.66
38.00	1.17	2.46	3.63	.71	2.50	.92	7.75
39.00	1.55	2.37	3.92	.38	2.50	.92	7.72
40.00	1.92	2.23	4.15	.21	2.50	.91	7.77
41.00	2.01	2.12	4.13	.24	2.50	.91	7.79
42.00	2.08	2.00	4.08	.41	2.50	.90	7.90
43.00	2.16	1.86	4.05	.70	2.50	.90	8.15
44.00	2.42	1.81	4.22	1.05	2.50	.95	8.72
45.00	2.68	1.72	4.41	1.39	2.50	.99	9.29
46.00	2.93	1.58	4.51	1.65	2.50	1.04	9.70
47.00	2.95	1.40	4.35	1.79	2.50	1.03	9.67
48.00	2.95	1.25	4.21	1.76	2.50	1.03	9.49
49.00	2.93	1.09	4.02	1.54	2.50	1.00	9.06
50.00	3.09	1.09	4.19	1.19	2.50	1.01	8.88
51.00	3.27	1.12	4.38	.83	2.50	1.02	8.73
52.00	3.42	1.15	4.57	.57	2.50	1.03	8.67
53.00	3.54	1.09	4.62	.50	2.50	1.07	8.69
54.00	3.60	.91	4.51	.60	2.50	1.13	8.73
55.00	3.68	.75	4.43	.81	2.50	1.18	8.92
56.00	3.81	.56	4.36	1.09	2.50	1.22	9.17
57.00	3.86	.35	4.21	1.37	2.50	1.24	9.32
58.00	3.89	.14	4.03	1.59	2.50	1.28	9.40
59.00	3.62	-.03	3.59	1.69	2.50	1.24	9.01
60.00	3.37	-.19	3.17	1.65	2.50	1.21	8.54
61.00	3.10	-.35	2.75	1.42	2.50	1.18	7.85
62.00	2.85	-.49	2.36	1.08	2.50	1.13	7.07
63.00	2.63	-.62	2.01	.73	2.50	1.08	6.32
64.00	2.39	-.71	1.68	.48	2.50	1.05	5.70
65.00	2.17	-.79	1.38	.40	2.50	1.00	5.28
66.00	1.95	-.86	1.10	.49	2.50	.96	5.04
67.00	1.75	-.91	.85	.71	2.50	.92	4.98
68.00	1.70	-.95	.75	1.00	2.50	.87	5.12
69.00	1.66	-1.00	.66	1.31	2.50	.84	5.30
70.00	1.59	-1.05	.54	1.59	2.50	.80	5.43
71.00	1.33	-1.08	.26	1.76	2.50	.75	5.27
72.00	1.10	-1.03	.07	1.79	2.50	.72	5.08
73.00	.89	-.97	-.07	1.64	2.50	.68	4.75
74.00	.71	-.92	-.20	1.31	2.50	.66	4.27
75.00	.55	-.88	-.32	.92	2.50	.64	3.74
76.00	.43	-.82	-.40	.58	2.50	.62	3.32

SUMMARY OF OPEN COAST STORM SURGE PROBLEM

HURRICANE CARLA VERIFICATION AT FREEPORT, TEXAS  
CENTRAL PRESSURE = 27.44 IN. HG.

SEP 1961 AEC 6/73  
PERIPHERAL PRESSURE = 29.92 IN. HG.

RADIUS TO MAXIMUM WIND = 46.0 N.M.

MAXIMUM WIND = 88.0 MI/HR.

TRANSLATION SPEED = 3.0 KNOTS

INITIAL RISE = 2.50 FEET

BOTTOM FRICTION FACTOR = .0050

WIND STRESS CORRECTION FACTOR = 1.00

C1 = 203.0

C2 = 106.0

C3 = 5280.0

TIME (HOURS)	SETUP Y-COMP (FT.)	SETUP Y-COMP (FT.)	TOT. WIND SETUP (FT.)	AST. TIDE LEVEL (FT. MLW)	INITIAL WATER LEVEL (FT.)	PRESSURE SETUP (FT.)	TOTAL WATER LEVEL (FT. MLW)
1.00	-.03	.06	.03	1.36	2.50	.32	4.21
2.00	-.02	.13	.11	1.05	2.50	.33	3.98
3.00	-.01	.18	.18	.80	2.50	.34	3.81
4.00	.00	.23	.24	.70	2.50	.35	3.78
5.00	-.02	.28	.26	.77	2.50	.36	3.89
6.00	-.04	.32	.28	.97	2.50	.37	4.12
7.00	-.07	.37	.29	1.25	2.50	.38	4.42
8.00	-.08	.40	.33	1.51	2.50	.39	4.73
9.00	-.09	.45	.36	1.67	2.50	.40	4.93
10.00	-.10	.49	.40	1.69	2.50	.41	5.00
11.00	-.13	.54	.41	1.51	2.50	.42	4.84
12.00	-.18	.59	.41	1.18	2.50	.43	4.52
13.00	-.24	.64	.40	.79	2.50	.44	4.13
14.00	-.26	.69	.43	.42	2.50	.46	3.80
15.00	-.28	.74	.46	.17	2.50	.47	3.60
16.00	-.30	.80	.50	.10	2.50	.48	3.58
17.00	-.33	.87	.54	.21	2.50	.50	3.75
18.00	-.35	.94	.59	.46	2.50	.48	4.03
19.00	-.36	1.01	.65	.80	2.50	.53	4.47
20.00	-.36	1.08	.72	1.16	2.50	.54	4.92
21.00	-.36	1.14	.79	1.49	2.50	.55	5.32
22.00	-.35	1.21	.85	1.72	2.50	.56	5.63
23.00	-.33	1.26	.93	1.80	2.50	.57	5.80
24.00	-.31	1.32	1.01	1.70	2.50	.58	5.79
25.00	-.28	1.38	1.10	1.44	2.50	.59	5.63
26.00	-.25	1.44	1.09	1.10	2.50	.61	5.30
27.00	-.24	1.50	1.06	.79	2.50	.63	4.98
28.00	-.23	1.57	1.03	.62	2.50	.65	4.81
29.00	-.22	1.65	1.00	.63	2.50	.67	5.01
30.00	-.21	1.74	1.39	.78	2.50	.70	5.37
31.00	-.21	1.84	1.63	1.03	2.50	.72	5.88
32.00	-.20	1.92	1.72	1.32	2.50	.74	6.29
33.00	-.20	1.99	1.79	1.56	2.50	.77	6.62
34.00	-.19	2.03	1.84	1.69	2.50	.79	6.82
35.00	-.18	2.09	2.12	1.67	2.50	.83	7.12
36.00	-.17	2.18	2.51	1.45	2.50	.87	7.33
37.00	-.16	2.29	2.95	1.10	2.50	.93	7.47
38.00	-.16	2.34	3.40	.71	2.50	.97	7.53
39.00	-.15	2.26	3.67	.38	2.50	.92	7.47
40.00	-.15	2.13	3.88	.21	2.50	.91	7.50
41.00	-.14	2.02	3.88	.24	2.50	.91	7.51
42.00	-.14	1.90	3.80	.41	2.50	.90	7.62
43.00	-.13	1.80	3.77	.70	2.50	.90	7.87
44.00	-.12	1.72	3.92	1.05	2.50	.95	8.42
45.00	-.11	1.64	4.09	1.39	2.50	.99	8.97
46.00	-.10	1.51	4.18	1.65	2.50	1.04	9.37
47.00	-.09	1.34	4.03	1.79	2.50	1.03	9.34
48.00	-.08	1.19	3.89	1.76	2.50	1.03	9.17
49.00	-.07	1.04	3.71	1.54	2.50	1.00	8.75
50.00	-.06	1.04	3.86	1.19	2.50	1.01	8.56
51.00	-.05	1.07	4.04	.83	2.50	1.01	8.39
52.00	-.04	1.10	4.22	.57	2.50	1.02	8.31
53.00	-.03	1.04	4.26	.50	2.50	1.03	8.33
54.00	-.02	.87	4.15	.60	2.50	1.07	8.38
55.00	-.01	.71	4.07	.81	2.50	1.18	8.56
56.00	.00	.54	4.01	1.09	2.50	1.22	8.81
57.00	.01	.34	3.87	1.37	2.50	1.24	8.98
58.00	.02	.14	3.69	1.59	2.50	1.28	9.06
59.00	.03	-.01	3.29	1.69	2.50	1.24	8.72
60.00	.04	-.16	2.91	1.65	2.50	1.21	8.27
61.00	.05	-.30	2.52	1.42	2.50	1.18	7.82
62.00	.06	-.44	2.16	1.08	2.50	1.13	6.87
63.00	.07	-.56	1.84	.73	2.50	1.08	6.08
64.00	.08	-.65	1.53	.48	2.50	1.05	5.55
65.00	.09	-.73	1.24	.40	2.50	1.00	5.15
66.00	.10	-.79	.99	.49	2.50	.96	4.93
67.00	.11	-.84	.75	.49	2.50	.92	4.88
68.00	.12	-.89	.66	.71	2.50	.87	5.03
69.00	.13	-.94	.57	1.00	2.50	.84	5.21
70.00	.14	-.99	.46	1.31	2.50	.80	5.35
71.00	.15	-1.02	.20	1.59	2.50	.75	5.21
72.00	.16	-.97	.03	1.76	2.50	.72	5.03
73.00	.17	-.92	-.10	1.79	2.50	.68	4.72
74.00	.18	-.87	-.22	1.64	2.50	.66	4.25
75.00	.19	-.83	-.33	1.31	2.50	.64	3.73
76.00	.20	-.78	-.39	.59	2.50	.62	3.32

SUMMARY OF OPEN COAST STORM SURGE PROBLEM

HURRICANE CAPLA VERIFICATION AT FREEPORT, TEXAS  
CENTRAL PRESSURE = 27.64 IN. HG.

SEP 1961 AEC 6/73  
PERIPHERAL PRESSURE = 29.92 IN. HG.

RADIUS TO MAXIMUM WIND = 46.0 N.M.

MAXIMUM WIND = 86.0 MI/HR.

