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City of San Antonio Departments

- Transportation
- Public Works
- Development Services
- · Disability Access Office
- Parks and Recreation
- Planning
- Metropolitan Heath District
- Neighborhood and Housing Services
- Economic Development
- Fire
- Police
- Office of Innovation
- Library
- Sustainability
- Communications and Engagement
- · City Attorney's Office

School Districts

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- Northside Independent School District

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¹ The asterisks "*" denote those City Council Districts located within the Culebra Road Multimodal Transportation Study

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			 (August 2, 2022) Appendix A-3: Culebra Road Transportation Study Documentation of October 2022 Community Open Houses (November 15, 2022) 	

EXECUTIVE SUMMARY

Projected to increase its population by approximately one million people between 2010 and 2040, the City of San Antonio (City) looks to plan for this growth in a smart, sustainable manner with policies and practices that consider both the opportunities and available funding for all modes of transportation. There is not one strategy or one funding source that will provide the needed resources for the City to implement a multimodal transportation plan. The City must continuously communicate a transportation strategy for the future, develop proposed improvements that address all modes, and provide a method of prioritizing projects with safety and equity at the forefront.

As part of this overall effort, in March 2018, the Alamo Area Metropolitan Planning Organization (AAMPO) Transportation Policy Board approved funding for Arterial Multimodal Mobility Planning as part of the AAMPO's 2019-2022 Transportation Improvement Program (TIP). Culebra Road was the first of four studies funded by the City and the AAMPO to conduct a detailed transportation planning and engineering analysis, leading to a concept design for implementation. Culebra Road was one of twelve corridors examined at a high-level as part of the San Antonio (SA) Tomorrow Multimodal Transportation Plan completed in 2016. Culebra Road also has one of the highest occurrences of severe pedestrian injuries when compared to other corridors in the City according to the City's 2017 Severe Pedestrian Injury Areas (SPIA) Report.

Transportation Study was to identify corridor deficiencies within the study limits, obtain community feedback on the corridor, prepare traffic counts and modeling, conduct an alternatives analysis of potential improvements, and document cost estimation of the preferred concepts. This effort aims to transition the corridor from an uncomfortable and unsafe environment for non-vehicular users to an equitable multimodal corridor for people who walk, bike, take transit, and drive.

The purpose of the Culebra Road Multimodal Corridor

Safety was identified as the number one concern to be addressed by this study. In addition, there were several other issues and challenges identified with the corridor, including and not limited to: disconnected sidewalks from adjoining businesses; ADA non-compliant and discontinuous sidewalks; long pedestrian crossing distances (both street width and frequency); narrow and unprotected bicycle facilities; transit facilities with no shelter; poorly marked crosswalks; and skewed intersections. Proposed concepts were developed, evaluated, and resulted in recommended preferred concepts for future implementation. The preferred concepts include providing wider and protected sidewalks; dedicated bicycle facilities; improved crosswalk connections; shade trees; and multi-purpose traffic calming devices.

This report provides a detailed summary of the corridor's existing conditions and challenges, public engagement process, proposed concepts, preferred concept recommendations, as well as an implementation matrix for priority segments by the City of San Antonio Transportation Department and their consultant team led by WSP USA, Inc., Poznecki-Camarillo, LLC, and MIG, Inc.

Initiated in Fall 2020, the study has progressed through the following phases:



Data Collection



Facilitated Community Visioning



Concept Refinement for Prioritization and Implementation



INTRODUCTION

INTRODUCTION Culebra Road Multimodal Corridor Transportation Study

INTRODUCTION

WHY A STUDY?

Culebra Road was one of twelve corridors examined at a high-level as part of the San Antonio (SA) Tomorrow Multimodal Transportation Plan completed in 2016. The Plan identified a 5-Year Action Plan with over 50 actions, one of which was to perform detailed corridor analysis on at least three corridors. As a result, Culebra Road was identified to be one of the first of four studies funded by City and AAMPO in 2018 to conduct a detailed transportation planning and engineer-

The Culebra Road Corridor extends approximately thirteen (13) miles along Culebra Road from I-10 west to Loop 1604. The corridor is located in the western part of the City of San Antonio as shown in FIGURE 1.

Located in Council Districts 1, 5, 6, and 7 in San Antonio.



i

Higher severe pedestrian injuries than other corridors in the City.

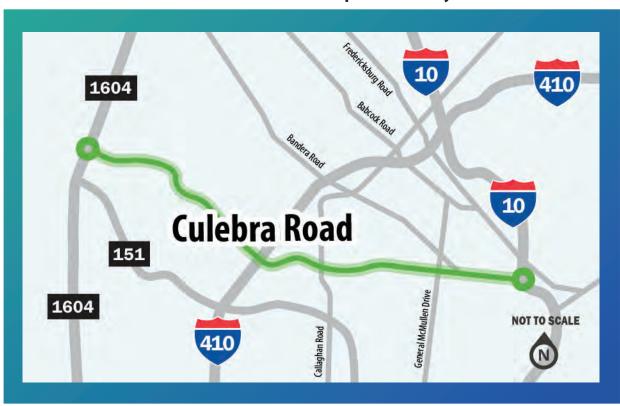
3 Hotspots





ing analysis, leading to a concept design for implementation. One of the reasons Culebra Road was chosen was due to the 2017 Severe Pedestrian Injury Areas (SPIA) Report, produced as part of the City's Vision Zero initiative, which showed higher severe pedestrian injuries than other corridors in the City, with three hotspots identified and a total of 12 fatalities and 14 incapacitating injuries based on 2011-2015 crash data.

FIGURE 1: Culebra Road Multimodal Corridor Transportation Study Area



The Culebra Road Multimodal Transportation Study represents the City's effort to proactively plan for safe, attractive, and sustainable transportation improvements consistent with the community's priorities.

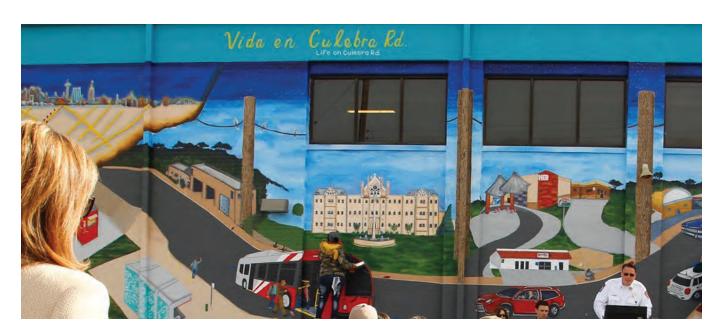
PPENDIX

PLANNING PROCESS

The City of San Antonio implemented a phased planning process in which the study team gathered data, facilitated visioning with key stakeholders and the community, and refined concepts for implementation as depicted in **FIGURE 2**.

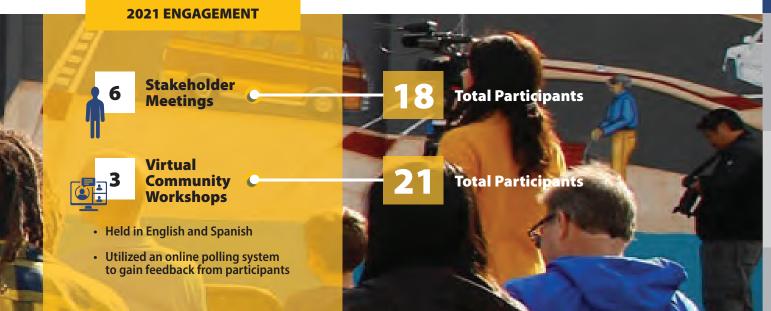
FIGURE 2: Culebra Road Multimodal Corridor Transportation Study Planning Process





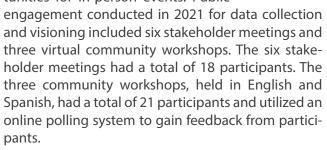
Meaningful public engagement has been essential to the development of this study's vision and priorities. Beginning in March 2021 and culminating in October 2022, the City of San Antonio provided multiple opportunities for the public to participate in the Culebra Road Transportation Study planning process and express their ideas, hopes, and concerns through numerous avenues.





The study successfully shifted to virtual engagement when the COVID-19 pandemic limited opportunities for in-person events. Public

THE PUBLIC PROCESS



Also conducted during this first round of public engagement, a three-month long Community Visioning Survey resulted in over 2,500 map survey responses.

Community Visioning Survey



Over 600 people participated in our outreach methods in 2021.



To maximize participation and engagement from community members for the community workshops, a multi-faceted outreach effort was implemented that included both traditional and non-traditional methods. These efforts included:



Newspaper Ads – reach of about 215,000 households



Postcards - 788 mailed



Email Blasts – 140 individuals emailed



Media Coverage – 6 news stories



Social Media Outreach – Facebook ads placed with about 73,000 views

Public engagement conducted in 2022 for proposed corridor concepts review included bilingual community open houses with in person and virtual options. The two in-person community open houses had 38 participants in person and 1,231 participants online with 37 comments received.



PHOTO 1: 2022 Community Open House at Holy Cross High School

To maximize participation and engagement from community members for the community workshops, a multi-faceted outreach effort was implemented that included both traditional and non-traditional methods. These efforts included:



Newspaper Ads – reach of about 215,000 households



Postcards - 3,890 mailed



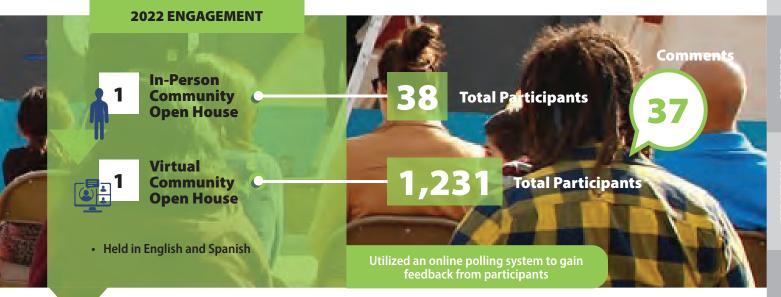
Email Blasts – 388 individuals emailed



Media Coverage – two news stories



Social Media Outreach – geofenced ads placed with about 850,000 views



Throughout the planning process, the City and consultants engaged and interviewed numerous corridor stakeholders from neighborhood associations (e.g., West End Hope in Action, Northwest Crossing, Thunderbird Hills, Great Northwest, University Park) to cycling groups (e.g., SATX Social Ride, Activate SA, Bike San Antonio, South Texas Off Road Mountains, Earn-a-Bike). Careful consideration was given to identify environmental justice (minority/low-income) populations as well as those with limited English proficiency.

In August 2022, the City convened a Technical Advisory Group composed of public agency partners to advise City staff and consultants on the development of the study's strategies and specific implementable short-term and long-term improvements.

See the corridor vision on page 13 for a summary of comments received, the detailed Community Engagement Summary Reports, and the Technical Advisory Group presentation can be found in **APPENDIX A**.

CULEBRA SAFETY CAMPAIGN

Adopted in 2016, Vision Zero SA is San Antonio's plan to eliminate traffic fatalities and to enhance the safety of the transportation system. The program uses a combined approach of six essential elements for a safe transportation system: Education, Encouragement, Engineering, Enforcement, Evaluation, and Equity. Each of these six elements are vital to help reach the program's goal. The engineering component is key to shape the way our roadways are designed and how people use them.

The City's Transportation Department launched the Culebra Road Safety Campaign (FIGURE 3) in June 2022 through August 2022 encouraging drivers and pedestrians to modify their behaviors. For drivers, the message was "Slow down and put your distractions away". For pedestrians, the message was to use marked crosswalks whenever possible and be aware of their surroundings. The educational campaign consisted of billboards, bus advertising, posters, and other outreach that informed people of safe habits and encouraged them to provide

input through an online survey. On August 11, 2022, the City held a press conference to present insights gained from the scheduled campaign. Approximately 20,000 mailers were sent to residents living along or near Culebra Road. More than a thousand responded to the mailer and participated in the online survey.

Through the online survey, the community expressed that their greatest challenges to safe travel on Culebra Road are people driving too fast, distracted drivers, and difficulty crossing the street. Approximately 92% of respondents drive as a primary mode of travel, while those who walk or ride a bicycle represent 4% each. Less than 1% of respondents travel using public transit as their primary mode.

A dynamic report of the survey can be viewed online:

https://publicinput.com/Report/u2dgb3tp4yx

CULEBRA ROAD SAFETY CAMPAIGN

66 Vision Zero SA is committed to bringing education and information to help prevent deaths along Culebra Road by focusing attention on drivers and pedestrians along this corridor.



STUDY GOALS Culebra Road Multimodal Corridor Transportation Study

STUDY GOALS

Goals implemented for the Culebra Road Multimodal Corridor Transportation Study included:

1

Create a safe corridor for all users of Culebra Road

2

Transform
Culebra into an
equitable, multimodal
corridor for people
who walk, bike,
take transit, and drive

3

Coordinate
and build upon
previous and concurrent
studies and initiatives

4

Develop conceptual designs for short-and long-term improvements

Ensure meaningful community and stakeholder engagement



CORRIDOR VISION Culebra Road Multimodal Corridor Transportation Study

CORRIDOR VISION

Through compilation of the ideas, aspirations, and concerns collected, the community vision emerged.

Improve safety for people walking, biking, taking transit and driving.

Manage traffic

speeds to

allow more

people to walk

and bike.

Participants repeatedly stressed safety as the number one concern for the corridor.

Participants noted the need for a balance between keeping vehicular traffic moving and the safety of pedestrian and bicyclists near highspeed traffic.

Provide safer walking and biking connections to adjacent neighborhood destinations and trail connections.

Participants noted the need for pedestrian/bicyclist connectivity including for that "last mile" between transit stops and their destination.

Although west San Antonio is thriving, participants would like to see opportunities for businesses to stay and succeed in the corridor. Participants especially desired more grocery shopping, dining options, and family-friendly establishments which could be better served by safe multimodal improvements.

Strengthen business and economic development.

Enhance and incorporate the natural environment.

Participants desired improved area aesthetics with proper landscape maintenance to overall improve the attractiveness of the corridor.

Improve overall shade and comfort.

Participants agreed that shade and comfort should accompany the implementation of pedestrian amenities especially in the San Antonio heat.

Improve neighborhood identity. The community on the west side of San Antonio are proud of their heritage and unique landmarks including St. Mary's University and the Basilica of the National Shrine of the Little Flower. There is a desire to showcase the Culebra Road corridor as a destination of choice within the City of San Antonio.

CORRIDOR OVERVIEW Culebra Road Multimodal Corridor Transportation Study

CORRIDOR OVERVIEW

CORRIDOR DESCRIPTION

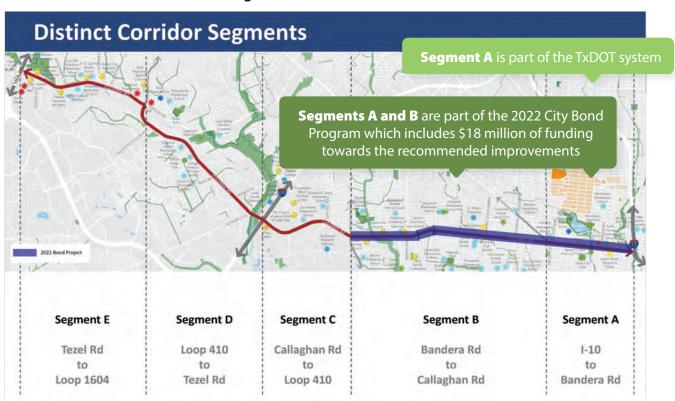
This corridor is a principal/minor arterial ranging from a four- to seven-lane cross section, and it spans several land use characteristics. The City of San Antonio Major Thoroughfare Plan identifies Culebra Road as a Primary Arterial Type A from I-10 to NW 24th Street / Bandera Road; a Secondary Arterial Type A from NW 24th Street / Bandera Road to Loop 410; and a Primary Arterial Type A from Loop 410 to Loop 1604.

The study considers 13 miles of the Culebra Road corridor from I-10 to Loop 1604.



Due to the length of the corridor and land use characteristics, **THE CORRIDOR WAS DIVIDED INTO FIVE (5) SEGMENTS**representing different context zones as shown in **FIGURE 4**.

FIGURE 4: Culebra Road Corridor Segments



Starting at I-10 and heading westward:

E	D	C	В	A
2.5 miles	3.2 miles	1.8 miles	Total: 3.7 miles	1.5 miles

Segment B1: 1.7 miles+ Segment B2: 2 miles





The study area is in an urbanized area in western San Antonio, Located within the Texas Blackland Prairies ecoregion, this area is characterized by a diverse assortment of perennial and annual grasses, as well as rich soil. Due to the urbanized nature of the study area, vegetation consists primarily of maintained, urban grasses and ornamental trees. Portions of the study area are located within Karst Zones 2, 3, and 5. Karst Zone 2 is defined as areas having a high probability of containing a suitable habitat for invertebrate species, Karst Zone 3 is defined as areas that probably do not contain endangered karst invertebrate species, and Karst Zone 5 is defined as areas, both cavernous and non-cavernous, that do not contain endangered karst invertebrate species. There are no critical habitat units for federally listed threatened and endangered species in the study area.

This section summarizes the existing overall environmental and transportation features within the **Culebra Road corridor.**

The study area is located within the Headwaters of San Antonio and Leon Creek watersheds, and several creeks cross Culebra Road within the study limits, including Apache Creek, Culebra Creek, Leon Creek, Martinez Creek, Zarzamora Creek, and associated tributaries. Three parks and recreational facilities are located adjacent to the corridor, including Alazan Creek Park, Culebra-Helotes Creek Greenway, and Leon Creek Greenway. The study area is not located within a National Register of Historic Places (NRHP) or City historic district. However, there is an adjacent City Landmark located at 1107 Culebra Road. See the Environmental Constraints Maps in **APPENDIX B**.



EQUITY INDICATORS

The City of San Antonio maintains an Equity Atlas, which is an interactive tool that highlights the demographics, disparities, and some infrastructure distribution within the city. Below is a summary of the equity indicators for the study corridor.

FIGURE 5: RACE

The census tracts comprising the study limits have a People of Color score of 3 to 5, as defined to the right:

- **5** The percent people of color is between 92.50 - 99.71 percent.
- **4** The percent people of color is between 83.59 - 92.36 percent.
- **3** The percent people of color is between 72.05 - 83.42 percent.



FIGURE 6: INCOME

The census tracts comprising the study limits have a Median Household Income score of 3 to 5, as defined to the right:

- **5** The median household income is between \$11,360 to \$35,900.
- **4** The median household income is between \$35,981 to \$42,377.
- **3** The median household income is between \$42,594 to \$55,351.



FIGURE 7: EDUCATION

The census tracts comprising the study limits have an Education Rank score of 3 to 5, as defined to the right:

- **5** The percent Education Less than High School Graduate or Equivalent is between 33.02 58.21 percent.
- **4** The percent Education Less than High School Graduate or Equivalent is between 19.02 32.97 percent.
- **3** The percent Education Less than High School Graduate or Equivalent is between 10.42 18.99 percent.



FIGURE 8: LANGUAGE

The census tracts comprising the study limits have a Language Rank score of 3 to 5, as defined to the right:

- **5** The percent Speak Other Language at Home – Speak English Less than "Very Well" is between 20.06 – 37.92 percent.
- **4** The percent Speak Other Language at Home – Speak English Less than "Very Well" is between 13.90 – 20.02 percent.
- **3** The percent Speak Other Language at Home – Speak English Less than "Very Well" is between 9.73 – 13.88 percent.







TRANSPORTATION - RELATED DEMOGRAPHICS

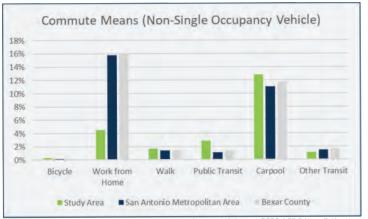
According to the U.S. Census Bureau, approximately 153,303 people live within the study area. The current unemployment rate in the study area is 8.0 percent. This is slightly lower than the 3.5 percent unemployment rate for San Antonio and the 3.8 percent unemployment rate for Bexar County. In addition to the low prevailing unemployment rate, Bexar County is predicted to undergo substantial population and employment growth over the next 30 years.

According to the Texas Water Development Board (TWDB), Bexar County is predicted to grow 0.9 percent between 2020 and 2070.

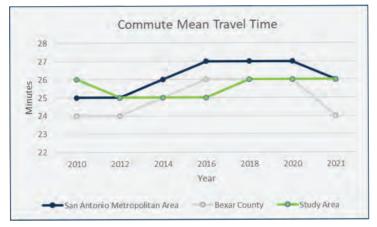
Vehicle ownership is widely prevalent among residents within the study area. According to the U.S. Census Bureau, only approximately 7.2 percent of households in the study area do not have access to a car. Further, approximately 56.9 percent of households in the study area have access to two or more vehicles, and approximately 76.4 percent of residents commute alone. The average household size is 3.3 people and there are approximately 22,000 children between the ages of five and 14 within the study area. Other key study area demographics are provided in the graphics below.

The provided demographic information does not consider people commuting from other areas to work in the study area. To some degree, commuters could be more likely to rely on modes other than single occupancy vehicles (e.g., public transportation, walking, biking, etc.).

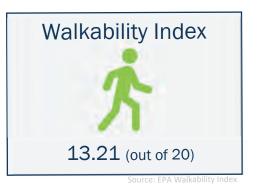
FIGURE 9: Transportation-Related Demographics



Source: US Census 2020 ACS 5-Year Estimate



Source: US Census 2010-2021 AC







Source: EF

² For the purpose of this section, the study area consists of U.S. Census Bureau census tracts adjacent to the Culebra Road corridor within the city limits.



SIDEWALK INFRASTRUCTURE

The City maintains an inventory of existing sidewalks, trails, and areas where severe pedestrian injuries have occurred. There are also several unsignalized mid-block crosswalks and Z-crossings throughout the corridor that have been installed by the City and TxDOT over the last several years in an effort to improve pedestrian safety. The severe pedestrian injury areas are shown below in **FIGURE 10**.

Sidewalks and crosswalks in the study area vary in design and state of repair. A detailed inventory of existing sidewalk conditions including missing sidewalks, existing pedestrian facilities which do not meet the 2010 ADA standards for accessible design, as well as locations missing "detectable warnings" and/or needs improvements are included in the Existing Conditions Technical Report (**APPENDIX C**).



Study Limits

ExistingSidewalks

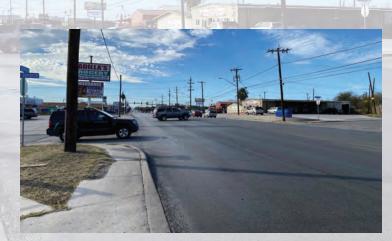
Existing Trails Severe Pedestrian Injury Areas

FIGURE 10: Pedestrian Facilities



Source: City of San Antonio, 2022

PHOTO 2: Discontinuous and Non-ADA Compliant Sidewalks





The City maintains an inventory of existing, lanes, routes, trails and areas where severe bicyclist injuries have occurred. The only segment that contained bicycle facilities along Culebra Road within the study limits was Segment D (refer to FIGURE 4). Severe bicyclist injury areas are noted in the following FIGURE 11. These connections improve multimodal connectivity and provide additional mode choice but serve limited users due to access restrictions.



Limits



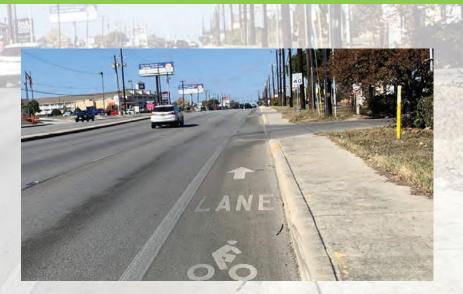
Severe Bicyclist Injury Areas

FIGURE 11: Bicycle Facilities



Source: City of San Antonio, 2022

PHOTO 3: Narrow and Unprotected Bicycle Lanes in Segment D





VIA Metropolitan Transit serves Culebra Road within the study limits. All segments had transit stops. As shown in **FIGURE 12**, eight routes (82, 88, 282, 288, 606, 610, 618 and 660) currently serve the Culebra corridor with varying service frequency. Routes and service frequency are subject to change in future.

Access to these bus routes is challenging. Gaps in the sidewalk network mean that people have no accommodating way to reach the stops. Access to the bus stops is further impeded by pedestrian crossing restrictions at key signalized intersections where bus stops are located. Additionally, most of the bus stops are little more than a sign on a post with no shelter.



Study Limits VIA Bus Routes

FIGURE 12: VIA Transit Routes



Source: VIA Metropolitan Transit, 2022

PHOTO 5: Image of bus stop along Culebra Road with no shelter





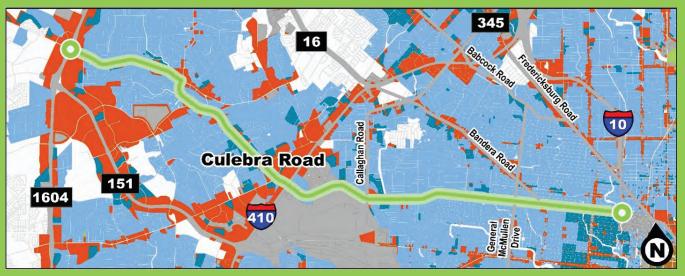
Land use within the project area is predominantly residential and commercial with approximately 839 adjacent properties. Industrial land uses exist between I-410 and Callaghan Road where Southwest Research Institute and Gustafson Stadium are located. Several single-family residential neighborhoods are located adjacent to the corridor, including Culebra Crossing, Culebra Park, Grissom Trails, Loma Park, Memorial Heights, Mountain View Acres, Pipers Meadow, Prospect Hill/West End Hope in Action, Reserve at Culebra Creek, Timber Ridge, University Park, and Uptown Neighborhood Association.

Nearby schools include the following: Beacon Hill Elementary School, Cardenas Center Dolores B. Linton

Elementary School, Esparza Elementary School, Fenwick Elementary School, HB Zachry Middle School, Huppartz Elementary School, IDEA Hidden Meadow, Kipp Aspire Academy, Little Leaps Academy, Little Flower Catholic School, Little Treehouse Early Learning Center, Lloyd M Knowlton Elementary School, Loma Park Elementary School, Memorial High School, Myers Elementary School, Nelson Early Childhood Center, Positive Solutions Charter, Roy Cisneros Elementary School, Southwest Preparatory, Southwest Prep Northwest, and St. Mary's University. Community facilities adjacent to the corridor include Memorial Library and Fire Station #26.



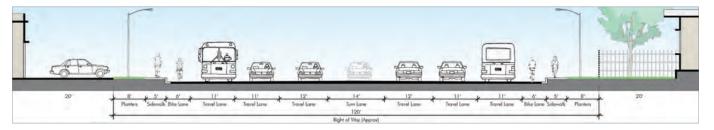
FIGURE 13: Existing Land Use





The roadway characteristics of the corridor were summarized visually for each of the five segments. **FIGURE 14** shows an example of the roadway cross section for Segment D. See the **IMPLEMENTATION** Section for the cross sections of each segment.

FIGURE 14: Existing Cross Section for Segment D



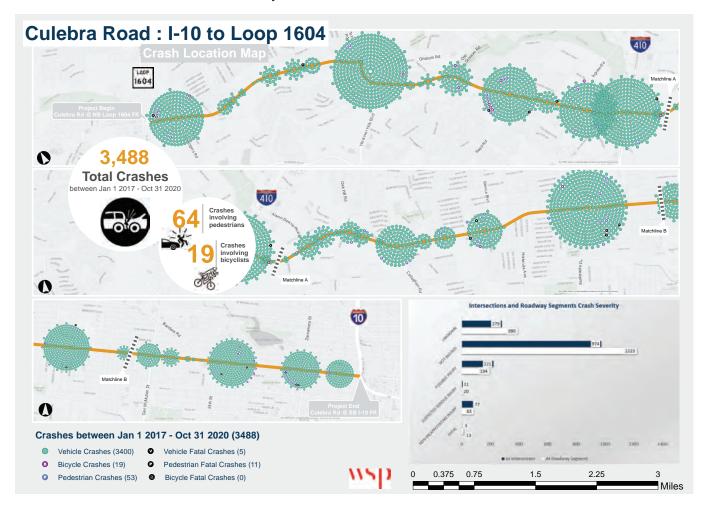
Right-of-way widths vary along the corridor from approximately 80 feet to 120 feet. Traffic volumes ranged from 22,000 to 51,000 Average Daily Traffic (ADT) along the corridor. The percentage of heavy vehicles (trucks) ranged from 1.4 percent to 3.7 percent. Posted speed limits ranged from 40-45 miles per hour (mph).

Because some of the traffic counts were collected during the COVID-19 pandemic when traffic volumes decreased, the Traffic Projections Methodology Memo in **APPENDIX D** includes a COVID-19 adjustment factor.



Overall, there were approximately 3,500 crashes along the corridor between January 2017 and October 2020. Pedestrians were involved in 64 of those crashes and bicyclists in 19. There were 16 fatal crashes, of which 11 were pedestrian fatalities. The other five fatalities were of persons traveling in a vehicle. **FIGURE 15** shows crash density for vehicle, pedestrian, and bicycle crashes.

FIGURE 15: Culebra Road Crash Density

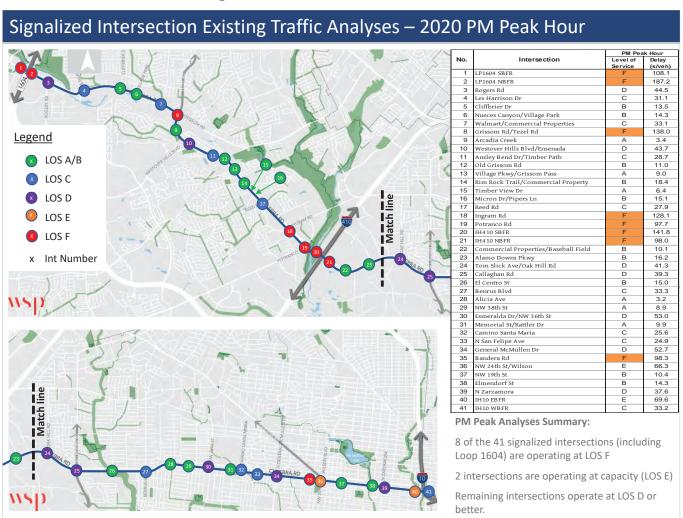




Synchro modeling provides a data-driven analysis of detailed congestion levels at intersections along the corridor. For vehicles, Level of Service (LOS) F indicates that traffic volumes exceed the roadway capacity and is measured in seconds of vehicle delay (sec/veh). LOS F is triggered when delay exceeds 80 seconds of vehicle delay at a signalized intersection. The LOS is measured in both the AM and PM peak hours when delay is anticipated to be at the highest point during a 24-hour day. In other words, LOS F is what the City deems as unacceptable and poor traffic flow for drivers.

Four of the five segments in the existing conditions model include signalized intersections currently experiencing LOS F. See **APPENDIX D** for more detail on the operational analysis.

FIGURE 16: Culebra Road Existing LOS



KEY FINDINGS

Data collection and input from agency stakeholders and the community have been instrumental in identifying four key findings for the Culebra Road Corridor.



KEY FINDINGS NO. 1

SAFETY

As noted earlier, safety is a concern in the corridor as seen by the data on crashes and fatalities as well as public input provided to the study team and through Vision Zero SA engagement. The community has specifically expressed concern about people driving too fast, distracted drivers, and difficulty crossing the street. Known as one of the deadliest roads in the City of San Antonio, a recent news article in the San Antonio Express-News notes that "more fatal vehicle crashes have occurred on Culebra than on any other road in San Antonio, excluding freeways." ³

FIGURE 17: Safety >>> Key Issues and Challenges



³ Pettaway, Taylor, Staff writer. "Like it's a speedway: The deadliest road in San Antonio has claimed more than 40 lives in the last decade". San Antonio Express-News, Sept. 12, 2022

PEDESTRIAN & BICYCLE

SIDEWALKS DISCONNECTED FROM ADJOINING BUSINES

Key findings for cyclists and pedestrians include the lack of sidewalks compliant with the Americans with Disabilities Act (ADA) design guidelines, discontinuous sidewalks, lack of sidewalks to access area businesses, narrow, unprotected bicycle lanes and non-existent bicycle lanes in the majority of the corridor.

FIGURE 18: Pedestrian and Bicycle >>> Key Issues and Challenges

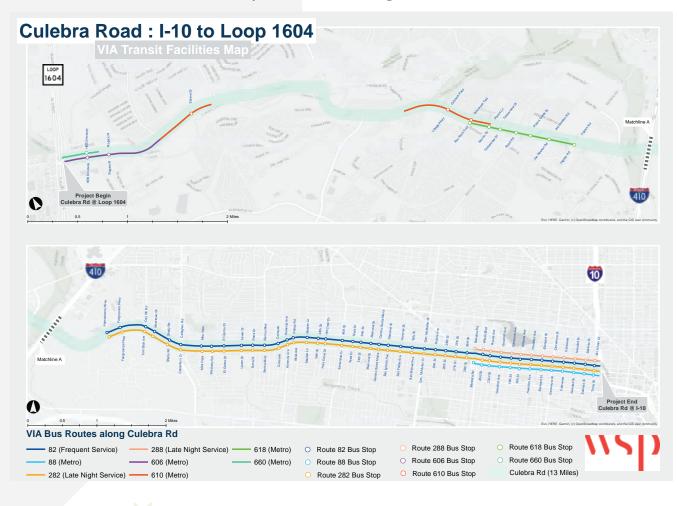






While VIA Metropolitan Transit serves the entirety of the corridor, discontinuous and non-ADA compliant sidewalks make access for the traveling public a challenge. Further, lack of shelter at transit stops can make long wait times cumbersome especially in the heat and rain.

FIGURE 19: Transit Facilities >>> Key Issues and Challenges



A key finding shows that intersections along Culebra Road within the study limits are dangerous and inefficient. Poorly marked sidewalks and skewed intersections make navigating the corridor difficult for users of all modes of transportation.

FIGURE 20: Intersections >>> Key Issues and Challenges



STRATEGIES & TOOLS Culebra Road Multimodal Corridor Transportation Study

STRATEGIES & TOOLS

Community Voices

"This roadway is one of the most dangerous and deadliest in our city and we should ensure that these improvements

"Safety is most important followed by improving the general environment through the corridor."









"All improvements are secondary to the safety issue. It is disheartening to see many crashes along Culebra Road. There is one intersection where there are four different roads coming in at different angles. Intersections like that need to be fixed. Please don't worry about beautification until the safety issue is handled."

"Roads are not just for cars! Provide safe paths for non-drivers."

OVERARCHING STRATEGIES

Six overarching strategies arose from our data analysis and public input review, and each can be implemented across the entire corridor.

FIGURE 21: Overarching Strategies

WIDER AND PROTECTED SIDEWALKS



IMPROVED CROSSWALK CONNECTIONS

IMPROVEMENTS TO TRANSIT FACILITIES AND THEIR LOCATIONS ALONG THE CORRIDOR

SHADE VIA TREES ALONG THE SIDEWALKS AS WELL AS THE MEDIANS

MULTI-PURPOSE TRAFFIC CALMING DEVICES



OVERARCHING STRATEGIES

Six overarching strategies arose from our data analysis and public input review, and each can be implemented across the entire corridor.

1 WIDER AND PROTECTED SIDEWALKS

Pedestrians and businesses thrive where sidewalks have been designed at an appropriate scale. Where a sidewalk is directly adjacent to moving traffic, a minimum 2-foot buffer from the curb is desired and wider buffers provide space for trees.



2 DEDICATED BICYCLE FACILITIES

These facilities would be protected from the moving traffic, and could include protected Class IV bicycle facilities, buffered bicycle lanes, and shared pedestrian / bicycle facilities.



IMPROVED CROSSWALK CONNECTIONS

These improvements could include making the crosswalk distance shorter, allowing for longer crosswalk timing, and providing a pedestrian refuge area on shaded mid-block crossings.

Where possible, improvements could include new crosswalks at key locations such as near existing schools, trailheads and other community destinations such as a grocery store.





NEX I SI EP

IMPROVEMENTS TO TRANSIT FACILITIES AND THEIR LOCATIONS ALONG THE CORRIDOR

Transit stops placed between the curb and facilities for pedestrians and bikes reduce conflict points, and improve safety and user experience.



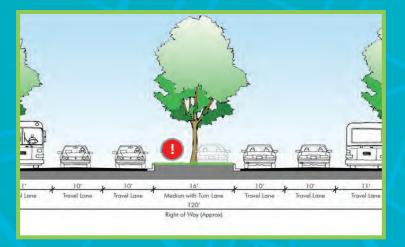
5 SHADE VIA TREES ALONG THE SIDEWALKS AS WELL AS THE MEDIANS

Street trees enhance city streets both functionally and aesthetically. Trees provide shade to homes, businesses, and pedestrians. Street trees frame the street and the sidewalk as discrete public realms, enriching each with a sense of rhythm and human scale. Trees may also provide a traffic calming effect.



MULTI-PURPOSE TRAFFIC CALMING DEVICES

These improvements could include medians, trees, reduced traveling widths, and better access management for vehicular traffic.



MULTIMODAL TOOLBOX

This toolbox demonstrates how we can achieve equity along the corridor by implementing the elements listed in this section. The elements within this toolbox should be appropriately implemented along the corridor based on the proposed concepts for Segments A to E as outlined later in this report.

FIGURE 22: Multimodal Toolbox Elements





wider and protected sidewalks along the corridor is crucial to create a corridor for all users. In all proposed concepts for the Segments along Culebra Road there are sidewalks that range from a minimum of five (5) feet wide in the most constrained right-of-way scenarios and up to ten (10) feet wide in Segments with wider right-of-way dedication. It is recommended to have a sidewalk at least six (6) feet wide where possible to provide sufficient space for two people to walk side by side comfortably. In addition, it is best practice to provide a planter buffer between the sidewalk and roadway to create a safe and comfortable environment for pedestrians. The planter buffer provides protection and a safe distance from the fast-moving traffic on Culebra Road, while also providing shade and a cooler environment for the pedestrian. Sidewalk spaces shall contain amenities such as lighting as well as trash and recycle bins. Maintenance considerations need to be discussed with the City during project development.





DEDICATED BICYCLE FACILITIES: Dedicated bicycle facilities include protected single-way bicycle lanes or two-way bicycle lanes along the roadway. Bicycle facilities are safest and most comfortable when they are protected or buffered from the roadway, which can be achieved by using planters or bollards for separation. At minimum, a single-way bicycle lane should be a minimum of five (5) feet wide to provide sufficient space for the user. Thus, two-way bicycle lanes shall be a minimum of ten (10) feet wide. It is also essential to provide directional arrows and markings to indicate that the dedicated bicycle facility space is reserved for bicyclists only.



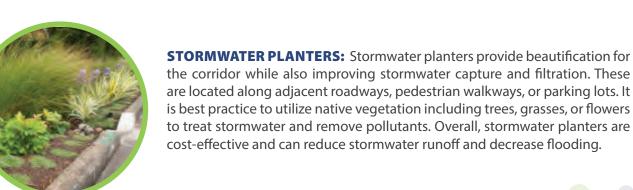
SHARED USE PATH: A shared use path is a space that is intended to be shared by both pedestrians and bicyclists. It is recommended that these spaces be a minimum of twelve (12) feet wide. These spaces can include amenities such as lighting as well as trash and recycle bins.



IMPROVED CROSSWALK CONNECTIONS: Improved crosswalk connections are an essential component to increased safety throughout the corridor. Crosswalks should be placed at all signalized intersections and must include markings. Markings must be clearly visible for all users, including fast-moving vehicles. Where possible, mid-block crosswalks should be provided when distances between existing crosswalks are too long and crossing distances at intersections should be narrowed through the use of curb extensions or bulbouts.



SHADE TREES: Shade trees shall be provided as often as possible to create shade for bicyclists and pedestrians, provide cooler temperature, and establish a more pleasant aesthetic for the corridor. It is essential that these shade trees be amply spaced to allow the tree roots ample room to reach full maturity. It is recommended to use White Crepe Myrtle, Mexican Sycamore, Cedar Oak, Monterrey Oak, Red Bud, or Chinkapin Oak in planters or stormwater drainage areas along the corridor. To avoid uprooting of adjacent sidewalk space or roadway, the best practice is to use structured soil under pavement and within the planter space. Shade trees also have the added benefit of helping calm traffic.





CORRIDOR WAYFINDING: Corridor wayfinding is crucial to providing identity and character to the corridor. Wayfinding consists of providing signage with corridor specific branding to direct users to key destinations and landmarks along Culebra Road, including the Basilica of the National Shrine of the Little Flower, St. Mary's University, or Culebra Creek.

TOOLBOX ELEMENTS - APPLICABLE BASED ON THE CORRIDOR SEGMENT

		SEGMENTS					
		A	В	C1	C2	D	E
ELEMENTS	Wider and Protected Sidewalks		•			•	
	Dedicated Bicycle Facilities					•	
	Shared Use Path						
	Improved Crosswalk Connections		•	•		•	
	Shade Trees					•	
	Stormwater Planters			•			
	Corridor Wayfinding		•	•		•	



IMPLEMENTATION Culebra Road Multimodal Corridor Transportation Study

IMPLEMENTATION

OVERVIEW

If implemented successfully, the concepts and strategies detailed in this report will contribute significantly to achieving the community's vision for Culebra Road. Each concept includes existing conditions, highlights, strengths, and limitations.

ALTERNATIVES ANALYSIS

As the character of Culebra Road changes throughout the corridor, there cannot be a blanket design solution or one-size fits all approach that will solve the corridor's issues and challenges.

There are **five (5) distinct corridor segments** identified throughout the entire project area and overarching improvements were individually recommended for each segment. The customized solution for each Segment was created in response to surrounding context such as land use, nearby key destinations, drainage, traffic volume, and existing street configuration. As noted earlier, the common improvements for these segments include, providing wider and protected sidewalks; dedicated bike facilities; improved crosswalk connections; shade trees; and multi-purpose traffic calming devices.

The best practices recommended in the proposed concepts for each segment build on City of San Antonio and NACTO practices and regulations. Collectively, these proposed improvements in the proposed concepts provide safety and connectivity for all modes of travel by re-purposing any excess right-of-way to better serve people who walk, bike, and take transit.

The proposed improvements for the Culebra Road Corridor are summarized and discussed in the following pages. Using the community's vision, overarching strategies, multimodal improvements toolbox, and feedback from the Technical Advisory Group, the study team developed maps, representative cross sections, and conceptual intersection layouts to illustrate two proposed concepts per segment throughout the corridor. Improvements are divided in five segments from north to south as described in the "Corridor Overview."

EXISTING CONDITIONS

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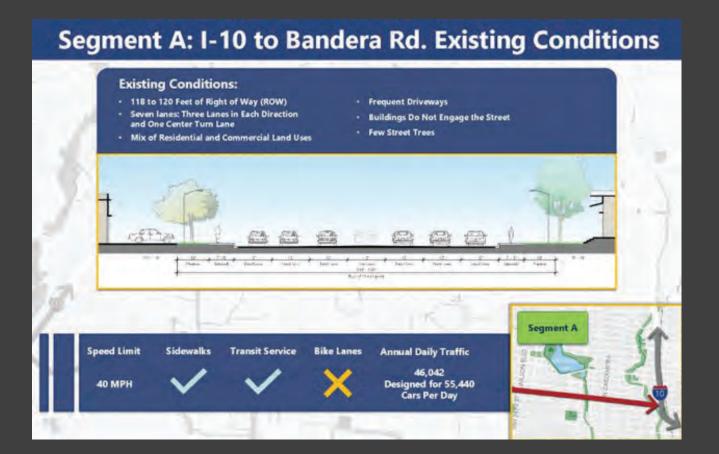
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STRATEGIES

FIGURE 23: Segment A - Existing Conditions



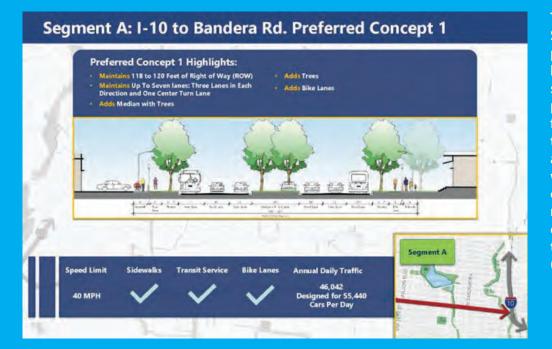
SEGMENT A

Segment A looks at Culebra Road from I-10 to Bandera Road. It is about 120 feet wide and has three lanes of traffic in each direction with a turn lane in the middle. There is a mix of residential and commercial users with multiple driveways and very few trees. There are no bicycle lanes in this segment. This segment falls within TxDOT's jurisdiction.

Α

7,963 feet (1.5 miles)

FIGURE 24: Segment A - Proposed Improvements Concept 1



The first concept for Segment A would include protected sidewalks and bicycle lanes on both sides of the street. These paths would be protected from moving traffic with tree lined planters. We also propose a median with trees in the middle of the street. This concept maintains the number of travel lanes within the existing right of way (ROW).

STRENGTHS

- Maintains the existing ROW
- · Maintains the same number of through lanes
- Adds a median with opportunities for trees and landscaping
- Adds dedicated and protected accommodations for both bicycles and pedestrians on both sides of the road

- Maintenance requirements for grass and trees in median and along ROW
- Median impacts adjacent property access
- Sidewalks remain at 6 feet when compared to Segment A Concept 2
- Possible utility conflicts at ROW
- · Variable ROW width
- Roadway centerline is not consistently in center of ROW



FIGURE 25: Segment A - Proposed Improvements Concept 2



The second concept for Segment A reduces the number of travel lanes from three lanes in each direction to two lanes in each direction. This will allow more space for folks to walk and bicycle along the street and will provide further opportunities for the shade and protection provided by new tree plantings.

STRENGTHS

- Maintains the existing ROW
- Adds a median with opportunities for trees and landscaping
- Adds dedicated and protected bicycle lanes on both sides of the road
- Widens sidewalks on both sides of the road from six feet to 10 feet

- Would require reducing the number of travel lanes in each direction from three to two
- Maintenance requirements for grass and trees in median and along ROW
- Median impacts adjacent property access
- · Possible utility conflicts at ROW
- Variable ROW width
- Roadway centerline is not consistently in center of ROW



EXISTING CONDITIONS

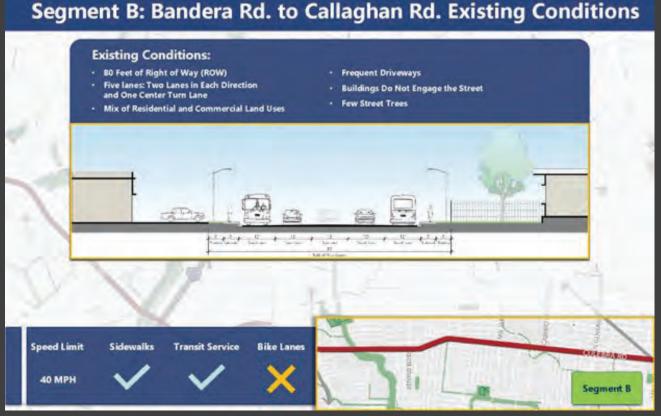
FIGURE 26: Segment B - Existing Conditions

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SEGMENT B

Segment B looks at Culebra Road from Bandera Road to Callaghan Road. It's a much narrower road – about 80 feet wide, and it has two lanes of traffic in each direction with one center turn lane. There is a mix of residential and commercial users and very few trees. There are no bicycle lanes in this segment.

В

Total: 20,033 feet (3.7 miles)

Segment B1: 9,195 feet(1.7 miles) + SegmentB2:10,838 feet (2 miles)

FIGURE 27: Segment B - Proposed Improvements Concept 1

Segment B: Bandera Rd. to Callaghan Rd. Preferred Concept 1



The first concept for Segment B would add a shared use path on one side of the street, which allows pedestrians and cyclists to move comfortably protected with a tree line buffer. On the other side of the street, we propose a sidewalk.

STRENGTHS

- Maintains the existing ROW
- Maintains the same number of through lanes
- Adds a median with opportunities for trees and landscaping
- Adds dedicated and protected shared-use path for both bicycles and pedestrians on one side of road
- Add a dedicated and protected sidewalk to the other side of the road

- · Does not add a dedicated and protected bicycle lane
- Maintenance requirements for grass and trees in median and along ROW
- Median impacts adjacent property access
- Possible utility conflicts at ROW
- · Variable ROW width
- Roadway centerline is not consistently in center of ROW



FIGURE 28: Segment B - Proposed Improvements Concept 2

Segment B: Bandera Rd. to Callaghan Rd. Preferred Concept 2



The second concept for Segment B would include protected sidewalks and bicycle lanes on both sides of the street. These paths would be protected from moving traffic with tree line planters. It has also been proposed to add a median with trees in the middle of the street. This number of travel lanes within the existing ROW.

STRENGTHS

- Maintains the existing ROW
- Maintains the same number of through lanes
- Adds a median with opportunities for trees and landscaping
- Adds dedicated and protected accommodations for both bicycles and pedestrians on both sides of the road

- · Maintenance requirements for grass and trees in median and along ROW
- Median impacts adjacent property access
- Possible utility conflicts at ROW
- Variable ROW width
- Roadway centerline is not consistently in center of ROW



EXISTING INDITONS

FIGURE 29: Segment C1 - Existing Conditions

Segment C1: Callaghan Rd. to Avenue G Existing Conditions



SEGMENT C1

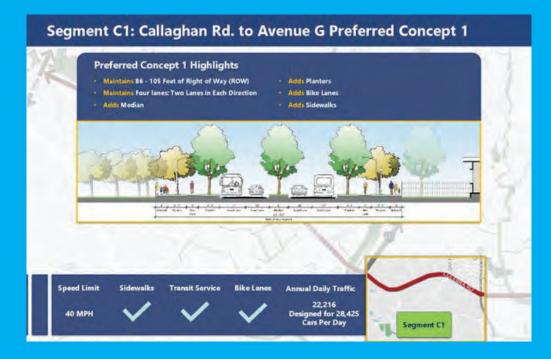
Segment C1 looks at Culebra Road from Callaghan Road to Avenue G. It varies in ROW, going from about 85 feet wide to about 100 feet wide. It has two lanes of traffic in each direction. There are not a lot of sidewalks in this area and several blocks of the road have homes that are located very close to the street.

Note: the study team split Segment C into two different segments, noted as C1 and C2, as they had differences necessitating separate review.

9,585 feet (1.8 miles)

46

FIGURE 30: Segment C1 - Proposed Improvements Concept 1



The first concept for Segment C1 adds a tree line median and provides a tree line buffer space to protect folks walking and cycling on both sides of the street. This concept maintains the number of travel lanes within the existing ROW.

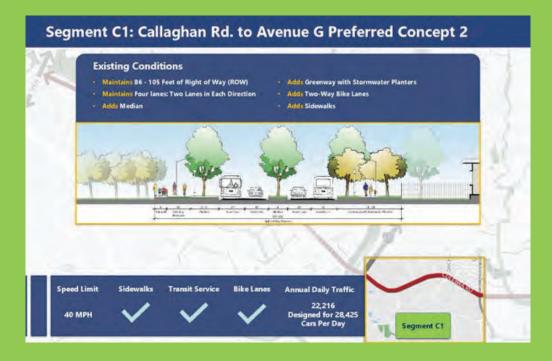
STRENGTHS

- Maintains the existing ROW
- Maintains the same number of through lanes
- Adds a median with opportunities for trees and landscaping
- Adds dedicated and protected accommodations for both bicycles and pedestrians on both sides of the road

- Maintenance requirements for grass and trees in median and along ROW
- Median impacts adjacent property access
- · Variable ROW width
- Roadway centerline is not consistently in center of ROW



FIGURE 31: Segment C1 - Proposed Improvements Concept 2



The second concept for Segment C1 provides for a sidewalk and two-way bicycle lanes on one side of the street. On the other side of the street, we propose a greenway with stormwater planters – this is a design element that we clearly heard that the community would like to see in this area.

STRENGTHS

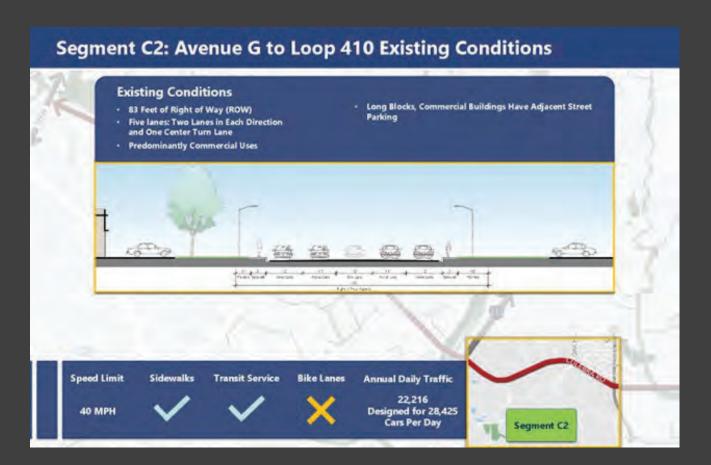
- Maintains the existing ROW
- · Maintains the same number of through lanes
- Adds a median with opportunities for trees and landscaping
- Adds dedicated and protected accommodations for two-way bicycle lanes on one side of the road
- Adds dedicated and protected sidewalks on both side of the road
- Add a greenway with stormwater planters on one side of the road

- Maintenance requirements for grass and trees in median and along ROW
- Median impacts adjacent property access
- · Variable ROW width
- Roadway centerline is not consistently in center of ROW



EXISTING CONDITIONS

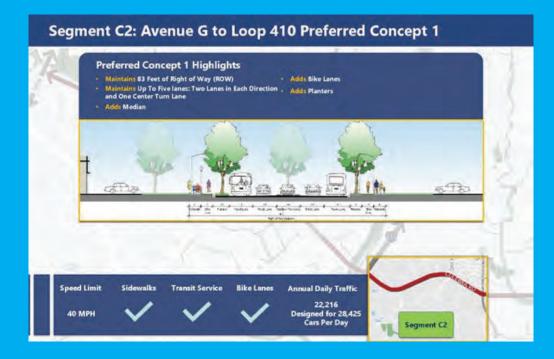
FIGURE 32: Segment C2 - Existing Conditions



Segment C2 looks at Culebra Road from Avenue G to Loop 410. It has about 80 feet of ROW with two lanes of traffic in each direction. It is predominately commercial and there are narrow sidewalks. There are no bicycle lanes in this segment.

49

FIGURE 33: Segment C2 - Proposed Improvements Concept 1



The first concept for Segment C2 adds a tree line median to help calm traffic and provides a tree line buffer space to protect folks walking and cycling on both sides of the street. This concept maintains the number of travel lanes within the existing ROW.

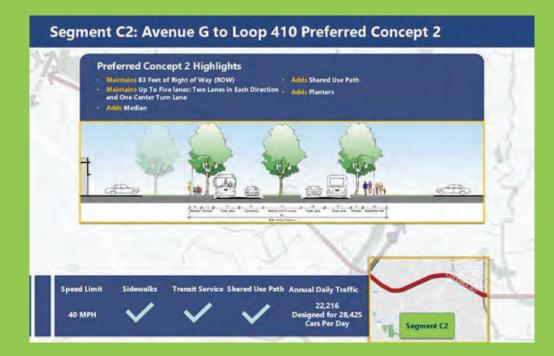
STRENGTHS

- Maintains the existing ROW
- Maintains the same number of through lanes
- Adds a median with opportunities for trees and landscaping
- Adds dedicated and protected accommodations for both bicycles and pedestrians on both sides of the road

- Maintenance requirements for grass and trees in median and along ROW
- Median impacts adjacent property access
- · Possible utility conflicts at ROW
- Variable ROW width
- Roadway centerline is not consistently in center of ROW



FIGURE 34: Segment C2 - Proposed Improvements Concept 2



The second concept for Segment C2 provides a shared use path for pedestrians and cyclists on one side of the street and a protected sidewalk on the other side of the street.

STRENGTHS

- Maintains the existing ROW
- Maintains the same number of through lanes
- Adds a median with opportunities for trees and landscaping
- Adds dedicated and protected shared-use path for both bicycles and pedestrians on one side of the road
- Add a dedicated and protected sidewalk on the other side of the road

- · Does not include a dedicated bicycle lane
- Maintenance requirements for grass and trees in median and along ROW
- Median impacts adjacent property access
- Possible utility conflicts at ROW
- · Variable ROW width
- Roadway centerline is not consistently in center of ROW



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FIGURE 35: Segment D - Existing Conditions



SEGMENT D

Segment D looks at Culebra Road from Loop 410 to Tezel Road. Currently, it is about 120 feet wide with three lanes of traffic in each direction and a center turn lane. This segment sees a mix of residential and commercial uses. While we do have sidewalks and bicycle lanes on both sides of the street, they are not buffered from the moving traffic. We heard from the community that folks do not feel comfortable walking or using their bicycle in this segment.

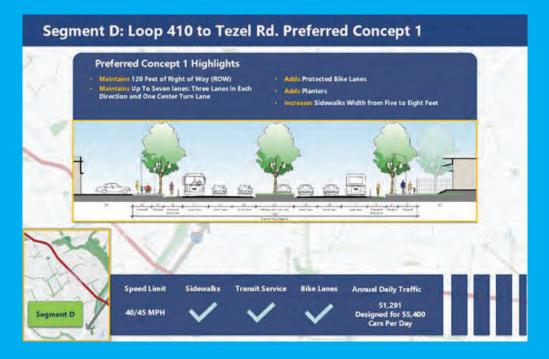
17,060 feet (3.2 miles)

D

52

Culebra Road Multimodal Corridor Transportation Study

FIGURE 36: Segment D - Proposed Improvements Concept 1



The first concept for Segment D provides for both bicycle and pedestrian paths on both sides of the street as well as a tree line median in the middle that provides for access management and helps calm traffic.

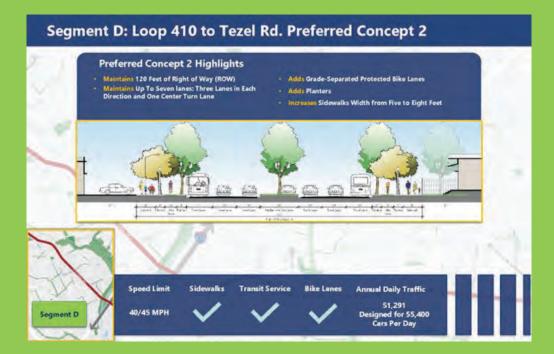
STRENGTHS

- Maintains the existing ROW
- Maintains the same number of through lanes
- Adds a median with opportunities for trees and landscaping
- Add protection to existing sidewalks and bicycle lanes on both sides of the road
- Increases existing sidewalk widths from five to eight feet

- Maintenance requirements for grass and trees in median and along ROW
- Median impacts adjacent property access
- Possible utility conflicts at ROW
- Variable ROW width
- Roadway centerline is not consistently in center of ROW



FIGURE 37: Segment D - Proposed Improvements Concept 2



The second concept for Segment D adds the buffer of a planted tree line between the bicycle and pedestrian paths on both sides of the street.

STRENGTHS

- Maintains the existing ROW
- Maintains the same number of through lanes
- Adds a median with opportunities for trees and landscaping
- Adds grade-separated protection to existing sidewalks and bicycle lanes on both sides of the road
- · Increases existing sidewalk widths from five to eight feet

- Maintenance requirements for grass and trees in median and along ROW
- Median impacts adjacent property access
- Possible utility conflicts at ROW
- · Variable ROW width
- Roadway centerline is not consistently in center of ROW



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FIGURE 38: Segment E - Existing Conditions



SEGMENT E

Segment E looks at Culebra Road from Tezel Road to Loop 1604. It is about 120 feet wide with two lanes of traffic in each direction. This segment sees a mix of residential and commercial uses. There are narrow sidewalks and no bicycle paths. We clearly heard from the community that all concepts for this segment allow for protected bicycle and pedestrians paths in a greenway setting.

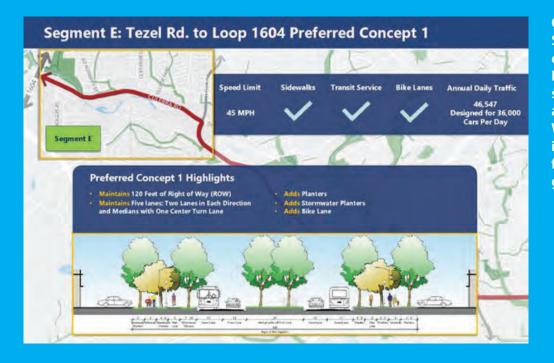
E

12,927 feet (2.5 miles)

55

Culebra Road Multimodal Corridor Transportation Study

FIGURE 39: Segment E - Proposed Improvements Concept 1



Our first concept for Segment E directly reflects community feedback. Folks can walk and bicycle safely and comfortably along tree lined corridors. We also propose a median in the middle that allows for two rows of trees to grow.

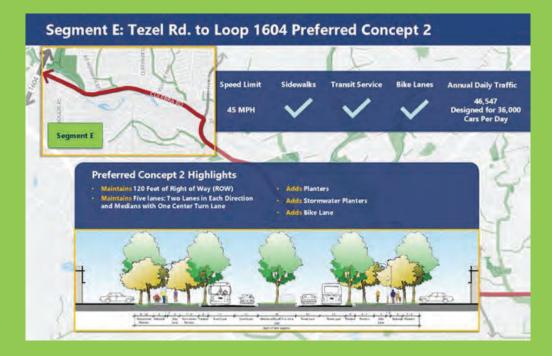
STRENGTHS

- Maintains the existing ROW
- · Maintains the same number of through lanes
- Adds a median with opportunities for two rows of trees and landscaping
- Adds dedicated and protected bicycle lanes on both sides of the road
- · Maintains sidewalks on both sides of the road

- Maintenance requirements for grass and trees in median and along ROW
- Median impacts adjacent property access
- · Possible utility conflicts at ROW
- Variable ROW width
- Roadway centerline is not consistently in center of ROW



FIGURE 40: Segment E - Proposed Improvements Concept 2



second for Segment E is a little different as we narrowed the proposed median to allow for more space for pedestrians and cyclists on both sides of the street.

STRENGTHS

- Maintains the existing ROW
- Maintains the same number of through lanes
- Adds a median with opportunities for trees and landscaping
- Adds stormwater planters to both sides of the road to increase buffer protection for cyclists and pedestrians
- Adds dedicated and protected bicycle lanes on both sides of the road
- Maintains sidewalks on both sides of the road

- · Maintenance requirements for grass and trees in median and along ROW
- · Median impacts adjacent property access
- Possible utility conflicts at ROW
- Variable ROW width
- Roadway centerline is not consistently in center of ROW



PREFERRED CONCEPTS

Feedback and comments from the October 2022 engagement effort resulted in overall support for proposed improvements, and no significant comments pulling in one way or another on the two proposed concepts for each segment. The community made it clear throughout the study process that their number one priority for the corridor was to improve safety. All of the concepts achieve that goal through the use of various design elements. These elements focus around reducing the time pedestrians spend crossing the street and creating physical separation between cars and other corridor users.

The top five favored elements

- (1) Adding trees to the corridor
- (2) Adding separated/protected pedestrian and bicycle paths
- (3) Adding frequent and safer pedestrian/bicycle crosswalks
- (4) Adding street lighting across all segments
- (5) Adding safety measures as safety was noted by many as a major need.

In comparison, there were a very small number of comments in opposition, and they centered on concerns about reducing the number of travel lanes. The following core design principles were used to develop all the concepts:

Travel Lane Width Reduction

Reducing travel lane widths reduces the crosswalk length and which in turn reduces the time it takes for pedestrians to cross the street. Implementing 10-foot travel lanes also serves to calm traffic which helps compound the safety benefit.

Protected Bicycle Lanes

Implementing protected bicycle lanes separates cars from cyclists, and also increases the separation distance between travel lanes and pedestrians.

Adding Trees

Incorporation of trees along the corridor allows for shade protection for pedestrians.

Therefore, as a result of the study process, data analysis, and public input, the team recommends the following as the preferred concept for each segment:

SEGMENT A

Concept 1

SEGMENT B

Concept 1

from Bandera

to 36th Street

SEGMENT B

Concept 2

from 36th St. to Callaghan Road

SEGMENT C1

Concept 2

SEGMENT C2

Concept 1

SEGMENT D

Concept 2

SEGMENT E

Concept 2

A preliminary design, also known as a schematic, was prepared for Segments A and B as this stretch of the corridor has potential funding through the 2022 Bond Program. This layout included the preferred concepts as mentioned above. In addition, renderings of an example intersection for the concepts are shown in **FIGURES 41-43** on the following page. In a future final design plan phase, these preferred concepts would be refined further to ensure an equitable, implementable design.

FIGURE 41: Segment A Preferred Concept 1 - Intersection

Segment A: I-10 to Bandera Road



FIGURE 42: Segment B Preferred Concept 1 - Intersection

Segment B: Bandera Road to Callaghan Road



FIGURE 43: Segment B Preferred Concept 2 - Intersection

Segment B: Bandera Road to Callaghan Road



FUNDING

This study has documented several reasons to prioritize funding for roadway improvements outside of capacity expansion, including community's desire to mitigate existing safety and accessibility concerns.

A combination of local, state, and federal funding sources will be necessary to fully implement the proposed improvements recommended in the Culebra Road Multimodal Corridor Transportation Study. As of the report's writing, funding has not been allocated to support all of the improvements recommended.

PRELIMINARY COST ESTIMATES

Cost estimates for the Culebra Road Multimodal Corridor Transportation Study were developed based on a combination of knowledge of prior improvement design for the urban areas in Central Texas and materials provided by the City of San Antonio. A cost range for the segments is included below to account for variable cost of certain items proposed, such as dedicated bicycle lanes, intersection improvements, sidewalks, and bridge crossings. The estimate took the 12-month average TxDOT bid prices for the San Antonio District up to September 2021 (12-consecutive month averages) and accounted for the main bulk of items that would go into these improvements.

All items considered preliminary engineering costs such as traffic control/impact to traffic, safety fencing, erosion and sedimentation control. Due to the concept-level of the improvements and unknown variables such as design material choices, a contingency was applied for the total estimated cost. Right-of-way costs were not included.

CORRIDOR SEGMENT	ESTIMATED COST
A: I-10 to Bandera Road	\$20,000,000
B: Bandera Road to Callaghan Road	\$34,000,000
C-1: Callaghan Road to Avenue G	\$10,000,000
C-2: Avenue G to Loop 410	\$5,000,000
D: Loop 410 to Tezel Road	\$36,000,000
E: Tezel Road to Loop 1604	\$24,000,000
Total Estimated Cost (includes planning, design, and utility coordination)	\$129,000,000

Dollar amounts represent general estimates of the level of effort required to design and construct the improvements; costs are likely to change after survey and detailed design.

POTENTIAL AVAILABLE FUNDING AND PRIORITIZATION

The City's Public Works Department has a proposed project in the 2022 Bond Program that considers improvements to Culebra Road from I-10 to Callaghan Road. This stretch of the corridor is noted in this study as Segments A and B. The project budget is \$18 million and the recommendations from this study will help inform the proposed project improvements. With the \$18 million budget, we recommend completing the detailed design for Segment B and constructing a first phase of Segment B between Bandera Road to 36th Street while pursuing additional funding for the next phase between 36th Street and Callaghan Road. Further discussions with TxDOT are required to determine an implementation approach for Segment A.

The table below summarizes the implementation priority for Segments A and B.

PRIORITY	CORRIDOR SEGMENT	ESTIMATED COST
1	B: Bandera Road to 36th Street	\$15,500,000
2	B: 36th Street to Callaghan Road	\$18,500,000
3	A: I-10 to Bandera Road	\$20,000,000

In addition to the 2022 Bond Program, in Fall 2021, TxDOT awarded the City with \$3.2M for safety improvements along Culebra Road from Shelly Street to NW 28th Street via their Highway Safety Improvement Program (HSIP).

This stretch of the corridor includes all of Segment B. The FY2022 budget also included \$5.2 million as part of the City's Vision Zero program towards the design and construction of 28 mid-block crossings on 8 corridors including Culebra Road. Discussion between the Public Works Department, Transportation Department, and stakeholders will continue to determine how best to leverage these multiple funding sources to fulfill the community's vision documented in the Culebra Road Multimodal Corridor Transportation Study.



NEXT STEPS
Culebra Road Multi Multimodal Transportation Study

NEXT STEPS

The Culebra Road Multimodal Corridor Transportation Study concluded in preferred concepts, visualizations, and preliminary designs as a result of the exploration of existing conditions, challenges and opportunities, and public input. The City will use these conclusions to create and construct an implementable design.

As Culebra Road from I-10 to Callaghan Road (Segments A and B) is part of the 2022 Bond Program, the City will develop final design plans for that stretch of the corridor, as appropriate, as the needed next step before any potential construction.

The schedule for any future construction is still to be determined. Currently there are no immediate next steps for Segments C through E; however, this study will serve as a foundation for pursuing future local, state, or federal funding opportunities.

This study will serve as a foundation for pursuing future local, state, or federal funding opportunities.





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TRATEGIES & TOOLS

APPENDIX A-1

Multimodal Corridor Transportation Study

Appendix A-1: Culebra Road Transportation Study Community Engagement Summary & Emerging Concepts (August 2021)

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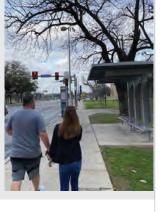














Culebra Road Transportation Study Community Engagement Summary & Emerging Concepts August 2021

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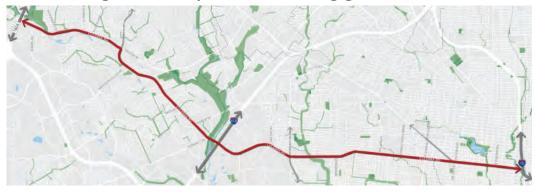
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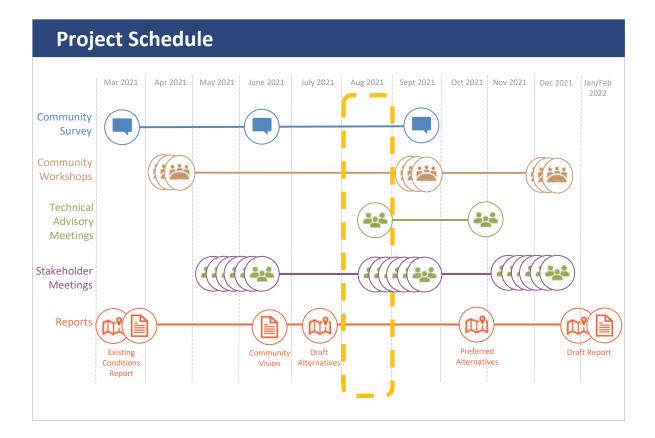
I. PROJECT BACKGROUND

Project Goals

- Create a safe corridor for ALL users of the street
- Transform Culebra into an equitable, multimodal corridor for people who walk, roll, bike, take transit and drive
- Enhance overall experience and connectivity for pedestrians, bicyclists & transit users
- Develop conceptual designs for short- and long-term improvements
- Coordinate and build upon previous and concurrent studies and initiatives
- Ensure meaningful community and stakeholder engagement



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II. COMMUNITY ENGAGEMENT PROCESS & SUMMARY

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Overall Community Visioning Engagement Process

- Multi-Pronged Community Engagement Included:
 - 3-month long in-person and virtual Community Visioning Survey
 - 5 Focus Group Stakeholder Meetings with neighborhood associations, pedestrian and bike advocacy groups and institutions
 - 3 Community Workshops in English and Spanish
 - NEARLY 600 ACTIVE PARTICIPANTS who attended stakeholder and community meetings and took the community survey
 - OVER 2000 CLICKS to multi-media outreach including email blasts, social media posts, newspaper and social media ads, media coverage and project website

Overall Community Visioning Outreach

Multifaceted Community Outreach Efforts advertised the community goals, Included:

- Postcards: Nearly 800 bilingual postcards mailed to adjacent property owners and stakeholders

 Stakeholders
- Newspaper Ads: 3 newspaper ads to advertise the project upcoming
 - La Prensa Texas (in Spanish) April 11th, 2021
 - San Antonio Express- News (in English) April 11th and April 13th, 2021



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Overall Community Visioning Outreach

Outreach Efforts Included:

- Email Blasts: 2 bilingual email with over 250 recipients
- Media Coverage: KSAT 12 website and Telemundo interview
- Social Media Posts: 22 social media posts that were shared or retweeted over 50 times on twitter and facebook
- Social Media Ads: 5 ads that were clicked more than 4000 time
- Culebra Workshop Website: https://culebraroadworkshops.org/



Community Survey

- Bi-Lingual English and Spanish
- March 12 to June 14th, 2021
 - Over 540 surveys
 - Over 2480 map responses
- Digital survey Multi-media and location based
- Hard copy survey: In-person and via mail
- Locations for pick up and drop

Memorial Library 3222 Culebra Rd. San Antonio, TX 78228

Great Northwest Library 9050 Wellwood St. 78250

Council District Offices



https://culebraroadworkshops.org/

Focus Group Stakeholder Meetings & Community Workshops



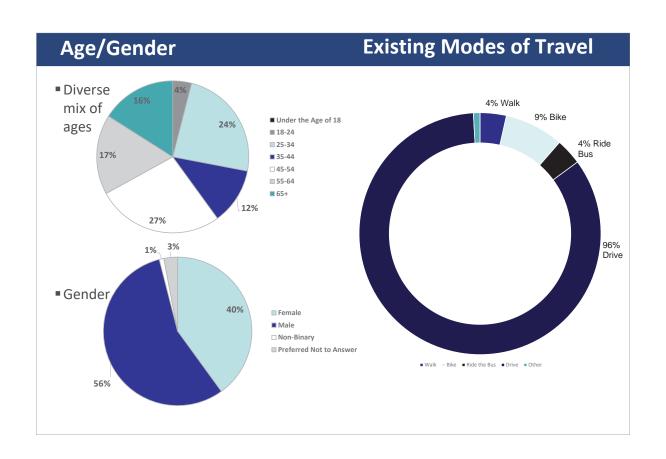
FOCUS GROUP STAKEHOLDER MEETINGS

- May 26th to June 8th, 2021
- Neighborhood Associations: West End Hope in Action, NW Crossing, Timber Ridge, Thunderbird Hills, Culebra Park, Great Northwest and University Park
- Bicycle & Pedestrian Advocacy Groups: Activate SA, AAMPO, Bike San Antonio, South Texas Off Road Mountains (STORM), and Earn a Bike

COMMUNITY WORKSHOPS (English and Spanish)

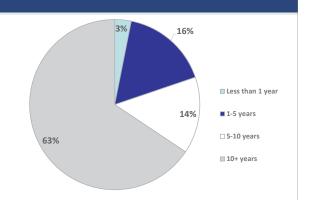
- Afternoon workshop on April 20, 2021
- Evening workshops on April 20 and April 22, 2021



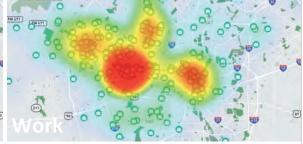


Live/Work

- Nearly 2/3rd of the respondents have lived in the area for more than 10 years
- Respondents from all key neighborhoods along the corridor
- Respondent represent small and large places of work including St Mary's University and Southwest Research Institute



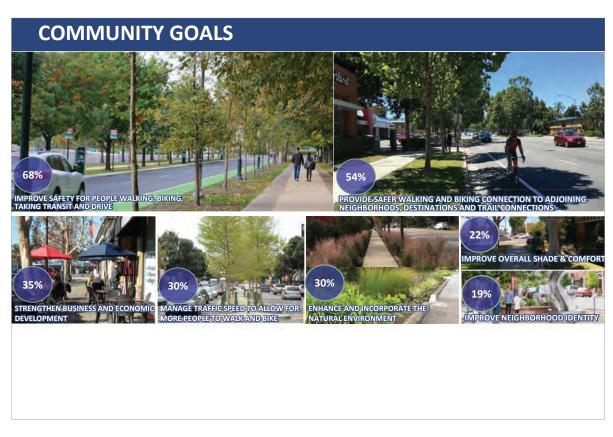






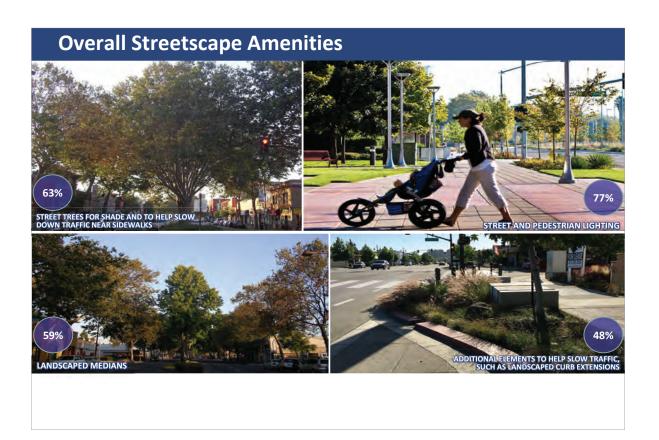
III. COMMUNITY VISION







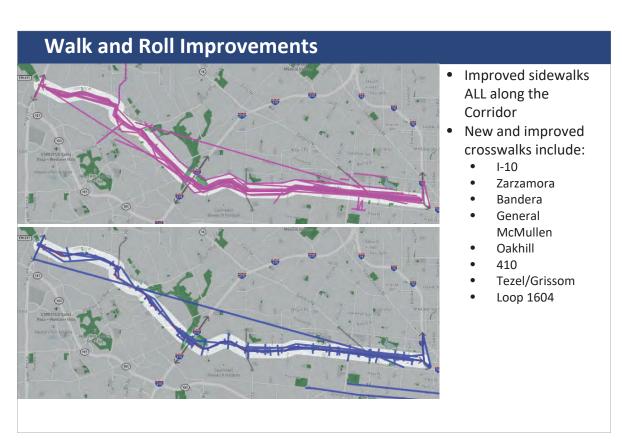
IV. OVERARCHING COMMUNITY STREETSCAPE FRAMEWORK



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Bike & Micromobility Improvements

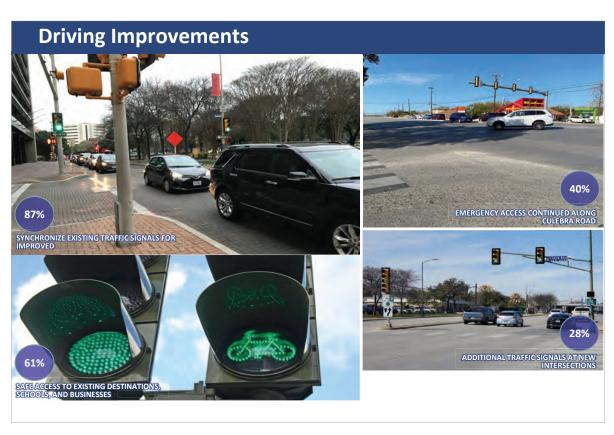




Bike & Micromobility Improvements: Type of Bike Facilities







Economic Development Improvements



Natural Environment & Community Identity Improvements



Other Community Feedback

BARRIERS

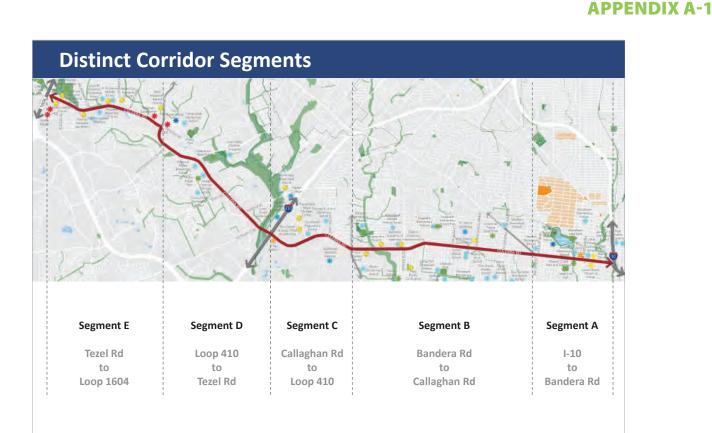
- "Lanes are too wide and incentivize dangerous driving"
- "Cars do not respect pedestrians or yield to bicyclists"
- "The sidewalks are right on the edge of the street.... I always think I am about to die when I walk on them"
- "Too many business signs makes it difficult to focus while driving"
- "Congestion at Culebra and 1604 is a nightmare"

OPPORTUNITIES

- "Reduce the lane widths to slow people down"
- "Prioritize pedestrian safety!"
- "You better build a protected bike lane!"
- "We need better illumination to see people crossing"
- "Add bioswales and LID features"
- "Some urban farming to share with the community"



V. INDIVIDUAL SEGMENT COMMENT SUMMARY & EMERGING CONCEPTS



Segment A: I-10 to Bandera Road

COMMUNITY FEEDBACK:

- 73% support wider and protected sidewalks for people
- 78% support wider sidewalks for business
- 72% support safe bike facilities for all ages with over 85% wanting protected, dedicated bike facilities
- Strong desire to improving existing crosswalks all along this segment, especially at I-10 interchange, Zarzamora, Hamilton and Wilson/Bandera; new pedestrian and bicycle access to Alazan Creek Trail and mid-block crosswalk between 19th and 24th
- Strong support for improved transit facilities all along egment B
 the corridor segment especially at Zarzamora and
 Bandera intersections and Alazan Creek Trail



Segment A

I-10 to Bandera Rd

Segment A: I-10 to Bandera Road Concept 1 Concept 2

Segment B: Bandera Road to Callaghan Road

COMMUNITY FEEDBACK

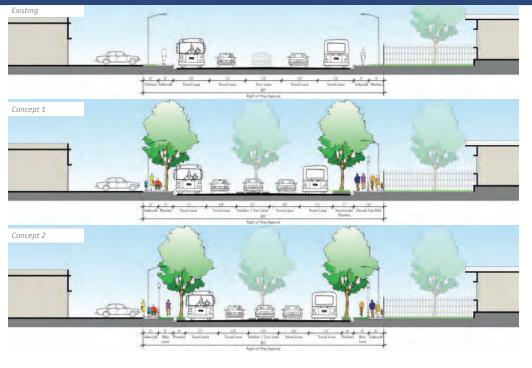
- 76% support wider sidewalks for people walking; 65% support wider sidewalks for business
- 64% support safe bike facilities for all ages; 76% want protected, dedicated bike facilities
- Strong desire for improving crosswalks at existing intersections including Culebra at 36th St/Esmeralda, General McMullen, Benrus and Callaghan; newand more frequent crosswalk opportunities all along the segment including areas between Maiden Lane and Yolanda, and between Callaghan and Mira.
- Strong desire for enhanced pedestrian, bicycle and micromobility connections between St Mary's University and surrounding neighborhoods including Woodlawn Heights and Woodlawn Lake
- Support enhanced transit facilities to St Mary's University



Segment B

Bandera Rd Callaghan Rd

Segment B: Bandera Road to Callaghan Road



Segment C: Callaghan Road to Loop 410

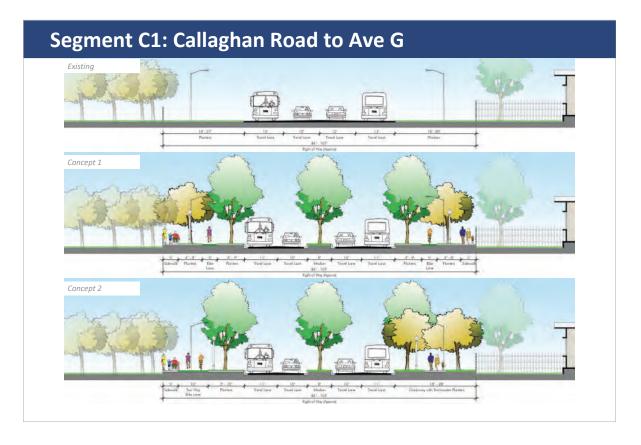
COMMUNITY FEEDBACK

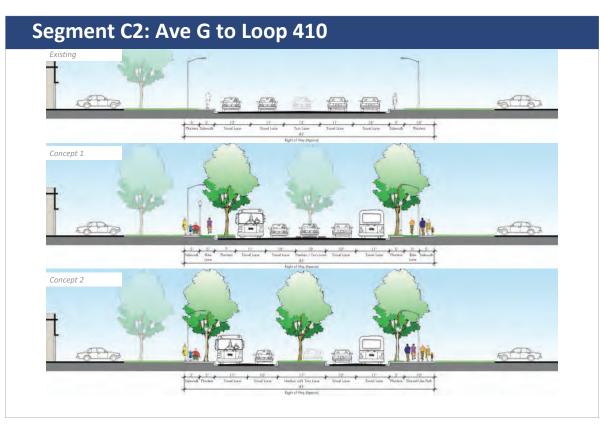
- 60% support wider sidewalks for people walking, 72% support wider sidewalks in front of businesses
- 56% support safe bike facilities for all ages and an overwhelming 90% want protected, dedicated bike facilities
- Strong desire for safer pedestrian and bike crosswalk connections at existing intersections including 410 interchange; new and improved crosswalks between the NISD athletic complex and nearby destinations including Southwest Research Institute and Leon Creek greenway trail system; and, new midblock cross walk between Fairgrounds Pkwy and Oak Hill
- Support new and improved transit facilities to Southwest Research Institute, especially at Oak Hill
- Over 80% want linear park with pedestrian and bicycle amenities along Culebra



Segment C

Callaghan Rd to Loop 410





Segment D: Loop 410 to Tezel Road

COMMUNITY FEEDBACK

- 57% support wider sidewalks for people; 71% support wider sidewalks for business
- 54% support safe bike facilities for all ages; 76% want protected, dedicated bike facilities
- A lot of comments that the existing bike lane is unsafe and should be separated and protected from fast moving traffic.
- Improve bike and pedestrian connectivity at Potranco Rd and nearby Potranco Trailhead, Reed Rd, and Rim Rock Trail; provide new crosswalk amenities around Van Ness intersection, and between Rim Rock Trail and Timber View
- Nearly 85% want linear park with pedestrian and bicycle amenities

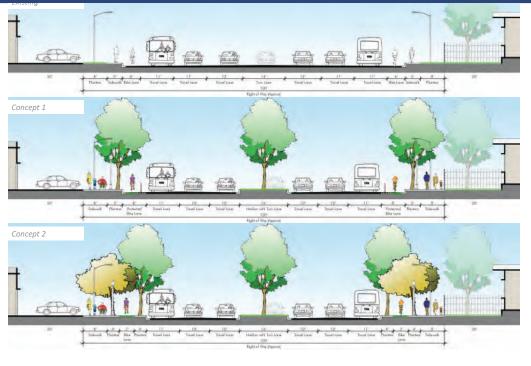


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Segment D

Loop 410 to Tezel Rd

Segment D: Loop 410 to Tezel Road



Segment E: Tezel Road to 1604

COMMUNITY FEEDBACK

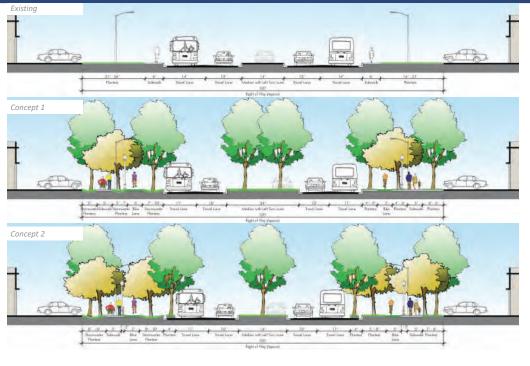
- 68% support wider sidewalks for people; 76% support wider sidewalks for business
- 75% want protected, dedicated bike facilities
- Improve pedestrian crosswalks at Tezel/Grissom, and Rogers Road; new and more frequent crosswalk opportunities all along the corridor segment including at IDEA Hidden Meadow school and Camino Rosa
- Strongly support new transit facilities near IDEA Hidden Meadow School and Tezel/Grissom intersection
- Over 80% want linear park
- Strong feedback about lack of maintenance and the need for more landscaping



Segment E

Tezel Rd to Loop 1604

Segment E: Tezel Road to Loop 1604



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Culebra Road Transportation Study
Community Engagement Summary & Emerging Concepts
August 2021



APPENDIX



Virtual Community Workshops and Outreach Summary-**Summary Report**

Culebra Road Transportation Study

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1. OVERVIEW

The City of San Antonio is conducting the Culebra Road Transportation Study to identify opportunities to enhance safety, pedestrian mobility, and micromobility access. The study received funding from the Alamo Area Metropolitan Planning Organization that is being administered through the Texas Department of Transportation. Culebra Road is the first of four roads that will be studied using this funding. The study will evaluate Culebra Road from Loop 1604 to 1-10 by applying a Vision Zero lens to develop improvements that will make the road friendlier and safer for drivers, pedestrians, and bicyclists.

The City of San Antonio, in cooperation with the project team, held three virtual community workshops and conducted extensive online outreach in April 2021 via Zoom video with teleconferencing platform. Due to the COVID-19 pandemic, the community workshops were held virtually to provide opportunities for engagement, while also adhering to public health officials' guidelines recommended by local health officials and the U.S. Centers for Disease Control and Prevention.

All three virtual community meetings were identical in content, with the one held on April 22, 2021, conducted using Spanish translation. **Table 1** lists the dates and times for each meeting. Meeting attendees, community members, and other stakeholders were invited to provide their feedback on Culebra Road.

Table 1 – Virtual Community Workshops Summary Table

Date	Time	Location
Tuesday, April 20, 2021 (in English)	11 a.m. – 1 p.m.	Virtual (Zoom)
Tuesday, April 20, 2021 (in English)	5:30 – 7:30 p.m.	Virtual (Zoom)
Thursday, April 22, 2021 (in Spanish)	6 – 8 p.m.	Virtual (Zoom)

2. MEETING STRUCTURE

The virtual community workshops were held online via Zoom and consisted of a narrated slide presentation, followed by breakout sessions. During the breakout sessions, participants were asked to provide their feedback through an online polling software, Mentimeter, and provide additional comments in small groups that were captured using another tool, Mural. A list of the attendees are included in **Appendix A**.

3. OUTREACH

To maximize participation and engagement from community members, a multi-faceted outreach effort was implemented that included both traditional and non-traditional methods. Following is a summary of the outreach conducted. Outreach materials for the virtual community meetings are included in **Appendix B**.

Newspaper Ads

Notices of the meetings were advertised in the primary regional newspapers of the area, including San Antonio Express-News, and a Spanish-language newspaper, La Prensa Texas. **Table 2** lists the date each newspaper ad was published and the circulation numbers for each newspaper. Copies of the newspaper ads can be found in **Appendix B**.

Table 2 – Newspaper Ads

Newspaper	Date	# of Subscribers
San Antonio Express-News (in English)	4/11/21, 4/13/21	645,000
La Prensa Texas (in Spanish)	4/11/21	15,000

Postcards

To ensure geographic coverage of the area, 788 bilingual postcards were mailed to adjacent property owners and stakeholders. A copy of the postcard can be found in **Appendix B**.

Email Blasts

Prior to the virtual community workshops, several bilingual email blasts were sent out to individuals on the Culebra Road Transportation Study stakeholder database. These email blasts notified individuals of the upcoming virtual community meetings.

Table 3 outlines the various email blasts that were sent, including the number of recipients and the open and click numbers for each. "Click numbers" refers to the number of individuals that clicked on a hyperlink included in the email. Copies of the email blasts can be found in **Appendix B**.

Table 3 – Email Blasts Distributio

Date	Email List	Email Content	# of Recipients	# of Opens	# of Clicks
4/6/21	Culebra Road	Online Survey and Virtual Community Workshops	117	33	10
4/19/21	Culebra Road	Reminder – Virtual Community Workshops	140	36	5

Media Coverage

Media coverage of the meetings included an article written ahead of the meetings on the KSAT 12 website and an interview on Telemundo. Below is a table summary and copies of the articles can be found in **Appendix B**.

Table 4 – Media Coverage

Pictured to the right: Hugo Tzintzun from PCI speaking on behalf of the Culebra Transportation Study Team for Telemundo.



Date	Media Outlet	Name of Article/Segment
4/8/21	News 4 SA	The City of San Antonio wants to hear from you about future improvements to Culebra Road" https://www.msn.com/en-us/news/us/the-city-of-san-antonio-wants-to-hear-from-you-about-future-improvements-to-culebra-road/ar-B81frlh0
4/14/2021	KSAT 12	-San Antonio's Public Works Department asks for input to plan Culebra Road' https://news4sanantonio.com/news/local/the-city-of-san- antonio-wants-to-hear-from-you-about-future-improvements- to-culebra-road
4/14/21	News Chant	"San Antonio's Public Works Department seeks input to plan Culebra Road" https://us.newschant.com/us-news/san-antonio/san-antonios- public-works-department-seeks-input-to-plan-culebra-road/
4/15/21	WOAI	"Transportation Department Seeks Input Regarding Culebra Road" https://woai.iheart.com/content/2021-04-15-public-works- seeks-input-regarding-upgrade-of-culebra-road/
4/15/21	Texas News Today	"San Antonio's utility sector wants input to plan Culebra Road" https://www.newsbreak.com/news/2204923425033/san- antonio-s-utility-sector-wants-input-to-plan-culebra-road
4/15/2021	Telemundo	Filmed but airing not confirmed.

Social Media Outreacl

Information about the virtual community workshops and the study in general was shared online via social media. In addition, several social media ads were strategically placed to allow for maximum reach in the community. Below is a table summay of the posts that were published and the ads that were placed. The shared social media posts can be found in **Appendix B**.

Table 5 – Social Media Posts

Date	Platform	Handle	Followers	Engagements	Shares/Retweets
3/15/21	Facebook	Alamo Area Metropolitan Planning Organization	4.059	1	0
3/15/21	Twitter	AlamoAreaMPO	2,720	3	4
3/16/21	Facebook	Pedal SATX	584	0	1

Date	Platform	Handle	Followers	Engagements	Shares/Retweets
4/16/21	Facebook	Jaque Oznog	76	0	0
4/19/21	Facebook	Mayo Caces	48	0	1
4/19/21	Facebook	Culebra Park Neighborhood Association	540	0	0
4/6/21	Facebook	San Antonio Bikes	3,828	45	22
4/9/21	Facebook	San Antonio City Council District 7	2,930	4	2
4/9/21	Twitter	AnaSandovalSATX	3,039	3	1
4/13/21	Facebook	San Antonio Bikes	3,828	53	18
4/13/21	Facebook	San Antonio City Council District 7	2,930	4	1
4/13/21	Twitter	AnaSandovalSATX	3,039	4	1
4/15/21	Facebook	San Antonio City Council District 7	2,930	1	0
4/15/21	Twitter	AnaSandovalSATX	3,039	4	0
4/16/21	Facebook	San Antonio City Council District 7	2,930	2	0
4/17/21	Facebook	San Antonio City Council District 7	2,930	1	0
4/17/21	Twitter	AnaSandovalSATX	3,039	1	0
4/19/21	Facebook	San Antonio City Council District 7	2,930	1	1
4/19/21	Twitter	AnaSandovalSATX	3,039	5	0
4/20/21	Facebook	Property Finder 210	50	0	0
4/20/21	Facebook	San Antonio City Council District 7	2,930	2	0
4/20/21	Twitter	AnaSandovalSATX	3,039	2	0

Table 6 – Social Media Ads

Date	Platform	Reach	Link Clicks
4/6/21	Facebook	13,760	266
4/6/21	Facebook	14,216	472
4/13/21	Facebook	24,082	969
4/13/21	Facebook	13,687	207

Date	Platform	Reach	Link Clicks
4/19/21	Facebook	6,981	218

4. PARTICIPATION

The combined attendance for all three virtual community workshops was 21 members of the public and elected officials. **Table 7** lists the dates and times for each meeting, the number of people who registered to attend, and the actual number of people who attended. Attrition from those who registered to those who participated is to be expected, but it does indicate that more people were aware of the virtual community meetings even if they were ultimately not able to attend.

Table 7 – Virtual Public Meetings Attendance

Date	Time	Location	Registered	Attendance
Tuesday, April 20, 2021	11 a.m. – 1 p.m.	Virtual	43	9
Tuesday, April 20, 2021	5:30 – 7:30 p.m.	Virtual	39	11
Thursday, April 22, 2021	6 – 8 p.m.	Virtual	8	1

5. WELCOME AND INTRODUCTIONS

Once the workshop began, Krystin Ramirez, MIG Project Team, welcomed all attendees and gave a brief introduction of the Zoom platform, breakout groups, and housekeeping rules. She then introduced Mentimeter, an online polling platform, which was used during the workshop.

At the two English workshops, Ms. Ramirez then went on to thank and introduce Councilman Robert Treviño, District 1, and Councilwoman Melissa Havrda, District 6, for participating in the workshop. She asked Councilman Treviño and Councilwoman Havrda to provide a brief welcome to the group.

Councilman Treviño thanked and welcomed attendees for their time and participation in the virtual community workshop. He stated that this is an important study as Culebra Road is an East/West corridor that extends from District 1 to District 6, from the center city, all the way to the Westside. In addition, he stated that this is an incredible opportunity to make Culebra Road safe for drivers, bicyclists, pedestrians, and transit users. He encouraged everyone to share ideas for Culebra as well as to stay involved with the study.

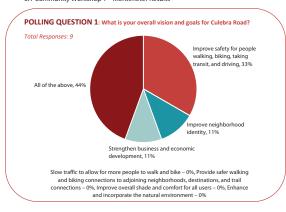
Councilwoman Havrda welcomed attendees and thanked them for their time. She stated this study will help shape reshape multimodal transportation across the San Antonio community. She then encouraged all attendees to share their thoughts and ideas to make Culebra Road a safer and friendlier ootion for all users.

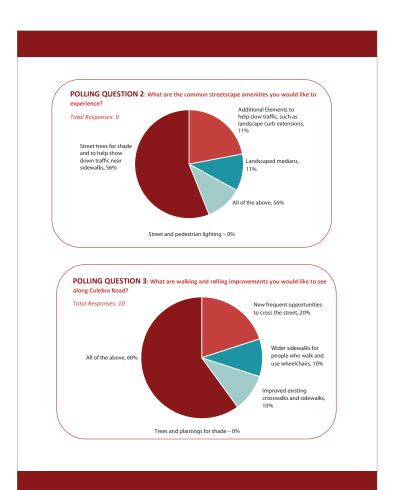
Ms. Ramirez then introduced Mukul Malhorta, with MIG. Mr. Malhorta stated that this is a multidisciplinary project which inlcudes WSP and Poznecki-Camarillo. He stated that this is the beginning of the project and the team is looking forward to hearing from the attendees on what improvements they want to see and their vision for the corridor.

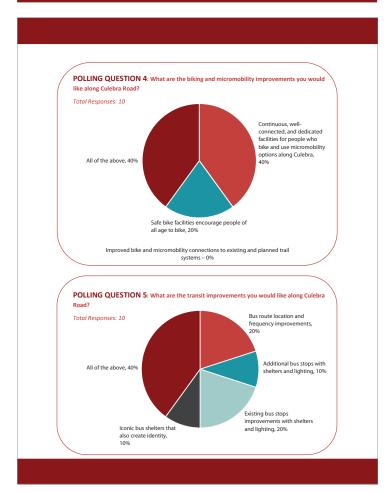
6. OVERVIEW OF COMMUNITY WORKSHOPS

Mr. Malhorta gave an overview of the project and discussed the project purpose, goals, and process. He then went on to reviewing the several different community assets, issues, and opportunities provided by Culebra Road. A copy of the presentation can be found in **Appendix** C. During the presentation, Ms. Ramirez, provided several questions using Mentimeter to allow participants to provide feedback on certain topics pertaining to their overall vision and goals for Culebra Road. Below are the questions and answers provided.

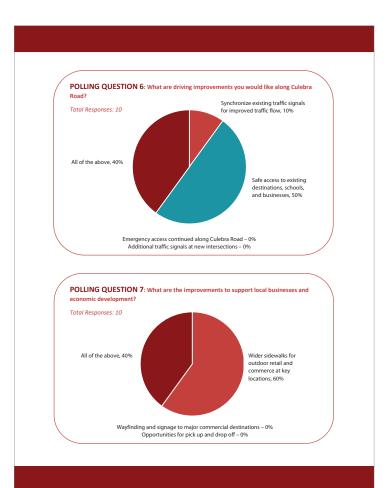
6.1 Community Workshop 1 – Mentimeter Results

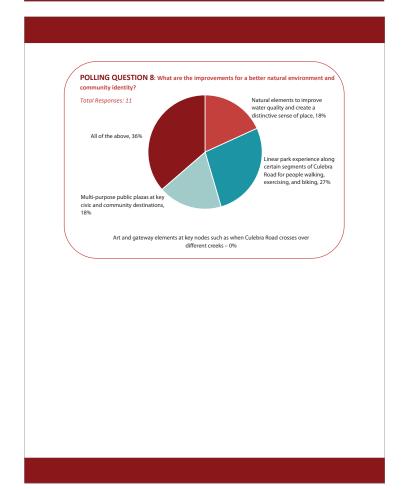


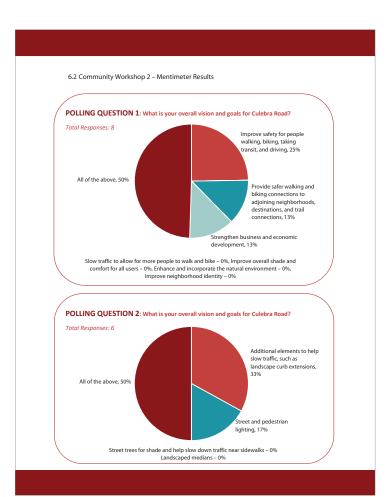




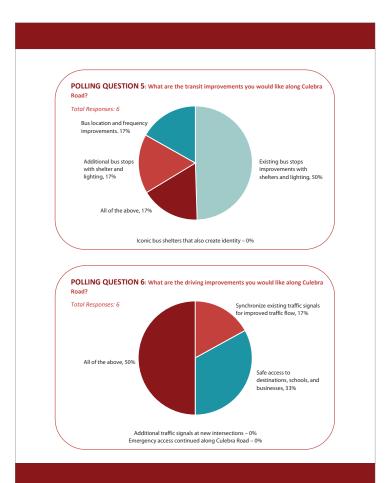


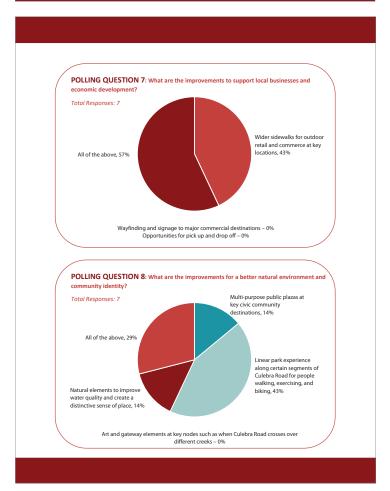












6.3 Community Workshop 3 – Mentimeter Results

Due to the low number of attendance for this workshop, the attendee had an open discussion about Culebra Road. The discussion from this workshop can be found in the 7.3 Community Workshop 3 -

7. SMALL GROUP DISCUSSION

After the main presentation, small breakout groups were held to facilitate verbal discussion regarding the information presented about Culebra Road. To make the workshop a success, feedback and opinions in the small breakout group discussions were encouraged to see what improvements the community wanted for the corridor. The community input provided by the participants will help develop a plan to improve the corridor.

The breakout session groups were pre-assigned during the registration process. Small group breakouts occurred at the end of main presentation and attendees were automatically placed in their groups. A facilitator led the discussion and a scribe recorded comments and questions. When the small group breakout ended, attendees were placed back into the main presentation and the scribe/facilitator provided a brief summary of the feedback received. Summaries of each breakout session are provided below.

7.1 Community Workshop 1 - Small Group Discussion

Gregory Reininger, City of San Antonio Kailey Butler, Poznecki-Camarillo

QUESTION 1: What is your overall vision and goals for Culebra Road?

- Managing traffic speed for I-10 to Culebra Road
 Increasing better walking and biking connections for the whole corridor
- Enhancing neighborhood identityShade and comfort

QUESTION 2: What are the common streetscape amenities you would like to experience?

- DISCUSSION:

 All of the above lack of shade

 Landscaping related to transit at stops and side roads
 Transit is important for making space available for bikin Transit is important for making space available for biking and pedestrian
 - Frequency of transit as well as amenity

OUESTION 3: What are walking and rolling improvements you would like to see along Culebra Road?

DISCUSSION:

- Shorter crosswalk distance between safe places to walk
 Street lighting is important for people walking at night
- Improve existing crosswalks
- Good detection when there is someone near the crosswalk let drivers know

- Improving lighting
 Z-crossings allow drivers to see pedestrians crossing
 Frequent opportunities more striped or signaled places to cross
- Extra signage pedestrian signage
 More inclusive design more aware of disabled users

QUESTION 4: What are the biking and micromobility improvements you would like along Culebra Road?

DISCUSSION:

- Continuous well-connected facilities
 Safe, protected continuous bike lane

- Protected bike lanes segment next to Loop 410 is narrow
 Important to recognize different type of cyclists (recreation, commuters, etc.)

QUESTION 5: What are the transit improvements you would like along Culebra Road?

- Identifying intersection where there are delays that impact on time performance whole
- corridor

 I like the iconic bus shelters, gives identity to the area

 Improve existing bus stops with shelters and lighting

 Update older shelters

QUESTION 6: What are driving improvements you would like along Culebra Road?

DISCUSSION:

- During winter storm surprisingly, high number of cars out treated it like a stop sign and worked Safe access – lower speed limits and enforcements
- Understand the purpose of the signal
 Lower the speed limits around schools

QUESTION 7: What are the improvements to support local businesses and economic development?

- Wider sidewalks better for pedestrian and can better businesses Wider sidSignage

- Adding plants, flowers, and curb appeal Finding off street areas for parking (competing with bike lanes and sidewalks) Consideration for no development zones Bringing retail out into the sidewalk enhances the character of the area

- All of the above
- Parking important to have parking for businesses
 District 1 regional parking plan for businesses

QUESTION 8: What are the improvements for a better natural environment and community identity?

- COSSION:

 * Public plaza and gateway elements locations where there is more on foot movement

 * Rain gardens could prevent runoff going into the street

 * St. Mary's would be a great gateway for greenway (Culebra and Zarzamora)

Linda Vela, Poznecki-Camarillo Lauren Simcic, City of San Antoni

QUESTION 1: What is your overall vision and goals for Culebra Road?

- Not conducive to businesses due to LT/RT/center lane all conflicting
 Creating ease of access to promote economic development
 Tezel improvements north of Culebra is positive plantings, green
 Aesthetically pleasing & wisible, not just concrete
 Long-term projects such as LRT
 Water Browner at the confirmation of the confirm

- Water flowing through, pervious pavement
 Attractive Culebra, warm
 Incorporate community plans

QUESTION 2: What are the common streetscape amenities you would like to experience?

- Amenities to supplement traffic
- Section E (Tezel Road) needs pedestrian amenities
 Larger crosswalks at businesses, protected bike/pedestrian paths
- Texas native plants
 Take a look at plantings on Tezel to see what is best for the region
- Protection for bikes/pedestrians on from I-10 to Bandera Road

QUESTION 3: What are walking and rolling improvements you would like to see along Culebra Road?

- DISCUSSION:

 Visibility of stop signs and lights due to overgrowth
 Mitigate sun glare
 Plantings on both sides of the sidewalk, serving as a buffer
 - Signage, causing drivers to stop, especially for offloading busses VIA and school

QUESTION 4: What are the biking and micromobility improvements you would like along Culebra Road?

DISCUSSION

- Avoid mixing bikes and distracted drivers safety!
 Connect to existing trail systems
 Make biking more feasible for all, focusing on kids
 Reiterating buffered sidewalks/bikeways

- Safe bike facilities leading into trailheads would not bike on Culebra as is

 ${\color{red} \textbf{QUESTION 5:}}\ \textbf{What are the transit improvements you would like along Culebra Road?}$

- SCUSSION:

 * What about light rail?

 * Iconic shelters similar to the I-35 bat bridge

 * Work with community health workers to address food/medical needs, and transit can help

 * Safer bus stops with buffer from roadway, lighting

QUESTION 6: What are driving improvements you would like along Culebra Road?

- Safe access to businesses, address LT/RT
 Reiterating glare, medians
 Slower speeds along busier sections of Culebra

QUESTION 7: What are the improvements to support local businesses and economic development?

- Sidewalks and planting at business frontage
 Hospitable to all users
- Kids walking on sidewalk that is effectively in the road, but not buffered

Signage that is more visible, compete with cell phone distraction

QUESTION 8: What are the improvements for a better natural environment and community identity?

DISCUSSION:

- CUSSION:

 Use natural elements to improve water quality

 Many resident requests for linear parks

 Creekwalk expansion just north on Tezel Road

 Enjoys trailheads on Grissom, possibly something similar on Culebra
- Wayfinding to trailheads

7.2 Community Workshop 2 – Small Group Discussion

Jessica Brunson, City of San Antonio

Linda Vela, Poznecki-Camarillo

QUESTION 1: What is your overall vision and goals for Culebra Road?

- Ny vision is to decrease travel time from there I access it (Bandera/Braun Road/Great NW/to downtown.

 Culebra is an alternate route for me to get to other parts of the city safely.
- Culebra is an alternate route for me to get to other parts of the city safely.
 One of the main things, I think is the priority to manage traffic speed. Regulate the speed
- limit.
- Posted speed limit seems to be disregarded quite a bit
 Would like to see better walking and biking connections because it is unsafe with current
- Would second improve walking/biking I'm at 1604 and Culebra and it has gotten really dangerous

- Sidewalk is right up against the road and sidewalk is missing in huge chunks
 Feels like you have to drive Culebra.
 Find a way to get traffic to flow better to downtown. Culebra is a good arterial to get to
- Id like to improve bike connections and pedestrian safety, as well as encourage businesses to be more bike/pedestrian friendly. Signs also in Spanish.

 Update bridges/culverts over creeks/tributaries

QUESTION 2: What are the common streetscape amenities you would like to experience?

DISCUSSION:

- I like using landscape to create a buffer between cars and pedestrians, so they feel safe.
- Most sidewalks are thin and up against the road need a buffer
- Street and pedestrian lighting
 When traveling, it is difficult when buses stop on main lanes -block lanes can we get bus turn in lanes?

QUESTION 3: What are walking and rolling improvements you would like to see along Culebra Road?

- Wider sidewalks definitely especially near shopping areas near Culebra/Texel and
- Culebra/Texel is scary to navigate due to free right.

 People do not walk much at Culebra and Alamo Ranch very car oriented.
- Buses do not give you enough time to board not ideal this is at Rogers Road/Culebra
- New and frequent opportunities to cross the street.
- Well landscaped medians to stop crossing left turns.
- Improved existing sidewalk/crosswalk and more opportunities to cross (on all segments).
 Generally, inside 410 going I-10 we tried Vision Zero focus on that area.
 Give 410 to I-10 infrastructure to ross safely to bring pedestrian crashes down.
- Pedestrian bridge crossings at major 4 pedestrian ways (Culebra & Ingram, Westover).
- Where people are already crossing that are not designated crosswalks? still analyzing. Some crosswalks are very far apart, and people will not walk that far.

QUESTION 4: What are the biking and micromobility improvements you would like along Culebra Road?

- DISCUSSION:
 From Tezel to Loop 1604 they have one side that backs up to Culebra Creek and isn't well
 - Instead, put extra west sidewalks on the side by Culebra Creek give them their own
 - Instead, but extra west sidewalks on the side by Culebra Creek give them their own thoroughfare.
 I biked for many years as my sole form of transportation very antagonistic when you bike need before between bikes and cars.
 Culebra between 410 & I-10 is very volatile for walk & roll of all forms. Agree with paved & divided sidewalks, or barrier protected bike lanes.

