



SOIL BORING LOG

ROUTE 132 / Grand Avenue DESCRIPTION Union Pacific Rail Bridge over IL 132 LOGGED BY RJC

SECTION 125X-N LOCATION NE Ret Wall SEC. 13, TWP. 45N, RNG. 11E, 3rd PM Latitude 42°22'14.71307" N, Longitude 87°53'42.99138" W

COUNTY Lake DRILLING METHOD HSA HAMMER TYPE AUTO

Table with columns for STRUCT. NO., Station, BORING NO., Station, Offset, Ground Surface Elev., and soil data columns (D, B, U, M, O, I, S, T) for Depth, Location, Consistency, Moisture, and Soil Type. Includes descriptions like '9 inches Concrete Pavement', 'Dark Brown, Moist SAND (Fill), trace gravel and clay', etc.

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer) The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, form 137 (Rev. 8-99)



SOIL BORING LOG

ROUTE 132 / Grand Avenue DESCRIPTION Union Pacific Rail Bridge over IL 132 LOGGED BY RJC

SECTION 125X-N LOCATION NE Ret Wall SEC. 13, TWP. 45N, RNG. 11E, 3rd PM Latitude 42°22'14.70889" N, Longitude 87°53'41.93790" W

COUNTY Lake DRILLING METHOD HSA HAMMER TYPE AUTO

Table with columns for STRUCT. NO., Station, BORING NO., Station, Offset, Ground Surface Elev., and soil data columns (D, B, U, M, O, I, S, T) for Depth, Location, Consistency, Moisture, and Soil Type. Includes descriptions like '9 inches Concrete Pavement', 'Dark Brown SAND (Fill), trace of gravel and clay', etc.

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer) The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, form 137 (Rev. 8-99)



SOIL BORING LOG

ROUTE 132 / Grand Avenue DESCRIPTION Union Pacific Rail Bridge over IL 132 LOGGED BY RJC

SECTION 125X-N LOCATION NE Ret Wall SEC. 13, TWP. 45N, RNG. 11E, 3rd PM Latitude 42°22'15.12871" N, Longitude 87°53'40.97554" W

COUNTY Lake DRILLING METHOD HSA HAMMER TYPE AUTO

Table with columns for STRUCT. NO., Station, BORING NO., Station, Offset, Ground Surface Elev., and soil data columns (D, B, U, M, O, I, S, T) for Depth, Location, Consistency, Moisture, and Soil Type. Includes descriptions like 'Black to Dark Brown, Moist TOPSOIL', 'Brown, Moist SILTY CLAY LOAM, trace organics and gravel', etc.

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer) The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, form 137 (Rev. 8-99)

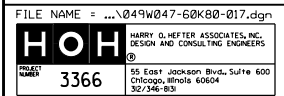


Table with columns for FILE NAME, USER NAME, DESIGNED, CHECKED, DRAWN, PLOT SCALE, PLOT DATE, REVISED, and REVISIONS.

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

SOIL BORING LOGS - SHEET 3 STRUCTURE NO. 049-W047

SHEET NO. 17 OF 18 SHEETS

Table with columns for F.A.P. RTE., SECTION, COUNTY, LAKE, TOTAL SHEETS, SHEET NO., and CONTRACT NO.

ILLINOIS FED. AID PROJECT



SOIL BORING LOG

Division of Highways GSG CONSULTANTS INC. FAU 1218 (Illinois Route 132 / Grand Avenue)

Date 4/28/11

ROUTE 132 / Grand Avenue DESCRIPTION Union Pacific Rail Bridge over IL 132 (Grand Ave.) LOGGED BY RJC

SECTION 125X-N LOCATION NE Ret Wall SEC. 13, TWP. 45N. RNG. 11E. 3rd PM

Latitude 42°22'15.15370" N, Longitude 87°53'40.11997" W

COUNTY Lake DRILLING METHOD HSA HAMMER TYPE AUTO

Table with columns for SOIL BORING LOG: DEPTH, BLOW COUNT, UNCONFINED COMPRESSIVE STRENGTH, MOISTURE, and SOIL DESCRIPTION.

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer) The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, form 137 (Rev. 8-99)



SOIL BORING LOG

Division of Highways GSG CONSULTANTS INC. FAU 1218 (Illinois Route 132 / Grand Avenue)

Date 4/29/11

ROUTE 132 / Grand Avenue DESCRIPTION Union Pacific Rail Bridge over IL 132 LOGGED BY RJC

SECTION 125X-N LOCATION NE Ret Wall SEC. 13, TWP. 45N. RNG. 11E. 3rd PM

Latitude 42°22'15.09277" N, Longitude 87°53'39.17377" W

COUNTY Lake DRILLING METHOD HSA HAMMER TYPE AUTO

Table with columns for SOIL BORING LOG: DEPTH, BLOW COUNT, UNCONFINED COMPRESSIVE STRENGTH, MOISTURE, and SOIL DESCRIPTION.

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer) The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, form 137 (Rev. 8-99)



SOIL BORING LOG

Division of Highways GSG CONSULTANTS INC. FAU 1218 (Illinois Route 132 / Grand Avenue)

Date 4/29/11

ROUTE 132 / Grand Avenue DESCRIPTION Union Pacific Rail Bridge over IL 132 LOGGED BY RJC

SECTION 125X-N LOCATION NE Ret Wall SEC. 13, TWP. 45N. RNG. 11E. 3rd PM

Latitude 42°22'15.03018" N, Longitude 87°53'38.21449" W

COUNTY Lake DRILLING METHOD HSA HAMMER TYPE AUTO

Table with columns for SOIL BORING LOG: DEPTH, BLOW COUNT, UNCONFINED COMPRESSIVE STRENGTH, MOISTURE, and SOIL DESCRIPTION.

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer) The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, form 137 (Rev. 8-99)

HOH logo and company information: HARRY O. HEFNER ASSOCIATES, INC. DESIGN AND CONSULTING ENGINEERS

Table with columns for USER NAME, DESIGNED, REVISIONS, CHECKED, REVISIONS, DRAWN, REVISIONS, CHECKED, REVISIONS.

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

SOIL BORING LOGS - SHEET 4 STRUCTURE NO. 049-W047

SHEET NO. 18 OF 18 SHEETS

Table with columns for F.A.P. RTE., SECTION, COUNTY, TOTAL SHEETS, SHEET NO., CONTRACT NO.

ILLINOIS FED. AID PROJECT

BENCH MARK:

TMB "A" - Elev. 696.20, NW Bolt of Old billboard Foundation +/- 50 ft South of Rte. 132 C.L. and +/- 50 ft. West of Pine Grove Ave.

EXISTING STRUCTURE: None

DESIGN SPECIFICATIONS

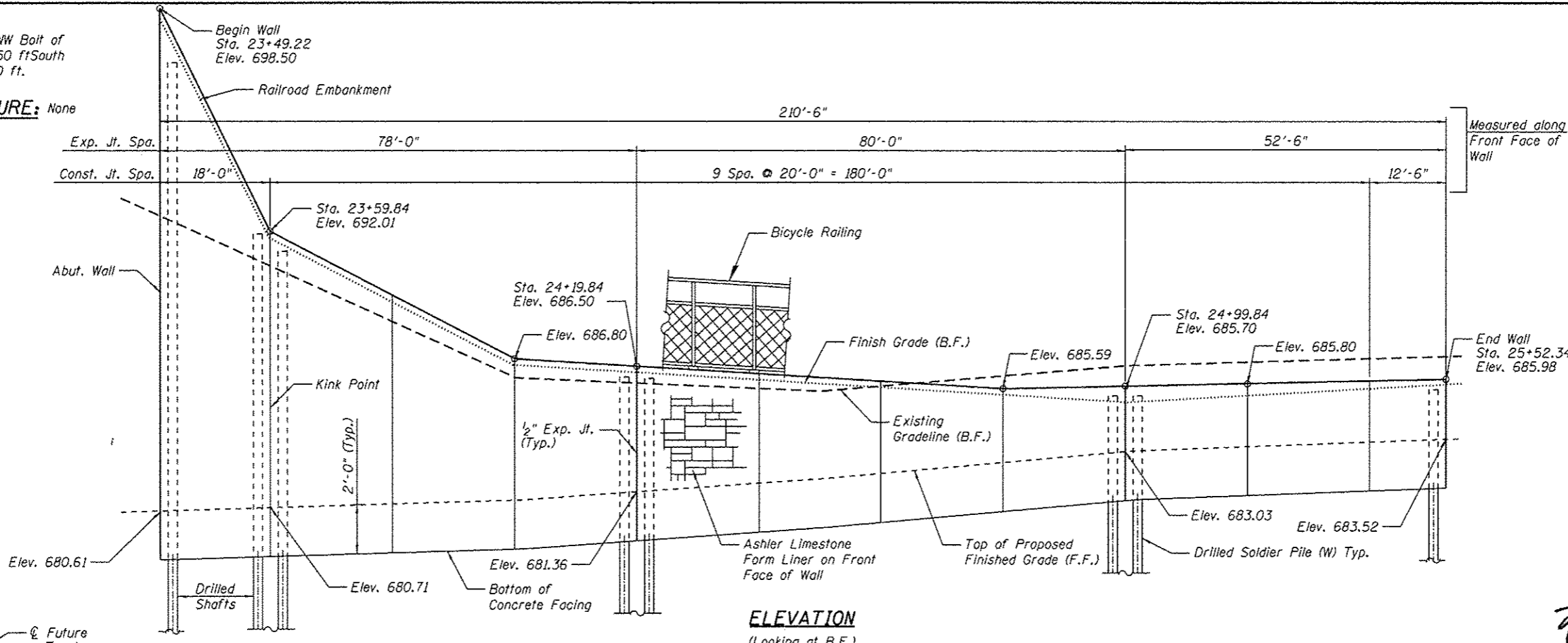
2012 AASHTO LRFD Bridge Design, 6th Edition

DESIGN STRESSES

FIELD UNITS
 f'c = 3,500 psi
 fy = 60,000 psi (Reinforcement)
 fy = 36,000 psi (Structural Steel M270 Grade 36)

SEISMIC DATA

Seismic Performance Zone (SPZ) = 1
 Design Spectral Acceleration at 1.0 sec. (S₀₁) = 0.054g
 Design Spectral Acceleration at 0.2 sec. (S₀₅) = 0.092g
 Soil Site Class = C



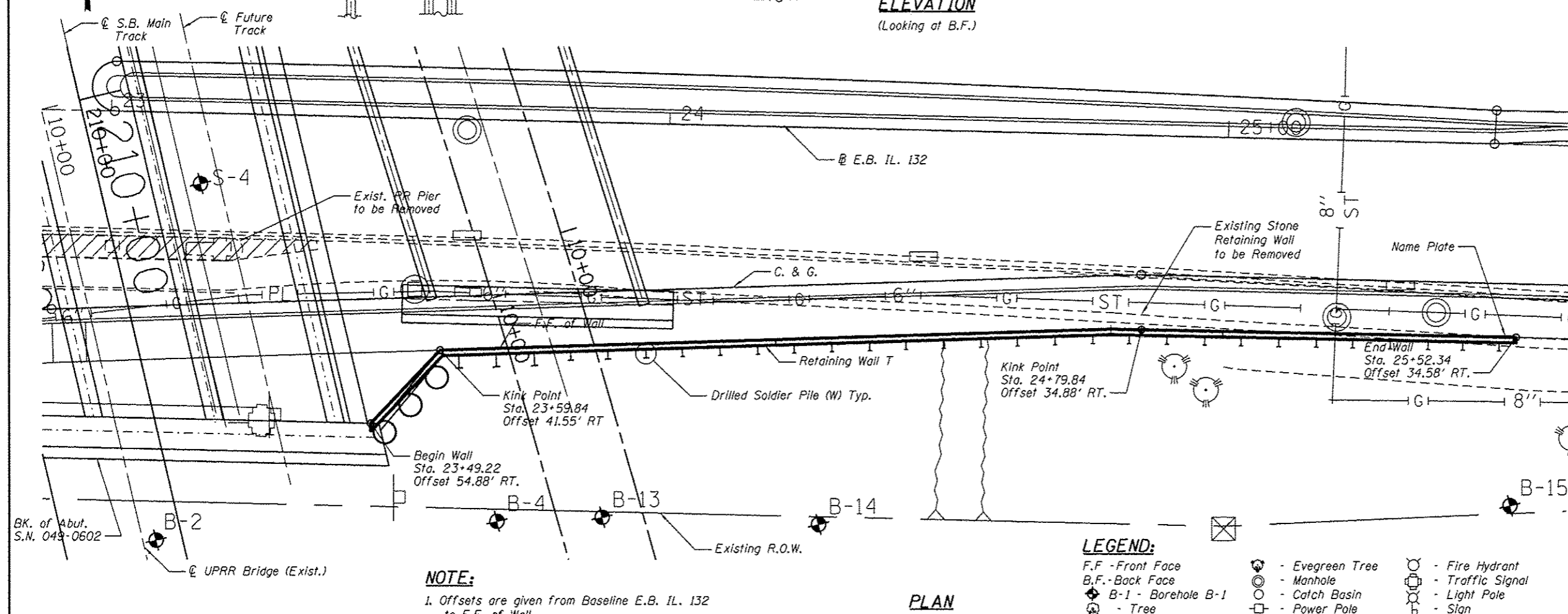
ELEVATION
(Looking at B.F.)

DAVID N. BILSON
 81003186
 LICENSED
 STRUCTURAL
 ENGINEER
 DATE: 6/21/16
 LICENSE EXPIRES 11/30/16

APPROVED

For Structural Adequacy Only

Engineer of Bridges & Structures



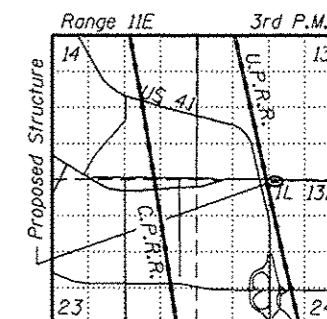
PLAN

NOTE:

1. Offsets are given from Baseline E.B. IL. 132 to F.F. of Wall.

LEGEND:

- F.F. - Front Face
- B.F. - Back Face
- B-1 - Borehole B-1
- Tree
- Bush
- Evergreen Tree
- Manhole
- Catch Basin
- Power Pole
- Hand Hole
- Fire Hydrant
- Traffic Signal
- Light Pole
- Sign



LOCATION SKETCH

GENERAL PLAN & ELEVATION
RETAINING WALL T
IL. ROUTE 132 (GRAND AVE.)
F.A.P. 346 - SEC. 125X-N&J-SB-B
LAKE COUNTY
STATION 23+49.22 TO
STATION 25+52.34
STRUCTURE NO. 049-W048

FILE NAME * ...049W048-60K80-001.dgn	USER NAME * eoffitzpatrick	DESIGNED - MMH	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	IL. ROUTE 132 E.B. RETAINING WALL T GENERAL PLAN AND ELEVATION	F.A.P. RTE. 346	SECTION 125X-N&J-SB-B	COUNTY LAKE	TOTAL SHEETS 361	SHEET NO. 203	
HOH	PLOT SCALE = 21.3333' / in.	CHECKED - DNB	REVISED -			SHEET NO. 1 OF 13 SHEETS		CONTRACT NO. 60K80		ILLINOIS FED. AID PROJECT	
3366	PLOT DATE = 10/7/2016	DRAWN - R.VEJAR	REVISED -								
		CHECKED - BCS	REVISED -								

INDEX OF SHEETS:

1. General Plan & Elevation
2. General Notes, Index of Drawings & Total Bill of Material
3. Wall Plan & Elevation
Sta. 23+47.75 to Sta. 24+19.84
4. Wall Plan & Elevation
Sta. 24+19.84 to Sta. 24+99.84
5. Wall Plan & Elevation
Sta. 24+99.84 to Sta. 25+52.34
6. Reinforcing Details, Drilled Shaft & Pile Summary
7. Wall Sections & Details - Sheet 1
8. Wall Sections & Details - Sheet 2
9. Bicycle Railing - Post Locations
10. Bicycle Railing Details
11. Soil Boring Logs - Sheet 1
12. Soil Boring Logs - Sheet 2
13. Soil Boring Logs - Sheet 3

GENERAL NOTES:

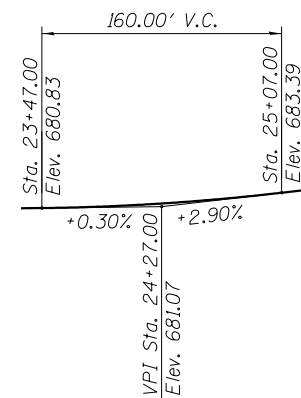
1. Wall stations and offsets are given to the front face of the concrete facing, and are measured from E.B. IL 132 Baseline.
2. Existing utilities in conflict with soldier pile wall construction shall be abandoned or relocated according to direction given in roadway plans
3. Reinforcement Bars designated (E) shall be Epoxy Coated.
4. All exposed concrete edges shall be chamfered $\frac{3}{4}$ " except as noted.
5. Steel piling shall be according to AASHTO M270, Grade 50.
6. No field welding is permitted except as specified in the contract documents.

TOTAL BILL OF MATERIAL

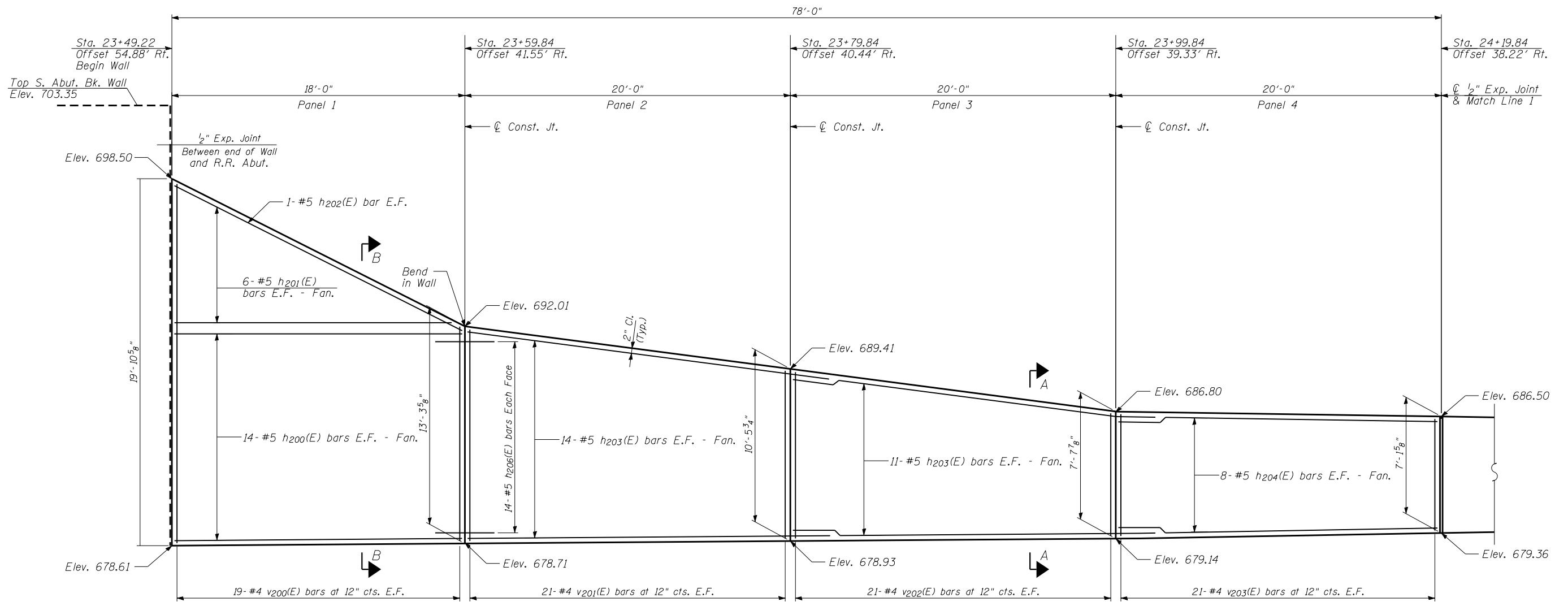
ITEM	UNIT	TOTAL
Structure Excavation	Cu. Yd.	203
Concrete Structures	Cu. Yd.	58
Stud Shear Connectors	Each	312
Untreated Timber Lagging	Sq. Ft.	1,570
Furnishing Soldier Piles (W Section)	Foot	560
Reinforcement Bars	Pound	21,670
Reinforcement Bars, Epoxy Coated	Pound	7,120
Drilled Shaft in Soil	Cu. Yd.	60
Geocomposite Wall Drain	Sq. Yd.	174
Pipe Underdrains for Structures, 4"	Foot	250
Drilling and Setting Soldier Piles (in Soil)	Cu. Ft.	3,064
Bicycle Railing	Foot	207
Name Plates	Each	1
Form Liner Textured Surface	Sq. Ft.	1,400
Furnish and Place Sandfill	Cu. Yd.	570
Staining Concrete Structure	Sq. Ft.	1400

STATION
23+49.22 to 25+52.34
E.B. IL 132
BUILT 20-- BY
STATE OF ILLINOIS
STRUCTURE NO. 049-W048

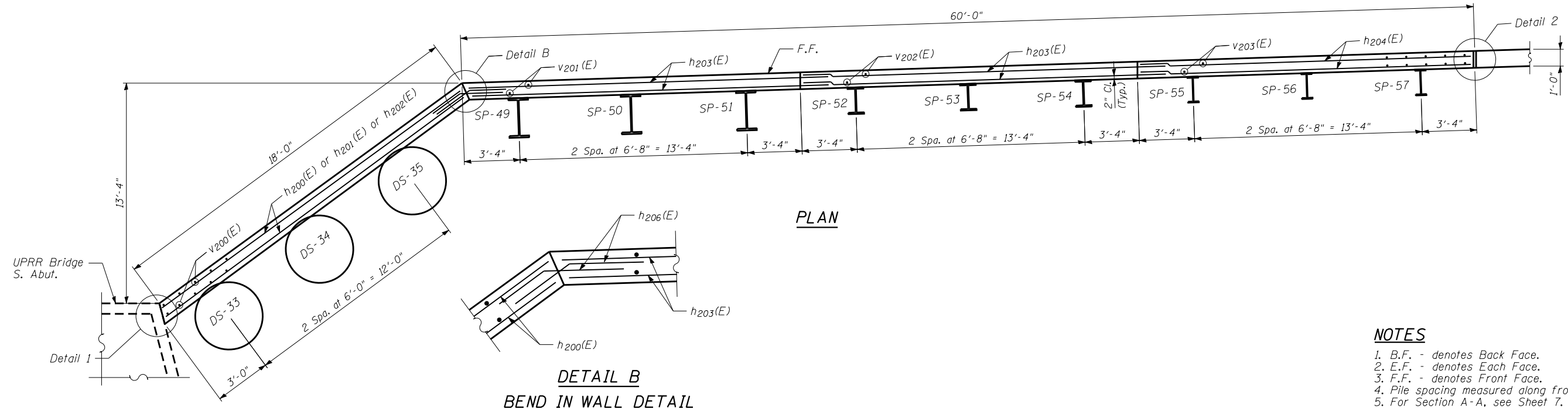
NAME PLATE
See Std. 515001



PROFILE GRADE
IL. 132 E.B. & W.B.



ELEVATION

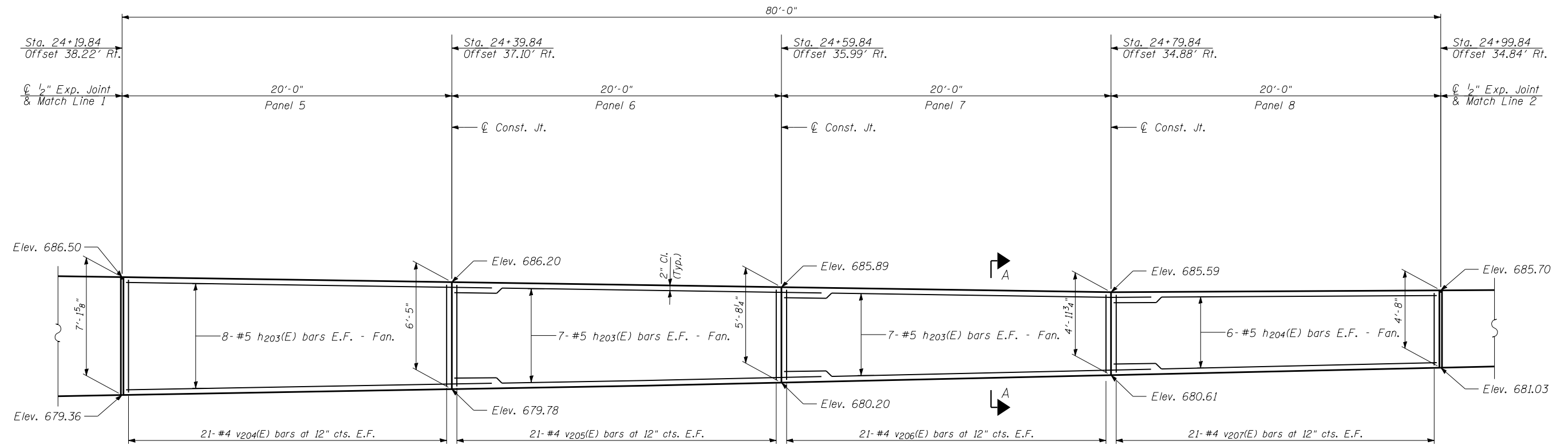


PLAN

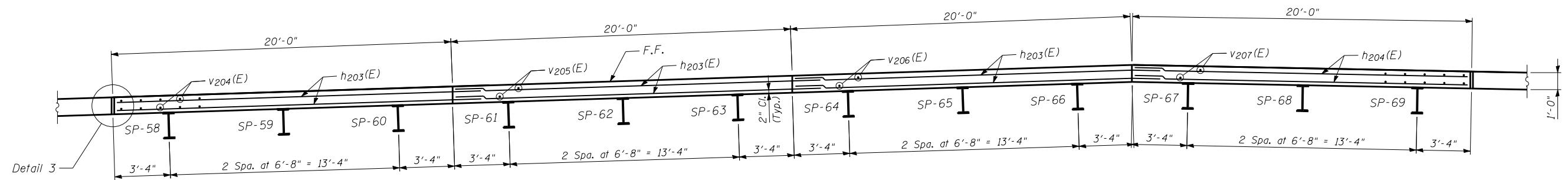
**DETAIL B
BEND IN WALL DETAIL**

- NOTES**
1. B.F. - denotes Back Face.
 2. E.F. - denotes Each Face.
 3. F.F. - denotes Front Face.
 4. Pile spacing measured along front face of wall.
 5. For Section A-A, see Sheet 7.
 6. For Section B-B, see Sheet 8.

FILE NAME - ...049W048-60K80-003.dgn HOH HARRY O. HEFTER ASSOCIATES, INC. DESIGN AND CONSULTING ENGINEERS 3366 55 East Jackson Blvd., Suite 600 Chicago, Illinois 60604 312/346-8131	USER NAME = aofitzpatrick DESIGNED - MMH CHECKED - DNB PLOT SCALE = 5/4,0000 '1' / in. DRAWN - R.VEJAR CHECKED - BCS PLOT DATE = 10/7/2016	DESIGNED - MMH CHECKED - DNB DRAWN - R.VEJAR CHECKED - BCS	REVISED - REVISED - REVISED - REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	PLAN AND ELEVATION STA. 23 + 49.22 TO STA. 24 + 19.84 STRUCTURE NO. 049-W048	F.A.P. RTE. 346 SECTION 125X-N&J-SB-B COUNTY LAKE TOTAL SHEETS 361 SHEET NO. 205 CONTRACT NO. 60K80	SHEET NO. 3 OF 13 SHEETS ILLINOIS FED. AID PROJECT
	SHEET NO. 3 OF 13 SHEETS						



ELEVATION



PLAN

NOTES

1. B.F. - denotes Back Face.
2. E.F. - denotes Each Face.
3. F.F. - denotes Front Face.
4. Pile spacing measured along front face of wall.
5. For Section A-A, see Sheet 7.

FILE NAME - ...049W048-60X80-004.dgn
HOH HARRY O. HEFTER ASSOCIATES, INC.
 DESIGN AND CONSULTING ENGINEERS
 3366 55 East Jackson Blvd., Suite 600
 Chicago, Illinois 60604
 312/346-8931

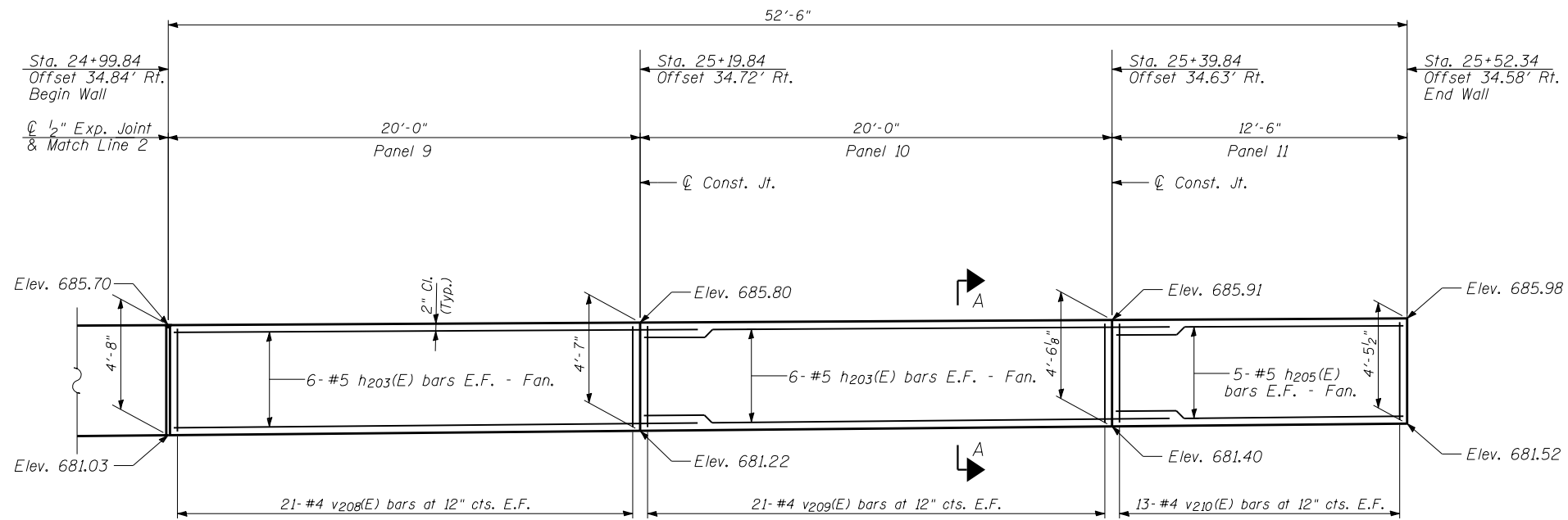
USER NAME = aefitzpatrick	DESIGNED - MMH	REVISED -
PLOT SCALE = 5/4.0000' = 1/4" = 1"	CHECKED - DNB	REVISED -
PLOT DATE = 10/7/2016	DRAWN - R.VEJAR	REVISED -
	CHECKED - BCS	REVISED -

**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

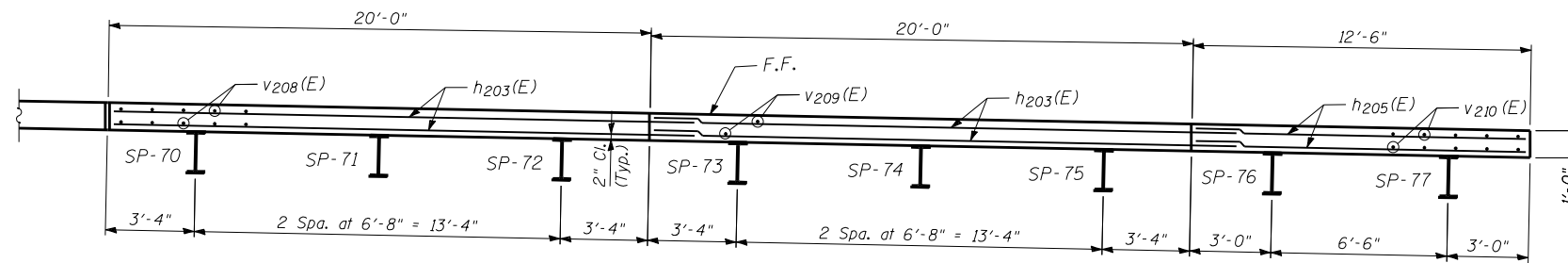
**PLAN AND ELEVATION STA. 24 + 19.84 TO STA. 24 + 99.84
 STRUCTURE NO. 049-W048**

SHEET NO. 4 OF 13 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
346	125X-N&J-SB-B	LAKE	361	206
CONTRACT NO. 60K80				
ILLINOIS FED. AID PROJECT				



ELEVATION



PLAN

NOTES

1. B.F. - denotes Back Face.
2. E.F. - denotes Each Face.
3. F.F. - denotes Front Face.
4. Pile spacing measured along front face of wall.
5. For Section A-A, see Sheet 7.

FILE NAME = ...049W048-60X80-005.dgn
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 DESIGN AND CONSULTING ENGINEERS
 3366 55 East Jackson Blvd., Suite 600
 Chicago, Illinois 60604
 312/346-8931

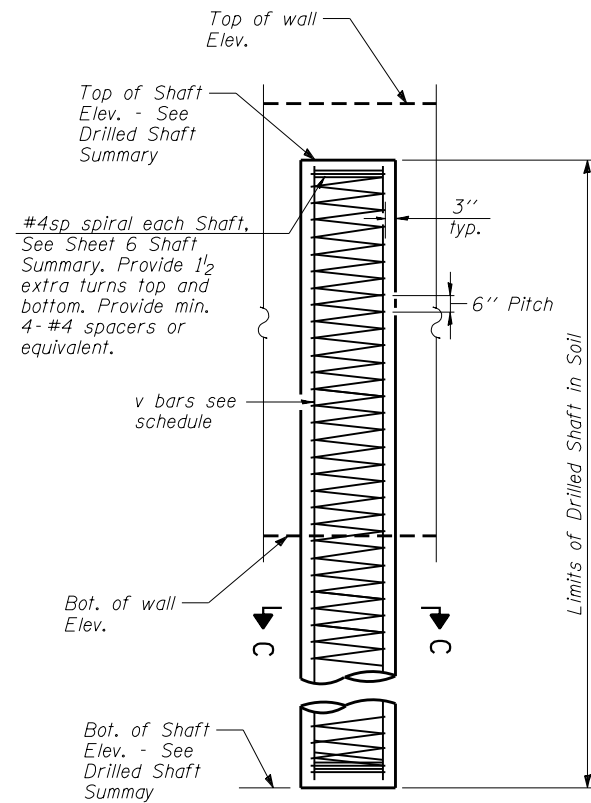
USER NAME = aefitzpatrick	DESIGNED - MMH	REVISED -
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PLOT DATE = 10/7/2016	DRAWN - R,VEJAR	REVISED -
	CHECKED - BCS	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

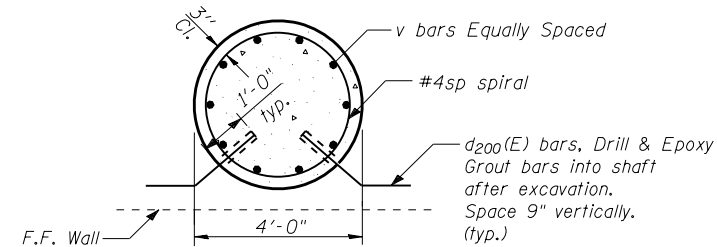
**PLAN AND ELEVATION STA. 24 + 99.84 TO STA. 25 + 52.34
STRUCTURE NO. 049-W048**

SHEET NO. 5 OF 13 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
346	125X-N&J-SB-B	LAKE	361	207
CONTRACT NO. 60K80				
ILLINOIS FED. AID PROJECT				



DRILLED SHAFT ELEVATION

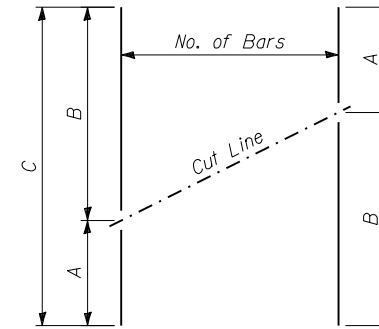


SECTION C-C

Drill & Epoxy Grout d100(E) bars according to Article 584 of the Standard Specifications. The cost shall be included with "Reinforcement Bars, Epoxy Coated".

BAR LIST AND BILL OF MATERIAL

Bar	No. of Sets Required	No. of Bars Per Set	A	B	C
h201(E)	1	6	2'-5"	14'-10"	17'-3"
v200(E)	1	19	12'-11"	19'-6"	32'-5"
v201(E)	1	21	10'-1"	12'-11"	23'-0"
v202(E)	1	21	7'-3"	10'-1"	17'-4"



SERIES OF BAR CUTTING DIAGRAM

See table for dimensions.
Order Bars Full Length, Cut as Shown Normal to Bar Axis and Use Remainder of Bars in Opposite Face.

DRILLED SHAFT SUMMARY

Shaft Mark	Diameter	Length	Top Elevation	Bottom Elevation	Reinforcing (Vertical)	Reinforcing (Horizontal)
DS-33	4'-0"	45	695.13	650.13	28v211	sp200
DS-34	4'-0"	43	692.97	649.97	28v212	sp201
DS-35	4'-0"	41	690.80	649.80	28v213	sp202

BAR LIST AND BILL OF MATERIAL

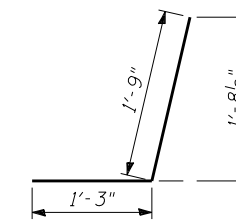
Bar	Number	Size	Length	Shape
d200(E)	350	#5	3'-0"	J
h200(E)	28	#5	17'-8"	—
h201(E)	6	#5	17'-3"	—
h202(E)	2	#5	18'-9"	—
h203(E)	118	#5	19'-8"	—
h204(E)	28	#5	22'-0"	—
h205(E)	10	#5	12'-2"	—
h206(E)	28	#5	4'-4"	—
v200(E)	19	#4	32'-5"	—
v201(E)	21	#4	23'-0"	—
v202(E)	21	#4	17'-4"	—
v203(E)	42	#4	6'-9"	—
v204(E)	42	#4	6'-1"	—
v205(E)	42	#4	5'-4"	—
v206(E)	42	#4	4'-8"	—
v207(E)	42	#4	4'-4"	—
v208(E)	42	#4	4'-3"	—
v209(E)	42	#4	4'-2"	—
v210(E)	26	#4	4'-1"	—
** SP200	1	#4	45'-0"	
** SP201	1	#4	43'-0"	
** SP202	1	#4	41'-0"	
v211	28	#11	44'-8"	—
v212	28	#11	42'-8"	—
v213	28	#11	40'-8"	—

Item	Unit	Quantity
Reinforcement Bars, Epoxy Coated	Pound	7,120
Concrete Structures	Cu. Yd.	58
Furnishing Soldier Piles (W Section)	Foot	560
Drilling and Setting Soldier Piles (in Soil)	Cu. Ft.	3,064
Drilled Shaft in Soil	Cu. Yd.	60
Reinforcement Bars	Pound	21,670

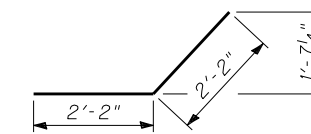
Minimum Lap for Spirals = 1/2 Turns
** Length is Height of Spiral.

LAP SPLICES

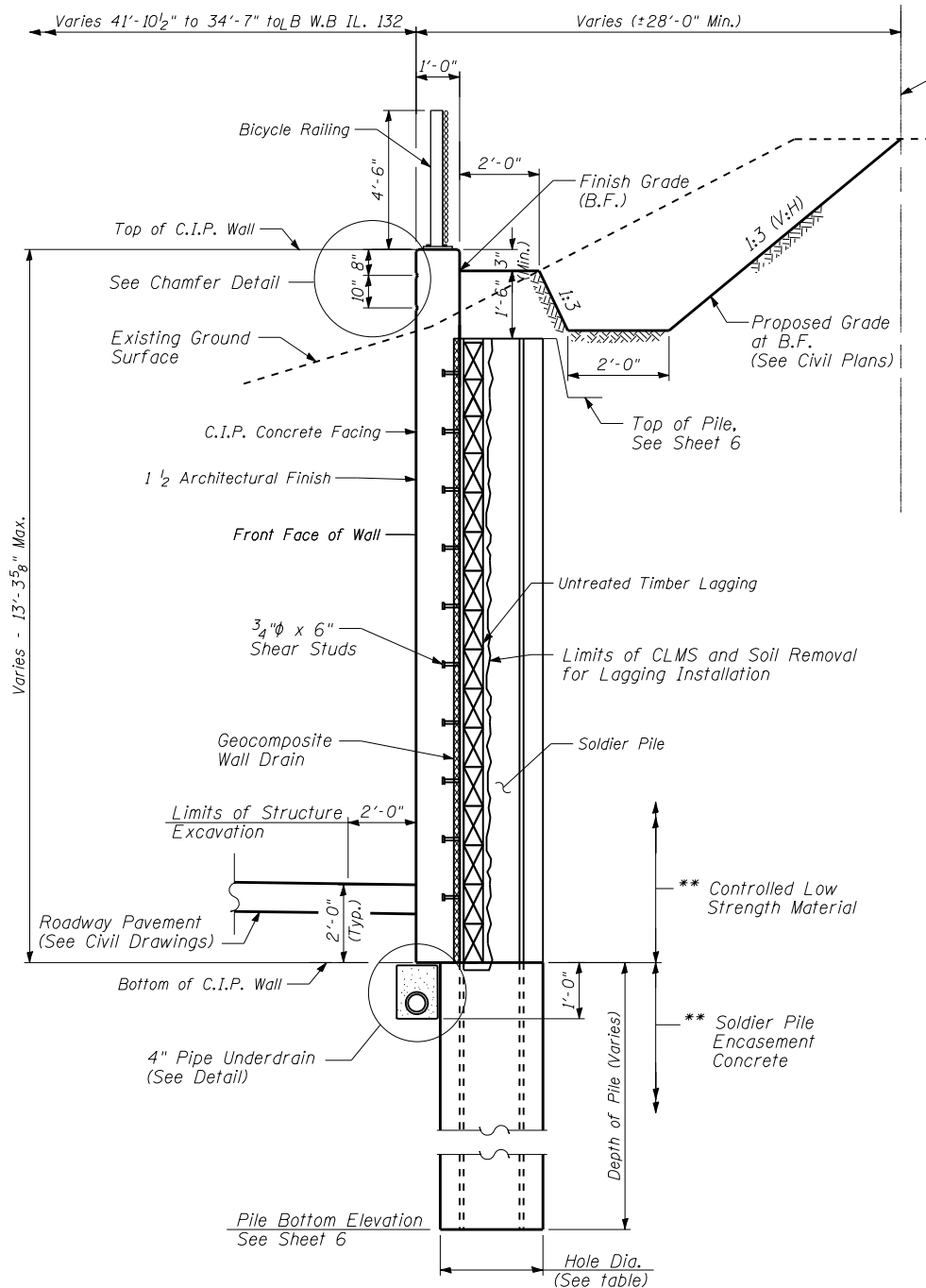
Bar	Lap
#4	1'-8"
#5	2'-2"
#6	2'-7"
#8	4'-6"
#11	9'-0"



BAR d200(E)



BAR h206(E)



SECTION A-A

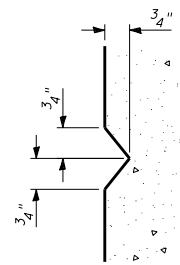
Sta. 23+59.85 to Sta. 25+52.34
(Drilled Soldier Pile Wall)

HOLE DIA.	
Pile Size	Hole Dia.
W36	4'-0"
W33	3'-6"
W30	3'-0"
W21	2'-6"
W18	2'-6"

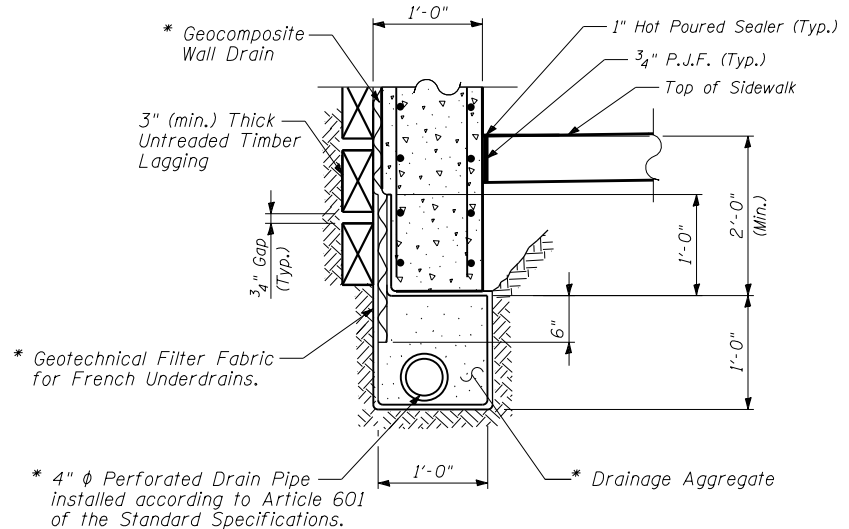
** Included in the cost of "Drilling and Setting Soldier Piles (in Soil)".

CHAMFER DETAIL

Cost of Chamfer included with "Concrete Structures"

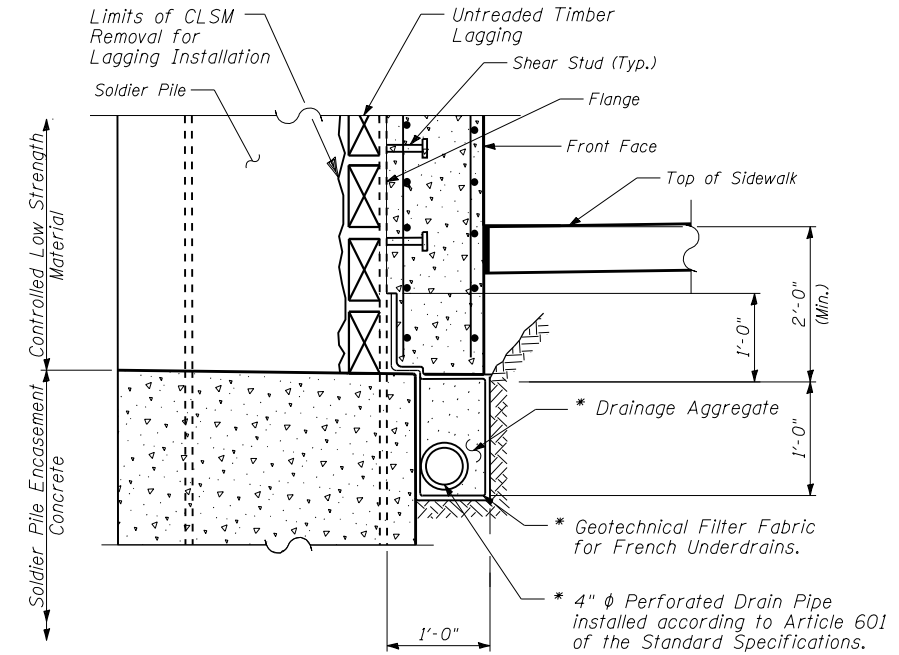


PIPE UNDERDRAIN DETAIL BETWEEN SOLDIER PILES

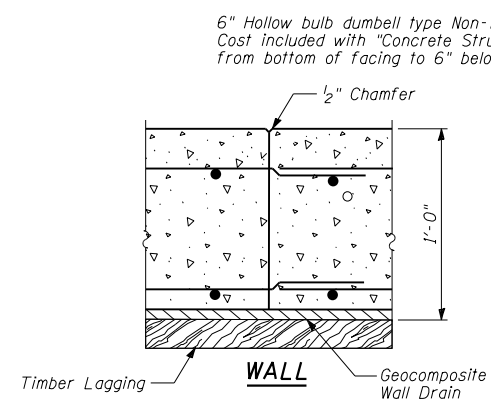


PIPE UNDERDRAIN DETAIL AT SOLDIER PILE

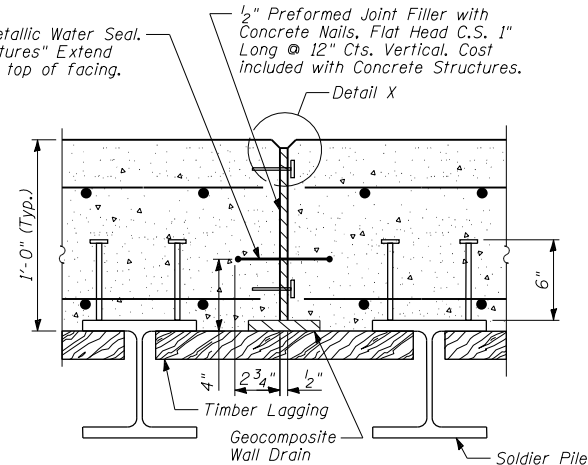
* Included in the cost of "Pipe Underdrains for Structures".



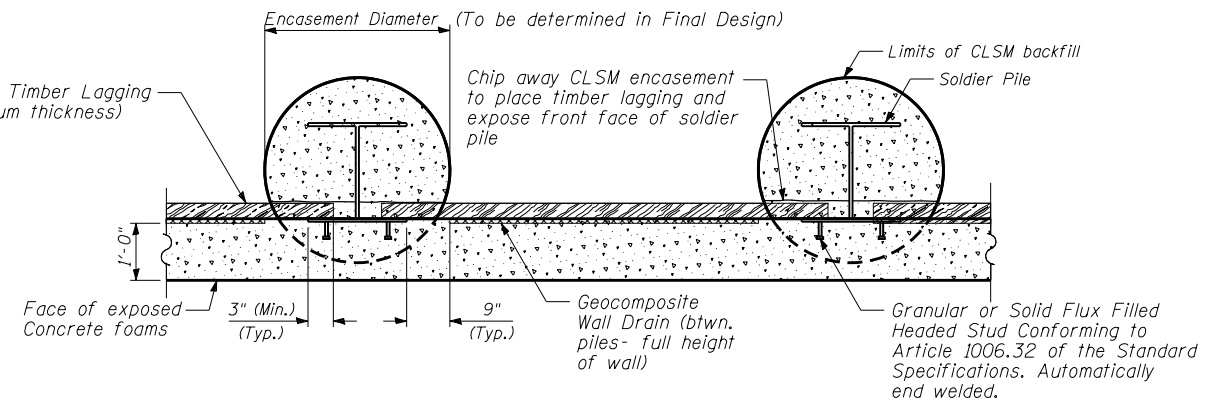
CONSTRUCTION JOINT DETAIL

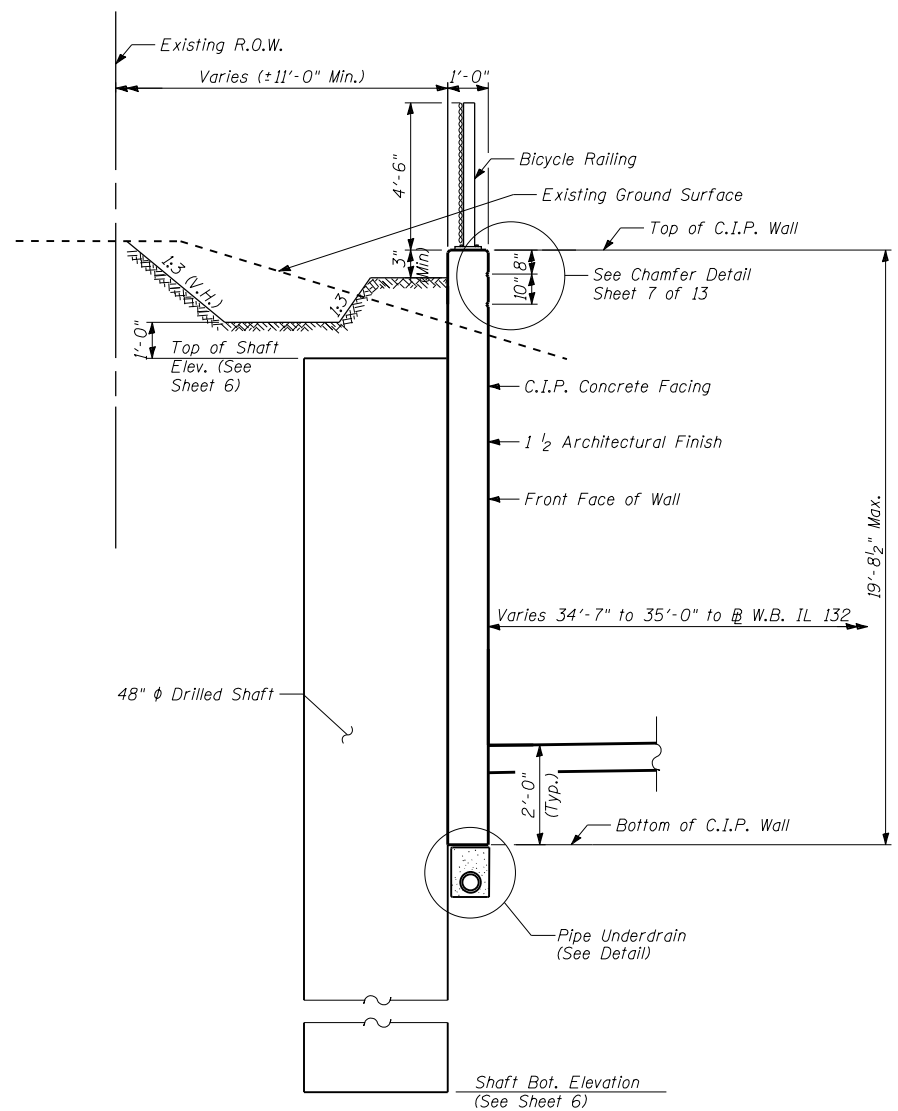


**EXPANSION JOINT DETAIL AT SOLDIER PILE WALL
DETAIL 2**

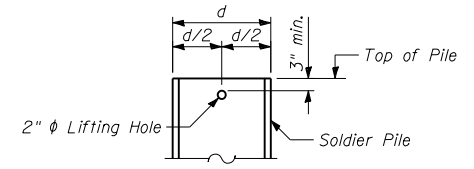


TYPICAL SECTION THRU DRILLED SOLDIER PILE WALL

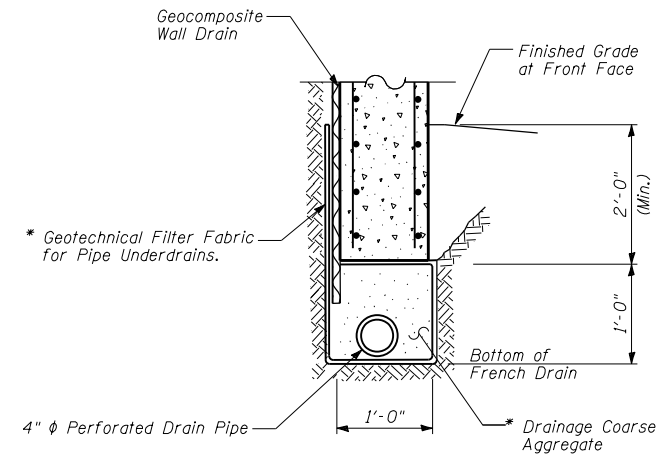




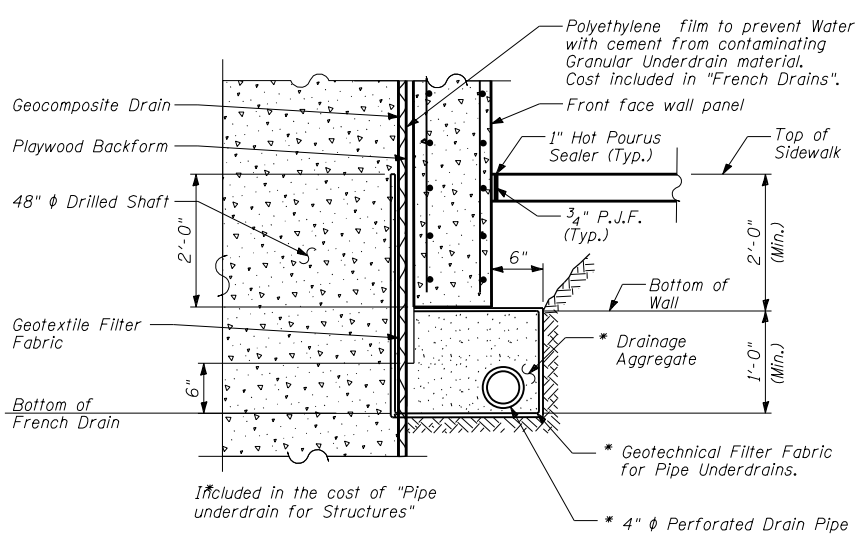
SECTION B-B
Sta. 23+47.75 to Sta. 23+59.84
(Tangent Pile Wall)



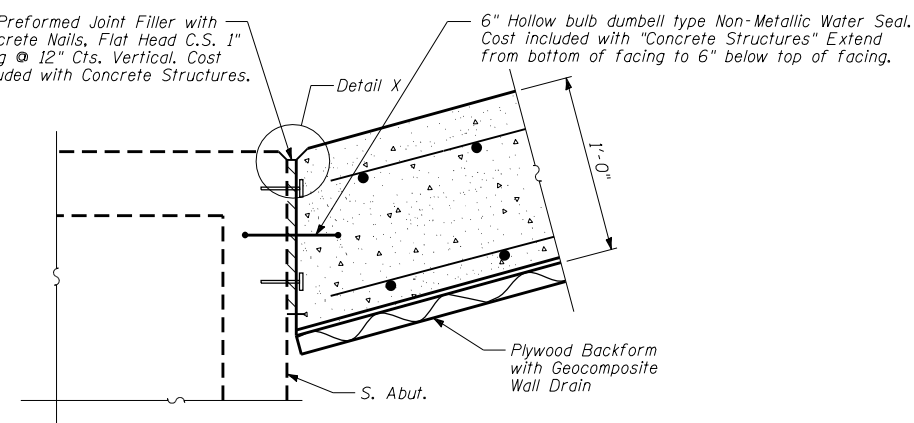
LIFTING HOLE DETAIL
Lifting hole to be provided if necessary.
Cost included with "Furnishing Soldier Piles (W Section)"



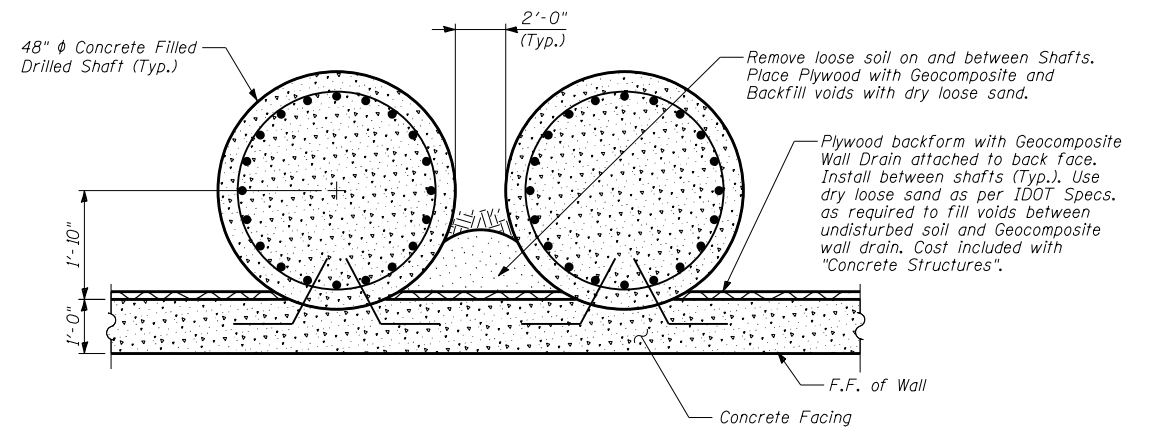
PIPE UNDERDRAIN DETAIL BETWEEN DRILLED SHAFTS



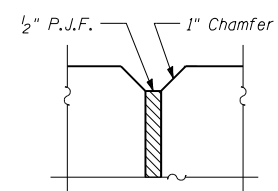
PIPE UNDERDRAIN DETAIL AT DRILLED SHAFT



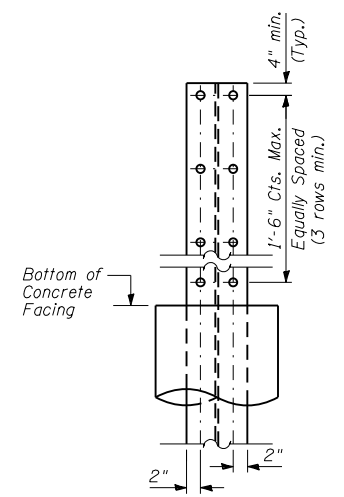
DETAIL 1



TYPICAL SECTION THRU TANGENT PILE WALL



DETAIL X



SHEAR STUD CONNECTOR DETAIL

SOLDIER PILE SUMMARY

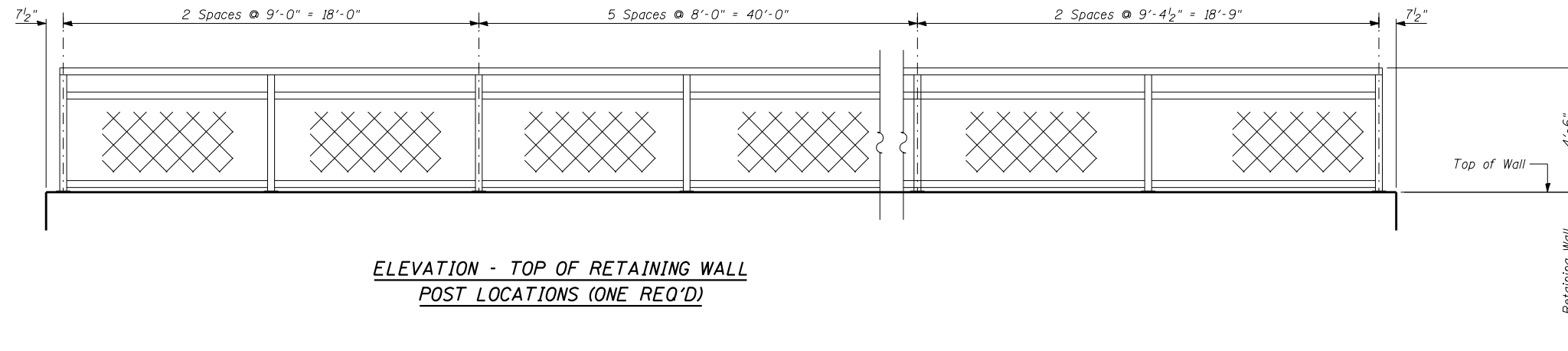
Pile Label	Pile Size	Length (ft.)	Top of Pile Elevation	Bot. of Pile Elevation
SP-49	W33X130	31	689.83	658.83
SP-50	W33X130	31	688.97	657.97
SP-51	W33X130	30	688.09	658.09
SP-52	W33X99	26	687.23	661.23
SP-53	W33X99	25	686.36	661.36
SP-54	W33X99	24	685.48	661.48
SP-55	W21X73	21	685.00	664.00
SP-56	W21X73	21	684.90	663.90
SP-57	W21X73	21	684.80	663.80
SP-58	W21X73	21	684.70	663.70
SP-59	W21X73	21	684.60	663.60
SP-60	W21X73	21	684.50	663.50
SP-61	W18X55	18	684.50	666.50
SP-62	W18X55	18	684.50	666.50
SP-63	W18X55	18	684.50	666.50
SP-64	W18X40	16	684.09	668.09
SP-65	W18X40	16	683.99	667.99
SP-66	W18X40	16	683.89	667.89
SP-67	W18X40	15	683.86	668.86
SP-68	W18X40	15	683.90	668.90
SP-69	W18X40	15	683.93	668.93
SP-70	W18X40	15	683.97	668.97
SP-71	W18X40	15	684.00	669.00
SP-72	W18X40	15	684.03	669.03
SP-73	W18X40	15	684.07	669.07
SP-74	W18X40	15	684.11	669.11
SP-75	W18X40	15	684.14	669.14
SP-76	W18X40	15	684.17	669.17
SP-77	W18X40	15	684.19	669.19

BILL OF MATERIAL

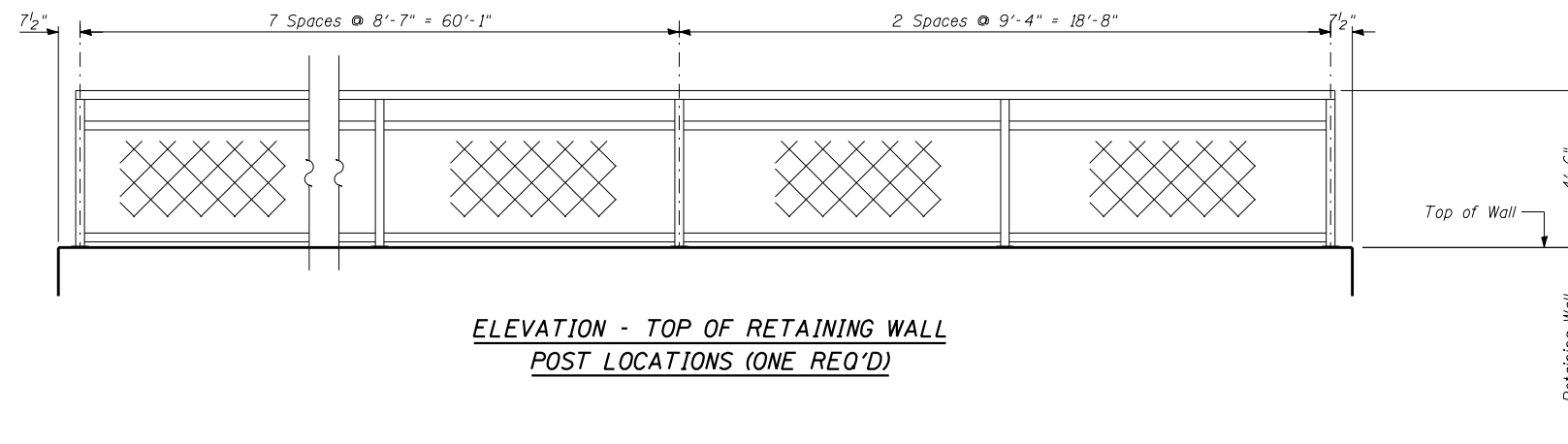
ITEM	UNIT	TOTAL
Structure Excavation	Cu. Yd.	203
Stud Shear Connectors	Each	312
Untreated Timber Lagging	Sq. Ft.	1,570
Geocomposite Wall Drain	Sq. Yd.	174
Pipe Underdrains for Structures, 4"	Foot	250

NOTES:

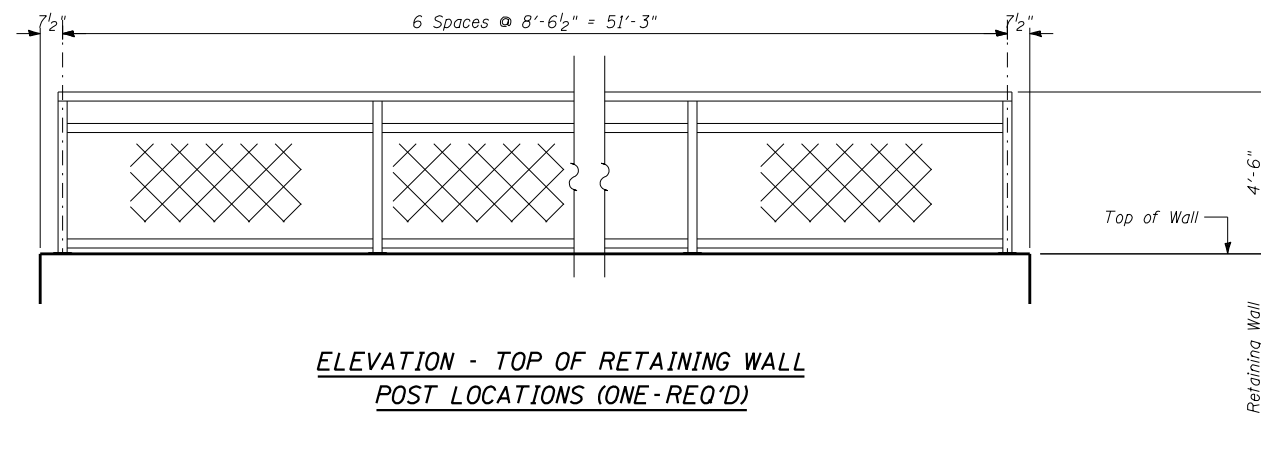
- The Geocomposite Wall Drain shall be constructed according to Section 591 of the Standard Specifications.
- The Contractor is responsible for the design and performance of the lagging using no less than 3" nominal rough-sawn thickness and the minimum tabulated unit stress in bending (f_b), used in the design of timber lagging shall be 1000 psi.
- Stud shear connectors shall be 3/4" ϕ x 6" granular or solid flux filled headed studs, automatically end welded to the front flange of the soldier piles.



ELEVATION - TOP OF RETAINING WALL
POST LOCATIONS (ONE REQ'D)

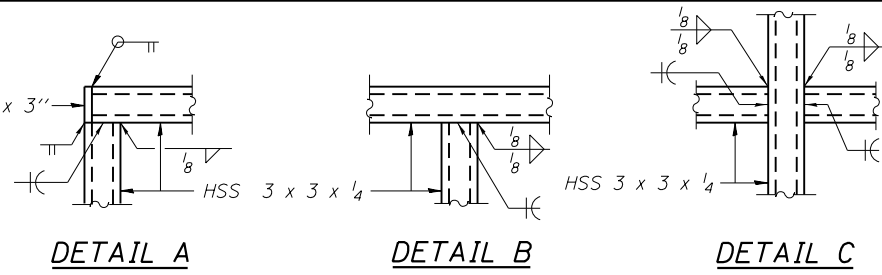
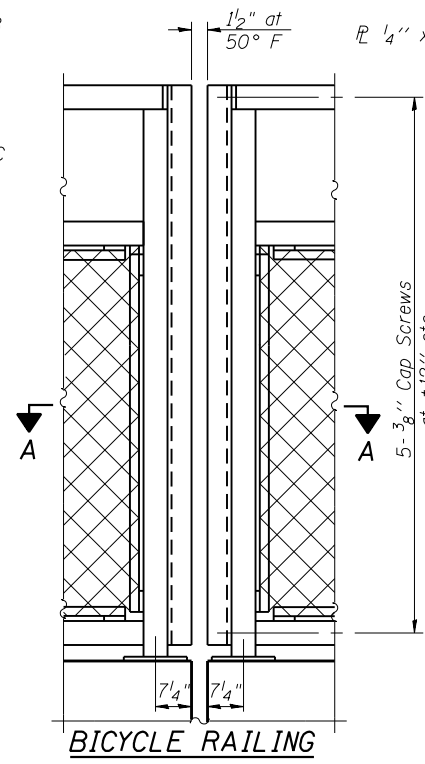
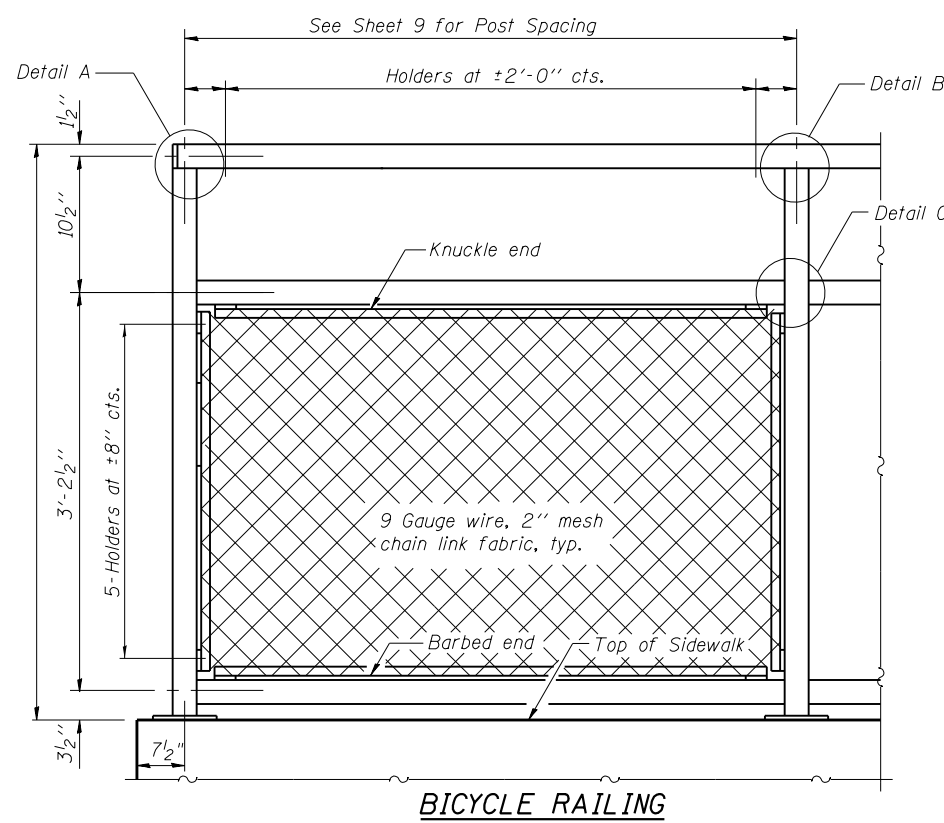


ELEVATION - TOP OF RETAINING WALL
POST LOCATIONS (ONE REQ'D)

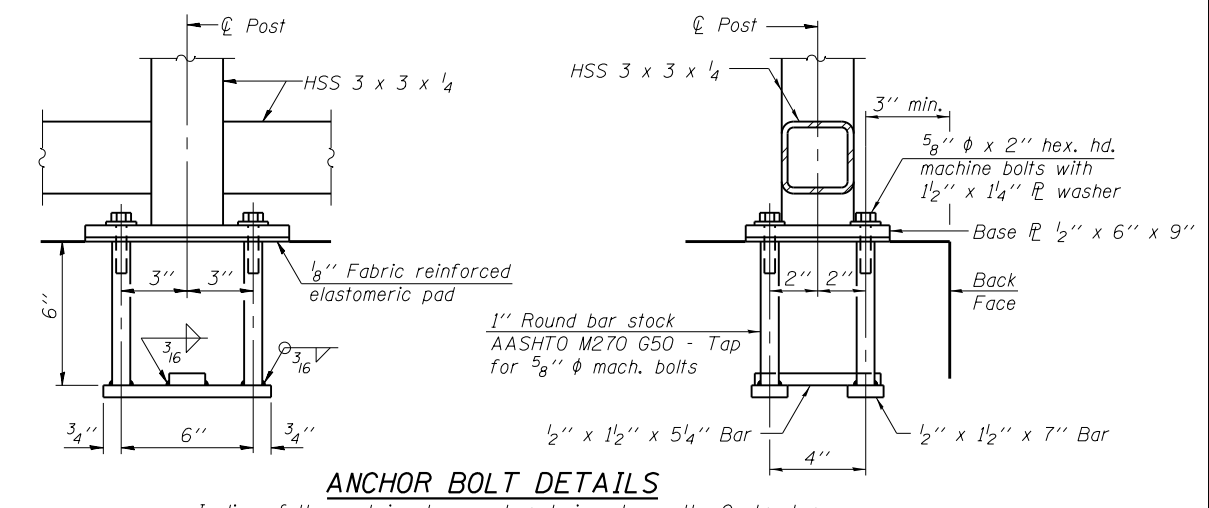
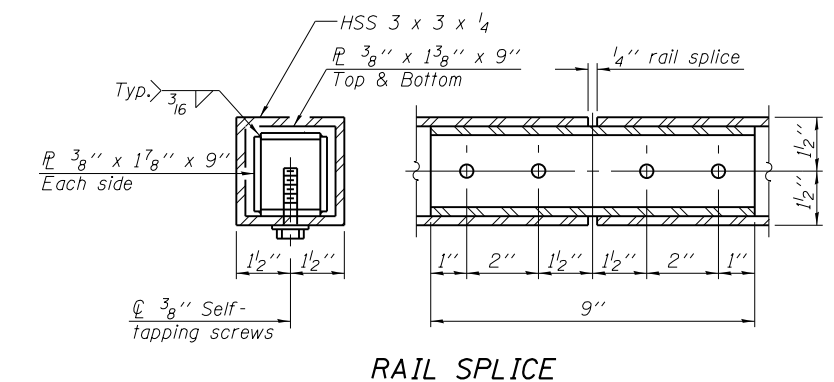
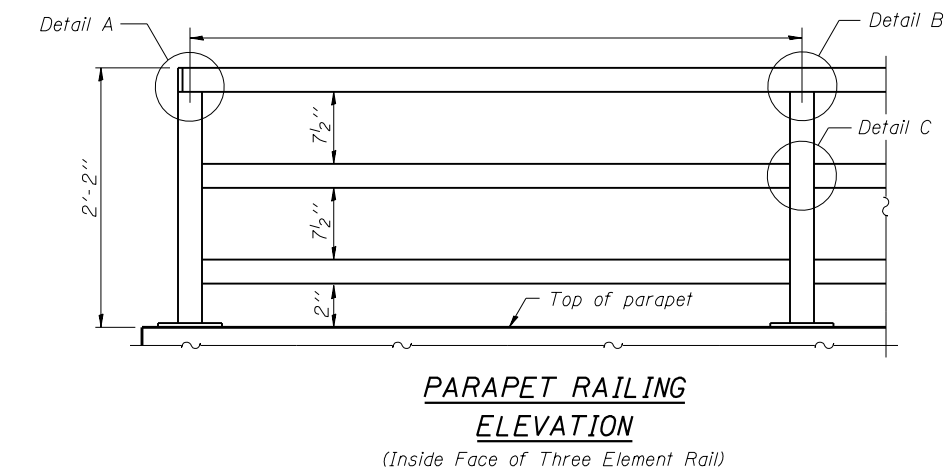
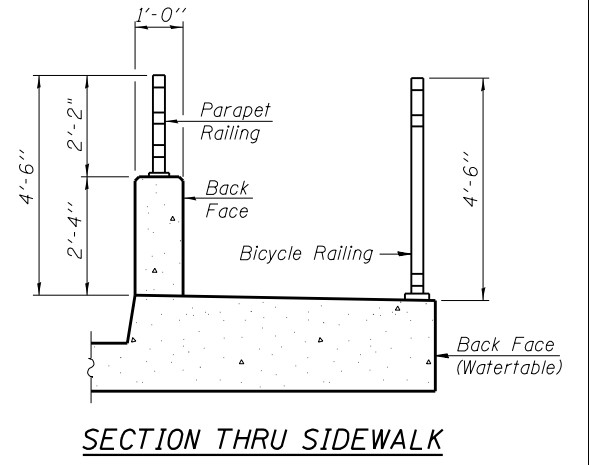
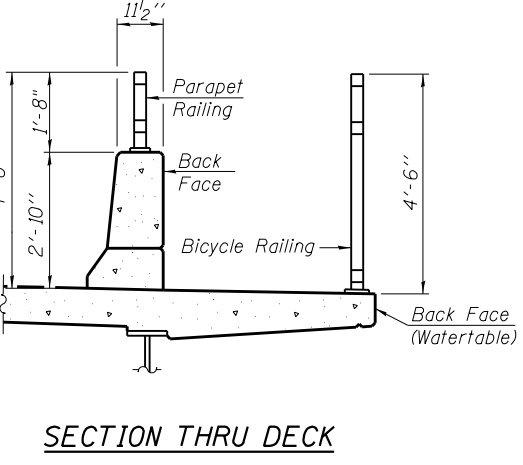
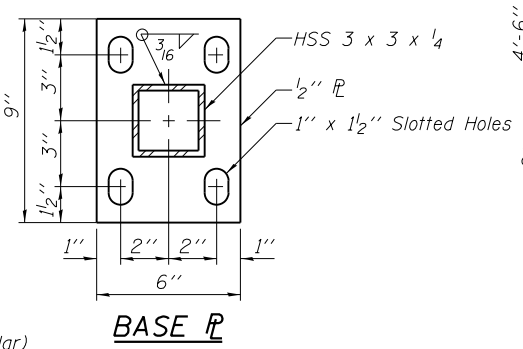
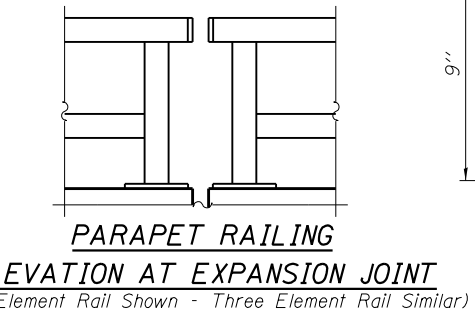
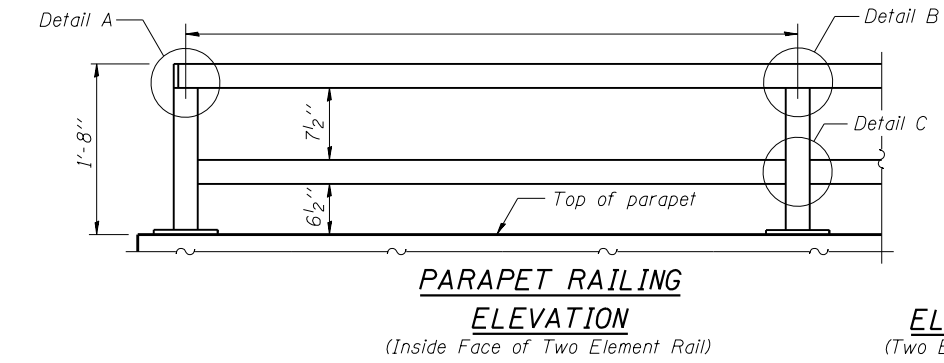
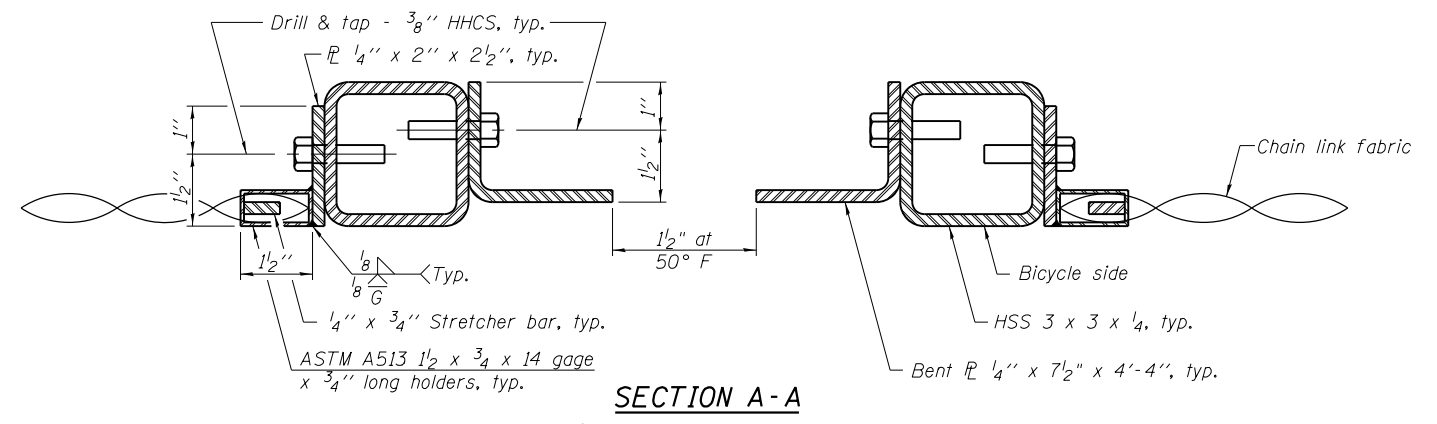


ELEVATION - TOP OF RETAINING WALL
POST LOCATIONS (ONE-REQ'D)

Note:
Posts are to be vertical. Fabricate railing sections to fit changes/slopes of wall top & maintain vertical orientation of posts.



All steel rail elements shall be galvanized according to Article 509.05 of the Standard Specifications.



In lieu of the cast-in-place anchor device shown, the Contractor has the option of drilling and setting 5/8" φ anchor rods according to Article 509.06 of the Standard Specifications. Embedment shall be according to the manufacturer's specifications.

BILL OF MATERIAL

Item	Unit	Quantity
Bicycle Railing	Foot	206.8
Parapet Railing	Foot	

R-29 1-12-15 (10'-0" Maximum Post Spacing)

FILE NAME - ...049W048-60K90-010.dgn	USER NAME - aefitzpatrick	DESIGNED - MMH	REVISED -
HOH HARRY O. HEFTER ASSOCIATES, INC. DESIGN AND CONSULTING ENGINEERS 3366 55 East Jackson Blvd., Suite 600 Chicago, Illinois 60604 312/346-8931	PLOT SCALE = @2.0000 '1' / in.	CHECKED - DNB	REVISED -
	PLOT DATE = 10/7/2016	DRAWN - R.V.EJAR	REVISED -
		CHECKED - BCS	REVISED -

STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

BICYCLE RAILING DETAILS
 STRUCTURE NO. 049-W048

SHEET NO. 10 OF 13 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
346	125X-N&J-SB-B	LAKE	361	212
CONTRACT NO. 60K80				

ILLINOIS FED. AID PROJECT



SOIL BORING LOG

Page 1 of 3 Date 5/26/11

ROUTE 132 / Grand Avenue DESCRIPTION Union Pacific Rail Bridge over IL 132 LOGGED BY RJC
SECTION 125X-N LOCATION S. Abut. SEC. 13, TWP. 45N, RNG. 11E, 3rd PM

Table with 4 columns: DEPTH (ft), BLOW COUNT (blows/ft), UNCONFINED COMPRESSIVE STRENGTH (tsf), and MOISTURE CONTENT (%). Includes sub-headers for Surface Water Elev., Stream Bed Elev., and Groundwater Elev.

Main data table for soil boring log showing soil descriptions (e.g., '4 inches crushed granite (ballast stone)', 'Very Stiff Gray, Moist SILTY LOAM'), depth markers, and blow count data.

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer) The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

Page 2 of 3 Date 5/26/11

ROUTE 132 / Grand Avenue DESCRIPTION Union Pacific Rail Bridge over IL 132 LOGGED BY RJC
SECTION 125X-N LOCATION S. Abut. SEC. 13, TWP. 45N, RNG. 11E, 3rd PM

Table with 4 columns: DEPTH (ft), BLOW COUNT (blows/ft), UNCONFINED COMPRESSIVE STRENGTH (tsf), and MOISTURE CONTENT (%). Includes sub-headers for Surface Water Elev., Stream Bed Elev., and Groundwater Elev.

Main data table for soil boring log showing soil descriptions (e.g., 'Very Stiff Gray, Moist SILTY LOAM (continued)', 'Gray, Fine to Coarse grain, Wet SAND, trace gravel'), depth markers, and blow count data.

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer) The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

Page 3 of 3 Date 5/26/11

ROUTE 132 / Grand Avenue DESCRIPTION Union Pacific Rail Bridge over IL 132 LOGGED BY RJC
SECTION 125X-N LOCATION S. Abut. SEC. 13, TWP. 45N, RNG. 11E, 3rd PM

Table with 4 columns: DEPTH (ft), BLOW COUNT (blows/ft), UNCONFINED COMPRESSIVE STRENGTH (tsf), and MOISTURE CONTENT (%). Includes sub-headers for Surface Water Elev., Stream Bed Elev., and Groundwater Elev.

Main data table for soil boring log showing soil descriptions (e.g., 'Very Stiff Gray, Moist SILTY LOAM, trace gravel (continued)', 'End of Boring'), depth markers, and blow count data.

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer) The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

Date 6/1/11

ROUTE 132 / Grand Avenue DESCRIPTION Union Pacific Rail Bridge over IL 132 LOGGED BY RJC

SECTION 125X-N LOCATION S. Abut. SEC. 13, TWP. 45N, RNG. 11E, 3rd PM Latitude 42°22'13.82437" N, Longitude 87°53'42.74013" W

COUNTY Lake DRILLING METHOD HSA HAMMER TYPE AUTO

Table with columns for Depth, Blows, UCS, Moisture, and Soil Description. Includes data for various soil layers and groundwater levels.

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer) The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206) BBS, form 137 (Rev. 8-99)



SOIL BORING LOG

Date 6/1/11

ROUTE 132 / Grand Avenue DESCRIPTION Union Pacific Rail Bridge over IL 132 LOGGED BY RJC

SECTION 125X-N LOCATION S. Abut. SEC. 13, TWP. 45N, RNG. 11E, 3rd PM Latitude 42°22'13.82437" N, Longitude 87°53'42.74013" W

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SOIL BORING LOG

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BENCH MARK:

TMB "A" - Elev. 696.20, NW Bolt of old billboard foundation ±50 ft. south of Rte. 132 & ±50 ft. west of Pine Grove Ave.

EXISTING STRUCTURE:

Existing bridge S.N. 049-0106 was constructed in 1938 for C&N Railway as Bridge No. 296 in Wisconsin Division. It is a three span structure with four steel beam stringers for each track supporting steel T-frames filled with concrete. The middle span is 51'-7" with end spans of 31'-6". Total length of the deck is 114'-7" and a width of 46 ft. Bridge center line is skewed 14°-33'. Substructure consists of wall type open abutments with slope walls and multi-column piers supported on spread footings.

The existing structure will be completely removed and replaced with a single span through girder superstructure. The total length of the new bridge will be 128'-0" between abutment faces at right angle to the centerline of IL 132. The deck width will be 63'-0" feet and it will have closed abutments on pile foundations. A temporary shoofly bridge will be constructed to maintain railroad traffic during construction. No salvage.