TRANSLATION SPEED = 3.0 KNOTS

INITIAL RISE = 2.50 FEET

BOTTOM FRICTION FACTOR = .0050

WIND STRESS CORRECTION FACTOR = .90

C1 = 203.0

C2 = 106.0

C3 = 5200.0

TIME (HOURS)	SETUP X-COMP. (FT.)	SETUP Y-COMP. (FT.)	TOT. WIND (FT.)	AST. TIDE LEVEL (FT. MLW)	INITIAL WATER LEVEL (FT.)	PRESSURE SETUP (FT.)	TOTAL WATER LEVEL (FT. MLW)
1.00	-.03	.06	.03	1.36	2.50	.32	4.21
2.00	-.02	.11	.09	1.05	2.50	.33	3.97
3.00	-.01	.16	.16	.80	2.50	.34	3.80
4.00	.00	.21	.21	.70	2.50	.35	3.76
5.00	-.02	.25	.24	.77	2.50	.36	3.87
6.00	-.04	.30	.26	.97	2.50	.37	4.10
7.00	-.06	.35	.27	1.25	2.50	.38	4.40
8.00	-.07	.37	.30	1.51	2.50	.39	4.70
9.00	-.08	.41	.33	1.67	2.50	.40	4.90
10.00	-.09	.46	.37	1.69	2.50	.41	4.97
11.00	-.12	.50	.38	1.51	2.50	.42	4.81
12.00	-.17	.55	.38	1.18	2.50	.43	4.49
13.00	-.22	.59	.37	.79	2.50	.44	4.11
14.00	-.24	.64	.40	.42	2.50	.46	3.78
15.00	-.26	.69	.44	.17	2.50	.47	3.57
16.00	-.27	.75	.47	.10	2.50	.48	3.56
17.00	-.29	.81	.51	.21	2.50	.50	3.72
18.00	-.31	.88	.56	.46	2.50	.48	4.01
19.00	-.33	.94	.61	.80	2.50	.53	4.44
20.00	-.32	1.01	.69	1.16	2.50	.54	4.88
21.00	-.32	1.07	.75	1.49	2.50	.55	5.29
22.00	-.32	1.13	.81	1.72	2.50	.56	5.59
23.00	-.30	1.19	.89	1.80	2.50	.57	5.75
24.00	-.28	1.24	.96	1.70	2.50	.58	5.74
25.00	-.25	1.30	1.05	1.44	2.50	.59	5.57
26.00	-.31	1.35	1.04	1.10	2.50	.61	5.25
27.00	-.39	1.41	1.02	.79	2.50	.63	4.93
28.00	-.48	1.47	1.00	.62	2.50	.65	4.77
29.00	-.40	1.55	1.15	.63	2.50	.67	4.95
30.00	-.31	1.64	1.33	.78	2.50	.70	5.30
31.00	-.19	1.73	1.54	1.03	2.50	.72	5.79
32.00	-.18	1.81	1.63	1.32	2.50	.74	6.19
33.00	-.18	1.87	1.70	1.56	2.50	.77	6.52
34.00	-.17	1.92	1.75	1.69	2.50	.78	6.72
35.00	-.12	1.98	2.00	1.67	2.50	.83	7.00
36.00	-.29	2.06	2.35	1.45	2.50	.87	7.17
37.00	-.59	2.16	2.75	1.10	2.50	.93	7.28
38.00	-.96	2.22	3.17	.71	2.50	.92	7.30
39.00	1.27	2.14	3.42	.38	2.50	.92	7.21
40.00	1.57	2.02	3.59	.21	2.50	.91	7.21
41.00	1.65	1.92	3.57	.24	2.50	.91	7.22
42.00	1.71	1.81	3.52	.41	2.50	.90	7.33
43.00	1.78	1.70	3.48	.70	2.50	.90	7.59
44.00	1.98	1.63	3.62	1.05	2.50	.95	8.12
45.00	2.21	1.56	3.76	1.39	2.50	.99	8.65
46.00	2.41	1.43	3.84	1.65	2.50	1.04	9.03
47.00	2.43	1.27	3.70	1.79	2.50	1.03	9.01
48.00	2.43	1.13	3.56	1.76	2.50	1.03	8.85
49.00	2.41	.98	3.39	1.54	2.50	1.00	8.43
50.00	2.34	.99	3.53	1.19	2.50	1.01	8.23
51.00	2.24	1.01	3.70	.83	2.50	1.02	8.04
52.00	2.11	1.04	3.85	.37	2.50	1.03	7.95
53.00	2.01	.98	3.90	.30	2.50	1.07	7.97
54.00	2.00	.82	3.79	.60	2.50	1.13	8.02
55.00	1.83	.68	3.71	.81	2.50	1.18	8.20
56.00	1.14	.41	3.65	1.09	2.50	1.22	8.46
57.00	1.18	.33	3.52	1.47	2.50	1.24	8.63
58.00	1.21	.15	3.36	1.59	2.50	1.28	8.72
59.00	2.08	.00	2.99	1.69	2.50	1.24	8.42
60.00	2.77	-.13	2.64	1.65	2.50	1.21	8.01
61.00	2.45	-.26	2.29	1.42	2.50	1.18	7.39
62.00	2.34	-.18	1.96	1.08	2.50	1.13	6.67
63.00	2.16	-.50	1.66	.73	2.50	1.08	5.98
64.00	1.96	-.54	1.38	.48	2.50	1.05	5.40
65.00	1.78	-.66	1.12	.40	2.50	1.00	5.02
66.00	1.60	-.73	.88	.49	2.50	.96	4.82
67.00	1.44	-.78	.66	.71	2.50	.92	4.79
68.00	1.39	-.82	.57	1.00	2.50	.87	4.94
69.00	1.36	-.87	.48	1.31	2.50	.84	5.13
70.00	1.30	-.92	.38	1.59	2.50	.80	5.27
71.00	1.09	-.95	.14	1.76	2.50	.75	5.16
72.00	.90	-.92	-.02	1.79	2.50	.72	4.99
73.00	.73	-.86	-.13	1.64	2.50	.68	4.69
74.00	.58	-.82	-.24	1.31	2.50	.66	4.24
75.00	.45	-.79	-.33	.92	2.50	.64	3.71
76.00	.35	-.74	-.39	.59	2.50	.62	3.33

SUMMARY OF OPEN COAST STORM SURGE PROBLEM

HURRICANE CARLA VERIFICATION AT FREEPORT, TEXAS  
CENTRAL PRESSURE = 27.64 IN. HG.

SEP 1961 AEC 6/73  
PERIPHERAL PRESSURE = 29.92 IN. HG.

RADIUS TO MAXIMUM WIND = 46.0 N.M.

MAXIMUM WIND = 88.0 MI/HR.

TRANSLATION SPEED = 3.0 KNOTS

INITIAL RISE = 2.50 FEET

BOTTOM FRICTION FACTOR = .0050

WIND STRESS CORRECTION FACTOR = .80

C1 = 203.0

C2 = 106.0

C3 = 5280.0

TIME (HOURS)	SETUP X-COMP. (FT.)	SETUP Y-COMP. (FT.)	TOT. WIND SETUP (FT.)	AST. TIDE LEVEL (FT. MLW)	INITIAL WATER LEVEL (FT.)	PRESSURE SETUP (FT.)	TOTAL WATER LEVEL (FT. MLW)
1.00	-.03	.05	.03	1.36	2.50	.32	4.21
2.00	-.02	.10	.08	1.05	2.50	.33	3.96
3.00	-.00	.15	.14	.80	2.50	.34	3.78
4.00	.00	.19	.19	.70	2.50	.35	3.74
5.00	-.01	.23	.21	.77	2.50	.36	3.84
6.00	-.03	.27	.23	.97	2.50	.37	4.07
7.00	-.06	.30	.24	1.25	2.50	.38	4.37
8.00	-.06	.34	.28	1.51	2.50	.39	4.67
9.00	-.07	.37	.31	1.67	2.50	.40	4.87
10.00	-.08	.41	.34	1.69	2.50	.41	4.94
11.00	-.11	.46	.35	1.51	2.50	.42	4.78
12.00	-.15	.50	.35	1.18	2.50	.43	4.46
13.00	-.19	.54	.35	.79	2.50	.44	4.08
14.00	-.21	.59	.38	.42	2.50	.46	3.75
15.00	-.23	.63	.41	.17	2.50	.47	3.55
16.00	-.24	.69	.45	.10	2.50	.48	3.53
17.00	-.26	.75	.48	.21	2.50	.50	3.69
18.00	-.28	.81	.53	.46	2.50	.51	3.97
19.00	-.29	.87	.58	.80	2.50	.53	4.41
20.00	-.29	.94	.65	1.16	2.50	.54	4.85
21.00	-.29	.99	.71	1.49	2.50	.55	5.25
22.00	-.28	1.05	.77	1.72	2.50	.56	5.55
23.00	-.27	1.10	.84	1.80	2.50	.57	5.71
24.00	-.25	1.16	.91	1.70	2.50	.58	5.68
25.00	-.22	1.21	.99	1.44	2.50	.59	5.51
26.00	-.28	1.26	.98	1.10	2.50	.61	5.19
27.00	-.35	1.32	.97	.79	2.50	.63	4.88
28.00	-.43	1.38	.95	.62	2.50	.65	4.72
29.00	-.35	1.45	1.00	.63	2.50	.67	4.90
30.00	-.28	1.53	1.25	.78	2.50	.70	5.23
31.00	-.17	1.62	1.45	1.03	2.50	.72	5.70
32.00	-.16	1.70	1.53	1.32	2.50	.74	6.10
33.00	-.16	1.75	1.60	1.56	2.50	.77	6.42
34.00	-.15	1.80	1.65	1.69	2.50	.79	6.62
35.00	.02	1.85	1.67	1.67	2.50	.83	6.87
36.00	.26	1.93	2.19	1.45	2.50	.87	7.01
37.00	.53	2.03	2.55	1.10	2.50	.93	7.08
38.00	.85	2.08	2.93	.71	2.50	.95	7.06
39.00	1.13	2.02	3.15	.38	2.50	.92	6.95
40.00	1.40	1.90	3.30	.21	2.50	.91	6.93
41.00	1.47	1.81	3.28	.24	2.50	.91	6.93
42.00	1.53	1.70	3.23	.41	2.50	.90	7.04
43.00	1.58	1.61	3.19	.70	2.50	.90	7.29
44.00	1.77	1.54	3.31	1.05	2.50	.95	7.81
45.00	1.97	1.47	3.43	1.39	2.50	.99	8.32
46.00	2.15	1.35	3.49	1.65	2.50	1.04	8.69
47.00	2.16	1.20	3.34	1.79	2.50	1.03	8.68
48.00	2.17	1.07	3.23	1.76	2.50	1.03	8.52
49.00	2.15	.93	3.08	1.54	2.50	1.00	8.12
50.00	2.27	.93	3.20	1.19	2.50	1.01	7.90
51.00	2.40	.95	3.35	.83	2.50	1.02	7.69
52.00	2.51	.98	3.49	.57	2.50	1.03	7.58
53.00	2.60	.93	3.52	.50	2.50	1.07	7.59
54.00	2.64	.78	3.62	.60	2.50	1.13	7.65
55.00	2.70	.64	3.35	.81	2.50	1.18	7.84
56.00	2.80	.44	3.29	1.09	2.50	1.22	8.09
57.00	2.84	.32	3.16	1.37	2.50	1.24	8.27
58.00	2.86	.15	3.01	1.59	2.50	1.28	8.38
59.00	2.66	.02	2.66	1.69	2.50	1.24	8.11
60.00	2.47	-.10	2.37	1.65	2.50	1.21	7.74
61.00	2.27	-.21	2.06	1.42	2.50	1.18	7.16
62.00	2.09	-.33	1.76	1.08	2.50	1.13	6.47
63.00	1.92	-.43	1.49	.73	2.50	1.08	5.80
64.00	1.75	-.52	1.23	.48	2.50	1.05	5.26
65.00	1.58	-.59	.99	.40	2.50	1.00	4.89
66.00	1.43	-.65	.77	.49	2.50	.96	4.72
67.00	1.28	-.71	.57	.71	2.50	.92	4.70
68.00	1.24	-.75	.48	1.00	2.50	.87	4.86
69.00	1.21	-.80	.41	1.31	2.50	.84	5.05
70.00	1.16	-.85	.31	1.59	2.50	.80	5.20
71.00	.97	-.88	.09	1.76	2.50	.75	5.11
72.00	.80	-.86	-.06	1.79	2.50	.72	4.95
73.00	.65	-.81	-.16	1.64	2.50	.68	4.66
74.00	.52	-.77	-.25	1.31	2.50	.66	4.22
75.00	.40	-.74	-.33	.92	2.50	.64	3.73
76.00	.31	-.69	-.38	.59	2.50	.62	3.33



# APPENDIX B-2. HURRICANE CARLA, GALVESTON, TEXAS

## SUMMARY OF OPEN COAST STORM SURGE PROBLEM

HURRICANE CARLA VERIFICATION AT GALVESTON, TEXAS  
CENTRAL PRESSURE = 27.64 IN. HG.