 - Downtown Alanta has protected bike corridors that effectively closed some streets to car traffic. Very unpopular option with voters but safer for micromobility/bike users.
 Improved connection to existing/planned trail systems

QUESTION 5: What are the transit improvements you would like along Culebra Road?

DISCUSSION:

- Bus pull out areas, so they do not block traffic.
- Rogers Road and Culebra stop hard to get to and buses don't wait long.

Bus turning from Culebra to Les Harrison – squeezes an extra turning lane and it is literally blocking the protected turn lane.

Perhaps they need to relocate that bus stop.

- This location has resulted in crashes and it is still pretty bad.
- There really is not transit that goes down Culebra and goes all the way downtown.

 That is very frustrating to people who live here. We want to go along Culebra road to
- è
- Definitely improve existing bus stops with shelters lighting.

 Co-located bike facilities (pumps, tools, etc.) with bus stops for commuters who use both lighting and emergency serviced button.

 Would like a real time bus info

QUESTION 6: What are driving improvements you would like along Culebra Road?

- DISCUSSION:

 It is 7-lane then 5 then 4 in different parts can we have a dedicated lane that goes faster than other lanes?

 - Like an HOV lane? Yes.

 Like an HOV lane? Yes.

 City used to have a traffic signal site on their website I have not seen anything synchronized are we doing it?

 What is being done in terms of traffic synchronization?

 - Culebra with a high-speed toll lane, interesting idea. Would that be elevated?
 Culebra from Loop 1604 inward to city is best on traffic times and entire weekend will back up from Loop 1604 pat Les Harrison needs more than widening Intersection of Culebra/Grissom/Tezel improvement

 - Are there enough carpooling areas? I think people use Culebra because it is quicker to Loop 410 than SH 151.
 - House 19 131.
 Improve crosswalk visibility in the Culebra/Bandera area. Most of those crosswalks are hard to see. Most motorists do not notice the crosswalk even when the crosswalk is flashing. Maybe some overhead lights to increase visibility from a further distance.
 - Could a displaced left-turn be a solution at Culebra/Loop 1604?

QUESTION 7: What are the improvements to support local businesses and economic development?

DISCUSSION:

- Incentives for businesses who make themselves bike friendly and accessible.
 Wider sidewalks at key locations, open spaces.
 Food truck park, there are open spaces that can be developed.

- Food truck park, there are open spaces that can be developed.
 Barrio Barista, family owned, bicycle friendly no solid route for cyclists to take. Creating a
- bilde route that does not take you on Culebra.
 Use alternate streets to local businesses so they can travel within the neighborhood but not necessarily on Culebra.
- Celebrate historic buildings, e.g., Little Flower Basilica, with open signage or open space for

The Greenway Trail French Creek is progressing. The old Conoco Station on Tezel is being transformed for CCD6 and park police facility.

 ${\color{blue} \textbf{QUESTION 8:}}\ \textbf{What are the improvements for a better natural environment and community identity?}$

DISCUSSION:

- Is there a way to have commercial interest that align with Culebra to put in pedestrian
- Short answer is yes especially where there is space. Critical thing is to provide protection.
 Public plazas and pathway designs that incorporate porous surfaces (think Confluence Park
- park pad) to aid watershed.
- Stormwater runoff management is critical.

GENERAL DISCUSSION:

- As far as economic development traffic has to be able to cross to get a business.
- My large multi-generational family lives at two different key points Zarzamora and West would love to go back and forth by bike or scooter! thank you for asking for our input!
- Combining driveways.
 Businesses need to be responsible car wash should have put up some bushes or something
- . Design guidelines/overlay districts could be discussed.
- * I love the idea of businesses getting together the idea that they could work together it would be beneficial.
- Too many places along Culebra where buildings are vacant overall, there is a very lost feeling along many of the areas along Culebra.
- If businesses start participating, feeling they are getting some help that the area in front of them will be improved maybe we can get some support

Gregory Reininger, City of San Antonio Akshaey Sabhanayagam, WSP

QUESTION 1: What is your overall vision and goals for Culebra Road?

- Shared use path is unsafe especially between Callaghan and Tezel, improve bike facilities
- with focus on safety, improve sidewalk and accesses
 Reduced speed
 Traffic calming to reduce speed.
- Riding a bike between Tezel and Loop 1604 is dangerous. Bicyclists are using trials just to be safe. Bicycle facilities near local businesses would be useful to provide access between shops
- and residences. Improve access

QUESTION 2: What are the common streetscape amenities you would like to experience?

DISCUSSION:

- Landscaping to provide shade
- Landscaping to provide shade
 Enhance crosswalk pavement markin
 Adding a park near Zarzamora Creek Enhance crosswalk pavement marking

- Street lighting
 Improve mid-block crosswalks by making it more visible to motorists.

QUESTION 3: What are walking and rolling improvements you would like to see along Culebra Road?

Add pedestrian bays or plaza which include pick-up/drop off center integrated with VIA.

QUESTION 4: What are the biking and micromobility improvements you would like along Culebra Road?

DISCUSSION:

- Separated bike lane is preferred
 Bike boxes at intersection
 Provide connectivity to trails
 Separate bicycle from vehicles

QUESTION 5: What are the transit improvements you would like along Culebra Road?

- Provide transit centers where traffic would consolidate in one place and are given access to WH-FI and convenience stores, etc. Example: At LYNX in Orlando at Central Station, the City of Orlando installed solar WH-FI poles.
 Make is easier for bicyclists to drop-off their bikes on buses.

- Add more bus stop locations.
 Add pedestrian bays or plaza which include pick-up/drop-off center integrated with VIA.

QUESTION 6: What are driving improvements you would like along Culebra Road?

- DISCUSSION:

 ◆ TWLTL are unsafe and hard to maneuver.

 ◆ Culebra at Loop 1604 could be improved like Bandera at Loop 1604 (DLT).

QUESTION 7: What are the improvements to support local businesses and economic development?

DISCUSSION:

Pick-up and drop-off bays would be a great fit.

QUESTION 8: What are the improvements for a better natural environment and community identity?

DISCUSSION

- Provide landscape at locations where it would enhance aesthetics. Example: Near creeks or greenway trails transitioning areas.
 Provide green buffer to improve aesthetics for slow speed riders (bicyclists).
- 7.3 Community Workshop 3 Small Group Discussion

QUESTION 1: What is your overall vision and goals for Culebra Road?

- Businesses need to take responsibility they should be asked to put up trees and fences.
 There is no getting away from the Yellow Bee Car Wash.
 Businesses need to not intrude onto areas visually as well as physically.
 There has been an incredible amount of building beyond Loop 1604.
 From 2 p.m. and on, it is impossible to get out of our neighborhood.

- I think we need a 3-lane road from Tezel three lanes in each direction I would like to see Culebra treated nicely/keep up with maintenance – for example, we have
- Twodulines used cultival treates interpretely put uniffication to example, we have weeds growing in the center concrete pieces.
 There are accidents waiting to happen at Bank of America/Goodwill people come out there and try to cross to make a left-turn. There is not a traffic light there.

- A possible side road to get to light .

 Three lanes each way, beautiful trees in center, sidewalks, bike lanes, having dedicated right turns.
 Longer turns for more popular areas.
 Culebra near St. Mary's needs bike lanes.

OUESTION 2: What are the common streetscape amenities you would like to experience?

DISCUSSION:

- Street lighting is very important.

 When services are upgraded people tend to respect the area more
- Street lighting is very important.
 When services are upgraded people tend to respe
 There are several places with no sidewalks.
 Nice to have coordinated lighting traffic signals.

QUESTION 3: What are walking and rolling improvements you would like to see along Culebra Road?

DISCUSSION:

- Near Chainey Road, there are people in their electric scooters who are disabled it is very disheartening knowing they have to face bumpy or broken sidewalks.
- Some of the sidewalks end or have to back into the road.
- Need to get practical things done first then beautification.
 The added trees along Tezel are very pleasant.
 I would like to see trees along Culebra.

QUESTION 4: What are the biking and micromobility improvements you would like along Culebra Road?

- I think adding biking and micromobility improvements to Culebra are a brilliant and wonderful idea.
- wonderrui idea.

 I ffacilities are properly marked, hopefully the crashes will go down.

 When people know what they are supposed to do, they tend to respect it more.

QUESTION 5: What are the transit improvements you would like along Culebra Road?

DISCUSSION:

- Nor protection from the weather.
 Shelters along Culebra for transit users.
 It know several people who do not feel safe when they ride the bus. They do not take their wallets.
 If knew I could get around town using transit I would not know what bus to take or where
- to get off.
- Do not want to face traffic going into downtown

QUESTION 6: What are driving improvements you would like along Culebra Road?

- Need a traffic light at Copper Tree exit because we cannot get out of our neighborhood from 2:30 - 7 p.m.

- We have three elementary gates at our back gate, so we are blocked in on both gates
 Addition of left-turn lanes
 If the Loop 1604 exit is too backed up, people go in the left-hand lane, make a U-turn, and come out by Goodwill.
- Perhaps a flying T at Copper Tree could work.

QUESTION 7: What are the improvements to support local businesses and economic development?

DISCUSSION:

- Signage would be nice especially with improved walking and biking.
 When you turn right from Tezel, on the left there is a giant shopping center where Well-Med is located. The left-hand turn is not very long and could be improved. Coming out of that center, you are coming out of a hill and that is a difficult right-hand turn.

QUESTION 8: What are the improvements for a better natural environment and community identity?

- Il love the idea of the outside art.
 The area needs more outdoor/community areas, such as greenspaces.
 Addition of trash cans in locations where they cannot be pushed over and thrown into the road.

8. NEXT STEPS/CONCLUSION

At the end of the virtual workshop, the attendees were thanked for their time and feedback. Mr. Malhorta mentioned the ongoing Culebra Road Community Workshops throughout the week as well as the Community Survey. The Community Survey allows for all members of the San Antonio community to provide their feedback/comments on an interactive map where they can place their comment(s) on a specific roadway. The survey will be available to participate in through Friday. June 4, 2021, All attendees were thanked again for their time and feedback, and the meeting was adjourned.

APPENDIX A
Attendee Reports

Emily Fleisher Clayton Ripps Steven Badowski Jason Rodriguez William Loudon Mauricio Hernandez Monica Delgado Taniesa Williams Kyle Sinclair Don Rios Roberto Trevino	Ripps				City	State	Zip Cod
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Joseph Raymond Lori Jones Prakash Shrestha Richard Ibarra Cynthia Coss EDWARD CASTOR Lexi Bachran Michael Brown Tomika Monterville		Email	Address	City	State	Z	ip Code
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Appendix B – Postcards



Don't have internet access? Call Linda Vela at (210) 349-3273 to ask questions or get materials presented at the community workshops. She's available between 8 a.m. and 5 p.m., Monday through Friday.

¿No tienes acceso al Internet? Llame a Linda Vela al (210) 349-3273 para hacer preguntas u obtener materiales presentados en los talleres comunitarios. Está disponible de 8 a.m. a 5 p.m., de lunes a viernes.



Culebra Road Transportation Study VIRTUAL COMMUNITY WORKSHOPS

The City of San Antonio is studying Culebra Road, from Loop 1604 to I-10, to see how the City can make it a safer and friendlier road for drivers, pedestrians, and bicyclists. Join us for one of three virtual community workshops to share your vision for the corridor and anything else you would like considered by the study team. There are three workshops: two in English and one in Spanish. All three will present the same information. You can also take our short online survey to share your vision for Culebra Road.

TO REGISTER OR TAKE OUR PROJECT SURVEY, VISIT:

www.CulebraRoadWorkshops.org

Once registered, you will receive the link to join the workshop. If you have any questions, please call Linda Vela at (210) 349-3273.

ENGLISH Tuesday, April 20, 2021 5:30 p.m. - 7:30 p.m.

Estudio de transporte de la calle Culebra TALLERES COMUNITARIOS VIRTUALES

La Ciudad de San Antonio está estudiando a la calle Culebra, desde Loop 1604 hasta I-10, para ver cómo la Ciudad puede convertirla en una calle más segura y amigable para conductores, peatones y ciclistas. Únase a nosotros en uno de los tres talleres comunitarios virtuales para compartir su visión del corredor y cualquier otra cosa que desee que el equipo de estudio considere. Hay tres talleres: dos en inglés y uno en español. Los tres presentarán la misma información. También puede realizar nuestra breve encuesta en línea para compartir su visión de la calle Culebra.

ENGLISH Tuesday, April 20, 2021 11 a.m. - 1 p.m.

www.CulebraRoadWorkshops.org **ESPAÑOL**

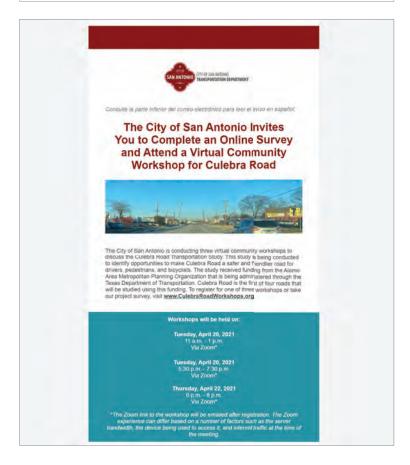
jueves, 22 de abril de 2021 6 - 8 p.m.

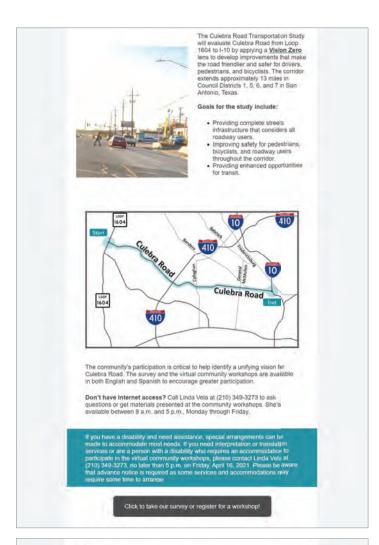
Una vez registrado, recibirá el enlace para unirse al taller . Si tiene alguna pregunta, por favor llame a Linda Vela al (210) 349-3273.

PARA REGISTRARSE O TOMAR

NUESTRA ENCUESTA, VISITE:

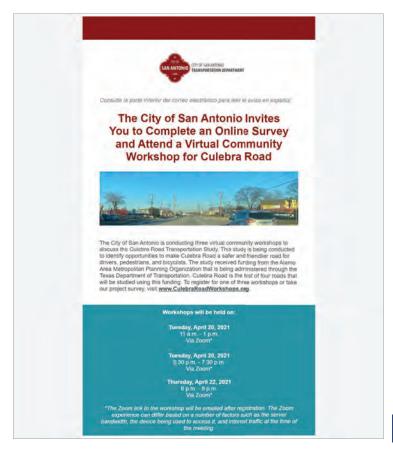
Appendix B – Email Blasts APPENDIX A-1







El Estudio de Transporte de la Calla Culebra vestuará la calle Culebra desde unidado de Visida Agre para desarrollar mejoras para hacer de Culebra una calla mada segura y a migraba para los conductores, peatones y colistas. El conductores para la calla colista compreta que consideran a callas compretas que consideran a callas compretas, que consideran a callas compretas que consideran a callas compretas que considera de la compreta de la calla compreta com ma discuspicado que requiera visua de partir de 2021 Tenga en cuantel que se requiera sun avento con anticipación y se para de partir de 2021 Tenga en cuantel que se requiera sun avento con anticipación y se para de partir de 2021 Tenga en cuantel que se requiera sun acultado de seguración de la compreta compreta compreta compreta compreta com ma aticipación y se para de partir de 2021 Tenga en cuantel que se requiera com ma dispusación



The Culebra Road Transportation Study will evaluate Culebra Road from Loop 1604 to 1-10 by applying a Wison Zero lens to develop improvements that make the road friendler and safer for drivers, pedestrians, and bicyclists. The corridor extends approximately 13 miles in Council Districts 1, 5, 8, and 7 in San Antonio, Texas. Goals for the study Include: Providing complete streets infrastructure that considers all roadway users. Improving safety for pedestrians, bicyclists, and roadway sears throughout the corridor. Providing enhanced opportunities for transit.

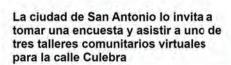
The community's participation is critical to help identify a unitying vision br-Culebra Road. The survey and the virtual community workshops are available in both English and Spanish to encourage greater participation.

Don't have internet access? Call Linda Vela at (210) 349-3273 to ask questions or get materials presented at the community workshops. She's available between 8 a.m. and 5 p.m., Monday through Friday.

1604

If you have a disability and need assistance, special arrangements can be made to accommodate most needs. If you need interpretation or translation services or are a person with a disability who requires an accommodation to participate in the virtual community workshops, please contact Linda Vells at (210),349-3273, no later than 5 p.m. on Friday, April 16, 2021. Please be aware that advance notice is required as some services and accommodations triay required service time to arrange.

Click to take our survey or register for a workshop





La ciudad de San Antonio esté llevando a cabo tres talieres comunitamos virtuales para discutir el Estudio de Transporte de la Calle Culebra. Este estudio se está llevando a cabo para identificar oportunidades para hacer de Culebra una calle más segura y amigable para conductores, peatones y ciclistas. El estudio recibió fondos de la Organización de Plantificación Metropolitana del Area de Alama que se administrarian a través del Departamento de Transporte de Texas. La calle Culebra es la primera de las cuatro carreteras que se estudiariar con este financiamento. Para registrárse en uno de los tres talleres o realizar la encuesta de nuestro proyecto, visite www.Culetra/GunAfvinshios.com.

Los talleres se llevarán a cabo en

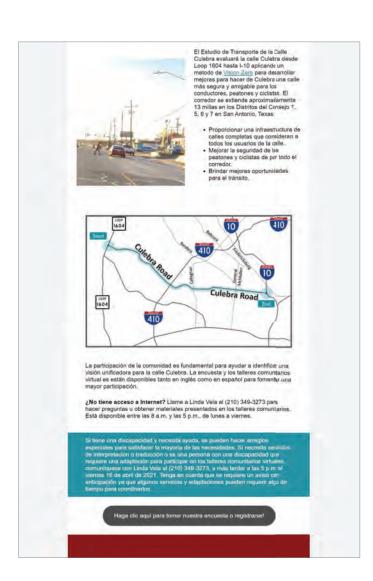
Taller de Inglés

martes 20 de abril de 2021 11 a.m. - 1 p.m. Via Zoom*

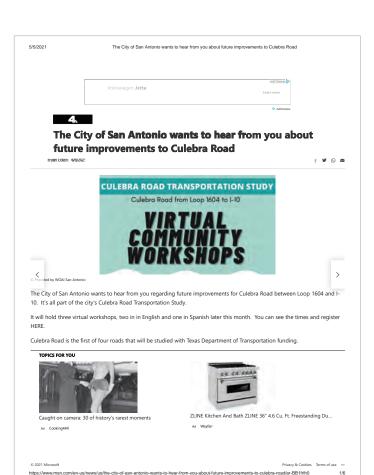
Taller de Inglès martes 20 de abril de 2021 5:30 p.m. - 7:30 p.m. Via Zoom*

Taller de español ueves 22 de abril de 2021 6 - 8 p.m.

* El enlace de Zoom al taller se enviará por correo electrónico después del registro. La experiencia de Zoom puede diferir en función dependiente in una serie de factores, como el ancho de banda del servidor, el dispositivo cue se

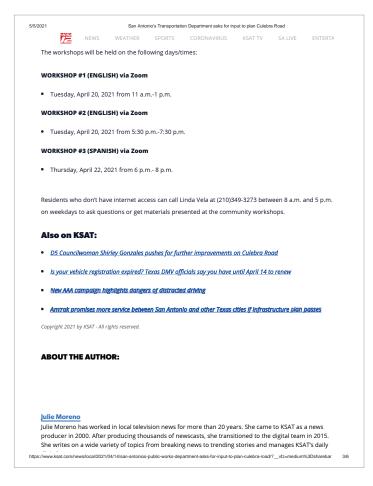


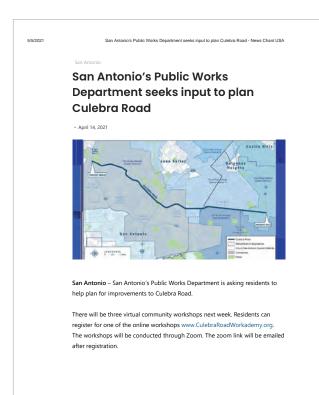
Appendix B – Media Coverage





5/5/2021 San Antonio's Transportation Department asks for input to plan Culebra Road SPORTS CORONAVIRUS KSAT TV SA LIVE Castle Hills Leon Valley SAN ANTONIO - San Antonio's Transportation Department is asking residents to help them make plans for improving Culebra Road. There will be three virtual community workshops next week. Residents can register for one of the workshops online at www.CulebraRoadWorkshops.org. The workshops will be held via Zoom. The Zoom link will be emailed after registration. The city aims at making Culebra a safer route for drivers, pedestrians and bicyclists. The workshops will support the Culebra Road Transportation Study, which will evaluate 13 miles of Culebra Road, from Loop 1604 to I-10, by applying a **Vision Zero** lens, officials said in a press release. Vision Zero is an initiative that focuses on reducing serious and fatal injuries in our area. The study received funding from the Alamo Area Metropolitan Planning Organization that is being administered through the Texas Department of Transportation. Culebra Road is the first of four roads that will be studied using this funding, officials said. Ad Google Fiber Google Fil Fast, reliable Find out if G gigabit internet. Google





https://us.newschant.com/us-news/san-antonio/san-antonios-public-works-department-seeks-input-to-plan-culebra-road

1/7

The city aims to make Culebra a safe passage for drivers, pedestrians and cyclists. The workshops will support the Culebra Road Transportation Study, which will evaluate 13 miles of Culebra Road, from Loop 1604 to 1-10, Sight zero Lens, officials said in a press release.

Vision Zero is an initiative focused on reducing serious and fatal injuries in our region.

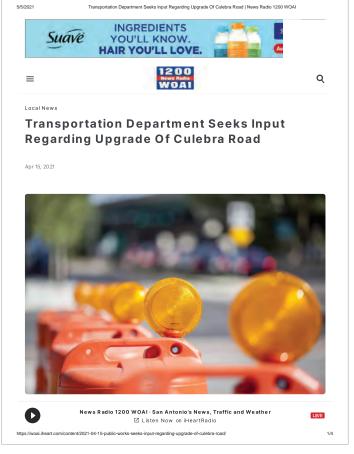
Cop who fatally shot Maya Bryant is a military-trained shooter: report

The study received funding from tl

Organization which is being admir

Read Next Story >





5/5/201

The city's Transportation Department is asking San Antonio residents for their input about how to improve Culebra Road. The Culebra Road Transportation Study is evaluating 13-miles of the road between Loop 1604 and I-10. The goal is to make Culebra safer for drivers, pedestrians and bicyclists. Three virtual community workshops are planned for next week via Zoom. The Zoom link will be emailed to people who register on their website.

Transportation Department Seeks Input Regarding Upgrade Of Culebra Road | News Radio 1200 WOAI

GO

Sponsored Content



[Pics] We All Had A Crush On Herln The 90's, This Is HerNow

Definition



Surgeon: Tinnitus? When The Ringing Won't Stop, Do This (Watch)

new healthylife.club



Doctors Can't Explain Why This Fruit May Cut Your Blood Sugar By 90%

Gluco Shield Pro



Mexico City Subway
Overpass Collapse
News Radio 1200 WOAI - San Antonio...



Federal Judge Throws Out
CDC Pandemic Related
Moratorium On Evictions
News Badio 1200 WOAL - San Antonio



WATCH: Woman Sets Fire To Home With Person Inside, Watches From Lawnchair

https://woai.iheart.com/content/2021-04-15-public-works-seeks-input-regarding-upgrade-of-culebra-road/

2/4

5/5/2021

San Antonio's utility sector wants input to plan Culebra Road - Texas News Today

San Antonio's utility sector wants input to plan Culebra Road

daviddalpe • 3 weeks ago



San Antonio – San Antonio's public works sector is asking residents to help develop plans for improving Culebra Road.

Next week there will be three virtual community workshops. Residents can enroll in one of the online workshops at www.CulebraRoadWorkshops.org. The

https://texasnewstoday.com/san-antonios-utility-sector-wants-input-to-plan-culebra-road/224976/

1

San Antonio's utility sector wants input to plan Culebra Road - Texas News Today workshop will be held via Zoom. The zoom link will be emailed to you after registration.

The city aims to make Klebra a safer route for drivers, pedestrians and cyclists. Officials said the workshop will support the Culebra Road Transportation Study, which evaluates the 13-mile Culebra Road from Loop 1604 to I-10 by applying the Vision Zero lens, officials said in a press release.

Vision Zero is an initiative focused on reducing serious and fatal injuries in our

Vision Zero is an initiative focused on reducing serious and fatal injuries in ou area.

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https://texasnewstoday.com/san-antonios-utility-sector-wants-input-to-plan-culebra-road/224970

2/5

5/5/2021

San Antonio's utility sector wants input to plan Culebra Road - Texas News Today

Advertising

The workshop will be held on the following day / time.

Zoom Workshop # 1 (English)

• •Tuesday, April 20, 2021 from 11:00 am to 1:00 pm

Zoom Workshop # 2 (English)

• •Tuesday, April 20, 2021 from 5:30 pm to 7:30 pm

Zoom Workshop # 3 (Spanish)

• •Thursday, April 22, 2021 6:00 pm to 8:00 pm

Residents without internet access can call Linda Vela (210) 349-3273 between 8 am and 5 pm on weekdays to ask questions and present materials at community workshops.

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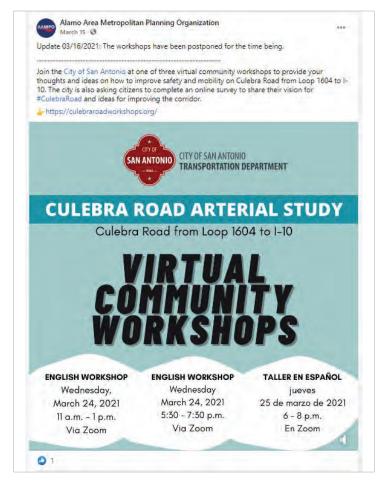
San Antonio's utility sector wants input to plan Culebra Road

Source link San Antonio's utility sector wants input to plan Culebra Road

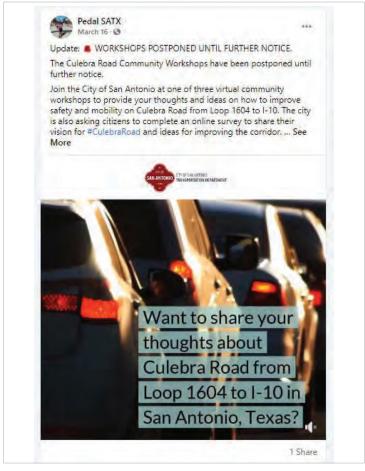
https://texasnewstoday.com/san-antonios-utility-sector-wants-input-to-plan-culebra-road/224976/

3/5

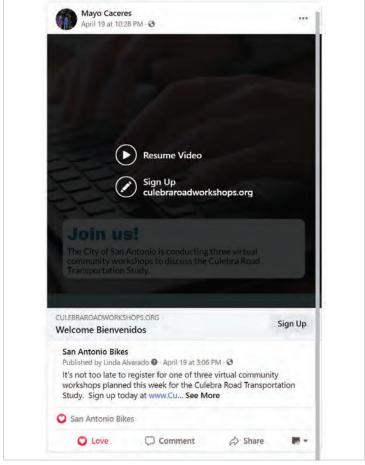
Appendix B – Social Media Outreach



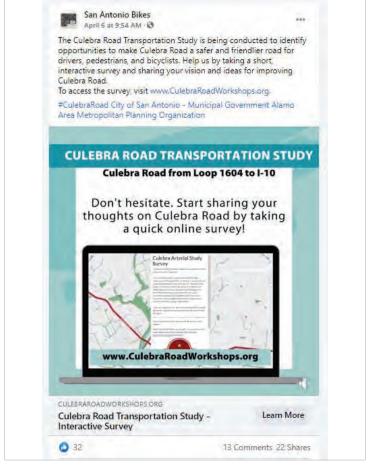






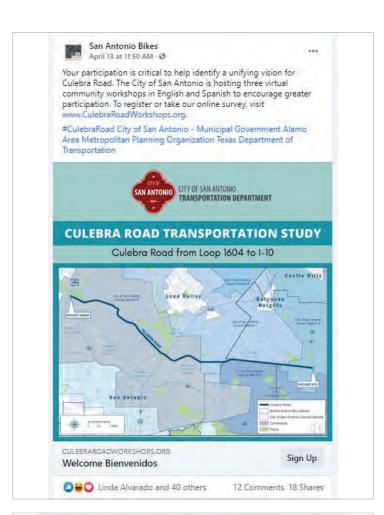


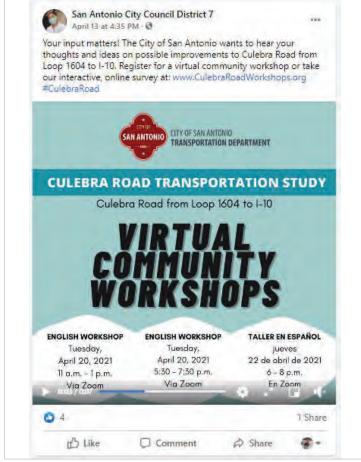




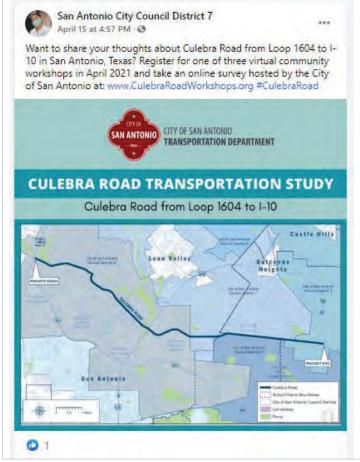






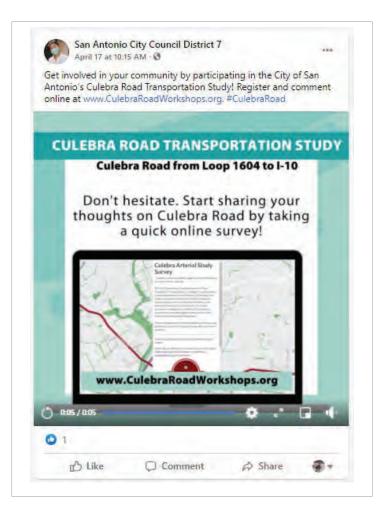




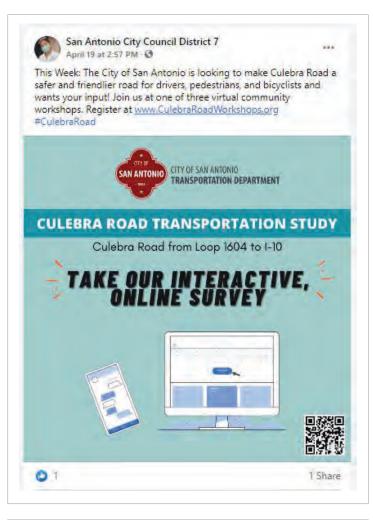


Ana Sandoval @AnaSandovalSATX · Apr 15 Want to share your thoughts about Culebra Road from Loop 1604 to I-10 in San Antonio, Texas? Register for one of three virtual community workshops in April 2021 and take an online survey hosted by the City of San Antonio at: CulebraRoadWorkshops.org #CulebraRoad CULEBRA ROAD TRANSPORTATION STUDY Culebra Road from Loop 1604 to I-10 Culebra Road from Loop 1604 to I-10









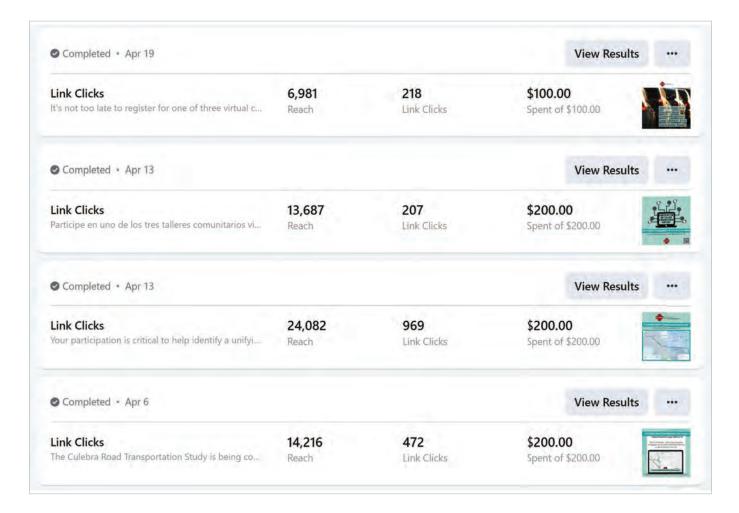






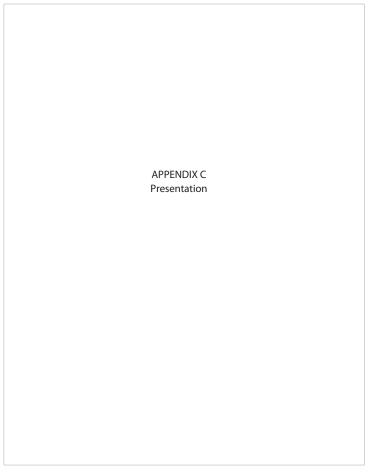
Ana Sandoval @AnaSandovalSATX · Apr 20 Do you travel along Culebra Road from Loop 1604 to I-10 on a regular basis? Then the City of San Antonio wants to hear from you! Register and comment online at CulebraRoadWorkshops.org, #CulebraRoad CULEBRA ROAD TRANSPORTATION STUDY Culebra Road from Loop 1604 to I-10 CULEBRA ROAD TRANSPORTATION DEPARTMENT COOD 37 views CULEBRA ROAD TRANSPORTATION DEPARTMENT COOD 37 views

Completed • Apr 6			View Results	***
Link Clicks Join the City of San Antonio at one of three virtual	13,760 Reach	266 Link Clicks	\$200.00 Spent of \$200.00	VIRTUAL CONHUNITY WORKSHOPS



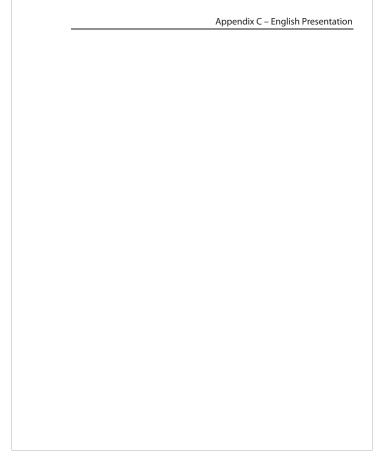






APPENDIX

APPENDIX A-1







Culebra Road Transportation Study
COMMUNITY VISIONING WORKSHOP

April 2021

Meeting Instructions

ZOOM:



- Name: Click the ... by your name to update your name – use First and Last Name
- **Unmute:** Have a question? Use the raise hand feature to ask a question



- Chat: Use the Chat feature to submit questions or comments during the meeting
- Can't access the Chat feature? Please text your questions to (210) ***-****.

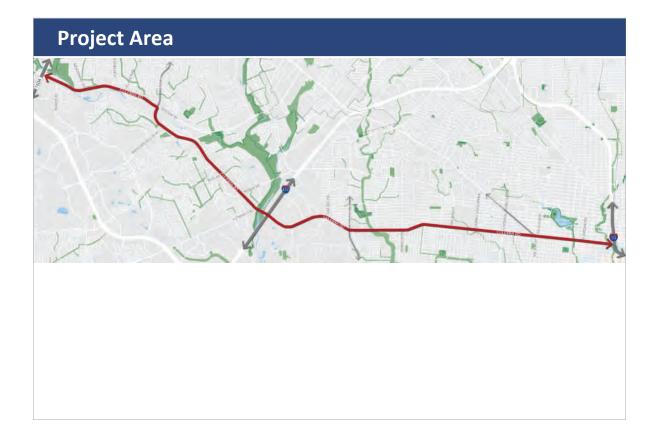
Mentimeter:

- **Description:** Mentimeter is an online polling tool
- Access: Use another browser tab or device to access Mentimeter
 - Visit: www.menti.com
 - Use Code: 5157 9754
- Can't access Mentimeter? Please text your responses to (210) ***-**** or enter your response in the chat

Go to www.menti.com and use the code 5157 9754

Agenda

- WELCOME & INTRODUCTIONS
- PROJECT PURPOSE, GOALS & PROCESS
- COMMUNITY ASSETS, ISSUES & OPPORTUNITIES
- EMERGING COMMUNITY VISION & GOALS
- GROUP DISCUSSIONS
- GROUP DISCUSSION REPORT BACK
- NEXT STEPS



Project Goals

- 1. Create a safe corridor for ALL users of the street
- 2. Transform Culebra into an equitable, multimodal corridor for people who walk, roll, bike, take transit and drive
- 3. Enhance the overall experience and connectivity for pedestrians, bicyclists and transit users
- 4. Develop conceptual designs for short- and long-term improvements
- 5. Coordinate and **build upon previous and concurrent studies** and initiatives
- 6. Ensure meaningful community and stakeholder engagement

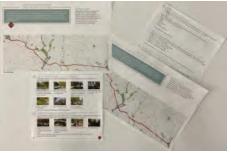
Project Process Oct 2021 | Nov 2021 | Dec 2021 Mar 2021 Apr 2021 May 2021 June 2021 July 2021 Aug 2021 Sept 2021 Community Survey Community Workshops Technical Advisorv Meetings Stakeholder Meetings Reports Preferred Community Conditions Alternatives Alternatives Report

Project Process: Community Survey

- Bi-Lingual English and Spanish
- Engaging, online, interactive survey
- Launched: March 12, 2021
 - Over 150 surveys
 - Over 2480 map responses
- Complimentary in-person survey







https://culebraroadworkshops.org/

Project Process: Community Survey

- Bi-Lingual English and Spanish
- Engaging, online, interactive survey
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 - Over 150 surveys
 - Over 2480 map responses
- Complimentary in-person survey
- Locations to pick up or drop the survey

Memorial Library

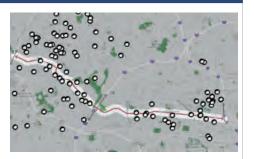
3222 Culebra Rd. San Antonio, TX 78228

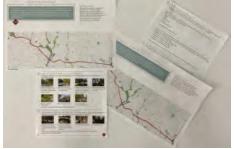
Great Northwest Library

9050 Wellwood St. 78250

Council District Offices

Or ask for a copy to be mailed to you!





https://culebraroadworkshops.org/

Project Process

Go to www.menti.com and use the code 5157 9754

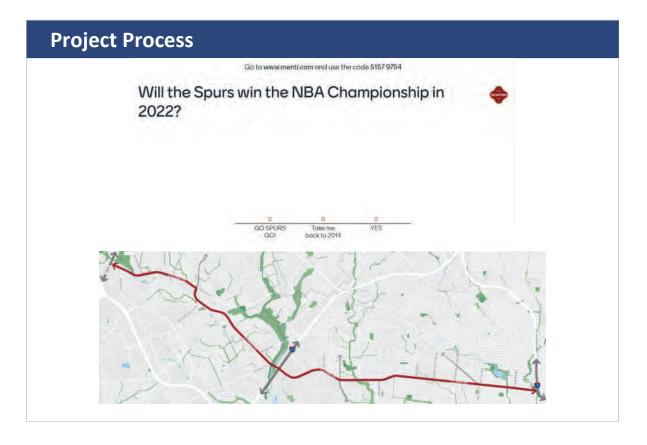
Type your name and relationship to Culebra Road.



Examples of Relationship to Culebra Road

- I live nearby
- I work nearby
- I shop nearby
- I travel down the road often

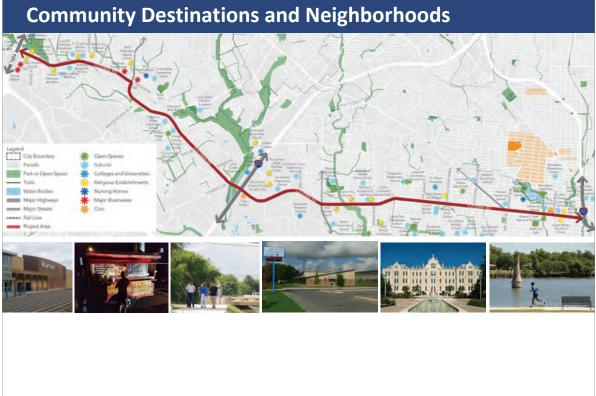






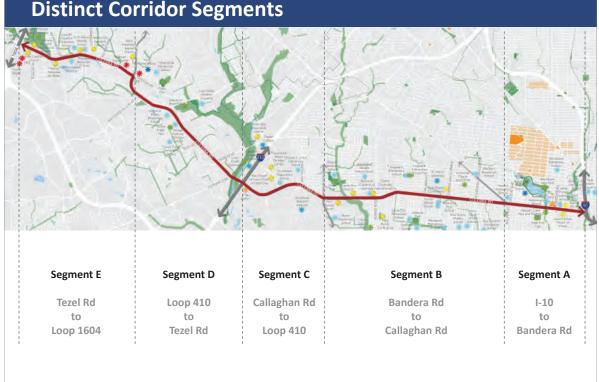
COMMUNITY ASSETS

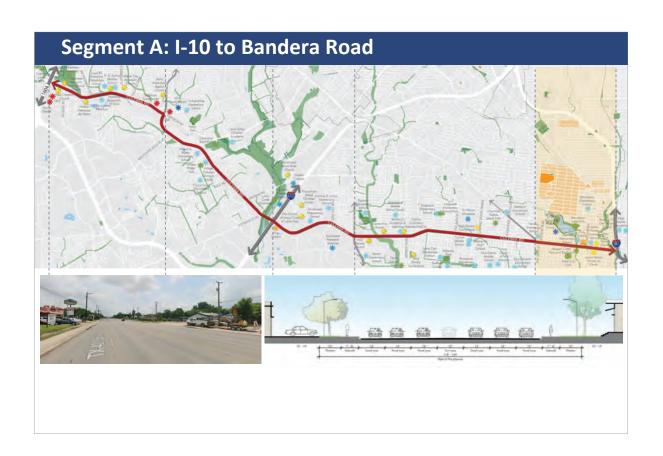
APPENDIX A-1 ommunity Destinations and Neighborhoods

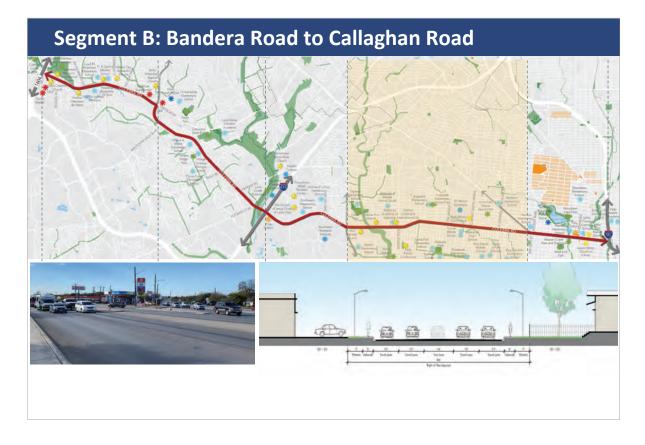


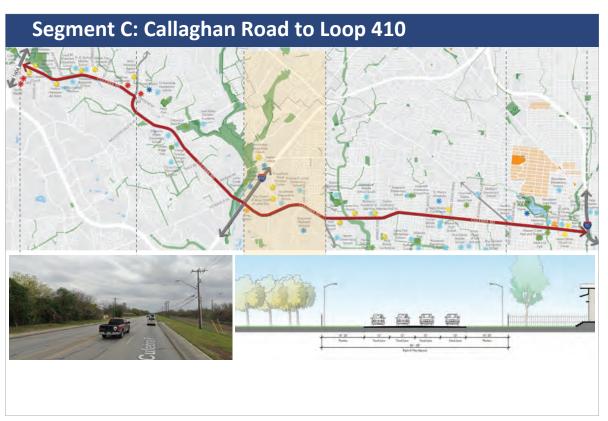


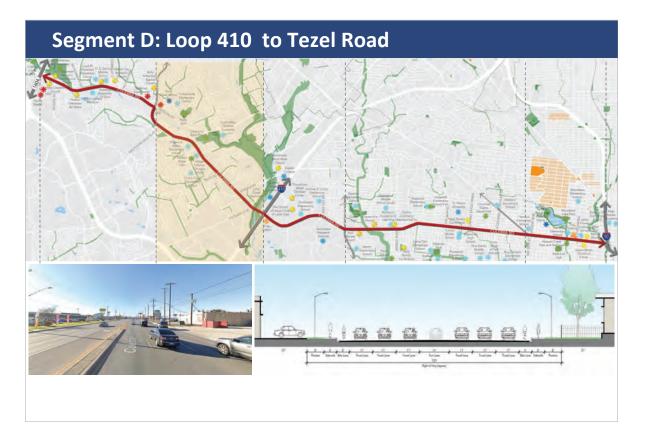
Distinct Corridor Segments















KEY CHALLENGES & OPPORTUNITIES



Pedestrian and Bicycle Facilities







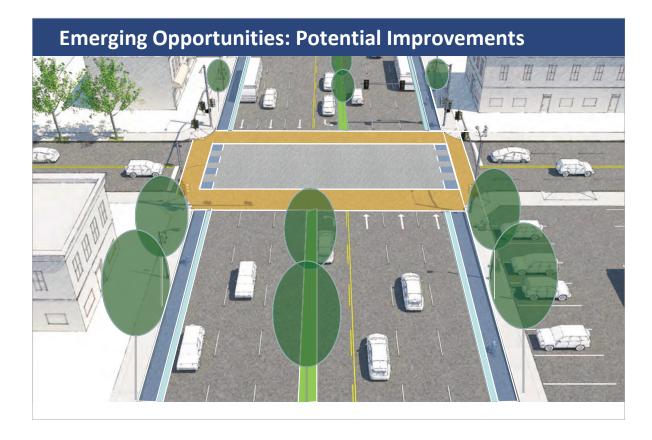


Transit Facilities













Emerging Opportunities: Potential Improvements

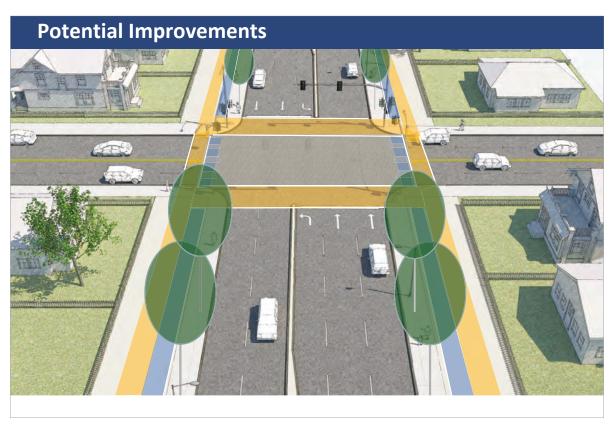




Emerging Opportunities: Potential Improvements







Emerging Opportunities: Potential Improvements

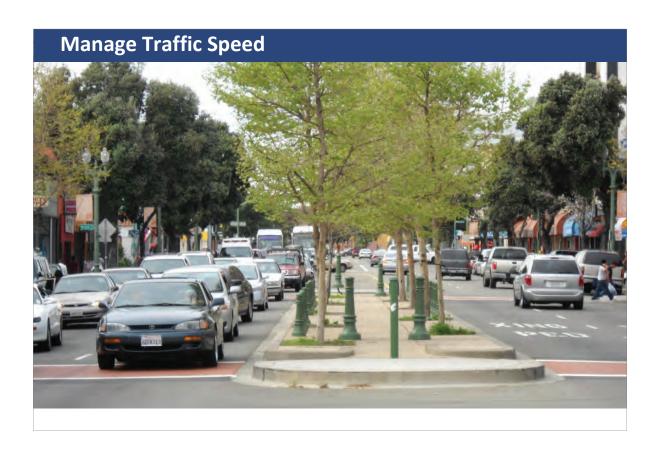






EMERGING VISION FOR CULEBRA ROAD

Increase Safety Increase Safety

















GROUP DISCUSSION



GROUP DISCUSSION

- What is your overall vision?
- What are the common streetscape amenities you would like to experience?
- What improvements you would like to see as you walk and bike?
- Other Discussion Items
 - What improvements you would like to see as you drive and take transit
 - What improvements would support local business and economic development?
 - What are the improvements for a better natural environment and community identity?



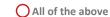
GROUP DISCUSSION #2: Overall Streetscape Amenities



Street trees for shade & comfort



Street and pedestrian lighting





Additional elements to help calm traffic



Landscaped Medians

Questions #2: What are the common streetscape amenities you would like to experience?

GROUP DISCUSSION #3: Walking and Rolling



New frequent opportunities to cross the street



Trees and plantings for shade

All of the above



Wider sidewalks for people who walk and use wheelchairs



Improved existing crosswalks and sidewalks

Questions #3: What are walking and rolling improvements you would like to see along Culebra Road?

GROUP DISCUSSION #4: Biking and Micromobility



Ocontinuous, well-connected, and dedicated facilities



 Improved bike and micromobility connections to existing and planned trail systems



Safe bike facilities encourage people of all ages to bike

All of the above

Questions #4: What are the biking and micromobility improvements you would like along Culebra Road?

GROUP DISCUSSION: Taking Transit



Existing bus stops improvements with shelters and lighting



Additional bus stops with shelters and lighting



O Bus route location and frequency improvements



lconic bus shelters that also create identity

All of the above

Questions #5: What are the transit improvements you would like along Culebra Road?

APPENDIX

APPENDIX A-1

GROUP DISCUSSION: Driving



Synchronize existing traffic signals for improved traffic flow



Additional traffic signals at new intersections

All of the above



O Safe access to existing destinations, schools, and businesses



Emergency access continued along Culebra Road

Questions #6: What are driving improvements you would like along Culebra Road?

GROUP DISCUSSION: Local Business & Economic Development



Wayfinding and signage to major commercial destinations



Wider sidewalks for outdoor retail and commerce at key locations



Opportunities for pick up and drop off

All of the above

Questions #7: What are improvements to support local businesses and economic development?

GROUP DISCUSSION: Natural Environment & Identity



O Natural elements to improve water quality and create a distinctive sense of place.



Multi-purpose public plazas at key civic and community destinations



Linear park experience along certain segments of Culebra Road for people walking, exercising and biking



Art and gateway elements at key nodes such as when Culebra Road crosses over different creeks

O All of the above

Questions #8: What are the improvements for a better natural environment and community identity?



GROUP DISCUSSION FEEDBACK

Next Steps

- Community Vision Engagement
 - Upcoming Community Meetings
 - Community Survey Ends (May 15th)
- Summarize Overarching Community Vision
- Develop Emerging Streetscape Concepts





Culebra Road Arterial Study
COMMUNITY VISIONING WORKSHOP
April 2021

Appendix C – Spanish Presentation



Culebra Road Transportation Study
Taller de Visión Comunitaria
April 2021

Instrucciones de Taller

ZOOM:



- Nombre: Haga clic en ... junto a su nombre para actualizar su nombre use **Primer** Nombre y Apellido
- **Chat:** Utilice la función de Chat para enviar preguntas o comentarios durante la reunión
- ¿No puede acceder a la función de Chat? Envíe sus preguntas por mensaje de texto a (210) 827-7183.

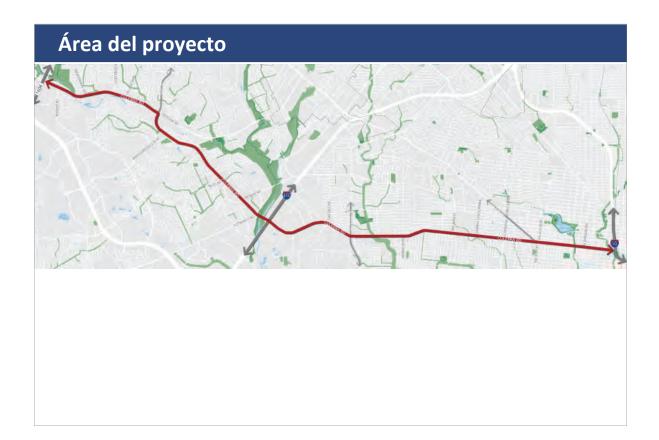


Agenda

- BIENVENIDA e INTRODUCCIONES
- PROPÓSITO, OBJETIVOS Y PROCESO DEL PROYECTO
- ACTIVOS, PROBLEMAS Y OPORTUNIDADES COMUNITARIAS
- VISIÓN Y OBJETIVOS COMUNITARIOS EMERGENTES
- DISCUSIONES
- PASOS SIGUIENTES

APPENDIX

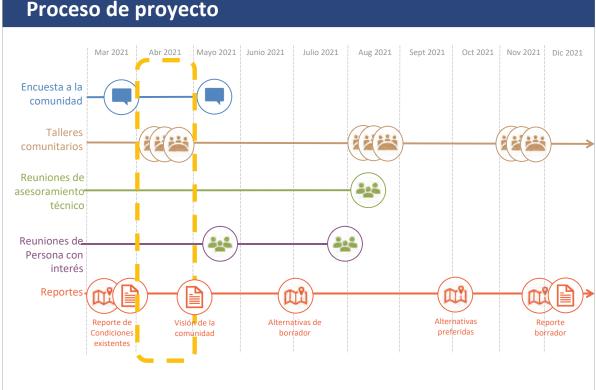
APPENDIX A-1 | 97



Objetivos del proyecto

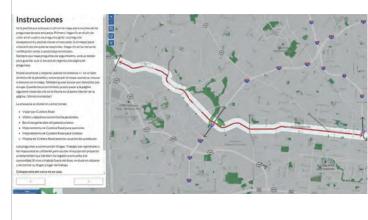
- 1. Crear un corredor **seguro para TODOS** los usuarios de la calle
- 2. Transformar Culebra en un corredor equitativo y multimodal para personas que caminan, ruedan, andan en bicicleta, toman el transporte público y conducen
- 3. Mejore la experiencia general y la conectividad para peatones, ciclistas y usuarios del transporte público
- 4. Desarrollar diseños conceptuales para mejoras a corto y largo plazo
- 5. Coordinar y aprovechar estudios e iniciativas anteriores y simultáneos
- 6. Asegurar una participación significativa de la comunidad y las partes interesadas

Proceso de proyecto

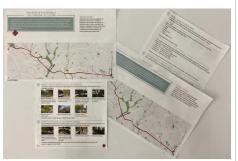


Proceso del proyecto: Encuesta a la comunidad

- Bilingüe Ingles y Español
- Encuesta interesante, en línea e interactiva
- Lanzado: 12 de Marzo, 2021
 - Más de 150 encuestas
 - Más de 2480 respuestas de mapas
- Encuesta en persona de cortesía







https://culebraroadworkshops.org/

APPENDIX

Proceso del proyecto: Encuesta a la comunidad

- Bilingüe Ingles y Español
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- Encuesta en persona de cortesía
- Ubicaciones para recoger o dejar la encuesta

Memorial Library

3222 Culebra Rd. San Antonio, TX 78228

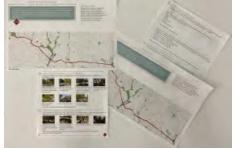
Great Northwest Library

9050 Wellwood St. 78250

Oficinas de Distrito del Consejo

• ¡O solicite que le envíen una copia por correo!





https://culebraroadworkshops.org/

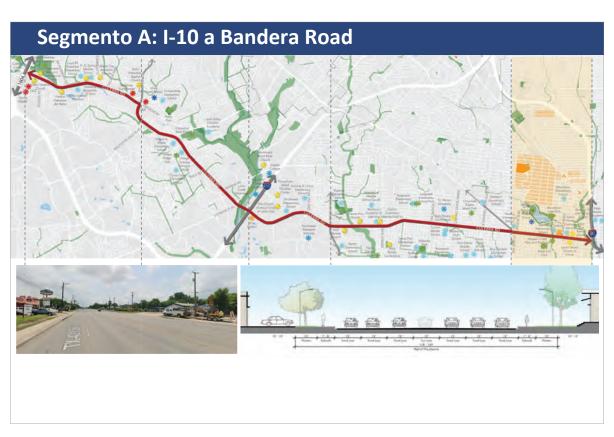


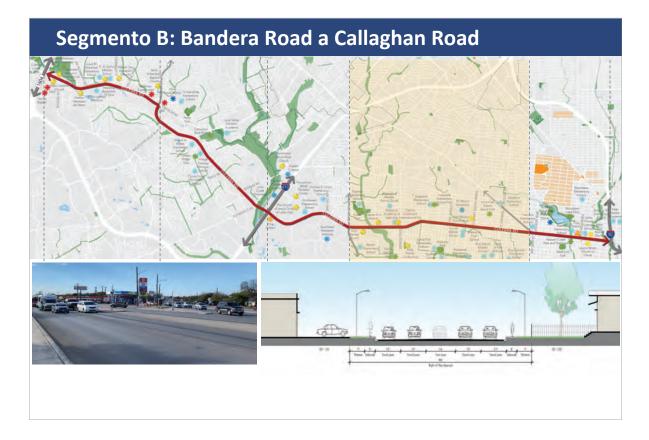
ACTIVOS COMUNITARIOS



















DESAFÍOS Y OPORTUNIDADES CLAVES



Instalaciones para peatones y bicicletas









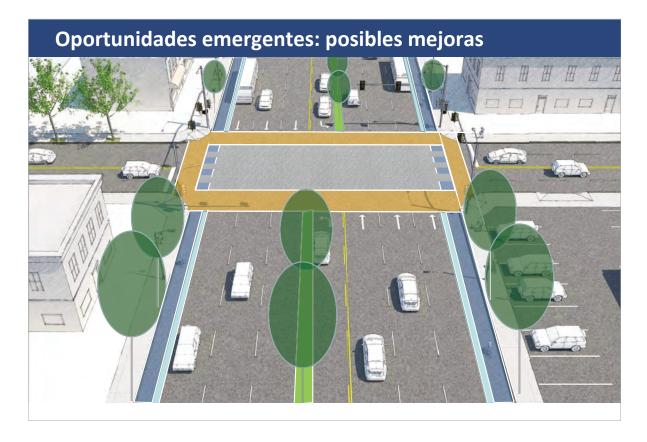
Instalaciones de tránsito









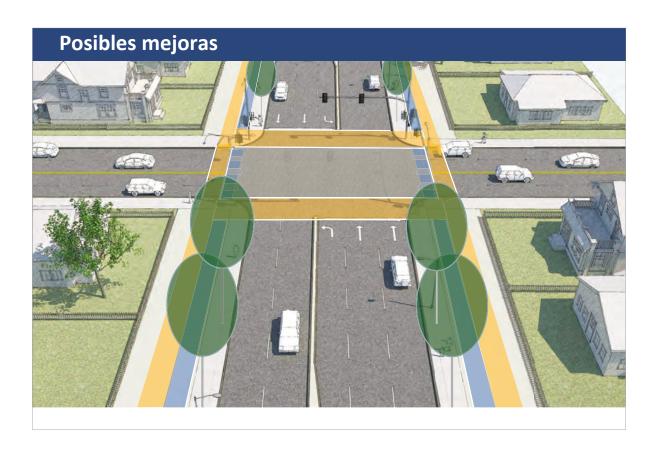












Oportunidades emergentes: posibles mejoras





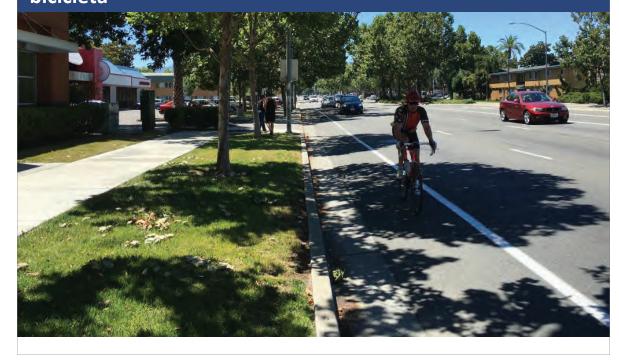


VISIÓN EMERGENTE DE CULEBRA ROAD





Proporcione mejores conexiones para caminar / andar en bicicleta



Mejora la sombra y la comodidad



APPENDIX A-1 lejorar el entorno natural













REUNIÓN DE DISCUSIÓN



DISCUSIÓN

- ¿Cual es su visión y objetivos generales por Culebra Road?
- ¿Cuáles son las comodidades del paisaje urbano quieres experimentar?
- ¿Cuáles son las mejoras para personas que caminan y van en ruedas que le gustaría ver?
- ¿Cuáles son las mejoras para personas que andan en bicicleta que le gustaría ver?
- Otros temas de discusión:
 - ¿Cuáles son las mejoras de transporte público que le gustaría ver?
 - ¿Cuáles son las mejoras para personas que conducen que le gustaría ver?
 - ¿Cuáles son las mejoras para apoyar a las empresas locales y el desarrollo económico?
 - ¿Cuáles son las mejoras del entorno natural y identidad de comunidad?



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DISCUSIÓN #2: Paisaje Urbano



Árboles de la calle para dar sombra y comodidad



Elementos adicionales para ayudar a ralentizar el tráfico



Alumbrado público y peatonal



Medianas ajardinadas

Todo lo Anterior

Pregunta #2: ¿Cuáles son las comodidades del paisaje urbano quieres experimentar?

DISCUSIÓN #3: Caminan y van en Ruedas



Nuevas y más frecuentes oportunidades para cruzar la calle de forma segura.



Árboles y plantaciones para sombra y como amortiguador del tráfico.



Aceras más amplias para personas que caminan y personas que usan sillas de ruedas



Mejorar los cruces peatonales existentes

O Todo lo Anterior

Pregunta #3: ¿Cuáles son las mejoras para personas que caminan y van en ruedas que le gustaría ver?

DISCUSIÓN #4: Andar en Bicicleta y Micro-movilidad



Instalaciones continuas, bien conectadas y dedicadas para personas que andan en bicicleta y usan opciones de micromovilidad a lo largo de Culebra.



Conexiones mejoradas para bicicletas y micromovilidad con los sistemas de senderos existentes y planificados



 Las instalaciones para bicicletas seguras animan a las personas de todas las edades a andar en bicicleta.

O Todo lo Anterior

Pregunta #4: ¿Cuáles son las mejoras para personas que andan en bicicleta que le gustaría ver?

DISCUSIÓN: Transporte Público



Mejoras en paradas de autobús existentes con marquesinas e iluminación



O Paradas de autobús adicionales con marquesinas e iluminación



Mejoras en la ubicación y frecuencia de las rutas de los autobuses



Marquesinas de autobús icónicas que también crean identidad

O Todo lo Anterior

Pregunta #5: ¿Cuáles son las mejoras de transporte público que le gustaría ver?

APPENDIX

APPENDIX A-1

DISCUSIÓN: Conducir



O Sincronizar las señales de tráfico existentes para mejorar el flujo de tráfico



O Semáforos adicionales en nuevas intersecciones

O Todo lo Anterior



Acceso seguro a destinos, escuelas y negocios existentes



Acceso de emergencia contínuo por Culebra Road

Pregunta #6: ¿Cuáles son las mejoras para personas que conducen que le gustaría ver?

DISCUSIÓN: Empresas Locales



Señalización y guías a los principales destinos comerciales



Aceras más amplias para venta Oportunidades para recoger y minorista y comercio al aire libre en ubicaciones clave



dejar pasajeros

O Todo lo Anterior

Pregunta #7: ¿Cuáles son las mejoras para apoyar a las empresas locales y el desarrollo económico?

DISCUSIÓN: Entorno Natural y Identidad



Elementos naturales para mejorar la calidad del agua y crear un sentido distintivo de lugar



Plazas públicas de usos múltiples en destinos cívicos y comunitarios clave



Experiencia de parque lineal a lo largo de ciertos segmentos de Culebra Road para personas que caminan, hacen ejercicio y andan en bicicleta

O Todo lo Anterior



Arte y elementos de entrada en nodos clave, como cuando Culebra Road cruza diferentes arroyos

Pregunta #8: ¿Cuáles son las mejoras del entorno natural y identidad de comunidad?



DISCUSIÓN

Próximos pasos

- Participación en Visión de la Comunidad: Encuesta a la comunidad – Termina May 15, 2021:
- Desarrollar conceptos emergentes de paisaje urbano









Culebra Road Transportation Study

Taller de Visión Comunitaria

April 2021

Culebra Road Transportation Study Stakeholder Meeting May 26, 2021 from 5:30 – 6:30 p.m.

PARTICIPANTS

- Carlos Gonzalez, Co-President, West End Hope In Action
- Greg Reininger, City of San Antonio
- Julio Ramos, WSP
- Mukul Malhotra, MIG
- Krystin Ramirez, MIG
- Linda Vela, PCI

MEETING SUMMARY

The meeting began with an overview of West End Hope in Action. Mr. Gonzalez, Co-President of West End Hope in Action, stated that the organization was formed 10 years ago with an AARP grant and has been doing grassroots community organizing since then. He said the group was challenged to become a neighborhood association and since has provided comments on Culebra numerous times because people keep getting killed, especially as the roadway has gotten wider and faster. He said the organization focuses on the area from 24th Street to Zarzamora but also extends from Zarzamora to I-10, and they primarily work out of the Frank Garrett Multi Service Center at 1226 NW 18th Street, 78207.

Next, Krystin Ramirez, with MIG, provided an overview of the Culebra Road Transportation Study. She got to a stopping point and asked Mr. Gonzalez to identify assets and opportunities along Section A of the corridor. Mr. Gonzalez said the character of the roadway changes at Bandera. He expressed appreciation that the traffic light on 19th Street was reinstalled. He said it had always been there and was then taken out and finally reinstalled once West End Hope in Action collaborated with the city to get it back.

Mr. Gonzalez recommended placing surveys in front of Torreon Restaurant and in front of Delicious Tamales and mentioned they were both great locations and places to collect surveys. Mr. Gonzalez said they really wanted to slow the speed of traffic. He said his experience is that electronic signs that show drivers how fast they are going compared to the posted speed limit work when combined with increased police enforcement. He said he used this when working for a school.

Mr. Gonzalez also recommended creative signage that encourages drivers to "keep our children safe" and "protect our neighborhood." He said that drivers do not seem to realize that after 24" Street they are in a residential area. Mr. Gonzalez said the traffic signal at 19" was very helpful and the additional z-crossings helped as well. He said that traffic signals help seniors and the disabled to have enough time to cross. He said the Alazan Creek trail also helps walkers get from one side of Culebra to the other safely. He said that Zakina Price who works for Councilman Roberto Treviño would have copies of their input and comments regarding Culabra Poat.

In terms of assets, Mr. Gonzalez also cited the Little Flower Basilica, which is on the corner of Culebra and Zaramora. He told the study team that there are pavers at Culebra and Zarzamora and thought they were put there when St. Thereas's remains were toured through the city. He said Little Flower just got a relatively new

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pastor and that he would try to get a good contact for them to the study team. He also cited the former Lerma's Night Club, the longest running conjunto live music venue, which is on the National Register of Historic Place.

 $Mr.\ Malhotra\ then\ asked for\ the\ best\ restaurants\ in\ the\ community.\ Mr.\ Gonzalez\ recommended\ Torreon,\ Valentinas,\ and\ Los\ Angeles.$

Mr. Gonzalez said that his organization helped District 1 get input on a previous survey asking questions about Culebra Road. He said Ms. Price with District 1 would have this information. Mr. Gonzalez said that the city installed an island at Hamilton which he felt might have been a mistake because emergency vehicles often have to jump the median when they respond.

He said his organization meets every two weeks at the Frank Garrett Multi Service Center with the next meeting being on Friday, June 4, 2021 at 9:15 a.m. He offered to have his folks take the survey at that meeting. Linda Vela, with PCI, offered to take surveys and offer a short overview of the study.

Mr. Gonzalez said that when they were working with District 1 to collect surveys, they talked to the woman operating Delicious Tamales and she indicated was involved in a crash. He said she agreed that people needed to go slower. He also said that day was also the night of the last fatality on Culebra that included a man and a woman.

Mr. Malhotra asked if a lot of students walk or bike. Mr. Gonzalez said that most of the kids from the Lincoln Court Public Housing have to cross at Elmendorf and Culebra because they go to Nelson Elementary about two blocks north of Culebra. He said that for a while, one of the crossing guards was a retired cop and that helped. He said West End Hope in Action works with the SAPD SAFE unit and every now and then they can help increase enforcement and help slow things down. He again mentioned the speed limit alert machine would only work if it is followed up with increased enforcement.

Mr. Ramos asked if connections from the Alazan Creek trail to Culebra Road would help. Mr. Gonzalez said they would. He said it would make things a lot easier. He said there is a natural foot path from the sidewalk to get to the trails but that a formal connection would be great.

Mr. Gonzalez also said Culebra Road used to be the dividing line between the northside and the rest of the community. He said it feels very symbolic to try to make things safer. He said, "the way things evolved, a lot of the lawn keepers and house keepers lived over here and worked over there."

Mr. Malhotra asked if Mr. Gonzales saw a lot of people walking or biking on Culebra Road. Mr. Gonzalez said he did not see anyone riding a bike on Culebra but that there are a ton of pedestrians.

Mr. Malhotra asked how long Mr. Gonzalez lived in the area. He said 64 years. Mr. Malhotra also asked what Culebra was like before widening. Mr. Gonzalez said one good thing was that it got rid of Mike's Lounge. He said there was a rumor about widening it for years before it actually got widened. He said speeds were always fast on Culebra Road but that when it got widened, they got even faster. He also mentioned that someone told them there were more fatalities on that segment of Culebra than any other in the country. Greg Reininger said it is definitely one of the highest fatality corridors in the city.

Mr. Gonzalez finished by saying he was excited to see new businesses coming in. He talked about a combo washateria and pizza place coming in next to the Circle K, the Anais Record Service expanding, and other signs that people are ready and willing to invest in Culebra.

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He wrapped up by talking about the assets near the Frank Garrett Multi Service Center in the West End Park, which is a historic park. He said it is also next to the senior nutrition center, and Parks and Recreation. He also said that West End Hope in Action coordinates a walk/talk with Chief McManus. He said the organization teaches people in the community they can report crime anonymously, come to the center, or contact one of the organization representatives. He said there are 10 people on the executive committee.

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Culebra Road Transportation Study NW Crossing NA and Timber Ridge NA Stakeholder Meeting June 1, 2021, from 1 p.m. to 2 p.m.

PARTICIPANTS

- Kenneth Pfeiffer, Northwest Crossing NA
- Dawn Tomaschefsky, Northwest Crossing NA
- Donald Page, Timber Ridge NA
- Julio Ramos, WSP
- Mukul Malhotra, MIG
- Krystin Ramirez, MIG
- Linda Vela, PCI

MEETING SUMMARY

This meeting was set to gather input from the Great Northwest on the Culebra Road Transportation Study. Mukul Malhotra, MIG, provided an overview of the study then asked participants for their thoughts on needed improvements along Culebra Road. Following is the information collected from the stakeholders listed above.

 $\label{thm:constraints} \textbf{Kenneth Pfeiffer with the Northwest Crossing Neighborhood Association made the following comments:}$

- The safety issue is critical and in addition to that is making sure that the traffic flows.
- He indicated that his neighborhood association as in communications with City of San Antonio Council District 6 regarding the linear parks and the linear trails. He said they are asking the city to extend a trail that runs from 1604 and Culebra Road through some undeveloped area. He said this also ends up right across of Schaenfield where a new hospital is being built. He indicated this trail could also take care of issues the neighborhood has experienced with a homeless camp and the crime and drugs that come with the homeless camp. He said that by bringing in the trails, it will bring more eyes to the area.
- Mr. Pfeiffer said that making sure that public right of way is used appropriately is something else that came up.

Donald Page with the Timber Ridge Neighborhood Association provided the following comments:

- He said most of his observations had to deal with safety concerns in Segment E.
- He said that speeding is a concern and that the speed limit signs on Potranco from Culebra to
 Military are 33% larger than the signs on Culebra. He said the city code does allow for larger
 signs to be used. He said Culebra is extremely busy and those speed limit signs easily get lost.
 He recommended that instead of using the minimum size of signs, the city should use larger
 signs on Culebra Road.

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- Secondly, he said that crossing Culebra is like trying to run the gauntlet. He said that as one is
 trying to cross or merge into Culebra you also have to try to avoid pedestrians who may be
 trying to cross.
- He said solid medians like on Hunt Lane would be nice on Culebra Road to limit left turns especially where you have six lanes. He said he would recommend raised medians from Rimrock Trail to Timberview, Micron to Reed Road, and Micron to Ingram.
- Mr. Page also mentioned that there was a median installed last month that he feels was improperly placed at Potranco. He said the City needs to make sure medians are installed correctly.
- He expressed a concern that from Micron to Ingram there are no signalized intersections or
 pedestrian crossings. He said there is an extended stay hotel in this area and that there are
 always people trying to cross the street. He said a median in this area would provide a refuge
 for pedestrians. He said this would benefit both the people who walk in this area and the people
 who drive.

Julio Ramos, WSP, said the team had been discussing the use of green T intersections. He said the team is also considering adding pedestrian signals and z-crossings because there are areas with pretty big gaps between pedestrian crossings.

One of the stakeholders asked if it was possible to do a fly over from Culebra Road to Loop 410 and specifically how the Bandera Road to Loop 410 fly over came to be. Mr. Ramos said this suggestion would be noted for consideration.

Mr. Pfeiffer said he thought all these ideas were good and that he hoped they would take root and move forward. Dawn Tomaschefsky, Northwest Crossing NA, said she had no additional ideas.

The consultant team asked if stakeholders ever saw kids cross Culebra to get to school. The stakeholders said that there may be some that cross at Timberview but that this has never been an issue or concern.

The consultant team said that one hope is to add more vegetation along the corridor. One of the stakeholders said that when they added more trees and shrubbery in the Tezel median, the foliage can sometimes make left turns a real challenge because you cannot see cars coming. They asked that the team be aware of the site distances associated with any sort of vegetation.

The study team also asked if there were any hilly areas on Culebra. Stakeholders said Culebra is generally pretty flat.

The consultant team asked for any other ideas and the stakeholders suggested connecting to adjacent creeks.

Stakeholders concluded their comments by pointing out that folks traveling to the new hospital on Loop 1604 would probably take Les Harrison.

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Culebra Road Transportation Study Thunderbird Hills NA and Culebra Park NA Stakeholder Meeting June 1, 2021, from 5:30 p.m. to 6:30 p.m.

PARTICIPANTS

- Daniel Rossiter, Thunderbird Hills NA
- Don Rios, Culebra Park NA
- Greg Reininger, City of San Antonio
- Mukul Malhotra, MIG
- Linda Vela, PCI

MEETING SUMMARY

This meeting was set to gather input from the Thunderbird Hills Neighborhood Association and the Culebra Park Neighborhood Association on the Culebra Road Transportation Study. Mukul Malhotra, MIG, provided an overview of the study then asked participants for their thoughts on needed improvements along Culebra Road. Following is the information collected from the stakeholders listed above.

Daniel Rossiter said he is a consultant with WSP working at the Southwest Research Institute but also the president of the Thunderbird Hills Neighborhood Association for the past three years. He said he was involved in meetings back in the spring to develop a vision for Culebra Road.

 $\label{thm:main} \mbox{Mukul Malhotra asked if either of them had taken the Culebra Road Transportation Study survey and Don Rios said he had.}$

Mr. Rossiter said the Thunderbird Hills NA extends from Callaghan to I-10. Mr. Rios said the Culebra Park NA extends from Benrus to Callaghan.

Mr. Rossiter said that to him everything is secondary to the safety issue. He said it is pretty disheartening to see all the crashes. He said there is one intersection where there are four different roads coming in at different angles and that intersections like that needed to be fixed. He asked the study team not to worry about beautification until the safety issue is handled.

Mr. Rios said that he concurred with Mr. Rossiter on safety as the paramount factor. He said that near his area, the study team needed to think about mass transit. He also said that he would like beautification and shade cover especially where Zarzamora Creek runs under the Culebra Road Bridge. He said there is an apartment complex by there and that stretch of road has lots of people that take the bus. He said another opportunity is near Brindle and Culebra. He said that opens into his neighborhood and would provide an opportunity. He also identified the area near Barrio Barrista as an area with retail going in and neighborhood visitors. He thought this would be an area with more pedestrian traffic. As a side bar, he recommended that the study team have the Westside Special if they go to Barrio Barrista.

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Mr. Rios also said the intersection at Bandera Road and Culebra Road is really challenging. He said he did not know how to fix it without causing significant impacts. Mr. Mulhotra said that one issue is the geometry, but the other issue is the amount of speeding.

The consultant team then asked for other issues or opportunities. The stakeholders said the sidewalks along that area are in disarray. They said there is no green buffer between the sidewalks and the road. They also said HEB draws huge amounts of pedestrian traffic. Stakeholders said they need wider sidewalks and sidewalks with a buffer away from the roads.

The Culebra Park NA said they were looking forward to the types of improvements being proposed. They said that how the proposed improvements sound very different from what is there now. Some of the concerns that were raised included the 300-unit apartment complex that is being constructed at Mirz Mista and Culebra Pacel.

Stakeholders said that most of those residents would be drivers but that it could also increase the number of people using the sidewalks. They also said that south of Mira Vista there is another 200-unit apartment complex coming into the area.

Mr. Mulhotra said all improvements will have to consider the future needs of the road. He said there are several locations where people have told the study team that they would like to see new crosswalks.

Mr. Mulhotra asked if there were any other developments that the study team should keep in mind. Stakeholders said there is another development that sits next to Thunderbird Hill NA. They said they were called the Majestic Ranch Apartments and that they were located at Bandera Road and Callaghan Road.

Stakeholders asked if the study team would be coordinating with the Southwest Research Institute (SWRI) and St. Mary's University. The study team said they were reaching out to try to schedule a meeting with both.

Mr. Rossiter said he works for WSP but is located at SWRI. He said the majority of SWRI employees do drive and enter via the main entrance, which is off Culebra, but that SWRI has five separate entrances — three of which are not on Culebra. He said most of the traffic is skewed towards Culebra Road since their main gate is there and has a signalized intersection.

Mr. Rossiter said It is dangerous trying to turn into and out of Bill Miller at Culebra and I-410 because you cannot see past the vehicles directly in front of you. He said this is a daily occurrence and turning into and out of Bill Miller is difficult. He said he would strongly recommend make getting back from those food areas safer.

He said the Northside Stadium is also a consideration. He said in the evenings the parking lot gets very crowded and you have lots of high schoolers trying to go to those food establishments. He also pointed out that there is a fire station and a police substation in this area.

Stakeholders also mentioned a proposal to the county commissioners to develop Zarzamora Creek into a trailway that would end at the Tierra Sol Park. They said it seems reasonable to extend that trailway northward. They indicated that they were confident that something positive would happen at Zarzamora Creek and asked that consultants consider access to a future trail.

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Stakeholders encouraged consultants to include protected bike lanes wherever possible instead of just a

Mr. Malhotra asked Mr. Rossiter how long his drive to work was. Mr. Rossiter said that on a good day it's between 2-5 minutes. He said he picked his home based on where he works so his commute is not had

Mr. Rossiter said he uses his bike recreationally and uses the sidewalks on Culebra Road but does not always feel safe. He said he liked the idea of shared use paths.

Mr. Malhotra concluded the meeting by indicating that Ms. Vela would send out the links to the survey and asked that they help disseminate to others who might be interested in the study.

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APPENDIX

Culebra Road Transportation Study Great Northwest Stakeholder Meeting June 4, 2021, from 10:30 – 11:30 a.m.

PARTICIPANTS

- Brian Stives, Great Northwest Chairman of the Board
- Bill McDunn, Great Northwest Resident
- Andy Greene, Great Northwest Resident
- Julio Ramos, WSP
- Linda Vela, PCI

MEETING SUMMARY

This meeting was set to gather input from the Great Northwest on the Culebra Road Transportation Study, Julio Ramos, WSP, and Linda Vela, PCI, provided an overview of the study then asked participants for their thoughts on needed improvements along Culebra Road. Following is the information collected from the stakeholders listed above.

- The I-10 section may be more dangerous for pedestrians since there are more pedestrians in that area.
- My concern is the Tezel intersection and having to make that left turn on Culebra Road. Is there
 a conceptual plan in place for that area? Not yet but the study team will be developing cross-sections.
- Stakeholders mentioned that Culebra Road from Tezel to Loop 1604 used to be a statemaintained section and that as part of TxDOT's turn back program, the medians were added.
 They indicated that the public had a hard time with this because there was not enough notice
 given. They indicated that the medians make it difficult to get out of the way of emergency
 vehicles, which can affect response times.
- Stakeholders said the traffic on the west end of Culebra is significant and that instead of two
 lanes in each direction there should probably be three lanes in each direction.
- Stakeholders were concerned that adding protected bicycle lanes from Tezel to Loop 1604 would make it even more difficult to get emergency responders through.
- They indicated that there is already a bike lane on Timber Path from Culebra Road to Les Harrison. They further said they would prefer for the city to complete the bike trail from Cathedral Rock to Loop 1604 and to go under Culebra Road near the HEB.
- They indicated that cyclists do not ride on Culebra Road and instead ride on the sidewalks.
- Stakeholders said the right of way is already narrow from Tezel to Loop 1604 and that adding a bicycle lane would make it difficult to add a third lane in each direction when needed.
- They said the owner of a limo company owns most of the land near Easterling and Village

 Parkway

 Parkway
- Mr. Ramos informed stakeholders that there are issues with a cement plant that are holding up
 the completion of the trail.
- Stakeholders mentioned that this issue is currently being handled by the courts as the cement
 plant owner is being cited for several violations related to an adjacent drainage channel. They
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 - reiterated that the neighborhood is most interested in seeing the trail connected. They said their observations are that most cyclists use the trails and not the road due to safety concerns.
- One of the stakeholders indicated that the intersection at Bandera Road and Loop 1604 seems
 to be working better since it was improved. He said he would prefer to travel through that area
 than through the Culebra Road/Loop 1604 intersection.
- Stakeholders also indicated the pedestrian crossings at Culebra/Tezel/Grissom have bad timing
 and they are very hard to cross if you are older or in a wheelchair. They said the intersection
 works well for cars but not for bicveles and pedestrians.
- Stakeholders also expressed concerns that the improvements that were just put in by TxDOT would get torn up as part of this project. They said they had just gone through construction and did not want to see the loss of those improvements.
- Stakeholders also said that people often come down Tezel and over to Timber Path to avoid the Culebra/Tezel/Grissom intersection.
- Stakeholders expressed concerns that since ATD funds were taken away from trail projects, the trail connection they were seeking would not get built.
- Stakeholders asked what the timeframe was for completion of the study. Mr. Ramos said the
 goal was to finish the study by the end of the year so that any proposed projects could be
 recommended for inclusion in the bond package.
- Andy Greene recommended reaching out to the District 6 office for an updated list of neighborhood associations.
- Mr. Ramos asked stakeholders if speeding was an issue. They said that congestion is too great during the day for any kind of speeding but that you can hear people speeding at night, particularly on Grissom.
- Ms. Vela asked about student travel patterns. Mr. Greene said that neither high school is within
 walking distance. He recommended checking school boundaries via the school district. He also
 recommended talking to Lindsey Place about this issue.
- Stakeholders did identify that younger children (8th graders/pre-teens) will cross Culebra to get to the gas station for snacks.
- Stakeholders also mentioned that there are about 100 homeless people living under the Culebra bridge in the vicinity of the proposed trail.

This concluded the stakeholder meeting. Mr. Ramos thanked stakeholders for their time and told them the plan was to conduct a public meeting later in the year.

Culebra Road Transportation Study Bicycle/Pedestrian Groups Stakeholder Meet June 4, 2021, from 1:30 – 2:30 p.m.

PARTICIPANTS

- William Long, Activate SA Board Member
- Joe Pawlik Active Transportation Planner with AAMPO
- Jeff More SATX Social Ride
- David Bemporad Activate SA
- Bryan Martin Bike San Antonio
- Brenda Gonzalez South Texas Off Road Mountains (STORM)
- Cristian Sandoval Earn-a-Bike
- Greg Reininger, City of San Antonio
- Julio Ramos, WSP Mukul Malhotra, MIG
- Krystin Ramirez. MIG
- Linda Vela, PCI

MEETING SUMMARY

The meeting began with introductions of the Study Team, participants, and an overview of the Culebra Road Transportation Study given by Julio Ramos, with WSP. Julio Ramos, WSP, went over the goals of the project and then asked the participants what their goals were for Culebra Road. During the presentation, William Long suggested that the Study Team attend the SATX Social Ride on Tuesday night to give riders the opportunity to take the survey. Jeff More, SATX Social Ride, said they would ensure to share the survey information with all riders. William Long also mentioned that a community member's water company came out to SATX Social Ride and was able to engage the group. At the conclusion of the presentation, the meeting was opened up to any questions of feedback regarding the study.

William Long started the questionnaire portion of the meeting and mentioned that he has lived off of Les Harrison near Loop 1604 and Easterling Dr., and on Easterling Dr. there is a dead end. He said a concrete company owns the entire horseshoe, and you can see the tracks and trails from their driving and it can be a big impact zone. David Bemporad, Activate SA, then asked what the traffic flows were and what the potential reduction is when you get closer to Downtown where segments A, B, and C exist. Julio responded that the Study Team has been running existing and projected models and came to the result that the 7-lane segment between I-10 and Bandera is a very congested segment. Having received the majority of feedback from the community, the Study Team is trying to make a five-lane section work and have specific improvements at signalized intersections. He mentioned the analysis is still in progress. He said on segment B, a lower volume section, the Study Team is looking at doing improvements at specific intersections and creating proposed cross-sections then go back to the public for feedback. From there, that will shape a preferred alternative and go back to the public once more to verify that we have the community's vision in place. Julio also said that they are looking at what to do with the right of way by Southwest Research Institute.

Linda Vela, PCI, then pulled up Google Maps and explained how when the Study Team met with West End Hope in Action, their biggest concern was slowing down traffic in the area, Brenda Gonzalez STORM, wanted to identify how many elementary and middle schools are nearby and how many kids

would have to cross Culebra to get there since most kids walk and ride their bikes. She mentioned she used to live in Sections E and D towards Loop 1604 and would bike to work taking Culebra Road with no bike lanes and then head out towards Talley Road (no sidewalks). She said she would have to take her mountain bike because it was dangerous. She said she also accesses Culebra Creek Park to reach $Terra\ Oaks\ Trailhead\ to\ get\ to\ the\ Greenaway's\ to\ use\ commuting, similar\ to\ others.\ Another\ concern$ she has is traffic during school hours not slowing down for kids, especially younger kids, who are

Cristian Sandoval, Earn-a-Bike, mentioned Culebra Road is not at its best right now and the people who are riding along Culebra Road are not recreational riders, they all ride to get to work, school, etc. He said recreational cyclists would even find it intimidating to ride with cars going 50-55 mph. He said he agrees that there should be a dedicated bike lane for riders.

David Bemporad, Activate SA, then briefed the team on Activate SA efforts and said they are working with neighborhood associations in the Woodlawn Lake area to get a complete street along Cincinnati which is a corridor north of Culebra. He also pointed out that the SA River authority has put out plans to receive funding from Bexar to extend Zarzamora from Roosevelt Park to Monterey Park and Commerce Street. The future goal is to further extend Zarzamora to reach Culebra Road to serve that potential need. He also said that Activate SA is working with communities in the South Westside at potential east/west bike routes to reach from Loop 1604 on one side to Loop 410 from East to West. He also worked with the City of San Antonio on a Vision Zero project and to potentially add more crosswalks to Culebra Road.

Mr. Bemporad continued with his topics and said a two-way cycle track at the northern edge of Culebra from I-10 out to Callaghan (Segments A and B) would be ideal - which would allow for greater connectivity to Woodlawn Lake and direct connection to St. Mary's University. He suggested that since there are no driveway approaches on Culebra Road along Section C, a cycle track should have a painted crossing at Culebra and Callaghan. He pointed out that it would help to have painted and separated signalized intersections along each of these segments. He concluded with mentioning that his main interaction with Culebra Road is as a transit user so he would love to see a great interaction with transit and cycling users with adding a cycle track bending around a bus stop.

Bryan Martin, Bike San Antonio, mentioned that the Study Team may not get an accurate representation of the community from the surveys due to numerous factors and to possibly find other ways to get feedback. He said he is very concerned with bus stops and bike lanes because the bike lanes go through the bus stops, and it becomes a safety issue.

Joe Pawlik, Active Transportation Planner with AAMPO, said to possibly show before and after images of what the study team is envisioning for certain corridors. He is hoping for efficient intersection improvements for bicycle infrastructure and said the Study Team needs to ensure the public knows

David Bemporad, Activate SA, mentioned he would send an email with more ideas for components on

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Culebra Road Transportation Study Bicycle/Pedestrian Groups Stakeholder Meeting June 8, 2021, from 6:30 – 7:30 p.m.

PARTICIPANTS

- Celina Escamilla, University Park NA
- Art, University Park NA
- Julio Ramos, WSP
- Mukul Malhotra, MIG
- Krystin Ramirez, MIG
- Lianna Ybarra, PCI

MEETING SUMMARY

The meeting began with introductions of the Study Team, participants, and an overview of the Culebra Road Transportation Study given by Julio Ramos, with WSP. Mr. Ramos went over the goals of the project and then passed it off to Krystin Ramirez, WSP. Ms. Ramirez provided a presentation of the Culebra Road Transportation Study. At the conclusion of the presentation, the meeting was opened up to any questions or feedback regarding the study.

Due to low attendance of the neighborhood association meeting, there were not any pressing questions. Celina Escamilla, University Park NA, asked Ms. Ramirez if she could provide a link to the presentation and survey so she could include it in the meeting minutes and send to everyone in the neighborhood association.

One of the attendees, Art, did mention that Culebra Road needed better safety for bicyclists. He said it is a very dangerous corridor for people who ride their bikes, and most people in cars do not slow down for them – it is the most dangerous in the afternoon during peak traffic time.

Ms. Escamilla then concluded the meeting and said she was going to ask her surrounding neighbors for their thoughts and feedback and said she would send the Study Team a summary.



NTRODUCTIO

STUDY GOAL!

CORRIDOR

APPENDIX A-2

Multimodal Corridor Transportation Study

Appendix A-2: Culebra Road Transportation Study Technical Advisory Group (TAG) Meeting (August 2, 2022)

APPENDIX A-2 | 1

APPENDIX

APPENDIX A-2





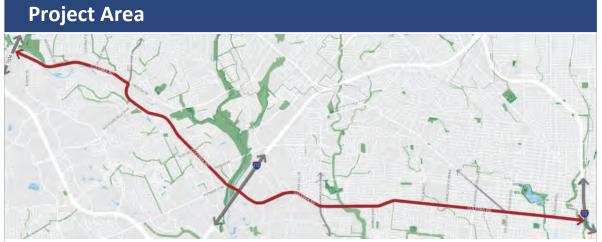
Culebra Road Transportation Study Technical Advisory Group (TAG) Meeting August 2, 2022

Meeting Purpose

- Provide project overview including engagement process with community members, stakeholders and City staff
- Review, discuss, and identify any changes to the proposed streetscape improvement options

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APPENDIX



- Project Limits: Loop 1604 to IH-10
- Project Area Length: Approximately 13 miles long
- Project Location:
 - Within northwest Bexar County, City of San Antonio, Texas
 - Western and eastern ends are part of TxDOT system. On the west, FM 471/Culebra Road extends from LP 1604 to Tezel Road. On the east, Spur 421/Culebra Road extends from NW 24th Street to I-10.

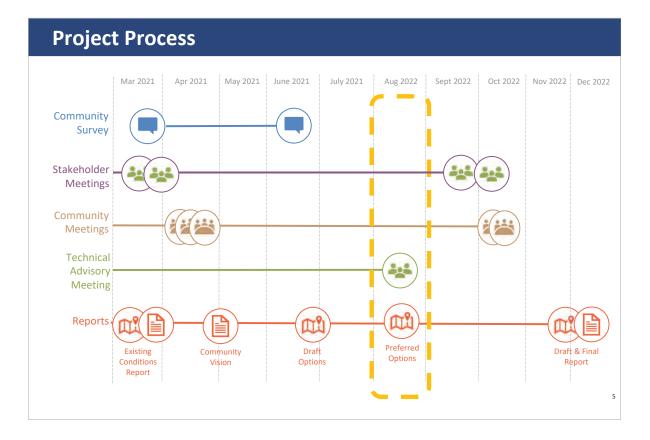
Project Goals

- 1. Create a safe corridor for ALL users of the street
- 2. Transform Culebra into an equitable, multimodal corridor for people who walk, roll, bike, take transit and drive
- 3. Enhance the overall experience and connectivity for pedestrians, bicyclists, transit users and drivers
- 4. Develop conceptual designs for short- and long-term improvements
- 6. Ensure meaningful community and stakeholder engagement
- 7. Coordinate and **build upon previous and concurrent studies** and initiatives









Community Outreach

Multi-Pronged Community Engagement Included:

- 3-month long in-person and virtual Community Visioning Survey
- 5 Focus Group Stakeholder Meetings with neighborhood associations, pedestrian and bike advocacy groups and institutions
- 3 **Community Workshops** in English and Spanish
- NEARLY 600 ACTIVE PARTICIPANTS who attended stakeholder and community meetings and took the community survey
- OVER 2000 CLICKS to multi-media outreach including email blasts, social media posts, newspaper and social media ads, media coverage and project website

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APPENDIX

APPENDIX A-2

Key Issues and Challenges: Pedestrian & Bicycle





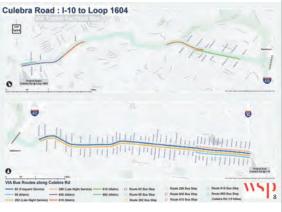




Key Issues and Challenges: Transit Facilities



- **Transit Routes:** Project area served by four routes: 82, 88, 610, and 618
- Ridership: Average combined daily weekday ridership for all routes in April 2022 was 3,519. Ridership on individual routes:
 - 82 -1,315 riders
 - 88 -1,674 riders
- **610** 280 riders
- 618 250 riders

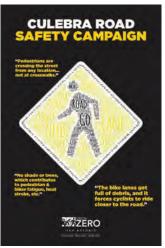






Culebra Safety Campaign

- Vision Zero SA is committed to bringing education and information to help prevent deaths along Culebra by focusing attention on drivers and pedestrians along this corridor.
- Data Collection Ends September: Link to draft report: https://publicinput.com/Report/u2dgb3tp4yx
- Publish Results in October: Results will be incorporated into the final Culebra Corridor Study Report







PROPOSED OPTIONS & DISCUSSIONS

Overarching Improvements

Individual Segment Prototypical Cross Sections

Overarching Improvements

Overarching Design Principles:

- Improve Overall Safety & Connectivity for all Modes of Travel
- Repurpose Any Excess Right-of-Way to better serve people who walk, bike and take transit
- Customize solutions to respond to surrounding context like land use, drainage, etc
- Explore long-term high-cost concepts to build on short-term, low-cost, improvements
- Build on best practices including, City of San Antonio, NACTO, etc



Short-Term, Low-Cost, Improvements May Include:

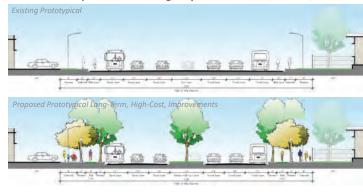
- Separated bike facilities
- Wider sidewalks
- Raised median
- Landscaping

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Overarching Improvements

Overarching Design Principles:

- Improve Overall Safety & Connectivity for all Modes of Travel
- Repurpose Any Excess Right-of-Way to better serve people who walk, bike and take transit
- Customize solutions to respond to surrounding context like land use, drainage, etc
- Explore long-term high-cost concepts to build on short-term, low-cost, improvements
- Build on best practices including, City of San Antonio, NACTO, etc

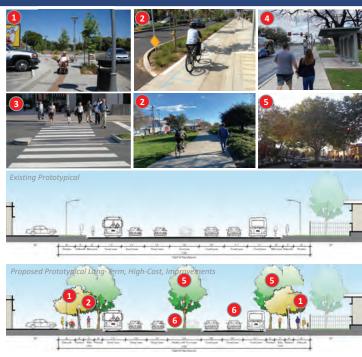


Long-Term, High-Cost, Improvements May Include Short-Term Improvements and:

- Moving the existing curb to shorten crosswalks
- Elevated and protected bike facilities
- Lighting and additional landscaping

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Overarching Improvements



Common Improvements:

- Wider & Protected Sidewalks:
- Dedicated Bike Facilities
 - Protected Class IV bike facilities pref.
 - Buffered bike lanes & shared ped/bike facilities for other situations

3 Improved Crosswalk Connections

- Improve existing crosswalks all along segment with shorter distances, longer crosswalk timings, and pedestrian refuge where possible
- Explore new crosswalks specially to existing schools, trail heads, community destinations, etc.
- Improved Transit Facilities
- **5** Shade Providing Trees
 - Along sidewalks and medians

6 Multi-purpose Traffic Calming Devices

 Explore mix of solutions to include access management, medians, trees, travel lane widths, etc.

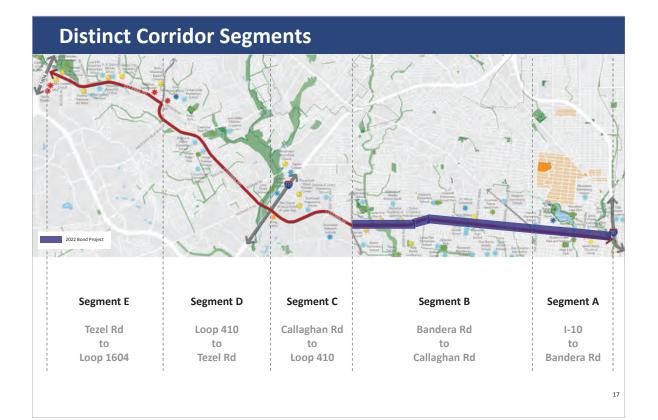
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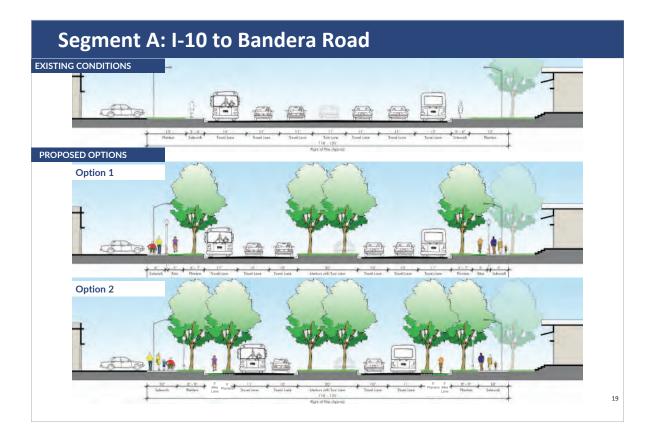


PROPOSED CONCEPTS & DISCUSSIONS

Overarching Improvements
Individual Segment Prototypical Cross Sections







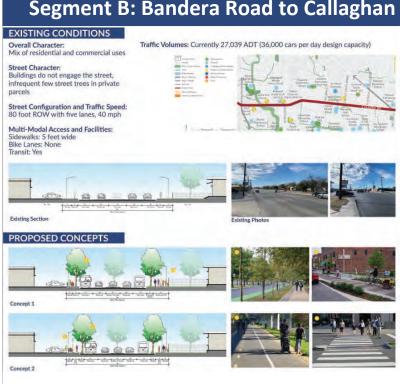




Segment A: I-10 to Bandera Road



Segment B: Bandera Road to Callaghan Road



COMMUNITY FEEDBACK:

Wider & Protected Sidewalks:

- 76% support overall;
- 65% support for wider sidewalks for business

Dedicated Bike Facilities:

• 76% support for protected

Improved Crosswalk Connection:

- Improve existing crosswalks all along segment, especially at 36th/Esmeralda, McMullen and Callaghan
- New crosswalks between Maiden Lane and Yolanda, Callaghan and

Improved Transit Facilities:

- Improve all along the corridor,
- Enhance connectivity to St Marys' University from surrounding neighborhoods

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APPENDIX A-2

Segment B: Bandera Road to Callaghan Road **EXISTING CONDITIONS** PROPOSED OPTIONS Option 1 Option 2

Segment B: Bandera Road to Callaghan Road



Segment B: Bandera Road to Callaghan Road



Segment B: Bandera Road to Callaghan Road



NEXT STEPS

- October: Final Public Meeting
 - -Report out TAG recommended concept
 - -Receive feedback and input from residents
- October November: Compile results and develop final report
- **December:** Publish results to Council and residents
 - -Provide final report to appropriate departments and agencies for incorporation into future infrastructure projects.

8.



Culebra Road Transportation Study
Technical Advisory Group (TAG) Meeting
August 2, 2022

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NTRODUCTIO

STUDY GOAL

CORRIDOR

CORRIDOR

TRATEGIES & TOOLS

APPENDIX A-3

Multimodal Corridor Transportation Study

Appendix A-3: Culebra Road Transportation Study Documentation of October 2022 Community Open Houses (November 15, 2022)

APPENDIX A-3 | 1



CULEBRA ROAD TRANSPORTATION STUDY

Documentation of October 2022 Community Open Houses

Draft date 11/15/2022

OVERVIEW	In October 2022, the City of San Antonio Transportation Department held the second series of meetings for the proposed Culebra Road Transportation Study. This study is looking at improvements to Culebra Road from Loop 1604 to I-10, a length of approximately 13 miles in Council Districts 1, 5, 6, and 7 in San Antonio, Texas. The following documents the October 2022 outreach. In total, the City had 38 participants in person and 1,231 participants online with a total of 33 commenters. Further details below.						
MEETING	In Person Meeting	In Person Meeting	Virtual Option				
MEETING DATE AND TIME	Oct. 1, 2022 9 am to 11 am	Oct. 4, 2022 6 pm to 8 pm	Oct. 1-Oct. 19, 2022 Beginning at 9 am				
MEETING LOCATIONS	Holy Cross High School – Convocation Center 426 N. San Felipe Street San Antonio, TX 78228 Inside Loop 410, District 5	Alamo City Apostolic Church 9302 Timber Path San Antonio, TX 78250 Outside Loop 410, District 6	culebraroadstudy.org				
TRANSLATION SERVICES	Spanish interpreters were available and materials were provided in Spanish	Spanish interpreters were available and materials were provided in Spanish	A Spanish version of the virtual public meeting webpage with Spanish materials was available.				
PRESENTERS	A pre-recorded presentation with audio and visual components	A pre-recorded presentation with audio and visual components	A pre-recorded presentation with audio and visual components				
ELECTED OFFICIALS IN ATTENDANCE	5	0	0				
TOTAL NUMBER OF ATTENDEES (APPROX.)	12	21	959 Virtual Meeting Page Unique Visitors and 212 Online Participants on SA Speak Up between Oct. 1-Oct. 19, 2022				
TOTAL NUMBER OF COMMENTERS	6	4	14				

DOCUMENTATION CONTENTS

- A. Summary of comments received
- B. Notices provided
- C. Comments received by Oct. 19, 2022
- D. Figures
- E. Additional comments received between Oct. 20-Oct. 31, 2022

A. Summary of Comments Received

Comments pertaining to the Community Open Houses must have been postmarked by Oct. 19, 2022. 33 comments were received. Comments received after this deadline are still being received and evaluated but are not reflected in the summary analysis.

The following is (1) a summary based on comments received as well as the written feedback provided during the in house open houses that were written on post it notes and placed on the exhibits and roll plots (see section C for details), and (2) a matrix of all comments received. The original comments can be found in section C.

MAJOR THEMES:

- NOTEMES:

 Overall support for proposed improvements overall and the proposed concepts presented at the meeting.

 Five top favored elements that were proposed:

 o (1) Adding trees to the corridor,

 (2) Adding separated/protected pedestrian and bicycle paths,

 o (3) Adding street plining across all segments, and

 o (5) Adding street plining across all segments, and

 o (5) Adding street plining across all segments, and

- o (5) Adding safety measures as safety was noted by many as a major need.
 Other elements that were supported:
 Beautifying the corridor,
 Reducing lane widths,
 Reducing lane widths,
 Reducing the number of travel lanes,
 Improving the road pavement (e.g., dips and potholes, consider PFC asphall),
 Removing center turn lane where appropriate,
 Adding stormwater planters in comparison to general planters, and
 Considering tree maintenance needs (e.g., watering, removal of dead trees)
 In comparison, there were a very small number of comments in opposition, and they centered on concerns about reducing the number of travel lanes. Other concerns included: new trees at intersection blocking views of the traveling public concerns included: new trees at intersection blocking views of the traveling public and causing maintenance hassles; concrete islands potentially being a hazard for cars; support for increasing speed rather than reducing it; and traffic noise concerns for adjacent neighborhoods.

SPECIFIC DESIGN CONSIDERATIONS:

- - Add bus pull outs (across all segments but noted especially in Segment D).

 Add privacy wooden fences between homes and new bicycle/pedestrian
 - facilities.

 o Keep sidewalk and bike lanes at grade with each other as it helps with
 - those getting off bus to access these facilities.

 - traffic acces
 - Laven and Culebra Road need intersection break to support access and U-

Culebra Road Transportation Study | Community Open Houses Documentation

- U-Turn access needs:

 o Consider U-turn access at El Centro and Culebra Road to support
- · Crosswalk needed
- Crosswalk needed

 Traffic light on Laven for pedestrian crossings needed.

 Segment E Specific:

 Consider adding turn lane or traffic light from EB Culebra Road to turn into Bank of America and Wells Fargo.

 Consider widening Segment E to three lanes in each direction and route bicycle/pedestrian traffic to Timber Path instead.
- Trail considerations

 o Consider adding trailhead at the lot by El Centro and Culebra Road near
- Zarzamora Creek.

 o Support for Zarzamora Creek south of Culebra Road to be parkland.
- Mobility improvements:
 o Improve mobility at Culebra Road and I-10 with longer left turn signal timings or improved on ramps.

CONSIDERATIONS ABOUT THE PROPOSED CONCEPTS PER SEGMENT:
There were not a lot of comments with preferences about one concept or another. Two
considerations for designers were:

• (a) the wish for continuity and equity across segments and

- (a) the wish for continuous,
 (b) symmetry where possible.

The following are specifics to each segment.

- Segment A:

 o Two comments in support of concept 1.
- - One comment in support of concept 1, three comments in support of concept 2.
- Segment C1: Two comments in support of concept 1, one comment in support of concept
- Segment C2:

 o One comment in support of concept 1, two comments in support of concept 2.
- - Segment D: One comment in support of concept 1, two comments in support of concept

- Segment E:
 - One comment in support of concept 1; three comments in support of concept 2.

OTHER COMMENTS

- Concerns regarding construction impacts (e.g., traffic flow, business access, safety).
- Concerns regarding financing for proposed improvements Coordination needed with adjacent projects/support for adjacent projects
 - Coordinate Culebra Road bike/ped improvements with Bandera Road bike/ped improvements so that it's continuous/consistent.
 Be complementary with the Westside creeks investments.

Culebra Road Transportation Study | Community Open Houses Documentation

- Zarzamora Creek Trail access at bridge needed.
 Improve existing trail between Grissom Road and Timberwilde by adding additional access to neighborhoods, call boxes and security cameras for

- additional access to neighborhoods, call boxes and security cameras for safety concerns.

 Improve bicycle/pedestrian facilities along Timber Path.

 Improve safety at the intersections of Culebra Road and Bandera Road, and Culebra Road and Loop 1604.

 Connect Culebra Road with Woodlawn Lake or the Greenway of Alazan Creek via new bicycle lanes

 Unrelated to scope of study:

 Concerns regarding city code compliance and enforcement including for those who abuse the center lane as an additional lane during congested times.

 Desire to move utility poles in the corridor underground.

 Drainage concerns.

 Consider removing traffic intersections for traffic circles/roundabouts.

 Need for traffic signals to be synchronized and have enhanced camera technology.
- Need to fr taffic signals to be synchronized and have enhanced came technology.
 Comments regarding VIA Metropolitan Transit
 Consider adding dedicated bus lanes.
 Need better bus stops.
 Need lighting at bus shelters.
 Need maintenance for lighting outages at bus shelters/stops.
 Appreciation for community open houses.

Culebra Road Transportation Study | Community Open Houses Documentation

Comment #	Date Received	Name	Type of Comment	What do you like about the proposed concepts?	What improvements are most important to you?	Other comments?
1	Sept. 20	Matthew T. Rodriguez	Email Comment			I began commuting to work last week. Leading up to my decision, I biked various routes on the weekends in order to identify an optimal way to get from my home (New Guilbeau/Tezel) to work (Culebrar/Detraco). I regularly find myself torn between two routes, which I call "The Direct Route" and "The Scenic Route". My direct route places me onto the Tezel/Culebra bike lane the entire way. Tezel does not concern me much, because there is just not that much traffic on my route (except for one school zone, but not an issue if I leave at the correct time). However, Culebra always has me worried. The bike lane disappears completely on the overpass once I cross over from Tezel to Culebra, which leaves me somewhat vulnerable, in the main lane for about 50 yards as I approach Westover Hills. Even when the bike lane reappears, it is hardly visible to motorists for the entire duration of my ride down Culebra. And to make matters worse, there is lots of debris in the lane, which is mainly gravel/rocks, but also has quite a bit of broken bike parts, screws, bits of plastic, etc, which makes me very nervous because it gives the appearance that bikers have attempted this road before me, only to be wiped out during their ride. The lane also needs a lot of sweeping, because in some areas the gravel is so bad that I am riding on the line that separates the driving lane and the bike lane. Due to the rough condition of the Culebra bike lane, I sometimes opt to take the scenic route when time is not a concern. This route utilizes the Greenways, which start at Tezel/Timber Lodge, goes through Cathedral Rock Park, and down towards the VIA transportation center where I can pop out at Potranco. This, however, adds almost 3 miles to my commute, not to mention many hills, twists, and turns that slow me down. I plan to attend the community open house, but just in case I cannol, I'd like to recommend that we begin be a dead of repainting from Tezel to 410 (possibly all the way to 1-10, but I never go that far down Culebra). In addition, the

Comment #	Date Received	Name	Type of Comment	What do you like about the proposed concepts?	What improvements are most important to you?	Other comments?
2	Sept. 23	Matt	Email Comment			We need to do better to protect bicyclists. https://www.ksat.com/news/local/2022/09/22/bicyclist-hit-killed-in-hit-and-run-crash-on-far-west-side-police-say/
3	Sept. 24	Hilda Ochoa	Email Comment			Hive at Coppertree Condos. Some of the homes, including mine, are close to Culebra Road. Traffic speed is ridiculous and it seems that its a disaster waiting to happen. I would like to see the City install a high concrete fence or some type of noise/safety barrier on the Culebra side to help lessen traffic noise and potential danger.
4	Sept. 27	Josiah	Email Comment			We want less car lanes , it brings a lot of noise and car shows to the neighborhoods creating dangerous environments. We want safer bike lanes , safer sidewalks and high density living not just offices
5	Oct. 1	Juan Varela	Email Comment			I would like more information.
6	Oct. 1	Ofelia Sanchez	Written Comment	The proposal looks like a great idea to improve the community but unless code compliance comes to uphold property owners who are hoarders and create unsanitary conditions like rat infestations that infestation affects nieghbors [sic] (Ace Ice House) wrecked vehicles and trash, useless dump.	Code compliance needs to inforce [sic] the no dumping in this area N. Gen McMullen and Culebra. The camera are not working, too many accidents. Placing trash deposits every comer. Sidewalks on W. Laurel needed. Speed bumps are also needed.	Project seems good for improvements.
7	Oct. 1	Jan Wells	Written Comment	Good people at the meeting & plenty of people who were knowledgeable	1604 & Culebra intersection area – driver/ pedestrian safety and rage problem	Section E should be combined with an F
8	Oct. 1	Edward F. Granado	Written Comment	Improving the streets, sidewalks, beautifying. Bike lanes on or close to sidewalk.	Street repair and sidewalks. Improving business, more lighting. Do not have bike lanes on street. We need to do something about homeless, hookers, drugies [sic], and crime in the area.	
9	Oct. 1	Jerid Morris	Written Comment	Protected ped/ bike lanes & proposed green canopy improvements	Decrease auto capacity & incentivize pedestrian/ bike traffic	Defund public safety budgets & use funds for meaningful, generational quality of life improvements
10	Oct. 1	Michael Stumpff	Written Comment	Focus on flora (?) & dedicated bike lanes & walk lanes and narrowed 10' lanes to slow down traffic.	Slowing traffic & reducing cut thru neighorhood [sic]	
11	Oct.1	Stacey Sinclair	Written Comment	Separate ped/ bike facilities - Reduced lanes/ lane widths - Trees in median - Enhanced safety	Safe multimodal facilities (w/ separation) Trees/ vegetation (to combat heat issues??) Providing options for safe travel (reduce SOV dependency)	Any considerations for dedicated bus lanes?
12	Oct. 1	Matthew Baiza	Online Comment	Overall, I think the proposed concepts will improve safety and mobility along the corridor for all modes of transportation. Regarding Segment A, I prefer Concept 1 which will create a wider buffer between vehicular traffic and cyclists. In addition, Concept 1 looks to keep the sidewalk and bike lane at grade with each other, which will be easier for those getting off the bus, especially for those with disabilities and/or mobility challenges. Regarding Segment B, I prefer Concept 1 because it similarly provides a wider buffer between vehicular traffic and cyclists. Regarding Segment C1, I prefer Concept 2 with the Greenway and storm planters.	Safety improvements, particularly ensuring there are wide barriers between whichual traffe and cyolists as well as adequate crosswalks and shading for pedestrians. In addition, improvements should not just look like a concrete jungle - I hope to see color and art integrated into bus shelters and other features.	This roadway is one of the most dangerous and deadliest in our city and we should ensure that these improvements make it safer for all.