DESIGN SPECIFICATIONS

2010 AREMA Manual for Railway Engineering
2007 BNSF Railway-Union Pacific
Railroad Guide for Railroad Grade
Separation Project

DESIGN STRESSES

FIELD UNITS

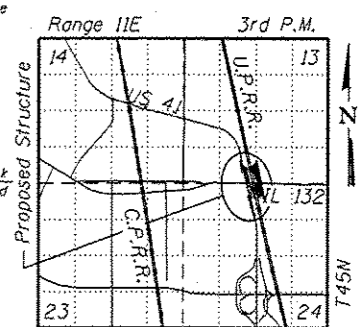
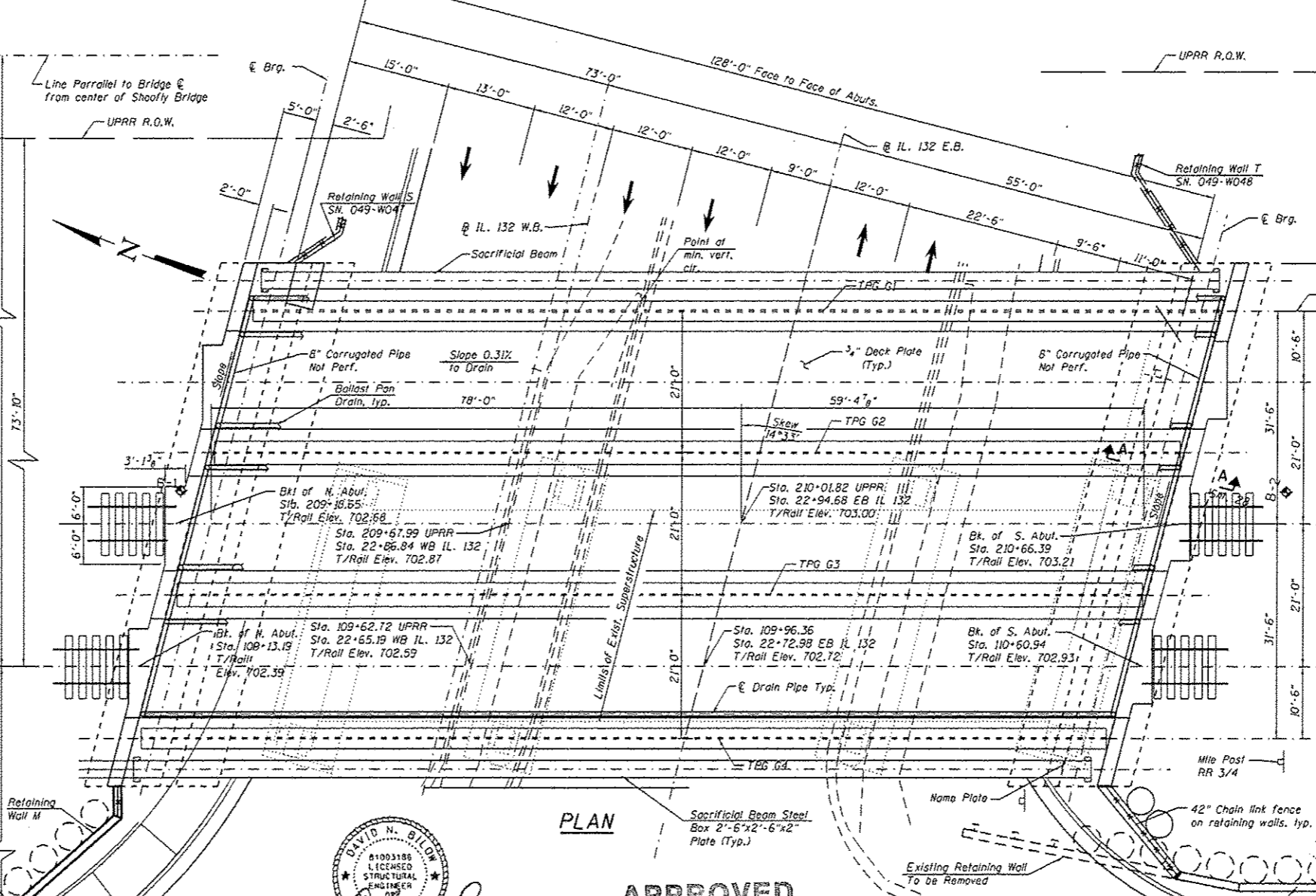
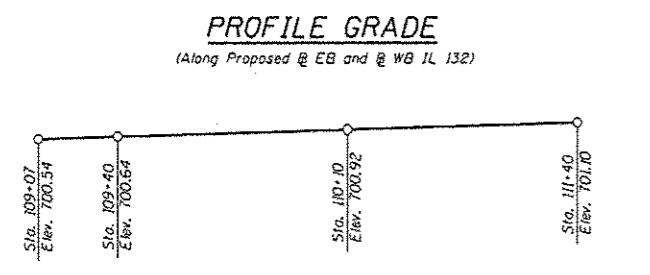
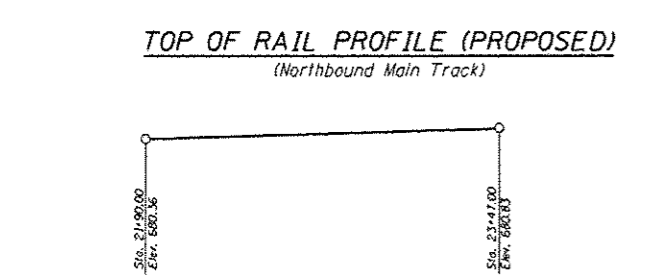
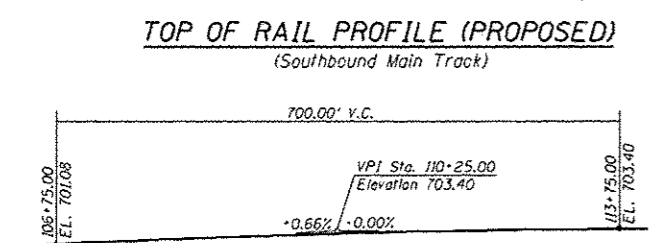
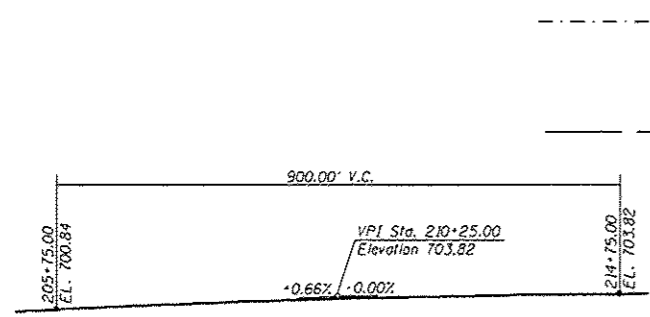
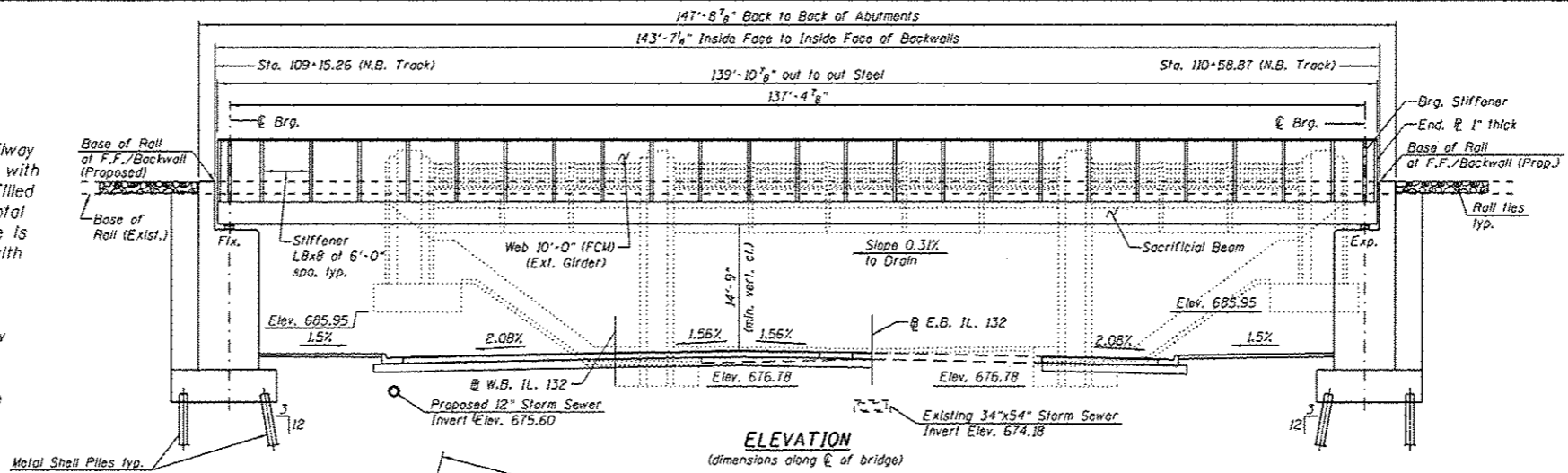
f'c = 4,000 psi
fy = 60,000 psi (Reinforcement)
fy = 50,000 psi (Structural Steel)
ASTM A709 Grade 50

LOADING

Cooper E80 or Alternative Load
on 4 Axles per AREMA Manual
Sacrificial beam vehicle impact load = 100 Kips

SEISMIC DATA

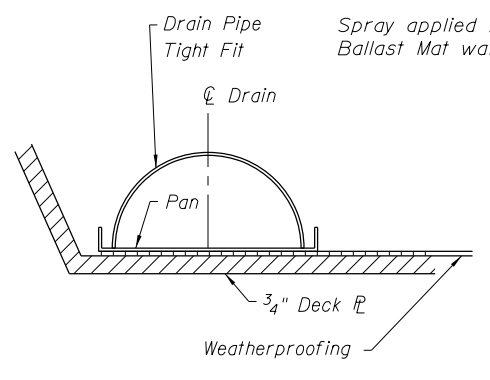
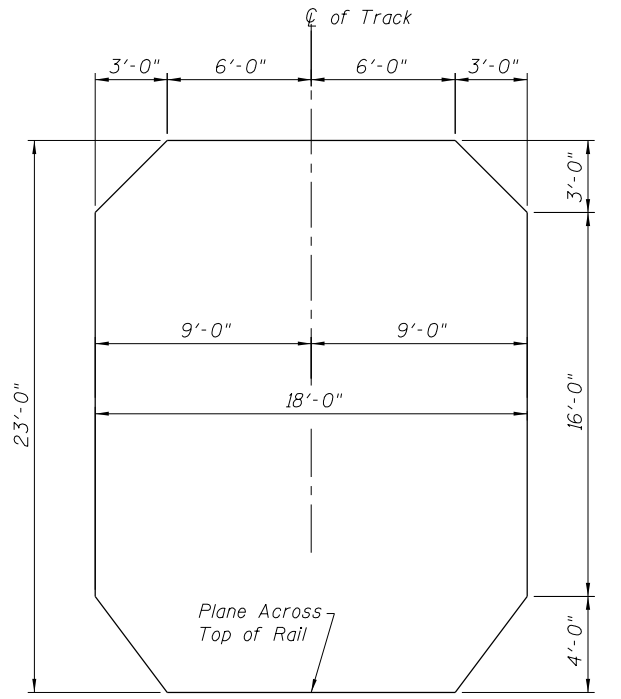
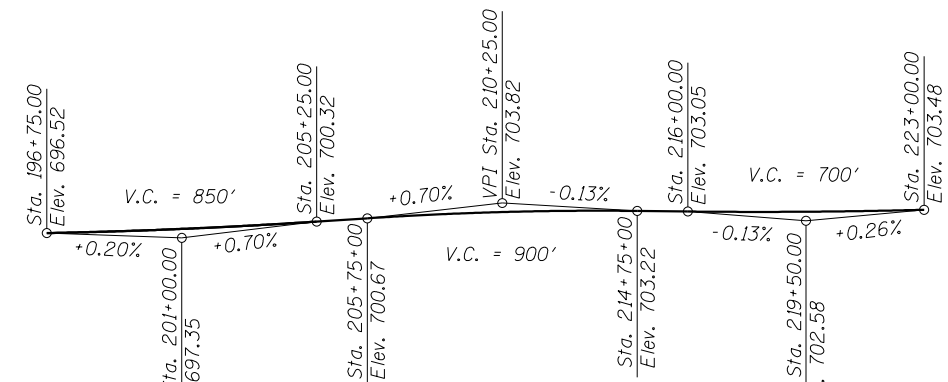
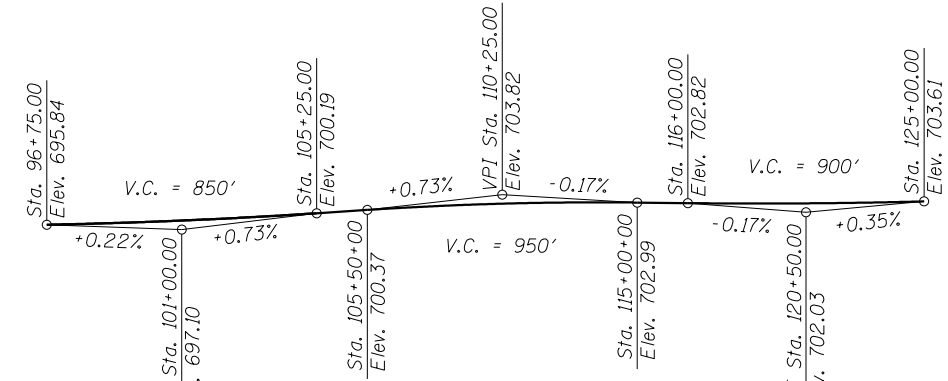
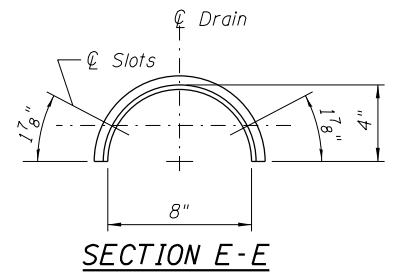
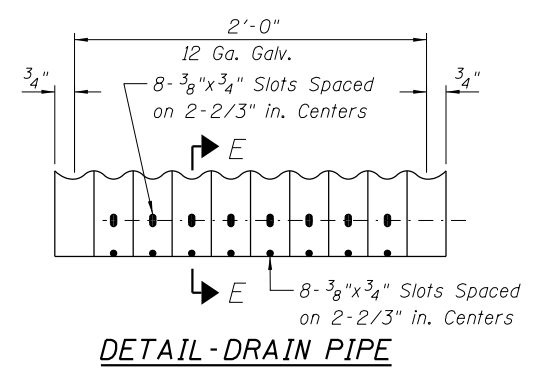
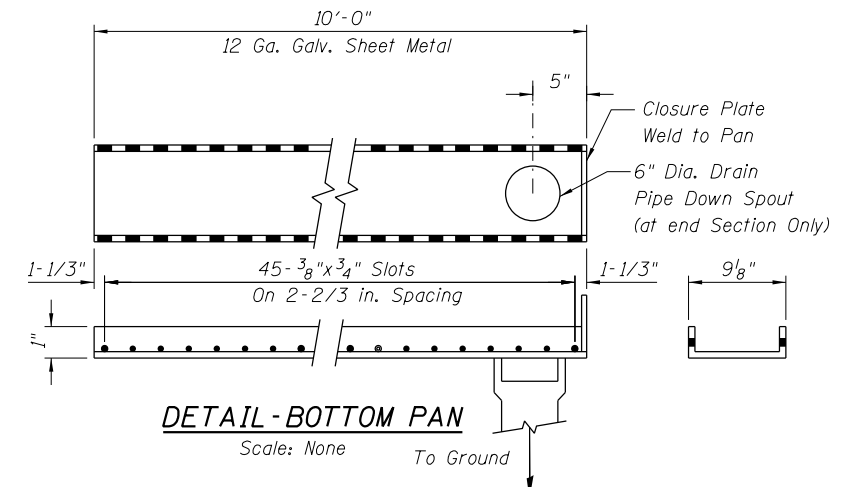
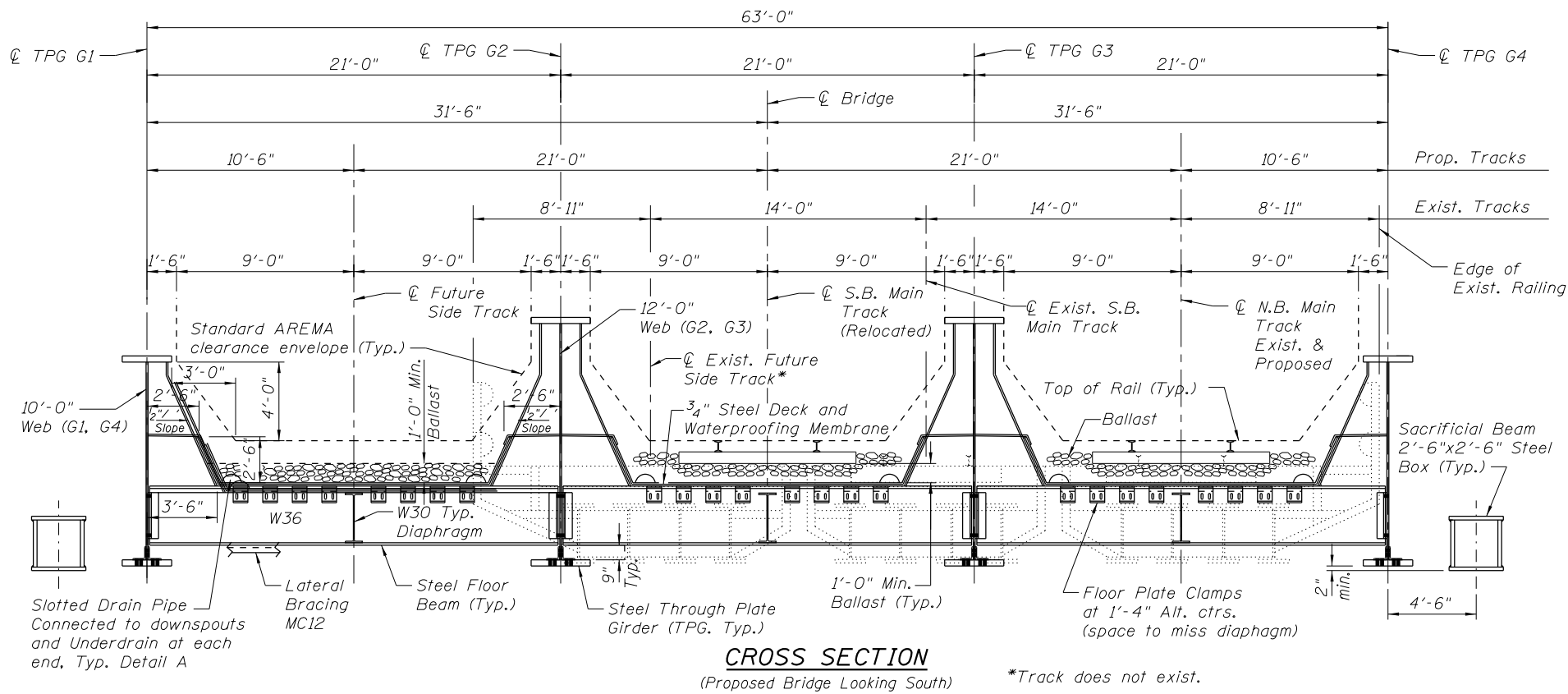
Base Acceleration Coefficient (AR) = 4.0%g (100yr.)
Site Coefficient (S) = 1.0
Damping Adjustment Factor (D) = 1.0
Seismic Response Coefficient = 10.0 (100yr.)



GENERAL PLAN
UPRR BRIDGE MP 37.75
MILWAUKEE SUBDIVISION
OVER IL. ROUTE 132 (GRAND AVE.)
F.A.P. 346 - SEC. 125X-N&J-SB-B
GURNEE, LAKE COUNTY, ILLINOIS
STATION 22+94.68EB
STRUCTURE NO. 049-0602

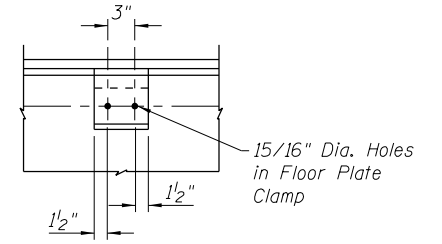
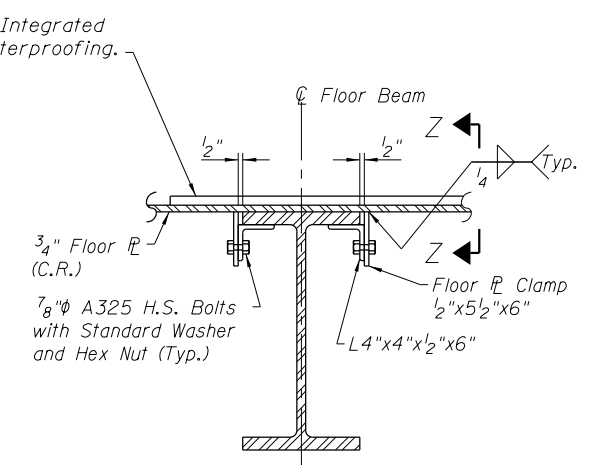
APPROVED
For Structural Adequacy Only
David H. Bilow
Engineer of Bridges & Structures
DATE: 6/21/16
LICENSE EXPIRES 11/30/16

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HOH 3366	PLOT SCALE : 2500.0000 1" = 110'	CHECKED - DNB	REVISED -			CONTRACT NO. 60K80				
15 East Johnson Street, Suite 100 Chicago, Illinois 60604 312-542-9372	PLOT DATE : 12/7/2016	DRAWN - R.VEJAR	REVISED -			ILLINOIS FED. AID PROJECT				
		CHECKED - BCS	REVISED -							



U.P.R.R.
BUILT 20-- BY
STATE OF ILLINOIS
F.A.P. 346 SEC 125X-N&J-SB-B
STATION 22+94.68 (EB)
LOADING COOPER E80 & ALT.
STRUCTURE NO. 049-0602

NAME PLATE
SEE Std. 515001



NOTE:
For curved track lateral clearance at each side of ϕ shall be increased $1\frac{1}{2}$ " per degree of curvature.

GENERAL NOTES

1. Workmanship and materials shall be in accordance with the GENERAL CONDITIONS AND SPECIFICATIONS Adopted by the Union Pacific Engineering Department and dated April 13, 2011 (UP General Conditions and Specification) except as modified by IDOT and the AREMA MANUAL for RAILWAY ENGINEERING dated 2010.
2. For any procedure, including demolition of existing structures, earth excavation, temporary shoring, sheeting installation, and erection that may affect rail operations, the Contractor shall submit for Engineer's review both work plans and shop drawings.
3. Dust shall be controlled by uniform applications of sprinkled water, only when directed by the Engineer.
4. Procurement of all permits, and payment thereof, shall be the Contractor's responsibility.
5. The Contractor shall be responsible for verifying all existing conditions and elevations at the site and must adapt this work to actual conditions in a manner approved by the Engineer. All construction debris shall be removed by the Contractor. It shall be Contractor's responsibility to verify the location of all utilities prior to starting construction. Contact J.U.I.E. at 800-892-0123.
6. No construction joints except those shown on the plans will be allowed unless approved by the Engineer.
7. Do not scale drawings for construction.
8. Dowel holes shall be drilled 1/4" larger than the diameter of the dowel.
9. Bearing seat surfaces shall be constructed or adjusted to the designated elevations within a tolerance of 1/8" inch. Adjustment shall be made either by grinding the surface or by shimming the bearing. Two 1/8" stainless steel adjusting shims of the dimensions of the bottom bearing plate shall be provided for each bearing in addition to all other plates or shims.
10. If cantilevered sheet piling is not feasible, additional members or other retention systems may be necessary. The Contractor shall submit a temporary soil retention system design including plan details and calculations for review and acceptance by the Engineer.
11. Backfill shall be placed behind the abutment after the superstructure has been poured and falsework removed. See Article 502.10 of the Standard Specifications.
12. All temporary shoring designs and details shall be signed and sealed by a structural engineer licensed in Illinois and submitted to the UPRR & IDOT for review.

SITE WORK

1. Site work shall be performed in accordance with DIVISION 2 SITE WORK SPECIFICATIONS of the UP General Conditions and Specification.

CAST-IN-PLACE CONCRETE

1. Cast-in-Place Concrete material, placement, and workmanship shall be in accordance with DIVISION 3 CONCRETE of the UP General Conditions and Specification and Chapter 8 of the AREMA Manual dated 2010. Minimum Compressive strength - $f_c' = 4000$ psi at 28 days.
2. Exposed surfaces shall be formed in a manner that will produce a smooth and uniform appearance without rubbing or plastering. Exposed edges of 90 degrees or less are to be chamfered 3/4"x3/4". Top surface to have a smooth finish, free of all float or trowel marks with the exception that a broom finish be used on all walkway surfaces.
3. Surface finish for bearing surfaces shall conform to American National Standard Institute Surface Roughness Requirements (A.N.S.I. B46.1 Surface Texture)
4. Cement shall be Type I, II or III Portland Cement per ASTM C150.
5. Fine aggregate shall be natural sand.
6. Admixtures, other than air entrainment, shall not be used without approval.
7. Membrane curing compound shall conform to ASTM C309 Type 2.
8. All construction joints shall be bonded.

REINFORCING STEEL

1. Reinforcing Steel shall be deformed, new billet bars per current ASTM A706 Specifications and meet Grade 60 requirements. Reinforcing Steel requiring field welding shall conform to ASTM A706 Specifications, Grade 60.
2. Fabrication of reinforcing steel shall be per Chapter 7 of the CRSI Manual of Standard Practice. Dimensions of bending details shall be out to out of bars.
3. Reinforcing steel is to be blocked to proper location and securely wired against displacement. Tack welding of reinforcing is prohibited. Minimum concrete cover not otherwise noted shall meet current AREMA requirements.

STRUCTURAL STEEL

1. Structural Steel material, fabrication, and erection shall be in accordance with Chapter 15 of the AREMA Manual dated 2010.
2. All structural steel shall conform to ASTM A709 Grade 50 unless otherwise noted on the plans or in the special provisions. Structural Steel is to be paid for at the Contract Lump Sum Price for "Furnishing and Erecting Structural Steel". Calculated Wt. of A709 Grade 50 steel is 1,927,482.0 Lbs, calculated Wt. of A709 Grade 50W Corrosion Resistant (c.r.) Steel is 54,163.0 Lbs, calculated Wt. of A325 Bolts is 31,739.0 Lbs, Total Weight is 2,013,384.0 Lbs.
3. The weights of structural steel, floor plate, sacrificial beams, nuts and high strength bolts are included in the weight of structural steel.
4. All bolted connections shall be made with galvanized high strength bolts conforming to ASTM A325. Bolts shall be 7/8" diameter unless otherwise noted. Holes shall be 1/6" larger than bolt size unless otherwise noted. Holes for shop fasteners shall be subpunched or subdrilled and reamed through a template in accordance with AREMA specifications and as specified in special provisions.
5. All welding, electrodes, flux, procedures, welder qualification, inspection, and testing must be in accordance with Article 505.04 (q), Welding of the IDOT Standard Specifications for Fracture Critical Members, and AREMA pg. 15-1-10 welding requirements.
6. Electro-slag, Electro-gas and FCAW Welding will not be permitted.
7. Structural steel for bridge must be fabricated by a fabricator certified under the American Institute of Steel Construction Quality Certification Program, Category III, Major Steel Bridges including Fracture Critical members in accordance with AREMA Specifications.
8. All tee and corner groove weld joints shall be ultrasonically inspected. All fillet welds shall be inspected by Magnetic Particle Procedure.
9. Steel for the Upper Floor Plate must have a raised pattern conforming to ASTM A786.
10. Steel deck shall conform to A709 specifications, Grade 50.
11. Anchor rods/ bolts shall conform to ASTM F1554 Grade 36 specifications.
12. End welded studs shall be C1015, C1017 or C1020 cold drawn steel, which conforms to ASTM A108 specification.
13. Cover plate, closure plates and anchor rods/bolts shall be galvanized after fabrication in accordance with ASTM A123, thickness Grade 100 if shown on plans.
14. Anchor rod washers shall be zinc coated in accordance with ASTM A153 specification if shown on plans.
15. No field welding is permitted except as specified in the contract documents.
16. All cross frames or diaphragms shall be installed as steel is erected and secured with erection pins and bolts except as otherwise noted. Individual cross frames or diaphragms at supports may be temporarily disconnected to install bearing anchor rods.
17. The webs and tension flanges of the through-plate girders are designated as "Fracture Critical Members" and shall conform to the fracture control plan for fracture critical members of the AREMA Specifications for zone 2. These members are noted on plans as (FCM). For all FCM members NTR shall be applied.
18. The main load carrying components subjected to tensile stress, other than fracture critical members, shall conform to supplemental requirements for notch toughness, Zone 2. These components are the wide flange beams and other components of main girders and are noted on the plans (NTR).
19. The Organic Zinc Rich Primer / Epoxy / Urethane Paint System shall be used for painting of new structural steel except where otherwise noted. The entire system shall be shop applied, with the exception that masked off connection surfaces, field installed fasteners and damaged areas shall be touched up in the field. The color of the final finish coat for all steel surfaces shall be Gray, Munsell No. 5B 7/1. See IDOT Standard Specification for Road and Bridge Construction. Painting of the structural steel will not be paid separately, but is included in the pay item "Furnish and Erect Structural Steel."

WATER PROOFING

1. Water proofing materials and installation shall be cold liquid applied Elastomeric Membrane with Integrated Ballast Mat in accordance with the AREMA MANUAL dated 2010 Chapter 8 Part 29 and special provision #7102.

FOUNDATIONS

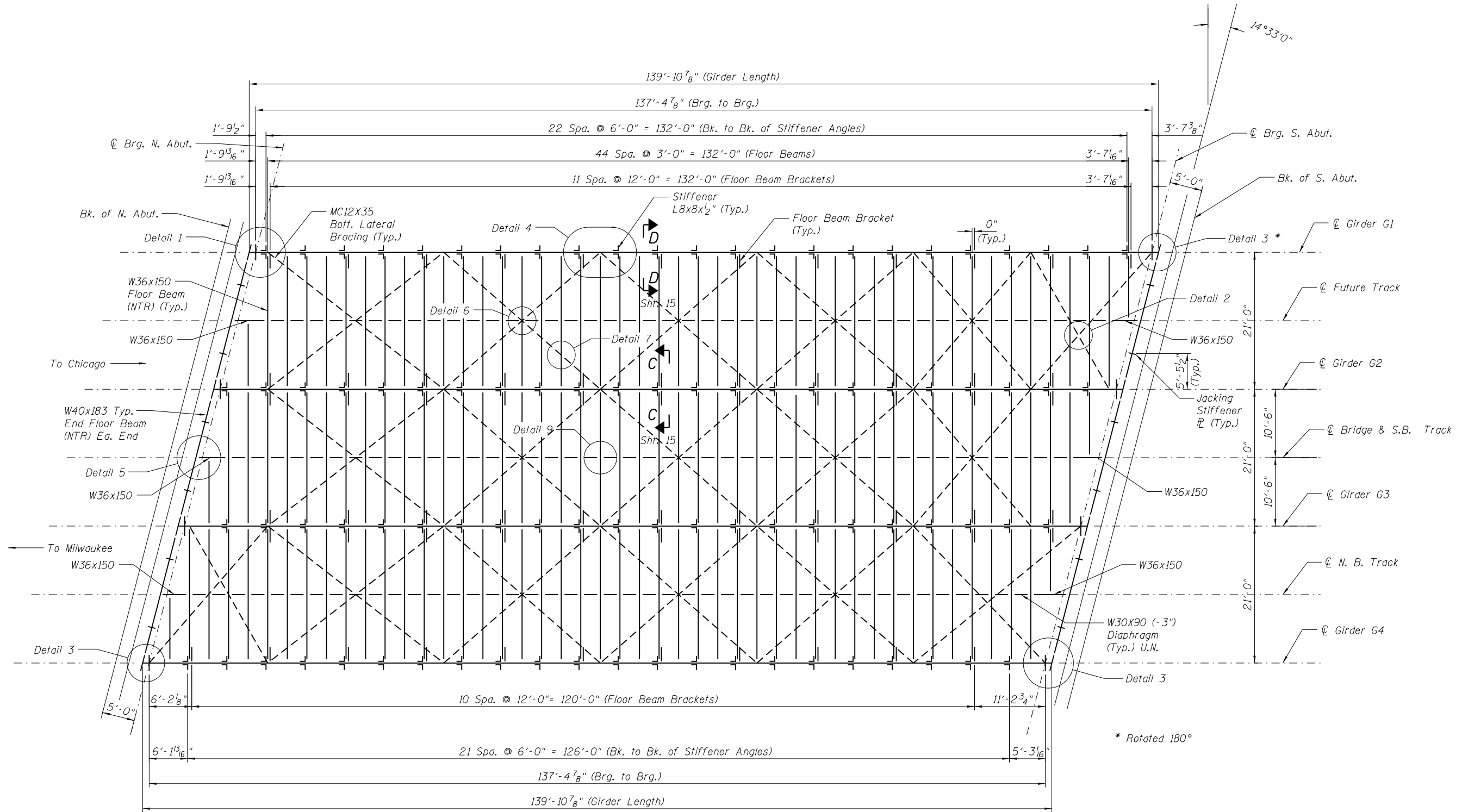
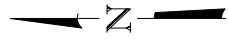
1. Metal shell piles shall be installed per IDOT Standard Specifications for Road and Bridge Construction Section 512. Use 14"φ with 0.312" walls. Estimated pile length 40 feet. $Q_u = 543$ Kips, $Q_a = 181$ Kips. Driving shoes shall be used.
2. Drilled Shafts for Wing Walls shall be installed per IDOT Standard Specification for Road and Bridge Construction Section 516.
3. Visually monitor for the presence of natural gas during pile driving. (See SGR pg 8.

TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
Structure Excavation	Cu. Yd.	-	2,305	2,305
Removal of Existing Structures No. 1	Each	-	1	1
Granular Backfill for Structures	Cu. Yd.	-	1,200	1,200
Concrete Structures	Cu. Yd.	-	1,429	1,429
Furnishing and Erecting Structural Steel	L Sum	0.82	-	0.82
Reinforcement Bars	Pound	-	15,796	15,796
Reinforcement Bars, Epoxy Coated	Pound	-	125,070	125,070
Drilled Shaft in Soil	Cu. Yd.	-	49	49
Furnishing Metal Shell Piles-14"x.312"	Foot	-	5,760	5,760
Driving Piles	Foot	-	5,760	5,760
Test Pile Metal Shells	Each	-	2	2
Membrane Waterproofing (Special)	Sq. Ft.	8,530	-	8,530
Geocomposite Wall Drain	Sq. Yd.	-	460	460
Pipe Underdrain for Structures 6"	Foot	-	220	220
Steel Bearing Assembly	Each	-	8	8
Drainage System	L Sum	0.7	-	0.7
Chain Link Fence, 42" Attached to Structure (Special)	Foot	-	93	93
Name Plates	Each	-	1	1
Deck Protection Ballast	Cu. Yd.	-	115	115
Concrete Sealer	S.F.	-	6610	6610
Pile Shoes	Each	-	144	144
Anchor Bolt 1/4"	Each	-	8	8
Anchor Bolt 1/2"	Each	-	8	8
Anchor Bolt 2"	Each	-	8	8
Anchor Bolt 2 1/2"	Each	-	8	8
Asphalt Waterproofing	Sq. Yd.	-	366	366
Rubbed Finish	Sq. Ft.	-	3,740	3,740
Structure Marker Sign-up	Each	-	4	4

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3. General Notes and Bill of Material
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8. Girder G2 Details
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42. Soil Boring Logs - 7
43. Soil Boring Logs - 8



STEEL FRAMING PLAN

Scale: 1/8" = 1'-0"

NOTE:
For Details see Sheets 11 & 12.

FILE NAME = ...04906202-60K80-004.dgn
HOH HARRY O. HEFTER ASSOCIATES, INC.
 DESIGN AND CONSULTING ENGINEERS
 3366 55 East Jackson Blvd., Suite 600
 Chicago, Illinois 60604
 312/346-8131

USER NAME = aefitzpatrick
 DESIGNED - MMH
 CHECKED - DNB
 DRAWN - R.VEJAR
 CHECKED - BCS
 PLOT SCALE = 16:0.0000 1/8" / 1"
 PLOT DATE = 10/7/2016

DESIGNED - MMH
 CHECKED - DNB
 DRAWN - R.VEJAR
 CHECKED - BCS

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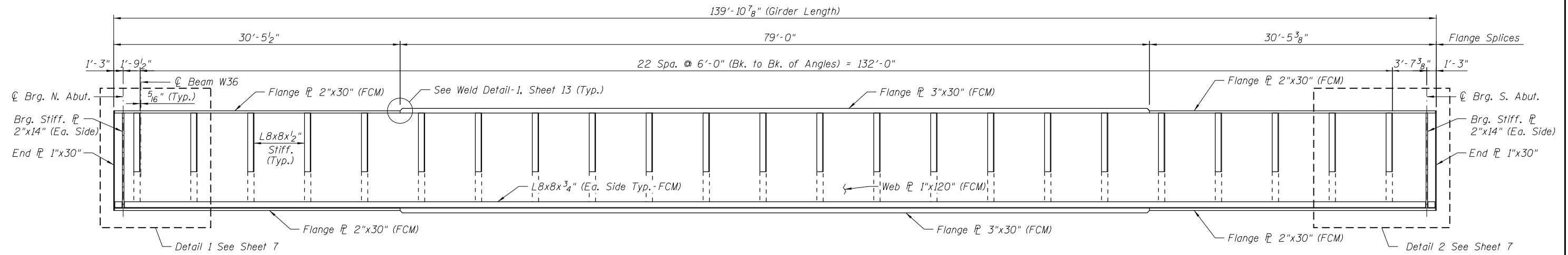
**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

**STEEL FRAMING PLAN
 STRUCTURE NO. 049-0602**

SHEET NO. 4 OF 43 SHEETS

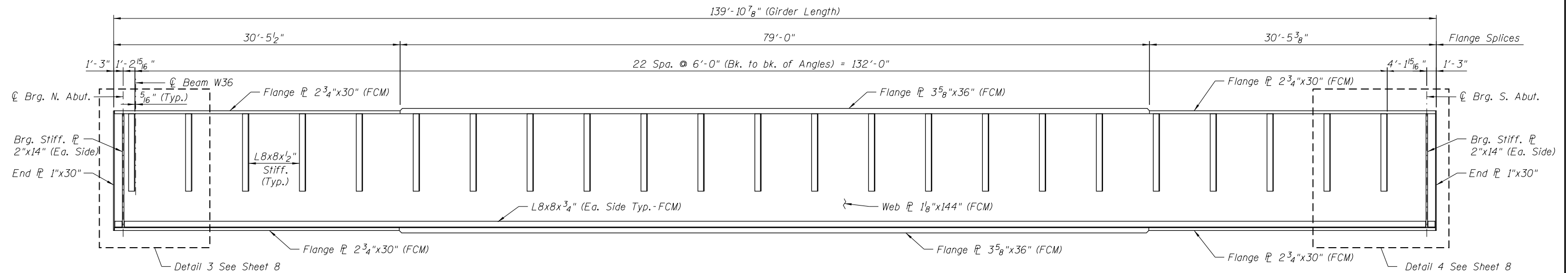
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346	125X-N&J-SB-B	LAKE	361	219
CONTRACT NO. 60K80				

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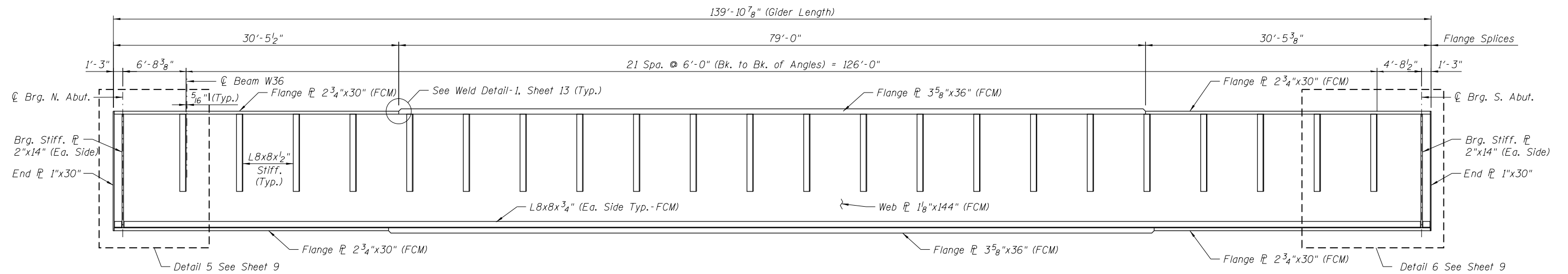
GIRDER G1 ELEVATION

Scale: $\frac{3}{16}'' = 1'-0''$
(Looking East)



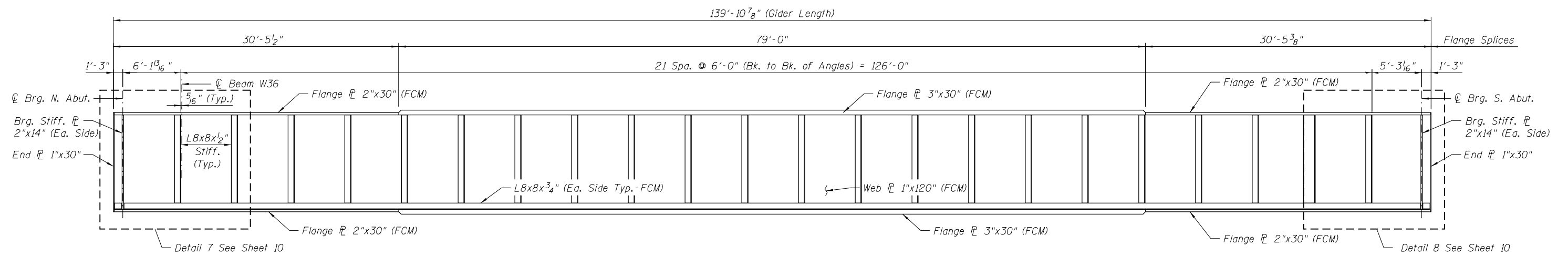
GIRDER G2 ELEVATION

Scale: $\frac{3}{16}'' = 1'-0''$
(Looking East)



GIRDER G3 ELEVATION

Scale: 3/16" = 1'-0"
(Looking East)



GIRDER G4 ELEVATION

Scale: 3/16" = 1'-0"
(Looking East)

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HOH HARRY O. HEFTER ASSOCIATES, INC.
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 3366 55 East Jackson Blvd., Suite 600
 Chicago, Illinois 60604
 312/346-8131

USER NAME = aefitzpatrick
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 PLOT DATE = 10/7/2016

DESIGNED - MMH
 CHECKED - DNB
 DRAWN - R.VEJAR
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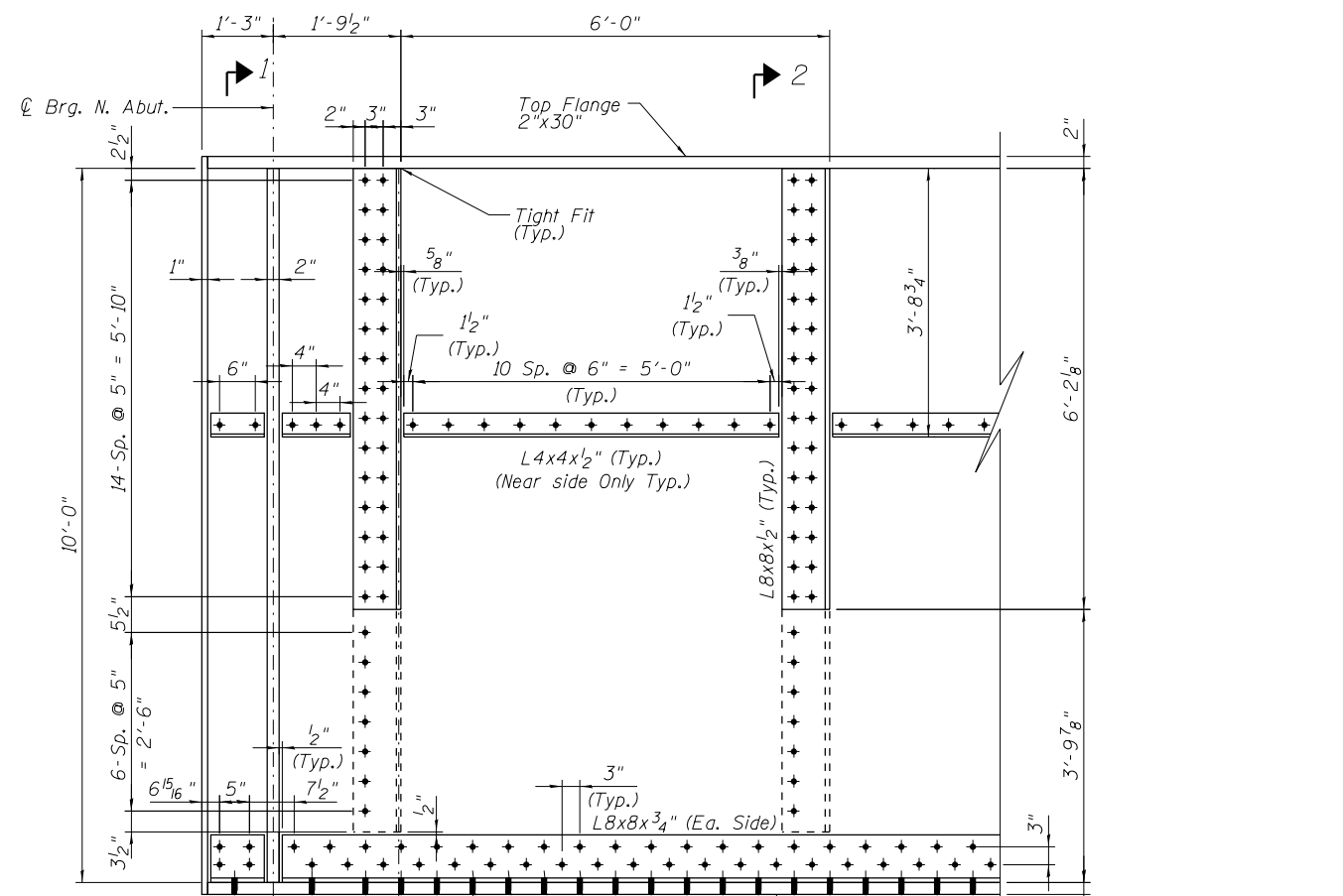
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**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

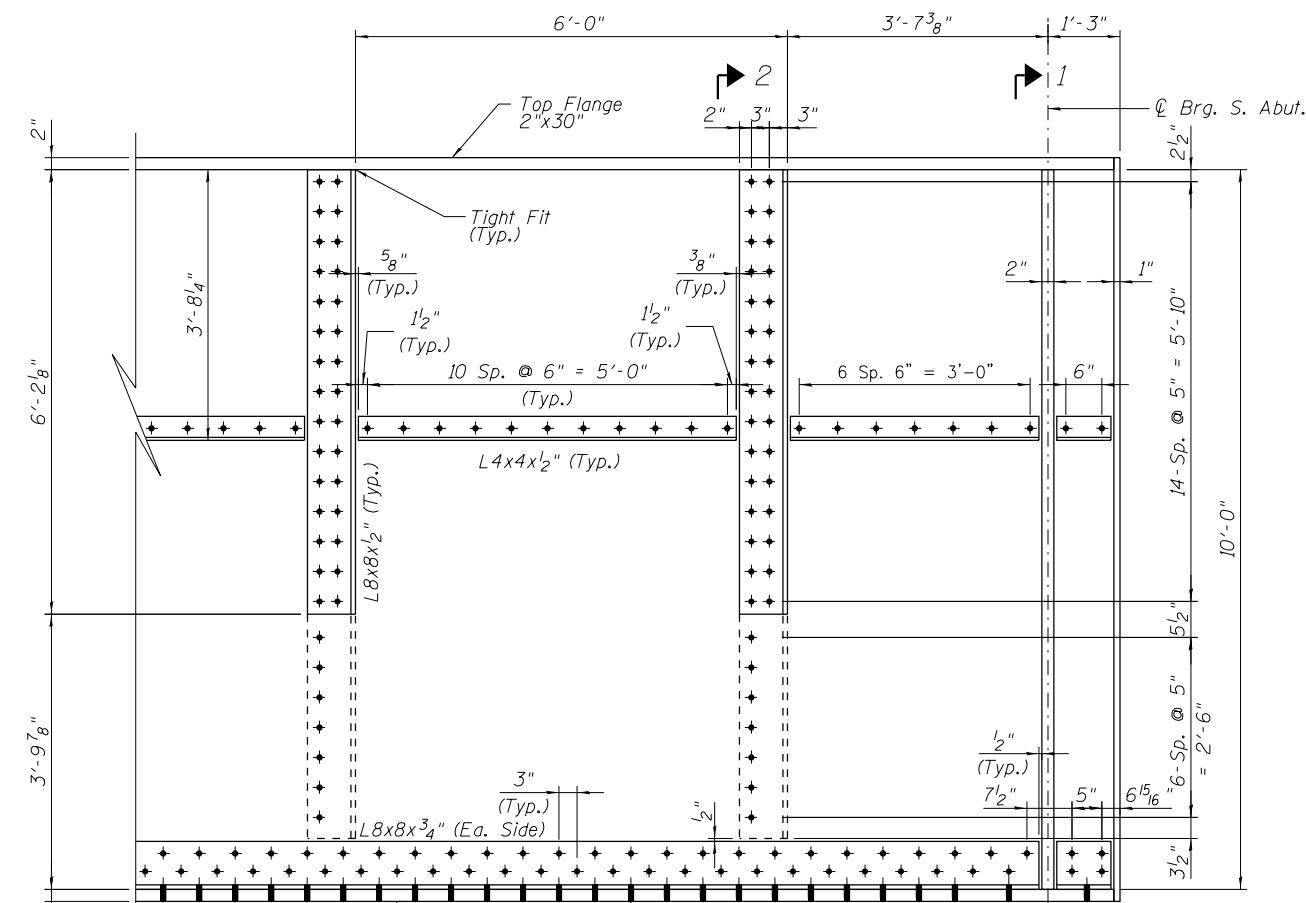
**GIRDERS G3 & G4
 STRUCTURE NO. 049-0602**

SHEET NO. 6 OF 43 SHEETS

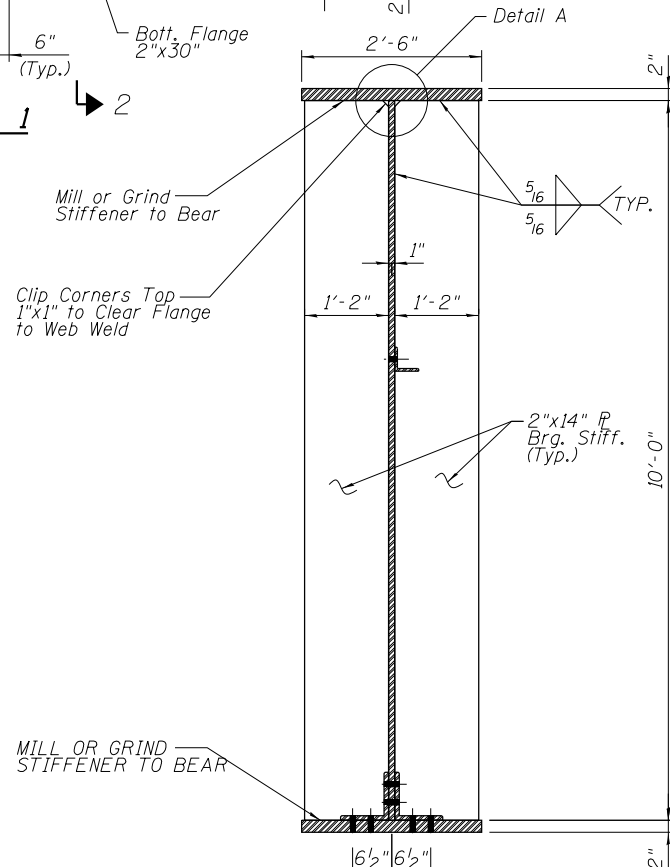
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CONTRACT NO. 60K80				
ILLINOIS FED. AID PROJECT				



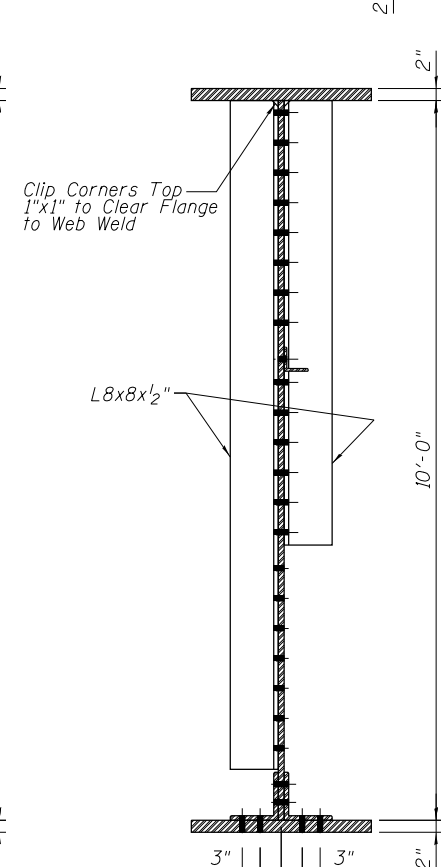
GIRDER G1 - DETAIL 1
Scale: 3/4" = 1'-0"



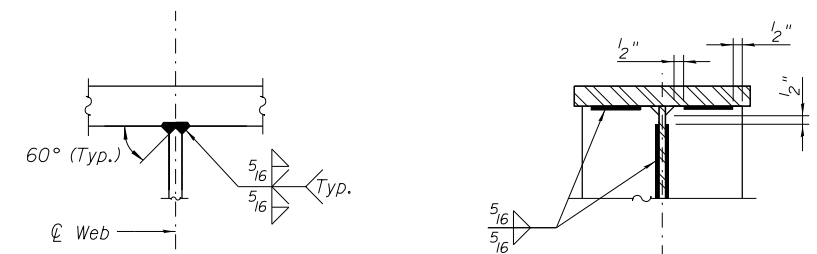
GIRDER G1 - DETAIL 2
Scale: 3/4" = 1'-0"



SECTION 1-1
Scale: 3/4" = 1'-0"

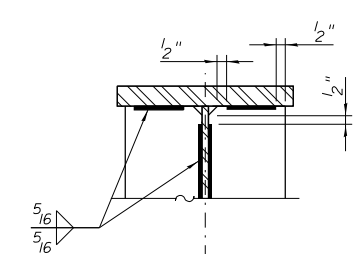


SECTION 2-2
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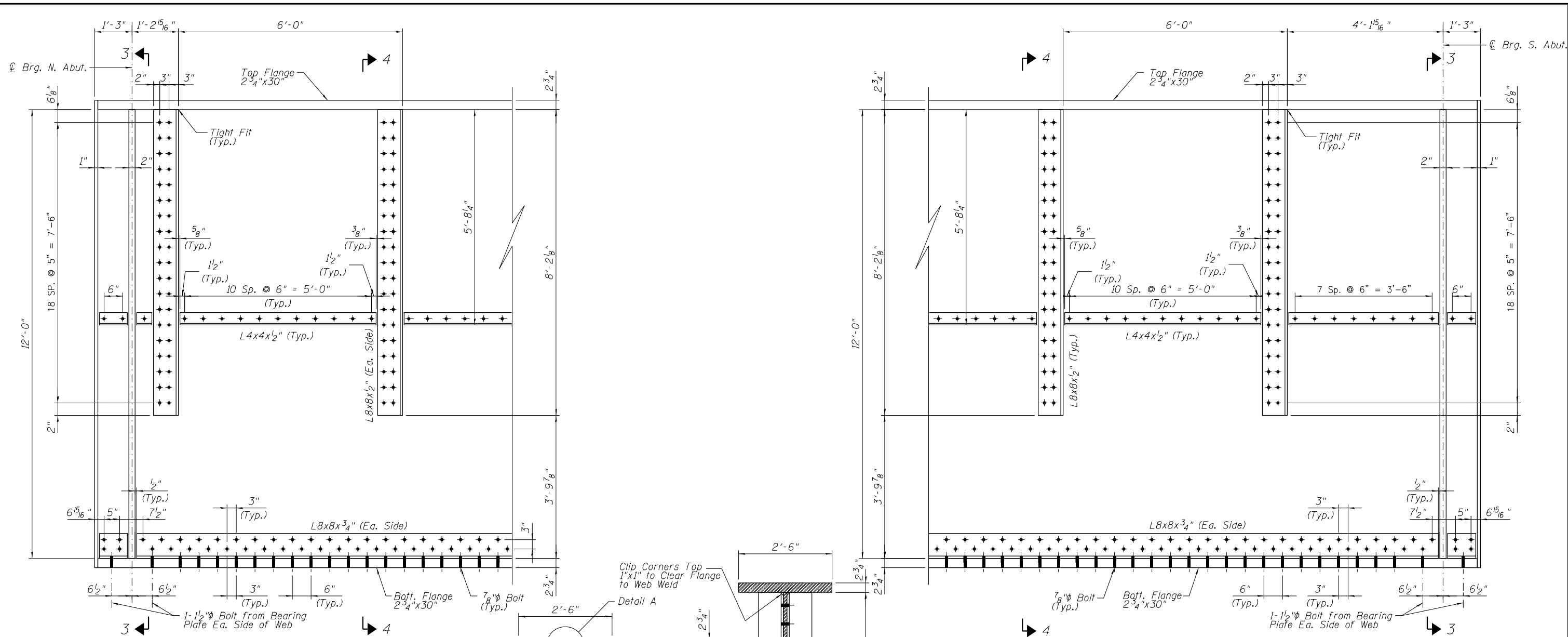


WEB TO FLANGE CONNECTION DETAIL - TYP.

NOTE:
Full penetration of joint required. Weld to be symmetrical about \bar{C} web. Excessive unbalance shall be cause for rejection. See Specifications for weld qualification. Gouge root to sound metal before welding second side.

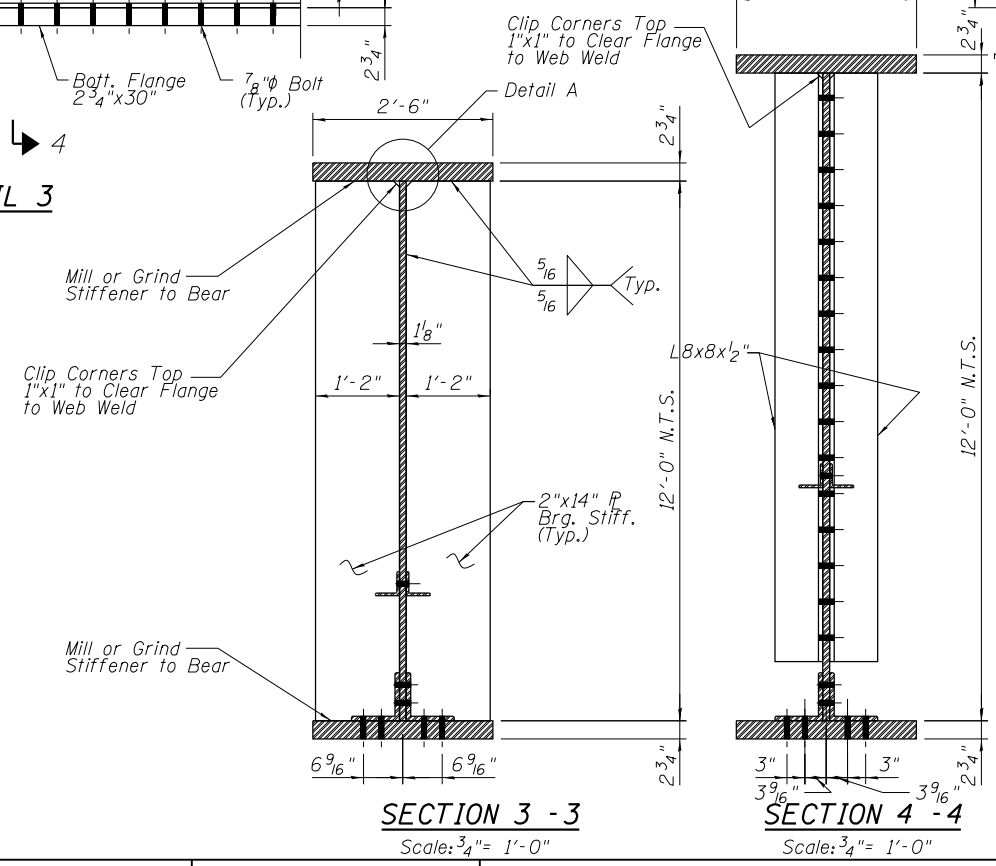


DETAIL A



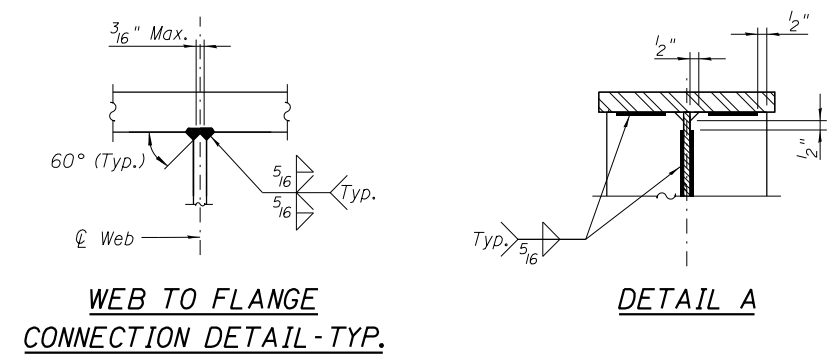
GIRDER G2 - DETAIL 3
Scale: 3/4" = 1'-0"

GIRDER G2 - DETAIL 4
Scale: 3/4" = 1'-0"



SECTION 3 - 3
Scale: 3/4" = 1'-0"

SECTION 4 - 4
Scale: 3/4" = 1'-0"



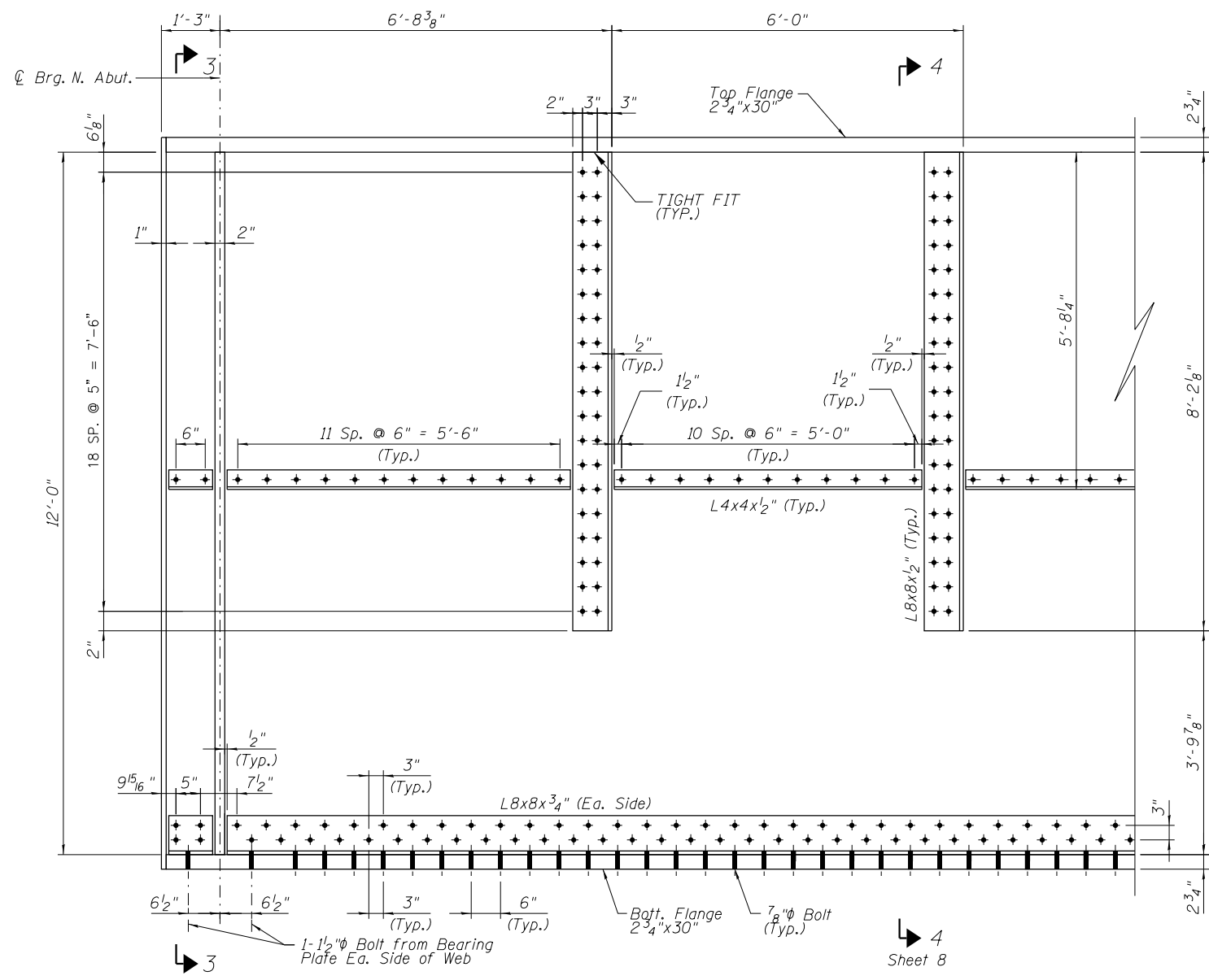
WEB TO FLANGE CONNECTION DETAIL - TYP.

DETAIL A

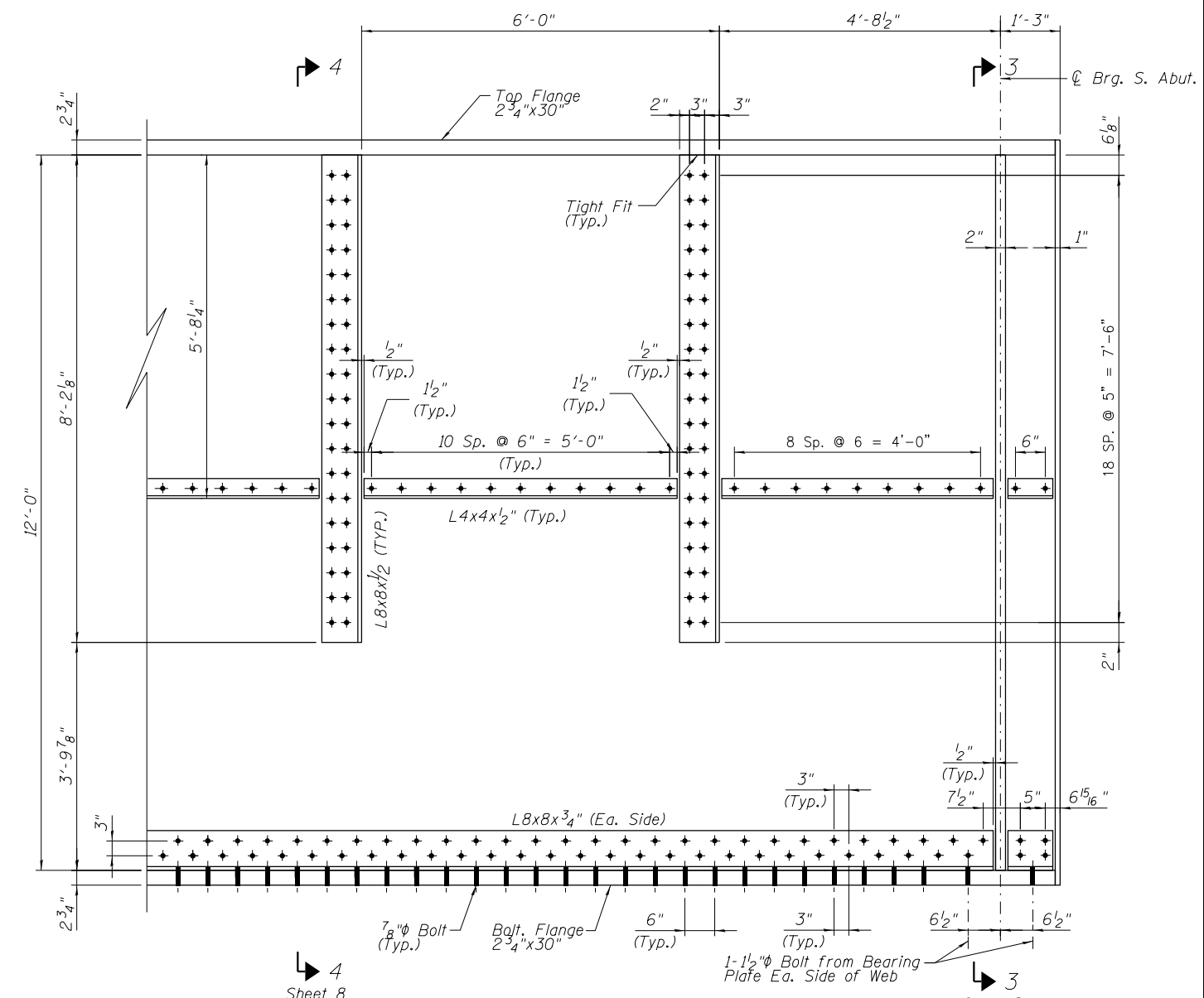
NOTE:
Full penetration of joint required. Weld to be symmetrical about \bar{C} web. Excessive unbalance shall be cause for rejection. See Specifications for weld qualification. Gauge root to sound metal before welding second side.

NOTE:
1. For Floor Beam and Bracket Connection Details. See Sheets 11, 12 & 13.

FILE NAME - ...04906202-60K00-008.dgn HOH HARRY O. HEFTER ASSOCIATES, INC. DESIGN AND CONSULTING ENGINEERS 3366 55 East Jackson Blvd., Suite 600 Chicago, Illinois 60604 312/346-8131	USER NAME = aefitzpatrick DESIGNED - MMH CHECKED - DNB DRAWN - R.VEJAR CHECKED - BCS PLOT SCALE = 2:810 '1' / in. PLOT DATE = 10/7/2016	DESIGNED - MMH CHECKED - DNB DRAWN - R.VEJAR CHECKED - BCS REVISED - REVISED - REVISED - REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	GIRDER G2 DETAILS STRUCTURE NO. 049-0602 SHEET NO. 8 OF 43 SHEETS	F.A.P. RT. SECTION COUNTY TOTAL SHEETS SHEET NO. 346 125X-N&J-SB-B LAKE 361 223 CONTRACT NO. 60K80 ILLINOIS FED. AID PROJECT
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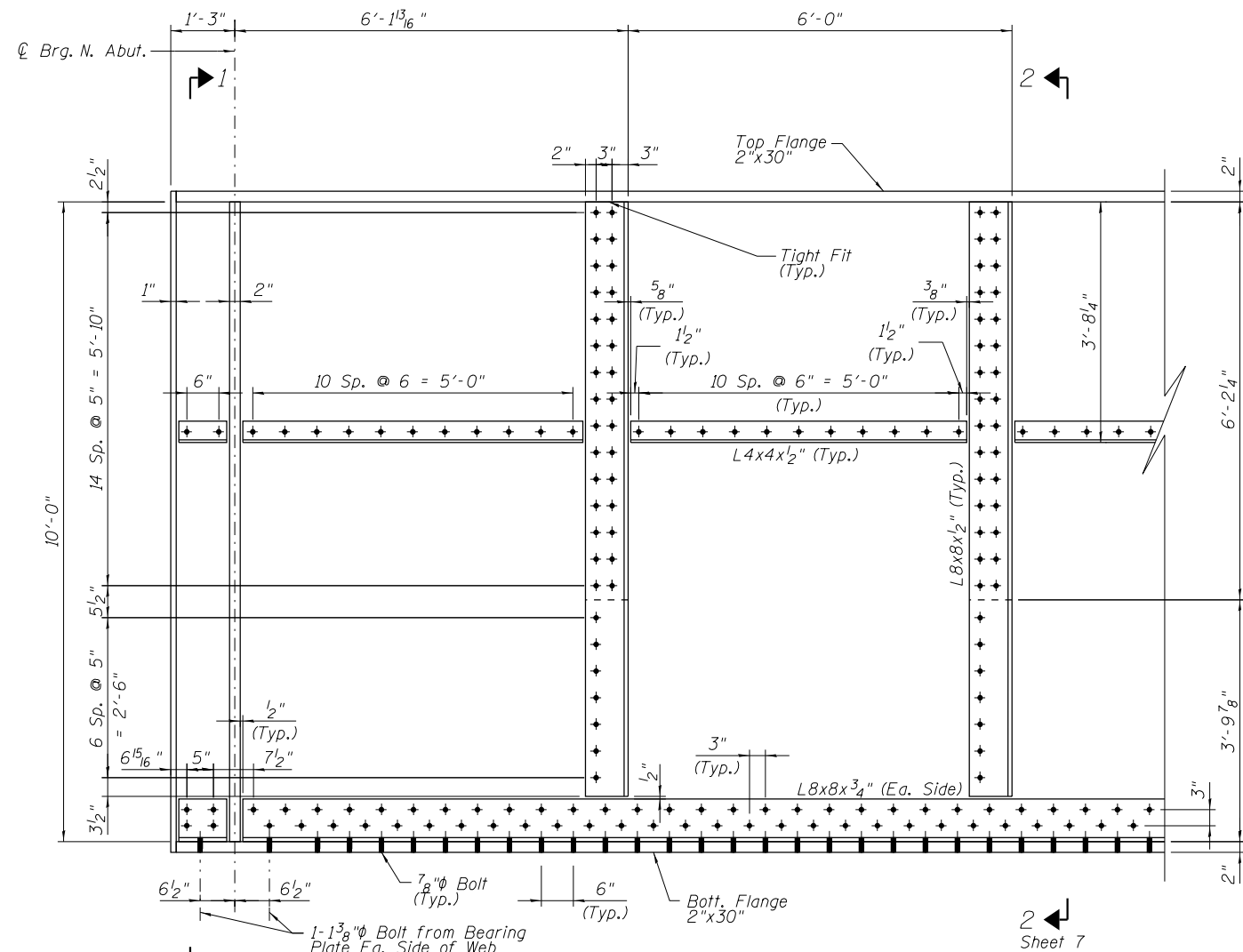
GIRDER G3 - DETAIL 5
Scale: 3/4" = 1'-0"



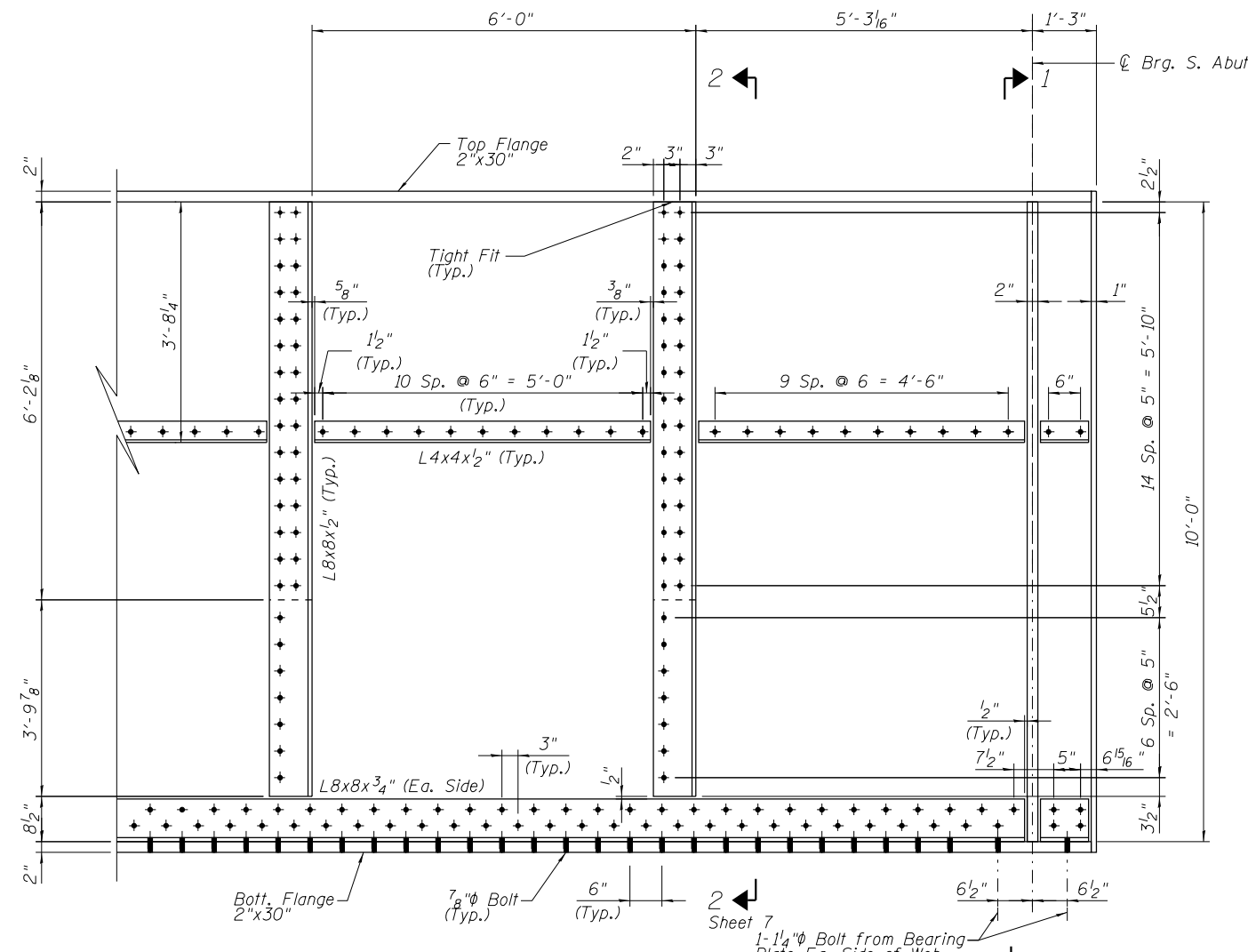
GIRDER G3 - DETAIL 6
Scale: 3/4" = 1'-0"

NOTE:
1. For Floor Beam and Bracket Connection Details, See Sheets 11, 12 & 13.

HOH HARRY O. HEFTER ASSOCIATES, INC. DESIGN AND CONSULTING ENGINEERS 55 East Jackson Blvd., Suite 600 Chicago, Illinois 60604 312/346-8931	USER NAME = aefitzpatrick PLOT SCALE = 2/8" = 1' / in. PLOT DATE = 10/7/2016	DESIGNED - MMH CHECKED - DNB DRAWN - R.VEJAR CHECKED - BCS	REVISED - REVISED - REVISED - REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	GIRDER G3 DETAILS STRUCTURE NO. 049-0602 SHEET NO. 9 OF 43 SHEETS	F.A.P. RTE. = 346 SECTION = 125X-N&J-SB-B COUNTY = LAKE TOTAL SHEETS = 361 SHEET NO. = 224 CONTRACT NO. 60K80	ILLINOIS FED. AID PROJECT



GIRDER G4 - DETAIL 7
Scale: 3/4" = 1'-0"



GIRDER G4 - DETAIL 8
Scale: 3/4" = 1'-0"

NOTE:
1. For Floor Beam and Bracket Connection Details, See Sheets 11, 12 & 13.

FILE NAME = ...04906202-60K80-010.dgn
HOH HARRY O. HEFTER ASSOCIATES, INC.
 DESIGN AND CONSULTING ENGINEERS
 3366 55 East Jackson Blvd., Suite 600
 Chicago, Illinois 60604
 312/346-8131

USER NAME = aefitzpatrick
 PLOT SCALE = 2:810 '1' / in.
 PLOT DATE = 10/7/2016

DESIGNED - MMH
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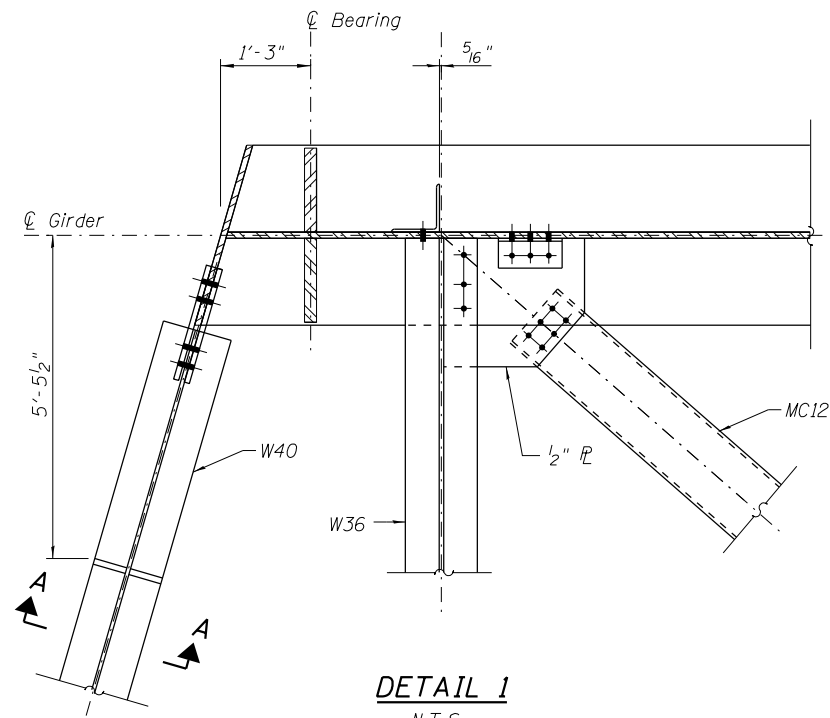
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**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

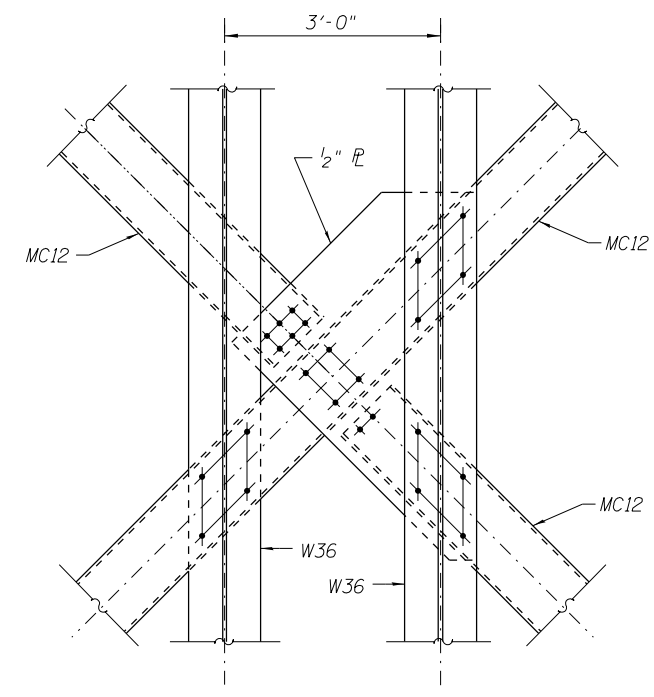
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 STRUCTURE NO. 049-0602**

SHEET NO. 10 OF 43 SHEETS

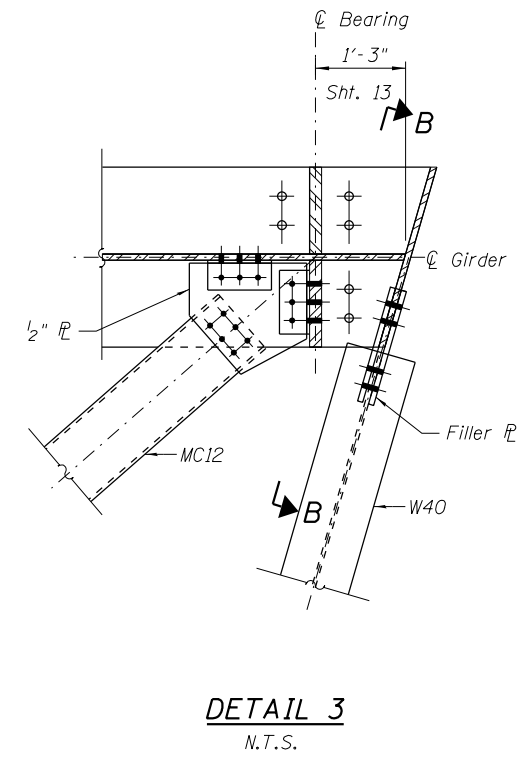
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CONTRACT NO. 60K80				
ILLINOIS FED. AID PROJECT				



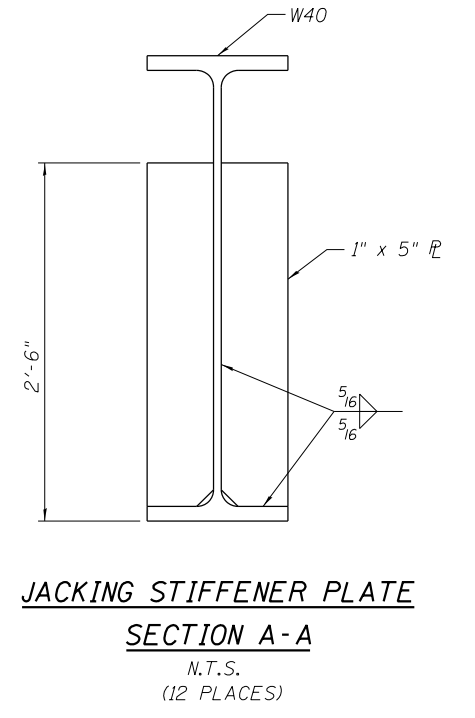
DETAIL 1
N.T.S.



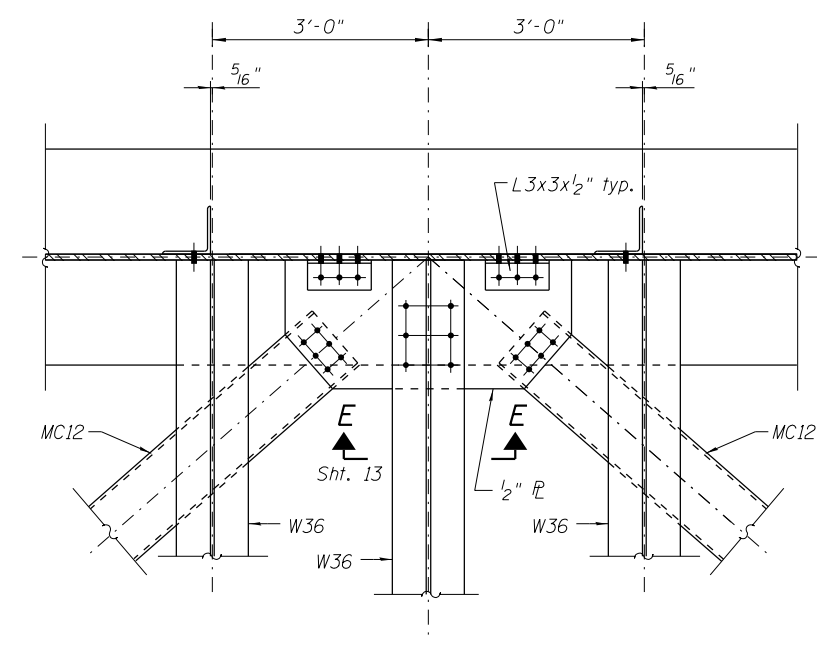
DETAIL 2
N.T.S.



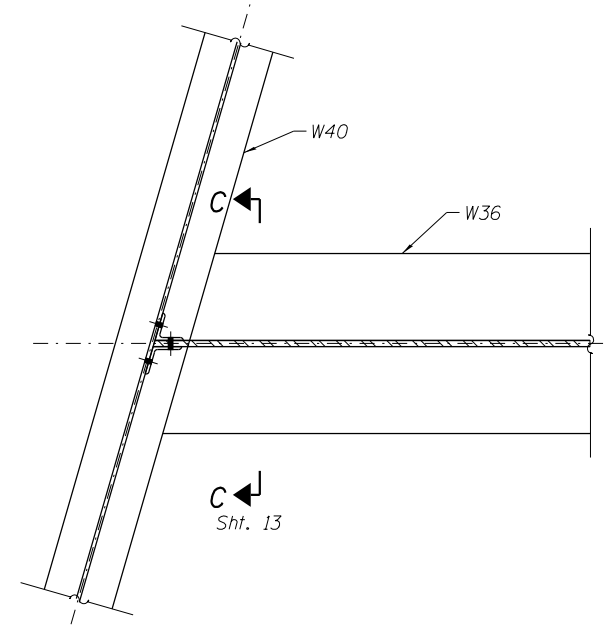
DETAIL 3
N.T.S.



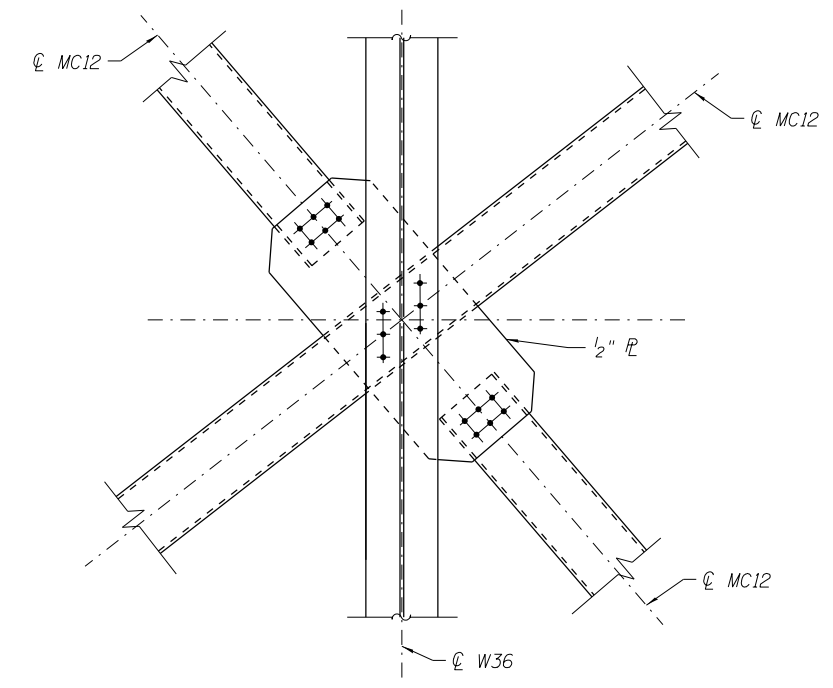
JACKING STIFFENER PLATE
SECTION A-A
N.T.S.
(12 PLACES)



DETAIL 4
N.T.S.



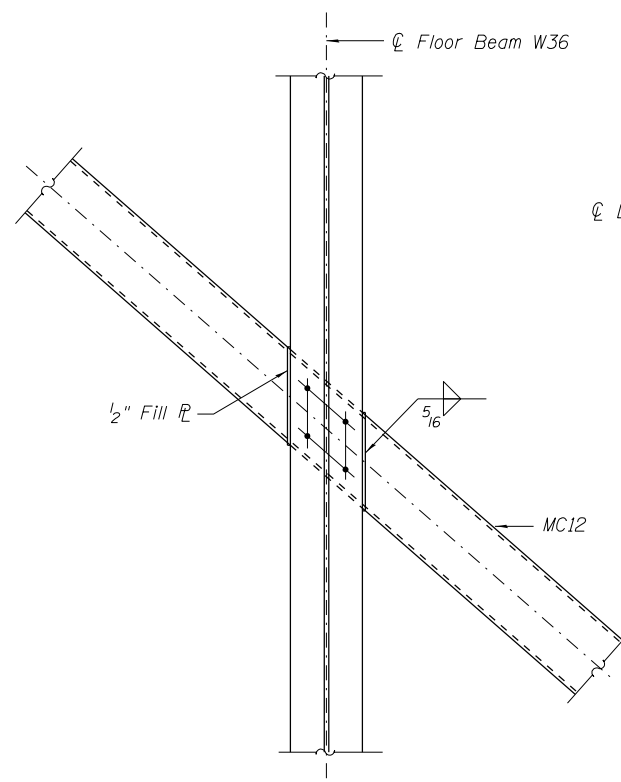
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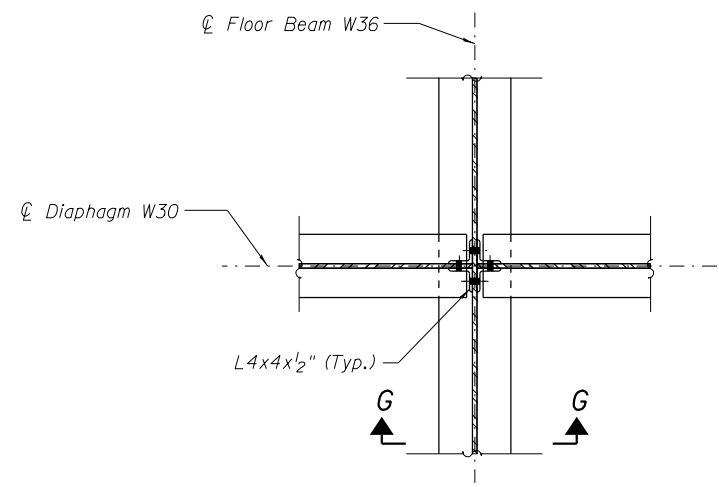
DETAIL 6
N.T.S.

NOTES:
1. For Sections see Sheet 13.
2. For Connection Plates 1 and 2 see Sheet 13.
3. For Detail locations see Sheet 4.