SEP 1961 AEC 6/73  
PERIPHERAL PRESSURE = 29.92 IN. HG.

RADIUS TO MAXIMUM WIND = 46.0 N.M.

MAXIMUM WIND = 88.0 MI/HR.

TRANSLATION SPEED = 3.0 KNOTS

INITIAL RISE = 1.90 FEET

BOTTOM FRICTION FACTOR = .0030

WIND STRESS CORRECTION FACTOR = 1.10

C1 = 203.0

C2 = 106.0

C3 = 5280.0

TIME (HOURS)	SETUP X-COMP. (FT.)	SETUP Y-COMP. (FT.)	TOT. WIND SETUP (FT.)	AST. TIDE LEVEL (FT. M.L.W.)	INITIAL WATER LEVEL (FT.)	PRESSURE SETUP (FT.)	TOTAL WATER LEVEL (FT. M.L.W.)
1.00	-.12	.13	.01	1.07	1.90	.32	3.31
2.00	-.11	.26	.15	.89	1.90	.33	3.27
3.00	-.10	.37	.27	.75	1.90	.34	3.26
4.00	-.09	.48	.39	.70	1.90	.35	3.34
5.00	-.08	.57	.49	.75	1.90	.36	3.50
6.00	-.08	.66	.58	.89	1.90	.37	3.74
7.00	-.07	.74	.67	1.07	1.90	.38	4.02
8.00	-.10	.81	.71	1.22	1.90	.39	4.21
9.00	-.15	.89	.74	1.30	1.90	.39	4.33
10.00	-.20	.98	.78	1.26	1.90	.40	4.33
11.00	-.25	1.06	.81	1.10	1.90	.41	4.22
12.00	-.33	1.15	.82	.84	1.90	.42	3.98
13.00	-.39	1.24	.84	.56	1.90	.44	3.74
14.00	-.42	1.33	.90	.30	1.90	.45	3.55
15.00	-.44	1.43	.99	.14	1.90	.46	3.49
16.00	-.47	1.54	1.07	.10	1.90	.47	3.54
17.00	-.50	1.66	1.16	.20	1.90	.48	3.75
18.00	-.49	1.74	1.30	.41	1.90	.50	4.10
19.00	-.51	1.93	1.42	.69	1.90	.51	4.51
20.00	-.52	2.06	1.54	.98	1.90	.51	4.93
21.00	-.52	2.18	1.66	1.25	1.90	.52	5.32
22.00	-.53	2.30	1.77	1.37	1.90	.53	5.57
23.00	-.52	2.40	1.88	1.39	1.90	.54	5.71
24.00	-.52	2.50	1.99	1.29	1.90	.55	5.72
25.00	-.51	2.60	2.09	1.11	1.90	.56	5.66
26.00	-.45	2.70	2.26	.89	1.90	.57	5.62
27.00	-.37	2.81	2.44	.70	1.90	.59	5.63
28.00	-.28	2.93	2.65	.61	1.90	.61	5.77
29.00	-.03	3.05	3.02	.62	1.90	.63	6.17
30.00	.26	3.17	3.43	.74	1.90	.64	6.71
31.00	.58	3.28	3.86	.91	1.90	.66	7.32
32.00	.60	3.36	3.96	1.10	1.90	.67	7.63
33.00	.62	3.43	4.04	1.24	1.90	.69	7.88
34.00	.64	3.47	4.12	1.30	1.90	.71	8.02
35.00	1.04	3.54	4.58	1.24	1.90	.74	8.46
36.00	1.50	3.64	5.14	1.06	1.90	.77	8.87
37.00	2.03	3.75	5.78	.80	1.90	.81	9.29
38.00	2.29	3.65	5.94	.53	1.90	.84	9.17
39.00	2.39	3.38	5.77	.31	1.90	.79	8.78
40.00	2.49	3.08	5.57	.21	1.90	.75	8.46
41.00	2.61	2.94	5.55	.24	1.90	.75	8.44
42.00	2.73	2.79	5.52	.39	1.90	.73	8.50
43.00	2.82	2.63	5.45	.62	1.90	.70	8.68
44.00	2.97	2.57	5.53	.90	1.90	.74	9.07
45.00	3.07	2.51	5.58	1.16	1.90	.79	9.43
46.00	3.19	2.46	5.65	1.33	1.90	.84	9.73
47.00	3.06	2.25	5.30	1.40	1.90	.83	9.43
48.00	2.56	2.05	4.61	1.34	1.90	.81	8.67
49.00	2.76	1.74	4.50	1.16	1.90	.80	8.36
50.00	2.96	1.76	4.71	.92	1.90	.81	8.34
51.00	3.14	1.81	4.95	.69	1.90	.81	8.35
52.00	3.32	1.90	5.22	.54	1.90	.81	8.47
53.00	3.33	1.86	5.19	.50	1.90	.85	8.43
54.00	3.29	1.56	4.85	.58	1.90	.84	8.21
55.00	3.18	1.19	4.37	.74	1.90	.92	7.94
56.00	3.35	1.03	4.37	.93	1.90	.94	8.14
57.00	3.51	.90	4.40	1.12	1.90	.96	8.38
58.00	3.71	.77	4.49	1.26	1.90	.97	8.62
59.00	3.50	.63	4.13	1.30	1.90	.95	8.28
60.00	3.26	.48	3.73	1.23	1.90	.94	7.80
61.00	3.05	.37	3.42	1.05	1.90	.91	7.28
62.00	3.02	.27	3.29	.82	1.90	.89	6.90
63.00	2.94	.19	3.12	.59	1.90	.88	6.50
64.00	2.90	.09	2.99	.44	1.90	.86	6.19
65.00	2.80	-.04	2.76	.40	1.90	.85	5.91
66.00	2.63	-.14	2.44	.48	1.90	.85	5.70
67.00	2.53	-.23	2.30	.65	1.90	.83	5.68
68.00	2.57	-.35	2.22	.88	1.90	.79	5.79
69.00	2.63	-.52	2.11	1.11	1.90	.76	5.67
70.00	2.63	-.72	1.91	1.29	1.90	.73	5.63
71.00	2.28	-.91	1.37	1.39	1.90	.70	5.36
72.00	2.00	-1.04	.96	1.37	1.90	.67	4.89
73.00	1.71	-1.13	.58	1.23	1.90	.64	4.35
74.00	1.43	-1.19	.24	.99	1.90	.63	3.76
75.00	1.16	-1.23	-.05	.73	1.90	.62	3.20
76.00	.96	-1.25	-.29	.51	1.90	.61	2.73

SUMMARY OF OPEN COAST STORM SURGE PROBLEM

HURRICANE CARLA VERIFICATION AT GALVESTON, TEXAS  
CENTRAL PRESSURE = 27.64 IN. HG.

SEP 1961 AEC 6/73  
PERIPHERAL PRESSURE = 29.92 IN. HG.

RADIUS TO MAXIMUM WIND = 46.0 N.M.

MAXIMUM WIND = 88.0 MI/HR.

TRANSLATION SPEED = 3.0 KNOTS

INITIAL RISE = 1.90 FEET

BOTTOM FRICTION FACTOR = .0030

WIND STRESS CORRECTION FACTOR = 1.00

C1 = 203.0

C2 = 106.0

C3 = 5280.0

TIME (HOURS)	SETUP X-COMP. (FT.)	SETUP Y-COMP. (FT.)	TOT. WIND SETUP (FT.)	AST. TIDE LEVEL (FT. MLW)	INITIAL WATER LEVEL (FT.)	PRESSURE SETUP (FT.)	TOTAL WATER LEVEL (FT. MLW)
1.00	-.11	.12	.01	1.07	1.90	.32	3.30
2.00	-.10	.23	.14	.89	1.90	.33	3.26
3.00	-.09	.34	.25	.75	1.90	.34	3.24
4.00	-.08	.44	.36	.70	1.90	.35	3.20
5.00	-.08	.52	.45	.75	1.90	.36	3.16
6.00	-.07	.61	.54	.89	1.90	.37	3.10
7.00	-.06	.68	.62	1.07	1.90	.38	3.07
8.00	-.10	.75	.65	1.22	1.90	.39	3.16
9.00	-.14	.83	.69	1.30	1.90	.39	4.28
10.00	-.18	.91	.72	1.26	1.90	.40	4.28
11.00	-.23	.99	.76	1.10	1.90	.41	4.17
12.00	-.30	1.07	.77	.84	1.90	.42	3.93
13.00	-.36	1.15	.80	.56	1.90	.42	3.69
14.00	-.39	1.24	.86	.30	1.90	.43	3.50
15.00	-.40	1.34	.94	.14	1.90	.44	3.44
16.00	-.43	1.44	1.02	.10	1.90	.47	3.49
17.00	-.45	1.56	1.11	.20	1.90	.48	3.69
18.00	-.45	1.68	1.23	.41	1.90	.50	4.04
19.00	-.46	1.81	1.35	.69	1.90	.51	4.44
20.00	-.48	1.94	1.46	.98	1.90	.51	4.86
21.00	-.47	2.05	1.58	1.23	1.90	.52	5.24
22.00	-.48	2.16	1.68	1.37	1.90	.53	5.48
23.00	-.47	2.27	1.80	1.39	1.90	.54	5.62
24.00	-.47	2.37	1.89	1.29	1.90	.55	5.63
25.00	-.46	2.46	2.00	1.11	1.90	.56	5.56
26.00	-.41	2.56	2.15	.89	1.90	.57	5.51
27.00	-.34	2.66	2.33	.70	1.90	.59	5.52
28.00	-.26	2.78	2.52	.61	1.90	.61	5.64
29.00	-.13	2.89	2.86	.62	1.90	.63	6.01
30.00	.24	3.00	3.24	.74	1.90	.64	6.52
31.00	.53	3.11	3.63	.91	1.90	.66	7.10
32.00	.54	3.19	3.74	1.10	1.90	.67	7.41
33.00	.56	3.25	3.82	1.24	1.90	.69	7.65
34.00	.59	3.30	3.89	1.30	1.90	.71	7.79
35.00	.94	3.37	4.31	1.24	1.90	.74	8.19
36.00	1.36	3.46	4.83	1.06	1.90	.77	8.56
37.00	1.85	3.57	5.41	.80	1.90	.81	8.92
38.00	2.08	3.68	5.57	.53	1.90	.80	8.79
39.00	2.18	3.22	5.40	.31	1.90	.79	8.41
40.00	2.27	2.94	5.20	.21	1.90	.78	8.09
41.00	2.38	2.80	5.18	.24	1.90	.75	8.08
42.00	2.49	2.66	5.15	.39	1.90	.73	8.17
43.00	2.57	2.51	5.08	.62	1.90	.70	8.31
44.00	2.71	2.45	5.15	.90	1.90	.74	8.69
45.00	2.80	2.40	5.20	1.16	1.90	.79	9.05
46.00	2.91	2.35	5.26	1.33	1.90	.84	9.33
47.00	2.79	2.14	4.93	1.40	1.90	.83	9.06
48.00	2.34	1.95	4.29	1.34	1.90	.81	8.34
49.00	2.52	1.66	4.18	1.16	1.90	.80	8.04
50.00	2.69	1.67	4.37	.92	1.90	.81	7.99
51.00	2.86	1.72	4.59	.69	1.90	.81	7.99
52.00	3.03	1.80	4.83	.54	1.90	.81	8.08
53.00	3.03	1.77	4.80	.50	1.90	.85	8.03
54.00	3.00	1.49	4.48	.58	1.90	.88	7.85
55.00	2.90	1.14	4.04	.74	1.90	.92	7.60
56.00	3.05	.98	4.03	.93	1.90	.94	7.80
57.00	3.20	.86	4.05	1.12	1.90	.96	8.03
58.00	3.39	.74	4.13	1.26	1.90	.97	8.26
59.00	3.19	.61	3.81	1.30	1.90	.95	7.96
60.00	2.97	.46	3.43	1.23	1.90	.94	7.50
61.00	2.78	.36	3.14	1.05	1.90	.91	7.01
62.00	2.75	.27	3.02	.82	1.90	.89	6.64
63.00	2.68	.19	2.87	.62	1.90	.88	6.24
64.00	2.64	.10	2.74	.59	1.90	.86	5.94
65.00	2.55	-.02	2.53	.44	1.90	.85	5.68
66.00	2.39	-.12	2.28	.40	1.90	.84	5.49
67.00	2.31	-.20	2.11	.48	1.90	.83	5.49
68.00	2.34	-.31	2.03	.65	1.90	.79	5.60
69.00	2.39	-.47	1.93	.88	1.90	.76	5.70
70.00	2.40	-.65	1.75	1.11	1.90	.73	5.66
71.00	2.08	-.83	1.25	1.29	1.90	.70	5.24
72.00	1.82	-.96	.86	1.39	1.90	.67	4.80
73.00	1.55	-1.04	.51	1.37	1.90	.64	4.28
74.00	1.30	-1.10	.20	1.23	1.90	.63	3.72
75.00	1.07	-1.14	-.07	.99	1.90	.62	3.18
76.00	.87	-1.17	-.30	.73	1.90	.61	2.72