APPENDIX

Comment #	Date Received	Name	Type of Comment	What do you like about the proposed concepts?	What improvements are most important to you?	Other comments?
				Regarding Segment C2, I prefer Concept 2 to be in line with Concept 2 in Segment C1 where the bike lane stays on one side of the street. Regarding Segment D, I strongly prefer Concept 2 as it keeps the bike lane at grade with the sidewalk. Traffic in this section is fast and there should be as much of a barrier as possible between vehicular traffic and cyclists. In addition, with the sidewalk and bike lane being at grade with each other, it should be easier for those getting off the bus, especially those with disabilities and/or mobility challenges. Regarding Segment E, I prefer Concept 2 which keeps the bike lane and sidewalk adiacent to each other.		
13	Oct. 2	Robert Perez	Online Comment	I guess the trees. But they do not address what will fix the common issues with this road with road delays.	Reduction in commute times by car and fixing the quality of the road, too many dips, and potholes.	No. Just no. None of the proposed concepts address key issues. Pedestrian traffic to cross needs to be segregated either by a bridge or tunnel for bikes and wheelchairs. They will help troubleshoot jaywalkers. If they must pass I would look into fencing to prevent jaywalkers. Secondly, no speed REDUCTION and/or lane reduction is acceptable. If anything the speed limit needs to be increased to 45-50mph. That is a comfortable cruising speed for this road. Third, Buses need a dedicated pull-out so as to not cause congestion. That is all they do currently, get in the way and cause frustration and accidents. Forth Renforce [sic] the road for the heavy weight of said via buses and school buses. They have caused dips and massive potholes that damage personal property. Effit is section A when going towards I 10 you have jerks that cause traffic by taking the center lane to cut off other drivers to either go left or right causing massive traffic and road rage. This section needs a solid barrier to prevent that to make traffic commit to that lane be it to go straight or turn left or right. That is my biggest issue. Enforcement should be present to send a message that this will not be tolerated, however, they should not shoot radar/laser to make a quick buck. In section C there is also a massive collection of traffic going towards 410 by the firehouse and police substation. This section should be widened to 3 lanes to promote laminar flow.
14	Oct. 3	Sean Tompkins	Online Comment	I am greatly in favor of protected bike and pedestrian lanes. I appreciate that designs have been created to not reduce the number of lanes of traffic while still incorporating these changes.	Bike lanes protected from traffic	
15	Oct. 3	Jesus Lopez	Online Comment	I like the inclusion of protected bicycle lane concepts across all segments. I also like the reduction of lanes as a traffic calming measure. And the addition of trees is favorable for the aesthetics and reduction of the heat island effect.	The most important improvements to me are the separate bicycle lanes, wider sidewalks, frequent pedestrian street crossings, and pedestrian islands at intersections.	I support the concepts with separate bicycle lanes on both sides of the street to establish a continuous bicycle corridor along Bandera. Mixing different implementations of bike infrastructure could interfere with the ease of use along all of Bandera.
16	Oct. 4	Vicki Larkins	Written Comment	I do like safe sidewalks and bicyle [sic] lanes where they aren't in the road.	To widen Culebra please safe turn lanes and more road to travel, I am on the road at least an hour to go home from Church and errands	Thank you for everything

Comment #	Date Received	Name	Type of Comment	What do you like about the proposed concepts?	What improvements are most important to you?	Other comments?
17	Oct. 4	Gerilynn Lee	Written Comment	I like the concepts of space from sidewalks & road	Cost efficient and safety	Who pays for this after all is said & done? (actually) - How will you help manage traffic during this? - What about the Businesses? Safety for everyone during the work?
18	Oct. 4	Nathan Alvarado	Written Comment	Trees good, safer crosswalks good	More pedestrian crossings between intersections	Bike lanes not a priority, better walking path yes
19	Oct. 4	Cathy Stein	Written Comment	Protected bike lanes		Require the construction company to actually work on the project until finished to limit the total time for the project, not tear up the road and leave it for 6 months as they did with the Timber Path extension & bridge years ago.
20	Oct. 4	Greg Singleton	Online Comment	I agree with the increased patrolling of Culebra. Speeding and reckless driving are a severe problem that must be controlled. But the pedestrians and cyclists that we are committing to protect must be patrolled also. They drift and wander across Culebra with no regard of the laws they must uphold.	Lighting in the area inside Loop 410 is a huge problem. Also there are random concrete islands in the middle of the road that are not marked or aren't even necessary. That, combined with the wandering pedestrians and cyclists and the insufficient lightning presents ongoing hazards every evening	have lived in San Antonio for 70 years, always on the northwest quadrant of the city. My first home was just off Culebra in the University Park neighborhood, when I was just born. I attended St. Mary's University and St Mary's Law School from 1971 to 1976. In 1979, my wife and I moved in the Timber Ridge area, off Culebra, between 410 and 1604. We lived there 18 years, when we moved to our current home, near Sea World. My son and his family recently renovated a home and live there, two blocks from St. Mary's, and one block off Culebra. Recently while visiting them there one night, a speeding vehicle on Culebra out me off, and I swerved to prevent getting hit. I didn't see the unmarked curb/sland in the middle of the street, and hit it, severely damaging my car. My son had hit the same curb/sland also, and blew a tire. When the repair facility on Culebra picked up my car, he told me the fisic) he, himself, had hit that island before, and many people came to his shop there on Culebra after the same type of incident.
21	Oct. 4	Laura Parker	Online Comment	Tiove the protected bike lanes and treed sidewalks!	Providing safe paths for non-drivers. So many people in the city do not have cars. For too long the city has been designed around cars, pedestrians are just as, if not more important than drivers when it comes to planning safe roads. Roads are not just for cars!	
22	Oct. 4	Brian Mast	Online Comment	Would prefer to see stormwater planters as opposed to general planters throughout the project as much as possible. Also, where trees are being planted, it would be good if they were implemented through tree wells so they could infiltrate stormwater as well.	Safety is most important followed by improving the general environment through the corridor. Between federal and local investments the westside creeks are poised to see an approximately \$224 million investment to improve the creek ecosystems. These road improvements can be done with that investment in mind so both the projects complement one another.	
23	Oct. 4	Natalie Dabbs	Online Comment	I like all the separated bike lanes and the added greenery. It's too scary to bike any place other than greenways and cars don't care about bike lanes.	I think caring about adding greenery is important since all the concrete is ugly.	

APPENDIX

Comment #	Date Received	Name	Type of Comment	What do you like about the proposed concepts?	What improvements are most important to you?	Other comments?
24	Oct. 5	Melissa Reyna	Online Comment	I like wider sidewalks and that the streets will be lined with greenery.	Safety of crosswalks. Maintenance of traffic lights and re paved streets removing potholes	Would like to see more trash bins, better bus stops, water fountains for pedestrians. Also, some of the strip centers need work to upgrade exterior buildings, graffiti control, city planning of what new business are added to street. Ex. Too many car washes in a small area
25	Oct. 6	Jacob Reyes	Online Comment	I like the addition of BUFFERED bike lanes (shared use paths are also appreciated when a buffered bike path can't be made)	BIKE LANES	This 13-mile corridor is ESSENTIAL for me, as I ride my bicycle to downtown frequently from the Great Northwest neighborhood. Please let this aspect come to fruition!! I beg you!!!
26	Oct. 6	Austin Kelm	Online Comment	Downsizing lanes from 3 lanes to 2. Adding PROTECTED bike lanes throughout the project. Adding landscaping and a median.	All bike lanes need to be PROTECTED bike lanes. A bike lane without protection isn't actually a bike lane at all. Shrinking the lane size down is also probably the most influential portion of this project.	Please make sure that the median is landscaped w/ NATIVE plants that are drought resistant and do not take much water. Also, landscaping does little to slow traffic. Big, tail, mature trees do more than anything else to slow down traffic. Please make sure you put as many as possible on every side of the roadway and median. When building the bike lane, EVERY inch of it needs to be protected. A non-protected bike lane is a dead cyclist. It also needs to be a solid curb, bollard, or something similar. A car should not be able to drive through it, even if it damages the car. Drivers are inside a glant steel box with protection all around them and bikers/pedestrians are not. They are the ones that need protection. Lastly, there are not nearly enough cross walks and protected intersections here. There need to be far more options for SAFELY crossing the street, and each crossing needs to have protection and prioritize the timing of the pedestrian/cyclist. No buttons that do nothing or waiting until the light cycles. They need to go IMMEDIATELY for bikers/pedestrians as they are the more efficient form of transportation. Please make sure that any crossing is also protected. This means a raised crossing and speed table at EVERY crossing and intersection, not just some. Lastly, please consider doing away with the traffic lights. They are one of the most inefficient forms of traffic routing. Traffic circles and its many variations would be much better for this area, allowing for more consistent and sustainable traffic to pass strough the area.
27	Oct. 10	Mark Anthony Rivera	Online Comment	The inclusion of shade throughout the corridor. It is a feature that mitigates the utrahan heat island effect and provides comfort for people walking.	The addition of consistent, protected bike facilities throughout the entire corridor.	I think it is contradictory to the vision zero goals of San Antonio to ad slip lanes along the corridor. They create confusion about who has the right of way and encourage drivers to speed through intersections, making the area more dangerous for all users.

Comment #	Date Received	Name	Type of Comment	What do you like about the proposed concepts?	What improvements are most important to you?	Other comments?
28	Oct. 10	Maria Vinton	YouTube Comment			Hello i live off culebra. my backyard backs up to culebra st. i live in the westover elms subd. my concerned is speeding. cars have been crashing into the fences of not only my subd but others in the area. I don't know how that can be resolved but i would like a resolution. I would like a metal ramp installed to protect our homes from getting run into by speeding vehicles. as for the stretch bwth tezel & 1604 i would like to see a third lane & extended turn lanes would help the traffic flow easier. trees & extended mediam are not the solutionwaste of taxpayer money. sure it would look beautiffl but tits not the solution. in have been living in the area for at least 20years i believe i have better input then someone who hasnt. traffic is very heavy in the area during busy timesdue to schools &business in the area4lanes in that area is not enoughbwn rogers 41604 is also a busy area & dangerous. we need more easy turn in lanes for businesses it is very dangerous therealways someone that gets inpatient and causes accidentstherefore traffic builts upsomeone needs to evaluate that area like vesterday pleases.
29	Oct. 10	Marie	Email Comment			evaluate that area ince yesteroday piesase 1) go up, taxpayers can't afford government to buy all the property and companies it would have to buy to expand road, so go up with entry (exit lanes at intervals which would give drivers less traffic lights. 2) you want shade and bike lanes, go up double decker. This will give shade and protection to bikers. Leave side walks alone and don't be causing More traffic problems by planting trees. Trees need maintenance, are you going to close Culebra down when the trees need maintenance? Not to mention storm/ice damage to limbs that can cause accidents. 3) law requires bikes, For Their Safety, follow same laws as vehicles. So having them travel the wrong way in traffic is a problem you can't handle. We already have hundreds of bikers who Don't follow the rules of the road, and their accidents are blamed on car drivers not the bad bicyclists! 4) also do you really believe going from three lanes to two, on such a buys street is a good idea. 5) do Not allow for any more commercial or residential buildings, that increase traffic, to be built on Culebra. The Idea you can continue to push more traffic onto the roads without tragic consequences is strange for a city that wants to decrease accidents. There is nothing that says every scrap of land Must have a building, for any use, built on it. Also for future road expansion push any buildings further back from street entry. A double decker might, and it might not, cost more but it beats the process you have now. Every road, crossing major highways, will be better served with double deckers, better commercial and residential planning to limit excess traffic on existing roads, and controlled traffic movement that may not be in a straight line. So if you are looking for a better direction then double deckers otherwise once completed your system will be obsolete and you will have to find another way to make traffic work for salt can only hope that you will rethink

B. Notices Provided

ELECTED OFFICIAL LETTER

On Aug. 31, 2022, elected officials were mailed a letter inviting them to participate in the Community Open Houses. The letter also included a copy of the bilingual flyer. The mailing list and an example copy of the letter packet are included below.

An emailed version of the letter was sent on Sept. 22, 2022, and an example copy

Elected Officials Sent Meeting Notice

Representing	First Name	Last Name	Title
San Antonio	Ron	Nirenberg	Mayor
San Antonio	Zack	Lyke	Chief of Staff, Mayor
San Antonio	Mario	Bravo	Councilmember, District 1
San Antonio	Mario	Llano	Chief of Staff, Councilmember, District 1
San Antonio	Teri	Castillo	Councilmember, District 5
San Antonio	Katy	Bravenec	Chief of Staff, Councilmember, District 5
San Antonio	Melissa	Cabello Havrda	Councilmember, District 6
San Antonio	Samantha	Hernandez	Chief of Staff, Councilmember, District 6
San Antonio	Ana	Sandoval	Councilmember, District 7
San Antonio	Andrew	Solano	Chief of Staff, Councilmember, District 7
Bexar County	Nelson	Wolff	Judge
Bexar County	Nicole	Erfurth	Chief of Staff, Judge
Bexar County	Justin	Rodriguez	Commissioner, Precinct 2
Bexar County	Francesca	Caballero	Chief of Staff, Commissioner, Precinct 2
Bexar County	Rebeca	Clay-Flores	Commissioner, Precinct 1
Bexar County	Frankie	Gonzales-Wolfe	Chief of Staff, Commissioner, Precinct 1
Bexar County	Tommy	Calvert	Commissioner, Precinct 4
Bexar County	Amy	Putney	Chief of Staff, Commissioner, Precinct 4
State of Texas	Trey	Martinez Fischer	Texas House of Representatives, District 116
State of Texas	Diego	Bernal	Texas House of Representatives, District 123
State of Texas	Ina	Minjarez	Texas House of Representatives, District 124
State of Texas	Ray	Lopez	Texas House of Representatives, District 125
State of Texas	José	Menéndez	Texas Senate, District 26

Culebra Road Transportation Study | Community Open Houses Documentation



August 31, 2022

Culebra Road Transportation Study Community Open House

As part of the project development process for the above-referenced study, the City of San Antonio will be conducting the second Community Open House series this October to present the proposed concepts and to receive public feedback. The City is studying transportation improvements to Culebra Road from Loop 1604 to I-10.

We invite you or a representative from your office to attend the community open houses by either viewing a pre-recorded presentation online or joining us in person at one of the open houses. For your convenience, a copy of the bilingual flyer is enclosed.

The in person community open houses will be held on:

Saturday, October 1, 2022

Tuesday, October 4, 2022 6:00 p.m. to 8:00 p.m. Holy Cross High School – Convocation Center 426 N. San Felipe Street, San Antonio, TX 78228 9302 Timber Path, San Antonio, TX 78250

A virtual option is being provided for individuals who would like to participate online instead of in person. The virtual option will be held via a pre-recorded presentation that will be available starting on Saturday, October 1, 2022, at 9 a.m. through Wednesday, October 19, 2022. To participate in the virtual community open house, please visit www.culebraroadstudy.org at the date and time indicated

Comments may be submitted at the in person meetings, at www.culebraroadstudy.org, or mailed to City of San Antonio Transportation Department c/o Poznecki-Camarillo, 5835 Callaghan Road, Suite 200, San Antonio, Texas 78228. All written comments must be postmarked or received on or before Wednesday, October 19, 2022 to be included in the official record of the community open houses

The previous community open houses series held on this project occurred virtually on April 20 and April 22, 2021. The City has heard from nearly 600 active participants. As a result of the valuable feedback received, our designers are studying the possibility of wider and protected sidewalks, dedicated bicycle facilities, improved crosswalk connections, improved transit facilities, shade-

P.O. Box 839966 • San Antonio, Texas 78283-3966

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providing trees, and multi-purpose traffic-calming devices. At the October 2022 community open houses, participants can see the proposed concepts and provide feedback.

We value your input and look forward to your participation in this critical study. If you have any questions, please feel free to contact Jacob Floyd, Transportation Planning Manager, at or via email at

FW: Open House: Culebra Road Transportation Study (Multi-Modal Corridor Study) Holy Cross High School - Convovation Center 426 N San Felipe Subject: Location

Sat 10/1/2022 9:00 AM Sat 10/1/2022 11:00 AM Start:

Show Time As: Tentative

Organizer: Catherine J Hernandez (Transportation)

Elizabeth.

This is the email invite that was sent to elected officials. For record tracking.

-Deb

-----Original Appointment----From: Catherine J Hernandez (Transportation)

Sent: Thursday, September 22, 2022 2:28 PM

To: Catherine J Hernandez (Transportation); Catherine J Hernandez (Transportation); Tomika Monterville (Transportation); Ana Sandoval (City Council); Mario Llano (City Council); Teri Castillo (City Council); Ane Council); Ana Cather (City Council); Ana Cather (Cit Teena Bailey (Transportation); Debora Gonzalez (Transportation); Melissa Cabello Havrda (City Council)
Cc: Melissa Ramirez (DSD); Adrian De Anda (City Council); Isaac Fellows (City Council)
Subject: Open House: Culebra Road Transportation Study (Multi-Modal Corridor Study)
When: Saturday, October 1, 2022 9:00 AM-11:00 AM (UTC-06:00) Central Time (US & Canada).

Where: Holy Cross High School - Convovation Center 426 N San Felipe

Good afternoon! Sending you this so it is on your calendars if you can drop by!

We invite you to attend the second round of Community Open House series for the Culebra Road Transportation Study from Loop 1604 to I-10. Whether you drive, walk, bike, or ride transit on Culebra Road, the City of San Antonio wants to hear from you!

Virtual Option
Participate online by visiting www.culebrasroadstudy.org from Saturday, October 1, 2022, through Wednesday, October 1, 2022, through Wednesday, October 1, 2022, through Wednesday, October 2, 2022, through Wednesday, October 3, 2022, through Wednesday, Oct 19, 2022. Identical information will be available at both in person options and the online option

The community open houses will be conducted in English and Spanish. Please contact us if you need special communication or accommodation arrangements. For more information, please view the attached flyer, visit www.culebraroadstudy.org or call Jacob Floyd at

Thank you.

The Culebra Road Corridor Study Team

Transportation Department 114 W. Commerce St. 9th Floor | San Antonio, TX 78205 O: 210.207.8085 www.sanantonio.gov/transportation www.visionzerosa.com CITY OF SAN ANTONIO TRANSPORTATION DEPARTMENT

Senior Transportation Planner

Elizabeth Story

FW: Open House: Culebra Road Transportation Study (Multi-Modal Corridor Study) Alamo City Apostolic Church 9302 Timber Path

Tue 10/4/2022 6:00 PM Start: End: Show Time As: Tue 10/4/2022 8:00 PM Tentative

Recurrence:

Organizer: Catherine J Hernandez (Transportation)

Elizabeth,

This is the email invite that was sent to elected officials. For record tracking.

-Deb

---Original Appointment---

-----Urginal Appointment ---From: Catherine J Hernandez (Transportation)
Sent: Thursday, September 22, 2022 2:31 PM
To: Catherine J Hernandez (Transportation); Catherine J Hernandez (Transportation); Tomika Monterville (Transportation); Ana Sandoval (City Council); Mario Llano (City Council); Teri Castillo (City Council); Andrew Solano (City Council); Raty Bravenec (City Council); Samantha Hernandez (City Council); Victor Landa (City Council); Jacob T. Floyd (Transportation); Margarita Hernandez (Transportation); Conger (Transportation); Borrita Jones (Transportation); Teena Bailey (Transportation); Debora Gonzalez (Transportation); Melissa Cabello Havrda (City Council)

Subject: Open House: Culebra Road Transportation Study (Multi-Modal Corridor Study)
When: Tuesday, October 4, 2022 6:00 PM-8:00 PM (UTC-06:00) Central Time (US & Canada).
Where: Alamo City Apostolic Church 9302 Timber Path

Good afternoon! Sending you this so it is on your calendars if you can drop by!

We invite you to attend the second round of Community Open House series for the Culebra Road Transportation Study from Loop 1604 to I-10. Whether you drive, walk, bike, or ride transit on Culebra Road, the City of San Antonio wants to hear from you!

Virtual Option
Participate online by visiting <u>www.culebrasroadstudy.org</u> from Saturday, October 1, 2022, through Wednesday, October 19, 2022. Identical information will be available at both in person options and the online option.

The community open houses will be conducted in English and Spanish. Please contact us if you need special communication or accommodation arrangements. For more information, please view the attached flyer, visit www.culebraroadstudy.org or call Jacob Floyd at

The Culebra Road Corridor Study Team

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Debora Gonzalez
Senior Transportation Planner
Transportation Department
114 W. Commerce St. 9th Floor | San Antonio, TX 78205
0: 210.207.8085

POSTCARD

www.sanantonio.gov/transportation www.visionzerosa.com

CITY OF SAN ANTONIO
TRANSPORTATION
DEPARTMENT

The week of Sept. 12, 2022, property owners and other interested parties were mailed a bilingual community open houses postcard which included public meeting details. Property owner information was obtained from Bexar County. There was a total of 3890 individuals who were mailed the notice. The mailing list and an example copy of the notice packet are included below.

	Representing
0781 Cu	lebra Land Investors Ltd
10673 C	ulebra Road LLC
1100 Ca	llaghan Road LLC
0781 Cu	lebra Land Investors Ltd
10673 C	ulebra Road LLC
1100 Ca	llaghan Road LLC
131 Adm	niral Dr Land Trust
1506 Lau	urel LLC
1507 Na	vidad Land Trust
1515 N T	rinity Land Trust
1803 Cu	lebra Inc
2003 Ep	person Revocable Living Trust
2040 W	Laurel Land Trust II
2610 Los	s Coyotes Diagonal Lp
2647 Cu	lebra LLC
3200 Tin	nber View Drive Co LLC
329 Hen	ry Street Trust
3M1B In	vestments
443 Alici	a Avenue Land Trust
4802 Blu	iff Street LLC
5530 Te	zel LLC
738 N G	eneral Mcmullen Land Tr
7770 Pip	ers LLC
7895 Cu	lebra Re LLC
7940 Pip	ers Creek LLC
7-Eleven	
8839 Cu	lebra LLC
9263 Cu	lebra Road LLC
A Estrad	a Homes LLC
	x Holdings LLC
	quivias Properties LLC
	as Capital LLC
	Broker Inc
AAA Rea	alty of Texas Inc

	roperties LLC
Abel M Ri	
	perties LLC
Abran Per	
	roperties LLC
	Estate Investments LLC
Adolfo and	d Bertha Gutierrez Revocable Trust
	es Investments Ltd
Ahani Ent	erprises LLC
Akaashan	nan LLC
Al Jazira F	Property Lp
Alamo Ma	naged Services LLC
Alco Rent	
Alejandro	Flores Jr Etal
Alessio Le	easing Inc
Alfaro Far	
Alfredo Ga	arza Business LLC
Alice S Tr	
	ez/Gomez Properties LLC
Alvera Ca	pital Investments LLC
	surance Services
	Properties LLC
	Homes 4 Rent Properties Eight LLC
Ameritex I	Homes LLC
	4-2 Borrower LLC
	5-2 Borrower Lp American Homes 4 Rent
5	Solutions LLC
Angelina (C Mares L/E
Anna Mar	ia Cendejas Etal
	Garza Etal
Apache S	elf Storage LLC
Applepine	
ARC DBP	CFBR001 LLC
ARC FES.	ANTX001
Archhisho	p of San Antonio

Archdiocese of San Antonio Inc Fbo Catholic Charities	BRE RC Alamo Ranch Tx Lp
Archland Property II Lp	BRE/LQ Tx Properties LP
Archon Growth & Income-Residential Lp	Breakie Holdings LLC
Arizpe Properties Ltd	Broadway National Bank
Armando & Patsy Gonzalez Rev Tr	Brody Allan J Trust Agreement Etal
Artemio Ortega Etal	Brundage Mini Storages Ltd
Artistic Remodeling LLC	Bubble Bath Properties Culebra LLC
Arturo E Macias Etal	Buffalo-Culebra Business Tr
Asa Fann LLC	Can Holdings LLC
Ashita LLC	Capital Plus Financial LLC
Ashwa Enterprises LLC	Carmelite Fathers of San Antonio Inc
Astha Pc LLC	Carmen E Hart Revocable L/Tr
Atc Sequoia LLC	Casa Del Rey Assembly of God Inc
Atw Investments Inc	Castellanos Construction LLC
Austin Chase Homes Lp	Castellanos Constructions LLC
Autozone Inc	Cemex Construction Materials South LLC
Avg Fitness Txok LLC	Centro Cristiano
AZ Alamo Ranch Development Lp	Cerrillos Inc
Baf Assets LLC	CF.SATX LLC
Bank of America	CG & DG Properties LLC
Bar K Design LLC	Chapa Family Trust
Bardoi Twins Ventures LLC	Chase J P Morgan Corp
Basel Investments Ltd	CHC Doral/Sutton LLC
Bazan Family Holdings Inc	Choe Chung S LLC
Bc Flip LLC	Citi Casa
Belac Inc	City of San Antonio
Belinda O Hernandez Etal	City of San Antonio
Ben Reyna Contracting Inc	City of San Antonio &
Benitez Raul Enterprise Inc	Citywide Metal Recycling
Bernardo Valdez Etal	CNL Funding 2000 A Lp
Betos Collision Inc	Cole Fd Portfolio I LLC
Betty Jeffery Trust	Congregacion Vida Nueva
Bexar County Mental Health & Mental Retardation Center	Copemex Partners Ltd
Bexar Rental Properties Lp	Coppertree Condominiums Owner Association
BGPT Investments Corporation	Costa Brava Otm Harmony Lp
Big Daddy Realty LLC	Cpi/Amherst Sfr Program Owner LLC
Big Diamond Inc	Creh Series LLC- Series 3/9
BLRE Tx Number One LLC	Cristo Centro Del Mundo
Blue Barn Properties LLC - Laguna Rio Series	CRJM Investments LLC-Laguna Rio Series
Bluefish Properties LLC	Crown Haven Homeowners Association Inc
Brambila Management LLC	Crown Meadows SA Partnership Ltd
Brazos De Santos Partners Ltd	Cuallix Homes Inc
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Culebra Affordable Housing Lp	Edgewood ISD
Culebra Center LLC	Elbaci LLC
Culebra Land L P	Elca 8227 LLC
Culebra North LLC	Emmanuel Baptist Church of San Antonio Inc
Culebra Oaks Owners Association, Inc	Esc LLC
Culebra Phillips Mart Inc	Esperanza Peace & Justice Center
Culebra Plaza LLC	Esquivias Properties LLC
Culebra Property LLC	Essin LLC
Culebra Road Properties Ltd	Estate of Adam Reyes
Culebra Square Owners Association Inc	Estate of Agapito C Zambrano
Culebra Vico LLC	Estate of Alfred Paniagua
Cuspid Realty LLC	Estate of Alfred Pete Santos
Custody of Our Lady of	Estate of Alfred S Pierson
CY Properties Texas LLC	Estate of Alicia G Garcia
D & O Developers LLC	Estate of Aucencio De Leon
Dakota South Properties Series LLC	Estate of Benjamin Green Sr
Darya Enterprises LLC	Estate of Clark Herman
Debra Asgari L/E	Estate of Dan Clark
Deer Creek Apartments J V	Estate of Domingo Cortez
Delacerda Investments Inc	Estate of Dora Ramirez
Delia E Lopez L/E	Estate of Dora Rodriguez
Delicious Inc	Estate of Elena Yanez
Dewo LLC	Estate of Elia Barrera
Diana R Salinas Trust	Estate of Elida Cantu
Dis Investments LLC	Estate of Elizabeth M Robalin
Discalced Carmelite Fathers	Estate of Ernest T & Isabel Garay
Discalced Carmelite Fathers	Estate of Fernando Inoyue
Discalced Carmelite Fathers-	Estate of Francisco Sonora Jr
Discalced Carmelite Nuns	Estate of Frank Kypuros
DN & K Properties Series LLC	Estate of Frank Thomas Jr
Dome Properties LLC	Estate of Hermi Salas
Donald M Greer 1989 Rev Trust	Estate of Hilario Gutierrez
Do-Re-Mi Holdings LLC	Estate of Ivy Sue Grissett
Doris K Garrett Rev L/Tr	Estate of John A Granado
Dsam Partners Limited Retirement Plan Roth 401K Plan	Estate of Josie Fernandez
Duncan Family Trust	Estate of Josie R Calvillo
Durissio LLC	Estate of Juan Reynaldo Garza
E M Richards Partnership Lp	Estate of Lillian Spears
EA Partners Ltd	Estate of Lily Herrera
Easterling Culebra Apartments Ltd	Estate of Lucia Rios
Eastgroup Properties Lp	Estate of Manuel & Mary C Guajardo
Eat 2006-022 LLC	Estate of Margaret M Villarreal

Estate of Margarita Garza Murillo	
Estate of Maria Pena	
Estate of Maria R Munoz	
Estate of Mary Rita Trevino	
Estate of Mittie Owens	
Estate of Neomi R Insuriaga	
Estate of Raul Aguilar	
Estate of Rodolfo N Jalomo	
Estate of Roger Lee Stack Etal	
Estate of Ronald Veilleux	
Estate of Severa M Zuniga	
Estate of Wilson Freeman I	
Estrada Salazar Santos L/E	
Eugene C Fuentes Sr L/E	
Eva Helen Fetters L/E	
E-Z Wash Inc	
Fabian Amador & Maria Hernande	z C/S
Fade 2 Black Management LLC	
Federal Home Loan Mortgage Cor	poration
FGIII Enterprise Inc	
Fibrcom Inc	
Firstmark Credit Union	
FJP Realty Lp	
Flamingo-Jones Ltd	
FLCR LLC	
Fountain of Life Mennonite	
Freds Fish Fry Inc	
Free Grace Baptist Church	
Gabriel Alfonso Valadez L/E	
Gallegini Properties LLC	
Garcia Properties Inc	
Garcia's Home Investments LLC	
Gardenia Home Investments LLC	
Gayatri Maa Properties LLC	
Gen2 Texas Properties LLC	
Getty Leasing Inc	
GFR-Hercules Properties LLC	
Global Evangelism Inc	
Global New Millennium Prtnrs Ltd	
Global Signal Acquisition LLC	
Gloria E Padilla Etal	
Gloria Ehrlich Etal	

Golden Arch Ltd Ptshp	1
Goodwill Industries	ł
Grant Holdings LLC & Granjoma Inc	ł
Great Northwest Community	\mathbf{I}
Grissom Trails Owners Assoc Inc	\mathbf{I}
GSN LLC	\mathbf{I}
Guadalupe Sanchez Gomez L/E	1
Guamnitz Inc	1
Guerra Living Trust	1
Guerrero Eloisa F RI/Tr	1
Gulf Latin Amer Assembly God	1
H E B Butt Grocery Co	1
Halle Properties LLC	1
Hank Sully LLC	\mathbf{I}
Hard Investors LLC	\mathbf{I}
Harold & Heather Green Living Trust	$\frac{1}{2}$
Hartman Gordon Family Foundation	$\frac{1}{2}$
Heb Grocery Company Lp	\mathbf{I}
Hendershot Levy LLC	ł
Herrero Properties LLC	ł
Hey Now Properties LLC	\mathbf{I}
Hilario Martinez Et Al C/S	ł
Hispanic Christian Communications Inc	ł
Hollywood Adventures Ltd	ł
Holy Rosary Church	ł
Hosanna Assembly of God San Antonio	ł
Hotchkiss Interests Inc	ł
Housing Auth City of S A	1
Housing Authority of Bexar County	1
HPA Texas Sub 2016-2 LLC	l
HPA Texas Sub 2017-1 MI LLC	l
Huebner Boneem Lp	1
Hutton San Antonio (Zarzamora) St LLC	1
Hutton San Antonio Culebra St LLC	1
Hvh Homes LLC	1
Idea Public Schools	l
Iglesia Centro Cristiano	1
Iglesia Ni Cristo Church of Christ	1
Ikonkar Enterprises Lp	l
Imelda & Mario A Torres Guzman Living Trust	1
Intown Suites Culebra Road	1
Inverterra Holdings LLC	1

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Ipac Properties LLC
Ivest Properties LLC
J & B Bar
J&C Quillian LLC
James A Dick Co
Jayden SRB LLC
Jayne Raymond Properties LLC
JBJ Car Wash Inc
JCLG LLC
JD Belmarez Properties LLC
Jessie & Sylvia Osoria C/S
Jesus G Chavez Etal
Jesusita Rodriguez L/E
Jimmie M Leal Etal
Jimmie Olivarez Et Al
Jma Eagles Nest LLC
Jo Ann Shaw Lewis L/E
Joe Cardenas Etal
Johnny A & Sandra P Castillo Trust
Jose Sierra & Josefa Perez C/S
Joy Star Holdings LLC
Jpmorgan Chase Bank NA
Juarez Living Trust
Kap Capital Investment LLC
Kara West Enterprises Inc
KD Homes 1 LLC
Kimia Realty Inc
Kirat Enterprises LLC
KJ Electric Co Inc
KK&LK Property LLC
Klaymeier Doug Revocable Trust
Klepetko Ronald Francis & Rayla-Jeanne Living Trust
Kosturakis Holdings LLC
Kuiraba Investments LLC
L John & Judith Milam Liv Trust
La Terraza LLC
Labor On Demand Inc
Lambeth Building Company
Lannalex Ventures LLC
Lar Del Ltd
Laredo Gonzalez Auto Parts Ltd
Level-rest II & C II C

	Street Church of Christ
	/ Creekside Ltd
	/ Home Investments LLC
_	Gas Wholesale Services Inc
	ealty Group LLC
LIDL U	IS Operations LLC
	orage Lp
	A Saldana Etal
	Culebra LLC
LJCRE	Ventures LLC
LI & M	Investments LLC
Lobo E	states LLC
Lomah	Offices LLC
Lone S	Star Monarch LLC
Lone S	Star Petroleum Inc
Lopez	Family Living Rev/Tr
Lopez	Ventures Family Ltd
Lucille	Lee L/E
Luz Ap	ostolica Assembly of
маан.	A9 LLC
Mag R	eal Estate Services LLC
Manso	ur Mansour
Margai	ret S Wong Trust
Maria I	Dela0Ruz I L/E
Maria I	H Ortiz L/E
Maria \$	S Jimenez L/E
Maribe	l V Valero Et Al
Markha	am Rayvin Jeannette Revocable Trust 2000
Mary C	Lorea L/E
Mary J	Abdo #1
Mauric	io Rivera L/E
Mayo 8	& Sons Construction Co
Mccom	nbs Family Ltd
Mcdon	alds Real Estate Co
Mcdon	alds Real Estate Company
	alds Real Estate Company
	H Series LLC-Series 5
	ola Garza Family Trust
	a Flores C/S
	Epstein Estate
-	Miller Developers LLC
	a Galindo L/E

Michael Litofsky Etal	Paredez Investments Ltd
Mireles Inc	PC5 Properties LLC
Miriama LLC	PDRE II III IV LLC
Mirta E Valle Etal	Pilgrim Center of Hope
MMM Texas Land Company LLC	Pipers SA Income Partners Lp
Monticello Manor Ltd / Ms 235	PLSP Ltd
Montoya Auto Supply	PNG-ELP LLC
Moonlight Land Trust	Pompa Capital LLC
Moreno Family 2011 Lllp	Potranco Hospitality LLC
Morsa Homes LLC	Progeny VII Partners Ltd
Multi Electric Co	Properties Pluss LLC - Series Z
Nameless Enterprises LLC	Propiedades Management LLC
Nams C Inc	Qirem & Milbis LLC
Nance & Associates	Qt South LLC
National Retail Properties Lp	Queen of Texas LLC
Naysin LLC	Quiktrip Corporation
Neal-Geoghegan Living Trust	R & D Brothers LLC
Net Lease Funding 2005 Lp	R & R Universal Llp
New Millenia Group	R & S Dairy Queens Inc
Newell Properties I LLC	R C P Holdings
Nieves G Canizalez L/E	R Koby Construction LLC
NIRAAJ LLC	Raid 9355 Culebra Road Lp
Noorani Investments Inc	Randolph Brooks Federal Credit Union
North America Car Co	Ranger SA-Tx Lp
North Park Condominiums LLC	Raul S Cantu Trust
Northside ISD	Reaglyn Investments LLC
NSA Property Holdings LLC	Realty Income Tx Prop L P
Nunez Family Living Trust	Redfinnow Borrower LLC
NW Texas Realty Lp	Redwood Estates 2 LLC
O Tara	Rendon Construction Co Inc
Obsvc San An 2 LLC	Rentia Corporation
Odilon & Maria Delarosa C/S	Republic Culebra Market LLC
OIS Investments Inc	Republic Culebra Market LLC
Olson Dale H LLC	Republic Culebra Mkt II LLC
O'Reilly Auto Enterprises LLC	Reserve At Culebra Creek Community Assocaition Inc
Oreilly-Wooten 2000 LLC	Reynaldo Perez Etal
Orn Ltd	Reynolds Dean Trust
Ortiz Mortuary Inc	Reysol Properties LLC
Ortiz Mortuary Inc	Richblo2D LLC 3 Kentucky Glen Series
Packard Enterprise Inc	Riparian Research Coporation
Pal Foundation	RJF - Joe Newton LLC
Panjwani Energy Properties LLC	Rkbe Properties LLC

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Robert Serna C/S	Sendero I Public Facility
Roble Hill LLC	Seven J Properties LLC
Rodcel Group Limited Partnership	Shantilal SSK LLC
Rogelio Castillo Puente Jr C/S	Sharique Properties LLC
Rogers Tanforan Ventures LLC	Sharon Peters Real Estate Inc
Rolando Hernandez Etal	Shiyam Real Estate LLC
Romaniacs Land Trust	Shree Mataji Corporation
Ronald F Reina Etal	Silver Moon Properties LLC
Ronstin Properties LLC	Singh Radhika Management Trust
Rosenstein Family Ltd Prtnrshp	Skipper Beverage Company Inc
Rossmore Enterprises	Smartchoice Builders LLC
RSM Holdings Inc	SMT Land Trust
RT Industries	Socorro Gomez L/E
Rudolfo Gonzales Et Al	Sorrento Plaza I I C
Ruelas Property LLC	Southwest Research
Ruth M Reed Etal	Southwest Research Institute
S J B C Corporation Inc	Southwest Winners Foundation
S N B Ltd Ptsho	Sovran Acquisition Ltd Prtnrshp
SA Creekside Assn Inc	Speedy Stop Food Stores LLC
SA Investments LLC	Spencer Anthony Cash Trust
SA Properties & More LLC	Spirit Master Funding X LLC
Safaiz International Inc	St Marys University of S A
Saikirpa LLC	Stanwich Mortgage Loan Trust A
Salazar Family Prop Mgmnt Tr	State of Texas
Salome Garcia Sanchez L/E	
	State of Texas Dept of Transportation
San Antonio Alternative Housing Corp San Antonio Culebra LLC	State of Texas Transportation Commissions Store Capital Acquisitions LLC
San Antonio Culebra LLC San Antonio Hotels LLC	Surrise America Homes LLC
San Antonio Housing Authority	Super Oz Holdings LLC
San Antonio Housing Facility Corporation San Antonio ISD	T C L Construction Enterprises Inc
San Antonio River Authority	Taylor Street Venture Lp
San Antonio Truss Company Inc	Teardrop Partners Lp
Sandoval Real Estate LLC	Telecom Lease Advisors LLC
Sandra P Castillo Etal	Templo Bautista Mexicano
Santikos Culebra Warehouse LLC	Teresa C Rios L/E
SATX Business Trust	Term Legacy Real Estate LLC
SATX Holdings LLC Series I	Terry Vincent Torres Etal
SBA 2012 TC Assets LLC	Texas Auto Inspection Inc
Scrubadoo Culebra LLC	Texas Project 28 LLC
Sdharod Enterprises Inc	Tezel Rd Holding LLC
Security Service FCU	TMA Realty Services Inc

Tomasa Flores G L/E Toscano Gene Inc	
RF Irrevocable Trust	
Frinidad M Parades L/E	
rinidad M Parades L/E	
Jamd LLC	
Jfund Realty LLC	
Jno Capital Commercial Investments LLC	
Jrban Bradley LLC	
JSA Ram Ltd	
/arela Management Inc	
/elsua Real Estate LLC	
/erai Krupa LLC	
/ictory Fellowship of Texas	
/ictory Outreach of Texas	
/illage Parkway Baptist Chur	
/ill's Rel Estate Investments LLC	
/irginia P Lopez Etal	
Vaf Texas Management Group LLC	
Vaf Texas Managment Group LLC	
Valberg-Cruz Living Trust	
Val-Mart Real Estate Bus Tr	
Vash Corral Incorporated	
VC Culebra Crossing Sa Lp	
Vells Fargo Bank N A	
Vendy's of San Antonio Inc	
Vest End Baptist Church	
Vestdale Hollows Tx L P	
Vestmount At Three Fountains Lp & LRE DC Proper	ty
Owner LLC Vestover Elms HOA Inc	
Vestover Hills Assembly of God Holdings	
Vestover Hills Medical And Professional Building LLC	3
Vestover Medical Plaza I LLC	
Vestover Valley HOA Inc	
Vet Cay LLC	
Veynand Enterprises Ltd	
Voodlawn Lake RE LLC	
VRO Corp	
/B Construction LLC	
/olanda D Garza	
folanda Saldana Aldana L/E	=
/S-CW Corporation	

YS-K Corporation Zen Lautus Inc	
Alamo Area Metropolitan Planning Organi	
Alamo Area Metropolitan Planning Organi	zation
Alamo RMA/Bexar County	
Alamo RMA/Bexar County	
San Antonio Mobility Coalition	
San Antonio Mobility Coalition	
Texas Department of Transportation	
VIA Metropolitan Transit	
7-Eleven	
AutoZone Auto Parts	
Bank of America	
Barrio Barista	
Bee Clean Car Wash #4	
Bill Miller Bar-B-Q	
Birrieria y Taqueria Ay Arandas	
Cali Motors Official Vehicle Inspection	
Caliente Harley Davidson	
Cash America Pawn	
Chase Bank	
Chevron	
Church's Chicken	
CPS Energy	
Culebra Meat Market 17	
Culebra Park	
CVS	
CVS	
Dairy Queen	
Delicious Tamales	
Discount Tire	
Dollar General	
Dollar General	
Doral Club Apartments	
Dunkin'	
El Canelo #6 Mexican Grill	
El Rodeo Seafood and Taqueria	
EZ Pawn	
Family Dollar	
Gardendale	
Gold's Gym San Antonio Culebra	
Great Northwest Library	

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HB Zachry Middle School	
H-E-B	
H-E-B	
Hernandez Tire & Muffler Shop	
IHOP	
IPAC Preowned Outlet	
Islas Automotive Tires	
Jack in the Box	
Jack in the Box	
Javies Wheel - Repair	
Jefferson Dental & Orthodontics	
KFC	
La Michoacana Meat Market	
La Popular Bakery	
La Popular Bakery	
Labor on Demand, Inc.	
Little Caesars Pizza	
Mariscos de Puerto	
McDonald's	
Memorial Library	
Mi Guadalajara Mexican Restaurant	
Monticello Manor Apartment Homes	
Natwel Supply	
Pizza Hut	
Popeyes Louisiana Kitchen	
QuikTrip	
RBFCU	
Restaurante el D.F.	
Roadrunner Food Mart	
Sahara Motel	
Shipley Do-Nuts	
Silver Oaks Apartments	
Southwest Research Institute	
Supernova Smoke Shop	
Taco Cabana	
Taqueria El Rodeo de Jalisco	
Taqueria Guadalajara	
The Corner Craft Beer	
Top Notch Wheels & More	
Tropic Express	
U-Haul Neighborhood Dealer	
Umberto's Italian Grill	

Villas at C	osta Brava
Walgreen:	
-	Supercenter
Wendy's	
Wok Inn A	sian Café
Zip's Car \	
	nio Fire Department
	nio Police Department
	st Police Substation
Alamo Cit	y Apostolic Church
	the National Shrine of the Little Flower
Holy Rosa	ry Catholic Church
-	Hall of Jehovah's Witnesses
The Churc Storehous	ch of Jesus Christ of Latter Day Saints Bishop's
	Hope in Action
Northwest	-
	y Farms Community Association
Crown Me	
Great Nor	
Hidden M	
Loma Par	
Loma Vist	
Memorial	
Mountain	
Pipers Me	
Pospect H	
	nio Creekside
Thunderbi	
Timber Ri	-
University	
Woodglen	
Woodlawr	
Culebra P	
Edgewood	
-	ISD Transportation Director
	y Chinese Cuisine
	Elementary School
	den Meadow
	Knowlton Elementary School
Martin Ele	mentary

Memorial High School
Mexican American Catholic College
Nelson Early Childhood Education Center
NISD Facilites and Operations
NISD Police Chief
NICD Description Applicati

NISD Transportation Director
Northside Activity Center
St. Mary's University
St. Mary's University School of Law
SAISD

Residents		
First Name	Last Name	
Solomon Abdo &	A L Hernden	
Damian R & Shelly R	Abbott	
Quader Imad	Abdel	
Solomon	Abdo	
Carlos Javier	Abelar	
Jessica	Abrego	
Rina	Acevedo	
Catherine	Achin	
Belinda G	Acosta	
Elia	Acosta	
Hector M & Linda	Acosta	
Jesus	Acosta	
Martha L	Acosta	
Victor	Acosta	
Sandra P	Adame	
Luis Moreno &	Adriana Vasquez	
Rafael E & Linda R	Agredano	
Alice	Aguallo	
Elisa Saldivar	Aguayo	
Lucia	Aguero	
Alfonso & Blanca	Aguilar	
Alfonso B & Marie	Aguilar	
Bertha	Aguilar	
Felix & Francisca	Aguilar	
Francine M	Aguilar	
Isabel Sendejo	Aguilar	
Jose A & Lorena D	Aguilar	
Juan L	Aguilar	
Kay M & Tony	Aguilar	
Paula	Aguilar	
Pedro	Aguilar	
Ricardo	Aguilar	
Sofia L	Aguilar	
Jose R & Juanita H	Aguilera	
Juan	Aguillon	
Arturo	Aguinaga	
Leticia	Aguirre	

Rodolfo & Vicenta	Aguirre
Socorro	Aguirre
Rabeya	Akhtar
Sadoon Basim	Al
Maria Isabel & Luis E	Alamillo
Alejandro	Alanis
Alejandro & Leticia	Alanis
Hector Hugo	Alanis
Jason Anthony	Alanis
David	Alaniz
Juan J & Mariana	Alaniz
Roland	Alaniz
Ramiro G	Alarcon
Yesenia Flores	Alarcon
Delia Rosa	Alcocer
Antonio S	Alcocer, Jr
Carlos	Aldaco, Jr
David G Jr & Maribel	Aldana
Alfredo S	Alderete
Nancy & Daniel	Alderete
Jose Delagarza	Alejandro
Vicky	Alejos
Benjamin & Diana H	Aleman
Irvin	Aleman
Irving G	Aleman
Jose	Aleman
Rebecca H	Aleman
Leroy Martinez &	Alexandra G Soto
Victor R & Maria R	Alfaro
Marissa Perez Hernandez &	Alfredo Villeda Arteaga
Josefina	Alfrido
Maricel	Aligno
Adam D. Saucedo &	Alison N. Lujan
John J & Carolyn M	Allen
Stacy W	Allen
Thomas G	Allison
Larry Scott Hermes &	Allison Sheppard
Hector Jesus	Almaguer
Jose R	Almanzar

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Yolanda P & Jose Angel	Almaraz
Michael	Almendarez
Javier & Gloria	Alonso
Albino A	Alonzo
Esteban & Silberia	Alonzo
Leonard & Antonia	Alonzo
Edward	Alonzo, Jr
Rosa	Altamirano
Diana L	Alvarado
Fidel G	Alvarado
Francisco & Alma	Alvarado
lda	Alvarado
Irene	Alvarado
Joseph	Alvarado
Juan F	Alvarado
Luis E & Arcilia	Alvarado
Marisol	Alvarado
Michelle	Alvarado
Ralph A	Alvarado, Jr
Virgilio	Alvarado, Jr
Benito	Alvarez
Benjamin & Maria	Alvarez
Daniel & Guadalupe	Alvarez
David & Felipa	Alvarez
Eleazar P & Yolanda A	Alvarez
Francisco & Wf	Alvarez
Gladys E	Alvarez
Jaime R	Alvarez
Lisa A	Alvarez
Maria	Alvarez
Maria J	Alvarez
Maria P	Alvarez
rancisco & Oralia	Alvarez
Robert G	Alvarez
Victor	Alvarez
Yolanda A	Alvarez
Bentio	Alvarez, Jr
Juan P & Valeria	Alvarez-Canchola
Paul	Alvear
Sergio E Venegas &	Alvina M Rogers
Rogelio S & Orvilia	Amador
Aaron M Coronado &	Amanda A Madison
Vincent A Canales &	Amanda Salazar-Canales
Jose A & Leticia C	Amaya
Erik	Amezquita
Tom & Pejman	Amrollah
Jacqueline & Stanley	Anderson
Martin V	Andrade

Nestor J Ramos Calderon	Andrea H Herrera Valdez
& Peirre R Elliott &	Andreen Elliott Peta-Gaye
Jerry Kagan &	Angela Maria Garcia
Leonal Lamas, Jr &	Angela Milagros Lamas
Carmen	Anguiano
Juan A & Janie	Anguiano
Noemi R.	Anguiano
Alicia Puente &	Antonio Maldonado
Beatrice R	Antu
Richard	Anyang
Daniel G	Aparicio
Ana E	Aponte
Shonda M	Applewhite
Carlos F Flores &	Araceli De Bustos
Agustin	Aranda
Gabino E	Araujo
Gilbert H & Dolores	Arevalo
Jose A & Guadalupe	Arevalo
Jesse Angel	Arevalos
Thomas S.	Arevalos, Jr.
Vanessa	Arguelles
Vannesa & Miguel Angel	Arguelles
Marco A & Mary Ann	Arias
Sandra D	Arias
Griselda	Arizmendi
Robert A	Arizola
Maria Magdalena	Arizpe
Ramiro Garza, Delores P Rangel &	Arnold L Garza
David C & Mary A	Arredondo
Fidel Garcia	Arredondo
Ignacio	Arredondo
Jesse	Arredondo, Jr
Melissa	Arreola
Salvador R	Arreola
Kim	Arroyo
Paula	Arroyo
Ruben & Carmen	Arroyos
Jessica M	Asebedo
Danial P	Asgari
Debra	Asgari
John Paul Queener, Jr &	Ashley Nicole Benavidez
Andres	Astorga
Karena	Auzenne
Juanita R	Avalos
Miguel	Avalos
Edward R & Linda	Avila
Maria Canderlaria	Avila

Culebra Road Transportation Study | Community Open Houses Documentation

APPENDIX

APPENDIX A-3

Victor M & Leticia C	Avila
Alfonso A	Avina
Jose Antonio	Avitia
Antonio & Martha	Ayala
Daniel & Maria R	Ayala
Mary De Los Angeles	Ayala
Oruc	Ayhan
Mary Margaret & Mike S	Baez
Debra D	Bailey
Kodi Duray & Sabrina	Bailey
Stacey Hall	Bain
Refugio S & German	Balcarcel
Dejesus Lydia A & Jose M & Aida	Balcazar
John R & Petra	Balderas
Jose D & Bertha P	Balderas
Mercedez D	Balderas
Clenton	Ballard, III
German & Gloria	Ballesteros
Gloria	Ballesteros
Maria H.	Baltazar
Rafaela	Banda
Robert	Banda
Robert Ruben & Gloria D	Banda
John Jesse	
	Barajas
Roseann	Barajas
Gilbert	Barba Barbosa
Encarnacion	
Henry G & Gloria	Barbosa
Martin & Ma Elena	Barbosa
Esteban	Barboza
Macario R	Barcena
Randy & Margarita	Barela
Mark A	Barker
Charissa	Barnes
Antonio F & Isabel	Barrera
Balde C & Lupe G	Barrera
Gloria	Barrera
Willie C & Concepcion B	Barrera
Gustavo G	Barreto
Heriberto V	Barrientes
Graciela	Barrientos
Guadalupe	Barrientos
Edwin R & Anamaria	Barrios
Hector O & Rosario A	Barrios
Jorge H	Barrios
Angel F & Maria C	Barron
Felipe M	Barron
Mario Jose & Tina A	Barron
	nonortation Study

Jose Luia	Basulto
Robert E & Michele K	Bates
Kevin D Lima	Batres
Michelle	Bautista
Ronald A & Susie S	Bayer
Joseph J	Bayona
Vikhyat	Bebarta
Matthew T & Jennifer L	Becker
Kim B, Ginger A &	Beckstead
Christopher B	
Michael	Bedient
Virginia	Beierly
Robert Arredondo &	Belkis Cuca-Arredondo
Rachel G	Benavides
Rosa P	Benitez
Geraldine L	Benke
Corey S	Benshoof
Duane L & Rebekah R	Benson
Michael & Andrea R	Beraducci
Timothy E & Rhea A	Bergauer
Jerome & Patricia	Berlanga
Joel	Berlanga
Cristobal J & Connie A	Bermea
Joe U & Sandra	Bermea
Andrew & Raven T	Bernal
Carlos & Anita S	Bernal
Claudia	Bernal
Edgardo I	Bernal
Francisco B	Bernal
Julie Ann	Bernal
Zoe X Luo &	Bernard J McDonald
Guadalupe R	Berrones
Jesus R & Norma C	Berrones
Johnroy R	Berrones
Jose Jesus & Sylvia Ann	Berrones
Duane	Berry
Kristine A	Besas
Arnoldo & Mayra	Betancourt
Betcourt Servando & Terry	Betancourt
Emmanuel B Castellanos	Bethany D Burbank
&	
Brandy Y & Marcus D	Beverly
Ansuya Gopal	Bhagat
Albert F & Josie	Bishop
Anthony E & Brenda J	Bishop
Luther Lorain	Bishop
Vernie M	Blackwell
Eric	Blaine
Jose Luis O. Santillan &	Blanca A. S. Pineda

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Pascual Zavala Davila &	Blanca Margarita
Hector R & Graciela	Blanco
James & Carrie	Blee
Robert	Bocanegra
Roberto & Juana Lorena	Bocanegra
Isable	Bohorquez
Philip	Bollman
Sylvia R	Bonales
Juan J & Silvia Y	Bonilla
Juan J & Silvia Y	Bonilla
Evelyn J	Boone
Felisitas	Borjon
Juan & Dalia	Borjon
Jose A & Raquel R	Botello
Marcela	Boughton-Maldonado
Caitlan Ann, Michael D, & Larry T	Bowlin
Ian C & Nancy Y	Boysen
Brook A	Brackett
Alicia	Brantton
Jorge M. & Maria Aracely	Bravo
Lori	Brazzill
Sinai Flores Alonso &	Brenda Lucia Camarillo
Chanel Y	Brewer
Rosaura V	Briones
Fernando T & Cynthia O	Briseno
Jose	Briseno
Maria T	Brito
Joe R Bustos Jr &	Brittany Henry
Alex M & Nohelli E	Brockman
Joseph	Brooks
Stephen John & Celina Teresa	Brotherman
Evelyn	Brothers
Alto	Brown
Brian	Brown
Margaret K	Brown
Robert E & Lenora G	Brown
Alan D & Raquel	Broyles
Antoinette	Bruner
Diana E	Bryan
Kermit Edwards	Bryant
Fredda M	Bryson
Rick A	Buchanan
Rodolfo & Linda A	Bueno
William D & Nadine M	Buhrman
Maria I	Bundick
Gabrielle A	Burchard
Guillermo	Burciaga

Eddie Lee	Burnett
Lillie Diane	Burnim
Bur Jesse J	Burns
Melissa White	Buschbaum
Roger R	Bustamante
Yolanda	Bustillos
Dolores	Bustos
Elvira	Bustos
Ruben	Bustos
David Cervantes	Bustos, Jr
Violet Sommers	Butler
Cluster R	Byars
Deborah	Byrd
Alicia	Cabrera
Enes	Cabric
Dorothy Maxine	Cadena
Mauricio & Soledad	Cagigas
Juan	Calbillo
Sandra	Caldera
Benito & Cynthia	Calderon
Carlos R & Claudia	Calderon
Irma	Calderon
James & Sylvia	Calderon
Javier F	Calderon
Jose L & Esmeralda	Calderon
Raul & Teresa A	Calderon
Ventura	Calderon, Jr
Faustino C	Calvario
Manuel & Rebecca	Calvillo
Jesse P & Linda	Calzada
Vicky	Calzada
Desidoro & Stella	Camacho
Guadalupe	Camacho
Jose E & Rosa G	Camacho
Pedro & Herlinda	Camacho
Jesus	Camacho, Jr
Wanda F	Camp
David J	Campa
Roberto G	Campa
Daniel	Campos
David A & Rosalinda	Campos
David A & Rosalinda	Campos
Hector Ivan	Campos
Joe Patrick	Campos
Juan	Campos
Martha Garza	Campos
Melissa	Campos
Nicolas	Campos

Campos
Campos
Camposano
Canales
Canales, Sr
Canela
Canizalez
Cano
Cano
Cantu
Cantu, Jr
Cantu, Jr
Cardenas
Cardenas
Cardenas
Cardenas
Cardona
Cardoza
Cardwell, II
Carielo
Carillo
arlos Cardenas
arlos Frederick
Carmona
Carolina Ziola
Carranza
Carrejo
Carreon
Carreon
Carreon
Carreon
Carrera
Carrero
Carrillo
Carrillo
Carrillo

Pedro M & Edith	Carrillo
Richard & Maria M	Carrillo
Hector R	Carrion
Curtis E	Carson, Jr.
Zita Beatrice	Carter
David R	Cartwright
Hugo Pena, Jr &	Casandra A Sanchez
Ilda Alicia	Casanova
Jessie Mae Arcos & Gilbert	Casares
John	
Angie	Casarez
Florence	Casarez
Richard	Casarez
Antonio & Diana	Casas
Lorenzo G	Casas, Jr
Juan J & Paz A	Casias
Thomas	Casias
Thomas	Casias
Abel	Casillas
Roseann R	Casillas
William Anthony	Casillas-Postalwait
Alma Graciela & Marco Antonio	Castaneda
Armando	Castaneda
Hilda V	Castaneda
Mario & Nora L	Castaneda
Ricardo G	Castaneda
Teresa De Jesus	Castaneda
Rose	Castano
Alfred	Castellano
Francisco	Castilla
Virginia	Castilleja
Alfred L	Castillo
Antonia	Castillo
Cesar A Avalos	Castillo
Daniel M. & Eva	Castillo
Eduardo Alonzo	Castillo
Eleodoro V & Irene	Castillo
Herlinda R	Castillo
Jamie & Linda V	Castillo
Jesse	Castillo
Michael R	Castillo
Minerva L	Castillo
Onesimo & Maria	Castillo
Pedro	Castillo
Ricardo J	Castillo
Rudolfo B	Castillo
Salvador & Avelina	Castillo
Sergio & Lucille S	Castillo

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Simon	Castillo
Velia	Castillo
Virginia M	Castillo
Gerardo & Karla Lira	Castro
Joe P	Castro
Luis Arredondo	Castro
Manuela	Castro
Marcelo & Matilde	Castro
Marcelo R & Lucia	Castro
Maurilio W	Castro
Maurillo W & Rebecca S	Castro
Roger R	Castro
Teresa	Castro
Andrew	Cavazos
Baldomero Perez	Cavazos
David J	Cavazos
Kristina	Cavazos
Linda	Cavazos
Luis Felipe	Cazares
Alex R	Ceciliano
Arturo G & Jesusa M	Cedillo
Olga N	Cedillo
Rodolfo M & Maria A	Cedillo
Hector	Cegueda
Silvano	Celestino
Joseph I & Rosalinda	Celis
Anthony & Sarah	Centeno
Darrell	Centeno
Maria Socorro	Cerna
Sara Anna, Arturo J, & Alberto John	Cerna
Javier	Cervantes
Jesse	Cervantes
Louis P	Cervantes
Manuel S	Cervantes
Mark Steven	Cervantes
Louis P	Cervantes, Jr
Ernestine	Cevallos
Rodolfo M & Martha M	Chacon
Gloria Ann	Chairez
Christian	Chamberlain
Cecil R & Rose	Chambless
Tinnakorn & Lamai	Chandaeng
Gloria D Emeteria	Chapa
Juana H	Chapa
Joe & Rita	Charles
Mary Elva	Chavarria
Raul	Chavera
Dolores & Gerardo	Chavera
Dolores & Gerardo	Criavez

Gilberto & Jeanette	Criavez
Gilberto P	Chavez
Gloria	Chavez
Graciela I	Chavez
Jose	Chavez
Maclovio E & Rosalinda E	Chavez
Manuel V	Chavez
Paul G & Ofelia M	Chavez
Paul N	Chavez
Willibaldo T	Chavez
Edward & Estella	Chavira
Shu Min & Guofan	Chen
David C	Cheraso
Clara	Chiquillo
Maria Cristina	Chiquillo
Clyde Dean & Donna Jean	Chlouber
Dennis W	Christensen
Kevin Wing-Hung	Chu
Li-O	Chu
Gabriel	Chvarria
Roger D & Mary E	Cirilo
Blanca E & Eleazar	Cisneros
Denisse & Phillip	Cisneros
Esperanza A	Cisneros
Gonzalo & Maria G	Cisneros
Debbie	Clark
Michael H & Doris J	Clay
Lewis M	Clements
Kristy	Clendening
Jennifer C	Coffey
Fermine	Colley
Roberto G & Mary E	Collazo
Christopher M Welfredo	Collins Colon
Juan M & Emma E	Colon
Jessica Danielle	Conrad
Angelica	Contreras
Antonio & Lucille	Contreras
Armando V & Sandra	Contreras
Frances & Juan A	Contreras
Patricia A	Contreras
Veronica	Copenhaver
Irene E	Corbitt
Servando	Cordova
Francisco R	Cordova
Servando & Irma Y	Cordova
Jasti John & Elin Grace	Cornelius
John & Kim	Corona

APPENDIX

APPENDIX A-3

Michele A	Corona
Diana D	Coronado
Martin H & Rita	Coronado
Mary Lou	Coronado
Rene	Coronado
Mario A & Juanita R	Cortes
Adam	Cortez
Brenda L	Cortez
Daniel M	Cortez
Joe J & Julia	Cortez
Richard & Janet	Cortez
Alfredo R	Cortinas
John R & Cynthia E	Cotlong
Willard Jay & Gloria J	Couey
Ruth M	Cowie
Grace Ann	Coy
Dan C & Carol J	Cozby
Brad E	Crabtree
Joaquina	Criado
Jose guadalupe Torres &	Crista Fernandez Aguero
Jesus Reyes-Ortiz &	Cristina Galvan-Lara
Ben & Willie M	Crockett
Yolanda	Crouch
Javier & Maria	Cruz
Manuel J	Cruz
Sonia	Cruz
Vanessa	Cruz
Alma	Cruz-Cardona
Tomas Delarosa &	Crystal Huey
James Michael Confer &	Crystal Lopez Confer
	Martiza Conter
Charles A Garza &	Crystal M Avila
Maria	Cuaresma-Jones
Dina Yvonne	Cuellar
Elizabeth D	Cuellar
Roland	Cuellar
Valentin L & Maria B	Cuellar
Maria De Jesus	Cueva
Kathleen R	Cummins
Andres C & Dominga	Cura
Elizabeth Ann	Curiel
Joe E & Gloria A	Curiel
Daniel A & Aubrie C	Cypert
Michael D & Libertad	Dabbs
Ronald E	Dailey
Juan Francisco Loera Ruiz	Daisy A Romeu
Deborah Aide	Dallo
Jesus	Dallo

Joe	Dallo
Claudia Catherine	Dallo-Martinez
Gloria Villanueva &	Daniel Deleon
Rebecca R Moreno &	Daniel Rodriguez
Arabella Parlati	Daniels
Michael	Daniels
Ulysses & Maria	Daniels
Baishakhi	Das
Teresa A	Daugherty
Santiago M Anguiano Jr &	David A Cisneros
Valerie Siller &	David Arriaga
Matthew Gonzalez &	David Hernandez
Antonio & Guadalupe	Davila
Diana C	Davila
Jesus	Davila
Andrew	Davis
Danny James & Marilyn	Davis
Robert E	Davis
Stacy	Davis
Beatrice	De Haro
Henry	De Hoyos
Madolena F	De Hoyos
Santiago G	De Hoyos
Juanita G	De La
Jesus M & Wf	De La Cruz
Cesar & Alejandra	De La Garza
Dario E	De La Garza
Noelia F	De La Paz
Janie	De La Rosa
Josephine	De La Rosa
James A	De Luna
Melissa A	Deans
Daniel Pineda &	Debra P Salinas
Eloisa G	Decordova
James & Theresa	Decosta
Gilbert	Dehoyos
Henry C & Armandina	Dehoyos
Pablo A & Giovanna C	Dehoyos
Thomas & Rosa	Dehoyos
Anna M	Delacruz
Gerry	Delacruz
Magdalena	Delacruz
Maryann	Delacruz
Leticia Ann	Delacruzeduardo
Christopher	Delagarza
Maria	Delagarza
Victoria S	Delagarza
	Delaluz

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	Delarosa
John A & Viola A	Delarosa
Margarita	Delarosa
Ramon H	Delarosa
Rebecca S	Delarosa
Silvia	Delarosa
Juan & Maria	Delbosque
Cynthia A & Miguel	Deleon
Francisco	Deleon
Gloria G	Deleon
Guadalupe	Deleon
Kyle Emil & Allison G	Deleon
Philip	Deleon
Rogelio	Deleon
Sylvia	Deleon
Victor V & Debbie M	Deleon
Abel John	Deleon, Jr
Larry	Deleon, Jr
Edward Lee	Delgado
Irma	Delgado
Jose A & Janie	Delgado
Jose A & Janie	Delgado
Irene	Delossantos
Jesus A Ordonez Villalva &	Delsa Arelie Jackson
Cindy	Deluna
Daniel & Margarita	Deluna
Indalecio	Dena, Jr
Jose Angel Lozano &	Denise Olea Alma
Juse Arigel Luzaliu &	
Daniel Vasquez &	Denise Perez
Daniel Vasquez & Jennifer	
Daniel Vasquez &	Denise Perez
Daniel Vasquez & Jennifer Winston L Patrick J	Denise Perez Dent-Bozzano
Daniel Vasquez & Jennifer Winston L Patrick J Balbina Diaz	Denise Perez Dent-Bozzano Deocampo Deore Deprez
Daniel Vasquez & Jennifer Winston L Patrick J Balbina Diaz Paula Bernardo &	Denise Perez Dent-Bozzano Deocampo Deore Deperez Derek Ng Ho
Daniel Vasquez & Jennifer Winston L Patrick J Balbina Diaz Paula Bernardo & Doreteo Joe	Denise Perez Dent-Bozzano Deocampo Deore Deperez Derek Ng Ho Desoto
Daniel Vasquez & Jennifer Winston L Patrick J Balbina Diaz Paula Bernardo & Doreteo Joe Gloria	Denise Perez Dent-Bozzano Deocampo Deore Deperez Derek Ng Ho
Daniel Vasquez & Jennifer Winston L Patrick J Balbina Diaz Paula Bernardo & Doreteo Joe Gloria Herlinda	Denise Perez Dent-Bozzano Deocampo Deore Deperez Derek Ng Ho Desoto
Daniel Vasquez & Jennifer Winston L Patrick J Balbina Diaz Paula Bernardo & Doreteo Joe Gloria Herlinda Erick Calvillo &	Denise Perez Dent-Bozzano Deocampo Deocampo Deore Deperez Derek Ng Ho Desoto Desoto Desoto Desotos Destry Copper
Daniel Vasquez & Jennifer Winston L Patrick J Balbina Diaz Paula Bernardo & Dorsteo Joe Gloria Herlinda Jovita Torres & Jovita Torres &	Denise Perez Dent-Bozzano Deocampo Deocampo Deore Deperez Denek Ng Ho Desoto Desoto Desousa Destrry Copper
Daniel Vasquez & Jennifer Winston L Patrick J Balbina Diaz Paula Bernardo & Doretto Joe Gloria Herlinda Erick Calvillo & Jovita Torres & Javier Barnera &	Denise Perez Dent-Bozzano Deocampo Deocampo Deore Deperez Derek Ng Ho Desoto Desoto Desoto Desotos Destry Copper
Daniel Vasquez & Jennifer Winston L Patrick J Balbins Diaz Paulia Bernardo & Doreteo Joe Gloria Herlinda Lerick Calvillo & Jovita Torres & Javier Barrera & William Akedo & William Akedo &	Denise Perez Deni-Bozzano Dencampo Decorampo Decore Deperez Derek Ng Ho Desoto
Daniel Vasquez & Jennifer Viniston L Patrick J Balbina Diaz Paula Bernardo & Doreteo Joe Gloria Herlinda Erick Calvillo & Jovita Tores & Javier Barrera & William Alcedo & Bryan C	Denise Perez Dent-Bozzano Dencempo Deocempo Deore Deperez Denet Ng Ho Desoto Dena M Reyes Diana M Reyes Diana Hisleh Dias
Daniel Vasquez & Jennifer Winston L Patrick J Balbina Diaz Paula Bernardo & Doreteo Joe Glora Herlinda Erick Calvillo & Joveta Tores & Joveta Tores & William Alcedo & Bryan C Alma Lecoor	Denise Perez Deni-Bozzano Dencampo Decorampo Decore Deperez Derek Ng Ho Desoto
Daniel Vasquez & Jennifer Winston L Patrick J Balbino Diaz Paula Bernardo & Doreteo Joe Gloria Herfinda Erick Calvillo & Jovita Torres & Javier Barrera & William Akceb & Bryan C Alma Leonor Jesse & Caprice	Denise Perez Dent-Bozzano Dencempo Deocempo Deore Deperez Denet Ng Ho Desoto Dena M Reyes Diana M Reyes Diana Hisleh Dias
Daniel Vasquez & Jennfer Winston L Patrick J Patrick J Balbina Diaz Patrick J Balbina Diaz Paula Bernardo & Gloria Herlinda Erink Catwillo & Jovita Torres & Javier Barrera & William Alcado & Beyan C Alma Leonor Jesso & Caprice Milary Milary & Caprice Jesso & Caprice	Denties Perez Dent-Bozzano Dencempo Decore Deprez Deprez Derek Ng Ho Desousa Desousa Desousa Desousa Destry Copper Diana A. Medina Diana M Reyes Diana Hsieh Dias Dias Diaz Diaz Diaz Diaz
Daniel Vasquez & Jennifer Winston L Patrick J Balbina Diaz Paula Bernardo & Doreteo Joe Gloria Herlinda Erick Calvillo & Jovita Torres & Jovita Torres & Javier Barrera & William Alcodo & Bryan C Alma Leonor Jesse & Caprice Jesse William Jesus	Denise Perez Deni-Bozzano Denocampo Deocampo Deore Deperez Derek Ng Ho Desoto D
Daniel Vasquez & Jennifer Jenn	Dentise Perez Dentis Perez Denti-Bozzano Deocompo Deoco Deoco Deperez Derek Ng Ho Descoto Desc
Daniel Vasquez & Jennifer Winston L Patrick J Balbina Diaz Paula Bernardo & Doreteo Joe Gloria Herlinda Erick Calvillo & Jovita Torres & Jovita Torres & Javier Barrera & William Alcodo & Bryan C Alma Leonor Jesse & Caprice Jesse William Jesus	Denise Perez Deni-Bozzano Denocampo Deocampo Deore Deperez Derek Ng Ho Desoto D

Maria Teresa	Diaz
Matilde	Diaz
Robert & Esther	Diaz
Newton Lloyd	Diem
Guadalupe	Dimas
J Carmelo Juarez &	Dionicia Montealvo
Joe S Garcia, Sr &	Dolores Jesusa
Antonio Sauceda	Dominguez
Carlos & Sylvia	Dominguez
David F & Amada S	Dominguez
Erlinda	Dominguez
Guadalupe	Dominguez
John A	Dominguez
Jose M & Elida R	Dominguez
Jose Maria	Dominguez
Joseph R & Teresa	Dominguez
Lisa	Dominguez
Maria R	Dominguez
Martina	Dominguez
Raul C & Carla T	Dominguez
Vilma	Dominguez
Enrique & San Juanita V	Donnell
Chris & Ann H	Donner
Linda O. Schubert &	Dorothy Solis, et. Al
Vicki R	Dorsey
Robert W	Douglas
Miguel Angel &	Dovalina
John M & Ana R	Drobiezewski
Carlos R	Duenas
Laura	Dugas
Adolph Gilbert	Duke
Bobbie	Duplessis
Francisco D & Vicenta R	Duque
Bonifacio & Martha A	Duran
Deborah R	Duran
Jesus J	Duran
Margarito & Beatrice	Duran
Maria R	Duran
Roberto & Petra V	Duran
Teresa O	Duran
Veronica Chavez	Duran
Earl L & Kirsten A	Duron
Rosa Maria	Eiten
Raphael	Eke
Albertina	Elias
Frances Y	Elias
Ray	Elias

Alberto G Arroyo &	Elizabeth M Ortiz
Mark	Elizalde
Diana E	Elizondo
Maria Elva Reyes	Elizondo
Maria Razo	Elizondo
Rogelio P	Elizondo
Tammy M	Elizondo
Theresa Ann	Elizondo
Sara Ann Brown &	Elleana M Illalan
James & Alda	Ellis
Gabriel Morales Alatorre &	Elvia Leticia Morales
Bruce E	Embrey
Edith D	Emeterio
Gabriel & Lilia	Enciso
Rita F	Enriquez
Steven Jay	Epstein
Francisco A	Erazo
Amber Bocanegra-Garza &	Eric A Tapia
William B & Caryn L	Erickson
Arlene C Rangel &	Ernest Roman, Jr
Francine V	Escamilla
Maria G	Escamilla
Norma	Escamilla
Daniel A & Christina D	Escarcega
Marie A & Martin A	Escareno
Andrew	Escobar
Byron J	Escobar
Cresencio	Escobar
Cruz & Blanca	Escobar
Epifanio & Adelaida	Escobar
Emest	Escobar
Maria	Escobar
Socorro	Escobar
Alberta L	Escobedo
Casimiro & Mary	Escobedo
Jose F J	Escobedo
Maria Del Rosario	Escobedo
Maria Linda	Escobedo
Paula R	Escobedo
Roberto G	Escobedo
Reza Khaladj	Esmaily
Anna Marie & Feliciano D	Esparza
Diana Veronica	Esparza
Gilberto & Olga	Esparza
Guillermo A	Esparza
Guillermo A. & Carmen	Esparza
Jose G & Gloria	Esparza
Mercedes C	Esparza Esparza
mercedes C	Сарагиа

Mercedes C	Esparza
Ramiro	Esparza
	·
Santiaga Sarita	Esparza
Cipriano	Esparza Juarez
	Espino
Cipriano & Josefina	Espino
Cipriano & Josefina	Espino
Melchor M	Espinosa
Anastasia Z	Espinoza
Anna M	Espinoza
Diane Salinas	Espinoza
Emily	Espinoza
Genaro	Espinoza
Jose A & Delia V	Espinoza
Fred B	Espronceda
Nora	Esquilbel
Cynthia A	Esquivel
David	Esquivel
Esther	Esquivel
Gregory B	Esquivel
Gregory B	Esquivel
Jeannette	Esquivel
Jesus Rosales & Kimberly	Esquivel
Jesus S & Asencion	Esquivel
Raul & Maria E	Esquivel
Raul G	Esquivel
Stephanie	Esquivel
Nelvin A & Jessica	Esteban
Thelma Inez Muraida &	Esther Ortiz
Braulio Villarreal & Isabel	Estrada
Marie	
Jose Garza	Estrada
Raymundo M & Raymundo Jr	Estrada
Roland	Estrada
Jesus S & Anita	Estrello
Molly	Eureste
Glenda Perez &	Eva Cervantes
Pablo E Campos &	Evangeline Ortiz
Julieta & Charles	Evans
Nick	Evans
Samuel Torres &	Evelyn Hernandez
Martin & Laurie	Everett
Bruce	Far
Mohamad Saeed Ebad	Fardzadeh
Carla	Farias
Martin A & Melissa J	Farias Farias
Romulo L	Farias
Syed Ali	Farooq

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Robert	Favela
Robert C	Favela
David Reynosa, III &	Faye Suarez
Oscar & Elba	Feernandez
Elia B	Felan
Jose & Maria D	Felan
Josefina Reyes	Felan
Linda Marie	Felan
Patricia A	Felan
Tiburcio	Felan
Maria G Garcia &	Felipita C Ybarra
Ricardo Cevallos, Jr &	Felisa Alonzo
Juanita	Felix
Agustin A	Fernandez
Carlos & Gloria	Fernandez
Daniel E	Fernandez
David Raymond & Debra A	Fernandez
Dolores Salazar	Fernandez
Eduardo	Fernandez
Estevan	Fernandez
Evangeline	Fernandez
Franklin Pupo	Fernandez
George R & Alice	Fernandez
Gilbert H. & Pam	Fernandez
Jorge F	Fernandez
Juan Francisco & Peggy	Fernandez
Ann Louis R	Fernandez
Monica	Fernandez
Noe Martinez & Orita A & Natalie	Fernandez
	Fernandez Fernandez
Ralph G & Lupe	
Rosio	Fernandez
Arthur	Fernandez, Jr
Enrique Perez Sosa &	Fernando Perez Moreno
Natasha	Ficello
Ofelia S	Fierce
Ulides C	Figueredo
Eva L	Figueroa
Edna	Fillmore
Charlotte E	Fisher
Jennifer D	Fisher
Patricia Ann	Fisher
Oliver	Flaig
Abel T & Laura B	Flores
Alejandro	Flores
Ana Laura	Flores
Armando & Grace &	Flores
Baldemar R & Wife	Flores

Basilio A	Flores
Beatrice L	Flores
Courtland M	Flores
Daniel N & Mary Lou	Flores
Donny & Don R	Flores
Encamacion	Flores
Eva Nicole	Flores
Francisco J	Flores
Hilda	Flores
Ismael & Maria	Flores
Jaime	Flores
Jose Antonio & Betty	Flores
Jose Damacio Ayala	Flores
Josephine	Flores
Juan I & Maria	Flores
Juan Jose Balderas	Flores
Maria	Flores
Maria Elisa	Flores
Mary P	Flores
Mericia	Flores
Micaela M	Flores
Ramiro & Cruz A	Flores
Ramon	Flores
Ramon A	Flores
Wenceslao B & Delia	Flores
Jesus Junior	Flores, III
Jose	Flores, Jr
Rodrigo	Flores, Jr
Gonzalo E	Floriano
Michele D & Gonzalo E	Floriano
Janie O	Fonseca
Merriel L	Forchion, Jr
Claudia	Fortunatti
Danielle Lomax & Michael	Fox
Alan	_
Sonia	Fox
Richard V	Fragoso
Jose	Fraire
Gloria	Franco
Christopher H & Cynthia E	Franklin
Uladzimir	Frantskevich
Gary & Karen	Franz
Steven R & Maria R	Frederick
Patrick & Nora	Freeman
Steven R & Heather J	Fritsche
Sr Antonio & Verna M	Frye
Linda P	Fuentes
Mercedes Nandan P	Fuentes Gad

 Armando & Grace & Flores
 Mercedes
 Fuertes

 Baldemar R & Wife
 Flores
 Nandan P
 Gad

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APPENDIX

Ashley	Gagnon
Henry E	Gagnon, Jr
Joel G & Ruby H	Galan
Joselito & Elsa	Galapate
Rafael C & Rosa	Galaviz
Theodore	Galindo, IV
Diana	Gallardo
Debra	Gallego
Alfredo & Carmen	Gallegos
Christopher M	Gallegos
Diane C	Gallegos
Martha L	Gallegos
Ramiro P	Gallegos
Rogelio & Sylvia	Gallegos
Seferino & Martha	Gallegos
Victor M	Gallegos
Mario	Galvan
Mary Lou	Galvan
Tiffany M	Galvan
David J & Guadalupe	Gamboa
Catarina R	Gamez
Edward M & Anna D	Gamez
Gail	Gamez
Laurencio & Maria G	Gamez
Perla	Gamez
Mary	Gantt
Felipe & Juanita	Gaona
Juan & Eulalia T	Garay
Raul	Garay
Richard	Garay
Robert Castaneda &	Garay
Theresa Jean	Garcez
Lorenzo T & Dominga	
Adam Alfredo & Ninfa B	Garcia Garcia
Andres	
	Garcia
Anita V	Garcia
Antonio Rojas	Garcia
Barbara Ann	Garcia
Benito & Diana	Garcia
Cesario & Maria C	Garcia
Christina & Christopher A	Garcia
Cynthia Ann	Garcia
Daniel & Frances	Garcia
David & Jennifer	Garcia
David D	Garcia
David F & Mirta	Garcia
Debra Loray	Garcia
Delia Araceli	Garcia
O. d. b	

Dominga	Garcia
Edward B	Garcia
Elias	Garcia
Elizabeth Jean	Garcia
Elvira & Manuel J & Daisy & Miguel	Garcia
Felipe	Garcia
Felipe D	Garcia
Francisco J	Garcia
Frank J	Garcia
Gustavo & Olga	Garcia
Hector M	Garcia
Ignacio & Maria Deroble	Garcia
Irene C	Garcia
Irene Jessica	Garcia
Irma M	Garcia
Ivan	Garcia
Jaclyne Renae	Garcia
Jorge	Garcia
Jose D & Cynthia	Garcia
Jose E & Angelina C	Garcia
Jose Miguel	Garcia
Juan A & Martha E	Garcia
Juan C & Aurora M	Garcia
Jubal	Garcia
Lawrence John & Elida O	Garcia
Lizeth Maria	Garcia
Louis & Brijida	Garcia
Manuel A	Garcia
Margaret	Garcia
Maria C	Garcia
Maria G	Garcia
Maria Victoria	Garcia
Martinliano Calderon	Garcia
Mary	Garcia
Mauricio	Garcia
Nelva A	Garcia
Norma	Garcia
Oscar S & Rosa M	Garcia
Patrick	Garcia
Paul	Garcia
Paul I	Garcia
Raul Luna	Garcia
Raul V & Maria A & Garcia Leonardo G & Elizabeth S	Garcia
Richard R	Garcia
Roberto & Monika	Garcia
Roberto & Rosalinda	Garcia
Roberto T & Mary C K	Garcia

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	Garcia
Rogelio	Garcia
Rosalinda Luna	Garcia
Rose Marie	Garcia
Ruben	Garcia
Ruby L	Garcia
Sandra Meyer	Garcia
Severa Q	Garcia
Steven	Garcia
Yolanda P	Garcia
Zeferina	Garcia
Daniel A.	Garcia, Jr
Gilbert M	Garcia, Jr
Sam	Garcia, Jr
Crespin & Pauline	Garibay
Maurice Armand	Garlet
Finley & Leslie F	Garrett
Shannon C	Garrett
Antonio & Leann	Garza
Cesar/Sergio/Juan	Garza
Cynthia Beatrice Martinez	Garza
Daniel	Garza
Daniel	Garza
Daniel & Amada S	Garza
Domingo D & Rosa M	Garza
Edgard U &	Garza
Esther	Garza
Guadalupe G	Garza
Gustavo M & Irma J	Garza
Hortencia R	Garza
Javier	Garza
Jose	Garza
Josie R	Garza
Juanita B	Garza
Luis P & Maria V	Garza
Melissa	Garza
Obdulia	Garza
Obdulia Pablo C & Blanca L Paulina	Garza
Obdulia Pablo C & Blanca L Paulina Presciliano	Garza Garza Garza Garza
Obdulia Pablo C & Blanca L Paulina Presciliano Ralph J	Garza Garza Garza Garza Garza
Obdulia Pablo C & Blanca L Paulina Presciliano Ralph J Ramiro & Sharon	Garza Garza Garza Garza Garza Garza Garza Garza Garza
Obdulia Pablo C & Blanca L Paulina Presciliano Ralph J Ramiro & Sharon Raul R	Garza
Obdulia Pablo C & Blanca L Paulina Presciliano Ralph J Ramiro & Sharon Raul R Rose Mary	Garza
Obdulia Pablo C & Blanca L Paulina Presciliano Ralph J Ramiro & Sharon Raul R Rose Mary Sara Morales	Garza
Obdulia Pablo C & Blanca L Paulina Presciliano Ralph J Ramiro & Sharon Raul R Rose Mary Sara Morales Frank L	Garza
Obdulia Pablo C & Blanca L Paulina Presciliano Raiph J Ramino & Sharon Raul R Rose Mary Sara Morales Frank L Carlos	Garza
Obdulia Pablo C & Blanca L Paulina Presciliano Ralph J Ramiro & Sharon Raul R Rose Mary Sara Morales Frank L	Garza

Baltazar	Gasca
Brittanie	Gathright
Juan A & Bartola L	Gatica
Dale	Gattis
Martin S & Velinda G	Gauna
Rodolfo Cruz	Gauna, Jr
Alejandra	Gaytan
Sebastian Isaiah	Gaytan
Janice L	Gerick
Ana Joy Madriaga	Germino
Fausto & Mckenzie	Gil
Guadalupe G	Gil
Reyes R & Simona L	Gil
Craig	Gillon
Laurel D	Ginsberg
Janie Roussin &	Gloria Gutierrez
Maria Rosario	Godina
Apolonio P	Godines
Robert A & Araceli	Godines
Juan Gerardo & Guadalupe	Godinez
Alma D & Juan	Godoy
Gloria Y	Godoy
Melissa	Goede
Anibal	Gomez
Catarino C & Cipriana	Gomez
David	Gomez
David O & Rachel	Gomez
Dora Luz	Gomez
Ernestina	Gomez
Esmeralda Gonzalez	Gomez
Estella A	Gomez
Everardo	Gomez
Fred & Hongwei	Gomez
George & Maria C	Gomez
Isidoro & Guadalupe	Gomez
Jaime E & Juanita A	Gomez
Jesus	Gomez
Jose & Maria Juana	Gomez
Jose F & Petra	Gomez
Juanita P	Gomez
Mary Esther	Gomez
Michael & Dolores	Gomez
Michael M & Diane R	Gomez
Yolanda B	Gomez
.lne	Gomez, Jr
Michael H & Isabel A	Gongora
Adalherto	Gonzales
Adam C	Gonzales
Auaill C	GUIZARS

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Al	Gonzales
Alonzo	Gonzales
Benny	Gonzales
Bertha Gutierrez	Gonzales
Carlos E & Sandy M	Gonzales
David R	Gonzales
Elias Jacob & Joy	Gonzales
Elias Jacob & Joy Elpidia G	Gonzales
Esther	Gonzales
Eugene E	Gonzales
	Gonzales
Evangelina M	Gonzales
Henry V & Betty J	
Joe R & Leticia M	Gonzales
Leroy & Edna G	Gonzales
Louis Ortiz & Gloria G.	Gonzales
Lydia T	Gonzales
M Nelda	Gonzales
Maria A	Gonzales
Maria Concepcion	Gonzales
Michael A	Gonzales
Pablo A & Esther	Gonzales
Pete	Gonzales
Robert C	Gonzales
Roger	Gonzales
Rosalinda	Gonzales
Rudolfo & Isabel	Gonzales
Rudy	Gonzales
Charles Anthony	Gonzales, Jr
Gilbert	Gonzales, Jr
Andres R & Enedina B	Gonzalez
Abelardo Duarte	Gonzalez
Alejandro & Gloria	Gonzalez
Alejandro & Gloria E	Gonzalez
Alma	Gonzalez
Arthur A	Gonzalez
Daniel	Gonzalez
David Javier	Gonzalez
Diana	Gonzalez
Douglas Carrillo	Gonzalez
Elizabeth V & Rodolfo Z	Gonzalez
Eriberto & Enelida	Gonzalez
Esteban	Gonzalez
Francisco R	Gonzalez
Gerardo & Maria E	Gonzalez
Jacinto & Lucia R	Gonzalez
Jamie Leal	Gonzalez
Jose	Gonzalez

Jose C & Martha C	Gonzalez
Juan Ramon & Raquel E	Gonzalez
Julian Guadalupe	Gonzalez
Kelly	Gonzalez
Luis M & Herlinda	Gonzalez
Oscar S	Gonzalez
Paul A	Gonzalez
Perfecta S	Gonzalez
Rafael	Gonzalez
Robert C & Gina C	Gonzalez
Robert J & Elena	Gonzalez
Rosa	Gonzalez
Rudy & Irma	Gonzalez
Sally	Gonzalez
Santos Rosalio	Gonzalez
Sergio & Azucena	Gonzalez
Sixto	Gonzalez
Frank F	Gonzalez Gonzalez, III
Victor H	Gonzalez, III Gonzalez, Jr
Alfonso & Alma Karina	Gonzalez, 31 Gonzalez-Betancourt
Amanda	Goodman
Marie M	Goodman
William F	Gorton
Daniel M & Sylvia M	Govea
David & Diane	Gracia
Bruce & Marion	Grady
Irene G	Granado
Krista Rae	Greenwell
Alan Greg	Grigorian
Michael	Guajardo
Andres	Guardado
Andres & Martha C	Guardado
Emilio Antonio	Guardiola
Gilbert	Guardiola
Jo Ann	Gubanche
Carlos	Guedea
Carlos & Paulette K	Guedea
Manuel	Guereca
Anna M	Guerra
Karina	Guerra
Leonard & Maria A	Guerra
Martha G	Guerra
Nora	Guerra
Raul Eduardo	Guerra
Samantha Graciela	Guerra
Agustin	Guerra, III
Jesse R	Guerra, Sr

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Henry D Guerrero Henry D Guerrero Irving Aleman Guerrero Irving Aleman Guerrero Irving Aleman Guerrero Un Benjamin Fores Guerrero Valente Jamie Guerrero Guillen Mariekz Guillen Guillenrez Jason Gumbardo Jim Maose Gunnels Adam Guillenrez Andrea Andrea Guillenrez Andrea Guillenrez Andrea Guillenrez Andrea Guillenrez Andrea Guillenrez Andrea Guillenrez Guillenrez Berigno & Anna Maria Guillenrez Berigno & Anna Maria Guillenrez Guillenrez Gerepa & Jamina Guillenrez Gerepa & Jusania Guillenrez Janie Guillenrez Allania Guillenrez Jusan Guillenrez Jusan Guillenrez Remond Allania Guillenrez Remond Allania Guillenrez Paul A Guillenrez Remond Y Guillenrez Eloy Guillenrez Eloy Guillenrez Eloy Guillenrez Eloy Guillenrez Eloy Guillenrez Ligrania Maria Guillenrez Halii Guillenrez Halii Guillenrez Halii Guillenrez Halii Franciacio Guillenren Theodore R Hansford Theodore R Hansford Theodore R Harsford	Emilia	Guerrero
Inving Aleman Petra Irene Guerrero Ull Benjamin Flores Guerrero Valente Jarnie Guerrero Guerrero Jarnie Guerrero Jarnie Guerrero Guerrero Jarnie Guerrero Guerrero Jarnie Guevara Roxanne M Guevara Guerrer Guerrero Jannie Guerrer Guerrero Jannie Guerrero Jannete Guillen Marieluz Janente Guillen Guerrero Jannels Guinerez Jannels Guinerez Andran Guinerez Andran Andran Guinerez Antrano Guinerez Antrano Guinerez Guerrero Antrano T & Anna Guinerez Guinerez Gererobe Bering J & Olivia A Guinerez Gererobe Gererobe Gererobe Guinerez Jannie Guinerez John James Guinerez John James Guinerez John James Guinerez Jose Aberto Guinerez	Henry D	Guerrero
Petra linne Uti Benjamin Fibres Valente Uti Benjamin Fibres Guerrero Valente Guerrero Jarnie Guevara Guillen Mariekz Guillen Guillen Gundels Adam Guterez Andrea Andrea Andrea Guillen Adam Guterez Andrea Guillen Andrea Guillen Andrea Guillen Andrea Andrea Andrea Guillen Andrea Guillen Guill	Humberto	Guerrero
Uit Benjamin Flores Valente Valente Jamine Guerrero Jamine Guerrero Guerrero Guerrero Guerrero Guerrero Guerrero Guerrero Guerrero Guerrero Jamine Guerrero Jamon Janette Guillen Marieluz Jason Gunbertez Anden Gulderrez Anden Addam Addam Gulderrez Armando R. & Anna M. Guderrez Armando R. & Anna Guderrez Junine Guderrez Junine Guderrez Junine Guderrez Jose Aberto Guderrez Junine Guderrez Junine Guderrez Junine Guderrez Junine Guderrez Junine Guderrez Guderrez Junine Guderrez Junine Guderrez Handren Guderrez Handren	Irving Aleman	Guerrero
Valente Jamie Quevara Roxannon M Guevara Janete Guevara Janete Guillen Mariekz Guillen Guillen Guillen Guillen Gurbardo Jim Maxie Gunnels Adam Guinerez Andrea Andrea Andrea Andrea Andrea Andrea Andrea Guilerrez Andrea Andrea Andrea Guilerrez Guilerrez Guilerrez Guilerrez Guilerrez Genigne & Anna Maria Guilerrez Gerigne & Anna Maria Guilerrez Genigne & Anna Maria Guilerrez Gentres Guilerrez Guilerrez Genope & Juania June Guilerrez John James Guilerrez John James Guilerrez Jose Alberto Guilerrez Jose Alberto Guilerrez Autin Guilerrez Ramna Maria Sucha Guilerrez Raymond & Rebecca Rebenca & Guilerrez Guilerrez Ruymond & Rebecca Roymond & Rebecca Roymond & Rebecca Guilerrez Guilerre	Petra Irene	Guerrero
Jamie Geward Roxanne M Guevard Anancte Guillon Marieluz Jason Gumbardo Jim Maxie Guillon Jim Maxie Guillon Jim Maxie Gunnels Adam Guiterrez Andrea Guiterrez Juhn Janne Guiterrez Juhn Andrea Guiterrez Juhn Andrea Guiterrez Harin Martha S Guiterrez Marin Allartha S Guiterrez Ramona Y Guiterrez Yuri Guiterrez Filoria Guiterrez Yuri Guiterrez Harin Guiterrez Guit	Uri Benjamin Flores	Guerrero
Roxanne M Guevarra Janette Guillen Marielux Guiterrez Jason Gurnbardo Jim Masie Gunnels Adam Gunnels Adam Guiterrez Andrea Adam Guiterrez Armando R. & Ana M. Guiterrez Admando R. & Ana M. Guiterrez Cuterrez Armando R. & Ana M. Guiterrez Armando R. & Ana M. Guiterrez Cuterrez Cuterrez Britan J. & Olivia A Guiterrez Guiterrez Guiterrez Guiterrez Guiterrez Guiterrez Guiterrez Janie Janie Jose Aberto Guiterrez John James Guiterrez Juan Guiterrez Juan Guiterrez Guiterrez Guiterrez Juan Guiterrez Fend A Guiterrez Ramona Y Guiterrez Ramona Y Guiterrez Remona Y Guiterrez Remona Guiterrez Remona Guiterrez Remona Guiterrez Guiterrez Feloy Guiterrez	Valente	Guerrero
Junette Gullen Marieluz Gulterez Jason Gurnbardo Jim Maxie Adam Gurnels Adam Gulterez Andrea Gulterez Genobe Gulterez Genobe Gulterez Genobe Justerez Genobe Justerez Juni Janie Gulterez Juni Janie Gulterez Juni Andrea Berigno Andrea Andrea Andrea Andrea Andrea Andrea Berigno Gulterez Andrea Andrea Andrea Andrea Andrea Andrea Andrea Berigno Gulterez Andrea Andrea Andrea Gulterez Andrea Andrea Gulterez Andrea Gulterez Andrea Gulterez Andrea Andrea Gulterez Andrea Andrea Gulterez Andrea Gulterez Andrea Andrea Gulterez Andrea Andrea Gulterez Andrea Andrea Gulterez Andrea Andrea Andrea Gulterez Andrea Andr	Jamie	Guevara
Marieluz Guiterrez Jason Gumbardo Jim Maxie Gunnels Adarin Guiterrez Andrea Guiterrez Andrea Guiterrez Andrea Guiterrez Andrea Guiterrez Andrea Guiterrez Antrando R. & Aria M. Guiterrez Antrando R. & Aria M. Guiterrez Berigno & Arna Maria Birian J & Olivia A. Guiterrez Guiterrez Berigno & Arna Maria Guiterrez Guiterrez Guiterrez Guiterrez Guiterrez Guiterrez Genobebo Guiterrez Guiterrez Guiterrez Janie Guiterrez Janie Guiterrez Jose Aberto Guiterrez Jose Aberto Guiterrez Jose Aberto Guiterrez Raman Guiterrez Maria A Martha Guiterrez Maria A Martha Guiterrez Remona Y Guiterrez Remona Y Guiterrez Remona Y Guiterrez Remona S Rebecca Guiterrez Remona Guiterrez Guiterrez Fichard & Gloria Guiterrez Guiterrez Guiterrez Remona Guiterrez Guiterrez Remona Guiterrez	Roxanne M	Guevara
Jason Gumbardo Jam Maoie Gumbardo Jam Maoie Gumbardo Jam Maoie Gumolais Adam Gusterrez Andrea Gusterrez Ammando R. & Ana M. Afturo T & Anna Benigno & Anne Maria Benigno & Anne Maria Benigno & Anne Maria Gusterrez Brian J & Olivia A Gusterrez Genoga & Justina Jusine Gusterrez Genopa & Justina Jusine Gusterrez Jusine Gusterrez Jusine Gusterrez Jusine Gusterrez Jusine Gusterrez Jusin Gusterrez Jusine Gusterrez Jusin Gusterrez Gusterrez Famona Y Gusterrez Ramona Y Gusterrez Ramona Y Gusterrez Reignon & Rebecca Richard & Gloria Fichian Gusterrez Gusterrez Filoy Gusterrez Gusterrez Harian Gusterrez Haria Gusterrez Gusterrez Gusterrez Gusterrez Harian Gusterrez Gusterrez Gusterrez Harian Gusterrez Gusterrez Harian G	Janette	Guillen
Jim Maxie Jim Maxie Jim Maxie Gunnels Adam Gutierrez Andrand R. & An M. Gutierrez Antrano R. & An M. Gutierrez Antrano R. & Anna Gutierrez Sentjoo & Anna Maria Bentjoo & Anna Maria Gutierrez Bentjoo & Anna Maria Gutierrez Carlos A. & Maria G. Gutierrez Genobebo Gutierrez Genobebo Gutierrez Janrie Gutierrez Janrie Gutierrez Janrie Gutierrez John James Gutierrez Jose Aherto Justierrez Jose Aherto Gutierrez Jose Aherto Gutierrez Gutierrez Haria Lutias Gutierrez Maria Kartha S. Gutierrez Ramona Y. Gutierrez Ramona Y. Gutierrez Yuri Gutierrez Yuri Gutierrez Yuri Gutierrez Haria Gutierrez Haria Gutierrez Gutierrez Yuri Gutierrez Gutierrez Yuri Gutierrez Haria Gutierrez H	Marieluz	Guiterrez
Adam Gutierrez Andrea Gutierrez Armando R. A Ana M. Gutierrez Armando R. A Ana M. Gutierrez Adrian T. & Anna Gutierrez Beringo A Anna Maria Gutierrez Brian J. & Olivia A Gutierrez Genorgo A Sunania Gutierrez Genobebo Gutierrez Genobebo Gutierrez Genobebo Gutierrez Junia Junia Junia Junia Gutierrez Jose Alberto Gutierrez Jose Alberto Gutierrez Gutierrez Jose Alberto Gutierrez Jose Alberto Gutierrez Jose Alberto Gutierrez Jose Alberto Gutierrez Gutierrez Jose Alberto Gutierrez Jose Alberto Gutierrez Gutierrez Gutierrez Gutierrez Gutierrez Gutierrez Ramna S Gutierrez Ramna Maria S Gutierrez Raymond & Rebecca Routierrez Gutierrez Fely Gutierrez Haria Gutierrez Gutierrez Gutierrez Gutierrez Gutierrez Gutierrez Haria Gutierrez Gutierrez Haria Gutierrez Gutierrez Haria Gutierrez Haria Harria Gutzman Gutierrez Haria Harria Gutzman Harria Gutzman Harria Harria Harria Harria	Jason	Gumbardo
Andrea Gutierrez Armando R. & Anna M. Gutierrez Armando R. & Anna M. Gutierrez Benigno & Anna María Benigno & Anna María Benigno & Anna María Brian J. & Givia A Gutierrez Carlos & María G Gutierrez Genobelo Gutierrez Genobelo Gutierrez Genobelo Gutierrez Genobelo Janire Gutierrez Janire Janire Juan Gutierrez John James Gutierrez John James Gutierrez John James Gutierrez Jose Aberto Gutierrez Jose Aberto Gutierrez Gutierrez Harrin Allastha Gutierrez Gutierrez Harrin Gutierrez Marin Allastha Gutierrez Marin Allastha Gutierrez Marin Allastha Gutierrez Harrin Gutierrez Fead A Gutierrez Yuri Gutierrez Yuri Gutierrez Holderez Eloy Gutierrez Harrin Gutierrez Harrin Marin Gutierrez Gutierrez Gutierrez Gutierrez Harrin Gutierrez Gutierrez Gutierrez Gutierrez Harrin Gutierrez Gutierrez Gutierrez Harrin Harrin Gutierrez Gutierrez Harrin Harrin Gutierrez Harrin Harri	Jim Maxie	Gunnels
Armando R. & Ana M. Gutierrez Arturo T & Arna M. Gutierrez Berigno & Anna Maria Berigno & Anna Maria Brian J & Olivia A Gutierrez Carlos & A Maria G Gutierrez Carlos & A Maria G Gutierrez Carlos & A Maria G Gutierrez Genobebo Gutierrez Genobebo Gutierrez Genobebo Gutierrez Janie Janie Janie John James Gutierrez Gutierrez Aller Gutierrez Gutierrez Gutierrez Gutierrez Gutierrez Aller Gutierrez Marin & Marrina S Gutierrez Ramona Y Gutierrez Ramona Y Gutierrez Remona Y Gutierrez Remona S Robecca Gutierrez Gutierrez Fichard & Gloria Gutierrez Gutierr	Adam	Gutierrez
Antro T A Anna Berigno A Anna Maria Berigno A Anna Maria Berigno A Anna Maria Gutierrez Carlor A & Maria G Gutierrez Carlor A & Maria G Gutierrez Genchebo Gutierrez Genchebo Gutierrez Genchebo Gutierrez Genchebo Janie Janie Janie Gutierrez Jun Gutierrez Jun Gutierrez Jun Gutierrez Haria Luia Gutierrez Maria S Gutierrez Ramona Y Gutierrez Ramona Y Gutierrez Ramona S Feloria Gutierrez Haria Gutierrez Gutierrez Gutierrez Gutierrez Gutierrez Gutierrez Gutierrez Gutierrez Gutierrez Haria Gutierrez Gutierrez Haria Gutierrez Haria Haria Gutierrez Haria Haria Gutierrez Haria Haria Haria Haria Haria Haria Haria Haria	Andrea	Gutierrez
Benigno & Anna Maria Brian J & Olivia A Gutierrez Carlos A & Maria G Gutierrez Carlos A & Maria G Gutierrez Carlos A & Maria G Gutierrez Gutierrez George & Juanita Gutierrez George & Juanita Jarie Gutierrez George & Juanita Gutierrez John James Gutierrez John James Gutierrez Josh Alema Gutierrez Josh Alema Gutierrez Gutierrez Gutierrez Gutierrez Gutierrez Gutierrez Gutierrez Maria & Martha S Gutierrez Maria & Martha S Gutierrez Maria & Martha S Gutierrez Gutierrez Famona Y Gutierrez Ramona Y Gutierrez Ramona S Rebecaa Gutierrez Richard & Gloria Gutierrez Richard & Gloria Gutierrez Filoy Gutierrez Gu	Armando R. & Ana M.	Gutierrez
Britan J & Olivia A Carlos A & Maria G Carlos A & Maria G Christina Cozaro Genchebo Gutierrez Genchebo Gutierrez Genchebo Gutierrez Junie Junie John James John James Gutierrez Juso Jose Alberto Gutierrez Juso Gutierrez Juso Gutierrez Juso Gutierrez Juso Gutierrez Maria Sudra Gutierrez Ramona Y Gutierrez Ramona Y Gutierrez Ramona Y Gutierrez Ramona S Gutierrez Roymond & Rebecca Roymond & Rebecca Roymond & Roberca Gutierrez Yuri Gutierrez Filoy Gutierrez H Aurora Guzzman Jeanette J Guzzman Martin Guzzman Martin Guzzman Guzzman Genzman Hartin F & Rosa M Guzzman Francisco Guzzman Hartin Hartin Guzzman Hartin Guzzman Hartin Guzzman Hartin Guzzman Hartin Hartin Guzzman Hartin Guzzman Hartin Hartin Guzzman Hartin Hart	Arturo T & Anna	Gutierrez
Carloe A & Maria G Christina Lozano Guiderrez Genobebo Guiderrez Genobebo Guiderrez George & Juanita Jaraie Guiderrez John James Guiderrez John James Guiderrez John James Guiderrez Jose Aberto Guiderrez Jose Aberto Guiderrez Jose Aberto Guiderrez Maria Luisa Guiderrez Maria Kartha S Guiderrez Guiderrez Famona Y Guiderrez Guiderrez Guiderrez Famona Y Guiderrez Guiderrez Famona Y Guiderrez Hanton Guiderrez Guiderrez Guiderrez Guiderrez Hanton Guiderrez Handon Hanton Guiderrez Handon Handon Guiderrez Handon Hangdon Hangdon Theodore R Handord	Benigno & Anna Maria	Gutierrez
Christina Lozano Gusterrez Genozbe Ausnita Gusterrez Genozbe Ausnita Jurie Jurie John James Gusterrez Josh James Gusterrez Josh Aberto Gusterrez Josh Alames Gusterrez Josh Alames Gusterrez Josh Alames Gusterrez Josh Alames Gusterrez Marin & Marrha S Gusterrez Marin & Marrha S Gusterrez Ramona Y Gusterrez Ramona Y Gusterrez Remona Y Gusterrez Richard & Gloria Gusterrez Richard & Gloria Gusterrez Virti Gusterrez Yuri Gusterrez Holderrez Holderrez Gusterrez Hantin Gusterren Gusterren Gusterren Gusterren Gusterren Gusterren Gusterren Gusterren Hantin Gusterren Hantin Roger & Mañal J Gusterren Hantin Gusterren Hantin Gusterren Hantin Gusterren Hantinon Hantinon Theodore R	Brian J & Olivia A	Gutierrez
Genobebo Gutierrez George & Juanita Janire Gutierrez Janire Gutierrez John James Gutierrez John James Gutierrez Jose Aberto Gutierrez Juan Gutierrez Juan Gutierrez Martin & Luisa Gutierrez Martin & Martin & Gutierrez Martin & Martin & Gutierrez Martin & Martin & Gutierrez Martin & Gutierrez Martin & Gutierrez Martin & Martin & Gutierrez Ramona Y Gutierrez Ramona Y Gutierrez Yuri Gutierrez Yuri Gutierrez Yuri Gutierrez Gutierrez Gutierrez Gutierrez Gutierrez Gutierrez Gutierrez Hamrin Gutierrez Gutierrez Gutierrez Hamrin Gutierrez Gutierrez Gutierrez Hamrin Gutierrez Gutierrez Hamrin Gutierrez Gutierrez Hamrin Gutierrez Gutierrez Hamrin Gutierrez Hamrin Gutierrez Hamrin Hartin Gutierrez Gutierrez Hamrin Hartin Gutierrez Hamrin Hartin Gutierrez Hamrin Hartin Gutierrez Hamrin Hartin Ha	Carlos A & Maria G	Gutierrez
George & Jusinita Jarie Jarie Jurie Gutierrez John James Gutierrez Joen Alleria Joen Alleria Joen Alleria Gutierrez Jusin Gutierrez Jusin Gutierrez Maria Gutierrez Maria Gutierrez Maria Martha S Gutierrez Marin & Martha S Gutierrez Ramona Y Gutierrez Ramona S Rebecaa Gutierrez Richard & Gloria Gutierrez Richard & Gloria Gutierrez Richard & Gloria Gutierrez Richard & Gloria Gutierrez Gutierrez Horia Gutierrez Haria Garret M & Katilyn D Hall Glaret M & Katilyn D Hansford Theodore R	Christina Lozano	Gutierrez
Janie Gutierrez John James Gütierrez Jose Alberto Gutierrez Juan Gutierrez Juan Gutierrez Gutierrez Maria Lutia Gutierrez Maria Sutierez Maria Sutierez Ramona Y Gutierrez Ramona Y Gutierrez Ramona Y Gutierrez Ramona Y Gutierrez Roymond & Rebecca Roymond & Robecca Gutierrez Gutierrez Yuri Gutierrez Eloy Gutierrez Gutierrez Gutierrez Gutierrez Gutierrez Gutierrez Gutierrez Gutierrez Gutierrez Haria Gutierrez Haria Aurora Guzman Guzman Martin Guzman Martin Guzman Martin Guzman Martin Gutierre Guzman Martin Guzman Haria Guzman Haria Guzman Haria Guzman Haria Guzman Haria Guzman Haria Haria Guzman Haria Guzman Haria Haria Guzman Haria Haria		Gutierrez
John James John James John James Juan Gutierrez Juan Gutierrez Maria Luisa Gutierrez Maria Luisa Gutierrez Maria Martha S Gutierrez Ramona Y Gutierrez Ramona Y Gutierrez Ramona Y Gutierrez Ramona Y Gutierrez Richard & Gloria Gutierrez Richard & Gloria Gutierrez Richard & Gloria Gutierrez Hartin Gutierrez Guzman Grissian Guzman Geristian Guzman Martin F & Rosa M Guzman Francisco Guzman Francisco Guzman Haria Naveren A Garrett M & Kalthyn D Hall Garrett M & Kalthyn D Hall Glansford Hansford Theodore R	George & Juanita	Gutierrez
Jose Alberto Juan Juan Gutierrez Juan Gutierrez Maria Lutia Gutierrez Maria Lutia Gutierrez Maria Santia Paul A Gutierrez Ramona Y Gutierrez Ramona Y Gutierrez Raymon & Rebecca Raymon & Rebecca Roymon & Rebecca Gutierrez Yuri Gutierrez Floy Gutierrez Hantin Barren Gutierrez Gutierrez Gutierrez Gutierrez Gutierrez Hantin Barren Gutierrez Gutierr	Janie	Gutierrez
Juan Gulderrez Meria Luisa Guiterrez Meria Luisa Guiterrez Marin & Martha S Guiterrez Paul A Guiterrez Rannona Y Guiterrez Ramnona S Rebecca Guiterrez Romnond & Rebecca Guiterrez Vuri Guiterrez Eloy Guiterrez Eloy Guiterrez, Ir Martin Guiterrez, Ir Aurora Guzman Jaenetle J Guzman Jaenetle J Guzman Martin Guzman Martin Guzman Harria Findicco Guzman Harria Guzman Findicco Guzman Harria Guzman Harria Findicco Guzman Harria Harria Guzman Harria Guzman Findicco Guzman Harria Harria Guzman Harria Guzman Harria Harria Guzman Harria	John James	Gutierrez
Meria Luisa Gutierrez Marin A Martha S Paul A Gutierrez Ramona Y Gutierrez Ramona Y Gutierrez Ramona S Rebeca Gutierrez Richard & Gloria Vori Gutierrez Richard & Gloria Gutierrez Richard & Gloria Gutierrez Vori Gutierrez Gutierrez Gutierrez Harin Gutierrez Guzman Christian Guzman Harin Guzman Francisco Guzman Harin Guzman Harin Garrett M & Katilyn D Hall Glaretor H Harington Theodoro R	Jose Alberto	Gutierrez
Martin & Martha S Guilderrez Ramona Y Guiderrez Ramona Y Guiderrez Ramona S Royer S Goria Guiderrez Yuri Guiderrez Yuri Guiderrez Yuri Guiderrez Eloy Guiderez Eloy Guiderez Aurora Aurora Guzman Guzman Grissian Guzman Guzman Martin Guzman Martin Guzman Harris Harris Harris Harris Guzman Harris		Gutierrez
Paul A Gutierrez Ramona Y Gutierrez Ramona Y Gutierrez Raymond & Rebecca Gutierrez Richard & Gloria Gutierrez Richard & Gloria Gutierrez Vuri Gutierrez Eloy Gutierrez, ut Martin Gutierrez, ut Aurora Guzman Christian Guzman Gristian Guzman Martin F & Rosa M Guzman Martin F & Rosa M Guzman Martin F & Rosa M Guzman Francisco Guzman Francisco Guzman Naveren A Haji Garrett M & Kalthyn D Hali Sinadama Hampton Theodore R Hansford		
Ramona Y Gutierrez Rammond & Rebecca Gutierrez Richard & Gloria Gutierrez Yuri Gutierrez Eloy Gutierrez, dr. Martin Gutierrez, dr. Aurora Guzman Christian Guzman Guzman Guzman Guzman Guzman Guzman Martin Guzman Martin Guzman Harria Guzman Martin Guzman Harria Harria Guzman Francisco Guzman, III Nevreen A Haji Garrett M & Katilyn D Hall Shadana Hampton Theodore R		
Raymond & Rebecca Richard & Gloria Guiterrez Yuri Guiterrez Eloy Guiterrez Bloy Guiterrez, Jr Autora Guzenan Guzenan Guzenan Guzenan Guzenan Jeanete J Guzenan Maria Maria Guzenan Maria Guzenan Maria Guzenan Guzenan Guzenan Haria Guzenan Haria Guzenan Haria Guzenan Haria Guzenan Roper & Maria J Guzenan Guzenan Hagi Garrett M & Kalityn D Hall Shadena Hampton Theodore R		
Richard & Gloria Gutlerrez Yori Gutlerrez Eloy Gutlerrez, Ir Martin Gutlerrez, Jr Aurora Guzman Christian Guzman Christian Guzman Martin Guzman Martin Guzman Martin Guzman Martin Guzman Martin & Rosa M Guzman Francisco Guzman Francisco Guzman Martin & Rosa M Guzman Francisco Guzman Francisco Haria J Guzman Francisco Haria J Guzman Francisco Guzman Haria Haria Garrett M & Katilyn D Hall Shadama Hampton Thacodore R		Gutierrez
Yuri Gutierrez		Gutierrez
Eloy Gutlerrez, Jr Martin Gutlerrez, Jr Aurora Guzman Christian Guzman Christian Guzman Jeanette J Guzman Martin F & Rosa M Guzman Martin F & Rosa M Guzman Martin F & Rosa M Guzman Francisco Guzman, III Navreen A Haji Garrett M & Kalthyn D Hali Sinadama Hampton Thacdore R Hansford		
Martin Guiterez, ir Aurora Guzman Christian Guzman Guzman Jeanette J Guzman Maria Guzman Maria Guzman Martin & Roger & Maria J Guzman Martin & Roger & Maria J Guzman Francisco Guzman, III Navreen A Haji Garrett M & Katilyn D Hall Shadana Hampton Theodore R Hansford		Gutierrez
Aurora Guzman Christian Guzman Jeanete J Guzman Maria Guzman Maria Guzman Maria Guzman Marin F S Rosa M Roper & Maria J Guzman Francisco Guzman, III Naveen A Haji Garrett M & Kalityn D Hall Shadena Hampton Theodore R Hansford	Eloy	Gutierrez, Jr
Christian Gizzman Jeanette J Guzman Marin Guzman Martin F & Rosa M Guzman Roger & Mariu J Guzman Francisco Guzman, III Nevreen A Haji Garret M & Katilyn D Hali Shadana Hampton Theodore R Hansford	Martin	Gutierrez, Jr
Jeanette J Guzman Maria Guzman Maria F Rosa M Guzman Roper & Maria J Guzman Francisco Guzman, III Naveren A Haji Garret M & Kaitlyn D Hall Shadana Hampton Theodore R Hanstord		
Maria Guzman Marin F & Rosa M Guzman Rosper & Maria J Guzman Francisco Guzman, III Naveren A Haji Garrett M & Kalityn D Hall Sinadama Hampton Theodore R Hansford	Christian	Guzman
Martin F & Rosa M Guzman Roger & Maria J Guzman Francisco Guzman, III Navreen A Haji Garrett M & Kashyn D Hall Shadana Hampton Theodore R Hansford		
Roger & Maria J Guzman Francisco Guzman, III Navreen A Haji Garrett M & Kaitlyn D Hall Shadana Hampton Theodore R Hansford		
Francisco Guzman, III Navreen A Haji Garrett M & Kaithyn D Hall Shadana Hampton Theodore R Hansford		
Navreen A Haji Garrett M & Kaitlyn D Hall Shadana Hampton Theodore R Hansford		
Garrett M & Kaitlyn D Hall Shadana Hampton Theodore R Hansford		
Shadana Hampton Theodore R Hansford		
Theodore R Hansford		
Fred & Patricia A Hardin		
	Fred & Patricia A	Hardin