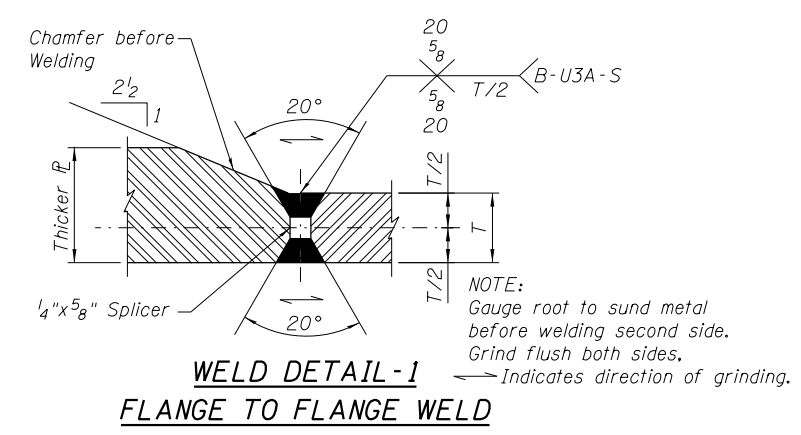
HOH HARRY O. HEFTER ASSOCIATES, INC. DESIGN AND CONSULTING ENGINEERS 55 East Jackson Blvd., Suite 600 Chicago, Illinois 60604 312/346-8131	USER NAME = aefitzpatrick DESIGNED - MMH CHECKED - DNB DRAWN - R.VEJAR CHECKED - BCS	REVISED - REVISED - REVISED - REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	STEEL DETAILS - 1 STRUCTURE NO. 049-0602	F.A.P. RTE. 346 SECTION 125X-N&J-SB-B COUNTY LAKE TOTAL SHEETS 361 SHEET NO. 226 CONTRACT NO. 60K80
	PLOT SCALE = 8:0.0000 '1' / in. PLOT DATE = 10/7/2016	SHEET NO. 11 OF 43 SHEETS ILLINOIS FED. AID PROJECT			



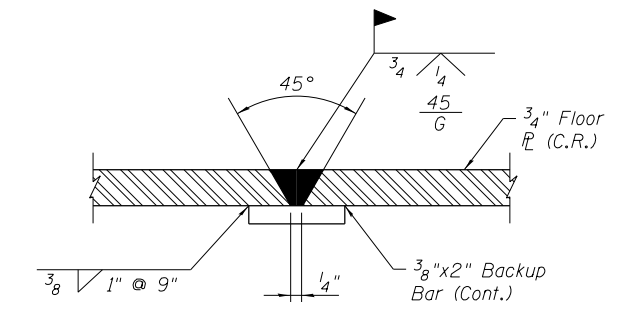
DETAIL 7
N.T.S.



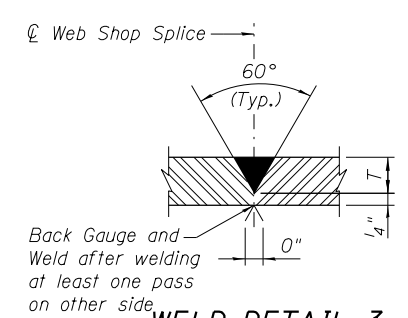
DETAIL 9
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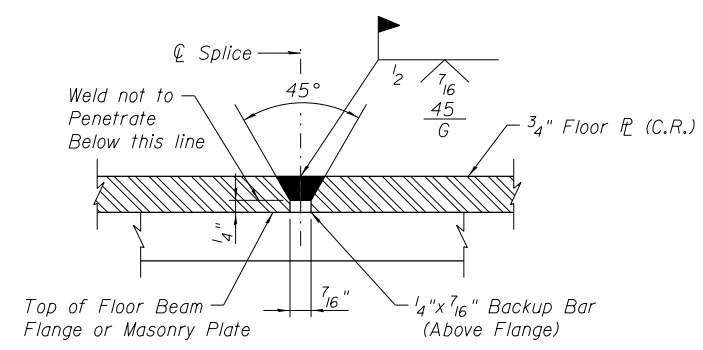
WELD DETAIL - 1
FLANGE TO FLANGE WELD



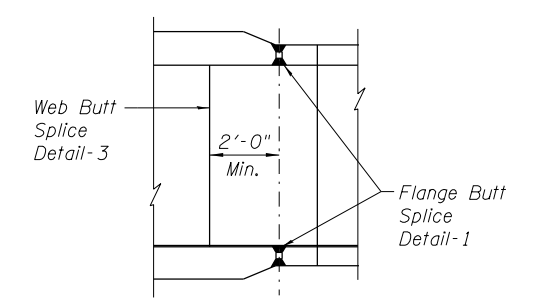
WELD DETAIL - 2
FLOOR PLATE FIELD SPLICE
BALLAST STOP PLATE SPLICE SIMILAR



WELD DETAIL - 3
WEB TO WEB BUTT WELD



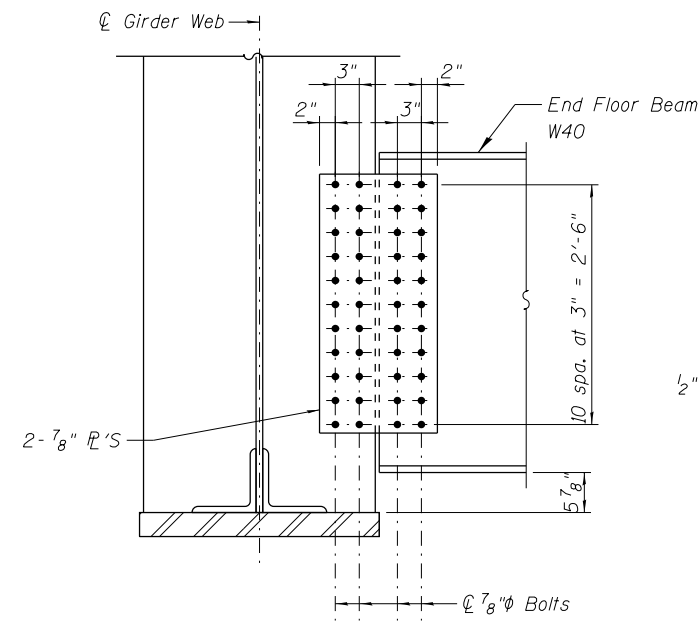
WELD DETAIL - 4
FIELD SPLICE OVER BEAM FLANGE



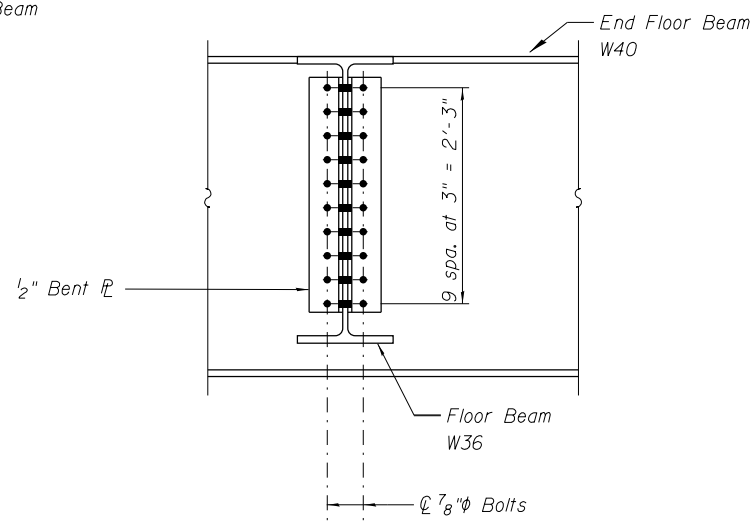
GIRDER SHOP SPLICE

NOTES:
1. For Sections see Sheet 13.
2. For Detail Locations see Sheet 4.
3. Girders shall be shop fabricated.
Field splice will not be allowed.

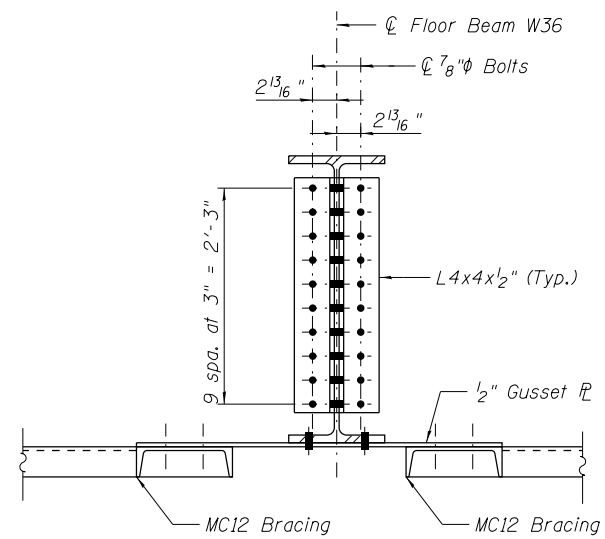
HOH HARRY O. HEFTER ASSOCIATES, INC. DESIGN AND CONSULTING ENGINEERS 55 East Jackson Blvd., Suite 600 Chicago, Illinois 60604 312/346-8931	USER NAME = aofitzpatrick PLOT SCALE = 8:0.0000 '1' / in. PLOT DATE = 10/7/2016	DESIGNED - MMH CHECKED - DNB DRAWN - R.VEJAR CHECKED - BCS	REVISED - REVISED - REVISED - REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	STEEL DETAILS - 2 STRUCTURE NO. 049-0602 SHEET NO. 12 OF 43 SHEETS	F.A.P. RTE. 346 SECTION 125X-N&J-SB-B COUNTY LAKE TOTAL SHEETS 361 SHEET NO. 227 CONTRACT NO. 60K80	ILLINOIS FED. AID PROJECT



SECTION B-B
Scale: 1"=1'-0"



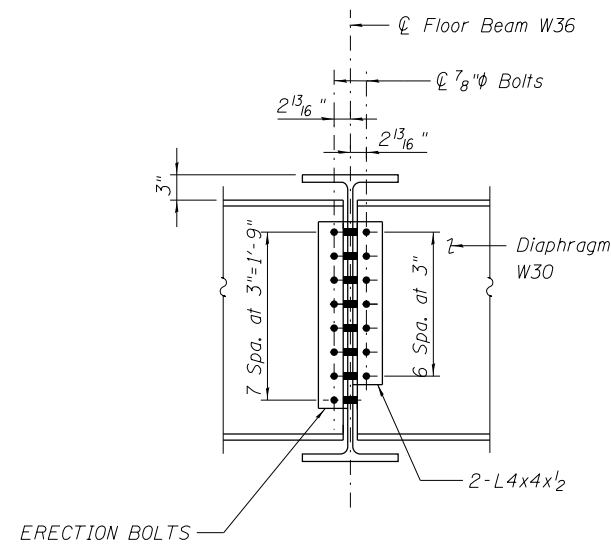
SECTION C-C
Scale: 1"=1'-0"



(Typ. Beam Connection at Girders G1 & G4)

SECTION E-E
Scale: 1"=1'-0"

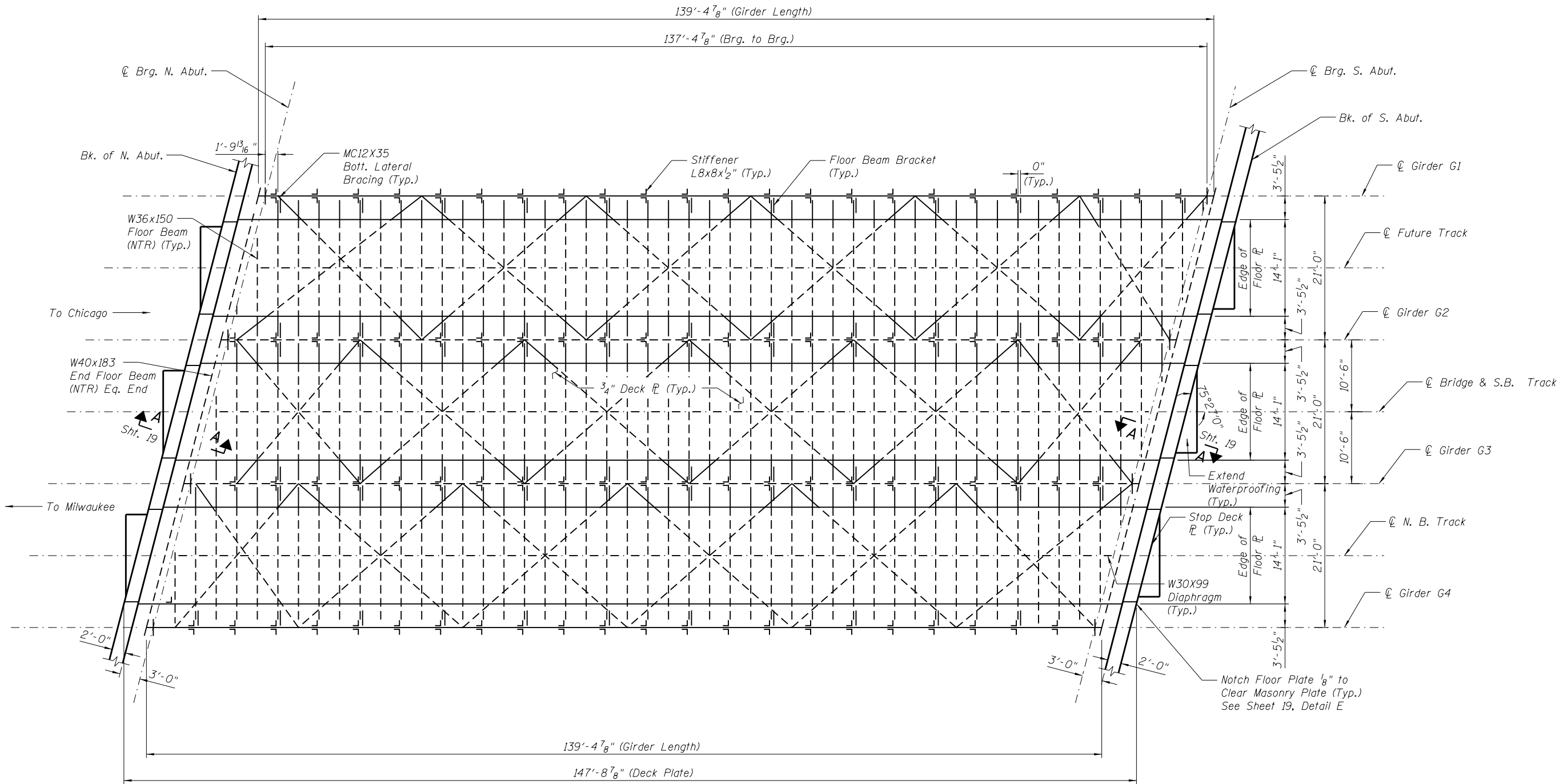
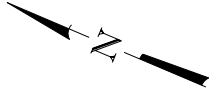
(Typ. Beam Connection)



ERECTION BOLTS

SECTION G-G
Scale: 1"=1'-0"

NOTE:
Work this Sheet with Sheet 11 & 12.



FLOOR DECK PLATE PLAN
Scale: 1/8" = 1'-0"

NOTE:
See Sheet 19 for Section A-A.

H:\Projects\3366\Drawings\Structural\Bridge\Plans\FINAL_PLANS\04906202-60K80-014.dgn

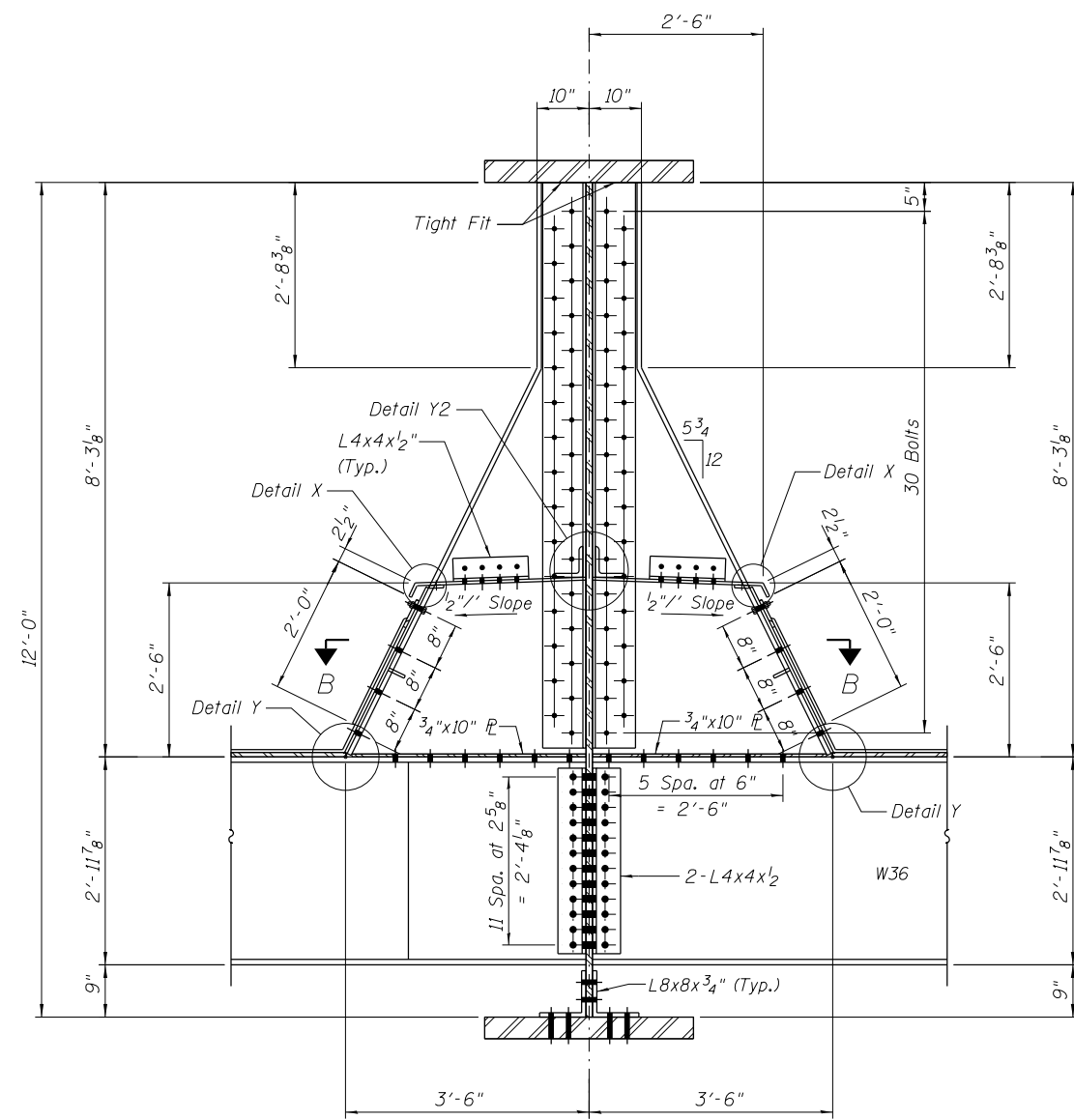
HOH HARRY D. HEFTER-ASSOCIATES, INC.
DESIGN AND CONSULTING ENGINEERS
55 East Jackson Blvd.
Chicago, Illinois 60604
312/346-8131

FILE NAME =	USER NAME = eefitzpatrick	DESIGNED - MMH	REVISED -
PROJECT NO. 3366		CHECKED - DNB	REVISED -
	PLOT SCALE = 16:0.0000 1/4" = 1'-0"	DRAWN - R.VEJAR	REVISED -
	PLOT DATE = 10/7/2016	CHECKED - DNB	REVISED -

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**FLOOR DECK PLATE PLAN
STRUCTURE NO. 049-0602**
SHEET NO. 14 OF 43 SHEETS

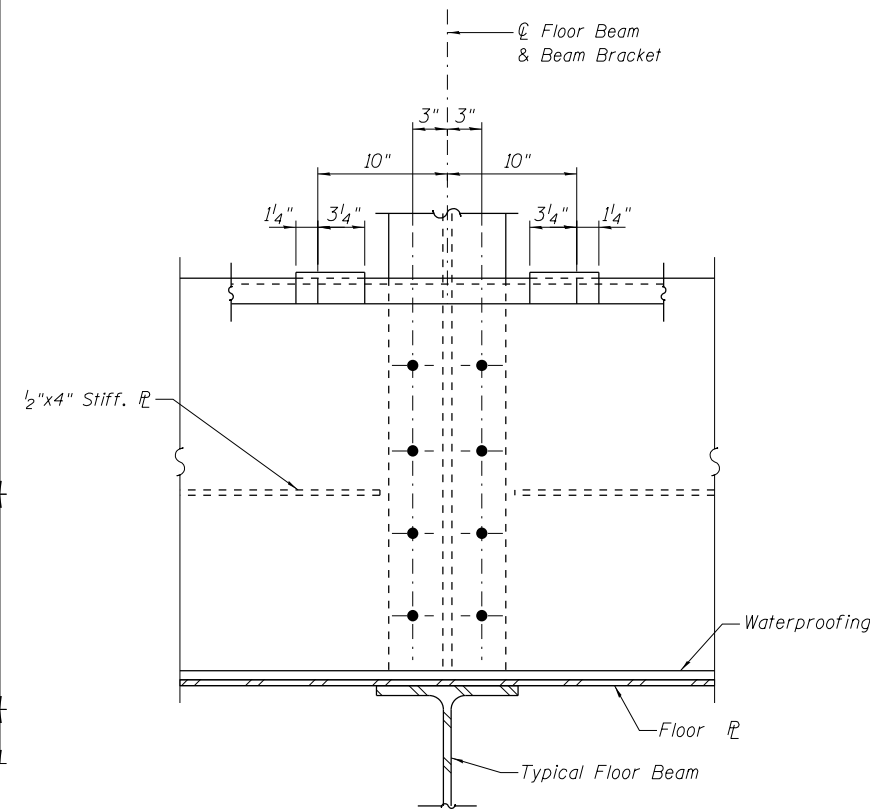
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
1218	125X-N	LAKE	361	229
CONTRACT NO. 60K80				
ILLINOIS FED. AID PROJECT				



SECTION C-C

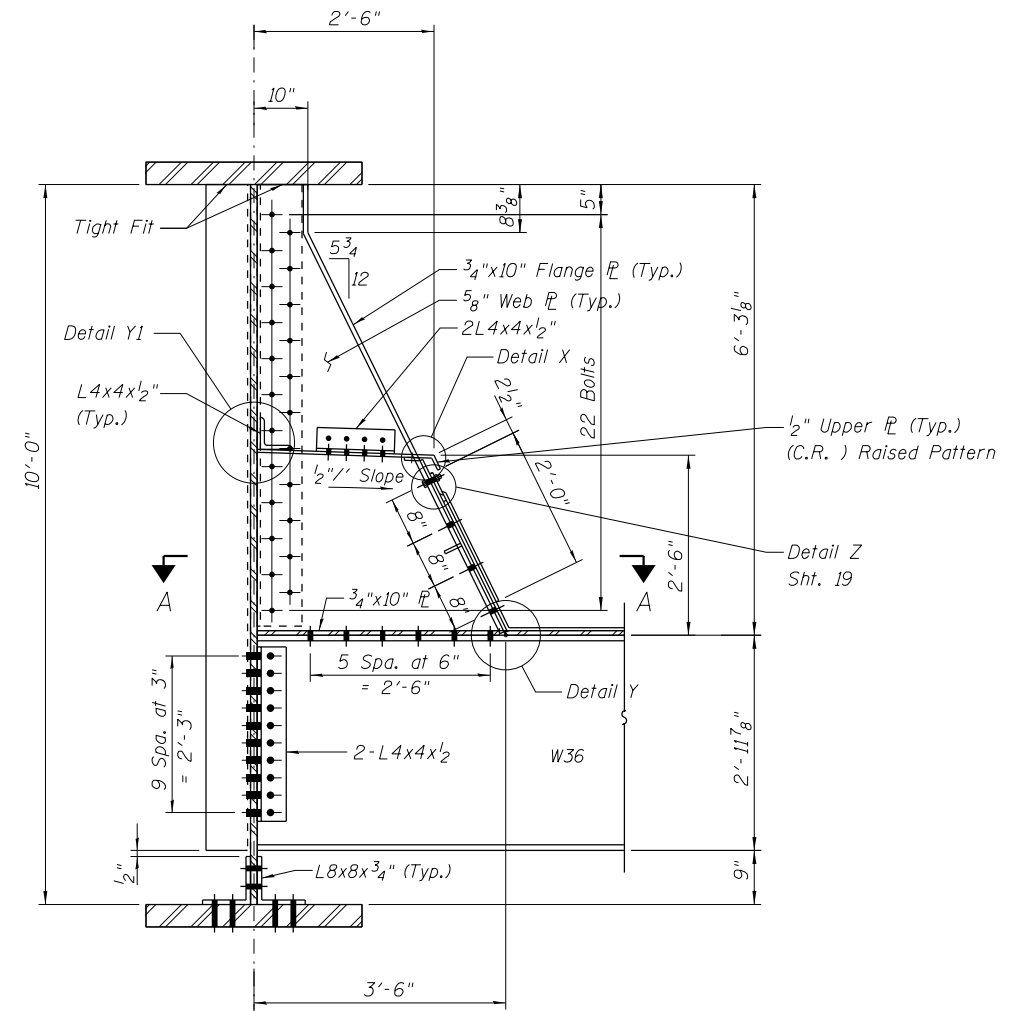
Girders G2 & G3
(At Beam Brackets)
N.T.S.

See Sheet 4 for Section Location
See Sheet 13 for Floor Beam Connections



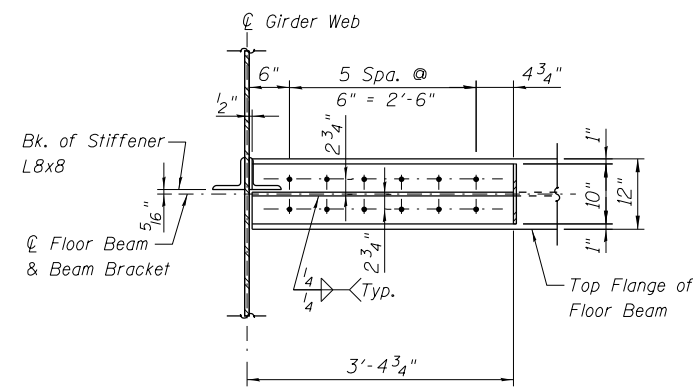
BALLAST PLATE ELEVATION

(At Beam Bracket Typical)
N.T.S.



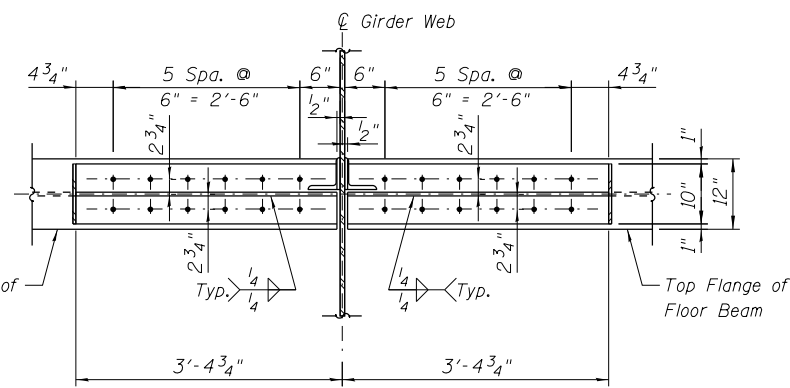
SECTION D-D

Girders G1 & G4
(At Beam Brackets)
N.T.S.



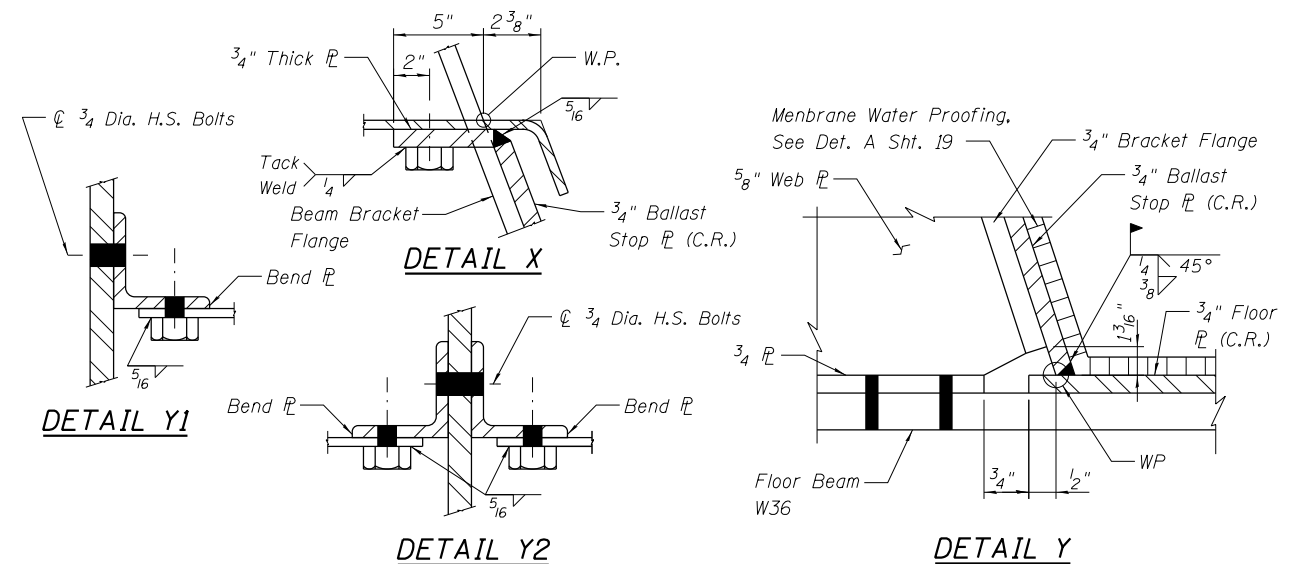
SECTION A-A

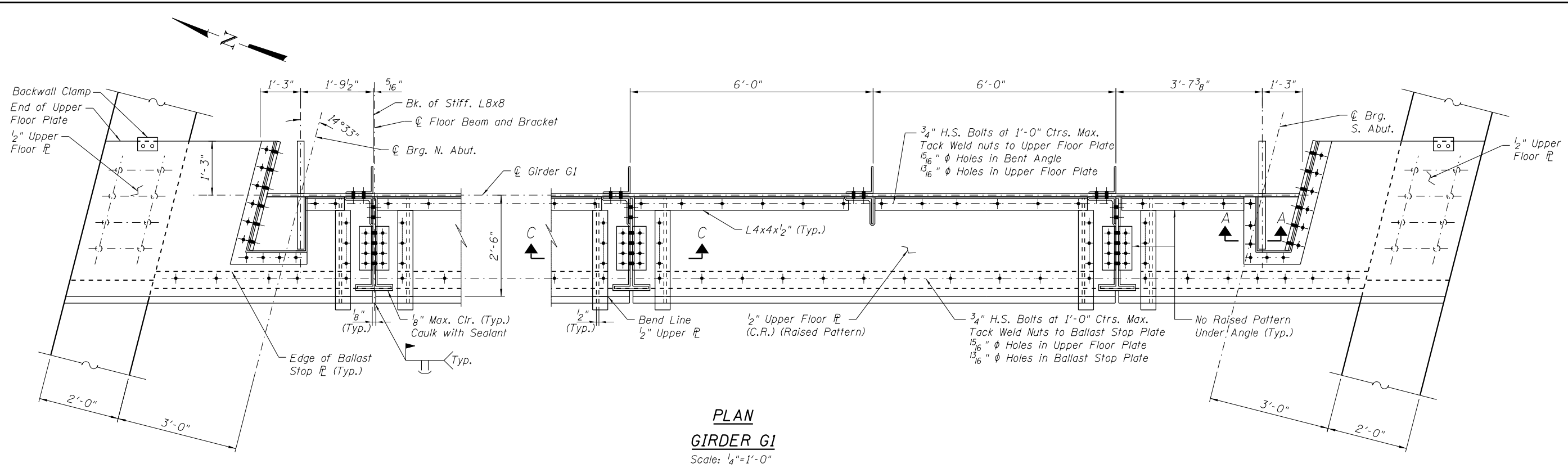
N.T.S.



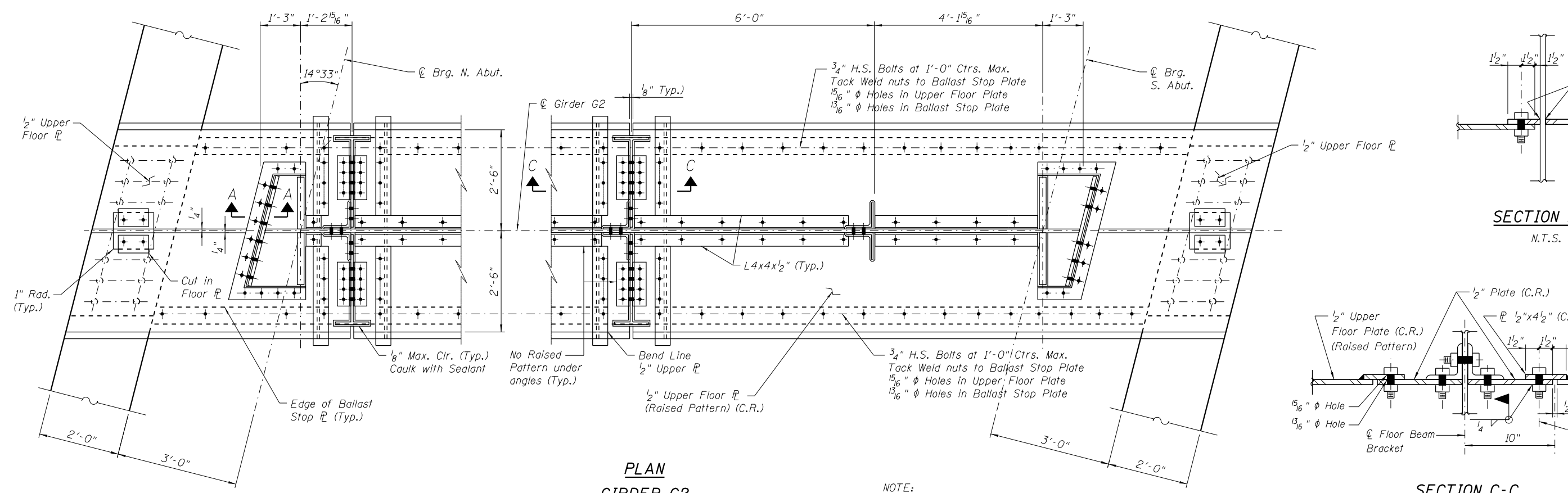
SECTION B-B

N.T.S.

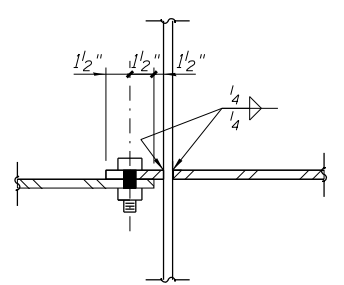




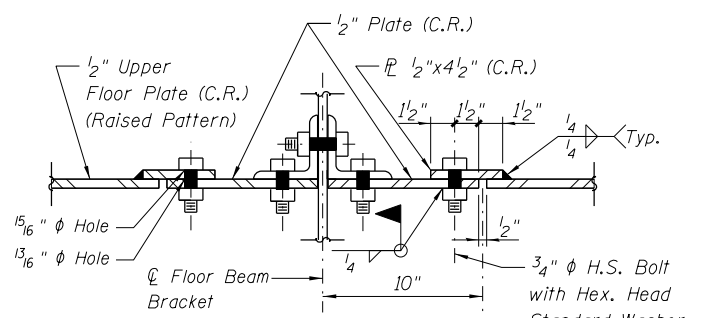
**PLAN
GIRDER G1**
Scale: 1/4"=1'-0"



**PLAN
GIRDER G2**
Scale: 1/4"=1'-0"



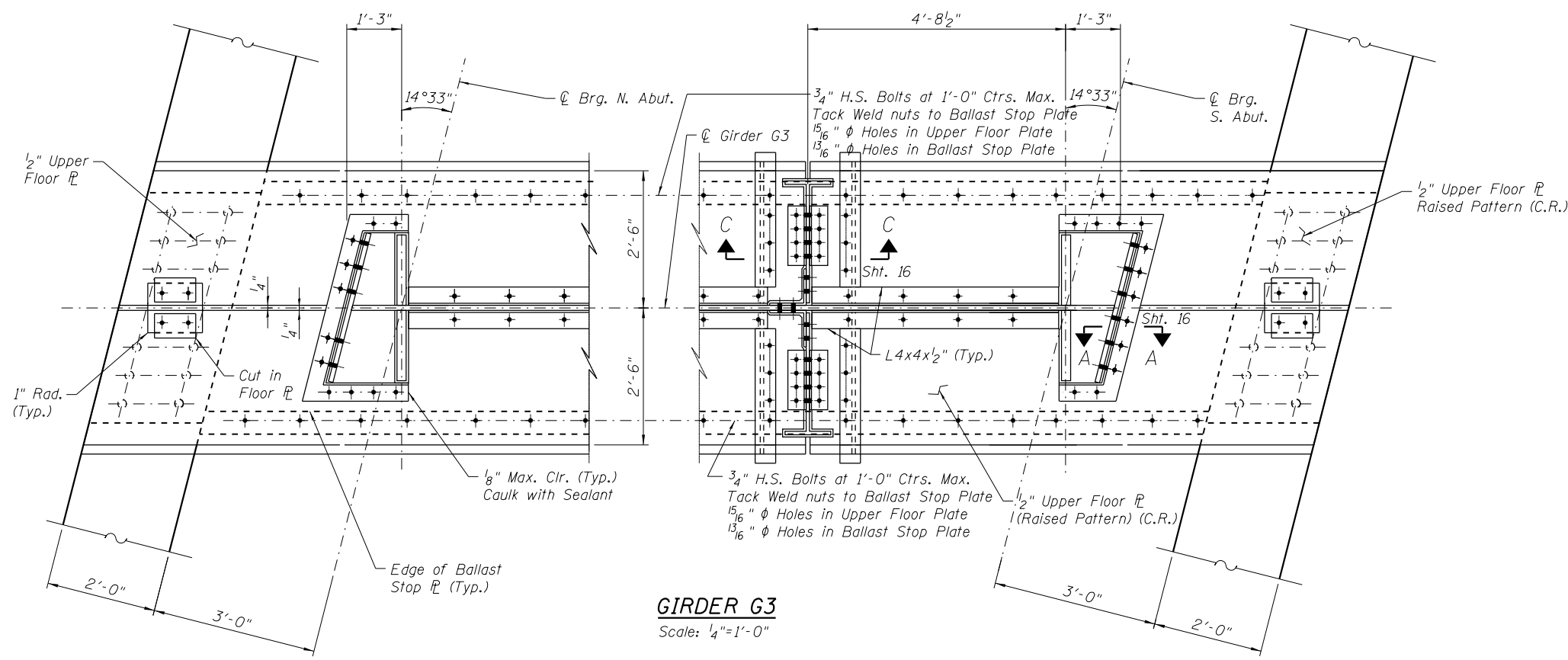
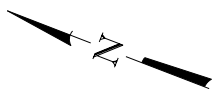
SECTION A-A
N.T.S.



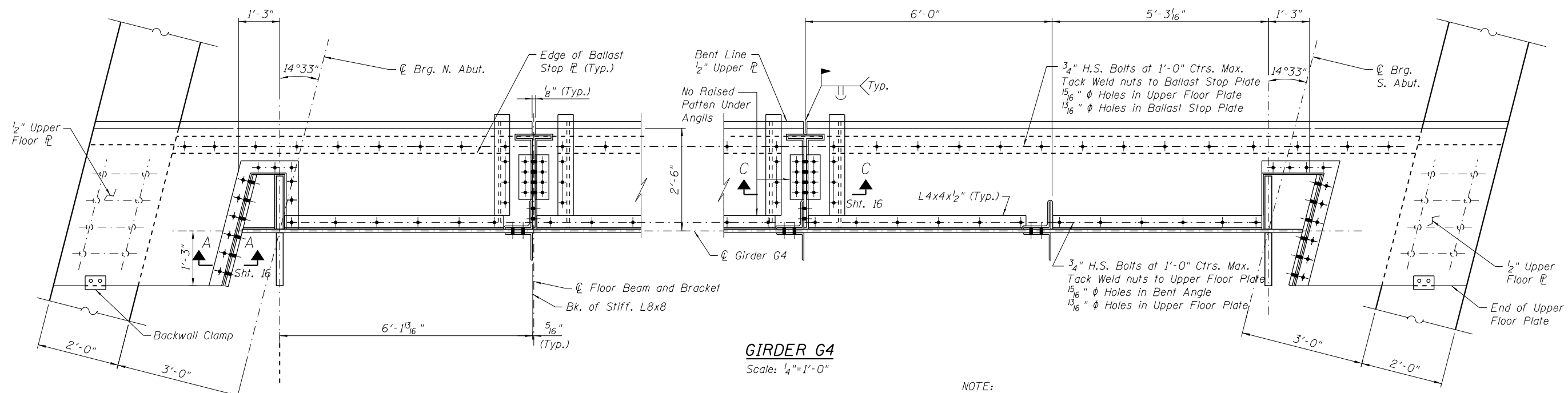
SECTION C-C
N.T.S.

NOTE:
1. Steel designated "C.R." shall meet the requirements of ASTM A588.
2. Field weld all openings to form a water tight surface.

FILE NAME - ...04906202-60K00-016.dgn HOH HARRY O. HEFTER ASSOCIATES, INC. DESIGN AND CONSULTING ENGINEERS 3366 55 East Jackson Blvd., Suite 600 Chicago, Illinois 60604 312/346-8131	USER NAME - aofitzpatrick DESIGNED - MMH CHECKED - DNB DRAWN - R.VEJAR CHECKED - BCS	REVISED - REVISED - REVISED - REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	UPPER FLOOR PLATE PLAN - 1 STRUCTURE NO. 049-0602 SHEET NO. 16 OF 43 SHEETS	F.A.P. R.T.E. - 346 SECTION - 125X-N&J-SB-B COUNTY - LAKE TOTAL SHEETS - 361 SHEET NO. - 231 CONTRACT NO. 60K80	ILLINOIS FED. AID PROJECT
	PLOT SCALE - 8:0.0000 '1' / in. PLOT DATE - 10/7/2016					



GIRDER G3
Scale: 1/4"=1'-0"



GIRDER G4
Scale: 1/4"=1'-0"

NOTE:
1. Steel designated "C.R." shall meet the requirements of ASTM A588.
2. Field weld all openings to form a water tight surface.

FILE NAME = ...04906202-60K00-017.dgn
HOH HARRY O. HEFTER ASSOCIATES, INC.
 DESIGN AND CONSULTING ENGINEERS
 3366 55 East Jackson Blvd., Suite 600
 Chicago, Illinois 60604
 312/346-8931

USER NAME = aofitzpatrick
 PLOT SCALE = 8:0.0000 1' = 1/8" in.
 PLOT DATE = 10/7/2016

DESIGNED - MMH	REVISD -
CHECKED - DNB	REVISD -
DRAWN - R.VEJAR	REVISD -
CHECKED - BCS	REVISD -

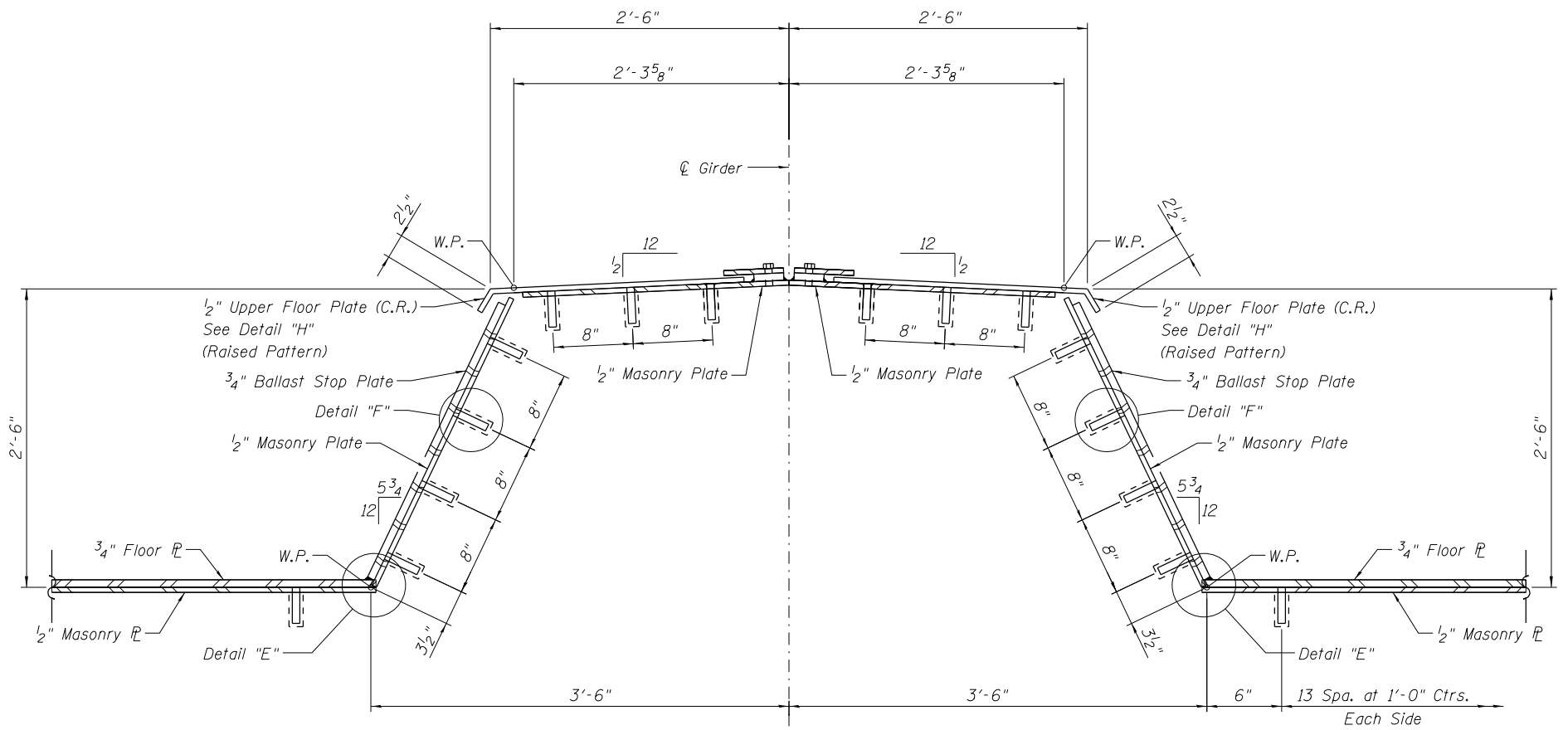
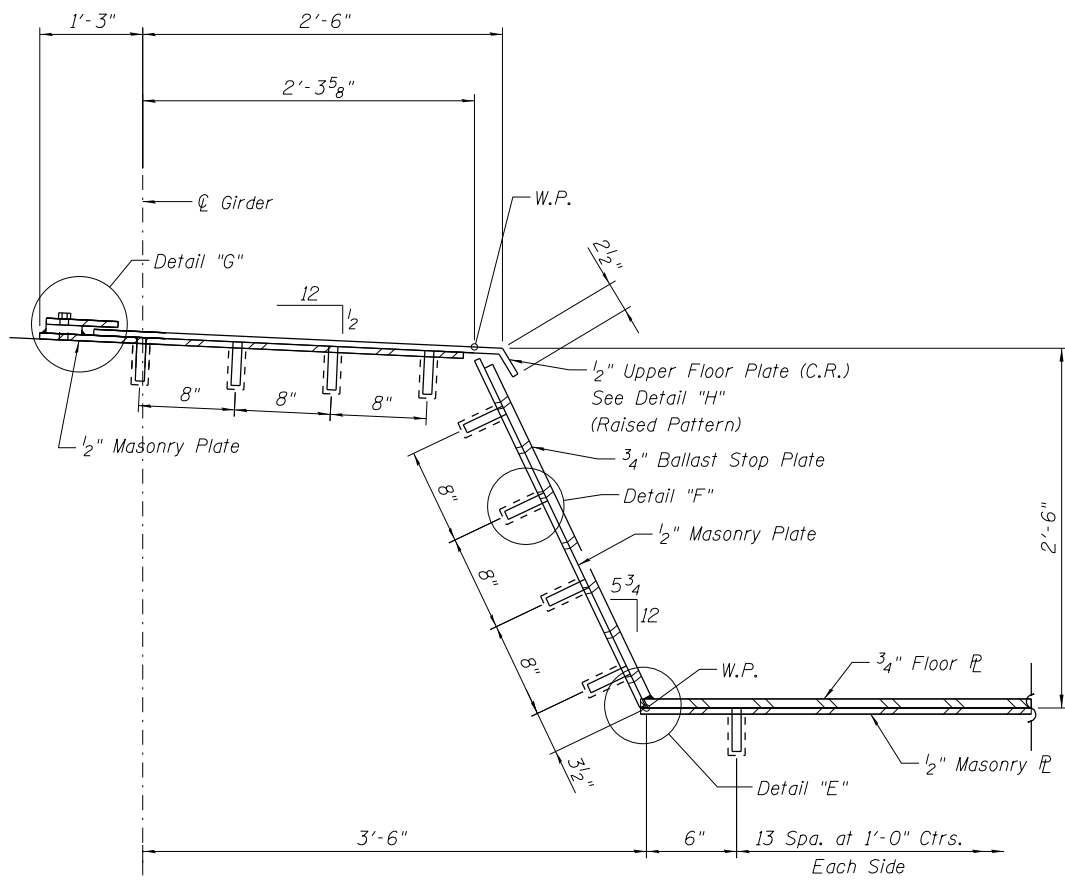
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

UPPER FLOOR PLATE PLAN - 2
STRUCTURE NO. 049-0602

SHEET NO. 17 OF 43 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
346	125X-N&J-SB-B	LAKE	361	232
CONTRACT NO. 60K80				

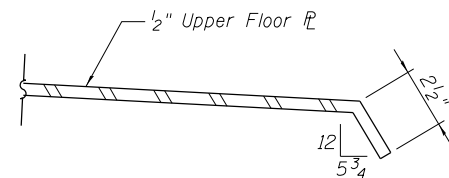
ILLINOIS FED. AID PROJECT



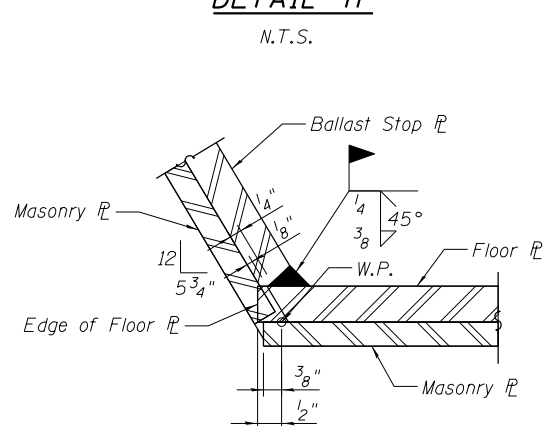
**ELEVATION
FLOOR PLATE DETAILS**

N.T.S.
(Looking North at North Abutment)
(South Abutment is Similar)
All horizontal dimensions are at rt. angle to ⊕ Girders.

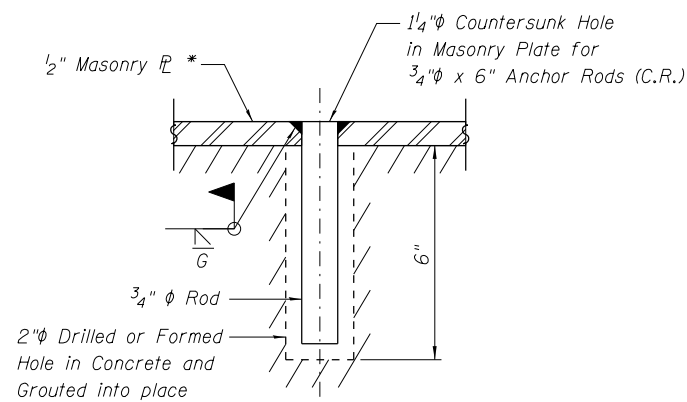
- Notes:
1. Steel designated "C.R." shall meet the requirements of ASTM A588
 2. Embedded masonry plates and rods included in the cost of membrane waterproofing.



DETAIL "H"
N.T.S.

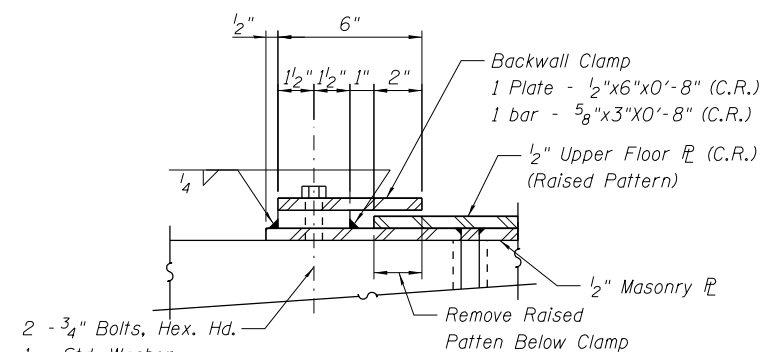


DETAIL "E"
N.T.S.



DETAIL "F"
N.T.S.

* Masonry plates to be welded to anchor rods in field and ground smooth.



2 - 3/4" Bolts, Hex. Hd.
1 - Std. Washer
1 - Lock Washer (C.R.)
Drill & Tap 1/2" Masonry PL and Spacer Bar

DETAIL "G"
N.T.S.

Note:
Hold 1/8" gap between backwall clamp and upper floor plate at expansion end.

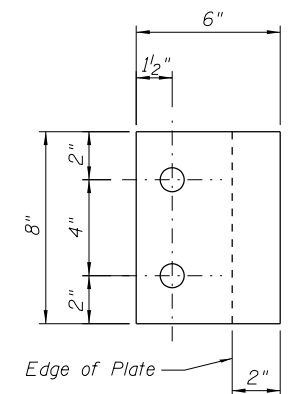
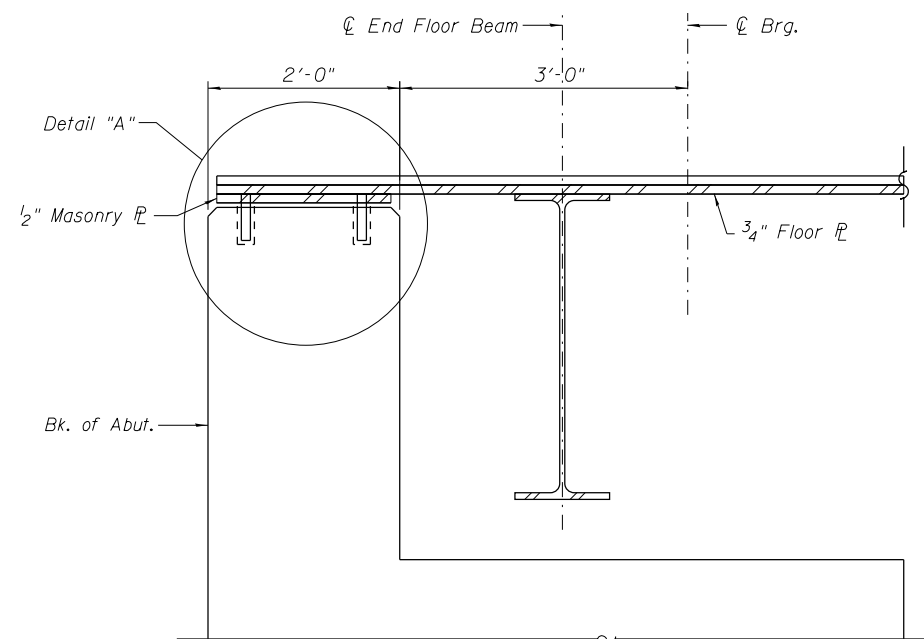
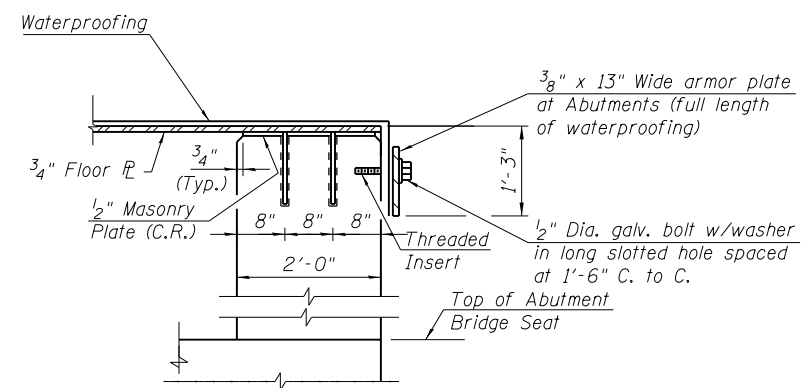


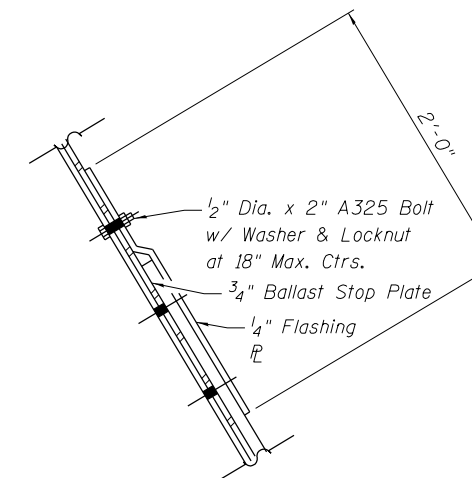
PLATE DETAIL
N.T.S.



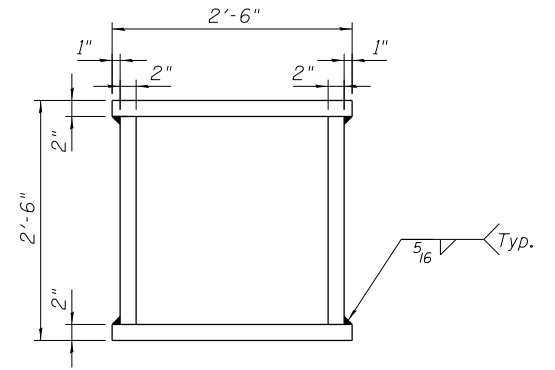
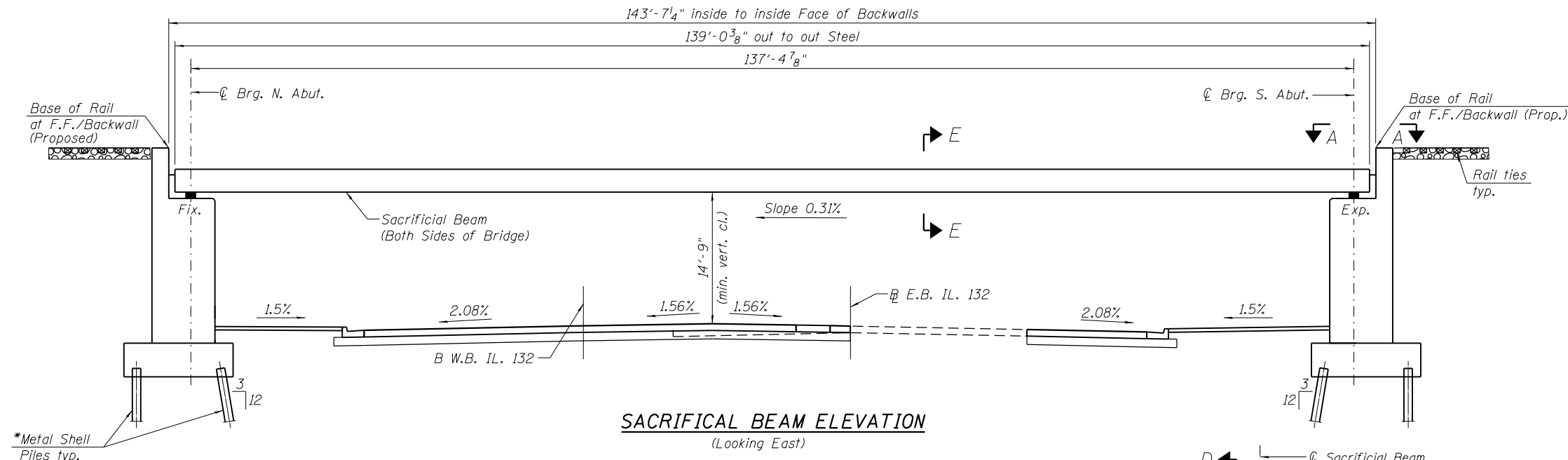
SECTION A-A
N.T.S.



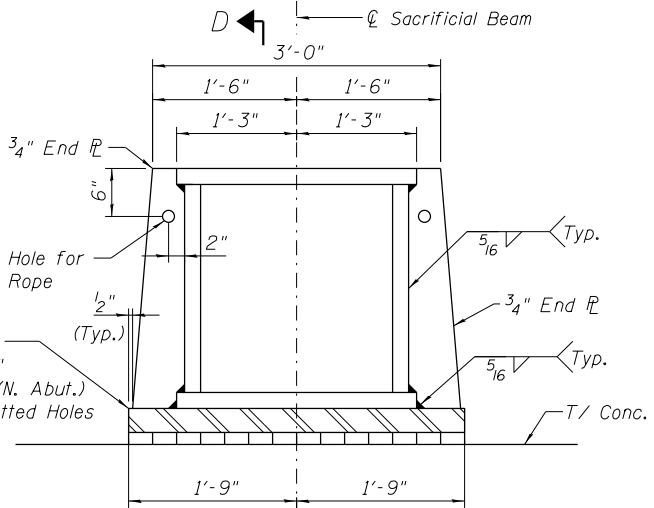
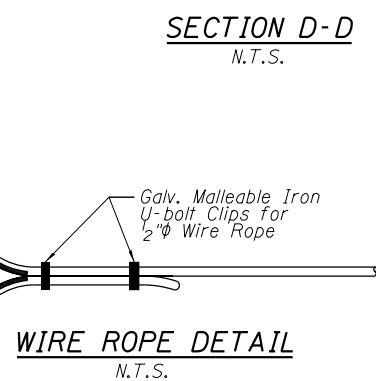
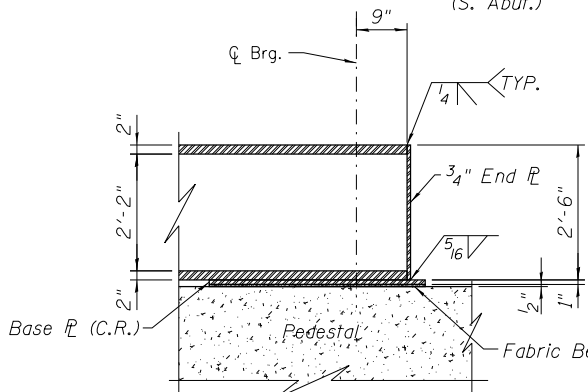
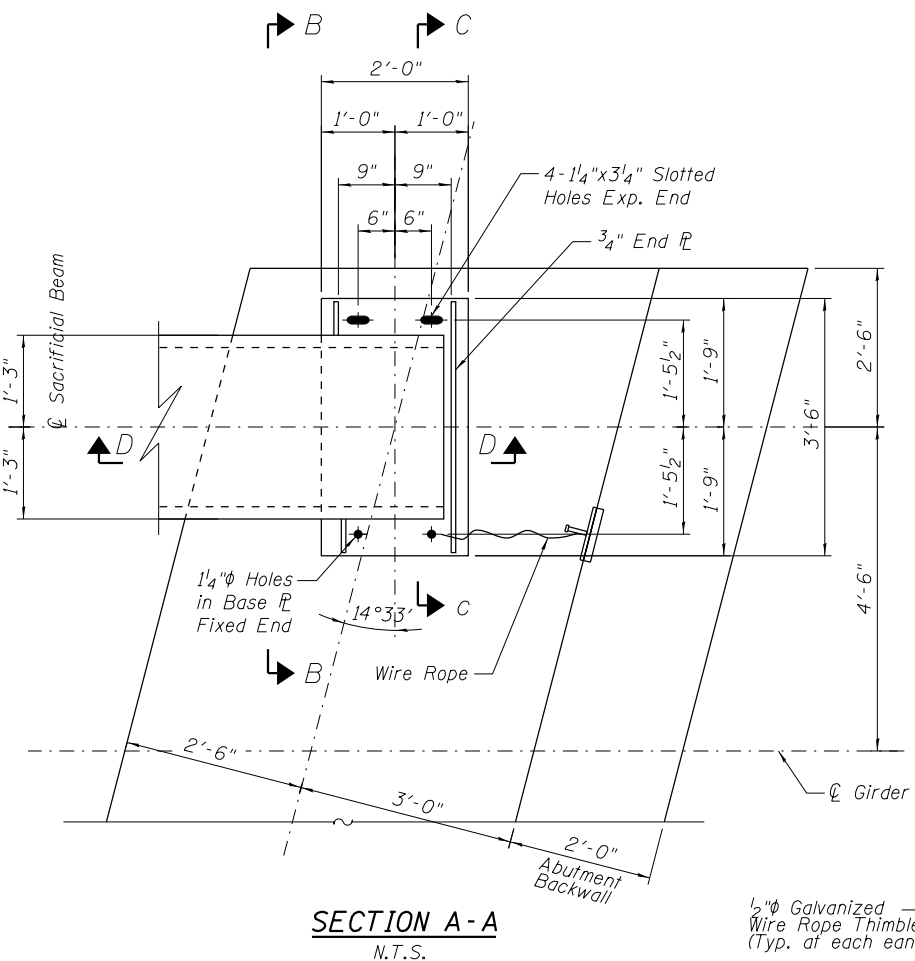
WATERPROOFING AT ABUTMENT BACKWALL
DETAIL A



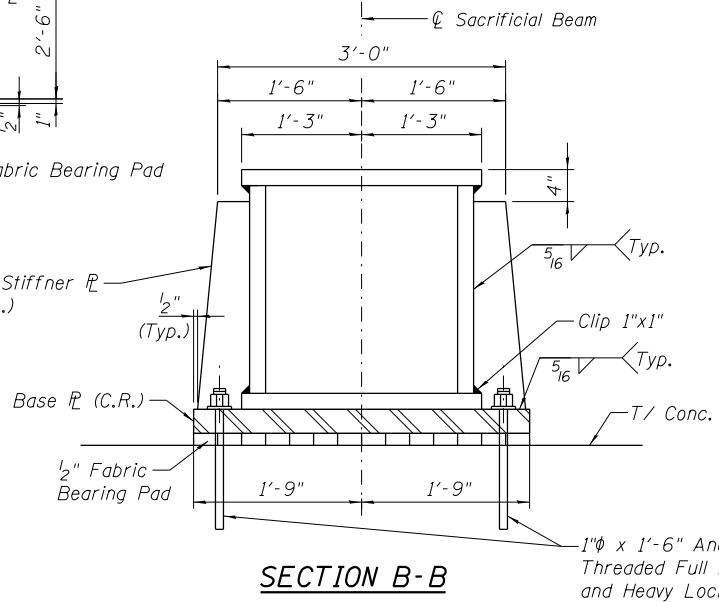
DETAIL Z
Scale: 1/2"=1'-0"
Sheet 16



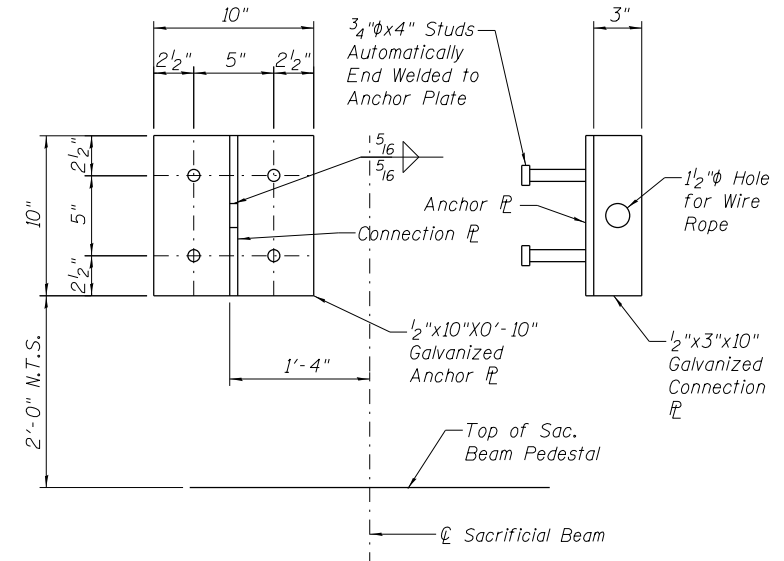
SACRIFICIAL BEAM ELEVATION
(Looking East)



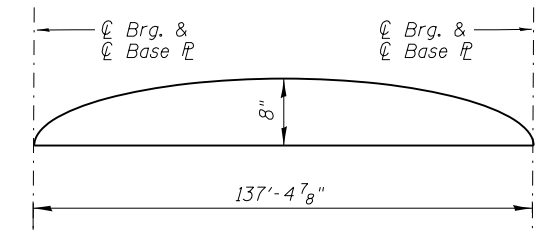
SECTION C-C



SECTION B-B



ANCHOR PLATE DETAIL



CAMBER DIAGRAM
(PARABOLIC CURVE)

FILE NAME = ...04906202-60K00-020.dgn
HOH HARRY O. HEFTER ASSOCIATES, INC.
 DESIGN AND CONSULTING ENGINEERS
 3366 55 East Jackson Blvd., Suite 600
 Chicago, Illinois 60604
 312/346-8931

USER NAME = aefitzpatrick
 PLOT SCALE = 2.0000 / in.
 PLOT DATE = 10/7/2016

DESIGNED - MMH
 CHECKED - DNB
 DRAWN - R.VEJAR
 CHECKED - BCS

REVISED -
 REVISED -
 REVISED -
 REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SACRIFICIAL BEAM DETAILS
STRUCTURE NO. 049-0602

SHEET NO. 20 OF 43 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
346	125X-N&J-SB-B	LAKE	361	235
CONTRACT NO. 60K80				

ILLINOIS FED. AID PROJECT

**PLATE GIRDER (G1 & G4)
MOMENT AND SHEAR**

MOMENT	
Description	At 0.5L
Dead Load	11,202.4
Live Load (E80)	11,957.8
Impact	2,570.9
Total	25,731.1
Web	1"
Flange	3"x30"
Gross I Furn.	895,700 in ⁴
Net I Furn.	815,117 in ⁴
Net Sect. Mod. - Bot.	13,107 in ³
Allow Max. Tensile Stress in Flange	27.5 ksi
Actual Max. Tensile Stress in Flange	23.6 ksi
Allow Max. Deflection (Live Load + Impact)	2.6 in
Actual Max. Deflection (Live Load + Impact)	2.0 in
SHEAR	
Dead Load	322.3 k
Live Load (E80)	384.8 k
Impact	82.7 k
Total	789.8 k
Web Shear	6.6 ksi
End Stiffener Column Area Reqd.	54.7 in ²
Section (2 Plates)	2"x14"
End Stiffener Column Area Furn.	68.0 in

**PLATE GIRDER (G2 & G3)
MOMENT AND SHEAR**

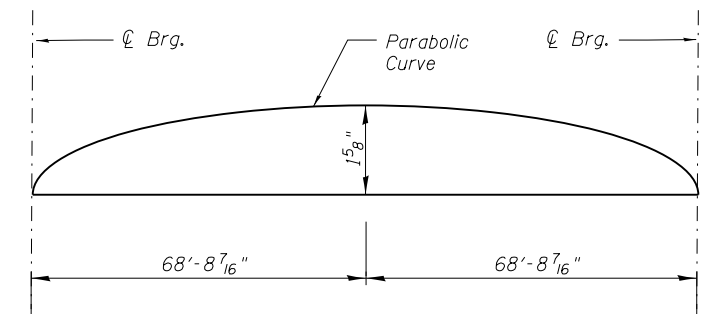
MOMENT	
Description	At 0.5L
Dead Load	20,348.3 ft-k
Live Load (E80)	23,915.7 ft-k
Impact	5,165.8 ft-k
Total	49,929.8 ft-k
Web	1 1/8"
Flange	3 5/8"x36"
Gross I Furn.	1,808,000 in ⁴
Net I Furn.	1,605,625 in ⁴
Net Sect. Mod. - Bot.	21,773 in ³
Allow Max. Tensile Stress in Flange	27.5 ksi
Actual Max. Tensile Stress in Flange	27.3 ksi
Allow Max. Deflection (Live Load + Impact)	2.6 in
Actual Max. Deflection (Live Load + Impact)	2.1 in
SHEAR	
Dead Load	588.9 k
Live Load (E80)	769.5 k
Impact	166.2 k
Total	1,521.6 k
Web Shear	9.4 ksi
End Stiffener Column Area Reqd.	55.5 in ²
Section (2 Plates)	2"x14"
End Stiffener Column Area Furn.	71.2 in

**FLOOR BEAM
MOMENT AND SHEAR**

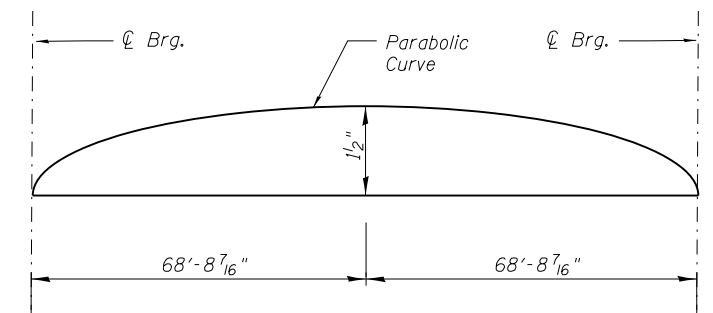
Description	MOMENT	
	Floor Beam (A709, GR.50) W36x150	End Floor Beam (A709, GR.50) W40x183
Dead Load	66.3 ft-k	1,451 ft-k (Jacking) 46.8 ft-k (Bridge)
Live Load (E80)	276.0 ft-k	253.1 ft-k
Impact	111.1 ft-k	99.2 ft-k
Knee Brace	647.4 ft-k	-
Total	1,106.8 ft-k	1,451 ft-k (Jacking) 399.1 ft-k (Bridge)
Section	W36x150	W40x183
Gross I Furn.	9,040 in ⁴	13,300 in ⁴
Net I Furn.	8,893 in ⁴	8,886 in ⁴
Net Sect. Mod. - Bot.	496.0 in ³	498.8 in ³
Allow Max. Tensile Stress in Flange	27.5 ksi	27.5 ksi
Actual Max. Tensile Stress in Flange	26.8 ksi	25.52 ksi (Jacking) 9.6 ksi (Bridge)
Allow Max. Deflection (Live Load + Impact)	0.394 in	0.367 in
Actual Max. Deflection (Live Load + Impact)	0.132 in	0.063 in
SHEAR		
Dead Load	10.4 k	322.3 k (Jacking) 9.3 k (Bridge)
Live Load (E80)	34.5 k	34.5 k (Bridge)
Impact	13.9 k	13.5 k (Bridge)
Knee Brace	185.0	-
Total	243.8 k	322.3 k (Jacking) 57.3 k (Bridge)
Web Shear	10.2 ksi	14.7 ksi (Jacking) 2.6 ksi (Bridge)

BEARING ON CONCRETE

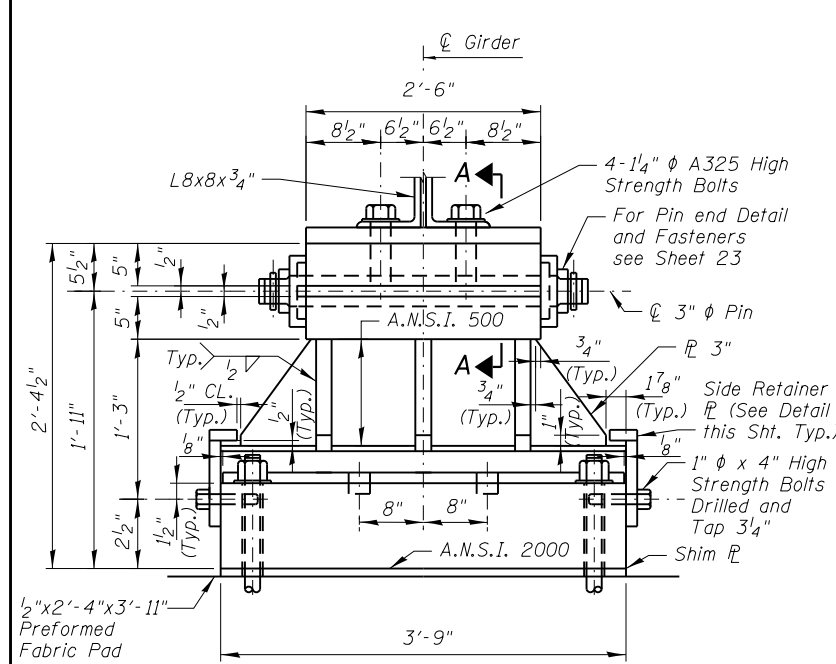
Description	Girder G1 or G4	Girder G2 or G3
Total Reaction	791 k	1,526 k
Bearing Area Furnished	1,170 in	1,536 in
Average Brg. Pressure	0.68 ksi	1.00 ksi



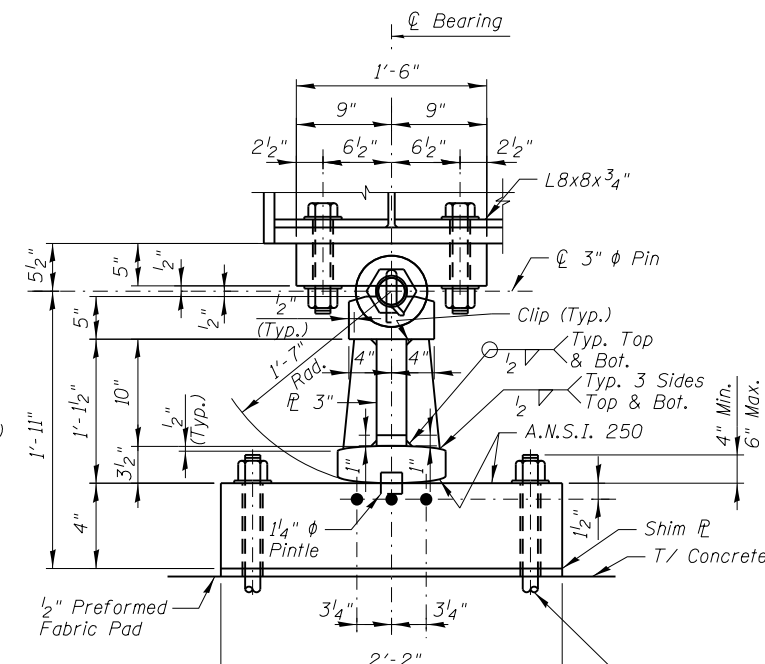
CAMBER DIAGRAM (GIRDERS G1 & G4)



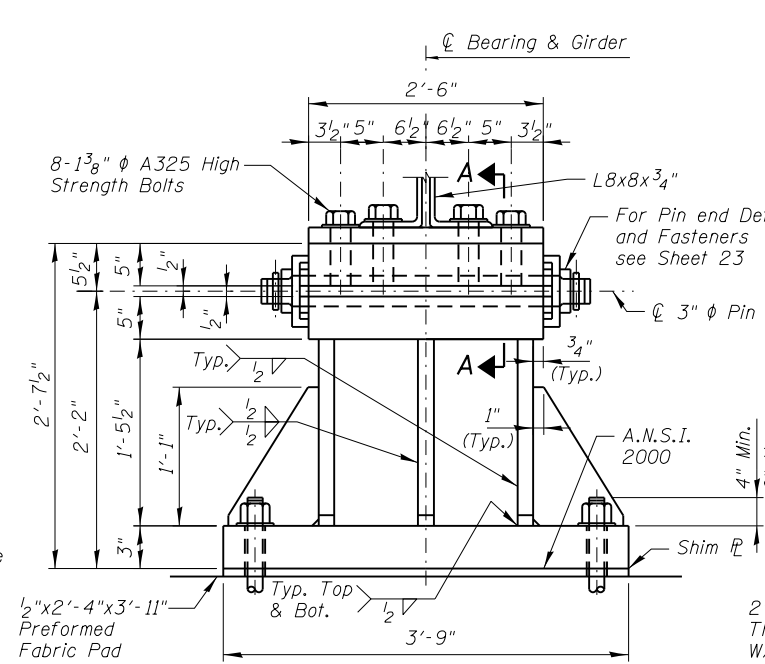
CAMBER DIAGRAM (GIRDERS G2 & G3)



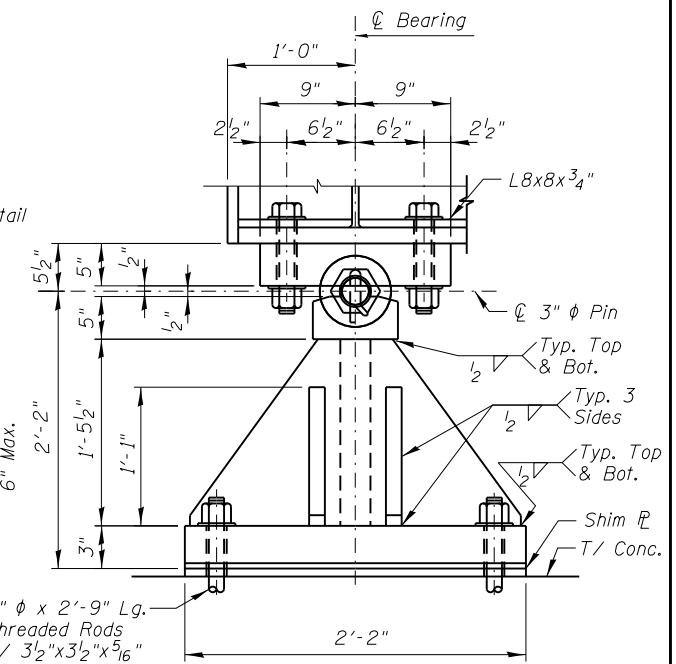
ELEVATION
(EXPANSION BEARING)
(S. Abutment)



SIDE VIEW
(EXPANSION BEARING)



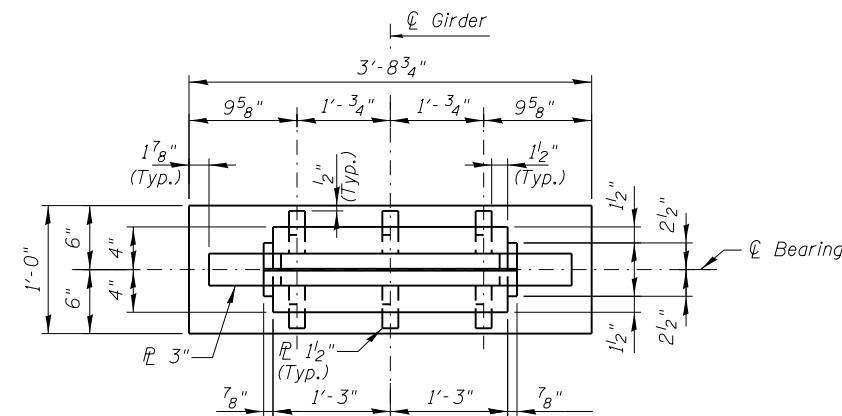
ELEVATION
(FIXED BEARING)
(N. Abutment)



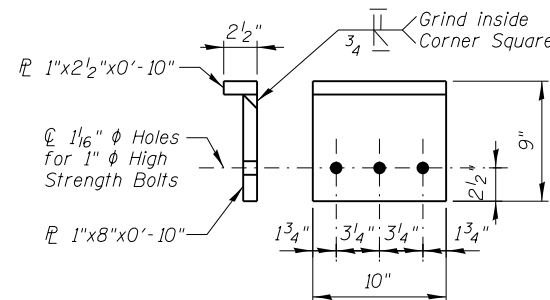
SIDE VIEW
(FIXED BEARING)

GIRDERS G1 & G4

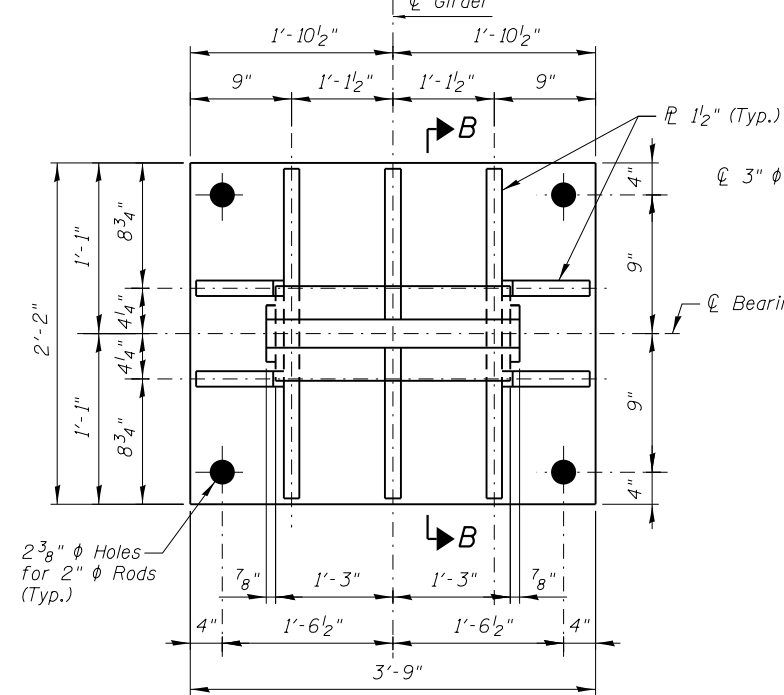
GIRDERS G1 & G4



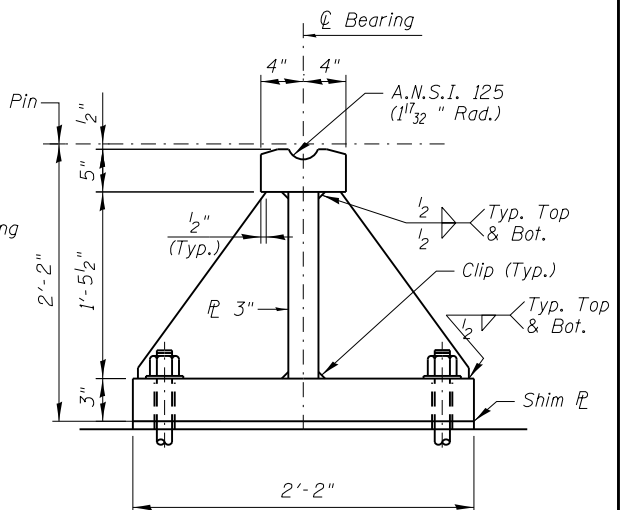
LOWER BEARING ASSEMBLY
(EXPANSION BEARING)



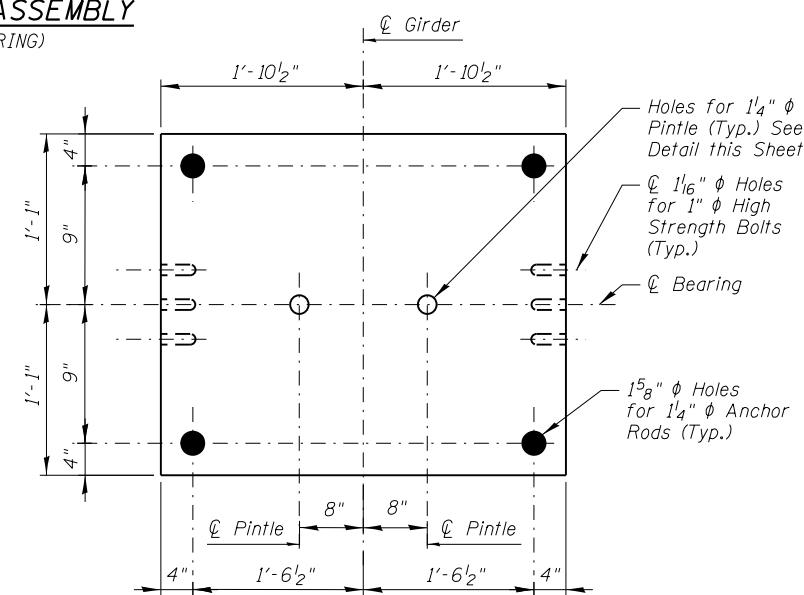
SIDE RETAINER DETAIL
(EXPANSION BEARING)



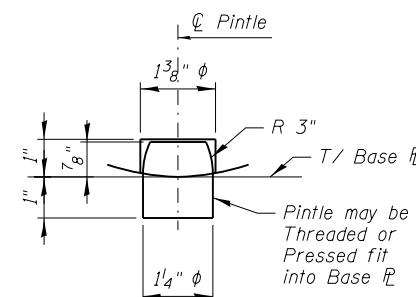
BOTTOM PLAN
(FIXED BEARING)



SECTION B-B
(FIXED BEARING)



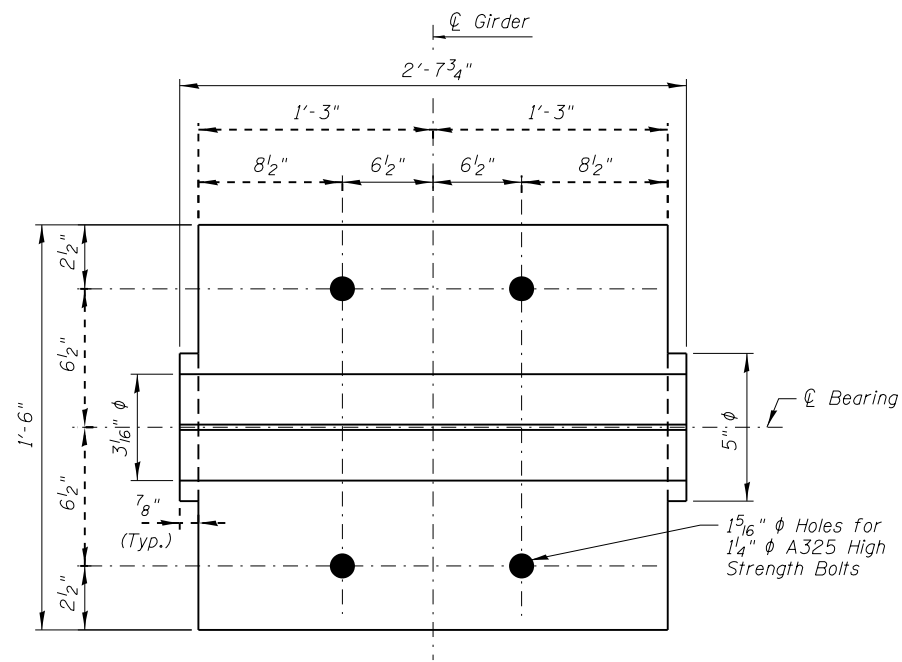
BASE PLATE PLAN
(EXPANSION BEARING)



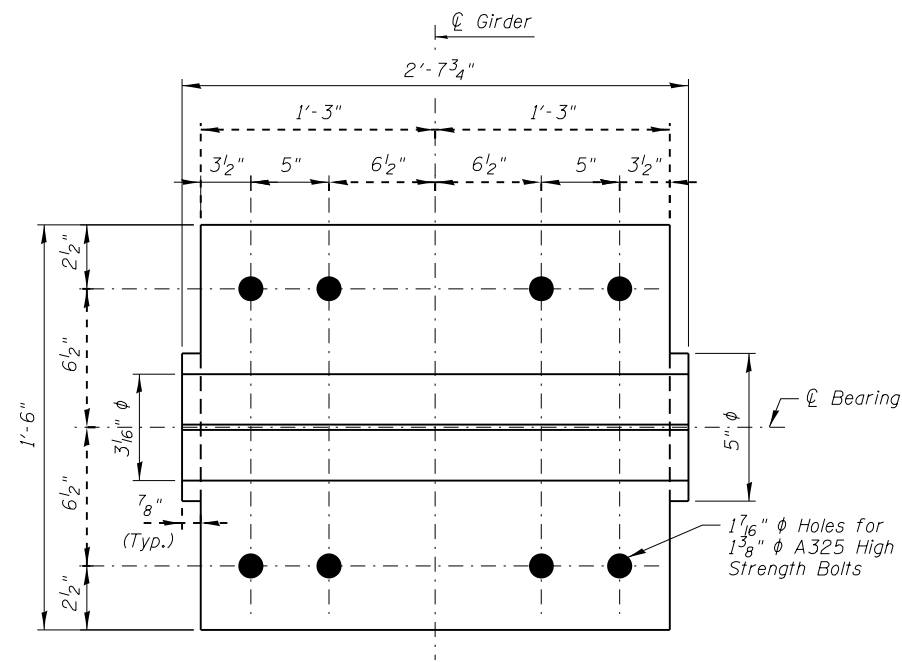
PINTLE DETAIL
(EXPANSION BEARING)

NOTES :

1. For Section A-A and Upper Bearing Assembly, See Sheet No. 23.
2. For Additional Notes See Sheet No. 23.
3. Grease Bearing Assembly before Installation.
4. Grease hole/fitting shall be on street side of Bearing.
5. Bearings shall be blocked during construction.