SUMMARY OF OPEN COAST STORM SURGE PROBLEM

HURRICANE CARLA VERIFICATION AT GALVESTON, TEXAS  
CENTRAL PRESSURE = 27.64 IN. HG.

SEP 1961 AEC 6/73  
PERIPHERAL PRESSURE = 29.92 IN. HG.

RADIUS TO MAXIMUM WIND = 46.0 N.M.

MAXIMUM WIND = 88.0 MI/HR.

TRANSLATION SPEED = 3.0 KNOTS

INITIAL RISE = 1.90 FEET

BOTTOM FRICTION FACTOR = .0030

WIND STRESS CORRECTION FACTOR = .90

C1 = 203.0

C2 = 106.0

C3 = 5200.0

TIME (HOURS)	SETUP X=COMP. (FT.)	SETUP Y=COMP. (FT.)	TUT. WIND SETUP (FT.)	AST. TIDE LEVEL (FT. MLW)	INITIAL WATER LEVEL (FT.)	PRESSURE SETUP (FT.)	TOTAL WATER LEVEL (FT. MLW)
1.00	-.10	.11	.01	1.07	1.90	.32	3.30
2.00	-.09	.21	.12	.89	1.90	.33	3.24
3.00	-.08	.31	.23	.75	1.90	.34	3.22
4.00	-.07	.39	.32	.70	1.90	.35	3.27
5.00	-.07	.48	.41	.75	1.90	.36	3.42
6.00	-.06	.55	.49	.89	1.90	.37	3.65
7.00	-.06	.62	.56	1.07	1.90	.38	3.91
8.00	-.09	.69	.60	1.22	1.90	.39	4.11
9.00	-.12	.76	.64	1.30	1.90	.39	4.23
10.00	-.17	.84	.67	1.26	1.90	.40	4.23
11.00	-.21	.91	.70	1.10	1.90	.41	4.11
12.00	-.27	.99	.72	.84	1.90	.42	3.88
13.00	-.32	1.07	.74	.56	1.90	.44	3.64
14.00	-.35	1.15	.80	.30	1.90	.45	3.45
15.00	-.36	1.24	.86	.14	1.90	.46	3.38
16.00	-.38	1.34	.96	.10	1.90	.47	3.43
17.00	-.41	1.45	1.04	.20	1.90	.48	3.62
18.00	-.41	1.57	1.16	.41	1.90	.50	3.97
19.00	-.42	1.69	1.27	.69	1.90	.51	4.37
20.00	-.43	1.81	1.38	.98	1.90	.51	4.78
21.00	-.42	1.92	1.50	1.23	1.90	.52	5.15
22.00	-.43	2.03	1.59	1.37	1.90	.53	5.39
23.00	-.43	2.13	1.70	1.39	1.90	.54	5.53
24.00	-.42	2.22	1.80	1.29	1.90	.55	5.53
25.00	-.42	2.31	1.89	1.11	1.90	.56	5.46
26.00	-.37	2.41	2.04	.89	1.90	.57	5.40
27.00	-.30	2.51	2.20	.70	1.90	.59	5.39
28.00	-.23	2.61	2.38	.61	1.90	.61	5.50
29.00	-.02	2.72	2.70	.62	1.90	.63	5.85
30.00	.22	2.83	3.05	.74	1.90	.64	6.33
31.00	.47	2.93	3.41	.91	1.90	.66	6.87
32.00	.49	3.01	3.50	1.10	1.90	.67	7.17
33.00	.51	3.07	3.58	1.24	1.90	.69	7.41
34.00	.53	3.12	3.65	1.30	1.90	.71	7.56
35.00	.45	3.19	4.04	1.24	1.90	.74	7.92
36.00	1.23	3.27	4.50	1.06	1.90	.77	8.23
37.00	1.66	3.37	5.04	.80	1.90	.81	8.55
38.00	1.88	3.37	5.18	.53	1.90	.84	8.41
39.00	1.97	3.06	5.02	.31	1.90	.79	8.03
40.00	2.04	2.79	4.83	.21	1.90	.74	7.72
41.00	2.15	2.66	4.81	.24	1.90	.75	7.70
42.00	2.25	2.52	4.77	.39	1.90	.73	7.79
43.00	2.32	2.38	4.70	.62	1.90	.70	7.93
44.00	2.44	2.32	4.76	.90	1.90	.74	8.31
45.00	2.53	2.27	4.80	1.16	1.90	.79	8.65
46.00	2.63	2.23	4.86	1.33	1.90	.84	8.93
47.00	2.52	2.03	4.55	1.40	1.90	.83	8.68
48.00	2.11	1.85	3.97	1.34	1.90	.81	8.02
49.00	2.28	1.57	3.85	1.16	1.90	.80	7.71
50.00	2.43	1.59	4.02	.92	1.90	.81	7.65
51.00	2.59	1.63	4.22	.69	1.90	.81	7.62
52.00	2.74	1.71	4.44	.54	1.90	.81	7.69
53.00	2.74	1.68	4.42	.50	1.90	.85	7.66
54.00	2.71	1.41	4.12	.58	1.90	.84	7.48
55.00	2.62	1.08	3.70	.74	1.90	.92	7.26
56.00	2.76	.93	3.69	.93	1.90	.94	7.45
57.00	2.89	.81	3.70	1.12	1.90	.96	7.68
58.00	3.06	.71	3.77	1.26	1.90	.97	7.91
59.00	2.89	.59	3.47	1.30	1.90	.95	7.62
60.00	2.68	.45	3.13	1.23	1.90	.94	7.20
61.00	2.51	.35	2.86	1.05	1.90	.91	6.73
62.00	2.48	.27	2.75	.82	1.90	.89	6.37
63.00	2.42	.20	2.61	.59	1.90	.88	5.99
64.00	2.39	.11	2.49	.44	1.90	.86	5.70
65.00	2.30	-.00	2.30	.40	1.90	.85	5.45
66.00	2.16	-.09	2.07	.48	1.90	.84	5.29
67.00	2.08	-.17	1.92	.65	1.90	.83	5.29
68.00	2.12	-.27	1.85	.86	1.90	.79	5.41
69.00	2.16	-.41	1.75	1.11	1.90	.76	5.52
70.00	2.16	-.58	1.58	1.29	1.90	.73	5.50
71.00	1.87	-.74	1.13	1.39	1.90	.70	5.12
72.00	1.84	-.86	.77	1.37	1.90	.67	4.71
73.00	1.40	-.95	.45	1.23	1.90	.64	4.22
74.00	1.17	-1.01	.16	.99	1.90	.63	3.68
75.00	.97	-1.05	-.09	.73	1.90	.62	3.18
76.00	.78	-1.08	-.30	.51	1.90	.61	2.72

SUMMARY OF OPEN COAST STORM SURGE PROBLEM

HURRICANE CARLA VERIFICATION AT GALVESTON, TEXAS  
CENTRAL PRESSURE = 27.64 IN. HG.

SEP 1961 AEC 6/73  
PERIPHERAL PRESSURE = 29.92 IN. HG.