Richard & Margarita	Harlow
Scott & Ashley	Harmeyer
Carlos	Harris
Leonard & Vernon L	Harris
Mekcos Mardell	Harris
Sharon L	Harrison
Henry B	Harrison, Sr
Debbie S & Kevin K	Hart
Kathryn Riley	Hayes
Richard Ewing &	Hector Flores
Amy R	Heflin
Derrick	Hegmon
Payam & Gol Laleh	Heidari
Trinie A	Hellman
William E & Rachelle	Helmick
Patrick & Romo Anna Marie	Hemby
Thomas	Henao
William R & Kathy	Henderson
Richardo & Eulalia	Heredia
Sandra K & Ricky M	Hermosa
Abraham & Nubia	Hernandez
Alberto & Norma	Hemandez
Alexandria Alberto Chiguichon	Hemandez
Alfredo	Hemandez
Alfredo	Hemandez
Alvaro A	Hemandez
Alvaro R & Carmen A	Hemandez
Angelina	Hemandez
Argentina	Hemandez
Argentina & Juan T	Hemandez
Aurora	Hemandez
Belinda H	Hemandez
Carrie Jovita	Hemandez
Connie M	Hemandez
Cynthia Ann	Hemandez
David	Hemandez
Elida	Hemandez
Elsa	Hernandez
Emma	Hemandez
Enrique Ramirez	Hemandez
Felix C	Hemandez
Filomeno & Anna Maria	Hemandez
Florinda Garcia	Hemandez
Frances	Hemandez
Frank	Hemandez
Gabina R	Hernandez
Genoveva	Hemandez

Gloria M	Hernandez
Guadalupe Eva	Hernandez
Gustavo Leija & Celia Huerto	Hernandez
Hector H	Hernandez
Hector J. & Maria S.	Hernandez
Humberto	Hernandez
Irma	Hernandez
Jaime	Hernandez
Jaime & Rafael	Hernandez
Janie	Hernandez
Jose & Maria	Hernandez
Joseph D	Hernandez
Josephine R	Hernandez
Juan & Roxanna	Hernandez
Juan A	Hernandez
Juan G & Roxana	Hernandez
Juan Garza & Helen Ann	Hernandez
Juan T & Argentina V	Hernandez
Linda Lou	Hernandez
Manuela M.	Hernandez
Margarita	Hernandez
Maria D	Hernandez
Maria De La Luz	Hernandez
Mario & Lesly	Hernandez
Mark & Denise	Hernandez
Martin V	Hernandez
Martin Vasquez & Emilia Marquez	Hernandez
Melissa	Hernandez
Michael P	Hernandez
Mike G & Irma I	Hernandez
Nadine J	Hernandez
Ninfa	Hernandez
Oscar & Amanda Marie	Hernandez
Paul	Hernandez
Rafael	Hernandez
Rafael & Jaime	Hernandez
Rafael & Rafaela	Hernandez
Ramon & Margarita	Hernandez
Raul Sanchez	Hernandez
Rocio Rocha & Javier A Rocha	Hernandez
Rogelio V	Hernandez
Rojelio G	Hernandez
Rosa A	Hernandez
Rosa Campos	Hernandez
Rosendo	Hernandez
Ruben & Rachel	Hernandez
Rudy M & Judee E	Hernandez

Simon G	Hernandez
Simona G	Hernandez
Timoteo & Carmen	Hernandez
Tony	Hernandez
Vanessa J	Hernandez
Veronica Jo	Hernandez
Yolanda & Jaime R	Hernandez
Victor R	Hemandez, II
Armen Garcia	Herrera
Gloria E	Herrera
Guadalupe Cardenas	Herrera
Joe	Herrera
Lee & Pauline G	Herrera
Mark	Herrera
Osvaldo R	Herrera
Pauline	Herrera
Roman & Alma	Herrera
Modesto	Herrera, Jr
Jose Manuel	Herrera, Sr
Hector Manuel	Herrera-Fernandez
Jason	Hiatt
Sergio & Claudia	Hidalgo
Sandy C, Carolyn B, &	Hill
Creston C Robert	1191
Estella	Hillyer
	Hinojosa
Ivan C & Angelina	Hinojosa Hinton
Terry W	
Zachary M	Hlavinka
Christina L	Ho
Adelina L	Hodges
Enrique & Martha L	Hoffman
George E	Hogan
Rosalinda	Holguin
Theresa L	Hooper
Dilder & Hasan Mehmuda	Hossain
Larry G & Julieta	Housley
Maya	Hsu
Yung Chien Tai &	Hua Lee Mei
Joshua	Hubbard
James D	Hubberd
Nancy Arias & William	Huber
William Lee	Huber
Nancy	Hudspeth
Arturo & Olivia	Huerta
Francisco	Huerta
Francisco M	Huerta
Francisco M	Huerta
Francisco M	Huerta

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Francisco M	Huerta
Francisco M	Huerta
Jesus M & Rita	Huerta
Victor Javier	Huerta Alcazar
Julia R & Oscar N	Huizar
Peter A & Linda L	Huizar
Rose Mary G &	Huizar
Charles P	Huron
Roger Frank & Carmen	Huron
Kum Sun	Hurst
Jose A & Norma A	Hurtado
Abigail R Espinoza &	Hurtado Reyes, Jr
Imtiaz	Hussain
Gary Mitchell & Brooke	Hydrick
Sammye Ruth	Hynes
Andres Alexis	Ibarra
Cecilia V	Ibarra
Edward J	Ibarra
Gerardo	Ibarra
Oralia A	Ibarra
Raul	Ibarra
Samuel C & Maria	Ibarra
Veronica	Ibarra
Walter H	Ibarra
Matin	Ibarrola
Luis Raul & Sara	Iglesias
Eusebio Garcia &	Iliana S Martinez
Vera	Iltis
Venise-Grace	Ingan
Refugio	Ipina
Martin	Iracheta
Juan S	Irineo
Arthur P Mazuca &	Irma M Garcia
Wilfredo A Sepulveda &	Irma Paredes
Carlos Campos-Davalos &	Irma Valdez-Cruz
Astin Genove	Irving
Juan O & Mary A	Isaac
Ashley & Benito	Islas
Cirilo	Islas
Cirilo Vargas	Islas
Cirilo Vargas	Islas
Cirilo Vargas & Josefa C	Islas
Khitam A & Mohammed	Ismail
Martiin & Lydia	Ituarte
Pablo	Iturbide
Michael D & Libertad	Ivarra
Sergio A	Izarraras
Mario & Maria	Izarraraz
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Mario & Maria De	IZBITATAZ
Ernestina Antu	Izquerdo
Ernestina Antu	Izquierdo
Victor Delgado &	Jackie Ramirez
Brenda Annette	Jackman
Norman & Sheryl	Jackson
Sean A	Jackson
Citlaytl Valenzuela &	Jacob Arocha
Meredith Hayles	Jacobsen
Matthew M Swartz &	Jaime A Rodriguez
Rebecca R Cerrillo &	Jaime Cerrillo Hernandez
Delia Nacianceno	Jalomo
Ruben Ysaac	Jalomo
Victor Isacc Flores	Jamarillo
Robert & Ruby	James
Wallace A & Micaela T	James
Veronica W Cisneros &	James Anthony Ravizee
Gwendolyn E	Janssen
Arnold & Felicita	Jasso
Eduardo R & Andrea I	Jauregui
Maria Del Carmen Mendez	Javier Rosendo
&	
Manuel Lopez &	Jeanette Robinson
Betty Sue	Jeffery
Patricia	Jenkins
Carlos Delarosa &	Jennifer A Campa
Andre & Jennifer	Jennings
Karin Gumbelevicius &	Jeremy Hammond
Yolanda Rivera &	Jesse Hernandez
Pedro Gonzales, II &	Jessica Raquel Martinez
Nicolette M	Jessie
Qinglai	Jiang
Michael Randolph Ivy &	Jim Lee
Cassandra	Jimenez
Abigail F	Jimenez
Alfred	Jimenez
Ana Soledad	Jimenez
Carmen & Henry	Jimenez
Delia	Jimenez
Eduardo & Rose Marie	Jimenez
Emilio Palacios	Jimenez
Gilbert	Jimenez
Jose Guadalupe Palacios	Jimenez
Marco Antonio & Edna	.limenez
Martinez	***************************************
Maria Teresa	Jimenez
Maria Z	Jimenez
Maricela S	Jimenez

Richard R & Maria M	Jimenez
Valentin Z	Jimenez
Randy & Natacha	Jimmerson
Graciela G	Jiron
Hector Colunga &	Jo Ann C Salas
Jennifer Jackson &	Joe Acosta
David R Casanova &	Johnny L Wilkins
Annie Jefferson	Johnson
Lawrence & Nadine	Johnson
Steven Ryan & Rachael	Johnson
Michelle Sumita T	Jonak
Thurman & Nancy A	Jones
Sadruddin K	Jooma
Glen G	
	Jorczak
Francisco	Jorge
Francisco & Mary J	Jorge
Cynthia Y Alanis &	Jorge H Segovia
G & Amelia	Jose
Maria Reyna Tinajero &	Jose Antonio Solorio
Gloria L. Alvarado &	Jose P. Gonzalez
Sandy Frances Jimenez, Ellen M Rios &	Juan C
Karla Angelica Romero &	Juana Soledad Palomino
Jesse A	Juarez
Jessica	Juarez
Juan D	Juarez
Norma	Juarez
Charles R	Juarez, Jr
Gerardo Castillo Rivera &	Judith Narvaez Arias
Juan Carlos Ramirez &	Judith Yaneth Herrera
George	Judson
Nidia Araksa Ramirez &	Julio Abraham Gonzalez
Jose Montelongo &	Karina Contreras
Juan M Hernandez Delacruz &	Kassandra Ramirez
J L & Dianna	Kaupert
David Wayne Lint &	Kelley E Pena
Edward	Kennedy
Kristi Renee	Kenney
Yesenia	Kem
Jay	Khadem
Isaiah & Essie Lee	Kidd
Long Tien	Kieu
Barbara Anne	Kill
Jimmy & Yen	Kim
Sylvia	King
Theora W	King
Brandon Lee	King
Gene G	Kingcaid

Matthew L & Victoria	Kleckner
Thomas W & Lynda D	Klimek
Tracy A & William D	
	Knight
Han Ma Eum	Korean
Monica	Kosta
Sanjay	Kumar
Michael D	Kung
Kalife	Kuri
Colin Sandford & Gloria A	Kyle
Jason Lee Brown &	Kylie B Grubb
Nicole C	Lackie
Josephine	Lafargue
Nisa N	Lagle
Luis A & Alicia G	Lainez
Ashley Ann & Larry D	Lamb
Kevin W & Kimberly K	Langbehn
Maria C	Lapenotiere
Cadena Johnny & Patsy Marie	Lara
Juan & Tina	Lara
Olivia C	Lara
Fernando & Irene	Lares
Juan Jose	Lares
Ralph G & Maria	Larochelle
Hao Chang Lan &	Laura Chen
Adolfo C Torres &	Laura J Segura
Juan J Arevalo &	Laura P Aleman
Cecilia Marie Becerra &	Lauren Catanza
Jerry D & Michelle	Laws
Christopher E	Lawton
Cesario	Lazaro
Duyen Ngoc	Lezaio
Arthur L	Leal
Irma Jean	
Irma Jean Jesus	Leal Leal
Priscilla S	Leal
Robert M	Leal
Ruben	Leal
Elizabeth M Hernanez &	Leal Adolfo Garza, Jr
David & Amy	Leavitt
Ricardo G & Amalia	Ledesma
Ricardo T & Sylvia	Ledesma
Debbie	Lee
Brian	Leeper
Don & Carla M	Leeseberg
Patricio	Leija
Zoila Beatriz & Maria Luz	Lemus

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Marciano & Maria	Leon
Enriqueta Jose R & Medina Julia	Leon-Zayas
Christina R & Ignacio E	Lerma
Lydia F	Lerma
Samuel Martinez Vigil &	Leticia Garza De Martinez
Consuelo M	Levine
Frank M & Rosalinda	Levine
Juanita I & Roderick D	Lewis
Kenneth M	Lewis
SL	Lewis
Michael A & Emmalyn T	Ligon
Gloria A Guevara &	Lilia A Mendez-Escobar
Carlos & Aurora	Limon
Constantino Gomez	Limon
Luis & Maria Luisa	Limon
Mario	Lincon
Mario	Lincon
Vicki Cisneroz &	Linda G Garcia
David Andrew Kotzur &	Linda Rodriguez
Robert	Lingo
Rojelio L	Lira
Alejandro	Livar
Rolando	Llanes
Evelyn Fisher	Long
Luz Maria	Long
Dora	Longoria
Grace	Longoria
Jose L, Eric M, & Christina M	Longoria
Leonides & Maria	Longoria
Maria	Longoria
Sofia	Longoria
Dagoberto & Ana L	Lopez
Alfonso O & Esther	Lopez
Alfredo & Herrera Nancy	Lopez
Arturo	Lopez
Arturo & Elizabeth	Lopez
Atanacio Ismael	Lopez
Benito	Lopez
Benito C & Oralia V	Lopez
Benny C	Lopez
Carmen J & Maria G	Lopez
Charles	Lopez
Desiree	Lopez
Edward E	Lopez
Edward G & Linda D	Lopez
Edward Rodriguez	Lopez
Guadalupe & Concepcion	Lopez

Isabel	Lopez
Jerardo & Janina A	Lopez
Jesse G & Jeanette R	Lopez
Jose & Maria D	Lopez
Jose P	Lopez
Juan Antonio	Lopez
Juan Carlos	Lopez
Juan F	Lopez
Juan Manuel Arroyo	Lopez
Juan R & Margarita	Lopez
Juanita	Lopez
Justo Sanchez	Lopez
Juventina O	Lopez
Livier Noemi Corona	Lopez
Magali	Lopez
Maria Isabel	Lopez
Maria Isela	Lopez
Mario Juarez	Lopez
Miguel E & Ruby	Lopez
Nicolas Rodgriguez	Lopez
Raul O	Lopez
Raymundo & Catalina	Lopez
Richard C	Lopez
Rosemary & Fidel A	Lopez
Ruben S.	Lopez
Ruben S. & Caroline D.	Lopez
Sandy	Lopez
Stephanie	Lopez
Susana	Lopez
Teresa	Lopez
Victor	Lopez
Louis	Lopez, Jr.
Oliver R	Lopez, Jr.
Saturnino	Loredo
Virginia	Loredo
Tomasa G	Losoya
Priscilla M & Joshua J	Lowery
Sanjuan Antonio	Lowery, Jr
Juanita	Lowry
Cesar	Loya
Helen	Lozano
Hortencia G	Lozano
Jamie	Lucio
Lucinda N	Lucio
Rebeca Malagon &	Lucio Plata
Javier & Mary D	Lugo
Mary S	Lugo
Rosario	Lugo

Diana M Gomez &	Luis Alejandro Sanchez
Cristina Lee Sandoval &	Luis Fernando White-
Abe Joe	Garcia Lujan
Jimmy G & Consuelo	Lujan
Annette G	Luna
Blanca E	Luna
Corina	Luna
David A	Luna
Estela M	Luna
Hilda	Luna
Jovita R	Luna
Mary E	Luna
Matilda	Luna
Ricardo C & Orlando	Luna
Rudy & Beatrice	Luna
Saul	Luna
Silvestre C	Luna
Erick Jon & Sharon G	Lundberg
Thoai Duc & An Thi	Ly
Manuel T Segura &	Ma G Molina Ramirez
Ruben C Perales &	Ma Guadalupe Martinez
Victor M & Brenda W	Macha
Theresa Ann	Machado
Gustavo Eric	Macharro
Richardo	Machorro
Miguel Eugenio	Macias
Jose A	Madrigal
Jose Alejandro	Madrigal
Miguel & Mary Ann H	Madrigal
D & Alicia O	Magallanez
Mabe Alejandra & Lezama	Magdaleno
Alfredo	
Suleman	Makani
Alejandra	Maldonado
Antonio & Virginia Felipe Barrera & Perla	Maldonado Maldonado
	Maldonado
Frank A Guillermo A & Esther I	
Jose Daniel	Maldonado Maldonado
Leticia	Maldonado
Mary E	Maldonado
Mary Teresa	Maldonado
Paula C	Maldonado
Sylvia	Mancha
Alfonso	Mandujano
James M & Crystal L	Mann
Maria J Morales &	Manuel Basio
Eduardo Rodriguez Olquin &	Maragarita Canchola Sanchez

Santiago H Arguelles &	Marcelina Ramirez
Rene S	Marco
Marco Antonio Lopez	Marco Antonio Higadera
Alvarez & Mary Ann	Lopez Jr Marek
Josefa E	Marez
Edgaras	Margevicius
Yadira Hernanez-Cortez &	Marggie Lizette Villarreal
Jose Eduardo Mani Villa &	Maria Abelina C Diaz
Octavio Dominguez Meza	Maria Antonio H Rodriguez
& _	
Arnoldo Betancourt &	Maria De Jesus
Jose Luis Aguilar &	Maria De Los A
Jose Luis Molina &	Maria Del Pilar
Fernando Valadez &	Maria Del Rosario
Pastor Chacha-Octavo &	Maria Dominguez
Marlon Rodriguez &	Maria Elena Casas
Eric Morin &	Maria I Martinez
Rsalinda G Lapp &	Maria I Martinez
Ricardo P Sottelo &	Maria Luisa Ramirez
Constaintino Quintanar &	Maria Martinez
Cristobal Cruz Echavarria	Maria Ortega
& Ricardo R Chacon &	Maria R Flores
Nohe Robledo &	Mariana Gaona
Jesus Tovar, Jr &	Maricela Cardenas
Alfonso & Alma R	Marin
Jose & Claudia	Marin
Jose Manuel	Marin
Maria Concepcion	Marin
Maria E & Jacqueline	Marin
Emmanuel & Yadira	Mariscal
Mario Francisco &	Mariscal
Kassandra Nikol	
Sergio Mario	Mariscal
Dora Ann & John W	Markle
Fernando Garza &	Marlene Villegas
Jose Juan Sanchez	Marmolejo
Carlos Ernesto	Marquez
Cindy	Marquez
Miguel & Anilu	Marquez
Ulises & Imelda	Marrufo
Armando Antonio	Martha E Maldonado
Vasquez, Jr & Eduardo G Vasquez &	Martha M Herrera
Rowland J	Martin, Jr
Amelia M	Martinez
Abel & Ernestina	Martinez
Alejandro M	Martinez
Amanda	Martinez
Andres & Martha	Martinez

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Andrew & Rosemary	Martinez
Andrew A	Martinez
Anita F	Martinez
Arturo	Martinez
Charles Anthony	Martinez
Charles B. & Rosa L.	Martinez
Charles L & Maria	Martinez
Concepcion C	Martinez
Delia Deleon	Martinez
Diana	Martinez
Diana L	Martinez
Dustin G	Martinez
Eliseo & Beatriz C	Martinez
Eloise R	Martinez
Estanislado	Martinez
Estela M & Isaura R	Martinez
Esther A	Martinez
Eva C	Martinez
Fernando G	Martinez
Francisco	Martinez
Gabino & Guadalupe	Martinez
Gil S	Martinez
Gilberto C & Gil S	Martinez
Gilberto C & Joshua	Martinez
Guadalupe L & Myong S	Martinez
Hector	Martinez
Hector L & Adela F	Martinez
Hector L & Adela F	Martinez
Hortencia	Martinez
Isabel T	Martinez
Jairo S	Martinez
Jesus	Martinez
Joel Jr	Martinez
John C	Martinez
John C. & Richard	Martinez
Jose A	
	Martinez
Josephine S	Martinez Martinez
Joshua	Martinez Martinez Martinez
Joshua Juan A	Martinez Martinez Martinez Martinez
Joshua Juan A Juan F	Martinez Martinez Martinez Martinez Martinez Martinez
Joshua Juan A Juan F Juan F.	Martinez Martinez Martinez Martinez Martinez Martinez Martinez Martinez
Joshua Juan A Juan F Juan F. Juan G & Estella	Martinez Martinez Martinez Martinez Martinez Martinez Martinez Martinez Martinez
Joshua Juan A Juan F Juan F. Juan G & Estella Juan Luis	Martinez
Joshua Juan A Juan F Juan G & Estella Juan Luis Krystle	Martinez
Joshua Juan A Juan F Juan F Juan G & Estella Juan Luis Krystle Lydia Garcia	Martinez
Joshua Juan A Juan F Juan F Juan G & Estella Juan Luis Krystle Lydia Garcia Marcelino & Maria	Martinez
Joshua Juan A Juan F Juan F Juan G & Estella Juan Luis Krystle Lydia Garcia	Martinez

Mary A	Martinez
Michael T	Martinez
Miguel Angel Torres	Martinez
Nicolas L	Martinez
Pablo F & Adriana	Martinez
Pamela	Martinez
Patricia B	Martinez
Patrick Lee	Martinez
Paul & Liza	Martinez
Regina	Martinez
Rene & Basti	Martinez
Richard	Martinez
Robert & Rachel P	Martinez
Robt & Guadalupe	Martinez
Salvador & Erlinda	Martinez
Salvador & Raquel	Martinez
Sonia Soto	Martinez
Stephanie C	Martinez
Steve E	Martinez
Steve E & Diana P	Martinez
Sylvia V	Martinez
Teresa	Martinez
Valerie Yvonne	Martinez
Victor	Martinez
Alfredo	Martinez, Jr
Julian R	Martinez, Jr
Manuel Dejesus	Martinez, Jr
Ramon	Martinez, Jr
Servando	Martinez, Jr
Hector	Martinez, Sr
Joshua Wesley	Mason
Jacquelyn Nicole	Masters
Angelita	Mata
Arturo Zavala	Mata
Jesus G	Mata
Linda R	Mata
Maria J	Mata
Marin	Mata
Valerie	Mata
Valerie Israel Olvera-Romero &	Matilde Salinas-Castanon
Theresa Blanca	Matide Salinas-Castanon Matito
Chloe Elizabeth Mullins &	Matthew De La Fuente
	Matthew James Sandoval
Hayde Guadalupe Garcia Mora & Andrew	Matthew James Sandoval Maurais
	Maurais Maxwell
Andrea Faye	
Thomas E & Jamie C	Mcbee
Beverly W	Mccall

Martin H & Linda

Kenneth Marlo	Manage
	Mccants
Malcolm M	Mcclendon
Evangeline	Mccolley
David A	Mcconkey
John J	Mccown
Russell L	Mccoy
Samuel & Viola L	Mcdaniel
Deborah	Mcgee
Emmer L	Mcintosh
Brian L & Patsy Jean	Mckibbin
Scott C Wm & Luz M	Mcmanus
	Mcmunn
William Lee	Mcmyne
Bradford	Mcvea
Rudy & Linda	Medelez
Blas V & Theresa F	Medellin
Idalia L	Medellin
Alicia	Medina
Andrew & Sandra A	Medina
Elvira L	Medina
Gloria	Medina
Jesus T	Medina
Jesus T & Nancy	Medina
Juana Leonor	Medina
Lionel J & Jo Ann	Medina
Maria L	Medina
Santa	Medina
Victor G.	Medina
Juan Jose	Medina, Jr
Lincon Maria	Medrano
Oscar J	Medrano, III
Efrain & Mejia Ivan	Mejia
Juan M	Mejia
Nancy M	Mejia
Rose Diane	Mejia
Fernando Conchas	Melendez
Oscar	Melendez
Alicia M	Melero
Alexander Christian	Melissa Leal
Schwartz, Sr. & James A	Melton
Iliana N	Menchaca
Richard G	Menchaca
Alicia & Pedro J	Mendez
Ernestina M.	Mendez
Hugo	Mendez
Jacquelyn R	Mendez
Norberto L & Carmen	Mendez
Nvia Loretta	Mendez

Rafael Lopez	Mendez
Raul H & Maria E	Mendez
Raul Matthew	Mendez
Daniel D	Mendez, Jr
Rosa M & Jacob	Mendiola
Elizabeth A	Mendoza
Guadalupe	Mendoza
Manuel H & Sylvia	Mendoza
Monica	Mendoza
Olga T	Mendoza
Antonio J & Maria	Menendez
Maria	Mercado
Ernesto Delcarmen	Mercedes Adriana
Galmiche & Paulo Cesar Yanez &	Castandea Mercedes Mireles
Beatrice	Merlos
John A	Merv
Veronica Esmerelda	Mesta
Joseph E & Peggy W	
	Meyer
Jamie Juan A	Meza Meza
	Meza
Mary Louise C	
Nicole	Meza
Melissa Torres &	Michael A Valdez
Sarah Beth Neira &	Michael Joseph Miller
Christopher L	Mick
Conrado Q Zurita, Maribel E Rosales, &	Miguel Angel Q Esquive
Teresa H Guerrero &	Miguela Hernandez
Casey L	Miller
Jeffrey C	Miller
Robert Wayne	Miller
Zoila O	Miller
Patrick	Milligan
Janet M	Mills
Fermin & Delia B	Miranda
Arthur	Mireles
Bianca	Mireles
Jesus I	Mireles
Juana	Mireles
Louis M & Guadalupe	Mireles
	Mireles
Magdalena	
Nadine	Mireles
Norma A	Mireles
Robert F	Mireles
Steban F	Mireles
Steve F	Mireles
Steve F	Mireles
Patrick	Mireur

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Bernadine	Mitchell
Frank	Mitchell
Frank	Mitchell
Frank & Zetta	Mitchell
David Cali	Mo
Viola	Moczygemba
Antonio & Norma	Mojica
Tina M	Molden
Alicia	Molina
David Danie	Molina
Hugo & Zandra	Molina
Illeana & Carlos	Molina
Manuel & Maria D	Molina
Norma Jean	Molina
Samantha J	Molina
Joseph	Molinaro
Jesse R & Esmeralda G	Molleda
Diamantina	Moncada
Fidencio & Jo Ann	Moncada
Ricardo	Moncada
Milon	Mondal
Gerardo Zuniga	Monreal
Agustin & Carmelina	Monsivas
Adiela Luz	Monssalve
Bertin H & Rosalina	Montalvo
Cynthia & Anna Lisa	Montalvo
Jose M.	Montalvo
Joseph & Brandy	Montalvo
Rebecca	Montalvo
Carlos Isidro & Suzanne H	Montanez
Rosaura I & Leon	Montealvo
Angelica B	Montejano
Juan E & Brenda	Montelongo
Jorge	Montes
Oscar	Montes
Oscar	Montoya
Russell	Montoya
Steve F	Montoya
Sylvia Ann	Montoya
Robert G	Mooneyham
Darwin L	Moore
Elaine E	Moore
Edwardo & Elia C	Mora
Guillermo	Mora
Fernando S	Mora, Jr
Antonio & Nancy R	Morado
Antonio G	Morado
Aileen	Morales

Chris Leann	Morales
Clementina Castaneda	Morales
Elena Reyes	Morales
Frank T & Dora V	Morales
Gabriel & Elvia L	Morales
Jesse & Angela	Morales
Joe G	Morales
Maria	Morales
Martin Reyes	Morales
Oziel S & Marisa	Morales
Rachel	Morales
Robert C	Morales
Sammy	Morales
Robbie M	Moralez
Richard O & Cecilia	Morante
Anna L	Moreno
Antonio & Juanita F	Moreno
Antonio Marco	Moreno
Christopher	Moreno
Crystal F	Moreno
David & Samanthia Y	Moreno
Eva A & Gilberto M	Moreno
Genesis	Moreno
Gregorio	Moreno
Guadalupe	Moreno
Guillermo F	Moreno
Jennifer Lopez	Moreno
Jesus G	Moreno
Jose & Marisela	Moreno
Juan J & Ofelia	Moreno
Lucia D	Moreno
Raul & Alma A	Moreno
Raymundo	Moreno
Roberto Daniel	Moreno
Rudolph	Moreno
Sofia	Moreno
Teodora	Moreno
Moses A	Moreno, III
Leslie	Morgan
Prince A	Morgan
Margarito & Ana L	Morones
Rosalie J	Morren
Michelle Gee	Morris
Lenny & Michelle E	Motley
Jimmy K & Janet	Moy
Ihsan	Mudassar
Charles E	Mueller, Jr
Juan	Mujica

APPENDIX

Agustin	Mungia
Cecilia O	Muniz
Daniel & Minerva	Muniz
Maria Elena Juarez	Muniz
Matt D	Muniz
Ramiro R	Muniz
Reynaldo R	Muniz
Alejandro C	Munoz
Bryan & Laurie	Munoz
Carlos & Gilda	Munoz
Cinthia Patricia	Munoz
Cynthia	Munoz
Juan Gabreil	Munoz
Salvador D & Martina	Munoz
Sylvia	Munoz
Treva N	Murphy
Minerva	Murrah
Samuel G & Norma K	Nami
Firoozeh	Namiranian
Jose R & Maria	Nandin
Joanne B	Nanez
Melissa	Narro
Fidel Salas, Jr. &	Nataly Duran Carreor
Diana & Stephan	Natividad
Leodegario M & Luz M	Nava
Francisco C	Navarro
Jesus R & Selene I	Navarro
Jose Carlos	Navarro
Olivia G & Martin	Navarro
Francisco C Navarro &	Nayeli C Rodriguez
Francisco C Navarro &	Nayeli C Rodriguez
Louis E & Klara S	Nelson
Hector M & Christine	Nerio
Samer M Zaqtan &	Nesreen Nefz Harb
Maria A	Netardus
Stepanie	Neumann
Lillian Inez	Newton
Adija & Solomon K	Nfor
An	Nguyen
Huan & Dao Le	Nguyen
Thuy Duong	Nguyen
Thuyet T & Vu Cuong	Nguyen
Destiny Marie Carrero-	Nicholas James
Scherrer &	
Scott & Sandra	Nichols
Francisco	Nieto
Marilina	Nieves
Barbara E	Nine
Patricia Colunga	Nino

Conrad K	Nishwitz
Amalia Moreno &	Noe Casas
Jose Luis	Nolasco
Joe & Frances	Nombrano
Antonio F	Noriega
John R	Noriega
John R & Linda R	Noriega
Manuel	Noriega
Digna M	Norris
Gregory	Nottingham
Terry	Noveroske
Gino	Nucio
Richard A & Christine R	Nugent
Christopher	Nunez
J Concepcion	Nunez
Ruben & Martha	Nunez
Edith G	Nunez-Morales
Garrett J R & Chelsea	Oats
Carol Amy	Obanon
David & Lydia	Obaya
Noe	Ochoa
Rene A	Ochoa, Jr
Joe C	Ocon
Luis A & Jessica	Ocura
Jesus Cristian Nunez Gaona &	Odalis H Hernandez
Joely	Odis
Pete G & Wife	Ojeda
Sergio & Debra	Ojeda
Kelly & Melissa	Okane
Adedolapo Enuma	Okolo
Benito S. Lucio &	Olga G. Aguilera
Francisco J & Maria A	Oliva
Filberto T	Olivarez
Ricardo	Olivarez, Jr
Maechal & Maria M	Olivarri
Belen R	Olivani
Germin E & Deborah	Olivo
Anita L	Olivo
Dolores Margarita	Olmos
German	Olvera
German & Monica	Olvera
Jonas & Sanjuana	Ontiveros
Edmundo	Oranday
Feliz M	Ordaz
San Juanita C	Ordaz
Manuel E & Iris G	Ornelas
Alfonso & Jennifer M	Orocio
Jesse	Orosco

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Jesse James	Orosco, III
Andrew V	Orozco
Aurora S	Orozco
Arnulfo B & Maria Refugio	Ortega
Aurelio	Ortega
Louis	Ortega
Monica G.	Ortega
Felix	Ortega, Jr.
Juan & Rosario	Ortegon
Tom & Susie E	Ortegon
Ulises	Ortegon
Ana Cristina Torres	Ortiz
Candido G	Ortiz
Candido G & Maria E	Ortiz
Candido G & Maria E	Ortiz
Edward S	Ortiz
Elizabeth M & Ana M	Ortiz
Frank O Brien & Viola	Ortiz
Irma	Ortiz
Javier C & Bertha Joe S & Erlinda	Ortiz Ortiz
	Ortiz
Juan U	
Maria & Mariano	Ortiz
Maria S	Ortiz
Robert	Ortiz
Rodolfo Lopez & Norma Eliza	Ortiz
Roland A	Ortiz
Gabriel E	Osoria
Sarah	Otey-Ciesielczyk
Michael J	Otoole
Pablo J & Lucia E	Oviedo
Carrie A	Owens
Quan T Nguy &	P Nguyen Bich
Robert Alexander	Pace, Jr
Dora C	Pacheco
Dorothy & Rupert	Pacheco
Frank G & Vicky G	Pacheco
Jose	Pacheco
Priscilla G	Pacheco
Augustina R	Padilla
David	Padilla
David & Grace	Padilla
David & Grace	Padilla
David & Maria G	Padilla
Elida S	Padilla
Gustavo & Maria G	Padilla
Tomas	Padilla

Cecilio & Hope	Paez
Alice	Pakravan
Antonio V	Palacios
Carmen	Palacios
Juan F & Jacoba H	Palacios
Juan F & Jacoba H	Palacios
Rafael A	Palacios
Jose De Jesus	Palacios, Jr
Yyolany E	Palma
Guadalupe B	Palmarez
Eva V	Palomarez
Anthony	Palomo
Joe F & Shannon	Palomo
Lorenzo S	Palomo
Robin	Pals
Ruben Lopez, Jr &	Pamela Victora Salazar
Thomas P	Pangilinan
George G	Paniagua
Samuel G	Paniagua
Catherine	Pantoja
Miguel A	Paredes
Herman Elanders	Parham
Jesus Espiririon	Parker
Edward	Parra
Justo C & Martha N	Parra
Mary Helen	Parra
Sharon D	Passmore
Leticia	Pastrano
Tejaskumar & Rital	Patel
Fernando	Patino
Filiberto & Quirina	Patlan
Joyce J & Wardell L	Paze
Curtis L	Pearson
Katherine Guerrero &	Pedro Hernandez
Alex E & Dolores	Pena
Carlos A & Sylvia	Pena
David Leo	Pena
Doris	Pena
Eristea	Pena
Ignacio R & Martha J	Pena
John P	Pena
Larry V	Pena
Lydia	Pena
Maria I	Pena
Maria Juanita	Pena
Melissa Yvonne	Pena
Michael R & Teresa G	Pena
Rudy L & Mary Lou	Pena
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Albert R	Pena, Jr
Jose H	Pena, Sr
Joshua Lee	Penrod
Alberto	Perales
Alberto & Rocio	Perales
Alma	Perales
Cruz	Perales
Angelica & Jason	Peralez
Aurelio Martin	Perea, Jr
Claudia	Pereida
Adela	Perez
Alicia	Perez
Benjamin D	Perez
Bonifacio Castro	Perez
Cristina Isabel	Perez
Daniel H & Rosemary V	Perez
Delia D	Perez
Francisco & Rosa Maria O	Perez
Gabino & Erika	Perez
German & Irman	Perez
Graciela Lina	Perez
Herminio M & Angelita	Perez
Ida Molina	Perez
Irma S	Perez
Janie V & Juan	Perez
Janie Vigil	Perez
Jesse A	Perez
Jesus E & Patricia H	Perez
Joe S & Lucy	Perez
Joel L	Perez
Jose	Perez
Juan G	Perez
Juan Gabriel	Perez
Juana L	Perez
Julio C & San Juanita	Perez
Lucy B	Perez
Luis R & Juliana A	Perez
Marcos	Perez
Maria Dejesus	Perez
Maria Del Socorro	Perez
Melody Gonzalez	Perez
Rafael	Perez
Raul & Alejandra	Perez
Reynaldo & Carmen	Perez
Reynaldo Jesus	Perez
Robert L	Perez
Rogelio & Sandra N	Perez
Rose Mary	Perez

Steven A & Julie L	Perez
Victor S & Maria T	Perez
Yvonne	Perez
Yvonne	Perez
Jose Luna	Perez, Jr
Marlenin	Periana-Lemas
DS	Perkins
Paul & Jemima	Perryman
Gloria Contreras	Pesina
Thanh	Pham
Darren T	Phelps
Winslow David & Lisa	Phillips
Cindy M & Joshua	Piccirillo
Ludy	Pimentel
Maria	Pina
Mark	Pina
Michael J	Pina
Ramon &	Pina
Raul	Pina, III
Juan G.	Pina, Jr.
Ernesto	Pinero
Darell M	Pittman
Victor F & Rachel J	Pitts
David Antonio	Pivaral
Elmer & Jennifer	Pivaral
Jose David	Pivarral
Leopoldo G & Nidia	Placeres
Cecilia	Plascencia
Norma I	Plata
Andres M & Carolina F	Plaza
Jose J	Poaree, Jr
Amparo R	Polendo
Pedro & Victoria Y	Pompa
Gonzalo & Pauline R	Ponce
Jamie T & Roselinda	Ponce
Jose Luis Fraire	Ponce
Antonio	Ponce, III
Antonio	Ponce, Jr
James E & Nancy H	Poole
Ronald & Christy	Poole
Maria Victoria	Portales
Miguel	Portales
Miguel Angel & Lizeth M	Portales
Gabriela	Portalez
Clifford & Suzette R	Porter
Antonio P & Betty	Portillo
Jorge S & Blanca G	Posada
Jose A & Maria D	Posada

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Alfredo	Posada-Cano, Jr
Karen M	Powlas
Maria Elisa	Powlas
Alfonso	Prado
Pete	Prado
Carmelo & Alice R	Prieto
Joel Alonso &	Priscilla Marie Gonzales
Carolyn & Martin L	Prosper
Arnulfo C	Pruneda
Rosemary	Puentes
Baldemar	Puga
Elsie J	Puryear
Patricia D	Quijada
Efrain & Esmeralda	Quinones
Juan & Rosalinda	Quinones
Oscar	Quintanar
Heliodoro & Bertha	Quintanilla
Lisa	Quintanilla-Seward
Marcella R	Quintero
Vanessa	Quintero
Emilio & Andrea	Quiroga
Vicenta V	Quiroga
Angelita	Rabago
Richard A	Rabago
Khalid A & Eynass	Rafati
Mahmoud D	Rafati
Nizar M	Rafati
Vincent & Mary Cruz	Raigoza
Albert & Linda	Ramirez
Alex B	Ramirez
Christopher & Leticia	Ramirez
Daniel A & Rachel P	Ramirez
Domingo & Florence	Ramirez
Ernestine R	Ramirez
Felipe	Ramirez
Francisco	Ramirez
Gisela	Ramirez
Javier	Ramirez
Jesse & Yolanda	Ramirez
Jesus T & Marianita	Ramirez
Jose Cruz	Ramirez
Jose E	Ramirez
Juan & Marta	Ramirez
Juan R & Nicolasa	Ramirez
Lara	Ramirez
Larry	Ramirez
Luis J & Emestina	Ramirez
Manuel R	Ramirez

Margaret & Louis	Ramirez
Mary Helen	Ramirez
Raymond S	Ramirez
Raymundo R	Ramirez
Robert & Paula	Ramirez
Rodolfo & Yassmin	Ramirez
Rosanna Michelle	Ramirez
Valerie V	Ramirez
Victor	Ramirez
Yvette	Ramirez
Francisco J	Ramirez, Jr
Isidro	Ramirez, Jr
Manuel V	Ramirez, Jr
Santiago	Ramirez, Jr
Salvador	Ramirez, Sr
Gerardo	Ramon
Joe R	Ramon
Luis	Ramon, Jr
Daniel	Ramos
David	Ramos
Elida	Ramos
Escolatica	Ramos
Irma Rosa	Ramos
Joe S & Sandra	Ramos
Maria Elena	Ramos
Raymundo & Laura E	Ramos
Robert & Maria F	Ramos
Roman B & Laura Ann	Ramos
Roman F & Linda	Ramos
Rose Mary	Ramos
Theodore & Veronica	Ramos
Willie & Maria Luisa	Ramos
Dennis R	Ramsey
Willie	Randle
Briana	Rangel
Florence C	Rangel
Henry	Rangel
Paul	Rangel
Raymond R	Rangel
Roman & Ramona	Rangel
Tommie O	Rangel
Sylvia R	Rangle
Natividad Villegas, Ariane I Molina, &	Raudel Villegas
Lucy V Rivera &	Raul L Perez
Curtis	Ray
Manuel	Razuri
Juan Miguel & Letticia	Rea
Ruben Al Robinson, Jr &	Rebecca Perez

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Eleazar A Ravinder & Diana L Roger & Marilyn	Recio Reddi
	Reeck
Jose G & Jezabel Y	Regino
Matilda L	Reid
Nery J Cortez-Mendez &	Reina M Alvarenga-Mejia
Leandro	Renaud
Antonio C	Rendon
David R	Rendon
Gabriel H & Juanita A	Rendon
Rebecka & Anthony	Rendon
Teresa	Rendon
Jesus A	Rendon-Limon
Jesus A	Rendon-Limon
Macrina	Renk
Heidi	Renteria
Yvonne	Renteria
Jose Antonio	Renteria, II
Jose Antonio Heriberto T & Matilde	Renteria, II Resendiz
Miguel Angel Arriaga	Resendiz
Adam	Reyes
Alma R Martiarena	Reyes
Celina Victoria	Reyes
Claudia	Reyes
Gilberto Salazar & Patricia T	Reyes
Irma F	Reyes
Javier	Reyes
Jose A & Irene	Reyes
Jose A & Irene	Reyes
Maria G	Reyes
Mary Lou M	Reyes
Oscar	Reyes
Ramiro	Reyes
Raymond & Angle	Reyes
Roberto C	Reyes
Roberto Lopez & Darling	Reyes
Ruben	Reyes
Ruben & Maria Elena	Reyes
Serena Andra & Michelle	Reyes
Thomas A & Rebecca L	Reyes
Tom C & Irene B	Reyes
Martin V	Reyes, Jr
Ruben	Reyes, Sr
Ernest V	Reygadas, Jr
Edward Q & Ophelia	Reyna
Louis & Maria P	Reyna
	Reyna
Thomas R	

Mildred J	Rhambo
Maria M Pachuca &	Ricardo Morales Avila
Regina Lydia Roseman &	Richard Louis Vargas
Paul Arthur	Richardson, Jr
Elsa C	Rico
Neal & Deborah	Riebe
Antonio R	Rigonan
Jorge G & Isabel N	Rincon
Rafael & Lucia G	Rincon
Antonio R	Riojas
Dolores	Riojas
Rosa Elia	Riojas
Edgar A	Rios
Guadalupe T	Rios
Jesse & Mary	Rios
John R & Emerald M	Rios
Jose Luis	Rios
Julio & Maria	Rios
Julio & Maria	Rios
Magalena R	Rios
Magdalena Mendez	Rios
Ricardo	Rios
Roy L & Esther M	Rios
Samantha & Anthony	Rios
Siomara & Edwardo Rios	Rios
Belisario P	Rivas
Drake & Caitlin J	Rivas
Judith G Baca	Rivas
Cesar & Anna	Rivas-Tellez
Alfredo Silva	Rivera
Antonio L & Luz A	Rivera
Edward & Brenda	Rivera
Jose	Rivera
Jose Manuel S	Rivera
	Rivera
Mary Lou	
Olga Cecilia	Rivera
Ruben B & Yolanda	Rivera
Susana Garcia	Rivera
Eva M Garcia &	Robert A Martinez
Rosalva Vela Guzman &	Robert Vela Guzman
Ethel	Robinson
Leyland G	Robinson
Robert Allan	Robinson
Herman G	Robledo
Juan & Lo Yun-Ju	Robledo
Leticia	Robledo
Abelino	Robles
rancisco Javier & Jacinto Carrera	Robles

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Lee & L S	Robles
Rigoberto & Maria	Robles
Rigoberto Gonzalez & Maria D	Robles
Virginia C	Robles
Virginia Carreon	Robles
Olga Leticia	Robleto
Thomas J	Rocco
Diana	Rocha
Robert	Rocha
San Juanita	Rocha
Alfredo & Guadalupe	Rodarte
Mary Beth	Rodarte
Brenda R Lara &	Rodrigo Dalva Hinojosa
Alma	Rodriguez
Adan Martinez	Rodriguez
Adrian	Rodriguez
Aladdin	Rodriguez
Alberto	Rodriguez
Alejandra R & Faustino A	Rodriguez
Alejandro H	Rodriguez
Alma J & Tomas	Rodriguez
Antonio D	Rodriguez
Antonio G	Rodriguez
Bernardino S	Rodriguez
Brisa D	Rodriguez
Christopher & Jacquelyn	Rodriguez
Claudia	Rodriguez
Dale	Rodriguez
E G & Minerva G	Rodriguez
Eduardo M & Leticia L	Rodriguez
Fleazar S	Rodriguez
Ernesto & Wf	Rodriguez
Esmeralda	Rodriguez
Estella	Rodriguez
Estella Sylvia	-
Francisco C & Laura P	Rodriguez
	Rodriguez
Frank R & Erminia Gabriel & Clarisa A	Rodriguez
	Rodriguez
Gary	Rodriguez
Gilbert & Maria C	Rodriguez
Guillermo A	Rodriguez
Hector V	Rodriguez
Henry D & Rosita L	Rodriguez
Hugo G & San J	Rodriguez
Idelfonso	Rodriguez
Jesse G. & Lupe R.	Rodriguez
Jesse M & Mary H	Rodriguez
Jesus & Rosa R	Rodriguez

Jim & Nancy Jimmy & Norma Joe A Jose F & Maria A	Rodriguez Rodriguez
Joe A	Rodriguez
Iono E & Maria A	Rodriguez
Juse i oxividita A	Rodriguez
Jose G	Rodriguez
Jose L & Aurora	Rodriguez
Jose P & Yolanda	Rodriguez
Jose T & Marisela S	Rodriguez
Juan C & Ana Maria	Rodriguez
Juan F	Rodriguez
Juan V	Rodriguez
Juanita Cruz	Rodriguez
Julio G	Rodriguez
Juventino	Rodriguez
Leticia	Rodriguez
Lillian	Rodriguez
Lori D	Rodriguez
Luis & Lidia	Rodriguez
Luis Alberto & Rosa Maria M	Rodriguez
Margaret J	Rodriguez
Maria Eugenia Soto	Rodriguez
Nilda H	Rodriguez
Noe S & Patricia P	Rodriguez
Oscar & Modesta S	Rodriguez
Raquel G	Rodriguez
Rene S	Rodriguez
Richard V & Aida S	Rodriguez
Robert & Mary Ann	Rodriguez
Robert L	Rodriguez
Roberta J	Rodriguez
Rodolfo A	Rodriguez
Rogelio	Rodriguez
Rogello Rosa Elia	Rodriguez
Rosa Ella Rosario S	Rodriguez
Rosano S Ruben	_
	Rodriguez
Sandra Veronica	Rodriguez Rodriguez
Veronica Vicenta P	-
	Rodriguez
Yvonne P	Rodriguez
Jesus Manuel	Rodriguez, Jr
Joe	Rodriguez, Jr
Jose	Rodriguez, Jr
Jose Luis	Rodriguez, Jr
Robert	Rodriguez, Jr
Catalina	Roel
Isaias T & Claudia R	Rojas
Jesus & Julia Ann	Rojas

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Raul Reyes	Rojas
Raymond & Maria Elena	Rojas
Enrique Edlin L	Rojas, Jr
Carlos A	Roldan Roman
Ernesto & Minerva	Roman
Olga	Roman
Raymundo	Roman
Abelardo T & Alma E	Romero
Adan	Romero
Alfredo & Sylvia	Romero
Libino	Romero
Raul H & Sylvia B	Romero
Sylvia	Romero
Viola B	Romero
Felix J & Diana R	Romo
Felix Joseph	Romo
Joaquin	Romo
Joaquin	Romo
Joaquin & Rosa	Romo
Priscilla	Romo
Sandra Renae	Romo
Eduardo	Romo, Jr
Jorge Adalberto G	Rosa O Huitron
Gonzalez & Esau Villegas &	Rosalba Velazquez
Francisca	Rosales
Victor Manuel	Rosales
Joseph Carreon &	Rosalinda Rodriguez
Maria	Rosas
Morayma	Rosas
Octobiano G	Rosas
Kyle G & Precious	Ross
Paulet	Rouse
Eleanor	Rowand
Neil Calfas &	Roy Fred Gonzalez
Mario Alberto Serrato	Rubio
Guy Gerald & Katrina	Rudes
Malburn J & Debbie	Rudesill
Nichelle N	Rudnicky
Ronald	Ruggiero
Andrew G	Ruiz
Isaac & Judith P	Ruiz
Jesse A & Susana	Ruiz
	Ruiz
Jose E & Blanca Leticia	
Jose L	Ruiz
Luis Garcia	Ruiz
Maria G	Ruiz
Raquel R	Ruiz

Soledad	Ruiz
Teresa M	Ruiz
Kevin Bradford	Russell. Jr
Jorge Saucedo Sifuentes	Ruth Garcia-Hernandez
Israel C Carias &	Ruth J Solis
Evelyn Villaflor	Sadsad
Barbara	Saenz
Efrain & Maria Dora	Saenz
Enrique R	Saenz
Maria	Saenz
Santiago Emmanuel & Mayda G	Sagastume
Ihsan A Al	Saidi
John Scott	Salamon
Christopher Thomas	Salas
Rosalinda	Salas
Jesse	Salas, Jr
Andrea	Salazar
Candelaria A	Salazar
Idalia T	Salazar
Joe L & Lou Ann Q	Salazar
Jorge A	Salazar
Jorge A & Julia	Salazar
Joy	Salazar
Manuel G	Salazar
Manuel G & Theresa	Salazar
Maria D	Salazar
Michelle Ann	Salazar
Pioquinto & Juana	Salazar
Raymundo C	Salazar
Salvador	Salazar
Silvia	Salazar
Antonia	Saldana
Carlos Contreras & Sabina	Saldana
Cristian Francisco	Saldana
Roxanna Rae	Saldana
Florestela	Saldivar
Hortencia Salas	Saldivar
Joe L & Margie O	Saldivar
Ricardo S	Saldivar
Frank	Salgado
Marjie	Salgado
Alfonso	Salgado
Ariel	Salinas
Aurora	Salinas
Benjamin A Eduardo	Salinas Salinas
Edward D & Irene D	Salinas

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Elda G	Salinas
Juanita L	Salinas
Michelle	Salinas
Natividad & Alicia	Salinas
Pablo & Eloina	Salinas
Paul D	Salinas
Rafael & Maria T	Salinas
Sergio Manzano &	Samantha Ann Torres
Mahamoudou & Miriam A	Samassekou
Norberto	Sanabria
Abel G	Sanchez
Amelia L	Sanchez
Ana Maria	Sanchez
Annette Lynn	Sanchez
Antonia G	Sanchez
Blanca E & Victor Manuel	Sanchez
Cruz R	Sanchez
Debbie Ann	Sanchez
Dickey E & Gloria B	Sanchez
Domingo G	Sanchez
Eduardo & Maria	Sanchez
Edward	Sanchez
Elizabeth Reyes	Sanchez
Enrique	Sanchez
Enrique & Maria	Sanchez
Estella P	Sanchez
Felipe Jr & Sandra	Sanchez
Frank N	Sanchez
Gerald & Beatrice	Sanchez
Hortencia	Sanchez
Jaime Rolando	Sanchez
Jesus & Lillian	Sanchez
Jesus Constante	Sanchez
Jose Luis	Sanchez
Josue & Esperanza	Sanchez
Juan M & Yolanda J	Sanchez
Leopoldo R	Sanchez
Luis & Josefa L	Sanchez
Lydia Esther	Sanchez
Maria E	Sanchez
Matthew Ray	Sanchez
Miriam Mar	Sanchez
Nicanor & Ana M	Sanchez
Ofelia	Sanchez
Paula	Sanchez
Ramon M & Romana J	Sanchez
Richard & Delma	Sanchez
Rodolfo	Sanchez

Rosalio & Raquel	Sanchez
Rosario Edieleyne	Sanchez
Vazquez Salvador	Sanchez
Salvador & Julia V.	Sanchez
Candelario	Sanchez, Jr
Esteban	Sanchez, Jr
Federico	Sanchez, Jr
Guadalupe	Sanchez, Jr
Nick	Sanchez, Jr
Nick & Ana	Sanchez, Jr
Angelina & Dennis A	Sanders
Eliseo	Sandoval
Hilaria	Sandoval
Joaquin & Cynthia D	Sandoval
Juan Carlos & Teresa R	Sandoval
Juan M & Christian	Sandoval
Manuel & Eloisa	Sandoval
Roy Vasquez	Sandoval
Roy Vasquez Rudy	Sandoval
	Sandoval
Steve	
Soledad M Martinez &	Sandy B Lopez
Luis Enrique	Santarelly
Juan C	Santellan
Aylin	Santos
Cecilia A	Santos
Gregorio	Santos
Javier J & Yvette O	Santos
Jesse	Santos
Juan A	Santos
Maria G	Santos
Vicente D	Santos
Yolanda Marie	Santos
Rudy	Santoscoy
Denis	Santoy
Jose Luis & Oralia	Sarabia
Petra T	Sarabia
Luis Daniel Baerga &	Sarah Baron-Baerga
Lauren Adriana Lundy &	Sarah Lee Lumberas
Adam & Lara Alison Nicole	Sauceda
Ascencion B & Maria	Sauceda
Elvia M	Sauceda
Bertha	Saucedo
Ruben	Saucedo
Nes	Sayag
Robert & R	Schilling
Roger & Libier Alcala	Schilling
Jason	Schmitt
Brian	Schoeman

APPENDIX

APPENDIX A-3

Therese Ann	Schroeder
Richard R & Sherre	Schuetze
Sean & Amber	Schupbach
Mary Jane	Sciaraffa
Roger W & Claudia	Scott
Blanca	Searuggs
Martin A & Kimberly Ann	Seawell
Tyler J & Sara K	Sebastian
Linda Carolyn Cisneros &	Sebastian Perales
Roger O	Segovia
Florinda	Segura
Aaron Esquivel &	Selina Jacqueline Flores
Sandra Davis	Sena
Maria	Serafin
Antoine A	Serkis
Robert G	Serna
Guadalupe	Serrato
Luis A	Serrato
Albert P	Serratos
Milaris Colon &	Seth Anthony Stevenson
Mohammas Saeed Ebad	Shahla Barekat
Fardzadeh &	0
Mrad A Rebeccahlynn Stern &	Shaker Shane Johnson
David B	Shea
Joel	Shell
Eldon & Gloria	Shewmaker
Jason & Short Tiffany	Short
Amanda L	Shotkoski
Marek	Sieczynski
Jose & Hortencia	Sifuentes
Maryann & Jerry	Sifuentes
Aureliano	Silva
Eduardo	Silva
Gustavo	Silva
Idalia A & Sidney	Silva
Luis A & Paula	Silva
Manuel S & Evangelina	Silva
Maria D	Silva
Zoe Alexandra	Silva
Edgar G Mercado &	Silvia Flores Mena
Patrick Scott	Sisente
Marisol	Sixtos
Johnny & Glenda	Slack
Jeffrey W & Leann	Smallen
Barbara D & Brian S	Smith
Elizabeth Carolina	Smith
Glenn Patrick	Smith
Johnnie D & Sonya R	Smith

Kaleena Felipe & Duncan	Smith
Vincent Peggy Laurene	Smythe
Perry E & Mary L	Snow
Morgan	Soe
Roberto	Sola Sola
Jesus & Maria	Solano
Jesus & Maria Eduardo H	
	Solis
Jorge Ernesto	Solis
Julia I	Solis
Mary L	Solis
Michael J	Solis
Rocio & Javier	Solis
Roland U & Natalie L	Solis
Gary A Tovar &	Sondra Crosthwait
Manuel Vallejo &	Sonia Garcia Filoteo
Dahlia Rodriguez &	Sophia Juarez
Armandina	Soria
Christina F	Sosa
Juan M	Soto
Maria C	Soto
Mary Jane	Soto
Robert	Soto
Guillermo P	Soto, Jr
William C	Sparks
Mark Steven	Spaw
Boyd	Spears
Nousha Parkhill Davison &	Spencer G Davison
Keith L	Springer
Jason W	Stacy
Charles	Stanford
Elvira	Starin
James M & Ruby C	Stein
Alan J & Carolyn B	Stephan
Jesse A Garcia &	Stephanie Bernique-Garcia
Catherine Elizabeth	Sterling
Roderick D	Steward
Robert	Stewart
Oscar L	Stillman
Donnie L & Barbara A	Stillwell
John & Geraldine	Stotts
Olivia L	Strange
Victor D	Stremciuc
Cynthia C	Stroman
Martin James & Minako	Stuhler
Faye	Suarez
Guillermo	Suarez
	Suarez
Jose Y & Romana	

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Thomas B Murray, III &	Suk Kyoung
Joseph Elliot	Swenning
Madison Arden & Kristi Kay-Lynn	Taber Smith
Santiago & Gloria M	Tailes
Victor E Colon &	Taisha Castro-Atiles
Marie	Takeshita
Elaine G	Talamantez
Jason M & Chalis F	Tam
Gregory	Tamez
Raymond & Estefana	Tamez
Anthony A Aguilar &	Tammie Moreno
Zhu & Guo	Tan
Benny & Elmo Quan	Tang
Nathan	Tanner
Ignacio	Tapia
Charles J	Tarver
Charles J & Kristi A	Tarver
Stefano Pietri &	Tatiana M Short
Lora & Loren	Tatum
Felix & Maria S	Tavarez
Lytle L	Taylor
Ng	Taylor
Sharon & Frederick D	Teal
Jeffrey D	Teart
Tadesse Gashaw & Aklilu Fre	Tegene
Inez R	Tellez
Nicholas	Tello
Albert	Teniente
Carlos	Tenorio
Juan Carlos	Tenorio
Robert	Tenorio
Alex A	Tenorio, Jr
Ricardo De Leon-Garcia &	Teresa De Jesus Armendariz-Ramirez
Fernando & Diane	Armendariz-Kamirez Terrazas
Gabriel M	Terrazas
Hector	Terrazas
Luis A & Maria D	Terrazas
Yolanda H	Terrazas
Iran F Ricardo &	Thacarli Bohorquez
Luis Garcia Ruiz &	Thalia Samantha Gutierrez
Barbara	Morales Thapa
Allen & Elena	Tharp
Kristie Chervl	Theis
Rene Guerrero &	Thelma Perez
Anthony T & Joni C	Thomas
Charles A & Teresa J	Thomas
Dottie Carol Jones	Thomas

Mychal	Thompson
Eric & Rosie	Thorne
Edgar O Cavazos &	Tiffany M Lopez
Clowean S	Tijerina
Eugene H & Lydia B	Tijerina
Mary Louise	Tijerina
Raul Puente & Lorene	Tijerina
Robert G	Tijerina
Robert G & Maria G	Tijerina
Roger	Tijerina
Juan	Tirado
David & Andrea	Tokar
Leonides	Tolentino
James W	Tolleson
Keanna Paige	Tollett
Amir	Tondivar
Melissa	Torralva
Celso M	Torres
David	Torres
Elizabeth	Torres
Ernestine Mosqueda	Torres
Esequiel	Torres
Esquiel	Torres
Eulalio A & Cruz F	Torres
Graciela	Torres
Gregory Jr & Martha C	Torres
Javier G	Torres
Josephine	Torres
Leticia	Torres
Louis R	Torres
Lucia	Torres
Maria G & Jesus R	Torres
Ralph G & Oralia V	Torres
Rosalva	Torres
Ruben G & Maria	Torres
Saul	Torres
Vidal	Torres
Jerry	Torres, Jr
Leondro	Torres, Jr
Juan	Torrijos
Gene	Toscano
Cecilia	Tovar
Celia	Tovar
Gilbert	Tovar
Joe A & Melissa A	Tovar
John & Herlinda L	Tovar
Jose Andres	Tovar
Vilma	Tovar

Dottle Carol Jones Thomas Valleta Road Transportation Study | Community Open Houses Documentation

APPENDIX

APPENDIX A-3

Dung & Hang Phung	Tran
Trung Binh	Tran
Roberto Rosas, Jr &	Travis Aaron Ethington
George & Amanda	Trejo
Irma	Trejo
Jesse & Joanna	Trejo
Mary Lou A	Trejo
Albino R & Linda A	Trevino
Ana Maria	Trevino
Berta	Trevino
Criselda A	Trevino
David & Elvira	Trevino
Guadalupe	Trevino
Gustavo & Carlos	Trevino
Juan A & M Leticia	Trevino
Juan A & Maria L	Trevino
Mary V	Trevino
Melanie C	Trevino
Meianie C Michael	Trevino
Oscar G	Trevino
Raymond D & Martha	Trevino
Rudy A & Amber F	Trevino
Jonathan Palacios &	Trinidad Sauceda-Vasquez
Santos & Claribel	Troche
Jaime Angel Ebergenyi	Trueba
Javier & Roxanna	Trujillo
Virginia	Trujillo
Awng Seng & Mu Pli &	Tsai Hkawng
Peggy Sue	Turner
Kristopher R & Rebecca G	Tyler
Evangelos S	Tzivopoulos
Cecilia	Ugwu
Oghenesume D & Blessing	Umugbe
Caleb & Castelleja Marisol	Urdiales
Pedro & Juanita	Urdiales
Virginia	Urdialez
Kathy A	Uribe
Maria & Benjamin	Uribe
Hector D	Uribe, Jr
Jesus Alejandro	Urrea
Derrick K & Davida M	Uzzle
	Uzzle Vache Chakmakian
Ani Baltayan &	
Debra & Kenneth	Vail
Amy	Valadez
Edward & Stephanie	Valadez
Irene G	Valadez
Jesus Castaneda	Valadez
Raul V, Maria Q, & Kenia Y	Valadez

Robert A & Leslie Jo	Valadez
Elvira	Valdez
Francisco	Valdez
Francisco G & Bertha	Valdez
Frank & Oralia	Valdez
Gerardo R	Valdez
Henry F & Gloria	Valdez
Herminia M	Valdez
Leslie D	Valdez
Mario A	Valdez
Norma R	Valdez
Veronica	Valdez
Enriqueta O & Henry	Valdillez
Carolina L	Valencia
Miguel Angel & San Juana	Valenciana
Youless Jimmy	Valentine, Jr
Juan & Pilar	Valenzuela
Gregorio	Valenzuela-Quezada
Christopher John Valdez &	Valerie V Rivera
Valentin A & Maria	Valero
Frailan O	Vallejo
Jose & Olivia	Vallejo
Jose M	Vallejo
Rita Eglantina	Vallejo
Sylvia E	Vallejo
Richard A & Rita Anne	Van Dyke
Emerson	Vance
William Ronald	Vanderford
Guadalupe	Vara. Jr
Elvin	Varela
Elvin & Claudia	Varela
Elvin A & Claudia	Varela
Pablo	Varela
Javier	Varela-Gaytan
Adam R	Vargas
Evodio T & Adriana C	Vargas
Guillermo M	Vargas
Maria	Vargas
Ruth	Vargas-Martinez
Anna	Vargas-Martinez
Anna	Vasquez
Gilbert	Vasquez
Gregorio V & Beatriz H	Vasquez
Hector M & Mary T	
Hector M & Mary I	Vasquez
	Vasquez
Jorge & Liliana M	Vasquez
Jorge P & Esperanza	Vasquez
Luis Ignacio & Melissa	Vasquez

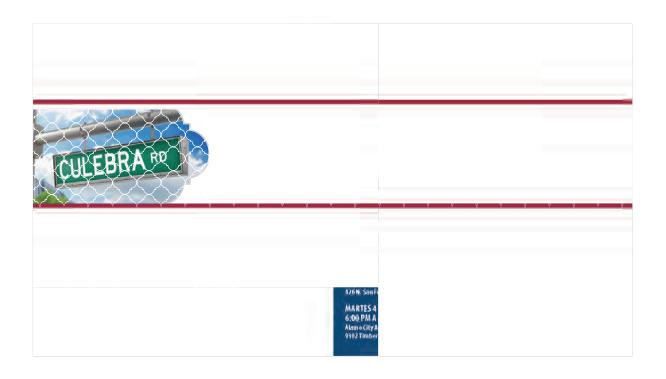
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Marco A	Vasquez
Maria Elena	Vasquez
Michelle	Vasquez
Reyna A	Vasquez
Ruben J	Vasquez
William G & Eloisa	Vasquez
Adan	Vasquez, Jr
Cruz	Vasquez, Jr
Oscar R	Vasquez, Jr
Masoud & Andrea J	Vaziri
Eliseo	Vazquez
Eliseo	Vazquez
Eliseo C	Vazquez
Jose J & Maria De Lourdes	Vazquez
Lorenzo A & Juanita	Vazquez
Valerie Siller	Vazquez
Ivan	Veda
Manuel & Erika	Vega
Margaruiete	Vega
Maria Del Consuelo	Vega
Reynaldo M	Vega
Adriana	Vela
Juan L Vela	Vela
Raul	Vela
Soledad	Vela
Esperanza Barron	Velasco
Angelica	Velasquez
Felicita	Velasquez
Guillermo	Velasquez
Jenny Lynn	Velasquez
Martin & Josephine	Velasquez
Robert M	Velasquez
Yolanda	Velasquez
Manuel & Simona A	Velazquez
Orfa Yoselin Trevino	Velazquez
Cesar R & Maria C	Velez
Evangeline	Velez
Pablo	Veliz
Baltazar	Vences
Luis Eduardo	Ventura-Diaz
Daniel U & Cinthia L	Vera
Jose	Verastique
Shailendra	Verma
Antonio Martinez &	Veronica Valenzuela
Arthur G	Viera, Jr
Heriberto C	Villa
Leticia	Villa
Tomas & Silvia	Villa

Joanne	Villalobos
Maria O	Villalobos
San Juana	Villalobos
Yolanda A & Victor M	Villalobos
Rosa Elia	Villalpando
Enrique	Villanueva
German & Cynthia	Villanueva
Juan Francisco	Villanueva
Pedro & Teresa	Villanueva
Roberto P	Villanueva
Teresa P	Villanueva
Encarnacion C	Villanueva, Jr
Arthur	Villarreal
Consuelo T	Villarreal
Diana G	Villarreal
Grace	Villarreal
Henry	Villarreal
Jaime Arnoldo & Diana	Villarreal
Juan C	Villarreal
Librado S & Maria D	Villarreal
Lorenzo R	Villarreal
Maria Luisa	Villarreal
Mary P	Villarreal
Melissa	Villarreal
Nancy Ann	Villarreal
Patricia M	Villarreal
Pedro	Villarreal
Raul & Carmen	Villarreal
Raymond & Sylvia	Villarreal
Andrea L	Villasenor
Giovani Daniel	Villasenor
Earl S & Amelia C	Villegas
Irene S	Villegas
Marcus A	Villegas
Moises	Villegas
Moises & Cynthia	Villegas
Moises B	Villegas
Rodrigo L & Sonia	Villegas
Maria C	Vinton
Alberto	Virgen
Alberto & Adriana	Virgen
Lena & Johannes	Visser
Yolanda	Viveiros
Lelia	Vosburgh
Thomas & Carmen	Vrzal
Henry	Vu
Khanh & Linh Ta	Vu
Michael & Mindy	Waala

Gloria F & Alva Norma F	Walker
Michael Todd & Robert	Walker
Tina M	Walker
John Warren	Walls
Steven Rene	Walsh
Gerald	Walter
Grace	Wanjie
Devin & Courtney	Warner
Jennifer S	Warren
Jennifer Nicoles	Waters
Charles M & Trina	Watson
Antonio J	Webb
Robert E & Maria H	Wechsler
John J & Gracie S	Wedell
Earl Benjamin & Betty Ann	Weimer
Samuel Edward	Welch
Jason E & Tina L	Wheat
Matthew & Alison	Whigham
Jeffrey Alan	White
Franklin T	Whitecotton
David	Whitson
Hector Capistran &	Wife
Emily & Kameron	Wilding
Lillian	Wiley
Robert	Willars
Melvin K	Williams
Risha A	Williams

Wendell	Williams, II
Brett D	Wilson
Jessie Mae	Woodard
Yuhunter	Woodard
Jeremy G	Wyles
Ashlie	Ximenez
Carlos Antelmo Figueroa Cardenas &	Yahaira Alejandra Hernandez
Cynthia	Yancy
Mary Jane	Yanez
Paulo Cesar	Yanez
Paulo Cesar & Mireles Mercedes	Yanez
Dong D	Yang
Li	Yang
Guadalupe G	Ybarra
Victoria M	Ybarra
Valerie A	Yeager
Emilio & Gladys A	Yebra
Maria G	Yebra
Kathy A	Yehl
Rebecca R	Yenne
David	York
Stella L	Young
Luis & Margarita A	Yruegas
Michael A	Ytuarte
Mary M	Yzaguirre
Armida & Jose	Zamarripa
Vanessa & Jose M	Zamarripa

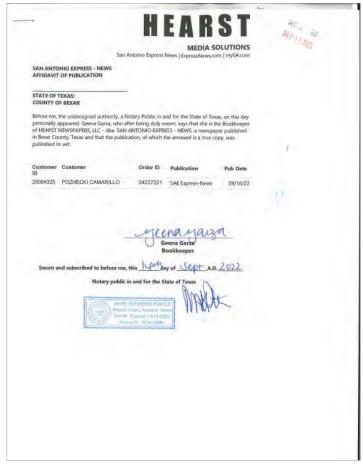


Culebra Road Transportation Study CULEBRA RD **COMMUNITY OPEN HOUSES** We want to share our proposed concepts with you, based on what we have learned! **IN PERSON** ONLINE SATURDAY, OCTOBER 1, 2022 Visit culebraroadstudy.org 9:00 AM TO 11:00 AM Available from October 1, 2022, through Holy Cross High School - Convocation Center October 19, 2022 426 N. San Felipe Street • San Antonio Identical information will be available at both TUESDAY, OCTOBER 4, 2022 in person options and online option 6:00 PM TO 8:00 PM **Alamo City Apostolic Church** 9302 Timber Path • San Antonio 1604 410 10 **Culebra Road** 151 1604



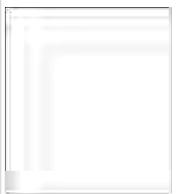
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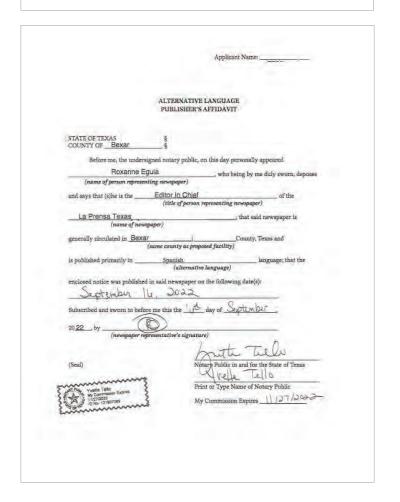


4th Annual abilitySTRONG Parade Kicking off October 1

By Laura H. Apin
The fourth annual ability-5
TRONG Parade, San Antonio's
annual Disability Price Gelebration, will take place on October
of more in a more in the street of the street





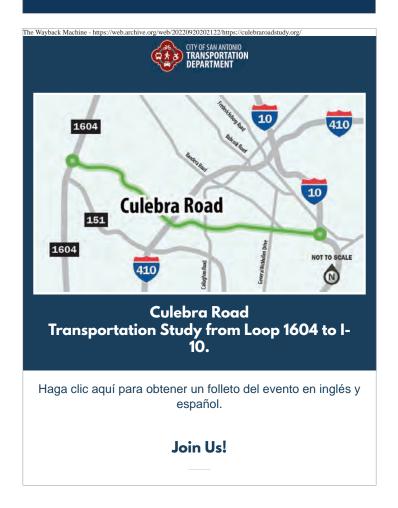


WEBSITE ANNOUNCEMENTS

On Aug. 31, 2022, the study webpage (culebraroadstudy.org) launched featuring the contents of the bilingual flyer and a link for interested persons to sign up for updates. On Sept. 16, 2022, the City of San Antonio launched the bilingual meeting information on www.SASpeakUp.com. The following documents both webpage updates.

Of note, between Aug. 31-Sept. 30, 2022, there were 684 unique visitors to the Culebraroadstudy.org, study webpage and 31 individuals signed up for future updates.

Culebra Road Transportation Study | Community Open Houses Documentation



APPENDIX A-3 | 37

Please join us for the second Culebra Road Community Open House series this October 2022! The City of San Antonio is studying transportation improvements to Culebra Road from Loop 1604 to I-10.

To sign up for email updates on the study, click the button below!

SIGN ME UP

In Person Community Open Houses

For individuals who would like to participate in person, please see below for details:

Saturday, October 1, 2022 9:00 a.m. to 11:00 a.m. Holy Cross High School - Convocation Center 426 N. San Felipe Street San Antonio, Texas 78228

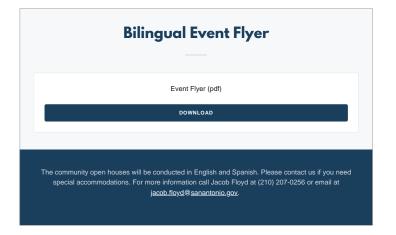
> Tuesday, October 4, 2022 6:00 - 8:00 p.m. Alamo City Apostolic Church 9302 Timber Path San Antonio, Texas 78250

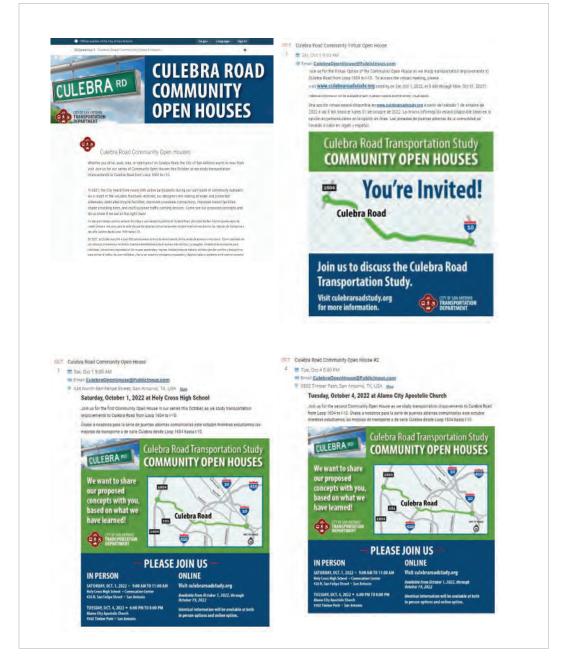
Virtual Community Open House

Persons interested in participating in the Virtual Community Open House can view the pre-recorded presentation and other materials on this website (culebraroadstudy.org) starting on Saturday, October 1, 2022 at 9:00 a.m.

The Virtual Community Open House will be available for viewing until Wednesday, October 19, 2022 at 11:59 p.m.

 $\label{lem:lemma:condition} \mbox{Identical information will be available at both in person and online options.}$





PARTNER COMMUNICATIONS EMAIL

On Sept. 16, 2022, an e-mail was sent to local partners and community organizations informing them of the meeting and asking for assistance getting the word out. The bilingual email included meeting details, a social media toolkit, social media graphics, and the flyer.

A follow up reminder e-mail was sent on Sept. 26, 2022.

The list of recipients and copies of the emails are included below.

Local Partners Sent Meeting Notice

Representing	
Alamo Area Metropolitan Planning Organization	1
San Antonio Mobility Coalition	
VIA Metropolitan Transit	
Bike San Antonio	
Activate San Antonio	
Pedal SATX	
WTS San Antonio	
San Antonio Wheelman	
San Antonio Walks	
ULI San Antonio - Urban Land Institute	
Texas Department of Transportation	
Great Northwest Library	
San Antonio Parks and Recreation Department	
San Antonio Parks Foundation	
San Antonio River Foundation	
Edgewood ISD	
Huppertz Elementary School	
IDEA Hidden Meadow	
Memorial High School	
Mexican American Catholic College	
Northside Independent School District	
San Antonio Independent School District	
St. Mary's University	
Alamo City Black Chamber of Commerce	
San Antonio Chamber of Commerce	
San Antonio Hispanic Chamber of Commerce	
South Texas Business Partnership	
San Antonio Women's Chamber of Commerce	

	Representing
Barrio	Barista
Birrieri	a y Taqueria Ay Arandas
El Roc	eo Seafood and Taqueria
Mario's	s Tacos
Mariso	os de Puerto
Culebr	a Creek Apartment Homes
Taurin	os Mexican Restaurant # 2
Alamo	City Apostolic Church
Basilio	a of the National Shrine of the Little Flower
	osary Catholic Church End Hope in Action Neighborhood ation
Northy	vest Crossing Neighborhood Association
Canter	bury Farms Community Association
Hidder	Meadow Neighborhood Association
Loma	Vista Neighborhood Association
Mount	ain View Neighborhood Association
Pipers	Meadow Neighborhood Association
Timbe	Ridge Neighborhood Association
Univer	sity Park Neighborhood Association
Wood	glen Neighborhood Association
Crown	Meadows Neighborhood Association
	a Park Neighborhood Association Northwest Community Improvement ation
Prospe	ect Hill Neighborhood Association
Thund	erbird Hills Neighborhood Association
Woodl	awn Lake Neighborhood Association

Culebra Road Transportation Study | Community Open Houses Documentation

Elizabeth Story

Jacob T. Floyd (Transportation) < Friday, September 16, 2022 4:05 PM Jacob T. Floyd (Transportation); Debora Gonzalez (Transportation); Catherine J Hernandez

(Halsportation) Join Us: Gulebra Road Transportation Study Community Open Houses Culebra Road Community Open House,pdf; Oct 1 2022 - Culebra Rd Community Open House.ics; Oct 4 2022 - Culebra Rd Community Open House.ics Subject: Attachments:

Dear Technical Advisory Group Member:

The City of San Antonio invites you to attend the second Community Open House series for the Culebra Road Transportation Study from Loop 1604 to I-10. Whether you drive, walk, bike, or ride transit on Culebra Road, the City of San Antonio wants to hear from you!

In 2021, the City heard from nearly 600 active participants during our last round of community outreach. As a result of the valuable feedback received, our designers are studying the possibility of wider and protected sidewalks, dedicated bicycle facilities, improved crosswalk connections, improved transit facilities, shade-providing trees, and multi-purpose

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Tuesday, October 4, 2022 6:00 p.m. to 8:00 p.m.

Alamo City Apostolic Church 9302 Timber Path San Antonio, TX 78250

Participate online by visiting www.culebrasroadstudy.org from Saturday, October 1, 2022, through Monday, October 31, 2022. Identical information will be available at both in person options and the online option. All comments are requested to be postmarked or received on or before Wednesday, October 19, 2022, to be included in the official documentation of the open house series. Additional comments will still be welcome until Monday, October 31, 2022 to be included in the project file.

The community open houses will be conducted in English and Spanish. Please contact us if you need special communication or accommodation arrangements. For more information, please view the attached flyer, visit www.culebraroadstudy.org or call Jacob Floyd at ...

The Culebra Road Corridor Study Team

APPENDIX A-3 | 40

Jacob Floyd, AICP, CNU-A

Transportation Planning Manager

Transportation Department 100 W. Houston St, 16th Floor | San Antonio, TX 78205 O: 210.207.0256

www.visionzerosa.com

www.sanantonio.gov/transportation



Elizabeth Story

Jacob T. Floyd (Transportation)
Monday, September 26, 2022 4:31 PM
Jacob T. Floyd (Transportation); Debora Gonzalez (Transportation); Catherine J Hernandez
(Transportation)

(transportation)
Reminder: Culebra Road Transportation Study Community Open Houses
Culebra Road Community Open House.ics; Oct
1 2022 - Culebra Rd Community Open House.ics; Oct Subject: Attachments:

4 2022 - Culebra Rd Community Open House.ics

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virtual uption

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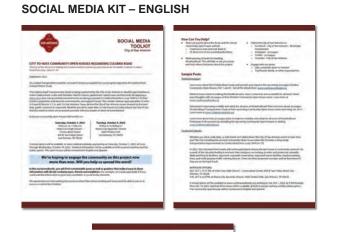
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Thank you,

The Culebra Road Corridor Study Team

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Email attachments included: IMAGES Culebra Road Transportation Study COMMUNITY OPEN HOUSES





TECHNICAL ADVISORY GROUP OUTREACH

On Sept. 16, 2022, an e-mail was sent to the membership of the study's Technical Advisory Group (TAG) to inform them of the meeting. The email included meeting details and the bilingual flyer.

A follow up reminder e-mail was sent on Sept. 26, 2022.

The list of recipients and copies of the emails are included below.

TAG Membership Sent Meeting Notice

Name	Representing
Abigail Kinnison	VIA Metropolitan Transit
Alanna Reed	City of San Antonio
Andrew Gutierrez	City of San Antonio
Arturo Herrera	VIA Metropolitan Transit
Bianca Thorpe	VIA Metropolitan Transit
Brandon Ross	City of San Antonio
Brenda Hicks-Sorensen	City of San Antonio
Brian Mast	San Antonio River Authority
Christina De La Cruz	City of San Antonio
Christopher Georges	City of San Antonio
Clifton Hall	Alamo Area Metropolitan Planning Organization
Clinton Eliason	City of San Antonio
Colleen Hord	City of San Antonio
Dale Picha	Texas Department of Transportation
Darcie Schipull	Texas Department of Transportation
David McBeth	City of San Antonio
David Pulido	Texas Department of Transportation
Dean White	San Antonio Police Department
Deborah Scharven	City of San Antonio
Emilio Rodriguez	City of San Antonio
Emily Royall	City of San Antonio
Eric Salazar	City of San Antonio
Erika Ragsdale	City of San Antonio
Guadalupe Campos	City of San Antonio
Harley Hubbard	City of San Antonio
Ian Benavidez	City of San Antonio
Isidro Martinez	Alamo Area Metropolitan Planning Organization
Jennifer Lopez-Garza	City of San Antonio
Jesse Quesada	City of San Antonio
Jessica Dovalina	City of San Antonio
Jose Salazar	City of San Antonio

Culebra Road Transportation Study | Community Open Houses Documentation

APPENDIX

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Name	Representing
Joshua Jaeschke	City of San Antonio
Kathleen Buckalew	City of San Antonio
Kimberly Rendon	City of San Antonio
Laura Parker	City of San Antonio
Laura Reyna	City of San Antonio
Leroy San Miguel	Northside Independent School District
Lilly Banda	City of San Antonio
Marc Jacobson	City of San Antonio
Marcus Hammer	City of San Antonio
Mark C Bird	City of San Antonio
Mark Loiselle	City of San Antonio
Martin Molina	Edgewood Independent School District
Matt Jimenez	San Antonio Fire Department
Nadia Islam	San Antonio Housing Authority
Nora Gonzales	City of San Antonio
Robert Potter	City of San Antonio
Roberta Sparks	City of San Antonio
Rudy Nino	City of San Antonio
Sek Choy	City of San Antonio
Sonia Jiménez	Alamo Area Metropolitan Planning Organization

Culebra Road Transportation Study | Community Open Houses Documentation

Elizabeth Story

Subject: Attachments:

Jacob T. Floyd (Transportation) <
Friday, September 16, 2022 4:05 PM
Jacob T. Floyd (Transportation); Debora Gonzalez (Transportation); Catherine J Hernandez
(Transportation)
Join U.S. Culebra Road Transportation Study Community Open Houses
Culebra Road Community Open House pdf; Oct 1 2022 - Culebra Rd Community Open House.ics; Oct
47323 Culebra Rd Community Open House pdf; Oct 1 2022 - Culebra Rd Community Open House.ics

4 2022 - Culebra Rd Community Open House.ics

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Tuesday, October 4, 2022 6:00 p.m. to 8:00 p.m. Alamo City Apostolic Church

9302 Timber Path San Antonio, TX 78250

virtual uption

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Thank you,

The Culebra Road Corridor Study Team

APPENDIX A-3 | 45

Jacob Flovd, AICP, CNU-A

Transportation Planning Manage Transportation Department

100 W. Houston St, 16th Floor | San Antonio, TX 78205

0: 210.207.0256

www.visionzerosa.com www.sanantonio.gov/transportation



Elizabeth Story

Jacob T. Floyd (Transportation) Monday, September 26, 2022 4:31 PM From:

Jacob T. Floyd (Transportation); Debora Gonzalez (Transportation); Catherine J Hernandez

Subject: Attachments:

Transportation (Transportation Study Community Open Houses Culebra Road Transportation Study Community Open Houses Culebra Road Community Open House pdf, Oct 1 2022 - Culebra Rd Community Open House pdf, Oct 1 2022 - Culebra Rd Community Open House ics; Oct 4 2022 - Culebra Rd Community Open House ics

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The Culebra Road Corridor Study Team

APPENDIX A-3 | 46

Jacob Floyd, AICP, CNU-A
Transportation Planning Manager
Transportation Department
100 W, Houston St, 16th Floor | San Antonio, TX 78205
O: 210.207.0256 www.visionzerosa.com www.sanantonio.gov/transportation CITY OF SAN ANTONIO
TRANSPORTATION
DEPARTMENT

NEWS RELEASE

On Sept. 20, 2022, the City of San Antonio emailed out a news release describing the project and advertising the community open houses. A copy is included below.

- meeting

Culebra Road Transportation Study | Community Open Houses Documentation

Elizabeth Story

Joe Conger (Transportation)
Tuesday, September 20, 2022 9:47 AM
Elizabeth Story From: Sent:

NEWS RELEASE: Residents invited to Share Feedback at Culebra Corridor Open Houses

From: City Communications & Engagement <Office.Communications@sanantonio.gov> Sent: Tuesday, September 20, 2022 9:46 AM

Subject: NEWS RELEASE: Residents invited to Share Feedback at Culebra Corridor Open Houses



FOR IMMEDIATE RELEASE CONTACT: Joe Conger, 210--207-5010

Residents invited to Share Feedback at Culebra Corridor Open Houses

SAN ANTONIO (September 20, 2022) — Whether you drive, walk, bike, or ride transit on Culebra Road, the City of San Antonio wants to share our proposed concepts with you, based on what we have learned from previous meetings with residents. Residents are invited to participate in a second Community Open House series this October as the City studies transportation improvements to Culebra Road from Loop 1604 to

In-person community open houses will be held on:

Saturday, October 1, 2022 9:00 a.m. to 11:00 a.m. Holy Cross High School – Convocation Center 426 N. San Felipe Street San Antonio, TX 78228

6:00 p.m. to 8:00 p.m. Alamo City Apostolic Church 9302 Timber Path San Antonio, TX 78250

Tuesday, October 4, 2022

Residents seeking to attend virtually may participate in an online open house at www.culebraroadstudy.org starting on Saturday, October 1, 2022, at 9 a.m. through Monday, October 31, 2022. The online open house will feature identical information as the in-person events. All events will be conducted in English and Spanish

The City is conducting the Culebra Road Transportation Study to identify opportunities to make Culebra Road a safer and friendlier road for drivers, pedestrians, transit users and bicyclists. The team has developed proposed improvements that enhance pedestrian and bicyclist environments, and support transit. The corridor extends approximately 13 miles in Council Districts 1, 5, 6, and 7 in San Antonio, Texas.

Since April, the City has heard input from nearly 600 residents. As a result of the feedback received, City designers are studying the possibility of wider and protected sidewalks, dedicated bicycle facilities, improved crosswalk connections, improved transit facilities, shade-providing trees, and multi-purpose traffic-calming devices. At the October 2022 community open houses, participants can see our proposed concepts and

Comments may be submitted at the in-person meetings, at www.culebraroadstudy.org, or mailed to City of San Antonio Transportation Department c/o Poznecki-Camarillo, 5835 Callaghan Road, Suite 200, San Antonio, Texas 78228. All comments are requested to be postmarked or received on or before Wednesday, October 19, 2022, to be included in the official documentation of the open house series. Additional comments will still be welcome until Monday, October 31, 2022 to be included in the project file.

The community's participation is critical to help identify a unifying vision for Culebra Road.

For more information or for special accommodations, visit www.culebraroadstudy.org or call Jacob Floyd at (210) 207-0256 or by email at

Culebra Road Transportation Study Area

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INTRODUCTIO

STUDY GOALS

CORRIDOR

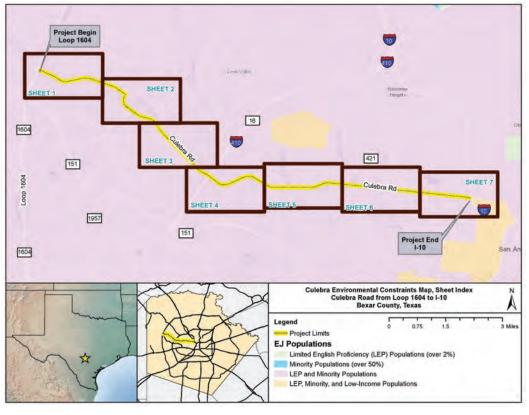
APPENDIX B

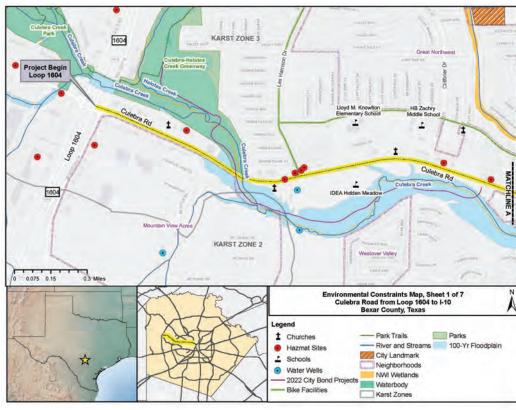
Multimodal Corridor Transportation Study

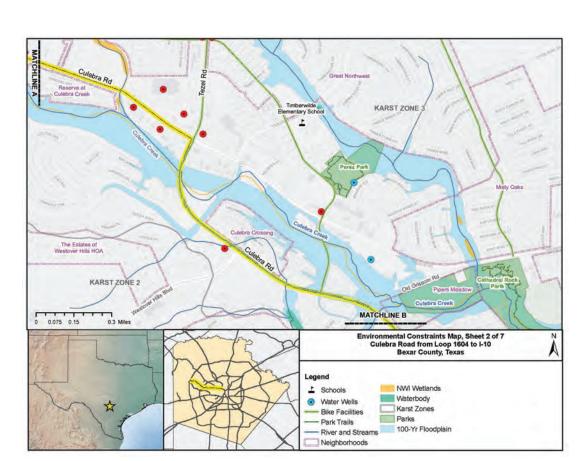
Appendix B: Culebra Environmental Constraints Map

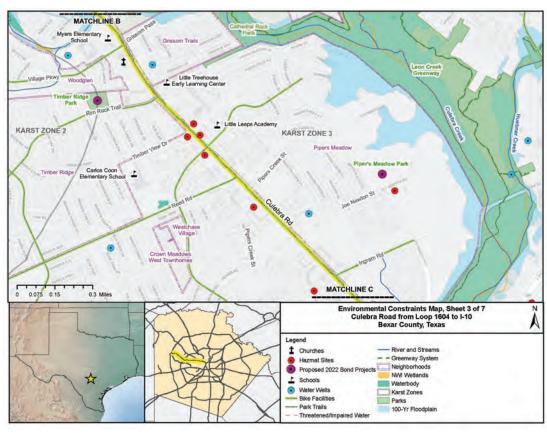
APPENDIX B | 1

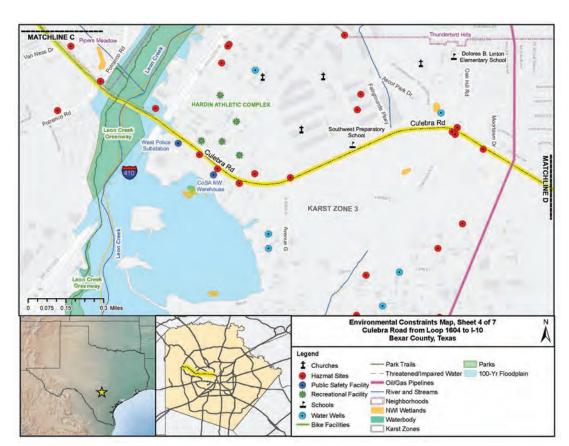
FIGURE 5: Environmental Constraints Map

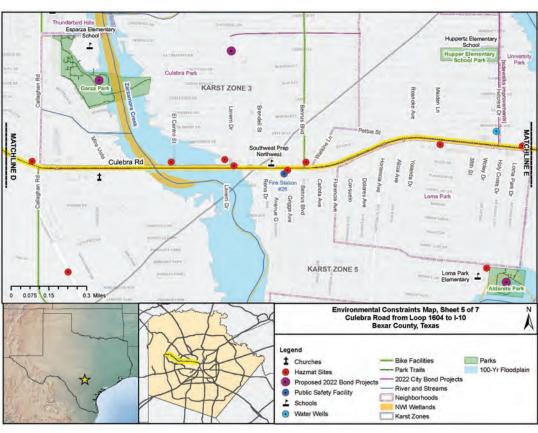


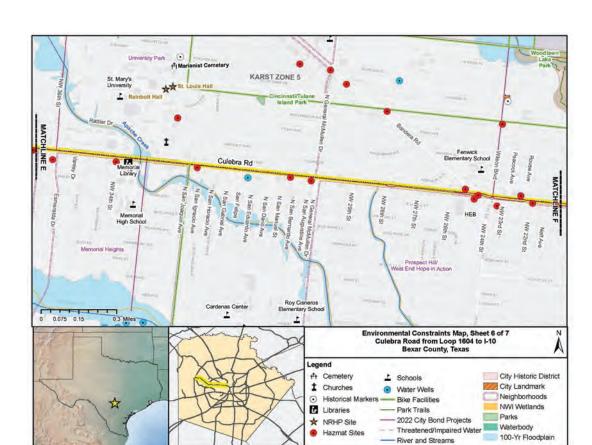


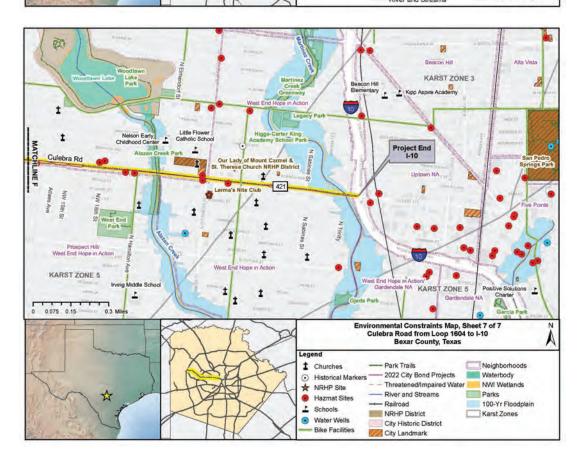














CORRIDOR



Multimodal Corridor Transportation Study

Appendix C: Culebra Road Corridor Study - Existing Conditions Memo (April 12, 2022)

APPENDIX

APPENDIX C

Existing Conditions Technical Report



MEMO

TO: Debora Gonzalez, Project Manager

Jemal Ali, PE, PMP; Michael Trueblood, PE, PTOE; Julio Ramos, PE, PTOE FROM:

SUBJECT: 30900399A Culebra Road Corridor Study; Task 3: Existing Conditions Memo Rev 01

DATE: April 12, 2022

EXISTING CONDITIONS SUMMARY

WSP was contracted by the City of San Antonio to complete a Multimodal Planning Study for Culebra Road. The project limits are from Loop 1604 to IH-10, and the corridor extends approximately 13 miles. The project is located within northwest Bexar County, City of San Antonio, Texas. This memo summarizes the existing conditions of the corridor and documents the corridor characteristics, establishes how the corridor is currently functioning, identifies existing issues that may limit or challenge potential solutions or recommendations, and identifies deficiencies or safety issues that should be addressed. This memo also describes the review of available data, crash analyses, traffic data collection and field visit notes.

The study area map showing the limits of the corridor is presented in Figure 1. The western and eastern ends of the project are part of the TxDOT system. On the west, FM 471/Culebra Road extends from Loop 1604 to Tezel Road. On the east, Spur 421/Culebra Road extends from NW 24th Street to I-10. Culebra Road is oriented in a northeasterly alignment. For purposes of this study, Culebra Road will be referred to as east-west and the intersecting roadways and side streets will be referred to as north-south orientation.

9311 San Pedro, Suite 700 San Antonio, TX 78216

Tel.: 210-247-4360 wsp.com



Figure 1 - Project Limits

APPENDIX





CORRIDOR CHARACTERISTICS

CORRIDOR REVIEW

Culebra Road is a 13 miles long project extends from IH-10 to Loop 1604. It is a principal/minor arterial ranging from four to seven lanes cross section spanning several land use characteristics.

Based on the land use characteristics, the corridor was reviewed in five sections as presented in $Table\ 1 and\ shown\ in\ \textbf{Figure}\ \textbf{2.}\ The\ functional\ classification\ of\ Culebra\ Road\ is\ split\ into\ Principal$ Arterial where the road is intended to serve through traffic where access is carefully controlled and minor Arterial with more emphasis on land use accesses and lower level of traffic mobility traffic mobility and the controlled arterial with more emphasis on land use accesses and lower level of traffic mobility and the controlled arterial with more emphasis on land use accesses and lower level of traffic mobility and the controlled arterial with more emphasis on land use accesses and lower level of traffic mobility and the controlled arterial with the controlled arterial with more emphasis on land use accesses and lower level of traffic mobility and the controlled arterial with more emphasis on land use accesses and lower level of traffic mobility and the controlled arterial with more emphasis on land use accesses and lower level of traffic mobility and the controlled arterial with more emphasis on land use accesses and lower level of traffic mobility and the controlled arterial with the controlled arterial withthan the Principal Arterial.

Table 1 - Culebra Road Segments and Functional Classification						
Sections	Locations	Functional Class				
Section A	I-10 to Bandera Road	Principal Arterial – Other				
Section B	Bandera Road to Callaghan Road	Minor Arterial				
Section C	Callaghan Road to IH-410	Minor Arterial				
Section D	IH-410 to Tezel Road	Minor Arterial				
Section E	Tezel Road to Loop 1604	Principal Arterial – Other				



- Corridor Segments



FIELD OBSERVATIONS

WSP performed site visits in December 2020 while traffic data was being collected and again in January 2021. The traffic data collection was conducted on a day with desirable weather conditions and good driving visibility. An overview of the information that was collected during the December field visit as well as collected from previous data is summarized below:

- Corridor length 13 miles
- Corridor roadway functional classification Principal Arterial Other
- Area Type Urbanized Area classification
- Posted Speed Limits:
 - o Loop 1604 to Reed Road 45 mph
 - o Reed Road to I-10 40 mph
- $\bullet \quad \text{Lane width existing Lane width varies throughout the corridor.} \ \text{The following are typical} \\$ approximate dimensions:
 - o 6-lane, 2-way with Two Way Left Turn Lane (TWLTL) measured between 19th Street and 18^{th} Street;
 - Outer lane 12 to 14 feet
 - Middle lane 11 feet
 - Inner lane 10 feet
 - TWLT 14 to 16 feet
 - o $\,$ 6-lane, 2-way with TWLTL with bike lane measured between Van Ness Road and Potranco Road:
 - Outer lane 11 feet
 - Middle lane 11 feet
 - Inner lane 12 feet
 - TWLT 16 feet
 - Bike lane 5 feet
 - o 4-lane, 2-way with TWLTL measured between Loma Park Drive and 36th Street;
 - Outer lane 12 feet
 - Inner lane 11 feet
 - TWLT 14 to 15 feet
 - o 4-lane, 2-way with median measured near idea school;
 - Outer lane 13 to 14 feet
 - Inner lane 11 feet
 - Median 17 feet
 - o 4-lane, 2-way without median measured West of Callaghan Road;
 - Outer lane 12 to 13 feet
 - Inner lane 11 to 12 feet

1120

- Roadway width
 - o Curb-to-curb width for Loop 1604 to Tezel/Grissom is approximately 65 to 66 feet, except along a segment east of Loop 1604 which is 62 feet wide.
 - $\circ\quad$ Tezel/Grissom to IH-410 is 92 to 94 feet wide
 - o IH-410 to Avenue G is 58 feet wide:
 - o Avenue G to Callaghan Road is 48 feet wide; and
 - o Callaghan Road to Wilson Boulevard is 61 feet wide; and
 - Wilson Boulevard to IH-10 is 82 feet wide.
- Right-of-Way

Apparent Right-of-Way (ROW) reviewed and approximate width is listed below. However, PCI is currently collecting ROW information along the corridor that will be included in the project base map.

- o 1604 to Tezel/Grissom is 120 feet
- o Tezel/Grissom to Old Grissom Rod is 100 feet
- $\circ\quad$ Old Grissom Rod to IH-410 is 100 feet wide
- o IH-410 to Moortown Drive = 80 feet
- o Moortown Drive to 490 feet West of El Centro = 100 feet
- o 490 feet West of El Centro St to Bandera = 80 feet
- o Bandera to IH-10 = 120 feet

Lane Assignment

- o 6-lane, 2-way with Two Way Left Turn Lane (TWLTL) between SB I-10 Frontage Road and Bandera Road
- o 4-lane, 2-way with TWLTL between Bandera Road Callaghan Road:
- o 4-lane, 2-way between Callaghan Road and Avenue G
- 4-lane, 2-way with TWLTL between Avenue G and IH-410;
 6-lane, 2-way with TWLTL between IH-410 and Grissom Road;
- o 4-lane, 2-way with TWLTL between Grissom Road and Loop 1604
- Roadway surface condition appears to need improvement between Bandera and IH-410
- During the field visit, several instances occurred where pedestrians were spotted crossing mid-block across Culebra Road, especially at bus stop locations. In one instance, a pedestrian was seen crossing mid-block but did not use the dedicated mid-block crosswalks which were within 500 feet.
- Sidewalk condition and ADA requirements

Sidewalk/curb conditions, missing sidewalk locations, and ADA requirements at the intersections were evaluated based on the site visit and a desktop review.

- o Missing sidewalks were observed at the following locations and included in the basemap - Appendix A:
 - Between Rogers Road to Nueces Canyon south side;
 - Between Joe Newton and Ingram Road north side;
 - Between Avenue G and Tomslick Ave south side; and



- Between Tomslick Ave and Callaghan Road- both north and south sides. It should be noted that there were a few discrepancies among sidewalk and sidewalk gaps between the data received from the City and the GIS database The corrections were marked during the field visit to replicate existing field conditions
- o 2010 ADA Standards for Accessible Design (403.5.3 Passing Spaces) require a 5 feet X 5 feet passing space for 200 feet where sidewalks width is less than 5 feet. Below is the list of locations where the existing sidewalk appeared to be less than 5 feet.
 - WB sidewalk between Moortown Dr and St Joseph's Way.
 - WB sidewalk between Mira Vista and Callaghan Road.
 - EB sidewalk between Mira Vista and Zarzamora Creek
 - WB sidewalk between Camino Santa Maria and NW 36th St.
- o Existing ADA ramps do not have "Detectable Warnings" at: Culebra Road at Culebra Station;
 - Culebra Road at NW 36th St;
 - Culebra Road at Memorial St
 - Culebra Road atd Camino Santa Maria;
 - Culebra Road at N San Felipe Ave;
- o Existing ADA ramps do not met/improvements are necessary at:
 - Culebra Road at Alamo Downs Pkwy has ADA ramps but no connectivity between South leg curbs ramps;
 - South leg crosswalk is missing at Culebra Road at NW 38th St,
 - The existing signal pole is in the center of the south leg curb ramp at the Culebra Road and IH-10 SBFR intersection; and
 - Several unsignalized intersections do not have detection warning surfaces and crosswalks.
- There are unsignalized mid-block crosswalks at the following locations between I-10 and IH-
- o Standard crossing between Mira Vista and Zarzamora Creek close to the bus stop; and o Z-type crossing between 28th Street and Rollins Avenue An existing Bike Lane along Culebra Road (5 feet bike lane separated by an 8" solid white line
- on both sides of the roadway) located between IH-410 and Grissom Road.
- · No bicyclists were spotted during field visit.
- . No bike amenities (bike racks) were spotted adjacent to the roadway
- . The bus stops summarized using GIS were consistent with those in the field.
- No on-street parking along the corridor.
- In the AM peak hour period, no congestion was spotted along the corridor. In the PM peak hour period, vehicles were queued at the IH-10, IH- 410 and Loop 1604 approaches. Loop 1604 queue extended to Rogers Road.
- Work zones were in place at the following locations; IH-410 and Loop 1604 interchange,





including on-going construction along the U-Turns. The sidewalk was closed in the eastbound direction approximately 50 feet east of Rogers Street.

CORRIDOR BASE MAPPING

The corridor base map was developed with an aerial image as background with apparent ROW identified. WSP extracted the following key elements from the GIS database provided by the City to include in the base map (refer to ${\bf Appendix}\;{\bf A})\!\!:$

- Bike facilities:
- Land uses:
- VIA bus network and bus stop locations;
- Apparent ROW; and
- Existing Sidewalk

During the site visit, there were a few discrepancies in sidewalk and sidewalk gaps compared with the GIS file received from the City. The data from the GIS layers are updated based on the field visit to reflect existing conditions as per the field visit.