**UPPER BEARING ASSEMBLY
BOTTOM VIEW
(EXPANSION BEARING)**



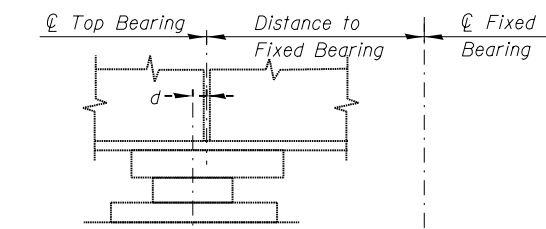
**UPPER BEARING ASSEMBLY
BOTTOM VIEW
(FIXED BEARING)**

NOTES :

1. Pin material must be ASTM A-668 Class F, harden to a Brinell Hardness of 200.
2. Material for bearing component parts must be ASTM A709, Grade 50, except as noted.
3. Bearing assembly weldments must be stress relieved by heat treating per AWS D1.1 prior to finish machining.
4. The parts of members in contact must be faced and bearing surfaces must be planed smooth before any welding is started.
5. Pintles must be stainless steel conforming to ASTM A276, Type 410 annealed.
6. Bearing pins or rockers and all surfaces in contact with pins or rockers must be given an all-over smooth finish.
7. Bearing seat surfaces will be constructed or adjusted to the designated elevations within a tolerance of 1/8\"/>
- 8. Shim plates will be placed between the base plate and the preformed fabric pad. Shim plates will match the footprint of the base plate. Heights of individual shim plates will be a minimum of 3/8\"/>
- 9. Concrete surface under bearing to be ground to a smooth level to give even bearing. Laminated fabric pads with 1/2\"/>
- 10. The cost of all steel weldments, plate washers, pins, recessed nuts, cap washers and preformed fabric pads is included in the cost of "Steel Bearing Assembly".

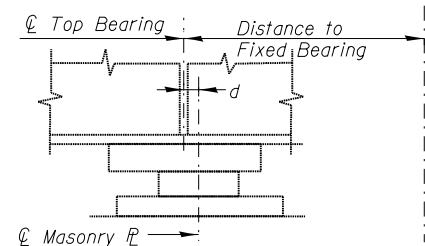
NOTE :

A.I.S.I. 1000 for Plates in Contract Unless noted Otherwise (typ.)



BELOW 50° F

(Move masonry \mathcal{R} away from fixed bearing)

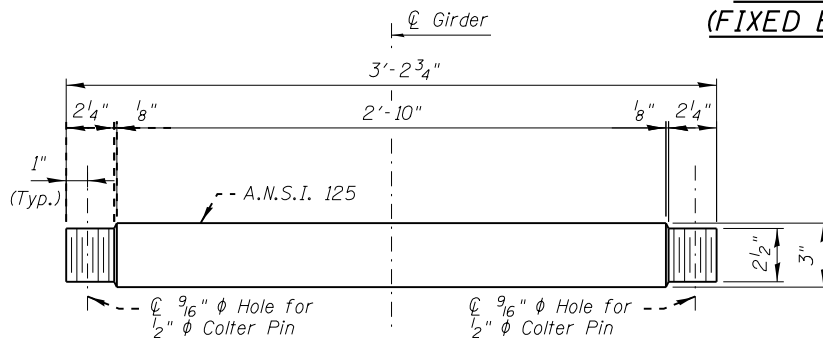


ABOVE 50° F

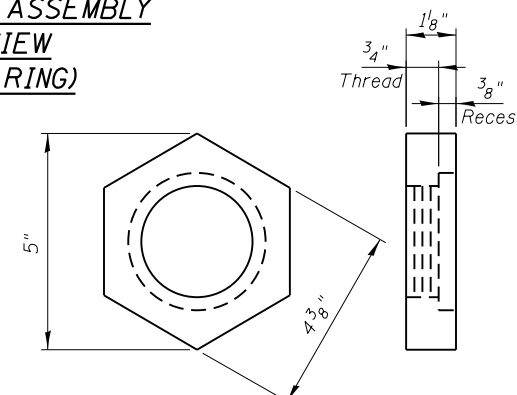
(Move masonry \mathcal{R} toward fixed bearing)

SETTING ANCHOR BOLTS AT EXPANSION BEARINGS

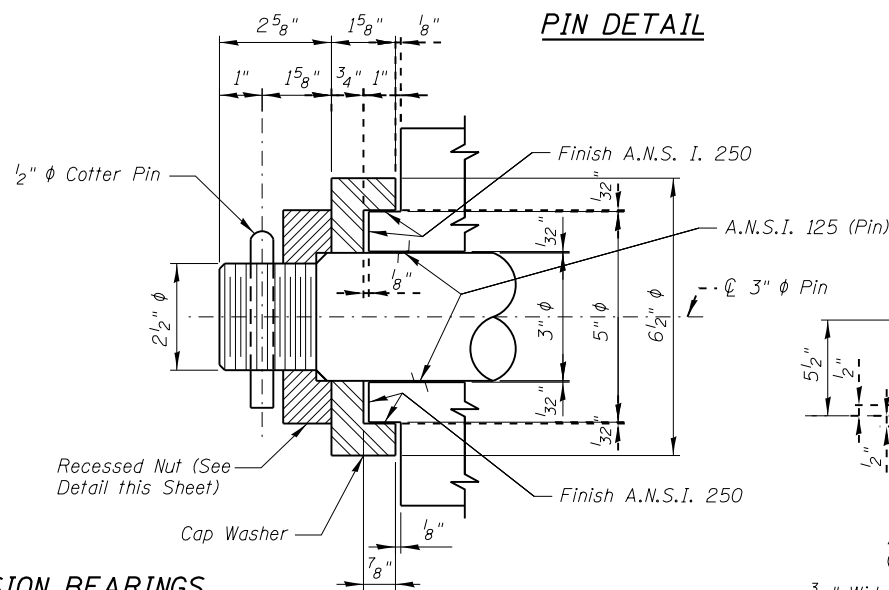
$d = \frac{1}{8}$ " per each 100' of expansion for every 15° F temperature change from the normal temperature of 50° F.



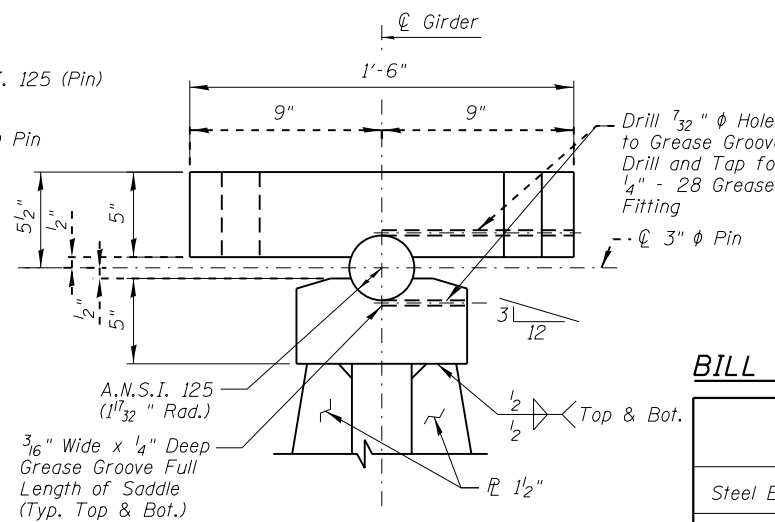
PIN DETAIL



RECESSED NUT DETAIL



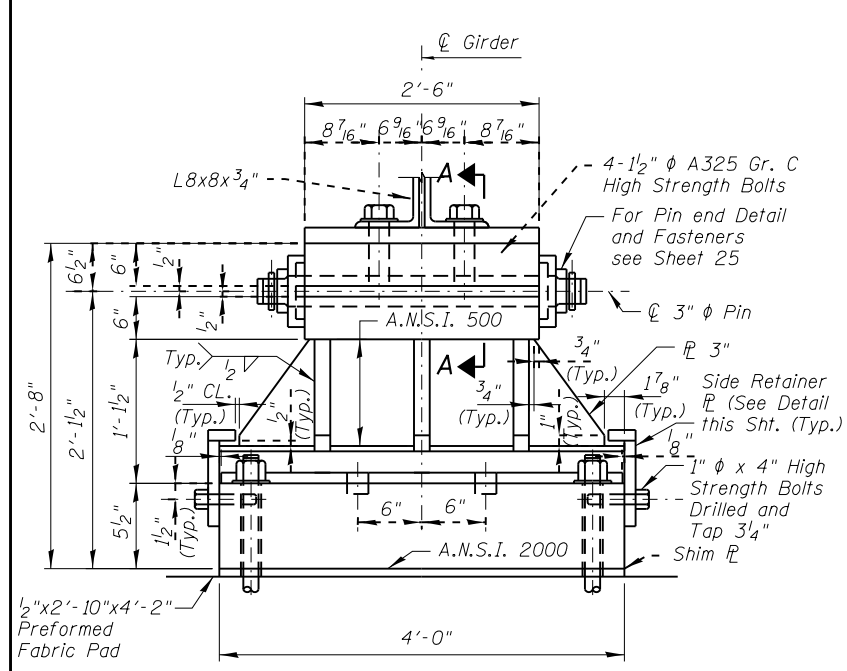
PIN END DETAIL



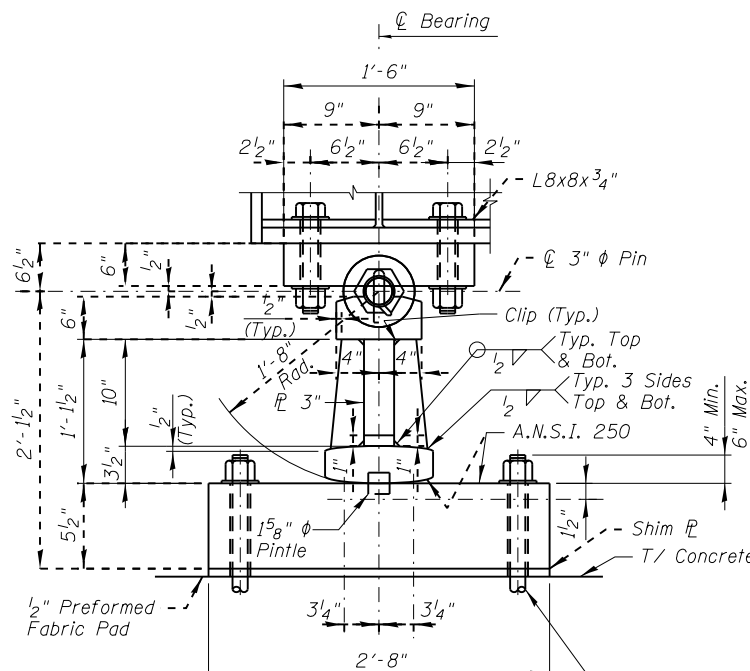
SECTION A-A

BILL OF MATERIAL

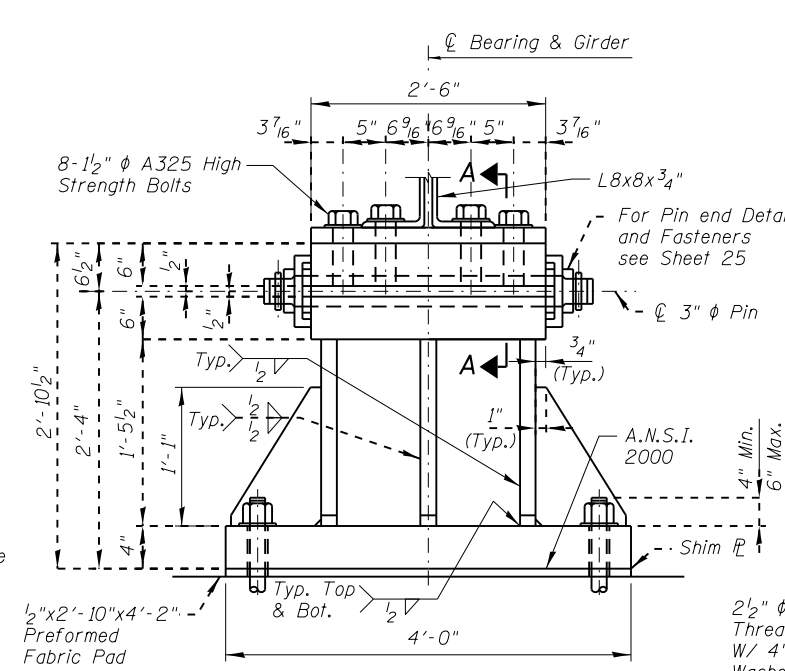
ITEM	UNIT	QUANTITY
Steel Bearing Assembly	Each	4
Anchor Bolts 1 1/4"	Each	8
Anchor Bolts 2"	Each	8



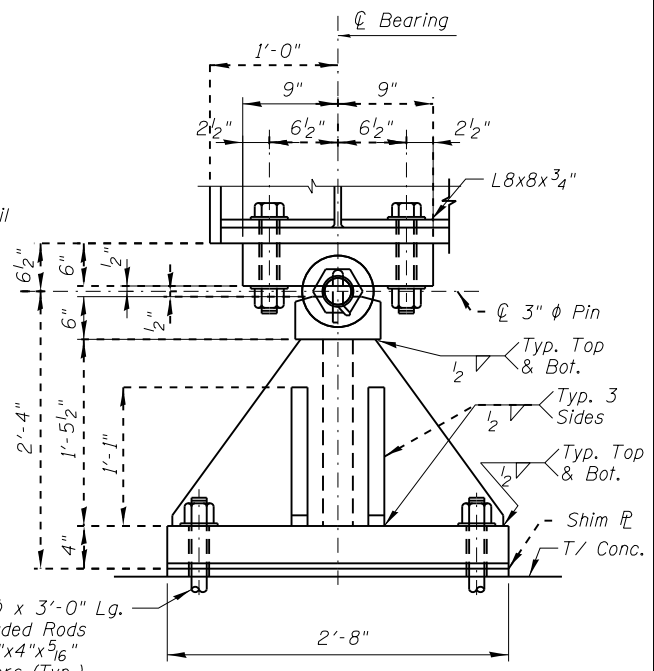
ELEVATION
(EXPANSION BEARING)
(S. Abutment)



SIDE VIEW
(EXPANSION BEARING)



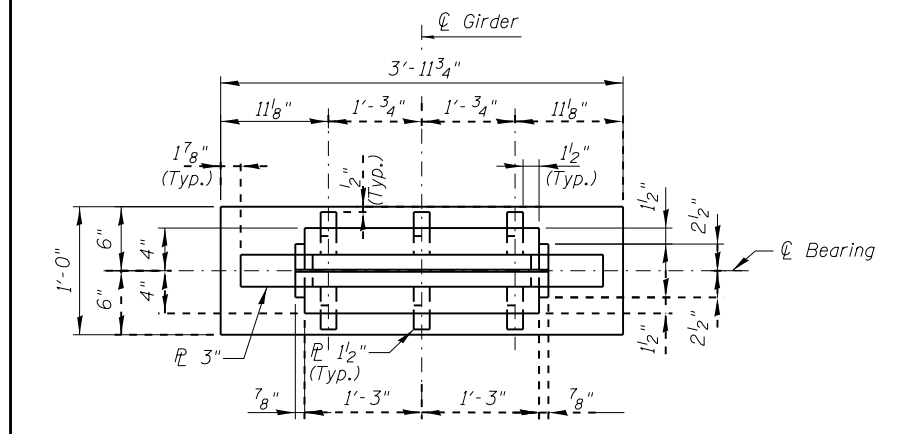
ELEVATION
(FIXED BEARING)
(N. Abutment)



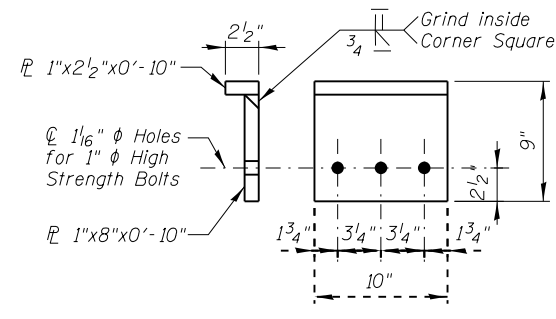
SIDE VIEW
(FIXED BEARING)

GIRDERS G2 & G3

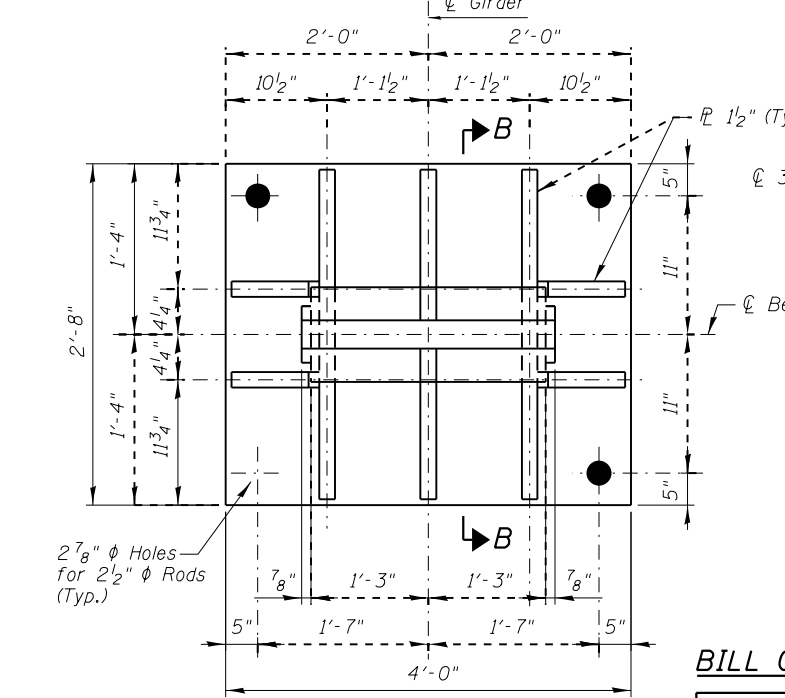
GIRDERS G2 & G3



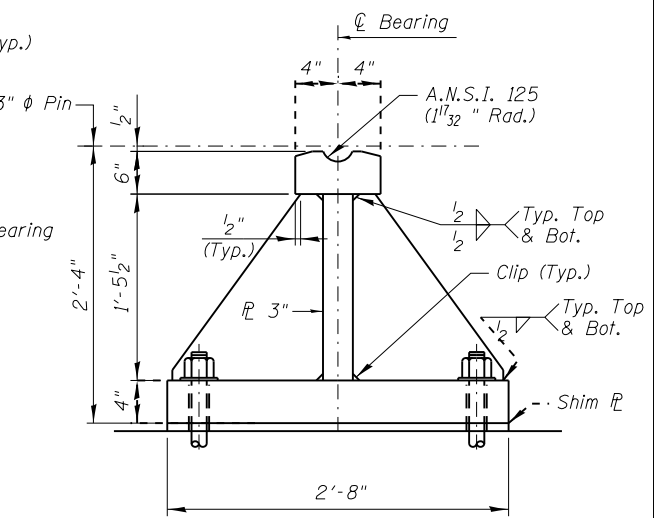
LOWER BEARING ASSEMBLY
(EXPANSION BEARING)



SIDE RETAINER DETAIL
(EXPANSION BEARING)



BOTTOM PLAN
(FIXED BEARING)



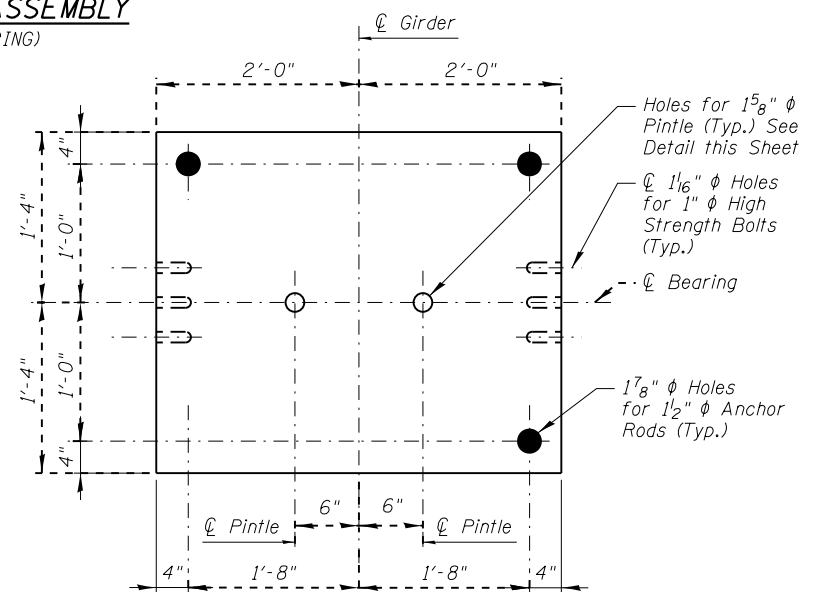
SECTION B-B
(FIXED BEARING)

BILL OF MATERIAL

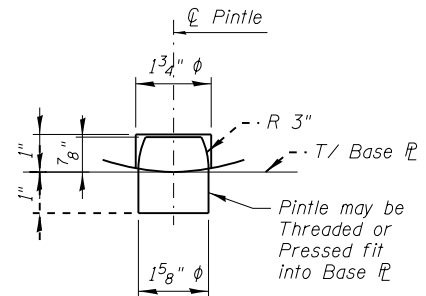
ITEM	UNIT	QUANTITY
Anchor Bolt 1 1/2"	Each	8
Anchor Bolt 2 1/2"	Each	8

NOTES :

1. For Section A-A and Upper Bearing Assembly, See Sheet No. 25.
2. For Additional Notes See Sheet No. 25.
3. Grease Bearing Assembly before Installation.
4. Grease Hole/Fitting shall be on street side of Bearing.
5. Bearing shall be blocked during construction.



BASE PLATE PLAN
(EXPANSION BEARING)



PINTLE DETAIL
(EXPANSION BEARING)

FILE NAME = ...04906202-60K00-024.dgn
HOH HARRY OLFERTER ASSOCIATES, INC.
 DESIGN AND CONSULTING ENGINEERS
 3366 55 East Jackson Blvd., Suite 600
 Chicago, Illinois 60604
 312/346-8131

USER NAME = roltean
 DESIGNED - MMH
 CHECKED - DNB
 DRAWN - R.VEJAR
 CHECKED - BCS
 PLOT SCALE = 24x0.0000 1' / in.
 PLOT DATE = 12/8/2016

DESIGNED - MMH
 CHECKED - DNB
 DRAWN - R.VEJAR
 CHECKED - BCS
 REVISED -
 REVISED -
 REVISED -
 REVISED -

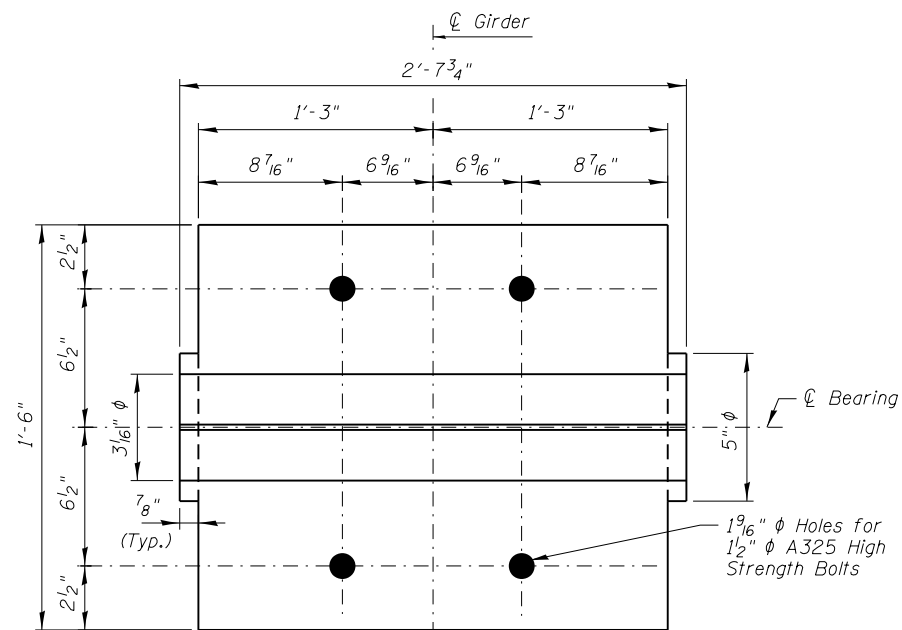
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

STEEL BEARING - 3 GIRDERS G2 & G3
STRUCTURE NO. 049-0602

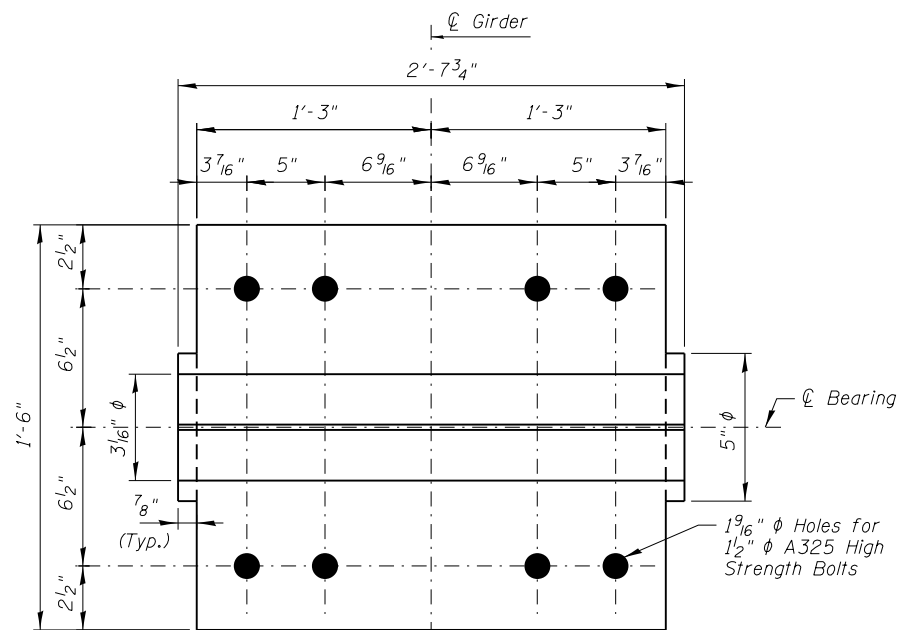
SHEET NO. 24 OF 43 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
346	125X-N&J-SB-B	LAKE	361	239
CONTRACT NO. 60K80				

ILLINOIS FED. AID PROJECT



**UPPER BEARING ASSEMBLY
BOTTOM VIEW
(EXPANSION BEARING)**



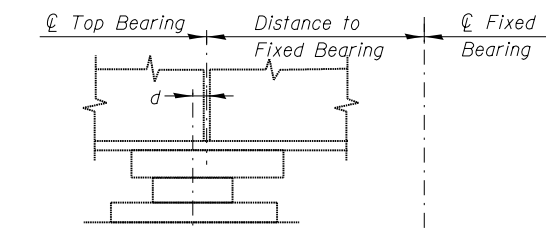
**UPPER BEARING ASSEMBLY
BOTTOM VIEW
(FIXED BEARING)**

NOTES :

1. Pin material must be ASTM A-668 Class F, harden to a Brinell Hardness of 200.
2. Material for bearing component parts must be ASTM A709, Grade 50, except as noted.
3. Bearing assembly weldments must be stress relieved by heat treating per AWS D1.1 prior to finish machining.
4. The parts of members in contact must be faced and bearing surfaces must be planed smooth before any welding is started.
5. Pintles must be stainless steel conforming to ASTM A276, Type 410 annealed.
6. Bearing pins or rockers and all surfaces in contact with pins or rockers must be given an all-over smooth finish.
7. Bearing seat surfaces will be constructed or adjusted to the designated elevations within a tolerance of 1/8\"/>
- 8. Shim plates will be placed between the base plate and the preformed fabric pad. Shim plates will match the footprint of the base plate. Heights of individual shim plates will be a minimum of 3/8\"/>
- 9. Concrete surface under bearing to be ground to a smooth level to give even bearing. Laminated fabric pads with 1/2\"/>
- 10. The cost of all steel weldments, pins, recessed nuts, cap washers and preformed fabric pads in the cost of "Steel Bearing Assembly".

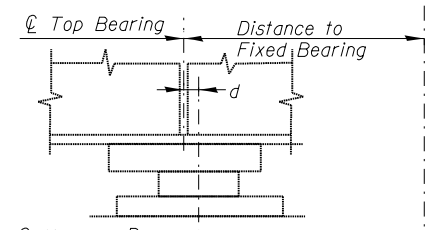
NOTE :

A.I.S.I. 1000 for Plates in Contract Unless noted Otherwise (typ.)



BELOW 50° F

(Move masonry line away from fixed bearing)

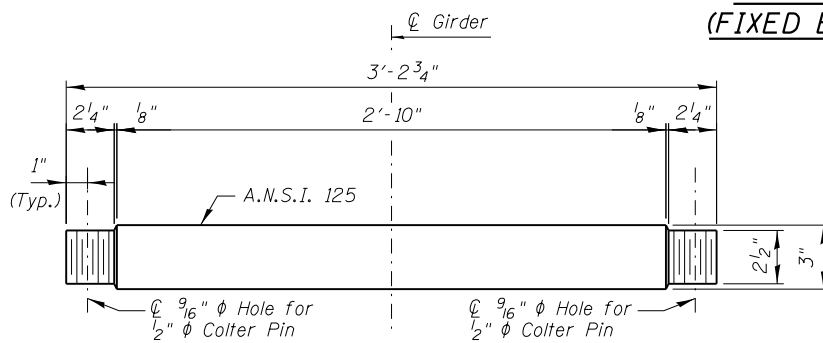


ABOVE 50° F

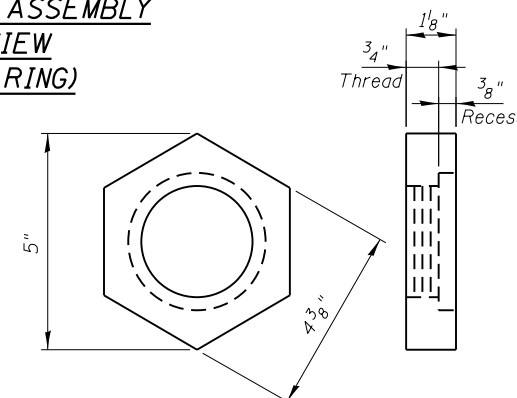
(Move masonry line toward fixed bearing)

SETTING ANCHOR BOLTS AT EXPANSION BEARINGS

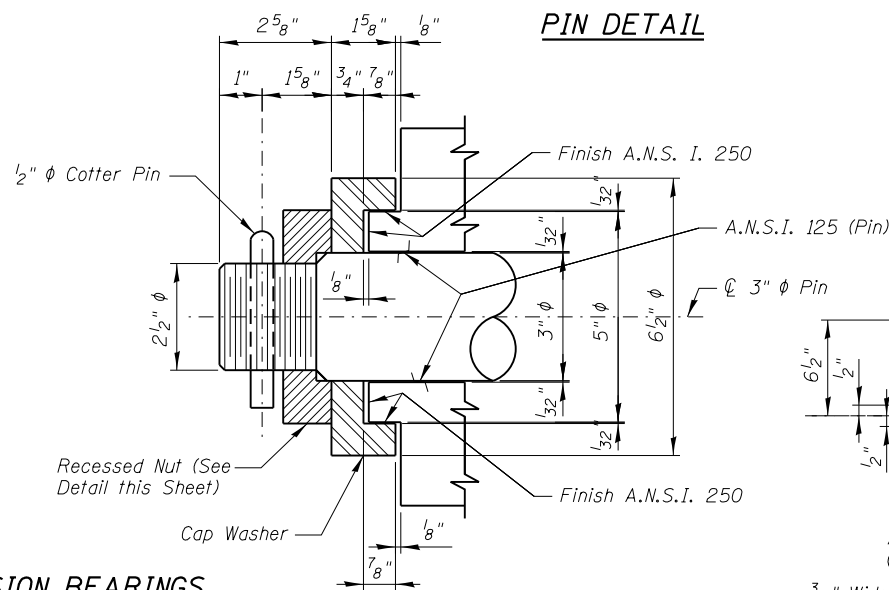
$d = \frac{1}{8}$ " per each 100' of expansion for every 15° F temperature change from the normal temperature of 50° F.



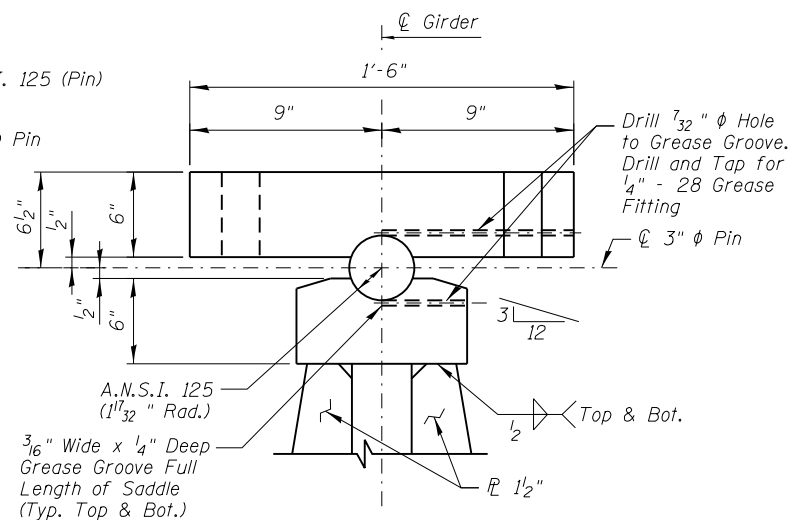
PIN DETAIL



RECESSED NUT DETAIL



PIN END DETAIL



SECTION A-A

BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Steel Bearing Assembly	Each	4

**ABUTMENT
BILL OF MATERIAL
(TWO ABUTMENTS)**

Bar	No.	Size	Length	Shape
h(E)	252	#6	28'-2"	—
h ₁ (E)	24	#6	10'-5"	—
h ₂ (E)	126	#6	18'-6"	└
h ₃ (E)	36	#7	28'-8"	—
h ₄ (E)	24	#6	10'-5"	—
h ₅ (E)	24	#6	6'-9"	—
n(E)	96	#10	13'-11"	└
n ₁ (E)	159	#6	8'-9"	└
n ₂ (E)	96	#10	12'-11"	└
n ₃ (E)	159	#6	7'-9"	└
t(E)	146	#10	22'-8"	—
t ₁ (E)	95	#9	22'-8"	—
t ₂ (E)	145	#10	19'-8"	—
t ₃ (E)	80	#9	19'-8"	—
u(E)	56	#6	13'-5"	└
u ₁ (E)	24	#6	14'-3"	└
u ₂ (E)	16	#6	15'-4"	└
v(E)	96	#10	13'-5"	—
v ₁ (E)	114	#6	13'-5"	—
v ₂ (E)	264	#6	10'-8"	└
v ₃ (E)	216	#6	12'-8"	└
v ₄ (E)	45	#6	20'-4"	└
v ₅ (E)	192	#6	14'-2"	└
v ₆ (E)	45	#6	20'-8"	└
v ₇ (E)	96	#10	14'-0"	—
v ₈ (E)	114	#6	14'-0"	—
w(E)	258	#6	28'-2"	—
Structure Excavation			Cu. Yd.	2,305
Concrete Structures			Cu. Yd.	1,367
Reinforcement Bars			Pound	115,632
Epoxy Coated				
Furnishing Metal Shell Piles, 14"φx0.312" Walls			Foot	5,760
Driving Piles			Foot	5,760
Test Pile Metal Shells			Each	2
Concrete Sealer			S.F.	4710
Asphalt Waterproofing			Sq. Yd.	366

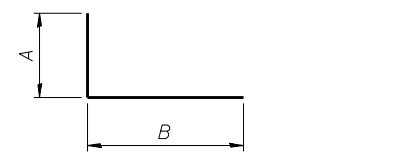
**BAR n(E), n₁(E)
n₂(E), & n₃(E)**

BAR DIMENSIONS

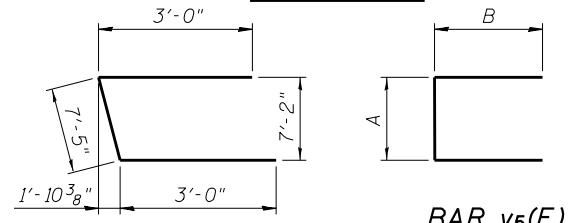
Bar	A	B
h ₂ (E)	4'-6"	14'-0"
v ₂ (E)	9'-0"	1'-8"
v ₃ (E)	11'-0"	1'-8"
v ₅ (E)	7'-2"	3'-6"
u ₁ (E)	4'-3"	5'-0"
u ₂ (E)	5'-4"	5'-0"

NOTES:

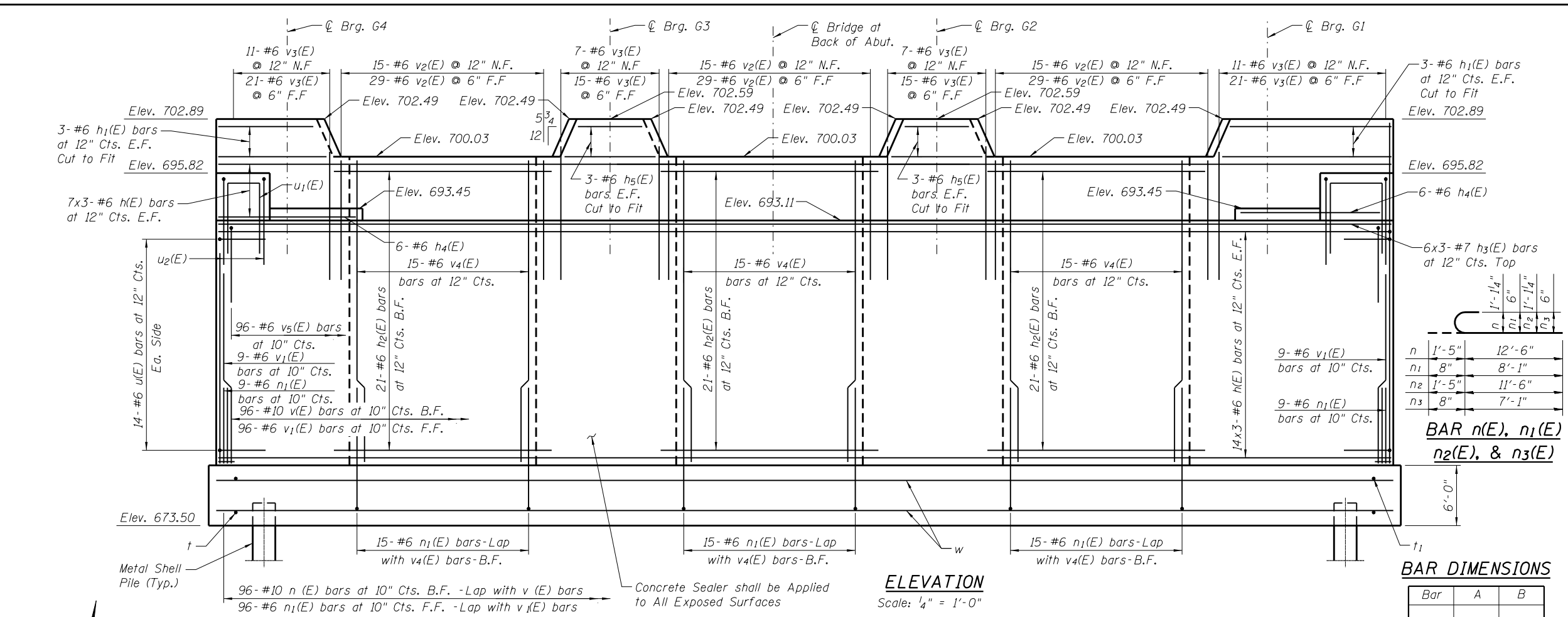
1. Bars designated (E) shall be Epoxy Coated.
2. Bars indicated thus 1x3-#5 Etc. indicates 1 line of bar with 3 lengths.



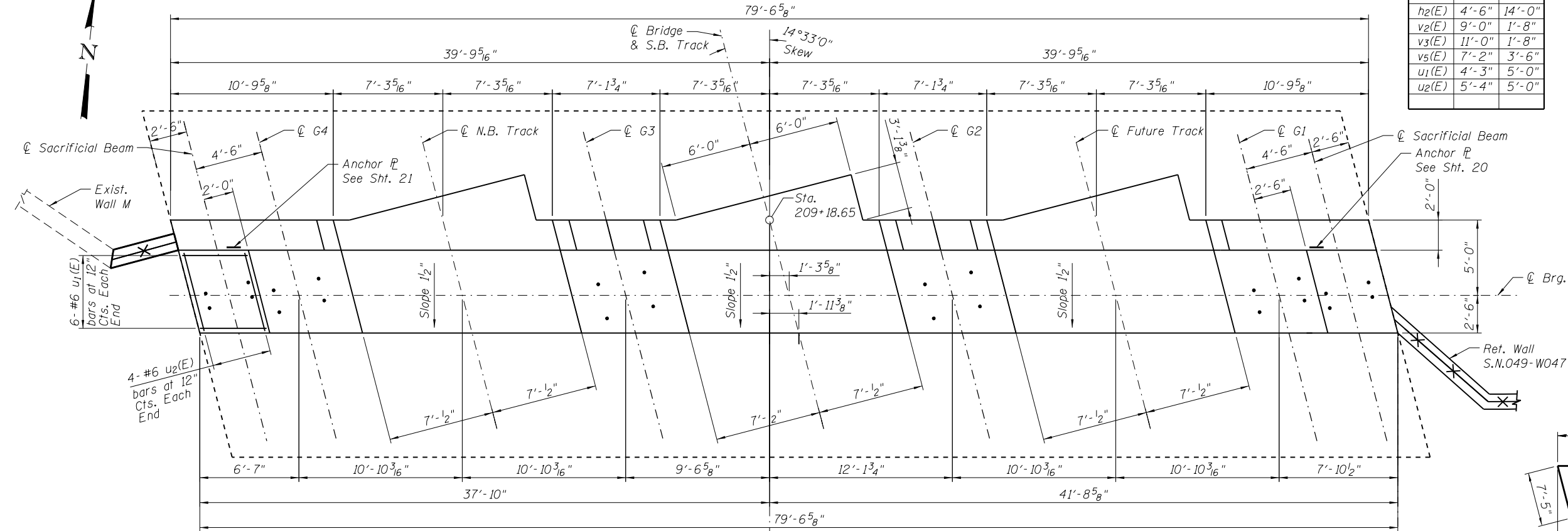
**BAR h₂(E),
v₂(E) & v₃(E)**



**BAR v₅(E),
u₁(E) & u₂(E)**



ELEVATION
Scale: 1/4" = 1'-0"



TOP VIEW
Scale: 1/4" = 1'-0"

FILE NAME: ...04906202-60K00-026.dgn
HOH HARRY OLFETER ASSOCIATES, INC.
 DESIGN AND CONSULTING ENGINEERS
 3366 55 East Jackson Blvd., Suite 600
 Chicago, Illinois 60604
 312/246-8931

USER NAME: raltean
 DESIGNED: MMH
 CHECKED: DNB
 DRAWN: R.V.EJAR
 CHECKED: BCS
 PLOT SCALE: 8:0.0000 1" / in.
 PLOT DATE: 12/8/2016

DESIGNED: MMH
 CHECKED: DNB
 DRAWN: R.V.EJAR
 CHECKED: BCS
 REVISED: -
 REVISED: -
 REVISED: -
 REVISED: -

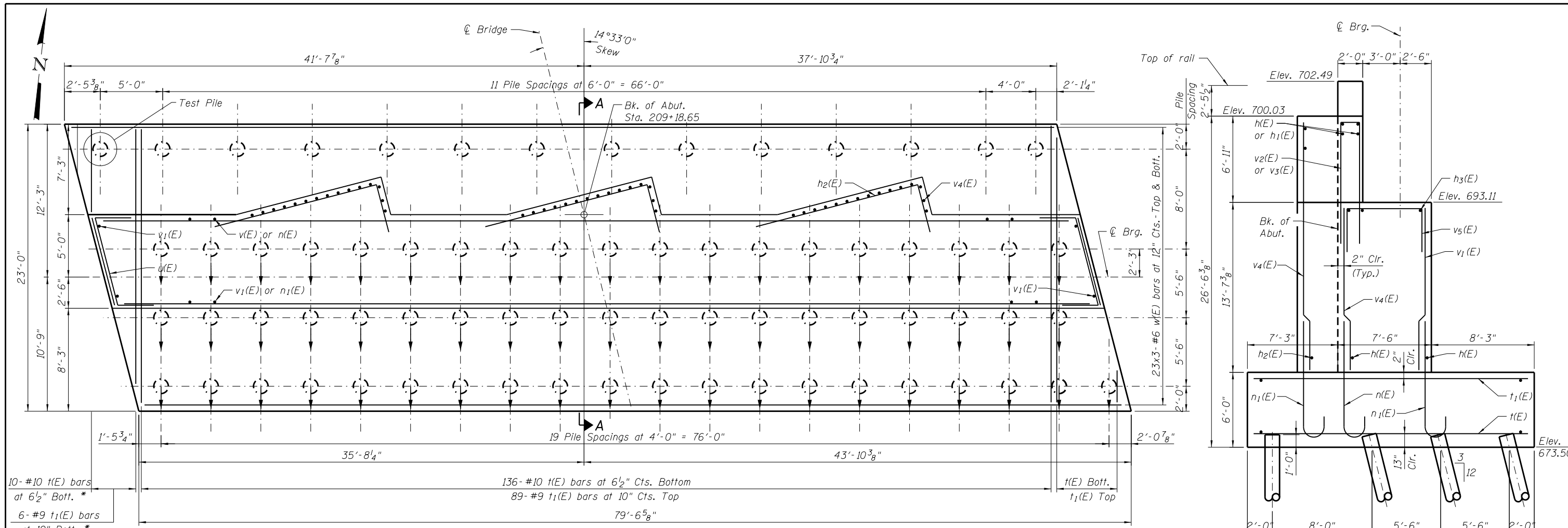
**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**NORTH ABUTMENT - 1
STRUCTURE NO. 049-0602**

SHEET NO. 26 OF 43 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
346	125X-N&J-SB-B	LAKE	361	241

CONTRACT NO. 60K80
ILLINOIS FED. AID PROJECT



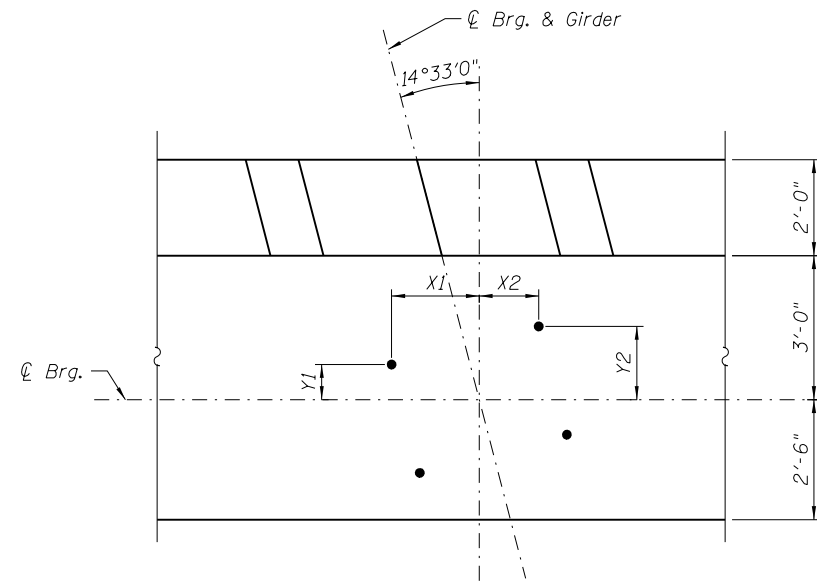
FOUNDATION PLAN
Scale: 1/4" = 1'-0"

SECTION A-A
Scale: 1/4" = 1'-0"

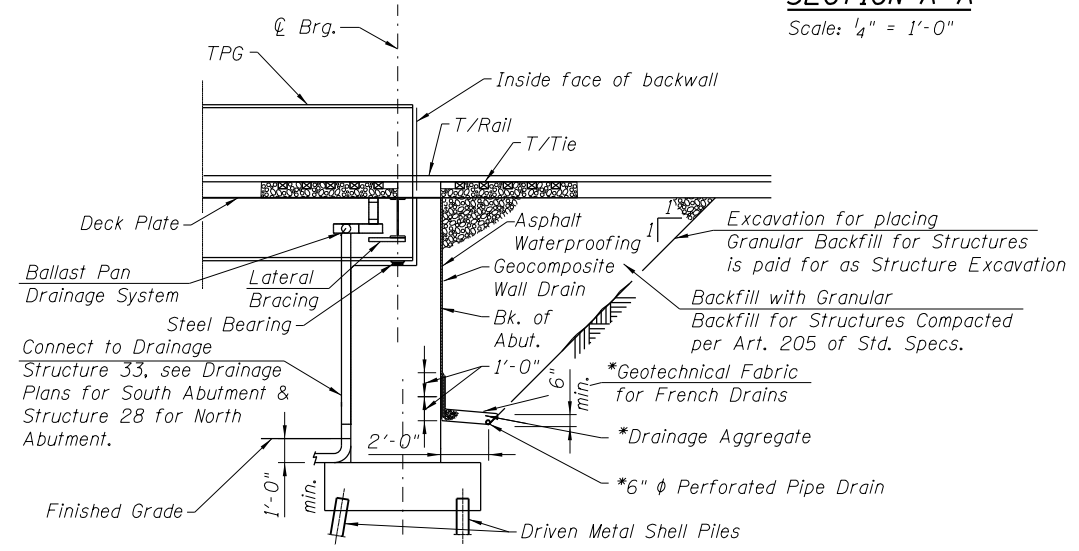
* Order t(E) and t1(E) bars full length. Cut to fit skew and use remainder of bars in opposite corners.

ANCHOR ROD LOCATION DIMENSION TABLE

Location	X1	Y1	X2	Y2
G1 & G4-S. Abut.	1'-8 3/8"	4 1/8"	1'-3 5/8"	1'-1 3/8"
G1 & G4-N. Abut.	1'-8 3/8"	4 1/8"	1'-3 5/8"	1'-1 3/8"
G2 & G3-S. Abut.	1'-10 3/8"	6 5/8"	1'-4 3/8"	1'-4 5/8"
G2 & G3-N. Abut.	1'-9 15/16"	8 3/4"	1'-2 7/8"	1'-6 5/16"
Sacrificial Beam	1'-6 7/16"	1 7/16"	1'-3 7/16"	10 3/16"

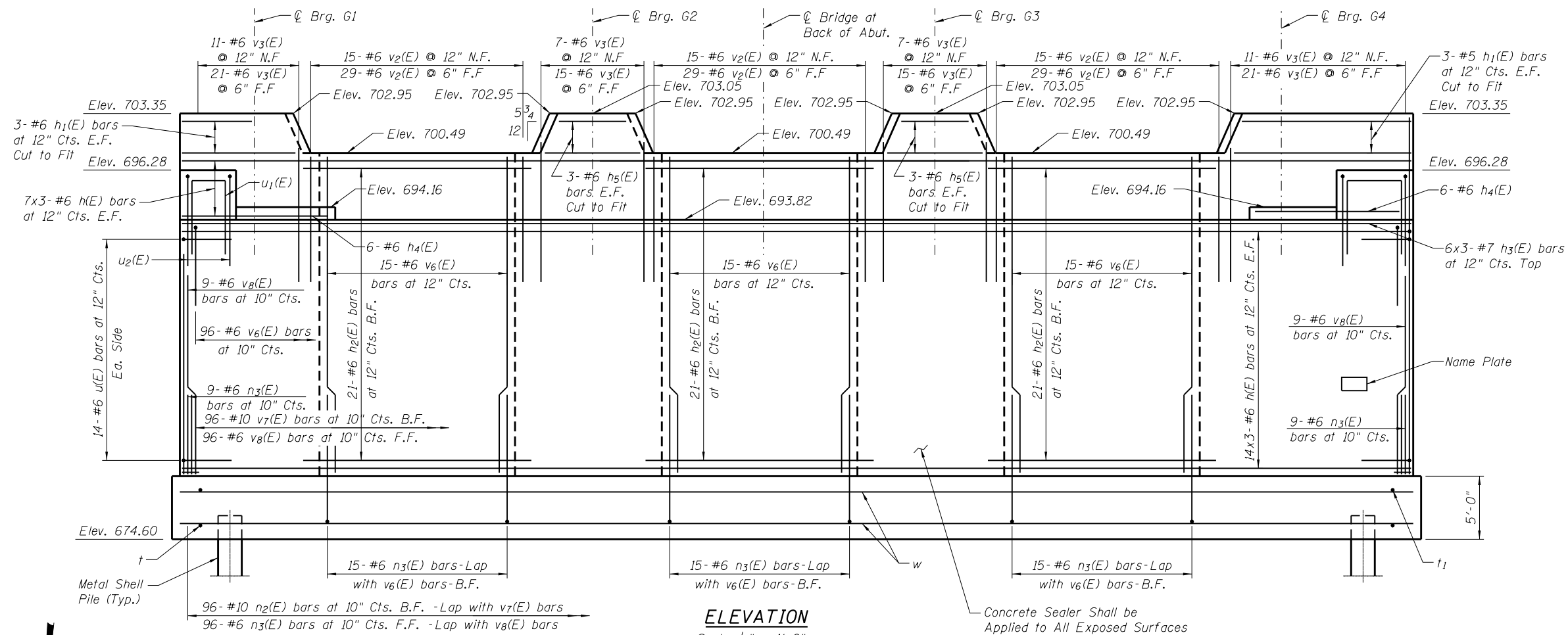


ANCHOR BOLT LOCATION PLAN
N.T.S.

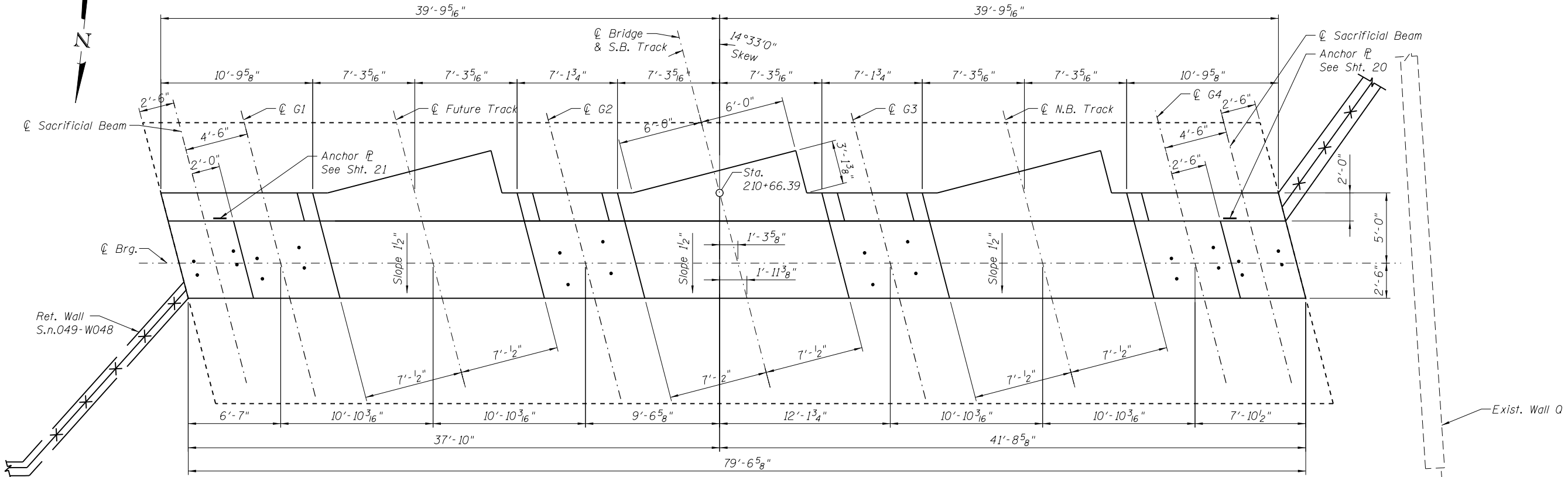


SECTION A-A ABUTMENT SECTION

* Included in cost of Pipe Underdrain for Structures.
Note: Backfill & drainage details are applicable to both abutments.



ELEVATION
Scale: 1/4" = 1'-0"



TOP VIEW
Scale: 1/4" = 1'-0"

FILE NAME - ...04906202-60K00-028.dgn
HOH HARRY O. HEFTER ASSOCIATES, INC.
 DESIGN AND CONSULTING ENGINEERS
 3366 55 East Jackson Blvd., Suite 600
 Chicago, Illinois 60604
 312/346-8831

USER NAME = aefitzpatrick
 DESIGNED - MMH
 CHECKED - DNB
 DRAWN - R.VEJAR
 CHECKED - BCS
 PLOT SCALE = 8:0.0000 1' = 1/8" in.
 PLOT DATE = 10/7/2016

DESIGNED - MMH
 CHECKED - DNB
 DRAWN - R.VEJAR
 CHECKED - BCS

REVISED -
 REVISED -
 REVISED -
 REVISED -

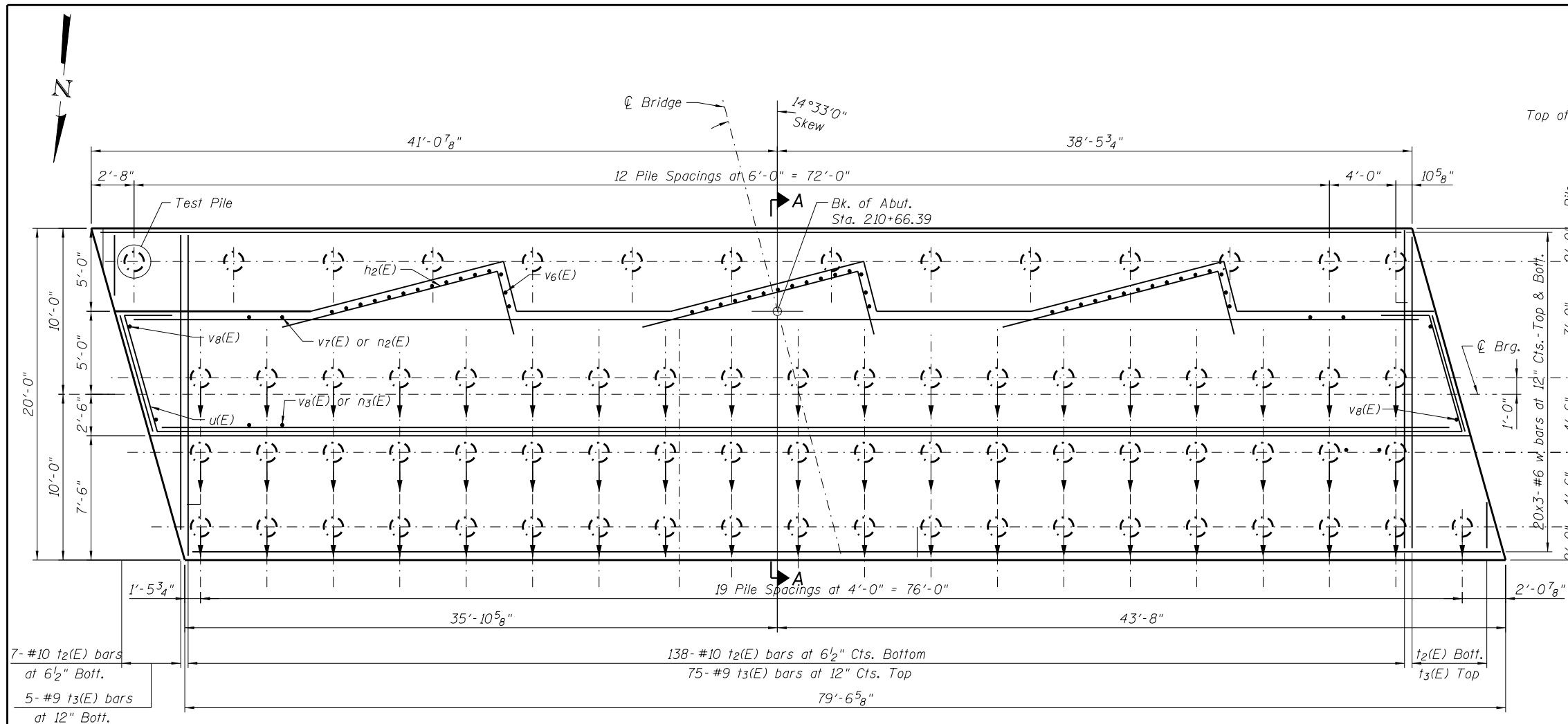
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SOUTH ABUTMENT - 1
STRUCTURE NO. 049-0602

SHEET NO. 28 OF 43 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
346	125X-N&J-SB-B	LAKE	361	243
CONTRACT NO. 60K80				

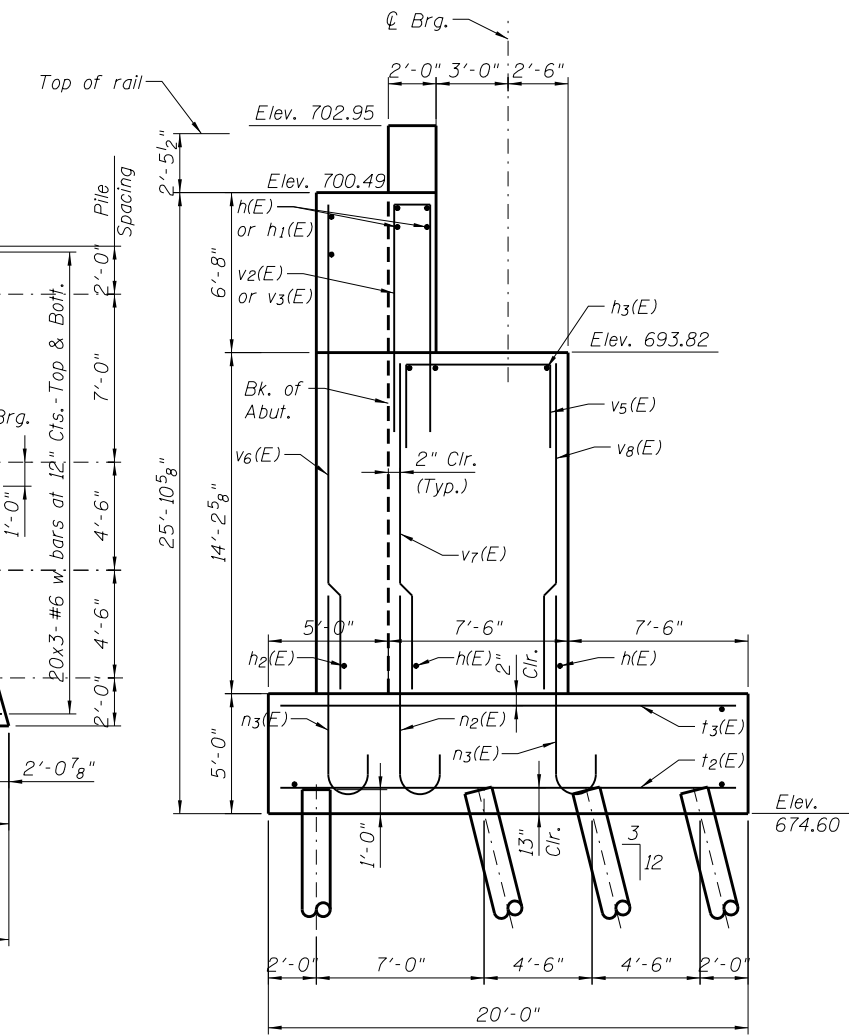
ILLINOIS FED. AID PROJECT



FOUNDATION PLAN

Scale: 1/4" = 1'-0"

* Order t(E) and t1(E) bars full length.
Cut to fit skew and use remainder
of bars in opposite corners.

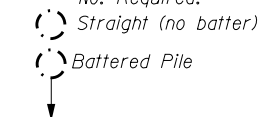


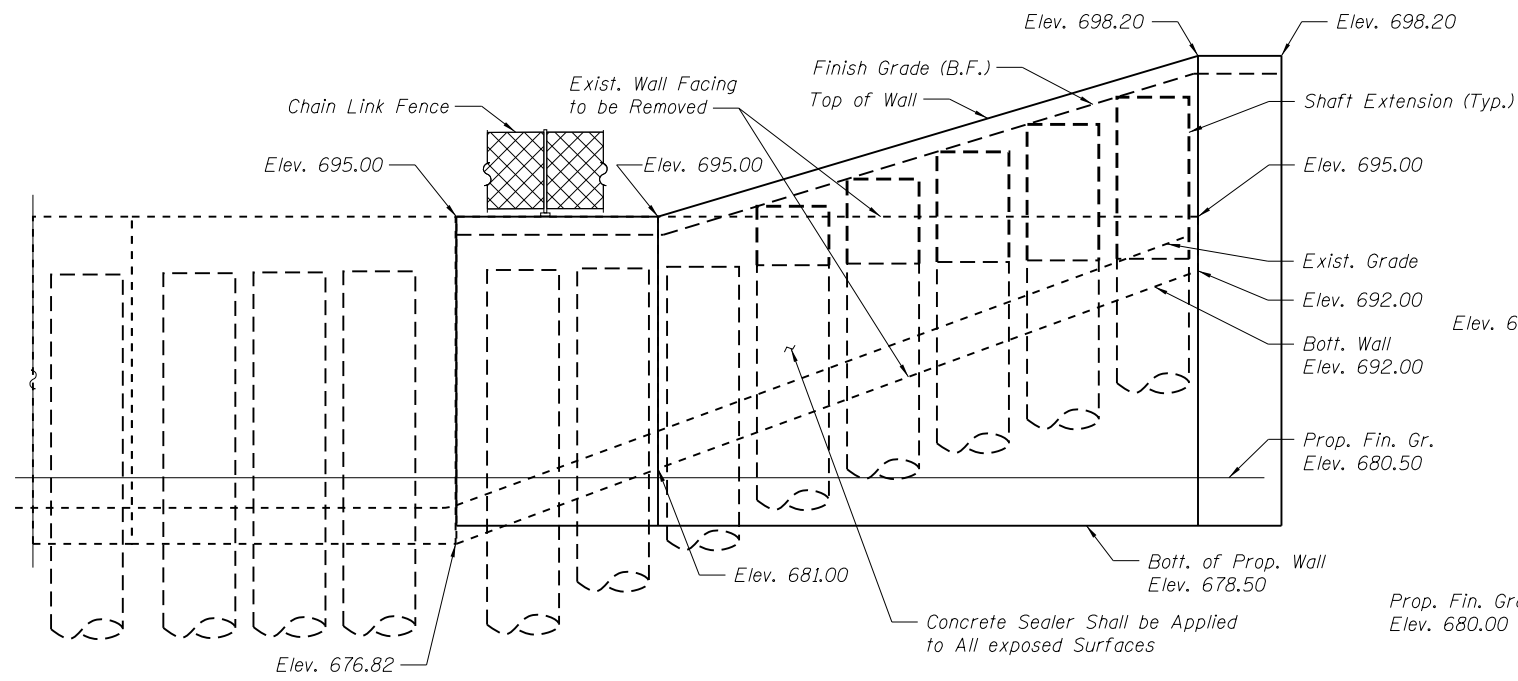
SECTION A-A

Scale: 1/4" = 1'-0"

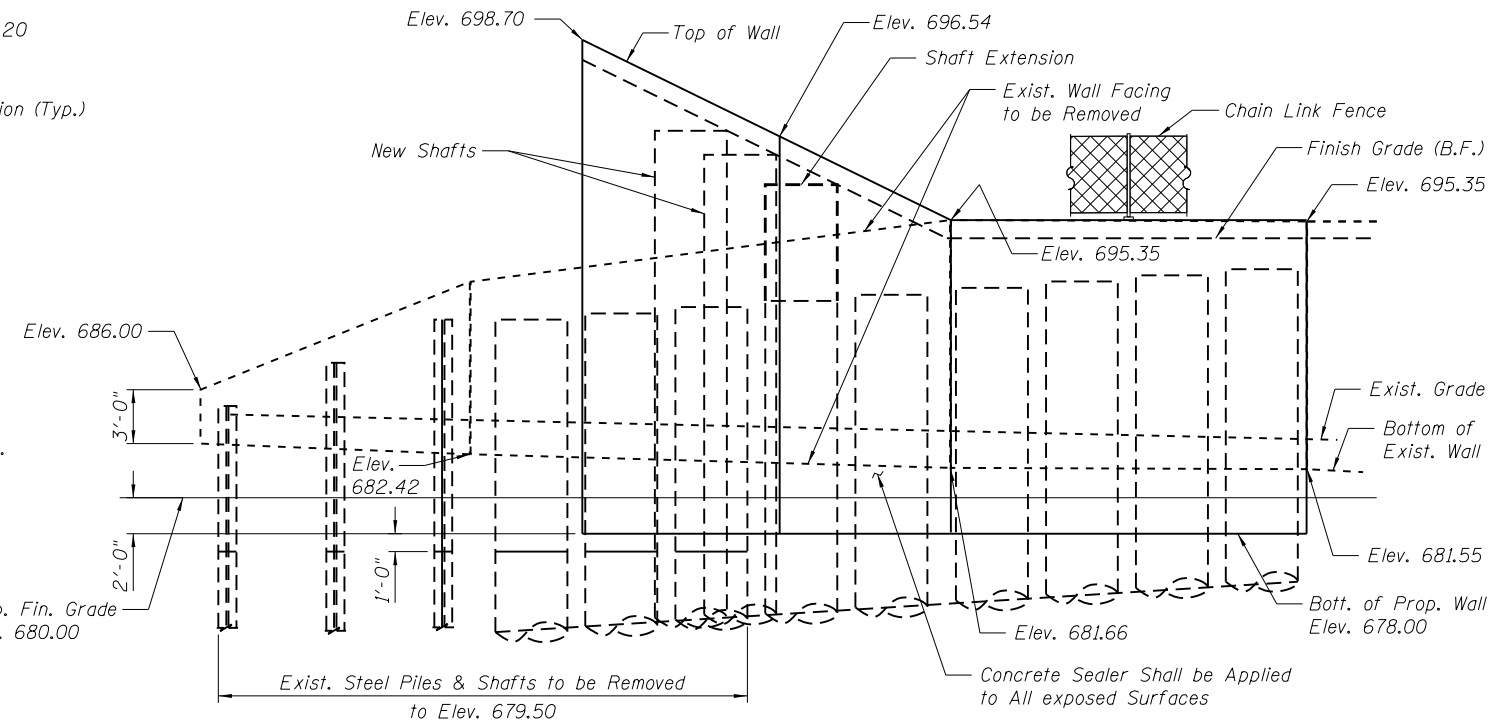
Pile Data for North and South Abutments

Type: Metal Shell 14" dia. x .312 walls with pile shoes
 Nominal Required Bearing: 516 Kips
 Allowable Resistance Available: 172 Kips
 Est. Length: 40 feet (vertical)
 No. Required: 144 (includes 2 test piles)

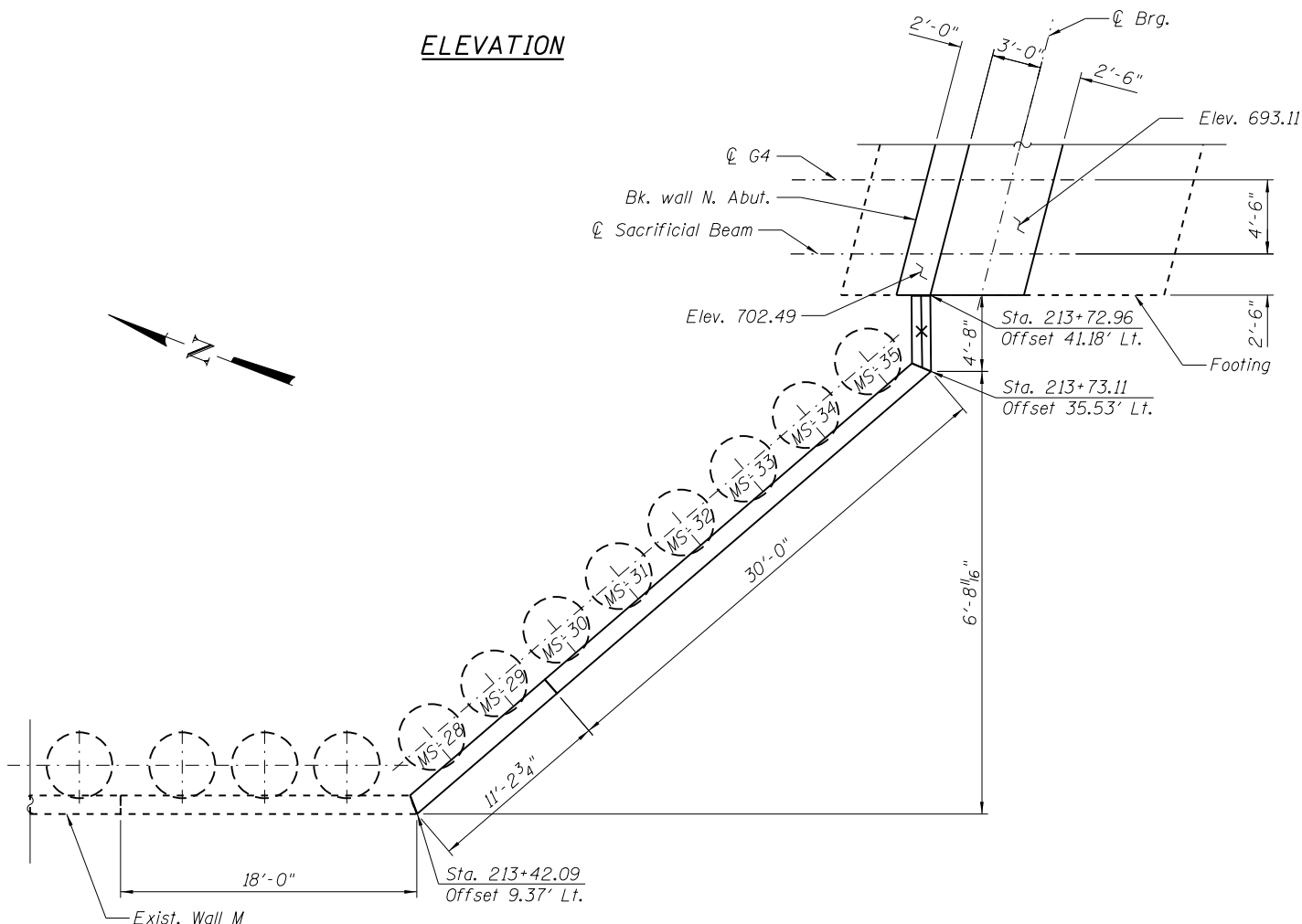




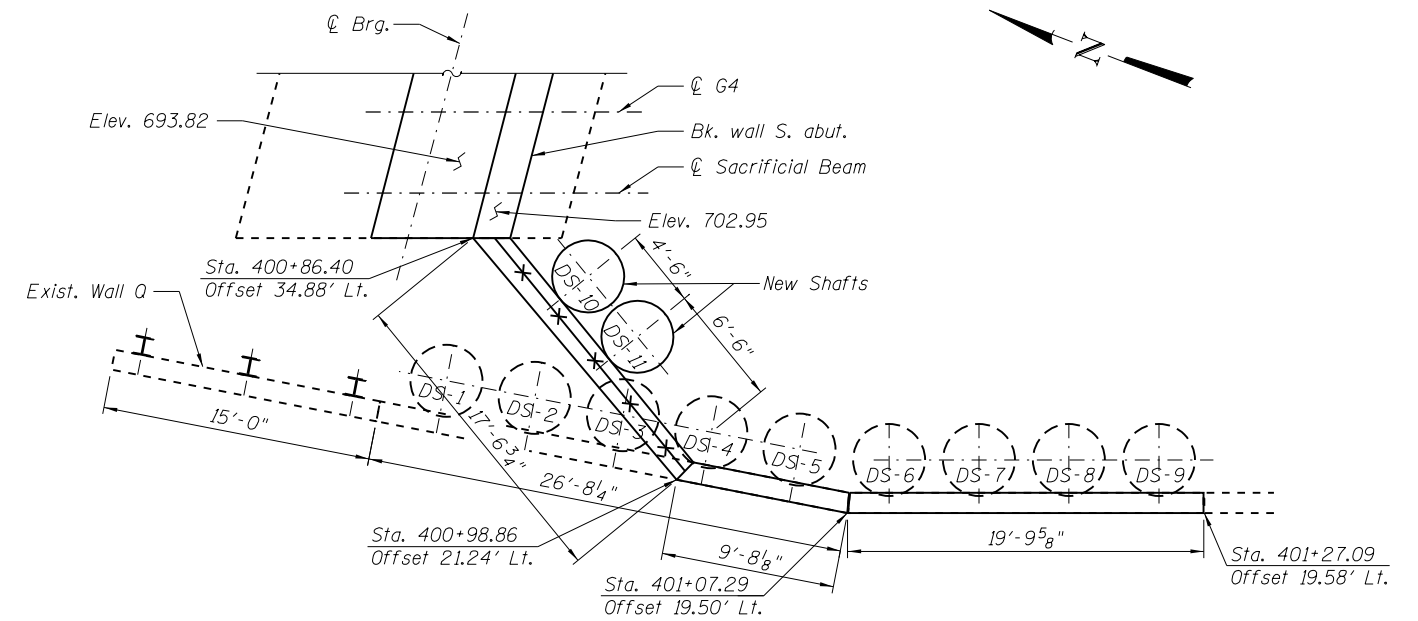
ELEVATION



ELEVATION



PLAN
N. Abutment



PLAN
S. Abutment

FILE NAME = ...04906202-60K00-030.dgn	USER NAME = aefitzpatrick	DESIGNED - MMH	REVISED -
HOH	HARRY O. HEFTER ASSOCIATES, INC. DESIGN AND CONSULTING ENGINEERS	CHECKED - DNB	REVISED -
3366	55 East Jackson Blvd., Suite 600 Chicago, Illinois 60604 312/346-9831	DRAWN - R.VEJAR	REVISED -
		CHECKED - BCS	REVISED -

PLOT SCALE = 8:0.0000 '1' / in.	
PLOT DATE = 10/7/2016	

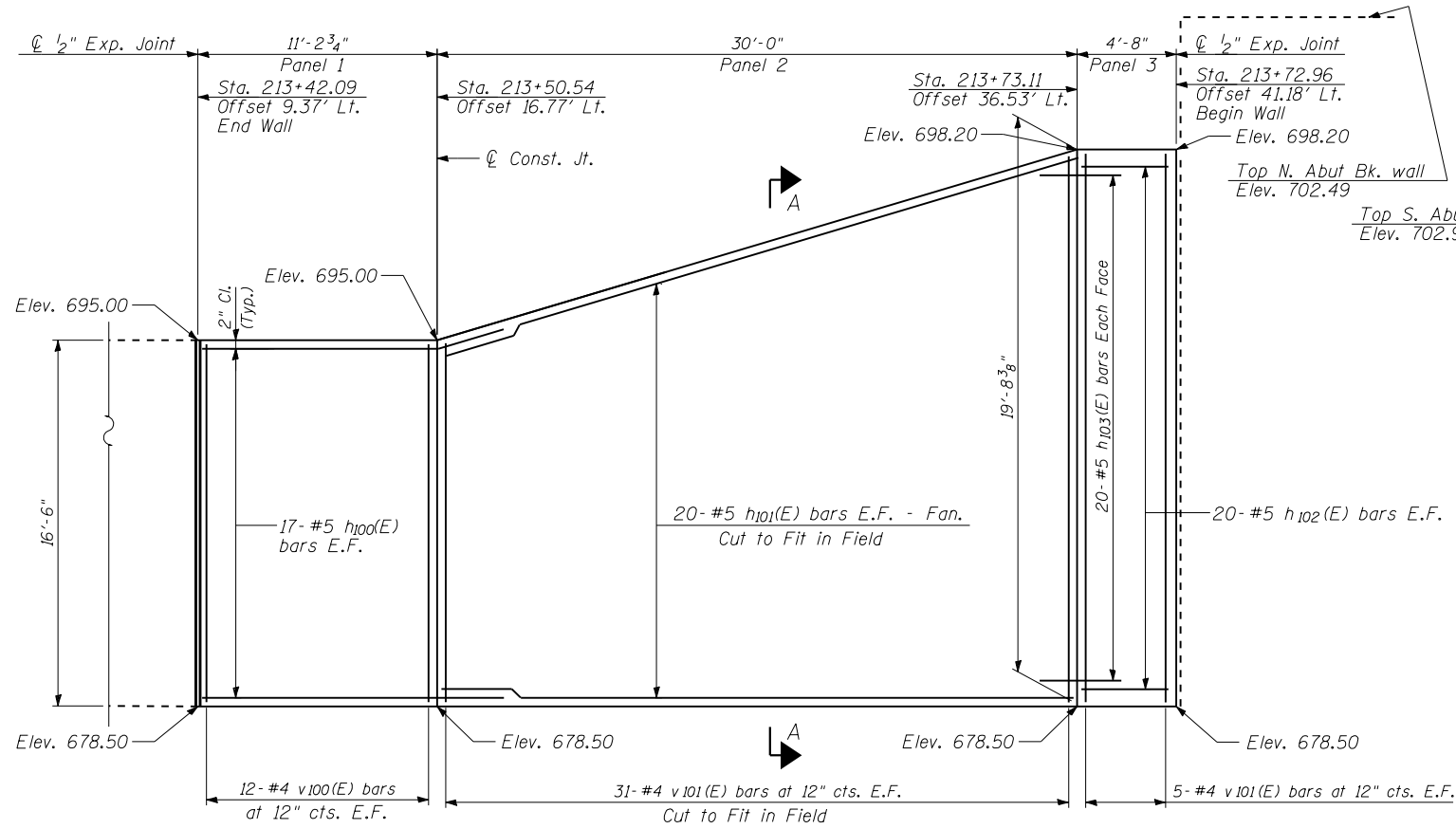
**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**WEST WING WALLS - GENERAL PLAN AND ELEVATION
STRUCTURE NO. 049-0602**

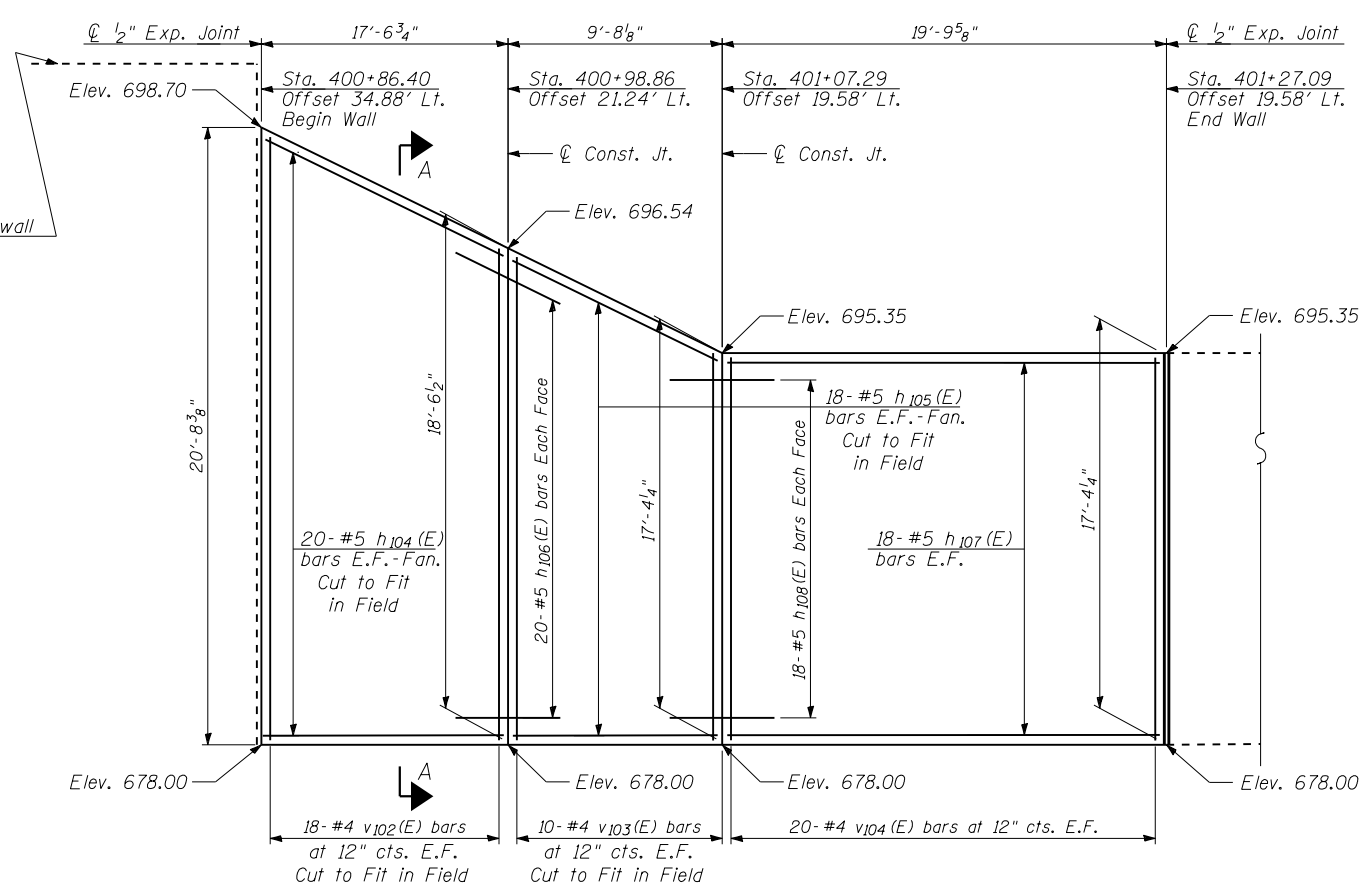
SHEET NO. 30 OF 43 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
346	125X-N&J-SB-B	LAKE	361	245
CONTRACT NO. 60K80				

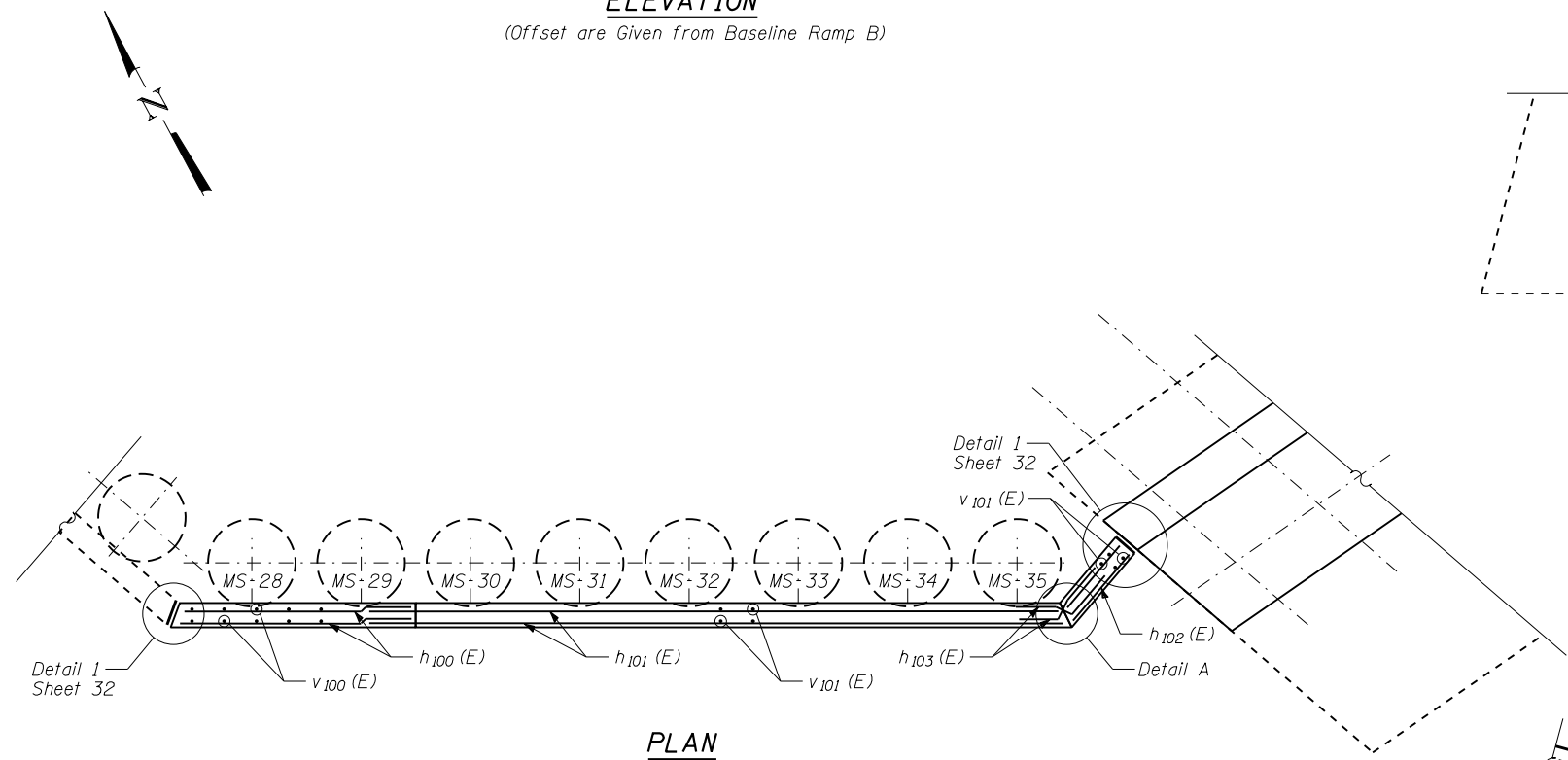
ILLINOIS FED. AID PROJECT



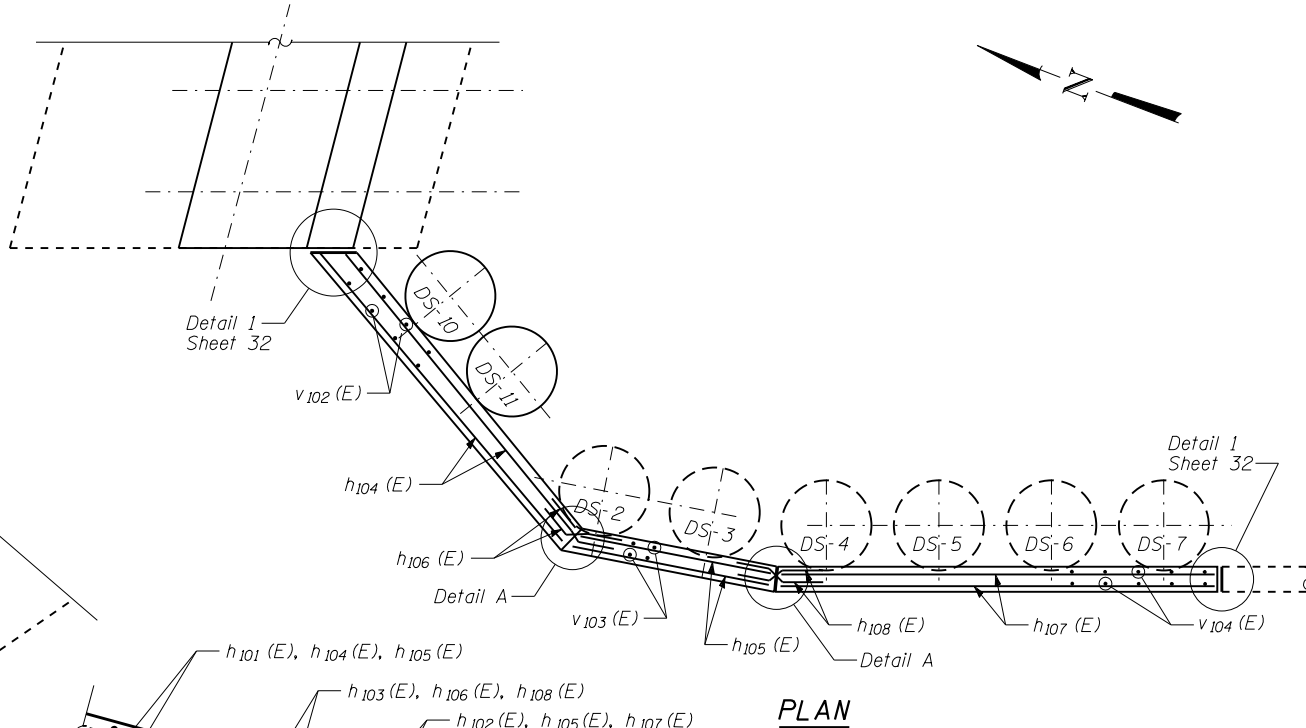
ELEVATION
(Offset are Given from Baseline Ramp B)



ELEVATION
(Offset are Given from Baseline Ramp D)



PLAN



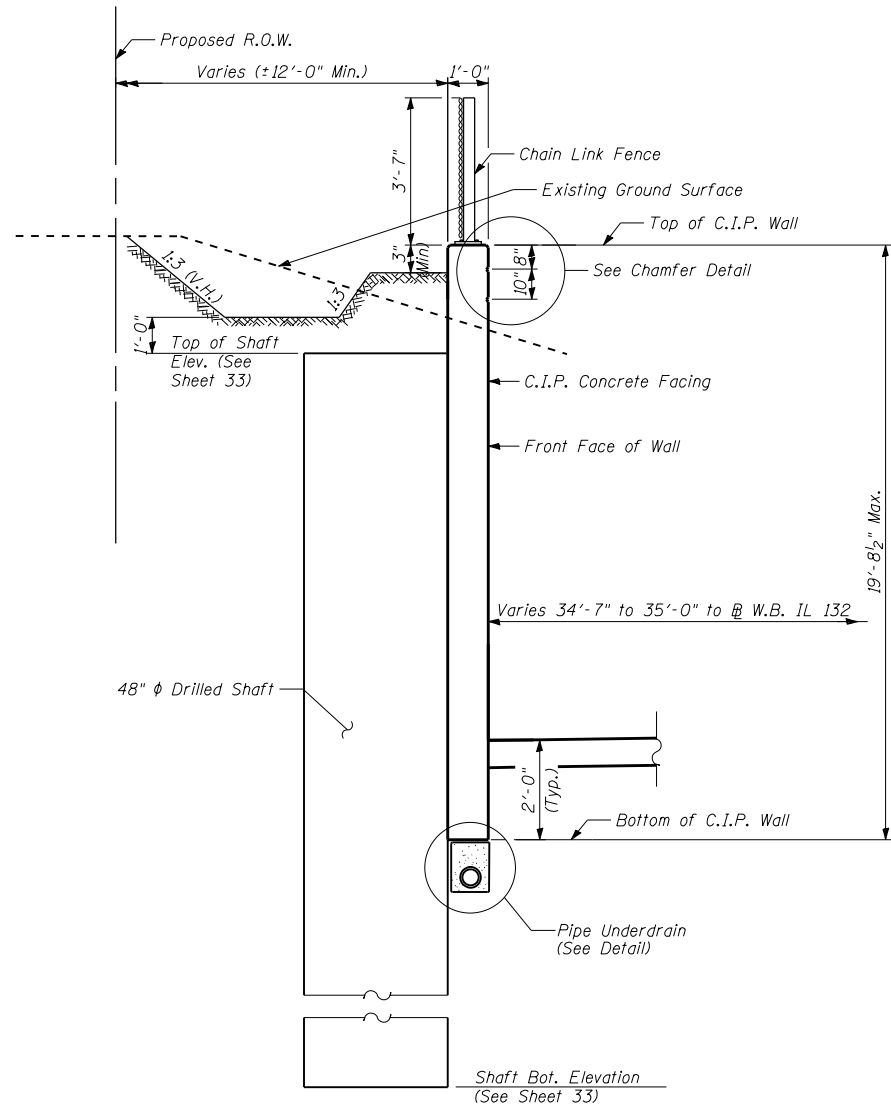
PLAN



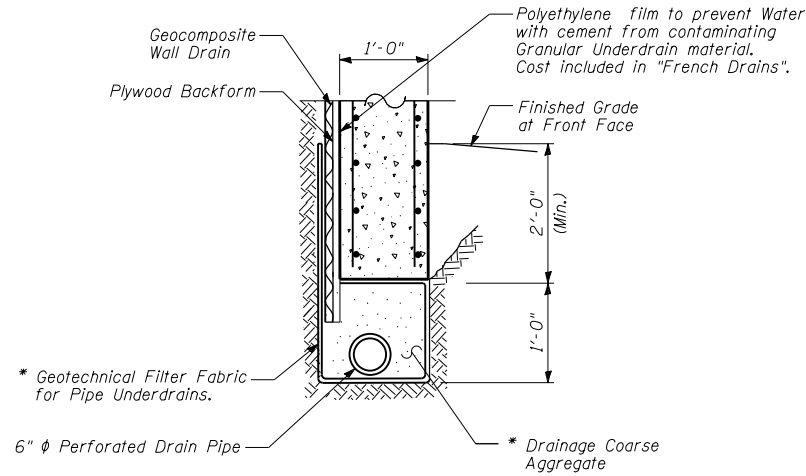
DETAIL A
BEND IN WALL DETAIL

- NOTES**
1. B.F. - denotes Back Face.
 2. E.F. - denotes Each Face.
 3. F.F. - denotes Front Face.
 4. For Section A-A, see Sheet 32.