RADIUS TO MAXIMUM WIND = 46.0 N.M.  
TRANSLATION SPEED = 3.0 KNOTS  
BOTTOM FRICTION FACTOR = .0030  
C1 = 203.0

MAXIMUM WIND = 88.0 MI/HR.  
INITIAL RISE = 1.90 FEET  
WIND STRESS CORRECTION FACTOR = .80  
C2 = 106.0 C3 = 5280.0

TIME (HOURS)	SETUP X-COMP. (FT.)	SETUP Y-COMP. (FT.)	TOT. WIND SETUP (FT.)	AST. TIDE LEVEL (FT. MLW)	INITIAL WATER LEVEL (FT.)	PRESSURE SETUP (FT.)	TOTAL WATER LEVEL (FT. MLW)
1.00	-.09	.10	.01	1.07	1.90	.32	3.30
2.00	-.08	.19	.11	.89	1.90	.33	3.23
3.00	-.07	.27	.20	.75	1.90	.34	3.19
4.00	-.06	.35	.29	.70	1.90	.35	3.24
5.00	-.06	.43	.37	.75	1.90	.36	3.37
6.00	-.05	.50	.44	.89	1.90	.37	3.60
7.00	-.05	.56	.51	1.07	1.90	.38	3.86
8.00	-.08	.62	.54	1.22	1.90	.39	4.05
9.00	-.11	.69	.58	1.30	1.90	.39	4.17
10.00	-.15	.76	.61	1.26	1.90	.40	4.17
11.00	-.18	.83	.65	1.10	1.90	.41	4.06
12.00	-.24	.90	.66	.84	1.90	.42	3.82
13.00	-.29	.98	.69	.56	1.90	.44	3.59
14.00	-.31	1.05	.75	.30	1.90	.45	3.39
15.00	-.32	1.14	.82	.14	1.90	.46	3.32
16.00	-.34	1.23	.89	.10	1.90	.47	3.36
17.00	-.36	1.34	.97	.20	1.90	.48	3.56
18.00	-.36	1.45	1.09	.41	1.90	.50	3.89
19.00	-.37	1.56	1.19	.69	1.90	.51	4.29
20.00	-.38	1.67	1.29	.98	1.90	.51	4.69
21.00	-.38	1.78	1.40	1.23	1.90	.52	5.06
22.00	-.39	1.88	1.50	1.37	1.90	.53	5.30
23.00	-.38	1.98	1.60	1.39	1.90	.54	5.43
24.00	-.38	2.07	1.69	1.29	1.90	.55	5.43
25.00	-.37	2.15	1.78	1.11	1.90	.56	5.35
26.00	-.33	2.24	1.92	.89	1.90	.57	5.28
27.00	-.27	2.34	2.07	.70	1.90	.59	5.26
28.00	-.21	2.44	2.23	.61	1.90	.61	5.36
29.00	-.02	2.54	2.52	.62	1.90	.63	5.67
30.00	.19	2.65	2.84	.74	1.90	.64	6.12
31.00	.42	2.74	3.17	.91	1.90	.66	6.63
32.00	.64	2.82	3.26	1.10	1.90	.67	7.03
33.00	.85	2.88	3.33	1.24	1.90	.69	7.17
34.00	.97	2.93	3.40	1.30	1.90	.71	7.31
35.00	.76	2.99	3.75	1.24	1.90	.74	7.63
36.00	1.09	3.07	4.17	1.06	1.90	.77	7.90
37.00	1.28	3.17	4.65	.80	1.90	.81	8.16
38.00	1.47	3.11	4.79	.53	1.90	.80	8.01
39.00	1.75	2.88	4.64	.31	1.90	.79	7.64
40.00	1.82	2.63	4.45	.21	1.90	.78	7.34
41.00	1.92	2.51	4.42	.24	1.90	.75	7.31
42.00	2.01	2.38	4.39	.39	1.90	.73	7.41
43.00	2.07	2.25	4.32	.62	1.90	.70	7.54
44.00	2.16	2.19	4.37	.90	1.90	.74	7.91
45.00	2.25	2.14	4.40	1.16	1.90	.79	8.25
46.00	2.35	2.10	4.44	1.33	1.90	.84	8.52
47.00	2.25	1.92	4.14	1.40	1.90	.83	8.29
48.00	1.89	1.75	3.63	1.34	1.90	.81	7.68
49.00	2.03	1.48	3.51	1.16	1.90	.80	7.37
50.00	2.17	1.49	3.66	.92	1.90	.81	7.29
51.00	2.31	1.54	3.84	.69	1.90	.81	7.25
52.00	2.44	1.60	4.04	.54	1.90	.81	7.38
53.00	2.45	1.58	4.02	.50	1.90	.85	7.27
54.00	2.42	1.33	3.74	.58	1.90	.88	7.11
55.00	2.34	1.02	3.35	.74	1.90	.92	6.92
56.00	2.46	.88	3.34	.93	1.90	.94	7.10
57.00	2.58	.77	3.35	1.12	1.90	.96	7.33
58.00	2.74	.67	3.41	1.26	1.90	.97	7.54
59.00	2.58	.56	3.14	1.30	1.90	.95	7.29
60.00	2.40	.43	2.83	1.23	1.90	.94	6.89
61.00	2.24	.34	2.58	1.05	1.90	.91	6.45
62.00	2.22	.27	2.48	.82	1.90	.89	6.10
63.00	2.16	.20	2.36	.59	1.90	.88	5.73
64.00	2.13	.12	2.24	.44	1.90	.86	5.45
65.00	2.05	.01	2.07	.40	1.90	.85	5.22
66.00	1.93	-.07	1.86	.48	1.90	.84	5.08
67.00	1.86	-.13	1.72	.65	1.90	.83	5.10
68.00	1.88	-.23	1.66	.86	1.90	.79	5.23
69.00	1.92	-.35	1.57	1.11	1.90	.76	5.34
70.00	1.93	-.51	1.42	1.29	1.90	.73	5.34
71.00	1.87	-.66	1.01	1.39	1.90	.70	4.00
72.00	1.46	-.77	.69	1.37	1.90	.67	4.62
73.00	1.25	-.85	.39	1.23	1.90	.64	4.16
74.00	1.04	-.91	.13	.99	1.90	.63	3.65
75.00	.86	-.96	-.10	.73	1.90	.62	3.15
76.00	.70	-.99	-.29	.51	1.90	.61	2.72

# APPENDIX B-3. HURRICANE AUDREY, EUGENE ISLAND, LOUISIANA

## SUMMARY OF OPEN COAST STORM SURGE PROBLEM

HURRICANE AUDREY VERIFICATION AT EUGENE ISLAND LA.  
CENTRAL PRESSURE = 27.95 IN. HG.

JUN 1957 AEC 7/73  
PERIPHERAL PRESSURE = 29.70 IN. HG.

RADIUS TO MAXIMUM WIND = 19.0 N.M.

MAXIMUM WIND = 95.0 MI/HR.

TRANSLATION SPEED = 14.0 KNOTS

INITIAL RISE = 1.00 FEET

BOTTOM FRICTION FACTOR = .0030

WIND STRESS CORRECTION FACTOR = 1.10

C1 = 203.0

C2 = 106.0

C3 = 5280.0

TIME (HOURS)	SETUP X-COMP. (FT.)	SETUP Y-COMP. (FT.)	TOT. WIND SETUP (FT.)	AST. TIDE LEVEL (FT. MLW)	INITIAL WATER LEVEL (FT.)	PRESSURE SETUP (FT.)	TOTAL WATER LEVEL (FT. MLW)
2.00	3.29	.95	4.24	.32	1.00	.24	5.80
4.00	2.92	1.16	4.08	1.00	1.00	.25	6.34
6.00	3.85	1.30	5.14	1.40	1.00	.27	7.81
8.00	4.27	1.36	5.63	1.27	1.00	.28	8.17
10.00	6.13	1.35	7.49	.82	1.00	.30	9.60
12.00	4.78	.69	5.47	.80	1.00	.29	7.55
14.00	3.51	.48	3.99	.98	1.00	.28	6.25
16.00	3.13	.20	3.34	.90	1.00	.26	5.50
18.00	3.62	.07	3.70	.30	1.00	.23	5.23
20.00	4.09	.12	4.22	-.60	1.00	.21	4.83
22.00	3.22	-.64	3.18	-.88	1.00	.20	3.49
24.00	2.29	-.08	2.21	-.58	1.00	.18	2.81
26.00	1.81	-.18	1.63	.07	1.00	.16	2.86

SUMMARY OF OPEN COAST STORM SURGE PROBLEM

HURRICANE AUDREY VERIFICATION AT EUGENE ISLAND LA.  
CENTRAL PRESSURE = 27.95 IN. HG.

JUN 1957 AEC 7/73  
PERIPHERAL PRESSURE = 29.70 IN. HG.

RADIUS TO MAXIMUM WIND = 19.0 N.M.

MAXIMUM WIND = 95.0 MI/HR.

TRANSLATION SPEED = 14.0 KNOTS

INITIAL RISE = 1.00 FEET

BOTTOM FRICTION FACTOR = .0025

WIND STRESS CORRECTION FACTOR = 1.10

C1 = 203.0

C2 = 106.0

C3 = 5280.0

TIME (HOURS)	SFTUP X-COMP. (FT.)	SETUP Y-COMP. (FT.)	TOT. WIND SETUP (FT.)	AST. TIDE LEVEL (FT. MLW)	INITIAL WATER LEVEL (FT.)	PRESSURE SETUP (FT.)	TOTAL WATER LEVEL (FT. MLW)
2.00	3.29	.99	4.28	.32	1.00	.24	5.84
4.00	2.92	1.24	4.15	1.00	1.00	.25	6.41
6.00	3.84	1.38	5.22	1.40	1.00	.27	7.89
8.00	4.26	1.45	5.70	1.27	1.00	.28	8.25
10.00	6.11	1.44	7.56	.82	1.00	.30	9.67
12.00	4.77	.74	5.50	.80	1.00	.29	7.59
14.00	3.50	.53	4.03	.98	1.00	.28	6.29
16.00	3.13	.25	3.37	.90	1.00	.26	5.53
18.00	3.62	.10	3.71	.90	1.00	.23	5.24
20.00	4.09	.14	4.23	-.60	1.00	.21	4.84
22.00	3.21	-.04	3.18	-.88	1.00	.20	3.49
24.00	2.29	-.06	2.21	-.58	1.00	.18	2.81
26.00	1.81	-.18	1.62	.07	1.00	.16	2.86

SUMMARY OF OPEN COAST STORM SURGE PROBLEM

HURRICANE AUDREY VERIFICATION AT EUGENE ISLAND LA.  
CENTRAL PRESSURE = 27.95 IN. HG.

JUN 1957 AEC 7/73  
PERIPHERAL PRESSURE = 29.70 IN. HG.

RADIUS TO MAXIMUM WIND = 19.0 N.M.

MAXIMUM WIND = 95.0 MI/HR.

TRANSLATION SPEED = 14.0 KNOTS

INITIAL RISE = 1.00 FEET

BOTTOM FRICTION FACTOR = .0025

WIND STRESS CORRECTION FACTOR = 1.00

C1 = 203.0

C2 = 106.0

C3 = 5280.0

TIME (HOURS)	SFTUP X-COMP. (FT.)	SETUP Y-COMP. (FT.)	TOT. WIND SETUP (FT.)	AST. TIDE LEVEL (FT. MLW)	INITIAL WATER LEVEL (FT.)	PRESSURE SETUP (FT.)	TOTAL WATER LEVEL (FT. MLW)
2.00	2.99	.93	3.91	.32	1.00	.24	5.48
4.00	2.69	1.16	3.85	1.00	1.00	.25	6.11
6.00	3.53	1.30	4.83	1.40	1.00	.27	7.50
8.00	3.92	1.36	5.28	1.27	1.00	.28	7.83
10.00	5.63	1.36	6.99	.82	1.00	.30	9.10
12.00	4.40	.70	5.09	.80	1.00	.29	7.18
14.00	3.22	.50	3.72	.98	1.00	.28	5.98
16.00	2.87	.24	3.11	.90	1.00	.26	5.27
18.00	3.31	.10	3.41	.30	1.00	.23	4.94
20.00	3.75	.14	3.89	-.60	1.00	.21	4.50
22.00	2.95	-.03	2.92	-.88	1.00	.20	3.24
24.00	2.10	-.08	2.03	-.58	1.00	.18	2.63
26.00	1.66	-.17	1.49	.07	1.00	.16	2.72

SUMMARY OF OPEN COAST STORM SURGE PROBLEM

HURRICANE AUDREY VERIFICATION AT EUGENE ISLAND L.A.  
CENTRAL PRESSURE = 27.95 IN. HG.