In addition, the base map includes pavement markings, lane designations, land uses, historic

The GIS maps highlighting Bike facilities and the VIA bus network are presented in Figure 3 on page 9 and Figure 4 on page 10. Eight routes (82, 88, 282, 288, 606, 610, 618 and 660) are serving the Culebra corridor as shown in Figure 4. Route 82 operates at 15 minute frequency during the peak hour. Routes 282 and 288 serve the corridor late nights. The rest of the routes are running with a 60 minute frequency.

LAND USES

 $WSP\ also\ mapped\ land\ uses\ from\ the\ GIS\ data\ received\ from\ the\ City.\ The\ corridor\ passes\ through$ predominantly residential areas throughout the project with commercial spaces on both sides of the road with the exception of the industrial land uses located between IH-410 and Callaghan Road where Southwest Research Institute and Gustafson Stadium are located. Schools (including Idea Hidden Meadow School located between Lee Harrison Drive and Selene Drive), churches and historical places (including Basilica of the National Shrine of the Little Flower) are scattered throughout the corridor. Five schools and seventeen churches are located within 500 feet of Culebra Road, as noted below:

- Schools
 - o Little Flower Catholic Private School
 - o Marin B. Fenwick Academy Public School
 - o Southwest Preparatory School Northwest Elementary Public School
 - o Pauline Nelson Early Childhood Education Center
 - o IDEA Hidden Meadow Public School



- Churches
 - o Our Lady of Mount Carmel Church*
 - Westover Hills Assembly of God Church
 Iglesia Cristiana Northwest Church

 - Saint Michaels Episcopal Church
 - o Our Lady of Mount Carmel and Saint Therese Church*
 - Basilica of The National Shrine of The Little Flower
 - Centro Cristiano International
 - Church of Christ Iglesiani Cristo
 - Congregation Vida Nueva Fuente de Vida Christian Church

 - Hosanna Assembly of God Church Korean United Baptist Church
 - Laurel Street Church of Christ
 - Luz Apostolica Church Redemption Tabernacle

 - o Village Parkway Baptist Church
 - o West End Baptist Church
- * Historical buildings

Figure 5 on page 11 illustrates the key land uses within the project limits.

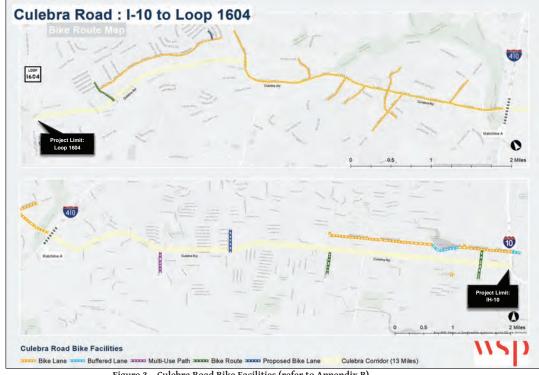


Figure 3 - Culebra Road Bike Facilities (refer to Appendix B)



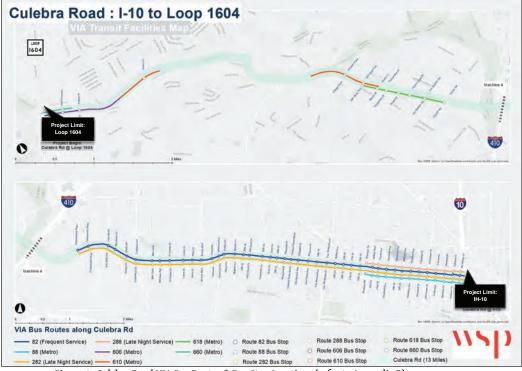


Figure 4 -Culebra Road VIA Bus Routes & Bus Stop Locations (refer to Appendix B)





Figure 5 - Land Use Map (refer to Appendix B)



TRAFFIC DATA COLLECTION

To identify current traffic patterns along the corridor, typical weekday traffic counts were conducted at 38 project intersections as well as three 24-hour bi-directional counts. The turning movement counts included passenger car, bus, and heavy vehicle traffic, with additional counts performed to record bicycle and pedestrian activity along the corridor on December 15, 2020 from 7:00 AM - 9:00 AM for the AM peak period and from 4:00 PM - 6:00 PM for the PM peak period.

Table 2 includes a summary of the three 24-hour count locations along Culebra Road. This information was used to determine the corridor-wide morning and afternoon peak hours. The peak hours were determined to be 7:15 AM to 8:15 AM and 5:00 PM to 6:00 PM.

Table 2 – 24-hour Count Summary							
Count Location	Eastbound	Westbound	Bi-Directional				
1 - Between Loop 1604 and Rogers Road	17,890	17,642	35,532				
2 - Between Ingram Road and Potranco Road	20,540	18,614	39,154				
3 - Between N Hamilton Ave and Elmendorf Street	18 153	16 004	35 147				

Table 3 summaries of the average AM and PM period heavy vehicle percentages along the corridor for each segment based on the AM and PM period counts. As show, Peak period counts collected are 7:00 AM to 9:00 AM and 4:00 PM to 6:00 PM. On average, the AM has 3.0% heavy vehicles estimate whilst the PM is about 1.6%. Detailed analyses for each intersection are provided in Appendix D.

Table 3 - AM/PM Peak Period Heavy Vehicle Percentage Summary

	A	AM P		
Segment	Eastbound	Westbound	Eastbound	Westbound
1: Loop 1604 to Tezel/Grissom	2.1%	3.2%	1.1%	0.8%
2: Tezel to Micron Dr/Pipers Ln	2.2%	3.5%	1.6%	0.8%
3: Micron Dr/Pipers Ln to IH-410	2.5%	3.9%	2.3%	0.8%
4: IH-410 to Callaghan Road	3.2%	4.9%	2.5%	2.9%
5: Callaghan Road to Wilson Blvd	2.1%	2.5%	1.4%	1.2%
6: Wilson Boulevard to IH-10	2.2%	4.4%	2.4%	1.8%
Overall	2.4%	3.7%	1.9%	1.4%
Bi-Directional	3.	0%	1.	6%

In addition to the data collection conducted in December 2020 the during COVID-19 pandemic, the following traffic data was reviewed:

 Traffic counts prior to 2020 – provided from the City of San Antonio (TMC collected in April and June 2017; ATR collected on Year 2014/2015);

Page 1



- 2. Traffic count data from SA Tomorrow Multimodal Transportation Plan TMC collected in
- 3. Historical count data obtained from the TxDOT Statewide Traffic Analysis and Reporting System (STARS II) Database or the TxDOT Statewide Planning Map.

Traffic data sources and Locations are presented in Figure 6:



Figure 6 - Traffic Data Location Map

Historical counts at Grissom Road/Tezel Road, Ingram Road, Callaghan Road and Zarzamora Street intersections are repeated in 2020 to develop COVID-19 factor (CF) to adjust the 2020 counts. Detailed traffic projection and COVID-19 count adjustment methodology memo is included in Appendix C. Detailed tabulated TMC and ATR data collected in December 2020 are included in Appendix D.

CRASH SUMMARY

Crash data was provided by City of San Antonio and consisted of crash data between January 1, 2017 - October 31, 2020. The data represented all vehicle, bicycle, and pedestrian crashes along on Culebra Road from Loop 1604 to 1-10. Crashes were summarized for each study intersection as well as between each intersection. The crash summary focused on an evaluation of crash severity by location for vehicles, pedestrians, and bicycle crashes.

Table 3 to Table 6 summarize the findings by crash year, travel mode, and severity.

A total of 3,488 crashes were recorded during January 1, 2017 - October 31, 2020 along Culebra Road. The majority of crashes (1,923) were corridor crashes that occurred between the study intersections, while the remaining 1,565 crashes occurred at intersections.

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APPENDIX

APPENDIX C



A total of 19 bicycle crashes were recorded, with 12 occurring at intersections. Similarly, 27 out of the 64 pedestrian crashes occurred at intersection

As shown in Table 6, from a total of 64 pedestrian crashes, 37 are along the corridor while 27 are located at intersections. Most of the crashes (15 crashes) are with in Bandera Road to SB IH-410 FR followed by 11 crashes between SB I-10 FR to Bandera Road. The Zarzamora St intersection has the most pedestrian crashes (7 crashes) followed by Pipers Ln / Micron Dr (3 crashes). There were 11 fatal pedestrian crashes as shown in Figure 8 on page 16.

Similarly, from a total of 19 bicycle crashes, 7 are along the corridor while 12 are located at intersections. Most of the crashes (3 crashes) are with in Bandera Road to SB IH-410 FR. The Cliffbrier Road intersection has the most bicycle crashes (3 crashes) followed by Reed Road (2 crashes). No bicycle fatal crashes were recorded during January 1, 2017 - October 31, 2020.

Detailed crash analyses tables for the corridor and intersections are presented in Appendix E.

Year	Total Crashes	Percentag	
2017	890	25.5%	
2018	898	25.7%	
2019	979	28.1%	
2020*	721	20.7%	

st 2020 crash counts are only from January 1 to October 31

Transportation Mode	Angle	Head On	Left Turn	No Data¹	Other	Rear End	Side swipe	Single Motor Vehicle	Total	Percent
Bicycle	0	0	0	0	0	0	1	18	19	0.5%
Pedestrian	0	0	0	0	0	2	0	62	64	1.8%
Vehicle	529	42	687	227	4	1243	334	339	3405	97.6%
Total	529	42	687	227	4	1245	335	419		
Percentage	15.2%	1.2%	19.7%	6.5%	0.1%	35.7%	9.6%	12.0%		
Total Crashes (Jan 1, 2017 - Oct	31, 2020))	T-	ble 6 – 0						3,488
								•		
	Un	known	No	ot Injure	d	Injured		Fatal	Total	Crashes
		669		2197		606		16		
Total		19.2%		63.0%		17.4%		0.5%		
Total Percentage Total Intersection										



Location	Unknown	Not Injured	Injured	Fatal	Total Crashes
	Pedestria	an			
At Intersection	1	3	21	2	27
On Segment	1	1	26	9	37
Total	2	4	47	11	64
Percentage	3.1%	6.3%	73.4%	17.2%	100.0%
	Bicycle				
At Intersection	0	1	11	0	12
On Segment	0	0	7	0	7
Total	0	1	18	0	19
Percentage	0.0%	5,3%	94.7%	0.0%	100.0%

 $\textbf{Figure 7} \ \text{next page and } \textbf{Figure 8} \ \text{on page 17} \ \text{illustrate the crashes locations and heat map along}$ the study corridor. Additional crash summaries are included in Appendix E.



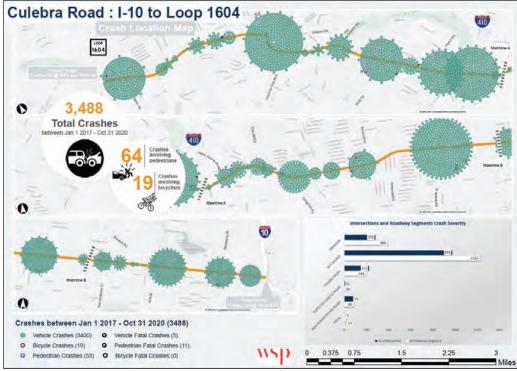


Figure 7 - Crash Map

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Figure 8 - Crash Heat Map



EXISTING CONDITION OPERATIONAL ANALYSIS

Study area geometry, signal timings, and turning movement counts for each study intersection were input into Synchro 10 to evaluate AM and PM peak hour traffic operational characteristics. Corridor and intersection geometry were derived from the SA Tomorrow Synchro network, field observations, and aerial imagery. Average intersection delay for each intersection was calculated for both AM and PM peak hour.

Delay is defined as additional travel time experienced by a driver beyond that required to travel at the desired speed, measured in seconds. Based on the calculated delay, a Level of Service (LOS) is assigned to represent quality of service. The Highway Capacity Manual (HCM) defines six levels of service, ranging from A to F based on a quantitative value of performance measures. LOS A represents the best operating conditions during analysis periods and LOS F represents the worst $conditions. \ A \ change \ of LOS \ indicates \ that \ roadway \ performance \ has \ transitioned \ from \ one \ given \ range \ of \ traveler-perceivable \ conditions \ to \ another \ range.$

 $\label{thm:constraints} \textbf{Table 8} \text{ shows the performance measures and the thresholds for assigning LOS.}$

Table 8 - Highway Capacity Manual Performance Measures

LOS Threshold	Signalized Intersection (sec/veh)	Unsignalized Intersection (TWSC)* (sec/veh)
LOS A	<=10	0-10
LOS B	>10-20	>10-15
LOS C	>20-35	>15-25
LOS D	>35-55	>25-35
LOS E	>55-80	>35-50
LOS F	>80	>50

* The overall intersection delay displayed for TWSC intersections is based on the highest turning movement delay calculated along the stop-controlled approach.

To develop 2020 existing traffic volumes for the traffic operation analyses, the December 2020 counts conducted during the COVID-19 pandemic were adjusted using a COVID-19 Factor (CF) of 1.30. The historical data collected in 2015 and 2017 were adjusted to 2020 using a growth rate developed from AAMPO's TDM model. Detailed traffic projection and COVID-19 count adjustment methodology memo is included in ${\bf Appendix}~{\bf C.}$

Table 8 and Table 9 show the Level of Service and Delay analysis results for the signalized andunsignalized intersections for the 2020 AM and PM Peak hours along the Culebra Road corridors. Synchro outputs are included in ${\bf Appendix}\ {\bf F.}$



Table 9 - Signalized Intersection Level of Service and Average Delay

	_	AM Peak Hour			ak Hour
ło.	Intersection	Level of Service	Delay (s/veh)	Level of Service	Delay (s/veh)
1	LP1604 SBFR	F	160.0	F	108.1
2	LP1604 NBFR	F	171.8	F	187.2
3	Rogers Road	С	23.6	D	44.5
4	Les Harrison Dr	С	24.9	С	31.1
5	Cliffbrier Dr	В	11.0	В	13.5
6	Nueces Canyon/Village Park	В	16.8	В	14.3
7	Walmart/Commercial Properties	A	10.0	С	33.1
8	Grissom Road/Tezel Road	F	178.0	F	138.0
9	Arcadia Creek	С	21.2	A	3.4
10	Westover Hills Blvd/Ensenada	F	95.0	D	43.7
11	Ansley Bend Dr/Timber Path	В	19.3	С	28.7
12	Old Grissom Road	A	9.8	В	11.0
13	Village Pkwy/Grissom Pass	С	26.4	A	9.0
14	Rim Rock Trail/Commercial Property	В	10.9	В	18.4
15	Timber View Dr	D	35.9	A	6.4
16	Micron Dr/Pipers Ln	В	17.9	В	15.1
17	Reed Road	F	82.7	С	27.9
18	Ingram Road	С	20.6	F	128.1
19	Potranco Road	F	245.8	F	97.7
20	IH-410 SBFR	F	160.0	F	141.8
21	IH-410 NBFR	F	171.8	F	98.0
22	Commercial Properties/Baseball Field	С	24.0	В	10.1
23	Alamo Downs Pkwy	В	16.5	В	16.2
24	Tom Slick Ave/Oak Hill Road	С	20.9	D	41.3
25	Callaghan Road	С	33.5	D	39.3
26	El Centro St	В	14.2	В	15.0
27	Benrus Blvd	D	38.6	С	33.3
28	Alicia Ave	A	6.6	A	3.2
29	NW 38th St	В	11.7	A	8.9
30	Esmeralda Dr/NW 36th St	D	49.6	D	53.0
31	Memorial St/Rattler Dr	A	2.2	A	9.9
32	Camino Santa Maria	В	16.0	С	25.6
33	N San Felipe Ave	В	10.4	С	24.9
34	General McMullen Dr	D	36.0	D	52.7
35	Bandera Road	E	77.4	F	98.3
36	NW 24th St/Wilson	С	29.8	E	66.3
37	NW 19th St	A	5.3	В	10.4
38	Elmendorf St	A	7.1	В	14.3
39	N Zarzamora	С	33.3	D	37.6
40	IH-10 EBFR	С	31.8	E	69.6
41	IH-10 WBFR	С	29.2	С	33.2

As shown in Table 9, eight of the 41 signalized intersections (including Loop 1604) are operating at LOS F, two are operating at capacity (LOS E) and the remaining are operating at LOS D or better.

APPENDIX

Table 10 - Unsignalized Intersection Level of Service and Average Delay

			Maior	AM Peak Hour				PM Peak Hour			
No	Intersection	Stop- Control Approach	Major Street Left Turn	LOS	Delay (s/veh)	Left Turn LOS	Left turn Delay (s/veh)	LOS	Delay (s/veh)	Left Turn LOS	Left turn Delay (s/veh)
1	Mountain View	NB	WBL	С	20.9	С	20.6	C	22.4	С	23.4
2	Coppertree Blvd	SB	EBL	F	78.7	В	10.1	F	>300	С	19.1
3	Camino Rosa	SB	EBL	C	20.8	В	10.1	F	77.3	С	24.7
4	Grissom Gate	WB	SBL	F	160.4	С	17.3	F	65.2	F	106.2
5	Pipers Creek St -SB	SB	EBL	F	56.3	С	15.5	F	>300	F	59.8
6	Pipers Creek St -NB	NB	WBL	F	218.3	F	>300	F	>300	F	155.5
7	Joe Newton St	SB	EBL	E	43.8	С	17.5	F	155.5	F	>300
8	Van Ness	EB	NBL	A	0	F	189.2	F	114.1	E	45.9
٥	van ness	WB	SBL	A	0	В	14.4	F	>300	F	90.7
9	Avenue G	NB	WBL	E	35.1	С	17.9	D	32.1	В	10.5
10	Fairgrounds Pkwy	SB	EBL	В	13.2	В	11.2	F	>300	В	12.9
11	Cantebury	NB	WBL	В	13.9	В	11	C	19.6	С	15.6
12	Hortencia Ave	NB	WBL	C	19.6	В	10.7	E	43.2	В	12.8
13	Mira Vista	SB	EBL	С	19.4	В	10.7	E	40.8	В	12.5
14	NW 34th St	NB	WBL	C	23.5	В	11.1	D	28.8	В	11.9
15	Neff Road	NB	WBL	F	156.4	D	33.8	F	>300	E	38.9
15	Neii Road	SB	EBL	F	107.3	С	16.3	F	>300	F	55.3
16	N Hamilton	NB	WBL	F	>300	D	33.1	F	>300	D	28.5
16	N Hamilton	SB	EBL	F	>300	Α	0	F	>300	E	47.6
17	Giffin Park Dr	NB	WBL	F	125.3	E	41.3	D	33.7	D	27
18	NW 28th St	NB	WBL	С	19.4	В	11.1	C	20.5	В	10.9
19	N Navidad Street	NB	WBL	D	30.6	A	0.0	C	19.8	A	0.0
19	in inavidad Street	SB	EBL	С	15.6	A	0.0	D	25.9	A	0.0

Similarly, as shown in Table 10, thirteen stop control movements are operating at LOS F during the PM Peak. Six of the major Street Left turn movements are operating at unacceptable levels of service (LOS F) with delays at some movements are more than 300 seconds per vehicle.

ISSUES AND OPPORTUNITIES

 $WSP\ noted\ locations\ along\ the\ Culebra\ Road\ corridor\ where\ challenges\ and\ issues\ were\ evident\ as$ well as locations where potential opportunities are present. Below is a brief summary of those items noted.

Generally, the issues noted below focused on safety and access for pedestrians and bicyclists. Motorists were also considered, but the primary focus is on the more vulnerable users of the

 The presence of a center two-way left-turn lane through most of the corridor adds to the $roadway\ width\ that\ pedestrians\ must\ navigate\ when\ crossing\ and\ allows\ for\ left-turn$ movements to be conducted from both directions, into and out of side streets and driveways, with no traffic control in place. Left-turns into side streets and driveways present opportunities for pedestrian and bicyclist conflicts, resulting in crashes.



- Daily traffic volumes are between 35,000 to 40,000 vehicles per day (vpd) along the corridor with peak hour volumes highest in the PM peak period, ranging from 2,500 to 3,700 vehicles per hour. The greatest volumes during the peak hour were recorded in the area around Potranco Road. Reallocating ROW from travel lanes to other uses may increase congestion and delay.
- Sections of the Culebra Road have residential properties fronting the corridor with driveways intersecting the roadway which may present a challenge when developing multimodal improvements.
- Dense driveway spacing is present in areas where small commercial parcels are closely spaced. This creates conflicts with pedestrians, bicyclists and motorists. Driveway reduction and consolidation should be considered.
- . Existing bike lanes are present on Culebra Road, between IH-410 and Tezel/Grissom Road. Bike facilities should be incorporated into the corridor further with connections to greenway trails and other bike facilities.
- The 5 feet bike lane is designated with an 8" solid white line, signing and the bike symbol.
 The most desirable bike facilities provide barrier separation from the traffic lanes or are off-roadway or are elevated, behind a curb. Most bicyclists do not feel safe riding close to high volumes of traffic traveling at 40 mph or higher.
- The corridor has gaps in sidewalks, lacks a buffer or separation from the adjacent traffic lane and lacks adequate lighting.
- Segments of the corridor include longtime neighborhood businesses with driveway and parking configurations that would not meet the City's current standards and will present challenges when developing improvements.
- A number of schools are located along Culebra Road or along intersecting cross-streets.
 Consideration must be given for school pick-up and drop-off operations, pedestrian and bicycle access when developing improvements
- . Numerous bridges and drainage structures are located along Culebra Road or cross the corridor and could limit the options where modifications are proposed. Also, consideration must be given to impacting drainage when proposing improvements.
- The segment of Culebra Road that includes the Southwest Research Institute (SWRI) has no curbs, sidewalk or drainage system. Also, the barbed-wire topped fencing along the SWRI property does not present an appealing "face" to the community
- . The 90-degree turn at the intersection with Tezel Road/Grissom Road creates confusion distracts unfamiliar drivers and is not easy for drivers to navigate.
- The channelized right-turn lanes at the intersection with Potranco Road are not $pedestrian-friendly\ since\ they\ encourage\ high-speed\ turns\ without\ being\ required\ to\ stop$ for the traffic signal.



OPPORTUNITIES:

The following items represent opportunities to improve the environment for pedestrians, transit users and bicyclists as well as improving the sense of place for the community.

- A significant portion of the corridor is wide with 5 lanes or 7 lanes. This presents an
 opportunity to reduce the number of lanes and to reallocate space to other users.
- The center two-way left-turn lane provides an opportunity to introduce a median to limit left-turn movements and reduce conflicts for pedestrians and bicyclists. The median would offer refuge space for pedestrians crossing the roadway and would reduce the total crossing distance. Medians will also offer opportunities for landscaping to create a sense of place and to narrow the field of vision for drivers which encourages slower travel speeds.
- Segments of the corridor have vacant parcels which could present opportunities for improvements that include transit, bicycle and pedestrian amenities as well as possible "pocket parks".
- Several existing and proposed greenway trails (Alazan Creek, Leon Creek) cross Culebra Road or are located nearby (Culebra Creek) and present opportunities for connections to bicycle facilities, pedestrian walkways and transit service.
- Several intersection alignments are non-traditional with skewed angles. These may be
 opportunities to improve the safety for all users and consider roundabouts.
- The segment of Culebra with the SWRI property offers an opportunity to introduce bike and pedestrian facilities with few driveway and cross-street conflicts. Landscaping and other treatments can help soften the security fencing,

APPENDICES

Appendix A – Culebra Road Base map

Appendix B – 11x17 Exhibits

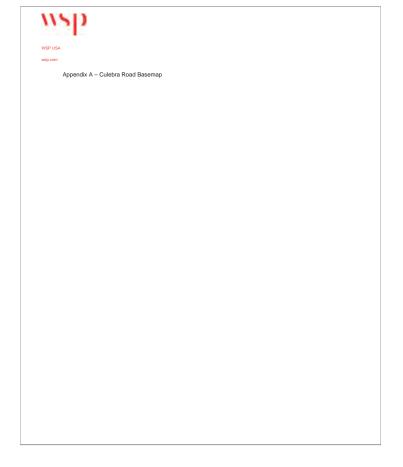
Appendix C - Traffic Projection Methodology

Appendix D - Tabulated Traffic Count Data - collected on December 15, 2020.

Appendix E - Crash Maps and Analyses Exhibits

Appendix F - Existing Condition Analyses Synchro Outputs

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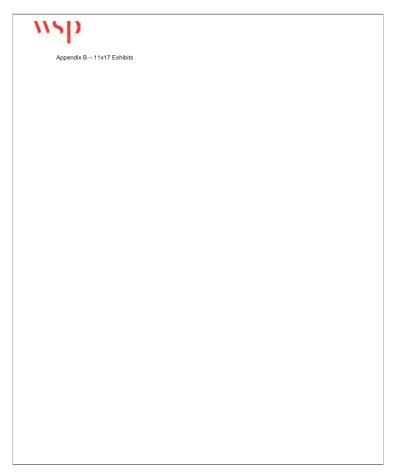


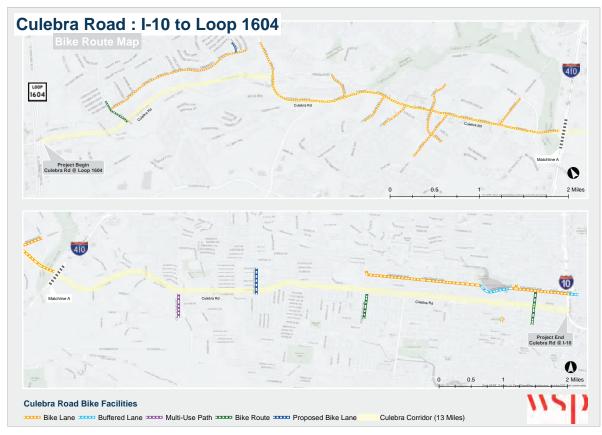


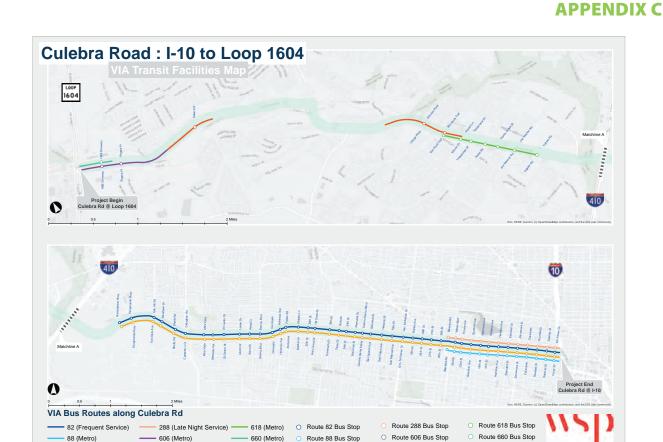


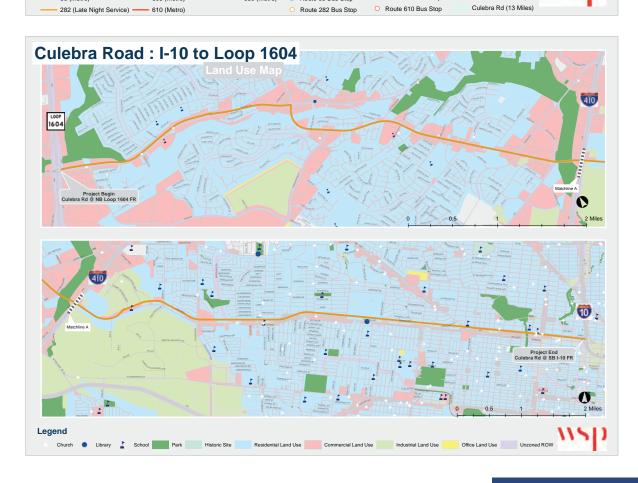












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Appendix C - Traffic Projection Methodology

TO: Bianca Thorpe, P.E., Capital Programs Manager

FROM: Kerri Collins, PE, PTOE, LEED AP; Michael Trueblood, PE, PTOE; Jemal Ali, PMP SUBJECT: Task 3: Culebra Road - Traffic Volume Projection Methodology Memo

DATE: February 2, 2021

This technical memorandum outlines our proposed methodology for the development of traffic projections as part of the Culebra Road Corridor Traffic Study located in San Antonio, Texas. Below is a map of the project showing the limits of the corridor at Loop 1604 and IH-10 (Figure 1).



Figure 1 – Study Area and Project Limits

The methodology was developed to forecast the existing traffic volumes to the future design year and to adjust 2020 traffic counts for pre-pandemic conditions. The projections will be used in the traffic analysis and evaluation of the proposed project options.

 $\label{thm:continuous} The \ traffic \ volumes \ will \ be \ developed \ to \ support \ analysis \ for \ the \ following \ years.$

- Base Year 2020; andFuture Analyses Year 2045

The following sources of data are used for the development of growth rates and adjustment factors:

- Traffic counts prior to 2020 provided from the City of San Antonio;
 Project collected traffic counts conducted in December 2020 during COVID-19 pandemic;

Historical count data - obtained from the TxDOT Statewide Traffic Analysis and Reporting System (STARS II) Database or the TxDOT Statewide Planning Map; and

Alamo Area Metropolitan Planning Organization (AAMPO) travel demand model (TDM) year 2017, 2025, 2035 and 2045 forecast traffic volumes

Note that the AAMPO travel demand model is used to develop traffic growth rates.

PROPOSED METHODOLOGY

The following steps were used for developing traffic projections. The first step of the methodology includes developing a COVID Factor (CF) to adjust the traffic volumes collected during the pandemic to better represent typical conditions by applying a growth rate to historical data to forecast normal 2020 $\,$ traffic patterns. The second step is to apply TDM-derived growth factors to the adjusted 2020 counts. The steps to develop traffic volumes and forecasts are as follows:

NORMALIZE 2020 BASE YEAR VOLUMES

Base year traffic volumes for the corridor will be normalized to pre-COVID conditions using a combination of counts collected in December 2020 adjusted with a COVID Factor using counts from previous years adjusted based upon a calculated growth rate to year 2020.

- $\textbf{1.} \quad \textbf{Pre-pandemic TMC Counts} \cdot \textbf{Apply the growth rate from the AAMPO model to traffic counts}$ conducted prior to 2020 to develop 2020 traffic volumes. Perform a reasonableness check.
- 2. Counts Collected During Pandemic the following process is used to develop a Covid Factor
 - Compare the adjusted 2020 pre-COVID TMC (developed in 1) with the similar count location of 2020 counts to develop a COVID Factor (four locations identified)
 - Compare 2020 projected STARS II count locations to 2020 ATR counts recorded at the same location to develop a COVID Factor (CF) and apply to the latest COVID 2020 counts
 - Compare and develop the most appropriate COVID Factor from the above two methods.
 - Conduct reasonableness check to make sure no turning movement volumes are increased where it is not reasonable, i.e., where development is not expected to increase, where roadway does not continue for a long distance, etc.

ANNUAL GROWTH FACTOR

The AAMPO TDM will be used to develop annual growth factors based on volume projections directly from the AAMPO travel demand forecasts for 2017, 2025, 2035 and 2045 models. The forecasts will be used to develop the annual growth factors for the corridor; a process that includes a review of the consistency and reasonableness of the growth trends from the model relative to historic trends.

2045 FUTURE VOLUMES

AAMPO TDM growth rates will be applied to the 2020 revised traffic volumes to develop the future 2045

VOLUME PROJECTION CALCULATION

For the purpose of developing annual growth rates and projected traffic volumes, , the study corridor was divided into six segments shown in Figure 2 and listed below based on the corridor characteristics such as land use, density, roadway configuration and connections with intersecting roads.

- Segment 1: Loop 1604 to Tezel/Grissom
- Segment 2: Tezel/Grissom to Micron Dr/Pipers Ln; Segment 3: Micron Dr/Pipers Ln to IH 410;
- Segment 4: IH 10 to Callaghan Road, Segment 5: Callaghan Road to Wilson Boulevard; and
- Segment 6: Wilson Boulevard to IH-10



Figure 2 - Corridor Segments

APPLICATION OF THE AAMPO GROWTH RATE

Traffic volume outputs for the 2017, 2025, 2035 and 2045 horizon years were extracted from the AAMPO TDM for the roadway links along the Culebra Road corridor study area. The growth rates were calculated for each specific link in the network and aggregated into six segments using a volume weighted average as shown in Table 1. Figure 3 shows a graph of how growth rates vary from link to link for different future year volumes. As shown in the table, the growth rate for each segment varies, thus the weighted average in the segment is used for each segment rate calculation. For the short-term future (2017 - 2025), the TDM annual growth rate for segment 2 and Segment 3 are 0.76%, -0.65%, respectively, which is very low. The 2017 TDM volumes for Segment 3 are higher compared with 2025 which results a negative growth rate, between Potranco Rd to IH 410 as an example. To be conservative, a 1.00% minimum growth $rate\ is\ assumed\ for\ all\ corridor\ segments.\ \textbf{Table\ 2}\ summarizes\ the\ average\ growth\ rates\ by\ segment.$

		Link from		Traffic Vo		Annual Growth Rates			
Segment	No.	LINK from	2017	2025	2035	2045	2017-2025	2025-2035	2035-204
	1	LP1604 SBFR	49603	61345	75053	87226	2.69%	2.04%	1.51%
_	2	LP1604 NBFR	36353	41435	48806	55812	1.65%	1.65%	1.35%
	3	Rogers Rd	36353	41435	48806	55812	1.65%	1.65%	1.35%
=	4	Mountain View	36353	41435	48806	55812	1.65%	1.65%	1.359
Segment 1	5	Les Harrison Dr	26012	29043	34301	40309	1.39%	1.68%	1.639
E	6	Cliffbrier Dr	25213	26978	31032	37009	0.85%	1.41%	1.789
Se	7	Nueces Canyon/Village Park	25213	26978	31032	37009	0.85%	1.41%	1.789
	8	Coppertree Blvd	25213	26978	31032	37009	0.85%	1.41%	1.789
	9	Walmart/Commercial Properties	25213	26978	31032	37009	0.85%	1.41%	1.78
	10	Camino Rosa	25213	26978	31032	37009	0.85%	1.41%	1.78
	11	Grissom Rd/Tezel Rd	42485	45225	50121	56540	0.78%	1.03%	1.21
	12	Arcadia Creek	42982	46184	53315	63389	0.90%	1.45%	1.75
	13	Westover Hills Blvd/Ensenada	37236	38683	46517	52216	0.48%	1.86%	1.16
2	14	Giffin Park Dr	37236	38683	46517	52216	0.48%	1.86%	1.16
ent	15	Ansley Bend Dr/Timber Path	37236	38683	46517	52216	0.48%	1.86%	1.16
Segment 2	16	Old Grissom Rd	46795	50162	61301	66050	0.87%	2.03%	0.75
S	17	Village Pkwy/Grissom Pass	46795	50162	61301	66050	0.87%	2.03%	0.75
	18	Grissom Gate	46795	50162	61301	66050	0.87%	2.03%	0.75
	19	Rim Rock Trail/Commercial Property	44503	47543	57417	60393	0.83%	1.90%	0.51
	20	Timber View Dr	44503	47543	57417	60393	0.83%	1.90%	0.51
	21	Micron Dr/Pipers Ln	41756	43711	53489	55628	0.57%	2.04%	0.39
	22	Reed Rd	48273	46511	55385	58881	-0.46%	1.76%	0.61
	23	Pipers Creek St (Both T Intersections)	49694	47654	56259	60038	-0.52%	1.67%	0.65
#	24	Joe Newton St	49694	47654	56259	60038	-0.52%	1.67%	0.65
a E	25	Ingram Rd	45122	43616	51456	55620	-0.42%	1.67%	0.78
Segment 3	26	Van Ness	45122	43616	51456	55620	-0.42%	1.67%	0.78
•,	27	Potranco Rd	58569	43878	51340	56858	-3.55%	1.58%	1.03
	28	IH410 SBFR	49765	48963	55791	60508	-0.20%	1.31%	0.81
	29	IH410 NBFR	27691	28960	32519	34984	0.56%	1.17%	0.73
	30	Commercial Properties/Baseball Field	22758	26555	29624	31877	1.95%	1.10%	0.74
Segment 4	31	Avenue G	22758	26555	29624	31877	1.95%	1.10%	0.74
mer	32	Alamo Downs Pkwy	26338	30353	33962	36119	1.79%	1.13%	0.62
- Be	33	Fairgrounds Pkwy	26208	30952	34128	36276	2.10%	0.98%	0.61
٠,	34	Oakhill-Tom Slick	25239	29954	32608	35144	2.16%	0.85%	0.75

	38 39	El Centro Benrus	24452 21876 21181	26914 24528 24261	29453 26921 26696	30819 28910 27921	1.21% 1.44% 1.71%	0.91% 0.94% 0.96%	0.45% 0.72% 0.45%
	39 40	Hortencia Ave	20820	23472	26026	26943	1.71%	1.04%	0.45%
	41	Alicia Ave	24027	25245	27456	29595	0.62%	0.84%	0.357
Ę	42	38th St	21904	22093	24153	27461	0.02%	0.90%	1.299
Segment 5	43	Esmeralda Dr/NW 36th St	24635	26871	29655	31431	1.09%	0.99%	0.589
Seg	43	NW 34th St	24635	26871	29655	31431	1.09%	0.99%	0.589
•,	44	Memorial St/Rattler Dr	23603	26308	28661	30520	1.09%	0.99%	0.58
		·							
	46	Camino Santa Maria	23135	25096	27185	29247	1.02%	0.80%	0.73
	47	N San Felipe Ave	26405	29801	32011	34475	1.52%	0.72%	0.74
	48	General McMullen Dr	23513	25246	28188	32246	0.89%	1.11%	1.35
	49	NW 28th St	23102	24835	27791	31769	0.91%	1.13%	1.35
	50	Bandera Rd	23102	24835	27791	31769	0.91%	1.13%	1.35
	51	Wilson	37260	43323	47019	54295	1.90%	0.82%	1.45
	52	Neff Rd	40055	46350	50387	57821	1.84%	0.84%	1.39
± 6	53	19th St	40587	46229	50543	57808	1.64%	0.90%	1.35
Segment 6	54	N Hamilton	40736	46702	51583	59297	1.72%	1.00%	1.40
89	55	Elmendorf	40736	46702	51583	59297	1.72%	1.00%	1.40
S	56	Zarzamora	47929	52997	58475	66872	1.26%	0.99%	1.35
	57	N Navidad Street	50165	55325	61166	69739	1.23%	1.01%	1.32
	58	IH10 EBFR	30014	32863	36324	41784	1.14%	1.01%	1.41
gment 1		LP 1604 to Grissom Rd/Tezel Rd	310739	349583	410932	480016	1.48%	1.63%	1.57
gment 2		Grissom Rd/Tezel Rd to Micron Dr/Pipers Ln	426566	453030	541724	595513	0.76%	1.80%	0.95
gment 3		Micron Dr/Pipers Ln to IH 410	415686	394563	463954	498175	-0.65%	1.63%	0.719
gment 4		IH 410 to Callaghan	123301	144369	159946	171293	1.99%	1.03%	0.69
gment 5		Callaghan to Wilson	372520	407818	448538	483835	1.14%	0.96%	0.769
egment 6		Wilson to IH 10	327482	370491	407080	466913	1.55%	0.95%	1.389

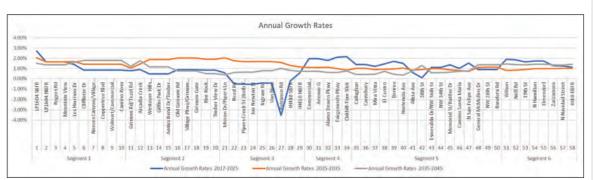


Figure 3 - Culebra Road Annual Growth Rates

Table 2: Annual Growth Rate Summary - from AAMPO TDM

Horizon Years	A	MPO TDM Growt	th Rate		Adjusted Growth R	late*
norizon tears	2017-2025	2025-2035	2035-2045	2017-2025	2025-2035	2035-2045
Segment 1	1.48%	1.63%	1.57%	1.48%	1.63%	1.57%
Segment 2	0.76%	1.80%	0.95%	1.00%	1.80%	1.00%
Segment 3	-0.65%	1.63%	0.71%	1.00%	1.63%	1.00%
Segment 4	1.99%	1.03%	0.69%	1.99%	1.03%	1.00%
Segment 5	1.14%	0.96%	0.76%	1.14%	1.00%	1.00%
Segment 6	1.55%	0.95%	1.38%	1.55%	1.00%	1.38%
Overall Corridor	0.88%	1.38%	1.03%	1.00%	1.38%	1.03%

^{*} Assuming minimum 1% annual growth rate.

Rates are calculated from the 2017, 2025, 2035 and 2045 TDM annual traffic volumes.



RECOMMENDED GROWTH RATE

The growth rates that will be used to project pre-COVID counts to 2020 analyses year and 2020 adjusted traffic volumes to 2045 are summarized in Table 3. The future 2020 to 2045 annual growth rate is calculated from the AAMPO TDM annual rates assuming a minimum of 1.00% growth rate between 2017 to 2045 shown in Table 2.

Table 3: Recommended Annual Growth Rates by Forecast Year

Locations	Annual Growth Rates							
Locations	2015 - 2020	2020 - 2045						
1 – Segment 1	1.50%	1.60%						
2 - Segment 2	1.00%	1.30%						
3 - Segment 3	1.00%	1.25%						
4 - Segment 4	2.00%	1.20%						
5 – Segment 5	1.15%	1.05%						
6 – Segment 6	1.55%	1.25%						

Corrections to traffic turn movement counts (TMCs) taken in 2020 during COVID 19 were developed using

- a. Pre-COVID Counts Apply annual growth rate developed in Table 3 to traffic counts
 conducted prior to 2020 [Pre-COVID] to develop adjusted 2020 [raffic volumes.
 b. COVID Factor via TMCs Compare adjusted 2020 [Pre-COVID] volumes with the actual
 2020 counts taken during COVID-19 to develop COVID Factor. The results of this evaluation at four locations are shown in Table 4. As shown, the TMC based COVID Factor varies between 1.23 and 1.39 along the corridor with an average value of 1.31.

Table 4: COVID Factor (CF) from Pre-COVID TMC Projections vs Actual Counts During COVID-19

	Pre-		Intersection	ı Volume	
TMC Counts Intersection Locations	COVID Count Year	Pre-COVID Count	2020 Adjusted Count	2020 Actual Count	COVID Factor (CF)
Culebra @ Grissom Rd/Tezel Rd	2015	19,100	20,600	14,800	1.39
Culebra @ Ingram Rd	2015	16,200	17,000	13,400	1.27
Culebra @ Callaghan	2017	16,100	16,500	12,300	1.34
Culebra @ Zarzamora	2017	14,300	15,000	12,200	1.23
Overall Ave	erage		69,100	52,700	1.31

Note: Volumes are sum of Intersection TMC from 7AM to 9AM and 4PM to 6PM



c. COVID Factor via ATRs - Compare the adjusted 2020 Pre-COVID TxDOT STARS II counts with a similar count location as actual 2020 COVID counts to develop COVID Factor. The results of this evaluation at three locations are shown in Table 5. As shown in Table 5, the ATR-based COVID Factor varies between 1.02 to 1.19 along the corridor with an average value of 1.10.

Table 5: COVID Factor (CF) from Pre-COVID ATR Projections vs Actual Counts During COVID-19

			ATR 24 HI	R Volumes	
ATR Counts Locations	STAR II Count Year	STARR II Total Volume	2020 Projected Volume	2020 Existing Volume	COVID Factor (CF)
ATR 1 - Between LP 1604 and Rogers Rd	2019	41,600	42,300	35,500	1.19
ATR 2 - Between Ingram Rd and Potranco Rd	2019	42,500	43,000	39,200	1.10
ATR 3 - Between N Hamilton Ave and Elmendorf Street	2019	35,400	35,900	35,100	1.02
Overall			121 200	109 800	1.10

There are significant differences between COVID Factor estimates derived by the above two approaches. It is our recommendation to use the COVID factor estimate from Pre COVID counts shown in **Table 4**, where the peak hour volumes are considered. This correction recognizes that the previous level of travel activity may never return due to the dramatic reduction in the use of commercial office and retail space in the economy.

- d. Develop Adjusted 2020 Traffic Volumes The 2020 base year traffic volumes for the study were calculated by applying the annual growth rates to Pre-COVID AM and PM peak hour traffic counts, or by applying the COVID Factor (CF) to the 2020 COVID, AM and PM peak hour traffic counts
- Step 4: Develop Future Traffic Volumes The future projected traffic volumes are calculated by applying the annual growth rates to adjusted 2020 daily, AM and PM peak hour volumes.

Based on available data and analyses results pertaining to corrections to counts for the COVID-19 $\,$ pandemic and future growth rates, it is recommended that:

- 1. traffic counts taken in 2020 should be factored by applying a COVID Factor (CF) of 1.30 to account the impact of COVID pandemic on those co
- 2. the growth rates shown in Table 3 be applied to the 2020 base year traffic volumes to generate the proposed future traffic volumes for the 2045 future analysis year.

wsp	
Appendix D – Tabulated Traffic Count Data – collected on December 15, 2020.	

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	Data Collected: December 15, 2020	
		1/27/2021
1/j30900309A CoSA Culebra Road Corridor Study/S. 0 Project Data/S. 1 Data Collection/S. 1.3 Data Tabulation/j3090	J0099A CoSA Culebra Road, AM, Rev1	1/2//2021

Project Number: 30900399A CoSA Culebra Road Corridor Study TX 1604 to I-10

AM TURNING MOVEMENT COUNTS

AM Turning Movement Counts

Data Collected: December 15, 2020

-1,109003999A CaSA Cullebra Road Corridor Study/S. D Project Data/S. 1 Data Collection/S. 1.3 Data Tabulation/3,0900399A CaSA Cullebra Road_AM_Rev1

1/27/2021

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Peak Hour Factor 0.250	2-hour Total	9	7	24	0	312	1312	8	0	122	13	152	0	61	1676	365	0	
Heavy Wehcle Percentage	7:15 AM - 8:15 AM*		0		0				0				0		961	204	0	
OnStreet Bicycle Count																		
C115 AM 16 8:15 AM Peak Hour represents the corridor peak not the intersection peak. T			0.00%	0.00%		1.92%	2.90%	0.00%		10.66%	0.00%	1.97%		1.64%	2.09%	3.29%		
** 7.15 AM - 8.15 AM Peak Hour represents the corridor peak not the intersection peak. R8FCU Driveway Culebra Rd Rogers Rd Culebra Rd Southbound Westbound Southbound Sout			0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
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8:00 AM to 9:00 AM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0			Bicyc	le Counts -	On Sidewal	k							961	→				
** SB/W - Pedestrian/cycle travelling southbound crossing eastbound approach (West leg) Diagram for: AM Peak Hour	7:00 AM to 8:00 AM	0	0	0	0	0	0	0	0				204	+				
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	** SB/W - Pedestrian/cycle tra	velling sout	hbound cro	ossing eastb	ound appro	oach (West I	eg)											

1/27/2021

The contract of the contract	North/South Street: East/West Street:					Les Harrison Culebra Re							Site Coo	de: Sig_2				
Les Harrison Dr SouthBound		AM	Date:	12/15/202	0			ollection\5.1.2	Raw Count D	ata\From GRA	M\12 30 202	0\Extracted T	MC\WSP - 12	-03713 - Culeb	ora Rd - TMCs			
Time			Les Har	rison Dr														
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** 7.15 AM - 8.15 AM Peak Hour represents the corridor peak not the intersection peak. Les Harrison Dr	OnStreet Bicycle Count				0								0				0	
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7:00 AM to 8:00 AM 0 0 1 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Time	SB/W**				NB/W	NB/E	EB/N	EB/S									
8:00 AM to 9:00 AM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	7-00 484 0-00 484	0					0				+	+						
Bicycle Counts - On Sidewalk 7:00 AM to 8:00 AM 0															U	54	9	29
8:00 AM to 9:00 AM 0 0 0 0 0 0 0 0 0 0 Diagram for: ** SB/W - Pedestrian/cycle travelling southbound crossing eastbound approach (West leg) Diagram for: AM Peak Hour	0.00 AW 10 3.00 AW																	
** SB/W - Pedestrian/cycle travelling southbound crossing eastbound approach (West leg) Diagram for: AM Peak Hour	7:00 AM to 8:00 AM	0	0	0	0	0	0	0	0				67	1				
								0	0						•			
Peak Hour: 7:15 AM - 8:15 AM*	** SB/W - Pedestrian/cycle tra	velling sout	thbound cro	ossing eastl	ound appr	oach (West I	eg)											
												Peak Hour	:	7:15 AM -	8:15 AM*			

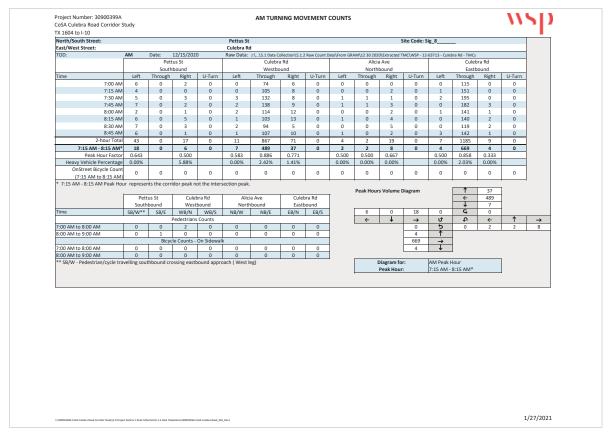
North/South Street:					Cliffbrier D	ır						Site Code: S	ig_3	_			
East/West Street:					Culebra Ro												
TOD:	AM		12/15/202	0	Raw Data:	J:\\5.1 Data C		Raw Count D	ata\From GR	AM\12 30 202	0\Extracted 1	MC\WSP - 12-	03713 - Cule				
			rier Dr			Culebr									ra Rd		
	. 6		bound		. 6	Westbo				North					ound		
Time	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	
7:00 AM 7:15 AM		0	13 22	0	0	121 150	8 19	0	0	0	0	0	9	217 222	0	0	
7:30 AM		0	28	0	0	147	16	0	0	0	0	0	18	301	0	0	
7:45 AM		0	22	0	0	172	25	0	0	0	0	0	18	291	0	0	
8:00 AM		0	31	0	0	154	18	0	0	0	0	0	23	204	0	0	
8:15 AM		0	35	0	0	152	25	0	0	0	0	0	22	167	0	0	
8:30 AM		0	24	0	0	149	19	0	0	0	0	0	16	189	0	0	
8:45 AM		0	20	0	0	166	18	0	0	0	0	0	9	176	0	0	
2-hour Total		0	195	0	0	1211	148	0	0	0	0	0	124	1767	0	0	
7:15 AM - 8:15 AM* Peak Hour Factor		0	103 0.831	0	0	623 0.906	78 0.780	0	0	0	0	0	68 0.739	1018 0.846	0	0	
Heavy Vehicle Percentage			2.05%			2.81%	2.70%						1.61%	2.09%			
OnStreet Bicycle Count																	
(7:15 AM to 8:15 AM)		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Time	SB/W**	SB/E	WB/N	WB/S	NB/W	NB/E	EB/N	EB/S		103	0	195	0	4	0		
			Pedestrian:	Counts						+	↓	→	Ů.	Ð	+	1	\rightarrow
7:00 AM to 8:00 AM 8:00 AM to 9:00 AM	0	0	0	0	0	0	0	0				68	5	0	0	0	0
8.00 AWI to 3.00 AWI				On Sidewal								1018	→	1			
7:00 AM to 8:00 AM	0	0	0	0	0	0	0	0				0	Ţ	1			
8:00 AM to 9:00 AM	0	0	0	0	0	0	0	0									
** SB/W - Pedestrian/cycle tra	velling sout	thbound cro	ssing eastl	ound appro	oach (West I	eg)					Diagram fo		AM Peak I				
											Peak Hour	:	7:15 AM -	8:15 AM*			

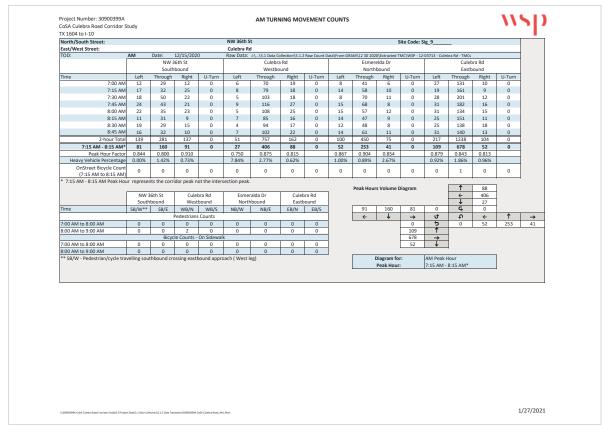
Project Number: 30900399A CoSA Culebra Road Corridor Study TX 1604 to I-10 AM TURNING MOVEMENT COUNTS WSD Village Park North/South Street: East/West Street: Site Code: Sig 4 e: 12/15/202 Village Park Southbound Southbound Through Right U-Turn 0 17 0 0 26 0 4 25 0 4 24 0 2 20 0 0 27 0 0 22 0 1 23 0 Westb Through 106 139 148 158 143 154 143 Through Right U-Turn 7:00 AM 7:15 AM 7:30 AM 7:45 AM 8:00 AM 8:15 AM 19 18 19 19 19 7 1 3 4 2 0 0 0 0 1 12 9 244 249 341 319 225 208 208 27 34 35 32 23 19 23 2 0 0 0 0 7 17 10 12 1 0 0 0 0 8:45 AM 143 1134 39 0 7 10 14 0.574 0.438 0.625 0.700 0.00% 0.00% 6.25% 9.09% 3.85% 7:15 AM - 8:15 AM* 124 **95** 0.913 588 63 1134 0.829 0.831 0.930 3.17% Heavy Vehicle Percentage 1.42% 21.43% 4.31% 1.62% OnStreet Bicycle Count (7:15 AM to 8:15 AM) 7:15 AM - 8:15 AM Peak Hou 0 0 0 0 0 0 0 0 0 0 0 0 0 0 ↑ 39 ← 588 ↓ 5 G 0 Village Park Southbound Culebra Rd Westbound Nueces Canyon Northbound Culebra Rd Eastbound SB/W*** S8/E WB/N WB/S NB/W NB/E EB/N EB/S 0 63 1134 3 Λ ← ↑ → 0 7 10 14 AM Peak Hour 7:15 AM - 8:15 AM*

East/West Street: Culebra Rd Date: 12/15/72020 Raw Date: 14: 14: 10: 20 (alcelorabit): 1.2 have Count Deal/Years (distance): 12 have Count Deal/Years (North/South Street:				W	/almart Drive						9	ite Code: S	ig_5	_			
Welshound	East/West Street:																	
Southbound	TOD:	AM			0	Raw Data:			Raw Count D	ata\From GR				03713 - Culei				
Time																		
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7:15 AM = 8:15 AM			1	44	0	18	1174	73	0	15	6	42	0	50	1930	46	0	
Heavy Vehicle Percentage 6.45% 0.00% 4.55% 0.00% 3.24% 5.48% 6.67% 0.00% 0.00% 0.00% 1.92% 0.00%	7:15 AM - 8:15 AM*		0		0				0								0	
On Street Bicycle Count O O O O O O O O O				0.893		0.563	0.942	0.750		0.667	0.250	0.625		0.682	0.947	0.714		
C7:15 AM to 8:15 AM Peak Hour represents the corridor peak not the intersection peak. Time	Heavy Vehicle Percentage	6.45%	0.00%	4.55%		0.00%	3.24%	5.48%		6.67%	0.00%	0.00%		0.00%	1.92%	0.00%		
(7:15 AM to 8:15 AM) * 7:15 AM - 8:15 AM Peak Hour represents the corridor peak not the intersection peak. Walmart Driveway Culebra Rd Westbound Northbound Eastbound Eastbound Sa/W** Sa/E Walmart Driveway Sa/W** Sa/E Walmart Driveway Northbound Eastbound Eastbound Culebra Rd Sa/W** Sa/E Walmart Driveway Northbound Eastbound Eastbound Culebra Rd Sa/W** Sa/E Walmart Driveway Northbound Eastbound Eastbound Culebra Rd Sa/W** Sa/E Walmart Driveway Northbound Eastbound Eastbound Culebra Rd Sa/W** Sa/W** Sa/W** Sa/W** Walmart Driveway Northbound Eastbound Eastbound Culebra Rd Sa/W** Sa/W** Sa/W** Walmart Driveway Northbound Eastbound Eastbound Eastbound Culebra Rd Sa/W** Sa/W** Walmart Driveway Northbound Eastbound Eastbound Culebra Rd Culebra Rd			0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Walmart Driveway Culebra Rd Salz Station Driveway Southbound Northbound Northbound Eastbound Eastbound Salz Station Driveway Culebra Rd Culebra																		
Walmart Driveway Culebra Rd Southbound Southbound	* 7:15 AM - 8:15 AM Peak Ho	ur represer	its the corri	dor peak n	ot the inter	section peak.					B 1 - 11				*	20	1	
Southbound Westbound Northbound Eastbound Eastbound Self		Molecost	Deimonnen	Culai	ara Del	Can Statio	na Daireannan	Culai	ara Del		reak Hour	s volume i	Jiagram		- 1		-	
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7:00 AM to 8:00 AM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		30/11				110/11	140/2	LUJII	20/3								1	→
Bicycle Counts - On Sidewalk 1045 → 20 1045 → 2	7:00 AM to 8:00 AM	0	0	0	0	0	0	0	2		,						1	
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8:00 AM to 9:00 AM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0			Bicyc	le Counts -	On Sidewal	k							1045					
** SB/W - Pedestrian/cycle travelling southbound crossing eastbound approach (West leg) Diagram for: AM Peak Hour													20	+				
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Peak Hour: /35 AM- 8:35 AM*	** SB/W - Pedestrian/cycle tra	velling sout	thbound cro	ssing easth	ound appr	oach (West l	eg)											
												Peak Hour	:	7:15 AM -	8:15 AM*		1	

Project Number: 30900399A CoSA Culebra Road Corridor Study TX 1604 to I-10 North/South Street: East/West Street: TOD: AM wsp AM TURNING MOVEMENT COUNTS Culebra Rd Site Code: Sig 6 Arcadia Creek Eastbound Tough Right U-Turn 0 30 0 0 25 0 0 23 0 0 31 0 0 7 0 0 22 0 0 22 0 0 22 0 0 16 0 0 176 0 7:00 AM 7:15 AM 7:30 AM 7:45 AM 8:00 AM 8:15 AM 8:30 AM 8:45 AM 109 146 141 170 140 138 139 150 236 284 323 323 272 192 197 216 27 13 18 20 17 25 19 14 9 11 10 11 149 68 0.850 4.03% 1202 36 0 0.930 0.643 1.66% 1.39% 0.00% 22 597 0.611 0.878 18.42% 2.47% 86 0.694 1.70% 7:15 AM - 8:15 AM* 0 Peak Hour Factor Heavy Vehicle Percentage OnStreet Bicycle Count (7:15 AM to 8:15 AM) 7:15 AM - 8:15 AM Peak Hour 0 0 0 0 0 0 Culebra Rd Southbound Westbound Culebra Rd Northbound Arcadia Creek SB/W** SB/E WB/N WB/S NB/W NB/E EB/N EB/S Ð 7:00 AM to 8:00 AM 8:00 AM to 9:00 AM 0 86 **→** AM Peak Hour 7:15 AM - 8:15 AM* Diagram for: Peak Hour:

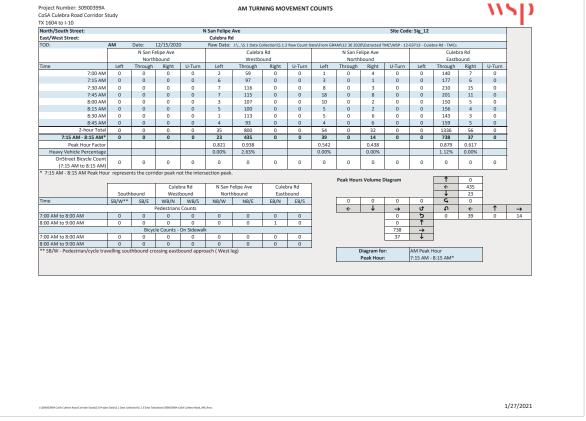
North/South Street: Start/Was Street Start/Wa	Project Number: 30900399A CoSA Culebra Road Corridor 5 TX 1604 to I-10	Study					AM TUR	NING MO	VEMENT	COUNTS							11	N
AM Date: 12/15/2020 Raw Oata: IA-\LS-1 Desc Clietton RGL Raw Count Deal/From GRAM\12 82000/Earneted TMC\WSP-12-01713 - Cueltor and Earthound Cueltor and Carbon Morthbound Cuelto					Base	eball Field D	riveway						Site Code: S	ig_7	_			
Baseball Field Driveway Culebra Rd Driveway Southbound Fastbound F						Culebra R	d								_			
Southbound	TOD:					Raw Data:	J:\\5.1 Data C	ollection\5.1.2	Raw Count D	ata\From GR	AM\12 30 202	0\Extracted T	MC\WSP - 12-	03713 - Culei				
Time					У													
7-00 AM 0 0 3 0 0 106 0 0 1 0 1 0 0 235 0 0																		
T-15 AM																		
7:30 AM 0 0 2 0 0 146 0 0 1 0 0 0 0 299 1 0																		
7.45 AM																		
8.00 AM 2																		
B.15 AM																		
8.30 AM 1 0 3 0 0 113 9 0 1 0 0 0 25 167 1 0																		
8.85 AM 5																		
2-hour Total 11																		
7.15 AM - 8.15 AM 5 0			_															1
Peak Hour Fator																		1
Heavy Vehicle Percentage 27.27% 24.00% 0.00% 4.24% 88.89% 75.00% 0.00% 90.91% 1.50% 0.00%																		
OnStreet Bicycle Count O																		1
** 7.15 AM Peak Hour represents the corridor peak not the intersection peak. Baseball Field		0	0	1	0	0	0	1	0	0	0	0	0	0	1	1	0	
Pedestrians Counts 7:00 AM to 8:00 AM		Baseb Driv	all Field eway	Culet	ora Rd bound	Dri Nort	veway hbound	Eastl	ound						1	514 1		
7:00 AM to 8:00 AM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Time	SB/W**				NB/W	NB/E	EB/N	EB/S								*	
800 AM to 9:00 AM	7-00 484 +0 8-00 484	0				0	0	0	1		_	•						
Bicycle Counts - On Sidewalk 1092															- U			-
7:00 AM to 8:00 AM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0							-	-	-					- -	i			
** SB/W - Pedestrian/cycle travelling southbound crossing eastbound approach (West leg) Diagram for: AM Peak Hour	7:00 AM to 8:00 AM	0					0	0	0						i			
	8:00 AM to 9:00 AM	0	0	0	0	0	0	0	0						,			
												Peak Hour	:	7:15 AM -	8:15 AM*			





Project Number: 30900399A CoSA Culebra Road Corridor Study TX 1604 to I-10 AM TURNING MOVEMENT COUNTS WSD North/South Street: East/West Street: Rattler Dr Site Code: Sig 10 Rattler Dr Southbound Memorial St Northbound Through 90 102 Through Right U-Turn Through Right U-Turn 7:00 AM 7:15 AM 7:30 AM 7:45 AM 8:00 AM 0 0 0 0 0 0 0 0 0 0 0 0 4 6 7 145 184 231 204 172 171 142 0 0 0 0 1 0 126 134 127 0 1 11 0 0 0 0 6 0 0 5 11 8:15 AM 8:30 AM 96 121 8:45 AM 489 0.912 2.87% 2 791 29 0.250 0.856 0.659 0.00% 1.76% 5.45% 7:15 AM - 8:15 AM* **32** 0.727 **4** 0.500 **21** 0.656 Heavy Vehicle Percentage 1.92% 3.85% 0.00% 0.00% OnStreet Bicycle Count (7:15 AM to 8:15 AM) 7:15 AM - 8:15 AM Peak Hou 0 0 0 0 0 0 0 0 0 0 0 0 0 0 4 489 32 0 ← 489 ↓ 32 **Ç** 0 **D** ← ↑ → 1 0 7 1 21 791 → 29 ↓ AM Peak Hour 7:15 AM - 8:15 AM*

TX 1604 to I-10																	
North/South Street:				Ca	imino Santa						S	ite Code: Si	g_11	_			
East/West Street:					Culebra R												
TOD:	AM		12/15/202	U	Raw Data:	J:\\5.1 Data C		Raw Count D	ata\From GR	AM\12 30 202	!0\Extracted T	MC\WSP - 12-	03713 - Cule				
			anta Maria ibound			Culebr				Manth	bound				ora Rd oound		
Time	Left		Right	U-Turn	Left	Through	Right	U-Turn	Left		Right	U-Turn	Left	Through	Right	U-Turn	
7:00 AM		Through	Right 8	U-Turn O	Leπ 0	1nrougn 82	Kight 5	0-Turn 0	υ 0	Through 0	Kight 0	0-Turn 0	Lett 9	140	Right	0-Turn 0	
7:15 AM		0	8	0	0	102	11	0	0	0	0	0	11	180	0	0	
7:30 AM		0	9	0	0	127	14	0	0	0	0	0	17	219	0	0	
7:45 AM		0	6	0	0	138	7	0	0	0	0	0	21	192	0	0	
8:00 AM		0	19	0	0	119	6	0	0	0	0	0	20	157	0	0	
8:15 AM		0	9	0	0	103	10	0	0	0	0	0	22	156	0	0	
8:30 AM		0	15	0	0	114	2	0	0	0	0	0	20	138	0	0	
8:45 AM		0	16	0	0	99	8	0	0	0	0	0	19	158	0	0	
2-hour Total		0	90	0	0	884	63	0	0	0	0	0	139	1340	0	0	
7:15 AM - 8:15 AM*		0	42	0	0	486	38	0	0	0	0	0	69	748	0	0	
Peak Hour Factor			0.553			0.880	0.679						0.821	0.854			
Heavy Vehicle Percentage OnStreet Bicycle Count			4.44%			2.94%	0.00%						0.72%	2.24%			
(7:15 AM to 8:15 AM)		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Time	South SB/W**		WB/N	WB/S	Norti NB/W	hbound NB/E	Eastl EB/N	EB/S		42	0	23	0	ç,	0		
7:00 AM to 8:00 AM	0	0	Pedestrians 0	Counts	0	0	0	0		+	1	→ 0	<u>t</u>	0 V	0	0	→
8:00 AM to 9:00 AM	1	0	0	0	0	0	0	0				69		- 0		0	U
				On Sidewal			-					748	<u>·</u>				
7:00 AM to 8:00 AM	0	0	0	0	0	0	0	0				0	Ţ				
8:00 AM to 9:00 AM	0	0	0	0	0	0	0	0									
											Peak Hour			8:15 AM*			



North/South Street: General McCullen Dr	n	0	und	Culeb								lon Dr	eneral McCull	Go				North/South Street:
Time	'n	0	und	Culeb		Site Cou								Gei				
Southbound	'n	0	und		03713 - Culeb	MC\WSP - 12-	\Extracted TI	AM\12 30 2020	hata\From GRA	Raw Count D	ollection\5.1.2			0	12/15/2020	Date:	AM	
Time	rn	0					cCullen Dr	General M			Rd	Culebra			1cCullen Dr	General M		
7:00 AM	rn_	0	Right															
7:15 AM 6 6 52 3 3 0 15 66 9 0 31 61 32 0 10 136 32 0 7:30 AM 8 55 5 0 26 83 6 0 34 109 46 0 12 176 22 0 7:45 AM 4 48 8 0 22 85 7 0 25 123 45 0 20 171 36 0 8:00 AM 10 49 5 0 32 80 5 0 32 78 35 0 12 114 31 0 8:15 AM 6 61 7 0 20 73 10 0 23 71 25 0 15 122 24 0 8:30 AM 8 53 6 0 21 71 6 0 32 82 82 82 0 10 112 24 0 8:30 AM 8 53 6 0 21 71 6 0 32 82 82 82 0 10 112 26 0 8:45 AM 7 58 10 0 28 65 11 0 27 81 31 0 10 112 26 0 8:45 AM 7 58 10 0 28 65 11 0 27 81 31 0 10 130 33 0 12 114 83 1 0 10 12 8 12 8 12 8 10 0 12 8 12 8 10 10 12 8 12 8																		
7:30 M 8 55 5 0 26 83 6 0 34 109 46 0 12 176 22 0 7:45 AM 4 48 8 0 22 85 7 0 25 85 7 0 25 85 7 0 25 85 7 0 25 85 7 0 25 85 7 0 25 85 7 0 25 85 7 0 25 85 7 0 25 85 7 0 25 85 7 0 25 85 7 0 25 85 7 0 25 85 7 0 25 85 7 0 25 85 7 0 25 85 85 9 0 22 174 36 0 20 171 36 0 0 23 815 AM 6 61 7 0 0 20 73 10 0 23 71 25 0 15 122 24 0 0 8:30 AM 8 53 6 0 21 71 71 6 0 32 82 82 28 0 10 112 26 0 0 8:45 AM 7 58 10 0 28 65 11 0 0 22 865 11 0 0 22 8 65 11 0 0 22 8 65 11 0 0 22 8 65 11 0 0 22 8 65 11 0 0 22 8 65 12 0 10 10 10 10 130 33 0 0 2 8 10 10 10 10 10 10 10 10 10 10 10 10 10																		
7-45 AM																		
8.00 M 10 49 5 0 32 80 5 0 32 78 35 0 12 114 31 0																		
8:15 AM 6 6 61 7 0 20 73 10 0 23 71 25 0 15 122 24 0 8:30 M 8 53 6 0 21 71 6 0 32 82 28 0 10 112 26 0 8:45 AM 7 58 10 0 28 65 11 0 27 81 31 0 10 130 33 0 2 2 2 8 0 10 112 26 0 0 2 2 8 65 11 0 27 81 31 0 10 10 130 33 0 2 8 2 2 8 0 10 10 112 26 0 0 10 10 10 10 10 10 10 10 10 10 10 10																		
8.30 M 8 53 6 0 21 71 6 0 32 82 28 0 10 112 26 0 8.45 AM 7 58 10 0 28 65 11 0 27 81 31 0 10 130 33 0 2-hour Total 53 413 48 0 179 578 59 0 228 660 271 0 100 1064 233 0 7.15 AM - 8.15 AM* 28 204 21 0 95 314 27 0 122 371 158 0 54 597 121 0 Peak Hour Factor 0.700 0.927 0.656 0.742 0.924 0.750 0.897 0.754 0.859 0.675 0.848 0.840 Heavy Wehicle Percentage 0.00% 4.60% 10.42% 1.65% 2.77% 0.00% 2.63% 3.48% 0.74% 0.00% 1.79% 0.86% OnStreet Bive/Le Count 0 0 0 0 0 0 0 0 0																		
2-hour Total 53 413 48 0 179 578 59 0 228 660 271 0 100 1064 233 0 7.15 AM - 8.15 AM* 28 204 21 0 95 314 27 0 122 371 158 0 54 597 121 0 Peak Hour Factor 0 700 0.927 0.656 0.742 0.924 0.750 0.897 0.754 0.899 0.675 0.848 0.840 Heavy Wehicle Percentage 0.00% 4.00% 10.42% 1.68% 2.77% 0.00% 2.63% 3.48% 0.74% 0.00% 1.79% 0.86% OnStreet Bive/Je Count 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0																		
7:15 AM - 8:15 AM* Pask Hour represents the corridor peak not the intersection peak. Pask Hour Factor 0.700 0.327 0.556 0.742 0.954 0.750 0.897 0.754 0.899 0.675 0.848 0.840 Heavy Wehick Percentage 0.00% 4.60% 10.42% 1.68% 2.77% 0.00% 2.63% 3.48% 0.74% 0.00% 1.79% 0.86% OnStreet Bicycle Count 0 0 0 0 0 0 0 0 0		0	33	130	10	0	31	81	27	0	11	65	28	0	10	58	7	8:45 AM
Peak Hour Factor 0.700 0.927 0.656 0.742 0.924 0.750 0.897 0.754 0.859 0.675 0.848 0.840	_	0	233	1064	100	0	271	660	228	0	59	578	179	0	48	413	53	2-hour Total
Heavy Vehicle Percentage		0	121	597	54	0	158	371	122	0	27	314	95	0	21	204	28	7:15 AM - 8:15 AM*
OnStreet Bicycle Count O O O O O O O O O																		
7:15 AM to 8:15 AM) Peak Hour represents the corridor peak not the intersection peak. Peak Hours Volume Diagram 1 27			0.86%	1.79%	0.00%		0.74%	3.48%	2.63%		0.00%	2.77%	1.68%		10.42%	4.60%	0.00%	
Peak Hours Volume Diagram 27 General McMullen Culebra Rd General McMullen Dr Culebra Rd ← 314		0	0	0	0	0	0	0	0	0	0		1					(7:15 AM to 8:15 AM)
General McMullen				_								÷	section peak.	ot the inters	idor peak no	nts the corri	ur represei	* 7:15 AM - 8:15 AM Peak Hor
				T		Diagram	s Volume D	Peak Hour	T	D.I	6.1.1	4-14-II D-		no Del	6.1.1			
Time SS/W** SB/E WB/S NB/W NB/E EB/N EB/S 21 204 28 0 G 0					0	28	204	21	1									Time
Pedestrians Counts	→	1				_			1	20/3	LUJII	140/2	140/44				30/11	
7:00 AM to 8:00 AM 0 3 0 0 2 0 1 0 0 5 0 122 371		371								0	1	0	2				0	7:00 AM to 8:00 AM
8:00 AM to 9:00 AM 0 1 0 3 0 2 1 0 54 ↑					1	54				0	1	2	0	3	0	1	0	8:00 AM to 9:00 AM
Bicycle Counts - On Sidewalk 597 →									and the same of th					On Sidewall	le Counts -			
7:00 AM to 8:00 AM 0 0 0 0 0 0 0 0 0 121					<u> </u>	121												
8.00 AM									J	0	0							
Peak Hour: 7:15 AM - 8:15 AM *				3:15 AM*	7:15 AM -		Peak Hour:											

North/South Street: East/West Street:					Bandera Ro Culebra Ro							Site Code:	SIG_14 AIV	п			
TOD:	AM	Date:	12/15/202	0		J:\\5.1 Data G	nllection\\$ 1	Raw Count D	ata\From GR	AM\12 30 202	0\Extracted 1	MC\WSP - 12-	03713 - Cule	hra Rd - TMCs			
			era Rd	-		Culebra				,	. (Culeb	ıra Rd		
		South	bound			Westbo	ound			North	bound			Eastb	ound		
Time	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	
7:00 AM	100	0	1	0	0	71	61	0	0	0	0	0	0	128	0	0	
7:15 AM	97	0	1	0	0	84	64	0	0	0	0	0	0	170	0	0	
7:30 AM	116	0	0	0	0	111	104	0	0	0	0	0	0	201	0	0	
7:45 AM		0	0	0	0	99	115	0	0	0	0	0	0	230	0	0	
8:00 AM 8:15 AM	96 93	0	4	0	0	113 109	98 102	0	0	0	0	0	0	141 164	0	0	
8:15 AM 8:30 AM		0	0	0	0	91	73	0	0	0	0	0	0	136	0	0	
8:30 AM 8:45 AM	87	0	2	0	0	106	88	0	0	0	0	0	0	166	0	0	
2-hour Total		0	9	0	0	784	705	0	0	0	0	0	0	1336	0	0	
7:15 AM - 8:15 AM*		0	5	0	0	407	381	0	0	0	0	0	0	742	0	0	
Peak Hour Factor			0.313			0.900	0.828	-				-		0.807			
Heavy Vehicle Percentage	2.91%		0.00%			2.30%	3.26%							1.50%			
OnStreet Bicycle Count (7:15 AM to 8:15 AM)		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
* 7:15 AM - 8:15 AM Peak Ho	Bande	era Rd	Cule	bra Rd bound	1	bound		ora Rd oound		Peak Hour	s Volume I	Diagram		↑ ← ↓	381 407 0		
Time	SB/W**	SB/E	WB/N	WB/S	NB/W	NB/E	EB/N	EB/S		5	0	436	0	G,	0		
			Pedestrian:							+	1	→	ď	Ð	+	1	→
7:00 AM to 8:00 AM 8:00 AM to 9:00 AM	0	0	0	0	0	0	0	0				0	5	0	0	0	0
8:00 AM to 9:00 AM	U		-	On Sidewal		U	U	U				742		+			
	0	0	0	0 Oil Sidewal	0	0	0	0				0	<u> </u>	+			
	0	0	1	0	0	0	0	0						4			
7:00 AM to 8:00 AM 8:00 AM to 9:00 AM	velling sout	thbound cro	ossing east	bound appro	oach (West I	eg)					Diagram fo	r:	AM Peak I	Hour			
											Peak Hour	:	7:15 AM -	8:15 AM*			
8:00 AM to 9:00 AM																	
8:00 AM to 9:00 AM																	
8:00 AM to 9:00 AM																	
8:00 AM to 9:00 AM																	
8:00 AM to 9:00 AM																	
8:00 AM to 9:00 AM																	
8:00 AM to 9:00 AM																	
8:00 AM to 9:00 AM																	

Project Number: 30900399A CoSA Culebra Road Corridor S TX 1604 to I-10	itudy					AM TUR	NING MO	VEMENT (COUNTS							11.	ור
North/South Street:					IH 10 EBF							Site Code:	Sig_15 AM				
East/West Street:					Culebra R												
TOD:	AM		12/15/202	0	Raw Data:	J:\\5.1 Data C		Raw Count D	ata\From GR	IAM\12 30 202	0\Extracted 1	MC\WSP - 12	-03713 - Culel				
			DEBFR			Culebra									ora Rd		
			bound			Westbo					bound				oound	U-Turn	
Time 7:00 AM	Left 20	Through 4	Right 98	U-Turn 0	Left 5	Through 98	Right 0	U-Turn 0	Left 0	Through 0	Right 0	U-Turn 0	Left 0	Through 104	Right 165	U-Turn 0	
7:00 AM 7:15 AM		2	98	0	12	105	0	0	0	0	0	0	0	104	165	0	
7:30 AM		6	110	0	22	120	0	0	0	0	0	0	0	168	202	0	
7:45 AM		8	110	0	7	122	0	0	0	0	0	0	0	163	201	0	
8:00 AM		11	83	0	12	146	0	0	0	0	0	0	0	151	210	0	
8:15 AM		5	88	0	16	130	0	0	0	0	0	0	0	155	183	0	
8:30 AM		4	86	0	17	104	0	0	0	0	0	0	0	136	147	0	
8:45 AM		8	81	0	13	109	0	0	0	0	0	0	0	147	153	0	
2-hour Total		48	749	0	104	934	0	0	0	0	0	0	0	1172	1429	0	
7:15 AM - 8:15 AM*		27	396	0	53	493	0	0	0	0	0	0	0	630	781	0	
Peak Hour Factor	0.717	0.614	0.900		0.602	0.844								0.938	0.930		
Heavy Vehicle Percentage	1.14%	8.33%	1.07%		1.92%	5.46%								2.65%	1.33%		
OnStreet Bicycle Count		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
(7:15 AM to 8:15 AM)							Ŭ	Ŭ			Ů	Ŭ	Ŭ	Ŭ	Ŭ	ŭ	
* 7:15 AM - 8:15 AM Peak Hor	ır represe	ents the corr	idor peak n	ot the inter	section peak	L								_		7	
										Peak Hour	s Volume I	Diagram		1	0	4	
		0 EBFR		bra Rd	No.	la la constant		ora Rd oound						+	493	-	
Time	SB/W**	hbound SB/E		bound WB/S	NB/W	hbound NB/E	EB/N	EB/S		396	27	109	0	\$	53	-	
Time	2B/W++		WB/N Pedestrians		NB/W	NB/E	EB/N	EB/S		←	¥		U	ð.	+	↑	
7:00 AM to 8:00 AM	0	0	0	1	0	0	0	0		←	*	→ 0	5	0	0	0	→ 0
8:00 AM to 9:00 AM	0	0	0	2	0	0	0	0				0	1	U	U	U	U
0.007441 (0 3.007441			ele Counts -			<u> </u>						630	→				
7:00 AM to 8:00 AM	0	0	0	0	0	0	0	0				781	į				
8:00 AM to 9:00 AM	0	0	0	0	0	0	0	0									
** SB/W - Pedestrian/cycle tra	velling sou	uthbound cr	ossing eastl	bound appr	oach (West	leg)				- 1	Diagram fo	r:	AM Peak F	lour		1	
											Peak Hour	:	7:15 AM -	8:15 AM*			

Project Number: 30900399A CoSA Culebra Road Corridor Study TX 1604 to I-10 North/South Street: East/West Street: TOD: AM **115D** AM TURNING MOVEMENT COUNTS Site Code: Sig_16 AM 6-TMCs Culebra Rd Eastbound Through Right U-Turn 49 0 0 0 58 0 0 74 0 0 82 0 0 67 0 0 52 0 0 0 0 0 0 0 0 12 30 2020\Extra IH 10 WBFR Northbound Left 86 110 118 121 119 117 119 98 7:00 AM 7:15 AM 7:30 AM 7:45 AM 8:00 AM 8:15 AM 8:30 AM 8:45 AM 80 89 108 105 120 115 91 16 20 17 16 13 14 10 13 888 480 468 281 0.967 0.857 0.79% 5.83% 253 130 0.833 9.09% 116 66 801 422 7:15 AM - 8:15 AM* 422 9 35 0.879 0.563 0.673 4.12% 0.00% 10.39% 281 Peak Hour Factor Heavy Vehicle Percentage OnStreet Bicycle Count (7:15 AM to 8:15 AM) 7:15 AM - 8:15 AM Peak Hou 0.825 1.72% 0 0 0 0 0 66 130 0 0 Peak Hours Volume Diagram Southbound Culebra Rd IH 10 WBFR Culebra Rd Southbound Westbound Northbound Eastbound SB/W** SB/E WB/N WB/S NB/W NB/E EB/N EB/S **Ω** ← 0 422 0 468 281 0 7:00 AM to 8:00 AM 8:00 AM to 9:00 AM **→** AM Peak Hour 7:15 AM - 8:15 AM* Diagram for: Peak Hour:

Tesel Rd	North/South Street: Site Code: Sig_ 17 AM	Project Number: 30900399A CoSA Culebra Road Corridor S TX 1604 to I-10	tudy					AM TURI	NING MO	VEMENT	COUNTS							11	SP
Time	Time						Tezel Ro	i						Site Code:	Sig_17 AM				_
Texel Rd Southbound Sout	Texel Rd Southbound Sout																		
Time	Time	TOD:	AM			0	Raw Data:			2 Raw Count E	Data\From GR			MC\WSP - 12-	-03713 - Culeb				
Time	Time																		
7:00 AM 24 136 6 0 17 45 112 0 58 76 10 0 4 137 98 1 1 7:15 AM 24 154 3 0 25 81 10 0 0 60 75 16 0 14 160 106 0 75 16 0 13 17:15 AM 24 154 3 0 25 81 10 0 0 60 75 16 0 13 212 137 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	7:00 AM 24 136 6 0 17 45 112 0 58 76 10 0 4 137 98 1 1 7:15 AM 24 154 3 0 25 81 10 0 0 60 75 16 0 14 160 106 0 75 16 0 13 17:15 AM 24 154 3 0 25 81 10 0 0 60 75 16 0 13 212 137 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1																		
7.15 AM 24 154 3 0 0 25 81 10 0 0 60 75 16 0 14 100 106 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	7.15 AM 24 154 3 0 0 25 81 10 0 0 60 75 16 0 14 100 106 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0																		
7-30 AM 39 174 5 0 177 67 13 0 61 76 15 0 13 212 137 1 1 7-45 AM 24 166 8 0 30 1012 199 0 70 108 16 0 26 220 137 0 8 16 1 1 1 1 16 131 100 3 1 17 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	7-30 AM 39 174 5 0 177 67 13 0 61 76 15 0 13 212 137 1 1 7-45 AM 24 166 8 0 30 1012 199 0 70 108 16 0 26 220 137 0 8 16 1 1 1 1 16 131 100 3 1 17 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1																		
7-35 AM 24 166 8 0 30 102 199 0 70 108 16 0 26 220 137 0 3 8 8 8 8 8 9 1 2 0 57 86 11 1 16 131 100 3 8 8 8 8 12 8 9 107 7 0 188 82 16 0 63 86 8 8 0 21 134 4 77 2 2 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	7-35 AM 24 166 8 0 30 102 199 0 70 108 16 0 26 220 137 0 3 8 8 8 8 8 9 1 2 0 57 86 11 1 16 131 100 3 8 8 8 8 12 8 9 107 7 0 188 82 16 0 63 86 8 8 0 21 134 4 77 2 2 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8																		
8:00 AM 33 161 9	8:00 AM 33 161 9																		
8:15 MM 29 107 7 0 18 82 16 0 0 83 86 8 0 21 134 77 2 8 836 MM 30 123 10 0 0 21 85 14 0 6 65 90 15 0 24 101 90 2 2 1 134 77 2 2 848 MM 12 110 11 0 0 25 74 14 0 65 90 15 0 24 101 90 2 2 1 132 112 8 19 10 11 1 0 0 25 74 14 0 65 90 15 0 24 101 90 2 2 1 134 17 4 2 8 14 14 14 17 17 14 17 17 18 19 11 1 1 7.15 AM -8.15 AM* 120 652 25 0 100 330 54 0 288 35 58 1 69 723 480 4 1 14 1217 819 11 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	8:15 MM 29 107 7 0 18 82 16 0 0 83 86 8 0 21 134 77 2 8 836 MM 30 123 10 0 0 21 85 14 0 6 65 90 15 0 24 101 90 2 2 1 134 77 2 2 848 MM 12 110 11 0 0 25 74 14 0 65 90 15 0 24 101 90 2 2 1 132 112 8 19 10 11 1 0 0 25 74 14 0 65 90 15 0 24 101 90 2 2 1 134 17 4 2 8 14 14 14 17 17 14 17 17 18 19 11 1 1 7.15 AM -8.15 AM* 120 652 25 0 100 330 54 0 288 35 58 1 69 723 480 4 1 14 1217 819 11 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1																		
8:30 M	8:30 M																		
8.84 SM 12 110 11 0 25 74 14 0 65 90 15 0 24 101 90 2	8.84 SM 12 110 11 0 25 74 14 0 65 90 15 0 24 101 90 2																		
7:15 AM - 8:15 AM 120	7:15 AM - 8:15 AM 120				11	0			14	0				0			90		
Peak Hour Factor 0.769 0.953 0.694 0.833 0.809 0.711 0.886 0.799 0.906 0.250 0.663 0.822 0.876 0.333 0.809 0.711 0.886 0.799 0.906 0.250 0.663 0.822 0.876 0.333 0.809 0.711 0.886 0.799 0.906 0.250 0.608 0.809 0.908 0.809 0.809 0.908 0.809 0.908	Peak Hour Factor 0.769 0.953 0.694 0.833 0.809 0.711 0.886 0.799 0.906 0.250 0.663 0.822 0.876 0.333 0.809 0.711 0.886 0.799 0.906 0.250 0.663 0.822 0.876 0.333 0.809 0.711 0.886 0.799 0.906 0.250 0.663 0.822 0.876 0.333 0.809 0.711 0.886 0.799 0.906 0.250 0.608 0.809 0.908 0.809	2-hour Total	215	1128	59	0	181	616	110	0	517	657	105	1	141	1217	819	11	
Heavy Wehicle Percentage 1.86% 1.33% 3.39% 3.39% 3.38% 4.55% 5.45% 2.71% 3.96% 1.90% 0.00% 2.84% 1.64% 1.95% 0.00%	Heavy Wehicle Percentage 1.86% 1.33% 3.39% 3.39% 3.87% 4.55% 5.45% 2.71% 3.96% 1.90% 0.00% 2.84% 1.64% 1.95% 0.00%	7:15 AM - 8:15 AM*	120	652	25	0	100	330	54	0	248	345	58	1	69	723	480	4	
On Street Bicycle Count O O O O O O O O O	On Street Bicycle Count O O O O O O O O O	Peak Hour Factor	0.769	0.953	0.694		0.833	0.809	0.711		0.886	0.799	0.906	0.250	0.663	0.822	0.876	0.333	
C15 AM to 8:15 AM 0	C 15 AM 16 3:15 AM 0		1.86%	1.33%	3.39%		3.87%	4.55%	5.45%		2.71%	3.96%	1.90%	0.00%	2.84%	1.64%	1.95%	0.00%	
C7:15 AM to 8:15 AM	C7:15 AM to 8:15 AM		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Tezel Rd Grissom Rd Southbound Sout	Tezel Rd Grissom Rd Southbound Sout								Ü	U	U	Ü	Ů	U	U	0	U	U	
Tezel Rd	Tezel Rd	* 7:15 AM - 8:15 AM Peak Hou	ir represe	nts the corr	idor peak n	ot the inter	section peak	ζ.								-		_	
Southbound Westbound Northbound Eastbound Eastbound Eastbound Eastbound Eastbound Northbound Eastbound	Southbound Westbound Northbound Eastbound Eastbound Eastbound Eastbound Eastbound Northbound Eastbound	1	Tax	al Dal	Ceina	ana Dal	Coll	ahaa Dal	Code	han Dal	1	Peak Hour	rs Volume I	Diagram				+	
Time	Time																	+	
Pedestrians Counts	Pedestrians Counts	Time										25	652	120	0			+	
7:00 AM to 8:00 AM 0 0 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	7:00 AM to 8:00 AM 0 0 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Time	30/ 44				140/44	NO/L	LD/IV	LD/3		_					_	↑	_
8.00 AM to 9:00 AM 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	8.00 AM to 9:00 AM 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	7:00 AM to 8:00 AM	0				0	0	0	0								345	
7:00.AM	7:00.AM																	0.0	
8:00 AM to 9:00 AM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	8:00 AM to 9:00 AM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0			Bicyc	le Counts -	On Sidewal	lk							723	→	1			
** SB/W - Pedestrian/cycle travelling southbound crossing eastbound approach (West leg) Diagram for: AM Peak Hour	** SB/W - Pedestrian/cycle travelling southbound crossing eastbound approach (West leg) Diagram for: AM Peak Hour								0	0				480	+	1			
									0	0						-		-	
Peak Hour: 7:15 AM - 8:15 AM*	Peak Hour: 7:15 AM - 8:15 AM*	** SB/W - Pedestrian/cycle tra	velling sou	thbound cr	ossing eastb	oound appr	oach (West	leg)											
													Peak Hour	:	7:15 AM -	8:15 AM*		1	

CoSA Culebra Road Corridor 5 FX 1604 to I-10	itudy															• • •	111
North/South Street:					Ingram Ro	1						Site Cod	e: Sig_18				_
East/West Street:					Culebra R												
TOD:	AM		12/15/2020)	Raw Data:	J:\\5.1 Data C		Raw Count D	ata\From GRA			MC\WSP - 12-	03713 - Culei				
			m Rd bound			Culebra				Ingra North	m Rd			Culet Eastb			
Time	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	
7:00 AM	3	0	9	0	7	132	0	0	6	5	15	0	44	342	1	0	
7:15 AM	7	5	22	0	5	136	4	0	5	10	26	0	46	449	6	0	
7:30 AM		4	27	0	3	181	7	0	5	16	27	0	72	437	2	0	
7:45 AM	8	4	24	0	6	195	6	0	6	13	21	0	62	425	3	0	
8:00 AM 8:15 AM	4	5	23 26	0	13 10	194 197	3 6	0	2	7	21 17	0	43 46	297 287	4	0	
8:15 AM 8:30 AM	4	1	25	0	6	167	7	0	4	6	14	0	34	287	2	0	
8:45 AM		5	29	0	8	195	3	0	4	4	7	0	36	269	3	0	
2-hour Total	52	30	185	0	58	1397	36	0	35	70	148	0	383	2805	22	0	
7:15 AM - 8:15 AM*	24	18	96	0	27	706	20	0	18	46	95	0	223	1608	12	0	
Peak Hour Factor		0.900	0.889		0.519	0.905	0.714		0.750	0.719	0.880		0.774	0.895	0.500		
Heavy Vehicle Percentage		6.67%	4.32%		1.72%	4.51%	5.56%		2.86%	0.00%	0.68%		2.35%	2.82%	0.00%		
OnStreet Bicycle Count (7:15 AM to 8:15 AM)		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
* 7:15 AM - 8:15 AM Peak Ho		nts the corn	idor neak ni	ot the inter-	section neak												
										Peak Hour	s Volume D	Diagram		1	20	1	
		am Rd		ıra Rd		am Rd		ıra Rd	Ī					+	706		
		hbound		oound		bound	Eastb							<u> </u>	27		
Time	SB/W**		WB/N	WB/S	NB/W	NB/E	EB/N	EB/S		96	18	24	0	G.	0		
7:00 AM to 8:00 AM	0	0	Pedestrians 0	0	0	0	0	0		+	↓	→ 0	か	0 D	+	46	→ 95
8:00 AM to 9:00 AM	0	0	0	0	1	0	3	0				223	Ť		10	40	33
			le Counts -	On Sidewal								1608	→	i			
7:00 AM to 8:00 AM	0	0	0	0	0	0	0	0				12	+				
8:00 AM to 9:00 AM	0	0	0	1	0	0	0	0								1	
** SB/W - Pedestrian/cycle tra	velling sou	thbound cro	ossing eastb	ound appro	oach (West I	eg)					Diagram for Peak Hour		AM Peak F 7:15 AM -				
											r eak Hour		7.13 AWI-	O.13 AW		1	

Project Number: 30900399A CoSA Culebra Road Corridor S	tudy					AM TURN	JING MO	VEMENT	COUNTS							"	וול
TX 1604 to I-10																	
North/South Street: East/West Street:					Callaghan I Culebra R							Site Code:	Sig_19 AM				
TOD:	AM	Date:	12/15/2020	1		J:\\5.1 Data Co	ellection\F 1 1	Daw Caust D	ata\Fram CB	44411220202	O February T	AACLUICO 12	02712 0.446	on Del TMCs			
100.	AIVI		12/15/2020 shan Rd	,	NdW Ddld.	Culebra		Kaw Count D	ata\From GK		han Rd	MC/WSP - 12-	J3/13 - Culet		ora Rd		+
	l		bound			Westbo					bound				oound		
Time	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	
7:00 AM	15	67	15	0	32	65	17	0	64	65	25	0	15	66	55	0	1
7:15 AM		79	20	0	34	76	14	0	65	115	33	0	25	94	69	0	
7:30 AM	26	101	18	0	39	121	27	0	93	112	27	0	19	96	68	0	
7:45 AM	40	100	20	0	38	111	24	0	97	115	37	0	19	89	68	0	
8:00 AM	28	80	13	0	26	81	25	0	75	102	29	0	23	94	41	0	
8:15 AM		53	10	0	34	95	22	0	74	76	34	0	20	74	46	0	
8:30 AM	25	62	15	0	25	76	24	0	47	71	31	0	14	75	39	0	
8:45 AM	20	57	16	0	30	91	31	0	57	79	35	0	16	75	63	0	
2-hour Total	211	599	127	0	258	716	184	0	572	735	251	0	151	663	449	0	1
7:15 AM - 8:15 AM*	117	360	71	0	137	389	90	0	330	444	126	0	86	373	246	0	1
Peak Hour Factor	0.731	0.891	0.888		0.878	0.804	0.833		0.851	0.965	0.851		0.860	0.971	0.891		1
Heavy Vehicle Percentage	2.37%	3.17%	7.09%		3.88%	3.21%	1.63%		2.97%	3.40%	3.98%		3.97%	3.62%	5.79%		
OnStreet Bicycle Count	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
(7:15 AM to 8:15 AM)							J	U	0	0	0	J	J	0	U	U	
* 7:15 AM - 8:15 AM Peak Hou	ır represer	its the corr	idor peak no	ot the inter	section peak											-	
									i	Peak Hour	s Volume I	Diagram		1	90	_	
		han Rd		ora Rd		ghan Rd	Culeb							+	389	_	
		bound		bound		hbound	Eastb							<u> </u>	137	_	
Time	SB/W**		WB/N	WB/S	NB/W	NB/E	EB/N	EB/S		71	360	117	0	G.	0		
7:00 AM to 8:00 AM	0	0	Pedestrians 0	Counts	0		0	1		+	1	→ 0	<u>t</u>	0 V	← 330	444	→ 126
8:00 AM to 9:00 AM	2	0	0	0	0	0	0	3				86	- →	U	330	444	120
0.00 AW 10 3.00 AW			ele Counts -			-						373	→	ł			
7:00 AM to 8:00 AM	0	0	0	0	0	0	0	0				246	Ť				
8:00 AM to 9:00 AM	0	0	0	0	0	0	0	0					•	ı			
** SB/W - Pedestrian/cycle tra	velling sout	thbound cre	ossing easth	ound appro	oach (West I	leg)			ļ.		Diagram fo	r:	AM Peak F	lour		1	
											Peak Hour	:	7:15 AM -	8:15 AM*			
																-	
																	1/27/2021

APPENDIX C Project Number: 30900399A CoSA Culebra Road Corridor Study TX 1604 to I-10 North/South Street: East/West Street: WSD AM TURNING MOVEMENT COUNTS Rd-TMCs Culebra Rd Eastbound Through Right U-Turn 249 22 0 237 19 0 327 28 0 314 21 0 271 23 0 255 13 0 261 26 0 228 30 0 ate: 12/15/2020 Zarzamora St Southbound Through Right U-Turn Through 142 128 163 147 178 138 156 hrough 23 49 33 41 45 39 32 7:00 AM 7:15 AM 7:30 AM 7:45 AM 8:00 AM 8:15 AM 8:30 AM 8:45 AM 0 0 0 0 0 0 0 0 0 0 0 0 14 25 22 37 26 32 24 25 33 38 31 33 34 37 8 14 9 25 21 10 17 37 30 48 158 33 228 7:15 AM - 8:15 AM* 132 0.943 **32** 0.727 **172** 0.827 616 17 0.607 110 168 0.743 0.857 135 0.888 **69** 0.690 1149 0.878 **91** 0.813 Peak Hour Factor 0.833 0.943 0.727 Heavy Vehicle Percentage 1.61% 7.46% 5.00% 3.97% 6.02% 1.14% 2.01% 1.65% OnStreet Bicycle Count (7:15 AM to 8:15 AM) 7:15 AM - 8:15 AM Peak Hou 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 17 616 172 0 ← 61b ↓ 172 **Ç** 0 **P** ← 0 110 Zarzamora St Culebra Rd Zarzamora St Culebra Rd Southbound Westbound Northbound Eastbound SB/W** SB/E WB/N WB/S NB/W NB/E EB/N EB/S

Diagram for: Peak Hour:

AM Peak Hour 7:15 AM - 8:15 AM*

7:00 AM to 8:00 AM 8:00 AM to 9:00 AM

1/27/2021

California Cal	Time	North/South Street: East/West Street:					Culebra R							Site Code:	UnSig_1 Al	И			
Time	Time		ΔM	Date:	12/15/202	n			ollection\5 1 1	Paul Count C	lata\Erom GBI	1841.12.20.202	n\Extracted 1	MC\WSD - 12	02712 - Cula	hra Dd - TAACe			
Southbound Westbound Wes	Southbound Westbound Wes	IOD.	AIVI	Date.	12/13/202		Naw Data.			Naw Coulie L	ata\rioiii Gio			MIC/WSF = 12	03713 - Cule				
7.00 AM 0 0 0 0 0 6 131 0 0 0 4 0 2 0 0 25 0 0 0 7.15 AM 0 0 0 0 0 7 209 0 0 0 3 0 3 0 0 225 1 0 0 7.30 AM 0 0 0 0 0 7 209 0 0 0 3 0 3 0 0 225 1 0 0 7.30 AM 0 0 0 0 0 13 219 0 0 0 2 0 4 0 0 0 309 1 0 0 7.45 AM 0 0 0 0 0 0 7 232 0 0 2 0 1 0 0 264 1 0 0 264 1 0 0 8.80 AM 0 0 0 0 0 0 10 197 0 0 0 3 0 2 0 0 202 1 0 0 8.81 AM 0 0 0 0 0 10 197 0 0 0 0 5 0 0 168 1 0 0 8.81 AM 0 0 0 0 0 5 107 0 0 3 0 2 0 0 171 1 0 0 8.81 AM 0 0 0 0 0 5 197 0 0 3 0 2 0 0 171 1 0 0 8.81 AM 0 0 0 0 0 8 217 0 0 3 0 2 0 0 171 1 0 0 8.81 AM 0 0 0 0 0 6 8 217 0 0 3 0 3 0 2 0 0 171 1 0 0 8.81 AM 0 0 0 0 0 6 8 217 0 0 3 0 3 0 0 189 2 0 0 171 1 1 0 0 8.81 AM 0 0 0 0 0 65 1609 0 0 0 20 0 22 0 0 171 1 1 0 0 8.81 AM 0 0 0 0 0 65 1609 0 0 0 20 0 22 0 0 171 3 8 0 0 189 2 0 0 171 AM 0 1 0 0 0 10 0 10 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	7.00 AM 0 0 0 0 0 6 131 0 0 0 4 0 2 0 0 25 0 0 0 7.15 AM 0 0 0 0 0 7 209 0 0 0 3 0 3 0 0 225 1 0 0 7.30 AM 0 0 0 0 0 7 209 0 0 0 3 0 3 0 0 225 1 0 0 7.30 AM 0 0 0 0 0 13 219 0 0 0 2 0 4 0 0 0 309 1 0 0 7.45 AM 0 0 0 0 0 0 7 232 0 0 2 0 1 0 0 264 1 0 0 264 1 0 0 8.80 AM 0 0 0 0 0 0 10 197 0 0 0 3 0 2 0 0 202 1 0 0 8.81 AM 0 0 0 0 0 10 197 0 0 0 0 5 0 0 168 1 0 0 8.81 AM 0 0 0 0 0 5 107 0 0 3 0 2 0 0 171 1 0 0 8.81 AM 0 0 0 0 0 5 197 0 0 3 0 2 0 0 171 1 0 0 8.81 AM 0 0 0 0 0 8 217 0 0 3 0 2 0 0 171 1 0 0 8.81 AM 0 0 0 0 0 6 8 217 0 0 3 0 3 0 2 0 0 171 1 0 0 8.81 AM 0 0 0 0 0 6 8 217 0 0 3 0 3 0 0 189 2 0 0 171 1 1 0 0 8.81 AM 0 0 0 0 0 65 1609 0 0 0 20 0 22 0 0 171 1 1 0 0 8.81 AM 0 0 0 0 0 65 1609 0 0 0 20 0 22 0 0 171 3 8 0 0 189 2 0 0 171 AM 0 1 0 0 0 10 0 10 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0			South	bound														
7.15 AM 0 0 0 0 0 7 209 0 0 3 0 3 0 3 0 0 225 1 0 0 7.730 AM 0 0 0 0 0 13 219 0 0 0 2 0 4 0 0 309 1 0 0 2 7.45 AM 0 0 0 0 0 0 0 1 3 219 0 0 0 2 0 1 0 0 0 264 1 0 0 309 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	7.15 AM 0 0 0 0 0 7 209 0 0 3 0 3 0 3 0 0 225 1 0 0 7.730 AM 0 0 0 0 0 13 219 0 0 0 2 0 4 0 0 309 1 0 0 2 7.45 AM 0 0 0 0 0 0 0 1 3 219 0 0 0 2 0 1 0 0 0 264 1 0 0 309 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		Left	Through	Right	U-Turn	Left		Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	
7.39 AM 0 0 0 0 13 219 0 0 2 0 4 0 0 309 1 0 0 2 7.45 AM 0 0 0 0 0 7 232 0 0 2 0 1 0 0 256 1 0 0 8.00 AM 0 0 0 0 0 9 207 0 0 0 3 0 2 0 0 202 1 0 0 8.15 AM 0 0 0 0 0 10 10 197 0 0 0 0 5 0 0 168 1 0 0 8.30 AM 0 0 0 0 0 5 197 0 0 0 3 0 2 0 0 171 1 1 0 0 8.81 AM 0 0 0 0 0 8 217 0 0 3 0 3 0 2 0 0 171 1 1 0 0 8.81 AM 0 0 0 0 0 8 217 0 0 3 0 3 0 2 0 0 171 1 1 0 0 8.81 AM 0 0 0 0 0 65 1609 0 0 0 20 0 22 0 0 171 1 1 0 0 189 2 0 0 171 1 1 0 0 1 0 0 1 0 0 1 0 0 0 1 0 0 0 1 0 0 0 1 0 0 0 1 0 0 0 1 0 0 0 1 0	7.39 AM 0 0 0 0 13 219 0 0 2 0 4 0 0 309 1 0 0 2 7.45 AM 0 0 0 0 0 7 232 0 0 2 0 1 0 0 256 1 0 0 8.00 AM 0 0 0 0 0 9 207 0 0 0 3 0 2 0 0 202 1 0 0 8.15 AM 0 0 0 0 0 10 10 197 0 0 0 0 5 0 0 168 1 0 0 8.30 AM 0 0 0 0 0 5 197 0 0 0 3 0 2 0 0 171 1 1 0 0 8.81 AM 0 0 0 0 0 8 217 0 0 3 0 3 0 2 0 0 171 1 1 0 0 8.81 AM 0 0 0 0 0 8 217 0 0 3 0 3 0 2 0 0 171 1 1 0 0 8.81 AM 0 0 0 0 0 65 1609 0 0 0 20 0 22 0 0 171 1 1 0 0 189 2 0 0 171 1 1 0 0 1 0 0 1 0 0 1 0 0 0 1 0 0 0 1 0 0 0 1 0 0 0 1 0 0 0 1 0 0 0 1 0																		
7.45 AM 0 0 0 0 0 7 232 0 0 0 2 0 1 0 0 264 1 0 0 264 1 0 0 8.8 0.0 AM 0 0 0 0 0 9 207 0 0 0 3 0 2 0 0 0 202 1 0 0 0 20 1 0 0 1 0 0 0 0	7.45 AM 0 0 0 0 0 7 232 0 0 0 2 0 1 0 0 264 1 0 0 264 1 0 0 8.8 0.0 AM 0 0 0 0 0 9 207 0 0 0 3 0 2 0 0 0 202 1 0 0 0 20 1 0 0 1 0 0 0 0																		
8:00 AM 0 0 0 0 9 207 0 0 3 0 2 0 0 202 1 0 0 8:15AM 0 0 0 0 0 10 197 0 0 0 0 5 0 0 1688 1 0 0 8:15AM 0 0 0 0 0 5 197 0 0 0 3 0 2 0 0 171 1 0 0 8:84 AM 0 0 0 0 0 5 197 0 0 0 3 0 2 0 0 171 1 1 0 0 1 0 0 0 0 0 0 0 0 0 0 0	8:00 AM 0 0 0 0 9 207 0 0 3 0 2 0 0 202 1 0 0 8:15AM 0 0 0 0 0 10 197 0 0 0 0 5 0 0 1688 1 0 0 8:15AM 0 0 0 0 0 5 197 0 0 0 3 0 2 0 0 171 1 0 0 8:84 AM 0 0 0 0 0 5 197 0 0 0 3 0 2 0 0 171 1 1 0 0 1 0 0 0 0 0 0 0 0 0 0 0																		
8.13 AM 0 0 0 0 0 10 197 0 0 0 0 0 5 5 0 0 168 1 0 0 8.30 AM 0 0 0 0 0 5 197 0 0 0 3 0 2 0 0 171 1 0 0 0 8.43 AM 0 0 0 0 0 0 8 217 0 0 3 3 0 3 0 0 189 2 0 0 171 1 1 0 0 0 0 0 0 5 197 0 0 0 3 0 0 2 0 0 171 1 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0	8.13 AM 0 0 0 0 0 10 197 0 0 0 0 0 5 5 0 0 168 1 0 0 8.30 AM 0 0 0 0 0 5 197 0 0 0 3 0 2 0 0 171 1 0 0 0 8.43 AM 0 0 0 0 0 0 8 217 0 0 3 3 0 3 0 0 189 2 0 0 171 1 1 0 0 0 0 0 0 5 197 0 0 0 3 0 0 2 0 0 171 1 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0																		
8 33 0 M 0 0 0 0 5 197 0 0 3 0 2 0 0 171 1 0 0 8 8.54 5M 0 0 0 0 8 217 0 0 0 3 0 3 0 0 189 2 0 0 2 2 0 0 172 1 1 0 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0	8 33 0 M 0 0 0 0 5 197 0 0 3 0 2 0 0 171 1 0 0 8 8.54 5M 0 0 0 0 8 217 0 0 0 3 0 3 0 0 189 2 0 0 2 2 0 0 172 1 1 0 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0																		
2-hour Total	2-hour Total 0																		
715 AM - 8.15 AM 0	715 AM - 8.15 AM 0	8:45 AM	0	0	0	0	8	217	0	0	3	0	3	0	0	189	2	0	
Peak Hour Factor	Peak Hour Factor		0	0	0	0			0	0	20	0		0	0	1733	8	0	
Heavy Vehicle Percentage	Heavy Vehicle Percentage		0	0	0	0			0	0		0		0	0			0	
On Street Bicycle Count O O O O O O O O O	On Street Bicycle Count O O O O O O O O O																		
C1:5 AM to 8:15 AM Peak Hour represents the comidor peak not the intersection peak Peak Hour represents the comidor peak not the intersection peak	C1:5 AM to 8:15 AM																		
T15 AM Peak Hour represents the corridor peak not the intersection peak. Culebra Rd	T15 AM Peak Hour represents the corridor peak not the intersection peak. Culebra Rd			0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Culebra Rd Culebra Rd Culebra Rd Culebra Rd Culebra Rd Eastbound	Culebra Rd	* 7:15 AM - 8:15 AM Peak Ho	ur represer	nts the corri	idor peak n	ot the inter	section peak	L.											
Southbound Westbound Northbound Eastbound Northbound Eastbound Northbound Eastbound Northbound Eastbound Northbound Eastbound Northbound Eastbound Northbound Northboun	Southbound Westbound Northbound Eastbound Northbound Eastbound Northbound Eastbound Northbound Eastbound Northbound Eastbound Northbound Eastbound Northbound Northboun										_	Peak Hour	rs Volume I	Diagram					
Time S8/N** S8/E W8/N W8/S N8/W N8/E E8/N E8/S 0 0 0 0 0 G 0 0 0 0 0 0 0 0 0 0 0 0 0	Time S8/N** S8/E W8/N W8/S N8/W N8/E E8/N E8/S 0 0 0 0 0 G 0 0 0 0 0 0 0 0 0 0 0 0 0																	_	
Pedestrians Counts	Pedestrians Counts														-			4	
7:00 AM to 8:00 AM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	7:00 AM to 8:00 AM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Time	2R/W++				NB/W	NB/E	EB/N	EB/S								4	
8:00 AM to 9:00 AM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	8:00 AM to 9:00 AM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	7:00 AM to 8:00 AM	0				0	0	0	0									
7:00 AM to 8:00 AM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	7:00 AM to 8:00 AM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	8:00 AM to 9:00 AM	0	0	0	0	0	0	0	0				0	1				
8:00 AM to 9:00 AM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	8:00 AM to 9:00 AM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0																		
** SB/W - Pedestrian/cycle travelling southbound crossing eastbound approach (West leg) Diagram for: AM Peak Hour	** SB/W - Pedestrian/cycle travelling southbound crossing eastbound approach (West leg) Diagram for: AM Peak Hour													4	1				
									0	0	J	<u> </u>	n: .					1	
Peak INVIII.	Peak Moul. 1/JJ mm - GJJ mm	SB/W - Pedestrian/cycle tra	veiling sout	inbound cre	JSSING EAST	ounu appr	nacii (west	ieg)											
													reak noui		7.13 AIVI -	0.13 AIVI		J	