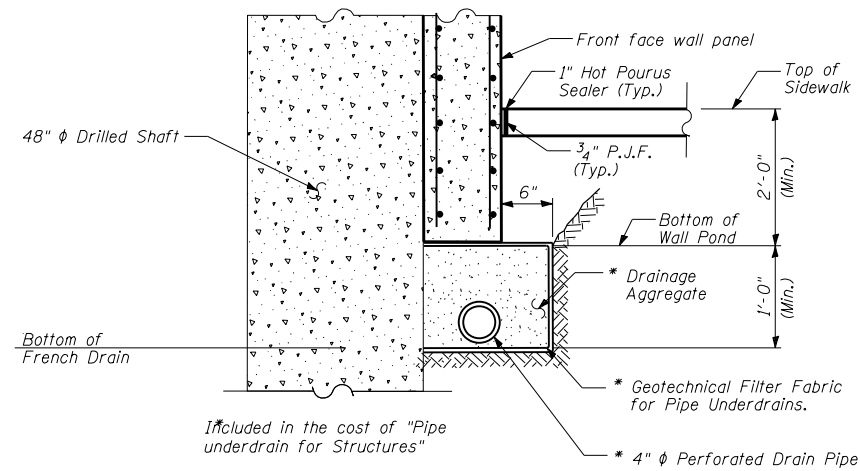
HOH HARRY O. HEFTER ASSOCIATES, INC. DESIGN AND CONSULTING ENGINEERS 3366 55 East Jackson Blvd., Suite 600 Chicago, Illinois 60604 312/346-8931	USER NAME = aefitzpatrick PLOT SCALE = 5:4.0000 '1' / 1in. PLOT DATE = 10/7/2016	DESIGNED - MMH CHECKED - DNB DRAWN - R.VEJAR CHECKED - BCS	REVISED - REVISED - REVISED - REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	WEST WING WALL REINFORCING DETAILS STRUCTURE NO. 049-0602 SHEET NO. 31 OF 43 SHEETS	F.A.P. R.T.E. = 346 SECTION = 125X-N&J-SB-B COUNTY = LAKE TOTAL SHEETS = 361 SHEET NO. = 246 CONTRACT NO. 60K80	ILLINOIS FED. AID PROJECT



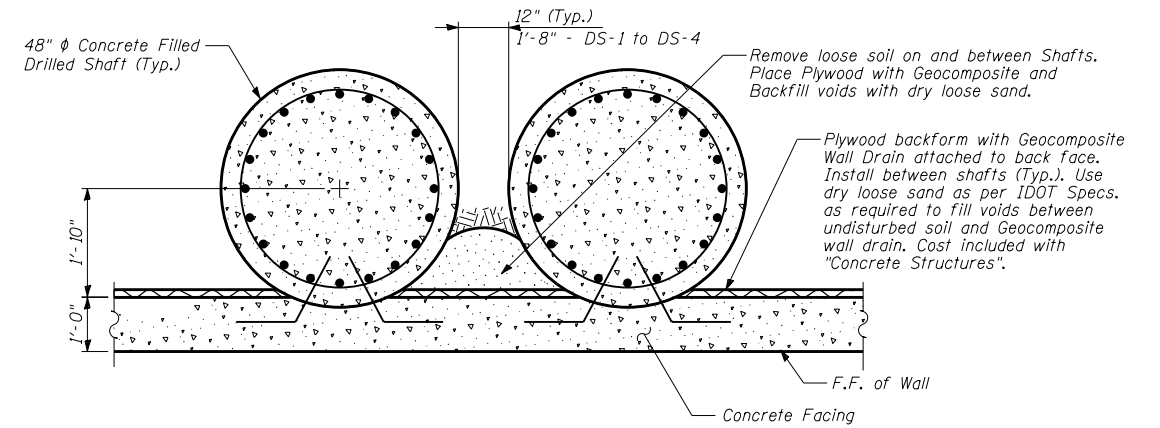
SECTION A-A
(Tangent Pile Wall)



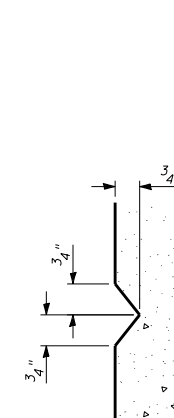
PIPE UNDERDRAIN DETAIL
BETWEEN DRILLED SHAFTS



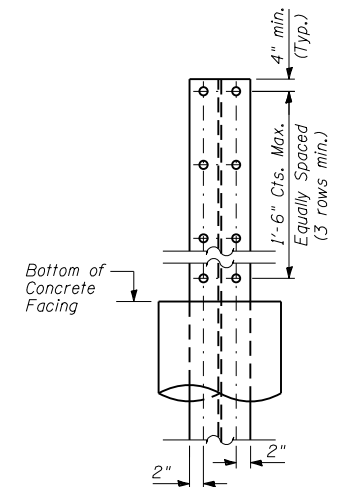
PIPE UNDERDRAIN DETAIL
AT DRILLED SHAFT



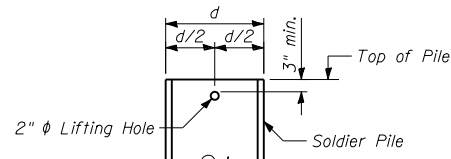
TYPICAL SECTION THRU
TANGENT PILE WALL



CHAMFER DETAIL
Cost of Chamfer included with "Concrete Structures"

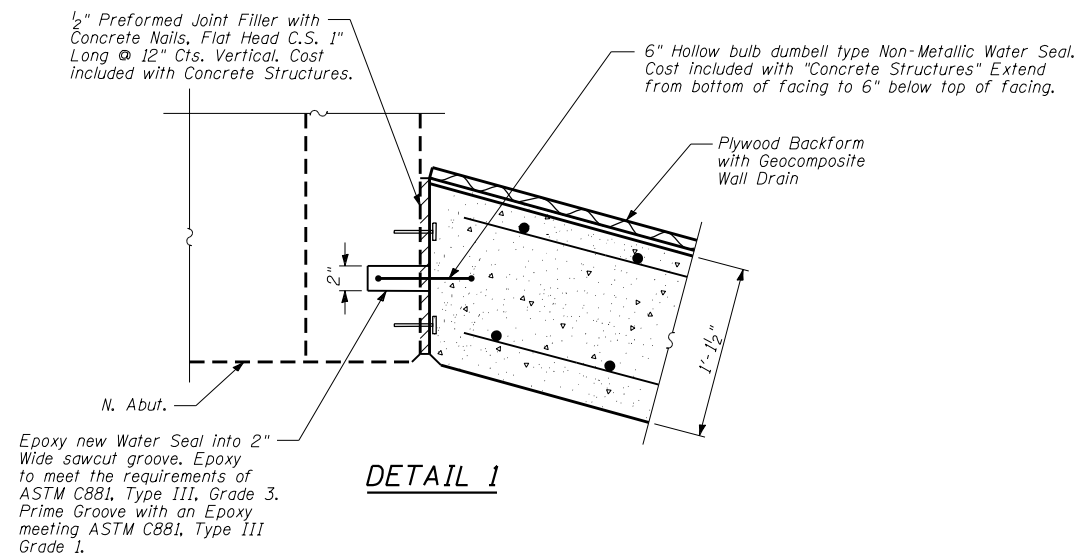


SHEAR STUD CONNECTOR DETAIL

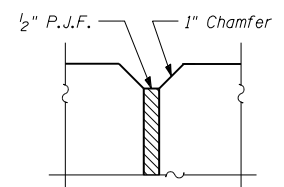


LIFTING HOLE DETAIL

Lifting hole to be provided if necessary. Cost included with "Furnishing Soldier Piles (W Section)"



DETAIL 1



DETAIL X

BILL OF MATERIAL

ITEM	UNIT	TOTAL
Geocomposite Wall Drain	Sq. Yd.	460
Pipe Underdrains for Structures 6"	Foot	220

NOTES:

- The Geocomposite Wall Drain shall be constructed according to Section 591 of the Standard Specifications.

BAR LIST AND BILL OF MATERIAL

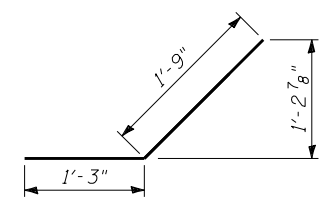
Bar	Number	Size	Length	Shape
d ₁₀₀ (E)	844	#5	3'-0"	└
#4		#4	1'-8"	
#5		#5	2'-2"	
#6		#6	2'-7"	
#8		#8	4'-6"	
#11		#11	9'-0"	
h ₁₀₀ (E)	34	#5	13'-0"	—
h ₁₀₁ (E)	40	#5	29'-8"	—
h ₁₀₂ (E)	40	#5	4'-3"	—
h ₁₀₃ (E)	40	#5	6'-2"	—
h ₁₀₄ (E)	40	#5	17'-3"	—
h ₁₀₅ (E)	36	#5	9'-4"	—
h ₁₀₆ (E)	40	#5	6'-2"	—
h ₁₀₇ (E)	36	#5	19'-5"	—
h ₁₀₈ (E)	36	#5	6'-2"	—
v ₁₀₀ (E)	24	#4	16'-2"	—
v ₁₀₁ (E)	72	#4	19'-4"	—
v ₁₀₂ (E)	36	#4	20'-4"	—
v ₁₀₃ (E)	20	#4	18'-2"	—
v ₁₀₄ (E)	40	#4	17'-0"	—
** SD ₁₀₀	2	#4	44'-6"	▩
S ₁₀₀	34	#4	10'-4"	○
v ₁₂₄	56	#11	44'-8"	—
v ₁₂₅	12	#8	3'-2"	—
v ₁₂₆	12	#8	3'-6"	—
v ₁₂₇	12	#8	3'-9"	—
v ₁₂₈	12	#8	4'-1"	—
v ₁₂₉	12	#8	4'-5"	—
Reinforcement Bars, Epoxy Coated	Pound		9,438	
Concrete Structures	Cu. Yd.		62	
Drilled Shaft in Soil	Cu. Yd.		49	
Reinforcement Bars	Pound		15,796	
Concrete Sealer	S.F.		1,900	

Minimum Lap for Spirals = 1 1/2 Turns
 ** Length is Height of Spiral.

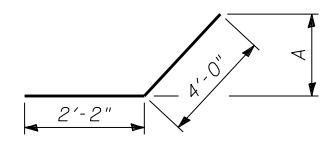
LAP SPLICES

Bar	Lap
#4	1'-8"
#5	2'-2"
#6	2'-7"
#8	4'-6"
#11	9'-0"

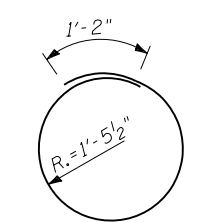
Bar	A
h ₁₀₃ (E)	3'-0"
h ₁₀₆ (E)	1'-6"
h ₁₀₈ (E)	0'-9"



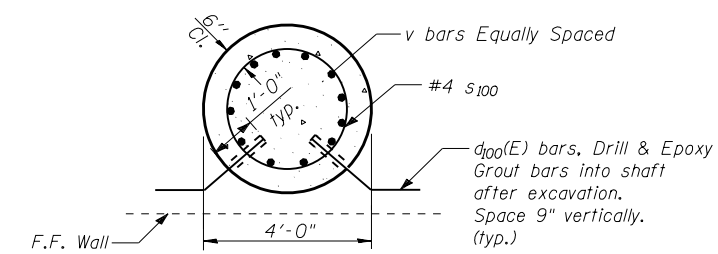
BAR d₁₀₀(E)



BAR h₁₀₃(E), h₁₀₆(E) & h₁₀₈(E)

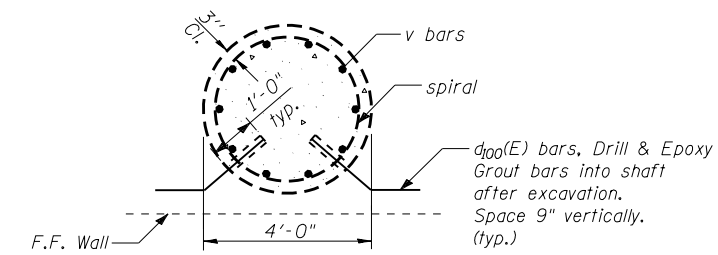


BAR s₁₀₀



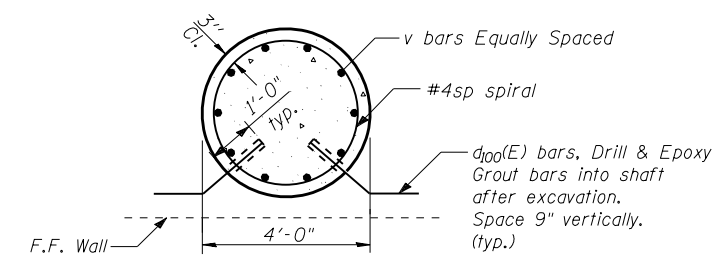
SECTION E-E

Drill & Epoxy Grout d₁₀₀(E) bars according to Article 584 of the Standard Specifications. The cost shall be included with "Reinforcement Bars, Epoxy Coated".



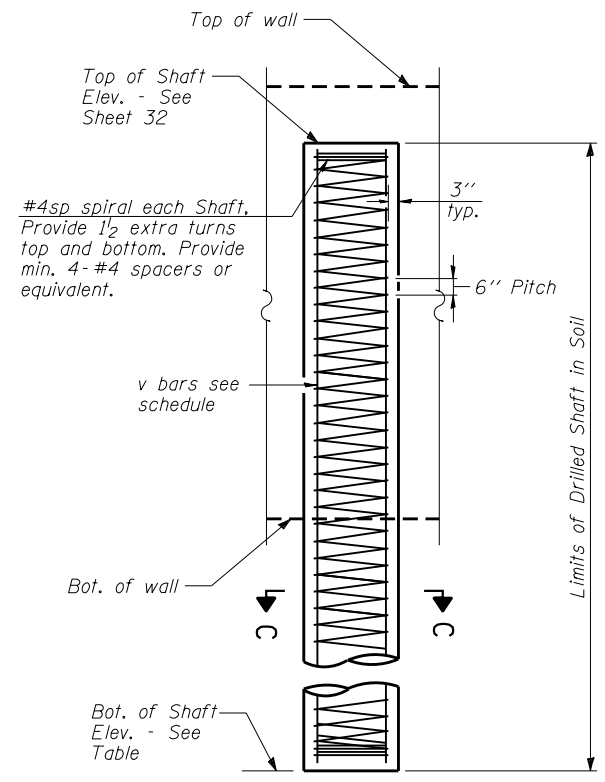
SECTION D-D

Drill & Epoxy Grout d₁₀₀(E) bars according to Article 584 of the Standard Specifications. The cost shall be included with "Reinforcement Bars, Epoxy Coated".

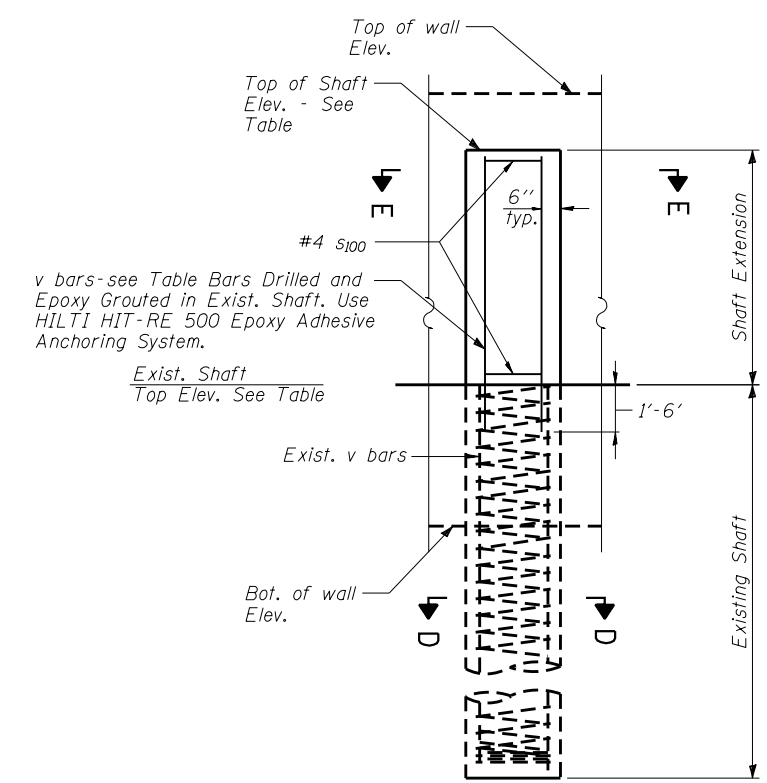


SECTION C-C

Drill & Epoxy Grout d₁₀₀(E) bars according to Article 584 of the Standard Specifications. The cost shall be included with "Reinforcement Bars, Epoxy Coated".



DRILLED SHAFT ELEVATION
Shafts DS-10 & DS-11



DRILLED SHAFT EXTENSION

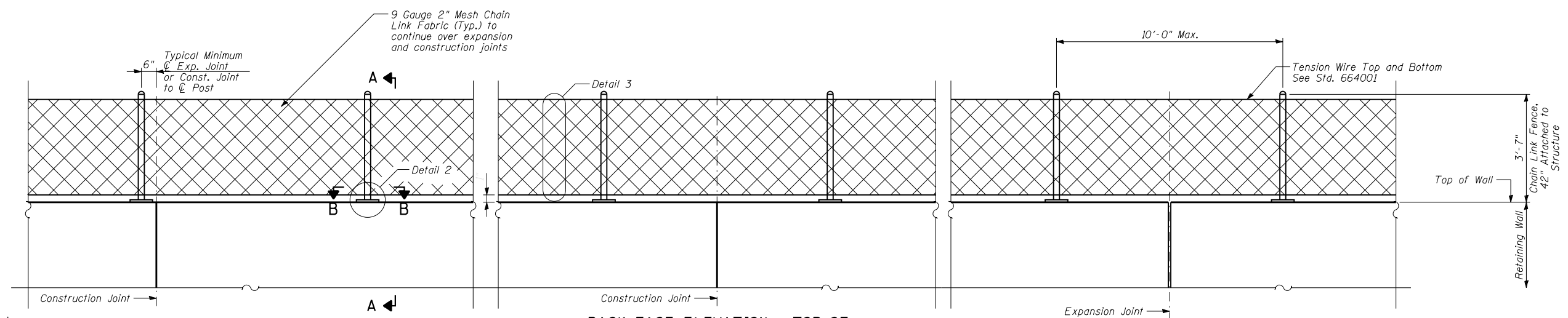
v bars see Table Bars Drilled and Epoxy Grouted in Exist. Shaft. Use HILTI HIT-RE 500 Epoxy Adhesive Anchoring System.

EXISTING SHAFT EXTENSION

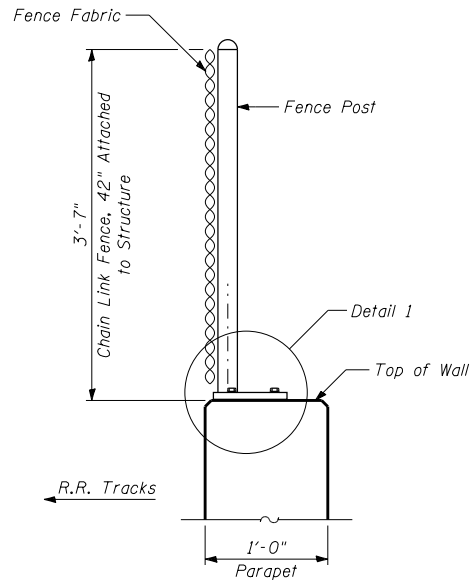
Shaft Mark	Diameter	Prop. Top Elevation	Exist. Top Elevation	Extension (Ft.)	Reinforcing (Vertical)	Reinforcing (Ties)
MS-31	4'-0"	694.11	692.31	1.80	12-v125	4-s100
MS-32	4'-0"	694.51	692.39	2.12	12-v126	5-s100
MS-33	4'-0"	694.91	692.48	2.43	12-v127	6-s100
MS-34	4'-0"	695.31	692.57	2.74	12-v128	6-s100
MS-35	4'-0"	695.71	692.66	3.05	12-v129	7-s100
DS-4	4'-0"	694.25	691.85	2.40	12-v127	6-s100

DRILLED SHAFT SUMMARY

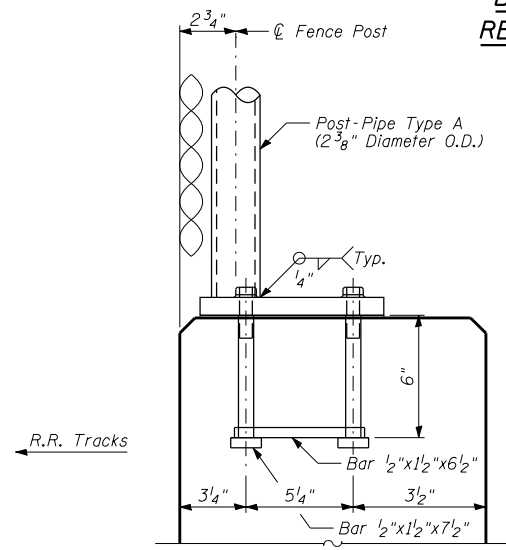
Shaft Mark	Diameter	Top Elevation	Bottom Elevation	Length	Reinforcing (Vertical)	Reinforcing (Spiral)
DS-10	4'-0"	696.41	651.41	45'-0"	28-v124	sp100
DS-11	4'-0"	696.41	651.41	45'-0"	28-v124	sp100



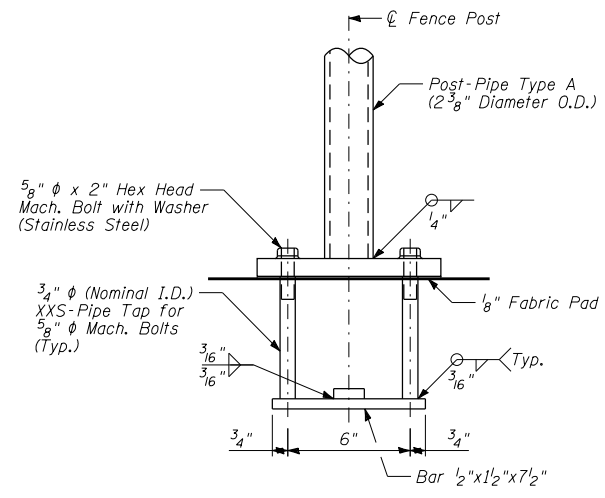
BACK FACE ELEVATION - TOP OF RETAINING WALL CHAIN LINK FENCE



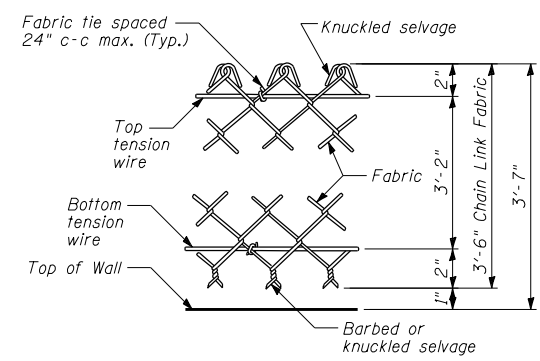
SECTION A-A



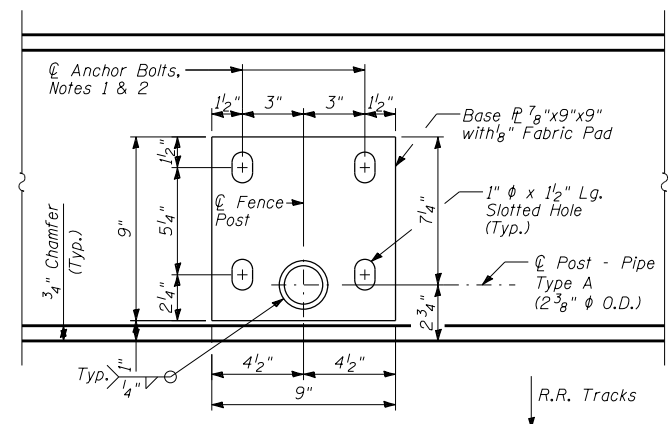
DETAIL 1



DETAIL 2



DETAIL 3



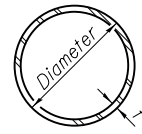
**SECTION B-B
BASE PLATE PLAN**

ITEM	UNIT	TOTAL
Chain Link Fence, 42" Attached to Structure (Special)	FOOT	93'-0"

BILL OF MATERIAL

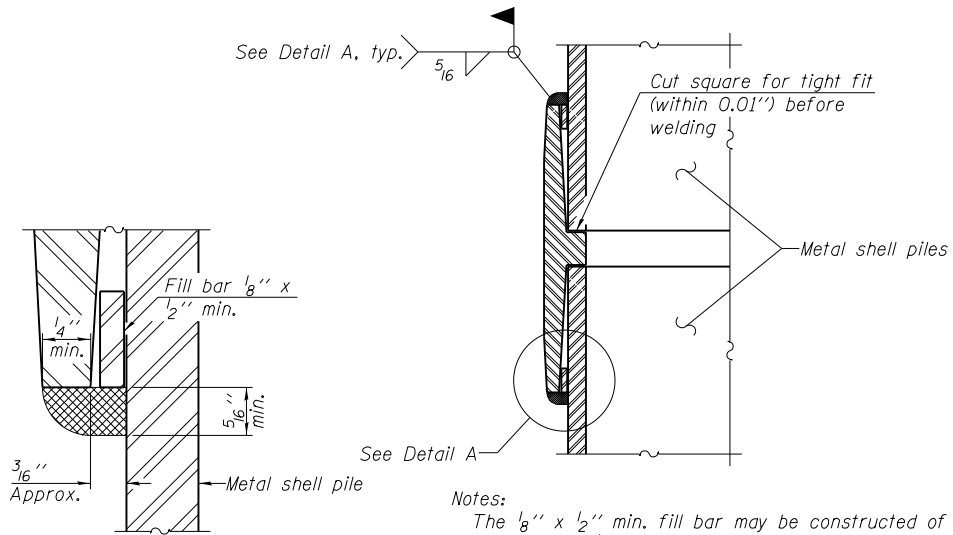
NOTES:

- In lieu of the cast-in-place anchor bolt assembly shown, the Contractor has the option of drilling and epoxy grouting 5/8" diameter anchor rods with 1/4" diameter washers. The Contractor shall use the capsule or the adhesive cartridge type anchor rods that have been previously tested and given prior approval by the Department. The Contractor shall install these anchor rods in pre-drilled holes according to the manufacturer's recommendations and procedures. The capsule or adhesive cartridge shall be sealed with pre-measured amounts of adhesive chemical. Anchor rod threading to be peened after nuts are installed.
- For additional chain link details, see Standard 664001.
- Adjust fence and post spacing to clear light pole.
- Fence post shall be vertical.



METAL SHELL PILE TABLE

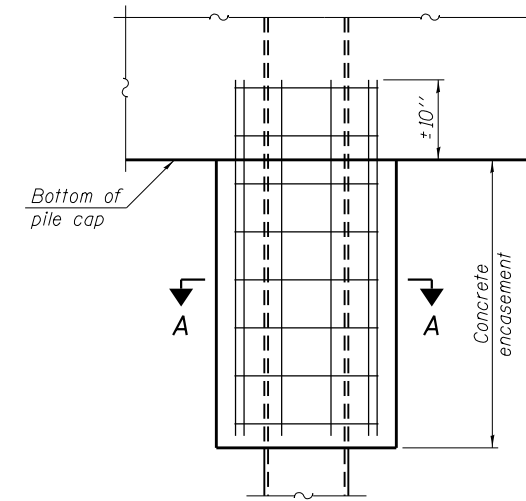
Designation and outside diameter	Wall thickness t	Weight per foot (Lbs./ft.)	Inside volume (yd. ³ /ft.)
PP12	0.179"	22.60	0.0274
PP12	0.250"	31.37	0.0267
PP14	0.250"	36.71	0.0368
PP14	0.312"	45.61	0.0361



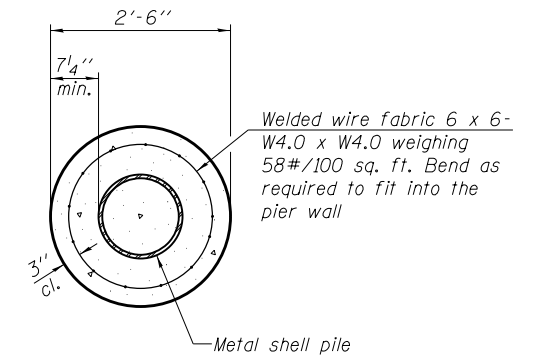
DETAIL A

Notes:
 The 1/8" x 1/2" min. fill bar may be constructed of 2 bars with a 1/8" max. gap between them.
 Pile segments shall be driven to solid contact with splicer before welding.

WELDED COMMERCIAL SPLICE



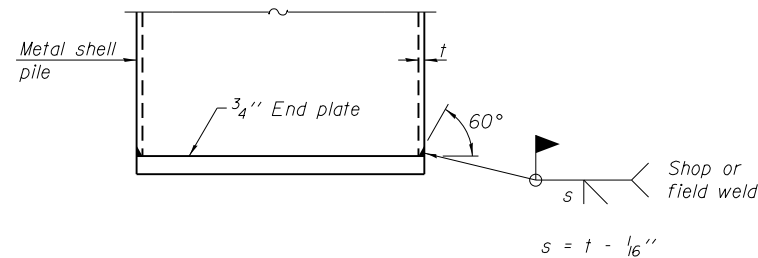
ELEVATION



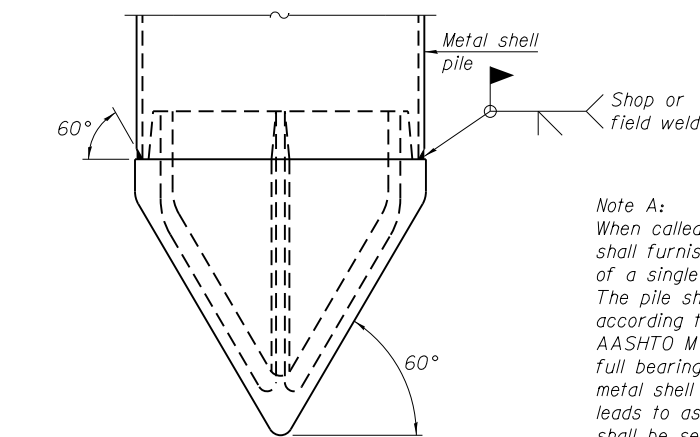
SECTION A-A

Note:
 Forms for encasement may be omitted when soil conditions permit.

CONCRETE ENCASEMENT AT PIERS



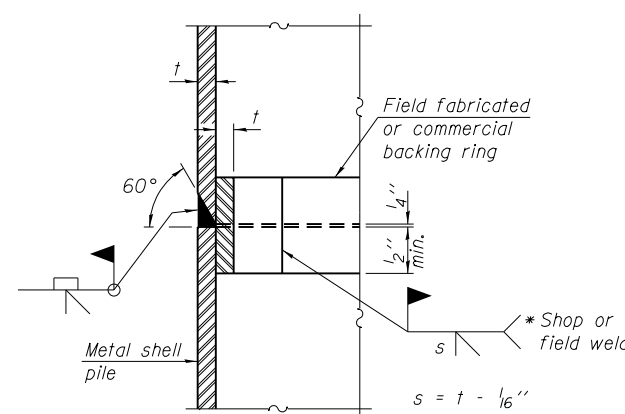
END PLATE ATTACHMENT



METAL SHELL PILE SHOE ATTACHMENT

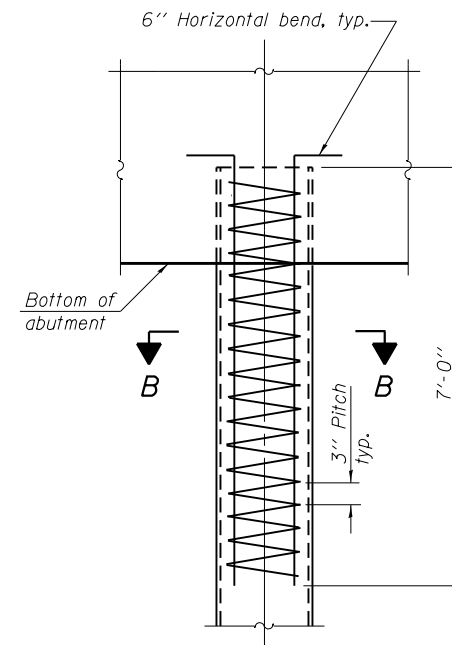
(See Note A)

Note A:
 When called for on the plans, the Contractor shall furnish metal shell pile shoes consisting of a single piece conical pile point as shown. The pile shoes shall be cast in one piece steel according to either ASTM A 148 Grade 90-60 or AASHTO M 103 Grade 65-35 and shall provide full bearing over the full circumference of the metal shell pile. The pile shoe shall have tapered leads to assure proper alignment and fitting and shall be secured to the pile with a circumferential weld.

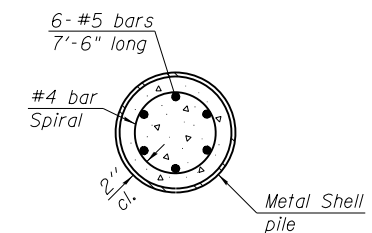


COMPLETE PENETRATION WELD SPLICE

* Field fabricated backing ring may be made from pile shell by removing segment to allow reducing circumference and vertically rejoin with partial joint penetration weld.



ELEVATION



SECTION B-B

METAL SHELL REINFORCEMENT AT ABUTMENTS

F-MS 1-27-12

Note:
 The metal shell piles shall be according to ASTM A 252 Grade 3.



USER NAME = aofitzpatrick
 PLOT SCALE = 2.0000" / in.
 PLOT DATE = 10/7/2016

DESIGNED - MMH
 CHECKED - DNB
 DRAWN - R.VEJAR
 CHECKED - BCS

REVISED -
 REVISED -
 REVISED -
 REVISED -

**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

**METAL SHELL PILE DETAILS
 STRUCTURE NO. 049-0602**

SHEET NO. 35 OF 43 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
346	125X-N&J-SB-B	LAKE	361	250
CONTRACT NO. 60K80				

ILLINOIS FED. AID PROJECT



Illinois Department of Transportation

Division of Highways
GSS CONSULTANTS INC.
FAU 1218 (Illinois Route 132 / Grand Avenue)

SOIL BORING LOG

Page 1 of 3

Date 5/24/11

ROUTE 132 / Grand Avenue DESCRIPTION Union Pacific Rail Bridge over IL 132 (Grand Ave.) LOGGED BY RJC
SECTION 125X-N LOCATION N. Abut., SEC. 13, TWP. 45N, RNG. 11E, 3rd PM
Latitude 42°22'15.35958" N, Longitude 87°53'44.03487" W
COUNTY Lake DRILLING METHOD HSA HAMMER TYPE AUTO

Table with columns for Depth (ft), Blows (blows/6"), Blow Count (tsf), and Moisture Content (%). Includes soil descriptions like 'Very Stiff Gray, Moist SILTY LOAM, trace gravel' and 'Medium Dense Gray, Moist SANDY LOAM'. Includes groundwater levels and surface elevations.

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)



Illinois Department of Transportation

Division of Highways
GSS CONSULTANTS INC.
FAU 1218 (Illinois Route 132 / Grand Avenue)

SOIL BORING LOG

Page 2 of 3

Date 5/24/11

ROUTE 132 / Grand Avenue DESCRIPTION Union Pacific Rail Bridge over IL 132 (Grand Ave.) LOGGED BY RJC
SECTION 125X-N LOCATION N. Abut., SEC. 13, TWP. 45N, RNG. 11E, 3rd PM
Latitude 42°22'15.35958" N, Longitude 87°53'44.03487" W
COUNTY Lake DRILLING METHOD HSA HAMMER TYPE AUTO

Table with columns for Depth (ft), Blows (blows/6"), Blow Count (tsf), and Moisture Content (%). Includes soil descriptions like 'Stiff to Very Hard Gray, Moist SILTY LOAM (continued)' and 'Medium Dense to Dense Gray, Fine to Medium Grain, Wet SAND, trace gravel'. Includes groundwater levels and surface elevations.

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)

HOH HARRY OLHEFTER ASSOCIATES, INC. DESIGN AND CONSULTING ENGINEERS 3366 55 East Jackson Blvd., Suite 600 Chicago, Illinois 60604 312/346-8931

Table with columns for USER NAME, DESIGNED, CHECKED, DRAWN, PLOT DATE, REVISED, and CHECKED.

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

SOIL BORING LOGS - 1 STRUCTURE NO. 049-0602 SHEET NO. 36 OF 43 SHEETS

Table with columns for F.A.P. RTE., SECTION, COUNTY, LAKE, TOTAL SHEETS, SHEET NO., and CONTRACT NO.



Illinois Department of Transportation
Division of Highways
GSS CONSULTANTS INC.

SOIL BORING LOG

Date 5/24/11

ROUTE 132 / Grand Avenue DESCRIPTION Union Pacific Rail Bridge over IL 132 (Grand Ave.) LOGGED BY RJC

SECTION 125X-N LOCATION N. Abut. SEC. 13, TWP. 45N, RNG. 11E, 3rd PM
Latitude 42°22'15.35958" N, Longitude 87°53'44.03487" W

COUNTY Lake DRILLING METHOD HSA HAMMER TYPE AUTO

STRUCT. NO. Station	D E P T H S T R I C T U R E S Q U A R E S T R I C T U R E S	B L O W S Q u A R E S	U C S Q u A R E S	M O I S T Q u A R E S	Surface Water Elev.	NA ft	
					Stream Bed Elev.	NA ft	
BORING NO. B-1 (STAIL Rte 132) Station Offset Ground Surface Elev.					22+76.56 80.94ft LT 699.70 ft	Groundwater Elev.: First Encounter Upon Completion After	664.9 ft NA ft - ft

Soil Description	(ft)	(/6")	(tsf)	(%)
Very Stiff to Hard Gray, Moist SILTY LOAM, trace gravel (continued)	0 to 85			
	5			
	9	2.8		17
	12	P		
	14			
	14	4.1		16
	22	B		
End of Boring	609.70	-90		
	-95			
	-100			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



Illinois Department of Transportation
Division of Highways
GSS CONSULTANTS INC.

SOIL BORING LOG

Date 5/26/11

ROUTE 132 / Grand Avenue DESCRIPTION Union Pacific Rail Bridge over IL 132 LOGGED BY RJC

SECTION 125X-N LOCATION S. Abut. SEC. 13, TWP. 45N, RNG. 11E, 3rd PM
Latitude 42°22'13.79422" N, Longitude 87°53'43.55376" W

COUNTY Lake DRILLING METHOD HSA HAMMER TYPE AUTO

STRUCT. NO. Station	D E P T H S T R I C T U R E S Q U A R E S	B L O W S Q u A R E S	U C S Q u A R E S	M O I S T Q u A R E S	Surface Water Elev.	NA ft	
					Stream Bed Elev.	NA ft	
BORING NO. B-2 (STAIL Rte 132) Station Offset Ground Surface Elev.					23+17.24 76.41ft RT 700.00 ft	Groundwater Elev.: First Encounter Upon Completion After	671.5 ft NA ft - ft

Soil Description	(ft)	(/6")	(tsf)	(%)
4 inches crushed granite (ballast stone)	0 to 699.67			
	7			
Brown and Dark Brown SAND (Fill), some gravel	698.50	3		25
Very Stiff Black and Dark Gray, Moist SILTY LOAM	0 to 678.60			
	4	2.2		18
	6	B		
	3			
	4	2.2		21
	5	B		
	-25			
Encountered Cobble at 26'	6			
	8			26
	10			
Wet at 28.5'	4			
	7	4.5		24
	7	B		
	-30			
	3			
	7	4.7		16
	9	B		
Very Stiff to Hard Gray and Brown, Moist SILTY LOAM	693.00	3	1.0	37
	1			
	3	2.0		38
	5	P		
	-10			
	2			
	7	5.7		23
	9	B		
Very Stiff to Hard Brown with occasional trace of Gray, Moist SILTY LOAM, trace gravel	686.50	5		
	7	5.7		23
	9	B		
	-15			
	2			
	4	3.3		18
	6	P		
	5			
	5	4.7		13
	9	B		
	-20			
11 inch layer of SAND, trace gravel, Gray, Fine to Coarse grain, Wet at 34'	665.10	4		17
	5			
Very Stiff Gray, Moist SILTY LOAM	0 to 665.10			
	4			
	7	3.5		14
	11	P		
	-40			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

FILE NAME : ...04906202-60K80-037.dgn	USER NAME : aefitzpatrick	DESIGNED - MMH	REVISED -	<p align="center">STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION</p>	<p align="center">SOIL BORING LOGS - 2 STRUCTURE NO. 049-0602</p>	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
3366		CHECKED - DNB	REVISED -			346	125X-N&J-SB-B	LAKE	361	252
		DRAWN - R,VEJAR	REVISED -			CONTRACT NO. 60K80				
		CHECKED - BCS	REVISED -			ILLINOIS FED. AID PROJECT				
				SHEET NO. 37 OF 43 SHEETS						



Illinois Department of Transportation
Division of Highways
GSS CONSULTANTS INC.

SOIL BORING LOG

Date 5/26/11

ROUTE 125X-N DESCRIPTION Union Pacific Rail Bridge over IL 132 LOGGED BY RJC

SECTION 125X-N LOCATION S. Abut. SEC. 13, TWP. 45N, RNG. 11E, 3rd PM

Latitude 42°22'13.79422" N, Longitude 87°53'43.55376" W

COUNTY Lake DRILLING METHOD HSA HAMMER TYPE AUTO

STRUCT. NO. _____
Station _____
BORING NO. B-2 (STAIL Rte 132)
Station 23+17.24
Offset 76.41ft RT
Ground Surface Elev. 700.00 ft

DEPTH (ft)	BLOW COUNT (blows/6")	UCS (tsf)	MOISTURE (%)	DESCRIPTION	DEPTH (ft)	BLOW COUNT (blows/6")	UCS (tsf)	MOISTURE (%)	DESCRIPTION
0				Very Stiff Gray, Moist SILTY LOAM (continued)	0				Very Stiff Gray, Moist SILTY LOAM, trace gravel (continued)
3					2				
8	3.8	12			3	3.1	15		
13	P				5	B			
653.00				Gray, Fine to Coarse grain, Wet SAND, trace gravel Switched to mud rotary at 47' Sand blow in at 47' Switched to mud rotary at 47' Drilled to 53' to clear out augers	631.50				Medium Dense to Dense Gray, Fine to Medium grain, Wet SAND, trace gravel
646.50					10				
					12		20		
					11				
					16				
	6			Very Stiff Gray, Moist SILTY LOAM, trace gravel	20		15		
	10	3.9	14		27				
	B				4				
					6	2.7	16		
					8	B			
					621.50				
	5			Very Stiff Gray, Moist SILTY LOAM, trace gravel					
	8	2.4	16						
	10	B							

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)



Illinois Department of Transportation
Division of Highways
GSS CONSULTANTS INC.

SOIL BORING LOG

Date 5/26/11

ROUTE 125X-N DESCRIPTION Union Pacific Rail Bridge over IL 132 LOGGED BY RJC

SECTION 125X-N LOCATION S. Abut. SEC. 13, TWP. 45N, RNG. 11E, 3rd PM

Latitude 42°22'13.79422" N, Longitude 87°53'43.55376" W

COUNTY Lake DRILLING METHOD HSA HAMMER TYPE AUTO

STRUCT. NO. _____
Station _____
BORING NO. B-2 (STAIL Rte 132)
Station 23+17.24
Offset 76.41ft RT
Ground Surface Elev. 700.00 ft

DEPTH (ft)	BLOW COUNT (blows/6")	UCS (tsf)	MOISTURE (%)	DESCRIPTION	DEPTH (ft)	BLOW COUNT (blows/6")	UCS (tsf)	MOISTURE (%)	DESCRIPTION
0				Very Stiff Gray, Moist SILTY LOAM, trace gravel (continued)	0				Very Stiff Gray, Moist SILTY LOAM, trace gravel (continued)
5					5				
7		19			7				
12					12				
610.00				End of Boring	10				
					10	3.3	17		
					13	P			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)

FILE NAME - ...04906202-60K80-038.dgn
 HARRY O. HEFTER ASSOCIATES, INC.
 DESIGN AND CONSULTING ENGINEERS
 3366 55 East Jackson Blvd., Suite 600
 Chicago, Illinois 60604
 312/346-8131

USER NAME = aefitzpatrick	DESIGNED - MMH	REVISED -
	CHECKED - DNB	REVISED -
PLOT SCALE = 24:0.0000 '1' / in.	DRAWN - R.VEJAR	REVISED -
PLOT DATE = 10/7/2016	CHECKED - BCS	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SOIL BORING LOGS - 3
STRUCTURE NO. 049-0602
SHEET NO. 38 OF 43 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
346	125X-N&J-SB-B	LAKE	361	253
CONTRACT NO. 60K80				
ILLINOIS FED. AID PROJECT				



SOIL BORING LOG

Date 5/27/11

ROUTE FAU 1218 (Illinois Route 132 / Grand Avenue) DESCRIPTION Union Pacific Rail Bridge over IL 132 LOGGED BY RJC

SECTION 125X-N LOCATION N. of Bridge, SEC. 13, TWP. 45N, RNG. 11E, 3rd PM, Latitude 42°22'15.84251" N, Longitude 87°53'44.17840" W

COUNTY Lake DRILLING METHOD HSA HAMMER TYPE AUTO

STRUCT. NO. Station	DEPTH H S	BLOW S Qu	UCS (tsf)	MOIST S T (%)	Surface Water Elev.		DEPTH H S (ft)	BLOW S Qu (/6")	UCS (tsf)	MOIST S T (%)
					NA ft	NA ft				
BORING NO. B-5 (STA: U.P. RR) Station 8+70.68 Offset 17.23ft LT Ground Surface Elev. 699.70 ft										
4 inches crushed granite (ballast stone)	699.37									
Brown, Fine to Medium grain, Moist SAND (Fill), trace gravel	6									
	8									
Brown and Gray, Moist SILTY LOAM (Fill)	9									
	3									
Stiff Gray, Moist SILTY CLAY LOAM	3									
	5									
Very Stiff to Hard Gray, Moist SILTY LOAM	5									
	6									
Gray, Moist SILTY LOAM (Fill)	5									
	8									
Black, Moist SILTY LOAM (Fill)	8									
	10									
Gray, Moist SILTY LOAM (Fill)	10									
	5									
Hard to Very Hard Brown, Moist SILTY LOAM, trace gravel	5									
	10									
Medium Dense Gray, Fine to Coarse grain, Wet SAND, trace gravel	13									
	4									
Gray trace Brown from 18.5' - 19.5'	8									
	12									
Very Stiff Gray, Moist SILTY LOAM	15									
	2									
End of Boring	5									
	8									

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)



SOIL BORING LOG

Date 5/25/11

ROUTE FAU 1218 (Illinois Route 132 / Grand Avenue) DESCRIPTION Union Pacific Rail Bridge over IL 132 LOGGED BY RJC

SECTION 125X-N LOCATION S. of Bridge, SEC. 13, TWP. 45N, RNG. 11E, 3rd PM, Latitude 42°22'13.31428" N, Longitude 87°53'43.39424" W

COUNTY Lake DRILLING METHOD HSA HAMMER TYPE AUTO

STRUCT. NO. Station	DEPTH H S	BLOW S Qu	UCS (tsf)	MOIST S T (%)	Surface Water Elev.		DEPTH H S (ft)	BLOW S Qu (/6")	UCS (tsf)	MOIST S T (%)
					NA ft	NA ft				
BORING NO. B-6 (STA: U.P. RR) Station 11+33.30 Offset 17.45ft LT Ground Surface Elev. 699.50 ft										
4 inches crushed granite (ballast stone)	699.17									
Brown, Fine to Medium grain, Moist SAND (Fill), trace gravel	3									
	3									
Gray and Black, Moist SILTY LOAM (Fill), trace gravel	3									
	2									
Very Stiff Brown and Gray, Moist SILTY LOAM, trace gravel	2									
	1									
Stiff to Very Stiff Gray, Moist SILTY CLAY LOAM, trace gravel	1									
	2									
Gray, Moist SILTY LOAM (Fill)	2									
	3									
Black, Moist SILTY LOAM (Fill)	3									
	1									
Gray, Moist SILTY LOAM (Fill)	1									
	2									
Very Stiff Brown and Gray, Moist SILTY LOAM, trace gravel	2									
	4									
Stiff to Very Stiff Gray, Moist SILTY CLAY LOAM, trace gravel	4									
	5									
Gray, Moist SILTY LOAM (Fill)	5									
	7									
Hard Brown, Moist SILTY LOAM, trace gravel	7									
	3									
Encountered Cobble at 14.5'	3									
	5									
Stiff to Very Hard Gray, Moist SILTY LOAM, trace gravel	5									
	7									
Very Stiff Gray, Moist SILTY LOAM	7									
	18									
End of Boring	19									
	4									
	6									
	9									

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)



Bridge Foundation Boring Log

P-91-804-76 PROJECT BRIDGE C&NW Railroad Date February 15, 1984 Sh. 1 of 2 Sh.

ROUTE FAP 120 over Route 132 Bored By D. Grow

SEC. 125-HB-1R STA. Checked By

COUNTY Lake

Boring No. S-4 Station 17+00 Offset 24' RT of C.L.

Table with columns for Elevation, N, Qu t/s.f., w (%), Surface Water El., Groundwater El. at Completion, Wash, After Hours, and soil descriptions. Includes data points for various soil layers like Stiff, Brown Silty Clay and Very Stiff, Gray Silty Clay (Till).

N-Standard Penetration Test-Blows per foot to drive 2" O.D. Split Spoon Sampler 12" with 140 No. hammer falling 30".

Qu-Unconfined Compressive Strength - t/sf w - Water Content - percentage of oven dry weight-%.

Type failure: B - Bulge Failure S - Shear Failure E - Estimated Value P - Penetrometer

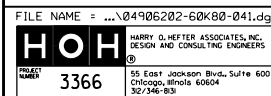
BD 137 (Rev. 4-78)

FORM NO. B D 137 REV. 9-80

Sh. 2 of 2 Sh.

BRIDGE FOUNDATION BORING LOG

Summary table for Bridge Foundation Boring Log with columns for Elevation, N, Qu t/s.f., w (%), and soil descriptions. Includes data for layers like Very Stiff, Gray Silty Clay (Till) and Dense, Gray Sand, Coarse Grained.



Metadata table with columns for FILE NAME, USER NAME, DESIGNED, CHECKED, DRAWN, PLOT SCALE, PLOT DATE, REVISED, and CHECKED.

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

SOIL BORING LOGS - 6 STRUCTURE NO. 049-0602

SHEET NO. 41 OF 43 SHEETS

Summary table with columns for F.A.P. RTE., SECTION, COUNTY, LAKE, TOTAL SHEETS, SHEET NO., and CONTRACT NO.

ILLINOIS FED. AID PROJECT



Illinois Department of Transportation
Division of Highways
GSG CONSULTANTS INC.

SOIL BORING LOG

Page 1 of 1

Date 5/3/11

ROUTE FAU 1218 (Illinois Route 132 / Grand Avenue) DESCRIPTION Union Pacific Rail Bridge over IL 132 LOGGED BY RJC

SECTION 125X-N LOCATION N. of Bridge, SEC. 13, TWP. 45N, RNG. 11E, 3rd PM.
Latitude 42°22'15.74706" N, Longitude 87°53'43.27555" W

COUNTY Lake DRILLING METHOD HSA HAMMER TYPE AUTO

STRUCT. NO.	DEPTHS	LOCATIONS	UCS	MOIST	Surface Water Elev.	DEPTHS	LOCATIONS	UCS	MOIST
Station	(ft)	(/6")	(tsf)	(%)	ft	(ft)	(/6")	(tsf)	(%)
BORING NO. B-22 (STA: U.P. RR)					NA				
Station 8+95.24					NA				
Offset 81.13ft LT									
Ground Surface Elev. 699.41									
Black to Dark Brown, Moist TOPSOIL	698.91			16					
Brown and Dark Brown, Moist SILTY LOAM (Fill), trace gravel and organics	4	3.0	14						
	3	P							
	5								
	3								
NOTE: A large amount of concrete debris and obstructions were encountered between 1' and 12'	12	3.0	17						
	6	P							
	-5								
	693.41								
Brown and Black, Moist GRAVEL, trace sand brick fragments, slag (Fill)	15		11						
	11								
	8								
	7								
	6								
	2		15						
	-10								
	688.41								
Brown and Dark Gray, Moist SILTY CLAY LOAM (Fill), trace gravel and brick fragments	4		25						
	5								
	9								
	4								
Hard Brown and Gray, Moist SILTY LOAM, trace gravel	7	7.0	13						
	7	P							
	-15								
	683.41								
Very Stiff Gray, Moist SILTY CLAY LOAM	3	2.6							
	5	B							
	7								
	2								
Stiff to Hard Gray, Moist SILTY LOAM, trace gravel	3	2.9	20						
	3	B							
	5								
	-20								
	680.91								
	2								
	3								
	8	2.4	13						
	5	B							
	-20								
	659.41								

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)



Illinois Department of Transportation
Division of Highways
GSG CONSULTANTS INC.

SOIL BORING LOG

Page 1 of 1

Date 5/3/11

ROUTE FAU 1218 (Illinois Route 132 / Grand Avenue) DESCRIPTION Union Pacific Rail Bridge over IL 132 LOGGED BY RJC

SECTION 125X-N LOCATION S. of Bridge, SEC. 13, TWP. 45N, RNG. 11E, 3rd PM.
Latitude 42°22'13.16175" N, Longitude 87°53'42.54124" W

COUNTY Lake DRILLING METHOD HSA HAMMER TYPE AUTO

STRUCT. NO.	DEPTHS	LOCATIONS	UCS	MOIST	Surface Water Elev.	DEPTHS	LOCATIONS	UCS	MOIST
Station	(ft)	(/6")	(tsf)	(%)	ft	(ft)	(/6")	(tsf)	(%)
BORING NO. B-23 (STA: U.P. RR)					NA				
Station 11+62.65					NA				
Offset 76.41ft LT									
Ground Surface Elev. 692.30									
Brown and Dark Brown, Moist SILTY CLAY LOAM (Fill), with organics	690.80	5		24					
	5	6.3	21						
	6	B							
	3								
Gray, Moist SILTY LOAM (Fill), trace gravel	2	2.3	20						
	2	P							
	-5								
	688.80								
Brown and Black, Moist SILTY LOAM (Fill), trace gravel	4								
	3	4.5	32						
	3	B							
	2								
Brown, Moist SILTY LOAM (Fill), trace gravel	3	4.5	20						
	3	B							
	2								
	5	4.0	20						
	6	P							
	7								
	7	1.3	18						
	5	P							
	-10								
	680.80								
Hard Gray, Moist SILTY LOAM, trace gravel	7	4.9	16						
	10	B							
	3								
	8	5.7	14						
	8	B							
	-15								
	676.30								
3 inch layer of SAND, Brown, Fine to Coarse grain, at 14.8'	4								
	5	4.3	19						
	8	B							
	6								
	10	5.7	19						
	13	B							
	-20								
	652.30								

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)

FILE NAME - ...04906202-60K00-042.dgn
USER NAME - aefitzpatrick
DESIGNED - MMH
CHECKED - DNB
DRAWN - R,VEJAR
CHECKED - BCS
REVISOR -
REVISOR -
REVISOR -
REVISOR -

PLOT SCALE = 24:0.0000 '1 / in.
PLOT DATE = 10/7/2016

DESIGNED - MMH
CHECKED - DNB
DRAWN - R,VEJAR
CHECKED - BCS
REVISOR -
REVISOR -
REVISOR -
REVISOR -

DESIGNED - MMH
CHECKED - DNB
DRAWN - R,VEJAR
CHECKED - BCS
REVISOR -
REVISOR -
REVISOR -
REVISOR -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SOIL BORING LOGS - 7
STRUCTURE NO. 049-0602

SHEET NO. 42 OF 43 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
346	125X-N&J-SB-B	LAKE	361	257
CONTRACT NO. 60K80				
ILLINOIS FED. AID PROJECT				



Division of Highways
GSG CONSULTANTS INC.
FAU 1218 (Illinois Route
132 / Grand Avenue)

SOIL BORING LOG

Date 5/25/11

ROUTE 125X-N DESCRIPTION Union Pacific Rail Bridge over IL 132 LOGGED BY RJC

SECTION 125X-N LOCATION S. of Bridge, SEC. 13, TWP. 45N, RNG. 11E, 3rd PM.
Latitude 42°22'12.34823" N, Longitude 87°53'43.11715" W

COUNTY Lake DRILLING METHOD HSA HAMMER TYPE AUTO

STRUCT. NO.	DEPTWHS	BLOWS	UCS (tsf)	MOSIST	Surface Water Elev.	DEPTWHS	BLOWS	UCS (tsf)	MOSIST
Station					NA ft				
BORING NO. B-24 (STA: U.P. RR)					Stream Bed Elev. NA ft				
Station 12+33.27					Groundwater Elev.:				
Offset 15.88ft LT					First Encounter 664.9 ft ∇				
Ground Surface Elev. 699.70 ft					Upon Completion 661.7 ft ∇				
					After - Hrs. - ft				
Dark Brown to Black, Moist GRAVEL (Fill), some sand		11			678.70				
Brown, Fine to Coarse grain, Moist SAND (Fill), trace gravel	2					5			
	3					9	4.5	14	
	3					11	P		
696.10	1					4			
Brownish-Gray with trace of Black, Moist SILTY CLAY LOAM (Fill), trace gravel	2	1.3	23			7	3.9	17	
	2	P				11	B		
	-5					-25			
693.70	1					3			
Gray and Brown with trace of Black, Moist SILTY LOAM (Fill), trace gravel	2	3.3	21			5	3.7	19	
	4	P				7	B		
	1					3			
	2	2.3	22			3	2.2	21	
	4	P				5	B		
	-10					-30			
	2								
	4	2.5	30						
687.60	5	P							
Black, Moist SILTY LOAM (Fill)									
686.20	4					3			
Brown and Gray, Moist SILTY LOAM (Fill), trace gravel	4	3.5	16			4	3.0	18	
	7	B				7	P		
684.80					2 inch layer of SAND, trace gravel, Gray, Fine to Medium grain, Moist at 34.3'	664.90			
Very Stiff Dark Gray with trace of Black, Moist SILTY LOAM	5					-35			
683.50	7	6.9	16		Gray, Fine to Medium grain, Wet SAND, trace gravel				
Hard Brown and Gray, Moist SILTY LOAM, trace gravel	11	B							
681.20	6				Very Stiff Gray, Moist SILTY LOAM, trace gravel	661.20	5		
Hard Brown, Moist SILTY LOAM, trace gravel	9	6.7	17				7	3.5	12
	12	B					11	P	
	-20								
					659.70	-40			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, form 137 (Rev. 8-99)

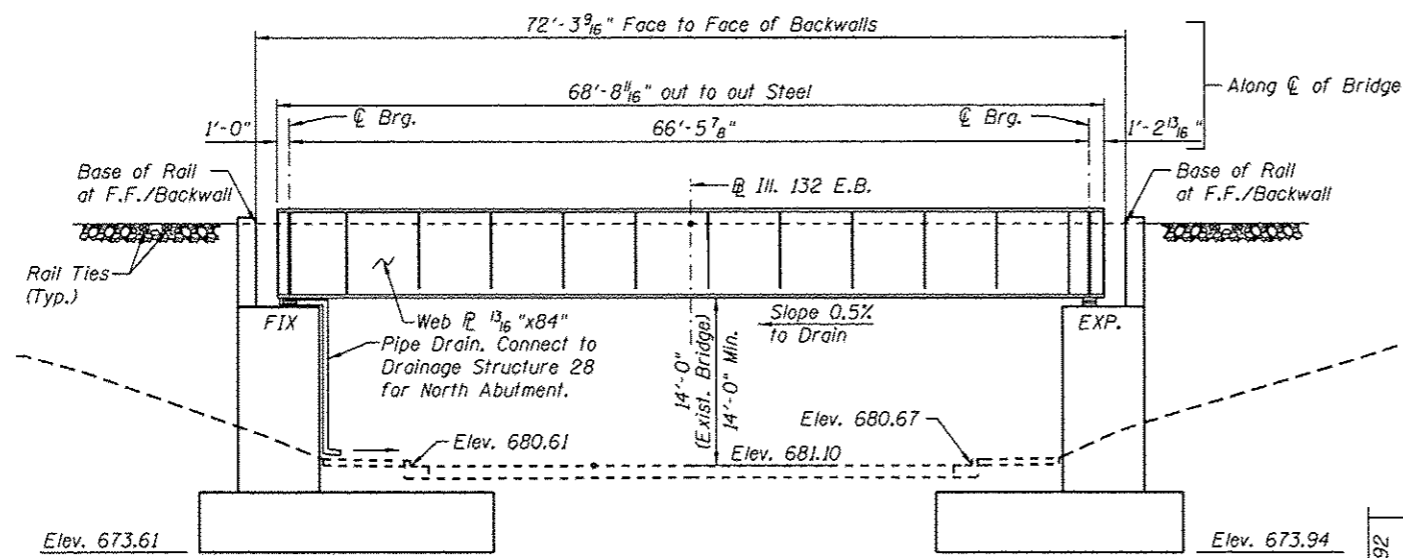
FILE NAME - ...04906202-60K80-043.dgn
HOH
 HARRY O. HEFTER ASSOCIATES, INC.
 DESIGN AND CONSULTING ENGINEERS
 3366
 55 East Jackson Blvd., Suite 600
 Chicago, Illinois 60604
 312/346-8931

USER NAME = aofitzpatrick	DESIGNED - MMH	REVISIONS
PLOT SCALE = 24x0.0000 "/ in.	CHECKED - DNB	REVISIONS
PLOT DATE = 10/7/2016	DRAWN - R.VEJAR	REVISIONS
	CHECKED - BCS	REVISIONS

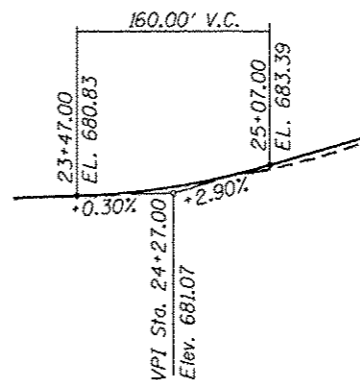
**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**SOIL BORING LOGS - 8
STRUCTURE NO. 049-0602**

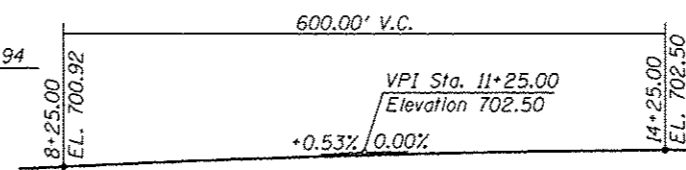
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
346	125X-N&J-SB-B	LAKE	361	258
				CONTRACT NO. 60K80
ILLINOIS FED. AID PROJECT				



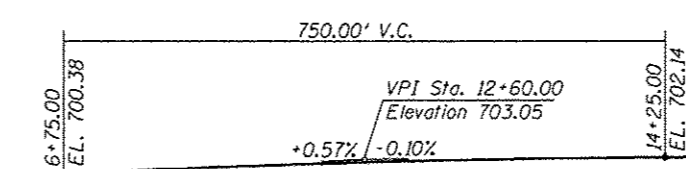
ELEVATION



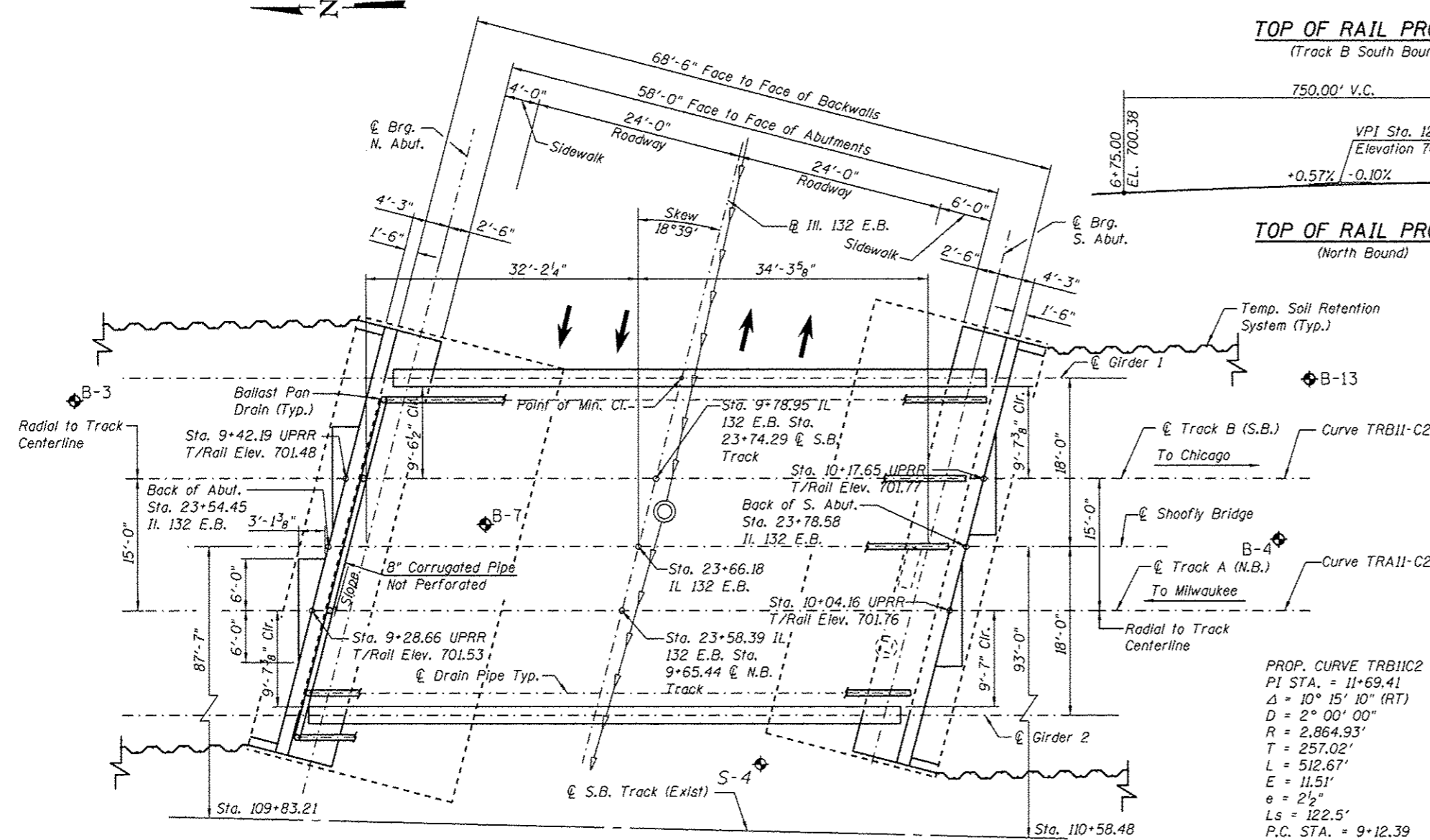
PROFILE GRADE
(ALONG EB IL 132)



TOP OF RAIL PROFILE
(Track B South Bound)



TOP OF RAIL PROFILE
(North Bound)



PLAN

LEGEND

- ◆ B-3 = Borehole B-3
- ◆ S-4 = Borehole S-4 (Exist.)

DESIGN SPECIFICATIONS

2010 AREMA Manual for Railway Engineering
2007 BNSF Railway-Union Pacific Railroad Guide for Railroad Grade Separation Project

LOADING

Cooper E80 or Alternative Load on 4 Axes per AREMA Manual

SEISMIC DATA

Base Acceleration Coefficient (AR) = 4.0%g (100yr.)
Site Coefficient (S) = 1.0
Damping Adjustment Factor (D) = 1.0
Seismic Response Coefficient = 10.0 (100yr.)

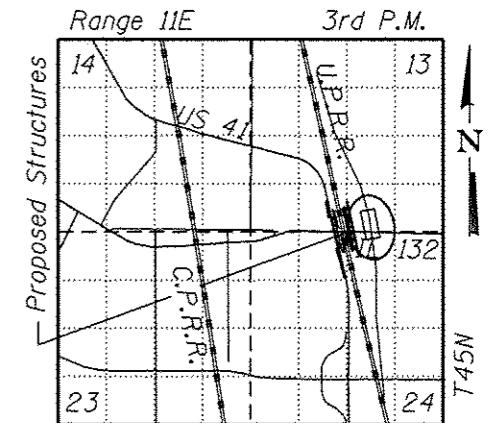
NOTES:

1. For limits of temporary soil retention system, existing contours, and existing and proposed utilities see Sht. 3.
2. For North Abutment see Sheets 17 and 18.
3. For South Abutment see Sheets 19 and 20.
4. The net allowable bearing capacity recommended for the design of foundations and retaining walls bearing on the materials described is 4,000 pounds per square foot (psf).

DANIO N. BILDW
LICENSED STRUCTURAL ENGINEER
DATE: 6/21/16
LICENSE EXPIRES 11/30/16

I certify that to the best of my knowledge, information and belief, this bridge design is structurally adequate for the design loading shown on the plans. The design is an economical one for the style of structure and complies with the requirements of the current AREMA Manual for Railroad Engineering (2010).

APPROVED
For Structural Adequacy Only
Dr. Carl Ruppberg
Engineer of Bridges & Structures



LOCATION SKETCH

GENERAL PLAN & ELEVATION
SHOOFLY BRIDGE OVER
IL. ROUTE 132 (GRAND AVE.)
F.A.P. 346 - SEC. 125X-N&J-SB-B
LAKE COUNTY
STATION 23+66.18 EB

PROP. CURVE TRB11C2
PI STA. = 11+69.41
Δ = 10° 15' 10" (RT)
D = 2° 00' 00"
R = 2,864.93'
T = 257.02'
L = 512.67'
E = 11.51'
e = 2 1/2"
Ls = 122.5'
P.C. STA. = 9+12.39
P.T. STA. = 14+25.06

PROP. CURVE TRA11C2
PI STA. = 11+59.99
Δ = 10° 15' 10" (RT)
D = 2° 00' 00"
R = 2,864.93'
T = 257.02'
L = 512.67'
E = 11.51'
e = 2 1/2"
Ls = 122.5'
P.C. STA. = 9+02.97
P.T. STA. = 14+15.63

FILE NAME * ...\SHOOFLY-60K80-001.dgn	USER NAME = eafitpatrick	DESIGNED - MMH	REvised -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	GENERAL PLAN AND ELEVATION SHOOFLY BRIDGE OVER IL RTE 132	F.A.P. RTE. 346	SECTION 125X-N&J-SB-B	COUNTY LAKE	TOTAL SHEETS 361	SHEET NO. 259		
HOH	PLOT SCALE = 1/8" = 1'-0"	CHECKED - DNB	REvised -			SHEET NO. 1 OF 25 SHEETS		CONTRACT NO. 60K80				
3366	PLOT DATE = 12/7/2016	DRAWN - R.VEJAR	REvised -			ILLINOIS FED. AID PROJECT						
		CHECKED - BCS	REvised -									

GENERAL NOTES

1. Workmanship and materials shall be in accordance with the GENERAL CONDITIONS AND SPECIFICATIONS Adopted by the Union Pacific Engineering Department and dated April 13, 2011 (UP General Conditions and Specification) except as modified by IDOT and the AREMA MANUAL for RAILWAY ENGINEERING dated 2010.
 2. For any procedure, including demolition of existing structures, earth excavation, temporary shoring, sheeting installation, and erection that may affect rail operations, the Contractor shall submit for Engineer's review both work plans and shop drawings.
 3. Dust shall be controlled by uniform applications of sprinkled water, only when directed by the Engineer.
 4. Procurement of all permits, and payment thereof, shall be the Contractor's responsibility.
 5. The Contractor shall be responsible for verifying all existing conditions and elevations at 6. the site and must adapt this work to actual conditions in a manner approved by the 7. Engineer.
- All construction debris shall be removed by the Contractor.
8. It shall be Contractor's responsibility to verify the location of all utilities prior to starting construction. Contact j.u.l.i.e. at 800-892-0123.
 9. No construction joints except those shown on the plans will be allowed unless approved by 10. the Engineer.
 11. Do not scale drawings for construction.
Dowel holes shall be drilled 1/4" larger than the diameter of the dowel.
Bearing seat surfaces shall be constructed or adjusted to the designated elevations within a tolerance of 1/8" inch. Adjustment shall be made either by grinding the surface or by 12. shimming the bearing. Two 1/8" adjusting shims of the dimensions of the bottom bearing plate shall be provided for each bearing in addition to all other plates or shims.
If cantilevered sheet piling is not feasible, additional members or other retention systems 13. may be necessary. The Contractor shall submit a temporary soil retention system design including plan details and calculations for review and acceptance by the Engineer.
 14. Backfill shall be placed behind the abutment after the superstructure has been poured and falsework removed. See Article 502.10 of the Standard Specifications.
All temporary shoring designs and details shall be signed and sealed by a structural engineer licensed in Illinois and submitted to the UPRR & IDOT for review.

SITE WORK

1. Site work shall be performed in accordance with DIVISION 2 SITE WORK SPECIFICATIONS of the UP General Conditions and Specification requirements.

CAST-IN-PLACE CONCRETE

1. Cast-in-Place Concrete material, placement, and workmanship shall be in accordance with DIVISION 3 CONCRETE of the UP General Conditions and Specification and Chapter 8 of the AREMA Manual dated 2010.
2. Minimum Compressive strength - $f_c' = 4000$ psi at 28 days.
3. Exposed surfaces shall be formed in a manner that will produce a smooth and uniform appearance without rubbing or plastering. Exposed edges of 90 degrees or less are to be chamfered 3/4"x3/4". Top surface to have a smooth finish, free of all float or trowel marks with the exception that a broom finish be used on all walkway surfaces.
4. Surface finish for bearing surfaces shall conform to American National Standard Institute Surface Roughness Requirements (A.N.S.I. B46.1 Surface Texture)
5. Cement shall be Type I, II or III Portland Cement per ASTM C150.
6. Fine aggregate shall be natural sand.
7. Admixtures, other than air entrainment, shall not be used without approval.
8. Membrane curing compound shall conform to ASTM C309 Type 2.
9. All construction joints shall be bonded.

REINFORCING STEEL

1. Reinforcing Steel shall be deformed, new billet bars per current ASTM A706 Specifications and meet Grade 60 requirements.
2. Reinforcing Steel requiring field welding shall conform to ASTM A706 Specifications, Grade 60.
3. Fabrication of reinforcing steel shall be per Chapter 7 of the CRSI Manual of Standard Practice. Dimensions of bending details shall be out to out of bars.
4. Reinforcing steel is to be blocked to proper location and securely wired against displacement. Tack welding of reinforcing is prohibited. Minimum concrete cover not otherwise noted shall meet current AREMA requirements.

STRUCTURAL STEEL

1. Structural Steel material, fabrication, and erection shall be in accordance with Chapter 15 of the AREMA Manual dated 2010.
2. All structural steel shall conform to ASTM A709 Grade 50 unless otherwise noted on the plans or in the special provisions. Structural Steel is to be paid for at the Contract Lump Sum Price for "Furnishing and Erecting Structural Steel". Calculated Wt. of A709 Grade 50 steel is 435,100 pounds.
3. The weights of structural steel floor plate, nuts and high strength bolts are included in the weight of structural steel.
4. All bolted connections shall be made with galvanized high strength bolts conforming to ASTM A325. Bolts shall be 7/8" diameter unless otherwise noted. Holes shall be 1/6" larger than bolt size unless otherwise noted. Holes for shop fasteners shall be subpunched or subdrilled and reamed through a template in accordance with AREMA specifications and as specified in special provisions.
5. All welding, electrodes, flux, procedures, welder qualification, inspection, and testing must be in accordance with Article 505.04 (q), Welding of the IDOT Standard Specifications for Fracture Critical Members, and Arema pg. 15-1-10 welding requirement
6. Electro-slag, Electro-gas and FCAW Welding will not be permitted.
7. Structural steel for bridge must be fabricated by a fabricator certified under the American Institute of Steel Construction Quality Certification Program, Category III, Major Steel Bridges including Fracture Critical members in accordance with AREMA Specifications.
8. All tee and corner groove weld joints shall be ultrasonically inspected. All fillet welds shall be inspected by Magnetic Particle Procedure.
9. Steel for the Upper Floor Plate must have a raised pattern conforming to ASTM A786.
10. Steel deck shall conform to A709 specifications, Grade 50.
11. Anchor rods/ bolts shall conform to ASTM F1554 Grade 36 specifications.
12. End welded studs shall be C1015, C1017 or C1020 cold drawn steel, which conforms to ASTM A108 specification.
13. Cover plate, closure plates and anchor rods/bolts shall be galvanized after fabrication in accordance with ASTM A123, thickness Grade 100 if shown on plans.
14. Anchor rod washers shall be zinc coated in accordance with ASTM A153 specification if shown on plans.
15. No field welding is permitted except as specified in the contract documents.
16. All cross frames or diaphragms shall be installed as steel is erected and secured with erection pins and bolts except as otherwise noted. Individual cross frames or diaphragms at supports may be temporarily disconnected to install bearing anchor rods.
17. The webs and tension flanges of the through-plate girders are designated as "Fracture Critical Members" and shall conform to the fracture control plan for fracture critical members of the AREMA Specifications for zone 2. These members are noted on plans as (FCM). For all FCM members NTR shall be applied.
18. The main load carrying components subjected to tensile stress, other than fracture critical members, shall conform to supplemental requirements for notch toughness, Zone 2. These components are the wide flange beams and other components of main girders and are noted on the plans (NTR).
19. The Organic Zinc Rich Primer / Epoxy / Urethane Paint System shall be used for painting of new structural steel except where otherwise noted. The entire system shall be shop applied, with exception that masked off connection surfaces, field installed fasteners and damaged areas shall be touched up in the field. The color of the final finish coat for all steel surfaces shall be Gray, Munsell No. 5b 7/1. See IDOT Standard Specification for Road and Bridge Construction.

WATER PROOFING

1. Water proofing materials and installation shall be cold liquid applied Elastomeric Membrane with integrated ballast mat in accordance with the AREMA MANUAL dated 2010 Chapter 8 Part 29 and special provision #7102..

TEMPORARY SOIL RETENTION SYSTEM

1. Driving sheet piling at the North shoofly abutment may be difficult because of existing rubble.

TOTAL BILL OF MATERIAL

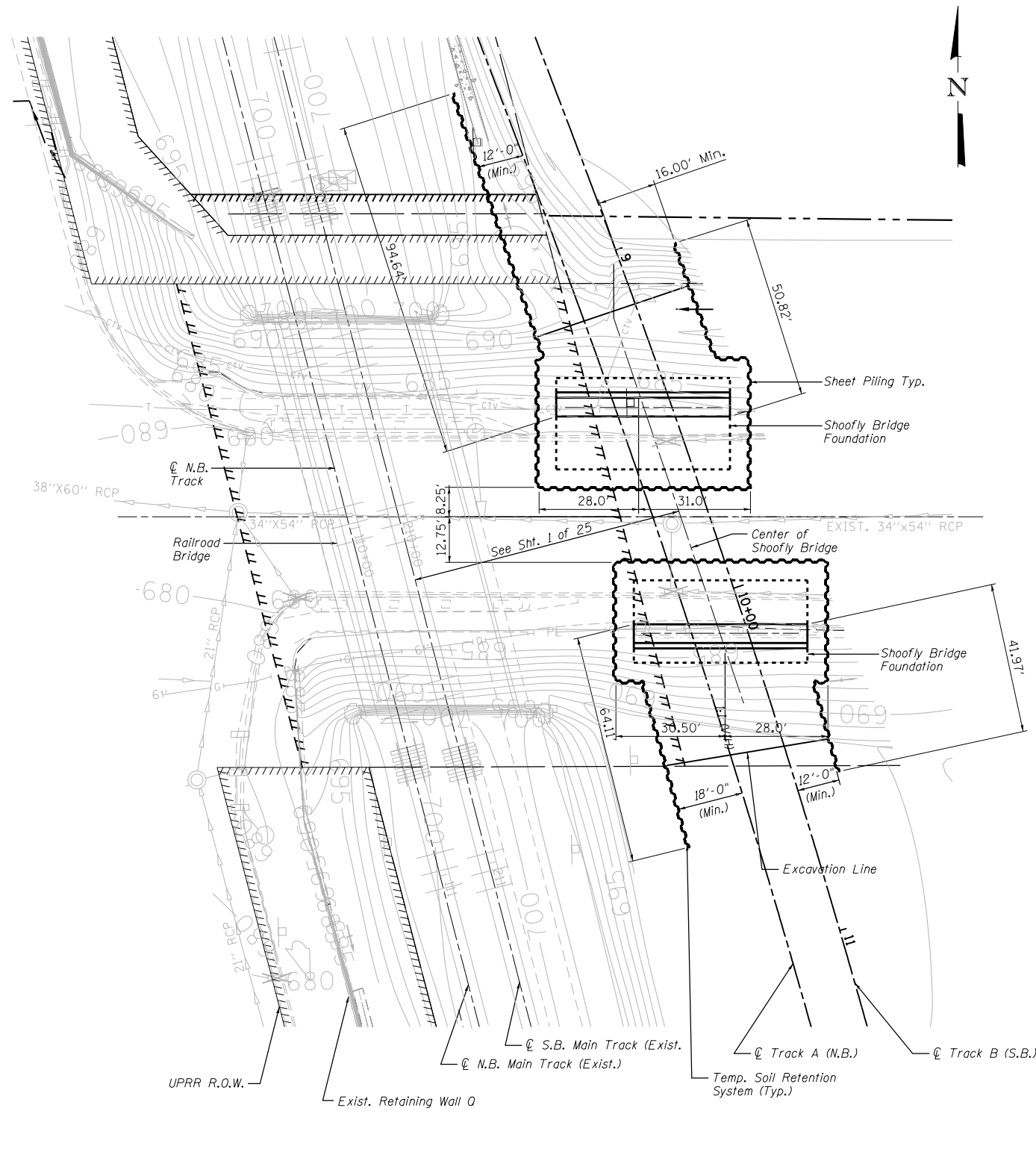
ITEM	UNIT	SUPER	SUB	TOTAL
Structure Excavation	Cu. Yd.	-	1,000	1,000
Granular Backfill for Structures	Cu. Yd.	-	183	183
Concrete Structures	Cu. Yd.	-	919	919
Furnishing and Erecting Structural Steel	L Sum	0.18	-	0.18
Reinforcement Bars	Pound	-	69,837	69,837
Pipe Handrail	Foot	148	-	148
Membrane Waterproofing (Special)	Sq. Ft.	2,640	-	2,640
Geocomposite Wall Drain	Sq. Yd.	-	300	300
Pipe Underdrain for Structures 6"	Foot	-	160	160
Temporary Soil Retention System	Sq. Ft.	-	17,700	17,700
Steel Bearing Assembly	Each	-	4	4
Drainage System	L Sum	0.3	-	0.3
Deck Protection Ballast	Cu. Yd.	-	31	31
Removal of Existing Structures No. 2	Each	-	1	1
Anchor Bolts 1 1/4"	Each	-	8	8
Anchor Bolts 2"	Each	-	8	8

INDEX OF SHEETS

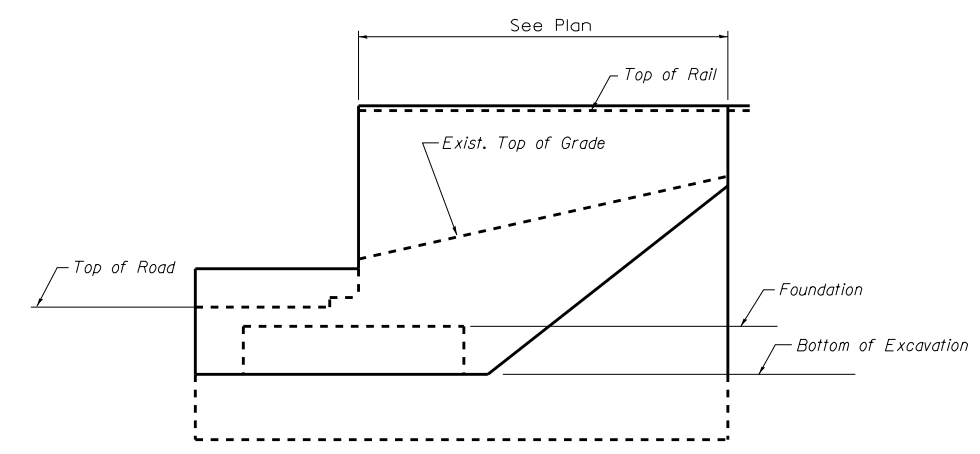
1. General Plan and Elevation
2. General Notes and Bill of Material
3. Temporary Soil Retention System
4. Steel Framing Plan
5. Typical Cross Section and Handrail Details
6. Girder Details
7. Steel Details - 1
8. Steel Details - 2
9. Floor Plate Plan
10. Sections and Details - 1
11. Sections and Details - 2
12. Upper Floor Plate and details
13. Floor Plate Details
14. Design Data and Misc. Details
15. Steel Bearing - 1
16. Steel Bearing - 2
17. North Abutment - 1
18. North Abutment - 2
19. South Abutment - 1
20. South Abutment - 2
21. Soil Boring Logs - 1
22. Soil Boring Logs - 2
23. Soil Boring Logs - 3
24. Soil Boring Logs - 4
25. Soil Boring Logs - 5

NOTES:

1. Prior to commencing any work, the Contractor shall submit for approval by the Railroad detailed plans indicating the nature and extent of the track protection system shoring proposed. The Contractor shall install the temporary soil retention system per the approved plans. Design of the temporary soil retention system (shoring) to comply with Union Pacific Railroad "Guidelines for Temporary Shoring". Plans and calculations must be signed and stamped by a Structural Engineer registered in Illinois & submitted to IDOT & UPRR for review.
2. At the north abutment location of shoofly bridge, all concrete, construction debris and unsuitable material shall be excavated and removed and replaced with structural fill as per structural geotechnical report Sec. S.2 See Appendix B of SAR for over excavation limits.
3. Temporary shoring at north abutment of shoofly bridge shall be designed to be able to penetrate the rubble/debris. If encountered, and support the existing mainline approach embankment shoring over excavation efforts mentioned in note 2.