JUN 1957 AEC 7/73  
PERIPHERAL PRESSURE = 29.70 IN. HG.

RADIUS TO MAXIMUM WIND = 19.0 N.M.

MAXIMUM WIND = 95.0 MI/HR.

TRANSLATION SPEED = 14.0 KNOTS

INITIAL RISE = 1.00 FEET

BOTTOM FRICTION FACTOR = .0025

WIND STRESS CORRECTION FACTOR = .90

C1 = 203.0

C2 = 106.0

C3 = 5200.0

TIME (HOURS)	SFTUP X-COMP. (FT.)	SFTUP Y-COMP. (FT.)	TOT. WIND SFTUP (FT.)	AST. TIDE LFVFL (FT. MLW)	INITIAL WATER LFVFL (FT.)	PRESSURE SETUP (FT.)	TOTAL WATER LEVEL (FT. MLW)
2.00	2.69	.86	3.55	.32	1.00	.24	5.11
4.00	2.46	1.08	3.54	1.00	1.00	.25	5.79
6.00	3.21	1.21	4.42	1.40	1.00	.27	7.09
8.00	3.58	1.27	4.85	1.27	1.00	.28	7.40
10.00	5.14	1.27	6.40	.82	1.00	.30	8.52
12.00	4.82	.65	4.67	.80	1.00	.29	6.76
14.00	2.93	.48	3.41	.98	1.00	.28	5.67
16.00	2.61	.24	2.84	.90	1.00	.26	5.00
18.00	3.01	.10	3.11	.30	1.00	.23	4.64
20.00	3.40	.13	3.54	-.60	1.00	.21	4.15
22.00	2.69	-.03	2.66	-.88	1.00	.20	2.98
24.00	1.91	-.07	1.85	-.58	1.00	.18	2.45
26.00	1.50	-.15	1.35	.07	1.00	.14	2.58

SUMMARY OF OPEN COAST STORM SURGE PROBLEM

HURRICANE AUDREY VERIFICATION AT EUGENE ISLAND L.A.  
CENTRAL PRESSURE = 27.95 IN. HG.

JUN 1957 AEC 7/73  
PERIPHERAL PRESSURE = 29.70 IN. HG.

RADIUS TO MAXIMUM WIND = 19.0 N.M.

MAXIMUM WIND = 95.0 MI/HR.

TRANSLATION SPEED = 14.0 KNOTS

INITIAL RISE = 1.00 FEET

BOTTOM FRICTION FACTOR = .0025

WIND STRESS CORRECTION FACTOR = .90

C1 = 203.0

C2 = 106.0

C3 = 5200.0

TIME (HOURS)	SETUP X-COMP. (FT.)	SETUP Y-COMP. (FT.)	TOT. WIND SETUP (FT.)	AST. TIDE LEVEL (FT. MLW)	INITIAL WATER LFVFL (FT.)	PRESSURE SETUP (FT.)	TOTAL WATER LEVEL (FT. MLW)
2.00	2.39	.79	3.18	.32	1.00	.24	4.74
4.00	2.22	1.00	3.22	1.00	1.00	.25	5.47
6.00	2.89	1.12	4.01	1.40	1.00	.27	6.68
8.00	3.23	1.18	4.41	1.27	1.00	.28	6.96
10.00	4.64	1.17	5.81	.82	1.00	.30	7.92
12.00	3.63	.61	4.24	.80	1.00	.29	6.32
14.00	2.64	.45	3.09	.98	1.00	.28	5.35
16.00	2.34	.23	2.57	.90	1.00	.26	4.73
18.00	2.70	.10	2.80	.30	1.00	.23	4.33
20.00	3.05	.13	3.18	-.60	1.00	.21	3.80
22.00	2.42	-.03	2.39	-.88	1.00	.20	2.71
24.00	1.72	-.06	1.66	-.58	1.00	.18	2.26
26.00	1.35	-.14	1.21	.07	1.00	.16	2.44

# APPENDIX B-4. HURRICANE CAMILLE, BILOXI, MISSISSIPPI

## SUMMARY OF OPEN COAST STORM SURGE PROBLEM

CAMILLE VF=13.0 KNOTS (APPROX.) R=24. N. MILES (APPROX.)

FIRST THREE LETTERS OF HURRICANE NAME (ITYPE) CAM  
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CENTRAL PRESSURE (PO) = 26.73 IN. HG.	PERIPHERAL PRESSURE (PN) = 29.92 IN. HG.
RADIUS TO MAXIMUM WIND (R) = 24.0 N.M.	MAXIMUM WIND (WX) = 125.0 MI/HR.
TRANSLATION SPEED (VF) = 13.0 KNOTS	INITIAL RISE (SE) = 1.20 FEET
BOTTOM FRICTION FACTOR (BFF) = .0030	WIND STRESS CORRECTION FACTOR (WKCOR) = 1.10
GEOGRAPHICAL LATITUDE (PHII) = 30.00	TRAVERSE BEARING (BEAR) = S 12.0 E

TIME (HOURS)	SETUP X-COMP. (FT.)	SETUP Y-COMP. (FT.)	TOT. WIND SETUP (FT.)	AST. TIDE LEVEL (FT. MLW)	INITIAL WATER LEVEL (FT.)	PRESSURE SETUP (FT.)	TOTAL WATER LEVEL (FT. MLW)
1.00	.81	.16	.97	1.04	1.20	.24	3.44
2.00	.65	.32	.98	.92	1.20	.25	3.35
3.00	.51	.49	1.00	.80	1.20	.27	3.31
4.00	.36	.67	1.03	.80	1.20	.28	3.31
5.00	.20	.84	1.04	.83	1.20	.29	3.36
6.00	-.00	1.00	.99	.94	1.20	.30	3.44
7.00	-.29	1.16	.87	1.10	1.20	.31	3.49
8.00	-.58	1.33	.74	1.28	1.20	.33	3.55
9.00	-.96	1.48	.52	1.42	1.20	.34	3.48
10.00	-1.37	1.63	.26	1.49	1.20	.36	3.31
11.00	-1.93	1.77	-.16	1.49	1.20	.37	2.90
12.00	-2.52	1.90	-.63	1.40	1.20	.39	2.36
13.00	-3.15	2.01	-1.34	1.27	1.20	.41	1.54
14.00	-4.00	2.14	-1.85	1.10	1.20	.43	.87
15.00	-4.15	2.28	-1.87	.95	1.20	.45	.73
16.00	-4.79	2.37	-2.42	.84	1.20	.47	.09
17.00	-5.35	2.45	-2.90	.80	1.20	.50	-.45
18.00	-5.92	2.56	-3.35	.81	1.20	.54	-.81
19.00	-7.03	2.67	-4.36	.86	1.20	.58	-1.72
20.00	-7.74	2.80	-3.54	.92	1.20	.63	-2.29
21.00	-6.66	3.07	-3.59	.98	1.20	.69	-.72
22.00	-9.63	3.39	-6.24	1.02	1.20	.78	-3.24
23.00	-6.73	3.84	-2.88	1.03	1.20	.85	-.20
24.00	-8.98	4.48	-4.10	1.03	1.20	.96	-.92
25.00	-5.18	4.84	-.34	1.03	1.20	1.09	2.98
26.00	25.43	2.70	28.13	1.05	1.20	1.27	31.65
27.00	19.87	1.29	21.16	1.11	1.20	1.52	24.98
28.00	18.78	.53	19.30	1.21	1.20	1.90	23.61
29.00	17.56	-.02	17.54	1.33	1.20	2.06	22.13
30.00	14.42	-.20	14.22	1.45	1.20	2.07	18.94
31.00	11.54	-.34	11.20	1.56	1.20	1.94	15.90
32.00	10.00	-.48	9.52	1.65	1.20	1.63	14.01
33.00	8.41	-.61	7.80	1.69	1.20	1.40	12.08
34.00	7.10	-.74	6.36	1.70	1.20	1.18	10.44
35.00	5.94	-.84	5.10	1.64	1.20	.99	8.93
36.00	5.07	-.88	4.19	1.54	1.20	.84	7.77
37.00	4.31	-.88	3.44	1.39	1.20	.70	6.72
38.00	3.80	-.79	3.01	1.22	1.20	.61	6.04
39.00	3.35	-.72	2.63	1.05	1.20	.55	5.43
40.00	3.09	-.63	2.45	.88	1.20	.51	5.04
41.00	3.03	-.56	2.44	.74	1.20	.47	4.85
42.00	2.84	-.50	2.36	.65	1.20	.44	4.65
43.00	2.94	-.43	2.51	.60	1.20	.42	4.74



SUMMARY OF OPEN COAST STORM SURGE PROBLEM

CAMILLE VFR=13.0 KNOTS (APPROX.) R=24. N. MILES (APPROX.)  
 CENTRAL PRESSURE = 26.73 IN. HG. PERIPHERAL PRESSURE = 29.92 IN. HG.  
 RADIUS TO MAXIMUM WIND = 24.0 N.M. MAXIMUM WIND = 125.0 MI/HR.  
 TRANSLATION SPEED = 13.0 KNOTS INITIAL RISE = 1.20 FEET  
 BOTTOM FRICTION FACTOR = .0030 WIND STRESS CORRECTION FACTOR = 1.00  
 C1 = 203.0 C2 = 106.0 C3 = 5260.0