Project Number: 30900399A CoSA Culebra Road Corridor Study TX 1604 to I-10 North/South Street: East/West Street: wsp AM TURNING MOVEMENT COUNTS Date: 12/15/2020 Coppertree Blvd Southbound Through Right U-Turn Through 110 159 144 175 149 177 151 7:00 AM 7:15 AM 7:30 AM 7:45 AM 8:00 AM 8:15 AM 8:30 AM 8:45 AM 0 0 0 0 0 152 7:15 AM - 8:15 AM* 11 Peak Hour Factor 0.393 Heavy Vehicle Percentage 6.25% 627 0.896 3.29% 6 0.375 7.14% 2 1281 0.250 0.852 0.00% 1.67% 0 0 OnStreet Bicycle Count (7:15 AM to 8:15 AM) 7:15 AM - 8:15 AM Peak Hou 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 6 627 0 0 ψ + → 0 2 1281 0 ↑ → 0 0 7:00 AM to 8:00 AM 8:00 AM to 9:00 AM AM Peak Hour 7:15 AM - 8:15 AM*

TX 1604 to 1-10 TX 1604 to	0
East/West Street: Culebra Rd All Date: 12/15/2020 Raw Data: 13/15/2020 Raw Data	0
TOD: AM Date: 12/15/2020 Raw Data: I_\\.\.\.\.\.\.\. base Count Deal/rom GRAN\12 30 2020/beracted TMC\WSP - 12-0373 - Culebra Rd - TMCs Culebra Rd - TMCs	0
Carbino Rosa Culebra Rd Northbound Culebra Rd Northbound Culebra Rd Southbound Culebra Rd Northbound Culebra Rd Southbound Culebra Rd Northbound Culebra Rd Culebra Rd Northbound Culebra Rd Culebra Rd Northbound	0
Southbound Westbound Westbound U-Turn Left Through Right	0
Time	0
7:00 AM	0
T;15 AM	7
7:30 AM	
8.00 AM	D .
8.15 AM 0 0 11 0 0 137 8 3 0 0 0 0 10 220 0 1 8.23 AM 4 0 17 0 0 144 4 7 0 0 0 0 7 201 0 0 8.45 AM 4 0 24 0 0 140 9 1 0 0 0 0 8 227 0 0 2-hour Total 15 0 129 0 0 1107 52 14 0 0 0 0 0 70 2162 0 1 7:15 AM - 8.15 AM 6 0 63 0 0 599 25 2 0 0 0 0 33 1238 0 0 Peak Hour Factor 0.500 0 0.788 0.930 0.481 0.500 0 0 0 0 0 0 0 Peak Hour Factor 0.500 0.788 0.930 0.481 0.500 0 0.750 0.910 Heavy White Perrentage 0.00% 3.10% 2.26% 3.85% 0.00% 0.26% 2.56% 2.50% 0.00% OnStreet Bicycle Count 0 0 0 0 0 0 0 0 0	0
8:30 AM	o .
8.45 AM	
2-hour Total 15 0 129 0 0 1107 52 144 0 0 0 0 70 2162 0 1 7:15 AM -8.15 AM* 6 0 63 0 0 599 25 2 0 0 0 0 33 1238 0 0 Peak Hour Factor 0.500 0.788 0.930 0.481 0.500 0.070 0.750 0.910 Heavy Vehicle Percentage 0.00% 3.10% 2.62% 3.85% 0.00% 2.86% 2.50% 0.00% OnStreet Bicycle Count (7:15 AM to 8:15 AM) 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	
7.15 AM - 8.15 AM 6	
Peak Hour Factor 0.500 0.7.88 0.930 0.481 0.500 0.750 0.910 0.750	
Heavy Vehicle Percentage 0.00% 3.10% 2.62% 3.85% 0.00% 2.86% 2.50% 0.00% 0.00% 0)
OnStreet Bicycle Count (7:15 AM to 8:15 AM) 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	
(7:15 AM to 8:15 AM)	0%
* 7:15 AM - 8:15 AM Peak Hour represents the corridor peak not the intersection peak. Camino Rosa Culebra Rd Culebra Rd Culebra Rd Southbound Westbound Westbound Eastbound Culebra Rd Culebra R	ð .
Pedestrians Counts ← ↓ → ௴ ♪ ← ↑	↑ →
7:00 AM to 8:00 AM 0 0 0 0 0 0 0 1 0 5 0 0 0	0 0
8:00 AM to 9:00 AM 0 0 0 0 0 0 0 0 33 ↑	
Bicycle Counts - On Sidewalk 1238 →	
7:00 AM to 8:00 AM 0 0 0 0 0 0 0 0 1 0 0 0 0 0 0 0 0 0 0	
** SB/W - Pedestrian/cycle travelling southbound crossing eastbound approach (West leg) Diagram for: AM Peak Hour	
Peak Hour: 7:15 AM - 8:15 AM*	

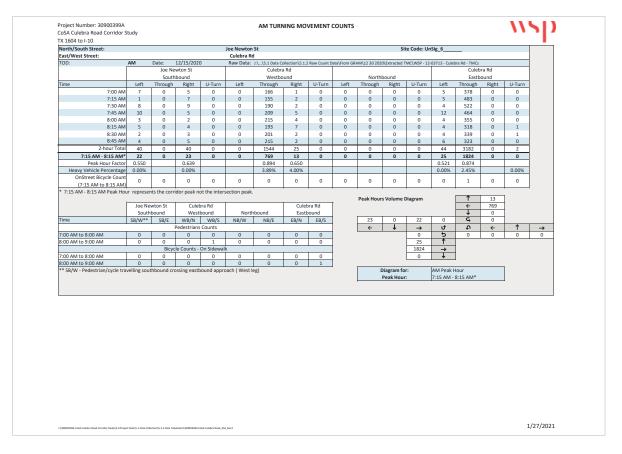
APPENDIX C WSD Project Number: 30900399A AM TURNING MOVEMENT COUNTS CoSA Culebra Road Corridor Study TX 1604 to I-10 North/South Street: 7:00 AM 7:15 AM 164 167 270 376 372 363 323 230 279 232 7:30 AM 7:45 AM 8:00 AM 8:15 AM 8:30 AM 8:45 AM 194 225 204 173 174 179 0 0 0 0 1434 0 0 0 Southbound Westbound Northbound Eastbound SB/W** SB/E WB/N WB/S NB/W NB/E EB/N EB/S ↑ → 0 0 7:00 AM to 8:00 AM 8:00 AM to 9:00 AM AM Peak Hour 7:15 AM - 8:15 AM*

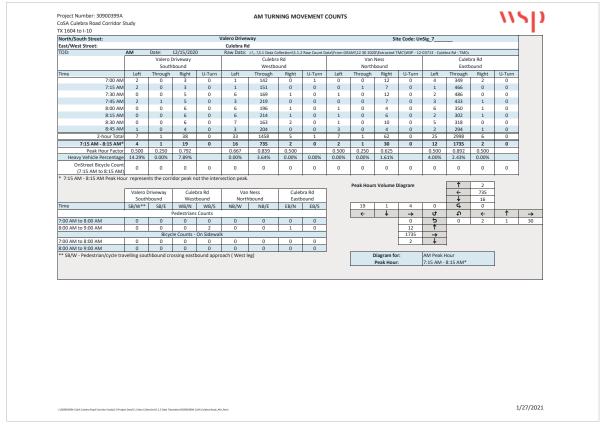
Project Number: 30900399A CoSA Culebra Road Corridor Study TX 1604 to I-10 **115D** AM TURNING MOVEMENT COUNTS North/South Street: East/West Street: Pipers Creek Culebra Rd Raw Data: J Site Code: UnSig 5 7:00 AM 7:15 AM 7:30 AM 7:45 AM 8:00 AM 8:15 AM 8:30 AM 8:45 AM 162 167 183 224 177 220 172 209 4 11 0 6 7:15 AM - 8:15 AM* 11 0 14 14 751 3 21 30 6 1775 24 0 Peak Hour Factor Heavy Vehicle Percentage 0.656 0.682 1.82% 0.889 2.47% OnStreet Bicycle Count (7:15 AM to 8:15 AM) 7:15 AM - 8:15 AM Peak Hou 0 0 0 0 0 0 0 0 0
 Pipers Creek
 Culebra Rd
 Pipers Creek
 Culebra Rd

 Southbound
 Westbound
 Northbound
 Eastbound

 SB/W**
 SB/E
 WB/N
 WB/S
 NB/W
 NB/E
 EB/N
 EB/S
 ひ ← ↑ → 0 30 :00 AM to 8:00 AM :00 AM to 9:00 AM ____ :00 AM to 8:00 AM :00 AM to 9:00 AM Diagram for: Peak Hour: AM Peak Hour 7:15 AM - 8:15 AM*

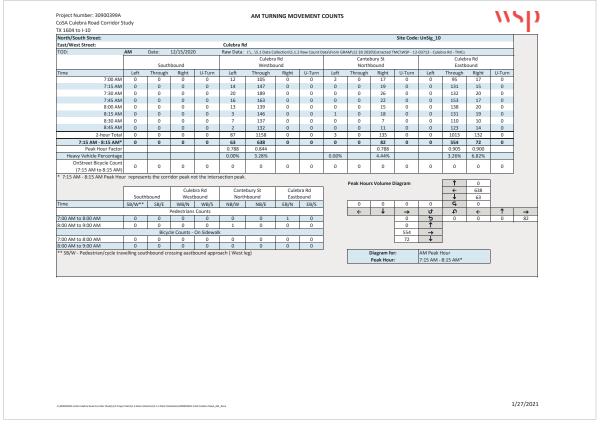
1/27/2021

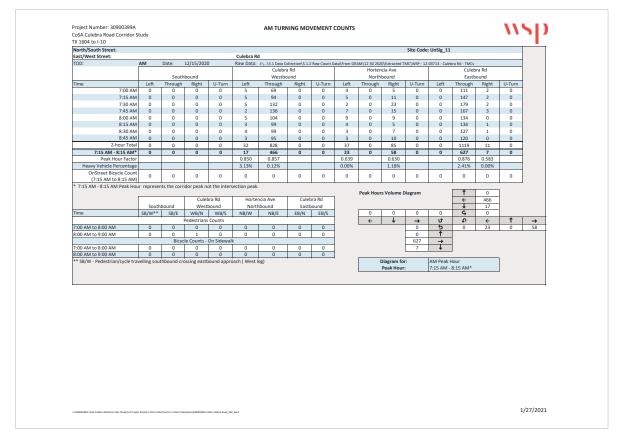




Project Number: 30900399A CoSA Culebra Road Corridor Study TX 1604 to I-10 North/South Street: East/West Street: **115D** AM TURNING MOVEMENT COUNTS Site Code: UnSig_8_ 43 29 45 44 49 34 15 7:00 AM 7:15 AM 7:30 AM 7:45 AM 8:00 AM 8:15 AM 8:30 AM 113 105 142 135 121 136 122 189 242 240 244 218 174 151 151 1025 503 0.886 6.05% 15 274 8:45 AM our Total 145 1603 2-hour Total 7:15 AM - 8:15 AM* Peak Hour Factor Heavy Vehicle Percentage OnStreet Bicycle Count (7:15 AM to 8:15 AM) 7:15 AM - 8:15 AM Peak Hou 944 167 0.967 0.852 2.18% 0.36% 3 0.375 0.00% 0.00% 25.00% 0 0 0 0 0 0 1 0 503 3 0 ← 503 ↓ 3 **G** 0 • ↑ → 0 Southbound Culebra Rd Westbound Avenue G Northbound Culebra Rd Eastbound SB/W** SB/E WB/N WB/S NB/W NB/E EB/N EB/S 7:00 AM to 8:00 AM 8:00 AM to 9:00 AM 944 → 167 ↓

7:00 AM 2 0 4 0 0 120 7:15 AM 9 0 1 0 0 137 7:30 AM 4 0 1 0 0 137 7:35 AM 7 0 4 0 0 137 7:35 AM 7 0 4 0 0 132 8:00 AM 5 0 8 0 0 134 8:10 AM 5 0 8 0 0 134 8:13 AM 9 0 11 0 0 149 8:30 AM 4 0 3 0 0 124 8:34 SAM 9 0 9 0 0 177 7:2 Abour Total 49 0 41 0 0 630 7:15 AM -8:15 AM 25 0 14 0 0 630 7:15 AM -8:15 AM 25 0 14 0 0 630 7:15 AM -8:15 AM Peak Hour represents the corridor peak not the intersection peak. Fairgrounds Pkwy Subthbound Northbound Northbound Northbound Fairgrounds Pkwy Subthbound Fairgrounds Pkwy Subthbound Northbound Northbound Fairgrounds Pkwy Subthbound Northbound Northbound Fairgrounds Pkwy Subthbound Northbound	d	Torn Left 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	North Through 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		U-Turn 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		Culet East Through 192 222 236 221 213 171 154 149 149 2.57% 0	Right 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	U-Turn 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 1	→
Time	d d	Torn Left 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	North Through 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Right O O O O O O O O O O O O O O O O O O O	U-Turn 0 0 0 0 0 0 0 0 0 0 0 0 0 Diagram	Left 9 15 11 11 11 10 6 2 75 48 0.800 1.33% 0	Culet East Through 192 222 236 221 213 171 154 149 149 2.57% 0	ora Rd oound Right 0 0 0 0 0 0 0 0 0 0 0 0 109 630 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	
Fairgrounds Phay Culebra Rd	d d	Torn Left 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	North Through 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Right O O O O O O O O O O O O O O O O O O O	U-Turn 0 0 0 0 0 0 0 0 0 0 0 0 0 Diagram	Left 9 15 11 11 11 10 6 2 75 48 0.800 1.33% 0	Culet East Through 192 222 236 221 213 171 154 149 149 2.57% 0	ora Rd oound Right 0 0 0 0 0 0 0 0 0 0 0 0 109 630 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	
Southbound	d Right U-Tur 9 0 20 0 33 0 32 0 24 0 20 0 16 0 175 0 199 0 0 826 Culebra Rd Eastbound EarlyN EB/S 0 1 109 0 0 0 0 0 100 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Through 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Right	0 0 0 0 0 0 0 0 0 0 0 0 0	9 15 11 11 11 10 6 2 75 48 0.800 1.33% 0	Through 192 222 236 221 213 171 154 149 1558 892 0.945 2.57% 0	Right 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	
7:00 AM	9 0 0 20 0 333 0 0 22 1 0 16 0 175 0 0 16 0 0 0 0 0 21 5 0 0 0 0 25 5 14% 0 0 0 Culebra Rd Eastbound EB/N EB/S	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0	9 15 11 11 11 10 6 2 75 48 0.800 1.33% 0	192 222 236 221 213 171 154 149 1558 892 0.945 2.57% 0	0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	
7.15 AM 9	20 0 33 0 32 0 24 0 20 0 21 0 16 0 175 0 109 0 0.826 5.14% 0 0 Culebra Rd Eastbound EB/N EB/S	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0	15 11 11 10 6 2 75 48 0.800 1.33% 0	222 236 221 213 171 154 149 1558 892 0.945 2.57% 0	0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	
7-30 AM	33 0 32 0 24 0 20 0 21 0 16 0 175 0 109 0 0.826 5.14% 0 0 Culebra Rd Eastbound EB/N EB/S	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0	11 11 11 10 6 2 75 48 0.800 1.33% 0	236 221 213 171 154 149 1558 892 0.945 2.57% 0	0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0	
7.45 AM	32 0 24 0 20 0 21 0 16 0 175 0 109 0 0.826 5.14% 0 0 Culebra Rd Eastbound EB/N EB/S	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0	0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0	11 11 10 6 2 75 48 0.800 1.33% 0	221 213 171 154 149 1558 892 0.945 2.57% 0	0 0 0 0 0 0 0 0	0 0 0 0 0 0 0	
8.00 AM	24 0 20 0 21 0 16 0 175 0 0.826 5.14% 0 0 Culebra Rd Eastbound EB/N EB/S	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0	0 0 0 0 0 0	0 0 0 0 0 0 0 0	11 10 6 2 75 48 0.800 1.33% 0	213 171 154 149 1558 892 0.945 2.57% 0	0 0 0 0 0 0 0	0 0 0 0 0 0	
8.15 AM 9 0 11 0 0 149	20 0 21 0 16 0 175 0 109 0 0.826 5.14% 0 0 Culebra Rd Eastbound EB/N EB/S	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0	0 0 0 0 0	0 0 0 0 0 0 0	10 6 2 75 48 0.800 1.33% 0	171 154 149 1558 892 0.945 2.57% 0	0 0 0 0 0 0	0 0 0 0 0	
8.30 AM	21 0 16 0 175 0 109 0 0.826 5.14% 0 0 Culebra Rd Eastbound EB/N EB/S	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0	0 0 0 0	0 0 0 0 0 0 Diagram	6 2 75 48 0.800 1.33% 0	154 149 1558 892 0.945 2.57% 0	0 0 0 0 0	0 0 0 0	
8.85 AM 9 0 9 0 0 177	16 0 175 0 109 0 0.826 5.14% 0 0 Culebra Rd Eastbound EB/N EB/S	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 Diagram	2 75 48 0.800 1.33% 0	149 1558 892 0.945 2.57% 0	0 0 0 0 109 630 0	0 0 0 0	
2-hour Total	175 0 109 0 0.826 5.14% 0 0 Culebra Rd Eastbound EB/N EB/S	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 O Peak Hour	0 0 0 ors Volume I	0 0 0 Diagram	75 48 0.800 1.33% 0	1558 892 0.945 2.57% 0	0 0 0 109 630 0	0	
7:15 AM - 8:15 AM 25	0 0.826 5.14% 0 0 Culebra Rd Eastbound EB/N EB/S	0 0 0 0 dd dd EB/S	0 0 Peak Hour	0 0 rs Volume	0 0 Diagram 25 →	48 0.800 1.33% 0	892 0.945 2.57% 0	0 0 109 630 0	0	
Peak Hour Factor 0.994	0.826 5.14% 0 0 Culebra Rd Eastbound EB/N EB/S	0 0 0 dddddddddddddddddddddddddddddddd	0 Peak Hour	0 rs Volume	0 Diagram 25 →	0.800 1.33% 0	0.945 2.57% 0	0 109 630 0	0	
OnStreet Bicycle Count O	0 0 Culebra Rd Eastbound EB/N EB/S	td dd dd EB/S	Peak Hour	rs Volume I	Diagram 25 →	0 0	0 ↑ ← ↓ G	109 630 0	1	
(7:15 AM to 8:15 AM)	Culebra Rd Eastbound EB/N EB/S	td dd dd EB/S	Peak Hour	rs Volume I	Diagram 25 →	0 U	↑ ← ↓ •	109 630 0	1	
C715 AM to 8:15 AM Peak Hour represents the corridor peak not the intersection peak.	Culebra Rd Eastbound EB/N EB/S	td dd dd EB/S	Peak Hour	rs Volume I	Diagram 25 →	0 U	↑ ← ↓ •	109 630 0	1	
Fairgrounds Pkwy Culebra Rd Northbound Northbound	Eastbound EB/N EB/S 0 1	EB/S	14	0	25 →	Ů.	÷ ÷	630 0 0		
Southbound Westbound Northbound	Eastbound EB/N EB/S 0 1	EB/S	14	0	25 →	Ů.	÷ ÷	630 0 0		
Southbound Westbound Northbound	Eastbound EB/N EB/S 0 1	EB/S			→	Ů.	₹ ↑	0		
SB/W** SB/E WB/N WB/S NB/W NB/E E	0 1	EB/S			→	Ů.	Ť.	0		
Pedestrians Counts	0 1	1			→	Ů.	Ď	_		
7.00 AM to 8.00 AM				_						
8:00 AM to 9:00 AM 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0							0	0	0	0
7:00 AM to 8:00 AM					48	1				
8:00 AM to 9:00 AM 0 0 0 0 0 0					892	→				
	0 0	0			0	+				
** SB/W - Pedestrian/cycle travelling southbound crossing eastbound approach (West leg)	0 0	0							_	
				Diagram fo Peak Hour		AM Peak F 7:15 AM -	8:15 AM*			





Project Number: 30900399A CoSA Culebra Road Corridor						AM TURI	NING MO	VEMENT	COUNTS							11	וול
FX 1604 to I-10 North/South Street:					Mira Vist							Cita Cada	: UnSig 12				_
East/West Street:					Culebra R							Site Code	: Unsig_12				
TOD:	AM	Date:	12/15/202	0		J:\\5.1 Data C	ollection\5.1.2	Raw Count D	ata\From GR	AM\12 30 202	0\Extracted T	MC\WSP - 12	-03713 - Culet	bra Rd - TMCs			
		Mira	a Vista			Culebr									ora Rd		
		South	hbound			Westbo	ound			North	bound			Eastb	ound		
Time	Left	Through		U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	
7:00 AN	1 3	0	3	0	0	110	3	0	0	0	0	0	4	95	0	0	
7:15 AN		0	7	0	0	143	3	0	0	0	0	0	9	142	0	0	
7:30 AM		0	4	0	0	190	2	0	0	0	0	0	12	135	0	0	
7:45 AN		0	9	0	0	184	6	0	0	0	0	0	8	160	0	0	
8:00 AN 8:15 AN		0	3	0	0	144 145	3 6	0	0	0	0	0	11 4	142 144	0	0	
8:15 AV 8:30 AV		0	6	0	0	125	2	0	0	0	0	0	7	120	0	0	
8:45 AN		0	7	0	0	141	0	0	0	0	0	0	3	112	0	0	
2-hour Tota		0	47	0	0	1182	25	1	0	0	0	0	58	1050	0	0	
7:15 AM - 8:15 AM*		0	23	0	0	661	14	1	0	0	0	0	40	579	0	0	
Peak Hour Factor			0.639			0.870	0.583	0.250					0.833	0.905			
Heavy Vehicle Percentage	7.69%		17.02%			2.54%	8.00%	0.00%					12.07%	2.86%			
OnStreet Bicycle Coun		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
(7:15 AM to 8:15 AM)						ŭ	Ü	Ů	Ů	Ů	Ů	Ů	Ŭ	Ů	ŭ	
* 7:15 AM - 8:15 AM Peak Ho	ur represe	nts the cor	ridor peak n	ot the inter	section peak											7	
	Adia	a Vista	Cula	bra Rd			Color	ora Rd		Peak Hour	s Volume I	Diagram		<u>↑</u>	14	4	
		hbound		bound	Nort	hbound		ound						<u> </u>	661	4	
Time	SB/W**		WB/N	WB/S	NB/W	NB/E	EB/N	EB/S		23	0	16	0	Ğ	1	1	
	,		Pedestrian:		,	,-	,	,-		+	+	→	t)	Ð	+	1	→
7:00 AM to 8:00 AM	0	0	0	0	0	0	0	0				0	5	0	0	0	0
8:00 AM to 9:00 AM	0	0	0	0	0	0	0	5				40	1				
				On Sidewal								579	→				
7:00 AM to 8:00 AM	0	0	0	1	0	0	0	0				0	1	Į.			
8:00 AM to 9:00 AM	0	0	0	0	0	0	0	0								1	
** SB/W - Pedestrian/cycle tra	avelling sou	rthbound cr	ossing east	bound appro	oach (West	leg)					Diagram fo Peak Hour		AM Peak F 7:15 AM -				
											Peak Hour		7.15 AIVI -	9.13 AIVI		J	

North/South Street: Culebra Rd	North/South Street: Culebra Rd	North/South Street: Culebra Rd Culebr	Project Number: 30900399A CoSA Culebra Road Corridor S TX 1604 to I-10	itudy					AM TUR	NING MO	VEMENT	COUNTS							115
TOD: AM Date: 12/15/2020 Raw Data: A. \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	Time	Time						NW 34th S	it						Site Code:	UnSig_13			
Southbound So	Southbound So	Southbound So																	
Southbound	Southbound	Southbound	TOD:	AM	Date:	12/15/2020	0	Raw Data:			Raw Count D	ata\From GR			MC\WSP - 12-	03713 - Culei			
Time	Time	Time																	
7:00 AM 0 0 0 0 0 1 80 0 0 0 0 0 1 80 0 0 0 0 0	7:00 AM 0 0 0 0 0 1 80 0 0 6 0 2 0 0 138 2 0 0 7:15 AM 0 0 0 0 0 0 0 188 0 0 0 3 0 0 0 0 190 5 0 0 97:15 AM 0 0 0 0 0 0 1 124 0 0 0 9 0 3 0 0 0 224 0 0 0 190 5 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	7:00 AM 0 0 0 0 0 1 80 0 0 6 0 2 0 0 138 2 0 0 7:15 AM 0 0 0 0 0 0 0 188 0 0 0 3 0 0 0 0 190 5 0 0 97:15 AM 0 0 0 0 0 0 1 124 0 0 0 9 0 3 0 0 0 224 0 0 0 190 5 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0																	
7:35 AM 0 0 0 0 0 108 0 0 3 0 0 0 0 199 5 0 0 7:30 AM 0 0 0 0 0 0 1 124 0 0 0 9 0 3 0 0 0 234 0 0 0 7:36 AM 0 0 0 0 0 0 2 145 0 0 0 6 0 2 0 0 210 4 0 0 0 8:35 AM 0 0 0 0 0 0 0 0 130 0 0 0 6 0 2 0 0 0 174 0 0 0 8:35 AM 0 0 0 0 0 0 0 0 99 0 0 0 6 0 1 0 0 181 0 0 181 0 0 181 0 0 0 8:35 AM 0 0 0 0 0 0 0 2 115 0 0 0 3 0 1 0 0 0 181 0 0 181 0 0 0 181 0 0 181 0 0 0 181 0 0 181 0 0 0 181 0 0 0 181 0 0 181 0 0 0 181 0 0 0 181 0 0 0 181 0 0 0 181 0 0 181 0 0 0 181 0 0 0 181 0 0 0 181 0 0 0 181 0 0 0 181 0 0 0 181 0 0 0 181 0 0 0 181 0 0 0 181 0 0 0 181 0 0 0 0	7:15 AM 0 0 0 0 0 1088 0 0 0 3 3 0 0 0 0 150 5 0 0 7:30 AM 0 0 0 0 0 1 124 0 0 0 9 0 3 0 0 0 224 0 0 0 224 0 0 0 7:30 AM 0 0 0 0 0 0 1 124 0 0 0 0 9 0 3 0 0 0 224 0 0 0 124 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	7:15 AM 0 0 0 0 0 1088 0 0 0 3 3 0 0 0 0 150 5 0 0 7:30 AM 0 0 0 0 0 1 124 0 0 0 9 0 3 0 0 0 224 0 0 0 224 0 0 0 7:30 AM 0 0 0 0 0 0 1 124 0 0 0 0 9 0 3 0 0 0 224 0 0 0 124 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0																	
7:30 MM 0 0 0 0 0 1 124 0 0 9 0 3 0 0 234 0 0 0 7:45 MP = 145 0 0 0 6 0 2 0 0 0 210 4 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	7:30 AM 0 0 0 0 0 1 1 124 0 0 0 9 0 3 0 0 224 0 0 0 2745 AM 0 0 0 0 0 0 2 1145 0 0 0 6 6 0 2 0 0 0 210 4 0 0 0 8:00 AM 0 0 0 0 0 0 0 130 0 0 0 4 0 1 1 0 0 174 0 0 0 0 8:15 AM 0 0 0 0 0 0 0 2 1115 0 0 0 3 0 1 1 0 0 151 3 1 0 0 151 3 1 1 8:45 AM 0 0 0 0 0 0 0 2 1115 0 0 0 3 0 1 1 0 0 0 151 3 1 1 0 0 151 3 3 1 1 8:45 AM 0 0 0 0 0 0 0 0 129 0 0 2 0 2 0 2 0 0 175 1 0 0 1745 AM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	7:30 AM 0 0 0 0 0 1 1 124 0 0 0 9 0 3 0 0 224 0 0 0 2 10 4 0 0 0 8:00 AM 0 0 0 0 0 0 0 130 0 0 0 4 0 1 1 0 0 174 0 0 0 8:15 AM 0 0 0 0 0 0 0 0 130 0 0 0 4 0 1 1 0 0 174 0 0 0 181 0 0 0 8:15 AM 0 0 0 0 0 0 0 2 1115 0 0 0 3 0 1 1 0 0 151 3 3 1 8:45 AM 0 0 0 0 0 0 0 2 1115 0 0 0 3 0 1 1 0 0 151 3 3 1 1 0 0 151 3 3 1 1 0 0 174 AM 1 1 1 0 0 0 174 AM 1 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0																	
7.45 AM 0 0 0 0 0 2 145 0 0 6 0 2 0 0 174 0 0 0 8.815 AM 0 0 0 0 0 0 0 0 199 0 0 6 0 1 0 0 174 0 0 0 8.815 AM 0 0 0 0 0 0 0 99 0 0 0 6 0 1 0 0 0 181 0 0 181 0 0 0 181 0 0 181 0 0 0 181 0 0 181 0 0 0 0	7.45 AM 0 0 0 0 0 2 145 0 0 0 6 0 2 0 0 124 0 0 0 124 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	7.45 AM 0 0 0 0 0 2 145 0 0 0 6 0 2 0 0 210 4 0 0 0 8.00 AM 0 0 0 0 0 0 0 131 0 0 0 0 0 0 0 174 0 0 0 0 8.15 AM 0 0 0 0 0 0 0 0 99 0 0 0 6 0 1 0 0 0 151 3 1 0 0 0 151 3 1 0 0 0 151 3 1 1 0 0 0 1 0 0 0 0 0 0 0 0 0 0 0 0																	
8.00 AM 0 0 0 0 0 0 130 0 0 4 0 1 0 0 174 0 0 0 8.15 AM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 181 0 0 0 181 0 0 0 8.15 AM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	8:00 AM 0 0 0 0 0 0 130 0 0 4 0 1 0 0 174 0 0 0 8:15 AM 0 0 0 0 0 0 0 9 9 0 0 6 6 0 1 0 0 0 181 0 0 0 181 0 0 0 8:30 AM 0 0 0 0 0 0 0 2 115 0 0 0 3 0 0 1 1 0 0 0 181 0 0 0 0	8:00 AM 0 0 0 0 0 0 130 0 0 4 0 1 0 0 174 0 0 0 8:15 AM 0 0 0 0 0 0 0 9 9 0 0 0 6 0 1 0 0 0 181 0 0 0 181 0 0 0 8:30 AM 0 0 0 0 0 0 0 2 115 0 0 0 3 0 0 1 0 0 0 151 3 1 1 0 0 0 181 0 0 0 0																	
8.15 AM 0 0 0 0 0 0 99 0 0 0 6 0 1 0 0 181 0 0 181 0 0 0 181 0 0 0 181 0 0 0 181 0 0 0 181 0 0 0 181 0 0 0 0	8:15 AM 0 0 0 0 0 0 99 00 0 6 0 1 0 0 151 3 1 0 0 151 3 1 1 0 0 0 151 3 1 1 0 0 0 151 3 1 1 0 0 0 151 3 1 1 0 0 0 151 3 1 1 0 0 0 151 3 1 1 0 0 0 151 3 1 1 0 0 0 151 3 1 1 0 0 0 151 3 1 1 0 0 1 151 3 1 1 0 0 1 151 3 1 1 0 0 1 151 3 1 1 0 0 1 151 3 1 1 1 0 1 1 1 1 1 1 1 1 1 1 1 1 1 1	Si3 AM 0 0 0 0 99 0 0 6 0 1 0 0 131 0 0 0																	
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8.845 M 0 0 0 0 0 129 0 0 2 0 2 0 0 175 1 0	S.85 AM 0 0 0 0 0 129 0 0 2 0 2 0 0 175 1 0	Sids AM			-						-			-					
7.15 AM - 8.15 AM 0	7.15 AM - 8.15 AM 0	7.15 AM - 8.15 AM 0																	
7.15 AM - 8.15 AM 0	7.15 AM - 8.15 AM 0	7.15 AM - 8.15 AM 0	2-hour Total															15	
Peak Hour Factor	Peak Hour Factor	Peak Hour Factor																	
On Street Bicycle Count O O O O O O O O O	Ostraret Bicycle Count O O O O O O O O O	Ostraret Bicycle Count O O O O O O O O O	Peak Hour Factor					0.375	0.874			0.611		0.500			0.863	0.450	
C15 AM to 8:15 AM 0	C7:15 AM to 8:15 AM C	C135 AM to 8:15 AM	Heavy Vehicle Percentage					0.00%	3.12%			0.00%		8.33%			2.00%	6.67%	0.00%
(7.15 AM to 8:15 AM) * 7:15 AM - 8:15 AM Peak Hour represents the corridor peak not the intersection peak. Culebra Rd NW 34th St Culebra Rd Southbound Northbound Eastbound Time S8/W** S8/E W8/N W8/S N8/W N8/E E8/N E8/S Pedestrians Counts Pedestrians Counts Pedestrians Counts **Oo 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	C15 AM to 8:15 AM Peak Hour represents the corridor peak not the intersection peak.	C15 AM to 8:15 AM Peak Hour represents the corridor peak not the intersection peak.		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Culebra Rd NW 34th St Culebra Rd NW 34th St Earlbound Northbound Eartbound Northbound Northbound Northbound Eartbound Northbound	Culebra Rd	Culebra Rd Southbound Culebra Rd New Status Culebra Rd Eastbound Eastbound Eastbound Eastbound Eastbound Culebra Rd Eastbound Eastbound Eastbound Eastbound Eastbound Culebra Rd Eastbound Eastbound Culebra Rd Eastbound Culebra Rd Eastbound Culebra Rd Eastbound Culebra Rd Cule																	
Culebra Rd Northbound Northbound Eastbound Northbound North	Culebra Rd	Culebra Rd	* 7:15 AM - 8:15 AM PEAK HOL	ur represe	nts the corn	oor peak n	ot the inter	section peak									•		1
Southbound Westbound Northbound Eatbound Eatbound Southbound Eatbound Southbound Southbound Eatbound Southbound Southbound Eatbound Southbound S	Southbound Westbound Northbound Eastbound Southbound Eastbound Southbound Eastbound Southbound Eastbound Southbound Eastbound Southbound Southboun	Southbound Westbound Northbound Eastbound Eastbound Eastbound Southbound Eastbound Southbound Eastbound Southbound Eastbound Southbound Eastbound Southbound Southbound				Culol	hra Pd	NDA/	2.4+h C+	Culol	ara Pd	T	Peak nour	s volume L	Jiagram				
Time	Time	Time		Sout	hbound														
Pedestrians Counts 7.00 AM to 8:00 AM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Pedestrians Counts 7.00 AM to 8:00 AM	Pedestrians Counts 7.00 AM to 8:00 AM	Time										0	0	0	0			
7:00 AM to 8:00 AM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	7.00 AM to 8:00 AM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	7.00 AM to 8:00 AM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		,				,.,	,-	,					→			_	1
8:00 AM to 9:00 AM 0 0 0 0 0 0 2 0 0 0 0 0 0 0 0 0 0 0 0	8:00 AM to 9:00 AM 0 0 0 0 0 0 2 0 0 0 0 0 0 0 0 0 0 0 0	8:00 AM to 9:00 AM 0 0 0 0 0 0 2 0 0 0 0 0 0 0 0 0 0 0 0	7:00 AM to 8:00 AM	0				0	0	0	0								
700.0 M to 8:00 AM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	7:00.AM	7:00.AM	8:00 AM to 9:00 AM	0	0	0	0	0	0	2	0				0	1			
8:00 AM to 9:00 AM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	8:00 AM to 9:00 AM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	8:00 AM to 9:00 AM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		•	Bicyc	le Counts -	On Sidewal	lk							808	→	Ī		
** SB/W - Pedestrian/cycle travelling southbound crossing eastbound approach (West leg) Diagram for: AM Peak Hour	** SB/W - Pedestrian/cycle travelling southbound crossing eastbound approach (West leg) Diagram for: AM Peak Hour	** SB/W - Pedestrian/cycle travelling southbound crossing eastbound approach (West leg) Diagram for: AM Peak Hour													9	<u> </u>	1		
										0	0								7
			,						-8/										

Project Number: 30900399A CoSA Culebra Road Corridor Study TX 1604 to I-10 North/South Street: East/West Street: **115D** AM TURNING MOVEMENT COUNTS Site Code: UnSig_14 hrough 226 288 354 387 268 276 229 7:00 AM 7:15 AM 7:30 AM 7:45 AM 8:00 AM 8:15 AM 8:30 AM 117 155 177 207 196 196 162 8:45 AM 180 1390 266 2294 5 1 7 0.417 0.250 0.438 0.00% 0.00% 0.00% 0 0 0 8 2294 6 4 1297 4 0.500 0.838 0.500 0.00% 2.01% 0.00% 7:15 AM - 8:15 AM* Peak Hour Factor 7:15 AM - 8:15 AM* Peak Hour Factor Heavy Vehicle Percentage OnStreet Bicycle Count (7:15 AM to 8:15 AM) 7:15 AM - 8:15 AM Peak Hour 0 0 0 0 0 0 0 0 0 1 735 2 0 Neff St Southbound Culebra Rd Westbound Neff St Northbound Culebra Rd Eastbound SB/W** SB/E WB/N WB/S NB/W NB/E EB/N EB/S ← 735 ↓ 2 G 0 ↑ ← ↑ → 7:00 AM to 8:00 AM 8:00 AM to 9:00 AM AM Peak Hour 7:15 AM - 8:15 AM*

7:00 AM 7:15 AM 7:30 AM 7:45 AM 8:00 AM 8:15 AM 8:30 AM	Left 0 0 0	Date: N Hamil South Through 0 0			N Hamilton A Culebra Ro Raw Data:						Sit	e Code: Un	Sia 15				
ast/West Street: OD: AM 7:00 AM 7:15 AM 7:30 AM 7:30 AM 8:00 AM 8:15 AM 8:30 AM 8:30 AM	Left 0 0 0	N Hamil South Through 0	ton Ave bound Right	U-Turn	Culebra Ro Raw Data:	J:\\5.1 Data Co Culebra					Sit	e Code: Un	Sig 15				
00: AM ime 7:00 AM 7:15 AM 7:30 AM 7:45 AM 8:00 AM 8:15 AM 8:30 AM	Left 0 0 0	N Hamil South Through 0	ton Ave bound Right	U-Turn	Raw Data:	J:\\5.1 Data Co Culebra							J-6_1J				
7:00 AM 7:15 AM 7:30 AM 7:45 AM 8:00 AM 8:15 AM 8:30 AM	Left 0 0 0	N Hamil South Through 0	ton Ave bound Right	U-Turn		Culebra											
7:00 AM 7:15 AM 7:30 AM 7:45 AM 8:00 AM 8:15 AM 8:30 AM	0 0 0	South Through 0 0	Right 1		Left			Raw Count Da	sta\From GRA	AM\12 30 202 N Hamil		VIC\WSP - 12-	33713 - Culeb	ra Rd - TMCs Culeb	04		
7:00 AM 7:15 AM 7:30 AM 7:45 AM 8:00 AM 8:15 AM 8:30 AM	0 0 0	Through 0 0	Right 1		Left						bound			Eastb			
7:00 AM 7:15 AM 7:30 AM 7:45 AM 8:00 AM 8:15 AM 8:30 AM	0 0 0	0	1			Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	
7:15 AM 7:30 AM 7:45 AM 8:00 AM 8:15 AM 8:30 AM	0 0 0	0			0	147	3	0	0	0	8	0	0	244	3	0	1
7:30 AM 7:45 AM 8:00 AM 8:15 AM 8:30 AM	0			0	0	158	2	0	0	0	19	0	0	289	5	0	
8:00 AM 8:15 AM 8:30 AM			0	0	0	184	2	0	0	0	15	0	0	374	8	0	
8:15 AM 8:30 AM		0	1	0	0	209	0	1	0	0	16	0	0	363	12	0	
8:30 AM	0	0	4	0	2	198	1	0	0	0	14	0	0	302	12	0	
	0	0	1	0	0	204	0	0	0	0	11	0	0	276	5	0	
	0	1	0	0	3	180	0	0	0	0	20	0	0	262	3	0	
	0	0	1	0	2	188	1	0	0	0	17	0	0	291	1	0	
	0	0	9	0	7 2	1468 749	9 5	1	0	0	120 64	0	0	2401 1328	49 37	0	
7:15 AM - 8:15 AM* Peak Hour Factor	U	U	6 0.375	U	0.250	0.896	0.625	0.250	0	0	0.842	U	U	0.888	0.771	0	-
Heavy Vehicle Percentage		0.00%	0.00%		0.230	3.61%	11.11%	0.230			2.50%			2.08%	0.00%		
On Channel Districts Course																	1
(7:15 AM to 8:15 AM)	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
7:15 AM - 8:15 AM Peak Hour re	epresent	s the corri	dor peak no	t the inters	ection peak.												•
										Peak Hour	s Volume D	iagram		1	5		
	N Hamilt		Culeb			lton Ave		ra Rd						+	749		
	Southb		Westb			bound		ound						<u> </u>	2		
ime SB	3/W**	SB/E	WB/N	WB/S	NB/W	NB/E	EB/N	EB/S		- 6	0	0	0	G,	1		
:00 AM to 8:00 AM	0	0	edestrians 1	Counts	0	0	0	0		+	+	→	<u>t</u>	t)	0	0	→ 64
	0	0	0	0	1	0	0	0				0	1	U	U	U	64
:00 AW to 9:00 AW	U		e Counts - 0			U		U				1328	<u>'</u>				
:00 AM to 8:00 AM	0	0	0	0	0	0	0	0				37	-				
	0	0	0	0	0	0	0	0					•				
* SB/W - Pedestrian/cycle travelling	ing south	bound cro	ssing eastb	ound appro	ach (West l	eg)		1		ı	Diagram for	:	AM Peak H	lour]	
											Peak Hour		7:15 AM -	8:15 AM*			

APPENDIX C Project Number: 30900399A CoSA Culebra Road Corridor Study TX 1604 to I-10 AM TURNING MOVEMENT COUNTS WSD North/South Street: East/West Street: Site Code: UnSig 16 AM Through 107 137 Through Right U-Turn 7:00 AM 7:15 AM 7:30 AM 7:45 AM 8:00 AM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 2 3 0 271 294 290 341 250 208 207 0 0 0 6 2 1 4 13 4 159 162 157 0 0 0 0 0 0 8:15 AM 8:30 AM 129 141 8:45 AM 227 2088 615 0.949 2.13% 7:15 AM - 8:15 AM* **9** 0.450 **22** 0.423 1175 0.861 Heavy Vehicle Percentage 1.39% 15.38% 16.67% 6.45% OnStreet Bicycle Count (7:15 AM to 8:15 AM) 7:15 AM - 8:15 AM Peak Hou 0 0 0 0 0 0 0 0 0 0 0 0

| Southbound | Culebra Bd | Grissom Park Dr | Culebra Rd | Eastbound | Soluthound | Eastbound | Soluthound | Soluthound | Eastbound | Soluthound | Soluthound | Eastbound | Soluthound | So

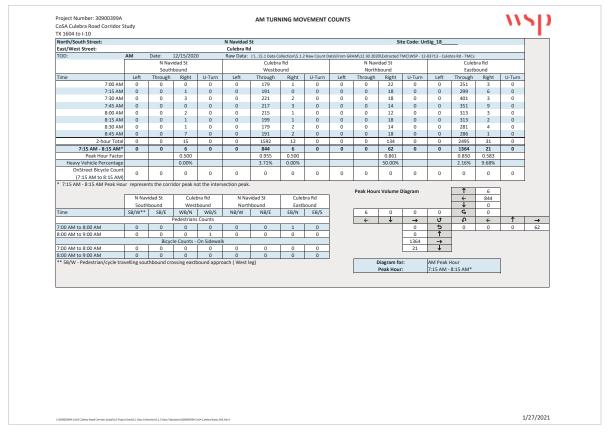
** SB/W - Pedestrian/cycle travelling southbound crossing eastbound approach (West Ieg)

1/j30900399A CoSA Cullebra Road Corridor Study(S.0 Project Data)(S.1 Data Collection)(S.1.3 Data Tabulation)(30900399A CoSA Cullebra Road_AM_Rev

1/27/2021

North/South Street:											Sit	e Code: Un	Sig 17			
East/West Street:					Culebra R	d										
TOD:	AM	Date:	12/15/202	0	Raw Data:	J:\\5.1 Data C		Raw Count D	ata\From GR			MC\WSP - 12-	03713 - Cule			
						Culebr					!8th St				ora Rd	
			bound			Westb					bound				ound	
Time	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Tu
7:00 AM	0	0	0	0	0	73	0	0	1	0	6	0	0	149	1	0
7:15 AM	0	0	0	0	1	94	0	0	2	0	3	0	0	162	0	0
7:30 AM	0	0	0	0	1	113	0	0	3	0	6	0	0	236	1	0
7:45 AM	-	0	0	0	-	111	0	0	5	0	4	0	-	209	-	0
8:00 AM 8:15 AM	0	0	0	0	0	126 110	0	0	4	0	2	0	0	169 141	0	0
8:30 AM	0	0	0	0	0	94	0	0	2	0	4	0	0	157	0	0
8:45 AM	0	0	0	0	0	109	0	0	2	0	4	0	0	163	0	0
2-hour Total	0	0	0	0	3	830	0	0	20	0	37	0	0	1386	2	0
7:15 AM - 8:15 AM*	0	0	0	0	2	444	0	0	14	0	15	0	0	776	1	0
Peak Hour Factor		_			0.500	0.881	-	-	0.700	-	0.625	-		0.822	0.250	·
Heavy Vehicle Percentage					0.00%	2.29%			0.00%		0.00%			1.59%	0.00%	
OnStreet Bicycle Count	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
(7:15 AM to 8:15 AM)	0	0	U	0	U	0	0	0	U	U	U	U	U	U	U	U
* 7:15 AM - 8:15 AM Peak Hou	ir represe	nts the corr	idor peak n	ot the inter	section peak											,
									,	Peak Hour	rs Volume [Diagram			0	
				bra Rd		28th St		ora Rd oound						+	444	
Time	SB/W**	nbound SB/E	WB/N	bound WB/S	NB/W	hbound NB/E	EB/N	EB/S		0	0	0	0	4	2	
Time	2B/W**		Pedestrians		NB/W	NB/E	EB/N	EB/S		÷	1	→	UT.	£	÷	4
7:00 AM to 8:00 AM	0	0	0	0	0	1	0	0		+	*	0	5	0	14	0
8:00 AM to 9:00 AM	0	0	1	0	0	0	0	0				0	1	- 0	14	
0.00 File to 5.00 AIN				On Sidewal								776	→			
7:00 AM to 8:00 AM	0	0	0	0	0	0	0	0				1	Ť	1		
8:00 AM to 9:00 AM	0	0	0	0	0	0	0	0								
** SB/W - Pedestrian/cycle tra	velling sou	thbound cr	ossing eastl	ound appr	oach (West I	leg)					Diagram for	r:	AM Peak I	Hour		
											Peak Hour		7:15 AM -	8:15 AM*		
																•

21/200002099A CoSA Culebra Road Corridor Study/S-D Project Data/S-1 Data Collection/S-1.2 Data Tabulation/20000209A CoSA Culebra Road_AM_Revi

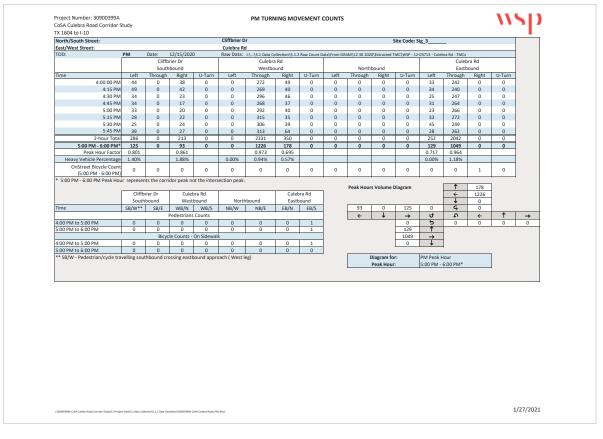


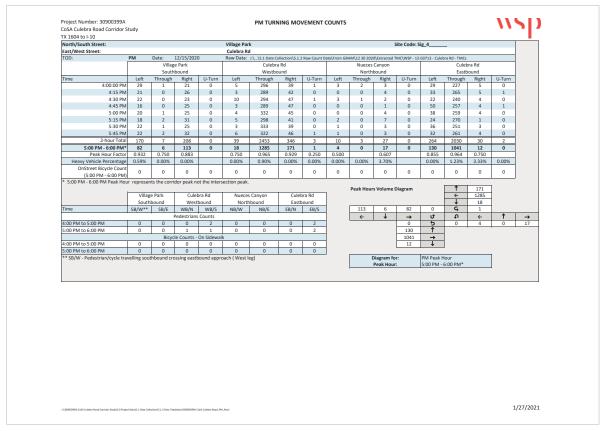


Project Number: 30900399A CoSA Culebra Road Corridor Study **115D** PM TURNING MOVEMENT COUNTS TX 1604 to I-10 North/South Street East/West Street: U-Turn 0 Right 34 48 51 63 48 45 45 45 Right 60 61 68 72 102 74 86 76 hrough 286 255 Left 18 15 14 9 15 19 19 4:00 PM 4:15 PM 49 57 66 59 82 86 64 114 63 68 4 215 217 217 233 259 257 247 279 255 4:30 PM 4:45 PM 5:00 PM 5:15 PM 269 250 262 242 17 22 20 16 20 7 53 55 51 53 57 69 0 0 0 0 0 321 242 143 **63** 1962 368 1038 172 26 14 602 **309** 41 15 599 **338** 230 2127 1067 47 23 5:00 PM - 6:00 PM* Peak Hour Factor Heavy Vehicle Percentage 0 0 0 0 0 0 14 1067 230 Culebra Rd Eastbound EB/N EB/S ð. 4:00 PM to 5:00 PM 5:00 PM to 6:00 PM 1038 172 **→**

Project Number: 30900399A CoSA Culebra Road Corridor Study TX 1604 to I-10 North/South Street: PM TURNING MOVEMENT COUNTS Les Harrison D Site Code: Sig 2 12/15/202 Les Harrison Dr Southbound Apartments Drive Northbound U-Turn 0 0 Right
41
52
58
68
47
55
67 hrough 247 Right 33 48 38 64 51 58 64 Left 66 63 76 61 79 74 86 69 4:00 PN 282 16 16 0 4:15 PM 4:30 PM 4:45 PM 253 223 0 291 283 211 0 0 0 0 262 5:00 PM 5:15 PM 257 264 274 254 0 5:30 PN 5:45 PN 281 261 271 1099 0.925 0.50% 2 19 0.250 0.528 0.00% 0.00% 308 1060 19 0.895 0.967 0.528 0.52% 0.90% 0.00% 5:00 PM - 6:00 PM* 141 236 0.881 **22** 0.688 **221** 0.863 12 0.74% 0 0 0 0 0 0 0 0 0 0 0 0 0 0 ↑ 221 ← 1099 ↓ 22 **Ģ** 2 Culebra Rd Eastbound SB/W** SB/E WB/N WB/S NB/W NB/E EB/N EB/S **↑** ← 0 19 ↑ → 3 12 1 308 1060 19 PM Peak Hour 5:00 PM - 6:00 PM*

1/27/2021





TX 1604 to 1-10 Month/South Street:	Mainst Driveway Site Code: Sig_5	Project Number: 30900399A CoSA Culebra Road Corridor S						PM TURN	NING MO	VEMENT (COUNTS							11	SD)
East/West Street: Culebra Rd Saw Data: 12/15/2020 Raw Data: 12/15/2020 Saw Data:	Section Column																		
Time	Dota: 12/15/2020 New Date: 12/15/2020 New Date: 14.35.10 has collection(\$1.2 kew Count both/from GRAM(12 20 2020) Station Driveway Southbound Station University					V							9	ite Code: S	ig_5	_			
Culebra Rd	Walmart Driveway South-Dund South-Dun		DNA	Datas	12/15/2020	`						1101222222	W.E	10011100 10	02742 0 1 1	0.1 70.40			
Time	Southbound Westbound Westbound Southbound Westbound W	IUD.	PIVI			,	Naw Data:			Kaw Count D	ata\From GK				03/13 - Culeo				
Time	Inter																		
## 4:00:00 PM	4:00:00 PM 8	Time	Left			U-Turn	Left			U-Turn	Left			U-Turn	Left			U-Turn	t
4-30 PM 13	4-30 PM 33																		İ
A43F M 13	4.45 PM 33 0 2.4 0 5 313 27 0 7 3 6 0 23 228 7 0 0 5.50 PM 22 1 38 0 6 343 24 0 4 1 9 0 21 267 10 0 5.15 PM 15 1 33 0 7 298 31 0 7 3 10 0 27 243 9 0 5.30 PM 10 5 34 0 18 346 29 0 7 1 10 0 22 243 7 0 5.45 PM 10 3 32 0 4 332 30 0 16 0 11 0 22 253 7 0 5.45 PM 10 3 32 0 4 332 30 0 16 0 11 0 22 253 7 0 5.40 PM 50 105 12 258 0 67 2557 223 0 57 10 69 0 203 1991 58 0 5.00 PM - 6.00 PM 57 10 137 0 35 1199 114 0 34 5 40 0 98 1016 33 0 Feak Hour Facto 0.648 0.500 0.050 0.396 0.456 0.053 0.919 0.531 0.417 0.090 0.875 0.951 0.255 Feak Hour Facto 0.648 0.500 0.050 0.396 0.456 0.053 0.019 0.006 0.006 0.006 0.006 0.006 Feak Hour Facto 0.648 0.500 0.006 0.396 0.456 0.006	4:15 PM	14	0	28	0	7	293	31	0	5	1	8	0	23	252	5	0	
S-00 PM 22	S-00 PM 22			1		0	8			0	5	0	6	0			7	0	
S15PM 15	S-15 PM 15																		
S-30 PM 10 S 34 0 18 346 29 0 7 1 10 0 28 243 7 0 0	S-30 PM 10 S 34 0 18 346 29 0 7 1 10 0 28 243 7 0																		
S.45 PM 10 3 32 0 4 332 30 0 16 0 11 0 22 253 7 0	S-45 PM 10 3 3 2 0 4 332 30 0 16 0 11 0 22 263 7 0 0																		
2-hour Total 105 12 258 0 67 2557 23 0 57 10 69 0 203 1991 58 0 5 50 PM -6:00 PM -0:00 -0:00 -0:00 PM -0:00 -0:00 PM -0	2-hour Total 105 12 258 0 67 2557 223 0 57 10 69 0 203 1991 58 0 500 M - 6.00 PM - 6.00 PM - 57 10 137 0 35 1319 114 0 34 5 40 0 98 1016 33 0 Peat Hour Factor 0 .648 0.500 0.901 0 .486 0.953 0.919 0 .531 0.417 0.909 0 .0.75 0.951 0.825 10.000 0.000 0.000 0 0 0 0 0 0 0 0 0 0																		
S.00 PM - 6.00 PM S.7	Side PM - 6.00 PM \$7 10 137 0 35 1319 114 0 34 5 40 0 98 1016 33 0					_												_	+
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On Street Bicycle Count O O O O O O O O O	On Street Bicycle Count C SOD PM - SOD PM																		
SoD PM - 6.00 PM Pask Hour represents the corridor peak not the intersection peak. Culebra Rd Southbound South	Sol PM -6:00 PM Peak Hour represents the corridor peak not the intersection peak Sol PM -6:00 PM Peak Hour represents the corridor peak not the intersection peak Walmart Driveway Sol PM -6:00 PM Sol PM -6:00 PM -6:00 PM Sol PM -6:00 PM -6:00 PM Sol PM -6:00 P									_									İ
Walmart Driveway South Surface Mark Surface Mar	Walmart Driveway Culebra Rd Southbound Westbound Southbound	1						0	0	0	0	0	0	0	0	0	0		
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Sicycle Counts - On Sidewalk	Bicycle Counts - On Sidewalk 1016 →															U	34	3	40
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** SB/W - Pedestrian/cycle travelling southbound crossing eastbound approach (West leg) Diagram for: PM Peak Hour	* SB/W - Pedestrian/cycle travelling southbound crossing eastbound approach (West leg) Diagram for: PM Peak Hour	5.00 PM to 6.00 PM						0	0	2				33	1	1			
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Peak Hour: 5:00 PM- 6:00 PM*	Peak Hour: 5:00 PM - 6:00 PM*	4:00 PM to 5:00 PM		0															
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TX 1604 to I-10 North/South Street:					Culebra R	1						Site Cor	de: Sig 6				_
East/West Street:					Culculati	•						Site Cot	ie. sig_u				
TOD:	PM		12/15/202	0	Raw Data:	J:\\5.1 Data O	ollection\5.1.	2 Raw Count E	Data\From GR			MC\WSP - 12	-03713 - Culeb	ıra Rd - TMCs			
			bra Rd								ora Rd				a Creek		
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Time	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left 7	Through	Right	U-Turn	Left	Through	Right	U-Turn	
4:00:00 PM 4:15 PM	0	242 260	24 43	0	0	0	0	0	14	287 375	0	0	24 14	0	11 7	0	
4:30 PM	0	261	38	0	0	0	0	0	23	421	0	0	20	0	5	0	
4:45 PM	0	275	29	0	0	0	0	0	16	409	0	0	26	0	19	0	
5:00 PM	0	276	34	0	0	0	0	0	21	415	0	1	35	0	12	0	
5:15 PM	0	268	39	0	0	0	0	0	31	404	0	0	22	0	16	0	
5:30 PM	0	284	33	0	0	0	0	0	16	426	0	0	19	0	12	0	
5:45 PM	0	255	31	0	0	0	0	0	32	422	0	0	21	0	11	0	
2-hour Total	0	2121	271	0	0	0	0	0	160	3159	0	1	181	0	93	0	_
5:00 PM - 6:00 PM*	0	1083	137	0	0	0	0	0	100	1667	0	1	97	0	51	0	
Peak Hour Factor		0.953	0.878		0.00%				0.781	0.978		0.250	0.693		0.797		
Heavy Vehicle Percentage									1.25%								
OnStreet Bicycle Count (5:00 PM - 6:00 PM)	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
* 5:00 PM - 6:00 PM Peak Hou	ir represei	nts the corr	idor peak n	ot the inters	ection peak.											7	
	6.1.1	bra Rd	1			bra Rd		ia Creek	Т	Peak Hour	rs Volume I	Diagram		+ T	0	-	
		bra Ku ibound	West	bound		nbound		bound						Ţ	0	-	
Time	SB/W**	SB/E	WB/N	WB/S	NB/W	NB/E	EB/N	EB/S		137	1083	0	0	Ġ	0	1	
			Pedestrians						1	+	+	→	Ů.	t t	+	1	→
4:00 PM to 5:00 PM	0	0	0	0	0	1	0	1	1			0	5	1	100	1667	0
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				On Sidewal								0	→				
4:00 PM to 5:00 PM 5:00 PM to 6:00 PM	0	0	0	0	0	0	0	0				51	↓				
** SB/W - Pedestrian/cycle tra							U	U	J		Diagram fo		PM Peak H	lour		1	
3b) w - redestrian/cycle tra	veiiiig sou	tiibouiiu ci	ossiiig easti	oound appro	Jacii (west i	cg)					Peak Hour		5:00 PM -				

Project Number: 30900399A CoSA Culebra Road Corridor Study TX 1604 to I-10 PM TURNING MOVEMENT COUNTS WSD Baseball Field Driveway North/South Street: East/West Street: Site Code: Sig 7 Date: 12/15/2020 Baseball Field Driveway Southbound Northbound Westb Through 271 243 251 257 311 242 232 4:00:00 PM 4:15 PM 4:30 PM 4:45 PM 5:00 PM 5:15 PM 5:30 PM 5:45 PM Through Right U-Turn 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 1 0 201 194 197 195 201 208 185 0 0 0 3 0 2 0 0 5 14 16 11 0 3 0 14 15 14 0 0 214 1595 5:45 PM 244 2051 1029 0.827 4.24% 5:00 PM - 6:00 PM* 48 35 808 Heavy Vehicle Percentage 27.27% 0.00% 90.91% 1.50% 0.00% 24.00% OnStreet Bicycle Count (5:00 PM - 6:00 PM) 5:00 PM - 6:00 PM Peak Hou 0 0 0 0 0 0 0 0 3 0 0 0 0 6 1029 0 0 | Baseball Field | Culebra Rd | Driveway | Culebra Rd | Driveway | Westbound | Northbound | Eastbound | SB/W** | SB/E | WB/N | WB/S | NB/W | NB/E | EB/N | EB/S | PM Peak Hour 5:00 PM - 6:00 PM*

orth/South Street: Pettus St Culebra Rd Culebra R	Project Number: 30900399A CoSA Culebra Road Corridor TX 1604 to I-10						PM TUR	NING MO	VEMENT	COUNTS							115
PM Date 12/15/2020 Raw	North/South Street:					Pettus St							Site Coo	le: Sig 8			
Pettus St	East/West Street:																
Memory Memory	TOD:	PM			0	Raw Data:			Raw Count D	ata\From GR/			MC\WSP - 12	03713 - Culei			
Metal According to According t																	
### 4.0000 PM 13	_																
# 15 PM 6 0 0 0 1 218 20 0 2 0 4 0 2 215 0 0 0 4.5 PM 13 0 1 0 9 211 30 0 2 1 4 0 0 0 193 2 0 0 0 0 0 0 0 0 0																	
4.30 PM 7 0 0 0 0 1 242 14 0 0 0 0 1 0 3 199 4 0 0 4 6.85 PM 13 0 1 1 0 9 211 30 0 0 2 1 1 4 0 0 0 133 199 4 0 0 5.00 PM 9 0 4 0 4 239 21 0 0 2 0 3 0 5 200 3 0 5 5.00 PM 7 1 3 0 0 2 2 60 2 0 3 0 5 200 3 0 0 5 5.30 PM 6 1 1 6 0 1 1 218 29 0 2 2 0 3 0 1 205 6 0 0 5.30 PM 6 1 1 6 0 0 1 218 29 0 0 2 1 1 3 0 0 0 218 4 0 0 5 5.30 PM 9 1 2 0 0 6 232 17 0 0 1 0 2 0 1 1 155 8 0 0 2 0 0 2 0 1 3 10 1 205 6 0 0 2 0 0 2 0 1 3 1 100 0 0 0 2 0 0 1 3 1 100 0 2 0 1 1 1 155 8 0 0 1 2 0 0 0 2 0 1 1 1 1 155 8 0 0 1 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0																	
S.O.PM 9																	
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S-545 FM 9 1 2 0 6 232 17 0 1 0 2 0 1 195 8 0																	
2-hour Total 70 3 2 0 0 28 1851 182 0 12 2 2 26 0 13 1609 30 0 1 S500 PM-600 PM* 31 3 15 0 13 869 95 0 7 1 11 0 7 818 21 0 Peak Hour Factor 0.861 0.750 0.625 0.562 0.913 0.819 0.875 0.250 0.917 0.350 0.938 0.656 Meany Yelkick Porcused 0.00% 0.																	
Stop PM - 6:00 PM 31 3 15 0 13 3-949 95 0 7 1 11 0 7 818 21 0																	
Peak Hour Factor 0.861											_			_			
Heavy Vehicle Percentage 0.00% 0					U				U				U				J
OnStreet Bicycle Count																	
Pettus St Southbound Southbound Southbound Northbound Southbound Northbound Southbound Northbound Southbound Northbound Southbound Northbound Southbound Sou	OnStreet Bicycle Coun	t o	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0
Pettus St	* 5:00 PM - 6:00 PM Peak Ho	ur represer	nts the corr	idor peak n	ot the inters	section peak.											
Southbound Westbound Northbound Eastbound Eastbound Southbound Eastbound Southbound Eastbound Southbound Eastbound Southbound Southbou											Peak Hour	s Volume I	Diagram		1		
me																	
Pedestrians Counts	Time										15	3	31	0			
00 PM to 5.00 PM		30/11				140/44	140/2	20/14	20/5				_				↑ →
Bicycle Counts - On Sidewalk	4:00 PM to 5:00 PM	0				0	0	0	0								
00 PM to 500 PM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	5:00 PM to 6:00 PM	0					0	0	0								,
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S8/W - Pedestrian/cycle travelling southbound crossing eastbound approach (West leg) Diagram for: Peak Hour: 5:00 PM - 6:00 PM*													21	+	l		
Peak Hour: 5:00 PM - 6:00 PM*												Diagram fo	r:	PM Peak H	lour		
	,						-0/										
DRODOWN CISA Cubes have Control transplic Design Control transplication Design Control																	1/27/20;

North/South Street:					NW 36th S	t						Site Cod	de: Sig_9				
East/West Street:					Culebra Ro												
TOD:	PM		12/15/2020	1	Raw Data:	J:\\5.1 Data Ci		Raw Count D	Data\From GRA			MC\WSP - 12-	03713 - Culeb				
			6th St bound			Culebra				Esmer	elda Dr				ora Rd oound		
lime .	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	
4:00:00 PM	37	67	29	0	16	218	30	0	25	57	13	0	19	175	20	0	
4:15 PM	45	86	38	0	17	198	25	0	23	59	10	0	23	187	11	0	
4:30 PM	29	70	32	0	11	205	35	0	32	74	11	0	31	193	13	0	
4:45 PM 5:00 PM	35 32	86 82	41 31	0	19 20	212 232	27 25	0	31 18	63 49	20 12	0	24 33	168 186	32 20	0	
5:15 PM	30	101	31	0	12	245	25	0	36	68	19	0	25	176	16	0	
5:30 PM	30	80	28	0	24	204	22	0	28	54	15	0	27	175	22	0	
5:45 PM	38	105	37	0	30	217	25	0	23	71	15	0	26	151	19	0	
2-hour Total 5:00 PM - 6:00 PM*	276 130	677 368	267 127	0	149 86	1731 898	214 97	0	216 105	495 242	115 61	0	208 111	1411 688	153	0	
Peak Hour Factor	0.855	0.876	0.858	0	0.717	0.916	0.970	U	0.729	0.852	0.803	0	0.841	0.925	77 0.875	U	
Heavy Vehicle Percentage	0.72%	0.30%	1.12%		0.00%	1.27%	0.93%		0.46%	0.00%	0.87%		0.48%	1.77%	0.65%		
OnStreet Bicycle Count	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
(5:00 PM - 6:00 PM) 5:00 PM - 6:00 PM Peak Hou																	
5:00 PIVI - 6:00 PIVI PEAK HOU	represe	its the com	not beak tic	it the inters	есноп реак.					Peak Hour	s Volume F	Niagram		_	97	1	
ĺ	NW:	36th St	Culeb	ra Rd	Esmer	alda Dr	Culet	ıra Rd	1	i cun i ioui	3 401011110 2	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		+	898		
		bound		ound		bound		ound						+	86	1	
Time	SB/W**		WB/N	WB/S	NB/W	NB/E	EB/N	EB/S	4	127	368	130	0	G.	0	1	
1:00 PM to 5:00 PM	0	0	Pedestrians 1	Counts	0	0	2	0		+	↓	→	t t	ů ů	← 105	T 242	→ 61
5:00 PM to 6:00 PM	0	0	1	0	0	0	1	0	1			111	Ť	-	103	242	01
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1:00 PM to 5:00 PM	0	0	0	0	0	0	0	0				77	+	ļ			
5:00 PM to 6:00 PM ** SB/W - Pedestrian/cycle trav	0	0	0	0 ound appro	0	0	0	0	J		Diagram for	,	PM Peak H	lour		1	
3b) W - Fedestriany cycle tras	vening sou	tribouriu cre	Jasiiig eastu	ошни аррго	Jacii (west ii	=B <i>)</i>					Peak Hour		5:00 PM -				
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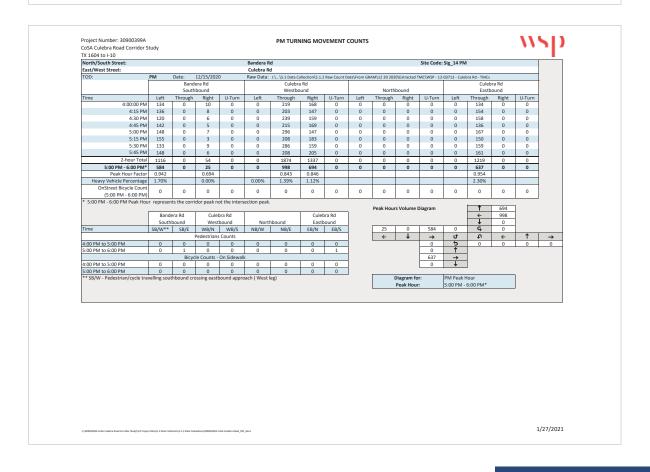
CoSA Culebra Road Corridor: TX 1604 to I-10	študy															• • •	117
North/South Street:					Rattler Dr							Site Code	e: Sig_10				
East/West Street:					Culebra Rd												
TOD:	PM		12/15/2020)	Raw Data:	J:\\5.1 Data Co		Raw Count D	ata\From GRA			MC\WSP - 12-	03713 - Culel				
		Rattl				Culebra				Memo				Culeb			
			bound			Westbo				North				Eastb			
Time	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	
4:00:00 PM		0	0	0	7	259	1	0	8	0	7	0	0	198	8	0	
4:15 PM		0	0	0	14	242	0	0	12	0	20	0	0	230	11	0	
4:30 PM		0	1	0	9	222	2	0	15	0	14	0	0	203	10	0	
4:45 PM		0	3	0	6	249	2	0	7	0	9	0	0	216	7	0	
5:00 PM		0	4	0	11	286	1	0	5	0	8	0	1	219	6	0	
5:15 PM 5:30 PM		0	0	0	10 10	279 267	0 2	0	12 2	0	7	0	0	227 200	7 6	0	
5:30 PM 5:45 PM		0	6	0	11	273	0	0	11	0	5	0	1	197	13	0	
2-hour Tota		0	14	0	78	2077	8	0	72	0	77	0	4	1690	68	0	
5:00 PM - 6:00 PM*		0	10	0	42	1105	3	0	30	0	27	0	4	843	32	0	
Peak Hour Factor		+ •	0.417		0.955	0.966	0.375		0.625		0.844		0.500	0.928	0.615		
Heavy Vehicle Percentage			0.00%		0.00%	1.20%	0.00%		0.00%		0.00%		0.00%	1.66%	1.47%		
OnStreet Bicycle Count		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
(5:00 PM - 6:00 PM)) 0					U	U	U	U	U	U	U	U	U	U	U	
* 5:00 PM - 6:00 PM Peak Ho	ur represer	nts the corri	dor peak no	ot the inters	ection peak.											_	
										Peak Hour	s Volume [iagram		1	3		
		tler Dr	Culeb			orial St		ora Rd						+	1105		
		hbound		bound		bound		oound						<u> </u>	42		
Time	SB/W**		WB/N	WB/S	NB/W	NB/E	EB/N	EB/S		10	0	5	0	G.	0		
4:00 PM to 5:00 PM	0	0	Pedestrians	Counts	0		2	0		+	1	→	<u>t</u>	0 Tu	←	0	→ 27
5:00 PM to 6:00 PM	0	0	1	0	0	0	1	0				4	1	U	30	U	21
3.00 FWI to 0.00 FWI	_ 0		le Counts -			- 0						843					
4:00 PM to 5:00 PM	0	0	0	0	0	0	1	1				32	Ţ	i			
5:00 PM to 6:00 PM	0	0	0	0	0	0	0	0						'			
** SB/W - Pedestrian/cycle tra	velling sou	thbound cro	ssing eastb	ound appro	oach (West le	eg)		•			Diagram fo	r:	PM Peak F	lour			
											Peak Hour		5:00 PM -	6:00 PM*			

Project Number: 30900399A **115D** PM TURNING MOVEMENT COUNTS CoSA Culebra Road Corridor Study TX 1604 to I-10 North/South Street: East/West Street: Site Code: Sig_11 Right 26 35 26 21 40 21 33 184 230 202 206 205 220 179 0 0 0 0 0 0 0 0 4:00:00 PM 4:15 PM 4:30 PM 4:45 PM 5:00 PM 5:15 PM 5:30 PM 22 26 18 28 24 24 24 25 13 11 13 13 15 13 10 240 226 225 233 275 255 260 11 15 10 12 5:45 PM 235 1949 12 181 2-hour Total 5:00 PM - 6:00 PM* Peak Hour Factor Heavy Vehicle Percentage OnStreet Bicycle Count (5:00 PM - 6:00 PM) 5:00 PM - 6:00 PM Peak Hour 93 785 0.930 0.892 1.07% 1.62% 131 0.819 1.26% 1025 0.932 1.33% 0.00% 0 0 0 0 0 0 0 49 1025 0 0 Camino Santa Maria Culebra Rd Culebra Rd Culebra Rd Southbound Westbound Northbound Eastbound SB/W** SB/E WB/N WB/S NB/W NB/E EB/N EB/S ← 1025 ↓ 0 G 0 ↑ ← ↑ → 4:00 PM to 5:00 PM 5:00 PM to 6:00 PM

Calebra RG	Same Columbia Co	Carly Nest Street:	Culebra Rd	TX 1604 to I-10 North/South Street:					V San Felipe	A						Cit- C-4	Ci- 12				_
Time	Discription 17/15/2020 Raw Date: 17/15	Time	Time					r								Site Cou	e: 51g_12				
N San Felipe Ave Northbound N San Felipe Ave Northbound N San Felipe Ave Northbound Northbound N San Felipe Ave Northbound N San Felipe Ave San	N San Felipe Ave Northbound	N San Felipe Ave NorthPound NorthPound	N San Felipe Ave Northbound N San Felipe Ave Northbound N San Felipe Ave Northbound Northbound N San Felipe Ave Northbound N San Felipe Ave San		PM	Date:	12/15/2020)			ollection\5.1.	2 Raw Count D	lata\From GR	AM\12 30 202	0\Extracted T	MC\WSP - 12-	03713 - Culel	bra Rd - TMCs			1
Time	Mathematics Mathematics	Time	Time																		1
4-00.00 PM 0 0 0 0 0 7 229 0 0 0 16 0 6 0 0 200 5 0 0 4 215 0 0 0 9 0 7 0 0 202 9 0 0 4 30 PM 6 4 30 PM 0 0 0 0 0 4 225 0 0 0 9 0 7 0 0 202 9 0 0 4 4 30 PM 6 0 0 0 0 0 4 225 0 0 0 9 0 7 0 0 202 9 0 0 4 4 30 PM 6 0 0 0 0 0 7 229 0 0 0 15 0 0 9 0 0 200 14 0 0 10 0 10 0 0 10 0 0 10 0 1	## 4:00:00 PM 0 0 0 0 7 229 0 0 15 0 6 0 0 200 5 0 ## 4:15 PM 0 0 0 0 0 4 225 0 0 9 0 7 0 0 202 9 0 ## 4:30 PM 0 0 0 0 0 4 231 0 0 10 0 0 0 210 12 0 ## 4:45 PM 0 0 0 0 0 7 229 0 0 15 0 9 0 0 200 14 0 ## 5:50 PM 0 0 0 0 0 9 273 0 0 15 0 9 0 0 220 14 0 ## 5:50 PM 0 0 0 0 0 0 0 0 0	4-00-00 PM 0 0 0 0 0 7 229 0 0 0 15 0 6 0 0 200 5 0 0 4 325 0 0 9 0 7 0 0 202 9 0 0 4 30 PM 0 0 0 0 0 4 225 0 0 9 0 7 0 0 202 9 0 0 4 30 PM 0 0 0 0 0 0 4 231 0 0 0 10 0 10 0 0 0 210 12 0 0 12 0 0 0 0	4-00.00 PM 0 0 0 0 0 7 229 0 0 0 16 0 6 0 0 200 5 0 0 4 215 0 0 0 9 0 7 0 0 202 9 0 0 4 30 PM 6 4 30 PM 0 0 0 0 0 4 225 0 0 0 9 0 7 0 0 202 9 0 0 4 4 30 PM 6 0 0 0 0 0 4 225 0 0 0 9 0 7 0 0 202 9 0 0 4 4 30 PM 6 0 0 0 0 0 7 229 0 0 0 15 0 0 9 0 0 200 14 0 0 10 0 10 0 0 10 0 0 10 0 1		1	North	bound			Westbo	ound			North	bound			Eastb	oound		i .
4.15 PM 0 0 0 0 0 4 225 0 0 0 0 9 0 7 0 0 202 9 9 0 0 4 33 PM 0 0 0 0 0 0 4 231 0 0 0 10 0 0 10 0 0 220 12 12 0 0 4 4.35 PM 0 0 0 0 0 0 7 229 0 0 0 15 0 9 0 0 0 200 14 4 0 0 15 0 0 9 0 0 0 0 0 0 0 0 0 0 0 0 15 0 0 10 0 0 0	## 4:15 PM	4.15 PM 0 0 0 0 0 4 225 0 0 0 9 0 7 0 0 0 202 9 0 0 4 33 1 0 0 10 0 10 0 0 202 9 0 0 4 4.35 PM 0 0 0 0 0 4 231 0 0 10 0 10 0 0 0 201 0 12 0 12 0 1	4.15 PM 0 0 0 0 0 4 225 0 0 0 0 9 0 7 0 0 202 9 9 0 0 4 33 PM 0 0 0 0 0 0 4 231 0 0 0 10 0 0 10 0 0 220 12 12 0 0 4 4.35 PM 0 0 0 0 0 0 7 229 0 0 0 15 0 9 0 0 0 200 14 4 0 0 15 0 0 9 0 0 0 0 0 0 0 0 0 0 0 0 15 0 0 10 0 0 0	Time	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	
4.30 PM 0 0 0 0 4 231 0 0 0 10 0 12 0 12 0 0 15 0 9 0 0 200 14 0 0 0 5.00 PM 0 0 0 0 0 7 229 0 0 0 15 0 9 0 0 200 14 0 0 5.00 PM 0 0 0 0 0 0 3 270 0 0 0 13 0 10 0 0 0 224 111 0 0 0 5.00 PM 0 0 0 0 0 0 3 270 0 0 0 13 0 10 0 0 0 224 111 0 0 0 5.30 PM 0 0 0 0 0 6 270 0 0 0 8 0 0 4 0 0 201 5 0 0 12 0 5 0 0 174 14 0 0 0 0 0 6 251 0 0 0 12 0 5 0 0 174 14 0 0 0 0 0 0 6 251 0 0 0 12 0 5 0 0 174 14 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	4:30 PM 0 0 0 0 4 231 0 0 10 0 10 0 120 12 0 0 5 0 45 PM 6	4:30 PM 0 0 0 0 0 4 231 0 0 10 0 10 0 0 210 12 0 0 6 6 8 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	4.30 PM 0 0 0 0 4 231 0 0 0 10 0 12 0 12 0 0 15 0 9 0 0 200 14 0 0 0 5.00 PM 0 0 0 0 0 7 229 0 0 0 15 0 9 0 0 200 14 0 0 5.00 PM 0 0 0 0 0 0 3 270 0 0 0 13 0 10 0 0 0 224 111 0 0 0 5.00 PM 0 0 0 0 0 0 3 270 0 0 0 13 0 10 0 0 0 224 111 0 0 0 5.30 PM 0 0 0 0 0 6 270 0 0 0 8 0 0 4 0 0 201 5 0 0 12 0 5 0 0 174 14 0 0 0 0 0 6 251 0 0 0 12 0 5 0 0 174 14 0 0 0 0 0 0 6 251 0 0 0 12 0 5 0 0 174 14 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0																		1
4.45 PM 0 0 0 0 0 7 229 0 0 0 13 0 0 0 0 200 144 0 0 5.00 PM 0 0 0 0 0 0 9 273 0 0 0 13 0 10 0 0 0 224 11 0 0 5.15 PM 0 0 0 0 0 0 3 3 270 0 0 0 9 0 6 0 0 206 7 0 0 5.30 PM 0 0 0 0 0 6 270 0 0 0 8 0 4 0 0 201 5 0 0 5.45 PM 0 0 0 0 0 6 251 0 0 0 12 0 5 0 0 174 146 0 2.45 PM 0 0 0 0 0 0 6 251 0 0 0 12 0 5 0 0 174 146 0 2.45 PM 0 0 0 0 0 0 6 251 0 0 0 12 0 5 0 0 174 146 0 2.45 PM 0 0 0 0 0 0 46 1958 0 0 0 12 0 5 0 0 174 146 0 2.45 PM 0 0 0 0 0 0 0 46 1958 0 0 0 22 0 5 7 0 0 0 1617 77 0 0 2.45 PM 0 0 0 0 0 0 24 1064 0 0 42 0 25 0 0 885 37 0 0 0 1617 77 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	A4.5 PM	4.45 PM 0 0 0 0 0 7 229 0 0 0 15 0 9 0 0 0 200 14 0 0 5.50 PM 0 0 0 0 0 0 9 273 0 0 0 13 0 0 10 0 0 0 224 11 0 0 5.15 PM 0 0 0 0 0 0 3 3 270 0 0 0 9 0 6 0 0 0 266 7 0 0 5.53 PM 0 0 0 0 0 6 270 0 0 0 8 0 4 0 0 0 201 5 0 0 5.45 PM 0 0 0 0 0 6 251 0 0 12 0 5 0 0 174 14 0 0 2.50 PM 0 0 0 0 0 6 251 0 0 12 0 5 0 0 174 14 0 0 2.50 PM 0 0 0 0 0 46 1978 0 0 0 92 0 57 0 0 1617 77 0 0 2.50 PM -6.00 PM 0 0 0 0 0 46 1978 0 0 0 92 0 57 0 0 1617 77 0 0 2.50 PM -6.00 PM Peak Hour Factor 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	4.45 PM 0 0 0 0 0 7 229 0 0 0 13 0 0 0 0 200 144 0 0 5.00 PM 0 0 0 0 0 0 9 273 0 0 0 13 0 10 0 0 0 224 11 0 0 5.15 PM 0 0 0 0 0 0 3 3 270 0 0 0 9 0 6 0 0 206 7 0 0 5.30 PM 0 0 0 0 0 6 270 0 0 0 8 0 4 0 0 201 5 0 0 5.45 PM 0 0 0 0 0 6 251 0 0 0 12 0 5 0 0 174 146 0 2.45 PM 0 0 0 0 0 0 6 251 0 0 0 12 0 5 0 0 174 146 0 2.45 PM 0 0 0 0 0 0 6 251 0 0 0 12 0 5 0 0 174 146 0 2.45 PM 0 0 0 0 0 0 46 1958 0 0 0 12 0 5 0 0 174 146 0 2.45 PM 0 0 0 0 0 0 0 46 1958 0 0 0 22 0 5 7 0 0 0 1617 77 0 0 2.45 PM 0 0 0 0 0 0 24 1064 0 0 42 0 25 0 0 885 37 0 0 0 1617 77 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0																		1
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S-15 PM	Side Side	S-15 PM	S-15 PM																		1
S.30 PM	Side PM	S-30 PM	S.30 PM																		1
SAS PM	SAS PM	S.45 PM	SAS PM																		l l
2-hour fotal 0 0 0 0 46 1978 0 0 0 9 2 0 57 0 0 1517 77 0 50 9M - 6.00 PM 0 0 0 0 24 1064 0 0 42 0 25 0 0 885 37 0 0 661 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	2-hour Total 0 0 0 0 46 1978 0 0 0 9 2 0 57 0 0 0 1617 77 0 0 500 PM 6.00 PM 1 0 0 0 0 0 42 1064 0 0 0 92 0 57 0 0 0 1617 77 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	2-hour Ictal 0	2-hour fotal 0 0 0 0 46 1978 0 0 0 9 2 0 57 0 0 1517 77 0 5 500 M-6.00 PM 0 0 0 0 24 1064 0 0 0 42 0 25 0 0 885 37 0 0 661 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1																		
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Peak Hour Factor	Peak Hour Factor	Peak Hour Factor	Peak Hour Factor																		1
Heavy Vehicle Percentage		Heavy Vehicle Percentage	Heavy Vehicle Percentage		- 0		- 0	0			- 0	-		-		0				-	1
Object Count Co	On Street Bicycle Count O O O O O O O O O	Objective Count Clos OPM - 6.00 PM Objective Count Clos OPM - 6.00 PM Objective Count Obj	Object Count Co																		1
**S.00 PM - 6.00 PM 0 0 0 0 0 0 0 0 0	S,00 PM - 6:00 PM Peak Hour represents the corridor peak not the intersection peak. Southbound Southbo	* 5.00 PM - 6.00 PM 0 0 0 0 0 0 0 0 0	**S.00 PM - 6.00 PM 0 0 0 0 0 0 0 0 0				_	_		_						_					i i
Culebra Rd	Culebra Rd Southbound Sou	Culebra Rd	Culebra Rd	(5:00 PM - 6:00 PM)		0	U	U	U	0	U	U	U	U	U	U	U	U	U	U	
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4:00 PM to 5:00 PM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	00 PM to 5:00 PM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	4.00 PM to 5:00 PM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	4:00 PM to 5:00 PM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Time	SB/W++				NB/W	NB/E	EB/N	EB/S							_		_
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4:00 PM to 5:00 PM	00 PM to 5:00 PM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Bicycle Counts - On Sidewalk 805 4.00 PM to 5:00 PM	4:00 PM to 5:00 PM																	-	_
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** SB/W - Pedestrian/cycle travelling southbound crossing eastbound approach (West leg) Diagram for: PM Peak Hour	SB/W - Pedestrian/cycle travelling southbound crossing eastbound approach (West leg) Diagram for: PM Peak Hour	** SB/W - Pedestrian/cycle travelling southbound crossing eastbound approach (West leg) Diagram for: PM Peak Hour	** SB/W - Pedestrian/cycle travelling southbound crossing eastbound approach (West leg) Diagram for: PM Peak Hour		0	0	0	0	0	0						37	+	1			
											0	0								_	
Peak Hour: 5:00 PM - 6:00 PM *	Peak Hour: 5:00 PM - 6:00 PM*	Peak Hour: 5:00 PM - 6:00 PM *	Peak Hour: 5.00 PM - 6.00 PM*	** SB/W - Pedestrian/cycle tra	velling sout	thbound cro	ossing eastb	ound appro	oach (West I	eg)											
															Peak Hour		5:00 PM -	6:00 PM*		ļ	

1/27/2021

Project Number: 30900399A CoSA Culebra Road Corridor Study TX 1604 to I-10 **115D** PM TURNING MOVEMENT COUNTS North/South Street General McCullen Dr Site Code: Sig 13 East/West Street: Rd - TMCs Culebra Rd Right U-Turn 14 0 11 0 4:00:00 PM 4:15 PM 4:30 PM 4:45 PM 5:00 PM 5:15 PM 5:30 PM 5:45 PM 127 98 99 99 119 124 138 113 53 50 58 49 65 59 63 53 132 105 133 117 106 108 124 114 40 48 35 48 45 40 33 47 13 24 25 25 9 17 24 24 54 50 48 53 43 44 41 44 121 133 125 134 145 161 129 132 11 15 10 15 22 13 43 52 55 47 54 51 166 192 199 200 194 186 207 0.941 0.00% 165 3 0.878 0.375 1.49% 0.00% 5:00 PM - 6:00 PM* 34 494 60 779 54 240 452 74 567 172 0.974 1.43% 0.880 0.977 2.31% 0.27% 0.923 0 0 0 0 0 0 54 779 207 Peak Hours Volume Diagram Dr Westbound Northbound SB/W** SB/E WB/N WB/S NB/W ← 3 240 4:00 PM to 5:00 PM 5:00 PM to 6:00 PM 567 → 172 ↓ PM Peak Hour Diagram for: Peak Hour: 5:00 PM - 6:00 PM* 1/27/2021



Project Number: 30900399A CoSA Culebra Road Corridor Study TX 1604 to I-10 PM TURNING MOVEMENT COUNTS WSD IH 10 EBFR North/South Street: East/West Street: : 12/15/20 IH 10 EBFR Southbound Southbound Through Right 7 134 5 144 3 137 3 133 3 128 7 119 3 131 1 135 ound Right U-Turn 162 0 142 0 129 0 143 0 137 0 139 0 118 0 130 0 Westb Through 230 255 228 239 256 268 290 4:00:00 PM 4:15 PM 4:30 PM 4:45 PM 5:00 PM 5:15 PM 5:30 PM 5:45 PM 0 20 22 27 14 15 18 138 133 152 140 139 131 154 0 25 19 18 24 10 16 12 0 0 0 0 5:45 PM 135 252 2018 130 1100 1066 0.919 1.83% 559 524 0.907 0.942 2.85% 2.64% 5:00 PM - 6:00 PM* 51 14 513 **53** 0.736 0.00% 0.73% 0.00% 1.13% OnStreet Bicycle Count (5:00 PM - 6:00 PM) 5:00 PM - 6:00 PM Peak Hou 0 0 0 0 0 0 0 0 0 0 0 0 ↑ 0 ← 1066 ↓ 53 **G** 0 1 ← 106b ↓ 53 G 0 ↑ ← ↑ → 0 0 0 0 559 → 524 ↓ PM Peak Hour 5:00 PM - 6:00 PM*

North North System: Culebra Rd Site Code: Sig_ 16	TX 1604 to I-10	tudy					PM TURN	III O IVIO	V LIVILIVI V	.001413							**	ור
PM Date: 12/15/2020 Ray Date: 13/15/2020 Ray Date: 13/15/202													Site Code	:: Sig_16				
Culebra RC																		
Southbound Westbound Westbound Northbound Eastbound Finding Westbound West	TOD:	PM	Date:	12/15/2020		Raw Data:			Raw Count D	ata\From GRA			MC\WSP - 12-0	03713 - Culet				
Time																		
## 4:00:00 PM 0 0 0 0 0 0 74 29 0 181 1 1 19 0 100 69 0 0 0 4:15 PM 0 0 0 0 0 0 65 22 0 189 2 18 0 101 60 0 0 0 0 4:35 PM 0 0 0 0 0 0 0 85 28 0 175 3 11 0 119 51 0 0 0 4:45 PM 0 0 0 0 0 0 0 73 21 0 174 4 8 0 108 64 0 0 0 5:00 PM 0 0 0 0 0 0 0 104 26 0 180 4 8 0 99 45 0 0 0 5:15 PM 0 0 0 0 0 0 76 22 0 200 5 8 0 104 61 0 0 0 5:30 PM 0 0 0 0 0 0 76 22 0 200 5 8 0 104 61 0 0 0 5:30 PM 0 0 0 0 0 0 0 76 22 0 200 5 8 0 104 61 0 0 0 5:30 PM 0 0 0 0 0 0 0 77 20 0 188 2 9 9 0 107 47 0 0 0 5:00 PM 6.00 PM 10 0 0 0 0 0 0 351 88 0 781 1 6 35 0 412 203 0 0 0 0 2 4 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Net .																	
## 4.15 PM 0 0 0 0 0 0 65 22 0 189 2 18 0 101 60 0 0 0 4.36 PM 0 0 0 0 0 0 0 85 28 0 175 3 11 0 119 51 0 0 0 0 4.45 PM 0 0 0 0 0 0 0 0 73 21 0 174 4 8 0 108 64 0 0 0 5.50 PM 0 0 0 0 0 0 0 0 76 22 0 20 200 5 8 0 104 61 0 0 0 5.15 PM 0 0 0 0 0 0 0 76 22 0 200 5 8 0 104 61 0 0 0 5.36 PM 0 0 0 0 0 0 77 20 0 188 2 9 0 107 47 0 0 0 5.45 PM 0 0 0 0 0 0 74 20 0 188 2 9 0 107 47 0 0 0 5.45 PM 0 0 0 0 0 0 74 20 0 188 2 9 0 107 47 0 0 0 5.45 PM 0 0 0 0 0 0 0 74 20 0 213 5 10 0 102 50 0 0 0 0 5.45 PM 0 0 0 0 0 0 0 5.45 PM 0 0 0 0 0 0 5.45 PM 0 0 0 0 0 0 5.45 PM 0 0 0 0 0 0 5.45 PM 0 0 0 0 0 0 5.45 PM 0 0 0 0 0 0 0 5.45 PM 0 0 0 0 0 0 0 5.45 PM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0																		
A 30 PM 0																		
## 445 PM 0 0 0 0 0 0 73 21 0 174 4 8 0 108 64 0 0 0 0 5 50 PM 0 0 0 0 0 0 104 26 0 180 4 8 0 99 45 0 0 0 0 5 15 PM 0 0 0 0 0 0 0 0 76 22 0 0 200 5 8 0 104 61 0 0 0 5 15 PM 0 0 0 0 0 0 0 77 20 0 188 2 9 0 107 47 0 0 0 5 53 PM 0 0 0 0 0 0 0 74 20 0 188 2 9 0 107 47 0 0 0 0 5 53 PM 0 0 0 0 0 0 0 74 20 0 188 2 9 0 107 47 0 0 0 0 0 5 545 PM 0 0 0 0 0 0 0 5 68 180 0 100 100 50 PM 0 0 0 0 0 0 5 68 180 0 100 100 100 100 100 100 100 100																		
S.00 PM																		
S.30 PM 0				0													0	
S.45 PM	5:15 PM	0	0	0	0	0		22	0	200	5	8	0	104	61	0	0	
2-hour Total 0				0		0												
S-00 PM - 6-00 PM 0						_												
Peak Hour Factor				_	_	_												
Heavy Vehicle Percentage		0	0	0	0	0			0				0			0	0	
Construct Biscycle Count C			<u> </u>															
So0 PM = 6.00 PM O O O O O O O O O						0.00%	3.24%	2.66%		1.33%	0.00%	0.00%		1.07%	5.15%			
**SOPM - 6:00 PM Peak Hour represents the corridor peak not the intersection peak. **Culebra Rd		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Culebra Rd Southbound Culebra Rd Northbound Eastbound Salth Southbound Salth		r renresen	ts the corri	dor neak no	t the inters	ertion neak												J
Culebra Rd H 10 M9FR Culebra Rd H 20 M9FR Culebra Rd Factorial Rd F	3.00 1 W 0.00 FW FERK HOL	represen	Co Corre	uoi peak iiu	c are alters	ссион реак.					Peak Hour	s Volume (Diagram		1	88	T	
Southbound Westbound Northbound Eastbound Northbound Eastbound Northbound Eastbound Northbound Eastbound Northbound Eastbound Northbound Eastbound Northbound Northbound Eastbound Northbound Northbound Eastbound Northbound				Culeb	ra Rd	IH 10	WBFR	Culet	ora Rd						+		1	
Pedestrians Counts		South	bound	Westl	ound	North	nbound	Eastb	ound						1	0	Ť	
4.00 PM to 5.00 PM 0 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Time	SB/W**	SB/E	WB/N	WB/S	NB/W	NB/E	EB/N	EB/S		0	0	0	0	¢	0	1	
5.00 PM 10 6.00 PM 0 0 0 0 0 1 1 1 0 0 4.00 PM 10 6.00 PM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0				Pedestrians							+	→					1	→
Bicycle Counts - On Sidewalk 4:00 PM to 5:00 PM															0	781	16	35
40.0PM 0.5.00PM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	5:00 PM to 6:00 PM	0					1	1	0						ļ			
5:00 PM to 6:00 PM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0																		
** SB/W - Pedestrian/cycle travelling southbound crossing eastbound approach (West leg) Diagram for: PM Peak Hour													0	+	ļ			
								U	U			Diagram fo		DM Dook H	lour		т	
	3b) W - Fedestrian/ Cycle tra	ening sout	noouna cre	assing eastu	ошни аррги	acii (west ii	cg)											
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1/2000000044 Colin Nasad Currelour Researd Currelour Researd Currelour Researd Currelour Researd (Protegor Case Cultiforms Researd Research Researd Research Res																		1/27/202

APPENDIX C WSD Project Number: 30900399A PM TURNING MOVEMENT COUNTS COSA Culebra Road Corridor Study TX 1604 to I-10 North/South Street: East/West Street: Tezel Rd U-Turn 3 1 25 22 32 25 36 15 27 36 Right 117 117 116 122 122 112 117 99 hrough 118 151 120 131 144 136 137 Right 11 15 10 19 20 16 20 21 Left 32 49 52 59 48 60 58 Through 173 204 255 222 253 236 226 232 Left 34 36 27 27 34 31 38 160 148 167 146 177 162 172 187 Left 40 31 31 32 26 49 38 37 132 114 118 109 116 150 92 151 171 159 152 171 4:15 PM 4:30 PM 4:45 PM 5:00 PM 5:15 PM 5:30 PM 5:30 PM 35 21 33 31 29 27 22 161 177 189 163 415 223 228 109 218 114 0 0 0 0 0 0 ↑ 109 ← 690 ↓ 223 **Ģ** 0 4:00 PM to 5:00 PM 5:00 PM to 6:00 PM

 Project Number: 30900339A
 PM TURNING MOVEMENT COUNTS

 CoSA Culebra Road Corridor Study
 TX 1604 to 1-10

 North/South Street:
 Ingram Rd
 Site Code: Sig_ 18

 East/West Street:
 Culebra Rd

East/West Street:					Culebra Ro	i											
TOD:	PM	Date:	12/15/2020)	Raw Data:	J:\\5.1 Data Co	ollection\5.1.2	Raw Count D	ata\From GR	AM\12 30 202	0\Extracted T	MC\WSP - 12	-03713 - Culet	bra Rd - TMCs			
		Ingra	m Rd			Culebra				Ingra				Culeb	ra Rd		
		South	bound			Westbo	und			North	bound			Eastb	ound		
Time	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	
4:00:00 PN	13	9	72	0	7	382	9	0	11	11	17	0	53	273	4	0	
4:15 PN	21	15	92	0	10	468	12	0	7	7	9	0	54	267	6	0	
4:30 PN	22	22	104	0	13	395	12	0	10	12	22	0	50	315	7	1	
4:45 PN	27	22	114	0	15	486	16	0	11	5	18	0	49	336	11	1	
5:00 PN	41	31	120	1	14	427	16	0	9	9	26	0	49	263	7	0	
5:15 PN	1 38	27	102	0	9	503	21	1	9	4	22	0	51	315	8	1	
5:30 PN	1 22	28	101	0	20	479	10	0	18	3	24	0	34	286	8	1	
5:45 PN	35	28	111	0	17	440	12	0	10	10	17	0	70	317	4	0	
2-hour Tota	219	182	816	1	105	3580	108	1	85	61	155	0	410	2372	55	4	
5:00 PM - 6:00 PM	136	114	434	1	60	1849	59	1	46	26	89	0	204	1181	27	2	
Peak Hour Facto	0.829	0.919	0.904	0.250	0.750	0.919	0.702	0.250	0.639	0.650	0.856		0.729	0.931	0.844	0.500	
Heavy Vehicle Percentag	0.91%	0.00%	1.23%	0.00%	0.00%	0.75%	0.00%	0.00%	0.00%	0.00%	1.94%		0.98%	2.70%	0.00%	0.00%	
OnStreet Bicycle Coun (5:00 PM - 6:00 PM		0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	
* 5:00 PM - 6:00 PM Peak Ho	ur represer	nts the corri	dor peak no	t the inters	ection peak.												
										Peak Hour	s Volume [Diagram		1	59		
	Ingra	ım Rd	Culet	ora Rd	Ingra	am Rd	Culeb	ora Rd						+	1849		
	South	bound	West	bound	North	bound	Eastb	ound						+	60		
Time	SB/W**	SB/E	WB/N	WB/S	NB/W	NB/E	EB/N	EB/S		434	114	136	1	4	1		
			edestrians	Counts						+	+	→	ď	5	+	1	Т
4:00 PM to 5:00 PM	0	0	0	0	0	0	0	0				2	5	0	46	26	8
5:00 PM to 6:00 PM	1	2	2	0	0	0	0	0				204	1			•	
		Bicyc	le Counts -	On Sidewal	k		•	•				1181	→	Ī			
4:00 PM to 5:00 PM	0	0	0	0	0	0	0	0				27	+				
5:00 PM to 6:00 PM	0	0	0	0	0	0	0	0									
** SB/W - Pedestrian/cycle tr	evelling sou	thbound cro	ssing eastb	ound appro	oach (West I	eg)					Diagram fo	r:	PM Peak H	lour			
											Peak Hour	:	5:00 PM -	6:00 PM*			

1/30000399A CoSA Culviors Road Corridor Study, S.O. Project Extr) S. 1. Data Collection (S. 1.2 Data Tabulation) 30000399A CoSA Culviors Road, J.M., Bav 1

1/27/2021

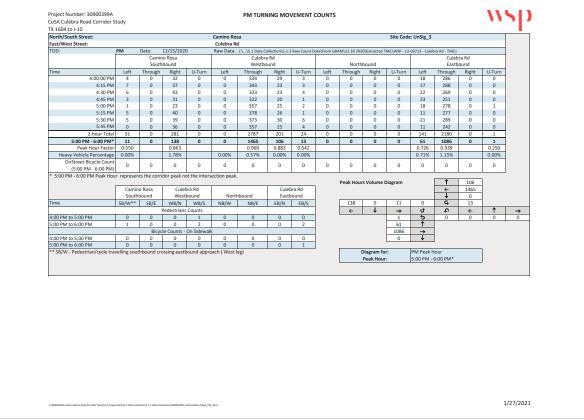
Project Number: 30900399A CoSA Culebra Road Corridor Study TX 1604 to 1-10 North/South Street: East/West Street: **115**D PM TURNING MOVEMENT COUNTS Left 50 53 50 47 43 46 53 58 hrough 110 115 109 113 125 115 136 119 Right 54 46 47 66 51 59 52 47 Through 131 128 114 126 122 147 128 130 84 81 81 75 80 67 80 76 4:00:00 PM 4:15 PM 4:30 PM 4:45 PM 5:00 PM 5:15 PM 24 12 15 21 19 24 17 43 39 49 36 48 38 43 36 98 133 115 136 135 139 160 133 142 145 136 139 143 130 66 84 66 83 96 71 79 2-hour Total 526 1049 214 \$50P M- 60P M* 29 567 108 Peak Hour Factor 0.942 0.856 0.844 Heavy Vehicle Percentage 0.95% 1.53% 2.80% OnStreet Bircycle Count (5.00 PM - 6.00 PM) 0 0 5.00 PM - 6.00 PM) each Hour represents the corridor peak in 209 0.886 0.95% 200 0.862 0.00% **555 495** 0.910 74 527 0.771 0.896 2.07% 3.22% 0.53% 2.74% 1.17% 1.44% 0 0 0 0 0 0 0 0 0 ↑ 165 ← 555 ↓ 200 G 0 Callaghan Rd Culebra Rd Callaghan Rd Culebra Rd Southbound Westbound Northbound Eastbound SB/W** SB/E WB/N WB/S NB/W NB/E EB/N EB/S → U 0 5 74 ↑ 527 → 303 ↓ 4:00 PM to 5:00 PM 5:00 PM to 6:00 PM

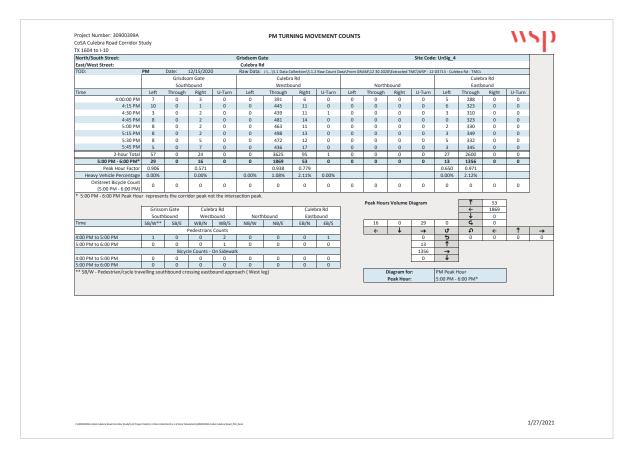
East/West Street Superior Culebra Rd Superior Culebra Rd Superior Culebra Rd Superior Superi	Time	New York Control of the Control of t
Culebra Rd South-bound Culebra Rd South-bound Survivous South-bound Sout	Culebra Br	New York Control of the Control of t
Southbound	Southbound	New York Control of the Control of t
Time	Time	Right U-Turn 25 0 50 0 35 0 48 0 33 0 48 0 41 0 47 0
4.00.00 PM 13	4-00-00 PM 13 44 23 0 37 272 5 0 49 69 53 0 24 239 415 PM 10 56 12 0 49 293 15 0 35 63 38 0 29 236 430 PM 9 54 19 0 43 262 13 0 45 79 49 0 18 217 445 PM 9 54 19 0 43 262 13 0 45 79 49 0 18 217 500 PM 15 61 17 0 42 280 13 0 50 73 52 0 17 210 515 PM 9 53 18 0 39 329 13 0 50 73 52 0 17 210 515 PM 9 53 18 0 39 329 13 0 50 73 52 0 17 210 515 PM 9 53 18 0 39 329 13 0 50 72 49 0 25 243 5.30 PM 14 64 21 0 38 316 9 0 49 65 35 0 21 213 5.00 PM 6.00 PM 14 64 21 0 38 316 9 0 49 65 35 0 21 213 5.00 PM 6.00 PM 14 64 21 0 38 316 9 0 49 65 35 0 21 213 5.00 PM 6.00 PM 14 64 21 0 38 316 9 0 49 65 35 0 21 213 5.00 PM 6.00 PM 54 23 75 0 164 1264 96 0 370 533 351 0 184 1802 5.00 PM 6.00 PM 54 233 75 0 164 1243 46 0 192 270 175 0 87 897 Peak How Factor 0.844 0.910 0.893 0.911 0.945 0.855 0.960 0.955 0.841 0.870 0.923 Heavy Wehclie Percentage 430% 2.91% 0.00% 1.0	25 0 50 0 35 0 48 0 33 0 48 0 41 0 47 0
4.15 PM 10 5 56 12 0 49 293 15 0 35 63 38 0 29 29 236 50 0 43 230 M 9 54 19 0 43 262 13 0 45 79 49 0 18 217 35 0 6 44 85 M 7 60 14 0 48 294 17 0 49 52 36 0 26 213 48 0 30 515 PM 9 53 18 0 39 329 13 0 50 73 52 0 17 210 33 0 515 PM 9 53 18 0 39 329 13 0 50 73 52 0 17 210 33 0 515 PM 9 53 18 0 39 329 13 0 50 73 52 0 17 210 33 0 5 50 PM 14 64 21 0 38 316 9 0 49 65 35 0 21 213 44 0 0 5 50 PM 16 55 19 0 45 318 11 0 43 60 39 0 24 231 41 0 0 5 50 PM 16 55 19 0 45 318 11 0 43 60 39 0 0 24 231 41 0 0 5 50 PM 6-60 PM 54 233 75 0 164 1243 46 0 192 270 175 0 87 897 169 0 0 50 PM 6-60 PM 54 233 75 0 164 1243 46 0 192 270 175 0 87 897 169 0 0 50 PM 6-60 PM 54 230 230 231 0 0.00% 1.69% 3.13% 1.00% 2.40% 1.42% 1.09% 2.66% 1.83% 1.00% 2.50 PM 6-60 PM 50 PM 6-60 PM 54 233 3 0 0.00% 1.69% 3.13% 1.00% 2.40% 1.42% 1.09% 2.66% 1.83% 1.00% 2.50 PM 6-50 PM Paak Hour represents the corridor peak not the intersection peak. Time **Peak Hour score** **Peak Hour represents the corridor peak not the intersection peak.** **Peak Hour score** **Peak Hour represents the corridor peak not the intersection peak.** **Peak Hour represents the corridor peak not the intersection peak.** **Peak Hour represents the corridor peak not the intersection peak.** **Peak Hour represents the corridor peak not the intersection peak.** **Peak Hour represents the corridor peak not the intersection peak.** **Peak Hour represents the corridor peak not the intersection peak.** **Peak Hour Score**	### A15PM 10 56 12 0 49 293 15 0 35 63 38 0 29 236 430PM 9 54 19 0 43 262 13 0 45 79 49 0 18 217 445PM 7 60 14 0 48 294 17 0 49 52 36 0 26 213 5:00PM 15 61 17 0 42 280 13 0 50 73 52 0 17 210 5:15PM 9 53 18 0 39 329 13 0 50 72 49 0 25 243 5:30PM 14 64 21 0 38 316 9 0 49 65 35 0 21 213 5:30PM 14 64 21 0 38 316 9 0 49 65 35 0 21 213 5:45PM 16 55 19 0 45 318 11 0 43 60 39 0 24 231 213 5:45PM 16 55 19 0 45 318 11 0 43 60 39 0 24 231 223 5:50PM 60 9M 65 65 65 75 60 21 213 5:45PM 16 55 19 0 45 60 39 60 370 533 351 0 184 1802 5:50PM 600PM 600PM 54 233 75 0 164 1243 46 0 192 270 175 0 87 897 898 899 898 899 899 899 899 899	50 0 35 0 48 0 33 0 48 0 41 0 47 0
4.45 PM 7 60 144 0 48 294 17 0 49 52 36 0 26 213 48 0 5 5 5 0 PM 15 61 17 0 42 280 13 0 50 73 52 0 17 210 33 0 5 5 5 15 PM 9 53 18 0 39 329 13 0 50 50 72 49 0 25 243 48 0 5 5 35 PM 14 64 21 0 38 316 9 0 49 56 35 0 21 213 41 0 5 5 6 19 0 45 318 11 0 43 60 39 0 24 231 47 0 2 5 6 5 4 7 143 0 341 2364 96 0 370 33 33 0 0 24 231 47 0 0 5 6 7 14 14 14 14 14 14 14 14 14 14 14 14 14	4:45 PM 7 60 14 0 48 294 17 0 49 52 36 0 26 213 5:00 PM 15 61 17 0 42 280 13 0 50 73 52 0 17 210 5:30 PM 14 64 21 0 38 316 9 0 49 65 35 0 21 213 5:45 PM 16 55 19 0 45 318 11 0 43 60 39 0 24 231 2-hour Total 93 447 143 0 341 2264 96 0 370 533 351 0 184 1802 5:00 PM -6:00 PM* 54 233 75 0 164 1243 46 0 192 270 175 0 87 897 Peak Hour Factor 0.844 0.510	48 0 33 0 48 0 41 0 47 0
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S.15 PM 9 53 18 0 39 329 13 0 50 72 49 0 25 243 48 0	5:15 PM 9 53 18 0 39 329 13 0 50 72 49 0 25 243 5:30 PM 14 64 21 0 38 316 9 0 49 65 35 0 21 213 5:30 PM 16 55 19 0 45 318 11 0 43 60 39 0 24 231 2-hour Total 93 447 143 0 341 2364 96 0 370 533 351 0 184 1802 5:00 PM - Ex0 PM* * * 423 75 0 164 1243 46 0 192 270 175 0 87 897 Peak Hour Factor 0.844 0.910 0.893 0.911 0.945 0.885 0.960 0.925 0.841 0.870 0.923 Heavy Vehicle Percentage 4.30% 2.91% <td>48 0 41 0 47 0</td>	48 0 41 0 47 0
5.30 PM 14 64 21 0 38 316 9 0 49 65 35 0 21 213 41 0	5:30 PM 14 64 21 0 38 316 9 0 49 65 35 0 21 213 S:45 PM 16 55 19 0 45 318 11 0 43 60 39 0 24 231 2-hour Total 93 447 143 0 341 2364 96 0 370 533 351 0 184 1802 S:00 PM: -600 PM* 54 223 75 0 164 1243 46 0 192 270 175 0 87 897 Perox Hour Factor 0.844 0.910 0.945 0.885 0.960 0.925 0.841 0.870 0.933 Heavy Vehicle Percentage 4.30% 2.91% 0.70% 0.00% 1.69% 3.13% 1.08% 2.44% 1.42% 1.09% 2.66% On Street Bixyled Count 0 0 0 0 0	41 0 47 0
S-45 PM 16 55 19 0 45 318 11 0 43 60 39 0 24 231 47 0	5:45 PM 16 55 19 0 45 318 11 0 43 60 39 0 24 231 2-hour fotal 93 447 143 0 341 236 96 0 370 533 351 0 184 1802 5:00 PM - 6:00 PM* 54 233 75 0 164 1243 46 0 192 270 175 0 87 897 Peak Hour Factor 0.844 0.910 0.893 0.911 0.945 0.885 0.960 0.925 0.841 0.870 0.923 Heavy Vehicle Percentage 4.30% 2.91% 0.70% 0.00% 1.69% 3.13% 1.08% 2.44% 1.42% 1.09% 2.66% OnStreet Bicycle Count 0 </td <td>47 0</td>	47 0
2-hour fotal 93 447 143 0 341 2264 96 0 370 533 351 0 184 1802 377 0 S.00 PM -6.00 PM* 54 233 75 0 164 1243 46 0 192 270 175 0 87 897 169 0 Peak Hour Factor 0.844 0.910 0.893 0.931 0.945 0.885 0.995 0.925 0.841 0.870 0.923 0.880 Heavy Vehicle Percentage 4.30% 2.91% 0.70% 0.00% 1.69% 3.13% 1.00% 2.44% 1.42% 1.00% 2.66% 1.83% OnStreet Bicycle Count (5.00 PM -6.00 PM) 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	2-hour Total 93 447 143 0 341 2364 96 0 370 533 351 0 184 1802 5.00 PM 6-00 PM 54 233 75 0 164 1243 46 0 192 270 175 0 87 897 Pesk Hour Factor 0.844 0.910 0.893 0.911 0.945 0.885 0.960 0.925 0.841 0.870 0.923 Heavy Wehicle Percentage 4.30% 2.91% 0.70% 0.00% 1.69% 3.13% 1.08% 2.44% 1.42% 1.09% 2.66% OnStreet Biveckle Count	
SoD PM - 6.00 PM* 54 233 75 0 164 1243 46 0 192 270 175 0 87 897 159 0	5:00 PM - 6:00 PM* 54 233 75 0 164 1243 46 0 192 270 175 0 87 897 Peak Hour Factor 0 0.844 0.910 0.893 0.911 0.945 0.885 0.960 0.925 0.841 0.870 0.923 Heavy Vehicle Percentage 4.30% 2.91% 0.70% 0.00% 1.69% 3.13% 1.08% 2.44% 1.42% 1.09% 2.66% OnStreed Bicycle Count 0	
Peak Hour Factor 0.844 0.910 0.893 0.911 0.945 0.885 0.885 0.906 0.925 0.841 0.923 0.880 Heavy Vehicle Percentage 4.30% 2.91% 0.70% 0.00% 1.69% 3.13% 1.08% 2.44% 1.42% 1.09% 2.66% 1.83% OnStreed Bicycle Count (S.00 PM - 6.00 PM) 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Peak Hour Factor 0.844 0.910 0.893 0.911 0.945 0.885 0.960 0.925 0.841 0.870 0.923 Heavy Vehicle Percentage 4.30% 2.91% 0.70% 0.00% 1.69% 3.13% 1.08% 2.44% 1.42% 1.09% 2.56% OnStreet Bicycle Count 0	
Heavy Wehicle Percentage	Heavy Vehicle Percentage 4.30% 2.91% 0.70% 0.00% 1.69% 3.13% 1.08% 2.44% 1.42% 1.09% 2.66% OnStreet Bicycle Count 0 0 0 0 0 0 0 0 0 0 1 0 0 0 0 0 0 0 0	
S.00 PM - 6.00 PM Peak Hour represents the corridor peak not the intersection peak. **S.00 PM - 6.00 PM Peak Hour represents the corridor peak not the intersection peak.** **Zarzamora St		
\$5.00 PM - 6.00 PM Peak Hour represents the corridor peak not the intersection peak. Culebra Rd Southbound South	(50004 50004)	0 0
Peak Hours Volume Diagram ↑ 46 ← 12.83	(5:00 PM - 6:00 PM)	0 0
Pedestrians Counts Pedestrians Counts Pedestrians Counts Pedestrians Counts Pedestrians Counts Pedestrians Counts Pedestrians Counts Pedestrians Counts Pedestrians Counts Pedestrians Count	Southbound Westbound Northbound Eastbound	164
.00 PM to 5:00 PM 2 3 3 3 0 1 6 4 2 2 870 175 0.00 PM to 6:00 PM 4 2 1 1 1 5 2 4 2 2 87	35/11 35/2 115/11 115/5 115/11 115/2 25/11 25/5	-
5:00 PM to 6:00 PM		
4:00 PM to 5:00 PM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		
5:00 PM to 6:00 PM		
** SB/W - Pedestrian/cycle travelling southbound crossing eastbound approach (West leg) Diagram for: PM Peak Hour		
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1/27/2021