PLAN

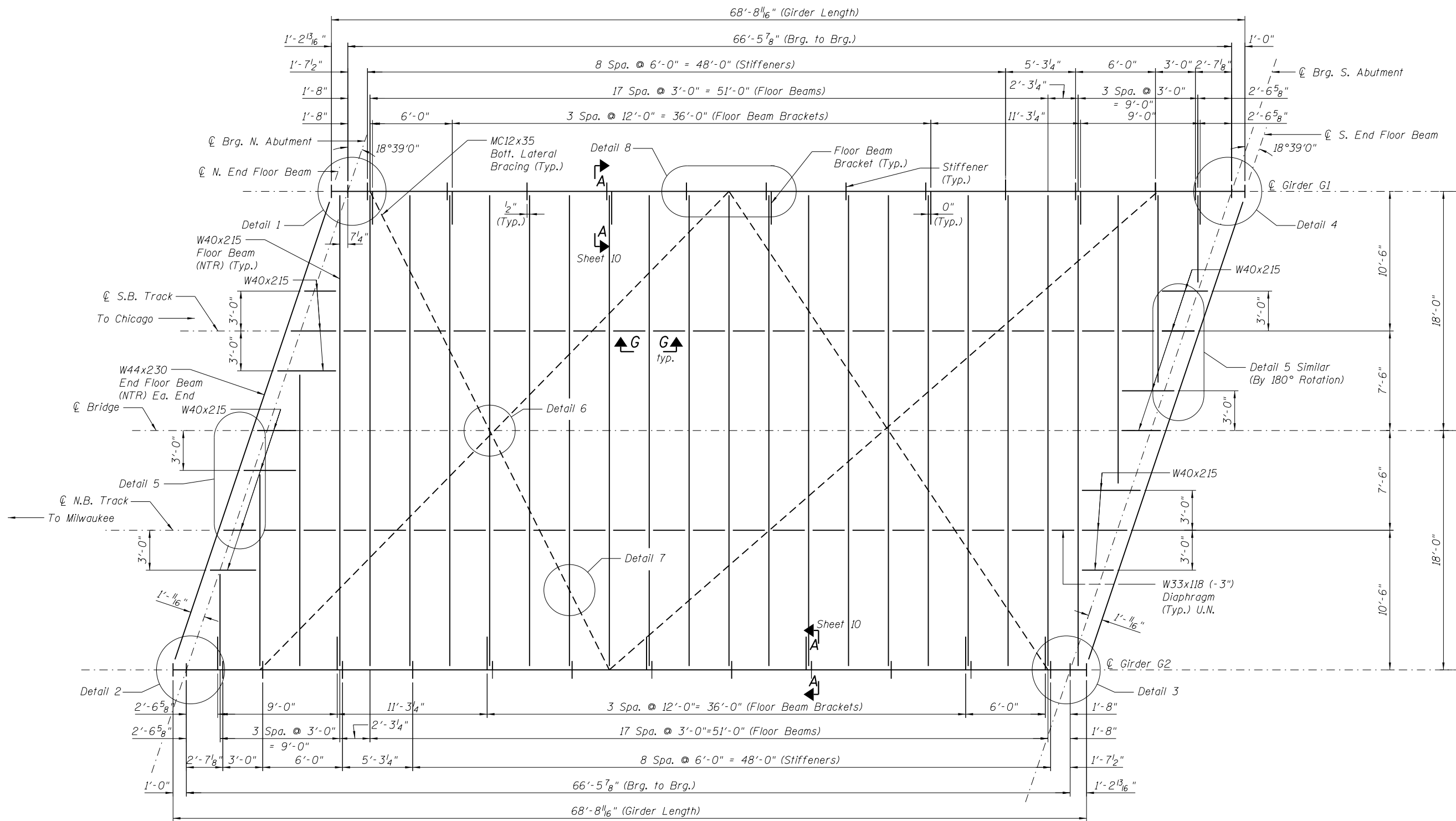
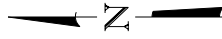


TYPICAL SHEET PILE ELEVATION AT FOOTINGS

SEQUENCE OF CONSTRUCTION:

1. Relocate utilities as required.
2. Install temporary soil retention system along shoofly tracks.
3. Construct Shoofly Bridge for two tracks.
4. Relocate existing N.B. Track and existing S.B. Track to Shoofly Bridge for West side Retaining Walls.
5. Install temporary soil retention system at each end of the railroad bridge for West side Retaining Walls.
6. Remove existing bridge superstructure and substructure.
7. Construct new bridge structure and West side Retaining Walls.
8. Remove temporary soil retention system at each end of the railroad bridge and complete railroad embankment.
9. Relocate N.B. Track and S.B. Track from Shoofly Bridge to new bridge structure.
10. Remove Shoofly structure and remaining soil retention system.
11. Construct retaining walls S & T.
12. Complete remaining work.

FILE NAME - ...SHOOFLY-60K80-003.dgn HOH HARRY O. HEFTER ASSOCIATES, INC. DESIGN AND CONSULTING ENGINEERS 55 East Jackson Blvd., Suite 600 Chicago, Illinois 60604 312/346-8931	USER NAME = aefitzpatrick	DESIGNED - MMH CHECKED - DNB	REVISED - REVISED - REVISED - REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	TEMPORARY SOIL RETENTION SYSTEM SHOOFLY BRIDGE OVER IL RTE 132	F.A.P. RTE. 346	SECTION 125X-N&J-SB-B	COUNTY LAKE	TOTAL SHEETS 361	SHEET NO. 261
	PLOT SCALE = 48.0000' / in. PLOT DATE = 10/7/2016	DRAWN - R.VEJAR CHECKED - BCS	CONTRACT NO. 60K80			SHEET NO. 3 OF 25 SHEETS		ILLINOIS FED. AID PROJECT		



STEEL FRAMING PLAN
Scale: 1/4"=1'-0"

- NOTE:
1. For Details see sheet 7.
 2. For Sect. G-G, see sheet 8.
 3. For Floor Beam Brackets, see sheet 11.

FILE NAME - ...SHOOFLY-60K80-004.dgn
HOH HARRY O. HEFTER ASSOCIATES, INC.
 DESIGN AND CONSULTING ENGINEERS
 3366 55 East Jackson Blvd., Suite 600
 Chicago, Illinois 60604
 312/346-8931

USER NAME = aefitzpatrick
 PLOT SCALE = 0:0.0000 '1' / in.
 PLOT DATE = 10/7/2016

DESIGNED - MMH
 CHECKED - DNB
 DRAWN - R.VEJAR
 CHECKED - BCS

REVISED -
 REVISED -
 REVISED -
 REVISED -

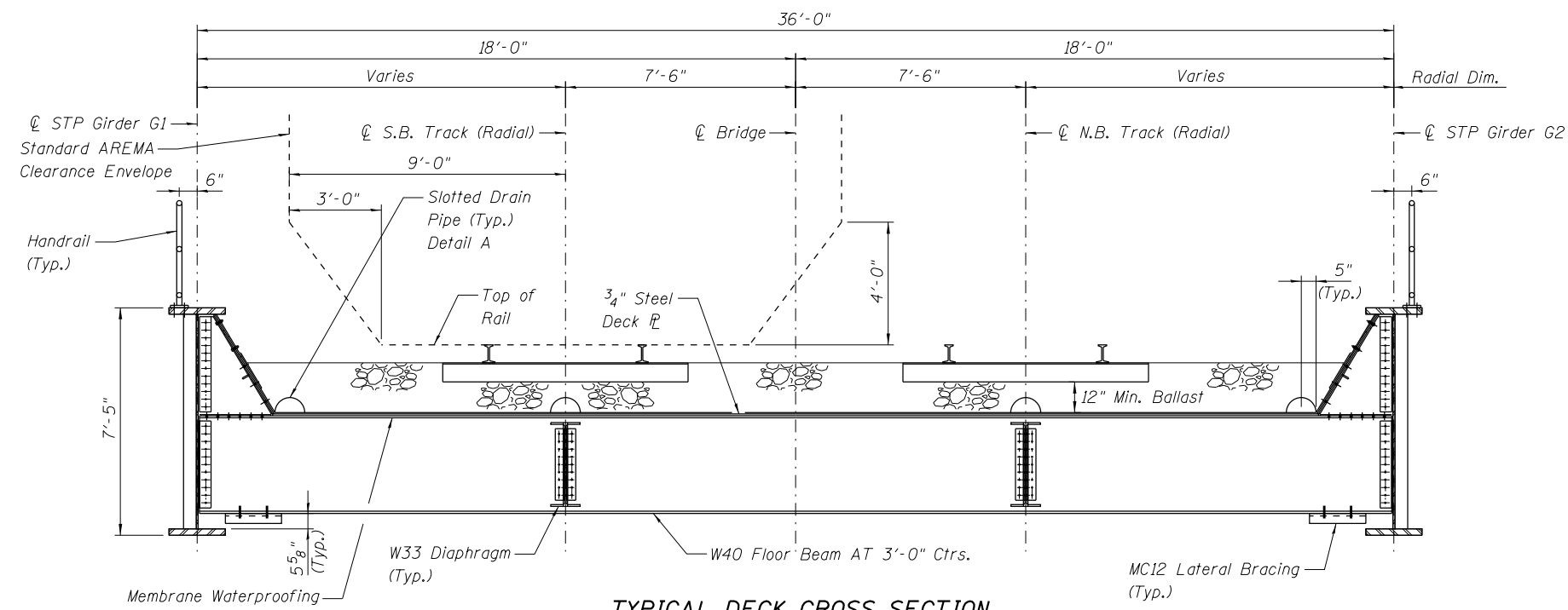
**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

**STEEL FRAMING PLAN
 SHOOFLY BRIDGE OVER IL RTE 132**

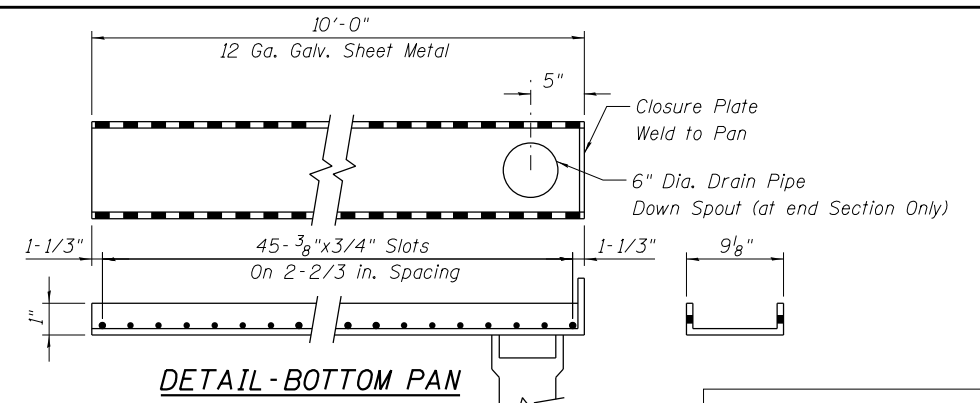
SHEET NO. 4 OF 25 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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CONTRACT NO. 60K80				

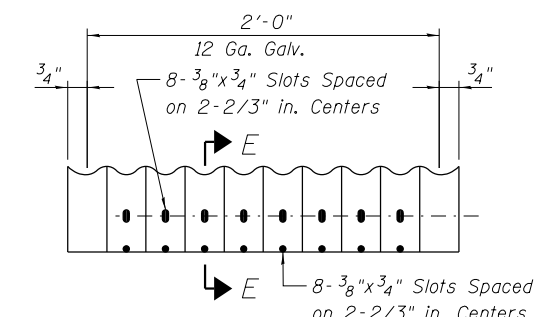
ILLINOIS FED. AID PROJECT



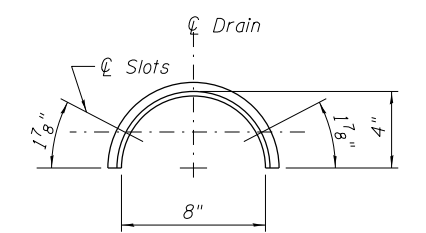
TYPICAL DECK CROSS SECTION
(Looking South)
Scale: 3/8"=1'-0"



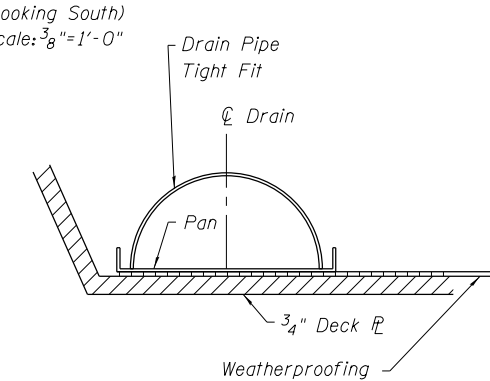
DETAIL - BOTTOM PAN
Scale: None



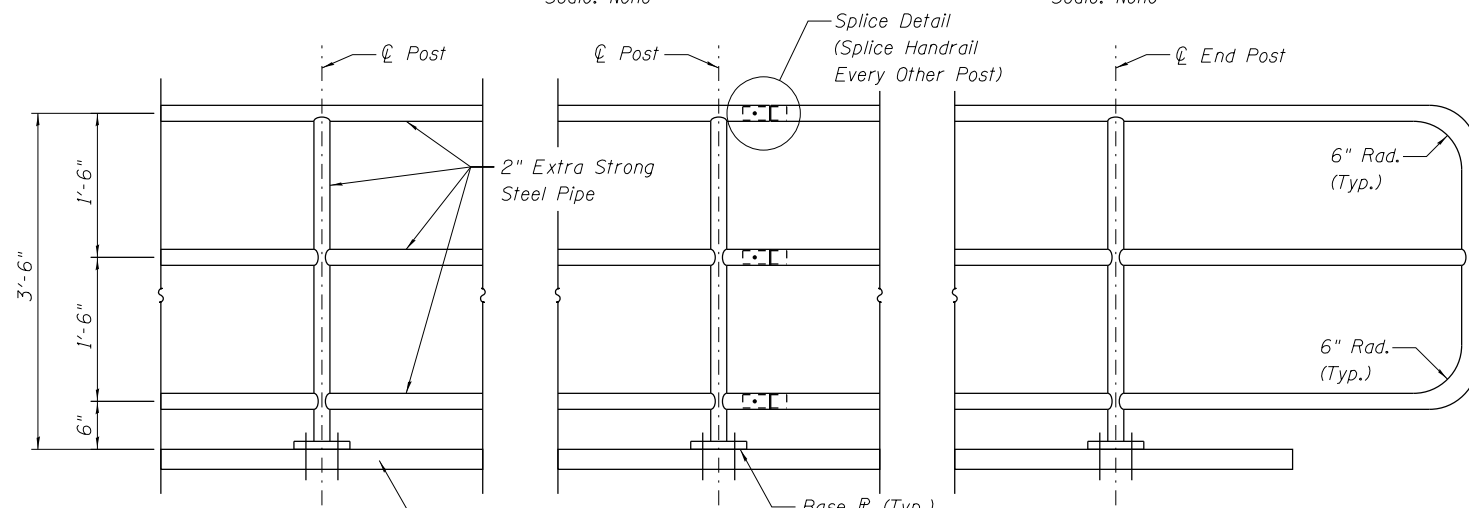
DETAIL - DRAIN PIPE
Scale: None



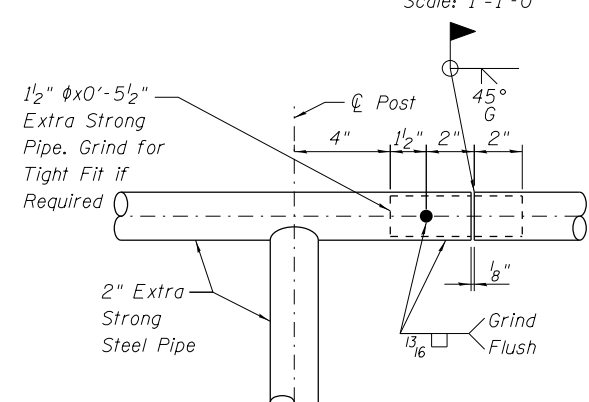
SECTION E-E
Scale: None



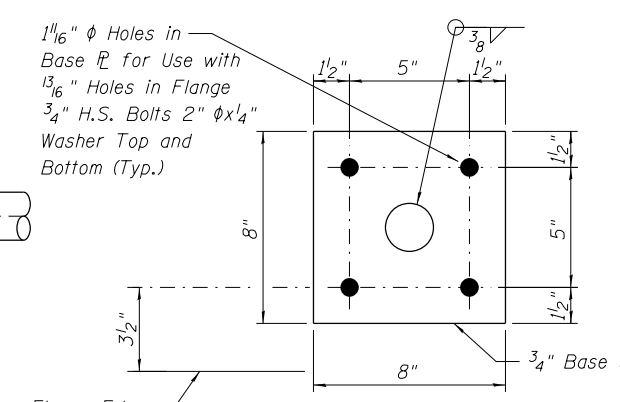
DETAIL - A



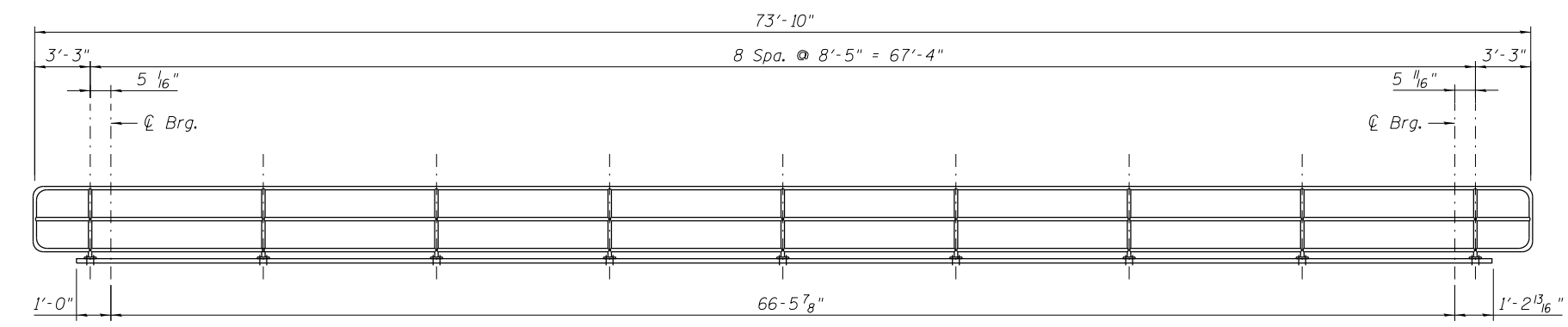
HANDRAIL DETAILS
Scale: 1"=1'-0"



SPlice DETAIL
Scale: 3"=1'-0"



BASE PLATE DETAILS
Scale: 3"=1'-0"



HANDRAIL OUTSIDE ELEVATION (GIRDER G1)
Girder G2 Similar with 180° Rotation
Scale: 1/4"=1'-0"

FILE NAME - ...SHOOFLY-60K80-005.dgn
HOH
 3366
 HOH ENGINEERING & CONSULTING ENGINEERS
 1000 W. WASHINGTON ST., SUITE 600
 CHICAGO, ILLINOIS 60604
 TEL: 773-831-8333

USER NAME = aefitzpatrick
 PLOT SCALE = 2.0000 "/in.
 PLOT DATE = 10/7/2016

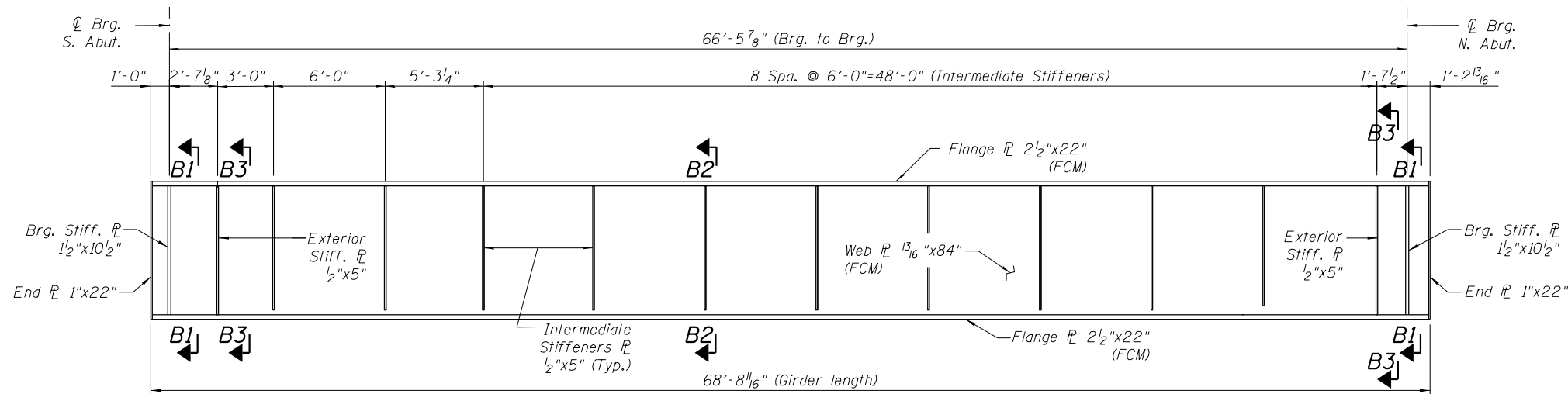
DESIGNED - MMH	REVISD -
CHECKED - DNB	REVISD -
DRAWN - R.V.EJAR	REVISD -
CHECKED - BCS	REVISD -

**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

**TYPICAL CROSS SECTION AND HANDRAIL DETAILS
 SHOOFLY BRIDGE OVER IL RTE 132**

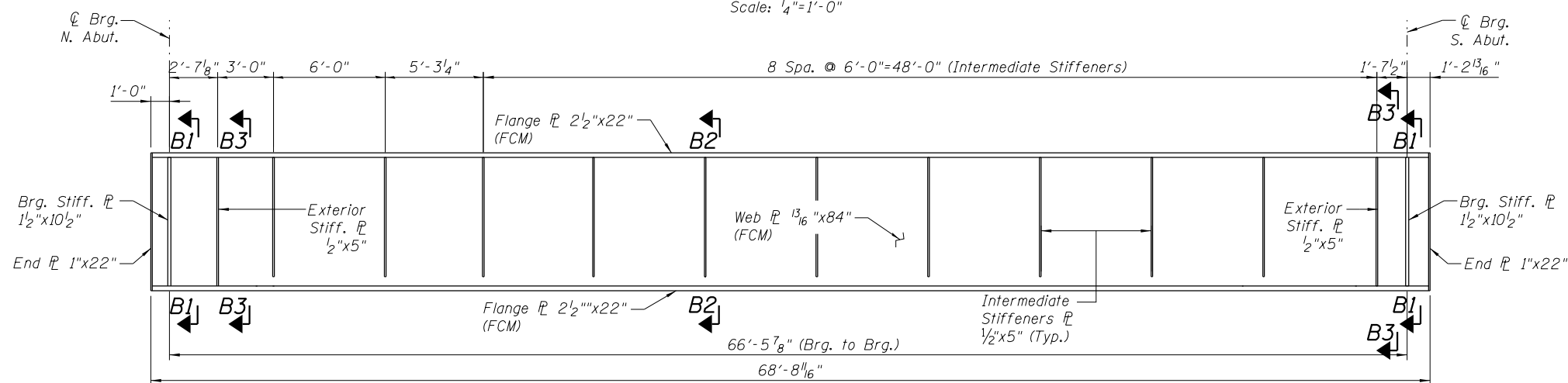
SHEET NO. 5 OF 25 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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CONTRACT NO. 60K80				
ILLINOIS FED. AID PROJECT				



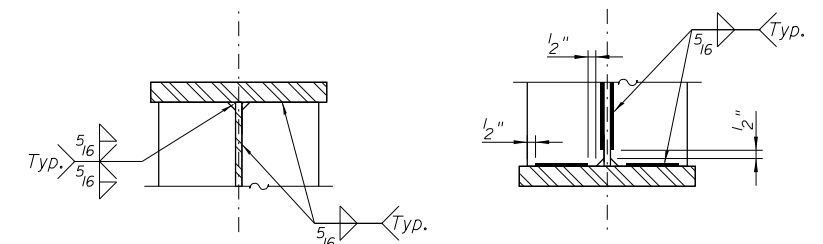
GIRDER G1 ELEVATION

(Looking West)
Scale: 1/4" = 1'-0"



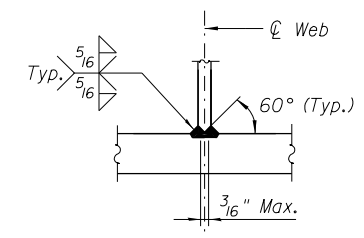
GIRDER G2 ELEVATION

(Looking East)
Scale: 1/4" = 1'-0"



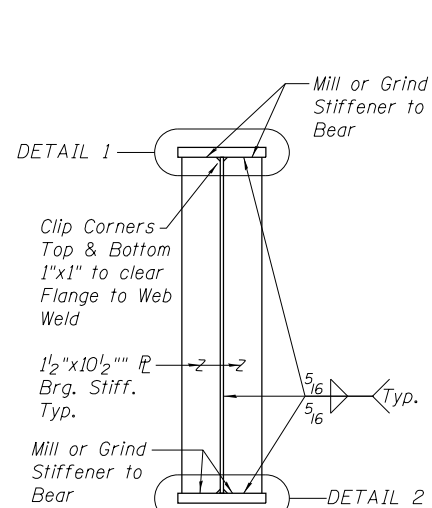
DETAIL 1

DETAIL 2

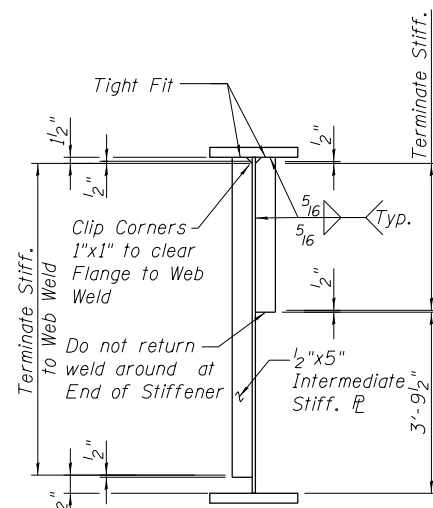


WEB TO FLANGE CONNECTION DETAIL - TYP.

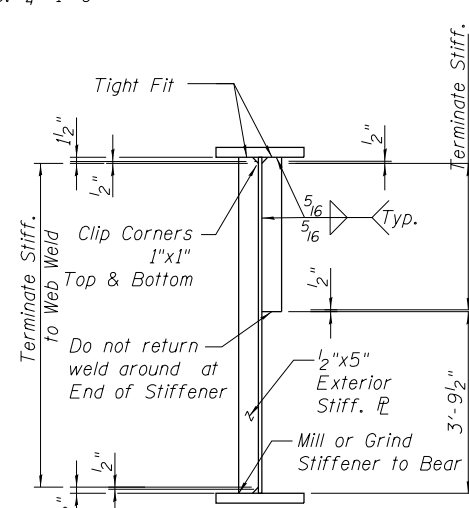
NOTE:
Full penetration of joint required. Weld to be symmetrical about \bar{C} web. Excessive unbalance shall be cause for rejection. See Specifications for weld qualification. Gauge root to sound metal before welding second side.



SECTION B1-B1 AT BEARING

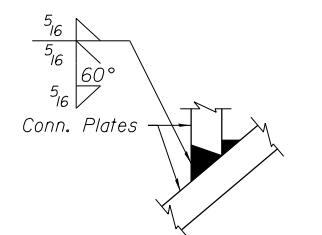


SECTION B2-B2 AT INTERMEDIATE

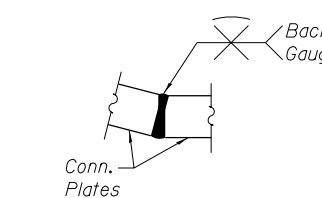


SECTION B3-B3 AT EXTERIOR

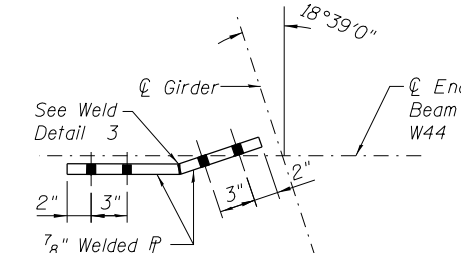
TYPICAL STIFFENER DETAILS



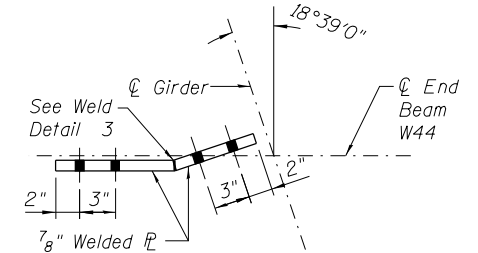
WELD DETAIL - 4



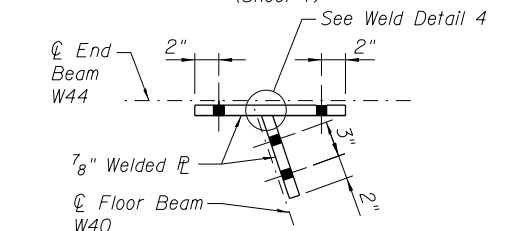
WELD DETAIL - 3



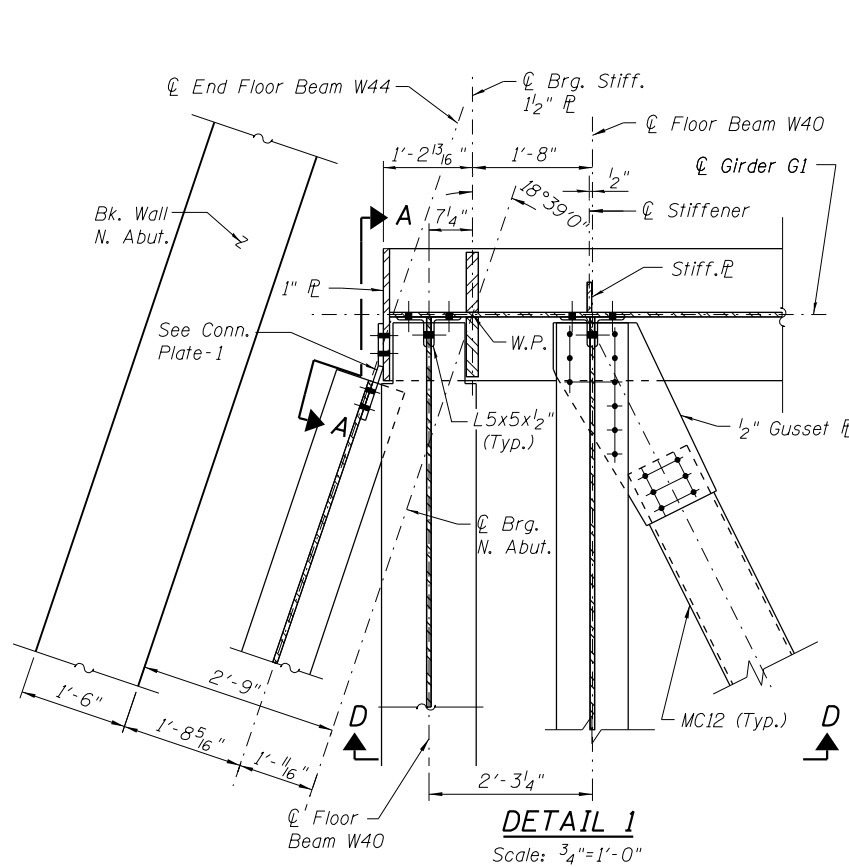
CONN. PLATE - 1



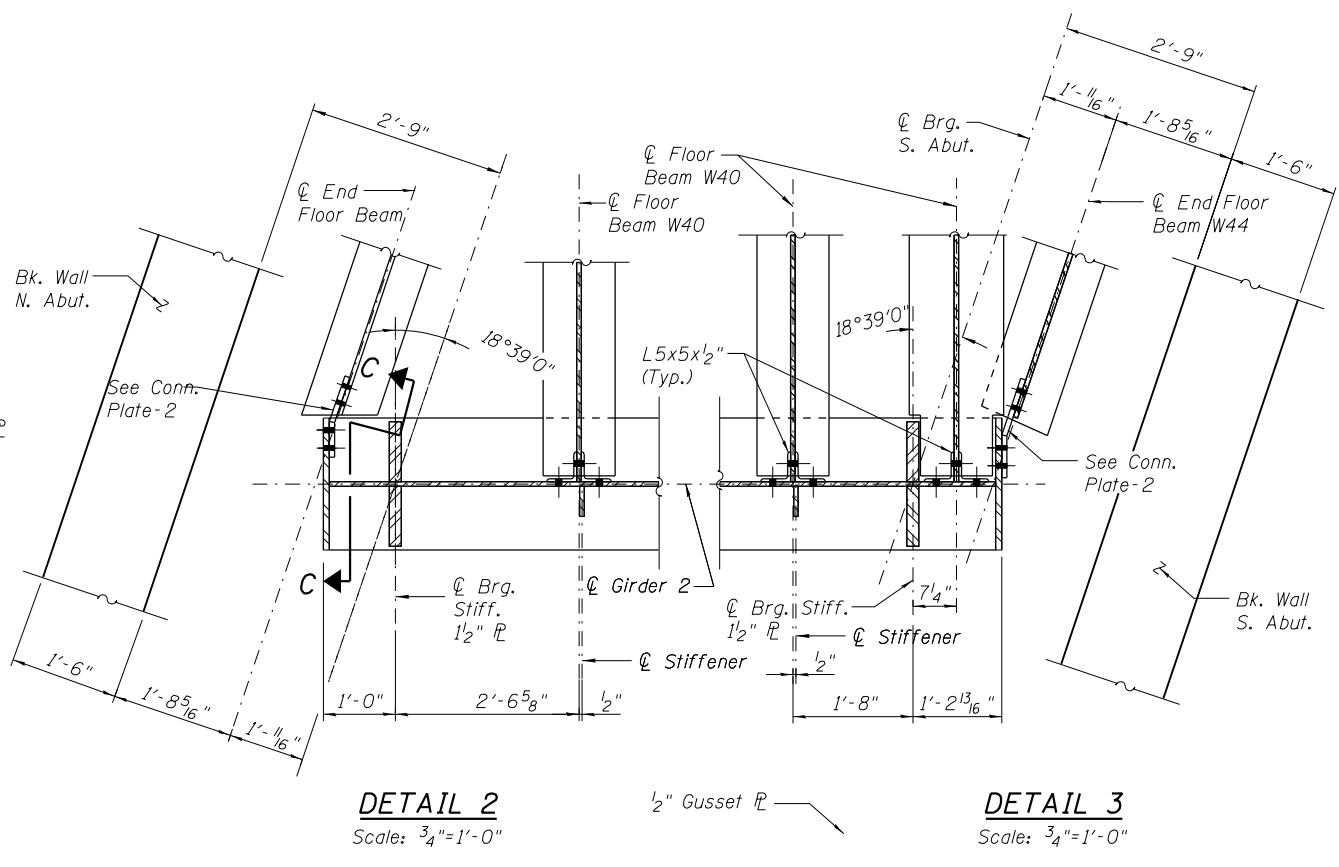
CONN. PLATE - 2



CONN. PLATE - 4

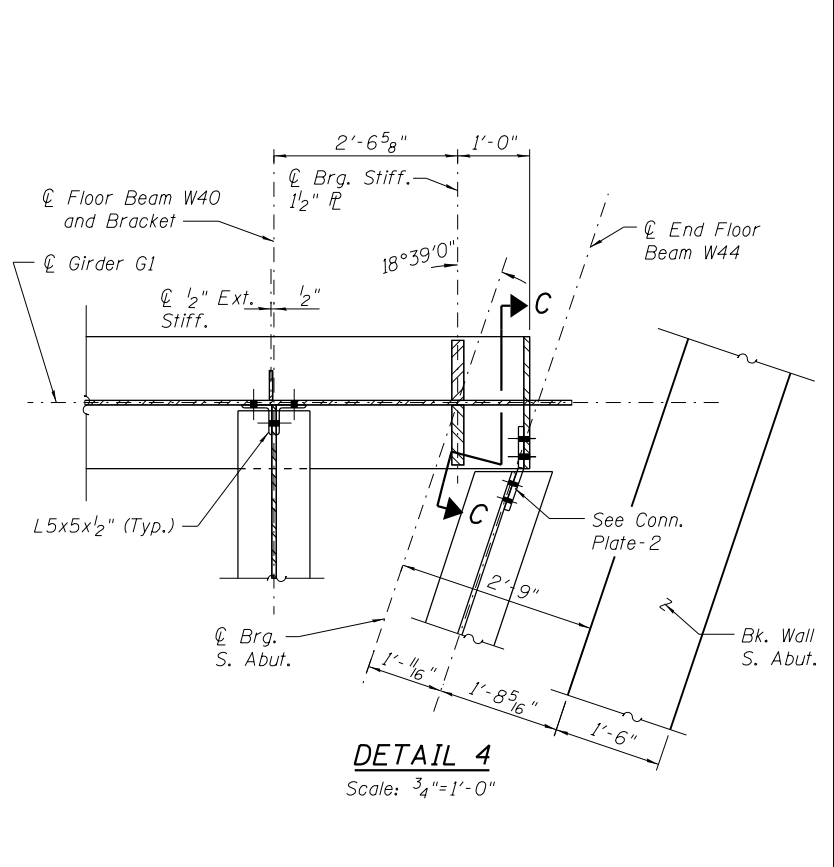


DETAIL 1
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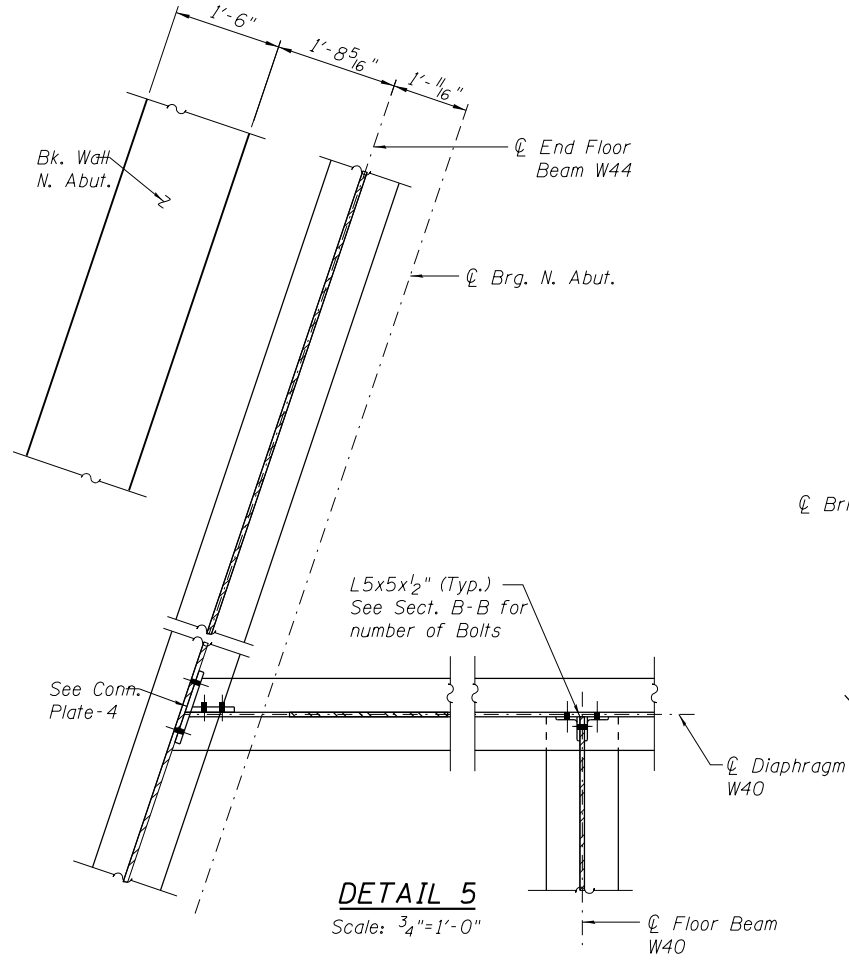


DETAIL 2
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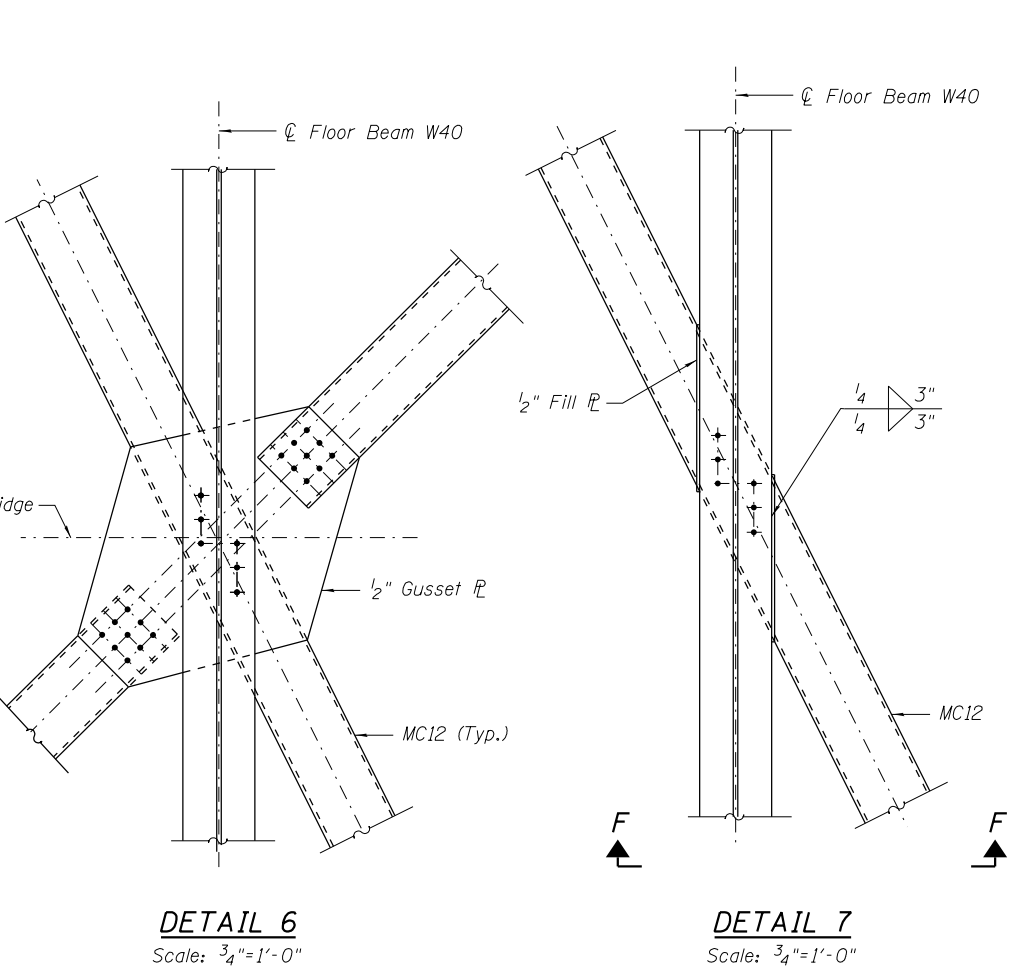
DETAIL 3
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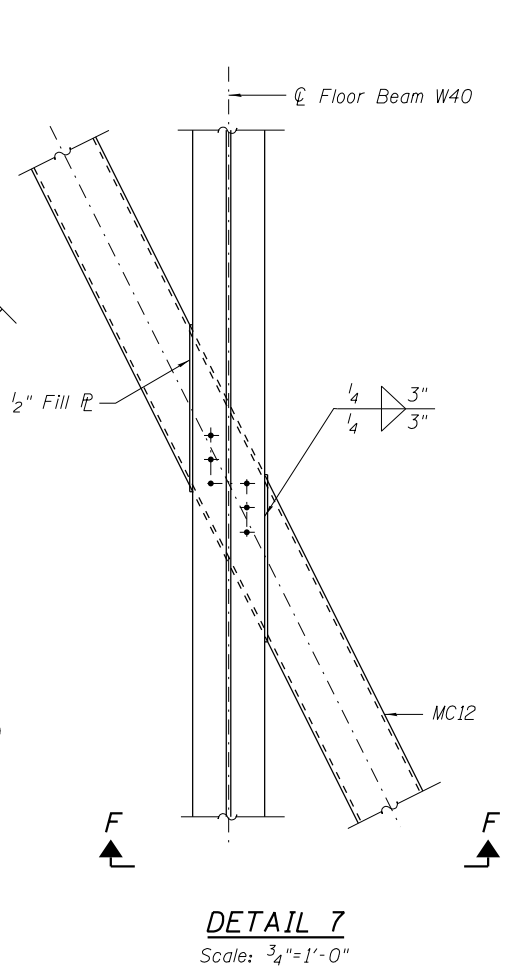
DETAIL 4
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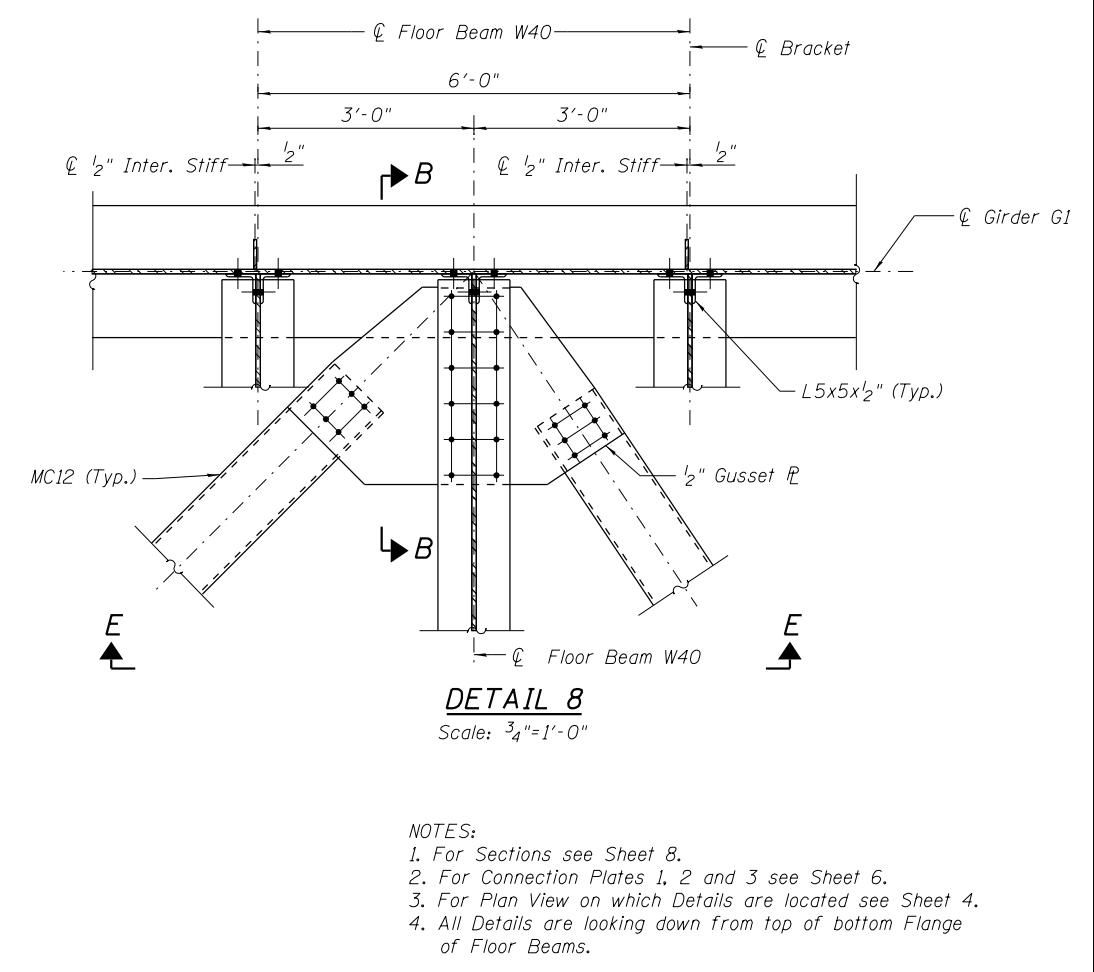
DETAIL 5
Scale: 3/4"=1'-0"



DETAIL 6
Scale: 3/4"=1'-0"



DETAIL 7
Scale: 3/4"=1'-0"



DETAIL 8
Scale: 3/4"=1'-0"

- NOTES:
1. For Sections see Sheet 8.
 2. For Connection Plates 1, 2 and 3 see Sheet 6.
 3. For Plan View on which Details are located see Sheet 4.
 4. All Details are looking down from top of bottom Flange of Floor Beams.

FILE NAME = ...SHOOFLY-60K80-007.dgn	USER NAME = aefitzpatrick	DESIGNED - MMH	REVISED -
HOH HARRY O. HEFTER ASSOCIATES, INC. DESIGN AND CONSULTING ENGINEERS 3366 55 East Jackson Blvd., Suite 600 Chicago, Illinois 60604 312/346-8931	PLOT SCALE = 8:0.0000 '1' / in.	CHECKED - DNB	REVISED -
	PLOT DATE = 10/7/2016	DRAWN - R.V.EJAR	REVISED -
		CHECKED - BCS	REVISED -

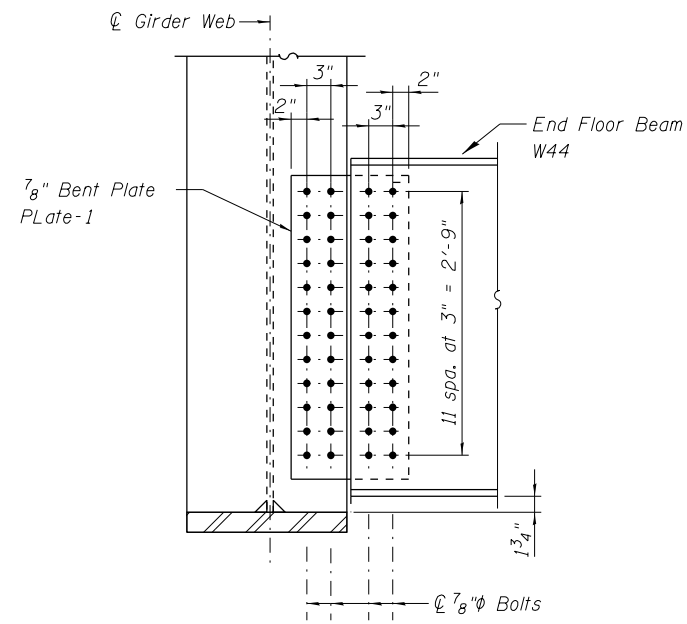
**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**STEEL DETAILS - 1
SHOOFLY BRIDGE OVER IL RTE 132**

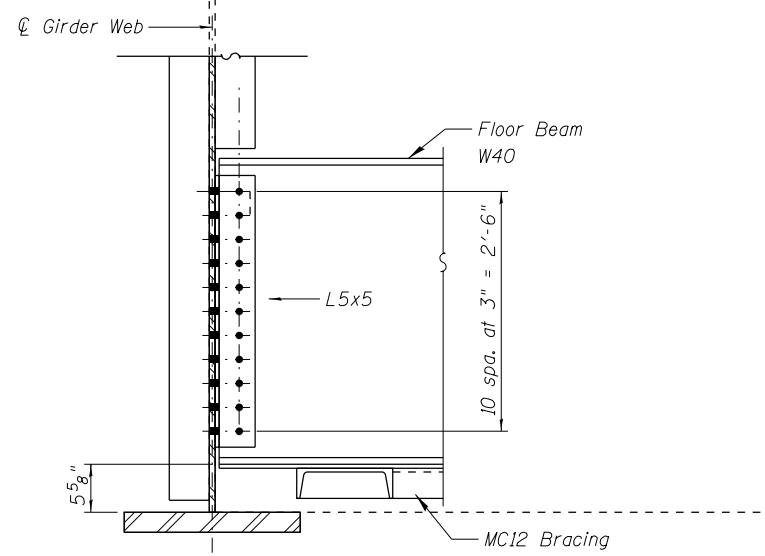
SHEET NO. 7 OF 25 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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CONTRACT NO. 60K80				

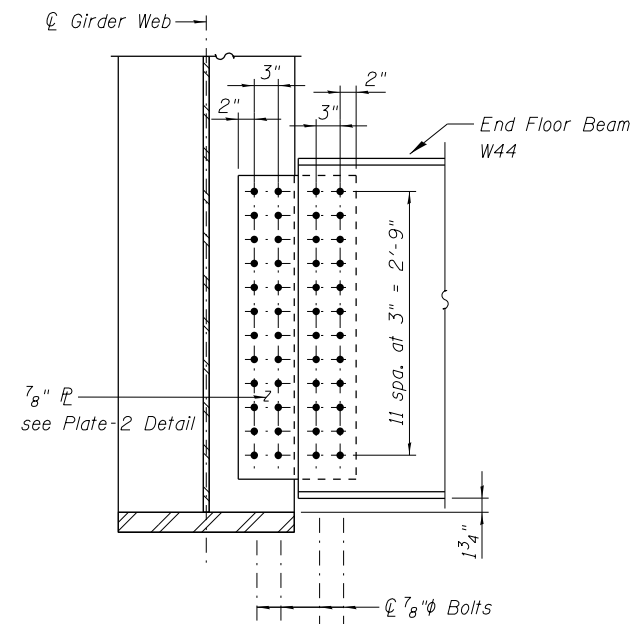
ILLINOIS FED. AID PROJECT



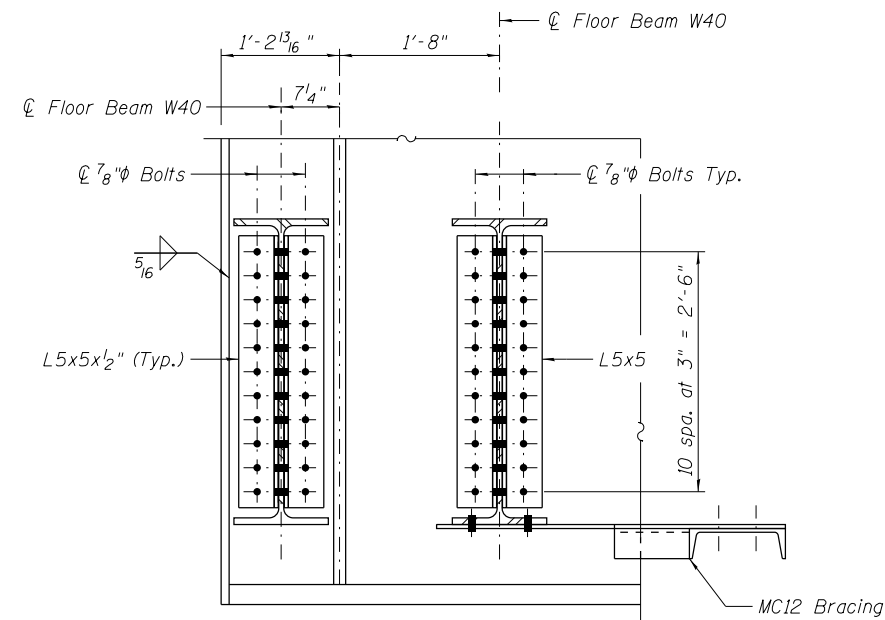
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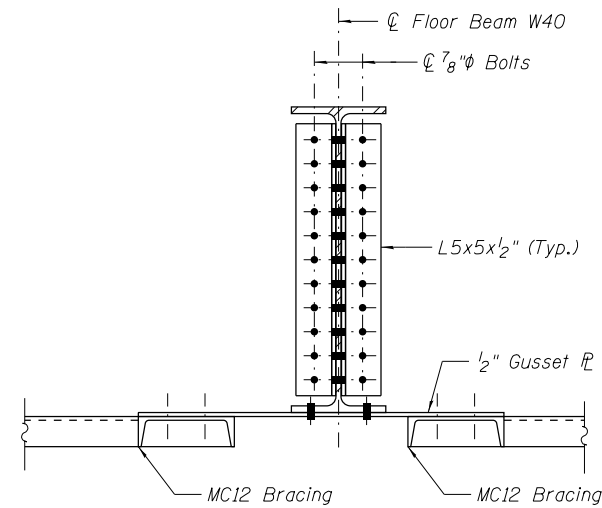
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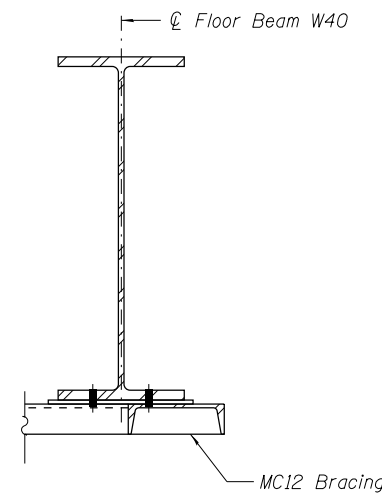
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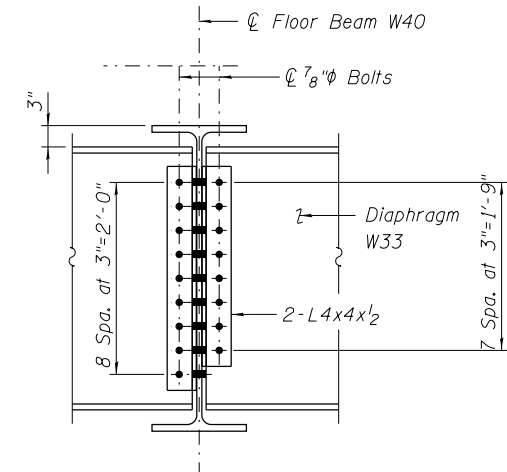
SECTION D-D
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SECTION E-E
Scale: 1"=1'-0"

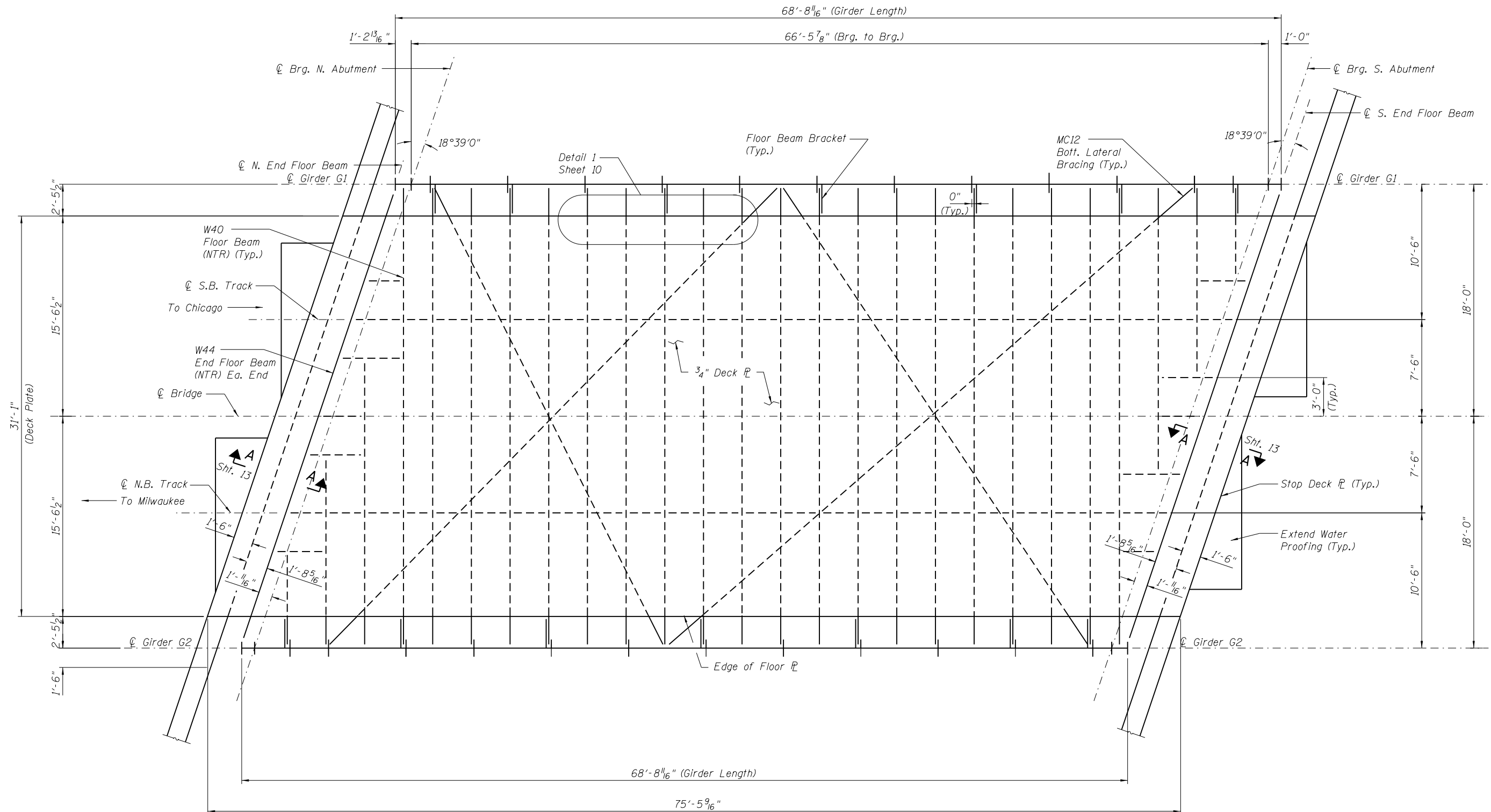
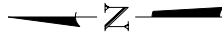


SECTION F-F
Scale: 1"=1'-0"



SECTION G-G
Scale: 1"=1'-0"

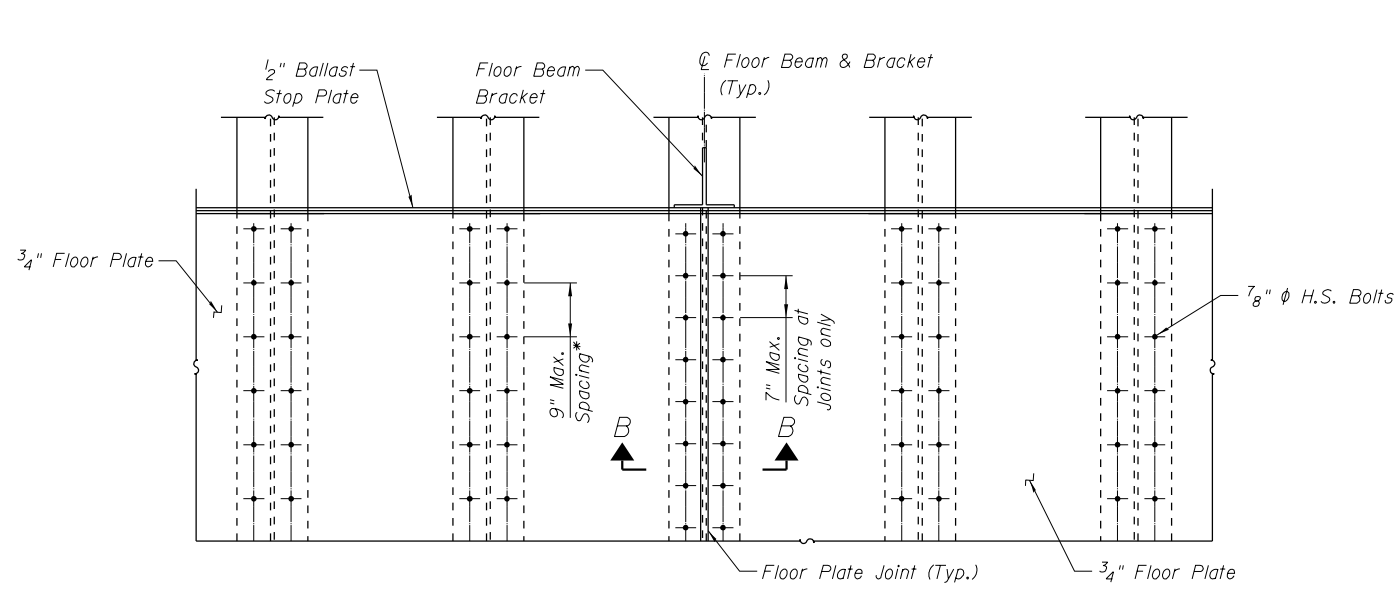
NOTE:
Work this Sheet with Sheet 4 & 7.



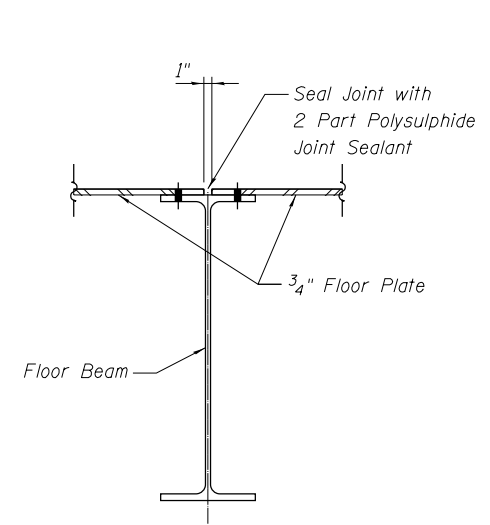
FLOOR PLATE PLAN
Scale: 1/4"=1'-0"

NOTE:
See Sheet 13 for Section A-A.

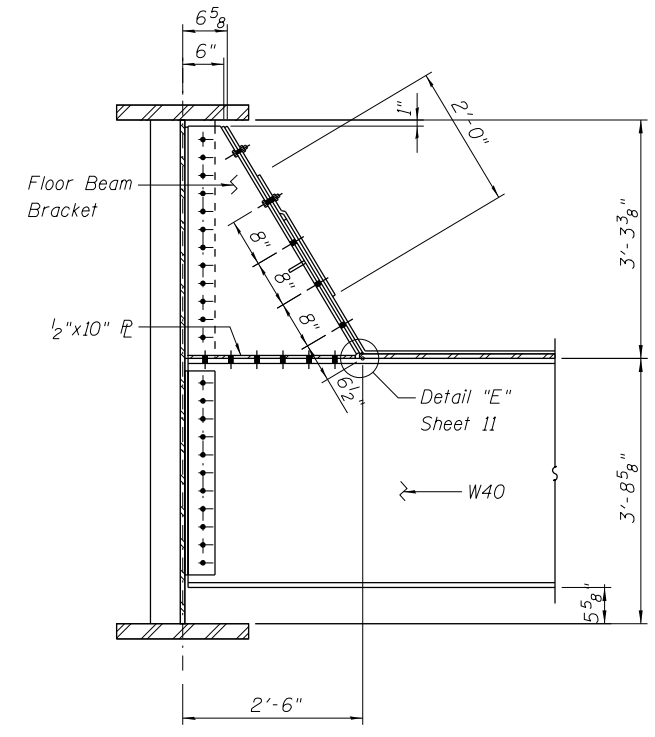
FILE NAME - ...SHOOFLY-60K80-009.dgn HOH HARRY O. HEFTER ASSOCIATES, INC. DESIGN AND CONSULTING ENGINEERS 3366 55 East Jackson Blvd., Suite 600 Chicago, Illinois 60604 312/346-8931	USER NAME = aefitzpatrick	DESIGNED - MMH	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	FLOOR PLATE PLAN SHOOFLY BRIDGE OVER IL RTE 132	F.A.P. RTE. 346	SECTION 125X-N&J-SB-B	COUNTY LAKE	TOTAL SHEETS 361	SHEET NO. 267
	PLOT SCALE = 8:0.0000 '1' / in. PLOT DATE = 10/7/2016	CHECKED - DNB DRAWN - R.VEJAR CHECKED - BCS	REVISED - REVISED - REVISED -			SHEET NO. 9 OF 25 SHEETS	CONTRACT NO. 60K80		ILLINOIS FED. AID PROJECT	



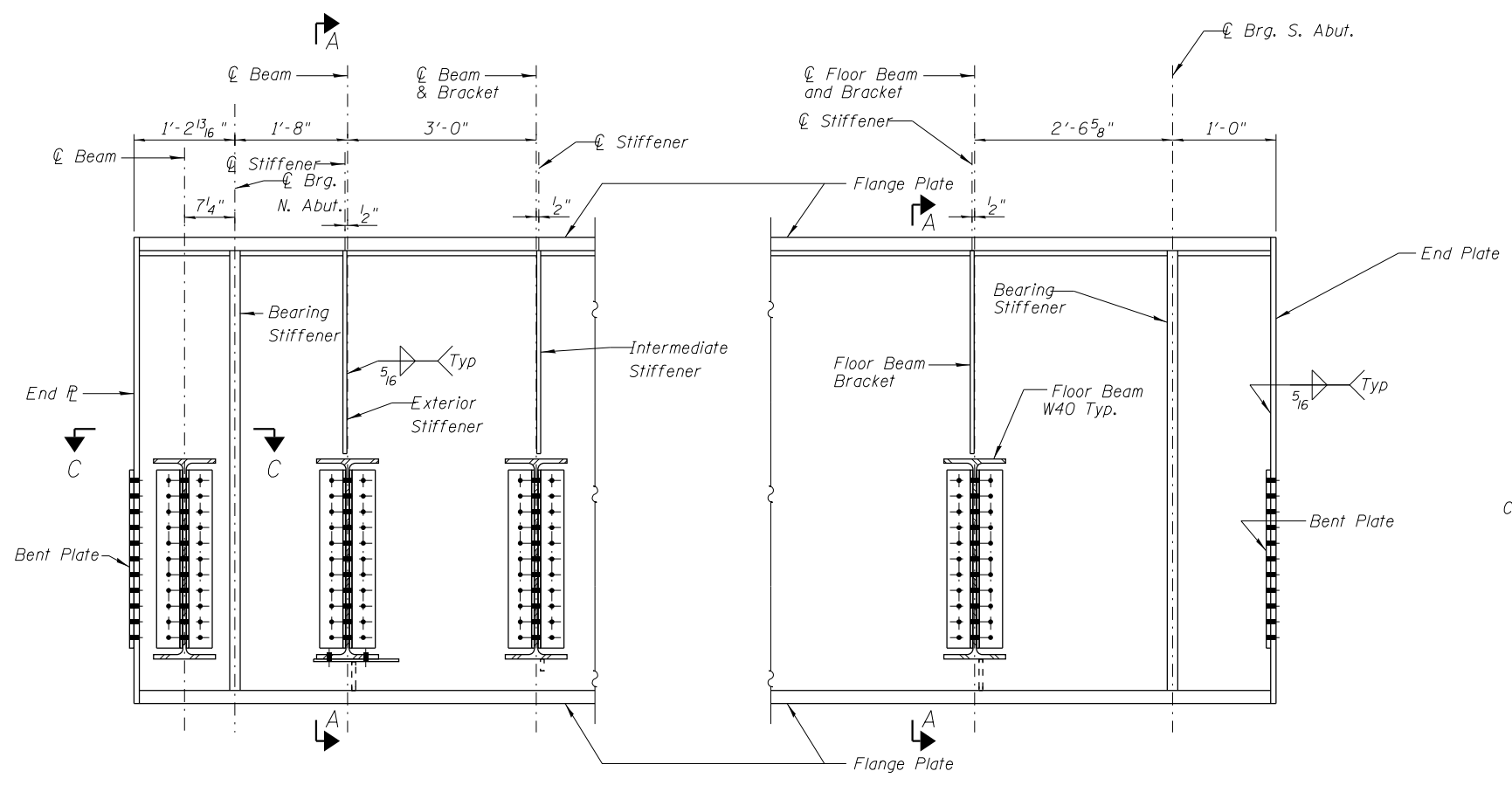
DETAIL 1
TYPICAL FLOOR PLATE CONNECTION PLAN
 Scale: 3/4" = 1'-0"



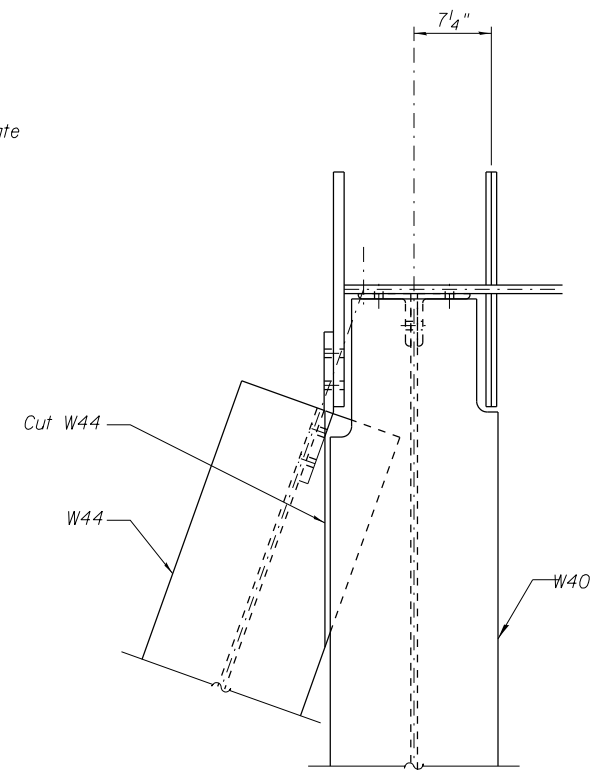
SECTION B-B
TYPICAL FLOOR PLATE JOINT DETAIL
 Scale: 1" = 1'-0"



SECTION A-A
 Scale: 3/4" = 1'-0"

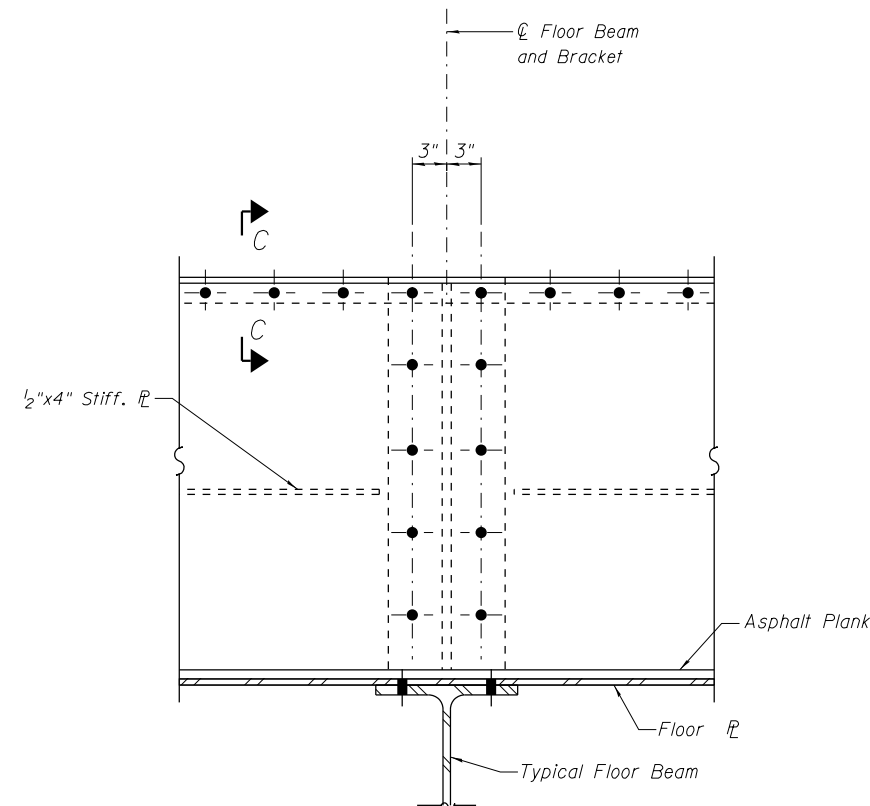


INSIDE ELEVATION - GIRDER G1
 (Girder G2 Similar by 180° Rotation)
 Scale: 3/4" = 1'-0"

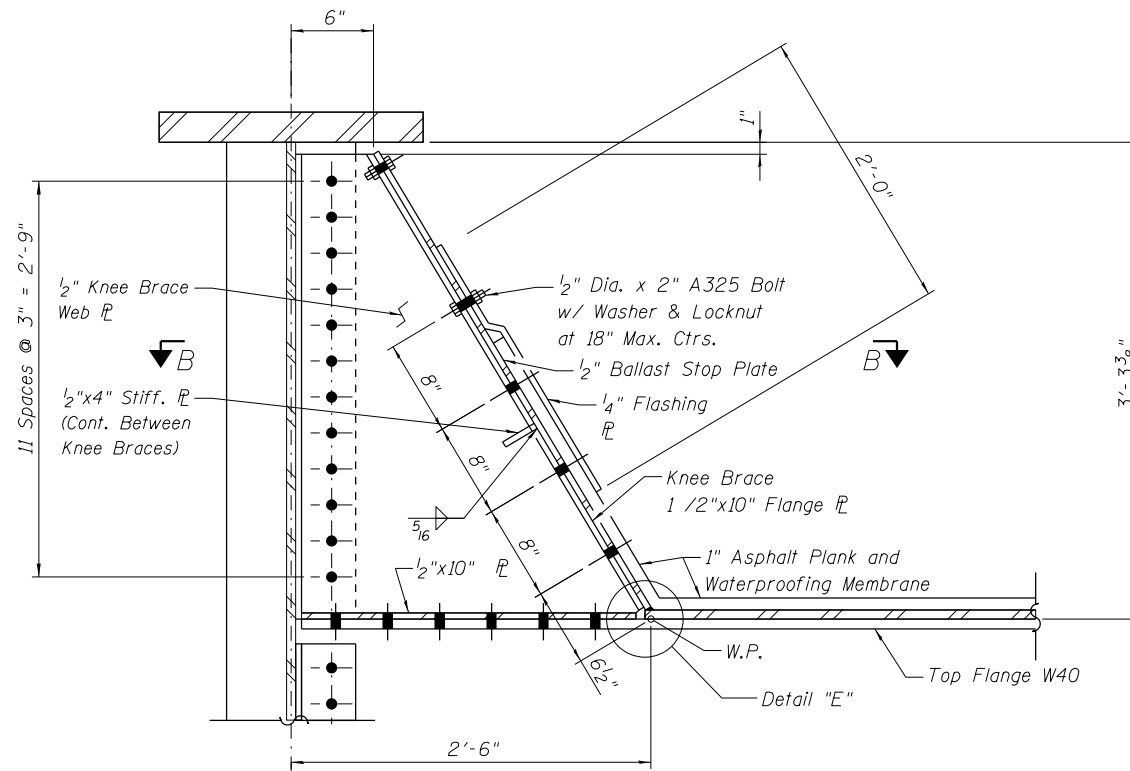


SECTION C-C

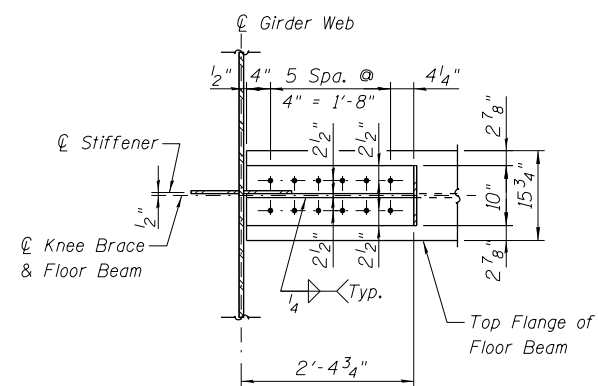
FILE NAME - ...SHOOFLY-60K80-010.dgn HOH HARRY O. HEFTER ASSOCIATES, INC. DESIGN AND CONSULTING ENGINEERS 3366 55 East Jackson Blvd., Suite 600 Chicago, Illinois 60604 312/346-8931	USER NAME = aefitzpatrick DESIGNED - MMH CHECKED - DNB DRAWN - R.VEJAR CHECKED - BCS	REVISED - REVISED - REVISED - REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	SECTIONS AND DETAILS - 1 SHOOFLY BRIDGE OVER IL RTE 132 SHEET NO. 10 OF 25 SHEETS	F.A.P. RTE. 346 SECTION 125X-N&J-SB-B COUNTY LAKE TOTAL SHEETS 361 SHEET NO. 268 CONTRACT NO. 60K80	ILLINOIS FED. AID PROJECT
	PLOT SCALE = 2.0000" / 1" PLOT DATE = 10/7/2016					



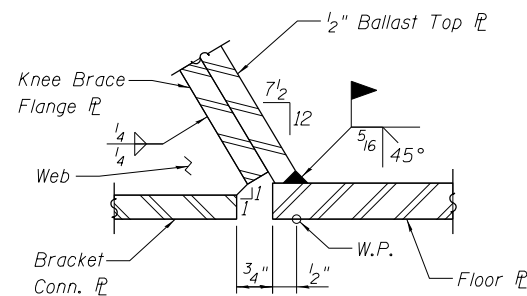
BALLAST PLATE ELEVATION
 (At Knee Brace Typical)
 Scale: 1/2"=1'-0"



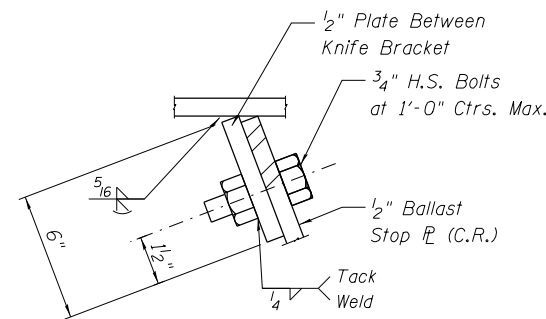
FLOOR BEAM BRACKET DETAIL
 Scale: 1/2"=1'-0"



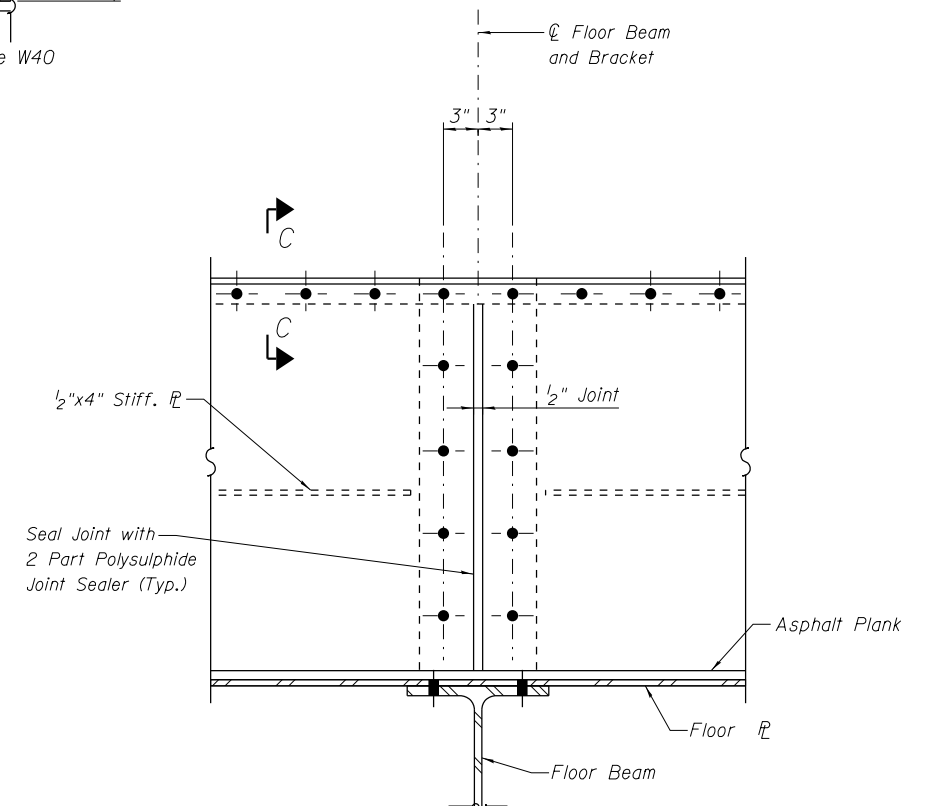
SECTION B-B
 Scale: 3/4"=1'-0"



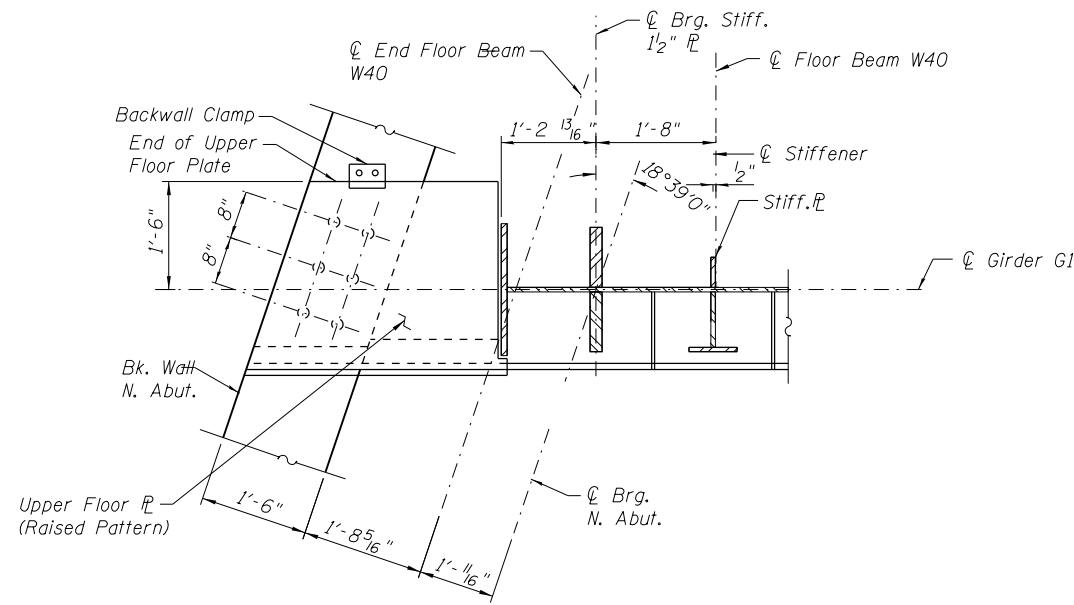
DETAIL "E"
 N.T.S.