TIME (HOURS)	SETUP X-COMP. (FT.)	SETUP Y-COMP. (FT.)	TOT. WIND SETUP (FT.)	AST. TIDE LEVEL (FT. MLW)	INITIAL WATER LEVEL (FT.)	PRESSURE SETUP (FT.)	TOTAL WATER LEVEL (FT. MLW)
1.00	.74	.15	.89	1.04	1.20	.24	3.37
2.00	.60	.24	.89	.92	1.20	.25	3.26
3.00	.46	.45	.92	.84	1.20	.27	3.22
4.00	.33	.62	.95	.80	1.20	.26	3.23
5.00	.18	.78	.96	.83	1.20	.29	3.28
6.00	-.00	.93	.93	.94	1.20	.30	3.37
7.00	-.26	1.09	.83	1.10	1.20	.31	3.44
8.00	-.53	1.24	.71	1.28	1.20	.33	3.52
9.00	-.47	1.39	.52	1.42	1.20	.34	3.48
10.00	-1.24	1.53	.29	1.49	1.20	.36	3.34
11.00	-1.75	1.67	-.09	1.49	1.20	.37	2.98
12.00	-2.29	1.79	-.50	1.40	1.20	.39	2.49
13.00	-3.03	1.90	-1.13	1.27	1.20	.41	1.75
14.00	-3.54	2.03	-1.56	1.10	1.20	.43	1.17
15.00	-3.70	2.16	-1.54	.95	1.20	.45	.86
16.00	-4.26	2.32	-2.01	.84	1.20	.47	.50
17.00	-4.68	2.47	-2.36	.80	1.20	.50	.14
18.00	-5.03	2.43	-2.59	.81	1.20	.54	-.05
19.00	-5.50	2.54	-2.95	.86	1.20	.58	-.32
20.00	-6.33	2.69	-3.64	.92	1.20	.63	-.88
21.00	-6.51	2.90	-5.61	.98	1.20	.69	-2.74
22.00	-4.96	3.16	-1.79	1.02	1.20	.78	1.21
23.00	-7.65	3.70	-3.95	1.03	1.20	.85	-.88
24.00	-10.14	4.22	-5.92	1.03	1.20	.96	-2.73
25.00	1.72	4.54	6.27	1.03	1.20	1.09	9.58
26.00	21.82	2.58	24.39	1.05	1.20	1.27	27.91
27.00	18.55	1.23	19.78	1.11	1.20	1.52	23.61
28.00	17.47	.52	18.00	1.21	1.20	1.90	22.30
29.00	16.32	.02	16.34	1.33	1.20	2.06	20.93
30.00	14.40	-.15	13.25	1.45	1.20	2.07	17.97
31.00	10.70	-.26	10.42	1.56	1.20	1.94	15.12
32.00	9.26	-.42	8.84	1.65	1.20	1.63	13.32
33.00	7.78	-.55	7.23	1.69	1.20	1.40	11.51
34.00	6.56	-.67	5.88	1.70	1.20	1.18	9.96
35.00	5.48	-.77	4.71	1.64	1.20	.99	8.54
36.00	4.67	-.82	3.85	1.54	1.20	.84	7.43
37.00	3.97	-.82	3.15	1.39	1.20	.70	6.44
38.00	3.49	-.75	2.74	1.22	1.20	.61	5.76
39.00	3.08	-.66	2.39	1.05	1.20	.55	5.20
40.00	2.83	-.60	2.23	.88	1.20	.51	4.82
41.00	2.76	-.56	2.22	.74	1.20	.47	4.63
42.00	2.62	-.46	2.14	.65	1.20	.44	4.44
43.00	2.70	-.41	2.29	.60	1.20	.42	4.31

SUMMARY OF OPEN COAST STORM SURGE PROBLEM

CAMILLE VF=13.0 KNOTS (APPROX.) R=24. N. MILES (APPROX.) PERIPHERAL PRESSURE = 29.92 IN. HG.  
 CENTRAL PRESSURE = 26.73 IN. HG.  
 RADIUS TO MAXIMUM WIND = 24.0 N.M.  
 MAXIMUM WIND = 125.0 MI/HR.  
 TRANSLATION SPEED = 13.0 KNOTS  
 INITIAL RISE = 1.20 FEET  
 BOTTOM FRICTION FACTOR = .0030  
 WIND STRESS CORRECTION FACTOR = .90  
 C1 = 203.0 C2 = 106.0 C3 = 5280.0

TIME (HOURS)	SETUP X=COMP. (FT.)	SETUP Y=COMP. (FT.)	TOT. WIND SFTUP (FT.)	AST. TIDE LEVEL (FT. MLW)	INITIAL WATER (FT.)	PRESSURE SETUP (FT.)	TOTAL WATER LEVEL (FT. MLW)
1.00	.66	.13	.80	1.04	1.20	.24	3.28
2.00	.54	.27	.80	.92	1.20	.25	3.18
3.00	.42	.41	.83	.84	1.20	.27	3.14
4.00	.30	.57	.86	.80	1.20	.28	3.14
5.00	.16	.72	.88	.83	1.20	.29	3.20
6.00	-.00	.86	.86	.94	1.20	.30	3.30
7.00	-.24	1.01	.77	1.10	1.20	.31	3.39
8.00	-.48	1.16	.68	1.28	1.20	.33	3.49
9.00	-.79	1.30	.51	1.42	1.20	.34	3.47
10.00	-1.12	1.43	.32	1.49	1.20	.36	3.36
11.00	-1.98	1.56	-.02	1.49	1.20	.37	3.05
12.00	-2.65	1.68	-.37	1.40	1.20	.39	2.62
13.00	-3.71	1.79	-.92	1.27	1.20	.41	1.96
14.00	-5.19	1.91	-1.29	1.10	1.20	.43	1.44
15.00	-7.28	2.03	-1.24	.95	1.20	.45	1.35
16.00	-9.76	2.11	-1.65	.84	1.20	.47	.86
17.00	-13.11	2.19	-1.92	.80	1.20	.50	.58
18.00	-17.37	2.29	-2.07	.81	1.20	.54	.47
19.00	-22.42	2.40	-2.32	.86	1.20	.58	.32
20.00	-28.29	2.54	-2.75	.92	1.20	.63	.00
21.00	-35.08	2.74	-3.34	.98	1.20	.69	-.47
22.00	-42.12	3.04	-4.08	1.02	1.20	.78	-1.08
23.00	-50.25	3.46	-4.79	1.03	1.20	.85	-1.71
24.00	-59.38	3.93	-5.45	1.03	1.20	.96	-2.44
25.00	-69.57	4.37	-6.19	1.03	1.20	1.09	-3.21
26.00	-80.05	4.84	-7.00	1.05	1.20	1.27	-4.02
27.00	-91.15	5.17	-7.82	1.11	1.20	1.52	-4.85
28.00	-103.13	5.52	-8.64	1.21	1.20	1.90	-5.65
29.00	-116.05	5.86	-9.51	1.33	1.20	2.06	-6.40
30.00	-130.15	6.10	-10.25	1.45	1.20	2.07	-7.07
31.00	-145.84	6.23	-10.91	1.56	1.20	1.94	-7.61
32.00	-163.50	6.36	-11.44	1.65	1.20	1.63	-8.02
33.00	-183.13	6.49	-11.84	1.69	1.20	1.40	-8.31
34.00	-204.00	6.60	-12.19	1.70	1.20	1.18	-8.48
35.00	-226.01	6.70	-12.50	1.64	1.20	.99	-8.44
36.00	-249.20	6.75	-12.68	1.54	1.20	.84	-8.29
37.00	-273.62	6.76	-12.79	1.39	1.20	.70	-8.05
38.00	-300.18	6.70	-12.88	1.22	1.20	.61	-7.72
39.00	-328.40	6.64	-12.95	1.05	1.20	.55	-7.31
40.00	-358.56	6.57	-13.01	.88	1.20	.51	-6.83
41.00	-390.53	6.53	-13.00	.74	1.20	.47	-6.29
42.00	-424.39	6.46	-13.03	.65	1.20	.44	-5.72
43.00	-460.15	6.39	-13.07	.60	1.20	.42	-5.13

SUMMARY OF OPEN COAST STORM SURGE PROBLEM

CAMILLE VF=13.0 KNOTS (APPROX.) R=24. N. MILES (APPROX.)  
 CENTRAL PRESSURE = 26.73 IN. HG. PERIPHERAL PRESSURE = 29.92 IN. HG.  
 RADIUS TO MAXIMUM WIND = 24.0 N.M. MAXIMUM WIND = 125.0 MI/HR.  
 TRANSLATION SPEED = 13.0 KNOTS INITIAL RISE = 1.20 FEET  
 BOTTOM FRICTION FACTOR = .0030 WIND STRESS CORRECTION FACTOR = .80  
 C1 = 203.0 C2 = 106.0 C3 = 5280.0

TIME (HOURS)	SETUP X=COMP. (FT.)	SETUP Y=COMP. (FT.)	TOT. WIND SETUP (FT.)	AST. TIDE LEVEL (FT. MLW)	INITIAL WATER LEVEL (FT.)	PRESSURE SETUP (FT.)	TOTAL WATER LEVEL (FT. MLW)
1.00	.59	.12	.71	1.04	1.20	.24	3.19
2.00	.48	.24	.72	.92	1.20	.25	3.09
3.00	.37	.37	.74	.84	1.20	.27	3.05
4.00	.27	.51	.78	.80	1.20	.28	3.05
5.00	.15	.65	.80	.83	1.20	.29	3.12
6.00	-.00	.79	.79	.94	1.20	.30	3.23
7.00	-.21	.93	.72	1.10	1.20	.31	3.33
8.00	-.43	1.07	.64	1.28	1.20	.33	3.45
9.00	-.70	1.20	.50	1.42	1.20	.34	3.46
10.00	-.99	1.33	.34	1.49	1.20	.36	3.38
11.00	-1.40	1.45	.05	1.49	1.20	.37	3.11
12.00	-1.82	1.57	-.26	1.40	1.20	.39	2.73
13.00	-2.40	1.67	-.73	1.27	1.20	.41	2.15
14.00	-2.91	1.78	-1.03	1.10	1.20	.43	1.70
15.00	-2.87	1.90	-.98	.95	1.20	.45	1.62
16.00	-3.30	1.98	-1.32	.84	1.20	.47	1.19
17.00	-3.58	2.05	-1.53	.80	1.20	.50	.97
18.00	-3.79	2.15	-1.64	.81	1.20	.54	.90
19.00	-4.08	2.25	-1.83	.86	1.20	.58	.81
20.00	-4.54	2.38	-2.16	.92	1.20	.63	.59
21.00	-5.11	2.57	-2.54	.98	1.20	.69	.33
22.00	-5.81	2.85	-2.76	1.02	1.20	.78	.24
23.00	-6.32	3.25	-3.07	1.03	1.20	.85	.00
24.00	-6.10	3.73	-2.37	1.03	1.20	.96	.82
25.00	-.71	4.11	3.40	1.03	1.20	1.09	6.71
26.00	18.03	2.30	20.33	1.05	1.20	1.27	23.86
27.00	15.70	1.11	16.81	1.11	1.20	1.52	20.63
28.00	14.73	.51	15.24	1.21	1.20	1.90	19.55
29.00	13.72	.10	13.83	1.33	1.20	2.08	18.42
30.00	11.26	-.05	11.21	1.45	1.20	2.07	15.93
31.00	8.96	-.18	8.78	1.56	1.20	1.94	13.48
32.00	7.72	-.30	7.42	1.65	1.20	1.63	11.90
33.00	6.46	-.42	6.04	1.69	1.20	1.40	10.33
34.00	5.43	-.54	4.89	1.70	1.20	1.18	8.97
35.00	4.53	-.63	3.89	1.84	1.20	.99	7.72
36.00	3.84	-.68	3.16	1.54	1.20	.84	6.74
37.00	3.26	-.69	2.57	1.39	1.20	.70	5.85
38.00	2.86	-.64	2.22	1.22	1.20	.61	5.26
39.00	2.52	-.60	1.92	1.05	1.20	.55	4.72
40.00	2.32	-.54	1.78	.88	1.20	.51	4.36
41.00	2.27	-.50	1.77	.74	1.20	.47	4.19
42.00	2.14	-.43	1.71	.65	1.20	.44	4.01
43.00	2.20	-.37	1.84	.60	1.20	.42	4.06