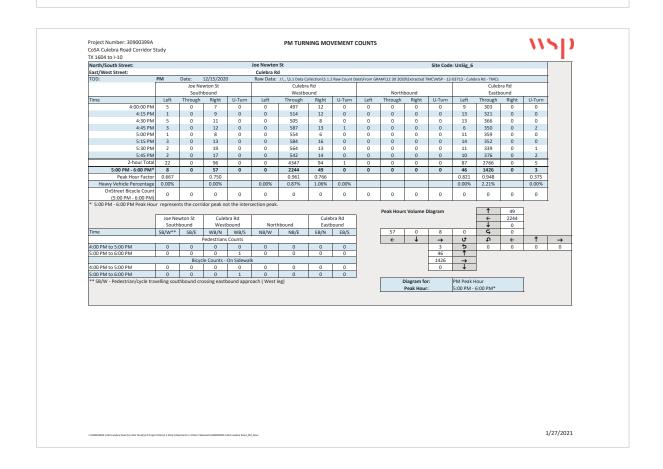
Company Comp	Column C	X 1604 to I-10 North/South Street:												Site Code	: UnSig 1				
Culebra Rd	Culebra Rd	East/West Street:					Culebra Ro	i											
Southbound	Southbound	FOD:	PM	Date:	12/15/2020)	Raw Data:			Raw Count D	ata\From GR			MC\WSP - 12-	03713 - Cule				
Time	Time																		
4.00:00 PM 0 0 0 0 0 6 330 0 0 0 0 0 1 0 0 291 1 0 0 4.15 PM 0 0 0 0 0 15 303 0 0 0 0 0 0 1 0 0 281 0 0 281 0 0 0 4.39 PM 0 0 0 0 0 13 296 0 0 0 0 0 4 0 0 286 2 0 0 5.00 PM 0 0 0 0 0 10 286 0 0 0 0 0 0 4 0 0 337 1 0 0 5.15 PM 0 0 0 0 0 12 300 0 0 0 0 4 0 0 337 1 0 0 5.15 PM 0 0 0 0 0 12 300 0 0 0 0 4 0 0 336 4 0 0 5.15 PM 0 0 0 0 0 12 300 0 0 0 0 0 3 3 0 0 335 0 0 0 5.30 PM 0 0 0 0 0 17 319 0 0 0 0 0 6 0 337 3 0 0 335 0 0 0 5.45 PM 0 0 0 0 0 16 278 0 0 0 0 0 6 0 0 337 3 0 0 335 0 0 0 5.45 PM 0 0 0 0 0 16 278 0 0 0 0 0 5 0 0 312 5 0 0 2-hour Total 0 0 0 0 0 55 1183 0 0 0 0 30 0 0 2525 16 0 0 5.00 PM - 6.00 PM 0 0 0 0 0 5 1 138 0 0 0 0 188 0 0 1324 12 0 0 0-NStreet Bicycle Count 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	4.00:00 PM 0 0 0 0 0 6 330 0 0 0 0 0 1 0 0 291 1 0 0 431 1 0 0 431 1 0 0 431 PM 0 0 0 0 0 15 303 0 0 0 0 0 0 1 1 0 0 0 281 0 0 0 0 1 1 0 0 281 0 0 0 0 0 1 1 0 0 0 281 0 0 0 0 0 1 1 0 0 0 281 0 0 0 0 0 0 1 1 0 0 0 281 0 0 0 0 0 0 1 1 0 0 0 281 0 0 0 0 0 0 1 1 0 0 0 0 0 1 1 0 0 0 0																		
# 115 PM	# 115 PM 0 0 0 0 0 15 303 0 0 0 0 0 1 0 0 281 0 0 0 0 0 4 4 0 0 0 286 2 0 0 0 0 0 0 0 0 0 0 0 286 2 0 0 0 0 0 0 0 0 0 0 286 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0																		
4.39 PM	4.39 PM																		
### ### ##############################	## 4:45 PM 0 0 0 0 0 6 293 0 0 0 0 0 6 0 0 337 1 0 0 5:00 PM 0 0 0 0 0 10 286 0 0 0 0 0 0 4 0 0 330 4 0 0 350 4 0 0 5:15 PM 0 0 0 0 0 12 300 0 0 0 0 0 0 3 0 0 355 0 0 0 355 0 0 0 355 9 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0																		
S.OP M	S.OP PM																		
S-30 PM	S-30 PM																		
S.45 PM	S.45 PM		0	0	0	0	12	300	0	0	0	0	3	0	0	335	0	0	
2-hour Total 0 0 0 0 0 95 2405 0 0 0 0 0 30 0 0 2525 16 0 0 0 0 0 30 0 0 2525 16 0 0 0 0 0 18 0 0 1324 12 0 0 0 0 18 0 0 1324 12 0 0 0 0 18 0 0 0 18 0 0 0 0 0 0 0 0 0 0	2-hour Total 0 0 0 0 0 95 2405 0 0 0 0 0 30 0 0 2525 16 0 0 0 0 0 30 0 0 2525 16 0 0 0 0 0 18 0 0 1324 12 0 0 0 18 0 0 18 0 0 18 0 0 18 0 0 18 0 0 18 0 0 18 0 0 18 0 0 0 18 0 0 0 0		0	0	0	0	17	319	0	0	0	0	6	0	0	347	3	0	
S00 PM - 600 PM 0	S00 PM - 600 PM 0		0	0	0	0	16	278	0	0	0	0	5	0	0	312	5	0	
Peak Hour Factor	Peak Hour Factor			0															
Heavy Wehicle Percentage	Heavy Wehicle Percentage		0	0	0	0			0	0	0	0		0	0			0	
On Street Bicycle Count O O O O O O O O O	On Street Bicycle Count				$\overline{}$														
** 5.00 PM - 6:00 PM Peak Hour represents the corridor peak not the intersection peak. Culebra Rd	** 5.00 PM - 6.00 PM Peak Hour represents the corridor peak not the intersection peak. Culebra Rd	OnStreet Bicycle Count		0	0	0			0	0	0	0		0	0			0	
4:00 PM to 5:00 PM 0 0 0 0 0 0 0 1 0 0 0 0 0 0 0 0 0 0 0	4:00 PM to 5:00 PM 0 0 0 0 0 0 0 1 0 0 0 0 0 0 0 0 0 0 0	Fime		SB/E	Westi WB/N	wB/S	North	bound	Easth	oound						¢	55 0	•	
5:00 PM to 6:00 PM	5:00 PM to 6:00 PM	1-00 DM +o E-00 DM	0				n	0	1	0		-							
Sicycle Counts - On Sidewalk 1324 → 132	# Bicycle Counts - On Sidewalk																		10
5:00 PM to 6:00 PM 0 0 0 0 0 0 0 0 0 0 Diagram for: PM Peak Hour	5:00 PM to 6:00 PM 0 0 0 0 0 0 0 0 0 0 Diagram for: PM Peak Hour									-					→	1			
** SB/W - Pedestrian/cycle travelling southbound crossing eastbound approach (West leg) Diagram for: PM Peak Hour	** SB/W - Pedestrian/cycle travelling southbound crossing eastbound approach (West leg) Diagram for: PM Peak Hour	1:00 PM to 5:00 PM	0	0	0	0	0	0	0	0				12	+	1			
									0	0						-			
		** SB/W - Pedestrian/cycle tra	velling sou	hbound cro	issing eastb	oound appro	oach (West I	eg)											

4:00:00 PM	dd Right U-Turn 0 0 0 0 0 0 0 0 0 0 0 0
East/Next Street: Culebra Rd	dd Right U-Turn 0 0 0 0 0 0 0 0 0 0 0 0
Top: PM Date: 12/15/2020 Raw Data: 13, \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	dd Right U-Turn 0 0 0 0 0 0 0 0 0 0 0 0
Coppertree Blvd	dd Right U-Turn 0 0 0 0 0 0 0 0 0 0 0 0
Time	Right U-Turn 0 0 0 0 0 0 0 0 0 0 0 0
Time	Right U-Turn 0 0 0 0 0 0 0 0 0 0 0 0
## 4:00:00 PM	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
## 4:3.9 M	0 0 0 0 0 0 0 0
4:30 PM	0 0 0 0 0 0
S.O.PM 3	0 0
S.O.PM 3	
S-30 PM 1	
S-30 PM 1	0 0
2-hour Total 15 0 18 0 0 2843 43 0 0 0 0 0 0 15 2243 0 0 0 5:00 PM -6:00 PM* 7 0 10 0 0 1470 27 0 0 0 0 0 0 15 2243 0 0 0 Peak Hour Factor 0.833 0.833 0.952 0.514 0.952 0.514 0.0588 0.956 0.006 0.	
S00 PM - 6:00 PM* 7	0 0
Peak Hour Factor 0.583 0.833 0.505 0.514 0.588 0	0 0
Peak Hour Factor 0.583 0.833 0.505 0.514 0.588 0	0 0
On Street Bicycle Count O O O O O O O O O	
S.00 PM - 6.00 PM O	
* 5:00 PM -6:00 PM Peak Hour represents the corridor peak not the intersection peak. Coppertree Blvd Culebra Rd Southbound Westbound Westbound Southbound Sauthbound Seatbound Southbound Culebra Rd Southbound Overhound Culebra Rd Southbound Overhound Culebra Rd Southbound Overhound Culebra Rd Sauthbound Overhound Culebra Rd Sauthbound Overhound Overh	0 0
	0
4:00 PM to 5:00 PM 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0
5:00 PM to 6:00 PM 0 0 0 1 0 0 0 2	
Bicycle Counts - On Sidewalk 1140 →	
4:00 PM to 5:00 PM 0 0 0 0 0 0 0 1	
5:00 PM to 6:00 PM	<u></u>
Peak Hour: 5:00 PM - 6:00 PM*	





Project Number: 30900399A PM TURNING MOVEMENT COUNTS **115**[) TX 1604 to I-10 North/South Street: East/West Street: 11 12 17 18 12 12 20 25 303 335 360 353 344 349 328 361 4:15 PM 4:30 PM 4:45 PM 5:00 PM 5:15 PM 10 6 14 14 7 11 13 18 17 15 13 512 539 549 548 559 541 9 10 5 13 8 7 **2197** 0.983 33 0.635 14 1382 0.700 0.957 Heavy Vehicle Percentage OnStreet Bicycle Coun 0.00% 0.00% 0.95% 7.14% 2.38% 1.57% 1.19% 0 0 0 0 0 0 0 0 0 0 0 0 0 ↑ 18 ← 2197 ↓ 58 **G** 2 eek Culebra Rd und Eastbound NB/E EB/N EB/S Pipers Creek Northbound 4:00 PM to 5:00 PM 5:00 PM to 6:00 PM PM Peak Hour 5:00 PM - 6:00 PM*



APPENDIX C Project Number: 30900399A CoSA Culebra Road Corridor Study TX 1604 to I-10 PM TURNING MOVEMENT COUNTS **115D** Valero Driveway North/South Street: East/West Street: Site Code: UnSig 7 Date: 12/15/2020 Valero Driveway Southbound Van Ness Northbound Through 393 443 4:00:00 PM 4:15 PM 4:30 PM 4:45 PM 5:00 PM 5:15 PM Through Right U-Turn Through Right U-Turn 0 0 0 0 0 0 297 465 473 513 343 375 360 337 12 9 0 0 73 5:45 PN 3697 0.949 0.60% 5:00 PM - 6:00 PM* 0.691 0.719 21 1432 0.550 0.00% 0.00% 2.53% 0.00% 1947 46 1 Valero Driveway Culebra Rd Van Ness Culebra Rd Southbound Westbound Northbound Eastbound SB/W*** S8/E WB/N WB/S NB/W NB/E EB/N EB/S

0 5 21 ↑ 1432 →

PM Peak Hour 5:00 PM - 6:00 PM*

1/27/2021

North/South Street:												Site Code	: UnSig_8				
East/West Street: TOD:	PM	Date:	12/15/202	0	Culebra Ro	J:\\5.1 Data C	-11)5 60.	111111111111111111111111111111111111111	0) 5 4 4 7	14011100 40	00740 0 1-1	h D.I. TAKO			
TOD:	PIVI	Date:	12/15/202	U	Raw Data:	Culebr		Raw Count D	ata\From GRA	AM\12 30 202		MC\WSP - 12-	03713 - Cule		ora Rd		
		South	bound			Westbo				North					oound		
Time	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	
4:00:00 PM		0	0	0	0	246	0	0	35	0	2	0	0	199	4	0	
4:15 PM	0	0	0	0	0	242	0	0	12	0	0	0	0	198	1	0	
4:30 PM	0	0	0	0	0	231	0	0	12	0	2	0	0	194	3	0	
4:45 PM		0	0	0	0	248	0	0	22	0	0	0	0	196	1	1	
5:00 PM		0	0	0	0	282	0	0	23	0	6	0	0	189	0	0	
5:15 PM		0	0	0	0	253	0	0	11	0	3	0	0	218	0	0	
5:30 PM		0	0	0	0	218	0	0	9	0	0	0	0	194	1	0	
5:45 PM 2-hour Total		0	0	0	0	238 1958	0	0	11 135	0	3 16	0	0	210 1598	12	0	
2-nour rotal 5:00 PM - 6:00 PM*		0	0	0	0	1958 991	0	0	135	0	16 12	0	0	1598 811	12 3	0	
Peak Hour Factor		U	U	U	U	0.879	U	U	0.587	U	0.500	U	U	0.930	0.375	U	
Heavy Vehicle Percentage					0.00%	2.25%			0.00%		0.00%			1.88%	0.00%	0.00%	
OnStreet Bicycle Count																	
(5:00 PM - 6:00 PM)		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Time	SB/W**		WB/N Pedestrian		NB/W	NB/E	EB/N	EB/S		0 ←	1	→	Ů	t t	0 ←	1	→
4:00 PM to 5:00 PM	0	0	0	0	0	0	0	0				0	5	0	54	0	12
5:00 PM to 6:00 PM	0	0	0	0 On Sidewal	0	0	0	0				0 811		-			
4:00 PM to 5:00 PM	0	0	0	0	0	0	0	0				3	- ₹	-			
5:00 PM to 6:00 PM	0	0	0	0	0	0	0	0					•	J			
** SB/W - Pedestrian/cycle tra									ı		Diagram fo	r:	PM Peak I	Hour		1	
											Peak Hour	:	5:00 PM -	6:00 PM*			

Project Number: 30900399A **115D** PM TURNING MOVEMENT COUNTS CoSA Culebra Road Corridor Study TX 1604 to I-10 North/South Street: East/West Street: Site Code: UnSig_9_ Right 13 18 11 11 11 12 17 5 4:00:00 PM 4:15 PM 4:30 PM 4:45 PM 5:00 PM 5:15 PM 5:30 PM 0 0 0 0 0 0 0 0 hrough 211 213 225 198 219 224 211 24 15 11 15 10 11 11 13 34 27 23 32 23 29 37 237 206 237 216 264 224 223 245 1852 956 0.905 2.70% 5:45 PM 220 1721 2-hour Total 5:00 PM - 6:00 PM* Peak Hour Factor Heavy Vehicle Percentage OnStreet Bicycle Count (5:00 PM - 6:00 PM) 5:00 PM - 6:00 PM Peak Hour 121 0.818 2.29% 1/21 18 874 0.563 0.975 14.81% 1.98% 38 0.559 3.30% 0.00% 0 0 0 0 0 0 0 0 0 ↑ 121 ← 956 ↓ 0 **G** 0 | Fairgrounds Pkwy | Culebra Rd | Culebra Rd | Southbound | Westbound | Northbound | Eastbound | SB/W** | SB/E | WB/N | WB/S | NB/W | NB/E | EB/N | EB/S | + 956 ↓ 0 G 0 A ← ↑ → 4:00 PM to 5:00 PM 5:00 PM to 6:00 PM

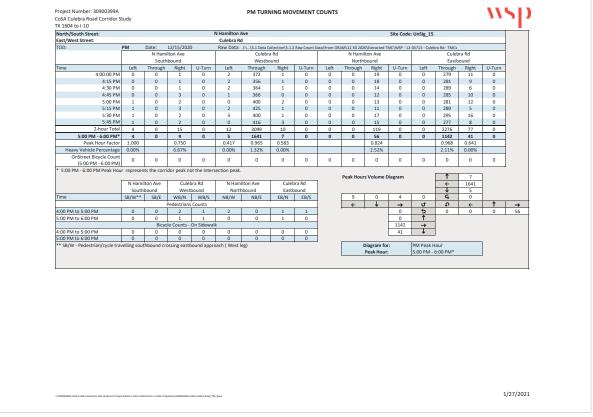
Company Comp	Project Number: 30900399A CoSA Culebra Road Corridor: TX 1604 to I-10	itudy					PM TURN	IIIVG IVIO	VEIVIEIVI	COUNTS							11	יור
EarlyMest Street:													Site Code	UnSig 10				
Top: PM						Culebra R	d						Site code.	011516_10				
Southbound		PM	Date:	12/15/2020)			ollection\5.1.2	Raw Count D	ata\From GR	AM\12 30 202	0\Extracted 1	MC\WSP - 12-	03713 - Culel	ora Rd - TMCs			
Time																		Ī
# 4.5PM 0 0 0 0 0 112 225 0 0 0 0 0 6 6 0 0 235 27 0 0 0 4.5PM 4.5PM 0 0 0 0 0 16 6 227 0 0 0 0 0 16 6 0 0 288 22 0 0 0 0 0 0 16 6 0 0 288 22 0 0 0 0 0 0 16 6 0 0 288 22 0 0 0 0 0 0 14 6 0 0 288 22 0 0 0 0 0 0 14 6 0 0 207 32 0 0 0 0 0 14 6 0 0 207 32 0 0 0 0 0 14 6 0 0 207 32 0 0 0 0 0 0 14 6 0 0 0 0 0 0 15 6 223 0 0 0 0 0 0 18 0 0 0 203 33 0 0 0 0 0 0 18 0 0 0 0 0 0 0 0 0 0 0 0			South					und			North	bound				ound		
## 4:15 PM																		
## 4:30 PM 0 0 0 0 22 237 0 0 0 0 21 0 212 25 0 0 4:4 5 PM 6 ## 4:54 PM 0 0 0 0 0 16 222 0 0 0 0 0 14 0 0 207 32 0 0 0 0 0 0 15 0 0 203 33 0 0 0 0 0 0 0 15 0 0 0 0 0 0 0 0 0 0 0																		
## 5.00 PM = 0 0 0 0 16 229 0 0 0 0 0 14 0 0 207 32 0 0 0 0 0 0 15 0 0 0 0 0 0 0 0 0 0 0 0																		
S.00 PM 0																		
S.15 PM																		
S.30 PM																		
2-heur Total																		
Southbound Sou	5:45 PM	0	0	0	0	17	239	0	0	1	0	15	0	0	221	31	0	
Peak Hour Factor 0.657 0.963 0.250 0.869 0.863 0.863 0.861	2-hour Total	0	0	0	0	137	1839	0	0	1	0	130	0	0	1742	251	0	
Heavy Vehicle Percentage			0	0	0			0	0		0		0	0			0	
Construct Dicycle Count Co																		
S.00 PM - 6.00 PM O						0.00%	1.63%			0.00%		0.00%			1.15%	1.59%		
S.00 PM - 6.00 PM Peak Hour represents the corridor peak not the intersection peak.			0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	
Southbound Supplies Suppli			to the count]
Southbound Culebra Rd Northbound Eastbound Eastbound Fastbound Eastbound Fastbound	* 5:00 PM - 6:00 PM Peak Ho	ır represen	ts the corri	.dor peak no	ot the inter	section peak					Book Hour	r Volumo I	Diagram		4	0	1	
Southbound Westbound Northbound Eastbound Satbound Sa				Culei	ra Rd	Canto	ahury St	Culet	ora Rd	1	reak noui	s volume i	Jiagi aiii					
Time		South	bound															
Pedestrians Counts 400 PM to 5:00 PM	Time										0	0	0	0				
4.00 PM to 5:00 PM 0 0 0 1 0 1 0 1 0 0 0 0 0 0 0 0 0 0 0						,	, -	/	,-		+	1	→	Ú	Ð	+	1	→
5.00 PM to 5:00 PM	4:00 PM to 5:00 PM	0				0	0	0	0									
4.00 PM to 5.00 PM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	5:00 PM to 6:00 PM	0	0	1	0	1	0	1	0				0	1		-		
5.00 PM to 6:00 PM 0 0 1 0 0 0 0 0 0 Diagram for: PM Peak Hour Peak Hour: 5:00 PM - 6:00 PM*			Bicyc	le Counts -	On Sidewal													
** SB,W - Pedestrian/cycle travelling southbound crossing eastbound approach (West leg) Diagram for: Peak Hour: PM Peak Hour:													145	+				
Peak Hour: 5:00 PM - 6:00 PM*								0	0								1	
	** SB/W - Pedestrian/cycle tra	velling sout	:hbound cro	ossing eastb	ound appr	oach (West I	leg)											
distance.																		
d Parlament																		
d Tables																		
d Parlings																		
4/7/1994																		
1/30000399A CoSA Culders Road Corridor Roady (Co. Project Data) (-) Data Collection (S. 1.3 Data Tabulation), 30000399A CoSA Culders Road J M. Revs 1																		

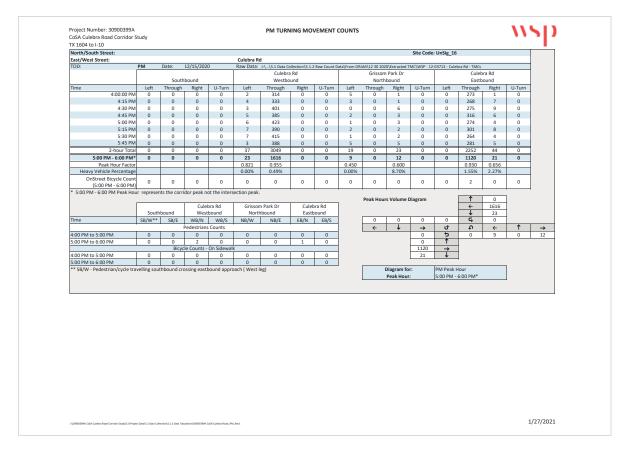
East/West Street: TOD: PN Time 4:00:00 PM 4:15 PM 4:30 PM		Date:	12/15/2020)	Paur Data:											
4:00:00 PM 4:15 PM	Loft					J:\\5.1 Data Co	llection\5.1.2	Raw Count D	ata\From GR	AM\12 30 202	0\Extracted T	MC\WSP - 12-	03713 - Cule	bra Rd - TMCs		
4:00:00 PM 4:15 PM	Loft					Culebra				Horten					ra Rd	
4:00:00 PM 4:15 PM	Loft		bound			Westbo				North					ound	
4:15 PM		Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn
	0	0	0	0	10	218	0	1	5	0	13	0	0	170	5	0
4:30 PM	0	0	0	0	3	234	0	0	4	0	14	0	0	203	7	0
	0	0	0	0	8	219	0	0	6	0	8	0	0	188	1	0
4:45 PM 5:00 PM	0	0	0	0	9	213 236	0	0	8	0	10 8	0	0	189 194	6	0
5:00 PM 5:15 PM	0	0	0	0	8	236	0	0	10	0	6	0	0	218	1	0
5:15 PM 5:30 PM	0	0	0	0	12	2/3	0	0	8	0	5	0	0	218	5	0
5:45 PM	0	0	0	0	9	227	0	0	9	0	12	0	0	201	7	0
2-hour Total	0	0	0	0	66	1832	0	1	56	0	76	0	0	1570	38	0
5:00 PM - 6:00 PM*	0	0	0	0	36	948	0	0	33	0	31	0	0	820	19	0
Peak Hour Factor					0.750	0.868			0.825		0.646			0.940	0.679	
Heavy Vehicle Percentage					0.00%	1.47%		0.00%	1.79%		1.32%			0.06%	5.26%	
OnStreet Bicycle Count	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
(5:00 PM - 6:00 PM)															Ů	
* 5:00 PM - 6:00 PM Peak Hour re	represent	s the corri	dor peak no	ot the inter	ection peak.											
			0.1.1	ora Rd		ncia Ave	0.1.1	ora Rd	i	Peak Hour	s Volume D	lagram		<u>↑</u>	0 948	
	Southb	ound		bound		nbound		ound						1	36	
Time SE	B/W**	SB/E	WB/N	WB/S	NB/W	NB/E	EB/N	EB/S		0	0	0	0	Ğ	0	
-	-,		edestrians		,	,	,	,-		+	1	→	ÚŤ.	۵	+	↑
4:00 PM to 5:00 PM	0	0	0	0	0	0	0	0				0	5	0	33	0
5:00 PM to 6:00 PM	0	0	0	0	0	0	1	0				0	Ť			
*		Bicyc	le Counts -	On Sidewal	k							820	→	1		
4:00 PM to 5:00 PM	0	0	0	0	0	0	0	0				19	→			
5:00 PM to 6:00 PM	0	0	0	0	0	0	0	0								
** SB/W - Pedestrian/cycle travelli	ling south	bound cro	ssing eastb	ound appr	oach (West I	eg)					Diagram for		PM Peak			
											Peak Hour		5:00 PM -	6:00 PM*		

Time	North/South Street: East/West Street:					Mira Vista Culebra Ro							Site Code:	UnSig_12				
Southbound		PM	Date:	12/15/2020)			ollection\5.1.2	Raw Count D	ata\From GR	AM\12 30 2020	\Extracted T	MC\WSP - 12-	03713 - Culel	ora Rd - TMCs			
Time			Mira	Vista			Culebra	a Rd							Culeb	ra Rd		
## 4:00:00 PM																		
415 PM																		
4:30 PM 6 0 7 0 0 237 10 0 0 0 0 5 197 0 0 0																		
A45PM 5																		
S.O.PM																		
S:30 PM 9 0 12 0 0 217 7 0 0 0 0 0 11 209 0 0																		
S.30 PM 9 0 12 0 0 217 7 0 0 0 0 0 11 209 0 0		7	0	18	0	0	242	9	0	0	0	0	0	10	238	0	0	
2-hour Total 53 0 82 0 0 1841 57 0 0 0 0 0 63 1755 0 0 5.00 PM -6.00 PM* 31 0 46 0 0 924 32 0 0 0 0 0 0 37 899 0 0 Peak hour Factor 0.861 0.639 0.055 0.889 0 0.841 0.944 0.944 Heavy Vehicle Percentage 0.00% 1.22% 0.00% 0.22% 1.75% 0.00% 0.23% 0.00% 0.23% 0.00% 0.23% 0.00% 0.23% 0.00% 0.23% 0.00% 0.23% 0.00% 0.23% 0.00% 0.23% 0.00% 0.23% 0.00% 0.00% 0.23% 0.00% 0.00% 0.23% 0.00% 0.00% 0.23% 0.00% 0.00% 0.23% 0.00% 0.00% 0.23% 0.00%		9																
S.00 PM - 6:00 PM 31																		
Peak Hour Factor 0.851 0.639 0.09% 0.22% 1.75% 0.00% 0.22% 1.75% 0.00% 0.22% 1.75% 0.00% 0.22% 0.00% 0.22% 1.75% 0.00% 0.23% 0.00%					_				_	_						_		
Heavy Wehicle Percentage 0.00% 1.22% 0.00% 0.22% 1.75% 0.00% 0.00% 0.23% 0.00%			0		0	0			0	0	0	0	0			0	0	
OnStreet Bicycle Count (\$5.00 PM - 6.00 PM) 0 0 0 0 0 1 0 0 0 1 1 0 0 0 0 0 1 1 1 0 0 0 0 0 0 0 1 1 1 0 0 0 0 0 0 0 0 1 1 1 0						0.000/												
**S.OP.PM -6.00 PM] O O O O O O O O O O O O O O O O O O O																		
** 5.00 PM - 6:00 PM Peak Hour represents the corridor peak not the intersection peak. Mira Vista		0	0	0	0	0	1	0	0	0	0	0	0	1	1	0	0	
Mira Vista Culebra Rd Southbound Westbound Northbound Eastbound Eastbound SB/W** SB/F WB/N WB/S NB/W NB/F ER/N EB/S 46 0 31 0 0 0 0 0 0 0 0 0	* 5:00 PM - 6:00 PM Peak Hou	r represen	its the corri	dor peak no	ot the inters	ection peak.												
Southbound Westbound Westbound Southbound Eastbound Southbound Southb											Peak Hour	s Volume D	Diagram					
Time																		
Pedestrians Counts											45		24					
4:00 PM to 5:00 PM	Time	SB/W**				NB/W	NB/E	EB/N	EB/S				_				•	
5:00 PM to 6:00 PM	4-00 DM +o E-00 DM	0				n	0	0	0		+	*						
## SB/W - Pedestrian/cycle travelling southbound crossing eastbound approach (West leg)															-			
5:00 PM to 6:00 PM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0			Bicyc	le Counts -	On Sidewal	c								→				
** SB/W - Pedestrian/cycle travelling southbound crossing eastbound approach (West leg) Diagram for: PM Peak Hour	4:00 PM to 5:00 PM												0	→				
								0	0								1	
<u>Реак ноиг</u> ј≾∪ РИ- 6.0∪ РИ-	** SB/W - Pedestrian/cycle tra	relling sout	hbound cro	ssing eastb	ound appro	ach (West I	eg)											
												Peak Hour		5:00 PM -	6:00 PM*			

TX 1604 to I-10																	
North/South Street:					NW 34th 9						Sit	e Code: Un	Sig_13				
East/West Street: TOD:	PM	Date:	12/15/2020		Culebra R	d J:\\5.1 Data Ci											
TOD:	PIVI	Date:	12/15/2020	J	Kaw Data:	J:\\5.1 Data G		Raw Count D	ata\From GR		4th St	MC\WSP - 12-	03713 - Culet		ora Rd		
		South	bound			Westbo					bound				oound		
Time	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	
4:00:00 PM	0	0	0	0	4	262	0	0	2	0	4	0	0	203	1	0	
4:15 PM		0	0	0	1	245	0	0	8	0	2	0	0	241	2	0	
4:30 PM		0	0	0	2	248	0	0	7	0	7	0	0	214	2	0	
4:45 PM		0	0	0	1	253	0	0	4	0	2	0	0	215	5	0	
5:00 PM		0	0	0	1	303	0	0	2	0	4	0	0	224	7	0	
5:15 PM 5:30 PM		0	0	0	2	282 270	0	0	5 4	0	5 2	0	0	218 209	7	0	
5:45 PM		0	0	0	3	265	0	0	7	0	1	0	0	198	9	0	
2-hour Tota		0	0	0	17	2128	0	0	39	0	27	0	0	1722	37	0	
5:00 PM - 6:00 PM*		0	0	0	9	1120	0	0	18	0	12	0	0	849	27	0	
Peak Hour Factor					0.750	0.924			0.643		0.600			0.948	0.750		
Heavy Vehicle Percentage					0.00%	1.32%			0.00%		11.11%			1.51%	2.70%		
OnStreet Bicycle Count		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
(5:00 PM - 6:00 PM)							_	_	_	_					_	
* 5:00 PM - 6:00 PM Peak Ho	ır represer	nts the corn	idor peak n	ot the inters	ection peak.					Peak Hour	- \/-l [·		1	0	Т	
			Cule	bra Rd	NW	34th St	Culei	ora Rd	1	reak noui	s volume L	лавтатт		+	1120	+	
	South	nbound		bound		bound		oound						Ì	9	t	
Time	SB/W**	SB/E	WB/N	WB/S	NB/W	NB/E	EB/N	EB/S		0	0	0	0	G,	0	1	
			Pedestrians							+	+	→	ዕ	t	+	1	→
4:00 PM to 5:00 PM	0	0	0	0	0	0	1	0				0	ð,	0	18	0	12
5:00 PM to 6:00 PM	0	0	1	0	0	0	4	0				0 849	1	1			
4:00 PM to 5:00 PM	0	0	0	On Sidewal	0	0	0	0				27	→	-			
5:00 PM to 6:00 PM	0	0	0	0	0	0	0	0				27		J			
** SB/W - Pedestrian/cycle tra						eg)			J		Diagram for	r:	PM Peak H	lour		ī	
											Peak Hour		5:00 PM -	6:00 PM*			

Project Number: 30900399A CoSA Culebra Road Corridor						PM TURN	NING MO	VEMENT	COUNTS							1151
TX 1604 to I-10					Neff Rd							674 - 6 - 1				
North/South Street: East/West Street:					Culebra R							Site Code	Unsig_14			
TOD:	PM	Date:	12/15/2020)		J:\\5.1 Data Co	nllaction\S 1 3	Pau Count D	ata\Erom GD/	M() 12 20 202	\\Evtracted T	MC\WSD - 12	12712 - Culai	ra Pd - TMCr		
100.			f Rd	,	nuw Dutu.	Culebra		Naw Count D	uta prom Gio	Nef		100/1001 - 22	33713 - Culci		ora Rd	
			bound			Westbo				North				Eastb		
Time	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through		U-Turn
4:00:00 PM		0	1	0	4	365	2	0	3	0	2	0	2	268	1	0
4:15 PM	1 2	0	0	0	3	323	5	0	1	0	1	0	0	291	3	0
4:30 PM	1 0	0	5	0	4	399	0	0	1	1	5	0	2	285	1	0
4:45 PM	0	0	7	0	1	338	1	0	1	0	2	0	5	250	5	0
5:00 PM		0	1	0	5	415	2	0	2	1	1	0	5	290	0	0
5:15 PM		0	2	0	2	381	0	0	1	0	3	0	2	273	1	0
5:30 PM	1 2	0	1	0	6	421	3	0	4	0	1	0	1	317	0	0
5:45 PM		0	1	0	5	386	1	0	1	0	2	0	1	270	3	0
2-hour Tota		0	18	0	30	3028	14	0	14	2	17	0	18	2244	14	0
5:00 PM - 6:00 PM		0	5	0	18	1603	6	0	8	1	7	0	9	1150	4	0
Peak Hour Factor			0.625		0.750	0.952	0.500		0.500	0.250	0.583		0.450	0.907	0.333	
Heavy Vehicle Percentage			0.00%		0.00%	1.42%	0.00%		0.00%	0.00%	0.00%		0.00%	1.96%	0.00%	
OnStreet Bicycle Coun (5:00 PM - 6:00 PM		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
* 5:00 PM - 6:00 PM Peak Ho		ats the corri	dor neak no	t the intere	ection neak											
3.00 TW 0.00 TW CURTO	ar represen	its the com	uoi peuk ne	re une miter.	rection peak					Peak Hour	s Volume D	Diagram		1	6	
	Ne	ff St	Culeb	ora Rd	Ne	eff St	Culet	ora Rd						+	1603	
		bound		oound		hbound		ound						Ţ	18	
Time	SB/W**	SB/E	WB/N	WB/S	NB/W	NB/E	EB/N	EB/S		5	0	5	0	4	0	
		•	Pedestrians	Counts		•		•		+	+	→	Ů.	t	+	↑ →
4:00 PM to 5:00 PM	0	1	1	0	0	0	2	0				0	5	0	8	1 7
5:00 PM to 6:00 PM	0	0	3	1	0	0	2	1				9	1			
			le Counts -					,				1150	→			
4:00 PM to 5:00 PM	0	0	0	0	0	0	0	0				4	+			
5:00 PM to 6:00 PM	0	0	1	0	0	0	0	0					PM Peak H			
** SB/W - Pedestrian/cycle tra	iveiling sour	tnbound cri	ossing easte	ound appro	bach (West i	ieg)					iagram for Peak Hour		5:00 PM -			
											Peak nour		5.00 PIVI -	D.UU PIVI		
7/30000000 GGG Cuber Hast Comiter Budgi S Pri	set Duta/S. 1 Duta Coll	lection),S.1.3. Data Tab	ulasion),20902399A C	sSA Culabra Road, Ph	d,flevi											1/27/202



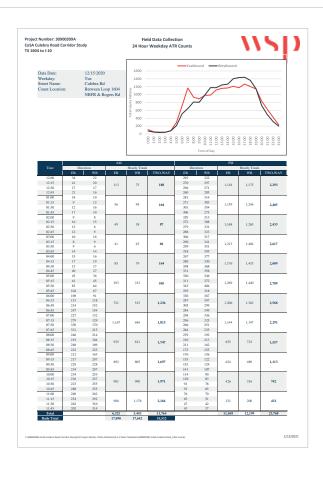


Project Number: 30900399A CoSA Culebra Road Corridor Study TX 1604 to I-10 North/South Street: East/West Street: **115D** PM TURNING MOVEMENT COUNTS Site Code: UnSig_17 4:00:00 PM 4:15 PM 4:30 PM 4:45 PM 5:00 PM 5:15 PM 5:30 PM 228 226 250 236 282 249 285 hrough 175 193 172 196 207 200 171 171 0 196 0 1510 4 774 2 0.935 0.500 2.12% 25.00% 234 1990 1050 0.921 1.21% 5:45 PN 0.250 0.750 0.00% 5.56% 0 0 2-hour Total 5:00 PM - 6:00 PM* Peak Hour Factor Heavy Vehicle Percentage OnStreet Bicycle Count (5:00 PM - 6:00 PM) 5:00 PM - 6:00 PM Peak Hour 9 0.450 0.00% 1 0.250 0.00% 0 0 0 0 0 0 0 1050 1 Southbound Culebra Rd Westbound NW 28th St Northbound Culebra Rd Eastbound SB/W** SB/E WB/N WB/S NB/W NB/E EB/N EB/S ← 1050 ↓ 1 G 1 n ← ↑ → 4:00 PM to 5:00 PM 5:00 PM to 6:00 PM

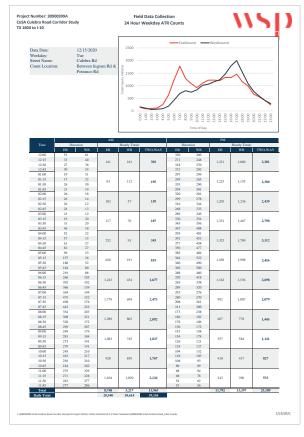
North/South Street:					N Navidad	St					Sit	e Code: Un	Sig 18				-
East/West Street:					Culebra R												
TOD:	PM		12/15/2020)						Data\From GRAM\12 30 2020\Extracted TMC\WSP - 12-03713 - Culebra Rd - TMCs							
		N Navi South	idad St		Culebra Rd Westbound				N Navidad St Northbound				Culebra Rd Eastbound				
Time	Left	Through	Right	Westbound U-Turn Left Through Right U-Turn					Northbound Left Through Right U-Turn				Left Through Right U-Turn				
4:00:00 PM		0	3	0	0	342	2	0	0	0	9	0	0	307	8	0	
4:15 PM		0	2	0	0	367	2	0	0	0	9	0	0	257	3	0	
4:30 PM		0	6	0	0	350	2	0	0	0	13	0	0	304	4	0	
4:45 PM	0	0	3	0	0	344	5	0	0	0	6	0	0	239	8	0	
5:00 PM	0	0	7	0	0	362	2	0	0	0	7	0	0	288	8	0	
5:15 PM		0	5	0	0	382	1	0	0	0	9	0	0	284	1	0	
5:30 PM		0	1	0	0	394	5	0	0	0	3	0	0	265	8	0	
5:45 PM		0	3	0	0	374	2	0	0	0	8	0	0	266	10	0	
2-hour Total		0	30	0	0	2915	21	0	0	0	64	0	0	2210	50	0	
5:00 PM - 6:00 PM*		0	16	0	0	1512	10	0	0	0	27	0	0	1103	27	0	
Peak Hour Factor			0.571			0.959	0.500				0.750			0.957	0.675		
Heavy Vehicle Percentage			0.00%		0.00%	1.82%	0.00%				1.56%			2.62%	2.00%		
OnStreet Bicycle Count (5:00 PM - 6:00 PM)		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
* 5:00 PM - 6:00 PM Peak Ho		nts the corri	dor neak no	t the inters	ection neak												
			р							Peak Hour	s Volume D	Diagram		1	10	1	
	N Nav	ridad St	t Culebra Rd			vidad St	Culet	ora Rd						+	1512		
	South	bound	West	oound	Norti	nbound	Eastb	ound						+	0		
Time	SB/W**	SB/E	WB/N	WB/S	NB/W	NB/E	EB/N	EB/S		16	0	0	0	G,	0		
			Pedestrians	Counts						+	+	→	Ú	ţ	+	1	→
4:00 PM to 5:00 PM	0	0	0	1	0	0	1	0				0	5	0	0	0	27
5:00 PM to 6:00 PM	0	2	0	2	0	0	0	0				0	1				
				On Sidewal								1103	→				
4:00 PM to 5:00 PM	0	0	0	0	0	0	0	0				27	↓	J			
5:00 PM to 6:00 PM ** SB/W - Pedestrian/cycle tra	0	0	0	0	0	0	0	0			Diagram for		PM Peak H			1	
,, -, -,											Peak Hour		5:00 PM -]	

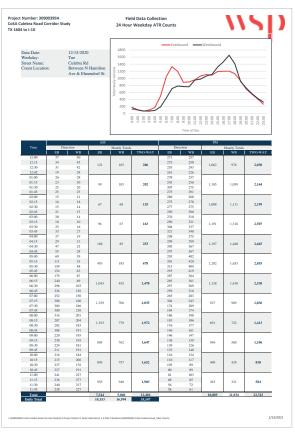
1/27/2021

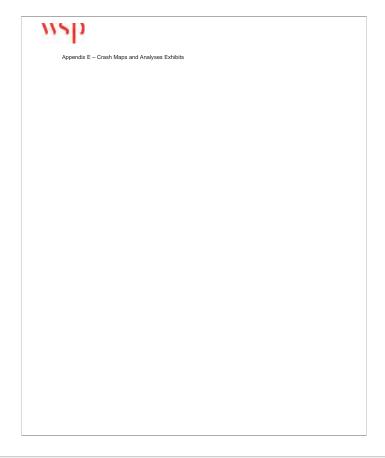
Project Number - 200002029. Cosis Culebra Road Correfor Study TX 1004 to 1-10 2808 ATR Counts Data Collected: December 15, 2020

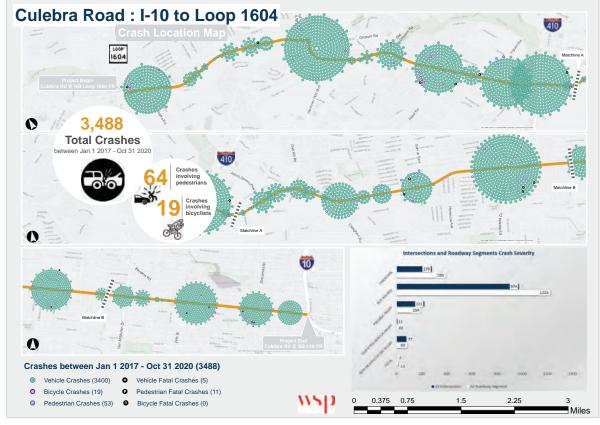


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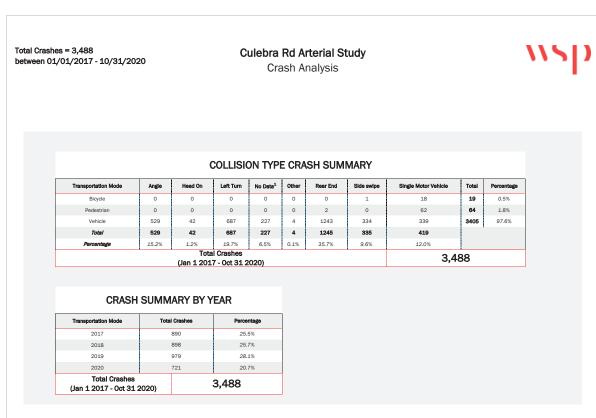




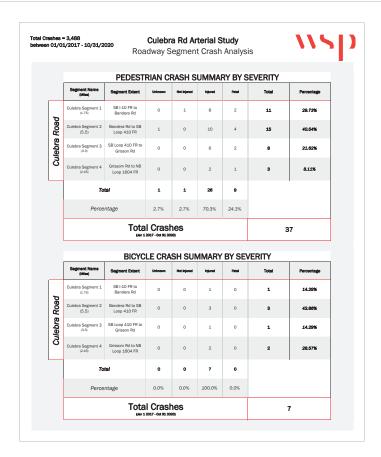


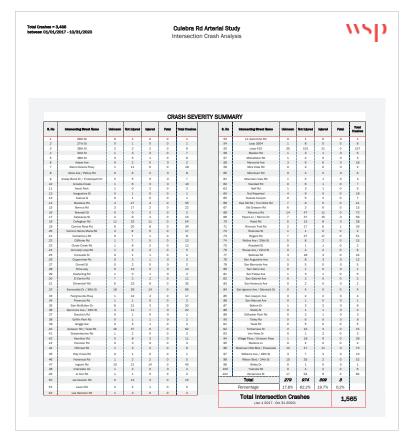


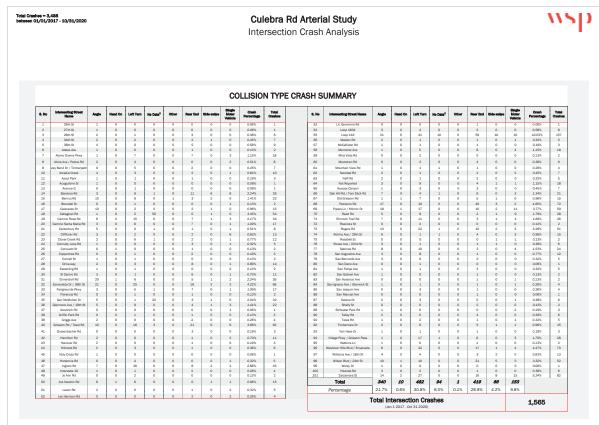




ehes = 3,488 01/01/2017 - 10/31/2020		Culebra Rd Arterial Study Roadway Segment Crash Analysis												
				CR.	ASH SL	JMMAR	Y BY CO	DLLISION 1						
	Segment Name (killes)	Segment Extent	Angle	Head On	Left Turn	No Data ¹	Other	Rear End	Side swipe	Single Motor Vehicle	Total	Percentage		
Ţ	Culebra Segment 1	SB I-10 FR to Bandera Rd	38	10	29	1	0	185	69	60	392	20.38%		
800	Cullebra Segment 2 (5.5)	Bandera Rd to SB Loop 410 FR	53	10	73	65	3	263	85	94	646	33.59%		
Pulahra Pradi	Culebra Segment 3	SB Loop 410 FR to Grisson Rd	67	7	59	43	0	243	78	73	570	29.64%		
Š	Culebra Segment 4	Grissom Rd to NB Loop 1604 FR	31	5	44	24	0	135	37	39	315	16.38%		
<u> </u>	To	i etal	189	32	205	133	3	826	289	266		•		
	Perce	ntage	9.8%	1.7%	10.7%	6.9%	0.2%	43.0%	14.0%	13.8%				
					Crashe						1,923			
		CRA	SH SUM	MMARY	BY SE	VERITY								
	Segment Name (killes)	Segment Extent	Unknown	Not Injured	Injured	Fetal	Total	Percentage						
-	Culebra Segment 1	SB I-10 FR to Bandera Rd	82	244	63	3	392	20.38%						
Pulahra Pradi	Culebra Segment 2 (5.5)	Bandera Rd to SB Loop 410 FR	139	394	108	5	646	33.59%						
sylva	Cullebra Segment 3	SB Loop 410 FR to Grisson Rd	106	377	84	3	570	29.64%						
Š	Culebra Segment 4 (2.45)	Grissom Rd to NB Loop 1604 FR	63	208	42	2	315	16.38%						
_	70	otal	390	1223	297	13								
	Perce	ntage	20.3%	63.6%	15.4%	0.7%								
		Total Crashes 1,923												







Total Crashes = 3,488 between 01/01/2017 - 10/31/2020

Culebra Rd Arterial Study Intersection Crash Analysis



BICYCLE INTERSECTION CRASH SUMMARY BY COLLISION TYPE

S. No	Intersecting Street Name	Rear End	Single Motor Vehicle	Total	Percentage
1	Cliffbrier Rd		3	3	25.00%
2	Hamilton Rd		1	1	8.33%
3	Ingram Rd		1	1	8.33%
4	Pipers Ln / Micron Dr		1	1	8.33%
5	Reed Rd		2	2	16.67%
6	Rimrock Trail Rd		1	1	8.33%
7	Rogers Rd	1		1	8.33%
8	Stillwater Park Rd		1	1	8.33%
9	Village Pkwy / Grissom Pass		1	1	8.33%
	Total	1	• 11		
	Percentage	8.33%	91.67%		
	Total Crashe	-			12

PEDESTRIAN INTERSECTION CRASH SUMMARY BY COLLISION TYPE

S. No	Intersecting Street Name	Rear End	Single Motor Vehicle	Total	Percentage
1	Alamo Downs Pkwy	0	1	1	3.70%
2	Alicia Ave / Pettus Rd	0	1	1	3.70%
3	Bandera Rd	0	1	1	3.70%
4	Camino Rosa Rd	0	1	1	3.70%
5	Esmeralda Dr / 36th St	0	1	1	3.70%
6	Laven Rd	0	1	1	3.70%
7	Memorial Ave	0	2	2	7.41%
8	Oak Hill Rd / Tom Slick Rd	0	1	1	3.70%
3	Patranco Rd	0	1	1	3.70%
4	Pipers Ln / Micron Dr	0	3	3	11.11%
5	Reed Rd	0	1	1	3.70%
6	Rogers Rd	0	1	1	3.70%
7	Rollins Ave / 29th St	0	1	1	3.70%
8	Sabinas Rd	0	2	2	7.41%
9	Wilson Blvd / 24th St	0	2	2	7.41%
10	Zarzamora St	1	6	7	25.93%
	Total	1	26		
	Percentage	3.70%	96.30%		
	Total Crashe (Jan 1 2017 - Oct 3			2	27

Culebra Rd Arterial Study Assumptions Total Crashes = 3,488 between 01/01/2017 - 10/31/2020

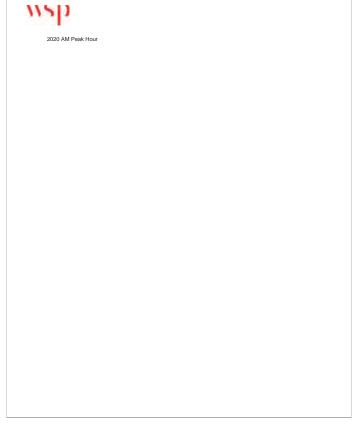
Crash Collision Type	Assumption
ANGLE - BOTH GOING STRAIGHT	Angle
ANGLE - BOTH LEFT TURN	LT
ANGLE - ONE LEFT TURN-ONE STOPPED	LT
ANGLE - ONE RIGHT TURN-ONE LEFT TURN	Angle
ANGLE - ONE RIGHT TURN-ONE STOPPED	Angle
ANGLE - ONE STRAIGHT-ONE BACKING	Angle
ANGLE - ONE STRAIGHT-ONE LEFT TURN	LT
ANGLE - ONE STRAIGHT-ONE RIGHT TURN	Angle
ANGLE - ONE STRAIGHT-ONE STOPPED	Angle
No Data ¹	No Data ¹
ONE MOTOR VEHICLE - BACKING	SMV
ONE MOTOR VEHICLE - GOING STRAIGHT	SMV
ONE MOTOR VEHICLE - OTHER	SMV
ONE MOTOR VEHICLE - TURNING LEFT	SMV
ONE MOTOR VEHICLE - TURNING RIGHT	SMV
OPPOSITE DIRECTION - BOTH GOING STRAIGHT	Head On
OPPOSITE DIRECTION - BOTH LEFT TURNS	LT
OPPOSITE DIRECTION - ONE BACKING-ONE STOPPED	Rear End
OPPOSITE DIRECTION - ONE LEFT TURN-ONE STOPPED	LT
OPPOSITE DIRECTION - ONE RIGHT TURN-ONE LEFT TURN	Angle
OPPOSITE DIRECTION - ONE STRAIGHT-ONE BACKING	Angle
OPPOSITE DIRECTION - ONE STRAIGHT-ONE LEFT TURN	LT
OPPOSITE DIRECTION - ONE STRAIGHT-ONE STOPPED	Head On
OTHER	Other
SAME DIRECTION - BOTH GOING STRAIGHT-REAR END	Rear End
SAME DIRECTION - BOTH GOING STRAIGHT-SIDESWIPE	Side Swipe
SAME DIRECTION - BOTH LEFT TURN	LT
SAME DIRECTION - BOTH RIGHT TURN	Angle
SAME DIRECTION - ONE LEFT TURN-ONE STOPPED	LT
SAME DIRECTION - ONE RIGHT TURN-ONE STOPPED	Angle
SAME DIRECTION - ONE STRAIGHT-ONE LEFT TURN	LT
SAME DIRECTION - ONE STRAIGHT-ONE RIGHT TURN	Angle
SAME DIRECTION - ONE STRAIGHT-ONE STOPPED	Rear End

Total Crashes = 3,488 between 01/01/2017 - 10/31/2020	Culebra Rd Arterial Study Assumptions	115]
		•

Crash Severity Type	Assumption
Killed	Fatal
Suspected serious injury	Injured
Incapacitating serious injury	Injured
Possible injury	Injured
Not injured	Not Injured
Unknown	Unknown

Road	Rd
Frontage Road	FR
Street	St
Avenue	Ave
Drive	Dr
Number	#
Serial Number	S.No
Left Turn	LT
Single Motor Vehicle	SMV
Not applicable since crashes were collected only until Oct 31 2020.	NA ¹
Southbound	NB
Northbound	SB
Eastbound	EB
Westbound	WB

Appendix F – Existing Condition Analyses Synchro Outputs



Lane Configurations Tinffic Volume (yph) 1247 1610 0 0 700 180 566 111 325 0 0 10 Future Volume (yph) 1247 1610 0 0 700 180 566 111 325 0 0 10 Future Volume (yph) 1247 1610 0 0 700 180 566 111 325 0 0 10 185 510 0 10 180 566 111 325 0 0 10 180 566 11 325 0		•	→	•	•	←	•	1	1	1	-	↓	4
Traffic Volume (ph) 247 1610 0 0 700 180 566 111 325 0 0 0 Itelael Flow (vphp) 247 1610 0 0 700 180 566 111 325 0 0 0 Itelael Flow (vphp) 2000 2000 2000 2000 2000 2000 2000 20	Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SB
Traffic Volume (ph) 247 1610 0 0 700 180 566 111 325 0 0 0 Itelael Flow (vphp) 247 1610 0 0 700 180 566 111 325 0 0 0 Itelael Flow (vphp) 2000 2000 2000 2000 2000 2000 2000 20		*											
	Traffic Volume (vph)		1610	0	0	700					0	0	
Total Lost time (s) 5.1 5.1 5.1 5.6 5.6 5.1 5.1 5.1 5.1 Lane UBL Factor 0.91 0.91 0.91 1.00 0.85 1.00 1.00 1.00 0.85 0.97 1.00 0.85 1.00 1.00 0.85 0.97 1.00 0.85 1.00 1.00 0.85 0.97 1.00 0.85 1.00 1.00 0.85 0.97 1.00 0.85 1.00 1.00 0.85 0.97 1.00 0.85 1.00 1.00 0.85 0.97 1.00 0.85 1.00 1.00 0.85 0.97 1.00 0.85 1.00 1.00 0.85 0.97 1.00 0.85 1.00 1.00 0.85 0.97 1.00 0.85 0.97 1.00 0.85 0.97 1.00 0.85 0.85 0.88 0.88 0.88 0.88 0.88 0	Future Volume (vph)	247	1610	0	0	700	180	566	111	325	0	0	
Lane UBL Factor 0.91 0.91 0.91 1.00 0.91 0.91 1.00 0.95 1.00 1.00 1.00 0.85 1.00 1.00 1.00 0.85 1.00 1.00 1.00 0.85 1.00 1.00 0.85 1.00 0.85 1.00 0.85 1.00 0.85 1.00 0.85 1.00 0.85 1.00 0.95 3.445 1.667 1.695 0.97 1.00 1.00 0.95 3.445 1.667 1.695 0.97 1.00 1.00 0.95 3.445 1.667 1.695 0.97 1.00 1.00 0.95 3.445 1.667 1.695 0.97 1.00 1.00 0.95 3.445 1.667 1.695 0.97 1.00 1.00 0.95 3.445 1.667 1.695 0.97 1.00 1.00 0.95 3.445 1.667 1.695 0.97 1.00 1.00 0.95 3.445 1.667 1.695 0.97 1.00 1.00 0.95 3.445 1.667 1.695 0.97 1.00 1.00 0.95 3.445 1.667 1.695 0.97 1.00 1.00 0.95 3.445 1.667 1.695 0.97 1.00 1.00 0.95 3.445 1.667 1.695 0.97 1.00 1.00 0.95 3.445 1.667 1.695 0.97 1.00 1.00 0.95 3.445 1.667 1.695 0.97 1.00 1.00 0.95 3.445 1.667 1.695 0.97 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0	Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	201
Fit Protected 0 95 1 00 1 00 1 00 0 85 1 00 1 00 0 85 Fit Protected 0 95 1 00 1 00 1 00 0 05 0 97 1 100 Sald Flow (prot) 1695 3666 5353 1667 1695 3445 1667 Fit Permitted 0 95 100 1 00 1 00 0 05 0 97 1 00 Sald Flow (perm) 1695 3666 5353 1667 1695 3445 1667 Feak-hour factor, PHF 0.88 0.88 0.88 0.88 0.88 0.88 0.88 0.8													
Ell Protected													
Sadd. Flow (prot) 1695 3666 5353 1667 1695 3445 1667 Fleemitted 0.95 1.00 1.00 1.00 0.95 0.97 1.00 Sadd. Flow (perm) 1695 3666 5353 1667 1695 3445 1667 Fleek-hour factor, PHF 0.88 0.88 0.88 0.88 0.88 0.88 0.88 0.8	Frt												
ER Permitted 0.95													
Sald Flow (perm) 1695 3666													
Peak-hour factor, PHF													
Growth Factor (vph)				0.00	0.00						0.00	2.01	_
Adj. Flow (yph)													
RTÓR Reduction (vph) 0 0 0 0 0 173 0 0 293 0 0 0 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1													108
Lane Group Flow (right)													
Heavy Vehicles (%)													
Turn Type													-
Protected Phases Aduated Green, G (s) File Promitted Phases File Promitted				2./0	2./0						2/0	2/0	
Pelmitted Phases 2							Pellii			Pellii			
Actuated Green, G (s) 73 9 73 9 30.4 30.4 19.9 19.9 19.9 Effective Green, g (s) 73.9 73.9 30.4 30.4 19.9 19.9 19.9 Actuated G (Ratio 0.53 0.53 0.22 0.22 0.14 0.14 0.14 Clearance Time (s) 5.5 0.5 0.5 0.22 0.22 0.14 0.14 0.14 Clearance Time (s) 5.6 5.6 5.1 5.1 5.1 Vehicle Extension (s) 1.0 1.0 1.0 1.0 1.0 1.0 Lane G (P Cap (vph) 894 1882 1162 361 240 489 236 vis Ratio P Common 1.0 0.0 1.0 0.0 1.0 0.0 1.0 vis Ratio P Common 1.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0		111	111				2	-	-	1			
Effective Green, g(s) 73.9 73.9 30.4 30.4 19.9 19.9 19.9 Actuated g(r Rein) Green, g(s) 73.9 0.53 0.22 0.22 0.14 0.14 0.14 0.14 0.14 0.14 0.14 0.14		73.9	73.9			30.4		19 9	19 9				
Actuated Q'C Ratio 0.53 0.53 0.22 0.22 0.14 0.14 0.14 0.14 Clearance Time (s) 5.6 5.6 5.1 5.1 5.1 vehicle Extension (s) 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0													
Clearance Time (s)		0.53	0.53			0.22	0.22	0.14	0.14	0.14			
Lane Gr (20 (Apr)) 894 1882 1162 361 240 489 236 wis Ratio Perm 0.16 c0.56 c0.16 c0.20 0.14 vis Ratio Perm 0.31 1.07 0.03 0.06 vis Ratio Perm 0.31 1.07 0.74 0.13 1.45 1.39di 0.45 Uniform Delay, d1 16.6 33.0 51.1 44.2 60.0 59.9 55.0 Progression Factor 0.13 0.52 1.12 2.15 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1	Clearance Time (s)												
vis Ratio Prof 0.16 c0.56 c0.16 c0.20 0.14 vis Ratio Prof 0.05 c0.20 0.14 0.06 vis Ratio Prem 0.31 1.07 0.74 0.13 1.45 1.39dl 0.45 Uniform Delay, d1 18.6 33.0 51.1 44.2 60.0 589 55.0 Progression Factor 0.13 0.52 1.12 2.15 10.0 1.00 1.00 incremental Delay, d2 0.1 35.3 2.1 0.1 222.6 37.5 0.5 Delay (s) 2.5 55.5 Evel of Service 9.0 28.6 97.5 55.5 Level of Service 4.0 9.0 28.6 97.5 55.5 Level of Service 66.7 136.1 0.0 0.0 Approach LOS 0.0 4.0 0.0 4.0 0.0 4.0 0.0 4.0 0.0 4.0 0.0 4.0 0.0 4.0 0.0 4.0 0.0 4.0 0.0 4.0 0.0	Vehicle Extension (s)					1.0	1.0	1.0	1.0	1.0			
wis Ratio Perm 0.03 0.06 vic Ratio 0.31 1.07 0.74 0.13 1.45 1.39d1 0.45 Uniform Delay, d1 18.6 33.0 51.1 44.2 60.0 59.9 55.0 Progression Factor 0.13 0.52 1.12 2.15 1.00 1.00 1.00 Incernmental Delay d2 0.1 33.3 2.1 0.1 22.6 37.5 0.5 Delay (s) 2.5 52.5 59.4 95.0 282.6 97.5 55.5 Opproach Delay (s) 45.5 66.7 136.1 0.0 Agproach Delay (s) 45.5 66.7 136.1 0.0 Agroach Delay (s) 45.0 Agroach Delay (s) 4.0 New Year (s) Agroach Cybe (s) 4.0 New Year (s) 4.0 New Year (s) Agroach (s) E F F F F Agroach (s)	Lane Grp Cap (vph)	894	1882			1162	361	240	489	236			
vic Ratio 0.31 1.07 0.74 0.13 1.45 1.38dI 0.45 Uniform Delay, d1 18.6 33.0 51.1 44.2 60.0 59.9 55.0 Progression Factor 0.13 0.52 1.12 2.15 1.00 1.00 1.00 Incremental Delay, d2 0.1 33.3 2.1 0.1 222.6 37.5 0.5 Delay (s) 2.5 52.5 59.4 95.0 28.6 97.5 55.5 Level of Service A D E F F F E Approach LOS D E F F F A Intersection Summary HCM 2000 Control Delay 75.3 HCM 2000 Level of Service E HCM 2000 Volume to Capacity atio 1.10 Accused Cycle Length (s) 1.40.0 Sum of lost time (s) 21.5 Intersection Capacity Utilization 10.2 % ICU Level of Service G G Analysis Period (min) 15 ICU Level at Lene. R	v/s Ratio Prot	0.16	c0.56			c0.16		c0.20	0.14				
Uniform Delay, d1 18.6 33.0 51.1 44.2 60.0 59.9 55.0 Progression Factor 0.13 0.52 1.12 2.15 1.00 1.00 1.00 Incremental Delay, d2 0.1 35.3 2.1 0.1 222.6 37.5 0.5 Delay (s) 2.5 52.5 59.4 95.0 282.6 97.5 55.5 Approach Delay (s) 46.5 66.7 136.1 0.0 Agrocab Delay (s) 46.5 66.7 136.1 0.0 Agrocab Delay (s) A interestion Summary F A A A A interestion Summary F A A B F F F A A B B F F F A A B B F F F E F A A B B B F F F A A B B B B B B B B	v/s Ratio Perm												
Progression Factor 0.13 0.52 1.12 2.15 1.00 1.00 1.00 Incremental Delay, d2 0.1 35.3 2.1 0.1 222.6 37.5 0.5 Delay (s) 2.5 52.5 594 95.0 282.6 97.5 55.5 Level of Service A D E F F F E Approach Delay (s) 46.5 66.7 138.1 0.0 0 0 A Mintersection Summary F A A A Mintersection Capacity The Company of the Capacity and th													
Incremental Delay, d2													
Delay (s)													
Level of Service													
Approach Delay (s) 46.5 66.7 136.1 0.0 Approach LOS D E F A Intersection Summary Intersection Summary F A HCM 2000 Control Delay 75.3 HCM 2000 Evel of Service E HCM 2000 Volume to Capacity ratio 1.10 A Actuated Cycle Length (s) 140.0 Sum of lost time (s) 21.5 Intersection Capacity Utilization 102.6% ICU Level of Service G Analysis Period (min) 15 G G Intersection Lane. Recode with 1 though lane as a left lane. G F													
Approach LOS D E F A Intersection Summary 1.00		А					F	F		E		0.0	
Intersection Summary 75.3													
HCM 2000 Centrol Delay 75.3 HCM 2000 Level of Service E HCM 2000 Volume to Capacity ratio 1.10 Adutated Cycle Length (s) 140.0 Sum of lost time (s) 21.5 Intersection Capacity Utilization 102.6% ICU Level of Service G Analysis Period (min) 15 dl Defacto Let Lane. Recode with 1 though lane as a let lane.	***		U									А	
HCM 2000 Volume to Capacity ratio 1.10 Actuated Cycle Length (s) 140.0 Sum of lost time (s) 21.5 Intersection Capacity Utilization 102.0% ICU Level of Service G Analysis Period (min) 15 d Defact Let Hane. Recode with 1 though lane as a let lane.													
Actuated Cycle Length (s) 140.0 Sum of lost time (s) 21.5 Intersection Capacity Utilization 102.6% ICU Level of Service G Analysis Period (min) 15 dl Defacto Left Lane. Recode with 1 though lane as a left lane.					Н	CM 2000	Level of	Service		E			
Intersection Capacity Utilization 102.6% ICU Level of Service G Analysis Period (min) 15 I Defacto Let Lane. Recode with 1 though lane as a left lane.		city ratio				()	Pare ()			04.5			
Analysis Period (min) 15 dl Defacto Left Lane. Recode with 1 though lane as a left lane.		e.											
dl Defacto Left Lane. Recode with 1 though lane as a left lane.		шоП			IC	U Level	o Service			G			
		odo with 1	though Is		oft lane								
Consum Lany Group		JOUE WITH T	u iough la	me as a 16	sit lätte.								
	C Cittical Latte Gloup												

	•	→	•	-	-		4	1	/	1	1	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SI
ane Configurations	LUL	444	7	*	414	WDI	NDL	INDI	INDIX	N N	414	
Fraffic Volume (vph)	0	1107	582	300	1000	0	0	0	0	762	449	2
uture Volume (vph)	0	1107	582	300	1000	0	0	0	0	762	449	2
deal Flow (vphpl)	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	20
Fotal Lost time (s)	2000	5.7	5.7	5.1	5.1	2000	2000	2000	2000	5.1	5.1	20
ane Util, Factor		0.91	1.00	0.91	0.91					0.91	0.91	- 1
rt		1.00	0.85	1.00	1.00					1.00	1.00	0
It Protected		1.00	1.00	0.95	1.00					0.95	0.98	1
Satd. Flow (prot)		5353	1667	1695	3563					1695	3490	16
It Permitted		1.00	1.00	0.95	1.00					0.95	0.98	- 1
Satd. Flow (perm)		5353	1667	1695	3563					1695	3490	16
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0
Growth Factor (vph)	108%	108%	108%	108%	108%	108%	108%	108%	108%	108%	108%	10
Adj. Flow (vph)	0	1233	648	334	1113	0	0	0	0	848	500	
RTOR Reduction (vph)	0	0	178	0	0	0	0	0	0	0	0	
ane Group Flow (vph)	0	1233	470	301	1146	0	0	0	0	441	907	
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	
Turn Type		NA	Perm	Split	NA					Perm	NA	Pe
Protected Phases		6		5 15	5 15						8	
Permitted Phases			6							8		
Actuated Green, G (s)		41.3	41.3	55.9	55.9					26.9	26.9	2
Effective Green, g (s)		41.3	41.3	55.9	55.9					26.9	26.9	2
Actuated g/C Ratio		0.29	0.29	0.40	0.40					0.19	0.19	0
Clearance Time (s)		5.7	5.7							5.1	5.1	
/ehicle Extension (s)		1.0	1.0							1.0	1.0	
ane Grp Cap (vph)		1579	491	676	1422					325	670	
/s Ratio Prot		0.23		0.18	c0.32							
/s Ratio Perm			c0.28							c0.26	0.26	0
/c Ratio		0.78	0.96	0.45	0.81					1.36	1.35	0
Jniform Delay, d1		45.2	48.5	30.7	37.2					56.5	56.5	4
Progression Factor		1.00	1.00	0.05	0.70					1.00	1.00	1
ncremental Delay, d2		3.9	31.4	0.1	0.3					179.5	168.9	
Delay (s)		49.1 D	79.9 E	1.8 A	26.5 C					236.0 F	225.4 F	4
evel of Service		59.7	E	A				0.0		F	199.7	
Approach Delay (s) Approach LOS		59.7 E			21.4 C			0.0 A			199.7	
pproach LOS					U			А			г	
ntersection Summary												
ICM 2000 Control Delay			94.1	Н	CM 2000	Level of	Service		F			
HCM 2000 Volume to Capacit	y ratio		1.02									
Actuated Cycle Length (s)			140.0		um of lost				21.5			
ntersection Capacity Utilization	n		102.6%	IC	CU Level of	of Service	1		G			
Analysis Period (min)			15									
: Critical Lane Group												
Culebra Arterial Study 7:15 an	n 12/15/2	9020 2020) AM Evic	ting Cond	lition					91	ynchro 10) Par

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SE
Lane Configurations	*	1	7	- 3	1	7	ř	** 1		ř	** 1	
Traffic Volume (veh/h)	154	18	38	99	24	27	20	900	35	37	2550	- 1
Future Volume (veh/h)	154	18	38	99	24	27	20	900	35	37	2550	- 1
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1856	1856	1856	1856	1856	1856	1856	1856	1856	1856	1856	18
Adj Flow Rate, veh/h	180	21	44	116	28	32	23	1050	41	43	2975	1
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.
Percent Heavy Veh, %	3	3	3	3	3	3	3	3	3	3	3	
Cap, veh/h	203	80	67	207	80	67	306	3625	141	329	2951	1
Arrive On Green	0.06	0.04	0.04	0.06	0.04	0.04	0.14	0.72	0.72	0.04	1.00	1.
Sat Flow, veh/h	1767	1856	1572	1767	1856	1572	1767	5002	195	1767	4922	2
Grp Volume(v), veh/h	180	21	44	116	28	32	23	709	382	43	2025	11
Grp Sat Flow(s),veh/h/ln	1767	1856	1572	1767	1856	1572	1767	1689	1820	1767	1689	18
Q Serve(g_s), s	7.0	1.5	3.9	1.3	2.1	2.5	0.0	10.2	10.2	1.5	0.0	83
Cycle Q Clear(g_c), s	7.0	1.5	3.9	1.3	2.1	2.5	0.0	10.2	10.2	1.5	0.0	83
Prop In Lane	1.00		1.00	1.00		1.00	1.00	0110	0.11	1.00	000#	0.
Lane Grp Cap(c), veh/h	203 0.89	80 0.26	67 0.65	207 0.56	80 0.35	67 0.47	306 0.08	2448 0.29	1319 0.29	329 0.13	2025 1.00	10
V/C Ratio(X)	222	187	158	226	187	158	306	2448	1319	419	2043	10
Avail Cap(c_a), veh/h HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	2.00	2.00	2.
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	0.97	0.97	0.97	0.25	0.25	0.
Uniform Delay (d), s/veh	62.4	64.9	66.0	60.5	65.1	52.7	51.3	6.7	6.7	13.1	0.23	(
Incr Delay (d2), s/veh	29.2	0.6	3.9	1.1	1.0	1.9	0.0	0.7	0.7	0.0	10.0	20
Initial Q Delay(d3).s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	(
%ile BackOfQ(50%),veh/ln	8.0	0.7	1.6	4.0	1.0	1.2	0.7	3.3	3.7	0.6	2.8	- 6
Unsig. Movement Delay, s/veh	0.0	0.,	1.0	1.0	1.0		0.7	0.0	0.1	0.0	2.0	
LnGrp Delay(d),s/veh	91.6	65.5	69.9	61.7	66.1	54.7	51.3	7.0	7.3	13.1	10.0	20
LnGrp LOS	F	E	E	Е	E	D	D	A	A	В	A	_
Approach Vol, veh/h		245			176			1114			3180	
Approach Delay, s/veh		85.5			61.1			8.0			13.8	
Approach LOS		F			E			A			B	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	7.8	106.8	13.5	11.9	25.0	89.6	13.5	11.9				
Change Period (Y+Rc), s	5.0	* 5.3	4.5	5.9	* 5.3	* 5.3	4.5	5.9				
Max Green Setting (Gmax), s	10.0	* 85	10.5	14.1	* 10	* 85	10.5	14.1				
Max Q Clear Time (q. c+l1), s	3.5	12.2	3.3	5.9	2.0	85.9	9.0	4.5				
Green Ext Time (p_c), s	0.0	2.3	0.0	0.0	0.0	0.0	0.0	0.0				
Intersection Summary												-
HCM 6th Ctrl Delay			17.9									
HCM 6th LOS			В									
Notes												
User approved pedestrian inter * HCM 6th computational engin						es crossi	ng the ba	rrier.				
Culebra Arterial Study 7:15 am	12/15/2	กวก วกวก	AM Evic	ina Cond	lition					Si	nchro 10	Pon

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
ane Configurations	*	1		7	ĵ.		7	^^		Ť	^^		
raffic Volume (veh/h)	77	30	167	123	48	116	124	850	25	43	2388	31	
uture Volume (veh/h)	77	30	167	123	48	116	124	850	25	43	2388	31	
nitial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0	
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Vork Zone On Approac		No			No			No			No		
\dj Sat Flow, veh/h/ln	1856	1856	1856	1856	1856	1856	1856	1856	1856	1856	1856	1856	
\dj Flow Rate, veh/h	93	36	202	148	58	140	150	1026	30	52	2882	37	
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	
ercent Heavy Veh, %	3	3	3	3	3	3	3	3	3	3	3	3	
Cap, veh/h	127	43	242	90	85	206	171	3489	102	435	3368	43	
rrive On Green	0.18	0.18	0.18	0.18	0.18	0.18	0.11	1.00	1.00	0.02	0.65	0.65	
at Flow, veh/h	1175	243	1366	1133	482	1164	1767	5058	148	1767	5155	66	
Grp Volume(v), veh/h	93	0	238	148	0	198	150	685	371	52	1884	1035	
Grp Sat Flow(s),veh/h/li		0	1610	1133	0	1646	1767	1689	1829	1767	1689	1844	
Serve(g_s), s	9.0	0.0	20.0	4.8	0.0	15.8	6.1	0.0	0.0	1.4	61.2	62.1	
ycle Q Clear(g_c), s	24.8	0.0	20.0	24.8	0.0	15.8	6.1	0.0	0.0	1.4	61.2	62.1	
rop In Lane	1.00		0.85	1.00		0.71	1.00		0.08	1.00		0.04	
ane Grp Cap(c), veh/h		0	285	90	0	292	171	2329	1261	435	2206	1204	
/C Ratio(X)	0.73	0.00	0.83	1.64	0.00	0.68	0.88	0.29	0.29	0.12	0.85	0.86	
vail Cap(c_a), veh/h	127	0	285	90	0	292	196	2329	1261	524	2206	1204	
ICM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	2.00	2.00	2.00	1.00	1.00	1.00	
pstream Filter(I)	1.00	0.00	1.00	1.00	0.00	1.00	0.94	0.94	0.94	0.65	0.65	0.65	
Iniform Delay (d), s/vel		0.0	55.6	69.0	0.0	53.9	40.1	0.0	0.0	7.7	19.0	19.2	
ncr Delay (d2), s/veh	16.9	0.0	17.9		0.0	5.2	27.2	0.3	0.6	0.0	3.0	5.5	
nitial Q Delay(d3),s/veh		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
lile BackOfQ(50%),vet		0.0	9.5	11.6	0.0	7.0	6.1	0.1	0.2	0.5	22.1	25.3	
Insig. Movement Delay													
nGrp Delay(d),s/veh	83.2	0.0		400.3	0.0	59.0	67.4	0.3	0.6	7.7	22.0	24.7	
nGrp LOS	F	A	E	F	A	E	E	A	A	A	С	С	
pproach Vol, veh/h		331			346			1206			2971		
pproach Delay, s/veh		76.2			205.0			8.7			22.7		
pproach LOS		Е			F			Α			С		
imer - Assigned Phs	1	2		4	5	6		8					
hs Duration (G+Y+Rc)	1 97 9	102.1		30.0	13.0	97.0		30.0					
hange Period (Y+Rc),		* 5.5		* 5.2	5.0	* 5.5		* 5.2					
lax Green Setting (Gm		* 90		* 25	10.0	* 90		* 25					
lax Q Clear Time (g_c		2.0		26.8	8.1	64.1		26.8					
Freen Ext Time (p_c), s		2.2		0.0	0.0	10.0		0.0					
tersection Summary													
CM 6th Ctrl Delay			35.9										
ICM 6th LOS			D										
lotes													
ser approved pedestri													
HCM 6th computation	al engir	ne requ	ires eq	ual dea	rance ti	mes for	the ph	ases cr	ossing t	the barr	ier.		

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Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	3	7		7	*	***	
Traffic Volume (veh/h)	213	109	867	202	110	2000	
Future Volume (veh/h)	213	109	867	202	110	2000	
Initial Q (Qb), veh	0	0	0	0	0	0	
Ped-Bike Adj(A_pbT)	1.00	1.00		1.00	1.00		
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	
Work Zone On Approac			No			No	
Adj Sat Flow, veh/h/ln	1856	1856	1856	1856	1856	1856	
Adj Flow Rate, veh/h	260	133	1059	247	134	2442	
Peak Hour Factor	0.86	0.86	0.86	0.86	0.86	0.86	
Percent Heavy Veh, %	3	3	3	3	3	3	
Cap, veh/h	283	252	3458	1074	367	3834	
Arrive On Green	0.16	0.16	0.68	0.68	0.07	1.00	
Sat Flow, veh/h	1767	1572	5233	1572	1767	5233	
Grp Volume(v), veh/h	260	133	1059	247	134	2442	
Grp Sat Flow(s),veh/h/li		1572	1689	1572	1767	1689	
Q Serve(g_s), s	20.3	10.9	11.7	8.3	3.2	0.0	
Cycle Q Clear(g_c), s	1.00	1.00	11.7	1.00	1.00	0.0	
Prop In Lane		252	3458	1074	367	3834	
Lane Grp Cap(c), veh/h V/C Ratio(X)	0.92	0.53	0.31	0.23	0.36	0.64	
Avail Cap(c_a), veh/h	367	327	3458	1074	451		
HCM Platoon Ratio	1.00	1.00	1.00	1.00	2.00	2.00	
Upstream Filter(I)	1.00	1.00	0.96	0.96	0.71		
Uniform Delay (d), s/vel		53.9	8.9	8.4	6.1	0.0	
Incr Delay (d2), s/veh		0.6	0.0	0.5	0.2	0.6	
Initial Q Delay(d3),s/veh		0.0	0.0	0.0	0.0	0.0	
%ile BackOfQ(50%),vel		4.4	4.0	2.7	1.0	0.2	
Unsig. Movement Delay		1					
LnGrp Delay(d),s/veh	78.9	54.6	9.1	8.8	6.3	0.6	
LnGrp LOS	Е	D	Α	Α	Α	Α	
Approach Vol, veh/h	393		1306			2576	
Approach Delay, s/veh	70.7		9.1			0.9	
Approach LOS	Е		Α			Α	
Timer - Assigned Phs	- 1	2				6	8
Phs Duration (G+Y+Rc)	£0.4					111.7	28.3
Change Period (Y+Rc),		* 5.7				5.7	5.9
Max Green Setting (Gm		* 83				99.3	29.1
Max Q Clear Time (g_c		13.7				2.0	22.3
Green Ext Time (p_c), s		2.7				9.9	0.1
Intersection Summary							
HCM 6th Ctrl Delay			9.8				
HCM 6th LOS			Α				
Notes							
* HCM 6th computation	al engi	ne requ	ires equ	ual clea	rance ti	mes for	he phases crossing the barrier.

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SE
Lane Configurations		4	7		4		36	^^			*	
Traffic Volume (vph)	66	7	102	2	3	19	56	1000	5	43	2336	
Future Volume (vph)	66	7	102	2	3	19	56	1000	5	43	2336	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	19
Total Lost time (s)		5.2	5.2		5.0		5.0	6.8		5.0	5.7	
Lane Util. Factor		1.00	1.00		1.00		1.00	0.91		1.00	0.91	
Frt		1.00	0.85		0.89		1.00	1.00		1.00	1.00	
Flt Protected		0.96	1.00		1.00		0.95	1.00		0.95	1.00	
Satd. Flow (prot)		1765	1568		1643		1752	5032		1752	5025	
Flt Permitted		0.73	1.00		0.98		0.04	1.00		0.22	1.00	
Satd. Flow (perm)		1339	1568		1617		69	5032		412	5025	
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.
Growth Factor (vph)	105%	105%	105%	105%	105%	105%	105%	105%	105%	105%	105%	105
Adj. Flow (vph)	77	8	119	2	4	22	65	1167	6	50	2725	
RTOR Reduction (vph)	0	0	99	0	20	0	0	0	0	0	1	
Lane Group Flow (vph)	0	85	20	0	8	0	65	1173	0	50	2765	
Turn Type	Perm	NA	Perm	Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4		4	8			2			6		
Actuated Green, G (s)		12.3	12.3		12.5		111.6	107.2		110.9	107.4	
Effective Green, g (s)		12.3	12.3		12.5		111.6	107.2		110.9	107.4	
Actuated g/C Ratio		0.09	0.09		0.09		0.80	0.77		0.79	0.77	
Clearance Time (s)		5.2	5.2		5.0		5.0	6.8		5.0	5.7	
Vehicle Extension (s)		1.0	1.0		1.0		1.0	1.0		1.0	1.0	
Lane Grp Cap (vph)		117	137		144		107	3853		359	3854	
v/s Ratio Prot							c0.02	0.23		0.00	c0.55	
v/s Ratio Perm		c0.06	0.01		0.00		0.46			0.11		
v/c Ratio		0.73	0.14		0.06		0.61	0.30		0.14	0.72	
Uniform Delay, d1		62.2	59.0		58.3		15.9	5.0		3.1	8.4	
Progression Factor		1.00	1.00		1.00		1.93	0.75		1.04	0.94	
Incremental Delay, d2		17.2	0.2		0.1		6.2	0.2		0.0	0.7	
Delay (s)		79.4	59.2		58.4		36.8	3.9		3.3	8.7	
Level of Service		Е	E		Е		D	Α		Α	Α	
Approach Delay (s)		67.6			58.4			5.7			8.6	
Approach LOS		Е			E			Α			Α	
Intersection Summary												
HCM 2000 Control Delay			10.9	Н	CM 2000	l evel of	Service		В			
HCM 2000 Volume to Capacit	tv ratio		0.72		2.7.2000		_ 3, 1,00					
Actuated Cycle Length (s)	,		140.0	Sı	um of lost	time (s)			17.0			
Intersection Capacity Utilization	on		73.1%		U Level c)		D			
Analysis Period (min)	J.1		15		0 2010.0	,, 00, 1,00						
c Critical Lane Group												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SE
Lane Configurations		4	7	ě	ĵ.		ň	^ ^		٦	^^	
Traffic Volume (vph)	111	20	230	19	32	27	130	915	12	6	2109	10
Future Volume (vph)	111	20	230	19	32	27	130	915	12	6	2109	1
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	19
Total Lost time (s)		5.9	5.9	5.9	5.9		5.0	5.1		5.0	5.5	
Lane Util. Factor		1.00	1.00	1.00	1.00		1.00	0.91		1.00	0.91	
Frt		1.00	0.85	1.00	0.93		1.00	1.00		1.00	0.99	
Flt Protected		0.96	1.00	0.95	1.00		0.95	1.00		0.95	1.00	
Satd. Flow (prot)		1769	1568	1752	1716		1752	5026		1752	4999	
Flt Permitted		0.71	1.00	0.95	1.00		0.05	1.00		0.25	1.00	
Satd. Flow (perm)		1310	1568	1752	1716		85	5026		464	4999	
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.
Growth Factor (vph)	105%	105%	105%	105%	105%	105%	105%	105%	105%	105%	105%	105
Adj. Flow (vph)	130	23	268	22	37	32	152	1068	14	7	2460	1
RTOR Reduction (vph)	0	0	197	0	24	0	0	1	0	0	3	
Lane Group Flow (vph)	0	153	71	22	45	0	152	1081	0	7	2584	
Turn Type	Perm	NA	Perm	Split	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		4		3	3		5	2		1	6	
Permitted Phases	4		4				2			6		
Actuated Green, G (s)		20.0	20.0	6.7	6.7		96.4	90.6		82.2	81.4	
Effective Green, g (s)		20.0	20.0	6.7	6.7		96.4	90.6		82.2	81.4	
Actuated g/C Ratio		0.14	0.14	0.05	0.05		0.69	0.65		0.59	0.58	
Clearance Time (s)		5.9	5.9	5.9	5.9		5.0	5.1		5.0	5.5	
Vehicle Extension (s)		1.0	1.0	1.0	1.0		1.0	1.0		1.0	1.0	
Lane Grp Cap (vph)		187	224	83	82		172	3252		279	2906	
v/s Ratio Prot				0.01	c0.03		c0.06	0.22		0.00	0.52	
v/s Ratio Perm		c0.12	0.05				c0.55			0.01		
v/c Ratio		0.82	0.32	0.27	0.55		0.88	0.33		0.03	0.89	
Uniform Delay, d1		58.2	53.9	64.3	65.2		44.0	11.1		12.0	25.4	
Progression Factor		1.00	1.00	1.00	1.00		1.41	0.56		0.87	0.77	
Incremental Delay, d2		22.4 80.6	0.3	0.6	4.5 69.7		35.9	0.3 6.5		0.0	3.5	
Delay (s)		80.6 F	54.2 D	64.9	69.7		98.0	0.5 A		10.4 B	23.0	
Level of Service Approach Delay (s)		63.8	U	Е	68.5		F	17.8		В	23.0	
		03.0 E			00.5 F			17.0 B			23.0 C	
Approach LOS		_			_			ь			C	
Intersection Summary												
HCM 2000 Control Delay			26.4	Н	CM 2000	Level of	Service		С			
HCM 2000 Volume to Capac	ity ratio		0.87									
Actuated Cycle Length (s)			140.0		um of los				22.3			
Intersection Capacity Utilizati	on		80.8%	IC	CU Level	of Service	•		D			
Analysis Period (min)			15									
c Critical Lane Group												

	•	-	*	1	—	•	1	†	1	-	↓	*
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SE
Lane Configurations		**		100	*			4		36	4	
Traffic Volume (vph)	106	1785	5	15	841	158	23	2	46	266	8	
Future Volume (vph)	106	1785	5	15	841	158	23	2	46	266	8	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	19
Total Lost time (s)	5.2	5.7		5.2	5.7			5.9		5.9	5.9	
Lane Util. Factor	1.00	0.91		1.00	0.91			1.00		0.95	0.95	
Frt	1.00	1.00		1.00	0.98			0.91		1.00	0.92	
Flt Protected	0.95	1.00		0.95	1.00			0.98		0.95	0.98	
Satd. Flow (prot)	1752	5034		1752	4917			1656		1665	1584	
Flt Permitted	0.95	1.00		0.95	1.00			0.98		0.95	0.98	
Satd. Flow (perm)	1752	5034		1752	4917			1656		1665	1584	
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.
Growth Factor (vph)	105%	105%	105%	105%	105%	105%	105%	105%	105%	105%	105%	105
Adj. Flow (vph)	120	2015	6	17	950	178	26	2	52	300	9	1
RTOR Reduction (vph)	0	0	0	0	18	0	0	49	0	0	26	
Lane Group Flow (vph)	120	2021	0	17	1110	0	0	31	0	210	175	
Turn Type	Prot	NA		Prot	NA		Split	NA		Split	NA	
Protected Phases	1	6		5	2		4	4		3	3	
Permitted Phases												
Actuated Green, G (s)	13.9	84.4		2.5	73.0			7.4		23.0	23.0	
Effective Green, g (s)	13.9	84.4		2.5	73.0			7.4		23.0	23.0	
Actuated g/C Ratio	0.10	0.60		0.02	0.52			0.05		0.16	0.16	
Clearance Time (s)	5.2	5.7		5.2	5.7			5.9		5.9	5.9	
Vehicle Extension (s)	1.0	1.0		1.0	1.0			1.0		1.0	1.0	
Lane Grp Cap (vph)	173	3034		31	2563			87		273	260	
v/s Ratio Prot	c0.07	c0.40		0.01	0.23			c0.02		c0.13	0.11	
v/s Ratio Perm											•	
v/c Ratio	0.69	0.67		0.55	0.43			0.35		0.77	0.67	
Uniform Delay, d1	61.0	18.4		68.2	20.7			64.0		56.0	55.0	
Progression Factor	1.24	0.52		1.20	0.45			1.00		1.00	1.00	
Incremental Delay, d2	0.9	0.1		9.9	0.5			0.9		11.1	5.3	
Delay (s)	76.5	9.7		91.5	9.9			64.9		67.1	60.3	
Level of Service	E	A		F	A			E		E	E	
Approach Delay (s)		13.5			11.1			64.9			63.8	
Approach LOS		В			В			E			E	
Intersection Summary												
HCM 2000 Control Delay			19.3		CM 2000	Laure La C	0		В			
HCM 2000 Control Delay HCM 2000 Volume to Capac	ity ratio		0.68	н	UNI ZUUU	FEARI OL:	SEL VICE		В			
Actuated Cycle Length (s)	ity fatto		140.0		um of lost	tions (a)			22.7			
Actuated Cycle Length (s) Intersection Capacity Utilizat	ion		71.2%		um of lost CU Level o				22.1			
Analysis Period (min)	1011		15	IC	O Level (i Service			U			
c Critical Lane Group			15									

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SB
Lane Configurations		^ ^;			*		ř	414	7	٦	ĵ»	
Traffic Volume (vph)	5	1300	354	450	425	30	297	7	514	37	83	1
Future Volume (vph)	5	1300	354	450	425	30	297	7	514	37	83	1
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	190
Total Lost time (s)	5.2	6.2		5.2	5.8		6.1	6.1	6.1	5.9	5.9	
Lane Util. Factor	1.00	0.91		1.00	0.91		0.91	0.91	1.00	1.00	1.00	
Frt	1.00	0.97		1.00	0.99		1.00	1.00	0.85	1.00	0.98	
Flt Protected	0.95	1.00		0.95	1.00		0.95	0.95	1.00	0.95	1.00	
Satd. Flow (prot)	1752	4874		1752	4987		1595	3204	1568	1752	1810	
Flt Permitted	0.44	1.00		0.07	1.00		0.95	0.95	1.00	0.95	1.00	
Satd. Flow (perm)	816	4874		133	4987		1595	3204	1568	1752	1810	
Peak-hour factor, PHF	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.8
Growth Factor (vph)	105%	105%	105%	105%	105%	105%	105%	105%	105%	105%	105%	105
Adj. Flow (vph)	6	1534	418	531	501	35	350	8	606	44	98	
RTOR Reduction (vph)	0	35	0	0	5	0	0	0	363	0	4	
Lane Group Flow (vph)	6	1917	0	531	531	0	175	183	243	44	108	
Turn Type	pm+pt	NA		pm+pt	NA		Split	NA	Perm	Split	NA	
Protected Phases	1	6		5	2		4	4		3	3	
Permitted Phases	6			2					4			
Actuated Green, G (s)	50.6	49.8		90.2	84.2		21.4	21.4	21.4	10.6	10.6	
Effective Green, g (s)	50.6	49.8		90.2	84.2		21.4	21.4	21.4	10.6	10.6	
Actuated g/C Ratio	0.36	0.36		0.64	0.60		0.15	0.15	0.15	0.08	0.08	
Clearance Time (s)	5.2	6.2		5.2	5.8		6.1	6.1	6.1	5.9	5.9	
Vehicle Extension (s)	1.0	1.0		1.0	1.0		1.0	1.0	1.0	1.0	1.0	
Lane Grp Cap (vph)	300	1733		488	2999		243	489	239	132	137	
v/s Ratio Prot v/s Ratio Perm	0.00	0.39		c0.27 c0.43	0.11		0.11	0.06	c0.15	0.03	c0.06	
v/c Ratio	0.01	1.11		1.09	0.18		0.72	0.37	1.01	0.33	0.79	
Uniform Delay, d1	28.6	45.1		45.1	12.4		56.4	53.3	59.3	61.3	63.6	
Progression Factor	1.62	1.21		1.04	1.57		1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	0.0	56.2		65.0	0.1		8.6	0.2	62.1	0.5	24.4	
Delay (s)	46.5	110.5		112.0	19.6		65.0	53.5	121.4	61.9	88.0	
Level of Service	-10.0 D	F		F	13.0 B		00.0 E	D	121.4 F	E	F	
Approach Delay (s)		110.3			65.6			98.3	-		80.7	
Approach LOS		F			E			F			F	
Intersection Summary												
HCM 2000 Control Delay			94.9	Н	CM 2000	Level of	Service		F			
HCM 2000 Volume to Capa	city ratio		1.08									
Actuated Cycle Length (s)	•		140.0	S	um of lost	time (s)			23.4			
Intersection Capacity Utiliza	ation		90.7%	IC	CU Level	of Service			Е			
Analysis Period (min)			15									
c Critical Lane Group												
Culebra Arterial Study 7:15	om 12/15/2	2020 2020	AM Evir	tina Cons	lition						nchro 10	Pone

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	٦	†		ሻ	ተተተ			4	7		4		
Traffic Volume (veh/h)	-11	2640	16	63	937	0	51	- 1	245	5	5	5	
uture Volume (veh/h)	11	2640	16	63	937	0	51	1	245	5	5	5	
nitial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0	
	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00	
	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Nork Zone On Approach		No	1050		No		1000	No	1050	1050	No	1050	
	13	1856 3046	1856 18	1856 73	1856 1081	0	1856 59	1856	1856 283	1856	1856	1856	
Adj Flow Rate, veh/h Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	
	3	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	
Percent Heavy Veh, % Cap, veh/h	415	2609	15	95	3760	0	236	4	217	80	79	62	
	0.01	0.73	0.73	0.02	0.74	0.00	0.14	0.14	0.14	0.14	0.14	0.14	
	1767	3593	21	1767	5233	0.00	1343	26	1572	334	571	452	
Grp Volume(v), veh/h	13	1493	1571	73	1081	0	60	0	283	18	0	0	
Grp Sat Flow(s), veh/h/ln1		1763	1852	1767	1689	0	1369	0	1572	1357	0	0	
Q Serve(q.s), s		101.7	101.7	1.6	9.8	0.0	0.0	0.0	19.3	0.0	0.0	0.0	
Cycle Q Clear(q_c), s	0.3	101.7	101.7	1.6	9.8	0.0	5.8	0.0	19.3	5.8	0.0	0.0	
	1.00	101.7	0.01	1.00	3.0	0.00	0.98	0.0	1.00	0.33	0.0	0.33	
	415	1280	1345	95	3760	0.00	240	0	217	221	0	0.00	
	0.03	1.17	1.17	0.77	0.29	0.00	0.25	0.00	1.31	0.08	0.00	0.00	
Avail Cap(c_a), veh/h	527	1280	1345	179	3760	0.00	240	0.00	217	221	0.00	0.00	
	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
	0.42	0.42	0.42	1.00	1.00	0.00	1.00	0.00	1.00	1.00	0.00	0.00	
Uniform Delay (d), s/veh		19.2	19.2	42.2	5.9	0.0	54.5	0.0	60.3	52.6	0.0	0.0	
Incr Delay (d2), s/veh	0.0	78.7	79.5	4.9	0.2	0.0	0.2	0.0	166.7	0.1	0.0	0.0	
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
%ile BackOfQ(50%),veh/	In0.1	61.2	64.6	1.9	3.0	0.0	1.9	0.0	17.7	0.6	0.0	0.0	
Unsig. Movement Delay,	s/veh												
LnGrp Delay(d),s/veh	5.2	97.8	98.7	47.1	6.1	0.0	54.7	0.0	227.1	52.6	0.0	0.0	
LnGrp LOS	Α	F	F	D	Α	Α	D	Α	F	D	Α	Α	
Approach Vol, veh/h		3077			1154			343			18		
Approach Delay, s/veh		97.9			8.7			196.9			52.6		
Approach LOS		F			Α			F			D		
Timer - Assigned Phs	- 1	2		4	5	6		8					
Phs Duration (G+Y+Rc),	s6.1	108.9		25.0	8.3	106.7		25.0					
Change Period (Y+Rc), s		* 5		* 5.7	* 4.9	* 5		* 5.7					
Max Green Setting (Gma		* 95		* 19	* 10	* 95		* 19					
Max Q Clear Time (g_c+		11.8		21.3	3.6	103.7		7.8					
Green Ext Time (p_c), s	0.0	2.7		0.0	0.0	0.0		0.0					
ntersection Summary													
HCM 6th Ctrl Delay			82.7										
HCM 6th LOS			F										
Votes													
Jser approved pedestria													
HCM 6th computational	engi	ne requ	ires equ	al dea	rance t	imes for	the ph	ases cr	ossing t	he barr	ier.		
Culebra Arterial Study 7:							tet.						Synchro 10 Rep

- < < + > > + .	1	•	•	*	-	٠	
WBT WBR NBL NBT NBR SBL SBT S	WBR NBI	WBT	WBL	EBR	EBT	EBL	Movement
<u>የ</u> ቀሴ ካ ብተ ፖ	,	** 1,			414	*	Lane Configurations
550 67 236 694 238 0 0			0	0	1600	1850	Traffic Volume (vph)
550 67 236 694 238 0 0	67 236	550	0	0	1600	1850	Future Volume (vph)
1900 1900 1900 1900 1900 1900 1900 19	1900 1900	1900	1900	1900	1900	1900	Ideal Flow (vphpl)
4.9 5.7 5.7 5.7					4.5	4.5	Total Lost time (s)
0.91 0.91 1.00					0.91	0.91	Lane Util. Factor
0.98 1.00 1.00 0.85					1.00	1.00	Frt
1.00 0.95 1.00 1.00					0.98	0.95	Flt Protected
4954 1595 3352 1568					3306	1595	Satd. Flow (prot)
1.00 0.95 1.00 1.00					0.58	0.95	Flt Permitted
4954 1595 3352 1568					1939	1595	Satd. Flow (perm)
0.95 0.95 0.95 0.95 0.95 0.95 0.95 0			0.95	0.95	0.95	0.95	Peak-hour factor, PHF
		105%	105%	105%	105%	105%	Growth Factor (vph)
608 74 261 767 263 0 0			0	0	1768	2045	Adj. Flow (vph)
11 0 0 0 122 0 0			0	0	0	0	RTOR Reduction (vph)
671 0 235 793 141 0 0			0	0	2566	1247	Lane Group Flow (vph)
NA Split NA Perm					NA	Prot	Turn Type
2 4 14 4 14	4 14	2			1 2 11	1 11	Protected Phases
4 14							Permitted Phases
19.1 39.4 39.4 39.4					86.0	66.9	Actuated Green, G (s)
19.1 39.4 39.4 39.4					86.0	66.9	Effective Green, g (s)
0.14 0.28 0.28 0.28	0.28				0.61	0.48	Actuated g/C Ratio
4.9							Clearance Time (s)
1.0							Vehicle Extension (s)
675 448 943 441					1844	762	Lane Grp Cap (vph)
0.14 0.15 c0.24	0.1	0.14			0.66	c0.78	v/s Ratio Prot
0.09					c0.19		v/s Ratio Perm
0.99 0.52 0.84 0.32					1.39	1.64	v/c Ratio
60.4 42.4 47.4 39.7					27.0	36.5	Uniform Delay, d1
1.28 1.00 1.00 1.00					0.39	0.40	Progression Factor
32.5 0.5 6.6 0.2					176.5	287.0	Incremental Delay, d2
		109.9			186.9	301.8	Delay (s)
F D D D					F	F	Level of Service
		109.9			224.5		Approach Delay (s)
F D A		F			F		Approach LOS
							Intersection Summary
M 2000 Level of Service F	evel of Service	CM 2000	Н	171.8			HCM 2000 Control Delay
				1.57		acity ratio	HCM 2000 Volume to Capa
n of lost time (s) 32.5	time (s)	um of lost	S	140.0			Actuated Cycle Length (s)
Level of Service H	f Service	CU Level	IC	124.9%		ation	Intersection Capacity Utiliza
				15			Analysis Period (min)
							c Critical Lane Group
	f Service			15			Intersection Capacity Utiliza Analysis Period (min) c Critical Lane Group

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SE
Lane Configurations		**		36	**		36	11	7	36	414	
Traffic Volume (vph)	13	2600	79	200	650	35	191	369	880	141	92	
Future Volume (vph)	13	2600	79	200	650	35	191	369	880	141	92	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	19
Total Lost time (s)	5.0	5.7		5.0	5.4		6.1	6.1	6.1	6.6	6.6	
Lane Util. Factor	1.00	0.91		1.00	0.91		1.00	0.95	1.00	0.91	0.91	
Frt	1.00	1.00		1.00	0.99		1.00	1.00	0.85	1.00	0.99	
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00	1.00	0.95	0.98	
Satd. Flow (prot)	1752	5014		1752	4998		1752	3505	1568	1595	3274	
Flt Permitted	0.36	1.00		0.06	1.00		0.95	1.00	1.00	0.95	0.98	
Satd. Flow (perm)	656	5014		104	4998		1752	3505	1568	1595	3274	
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.
Growth Factor (vph)	105%	105%	105%	105%	105%	105%	105%	105%	105%	105%	105%	105
Adj. Flow (vph)	14	2844	86	219	711	38	209	404	962	154	101	
RTOR Reduction (vph)	0	2	0	0	3	0	0	0	163	0	2	
Lane Group Flow (vph)	14	2928	0	219	746	0	209	404	800	86	174	
Turn Type	pm+pt	NA		pm+pt	NA		Split	NA	Perm	Split	NA	
Protected Phases	1	6		5	2		4	4		3	3	
Permitted Phases	6			2					4			
Actuated Green, G (s)	67.5	65.7		86.6	79.8		24.8	24.8	24.8	10.5	10.5	
Effective Green, g (s)	67.5	65.7		86.6	79.8		24.8	24.8	24.8	10.5	10.5	
Actuated g/C Ratio	0.48	0.47		0.62	0.57		0.18	0.18	0.18	0.08	0.08	
Clearance Time (s)	5.0	5.7		5.0	5.4		6.1	6.1	6.1	6.6	6.6	
Vehicle Extension (s)	1.0	1.0		1.0	1.0		1.0	1.0	1.0	2.0	2.0	
Lane Grp Cap (vph)	330	2352		247	2848		310	620	277	119	245	
v/s Ratio Prot	0.00	c0.58		c0.10	0.15		0.12	0.12		c0.05	0.05	
v/s Ratio Perm	0.02			0.45					c0.51			
v/c Ratio	0.04	1.24		0.89	0.26		0.67	0.65	2.89	0.72	0.71	
Uniform Delay, d1	18.9	37.1		46.1	15.2		53.8	53.6	57.6	63.3	63.3	
Progression Factor	1.00	1.00		1.08	1.43		1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	0.0	113.9		25.2	0.2		4.5	1.9	859.6	16.7	7.8	
Delay (s)	18.9	151.1		74.9	22.0		58.3	55.5	917.2	80.0	71.1	
Level of Service	В	F		E	С		E	Е	F	F	Е	
Approach Delay (s)		150.5			34.0			582.4			74.0	
Approach LOS		F			С			F			E	
Intersection Summary												
HCM 2000 Control Delay			245.8	Н	CM 2000	Level of	Service		F			
HCM 2000 Volume to Capac	city ratio		1.50									
Actuated Cycle Length (s)			140.0	S	um of lost	time (s)			23.4			
Intersection Capacity Utilizal	tion		132.1%		U Level				Н			
Analysis Period (min)			15									
c Critical Lane Group												
Culebra Arterial Study 7:15											rnchro 10	

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			*	₩.	MDT	WDE			/ LIDE	001	•	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SE
Lane Configurations Traffic Volume (vph)	0	*** 2900	700	250	4 ↑↑ 500	0	0	0	0	551	41 ↑ 74	3
Future Volume (vph)	0	2900	700	250	500	0	0	0	0	551	74	3
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	19
Total Lost time (s)	1300	4.9	4.9	4.8	4.8	1300	1900	1900	1900	5.9	5.9	13
Lane Util. Factor		0.91	1.00	0.91	0.91					0.91	0.91	1.
Ert		1.00	0.85	1.00	1.00					1.00	1.00	0.
Fit Protected		1.00	1.00	0.95	1.00					0.95	0.96	1.
Satd, Flow (prot)		5036	1568	1595	3349					1595	3230	15
Flt Permitted		1.00	1.00	0.95	0.74					0.95	0.96	1.
Satd. Flow (perm)		5036	1568	1595	2487					1595	3230	15
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.
Growth Factor (vph)	105%	105%	105%	105%	105%	105%	105%	105%	105%	105%	105%	105
Adj. Flow (vph)	0	3205	774	276	553	0	0	0	0	609	82	4
RTOR Reduction (vph)	0	0	187	0	0	0	0	0	0	0	0	3
Lane Group Flow (vph)	0	3205	587	248	581	0	0	0	0	304	387	
Turn Type		NA	Perm	Prot	NA					Split	NA	Pe
Protected Phases		6		5 15	5 6 15					8 18	8 18	
Permitted Phases		01.1	6	10.0						00.0	00.0	8
Actuated Green, G (s)		61.1	61.1	40.3	101.4					23.0	23.0	23
Effective Green, g (s)		61.1 0.44	61.1 0.44	40.3 0.29	101.4					23.0	23.0	23
Actuated g/C Ratio Clearance Time (s)		4.9	4.9	0.29	0.72					0.16	0.16	0.
Vehicle Extension (s)		1.0	1.0									
Lane Grp Cap (vph)		2197	684	459	2049					262	530	2
v/s Ratio Prot		c0.64	004	c0.16	0.08					c0.19	0.12	
v/s Ratio Perm		00.07	0.37	00.10	0.12					00.10	0.12	0
v/c Ratio		1.46	0.86	0.54	0.28					1.16	1.12dl	0.
Uniform Delay, d1		39.5	35.6	42.0	6.7					58.5	55.6	51
Progression Factor		1.13	1.48	0.23	0.47					1.00	1.00	1.
Incremental Delay, d2		206.7	1.4	0.3	0.0					106.0	4.4	(
Delay (s)		251.1	54.1	10.0	3.2					164.5	60.0	51
Level of Service		F	D	Α	Α					F	Е	
Approach Delay (s)		212.8			5.2			0.0			85.6	
Approach LOS		F			Α			Α			F	
Intersection Summary												
HCM 2000 Control Delay			160.0	Н	CM 2000	Level of S	Service		F			
HCM 2000 Volume to Capac	city ratio		1.28									
Actuated Cycle Length (s)			140.0		um of lost				32.5			
Intersection Capacity Utilizat	tion		124.9%	IC	CU Level o	of Service			Н			
Analysis Period (min)			15									
dl Defacto Left Lane. Rec	ode with 1	though la	ne as a le	ett lane.								
c Critical Lane Group												
												_