SECTION C-C

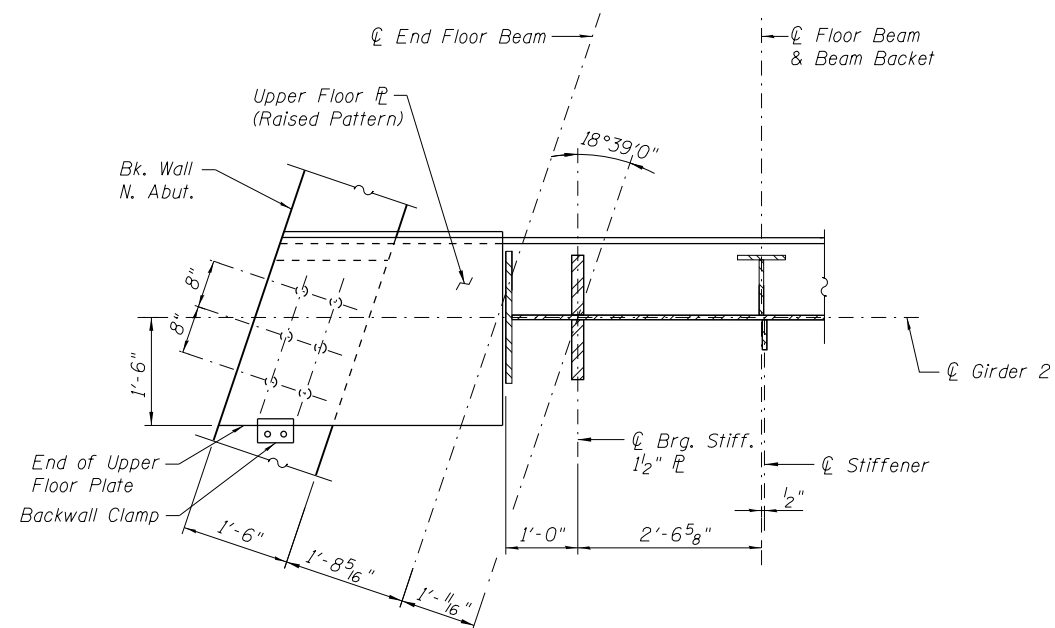
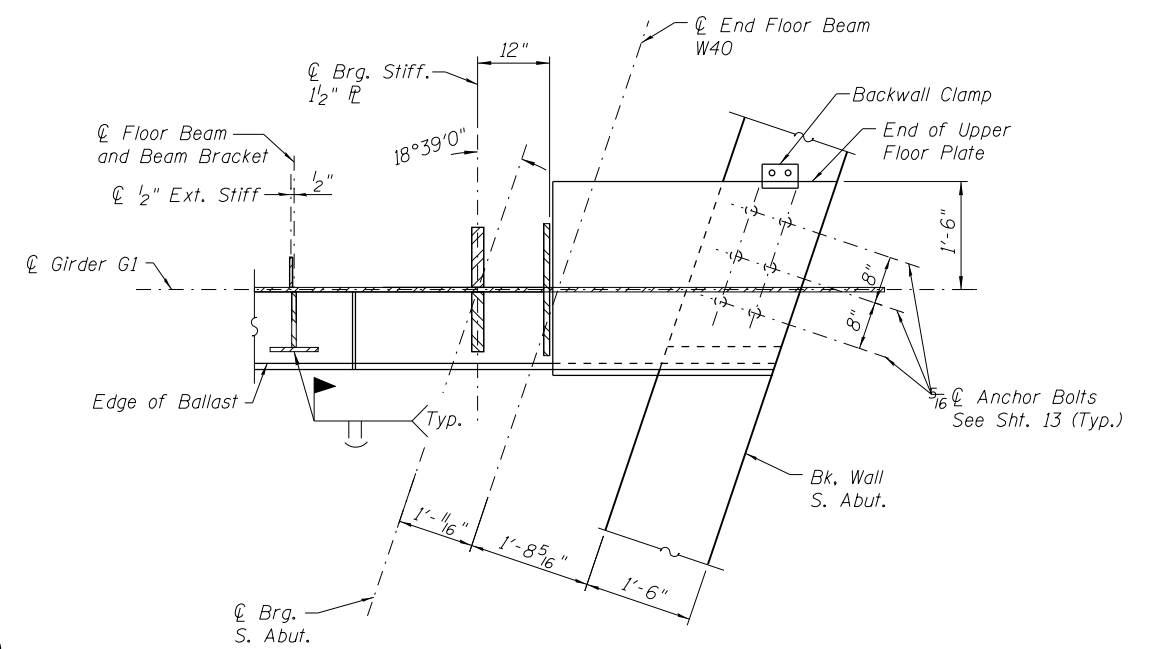


ELEVATION AT BALLAST STOP PLATE SPLICE
 Scale: 1/2"=1'-0"



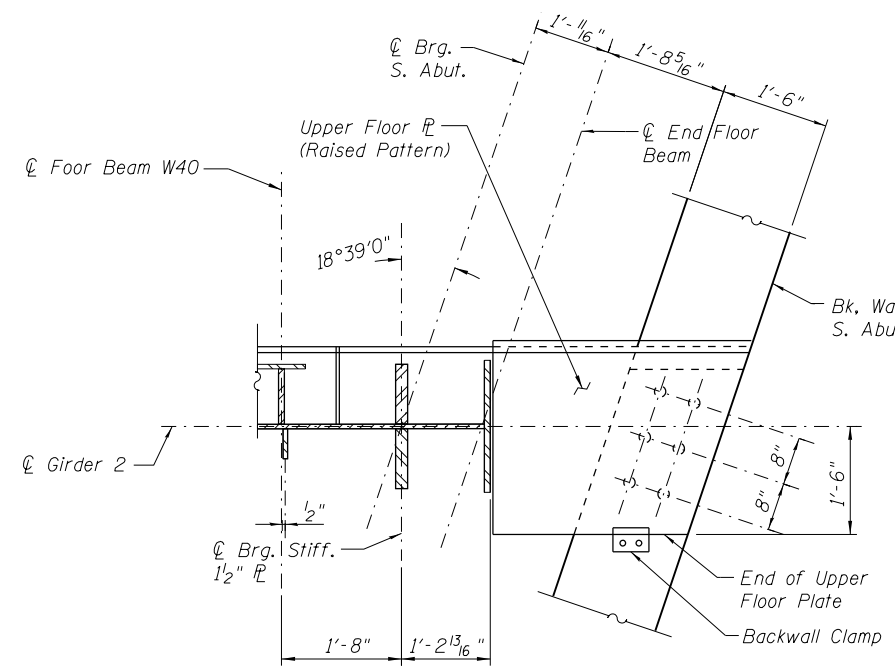
UPPER FLOOR PLATE PLAN AT GIRDER G1 (PART PLAN)

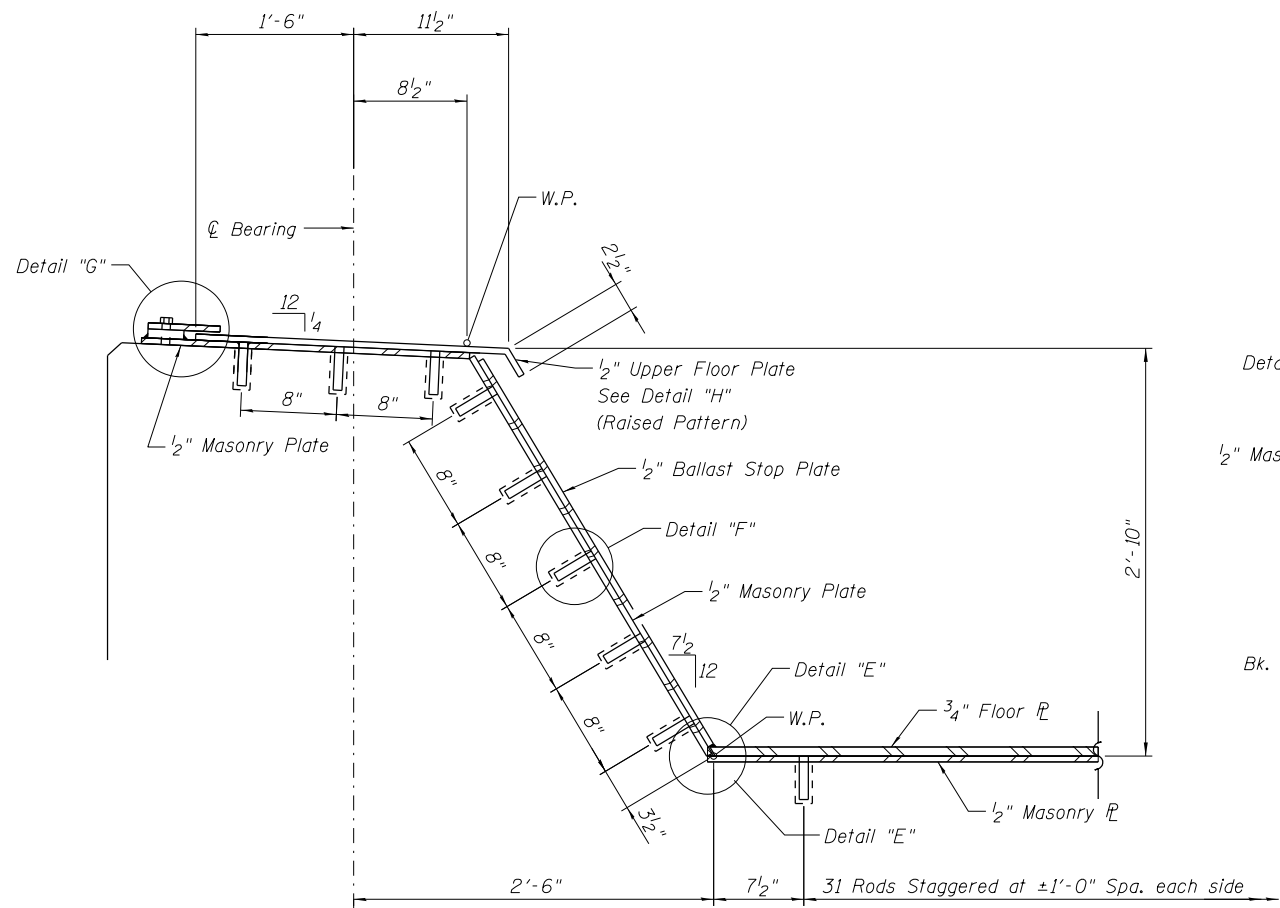
Scale: 1/4"=1'-0"



UPPER FLOOR PLATE PLAN AT GIRDER G2 (PART PLAN)

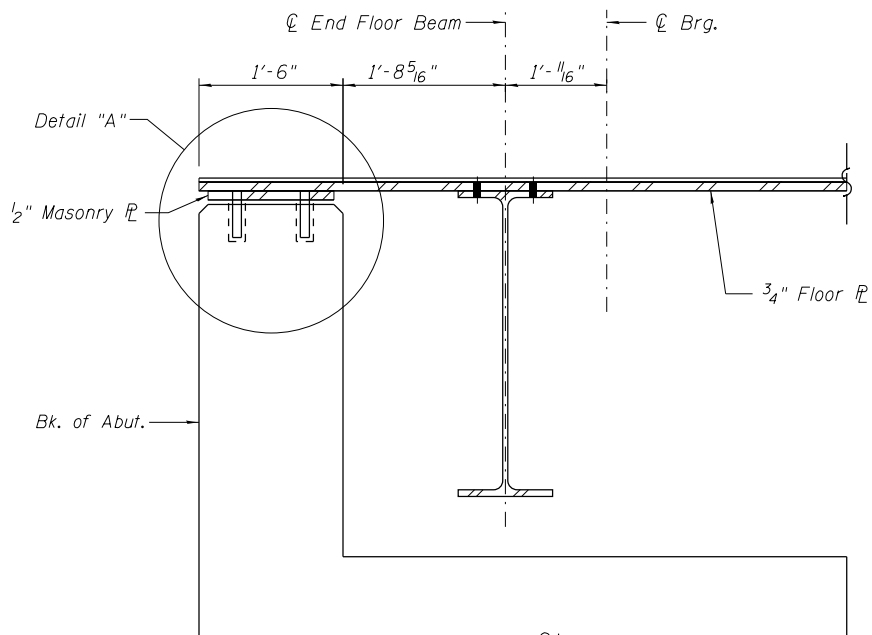
Scale: 1/4"=1'-0"





BALLAST STOP PLATE DETAIL

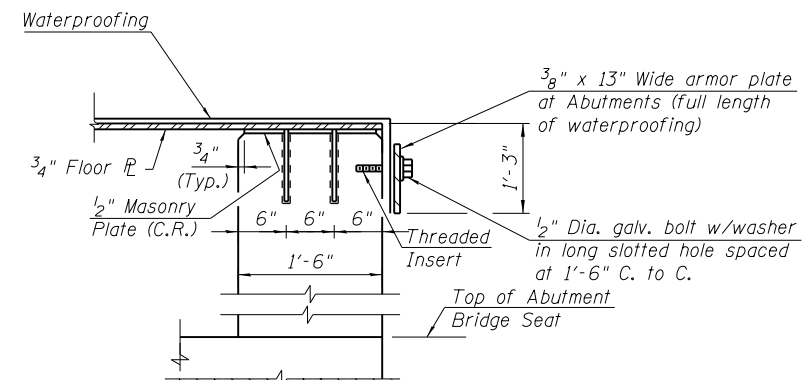
N.T.S.



SECTION A-A

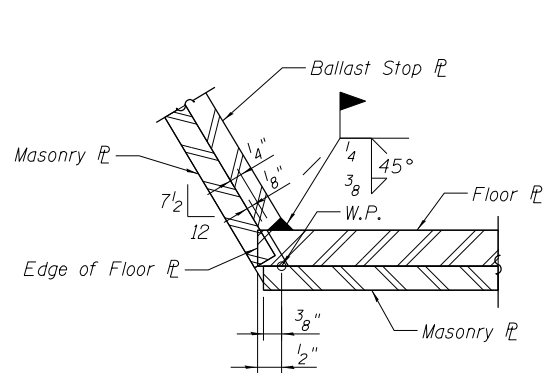
N.T.S.

NOTE:
For Waterproofing at abutment wall see Detail A.



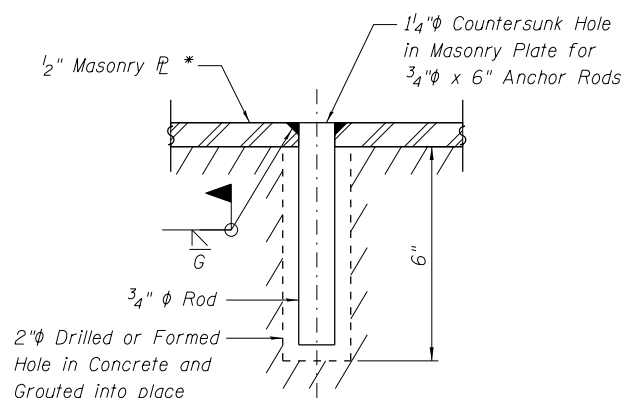
WATERPROOFING AT ABUTMENT BACKWALL

DETAIL A



DETAIL "E"

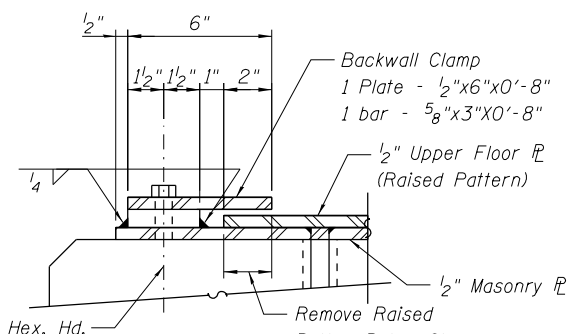
N.T.S.



DETAIL "F"

N.T.S.

* Masonry plates to be welded to anchor rods in field and ground smooth.

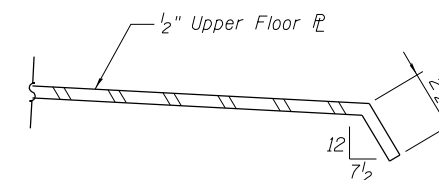


DETAIL "G"

N.T.S.

2 - 3/4" Bolts, Hex. Hd.
1 - Std. Washer
1 - Lock Washer
Drill & Tap 1/2"
Masonry P

Note:
Hold 1/8" gap between backwall clamp and upper floor plate at expansion end.



DETAIL "H"

N.T.S.

**PLATE GIRDER
MOMENT AND SHEAR**

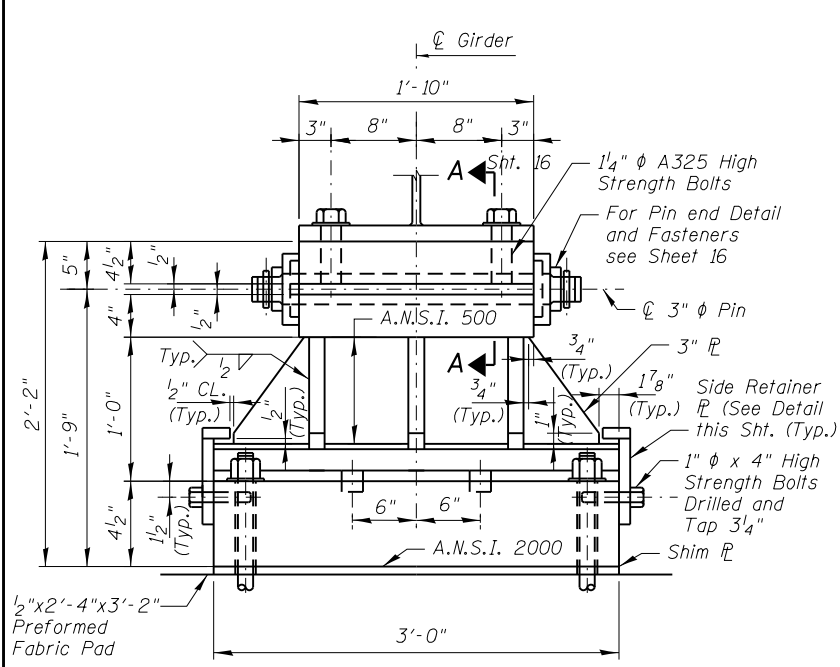
MOMENT	
Description	Girder (A709, GR.50)
Dead Load	4,046 ft-k
Live Load (E80)	6,233 ft-k
Impact	1,977 ft-k
Total	12,256 ft-k
Web	13/16 "
Flange	2 1/2"x22"
Gross I Furn.	246,000 in ⁴
Net I Furn.	242,692 in ⁴
Net Sect. Mod. - Bot.	5,527 in ³
Allow Max. Tensile Stress in Flange	27.5 ksi
Actual Max. Tensile Stress in Flange	26.6 ksi
Allow Max. Deflection (Live Load + Impact)	1.22 in
Actual Max. Deflection (Live Load + Impact)	0.81 in
SHEAR	
Dead Load	243 k
Live Load (E80)	417 k
Impact	133 k
Total	793 k
Web Shear	11.7 ksi
End Stiffener Column Area Reqd.	19.3 in ²
Section (2 Plates)	1 1/2"x10 1/2"
End Stiffener Column Area Furn.	39.4 in ²

**FLOOR BEAM
MOMENT AND SHEAR**

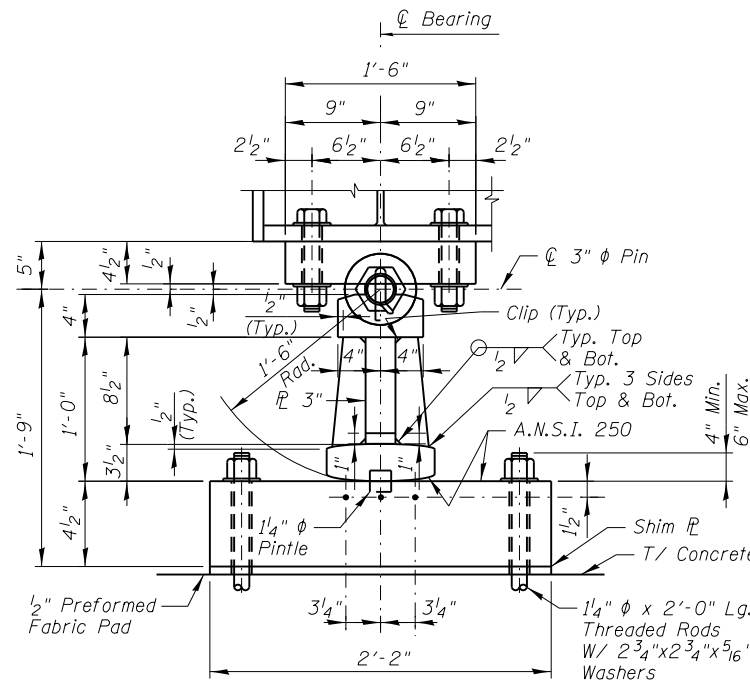
MOMENT		
Description	Typical Floor Beam (A709, GR.50) W40x215	End Floor Beam (A709, GR.50) W44x230
Dead Load	209.8 ft-k	421.1 ft-k
Live Load (E80)	724.5 ft-k	764.7 ft-k
Impact	245 ft-k	258.6 ft-k
Total	1179.3 ft-k	1,444.4 ft-k
Section	W40x215	W44x230
Gross I Furn.	16,700 in ⁴	20,800 in ⁴
Net I Furn.	15,571 in ⁴	20,140 in ⁴
Net Sect. Mod. - Bot.	859 in ³	971 in ³
Allow Max. Tensile Stress in Flange	27.5 ksi	27.5 ksi
Actual Max. Tensile Stress in Flange	16.5 ksi	17.85 ksi
Allow Max. Deflection (Live Load + Impact)	0.675 in	0.755 in
Actual Max. Deflection (Live Load + Impact)	0.61 in	0.574 in
SHEAR		
Dead Load	19.9 k	39.1 k
Live Load (E80)	69.0 k	69.0 k
Impact	23.3 k	23.3 k
Knee Brace	63.6 k	—
Total	175.8 k	131.4 k
Web Shear	6.93 ksi	4.31 ksi

BEARING ON MASONRY

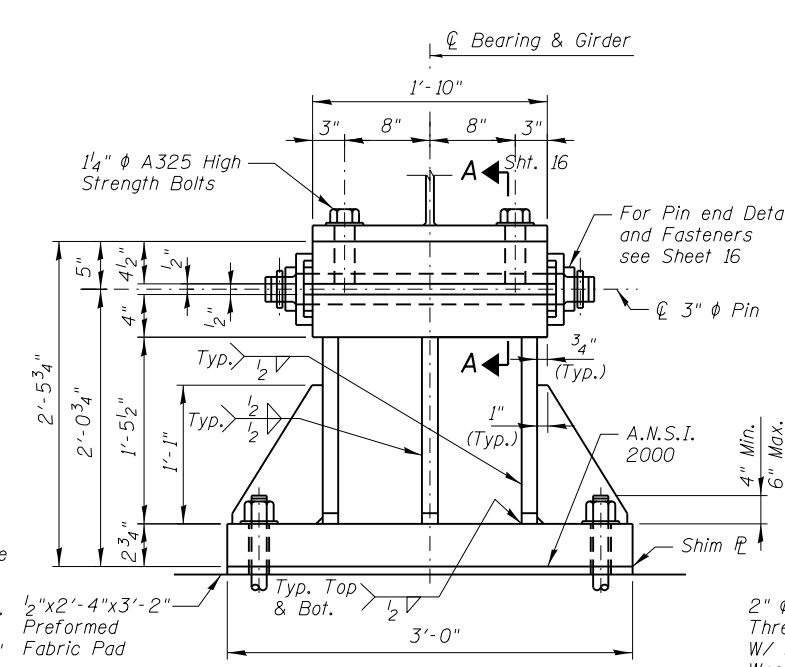
Description	Girder G1 or G2
Total Reaction	790.2 k
Bearing Area Furnished	936 in ²
Average Brg. Pressure	0.84 ksi



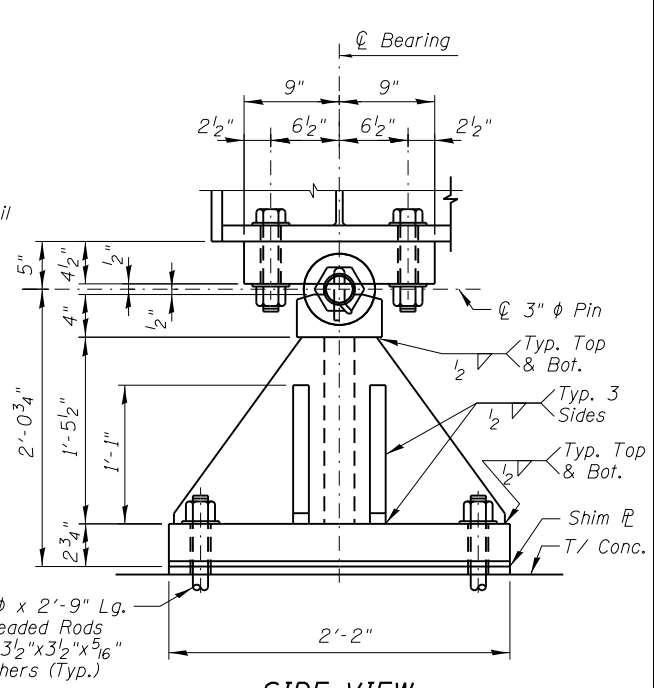
ELEVATION
(EXPANSION BEARING)
(S. Abutment)



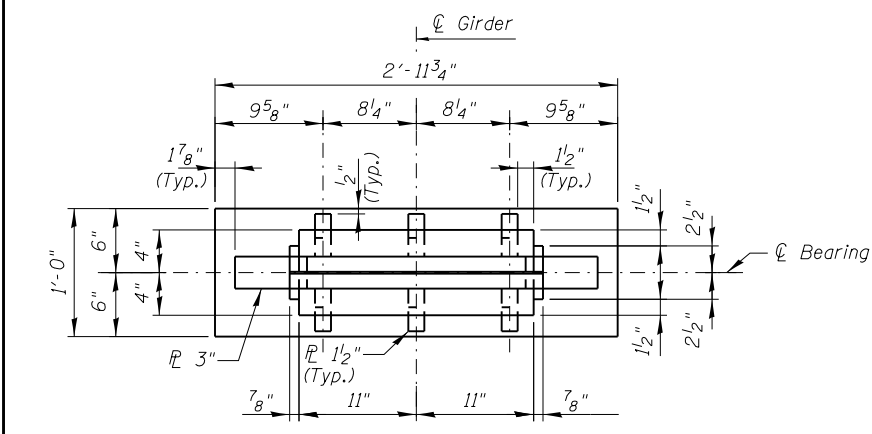
SIDE VIEW
(EXPANSION BEARING)



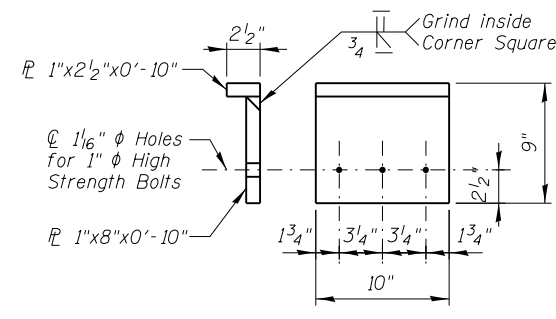
ELEVATION
(FIXED BEARING)
(N. Abutment)



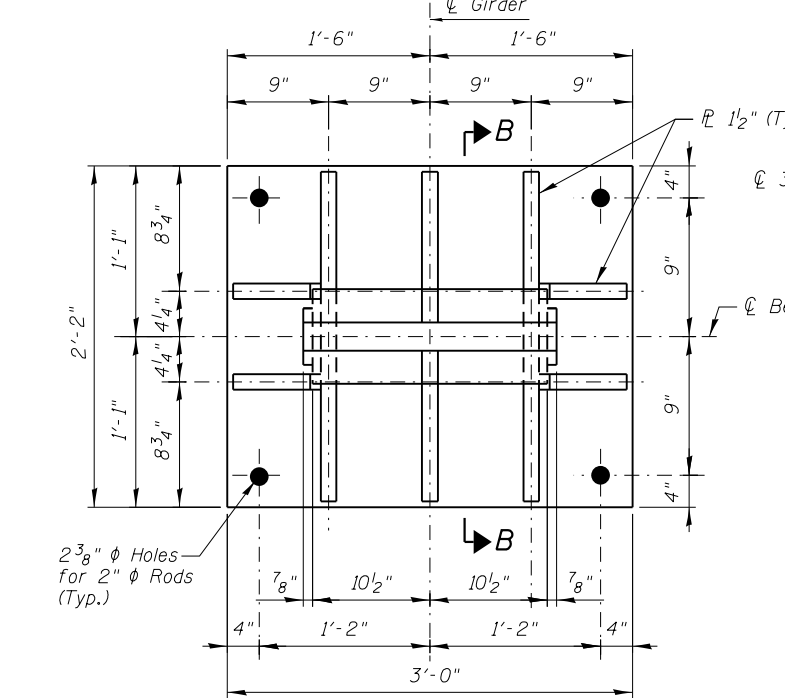
SIDE VIEW
(FIXED BEARING)



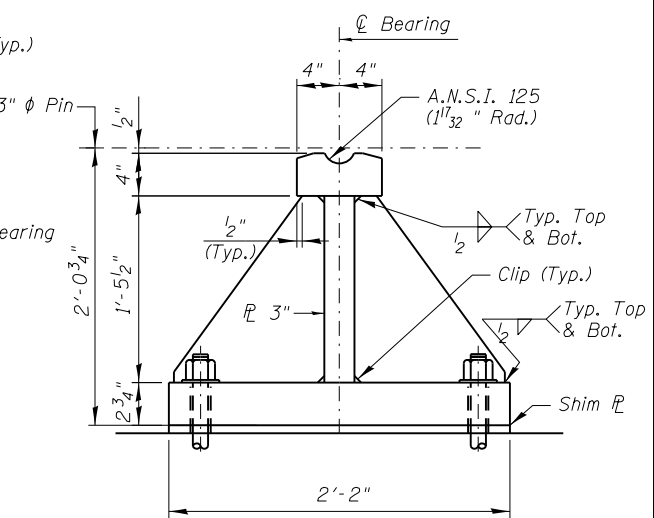
LOWER BEARING ASSEMBLY
(EXPANSION BEARING)



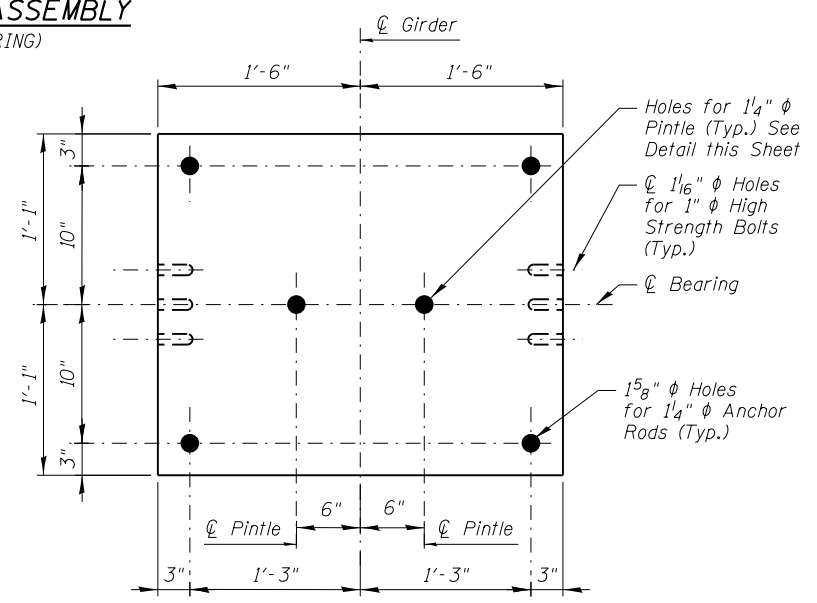
SIDE RETAINER DETAIL
(EXPANSION BEARING)



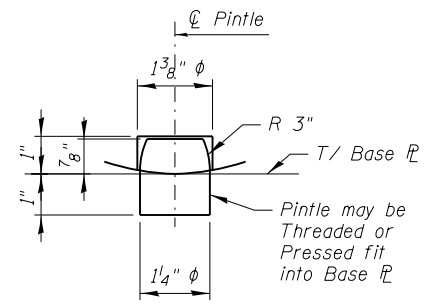
BOTTOM PLAN
(FIXED BEARING)



SECTION B-B
(FIXED BEARING)



BASE PLATE PLAN
(EXPANSION BEARING)



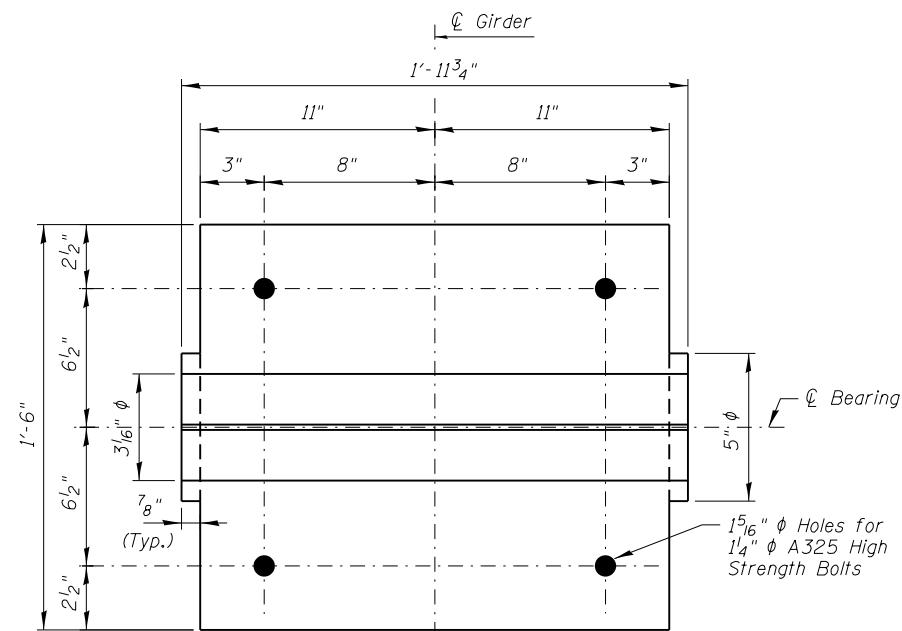
PINTLE DETAIL
(EXPANSION BEARING)

BILL OF MATERIAL

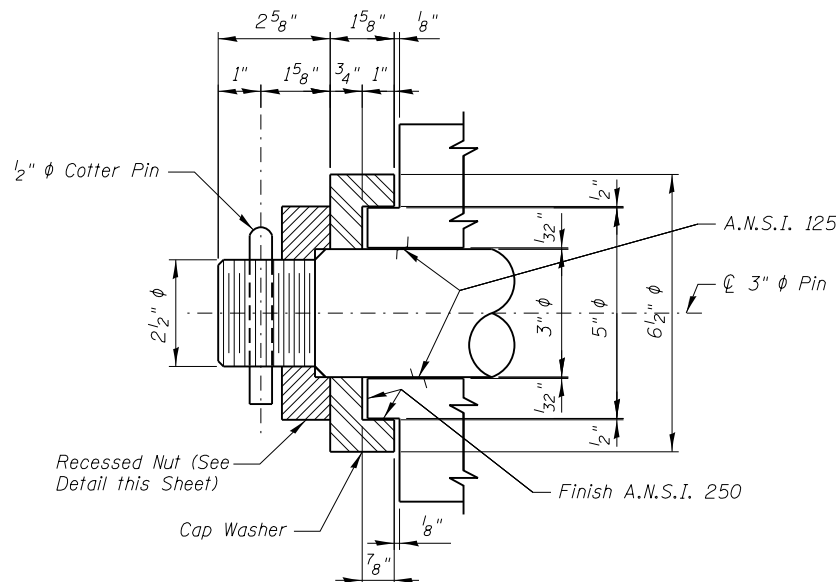
ITEM	UNIT	QUANTITY
Anchor Bolt 1/4"	Each	8
Anchor Bolt 2"	Each	8

NOTES :

1. For Section A-A and Upper Bearing Assembly, See Sheet No. 16.
2. For Additional Notes See Sheet No. 16.
3. Grease bearing assembly before installation.
4. Grease hose/fitting shall be steel side of bearing.
5. Bearings shall be blocked during construction.



**UPPER BEARING ASSEMBLY
BOTTOM VIEW**



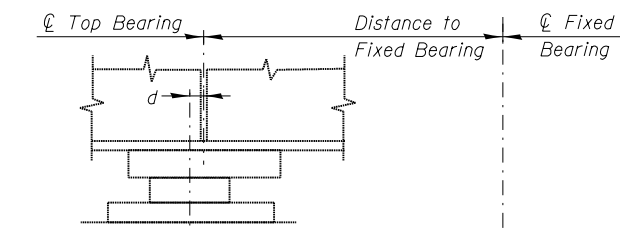
PIN END DETAIL

NOTES :

1. Pin material must be ASTM A-668 Class F, harden to a Brinell Hardness of 200.
2. Material for bearing component parts must be ASTM A709, Grade 50, except as noted.
3. Bearing assembly weldments must be stress relieved by heat treating per AWS D1.1 prior to finish machining.
4. The parts of members in contact must be faced and bearing surfaces must be planed smooth before any welding is started.
5. Pintles must be stainless steel conforming to ASTM A276, Type 410 annealed.
6. Bearing pins or rockers and all surfaces in contact with pins or rockers must be given an all-over smooth finish.
7. Bearing seat surfaces will be constructed or adjusted to the designated elevations within a tolerance of 1/8". Adjustment must be made by grinding the surface or by shimming the bearing. Two 1/8" adjusting shims, of the dimensions of the bottom bearing plates, will be provided for each bearing in addition to all other plates or shims.
8. Shim plates will be placed between the base plate and the preformed fabric pad. Shim plates will match the footprint of the base plate. Heights of individual shim plates will be a minimum of 3/8" and a maximum of 2". The fabricator will determine the thickness of individual plates to obtain the equivalent shim height required.
9. Concrete surface under bearing to be ground to a smooth level to give even bearing. Laminated fabric pads with 1/2" thickness and specified durometer hardness will be placed under each bearing.
10. The cost of all steel weldments, pins, recessed nuts, cap washers and preformed fabric pads, is included in the cost of "Steel Bearing Assembly".

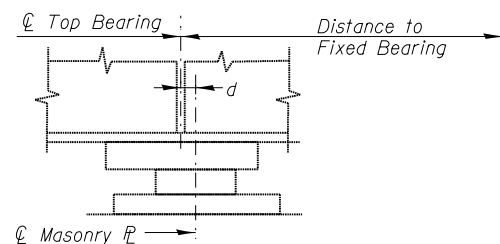
NOTE :

A.I.S.I. 1000 for Plates in Contract Unless noted Otherwise (typ.)



BELOW 50° F

(Move masonry from fixed bearing)

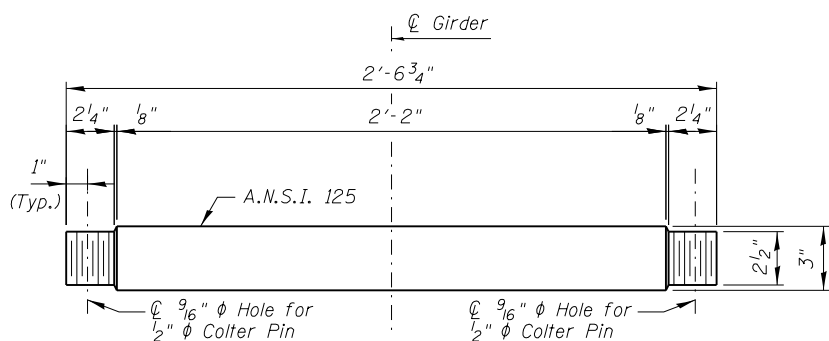


ABOVE 50° F

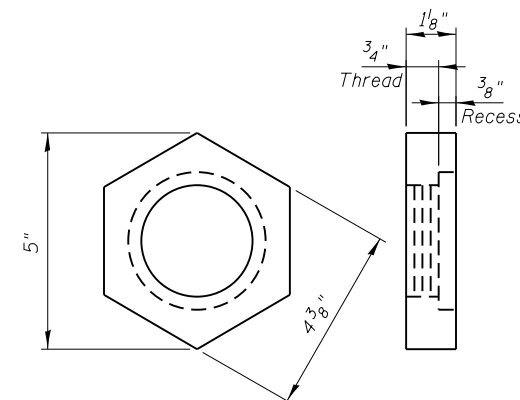
(Move masonry toward fixed bearing)

SETTING ANCHOR BOLTS AT EXPANSION BEARINGS

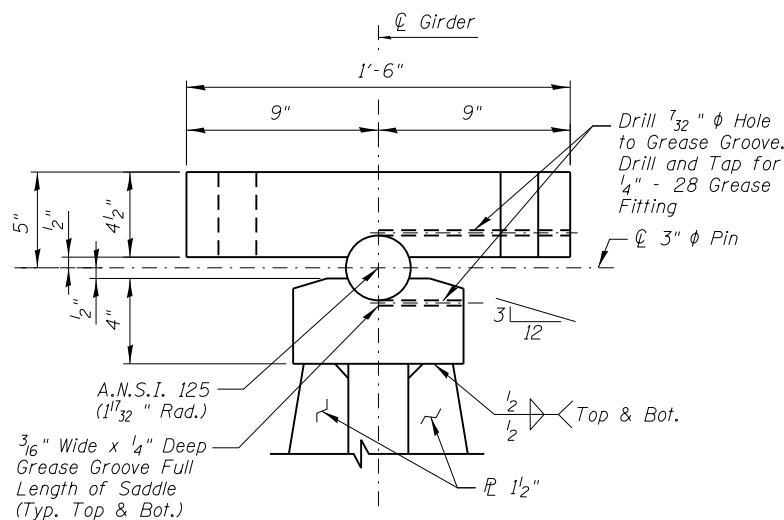
$d = \frac{1}{8}$ " per each 100' of expansion for every 15° F temperature change from the normal temperature of 50° F.



PIN DETAIL



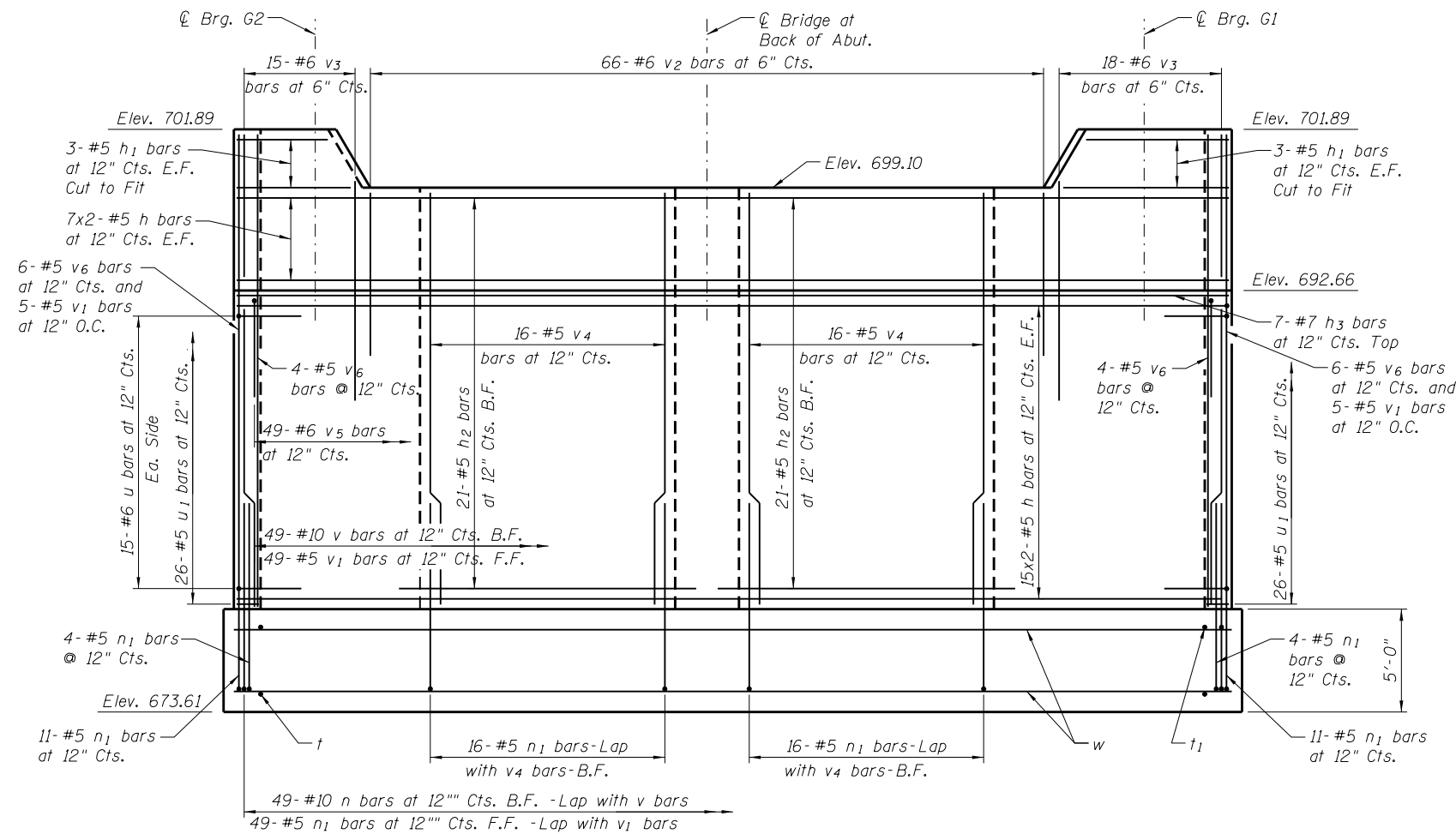
RECESSED NUT DETAIL



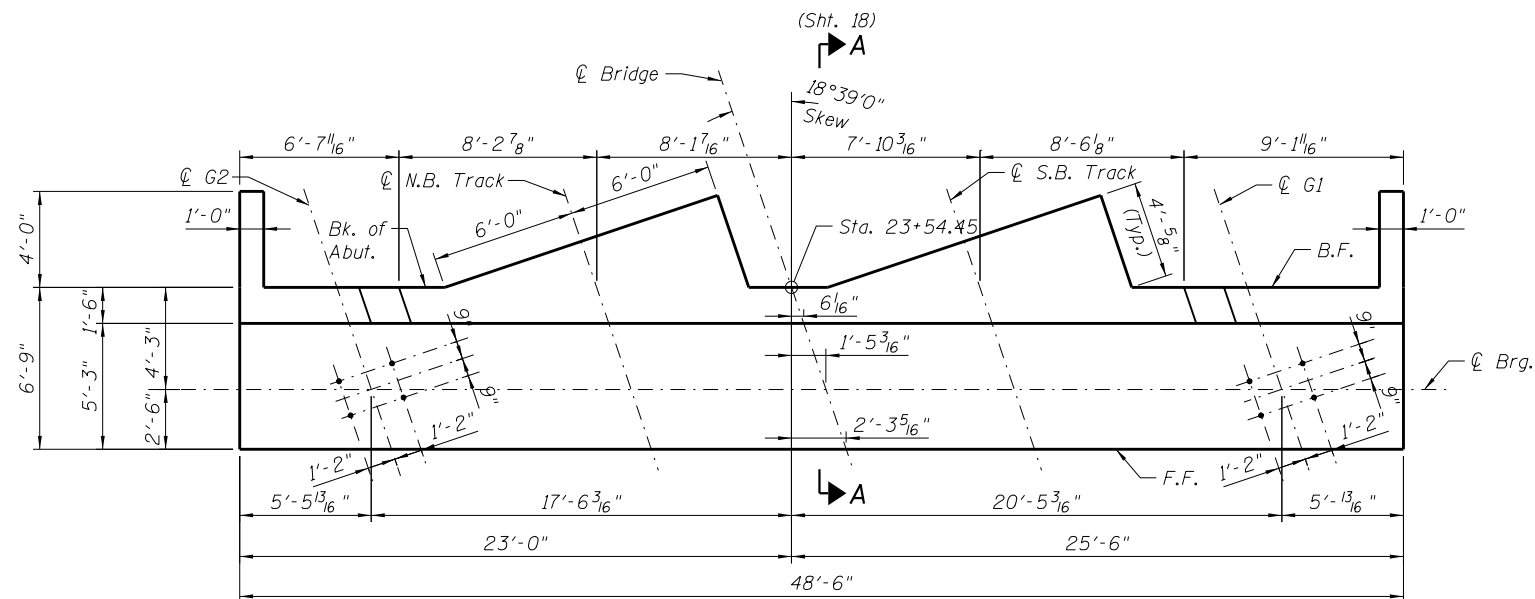
SECTION A-A

BILL OF MATERIAL

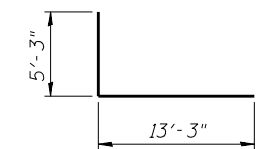
ITEM	UNIT	QUANTITY
Steel Bearing Assembly	Each	4



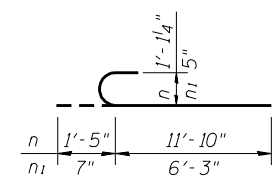
ELEVATION
Scale: 1/4" = 1'-0"



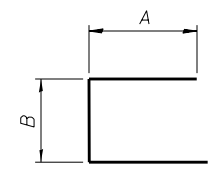
TOP VIEW
Scale: 1/4" = 1'-0"



BAR h2



BAR n, n1



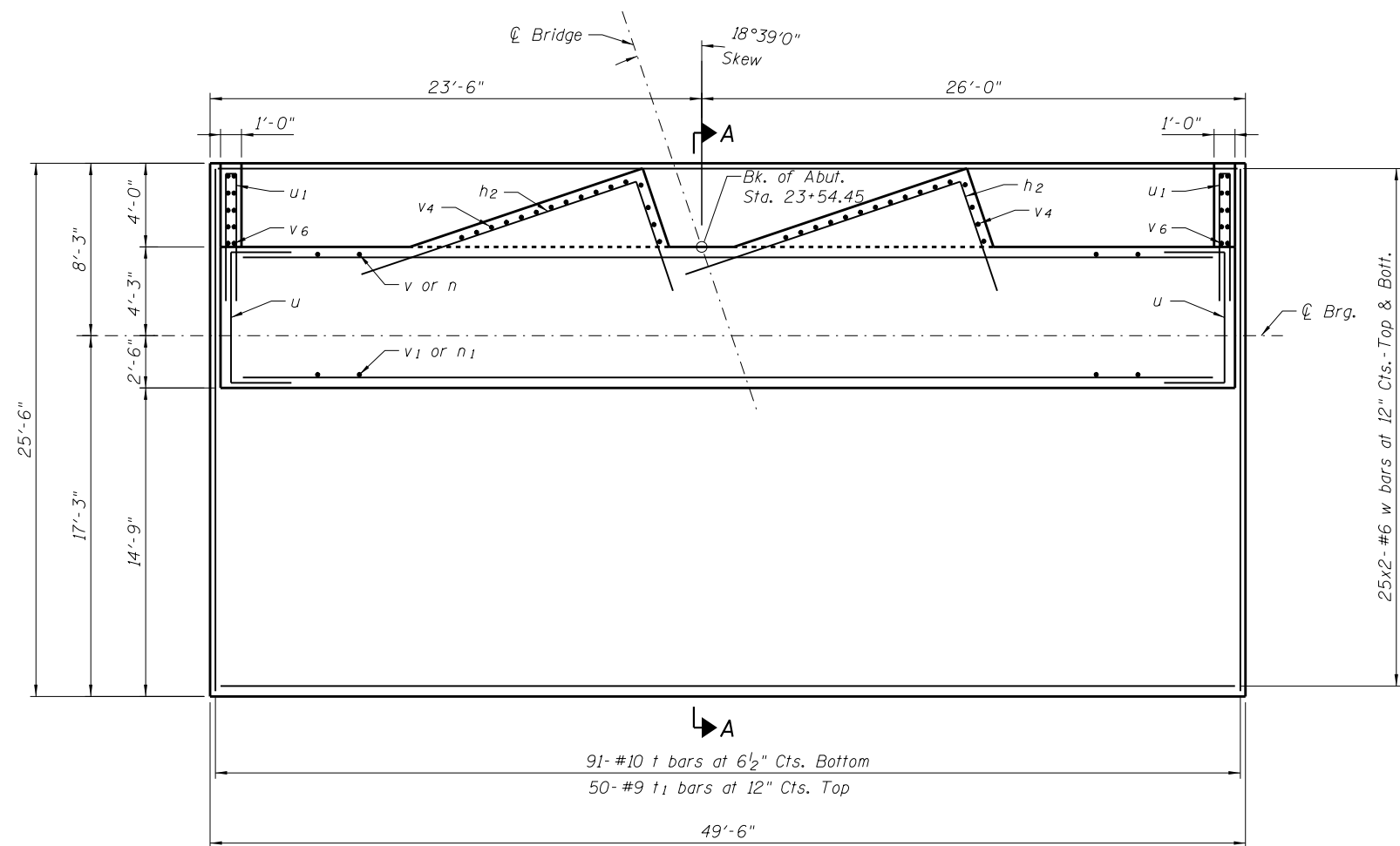
BAR u, u1, v2, v3, v5

BAR DIMENSIONS

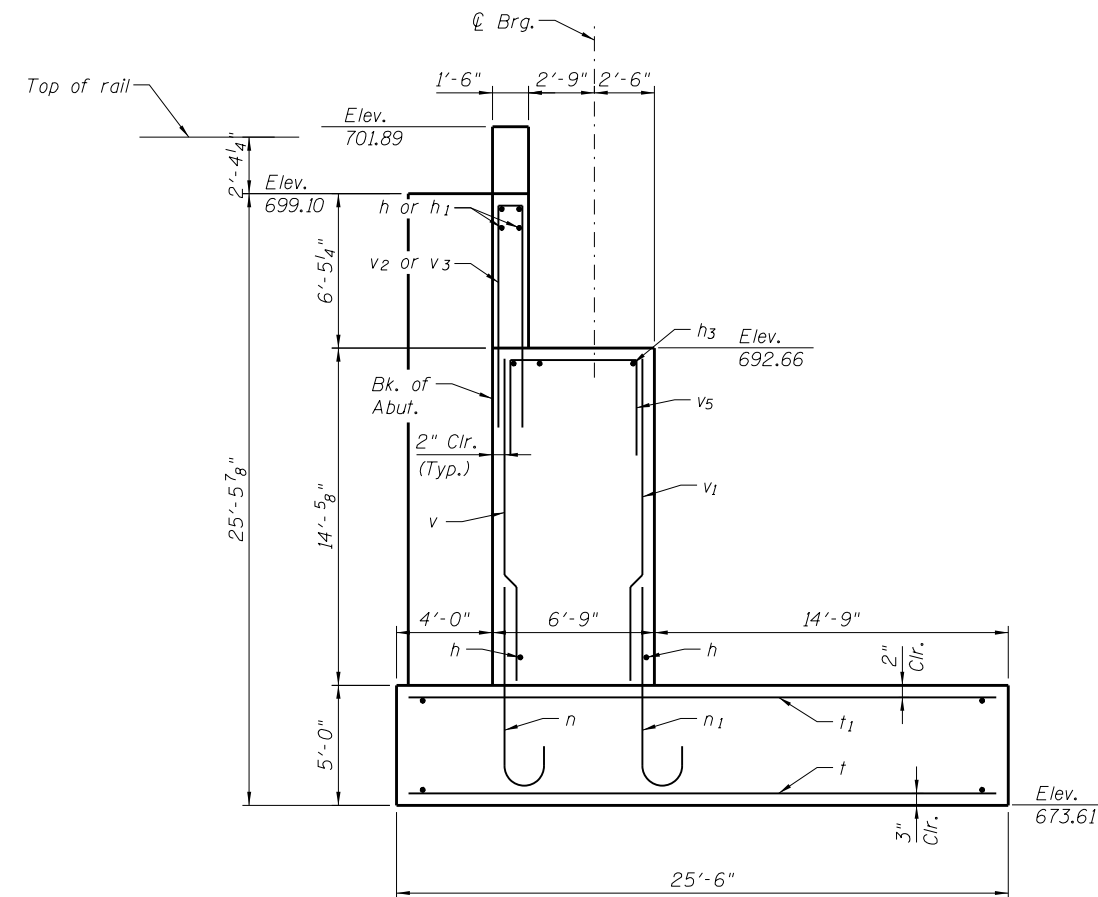
Bar	A	B
u	3'-0"	6'-5"
u1	5'-2"	0'-8"
v2	8'-10"	1'-2"
v3	11'-7"	1'-2"
v5	2'-7"	6'-5"

**ABUTMENT
BILL OF MATERIAL
(TWO ABUTMENTS)**

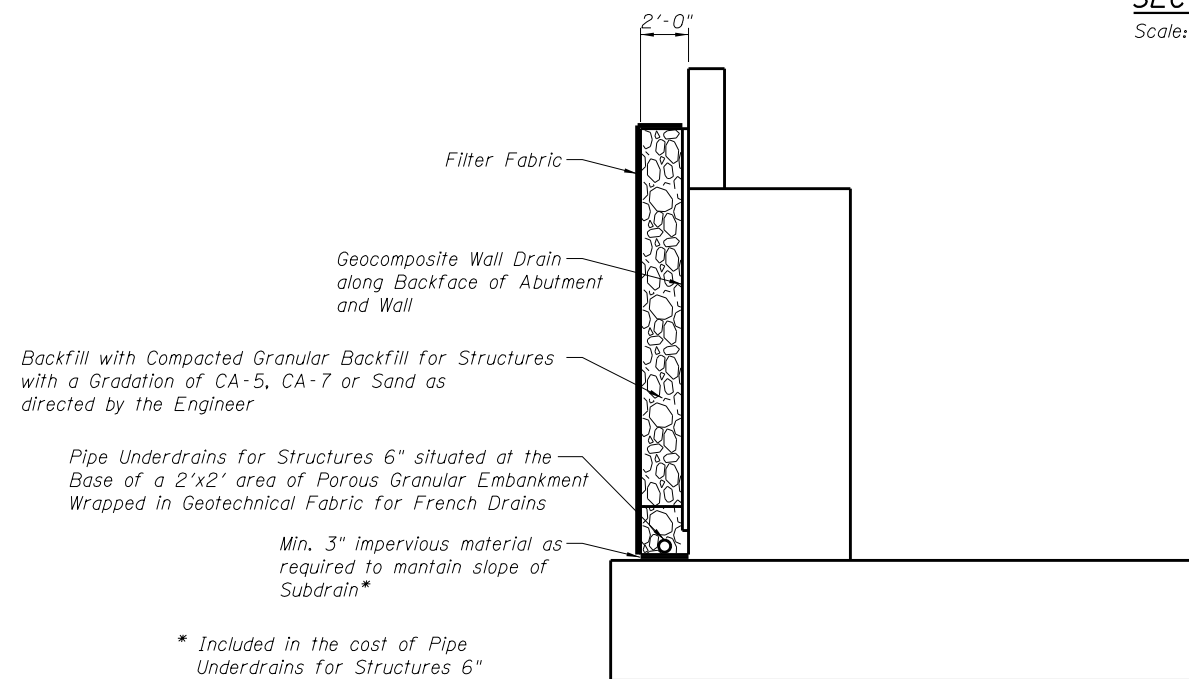
Bar	No.	Size	Length	Shape
h	176	#5	25'-2"	—
h1	24	#5	8'-9"	—
h2	84	#5	18'-6"	L
h3	28	#7	25'-9"	—
n	98	#10	13'-3"	C
n1	222	#5	6'-10"	C
t	91	#10	25'-0"	—
t1	50	#9	25'-0"	—
t2	91	#10	22'-8"	—
t3	50	#9	22'-8"	—
u	60	#6	12'-5"	—
u1	104	#5	11'-0"	—
v	49	#10	13'-10"	—
v1	69	#5	13'-10"	—
v2	132	#6	18'-10"	—
v3	66	#6	24'-4"	—
v4	64	#5	20'-4"	—
v5	98	#6	11'-7"	—
v6	40	#5	25'-4"	—
v7	49	#10	14'-1"	—
v8	49	#5	14'-1"	—
w	200	#6	25'-11"	—
Structure Excavation			Cu. Yd.	1,000
Concrete Structures			Cu. Yd.	919
Reinforcement Bars			Pound	69,837



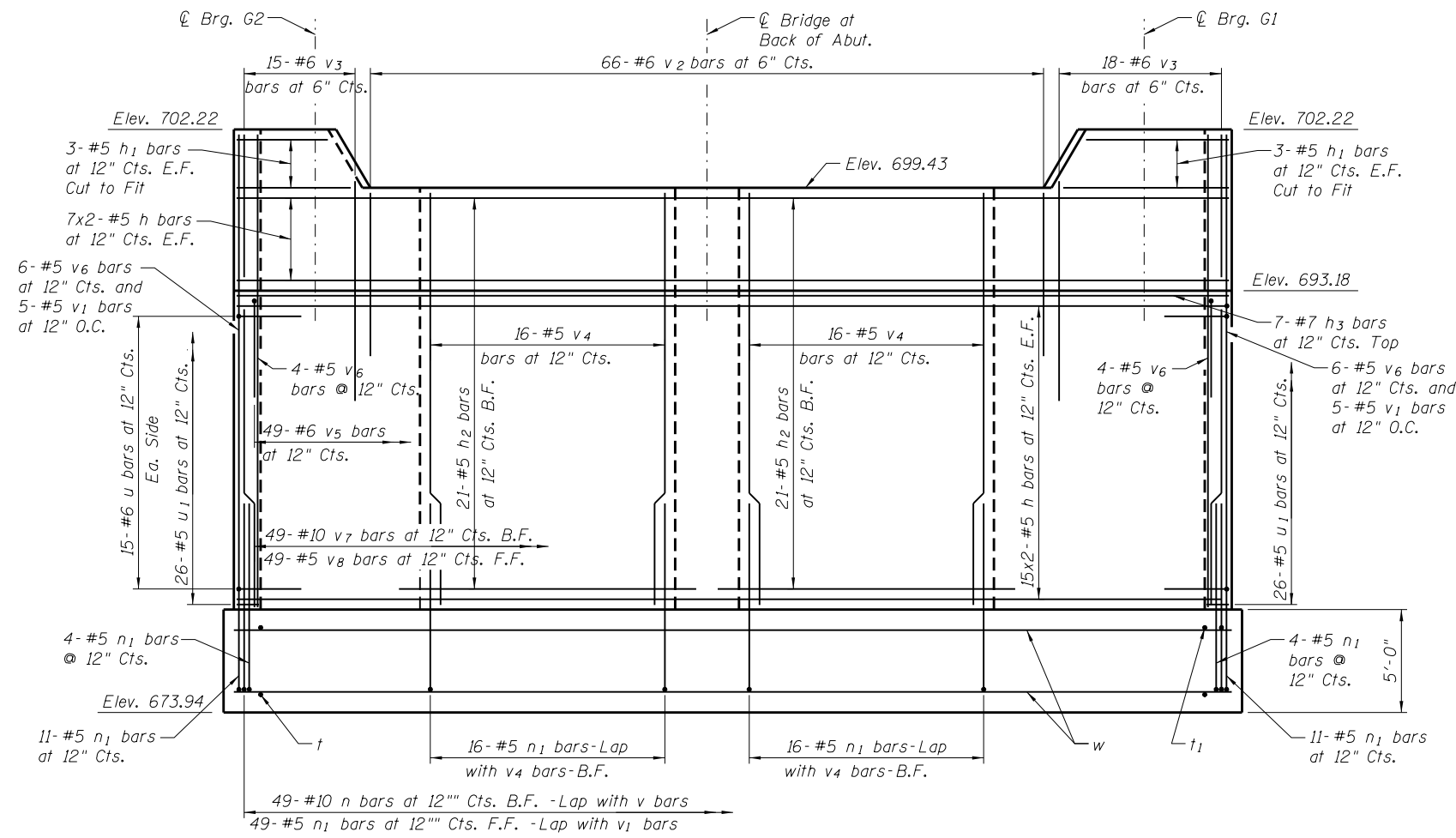
FOUNDATION PLAN
Scale: 1/4" = 1'-0"



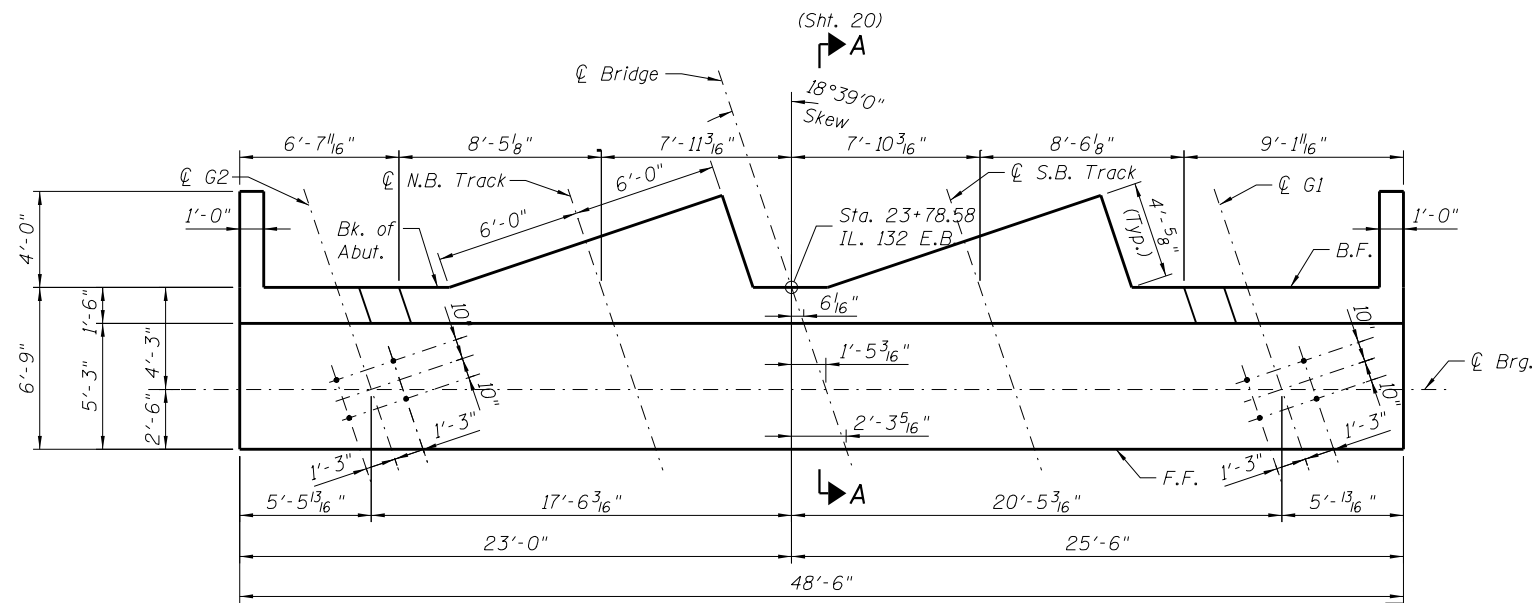
SECTION A-A
Scale: 1/4" = 1'-0"



TYPICAL DRAINAGE SECTION - NORTH AND SOUTH ABUTMENT
(Abutments)



ELEVATION
Scale: 1/4" = 1'-0"



TOP VIEW
Scale: 1/4" = 1'-0"

FILE NAME = ...SHOOFLY-60K80-019.dgn
HOH HARRY O. HEFTER ASSOCIATES, INC.
 DESIGN AND CONSULTING ENGINEERS
 3366 55 East Jackson Blvd., Suite 600
 Chicago, Illinois 60604
 312/346-8931

USER NAME = aefitzpatrick
 PLOT SCALE = 8:0.0000 1' / in.
 PLOT DATE = 10/7/2016

DESIGNED - MMH
 CHECKED - DNB
 DRAWN - R.VEJAR
 CHECKED - BCS

REVISED -
 REVISED -
 REVISED -
 REVISED -

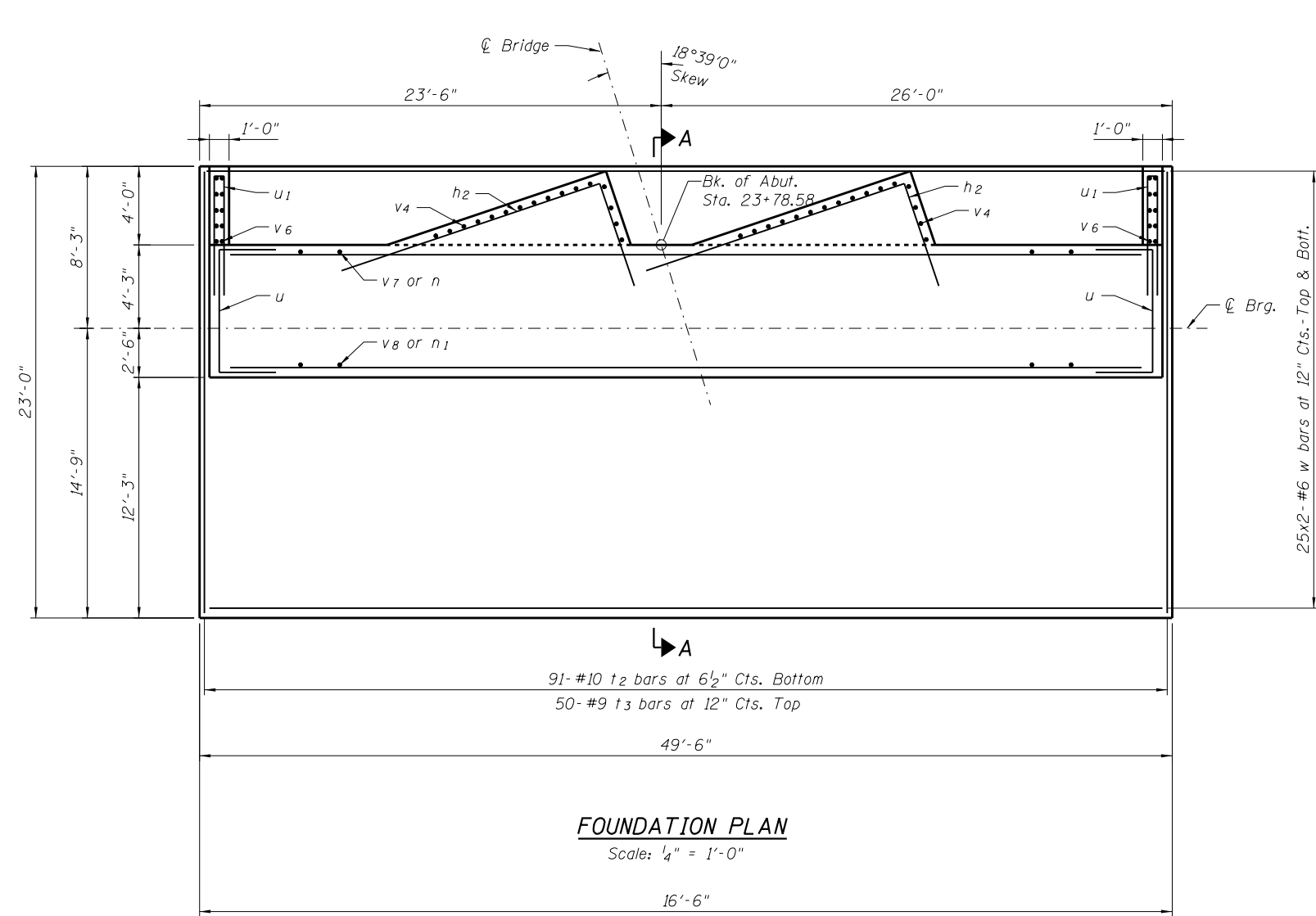
**STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION**

**SOUTH ABUTMENT - 1
 SHOOFLY BRIDGE OVER IL RTE 132**

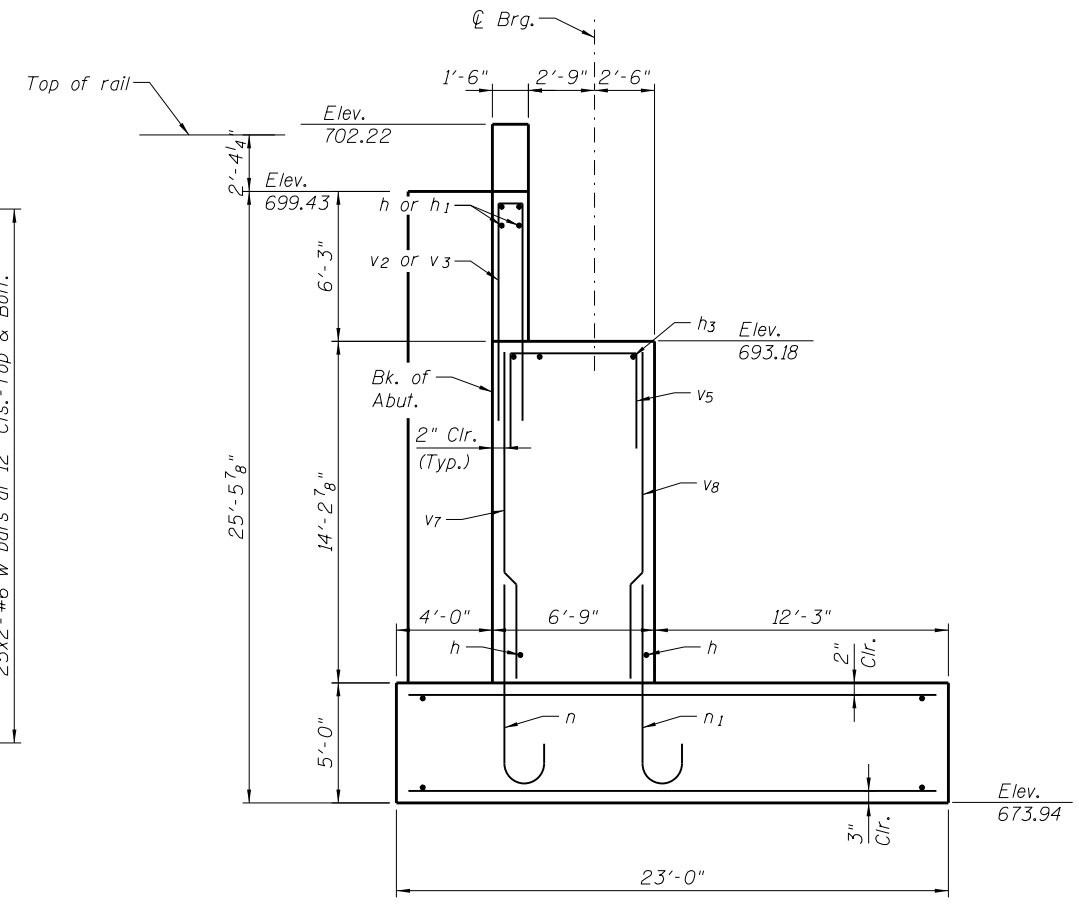
SHEET NO. 19 OF 25 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
346	125X-N&J-SB-B	LAKE	361	277
CONTRACT NO. 60K80				

ILLINOIS FED. AID PROJECT



FOUNDATION PLAN
Scale: 1/4" = 1'-0"



SECTION A-A
Scale: 1/4" = 1'-0"

HOH HARRY O. HEFTER ASSOCIATES, INC. DESIGN AND CONSULTING ENGINEERS 55 East Jackson Blvd., Suite 600 Chicago, Illinois 60604 312/346-8131	USER NAME = aefitzpatrick	DESIGNED - MMH	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	SOUTH ABUTMENT - 2 SHOOFLY BRIDGE OVER IL RTE 132	F.A.P. RTE. 346	SECTION 125X-N&J-SB-B	COUNTY LAKE	TOTAL SHEETS 361	SHEET NO. 278
	PLOT SCALE = 8:0.0000 '1' / in.	DRAWN - R.VEJAR	CHECKED - DNB			REVISED -	CONTRACT NO. 60K80			SHEET NO. 20 OF 25 SHEETS



Illinois Department
of Transportation

Division of Highways
GSG CONSULTANTS INC.
FAU 1218 (Illinois Route
132 / Grand Avenue)

SOIL BORING LOG

Date 5/2/11

ROUTE 132 / Grand Avenue DESCRIPTION Union Pacific Rail Bridge over IL 132 LOGGED BY RJC

SECTION 125X-N LOCATION N. Abut., SEC. 13, TWP. 45N, RNG. 11E, 3rd PM,
Latitude 42°22'15.54542" N, Longitude 87°53'43.06994" W

COUNTY Lake DRILLING METHOD HSA HAMMER TYPE AUTO

STRUCT. NO. Station	D E P T H	B L O W S	U C S Qu	M O I S T T	Surface Water Elev.		D E P T H	B L O W S	U C S Qu	M O I S T T
					ft	ft				
BORING NO. B-3 (STA: U.P. RR) Station 9+18.58 Offset 91.62R LT Ground Surface Elev. 699.36 ft	(ft)	(/6")	(tsf)	(%)		NA				
	3			16	Dark Brown, Moist TOPSOIL - trace, organics, sand, gravel	698.36				
	25	4.0		19	Brown and Gray, Moist SILTY CLAY LOAM (Fill), trace organics and gravel					
	2	P								
	6									
	6	2.5		21						
	6	P								
	-5				NOTE: A large amount of concrete debris and obstructions were encountered between 1' and 12'					
	4									
	4			12		673.36				
	4				Very Stiff Gray, Moist SILTY LOAM, trace gravel			3		
	4							2.6		19
	4							B		
	1				Brick Fragments at 8.5'					
	2			15		670.86		3		
	4				Very Stiff Gray, Moist SILTY CLAY LOAM			3.3		22
	-10							B		
	7									
	19			15						
	6					686.86				
	4				Very Stiff Brown, Moist SILTY CLAY LOAM, trace gravel					
	4	3.9		22		665.36		3		
	6	B			6 inch layer of SAND, trace gravel, Gray, Medium to Coarse grain, Wet at 33.5'			1.3		14
	-15				Stiff Greenish Gray, Moist SILTY LOAM, trace gravel			P		
	7				Water encountered at 16'					
	6	3.9		21						
	8	B				682.26				
	4				Very Stiff Gray, Moist SILTY CLAY LOAM, trace gravel					
	6					660.86		3		
	6	2.9		21	Very Stiff Gray, Moist SILTY CLAY LOAM, trace gravel			6		14
	8	B				679.36		2.9		
	-20							B		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)



Illinois Department
of Transportation

Division of Highways
GSG CONSULTANTS INC.
FAU 1218 (Illinois Route
132 / Grand Avenue)

SOIL BORING LOG

Date 5/2/11

ROUTE 132 / Grand Avenue DESCRIPTION Union Pacific Rail Bridge over IL 132 LOGGED BY RJC

SECTION 125X-N LOCATION N. Abut., SEC. 13, TWP. 45N, RNG. 11E, 3rd PM,
Latitude 42°22'15.54542" N, Longitude 87°53'43.06994" W

COUNTY Lake DRILLING METHOD HSA HAMMER TYPE AUTO

STRUCT. NO. Station	D E P T H	B L O W S	U C S Qu	M O I S T T	Surface Water Elev.		D E P T H	B L O W S	U C S Qu	M O I S T T
					ft	ft				
BORING NO. B-3 (STA: U.P. RR) Station 9+18.58 Offset 91.62R LT Ground Surface Elev. 699.36 ft	(ft)	(/6")	(tsf)	(%)		NA				
	3				Very Stiff Gray, Moist SILTY CLAY LOAM, trace gravel	655.86				
	5	2.2		19						
	7	B								
	6									
	7	2.2		20						
	9	B								
	-25					673.36		3		
	4				Very Stiff Gray, Moist SILTY LOAM, trace gravel			4	2.6	22
	4							6	B	
	1				Brick Fragments at 8.5'			3		
	2			15	Very Stiff Gray, Moist SILTY CLAY LOAM	670.86		4	3.3	22
	4	B						6	B	
	7									
	19			15				7	2.5	13
	6					686.86		8	B	14
	4				Very Stiff Brown, Moist SILTY CLAY LOAM, trace gravel			7		
	4	3.9		22		665.36		3	1.3	14
	6	B			6 inch layer of SAND, trace gravel, Gray, Medium to Coarse grain, Wet at 33.5'			4	P	
	-15				Stiff Greenish Gray, Moist SILTY LOAM, trace gravel					
	7				Water encountered at 16'					
	6	3.9		21		682.26		7		
	8	B						8	3.7	14
	4				Very Stiff Gray, Moist SILTY CLAY LOAM, trace gravel			11	B	
	6					660.86				
	6	2.9		21	Very Stiff Gray, Moist SILTY CLAY LOAM, trace gravel			6		
	8	B				679.36		7	2.9	21
	-20							9	B	

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)



Illinois Department of Transportation

Division of Highways GSG CONSULTANTS INC. FAU 1218 (Illinois Route 132 / Grand Avenue)

SOIL BORING LOG

Page 2 of 3

Date 6/1/11

ROUTE 132 / Grand Avenue DESCRIPTION Union Pacific Rail Bridge over IL 132 LOGGED BY RJC
SECTION 125X-N LOCATION S. Abut., SEC. 13, TWP. 45N, RNG. 11E, 3rd PM. Latitude 42°22'13.82437" N, Longitude 87°53'42.74013" W
COUNTY Lake DRILLING METHOD HSA HAMMER TYPE AUTO

Table with columns for Depth (ft), SPT Blows (6" / 6"), UCS (tsf), and Moisture (%). It details soil layers from 0 to 23 feet depth, including descriptions like 'Medium Dense Gray, Fine to Medium grain, Wet SAND' and 'Stiff to Very Stiff Gray, Moist SILTY LOAM'.

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)



Illinois Department of Transportation

Division of Highways GSG CONSULTANTS INC. FAU 1218 (Illinois Route 132 / Grand Avenue)

SOIL BORING LOG

Page 3 of 3

Date 6/1/11

ROUTE 132 / Grand Avenue DESCRIPTION Union Pacific Rail Bridge over IL 132 LOGGED BY RJC
SECTION 125X-N LOCATION S. Abut., SEC. 13, TWP. 45N, RNG. 11E, 3rd PM. Latitude 42°22'13.82437" N, Longitude 87°53'42.74013" W
COUNTY Lake DRILLING METHOD HSA HAMMER TYPE AUTO

Table with columns for Depth (ft), SPT Blows (6" / 6"), UCS (tsf), and Moisture (%). It details soil layers from 0 to 22 feet depth, including descriptions like 'Stiff to Very Stiff Gray, Moist SILTY LOAM' and 'Medium Dense Gray, Medium to Coarse grain, Wet SAND'.

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)

Metadata table with columns: FILE NAME, USER NAME, DESIGNED, CHECKED, DRAWN, PLOT DATE, REVISED.

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

SOIL BORING LOGS - 3 SHOOFLY BRIDGE OVER IL RTE 132 SHEET NO. 23 OF 25 SHEETS

Summary table with columns: F.A.P. RTE., SECTION, COUNTY, TOTAL SHEETS, SHEET NO., CONTRACT NO.



Illinois Department of Transportation
 Division of Highways
 GSG CONSULTANTS INC.
 FAU 1218 (Illinois Route 132 / Grand Avenue)

SOIL BORING LOG

Page 1 of 1

Date 5/31/11

ROUTE 132 / Grand Avenue DESCRIPTION Union Pacific Rail Bridge over IL 132 LOGGED BY RJC

SECTION 125X-N LOCATION SE Ret. Wall, SEC. 13, TWP. 45N, RNG. 11E, 3rd PM

Latitude 42°22'13.83067" N, Longitude 87°53'42.48955" W

COUNTY Lake DRILLING METHOD HSA HAMMER TYPE AUTO

STRUCT. NO.	Station	D E P T H (ft)	B L O W S (/6")	U C S (tsf)	M O I S T (%)	Surface Water Elev.	Stream Bed Elev.	Groundwater Elev.:	First Encounter	Upon Completion	After - Hrs.	D E P T H (ft)	B L O W S (/6")	U C S (tsf)	M O I S T (%)
						NA	NA								
BORING NO. B-13 (STA IL Rte 132)	23+96.98								667.5	667.5					
	Offset 70.42ft RT														
	Ground Surface Elev. 692.09														
Brown and Dark Brown, Moist SILTY CLAY LOAM (Fill), with organics, trace gravel	691.09		4		20			Very Stiff to Hard Gray, Moist SILTY LOAM, trace gravel (continued)					3		
Brown and Black, Moist SILTY CLAY LOAM (Fill), trace organics, gravel			5	2.6	24								4	2.9	19
			6	B									6	B	
			4										4		
			5	4.0	20								8	1.5	14
	687.19		8	P									10	P	
Hard Brown, Moist SILTY LOAM, trace gravel			2					Medium Dense Gray, Fine to Coarse grain, Wet SAND, trace gravel					6		
			2	4.8	20				665.19				6	2.5	19
			4	P				Very Stiff Gray, Wet SILTY LOAM	664.59				6	P	
			4					End of Boring							
			5	6.0	17										
			9	P											
	680.59		5												
Hard Gray and Brown, Moist SILTY LOAM, trace gravel			7	4.7	16										
			10	B											
	678.59		3												
Very Stiff to Hard Gray, Moist SILTY LOAM, trace gravel			6	2.4	18										
			7	B											
			5												
			6	2.2	19										
			8	B											
			3												
			7	7.0	22										
			9	P											

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
 The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, form 137 (Rev. 8-99)



FILE NAME - ...SHOOFLY-60K80-025.dgn	USER NAME - eefitzpatrick	DESIGNED - MMH	REVISED -
		CHECKED - DNB	REVISED -
		DRAWN - R,VEJAR	REVISED -
		CHECKED - BCS	REVISED -

STATE OF ILLINOIS
 DEPARTMENT OF TRANSPORTATION

SOIL BORING LOGS - 5
 SHOOFLY BRIDGE OVER IL RTE 132

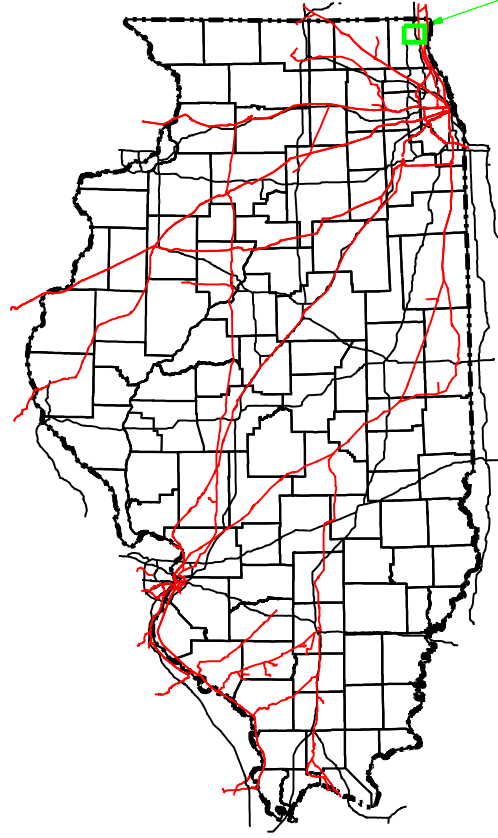
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
346	125X-N&J-SB-B	LAKE	361	283
CONTRACT NO. 60K80				

SHEET NO. 25 OF 25 SHEETS

ILLINOIS FED. AID PROJECT

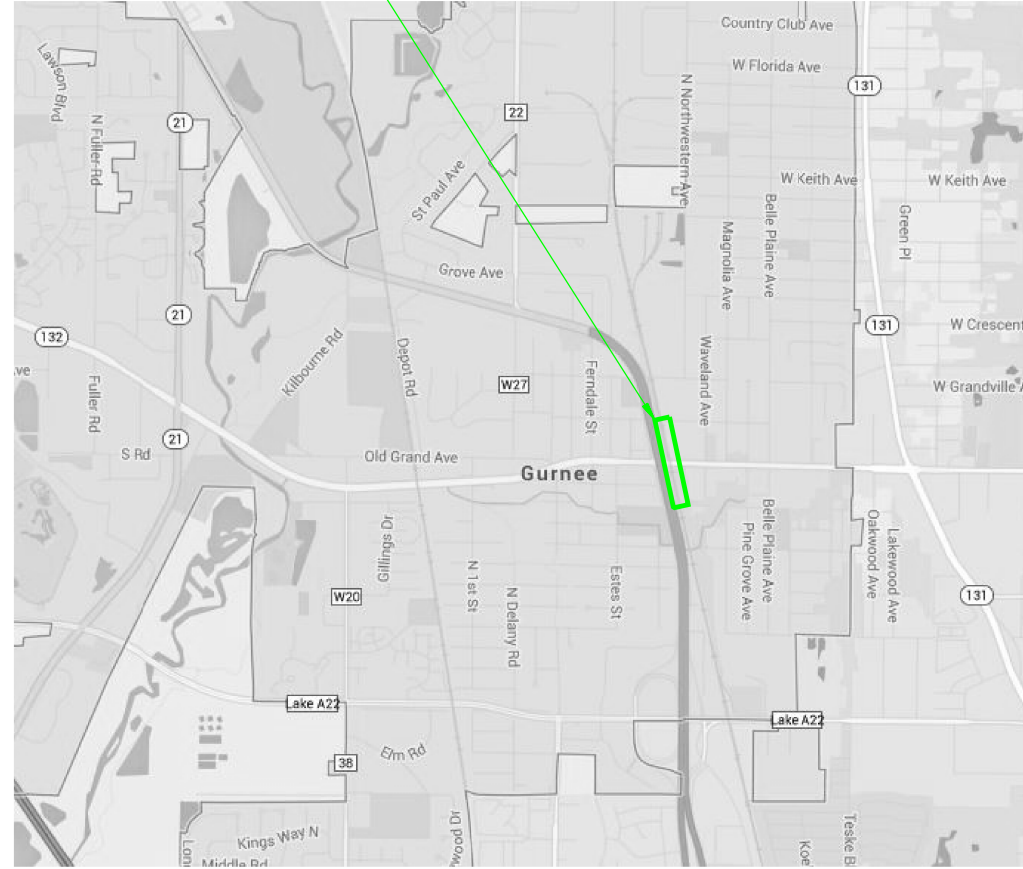


ENGINEERING DESIGN & CONSTRUCTION



State of Illinois

PROJECT LOCATION



Project Location Map

GURNEE, ILLINOIS
MILWAUKEE SUBDIVISION
MILE POST 37.5 TO 38.00
UPRR BRIDGE OVER IL RTE. 132 (GRAND AVE.)

**LAST REVISED
JUNE 7, 2016**

FINAL
NOT FOR CONSTRUCTION

WORK ORDER:
PROJECT NUMBER:
BUDGET REFERENCE:

PROJECT INDEX

PROJECT DESIGN	DESCRIPTION
G001	COVER SHEET WITH VICINITY MAP
G002	PROJECT INDEX & REVISION SHEET
G003	GENERAL NOTES & PROJECT CONTACTS
G004	ABBREVIATIONS & LEGEND
G005	CONTROL POINTS AND GEOMETRY
G006	WORK SCHEDULE
G007 to G008	SUGGESTED TRACK STAGING PLAN
G009	TRACKWORK SCOPE OF WORK
C001	SHOOFLY TRACKS - TYPICAL SECTIONS
C002	MAINLINE TRACKS - TYPICAL SECTIONS
C003 to C004	SHOOFLY TRACK 1 - PLAN AND PROFILE
C005 to C006	SHOOFLY TRACK 2 - PLAN AND PROFILE
C007 to C09	MAINLINE TRACK 1 - PLAN AND PROFILE
C010 to C012	MAINLINE TRACK 2 - PLAN AND PROFILE
C013	MAINLINE SPUR TRACK 812 - PLAN AND PROFILE
X001 to X008	SHOOFLY TRACKS - CROSS SECTIONS
X009 to X018	MAINLINE TRACKS - CROSS SECTIONS

PROJECT INDEX (CONTINUED)

STANDARDS	DESCRIPTION
Std. Dwg. 0001C	Roadbed Section Heavy Tonnage
Std. Dwg. 0010E	Ballast and Subballast Gradation Table
Std. Dwg. 0013A	Grain Size Distribution for Subgrade Soils - Subballast Thickness
Std. Dwg. 0015	Curve Marking Standard
Std. Dwg. 0021E	Superelevation of Curves, 1" Unbalance
Std. Dwg. 0025D	Rail Markings for Engines, Cars or Equipment Clear of Road Grade Crossings
Std. Dwg. 0026B	Clearance Point Marking
Std. Dwg. 0030C	Standard Earth Dumber for End of Spur Track
Std. Dwg. 0045B	Rail Laying Temperatures for Continuous Welded Rail
Std. Dwg. 0441B	Double Shoulder Tie Plate for 132 Lb. And 136 Lb. Rail
Std. Dwg. 0453D	Spiking Pattern
Std. Dwg. 0460G	Rail Anchor Patterns
Std. Dwg. 0519B	Culvert Location Tie Marking
Std. Dwg. 0575B	Standard Barricade for Placement at Bridge Backwalls
Std. Dwg. 2006	Single Switch Point Derail
Std. Dwg. 6010L	Approved Trackwork Suppliers
Com. Std. Dwg. 130005	6" Track Spike
Com. Std. Dwg. 343000	No. 11 Turnout 136/141 Lb. Panel No. 1476
Com. Std. Dwg. 343001	No. 11 Turnout 136/141 Lb. Panel No. 2 270
Com. Std. Dwg. 343002	No. 11 Turnout 136/141 Lb. Panel No. 3 With RBM Frog
Com. Std. Dwg. 343003	No. 11 Turnout 136/141 Lb. Panel No. 4 155
Com. Std. Dwg. 343004	No. 11 Turnout 136/141 Lb. Panel No. 3 SMSG Frog
Com. Std. Dwg. 343005	No. 11 Turnout 136/141 Lb. Panel No. 3 with Optional Jump Frog
Com. Std. Dwg. 343100	No. 11 Turnout 19'-6" Curved Switch, Turnout Geometry
Com. Std. Dwg. 343200	No. 11 Turnout with RBM, SMSG, and Spring Frogs, Bill of Materials
Com. Std. Dwg. 680000	General Notes and Details for Round Steel Pipe Culverts
Com. Std. Dwg. 680010	Construction Notes and Tables for Smooth Steel Pipe Culverts
Com. Std. Dwg. 680020	Construction Notes and Tables for Corrugated Steel Pipe Culverts

PROJECT INDEX (CONTINUED)

STRUCTURES DESIGN DESCRIPTION

SEE SEPARATE BRIDGE DRAWING SETS
(PREPARED BY HARRY O. HEFTER-ASSOCIATES, INC)

MECHANICAL DESIGN DESCRIPTION

ELECTRICAL DESIGN DESCRIPTION

PROJECT REVISIONS				
REV. #	BY	DATE:	SHEET:	DESCRIPTION

	DRAWN BY: AAF-HOH	UNION PACIFIC RAILROAD Office of Assistant Vice President Engineering Design/Construction
	CHECKED BY: BAP-HOH	
	DATE: 06/07/16	
	SHEET NUMBER: G002 of 009	
LOCATION & DESCRIPTION: GURNEE, LAKE COUNTY, ILLINOIS MILWAUKEE SUBDIVISION MP 37.50 TO MP 38.00 UPRR BRIDGE OVER IL RTE. 132 (GRAND AVE.)		SHEET TITLE: PROJECT INDEX & REVISIONS

GENERAL NOTES

1. Contractors shall notify Service Alert, (800) 642-2444 and UPRR Fiber Optics Hotline (800) 336-9193, 48 hours prior to any excavation. The USA Authorization Numbers shall be kept at the job site.
2. No work whatsoever shall be commenced without first notifying the UPRR Engineer.
3. The Contractor shall comply with all Federal, State, County, and City Laws and Ordinances and Regulations of the Department of Industrial Relations, OSHA, NPDES and Industrial Accident Commission related to the safety and character of the work, equipment and labor personnel.
4. Contractor shall be responsible for coordinating with all Utility agencies.
5. Contractor shall protect in place (by any means necessary) all existing utilities to remain unless otherwise specified herein, contractor shall be responsible for the complete repair at his expense, for any damage to existing utilities, structures, or other site features, as a result of his work.
6. Prior to placing curbs, pavements, base, subbase, track, etc., all underground utilities shall be installed, backfill completed, and the Engineer notified by each of the utility companies having facilities within the work area, that the utility installation has satisfactorily passed acceptance tests.
7. All existing underground utilities, that are not to be re-used shall be abandoned in place. All existing pipelines to be abandoned in place shall be cement slurry filled and capped at least 3'-0" below top of proposed subgrade.
8. Contractor shall verify locations and elevations of existing utilities whether known or unknown prior to beginning construction.
9. Any underground structures such as cesspools, cisterns, mining shafts, tunnels, septic tanks, wells, and pipelines not located prior to construction shall be brought to the attention of the engineer for determination of appropriate action such as removal or treatment in a manner judged suitable to the engineer.
10. Contractor shall coordinate location of all proposed utilities with UPRR to assure accuracy of utility connections and compliance with local codes.
11. Any existing conditions found to be a variance with these drawings must be immediately reported to the Engineer.
12. Contractor shall maintain and clean to the satisfaction of the Engineer, all access and service roads used during construction.
13. Contractor shall perform all construction in such a manner as to protect adjacent existing buildings, and other site elements which are to remain in service.
14. Contractor shall provide As-built Drawings for all improvements.
15. No field changes will be permitted without direct written authorization from the UPRR Engineer or his representative.
16. Contractor shall coordinate work which affects adjacent property owners. Any questions or agreements between adjacent property owners and contractor shall be made in writing. A copy of such agreement shall be provided to the UPRR Engineer or his representative.
17. The contractor is responsible for preparing a Stormwater Pollution Prevention Plan (SWPPP) to comply with State regulations. General specifications and typical erosion control details are included in the plan set.
18. Right-of-way lines shown on the plans were taken from existing UPRR right-of-way map and are approximate.
19. Match lines for sheets are based on the existing Main Line stationing unless otherwise specified.
20. Track laying, ballasting, and installation of road crossing panels will be done by UPRR unless otherwise stated.
21. Where existing culverts are to be extended, the contractor shall expose existing drainage structures and field verify size and type before ordering.
22. The contractor is responsible for the removal of all pavement markings that will be in conflict with the proposed work.
23. Contractor shall comply with all STATE and CITY standard specifications for construction of public improvements requirements. CITY standard specifications shall prevail.
24. Contractor shall maintain at least one access to all affected business. If necessary, multiphase construction shall be utilized.
25. All demolished and surplus track material shall become the property of the IDOT Contractor and shall be removed from the site.
26. Contractor shall comply with the UPRR Specifications as provided in Section B of the Special Provisions.