# APPENDIX B-5. HURRICANE CAROL, NARRAGANSETT PIER, RHODE ISLAND

## SUMMARY OF OPEN COAST STORM SURGE PROBLEM

CAROL      VF = 33.3 KNOTS      R = 25. N. MILES  
 CENTRAL PRESSURE = 28.69 IN. HG.      PERIPHERAL PRESSURE = 29.92 IN. HG.  
 RADIUS TO MAXIMUM WIND = 25.0 N.M.      MAXIMUM WIND = 95.0 MI/HR.  
 TRANSLATION SPEED = 33.3 KNOTS      INITIAL RISE = .50 FEET  
 BOTTOM FRICTION FACTOR = .0030      WIND STRESS CORRECTION FACTOR = 1.10  
 C1 = 203.0      C2 = 106.0      C3 = 5280.0

TIME (HOURS)	SETUP X-COMP. (FT.)	SETUP Y-COMP. (FT.)	TOT. WIND SETUP (FT.)	AST. TIDE LEVEL (FT. MLW)	INITIAL WATER LEVEL (FT.)	PRESSURE SETUP (FT.)	TOTAL WATER LEVEL (FT. MLW)
1.00	.04	.06	.10	.00	1.20	.11	1.41
2.00	.00	.12	.12	-.20	1.20	.12	1.25
3.00	-.00	.18	.18	.10	1.20	.14	1.62
4.00	-.01	.28	.27	.40	1.20	.16	2.03
5.00	.07	.48	.55	.90	1.20	.19	2.84
6.00	.30	.79	1.09	1.99	1.20	.23	4.51
7.00	1.18	1.20	2.38	3.10	1.20	.29	6.96
8.00	2.30	1.21	3.50	3.90	1.20	.40	9.00
9.00	2.83	.50	3.33	4.00	1.20	.61	9.14
10.00	2.53	-.71	1.82	3.60	1.20	.87	7.48
11.00	1.94	-2.16	-.22	2.70	1.20	.63	4.31
12.00	1.19	-3.21	-2.02	1.90	1.20	.41	1.09
13.00	.57	-3.46	-2.88	.70	1.20	.34	-.65
14.00	.29	-2.91	-2.62	.30	1.20	.27	-.86
15.00	.16	-2.22	-2.07	.40	1.20	.22	-.24

SUMMARY OF OPEN COAST STORM SURGE PROBLEM

CAROL VF = 33.3 KNOTS R = 25.0 N. MILES  
 CENTRAL PRESSURE = 28.69 IN. HG. PERIPHERAL PRESSURE = 29.92 IN. HG.  
 RADIUS TO MAXIMUM WIND = 25.0 N.M. MAXIMUM WIND = 95.0 MI/HR.  
 TRANSLATION SPEED = 33.3 KNOTS INITIAL RISE = .50 FEET  
 BOTTOM FRICTION FACTOR = .0030 WIND STRESS CORRECTION FACTOR = 1.00  
 C1 = 203.0 C2 = 106.0 C3 = 5280.0

TIME (HOURS)	SETUP X-COMP. (FT.)	SETUP Y-COMP. (FT.)	TOT. WIND SETUP (FT.)	AST. TIDE LEVEL (FT. MLW)	INITIAL WATER LEVEL (FT.)	PRESSURE SETUP (FT.)	TOTAL WATER LEVEL (FT. MLW)
1.00	.03	.00	.09	.00	1.20	.11	1.40
2.00	.00	.11	.11	-.20	1.20	.12	1.24
3.00	-.00	.16	.16	.10	1.20	.14	1.60
4.00	-.01	.26	.25	.40	1.20	.16	2.01
5.00	.07	.43	.50	.90	1.20	.19	2.79
6.00	.27	.72	1.00	1.99	1.20	.23	4.41
7.00	1.07	1.09	2.17	3.10	1.20	.29	6.75
8.00	2.09	1.11	3.20	3.90	1.20	.40	8.70
9.00	2.58	.47	3.05	4.00	1.20	.61	8.86
10.00	2.30	-.63	1.67	3.60	1.20	.87	7.34
11.00	1.76	-1.96	-.14	2.70	1.20	.63	4.33
12.00	1.06	-2.93	-1.85	1.50	1.20	.41	1.26
13.00	.52	-3.21	-2.69	.70	1.20	.34	-.46
14.00	.26	-2.75	-2.48	.30	1.20	.27	-.71
15.00	.14	-2.10	-1.96	.40	1.20	.22	-.13

SUMMARY OF OPEN COAST STORM SURGE PROBLEM

CAROL VF = 33.3 KNOTS R = 25.0 N. MILES  
 CENTRAL PRESSURE = 28.69 IN. HG. PERIPHERAL PRESSURE = 29.92 IN. HG.  
 RADIUS TO MAXIMUM WIND = 25.0 N.M. MAXIMUM WIND = 95.0 MI/HR.  
 TRANSLATION SPEED = 33.3 KNOTS INITIAL RISE = .50 FEET  
 BOTTOM FRICTION FACTOR = .0030 WIND STRESS CORRECTION FACTOR = 1.00  
 C1 = 203.0 C2 = 106.0 C3 = 5280.0

TIME (HOURS)	SETUP X-COMP. (FT.)	SETUP Y-COMP. (FT.)	TOT. WIND SETUP (FT.)	AST. TIDE LEVEL (FT. MLW)	INITIAL WATER LEVEL (FT.)	PRESSURE SETUP (FT.)	TOTAL WATER LEVEL (FT. MLW)
1.00	.04	.06	.09	.00	1.20	.11	1.40
2.00	.00	.11	.11	-.20	1.20	.12	1.24
3.00	-.00	.16	.16	.10	1.20	.14	1.60
4.00	-.01	.26	.25	.40	1.20	.16	2.01
5.00	.07	.43	.50	.90	1.20	.19	2.79
6.00	.27	.72	1.00	1.99	1.20	.23	4.42
7.00	1.08	1.10	2.17	3.10	1.20	.29	6.76
8.00	2.12	1.11	3.23	3.90	1.20	.40	8.73
9.00	2.62	.47	3.09	4.00	1.20	.61	8.90
10.00	2.35	-.64	1.71	3.60	1.20	.87	7.38
11.00	1.80	-1.96	-.16	2.70	1.20	.64	4.38
12.00	1.10	-2.94	-1.84	1.50	1.20	.42	1.28
13.00	.53	-3.22	-2.69	.70	1.20	.34	-.45
14.00	.27	-2.75	-2.48	.30	1.20	.27	-.71
15.00	.15	-2.10	-1.96	.40	1.20	.22	-.13

SUMMARY OF OPEN COAST STORM SURGE PROBLEM

CAROL VF = 33.3 KNOTS R = 25.0 MILES  
 CENTRAL PRESSURE = 28.69 IN. HG. PERIPHERAL PRESSURE = 29.92 IN. HG.  
 RADIUS TO MAXIMUM WIND = 25.0 N.M. MAXIMUM WIND = 95.0 MI/HR.  
 TRANSLATION SPEED = 33.3 KNOTS INITIAL RISE = .50 FEET  
 BOTTOM FRICTION FACTOR = .0030 WIND STRESS CORRECTION FACTOR = .90  
 C1 = 203.0 C2 = 106.0 C3 = 5280.0

TIME (HOURS)	SETUP X-COMP. (FT.)	SETUP Y-COMP. (FT.)	TOT. WIND SETUP (FT.)	AST. TIDE LEVEL (FT. MLW)	INITIAL WATER LEVEL (FT.)	PRESSURE SETUP (FT.)	TOTAL WATER LEVEL (FT. MLW)
1.00	.03	.05	.08	.00	1.20	.11	1.39
2.00	.00	.10	.10	-.20	1.20	.12	1.23
3.00	-.00	.15	.15	.10	1.20	.14	1.59
4.00	-.01	.23	.22	.40	1.20	.16	1.98
5.00	.06	.39	.45	.90	1.20	.19	2.74
6.00	.25	.65	.90	1.99	1.20	.23	4.52
7.00	.97	.99	1.96	3.10	1.20	.29	6.55
8.00	1.91	1.01	2.92	3.90	1.20	.40	8.42
9.00	2.36	.44	2.80	4.00	1.20	.61	8.61
10.00	2.12	-.56	1.56	3.60	1.20	.87	7.22
11.00	1.62	-1.76	-.14	2.70	1.20	.64	4.40
12.00	.99	-2.66	-1.67	1.50	1.20	.42	1.45
13.00	.48	-2.96	-2.49	.70	1.20	.34	-.25
14.00	.24	-2.57	-2.33	.30	1.20	.27	-.56
15.00	.13	-1.98	-1.84	.40	1.20	.22	-.02

SUMMARY OF OPEN COAST STORM SURGE PROBLEM

CAROL VF = 33.3 KNOTS R = 25.0 MILES  
 CENTRAL PRESSURE = 28.69 IN. HG. PERIPHERAL PRESSURE = 29.92 IN. HG.  
 RADIUS TO MAXIMUM WIND = 25.0 N.M. MAXIMUM WIND = 95.0 MI/HR.  
 TRANSLATION SPEED = 33.3 KNOTS INITIAL RISE = .50 FEET  
 BOTTOM FRICTION FACTOR = .0030 WIND STRESS CORRECTION FACTOR = .80  
 C1 = 203.0 C2 = 106.0 C3 = 5280.0

TIME (HOURS)	SETUP X-COMP. (FT.)	SETUP Y-COMP. (FT.)	TOT. WIND SETUP (FT.)	AST. TIDE LEVEL (FT. MLW)	INITIAL WATER LEVEL (FT.)	PRESSURE SETUP (FT.)	TOTAL WATER LEVEL (FT. MLW)
1.00	.03	.05	.07	.00	1.20	.11	1.39
2.00	.00	.09	.09	-.20	1.20	.12	1.21
3.00	-.00	.13	.13	.10	1.20	.14	1.57
4.00	-.01	.21	.20	.40	1.20	.16	1.96
5.00	.05	.35	.40	.90	1.20	.19	2.69
6.00	.22	.58	.80	1.99	1.20	.23	4.22
7.00	.86	.88	1.75	3.10	1.20	.29	6.33
8.00	1.70	.91	2.61	3.90	1.20	.40	8.10
9.00	2.11	.41	2.51	4.00	1.20	.61	8.32
10.00	1.88	-.48	1.40	3.60	1.20	.87	7.07
11.00	1.44	-1.55	-.11	2.70	1.20	.64	4.43
12.00	.88	-2.37	-1.49	1.50	1.20	.42	1.63
13.00	.42	-2.69	-2.27	.70	1.20	.34	-.03
14.00	.21	-2.38	-2.17	.30	1.20	.27	-.40
15.00	.12	-1.84	-1.72	.40	1.20	.22	-.10

Pararas-Carayannis, George

Verification study of a bathystrophic storm surge model. Fort Belvoir, Va., U.S. Coastal Engineering Research Center, 1975.

248 p. illus. (U.S. Coastal Engineering Research Center. Technical memorandum no. 50).

Bibliography: p. 132-136.

Verification of a bathystrophic storm surge numerical model is presented. Historical hurricane data from traverses on the gulf and east coasts were used to calibrate combined values of wind and bottom-stress coefficients in hydrodynamic equations for a numerical computation.

1. Hurricane data-Data processing. 2. Coastal engineering. 3. Hurricanes. 4. Storm surges. I. Title. (Series)

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