DESIGN CRITERIA

1. UPRR standard plans and trackworks
2. Illinois Department of Transportation Roadway Standards

SURVEY NOTES

1. Railroad stationing for project profiles and alignments is based on stations established for chord definition spiraled curves at the centerline of the existing UPRR Main Line unless otherwise noted.
2. The contractor is responsible for the preservation of all survey control monuments. In the event monuments are damaged or destroyed by the contractor, the Engineer will replace the monument solely at the contractor's expense.

	DATUM
HORIZONTAL	NAD 83
VERTICAL	NAVD 88

TRAFFIC NOTES

1. All barricades, warning signs, lights, devices, etc. for the guidance of vehicle traffic and pedestrians must conform to the installation shown in the Manual on Uniform Traffic Control Devices (MUTCD), current edition.
2. Contractor shall make twice daily inspections of barricades and flashing lights to ensure proper placement and functioning of warning devices.
3. Grade crossings closed to traffic during construction shall be barricaded in accordance with the MUTCD.
4. At all grade crossings, all grade crossing warning signs (crossbuck) shall temporarily be relocated during construction and reset after the grade crossings construction is completed to a point adjacent to the roadway and 15 feet from the centerline of the near track as stated in the MUTCD except where automatic grade crossing warning signals/gates exist. All automatic warning devices are the responsibility of UPRR. At no time shall a crossing be left open without proper warning signs in place.
5. Contractor shall submit traffic control plans to CITY Traffic Department for approval at least 2 weeks prior to each road closure. Plans shall be 11" x 17" engineered drawings, sealed by a professional engineer from the STATE.
6. The contractor is responsible for the prompt replacement and/or repair of all traffic control devices and appurtenances damaged or disturbed due to construction.

PROJECT CONTACTS

CONTACT	PHONE NUMBER	UPRR
JOHN VENICE JAMES NUDEIRA	(312) 777-2043 (708) 649-5280	MANAGER OF TRACK PROJECTS

CONTACT	PHONE NUMBER	FIBER
CARL DONOHUE JIM BURTON KEVIN HUFF ROBERT SCHULTER	(847) 705-4257 (847) 318-3100 (708) 388-9684 (630) 600-6349	AT&T SPRINT QUEST/CENTURY LINK COMCAST

CONTACT	PHONE NUMBER	UTILITIES
CARL DONOHUE TOM RIGWOOD GERRY GLOGOVSKY ROBERT SCHULTER	(847) 705-4257 (847) 599-7550 (708) 327-6611 (630) 600-6349	TELEPHONE (AT&T) WATER (VILLAGE OF GURNEE) GAS (NORTH SHORE GAS) COMCAST CABLE

CONTACT	PHONE NUMBER	FEDERAL AND LOCAL GOVERNMENT AGENCY
CRAIG BAUER	(847) 705-4265	ILLINOIS DEPARTMENT OF TRANSPORTATION

PHONE NUMBER	GENERAL
(800) 336-9193 (888) 258-0808 (888) 877-7267	UPRR CALL BEFORE YOU DIG CALL BEFORE YOU DIG (NATIONAL DIRECTORY) UPRR Response Management Communications Center (RMCC)

	DRAWN BY: AAF-HOH	UNION PACIFIC RAILROAD Office of Assistant Vice President Engineering Design/Construction
	CHECKED BY: BAP-HOH	
	DATE: 06/07/16	
	SHEET NUMBER: G003 of 009	
LOCATION & DESCRIPTION: GURNEE, LAKE COUNTY, ILLINOIS MILWAUKEE SUBDIVISION MP 37.50 TO MP 38.00 UPRR BRIDGE OVER IL RTE. 132 (GRAND AVE.)		SHEET TITLE: GENERAL NOTES AND PROJECT CONTACTS

ABBREVIATIONS

MISCELLANEOUS

Ac.	Acres
Ave.	Avenue
Blvd.	Boulevard
Bldg.	Building
BNSF	BNSF Railway
C.Y.	Cubic Yards
Conc.	Concrete
°	Degree (s)
Dia.	Diameter
Dr.	Drive
Dwg.	Drawing
E	East
Elev.	Elevation
Exist.	Existing
'	Foot, Feet or Minute (s)
F.S.	Finished Surface
Horiz.	Horizontal
"	Inch, Inches or Second (s)
Inv.	Invert
Lt.	Left
L	Length
L.F.	Lineal Feet
Max.	Maximum
Min.	Minimum
N	North
NTS	Not to Scale
No.	Number
OH	Overhead
Prop.	Proposed
RR	Railroad
Rwy	Railway
ROW	Right of Way
Rt.	Right
S	South
S.F.	Square Feet
Sta.	Station
Std.	Standard
St.	Street
Twp.	Township
Typ.	Typical
UG	Underground
UPRR	Union Pacific Railroad
V	Velocity
Wt.	Weight
W	West
X-ing	Crossing

SIGNAL

ABS	Automatic Block Signal
ATC	Automatic Train Control
CTC	Centralized Traffic Control
DED	Dragging Equipment Detector
DTC	Direct Traffic Control
ELTO	Electric Lock Turnout
HBD	Hot Box Detector
HTTO	Hand Throw Turnout
HWD	High Wide Dectector
POTO	Power Operated Turnout
TWC	Track Warrant Control
WILD	Wheel Impact Load Dectector

STRUCTURES

Bldg.	Building
Br.	Bridge
CB	Catch Basin
CPT	Concrete Pile Trestle - Ballast Deck
CIP	Cast Iron Pipe
CMP	Corrugated Metal Pipe
CMPA	Corrugated Metal Pipe Arch
CSP	Corrugated Steel Pipe
Culv.	Culvert
DI	Drop Inlet
DPGBD	Deck Plate Girder - Ballast Deck
DPGOD	Deck Plate Girder - Open Deck
EBW	East Backwall
F.L.	Flowline
F.F.	Finished Floor
GIP	Galvanized Iron Pipe
Hdw	Headwall
NBW	North Backwall
PSCT	Prestressed Concrete Trestle
RCA	Reinforced Concrete Arch
RCB	Reinforced Concrete Box
RCP	Reinforced Concrete Pipe
SBW	South Backwall
SSP	Smooth Steel Pipe
SPTBD	Steel Pile Trestle - Ballast Deck
SPTOD	Steel Pile Trestle - Open Deck
SPP	Structural Plate Pipe
TPGBD	Through Plate Girder - Ballast Deck
TPGOD	Through Plate Girder - Open Deck
TPTBD	Timber Pile Trestle - Ballast Deck
TPTOD	Timber Pile Trestle - Open Deck
TTBD	Through Truss - Ballast Deck
TTOD	Through Truss - Open Deck
TWB	Treated Wood Box
VCP	Vitrified Clay Pipe
Viad.	Viaduct
WBW	West Backwall
WIP	Wrought Iron Pipe

TRACK

ATR	Above Top of Rail
Align.	Alignment
BBR	Below Base of Rail
Cntrs.	Centers
CWR	Continuous Welded Rail
D	Degree of Curvature
DSPD	Double Switch Point Derail
EOT	End of Track
HH	Head Hardened
Jtd.	Jointed Rail
LH	Left Hand
L/C	Length of Curve
L/S	Length of Spiral
ML	Main Line
MM	Mile Marker
MP	Mile Post
NSC	Not Sufficient Clearance
OTM	Other Track Material
PCC	Point of Compound Curve
PC	Point of Curve
PCS	Point of Curve to Spiral
POC	Point on Curve
PF	1/2" Point of Frog
PI	Point of Intersection
PITO	Point of Intersection of Turnout
PS	Point of Spiral
PSC	Point of Spiral to Curve
POS	Point on Spiral
PT	Point of Tangent
POT	Point on Tangent
Pt. Sw.	Point of Switch
PVC	Point of Vertical Curve
PVI	Point of Vertical Intersection
PVT	Point of Vertical Tangent
Rel.	Relocate
RH	Right Hand
SE	Superelevation
SH	Second Hand
SSPD	Single Switch Point Derail
TC	Track Centers
Temp.	Temporary
T.F.	Track Feet
T/R	Top of Rail
Trk.	Track
UXO	Universal Cross-Over
X-Over	Cross-Over

UTILITIES

— AIR —	AIR -	Compressed Air
— F/O —		Fiber Optic Cable
— G —		Gas Pipeline
— O+P —		Overhead Power Line
— SS —		Sanitary Sewer
— — — UGS — — —		Underground Signal Line
— — — — —		Steam Line
— S —	S	Storm Sewer
— T —		Telephone
— — — UGE — — —		Underground Electric
— W —		Water Main
— — — — —		Underground Wire
— — — — —		Under Drain
— — — — —		Tree Line

IN.	Inlet
C.B.	Catch Basin
M.H.	Manhole
P.P.	Power Pole
D.H.	Double Handhole
T.S.	Traffic Signal
T.P.	Telephone Pole
	Control Cabinet

TRACK

	Existing
	Proposed
	Remove
	Shift
	Future
	Turnout
	Earthen Bumber
	Single Switch Point Derail

PROPERTY

	Proposed IDOT Right of Way
	Existing Right of Way
	Proposed Permanent Easement
	Existing Permanent Easement
	Temporary Easement

SYMBOLS

ROAD CROSSING WARNING DEVICES

	Crossbuck Sign
	Flashing Light Warning Device
	Flashing Light Warning Device with Gate
	Cantilever Flashing Light Warning Device
	Cantilever Flashing Light Signal with Gate

SIGNAL

	Absolute Signal
	Signal Bridge
	Cantilever Signal
	ACS or CTC Signal
	Dwarf Signal
	Begin CTC
	Microwave Tower
	AEI
	Battery Box
	Dragging Equipment Detector
	Generator
	Hot Box Detector

STRUCTURES

	Culvert
	Culvert with Headwalls
	Building
	Flag Pole
	Bridge Temporary Sheet Piling

LIGHTING

	Light Pole
	Light Tower

SIGNS

	Stop
	Yard Limit
	1 Mile to Yard Limit
	Whistle Post
	Flanger
	Station
	Reduce Speed
	Resume Speed
	General Purpose

FENCES

	Barbed Wire
	Chain Link
	Snow / Sand
	Cattle Guard

ROADS

	Paved Road
	Unimproved Road
	Interstate Highway
	Federal Highway
	State Highway
	County Highway

OTHER

	Wetlands
	River or Lake
	Existing Tree
	Embankment
	Flow Line
	Milepost
	Milemarker
	Control Point
	Revision Number
	Revision Cloud

CONSTRUCTION

	Note (Shift by UPRR)
	Note (Proposed by Contractor)
	Note (Removal by Contractor)
	Cut Lines
	Fill Lines

	DRAWN BY: AAF-HOH	UNION PACIFIC RAILROAD Office of Assistant Vice President Engineering Design/Construction
	CHECKED BY: BAP-HOH	
	DATE: 06/07/16	
	SHEET NUMBER: G004 of 009	
LOCATION & DESCRIPTION: GURNEE, LAKE COUNTY, ILLINOIS MILWAUKEE SUBDIVISION MP 37.50 TO MP 38.00 UPRR BRIDGE OVER IL RTE. 132 (GRAND AVE.)		SHEET TITLE: ABBREVIATIONS & LEGEND

Horizontal Curves Control Points Table

Description	Station	Northing	Easting
Shoofly Track 1			
Curve 1			
PS	0+00.00	2078903.29	1102327.22
PSC	1+22.50	2078784.23	1102356.04
PI	2+28.53	2078681.49	1102382.45
PCS	3+34.57	2078580.97	1102416.35
PT	4+57.07	2078465.46	1102457.14
Curve 2			
PS	7+81.34	2078160.47	1102567.29
PSC	9+03.84	2078044.96	1102608.07
PI	11+60.17	2077801.41	1102690.18
PCS	14+16.50	2077547.14	1102727.63
PT	15+39.00	2077425.70	1102743.75
Curve 3			
PS	19+72.30	2076995.78	1102797.71
PSC	20+94.80	2076874.35	1102813.83
PI	21+85.52	2076784.57	1102827.04
PCS	22+76.23	2076695.80	1102845.88
PT	23+98.73	2076576.34	1102873.01
Shoofly Track 2			
Curve 1			
PS	0+00.00	2078914.83	1102338.89
PSC	1+14.19	2078795.77	1102367.71
PI	2+19.88	2078693.88	1102393.92
PCS	3+25.57	2078594.18	1102427.54
PT	4+48.44	2078478.67	1102468.32
Curve 2			
PS	7+81.34	2078165.57	1102581.39
PSC	9+03.52	2078050.06	1102622.18
PI	11+60.17	2077804.94	1102704.86
PCS	14+16.82	2077549.00	1102742.52
PT	15+39.00	2077427.57	1102758.64
Curve 3			
PS	19+81.80	2076988.23	1102813.78
PSC	21+04.66	2076866.80	1102829.90
PI	21+95.04	2076777.80	1102843.01
PCS	22+85.42	2076689.81	1102861.67
PT	23+98.73	2076570.35	1102888.79
Prop. Main Track 2			
Curve 1			
PS	197+03.68	2079249.31	1102260.44
PSC	197+36.68	2079217.19	1102267.99
PI	197+95.17	2079160.25	1102281.43
PCS	198+53.67	2079103.46	1102295.45
PT	198+86.67	2079071.43	1102303.39
Curve 2			
PS	202+38.49	2078729.99	1102388.21
PSC	202+71.49	2078697.96	1102396.15
PI	203+29.98	2078641.17	1102410.17
PCS	203+88.48	2078584.24	1102423.61
PT	204+21.48	2078552.11	1102431.16
Curve 3			
PS	215+70.45	2077433.50	1102693.53
PSC	216+03.45	2077401.37	1102701.05
PI	216+61.99	2077344.35	1102714.34
PCS	217+20.54	2077287.20	1102727.04
PT	217+53.54	2077254.98	1102734.17
Curve 4			
PS	221+04.90	2076911.88	1102809.92
PSC	221+37.90	2076879.66	1102817.05
PI	221+96.45	2076822.51	1102829.75
PCS	222+54.99	2076765.49	1102843.04
PT	222+87.99	2076733.35	1102850.56
Prop. Spur 812			
Pt. Sw.	0+00.00	2078412.38	1102463.93
End	4+39.55	2078847.13	1102400.01

VERTICAL CONTROL POINTS
TBM H
SQUARE CUT ON E'LY SIDE OF CONCRETE BASE FOR LIGHT POLE AT THE NORTH SIDE OF US 41 AND IL RTE 132 NE EXIT RAMP +/- 800 FT SOUTH OF IL RTE 132
N 2077269.505
E 1102654.015
EL 690.08
STA 17+22.86 (UPRR)
OFFSET 61.9' RT
TBM K
SQUARE CUT ON E'LY SIDE OF CONCRETE BASE FOR LIGHT POLE AT THE NE CORNER OF US RTE 41 AND GRANDVILLE AVE. BETWEEN US RTE 41 AND RAILROAD TRACKS
N 2079356.954
E 1102184.255
EL 695.69
STA N/A
OFFSET N/A

SURVEY CONTROL POINTS			
#1000	#1001	#1002	#1004
FOUND CUT CROSS IN SIDEWALK	FND MAG NAIL	FOUND IRON ROD	FOUND CUT CROSS IN SIDEWALK
N 2078031.487	N 2076150.872	N 2080159.781	N 2077888.344
E 1101005.327	E 1102838.977	E 1101172.359	E 1100790.203
EL 683.79	EL 686.89	EL 687.32	EL 678.83
#1005	#1006	#1007	#1008
FOUND CUT CROSS IN SIDEWALK	SET CUT CROSS IN SIDEWALK	SET CUT CROSS ON W. SIDE OF ISLAND	SET CUT CROSS IN SIDEWALK
N 2078049.961	N 2077972.982	N 2077962.101	N 2077949.961
E 1101270.947	E 1101836.102	E 1102308.960	E 1102820.533
EL 686.59	EL 686.81	EL 680.79	EL 684.50
#1009	#1010	#1011	#1012
SET CUT CROSS IN SIDEWALK	SET CUT CROSS IN SIDEWALK	SET IRON ROD	SET IRON ROD
N 207935.315	N 2077992.583	N 2079275.563	N 2078119.784
E 1103462.925	E 1103796.570	E 1102218.603	E 1102494.451
EL 694.52	EL 694.11	EL 694.88	EL 695.87
#1013	#1016	#2541	#2547
SET IRON ROD W/ CAP	SET IRON ROD	SET CUT CROSS ON SIDEWALK	SET CUT CROSS ON CONCRETE BASE
N 2078740.279	N 2076645.146	N 2077945.523	N 2077580.150
E 1102343.387	E 1102886.622	E 1102397.196	E 1102602.617
EL 696.64	EL 699.83	EL 680.09	EL 690.50

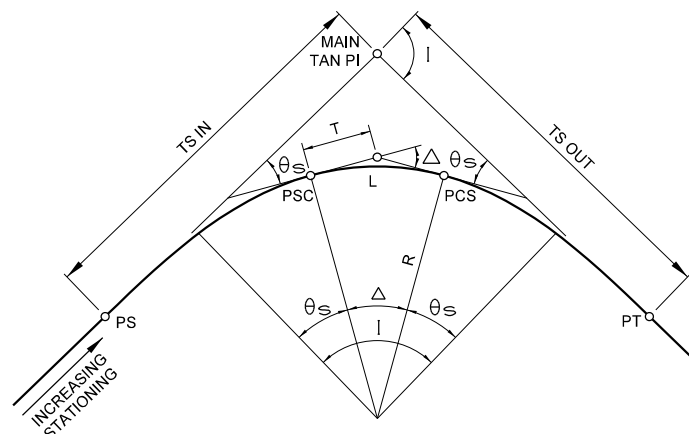


FIGURE A
CIRCULAR CURVES
WITH SPIRAL TRANSITION

I - TOTAL INTERSECTION ANGLE
 θ_s - SPIRAL ANGLE = $\frac{AI^2}{2}$
 Δ - CENTRAL ANGLE OF CIRCULAR CURVE = $I - 2\theta_s$
 D_c - DEGREE OF CURVE
 A - RATE OF CHANGE OF DEGREE OF CURVE PER 100-FT. OF LENGTH = $\frac{D_c}{L}$
 R - RADIUS OF CIRCULAR CURVE
 T - TANGENT LENGTH OF CIRCULAR CURVE = $R \tan \frac{\Delta}{2}$
 L - LENGTH OF CIRCULAR CURVE = $\frac{\Delta}{D_c} \times 100$
 PS - TANGENT TO SPIRAL
 PSC - SPIRAL TO CURVE
 PCS - CURVE TO SPIRAL
 PT - SPIRAL TO TANGENT
 $MAIN TAN PI$ - POINT OF INTERSECTION OF MAIN TANGENTS
 $(TS IN)$
 $(TS OUT)$ - TANGENT LENGTH OF COMPLETE CURVE = $(R+o) \tan \frac{I}{2} + t$
 (WHEN SPIRALS OF EQUAL LENGTH ARE USED ON BOTH SIDES OF CIRCULAR CURVE, SEE FIGURE C. FOR o AND t).

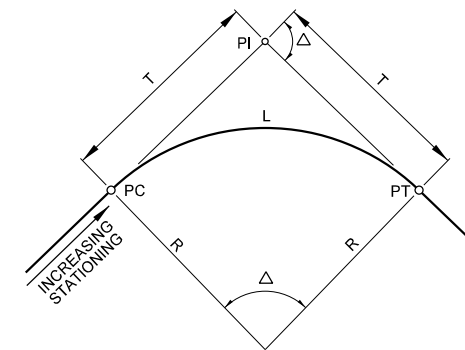


FIGURE B
SIMPLE CIRCULAR CURVE

R = RADIUS OF CIRCULAR CURVE
 Δ = CENTRAL ANGLE OF CIRCULAR CURVE
 $T = R \tan \frac{\Delta}{2}$
 $L = \frac{\Delta}{D_c} \times 100$
 $D_c = 2 \sin^{-1} (50/R) =$ DEGREE OF CURVE (CHORD DEFINITION)

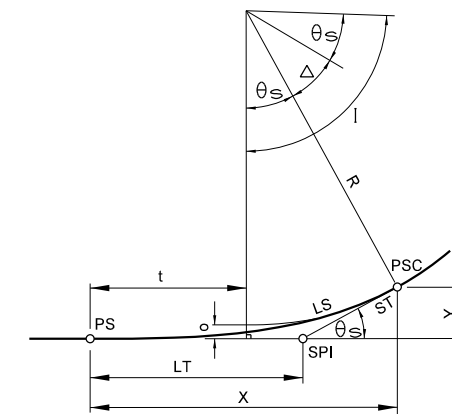


FIGURE C
SPIRAL TRANSITION CURVE

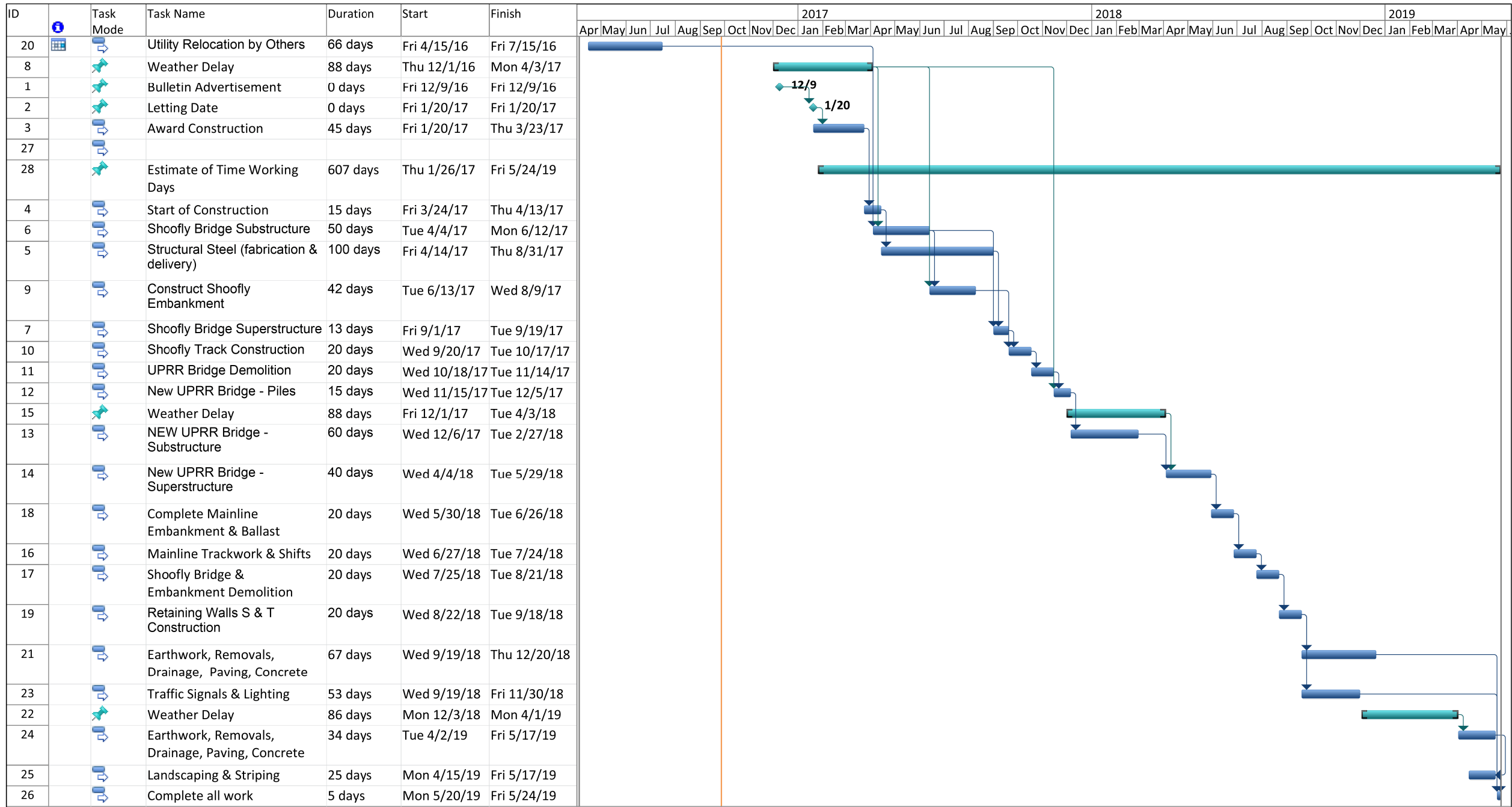
SPIRAL TRANSITION CURVE DATA:
 THE SPIRAL USED IS DEFINED BY THE TALBOT SPIRAL.

LS = LENGTH OF SPIRAL (TS TO PSC)
 $\theta_s = \frac{AL^2}{2}$
 $X = 100 L_1 - 0.000762A^2 L_1^5$
 $Y = 0.291AL_1^3 - 0.00000158A^3 L_1^7$
 $o = 0.0727AL_1^3$
 $t = 50L_1 - 0.000127A^2 L_1^5$
 $ST = \frac{Y}{\sin \theta_s}$
 $LT = X - \frac{Y}{\tan \theta_s}$
 $D_c = 2 \sin^{-1} (50/R) =$ DEGREE OF CURVE (CHORD DEFINITION)
 L_1 - TOTAL NO. OF STATIONS IN SPIRAL
 SPI - SPIRAL POINT OF INTERSECTION
 NOTE: D_c , θ_s , Δ , AND I ARE IN DEGREES.
 ALL OTHERS DIMENSIONS ARE FEET.



DRAWN BY:
 AAF-HOH
 CHECKED BY:
 BAP-HOH
 DATE:
 06/07/16
 SHEET NUMBER
 G005 of 009

UNION PACIFIC RAILROAD Office of Assistant Vice President
 Engineering Design/Construction
 LOCATION & DESCRIPTION: GURNEE, LAKE COUNTY, ILLINOIS
 MILWAUKEE SUBDIVISION
 MP 37.50 TO MP 38.00
 UPRR BRIDGE OVER IL RTE. 132 (GRAND AVE.)
 SHEET TITLE:
CONTROL POINTS AND GEOMETRY

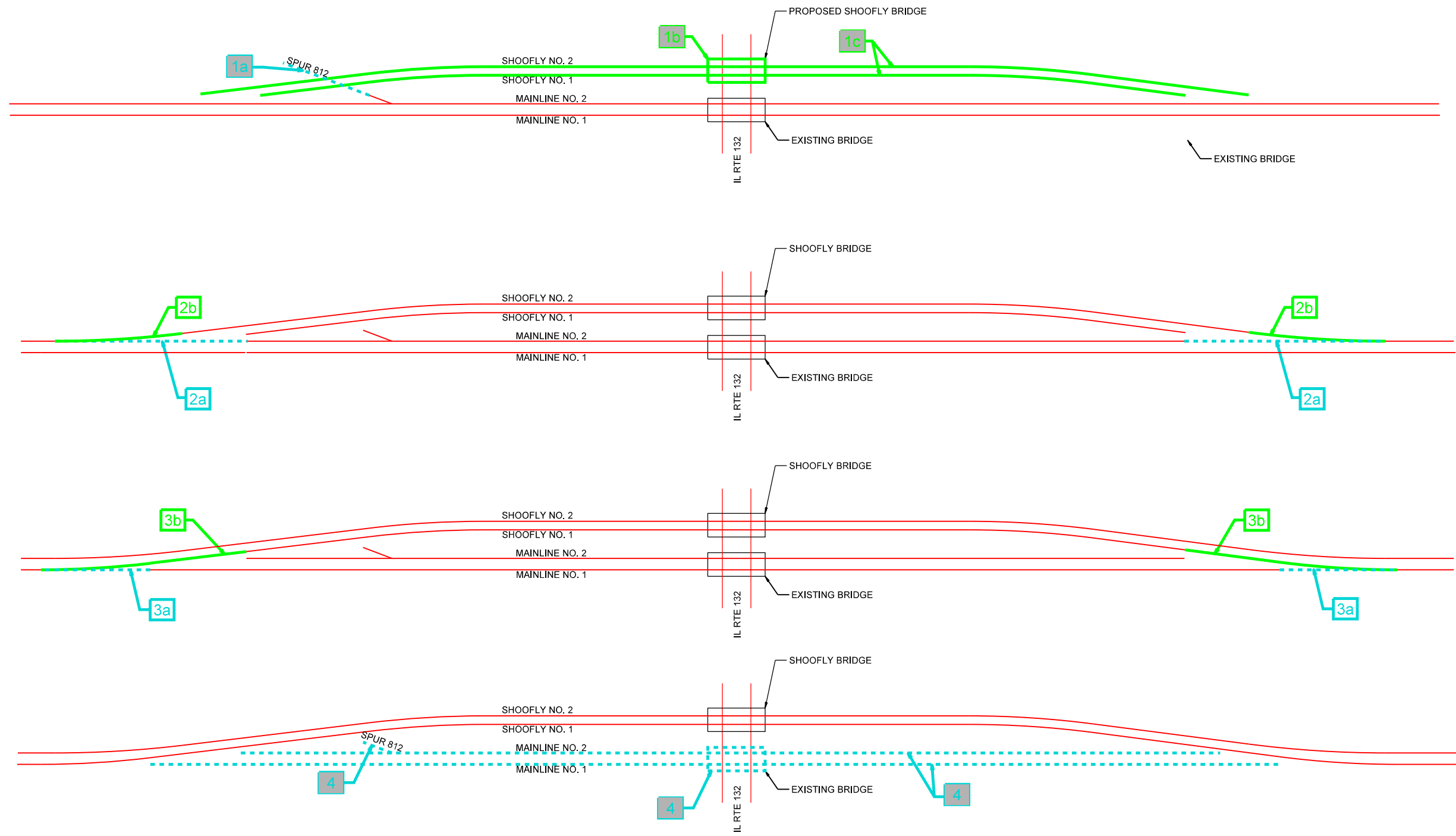


Project: UPRR over IL 132
Gurnee, IL - Construction Sched.
Date: Tue 9/27/16

Task		Project Summary		Inactive Milestone		Manual Summary Rollup		Deadline	
Split		External Tasks		Inactive Summary		Manual Summary		Progress	
Milestone		External Milestone		Manual Task		Start-only			
Summary		Inactive Task		Duration-only		Finish-only			

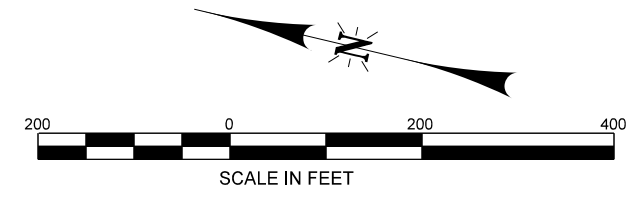
PRELIMINARY WORK SCHEDULE

	DRAWN BY: AAF-HOH	UNION PACIFIC RAILROAD Office of Assistant Vice President Engineering Design/Construction
	CHECKED BY: BAP-HOH	
	DATE: 06/07/16	
	SHEET NUMBER G006 of 009	
LOCATION & DESCRIPTION: GURNEE, LAKE COUNTY, ILLINOIS MILWAUKEE SUBDIVISION MP 37.50 TO MP 38.00 UPRR BRIDGE OVER IL RTE. 132 (GRAND AVE.)		SHEET TITLE: WORK SCHEDULE



PRESTAGE:
 PRIOR TO TRACK CONSTRUCTION, THE AREA OF THE SHOOFLY EMBANKMENT WILL BE CLEARED AND THE SHOOFLY EMBANKMENT WILL BE CONSTRUCTED ALONG WITH THE CULVERT AND OTHER DRAINAGE COMPONENTS AND EROSION CONTROL FEATURES. SEE THE MAINTENANCE OF TRAFFIC SHEETS WHICH ARE PART OF IL 132 RECONSTRUCTION.

- 1a** REMOVE SPUR TRACK 812 TO 13' CLEAR POINT OF MAINLINE 2. (BY IDOT CONTRACTOR)
- 1b** CONSTRUCT SHOOFLY BRIDGE. (BY IDOT CONTRACTOR)
- 1c** CONSTRUCT SHOOFLY TRACKS TO 13' CLEAR POINT OF MAINLINE 2. (BY IDOT CONTRACTOR)
- 2a** REMOVE MAINLINE 2 TO 13' CLEAR POINT AT SHOOFLY 1 (BY UPRR).
- 2b** CONSTRUCT NEW TRACK ON MAINLINE 2 AND AND CONNECT MAINLINE 2 TO SHOOFLY 2. (BY UPRR)
 PLACE MAINLINE TRACK 2/SHOOFLY TRACK 2 INTO SERVICE. (BY UPRR)
- 3a** REMOVE MAINLINE 1 TO 13' CLEAR POINT AT SHOOFLY 1 (BY UPRR).
- 3c** CONSTRUCT NEW TRACK ON SHOOFLY 1 TO 13' CLEAR POINT AT MAINLINE 2. (BY UPRR)
- 4** REMOVE REMAINING PORTIONS OF MAINLINE 1, 2 AND SPUR TRACK 812 INCLUDING BALLAST AND SUBBALLAST. (BY IDOT CONTRACTOR)
 DEMOLISH EXISTING UPRR BRIDGE. (BY IDOT CONTRACTOR)

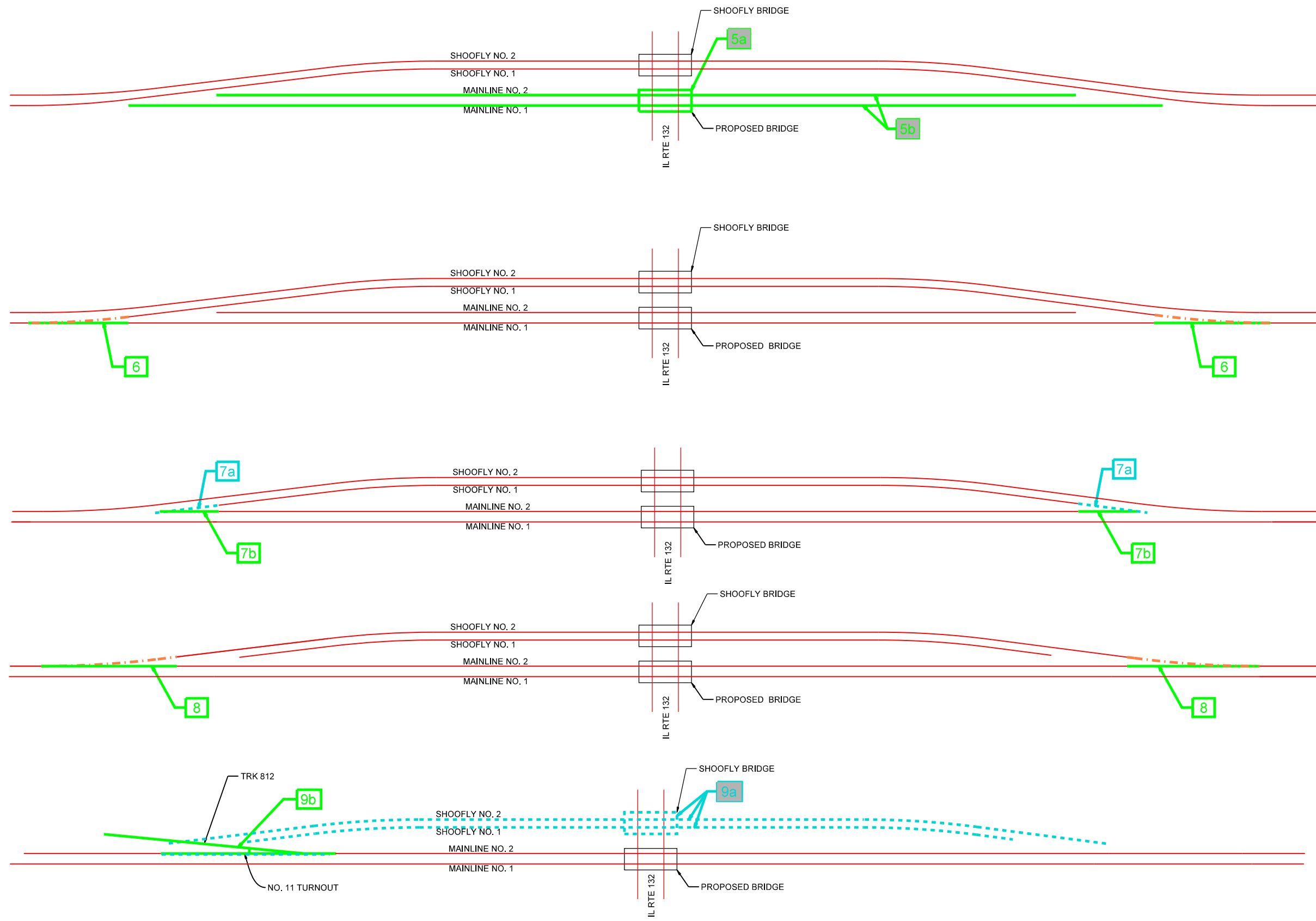


NOTES:
 1. THE INFORMATION IS SUBJECT TO REVIEW BY UPRR CIVIL/TRACK CONSTRUCTION.
 2. ANY TRACK AND TIES THAT ARE REMOVED SHALL BECOME THE PROPERTY OF THE IDOT CONTRACTOR.

LEGEND

	Existing
	Proposed
	Remove
	Shift

	DRAWN BY: AAF-HOH	UNION PACIFIC RAILROAD Office of Assistant Vice President Engineering Design/Construction
	CHECKED BY: BAP-HOH	
	DATE: 06/07/16	
	SHEET NUMBER: G007 of 009	
LOCATION & DESCRIPTION: GURNEE, LAKE COUNTY, ILLINOIS MILWAUKEE SUBDIVISION MP 37.50 TO MP 38.00 UPRR BRIDGE OVER IL RTE. 132 (GRAND AVE.)		SHEET TITLE: SUGGESTED TRACK STAGING PLAN

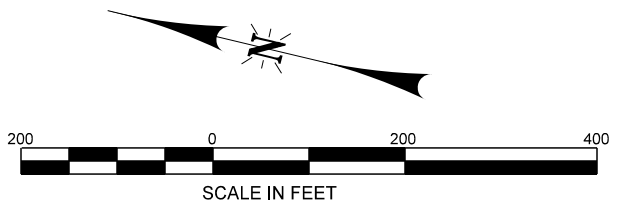


- 5a** CONSTRUCT BRIDGE. (BY IDOT CONTRACTOR)
- 5b** CONSTRUCT NEW MAINLINE TRACKS TO 13' CLEAR POINT OF SHOOFLY 1. (BY IDOT CONTRACTOR)
- 6** SHIFT TRACK AND CONNECT MAINLINE 1 TO NEW MAINLINE 1 (BY UPRR)
PLACE MAINLINE TRACK 1 INTO SERVICE (BY UPRR)
- 7a** REMOVE SHOOFLY 1 TO 13' CLEAR POINT AT NEW MAINLINE 2 (BY UPRR).
- 7b** EXTEND NEW MAINLINE 2 TO 13' CLEAR POINT AT SHOOFLY 2 - (SOUTH END ONLY) (BY UPRR).
- 8** SHIFT TRACK AND CONNECT EXISTING MAINLINE 2 TO NEW MAINLINE 2 (BY UPRR)
- 9a** REMOVE REMAINING PORTIONS OF SHOOFLY TRACKS AND SHOOFLY BRIDGE. (BY IDOT CONTRACTOR)
REMOVE SHOOFLY EMBANKMENT. (BY IDOT CONTRACTOR)
- 9b** REMOVE MAINLINE TRACK (AT TURNOUT AREA).
INSTALL TURNOUT AND CONSTRUCT SPUR TRACK 812.
PLACE MAINLINE TRACK 2 AND SPUR TRACK 812 INTO SERVICE. (BY UPRR)

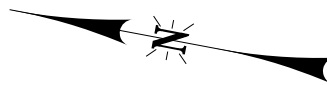
LEGEND

- Existing
- Proposed
- - - Remove
- - - Shift

- NOTES:**
1. THE INFORMATION IS SUBJECT TO REVIEW BY UPRR CIVIL/TRACK CONSTRUCTION.
 2. ANY TRACK AND TIES THAT ARE REMOVED SHALL BECOME THE PROPERTY OF THE IDOT CONTRACTOR.



	DRAWN BY: AAF-HOH	UNION PACIFIC RAILROAD Office of Assistant Vice President Engineering Design/Construction
	CHECKED BY: BAP-HOH	
	DATE: 06/07/16	
	SHEET NUMBER G008 of 009	
LOCATION & DESCRIPTION: GURNEE, LAKE COUNTY, ILLINOIS MILWAUKEE SUBDIVISION MP 37.50 TO MP 38.00 UPRR BRIDGE OVER IL RTE. 132 (GRAND AVE.)		SHEET TITLE: SUGGESTED TRACK STAGING PLAN



STATION	TRACK	ITEM	QUANTITY	CONTRACTOR TOTAL (TRACK FT.)	UPRR TOTAL (TRACK FT.)
SHOOFLY TRACK 2 STATION 0+00 to 24+17	SHOOFLY TRACK 2	332 (BY UPRR)	1753	1753	664
		332 (BY UPRR)	1753	1753	664
		332 (BY UPRR)	1753	1753	664
	2417-SURFACING AND MAINTENANCE (BY UPRR)			0	2417
	1678			1678	0
SHOOFLY TRACK 1 STATION 0+00 to 23+99	SHOOFLY TRACK 1	461 (BY UPRR)	1468	1468	930
		461 (BY UPRR)	1468	1468	930
		461 (BY UPRR)	1468	1468	930
	2399-SURFACING AND MAINTENANCE (BY UPRR)			0	2399
	188 (BY UPRR) 1376 168 (BY UPRR)			1376	356
MAINLINE TRACK 2 STATION 197+04 to 224+55	MAINLINE TRACK 2	135 (BY UPRR)	1370	1370	300
		135 (BY UPRR)	1370	1370	300
		135 (BY UPRR)	1370	1370	300
	737 (BY UPRR)			0	1081
	2751-SURFACING AND MAINTENANCE (BY UPRR)			0	2751
MAINLINE TRACK 1 STATION 100+56 to 124+46	MAINLINE TRACK 1	333 (BY UPRR)	1724	1724	666
		333 (BY UPRR)	1724	1724	666
		333 (BY UPRR)	1724	1724	666
	2390-SURFACING AND MAINTENANCE (BY UPRR)			0	2390
	333 (BY UPRR)			1724	666

**UPRR OVER IL132 - CONTRACT 60K80
TRACKWORK SCOPE OF WORK AND MATERIALS
SCHEMATIC DIAGRAM IN LINEAR FEET**

Notes:

- All work shall be performed by the IDOT contractor except where noted "by UPRR".
- All materials shall be provided by the IDOT contractor.
- Proposed spur track 812 quantities are not shown in the schematic diagram but are included in the summary of quantities totals.

SUMMARY OF QUANTITIES - RAIL WORK*

ITEM	UNIT	BY IDOT CONTRACTOR	BY UPRR	TOTAL
TREE REMOVAL (6 TO 16 UNITS)	UNIT	32		32
TREE REMOVAL, ACRES	ACRE	2		2
EARTH EXCAVATION	CUYD	35,467		35,467
BORROW EXCAVATION, EMBANKMENT AND OTHER FILLS	CUYD	11,174		11,174
TEMPORARY SOIL RETENTION SYSTEM	SQ FT	5,100		5,100
BALLAST (ON BRIDGE DECKS)	CUYD	146		146
POROUS GRANULAR EMBANKMENT	CUYD	3,846		3,846
TOPSOIL FURNISH AND PLACE, 4"	SQ YD	9,669		9,669
SEEDING, CLASS 2A	ACRE	2		2
NITROGEN FERTILIZER NUTRIENT	POUND	120		120
POTASSIUM FERTILIZER NUTRIENT	POUND	120		120
RAILROAD PROTECTIVE LIABILITY INSURANCE	L SUM	1		1
TRACK REMOVAL	TRACK FT	6,717	2,095	8,811
SINGLE SWITCH POINT DERAIL, TYPE 1	EACH		1	1
TRACK WORK (SEE NOTE 1)	TRACK FT	6,315	2,223	8,538
TRACK SHIFT	TRACK FT		1,747	1,747
SUBBALLAST	CUYD	9,170		9,170
INSTALL TURNOUT, NO 11 (SEE NOTE 3)	EACH		1	1
TURNOUT REMOVAL	EACH	1		1
UPRR TRACKWORK MATERIALS ONLY (SEE NOTE 2)	TRACK FT	10,285		10,285
REMOVAL AND DISPOSAL OF UNSUITABLES	CUYD	3,845		3,845
PAVEMENT REPLACEMENT, SPECIAL	SQ YD	220		220
PIPE CULVERTS, CLASS A, TYPE 3 30", TEMPORARY	FOOT	80		80
PIPE CULVERTS, SSP, 24"	FOOT	148		148
CHAIN LINK FENCE REMOVAL	FOOT	800		800
FENCING INSTALLATION, 6" CHAIN LINK	FOOT	800		800

*Bridge quantities not included in this table, for bridge quantities refer to the Bridge plans (separate package).
**Quantities shown are not final quantities and may be adjusted in the Final IDOT submittal package.

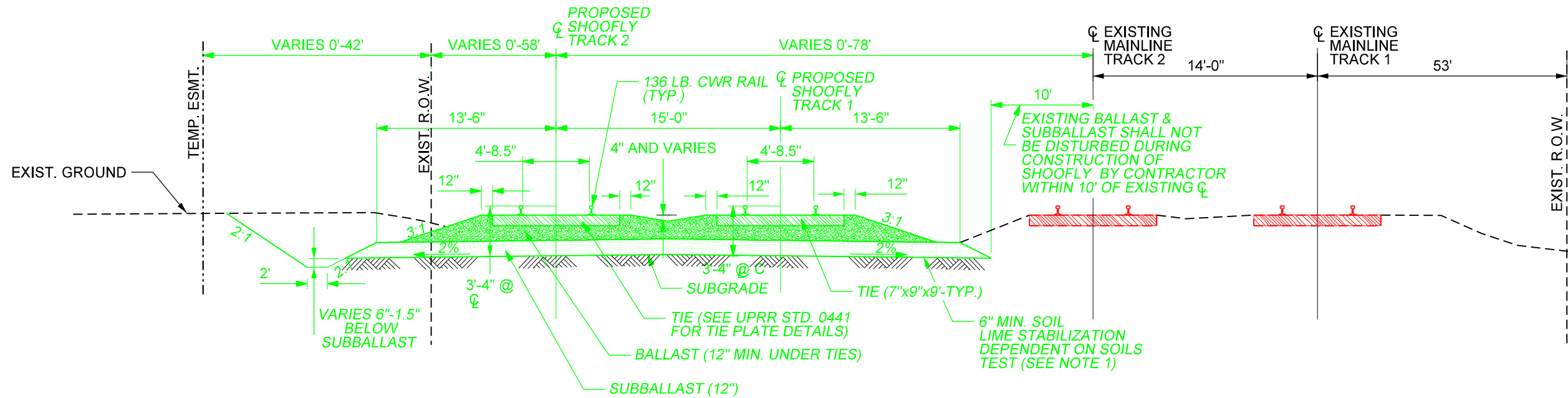
Summary of Quantity Notes:

- Track construction Foot of Track includes ballast, labor, ties, rails etc.
- Providing all track materials for installation by UPRR Foot of Track
- Installation only, track materials to be provided by IDOT Contractor.

LEGEND

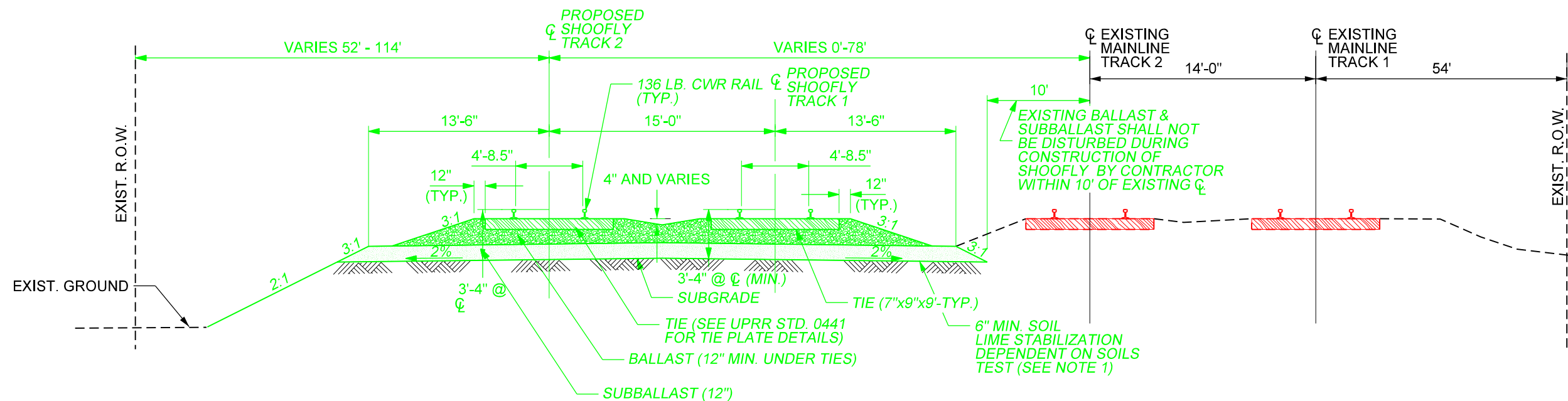
 CONSTRUCT SUBBALLAST
 INSTALL TIES
 INSTALL RAIL & BALLAST
 TRACK REMOVAL - INCLUDES BALLAST AND SUBBALLAST
 TRACK SURFACING AND MAINTENANCE
 TRACK SHIFT-INCLUDES BALLAST AND SUBBALLAST

	DRAWN BY: AAF-HOH	UNION PACIFIC RAILROAD Office of Assistant Vice President Engineering Design/Construction
	CHECKED BY: BAP-HOH	
	DATE: 06/07/16	
	SHEET NUMBER: G009 of 009	
LOCATION & DESCRIPTION: GURNEE, LAKE COUNTY, ILLINOIS MILWAUKEE SUBDIVISION MP 37.50 TO MP 38.00 UPRR BRIDGE OVER IL RTE. 132 (GRAND AVE.)		SHEET TITLE: TRACKWORK SCOPE OF WORK



**SHOOFLY TRACKS NORTH OF IL. RTE. 132
TYPICAL SECTION**

SHOOFLY TRACK 1 (STA. 0+00.00 TO STA. 9+36.87)
SHOOFLY TRACK 2 (STA. 0+00.00 TO STA. 9+34.81)

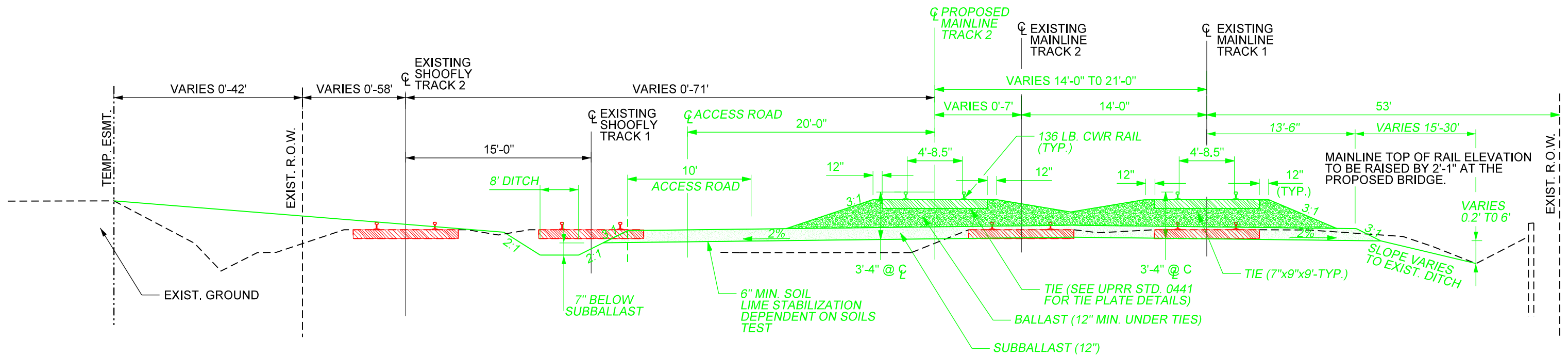


**SHOOFLY TRACKS SOUTH OF IL. RTE. 132
TYPICAL SECTION**

SHOOFLY TRACK 1 (STA. 10+12.76 TO STA. 23+98.73)
SHOOFLY TRACK 2 (STA. 10+09.87 TO STA. 24+16.85)

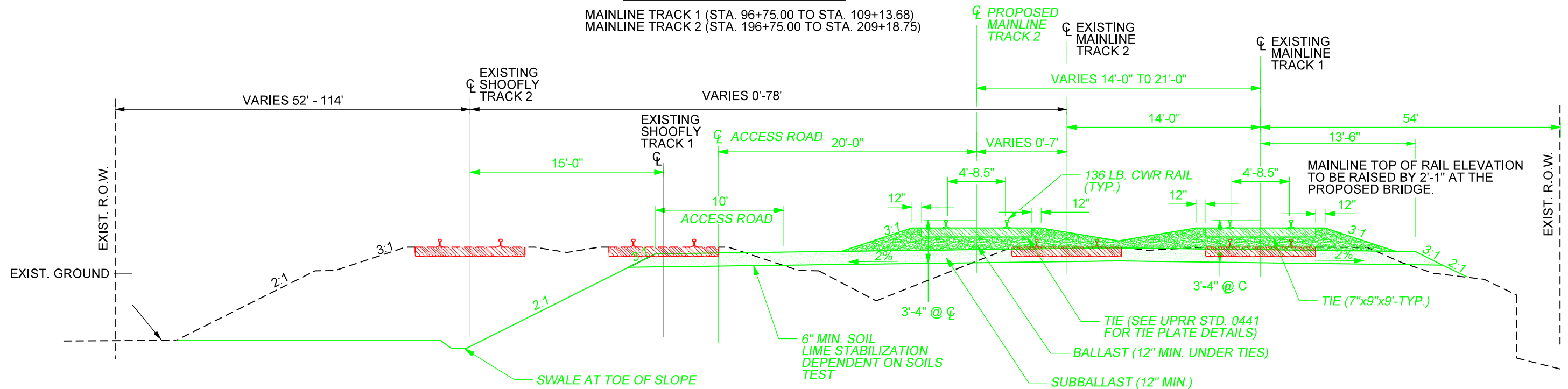
NOTE 1
AS PER THE GEOTECHNICAL REPORT, UNDERCUTTING
MAY BE REQUIRED FROM STA. 8+00 TO THE SHOOFLY
BRIDGE.

	DRAWN BY: AAF-HOH	UNION PACIFIC RAILROAD Office of Assistant Vice President Engineering Design/Construction
	CHECKED BY: BAP-HOH	
	DATE: 06/07/16	
	SHEET NUMBER C001 of 014	
LOCATION & DESCRIPTION: GURNEE, LAKE COUNTY, ILLINOIS MILWAUKEE SUBDIVISION MP 37.50 TO MP 38.00 UPRR BRIDGE OVER IL RTE. 132 (GRAND AVE.)		SHEET TITLE: SHOOFLY TRACKS - TYPICAL SECTIONS



**MAINLINE TRACKS NORTH OF IL. RTE. 132
TYPICAL SECTION**

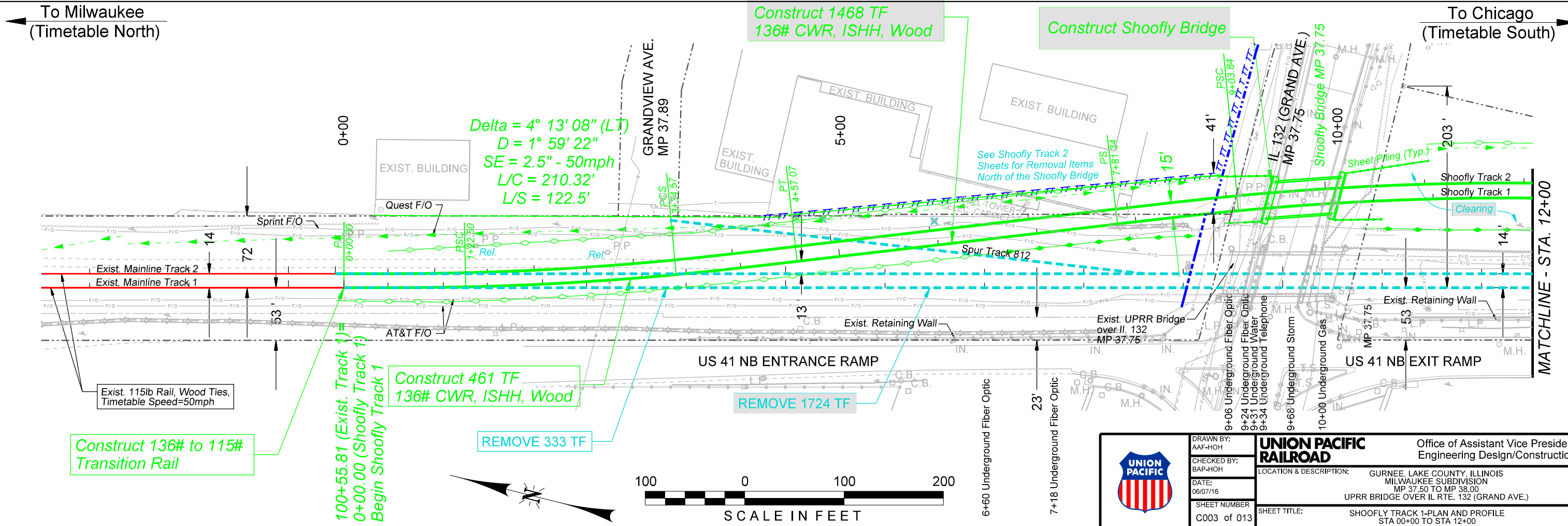
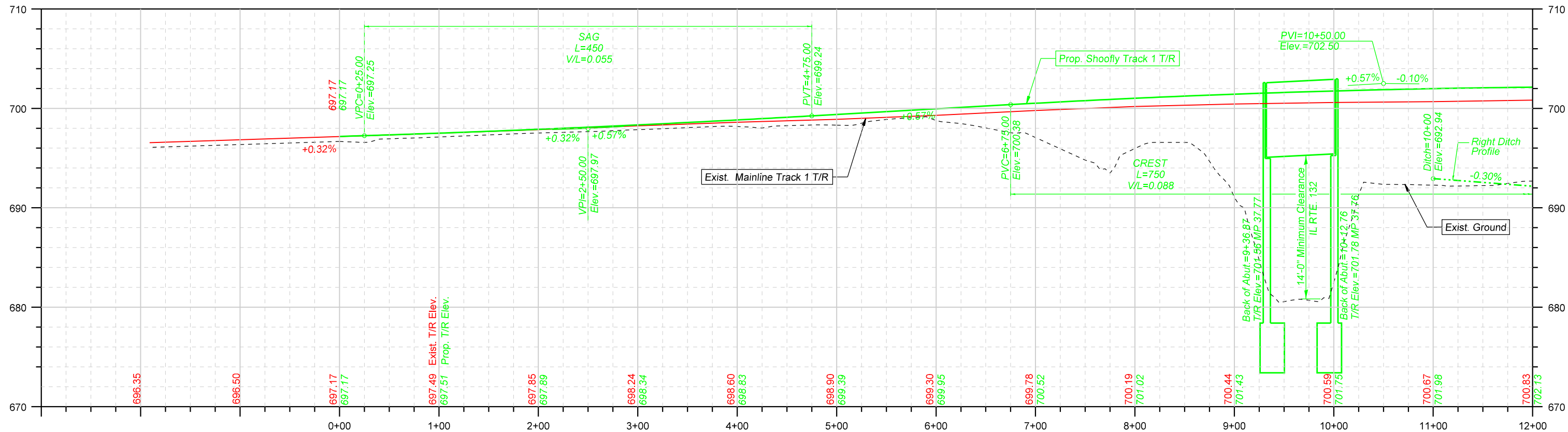
MAINLINE TRACK 1 (STA. 96+75.00 TO STA. 109+13.68)
MAINLINE TRACK 2 (STA. 196+75.00 TO STA. 209+18.75)



**MAINLINE TRACKS SOUTH OF IL. RTE. 132
TYPICAL SECTION**

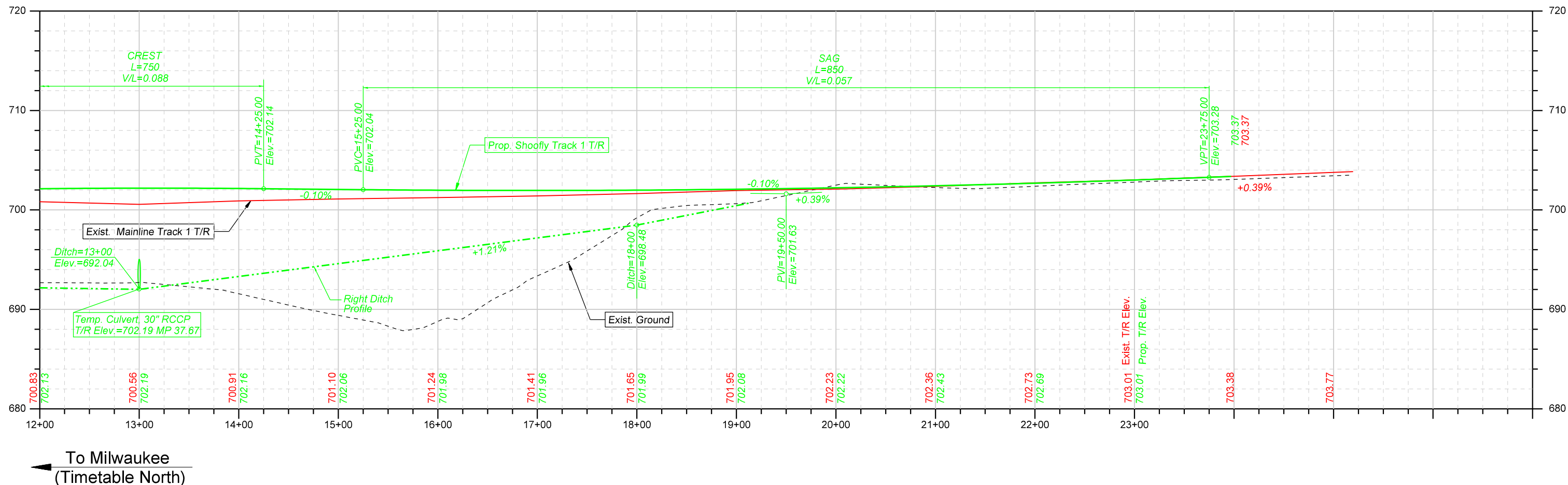
MAINLINE TRACK 1 (STA. 110+60.82 TO STA. 125+00.00)
MAINLINE TRACK 2 (STA. 210+66.31 TO STA. 225+00.00)

	DRAWN BY: AAF-HOH	UNION PACIFIC RAILROAD Office of Assistant Vice President Engineering Design/Construction
	CHECKED BY: BAP-HOH	
	DATE: 06/07/16	
	SHEET NUMBER C002 of 014	
LOCATION & DESCRIPTION: GURNEE, LAKE COUNTY, ILLINOIS MILWAUKEE SUBDIVISION MP 37.50 TO MP 38.00 UPRR BRIDGE OVER IL RTE. 132 (GRAND AVE.)		SHEET TITLE: MAINLINE TRACKS - TYPICAL SECTIONS



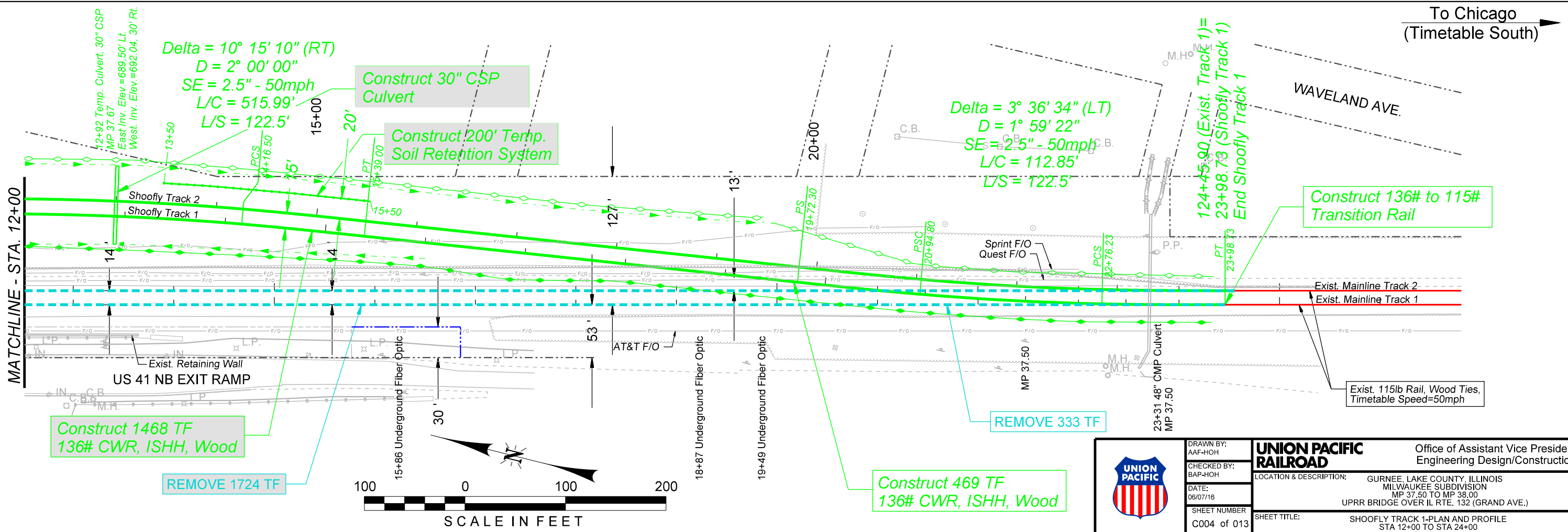
DRAWN BY: AAF-HOH
 CHECKED BY: BAP-HOH
 DATE: 06/07/16
 SHEET NUMBER: C003 of 013

UNION PACIFIC RAILROAD
 Office of Assistant Vice President Engineering Design/Construction
 LOCATION & DESCRIPTION: GURNEE, LAKE COUNTY, ILLINOIS MILWAUKEE SUBDIVISION MP 37.50 TO MP 38.00 UPRR BRIDGE OVER IL RTE. 132 (GRAND AVE.)
 SHEET TITLE: SHOOFLY TRACK 1-PLAN AND PROFILE STA 0+00 TO STA 12+00

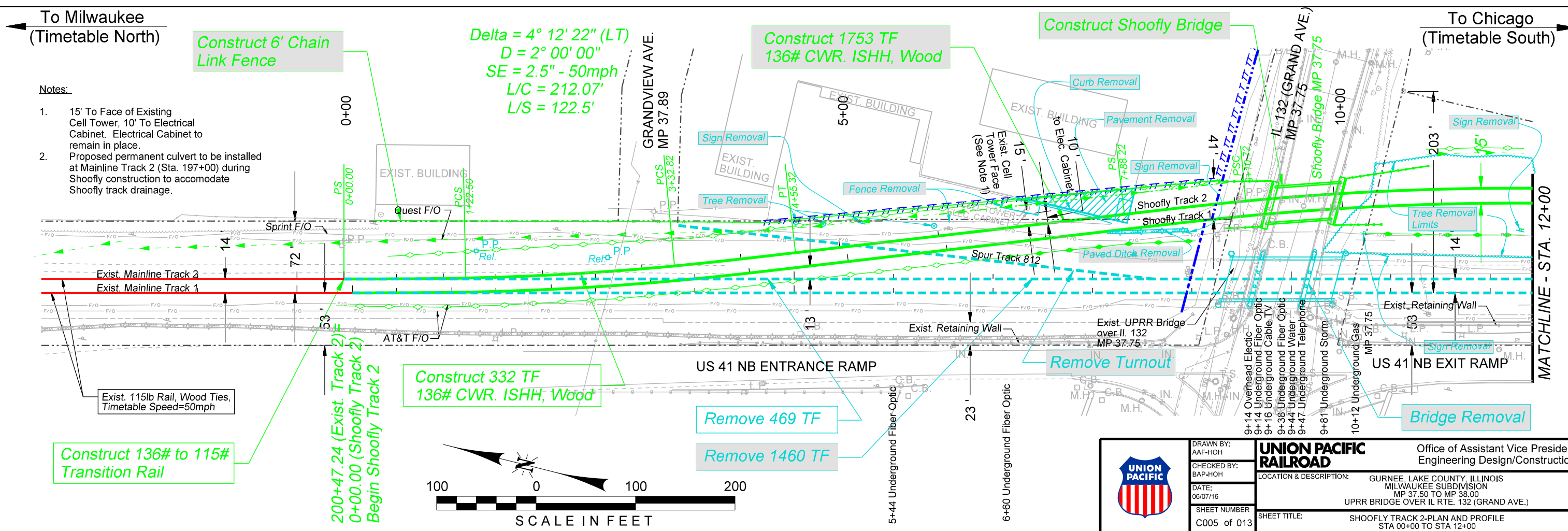
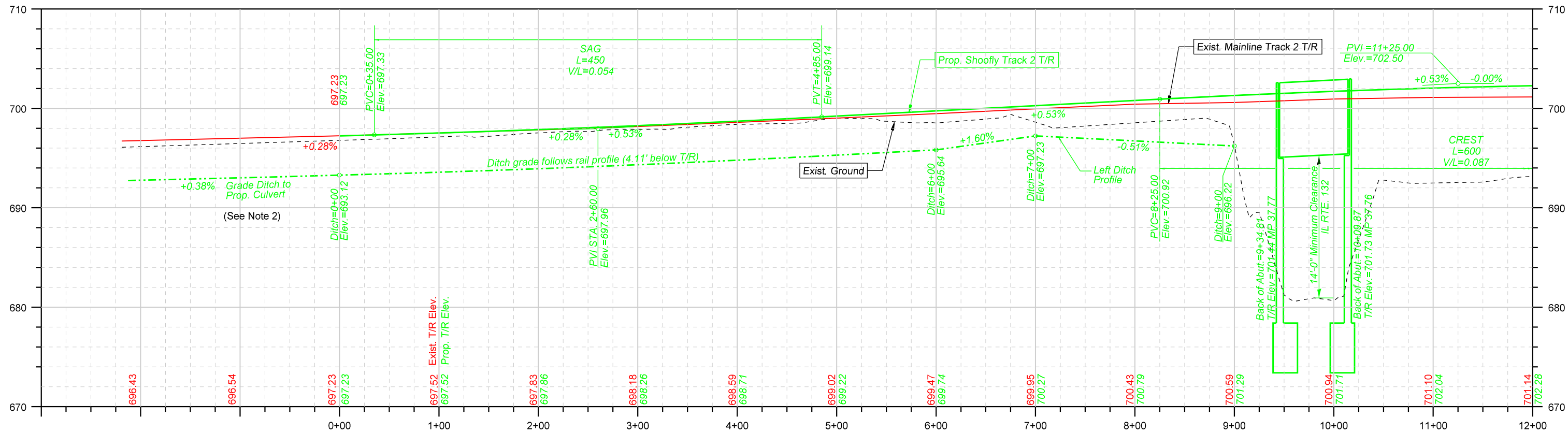


← To Milwaukee
 (Timetable North)

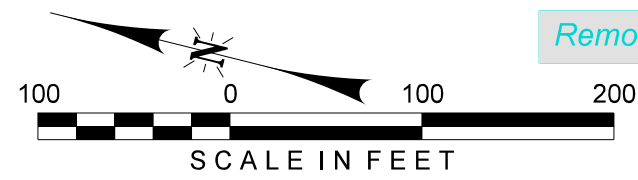
To Chicago
 (Timetable South) →



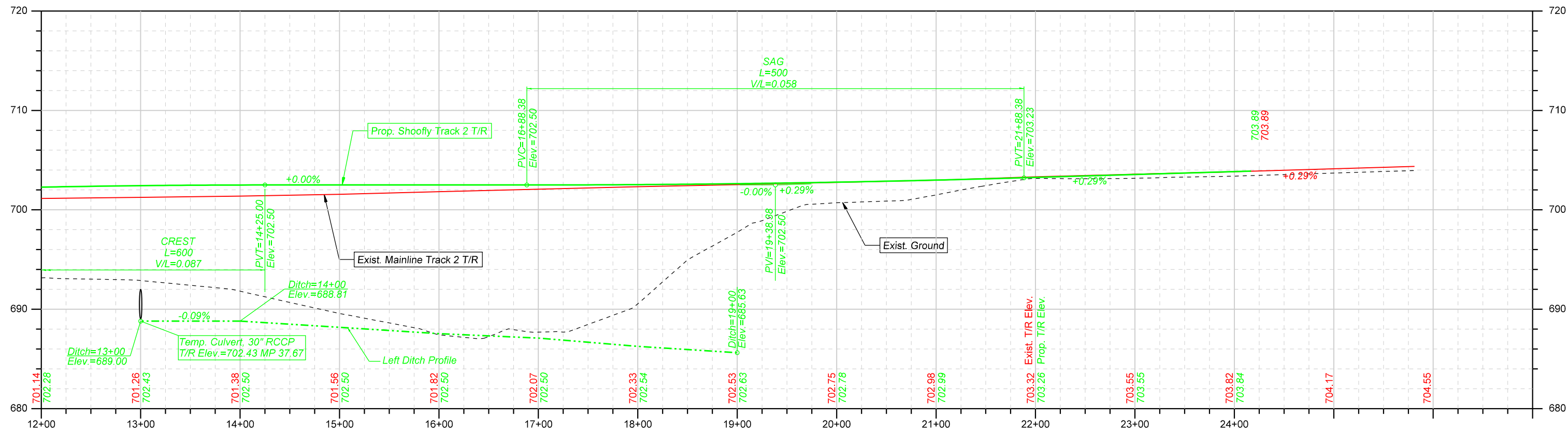
	DRAWN BY: AAF-HOH	UNION PACIFIC RAILROAD Office of Assistant Vice President Engineering Design/Construction
	CHECKED BY: BAP-HOH	
	DATE: 06/07/16	
	SHEET NUMBER: C004 of 013	
LOCATION & DESCRIPTION: GURNEE, LAKE COUNTY, ILLINOIS MILWAUKEE SUBDIVISION MP 37.50 TO MP 38.00 UPRR BRIDGE OVER IL RTE. 132 (GRAND AVE.)		SHEET TITLE: SHOOFLY TRACK 1-PLAN AND PROFILE STA 12+00 TO STA 24+00
SHEET TITLE:		



- Notes:
- 15' To Face of Existing Cell Tower, 10' To Electrical Cabinet. Electrical Cabinet to remain in place.
 - Proposed permanent culvert to be installed at Mainline Track 2 (Sta. 197+00) during Shoofly construction to accommodate Shoofly track drainage.

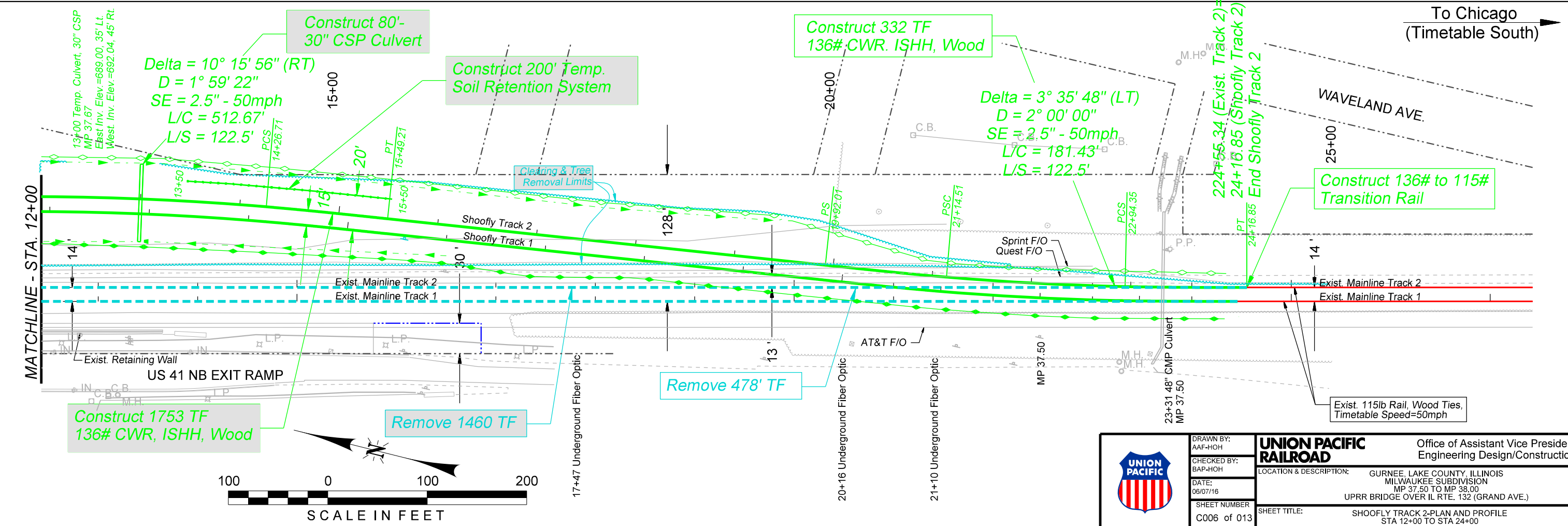


	DRAWN BY: AAF-HOH	UNION PACIFIC RAILROAD Office of Assistant Vice President Engineering Design/Construction GURNEE, LAKE COUNTY, ILLINOIS MILWAUKEE SUBDIVISION MP 37.50 TO MP 38.00 UPRR BRIDGE OVER IL RTE. 132 (GRAND AVE.)
	CHECKED BY: BAP-HOH	
	DATE: 06/07/16	
	SHEET NUMBER: C005 of 013	
SHEET TITLE: SHOOFLY TRACK 2-PLAN AND PROFILE STA 00+00 TO STA 12+00		

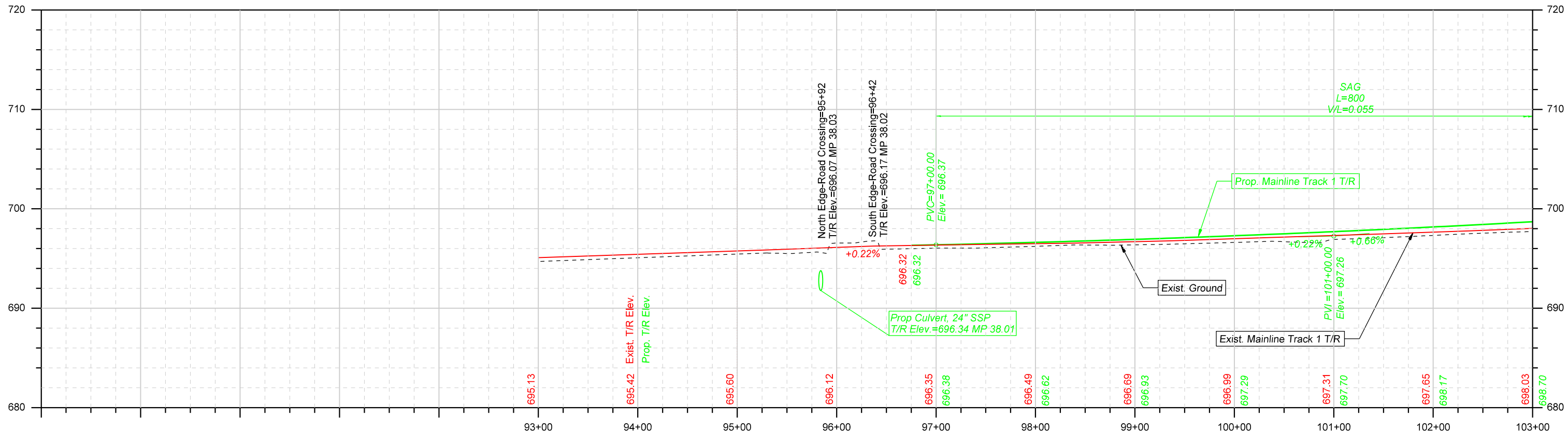


← To Milwaukee (Timetable North)

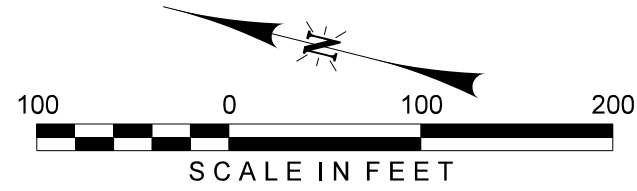
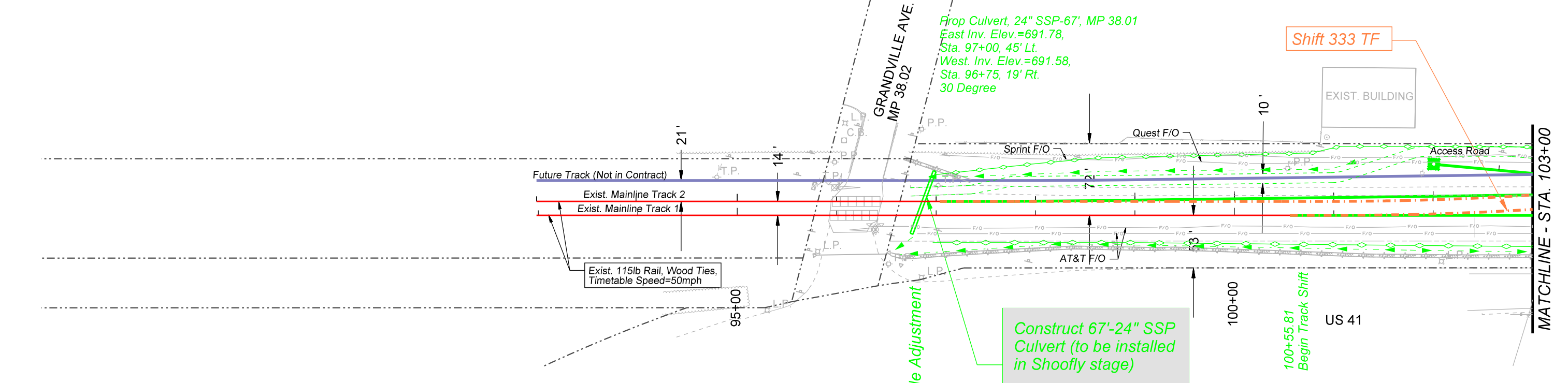
To Chicago (Timetable South) →



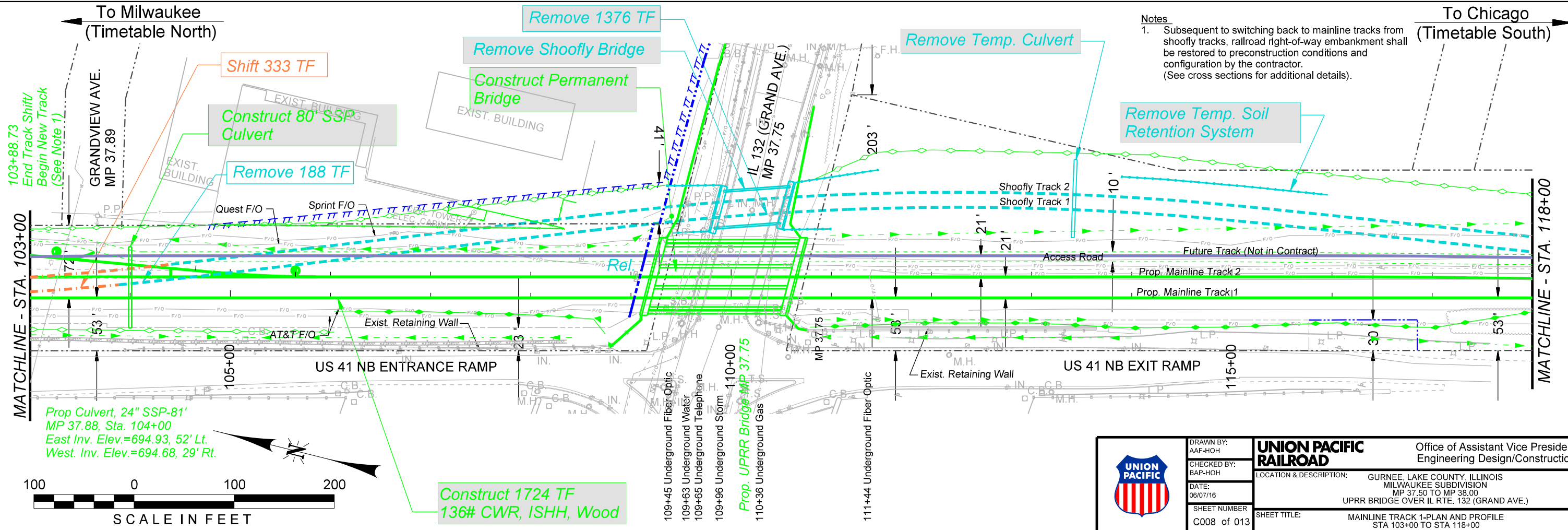
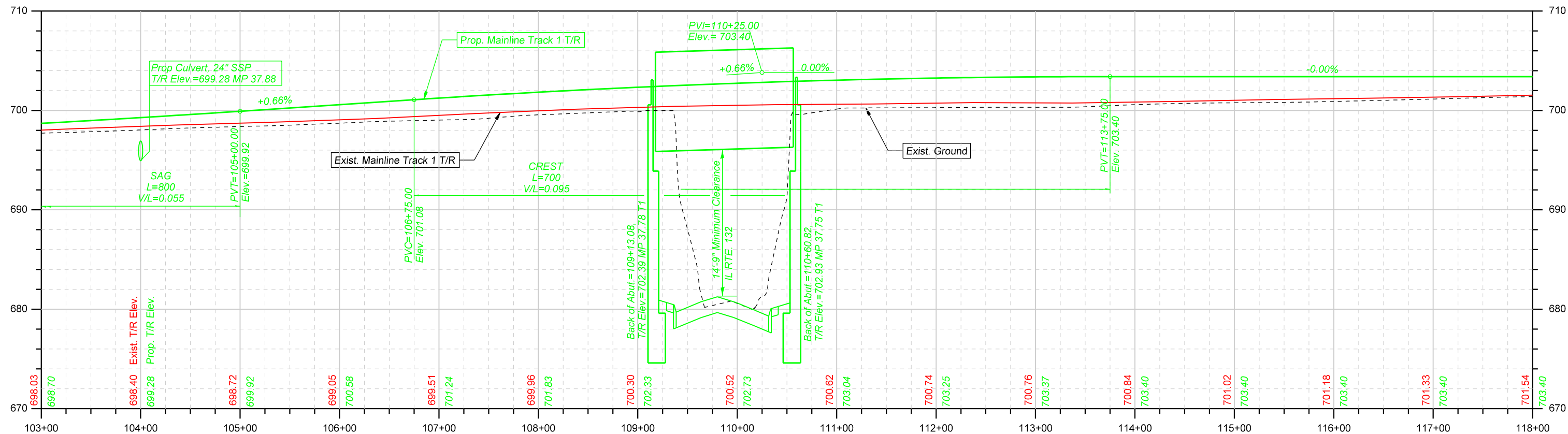
	DRAWN BY: AAH-HOH	UNION PACIFIC RAILROAD Office of Assistant Vice President Engineering Design/Construction
	CHECKED BY: BAP-HOH	
	DATE: 06/07/16	
	SHEET NUMBER: C006 of 013	
LOCATION & DESCRIPTION: GURNEE, LAKE COUNTY, ILLINOIS MILWAUKEE SUBDIVISION MP 37.50 TO MP 38.00 UPRR BRIDGE OVER IL RTE. 132 (GRAND AVE.)		SHEET TITLE: SHOOFLY TRACK 2-PLAN AND PROFILE STA 12+00 TO STA 24+00
SCALE IN FEET 100 0 100 200		



← To Milwaukee (Timetable North) To Chicago (Timetable South) →



	DRAWN BY: AAF-HOH	UNION PACIFIC RAILROAD	Office of Assistant Vice President Engineering Design/Construction
	CHECKED BY: BAP-HOH		LOCATION & DESCRIPTION:
	DATE: 06/07/16		GURNEE, LAKE COUNTY, ILLINOIS MILWAUKEE SUBDIVISION MP 37.50 TO MP 38.00 UPRR BRIDGE OVER IL RTE. 132 (GRAND AVE.)
	SHEET NUMBER: C007 of 013		SHEET TITLE: MAINLINE TRACK 1-PLAN AND PROFILE STA 93+00 TO STA 103+00



Notes
 1. Subsequent to switching back to mainline tracks from shoofly tracks, railroad right-of-way embankment shall be restored to preconstruction conditions and configuration by the contractor. (See cross sections for additional details).

	DRAWN BY: AAF-HOH	UNION PACIFIC RAILROAD Office of Assistant Vice President Engineering Design/Construction
	CHECKED BY: BAP-HOH	
	DATE: 06/07/16	
	SHEET NUMBER: C008 of 013	
LOCATION & DESCRIPTION: GURNEE, LAKE COUNTY, ILLINOIS MILWAUKEE SUBDIVISION MP 37.50 TO MP 38.00 UPRR BRIDGE OVER IL RTE. 132 (GRAND AVE.)		SHEET TITLE: MAINLINE TRACK 1-PLAN AND PROFILE STA 103+00 TO STA 118+00