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EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SE
- 1	44	7	36	44		36	1	7	36	1	
120	1050	250	36	750	200	11	8	5	123	71	- 1
120	1050	250	36	750	200	11	8	5	123	71	1
											19
4.4	5.3	5.3	4.4	4.9		5.2	5.2	5.2	5.1	5.1	
1.00	0.95	1.00	1.00	0.95		1.00	1.00	1.00	1.00		
1.00	1.00	0.85	1.00	0.97		1.00	1.00	0.85	1.00	0.91	
0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	
1736	3471	1553	1736	3362		1736	1827	1553	1736	1661	
0.18	1.00	1.00	0.18	1.00		0.95	1.00	1.00	0.95	1.00	
330	3471	1553	329	3362		1736	1827	1553	1736	1661	
0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.8
											106
	1251										1
							0				- "
	1251	216	43	1118	0	13	10	0	146	173	
		4%	4%		4%	4%	4%	4%	4%		4
					170						
		I GIIII						I GIIII			
		6					-	1			
	92.2			87.1		6.0	6.0		17.0	17.0	
		1022						00			
	00.00	0.14		0.00		60.01	0.01	0.00	0.00	60.10	
	0.66			0.52		0.10	0.12		0.66	0.92	
		ь	ь								
	ь										
		20.9	H	CM 2000	Level of	Service		С			
ratio		0.57									
		140.0		um of lost				20.0			
		61.7%	IC	U Level	of Service			В			
	1.00 1.00 0.95 1736 0.18	44 5.3 1.00 0.95 1.00 1.00 0.95 1.00 1.00 0.95 1.00 1.00 0.95 1.00 1.00 0.95 1.00 1.00 1.00 0.95 1.00 0.89	4.4 4.5.3 5.3 5.3 1.100 0.95 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0	4.4 5.3 5.3 4.4 1.00 0.95 1.00 1.00 0.85 1.00 1.00 0.85 1.00 0.00 0.95 1.00 1.00 0.95 1.00 1.00 0.95 1.00 1.00 0.95 1.00 1.00 0.95 1.00 1.00 0.95 1.00 1.00 0.95 1.00 1.00 1.00 0.95 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0	4.4 5.3 5.3 4.4 4.9 1.00 0.95 1.00 1.00 0.55 1.00 1.00 0.85 1.00 0.97 1.00 1.00 0.85 1.00 0.97 1.00 1.00 0.85 1.00 0.97 1.00 1.00 0.95 1.00 0.97 1.00 1.00 0.95 1.00 0.97 1.00 1.00 0.95 1.00 0.97 1.00 1.00 0.95 1.00 0.97 1.00 1.00 0.95 1.00 0.97 1.00 0.0 0.97 1.00 0.99 0.99 0.89 0.89 1.00 0.0 0.0 0.90 0.89 0.89 1.00 0.0 0.0 0.90 0.89 0.89 1.00 0.0 0.0 0.0 0.90 0.89 1.00 0.0 0.0 0.0 0.30 1.00 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	4.4 5.3 5.3 4.4 4.9 1.00 0.95 1.00 1.00 0.95 1.00 1.00 0.85 1.00 0.97 0.95 1.00 1.00 0.95 1.00 1.00 0.85 1.00 0.97 1.03 3471 1553 1736 3362 0.18 1.00 1.00 0.18 1.00 1.736 330 4471 1553 3293 3862 0.89 0.89 0.89 0.89 0.89 0.89 0.89 1.43 1251 298 43 893 238 0 0 0 62 0 13 0 0 143 1251 216 43 1118 0 144 45 44 45 44 45 44 45 118 0 1 6 6 6 2 1 0.05 92 92 91.0 87.1 100.5 92.2 92.2 91.0 87.1 100.5 92.2 92.2 91.0 87.1 100.5 92.2 92.2 91.0 87.1 100.5 92.2 92.2 91.0 87.1 100.5 92.2 92.2 91.0 87.1 100.5 92.2 92.2 91.0 87.1 100.5 92.3 92.2 92.0 1.0 0.72 0.66 0.66 0.65 0.62 4.4 5.3 5.3 4.4 4.9 1.0 2.0 1.0 1.00 0.38 0.00 0.33 1.285 1022 253 2091 0.03 0.36 0.00 0.33 1.285 1022 253 2091 0.48 0.14 0.11 0.43 0.55 0.21 0.17 0.53 0.95 1.28 9.5 9.9 15.0 1.05 0.96 1.33 1.00 1.00 1.02 1.29 1.29 1.29 1.0 1.0 1.04 0.5 0.96 1.33 1.00 1.00 1.02 1.9 1.9 1.9 1.0 1.0 1.05 0.96 1.33 1.00 1.00 1.02 1.9 1.9 1.9 1.5 1.7 1.06 1.9 1.9 1.9 1.9 1.9 1.9 1.0 1.07 1.08 1.08 1.09 1.00 1.09 1.33 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00	4.4	4.4 5.3 5.3 4.4 4.9 5.2 5.2 5.2 1.00 1.00 0.95 1.00 1.00 0.95 1.00 1.00 0.95 1.00 1.00 0.95 1.00 1.00 0.95 1.00 1.00 0.95 1.00 1.00 0.95 1.00 0.95 1.00 0.95 1.00 0.95 1.00 1.00 0.95 1.20 0.95 1.00	4.4 5.3 5.3 4.4 4.9 5.2 5.2 5.2 5.0 1.00 0.00 0.95 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0	4.4	4.4 5.3 5.3 4.4 4.9 5.2 5.2 5.2 5.1 5.1 5.1 100 0.95 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0

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Movement	EBL	EBT	WBT	WBR	SBL	SBR		
Lane Configurations		414	↑ ₽		*	7		
Traffic Volume (vph)	54	1450	620	150	67	86		
Future Volume (vph)	54	1450	620	150	67	86		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Total Lost time (s)		5.3	5.3		5.0	5.0		
Lane Util. Factor		0.95	0.95		1.00	1.00		
Frt		1.00	0.97		1.00	0.85		
Flt Protected		1.00	1.00		0.95	1.00		
Satd. Flow (prot)		3465	3370		1736	1553		
Flt Permitted		0.85	1.00		0.95	1.00		
Satd. Flow (perm)		2962	3370		1736	1553		
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91		
Growth Factor (vph)	106%	106%	106%	106%	106%	106%		
Adj. Flow (vph)	63	1689	722	175	78	100		
RTOR Reduction (vph)	0	0	9	0	0	90		
Lane Group Flow (vph)	0	1752	888	0	78	10		
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%		
Turn Type	Perm	NA	NA		Prot	Perm		
Protected Phases		6 26	2 22		8 28			
Permitted Phases	6 26					8 28		
Actuated Green, G (s)		105.6	105.6		13.8	13.8		
Effective Green, g (s)		105.6	105.6		13.8	13.8		
Actuated g/C Ratio		0.75	0.75		0.10	0.10		
Clearance Time (s)								
Vehicle Extension (s)		2234	2541		171	153		
Lane Grp Cap (vph) v/s Ratio Prot		2234	0.26		c0.04	153		
v/s Ratio Prot v/s Ratio Perm		c0.59	0.26		CU.U4	0.01		
v/s Ratio Perm v/c Ratio		0.78	0.35		0.46	0.01		
Uniform Delay, d1		10.3	5.7		59.6	57.2		
Progression Factor		1.53	1.23		1.00	1.00		
Incremental Delay, d2		1.00	0.0		0.7	0.1		
Delay (s)		17.1	7.1		60.3	57.3		
Level of Service		В	A		E	E E		
Approach Delay (s)		17.1	7.1		58.6	_		
Approach LOS		В	A		E			
Intersection Summary								
HCM 2000 Control Delay			16.5	H	CM 2000	Level of Service	В	
HCM 2000 Volume to Capaci	ty ratio		0.75	_		Para (a)	00.0	
Actuated Cycle Length (s)	_		140.0		um of lost		20.6	
Intersection Capacity Utilization	on		85.4% 15	IC	U Level C	of Service	E	
Analysis Period (min) c Critical Lane Group			13					
C Chical Lane Gloup								

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	7	†		7	*			4			4		
Traffic Volume (veh/h)	100	720	8	7	550	75	21	30	32	126	18	66	
Future Volume (veh/h)	100	720	8	7	550	75	21	30	32	126	18	66	
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0	
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Work Zone On Approac		No	1070		No	4000		No			No	4000	
Adj Sat Flow, veh/h/ln	1870	1870 840	1870	1870	1870 642	1870 88	1870 24	1870	1870	1870 147	1870	1870 77	
Adj Flow Rate, veh/h Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	
Percent Heavy Veh, %	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	
Cap, veh/h	304	986	11	812	1960	268	93	133	118	208	24	85	
Arrive On Green	0.02	0.09	0.09	0.82	1.00	1.00	0.19	0.19	0.19	0.19	0.19	0.19	
Sat Flow, veh/h	1781	3602	39	1781	3140	430	299	714	636	864	132	456	
Grp Volume(v), veh/h	117	414	435	8	363	367	96	0	030	245	0	0	
Grp Sat Flow(s), veh/h/lr		1777	1863	1781	1777	1793	1649	0	0	1452	0	0	
Q Serve(q.s), s	6.3	27.6	27.6	0.0	0.0	0.0	0.0	0.0	0.0	14.1	0.0	0.0	
Cycle Q Clear(q_c), s	6.3	27.6	27.6	0.0	0.0	0.0	5.7	0.0	0.0	19.8	0.0	0.0	
Prop In Lane	1.00	21.0	0.02	1.00	0.0	0.24	0.25	0.0	0.39	0.60	0.0	0.0	
Lane Grp Cap(c), veh/h		486	510	812	1109	1119	343	0	0.00	317	0	0.01	
V/C Ratio(X)	0.39	0.85	0.85	0.01	0.33	0.33	0.28	0.00	0.00	0.77	0.00	0.00	
Avail Cap(c_a), veh/h	340	887	930	812	1109	1119	509	0.00	0.00	468	0.00	0.00	
HCM Platoon Ratio	0.33	0.33	0.33	2.00	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00	
Upstream Filter(I)	0.93	0.93	0.93	1.00	1.00	1.00	1.00	0.00	0.00	1.00	0.00	0.00	
Uniform Delay (d), s/veh	137.5	52.2	52.2	5.6	0.0	0.0	42.1	0.0	0.0	47.8	0.0	0.0	
Incr Delay (d2), s/veh	0.3	16.0	15.4	0.0	0.8	0.8	0.2	0.0	0.0	2.3	0.0	0.0	
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
%ile BackOfQ(50%),veh	/ln2.9	15.3	15.9	0.0	0.2	0.2	2.5	0.0	0.0	7.4	0.0	0.0	
Unsig. Movement Delay	, s/veh												
LnGrp Delay(d),s/veh	37.8	68.2	67.6	5.6	0.8	0.8	42.3	0.0	0.0	50.1	0.0	0.0	
LnGrp LOS	D	Е	Е	Α	Α	Α	D	Α	Α	D	Α	Α	
Approach Vol, veh/h		966			738			96			245		
Approach Delay, s/veh		64.2			0.8			42.3			50.1		
Approach LOS		Е			Α			D			D		
Timer - Assigned Phs	- 1	2		4	5	6		8					
Phs Duration (G+Y+Rc)		80.0		27.5	54.6	37.9		27.5					
Change Period (Y+Rc),		* 5.1		* 5.2	* 5.1	* 5.1		* 5.2					
Max Green Setting (Gm		* 60		* 35	* 11	* 60		* 35					
Max Q Clear Time (g. c-		2.0		7.7	2.0	29.6		21.8					
Green Ext Time (p_c), s		2.8		0.2	0.0	3.3		0.5					
Intersection Summary													
HCM 6th Ctrl Delay			38.6										
HCM 6th LOS			D										
Notes													
* HCM 6th computations	al engir	ne requ	ires equ	ıal clea	rance ti	mes for	the ph	ases cr	ossing t	he barr	ier.		
Culebra Arterial Study 7	:15 am	12/15/	2020 2	120 AM	Fxistin	a Cond	ition						Synchro 10 Rep

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Movement	EBL	EBT	WBT	WBR	SBL	SBR		
Lane Configurations		44	† 1>		14			
Traffic Volume (vph)	30	800	700	15	25	104		
Future Volume (vph)	30	800	700	15	25	104		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Total Lost time (s)	4.4	5.1	5.1		5.2			
Lane Util. Factor	1.00	0.95	0.95		1.00			
Frt	1.00	1.00	1.00		0.89			
Flt Protected	0.95	1.00	1.00		0.99			
Satd, Flow (prot)	1770	3539	3528		1644			
Flt Permitted	0.95	1.00	1.00		0.99			
Satd. Flow (perm)	1770	3539	3528		1644			
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91		
Growth Factor (vph)	105%	105%	105%	105%	105%	105%		
Adi, Flow (vph)	35	923	808	17	29	120		
RTOR Reduction (vph)	0	0	1	0	102	0		
Lane Group Flow (vph)	35	923	824	0	47	0		
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%		
Turn Type	Prot	NA.	NA.	2,0	Prot	270		
Protected Phases	1	6 26	2 22		8 28			
Permitted Phases		0 20	2 22		0 20			
Actuated Green, G (s)	4.3	81.3	72.6		18.1			
Effective Green, g (s)	4.3	81.3	72.6		18.1			
Actuated g/C Ratio	0.04	0.68	0.60		0.15			
Clearance Time (s)	4.4	0.00	0.00		0.15			
Vehicle Extension (s)	1.0							
	63	2397	2134		247			
Lane Grp Cap (vph) v/s Ratio Prot	c0.02	c0.26	c0.23		c0.03			
	CU.U2	CU.26	CU.23		CU.U3			
v/s Ratio Perm v/c Ratio	0.56	0.39	0.39		0.19			
Vic Ratio Uniform Delay, d1	56.9	8.4	12.2		44.5			
		1.00	1.06		1.00			
Progression Factor	1.00							
Incremental Delay, d2	5.9	0.0	0.0		0.1			
Delay (s) Level of Service	62.8 F	8.5	13.0 B		44.7 D			
	E	A						
Approach Delay (s)		10.5	13.0 B		44.7			
Approach LOS		В	В		D			
Intersection Summary								
HCM 2000 Control Delay			14.2	Н	CM 2000	Level of Service	В	
HCM 2000 Volume to Capa	city ratio		0.37					
Actuated Cycle Length (s)			120.0		um of lost		25.0	
Intersection Capacity Utiliza	tion		44.8%	IC	U Level o	of Service	A	
			15					
Analysis Period (min) c Critical Lane Group			15			N CONTIGO		
Culebra Arterial Study 7:15	am 12/15/2	020 2020	AM Exis	tina Cond	lition			Synchro 10 Re

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Movement	EBT	EBR	WBL	WBT	NBL	NBR		
Lane Configurations	† }		36	† †	N/A			
Traffic Volume (vph)	960	12	17	680	65	128		
Future Volume (vph)	960	12	17	680	65	128		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Total Lost time (s)	5.4		5.4	5.4	5.2			
Lane Util. Factor	0.95		1.00	0.95	1.00			
Frt	1.00		1.00	1.00	0.91			
Flt Protected	1.00		0.95	1.00	0.98			
Satd. Flow (prot)	3533		1770	3539	1668			
FIt Permitted	1.00		0.23	1.00	0.98			
Satd. Flow (perm)	3533		427	3539	1668			
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96		
Growth Factor (vph)	105%	105%	105%	105%	105%	105%		
Adj. Flow (vph)	1050	13	19	744	71	140		
RTOR Reduction (vph)	1	0	0	0	68	0		
Lane Group Flow (vph)	1062	0	19	744	143	0		
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%		
Turn Type	NA		Perm	NA	Prot			
Protected Phases	6 26			2 22	4 24			
Permitted Phases			2 22					
Actuated Green, G (s)	81.4		81.4	81.4	17.4			
Effective Green, g (s)	81.4		81.4	81.4	17.4			
Actuated g/C Ratio	0.68		0.68	0.68	0.14			
Clearance Time (s)								
Vehicle Extension (s)								
Lane Grp Cap (vph)	2396		289	2400	241			
v/s Ratio Prot	c0.30			0.21	c0.09			
v/s Ratio Perm			0.04					
v/c Ratio	0.44		0.07	0.31	0.60			
Uniform Delay, d1	8.9		6.5	7.9	48.0			
Progression Factor	1.13		0.44	0.41	1.00			
Incremental Delay, d2	0.0		0.0	0.0	2.6			
Delay (s)	10.0		2.9	3.2	50.6			
Level of Service	В		Α	Α	D			
Approach Delay (s)	10.0			3.2	50.6			
Approach LOS	В			Α	D			
Intersection Summary								
HCM 2000 Control Delay			11.7	H	CM 2000	Level of Service	В	
HCM 2000 Volume to Capa	acity ratio		0.47					
Actuated Cycle Length (s)			120.0		um of lost		21.2	
Intersection Capacity Utiliza	ation		49.1%	IC	U Level	of Service	A	
Analysis Period (min)			15					
c Critical Lane Group								
Culebra Arterial Study 7:15								

Movement	•	\rightarrow	•	•	—	•	1	†	1	-	¥	4
	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SB
Lane Configurations	Ä	** 1		ě	iiii;		Ť	†	7	Ĭ	† 1>	
Traffic Volume (vph)	24	1350	136	146	736	42	145	145	201	56	127	4
Future Volume (vph)	24	1350	136	146	736	42	145	145	201	56	127	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	190
Total Lost time (s)	5.0	4.4		5.0	13.4		4.4	6.0	6.0	4.4	6.4	
Lane Util. Factor	1.00	0.91		1.00	0.86		1.00	1.00	1.00	1.00	0.95	
Frt	1.00	0.99		1.00	0.99		1.00	1.00	0.85	1.00	0.96	
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1770	5016		1770	6356		1770	1863	1583	1770	3401	
Flt Permitted	0.95	1.00		0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (perm)	1770	5016		1770	6356		1770	1863	1583	1770	3401	-
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.9
Growth Factor (vph)	105%	105%	105%	105%	105%	105%	105%	105%	105%	105%	105%	105
Adj. Flow (vph)	27	1541	155	167	840	48	165	165	229	64	145	
RTOR Reduction (vph)	0	9	0	0	6	0	0	0	188	0	32	
Lane Group Flow (vph)	27	1687	0	167	882	0	165	165	41	64	164	
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2
Turn Type	Prot	NA		Prot	NA		Prot	NA	Perm	Prot	NA	
Protected Phases	1	6		5	2		7	4		3	8	
Permitted Phases							40.0		4			
Actuated Green, G (s)	3.6	60.4		12.0	42.8		18.2	21.3	21.3	6.5	9.2	
Effective Green, g (s)	3.6	60.4		12.0	42.8		18.2	21.3	21.3	6.5	9.2	
Actuated g/C Ratio	0.03 5.0	0.50		0.10 5.0	0.36		0.15	0.18	0.18	0.05	0.08	
Clearance Time (s) Vehicle Extension (s)	0.5	0.5		0.5	13.4		0.5	6.0 1.0	6.0 1.0	0.5	1.0	
	53	2524		177	2266		268	330		95	260	
Lane Grp Cap (vph) v/s Ratio Prot	0.02	c0.34		c0.09	0.14		c0.09	c0.09	280	0.04	c0.05	
v/s Ratio Prot v/s Ratio Perm	0.02	CU.34		CU.U9	0.14		CU.U9	CU.U9	0.03	0.04	CU.U5	
v/s Ratio Perm v/c Ratio	0.51	0.67		0.94	0.39		0.62	0.50	0.03	0.67	0.63	
Uniform Delay, d1	57.3	22.3		53.7	28.8		47.6	44.5	41.7	55.7	53.7	
Progression Factor	1.52	0.19		0.89	1.56		1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	0.3	0.19		50.1	0.5		2.9	0.4	0.1	13.8	3.4	
Delay (s)	87.4	4.3		97.7	45.4		50.6	45.0	41.8	69.5	57.2	
Level of Service	67.4	4.3 A		51.1	43.4 D		JU.0	45.0 D	41.0 D	09.5	57.2 E	
Approach Delay (s)	- 1	5.6			53.7		D	45.3	D		60.2	
Approach LOS		J.0			D.7			TO.0			E	
		- "										
Intersection Summary												
HCM 2000 Control Delay			29.8	Н	CM 2000	Level of	Service		С			
HCM 2000 Volume to Capaci	ty ratio		0.88									
Actuated Cycle Length (s)			120.0		um of lost				41.6			
Intersection Capacity Utilization	on		69.5%	IC	U Level o	of Service			С			
Analysis Period (min)			15									

و		→	*	1	•	•	4	†	-	/	Ţ	4	
Movement EE	3L	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
ane Configurations	7	^^^		ň	<u>ተ</u> ቀሴ			4			4		
raffic Volume (veh/h) 11	10	1500	115	34	980	42	40	42	18	40	42	18	
uture Volume (veh/h) 11	0	1500	115	34	980	42	40	42	18	40	42	18	
nitial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0	
Ped-Bike Adj(A_pbT) 1.0	00		1.00	1.00		1.00	1.00		1.00	1.00		1.00	
Parking Bus, Adj 1.0	00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Vork Zone On Approach		No			No			No			No		
dj Sat Flow, veh/h/ln 185	6	1856	1856	1856	1856	1856	1856	1856	1856	1856	1856	1856	
dj Flow Rate, veh/h 12	24	1694	130	38	1106	47	45	47	20	45	47	20	
Peak Hour Factor 0.9	93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	
Percent Heavy Veh, %	3	3	3	3	3	3	3	3	3	3	3	3	
Cap, veh/h 48	31	3675	282	254	3743	159	89	67	25	89	67	25	
Arrive On Green 0.0		0.77	0.77	0.04	1.00	1.00	0.09	0.09	0.09	0.09	0.09	0.09	
Sat Flow, veh/h 176	67	4799	368	1767	4983	212	525	758	279	525	758	279	
Grp Volume(v), veh/h 12	24	1191	633	38	749	404	112	0	0	112	0	0	
Grp Sat Flow(s),veh/h/ln176	67	1689	1789	1767	1689	1817	1562	0	0	1562	0	0	
Serve(a s), s 2	.0	15.3	15.4	0.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Cycle Q Clear(g_c), s 2	.0	15.3	15.4	0.6	0.0	0.0	8.4	0.0	0.0	8.4	0.0	0.0	
Prop In Lane 1.0	00		0.21	1.00		0.12	0.40		0.18	0.40		0.18	
ane Grp Cap(c), veh/h 48	31	2587	1370	254	2537	1365	181	0	0	181	0	0	
//C Ratio(X) 0.2	26	0.46	0.46	0.15	0.30	0.30	0.62	0.00	0.00	0.62	0.00	0.00	
Avail Cap(c_a), veh/h 60)3	2587	1370	401	2537	1365	488	0	0	488	0	0	
CM Platoon Ratio 1.0	00	1.00	1.00	2.00	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00	
Jpstream Filter(I) 1.0	00	1.00	1.00	0.86	0.86	0.86	1.00	0.00	0.00	1.00	0.00	0.00	
Jniform Delay (d), s/veh 3	.0	5.1	5.1	4.1	0.0	0.0	53.6	0.0	0.0	53.6	0.0	0.0	
ncr Delay (d2), s/veh 0	.1	0.6	1.1	0.1	0.3	0.5	1.3	0.0	0.0	1.3	0.0	0.0	
nitial Q Delay(d3),s/veh 0	.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
6ile BackOfQ(50%),veh/ln0	.5	4.3	4.8	0.2	0.1	0.2	3.4	0.0	0.0	3.4	0.0	0.0	
Jnsig. Movement Delay, s/v	/eh												
.nGrp Delay(d),s/veh 3	.1	5.7	6.2	4.2	0.3	0.5	54.9	0.0	0.0	54.9	0.0	0.0	
nGrp LOS	Α	Α	Α	Α	Α	Α	D	Α	Α	D	Α	Α	
Approach Vol, veh/h		1948			1191			112			112		
Approach Delay, s/veh		5.7			0.5			54.9			54.9		
Approach LOS		Α			Α			D			D		
imer - Assigned Phs	1	2		4	5	6		8					
hs Duration (G+Y+Rc), s8	7	95.0		16.2	7.0	96.8		16.2					
Change Period (Y+Rc), so		4.9		* 5.6	* 4.8	4.9		* 5.6					
Jnange Period (Y+RC), S 4 Max Green Setting (Gmax):		58.1		* 34	* 12	58.1		* 34					
Max Q Clear Time (g_c+l14)		2.0		10.4	2.6	17.4		10.4					
Green Ext Time (p_c), s 0		2.7		0.2	0.0	5.1		0.2					
ntersection Summary													
ICM 6th Ctrl Delay			7.1										
ICM 6th LOS			A										
lotes													
Iser approved pedestrian in HCM 6th computational er								ases cr	ossing t	he barr	ier.		
Culebra Arterial Study 7:15	_												Synchro 10 Re

· 🕨 📗 🗸	r '	†	1	•	—	1	•	-	٠	
R SBL SBT SE	NBR S	NBT	NBL	WBR	WBT	WBL	EBR	EBT	EBL	Movement
4		4			**†	7		* **	*	Lane Configurations
	53	1	39	1	930	41	28	1590	7	Traffic Volume (vph)
7 5	53	1	39	1	930	41	28	1590	7	Future Volume (vph)
1900 1900 19	1900 19	1900	1900	1900	1900	1900	1900	1900	1900	Ideal Flow (vphpl)
5.0		5.0			5.6	4.8		5.6	4.8	Total Lost time (s)
1.00		1.00			0.91	1.00		0.91	1.00	Lane Util. Factor
0.99		0.92			1.00	1.00		1.00	1.00	Frt
0.97		0.98			1.00	0.95		1.00	0.95	Flt Protected
1781		1667			5035	1752		5023	1752	Satd. Flow (prot)
1.00		0.86			1.00	0.09		1.00	0.26	Flt Permitted
1828		1460			5035	172		5023	486	Satd. Flow (perm)
		0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	Peak-hour factor, PHF
		105%	105%	105%	105%	105%	105%	105%	105%	Growth Factor (vph)
	59	1	43	1	1028	45	31	1757	8	Adj. Flow (vph)
	0	44	0	0	0	0	0	1	0	RTOR Reduction (vph)
	0	59	0	0	1029	45	0	1787	8	Lane Group Flow (vph)
Perm NA	Pe	NA	Perm		NA	pm+pt		NA	pm+pt	Turn Type
3		4			2	5		6	1	Protected Phases
3			4			2			6	Permitted Phases
2.6		10.9			85.4	88.9		82.6	83.3	Actuated Green, G (s)
2.6		10.9			85.4	88.9		82.6	83.3	Effective Green, g (s)
0.02		0.09			0.71	0.74		0.69	0.69	Actuated g/C Ratio
5.0 1.0		5.0			5.6	4.8		5.6	4.8	Clearance Time (s)
		1.0			1.0	1.0		1.0	0.5	Vehicle Extension (s)
39		132			3583	173		3457	344	Lane Grp Cap (vph)
c0.01		c0.04			0.20	c0.01		c0.36	0.00	v/s Ratio Prot v/s Ratio Perm
0.36		0.45			0.29	0.18		0.52	0.02	v/s Ratio Perm v/c Ratio
0.36 57.9		51.7			6.3	5.9		9.0		V/C Ratio Uniform Delay, d1
1.00		1.00			0.31	1.20		0.39	5.6 0.34	Progression Factor
2.1		0.9			0.31	0.3		0.59	0.04	
59.9		52.6			2.1	7.4		4.0	1.9	Incremental Delay, d2 Delay (s)
59.9 F		52.0 D			2.1 A	7.4 A		4.0 A	1.9 A	Level of Service
59.9		52.6			24	А		4.0	А	Approach Delay (s)
55.5 F		D D			2.4 A			4.0 A		Approach LOS
-		U			^			^		**
										Intersection Summary
i	Α		ervice	Level of S	CM 2000	Н	5.3			HCM 2000 Control Delay
									city ratio	
i e	A			f Service	U Level o	IC			tion	
							15			
	20.4 A		ervice	time (s)	CM 2000 um of lost CU Level o	S	5.3 0.50 120.0 52.9% 15		,	HCM 2000 Control Delay HCM 2000 Volume to Capar Actuated Cycle Length (s) Intersection Capacity Utiliza Analysis Period (min) c Critical Lane Group

. Mountain v	/iew 8	Cule	ebra l	≺d				
rsection								
Delay, s/veh	0.5							
vement	EBT	EBR	WBL	WBT	NBL	NBR		
e Configurations	† 1>	LUIT	1	44	1100	7		
fic Vol, veh/h	1350	5	36	867	10	10		
re Vol. veh/h	1350	5	36	867	10	10		
flicting Peds, #/hr	0	0	0	0	0	0		
Control	Free	Free	Free	Free	Stop	Stop		
Channelized	-	None		None	-	None		
age Length		-	160	-	-	0		
in Median Storag		-		0	0			
de, %	0	-	-	0	0	-		
k Hour Factor	88	88	88	88	88	88		
vy Vehicles, %	2	2	2	2	2	2		
nt Flow	1994	7	53	1281	15	15		
as/Mines	Majort		Maias		Minart			
or/Minor flicting Flow All	Major1 0	0	Major2 2001	0	Minor1 2745	1001		
Stage 1	-	-	2001	-	1998	1001		
Stage 1 Stage 2		- 1	- 1	- 1	747	- 1		
cal Hdwy	- 1	- 1	4.14	- :	6.84			
cal Hdwy Stg 1	- 1	- 1	4.14		5.84	0.54		
cal Hdwy Stg 2		- 1	- 1	- 1	5.84	- :		
ow-up Hdwy			2.22		3.52	3.32		
Cap-1 Maneuver			283		16	241		
Stage 1			-		91			
Stage 2					429			
oon blocked, %		-		-				
Cap-1 Maneuver		-	283		~ 13	241		
Cap-2 Maneuver		-	-	-	70			
Stage 1			-	-	91			
Stage 2		-	-	-	349	-		
roach	EB		WB		NB			
M Control Delay, s	0		0.8		20.9			
ILOS					С			
or Lano/Major Ma	mt	NBLn1	EBT	EBR	WBL	WBT		
or Lane/Major Mvi acity (veh/h)	iiit	241	EB1	EDIK	283	WBI		
M Lane V/C Ratio		0.061	- 1	- 1	0.188	- 1		
	:)	20.9	- :	- :	20.6	- :		
	7	20.5 C	- 1	- 1	20.0	- 1		
M Control Delay (s		0.2			0.7	- 1		
M Control Delay (s M Lane LOS	h)							
M Control Delay (s M Lane LOS M 95th %tile Q(vel	h)							
M Control Delay (s M Lane LOS M 95th %tile Q(vel es					20-	. 0	teller Net Defend . * Allere's colo	and the others
M Control Delay (s M Lane LOS M 95th %tile Q(vel			elay exc	eeds 3	00s	+: Com	outation Not Defined *: All major volume	me in platoo

itersection							
t Delay, s/veh	0.8						
lovement	SBL	SBR	SEL	SET	NWT	NWR	
ane Configurations	¥			414	† ‡		
raffic Vol, veh/h	6	63	33	1340	599	25	
uture Vol, veh/h	6	63	33	1340	599	25	
onflicting Peds, #/hr	0	0	0	0	0	0	
ign Control	Stop	Stop		Free	Free	Free	
T Channelized		None		None		None	
torage Length	0	-	-	-	-	-	
eh in Median Storage,			-	0	0		
irade, %	0	-	-	0	0	-	
eak Hour Factor	92	92	92	92	92	92	
eavy Vehicles, %	3	3	3	3	3	3	
lvmt Flow	8	89	47	1893	846	35	
to different	····		Andread .		1-1-0		
	Minor2		Aajor1		Major2		
onflicting Flow All	1905 864	441	881	0		0	
Stage 1			-			-	
Stage 2	1041	- 00	1.40		-		
itical Hdwy	6.86	6.96	4.16	-	-	-	
itical Hdwy Stg 1	5.86	- 1	-		- 1		
ritical Hdwy Stg 2	5.86 3.53	3.33	2.23	- :	- 1		
ollow-up Hdwy				- 1	- 1		
ot Cap-1 Maneuver	60 371	561	757	- 1	- 1	- 1	
Stage 1 Stage 2	299	- 1	- 1	- 1	- 1		
Stage 2 latoon blocked, %	299				- 1	- 1	
lov Cap-1 Maneuver	60	561	757	- :	- 1	- 1	
lov Cap-1 Maneuver lov Cap-2 Maneuver	60	561	/5/		- 1	- 1	
			-	- 1			
		-					
Stage 1	371						
	371 299		-				
Stage 1 Stage 2	299				NW		
Stage 1 Stage 2 pproach	299 SB		SE		NW		
Stage 1 Stage 2 pproach CM Control Delay, s	299 SB 20.8				NW 0		
Stage 1 Stage 2 oproach CM Control Delay, s	299 SB		SE	_			
Stage 1 Stage 2 pproach CM Control Delay, s CM LOS	299 SB 20.8 C		SE 0.2		0	SBLn1	
Stage 1 Stage 2 pproach CM Control Delay, s CM LOS linor Lane/Major Mvml	299 SB 20.8 C		SE	SEL	0	SBLn1 325	
Stage 1 Stage 2	299 SB 20.8 C	NWT	SE 0.2 NWR	SEL	0 SET:		
Stage 1 Stage 2 pproach CM Control Delay, s CM LOS linor Lane/Major Mvml apacity (veh/h) CM Lane V/C Ratio	299 SB 20.8 C	NWT -	SE 0.2 NWR	SEL 757 0.062	SET:	325 0.3	
Stage 1 Stage 2 opproach CM Control Delay, s CM LOS inor Lane/Major Myml apacity (veh/h)	299 SB 20.8 C	NWT -	SE 0.2 NWR	SEL 757 0.062 10.1	SET:	325 0.3	

	۶	→	*	1	—	•	1	Ť	1	/	ţ	4	
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
ane Configurations	1	♠ ₽		×	↑ ↑		7	^	7		ની	7	
raffic Volume (veh/h)	70	1300	3	5	588	39	7	10	14	124	10	95	
uture Volume (veh/h)	70	1300	3	5	588	39	7	10	14	124	10	95	
nitial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0	
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Vork Zone On Approac		No			No			No			No		
Adj Sat Flow, veh/h/ln	1969	1969	1969	1969	1969	1969	1969	1969	1969	1969	1969	1969	
Adj Flow Rate, veh/h	103	1920	4	7	869	58	10	15	21	183	15	140	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2	
Cap, veh/h	503	2776	6	179	2519	168	80	311	264	257	17	276	
Arrive On Green Sat Flow, veh/h	0.04 1875	0.72 3830	0.71	0.01	0.71 3559	0.69	0.16	0.16 1969	0.16	0.17	0.16	0.17	
,	10/3	937	987	7	457	470	10	15	21	198		140	
Grp Volume(v), veh/h Grp Sat Flow(s),veh/h/lr		1870	1967	1875	1870	1926	1297	1969	1668	1419	0	1668	
	2.1	38.7	38.7	0.2	13.2	13.3	1.1	0.9	1.5	18.2	0.0	10.7	
Q Serve(g_s), s Cycle Q Clear(g_c), s	2.1	38.7	38.7	0.2	13.2	13.3	20.1	0.9	1.5	19.1	0.0	10.7	
Prop In Lane	1.00	30.1	0.00	1.00	13.2	0.12	1.00	0.9	1.00	0.92	0.0	1.00	
ane Gro Cap(c), veh/h		1356	1426	179	1324	1363	80	311	264	291	0	276	
//C Ratio(X)	0.20	0.69	0.69	0.04	0.34	0.34	0.12	0.05	0.08	0.68	0.00	0.51	
Avail Cap(c_a), veh/h	623	1356	1426	331	1324	1363	211	510	433	440	0.00	445	
CM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Jpstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	
Jniform Delay (d), s/veh	5.8	10.6	10.6	11.0	7.9	8.0	67.5	50.0	50.2	57.3	0.0	53.2	
ncr Delay (d2), s/veh	0.1	2.9	2.8	0.0	0.7	0.7	0.3	0.0	0.0	1.0	0.0	0.5	
nitial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
6ile BackOfQ(50%),veh	/ln0.7	14.6	15.3	0.1	5.0	5.2	0.4	0.5	0.6	6.7	0.0	4.5	
Jnsig. Movement Delay													
.nGrp Delay(d),s/veh	5.9	13.5	13.4	11.0	8.6	8.7	67.7	50.0	50.3	58.3	0.0	53.8	
nGrp LOS	A	В	В	В	A	A	Е	D	D	E	A	D	
Approach Vol, veh/h		2027			934			46			338		
Approach Delay, s/veh		13.1			8.7			54.0			56.4		
Approach LOS		В			Α			D			Е		
imer - Assigned Phs	1	2		4	5	6		8					
hs Duration (G+Y+Rc)	s9.1	103.1		27.8	6.7	105.5		27.8					
Change Period (Y+Rc).		* 6.1		* 5.7	5.0	* 6.1		* 5.7					
Max Green Setting (Gm		* 74		* 36	13.0	* 74		* 36					
Max Q Clear Time (g. c-		15.3		22.1	2.2	40.7		21.1					
Green Ext Time (p_c), s	0.0	0.6		0.0	0.0	1.6		0.3					
ntersection Summary													
HCM 6th Ctrl Delay			16.8										
ICM 6th LOS			В										
lotes	al a a cit			- عام امر			alle e e		analar 1	ha ha			
HCM 6th computations	aı engi	ne requ	iies edi	лап стеа	rance t	mes to	uie pn	uses cr	ussing 1	ne oarr	ei.		

Intersection							
Int Delay, s/veh	0.7						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	7	44	† \$		¥		
Traffic Vol, veh/h	5	1430	627	6	- 11	8	
Future Vol, veh/h	5	1430	627	6	11	8	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None		None		None	
Storage Length	290		-		0		
Veh in Median Storage,	# -	0	0		0		
Grade, %	-	0	0		0		
Peak Hour Factor	86	86	86	86	86	86	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	8	2162	948	9	17	12	
Major/Minor M	lajor1		Najor2		Minor2		
Conflicting Flow All	957	0	riajuiz		2050	479	
Stage 1	331	-	-	-	953	413	
Stage 2					1097		
	4.14				6.84	6.94	
Critical Hdwy Stg 1	4.14	- :	- 1	- 1		0.54	
Critical Hdwy Stg 2					5.84		
	2.22				3.52	3.32	
Pot Cap-1 Maneuver	714				48	533	
Stage 1					335	-	
Stage 2					281		
Platoon blocked, %			-				
Mov Cap-1 Maneuver	714	-			47	533	
Mov Cap-2 Maneuver	-		-		47		
Stage 1	-				331		
Stage 2		-	-	-	281	-	
Anneach	EB		WB		SB		
Approach HCM Control Delay, s	0		0		78.7		
HCM LOS	U		U		70.7 F		
HCM LUS							
Minor Lane/Major Mymt		EBL	EBT	WBT	WBR :	SBLn1	
Capacity (veh/h)		714			-	76	
HCM Lane V/C Ratio		0.011	-			0.378	
HCM Control Delay (s)		10.1			-	78.7	
HCM Lane LOS		В			-	F	
HCM 95th %tile Q(veh)		0	-	-	-	1.5	
HCM Lane LOS		В				F	

	•	-	*	1	—	•	1	†	/	/	Į.	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SE
Lane Configurations	-	† †	7		↑ ₽		-	4	7		1	
Traffic Volume (vph)	21	1300	240	185	669	2	57	9	75	1	5	
Future Volume (vph)	21	1300	240	185	669	2	57	9	75	1	5	
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	201
Total Lost time (s)	4.7	4.0	5.3	4.0	4.0		4.0	5.1	4.4	5.1	5.1	5
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95		0.95	0.95	1.00	1.00	1.00	1.0
Frt	1.00	1.00	0.85	1.00	1.00		1.00	1.00	0.85	1.00	1.00	0.8
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	0.96	1.00	0.95	1.00	1.0
Satd. Flow (prot)	1845	3689	1650	1845	3688		1752	1779	1650	1845	1942	16
Flt Permitted	0.29	1.00	1.00	0.04	1.00		0.95	0.96	1.00	0.95	1.00	1.0
Satd. Flow (perm)	562	3689	1650	86	3688		1752	1779	1650	1845	1942	16
Peak-hour factor, PHF	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.8
Growth Factor (vph)	130%	130%	130%	130%	130%	130%	130%	130%	130%	130%	130%	130
Adj. Flow (vph)	31	1920	355	273	988	3	84	13	111	1	7	
RTOR Reduction (vph)	0	0	49	0	0	0	0	0	104	0	0	
Lane Group Flow (vph)	31	1920	306	273	991	0	48	49	7	1	7	
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3
Turn Type	pm+pt	NA	Perm	pm+pt	NA		Split	NA	Perm	Split	NA	Per
Protected Phases	1	6		5	2		4	4		3	3	
Permitted Phases	6		6	2					4			
Actuated Green, G (s)	87.9	85.4	85.4	113.6	106.4		7.8	7.8	7.8	2.4	2.4	2
Effective Green, g (s)	87.9	87.8	86.5	114.3	108.4		8.9	7.8	8.5	2.4	2.4	2
Actuated g/C Ratio	0.63	0.63	0.62	0.82	0.77		0.06	0.06	0.06	0.02	0.02	0.1
Clearance Time (s)	4.7	6.4	6.4	4.7	6.0		5.1	5.1	5.1	5.1	5.1	5
Vehicle Extension (s)	1.0	1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0	1.0	1
Lane Grp Cap (vph)	375	2313	1019	369	2855		111	99	100	31	33	
v/s Ratio Prot	0.00	c0.52		c0.13	0.27		0.03	c0.03		0.00	c0.00	
v/s Ratio Perm	0.05		0.19	0.48					0.00			0.0
v/c Ratio	0.08	0.83	0.30	0.74	0.35		0.43	0.49	0.07	0.03	0.21	0.0
Uniform Delay, d1	9.9	20.3	12.5	45.4	4.9		63.1	64.2	62.0	67.7	67.9	67
Progression Factor	1.13	1.23	1.65	1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.0
Incremental Delay, d2	0.0	0.3	0.1	6.6	0.3		1.0	1.4	0.1	0.2	1.2	0
Delay (s)	11.2	25.3	20.8	51.9	5.2		64.1	65.6	62.1	67.8	69.0	67
Level of Service	B	C	C	D	A		E	E	F	E	E	
Approach Delay (s)		24.4	·		15.3		_	63.4	_	_	68.5	
Approach LOS		C			В			E			E	
Intersection Summary												
HCM 2000 Control Delay			23.6	Н	CM 2000	Level of	Service		С			
HCM 2000 Volume to Capa	acity ratio		0.78									
Actuated Cycle Length (s)	. ,		140.0	S	um of los	time (s)			18.9			
Intersection Capacity Utiliza	ation		76.9%	IC	U Level	of Service			D			
Analysis Period (min)			15									
c Critical Lane Group												
Culebra Arterial Study 7:15	am 12/15/2	020 2020	AM Exis	ating Cond	lition					Si	nchro 10	Reno

89 89 2000 4.0 1.00	1100 1100 2000 4.0	117 117 1900	1 1	WBL 46 46	WBT 632 632	WBR 50 50	NBL 54	NBT	NBR 59	SBL 176	SB 1
89 89 2000 4.0 1.00	1100 1100 2000	117	1	46	632		54	9	59		-
89 2000 4.0 1.00	1100 2000	117	1		632				59	176	
2000 4.0 1.00	2000			46	632						
4.0 1.00		1900					54	9	59	176	
1.00	4.0		2000	1900	2000	2000	1900	1900	1900	2000	190
				4.9	4.0		5.4	5.4		4.2	5
	0.95			1.00	0.95		1.00	1.00		1.00	1.0
1.00	0.99			1.00	0.99		1.00	0.87		1.00	0.8
0.95	1.00			0.95	1.00		0.95	1.00		0.95	1.0
											158
											1.0
											158
											0.9
											130
											- 2
											19
											13
		3%				3%			3%		3
pm+pt							Perm			Perm	N
	6				2			4			
			2								
											24
											24
											0.2
											5
											1
							72			292	32
	c0.48				0.27			0.02			0.0
											0.4
											41
											1.0
											41
A				В			r			E	52
	16.4 B				12.4 B			98.4 F			52
		24.9	Н	ICM 2000	Level of S	ervice		С			
ratio		0.81									
		120.0						14.3			
1		85.6%	10	CU Level o	of Service			E			
		15									
	1 6 80.4 82.2 0.69 4.9 0.5 386 c0.02 0.21 0.33 7.7 1.00 0.2 7.9 A	0.23 1.00 4.53 3636 0.91 0.91 1.30% 1.30% 1.30% 1.30% 1.30% 1.30% 1.27 1.571 1.27 1.571 1.27 1.573 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.	0.23	0.23 1.00 453 3636 0.91 0.91 0.91 0.91 0.91 130% 130% 130% 130% 130% 130% 130% 130	0.23 1.00 0.06 4453 3636 117 0.91 0.91 0.91 0.91 0.91 1.00% 1.00% 1.00% 1.00% 1.00% 0 5 0 0 0 0 75 0 0 0 0 75 0 0 0 0 75 0 0 0 0 75 0 0 0 0 75 0 0 0	0.23 1.00 0.06 1.00 1.00 1.00 1.00 1.00 1.00	0.23 1.00	0.23 1.00 0.06 1.00 0.19 453 3836 1177 3649 330 0.91 0.91 0.91 0.91 0.91 0.91 0.91 0.91	0.23 1.00 0.06 1.00 0.19 1.00	0.23 1.00	0.23 1.00

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Movement	EBL	EBT	WBT	WBR	SBL	SBR		
Lane Configurations	*	44	†ĵ₃		,	7		
Traffic Volume (vph)	68	1218	623	78	195	103		
Future Volume (vph)	68	1218	623	78	195	103		
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000		
Total Lost time (s)	4.0	4.0	4.0		4.0	4.0		
Lane Util. Factor	1.00	0.95	0.95		1.00	1.00		
Frt	1.00	1.00	0.98		1.00	0.85		
Flt Protected	0.95	1.00	1.00		0.95	1.00		
Satd. Flow (prot)	1863	3725	3663		1863	1667		
Flt Permitted	0.18	1.00	1.00		0.95	1.00		
Satd. Flow (perm)	352	3725	3663		1863	1667		
Peak-hour factor, PHF	0.89	0.89	0.89	0.89	0.89	0.89		
Growth Factor (vph)	130%	130%	130%	130%	130%	130%		
Adj. Flow (vph)	99	1779	910	114	285	150		
RTOR Reduction (vph)	0	0	11	0	0	0		
Lane Group Flow (vph)	99	1779	1013	0	285	150		
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%		
Turn Type	pm+pt	NA	NA		Prot	pt+ov		
Protected Phases	1	6	2		8	18		
Permitted Phases	6					6		
Actuated Green, G (s)	41.0	41.0	32.2		13.4	59.4		
Effective Green, g (s)	42.0	42.6	33.8		14.4	60.4		
Actuated g/C Ratio	0.65	0.66	0.52		0.22	0.93		
Clearance Time (s)	5.0	5.6	5.6		5.0			
Vehicle Extension (s)	0.5	1.0	1.0		1.0			
Lane Grp Cap (vph)	339	2441	1904		412	1549		
v/s Ratio Prot	0.02	c0.48	0.28		c0.15	0.03		
v/s Ratio Perm v/c Ratio	0.17	0.73	0.53		0.69	0.06		
	6.0	7.4	10.4		23.3	0.10		
Uniform Delay, d1	1.00	1.00	1.00		1.00	1.00		
Progression Factor Incremental Delay, d2	0.2	2.0	1.00		4.0	0.0		
Delay (s)	6.1	9.3	11.4		27.3	0.0		
Level of Service	0.1 A	9.5 A	11.4 B		21.3 C	0.2 A		
Approach Delay (s)	A	9.2	11.4		17.9	^		
Approach LOS		3.2 A	В		17.5 B			
Intersection Summary								
HCM 2000 Control Delay			11.0	H	CM 2000	Level of Service	В	
HCM 2000 Volume to Capa	city ratio		0.77					
Actuated Cycle Length (s)			65.0		um of lost		12.0	
Intersection Capacity Utiliza	tion		61.6%	IC	U Level	of Service	В	
Analysis Period (min)			15					
c Critical Lane Group								

	٠	→	•	•	←	•	4	†	/	-	ļ	*
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SI
Lane Configurations	*	† 12			44	7		ħ			ħ	
Traffic Volume (vph)	30	1350	30	9	599	39	8	1	15	14	5	
Future Volume (vph)	30	1350	30	9	599	39	8	1	15	14	5	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	19
Total Lost time (s)	4.5	5.8		4.5	5.8	5.8	5.4	5.4		5.4	5.4	
Lane Util. Factor	1.00	0.95		1.00	0.95	1.00	1.00	1.00		1.00	1.00	
Frt	1.00	1.00		1.00	1.00	0.85	1.00	0.86		1.00	0.88	
Flt Protected	0.95	1.00		0.95	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1770	3528		1770	3539	1583	1770	1597		1770	1631	
Flt Permitted	0.32	1.00		0.09	1.00	1.00	0.82	1.00		0.77	1.00	
Satd. Flow (perm)	594	3528		174	3539	1583	1521	1597		1433	1631	
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0
Growth Factor (vph)	130%	130%	130%	130%	130%	130%	130%	130%	130%	130%	130%	13
Adj. Flow (vph)	40	1809	40	12	803	52	11	1	20	19	7	
RTOR Reduction (vph)	0	1	0	0	0	12	0	19	0	0	33	
Lane Group Flow (vph)	40	1848	0	12	803	40	11	2	0	19	8	
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	
Turn Type	pm+pt	NA		pm+pt	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	1	6		5	2			4			3	
Permitted Phases	6			2		2	4			3		
Actuated Green, G (s)	112.0	112.0		111.9	110.6	110.6	4.9	4.9		5.2	5.2	
Effective Green, q (s)	112.0	112.0		111.9	110.6	110.6	4.9	4.9		5.2	5.2	
		0.77		0.77	0.76	0.76	0.03	0.03		0.04	0.04	
Actuated g/C Ratio	0.77											
Actuated g/C Ratio Clearance Time (s)	4.5	5.8		4.5	5.8	5.8	5.4	5.4		5.4	5.4	
Actuated g/C Ratio								5.4 0.5			5.4 0.5	
Actuated g/C Ratio Clearance Time (s)	4.5	5.8		4.5	5.8	5.8	5.4	0.5		5.4		
Actuated g/C Ratio Clearance Time (s) Vehicle Extension (s)	4.5 0.5	5.8 0.5		4.5 1.0	5.8 1.0	5.8 1.0	5.4 0.5	0.5		5.4 0.5	0.5	
Actuated g/C Ratio Clearance Time (s) Vehicle Extension (s) Lane Grp Cap (vph) v/s Ratio Prot	4.5 0.5 484	5.8 0.5 2725 c0.52		4.5 1.0 154	5.8 1.0 2699 c0.23	5.8 1.0	5.4 0.5	0.5 53 0.00		5.4 0.5	0.5 58 0.01	
Actuated g/C Ratio Clearance Time (s) Vehicle Extension (s) Lane Grp Cap (vph)	4.5 0.5 484 0.00	5.8 0.5 2725		4.5 1.0 154 0.00	5.8 1.0 2699	5.8 1.0 1207	5.4 0.5 51	0.5		5.4 0.5 51	0.5	
Actuated g/C Ratio Clearance Time (s) Vehicle Extension (s) Lane Grp Cap (vph) v/s Ratio Prot v/s Ratio Perm	4.5 0.5 484 0.00 0.06	5.8 0.5 2725 c0.52		4.5 1.0 154 0.00 0.06	5.8 1.0 2699 c0.23	5.8 1.0 1207 0.03	5.4 0.5 51 c0.01	0.5 53 0.00		5.4 0.5 51 c0.01	0.5 58 0.01	
Actuated g/C Ratio Clearance Time (s) Vehicle Extension (s) Lane Grp Cap (vph) v/s Ratio Prot v/s Ratio Perm v/c Ratio Uniform Delay, d1	4.5 0.5 484 0.00 0.06 0.08	5.8 0.5 2725 c0.52 0.68		4.5 1.0 154 0.00 0.06 0.08	5.8 1.0 2699 c0.23	5.8 1.0 1207 0.03 0.03	5.4 0.5 51 c0.01 0.22	0.5 53 0.00 0.03		5.4 0.5 51 c0.01 0.37	0.5 58 0.01 0.14	
Actuated g/C Ratio Clearance Time (s) Vehicle Extension (s) Lane Grp Cap (vph) v/s Ratio Prot v/c Ratio Perm v/c Ratio Uniform Delay, d1 Progression Factor Incremental Delay, d2	4.5 0.5 484 0.00 0.06 0.08 4.2 1.00	5.8 0.5 2725 c0.52 0.68 7.9 1.00		4.5 1.0 154 0.00 0.06 0.08 12.4 1.00 0.1	5.8 1.0 2699 c0.23 0.30 5.3 1.00 0.3	5.8 1.0 1207 0.03 0.03 4.2 1.00 0.1	5.4 0.5 51 c0.01 0.22 68.2 1.00 0.8	0.5 53 0.00 0.03 67.8 1.00 0.1		5.4 0.5 51 c0.01 0.37 68.3 1.00	0.5 58 0.01 0.14 67.7 1.00 0.4	
Actuated g/C Ratio Clearance Time (s) Vehicle Extension (s) Lane Grp Cap (vph) v/s Ratio Prot v/s Ratio Perm v/c Ratio Uniform Delay, d1 Progression Factor Incremental Delay, d2 Delay (s)	4.5 0.5 484 0.00 0.06 0.08 4.2 1.00	5.8 0.5 2725 c0.52 0.68 7.9 1.00		4.5 1.0 154 0.00 0.06 0.08 12.4 1.00 0.1 12.5	5.8 1.0 2699 c0.23 0.30 5.3 1.00	5.8 1.0 1207 0.03 0.03 4.2 1.00 0.1 4.2	5.4 0.5 51 c0.01 0.22 68.2 1.00 0.8 69.0	0.5 53 0.00 0.03 67.8 1.00 0.1 67.8		5.4 0.5 51 c0.01 0.37 68.3 1.00 1.7	0.5 58 0.01 0.14 67.7 1.00 0.4 68.1	
Actuated g/C Ratio Cleaterance Time (s) Vehicle Extension (s) Lane Grp Cap (vph) v/s Ratio Prot v/s Ratio Prot v/s Ratio Perm v/c Ratio Uniform Delay, d1 Progression Factor Incremental Delay, d2 Delay (s) Level of Service	4.5 0.5 484 0.00 0.06 0.08 4.2 1.00	5.8 0.5 2725 c0.52 0.68 7.9 1.00 1.4 9.3		4.5 1.0 154 0.00 0.06 0.08 12.4 1.00 0.1	5.8 1.0 2699 c0.23 0.30 5.3 1.00 0.3 5.6 A	5.8 1.0 1207 0.03 0.03 4.2 1.00 0.1	5.4 0.5 51 c0.01 0.22 68.2 1.00 0.8	0.5 53 0.00 0.03 67.8 1.00 0.1 67.8 E		5.4 0.5 51 c0.01 0.37 68.3 1.00	0.5 58 0.01 0.14 67.7 1.00 0.4 68.1 E	
Actuated g/C Ratio Clearance Time (s) Vehicle Extension (s) Lane Grp Cap (vph) vis Ratio Prot vis Ratio Port vis Ratio Derm vic Ratio Uniform Delay, d1 Progression Factor Incremental Delay, d2 Delay (s) Level of Service Approach Delay (s)	4.5 0.5 484 0.00 0.06 0.08 4.2 1.00 0.0 4.2	5.8 0.5 2725 c0.52 0.68 7.9 1.00 1.4 9.3 A 9.2		4.5 1.0 154 0.00 0.06 0.08 12.4 1.00 0.1 12.5	5.8 1.0 2699 c0.23 0.30 5.3 1.00 0.3 5.6 A	5.8 1.0 1207 0.03 0.03 4.2 1.00 0.1 4.2	5.4 0.5 51 c0.01 0.22 68.2 1.00 0.8 69.0	0.5 53 0.00 0.03 67.8 1.00 0.1 67.8 E		5.4 0.5 51 c0.01 0.37 68.3 1.00 1.7	0.5 58 0.01 0.14 67.7 1.00 0.4 68.1 E 68.7	
Actuated g/C Ratio Clearance Time (s) Vehicle Extension (s) Lane Grp Cap (vph) vis Ratio Prot vis Ratio Port vis Ratio Derm vic Ratio Uniform Delay, d1 Progression Factor Incremental Delay, d2 Delay (s) Level of Service Approach Delay (s)	4.5 0.5 484 0.00 0.06 0.08 4.2 1.00 0.0 4.2	5.8 0.5 2725 c0.52 0.68 7.9 1.00 1.4 9.3		4.5 1.0 154 0.00 0.06 0.08 12.4 1.00 0.1 12.5	5.8 1.0 2699 c0.23 0.30 5.3 1.00 0.3 5.6 A	5.8 1.0 1207 0.03 0.03 4.2 1.00 0.1 4.2	5.4 0.5 51 c0.01 0.22 68.2 1.00 0.8 69.0	0.5 53 0.00 0.03 67.8 1.00 0.1 67.8 E		5.4 0.5 51 c0.01 0.37 68.3 1.00 1.7	0.5 58 0.01 0.14 67.7 1.00 0.4 68.1 E	
Actuated gC Ratio Clearance Time (s) Vehicle Extension (s) Lane Grp Cap (vph) vis Ratio Prot vis Ratio Prot vis Ratio Prot vis Ratio Prot vis Ratio Prot Progression Factor Incremental Delay, d1 Delay (s) Level of Service Approach Delay (s) Approach LOS	4.5 0.5 484 0.00 0.06 0.08 4.2 1.00 0.0 4.2	5.8 0.5 2725 c0.52 0.68 7.9 1.00 1.4 9.3 A 9.2		4.5 1.0 154 0.00 0.06 0.08 12.4 1.00 0.1 12.5	5.8 1.0 2699 c0.23 0.30 5.3 1.00 0.3 5.6 A	5.8 1.0 1207 0.03 0.03 4.2 1.00 0.1 4.2	5.4 0.5 51 c0.01 0.22 68.2 1.00 0.8 69.0	0.5 53 0.00 0.03 67.8 1.00 0.1 67.8 E		5.4 0.5 51 c0.01 0.37 68.3 1.00 1.7	0.5 58 0.01 0.14 67.7 1.00 0.4 68.1 E 68.7	
Actuated gC Ratio Clearance Time (s) Vehicle Extension (s) Lane Grp Cap (vph) vis Ratio Port vis Ratio Port vis Ratio Perm vic Ratio Uniform Delay, d1 Progression Factor Incremental Delay, d2 Delay (s) Approach Delay (s) Approach DOS Intersection Summany	4.5 0.5 484 0.00 0.06 0.08 4.2 1.00 0.0 4.2	5.8 0.5 2725 c0.52 0.68 7.9 1.00 1.4 9.3 A 9.2	10.0	4.5 1.0 154 0.00 0.06 0.08 12.4 1.00 0.1 12.5 B	5.8 1.0 2699 c0.23 0.30 5.3 1.00 0.3 5.6 A 5.6	5.8 1.0 1207 0.03 0.03 4.2 1.00 0.1 4.2 A	5.4 0.5 51 c0.01 0.22 68.2 1.00 0.8 69.0 E	0.5 53 0.00 0.03 67.8 1.00 0.1 67.8 E		5.4 0.5 51 c0.01 0.37 68.3 1.00 1.7	0.5 58 0.01 0.14 67.7 1.00 0.4 68.1 E 68.7	
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4	69	740	520	100	330	54	1	248	345	58	12
1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	190
	4.4	5.7	5.7	4.8	5.7			4.9	5.8		4
	1.00	0.95	1.00	1.00	0.95			0.97	0.95		0.9
	1.00	1.00	0.85	1.00	0.98			1.00	0.98		1.0
	0.95	1.00	1.00	0.95	1.00			0.95	1.00		0.9
	1752	3505	1568	1752	3431			3400	3429		340
	0.20	1.00	1.00	0.95	1.00			0.26	1.00		0.9
	375	3505	1568	1752	3431			948	3429		340
0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.8
130%	130%	130%	130%	130%	130%	130%	130%	130%	130%	130%	130
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	19.7	45.1	45.1	14.5	40.3			15.1	48.6		10
	0.14	0.32	0.32	0.10	0.29			0.11	0.35		0.0
	4.4	5.7	5.7	4.8	5.7			4.9	5.8		4
	1.0	1.0	1.0	1.0	1.0			1.0	1.0		- 1
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	2.10	0.98	1.19	0.82	0.57			3.65	0.50		0.6
	60.1	47.0	47.5	61.5	42.5			62.5	36.1		62
	1.00	1.00	1.00	1.00	1.00			1.32	0.76		1.0
	552.4	22.3	103.7	24.0	2.4			1214.7	0.1		5
	612.5	69.3	151.2	85.5	44.9			1297.5	27.4		68
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Peak-hour factor, PHF	Satd. Flow (perm)	3486			
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64: Griffin Park	UF &	Cuie	ыга Н	tu				02/03/20
Intersection								
Int Delay, s/veh	1.5							
Movement		EBR	WBL		NBL	NBR		
Lane Configurations	<u>ቀቀ</u> ነ			411		7		
Traffic Vol, veh/h	1480	10	4	750	9	22		
Future Vol, veh/h	1480	10	4	750	9	22		
Conflicting Peds, #/hr		0	0	0	0	0		
Sign Control		Free			Stop	Stop		
RT Channelized	- 1	None	- 1	None	- 0	None 0		
Storage Length Veh in Median Storage				0	0	-		
Grade. %	0	- :	- 1	0	0	- :		
Peak Hour Factor	90	90	90	90	90	90		
Heavy Vehicles, %	2	2	2	2	2	2		
Mymt Flow	2138	14	6	1083	13	32		
invinci ion	2100		•	1000	10	O.L		
Major/Minor N	Major1	- 1	Major2	- 1	Minor1			
Conflicting Flow All	0		2152		2590	1076		
Stage 1	-		-		2145			
Stage 2	-		-	-	445	-		
Critical Hdwy	-		5.34		5.74	7.14		
Critical Hdwy Stg 1	-							
Critical Hdwy Stg 2	-		-		6.04			
Follow-up Hdwy	-			-		3.92		
Pot Cap-1 Maneuver Stage 1	- :	- :	105	- :	45 46	185		
Stage 2					560			
Platoon blocked. %					000			
Mov Cap-1 Maneuver			105		39	185		
Mov Cap-2 Maneuver	-		-		42			
Stage 1	-				46			
Stage 2	-		-		479			
Approach	EB 0		2.1		NB 56.6			
HCM Control Delay, s HCM LOS	0		2.1		56.6 F			
HOM LOS								
Minor Lane/Major Mvm		NBLn11	VRI n2	EBT	FRR	WBL	WBT	
Capacity (veh/h)		42		-	-	105		
HCM Lane V/C Ratio			0.172	-		0.055		
HCM Control Delay (s)			28.5		-	41.3	1.9	
HCM Lane LOS		F	D	-	-		A	
HCM 95th %tile Q(veh)		1	0.6		-	0.2		
Culebra Arterial Study	7·15 an	12/15/	2020.2	020 AM	Evietin	a Cond	ition	Synchro 10 Rep

1000: Culebra F	Ka &	a & C	riss	om (ate		
Intersection							
Int Delay, s/veh	2.5	2.5					
Movement	SEL	SEL	SET	NWT	NWR	SWL	SWR
Lane Configurations				ተ ቀሴ		N/	
Traffic Vol, veh/h	4		1900	875	22	40	5
Future Vol, veh/h	4		1900	875	22	40	5
Conflicting Peds, #/hr			0	0	0	0	0
Sign Control	Free		Free	Free	Free	Stop	Stop
RT Channelized			None	-		-	None
Storage Length	e#-		0	0		0	- 1
Veh in Median Storage Grade. %	e,# -		0	0		0	- 1
Peak Hour Factor	95		95	95	95	95	95
Heavy Vehicles, %	95		95	95	95	95	95
Mymt Flow			2600	1197	30	55	7
minici ion	·		2000		00	- 00	
Major/Minor I	Major1	nior1		Major2	,	Minor2	
Conflicting Flow All	1227		0	najorz	0	2262	614
Stage 1	1221		-		-	1212	014
Stage 2						1050	
Critical Hdwy	5.38	5.38				5.78	7.18
Critical Hdwy Stg 1			-			6.68	
Critical Hdwy Stg 2	-		-			6.08	-
Follow-up Hdwy	3.14	3.14	-	-		3.84	3.94
Pot Cap-1 Maneuver	298	298	-			66	369
Stage 1	-	-	-	-	-	176	-
Stage 2		-	-			265	
Platoon blocked, %			-	-	-		
Mov Cap-1 Maneuver			-	-		66	369
Mov Cap-2 Maneuver	-		-	-	-	66	-
Stage 1	-		-	-	-	176	
Stage 2		-		-		265	
Approach	SE			NW		SW	
HCM Control Delay, s	0	0		0		160.4	
						F	
HCM LOS							
HCM LOS			NWT	NWR	SEL		SWLn1
Minor Lane/Major Mvm	nt		-	-	298	-	10
Minor Lane/Major Mvm Capacity (veh/h)	nt				0.018		0.844
Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio						0 A	160.4 F
Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)			-			A	
Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS)				C		4.4
Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS)				0.1	-	4.1
Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s))					•	4.1
Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS HCM 95th %sile Q(veh)) a)		•	•	0.1		
Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS) a)	15 am	•	•	0.1		

	۶	•	1	Ť	ļ	4	
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	ě	7	ě	444	**†		
Traffic Volume (veh/h)	68	86	22	597	1202	36	
Future Volume (veh/h)	68	86	22	597	1202	36	
Initial Q (Qb), veh	0	0	0	0	0	0	
Ped-Bike Adj(A_pbT)	1.00	1.00	1.00			1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	
Work Zone On Approach	No			No	No		
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	
Adj Flow Rate, veh/h	99	126	32	872	1756	53	
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89	
Percent Heavy Veh, %	2	2	2	2	2	2	
Cap, veh/h	166	148	770	4206	1801	54	
Arrive On Green	0.09	0.09	0.43	0.82	0.71	0.71	
Sat Flow, veh/h	1781	1585	1781	5274	5261	154	
Grp Volume(v), veh/h	99	126	32	872	1173	636	
Grp Sat Flow(s),veh/h/ln	1781	1585	1781	1702	1702	1843	
Q Serve(g_s), s	7.5	11.0	1.5	5.1	45.5	45.6	
Cycle Q Clear(g_c), s	7.5	11.0	1.5	5.1	45.5	45.6	
Prop In Lane	1.00	1.00	1.00			0.08	
Lane Grp Cap(c), veh/h	166	148	770	4206	1204	652	
V/C Ratio(X)	0.60	0.85	0.04	0.21	0.97	0.98	
Avail Cap(c_a), veh/h	447	397	770	4206	1914	1036	
HCM Platoon Ratio	1.00	1.00	1.00	1.00	2.00	2.00	
Upstream Filter(I)	1.00 60.9	1.00 62.5	0.96 23.0	0.96	0.09	0.09	
Uniform Delay (d), s/veh	1.3	5.2	0.0	0.1	3.9	6.3	
Incr Delay (d2), s/veh	0.0	0.0	0.0	0.1	0.0	0.0	
Initial Q Delay(d3),s/veh	3.5	9.6	0.0	1.2	8.7	9.9	
%ile BackOfQ(50%),veh/ln Unsig. Movement Delay, s/veh	3.5	9.6	0.0	1.2	8.7	9.9	
LnGrp Delay(d),s/veh	62.2	67.7	23.0	2.7	23.8	26.2	
LnGrp LOS	02.2 E	67.7 E	23.0 C	Α.	23.0 C	20.2 C	
Approach Vol, veh/h	225			904	1809		
	65.3			3.4	24.6		
Approach Delay, s/veh Approach LOS	65.3 F			3.4 A	24.6 C		
•••					C		
Timer - Assigned Phs		2		4	5	6	
Phs Duration (G+Y+Rc), s		121.0		19.0	66.2	54.8	
Change Period (Y+Rc), s		5.7		5.9	5.7	* 5.3	
Max Green Setting (Gmax), s		93.3		35.1	10.0	* 79	
Max Q Clear Time (g_c+l1), s		7.1		13.0	3.5	47.6	
Green Ext Time (p_c), s		1.0		0.1	0.0	1.9	
Intersection Summary							
HCM 6th Ctrl Delay			21.2				
HCM 6th LOS			C				
Notes							
* HCM 6th computational engin	e requir	es equal	clearance	times fo	r the phas	ses crossir	ng the barrier.

0.5 EBL 6 6 0 Free	2300		WBR		
6 6 0 Free	4 ↑↑ 2300		WPP		
6 6 0 Free	4 ↑↑ 2300		WDD		
6 0 Free	4 ↑↑ 2300		M/DD		
6 0 Free	4 ↑↑ 2300			SBL	SBR
6 0 Free	2300			¥	
0 Free		751	3	11	14
Free	2300	751	3	11	14
	0	0	0	0	0
	Free	Free	Free	Stop	Stop
	None	-	None	-	None
				0	
e,# -	0	0		0	
-,	0	0		0	
90	90	90	90	90	90
3	3	3	3	3	3
			4	16	20
	OULL	1000			
1089					545
-		-			
					7.16
	-				-
					3.93
			-		411
-	-				-
-				183	
	-				
352					411
-	-	-	-		-
-	-	-	-		-
-		-	-	183	
FB		WB		SB	
U		U			
nt			WBT	WBR :	
		-	-	-	105
		-	-	-	0.344
)					56.3
			-	-	F
)	0.1	-	-	-	1.4
	3 3 9 Major1 1089	Major1 1089 0 0 0 0 0 0 0 0 0	3 3 3 3 3 3 3 3 3 3	3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3	3 3 3 3 3 3 3 3 3 3

Intersection Int	70: Pipers Cree		()	<u> </u>						02/03/20
Int Delay, s/veh 33.7 Movement EBT EBR WBL WBT NBL NBR Traffic Vol, vehlh 2300 24 14 800 21 30 Cunflicing Peds, #hr 0 0 0 0 0 0 0 Conflicing Peds, #hr 0 0 0 0 0 0 0 Conflicing Peds, #hr 0 0 0 0 0 0 0 Conflicing Peds, #hr 0 0 0 0 0 0 0 Conflicing Peds, #hr 0 0 0 0 0 0 0 Conflicing Peds, #hr 0 0 0 0 0 0 0 Conflicing Peds, #hr 0 0 0 0 0 0 0 Conflicing Peds, #hr 0 0 0 0 0 0 0 Conflicing Peds, #hr 0 0 0 0 0 0 0 Conflicing Peds, #hr 0 0 0 0 0 0 0 Conflicing Peds, #hr 0 0 0 0 0 0 0 Conflicing Peds, #hr 0 0 0 0 0 0 0 Conflicing Feds Peds Peds Peds Peds Peds Peds Peds P	Interception									
Movement		33.7								
Lane Configurations	**	FRT	FRR	WRI	WRT	MRI	NRP			
Traffic Vol. vehih 2900 24 14 800 21 30 Conflicing Peds, #hr 200 24 14 800 21 30 Conflicing Peds, #hr 200 24 14 800 21 30 Conflicing Peds, #hr 0 0 0 0 0 0 0 Conflicing Peds, #hr 0 0 0 0 0 0 0 Conflicing Peds, #hr 0 0 0 0 0 0 0 Conflicing Peds, #hr 0 0 0 0 0 0 0 Conflicing Peds, #hr 0 0 0 0 0 0 0 Conflicing Peds, #hr 0 0 0 0 0 0 0 Conflicing Peds, #hr 0 0 0 0 0 0 0 0 Conflicing Peds, #hr 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0			LUIT	WIDE			INDIX			
Future Vol. vehih 2300 24 14 880 21 30 Conflicting Peris, #thir 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0			24	14			30			
Sign Control Free Free Free Free Free Stop Stop RT Chammelzed - None -			24	14		21	30			
RI Channelized None None None None Storage Length 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 -	Conflicting Peds. #/hr	0	0	0	0	0	0			
RI Channelized None None None None Storage Length 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 -	Sign Control	Free	Free	Free	Free	Stop	Stop			
Weh in Median Storage, # 0	RT Channelized		None		None	-				
Grade, % 0 0 0 Peak Hour Factor 90 80 90 90 90 90 90 90 90 90 90 90 90 90 90				-	-	0	-			
Peak Hour Factor 90 90 90 90 90 90 90 Hour Factor 90 90 90 90 90 Heavy Vehicles, % 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3							-			
Heavy Vehicles, % 3 3 3 3 3 3 3 3 3										
Major/Minor										
MajorMinor Major1 Major2 Minor1										
Conflicting Flow All 0 0 3357 0 3842 1679 Stage 1 3340 Stage 2 502 Critical Helwy Stag 1 6.66 Critical Helwy Stag 2 6.66 Critical Helwy Stag 2 6.69 Critical Helwy Stag 2 6.69 Critical Helwy Stag 2 6.99 Critical Helwy Stag 2 6.99 Critical Helwy Stag 2 5.21 Stage 1 7 - Stage 2 5.21 Wor Cap-1 Maneuver - 24 - 0.71 Wor Cap-2 Maneuver 24 - 0.71 Wor Cap-1 Maneuver 24 - 0.71 Wor Cap-2 Maneuver 0 Stage 1 7 Stage 2 0 Stage 1 7 Stage 1 7 Stage 1 7 Stage 2 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 2 7 Stage 1 7 Stage 1 Stage 1	Mvmt Flow	3322	35	20	1156	30	43			
Conflicting Flow All 0 0 3357 0 3842 1679 Stage 1 3340 Stage 2 502 Critical Helwy Stag 1 6.66 Critical Helwy Stag 2 6.66 Critical Helwy Stag 2 6.69 Critical Helwy Stag 2 6.69 Critical Helwy Stag 2 6.69 Critical Helwy Stag 2 6.99 Critical Helwy Stag 2 5.70 2.14 8.71 Stage 1 7 Stage 2 5.21 How Cap 2 Maneuver - 24 - 0.71 Mov Cap 2 Maneuver - 24 - 0.71 Mov Cap 2 Maneuver 24 - 0.71 Mov Cap 2 Maneuver 24 - 0.71 Mov Cap 2 Maneuver 2 - 0 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 2 0 Mov Cap 2 Maneuver 2 - 0 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 2 0 Stage 2 0 Stage 2 7 Stage 1 7 Stage 1 7 Stage 2 7 Stage 2 7 Stage 2 7 Stage 1 7 Stage 2 7 Stage 2 7 Stage 1 7 Stage 2 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 2 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 1 7 Stage 2 7 Stage 1 7 Stage 2 7 Stage 2 7 Stage 2 Stage 1										
Stage 2										
Stage 2										
Critical Holwy Sig 1										
Critical Holyy Stg 1 - - 6.66 - Critical Holyy Stg 2 - - 6.06 - Follow-up Holyy - 3.13 - 3.83 3.93 Pot Cap-1 Maneuver - 24 - - 7 Stage 1 - - 7 - Plation blooked, % - - 0 - Mov Cap-1 Maneuver - 24 0 71 Mov Cap-2 Maneuver - - 0 - Stage 2 - - 7 - Stage 2 - 0 - - Approach EB WB NB - HCM Los F - - 7 Minor Lane/Major Mvmt NBLT EBF WBL WBT Capacity (vehr) 71 - 24 - HCM Lane V/C Ratio 10.38 - 0.843 - - HCM Lane LOS </td <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>										
Critical Haw's Sig 2										
Follow-up Howy - 3.13 - 3.83 3.93 PRC Cap-1 Maneuver - 24 - 8 71 Slage 1 7 - 8 Slage 2 521 - 9 Plation blocked , % 7 Mov Cap-1 Maneuver - 24 - 0 71 Mov Cap-1 Maneuver - 24 - 0 71 Mov Cap-2 Maneuver 0 - 9 Slage 1 7 - 9 Slage 2 0 - 9 Slage 2 0 - 9 Slage 2 0 - 9 Slage 1 7 - 9 Slage 2 0 - 9 Slage 1 7 - 9 Slage 2 0 - 9 Morrison EB WB NB HCM Control Delay, s 0 118.4 218.3 HCM LOS F F F F HCM Capacity (vehit) 71 - 24 - 9 HCM Lane V/C Ratio 1,038 - 0,843 - 9 HCM Control Delay (s) 218.3 - \$,800.7 114.2 HCM Lane LOS F - F F HCM Ostrin Delay (s) 5.4 - 2.5 - 9 HCM S5th Kille Q(veh) 5.4 - 2.5 - 9 Hold S5th Kille Q(veh) 5.4 - 2.5 - 9 Hold S6th Kille Q(veh) 5.4 - 2.5 - 9							-			
Pot Cap-1 Maneuver			-				-			
Stage 1										
Slage 2										
Platon tokxed, %				-						
Mov Cap 2 Maneuver						521	-			
Mov Cap 2 Maneuver 0 - Stage 1 7 - Stage 2 0 - O - O - O - O - O - O - O - O -				24		٥	71			
Slage 1										
Stage 2										
Approach EB WB NB HCM Control Delay, s 0 118.4 218.3 HCM LOS Minor Lane/Major Mvmt NBLn1 EBT EBR WBL WBT Capacity (veh/h) 71 - 24 - HCM Lane V/C Ratio 1.038 - 0.843 - HCM Lane LOS F - 5 F F HCM Stin Kelle Q(veh) 5.4 - 2.5 - Notes										
HCM Control Delay, s 0 118.4 218.3 F HCM LOS F Gapacity (reh/h) 71 - 24 - HCM Lone V/C Ratio 1.038 - 0.843 - HCM Control Delay (s) 218.3 - \$,360.7 114.2 HCM Lane LOS F - F F HCM S9th *Mile Q(veh) 5.4 - 2.5 - Notes										
HCM LOS F Minor Lane/Major Mvmt NBLn1 EBT EBR WBL WBT Capacity (vehrh) 71 - 24 - HCM Lane V/C Ratio 1.038 - 0.843 - HCM Lane V/C Ratio 1.038 - \$.860.7 114.2 HCM Lane LOS F - F F HCM 55th %Hile Q(veh) 5.4 - 2.5 - Notes	Approach	EB		WB		NB				
Minor Lane/Major Mvmt NBLn1 EBT EBR WBL WBT Capacity (veh/h) 71 - 24 - HCM Lane V/C Ratio 1.038 - 0.843 - HCM Control Delay (s) 218.3 - \$.560.7 114.2 HCM Lane LOS F - F F HCM 55th Wille Q(veh) 5.4 - 2.5 - Notes	HCM Control Delay, s	0		118.4		218.3				
Capacity (veh/h) 71 - 24 - HCM Lane V/C Ratio 1.038 0.843 - HCM Cantrol Delay (s) 218.3 - \$800.7 114.2 HCM Lane LOS F - F F HCM S5th Kelle Q(veh) 5.4 - 2.5 - Notes	HCM LOS					F				
Capacity (veh/h) 71 - 24 - HCM Lane V/C Ratio 1.038 0.843 - HCM Cantrol Delay (s) 218.3 - \$800.7 114.2 HCM Lane LOS F - F F HCM S5th Kelle Q(veh) 5.4 - 2.5 - Notes										
HCM Lane V/C Ratio 1.038 - 0.843 - HCM Canter V/C Ratio 1.038 - 3.80.07 114.2 - HCM Centro Delay (s) 2.18.3 - \$.360.07 114.2 - F. F. F. F. F. F. F. F. F. F. F. F. F.	Minor Lane/Major Mvm	ıt l	NBLn1	EBT	EBR	WBL	WBT			
HCM Control Delay (s) 218.3 - \$360.7 114.2 HCM Lane LOS F - F F HCM 55th Waite Q(veh) 5.4 - 2.5 - Notes							-			
HCM Lane LOS F F F HCM 95th %alle Q(veh) 5.4 2.5 - Notes										
HCM 95th %tile Q(veh) 5.4 2.5 - Notes										
Notes							F			
	HCM 95th %tile Q(veh)		5.4	-		2.5	-			
:: Volume exceeds capacity \$: Delay exceeds 300s +: Computation Not Defined *: All major volume in platoon										
	~: Volume exceeds cap	acity	\$: De	lay exc	eeds 30	00s	+: Comp	outation Not Defined	*: All major volume in platoon	

Intersection						
Int Delay, s/veh	0.8					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	7		ተ ቀተ»		¥	
Traffic Vol, veh/h	25	2300	800	13	23	22
Future Vol. veh/h	25	2300	800	13	23	22
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized		None		None	-	None
Storage Length	210				0	-
Veh in Median Storage	.# -	0	0	-	0	-
Grade, %	-	0	0		0	
Peak Hour Factor	91	91	91	91	91	91
Heavy Vehicles, %	3	3	3	3	3	3
Mvmt Flow	36		1143	19	33	31
Major/Minor	Major1		Major		Minor2	
			Major2			E04
Conflicting Flow All	1162	0		0		581
Stage 1		- 1	- 1	- :		
Stage 2	E 20					7.10
Critical Hdwy	5.36		-			7.16
Critical Hdwy Stg 1		- 1		- 1	6.66	- 1
Critical Hdwy Stg 2	3.13				3.83	3.93
Follow-up Hdwy	3.13	- 1		- 1	3.83	3.93
Pot Cap-1 Maneuver						
Stage 1	-			- 1	193	
Stage 2 Platoon blocked, %		- 1	- 1	- 1	1/4	
Mov Cap-1 Maneuver	324			- 1	42	390
	324	- 1	- 1	- 1	98	390
Mov Cap-2 Maneuver		- 1		- 1	172	- 1
Stage 1						- :
Stage 2					174	-
Approach	EB		WB		SB	
HCM Control Delay, s	0.2		0		43.8	
HCM LOS					Е	
Minor Lane/Major Mvm	+	EBL	EBT	WBT	WRP	SBLn1
Capacity (veh/h)		324	EDI -	WDI	WDR	155
HCM Lane V/C Ratio		0.11	- 1	- :		0.415
HCM Control Delay (s)		17.5	- 1	-	- 1	
HCM Control Delay (s) HCM Lane LOS		17.5 C	- 1	- 1		
		0.4		-	-	1.8
HCM 95th %tile Q(veh)		0.4			-	1.8
Culebra Arterial Study : WSP - San Antonio	7:15 an	n 12/15	2020 2	020 AM	Fxistin	n Cond

Intersection													
Int Delay, s/veh	15.1												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	LUL	4	LUIT	1100	4	11011		41412	HUIT	ODL	at the	ODIT	
Traffic Vol. veh/h	2	1	30	4	1	19	16	700	2	12	2150	4	
Future Vol. veh/h	2	1	30	4	- 1	19	16	700	2	12	2150	4	
Conflicting Peds. #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None	-	-	None			None			None	
Storage Length	-		-		-					-			
Veh in Median Storage	.# -	0	-		0	-		0	-		0	-	
Grade, %	-	0	-		0	-		0	-	-	0	-	
Peak Hour Factor	94	94	94	94	94	94	94	94	94	94	94	94	
Heavy Vehicles, %	3	3	3	3	3	3	3	3	3	3	3	3	
Mvmt Flow	3	- 1	41	6	1	26	22	968	3	17	2973	6	
Major/Minor 1	Minor2			/linor1			Major1		N	Major2			
Conflicting Flow All	3442	4025	1490	2238	4027	486	2979	0	0	971	0	0	
Stage 1	3010	3010	-	1014	1014		-				i.		
Stage 2	432	1015	-	1224	3013		-			-			
Critical Hdwv	6.46	6.56	7.16	6.46	6.56	7.16	5.36			5.36			
Critical Hdwy Stg 1	7.36	5.56	-	7.36	5.56								
Critical Hdwy Stg 2	6.76	5.56	-	6.76	5.56								
Follow-up Hdwy	3.83	4.03	3.93	3.83	4.03	3.93	3.13	-		3.13		-	
Pot Cap-1 Maneuver	7	3	96	44	3	449	38	-	-	401		-	
Stage 1	7	30	-	193	312	-	-	-	-	-		-	
Stage 2	521	312	-	169	30							-	
Platoon blocked, %								-			-	-	
Mov Cap-1 Maneuver	-	0	96		0	449	38	-		401			
Mov Cap-2 Maneuver		0	-		0	-		-	-			-	
Stage 1	7	30		193	0		-	-		-			
Stage 2		0		92	30		-	-				-	
Approach	EB			WB			NB			SB			
HCM Control Delay, s							61.7			0.1			
HCM LOS	-			-									
Minor Lane/Major Mvm	t	NBL	NBT	NBR I	EBLn1V	VBLn1	SBL	SBT	SBR				
Capacity (veh/h)		38	-		-	-	401		-				
HCM Lane V/C Ratio		0.582			-		0.041	-					
HCM Control Delay (s)		189.2	59	-	-		14.4	0					
HCM Lane LOS		F	F		-		В	Α					
HCM 95th %tile Q(veh)		2.1	-				0.1						

	•	-	*	1	•	•	1	†	1	-	Ų.	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SI
Lane Configurations	*	*		*	*			1	7	100	1	
Traffic Volume (veh/h)	223	2050	20	27	706	20	18	46	95	24	18	
Future Volume (veh/h)	223	2050	20	27	706	20	18	46	95	24	18	
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1841	1841	1841	1841	1841	1841	1841	1841	1841	1841	1841	18
Adj Flow Rate, veh/h	315	2897	28	38	998	28	25	65	134	34	25	1
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0
Percent Heavy Veh, %	4	4	4	4	4	4	4	4	4	4	4	
Cap, veh/h	492	3737	36	108	3391	95	186	175	149	162	186	- 1
Arrive On Green	0.07	0.73	0.73	0.02	0.67	0.67	0.02	0.10	0.10	0.02	0.10	0
Sat Flow, veh/h	1753	5132	49	1753	5025	141	1753	1841	1560	1753	1841	15
Grp Volume(v), veh/h	315	1888	1037	38	665	361	25	65	134	34	25	_ 1
Grp Sat Flow(s),veh/h/ln	1753	1675	1832	1753	1675	1815	1753	1841	1560	1753	1841	15
Q Serve(g_s), s	7.9	51.9	52.5	1.0	11.9	11.9	1.9	4.9	12.6	2.6	1.8	1
Cycle Q Clear(g_c), s	7.9	51.9	52.5	1.0	11.9	11.9	1.9	4.9	12.6	2.6	1.8	1
Prop In Lane	1.00		0.03	1.00		0.08	1.00		1.00	1.00		1
Lane Grp Cap(c), veh/h	492	2439	1334	108	2261	1225	186	175	149	162	186	- 1
I/C Ratio(X)	0.64	0.77	0.78	0.35	0.29	0.29	0.13	0.37	0.90	0.21	0.13	0
Avail Cap(c_a), veh/h	785	2439	1334	199	2261	1225	203	175	149	168	186	
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1
Uniform Delay (d), s/veh	6.9	12.5	12.6	19.5	9.8	9.8	59.1	62.8	66.3	58.7	60.6	6
Incr Delay (d2), s/veh	0.5	2.5	4.5	0.7	0.3	0.6	0.3	0.5	45.1	0.6	0.1	3
nitial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
%ile BackOfQ(50%),veh/ln	2.5	17.3	19.9	0.6	4.2	4.6	0.9	2.3	6.8	1.2	0.9	
Unsig. Movement Delay, s/veh	7.4	45.0	17.1	20.2	40.4	10.4	59.4	63.3	444.0	59.4	60.7	9
LnGrp Delay(d),s/veh		15.0			10.1				111.3			9
LnGrp LOS	A	В	В	С	В	В	E	E	F	E	E	_
Approach Vol, veh/h		3240			1064			224			195	
Approach Delay, s/veh		14.9			10.5			91.6			87.6	
Approach LOS		В			В			F			F	
Timer - Assigned Phs	- 1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	15.3	105.3	7.4	20.0	7.4	113.2	6.6	20.9				
Change Period (Y+Rc), s	5.0	* 5.4	4.0	5.9	5.0	5.4	4.0	5.9				
Max Green Setting (Gmax), s	35.0	* 75	4.0	14.1	10.0	99.6	4.0	14.1				
Max Q Clear Time (g. c+l1), s	9.9	13.9	4.6	14.6	3.0	54.5	3.9	14.7				
Green Ext Time (p_c), s	0.4	2.1	0.0	0.0	0.0	11.4	0.0	0.0				
Intersection Summary												
HCM 6th Ctrl Delay			20.6									
HCM 6th LOS			С									
Votes												
Jser approved pedestrian inter HCM 6th computational engin						ses crossi	ng the ba	rrier.				

ntersection							
nt Delay, s/veh	0.2						
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
ane Configurations	† 1>		٦	44	¥		
raffic Vol, veh/h	1250	200	3	550	5	5	
uture Vol, veh/h	1250	200	3	550	5	5	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free		Free	Free	Stop	Stop	
RT Channelized	-					None	
Storage Length	-		120	0	0		
/eh in Median Storage, Grade. %	# 0	- :	- 1	0	0	- 1	
Peak Hour Factor	95		95	95	95	95	
leavy Vehicles, %	3	95	95	95	95	95	
Nymt Flow	1711		4	753	7	7	
				100			
Major/Minor N	lajor1	1	Major2	,	Minor1		
Conflicting Flow All	0		1985		2233	993	
Stage 1	-	-	1303	-		333	
Stage 2							
Critical Hdwv			4.16			6.96	
Critical Hdwy Stg 1		-					
Critical Hdwy Stg 2	-	-			5.86		
ollow-up Hdwy	-					3.33	
ot Cap-1 Maneuver	-		283		36	242	
Stage 1	-	-	-		109	-	
Stage 2	-		-	-	654	-	
Platoon blocked, %	-						
Mov Cap-1 Maneuver			283		35 92	242	
Nov Cap-2 Maneuver			-		109		
Stage 1 Stage 2	- 1	- 1	- 1		645	- :	
Stage 2					040	- 1	
	FB		14/0		ND		
Approach HCM Control Delay, s	0		0.1		NB 35.1		
ICM Control Delay, s	U		0.1		35.1 F		
ICM EOS							
fines Lana (Maias Mum)		NBLn1	EBT	EDD	WBL	WBT	
Minor Lane/Major Mvmt Capacity (veh/h)		133	EB1	EBR -		WB1	
		0.103	- 1		0.015	- 1	
ICM Lane V/C Ratio		35.1	- 1		17.9	- :	
ICM Lane V/C Ratio							
ICM Lane V/C Ratio ICM Control Delay (s) ICM Lane LOS		E					
ICM Control Delay (s)		0.3			0		

127: Culebra Rd	& F	airgro	ound	Park	way	
Intersection						
Int Delay, s/veh	3.6					
Movement	FBI	EBT	WBT	WBR	SBL	SBR
Lane Configurations	LDL	414	†1 >	WDIN	N/	JUN
Traffic Vol. veh/h	48	1190	650	100	25	14
Future Vol. veh/h	48	1190	650	100	25	14
Conflicting Peds. #/hr	-0	1130	0.00	000	0	0
	Free		Free	Free	Stop	Stop
RT Channelized		None		None	otop	None
Storage Length		-		-	0	-
Veh in Median Storage,		0	0	- 1	0	- 1
Grade. %	# -	0	0	- 1	0	- 1
Peak Hour Factor	93	93	93	93	93	93
	93	93	93	93	93	93
Heavy Vehicles, % Mymt Flow	67	1663	909	140	35	20
MVMt Flow	6/	1003	909	140	35	20
	lajor1		Major2		Minor2	
Conflicting Flow All	1049	0		0	1945	525
Stage 1	-	-		-	979	
Stage 2	-	-		-	966	
Critical Hdwy	4.18				6.88	6.98
Critical Hdwy Stg 1	-	-		-	5.88	
Critical Hdwy Stg 2		-		-	5.88	
Follow-up Hdwy	2.24	-		-	3.54	3.34
Pot Cap-1 Maneuver	647	-		-	55	492
Stage 1	-	-		-	320	
Stage 2		-		-	325	
Platoon blocked, %						
Mov Cap-1 Maneuver	647				0	492
Mov Cap-1 Maneuver	041				0	752
Stage 1					0	
Stage 2					325	
Stage 2	-			_	323	_
Approach	EB		WB		SB	
HCM Control Delay, s	5.4		0		13.2	
HCM LOS					В	
Minor Lane/Major Mvmt		EBL	EBT	WBT	WBR	SBLn1
		647		-		492
		0.104		-		0.111
Capacity (veh/h)						
Capacity (veh/h) HCM Lane V/C Ratio			5.2			
Capacity (veh/h)		11.2 B	5.2 A		- 1	13.2 B

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	٦	44	7	1	†î»		14	†î>		ኘኘ	†î₃		
Traffic Volume (veh/h)	100	500	290	137	400	100	330	444	126	117	360	71	
Future Volume (veh/h)	100	500	290	137	400	100	330	444	126	117	360	71	
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0	
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Work Zone On Approac		No			No			No			No		
Adj Sat Flow, veh/h/ln	1856	1856	1856	1856	1856	1856	1856	1856	1856	1856	1856	1856	
Adj Flow Rate, veh/h	143	714	0	196	571	0	471	634	0	167	514	0	
Peak Hour Factor	0.91		0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	
Percent Heavy Veh, %	3	3	3	3	3	3	3	3	3	3	3	3	
Cap, veh/h	434	1366		393	1433		537	909		232	596		
Arrive On Green	0.07	0.39	0.00	0.09	0.41	0.00	0.16	0.26	0.00	0.07	0.17	0.00	
Sat Flow, veh/h	1767		1572	1767	3618	0	3428	3618	0	3428	3618	0	
Grp Volume(v), veh/h	143	714	0	196	571	0	471	634	0	167	514	0	
Grp Sat Flow(s),veh/h/li		1763	1572	1767	1763	0	1714	1763	0	1714	1763	0	
Q Serve(g_s), s	4.8	15.6	0.0	6.6	11.5	0.0	13.4	16.3	0.0	4.8	14.2	0.0	
Cycle Q Clear(g_c), s	4.8	15.6	0.0	6.6	11.5	0.0	13.4	16.3	0.0	4.8	14.2	0.0	
Prop In Lane	1.00	1000	1.00	1.00		0.00	1.00	000	0.00	1.00	=00	0.00	
Lane Grp Cap(c), veh/h				393	1433		537	909		232	596		
V/C Ratio(X)	0.33	0.52		0.50	0.40		0.88	0.70		0.72	0.86		
Avail Cap(c_a), veh/h	589			551	1433		703	955		497	744		
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Upstream Filter(I)	1.00		0.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00 45.7	1.00	0.00	
Uniform Delay (d), s/vel Incr Delay (d2), s/veh		23.5	0.0	17.2	21.0	0.0	8.2	1.7	0.0	1.6	7.3	0.0	
Initial Q Delay(d3), s/veh	0.2	0.0	0.0	0.4	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
%ile BackOfQ(50%),vel		6.4	0.0	2.5	4.6	0.0	6.2	7.1	0.0	2.1	6.7	0.0	
Unsig. Movement Delay			0.0	2.5	4.0	0.0	0.2	7.1	0.0	2.1	0.7	0.0	
LnGrp Delay(d),s/veh		25.0	0.0	17.6	21.9	0.0	49.4	35.3	0.0	47.2	47.7	0.0	
LnGrp LOS	B	C	0.0	17.0 B	C C	0.0	TJ.4	D.0	0.0	T1.2	D.	0.0	
Approach Vol, veh/h		857	Α	ь	767	А		1105	Α		681	Α	
Approach Delay, s/veh		23.6			20.8			41.3			47.6	Α.	
Approach LOS		23.0 C			20.0			41.3			47.0		
Timer - Assigned Phs	1	2	3	4	5	6	7	8					
Phs Duration (G+Y+Rc)		45.8	11.3	31.7	13.1	44.0	20.2	22.8					
Change Period (Y+Rc),		* 5.2	4.5	* 5.9	4.5	* 5.2	4.5	* 5.9					
Max Green Setting (Gm		* 23	14.5	* 27	17.5	* 21	20.5	* 21					
Max Q Clear Time (g_c		13.5	6.8	18.3	8.6	17.6	15.4	16.2					
Green Ext Time (p_c), s	0.0	1.0	0.1	1.3	0.1	0.7	0.2	0.7					
Intersection Summary													
HCM 6th Ctrl Delay			33.5										
HCM 6th LOS			С										
			_										
Notes				the second									
User approved pedestri										h - h -			
* HCM 6th computation												interes :	tion delete
Unsignalized Delay for	INRK, I	EBK, W	DK, SB	rk] IS 63	kuluaed	11 Om Ca	aculatio	ris of th	ie appro	Jacn de	iay and	intersec	uon uėlay.

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	٠	-	•	•	•	•	4	†	1	-	↓	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SE
Lane Configurations	75	Ŷβ		7	† 124			4			413	
Traffic Volume (vph)	9	1450	1	1	514	1	2	5	1	5	5	
Future Volume (vph)	9	1450	1	1	514	1	2	5	1	5	5	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	19
Total Lost time (s)	4.4	5.8		4.4	5.0			5.8			5.8	
Lane Util. Factor	1.00	0.95		1.00	0.95			1.00			0.95	
Frt	1.00	1.00		1.00	1.00			0.99			0.94	
Flt Protected	0.95	1.00		0.95	1.00			0.99			0.99	
Satd. Flow (prot)	1719	3438		1719	3437			1763			3177	
FIt Permitted	0.34	1.00		0.06	1.00			0.90			0.87	
Satd. Flow (perm)	616	3438		104	3437			1606			2808	
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.
Growth Factor (vph)	130%	130%	130%	130%	130%	130%	130%	130%	130%	130%	130%	130
Adj. Flow (vph)	13	2071	1	1	734	1	3	7	1	7	7	100
RTOR Reduction (vph)	0	0	0	0	0	0	0	1	0	0	9	
Lane Group Flow (vph)	13	2072	0	1	735	0	0	10	0	0	15	
Heavy Vehicles (%)	5%	5%	5%	5%	5%	5%	5%	5%	5%	5%	5%	
Turn Type	pm+pt	NA.	370	pm+pt	NA.	370	Perm	NA.	370	Perm	NA.	
Protected Phases	1	6 26		5	2 22		1 Gilli	4 24		1 Gilli	8 28	
Permitted Phases	6 26	0 20		2 22	2 22		4 24	4 24		8 28	0 20	
Actuated Green, G (s)	104.6	104.6		105.9	105.3		4 24	7.2		0 20	7.2	
Effective Green, g (s)	104.6	104.6		105.9	105.3			7.2			7.2	
Actuated g/C Ratio	0.75	0.75		0.76	0.75			0.05			0.05	
Clearance Time (s)	4.4	0.73		4.4	0.75			0.03			0.03	
Vehicle Extension (s)	1.0			0.2								
	472	2568		85	2585			82			144	
Lane Grp Cap (vph) v/s Ratio Prot					0.21			82			144	
	0.00	c0.60		c0.00	0.21							
v/s Ratio Perm	0.02			0.01				c0.01			0.01	
v/c Ratio	0.03	0.81		0.01	0.28			0.12			0.10	
Uniform Delay, d1	7.1	11.3		21.2	5.5			63.4			63.3	
Progression Factor	1.09	2.62		1.12	1.13			1.00			1.00	
Incremental Delay, d2	0.0	0.2		0.0	0.0			0.2			0.1	
Delay (s)	7.8	29.7		23.7	6.2			63.6			63.4	
Level of Service	Α	С		С	Α			Е			Е	
Approach Delay (s)		29.5			6.2			63.6			63.4	
Approach LOS		С			Α			Е			Е	
Intersection Summary												
HCM 2000 Control Delay			24.0	H	CM 2000	Level of S	Service		С			
HCM 2000 Volume to Capac	ity ratio		0.76									
Actuated Cycle Length (s)			140.0	S	um of lost	time (s)			27.6			
Intersection Capacity Utilizat	ion		66.8%	IC	U Level o	of Service			С			
Analysis Period (min)			15									
Critical Lane Group												

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28T EBR 0 80 10 80 10 80 10 80 10 80 10 80 10 80 10 80 10 91 11 91 11 114	WBL 63 63 0 Free - 160 - - 91 3	WBT 638 638 0 Free None	NBL 0 0 0 Stop	NBR 82 82 0 Stop
T EBR 0 80 0 80 0 0 0 0 0 0 0 Free - None 0 0 - 0 0 1 91 3 3	63 63 0 Free - 160 - - 91 3	638 638 0 Free None	0 0 0 Stop	82 82 82 0
0 80 0 80 0 0 0 ee Free - None - 0 - 0 - 0 1 91 3 3	63 63 0 Free - 160 - - 91 3	638 638 0 Free None	0 0 0 Stop	82 82 82 0
0 80 0 0 0 ee Free - None 0 - 0 - 01 91 3 3	63 63 0 Free - 160 - - 91 3	638 638 0 Free None	0 0 Stop	82 82 0
0 80 0 0 0 ee Free - None 0 - 0 - 01 91 3 3	63 0 Free - 160 - - 91 3	638 0 Free None	0 0 Stop	82
0 0 ee Free - None 0 0 - 01 91 3 3	0 Free - 160 - - 91 3	0 Free None - 0	0 Stop	0
ee Free - None 0 0	Free - 160 - - 91 3	Free None -	Stop -	
- None 0 - 0 - 01 91 3 3	160 - - 91 3	None - 0	-	
0 - 0 - 91 91 3 3	160 - - 91 3	- 0		
0 - 0 - 91 91 3 3	91	0		Stop
0 - 91 91 3 3	91 3		0	0
91 91 3 3	91 3		0	- :
3 3	3	91	91	91
1 114		911	3	117
	30	911	U	117
r1 l	Major2	, N	Minor1	
0 0	985	0	-	493
	4.16		-	6.96
	-		-	
	2.23			3.33
	691		0	519
	-		0	
	-		0	-
		-		
	691		-	519
	-	-	-	-
	-		-	-
	-	-	-	-
R	WR		NR	
U	- 1			
			В	
NBLn1	EBT	EBR	WBL	WBT
519	-	-	691	-
0.226	-	-	0.13	-
13.9	-		11	-
В	-	-	0.4	-
0.9				
	NBLn1 519 0.226 13.9 B	- 4.16 2.23 691 691 691 691 10 1	- 4.16	- 4.16

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Intersection						
Int Delay, s/veh	1.2					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		414	† 1>		¥	
Traffic Vol, veh/h	40	650	670	15	16	23
Future Vol, veh/h	40	650	670	15	16	23
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized		None		None	- 0	None
Storage Length Veh in Median Storage,		0	0	- 1	0	- 1
Grade %	* :	0	0	- 1	0	- 1
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	3	3	3	3	3	3
Mvmt Flow	58	939	968	22	23	33
Major/Minor N	lajor1		Major2		Minor2	
Conflicting Flow All	990	0		0	1565	495
Stage 1				-	979	
Stage 2	-		-	-	586	
Critical Hdwy	4.16		-		6.86	6.96
Critical Hdwy Stg 1	-		-	-	5.86	-
Critical Hdwy Stg 2	- 0.00				5.86	3.33
Follow-up Hdwy	2.23 688	-	- 1		3.53	517
		-			322	517
Pot Cap-1 Maneuver	000	_				
Pot Cap-1 Maneuver Stage 1			- 1		517	
Pot Cap-1 Maneuver				- :	517	
Pot Cap-1 Maneuver Stage 1 Stage 2			-		517	517
Pot Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, %		i	- :		83 193	
Pot Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1	688	-	:		83 193 265	517
Pot Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver	688	-		:	83 193	517
Pot Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1	688			:	83 193 265	517 - -
Pot Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1	688			:	83 193 265	517 - -
Pot Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2 Approach HCM Control Delay, s	688		-	:	83 193 265 517 SB 19.4	517 - -
Pot Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2	688 -		- - - - - WB	:	83 193 265 517	517 - -
Pot Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2 Approach HCM Control Delay, s	688 -		- - - - - WB	:	83 193 265 517 SB 19.4	517 - -
Pot Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS	688 - - - EB 1.4		- - - - - WB	:	83 193 265 517 SB 19.4	517
Pot Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2 Approach HCM Control Delay, s	688 - - - EB 1.4		- - - - - - WB		83 193 265 517 SB 19.4 C	517
Pot Cap-1 Maneuver Stage 1 Stage 2 Platon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvml	688 - - - EB 1.4	EBL 688 0.084			83 193 265 517 SB 19.4 C	517 - - - - SBLn1 306 0.184
PRIC Cap-1 Meneuver Stage 1 Stage 2 Platon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvml Capacity (vehh) HCM LON VC Ratio	688 - - - EB 1.4	EBL 688 0.084		WBT	83 193 265 517 SB 19.4 C	517 - - - SBLn1 306 0.184 19.4
Pot Cap-1 Maneuver Slage 1 Slage 2 Platon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Slage 1 Slage 2 Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvml Capacity (wehth)	688 - - - EB 1.4	EBL 688 0.084		WBT	83 193 265 517 SB 19.4 C	517 - - - - SBLn1 306 0.184 19.4

1.5 BT EB 100 100 100 100 100 100 100 1	7 17 7 17 0 0 ee Free	WBT ♣↑ 555 555	NBL 23 23	NBR 58
700 700 0 0 ree Fre	7 17 7 17 0 0 ee Free	4 ↑ 555 555	1 7	
700 700 0 ree Fre - Nor	7 17 7 17 0 0 ee Free	4 ↑ 555 555	1 7	
700 700 0 ree Fre - Nor	7 17 7 17 0 0 ee Free	4 ↑ 555 555	1 7	
700 700 0 ree Fre - Nor	7 17 0 0 e Free	555 555	23	58
00 0 ree Fre - Nor	0 0 e Free	555	00	
0 ree Fre - Nor	0 0 e Free			58
- Nor		0	0	0
- Nor		Free	Stop	Stop
	ne -		-	None
0			0	
		0	0	
0		0	0	
	37 87	87	87	87
1	1 1	1	1	1
		829	34	87
				-
or1	Major2	ħ	Minor1	
0			1516	528
-	- 1000	-		320
				6.92
	- 4.12			0.52
	- 2.21			3.31
				498
		-	602	-
	- 661		103	498
			220	-
			300	
			560	
EB	WB		NB	
0	0.7		19.6	
			С	
NBLr	1 EBT	EBR	WBL	WBT
			661	
19	.6 -		10.7	0.4
	C -		0.1	Α
	NBLT 360 330 330 330 330 330 330 330 330 330	446 10 25 crt Major2 0 0 1056 4.12 661 661 661 661 7 - 7 - 888 W8 0 0 0.7	A46 10 25 829 ort Major2 1 0 0 1056 0 4.12	A46 10 25 829 34 ort Major2 Minor1 0 0 1056 0 1516 1051 4.12 6.822 5.82 2.21 3.51 - 661 - 111 602 601 - 20 601 602 600 - 600

0.6					
EBT	EBR	WBL	WBT	NBL	NBR
† ℃				N/	
808	9	3	507	22	6
808	9	3	507	22	6
0	0	0	0	0	0
Free	Free	Free	Free		Stop
				-	None
				0	
# 0					
	-	-			-
91	91	91		91	91
2	2	2	2	2	2
					9
					_
Major1		Major?	,	dinor1	
					584
					304
		4.14		6.84	6.94
					0.01
		2.22			3.32
					455
					.00
				669	
		594		107	455
					.00
				002	
		14100			
0		0.2			
				С	
ıt l		EBT	EBR	WBL	WBT
	234			594	
		-	-		-
	23.5			11.1	0.1
	С	-	-	В	Α
				0	
	**************************************	EBT EBR S08 9 808 9 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	BBT BBR WBL The property WBL WBL The property WBL WBL The property WBL W	EBT EBR WBL WBT	EBT EBR WBL WBT NBL ↑↑ ↑ ↑ ↑ 808 9 3 507 22 808 9 3 507 22 808 9 3 507 22 808 9 - None - N

ntersection						
nt Delay, s/veh	0.5					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
ane Configurations	† \$	LUIT	*****	414	W	HUIT
Fraffic Vol. veh/h	776	- 1	2	405	14	15
uture Vol. veh/h	776	1	2	405	14	15
Conflicting Peds. #/hr	0	0	0	0	0	.0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized		None		None	-	None
Storage Length		-		-	0	-
eh in Median Storage.	,# 0			0	0	
Grade, %	0		-	0	0	
Peak Hour Factor	87	87	87	87	87	87
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	1160	1	3	605	21	22
					-	
Major/Minor N	Major1		Major2		Minor1	
Conflicting Flow All	viajor i		1161	0	1470	581
Stage 1		-	1101	-	1161	301
Stage 2			- 1	- :	309	
Critical Hdwv	_	_	4.14	- :	6.84	6.94
Critical Hdwy Stg 1	- 1	- :	4.14	- :	5.84	0.94
Critical Hdwy Stg 2	- 1	_	- 1	- 1	5.84	- 1
Follow-up Hdwy	- 1	- 1	2.22	- 1	3.52	3.32
Pot Cap-1 Maneuver	_	- :	597	- :	118	457
Stage 1	- 1	- :	597	- :	260	437
Stage 2	_	_		_	718	_
Platoon blocked. %	- 1	- 1		- 1	710	
Mov Cap-1 Maneuver	_	_	597	_	117	457
Mov Cap-1 Maneuver	- 1	- 1	597	- :	212	401
Stage 1	- 1	- 1	- 1	- 1	260	- 1
Stage 2	- 1	- 1	- 1	- 1	712	- 1
Stage 2	-				/12	
\	EB		WB		NB	
Approach HCM Control Delay, s			0.1		19.4	
HCM LOS	U		0.1		19.4 C	
TOM LOS					U	
Frank and Malanta		IDI4	COT	500	MIDI	MOT
Minor Lane/Major Mvm	L P	VBLn1 293	EBT	EBR	WBL 597	WBT
Capacity (veh/h) HCM Lane V/C Ratio		0.148			0.005	- 1
HCM Lane V/C Ratio HCM Control Delay (s)		19.4	- 1	- 1	11.1	0
HCM Control Delay (s)		19.4 C		- 1	11.1 B	A
		0.5		- 1	0	Α -
HCM 95th %tile Q(veh)						

	•	→	•	•	•	•	4	1	-	-	Ţ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SE
ane Configurations	*	† 15		76	ተተ			4			4	
raffic Volume (vph)	4	669	4	7	489	0	2	2	8	18	0	
uture Volume (vph)	4	669	4	7	489	0	2	2	8	18	0	
deal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	19
otal Lost time (s)	5.1	5.1		5.1	5.1			5.2			5.2	
ane Util. Factor	1.00	0.95		1.00	0.95			1.00			1.00	
rt	1.00	1.00		1.00	1.00			0.91			0.97	
It Protected	0.95	1.00		0.95	1.00			0.99			0.96	
Satd. Flow (prot)	1770	3536		1770	3539			1681			1734	
It Permitted	0.37	1.00		0.27	1.00			0.94			0.77	
Satd. Flow (perm)	692	3536		508	3539			1592			1385	
Peak-hour factor, PHF	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.
Growth Factor (vph)	130%	130%	130%	130%	130%	130%	130%	130%	130%	130%	130%	130
Adj. Flow (vph)	6	977	6	10	714	0	3	3	12	26	0	
RTOR Reduction (vph)	0	0	0	0	0	0	0	11	0	0	33	
ane Group Flow (vph)	6	983	0	10	714	0	0	7	0	0	2	
leavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2 22			6 26			4 24			8 28	
Permitted Phases	2 22			6 26			4 24			8 28		
Actuated Green, G (s)	92.5	92.5		92.5	92.5			7.1			7.1	
Effective Green, g (s)	92.5	92.5		92.5	92.5			7.1			7.1	
Actuated g/C Ratio	0.77	0.77		0.77	0.77			0.06			0.06	
Clearance Time (s)												
/ehicle Extension (s)												
ane Grp Cap (vph)	533	2725		391	2727			94			81	
/s Ratio Prot		c0.28			0.20							
/s Ratio Perm	0.01			0.02				c0.00			0.00	
/c Ratio	0.01	0.36		0.03	0.26			0.07			0.03	
Jniform Delay, d1	3.2	4.4		3.2	3.9			53.3			53.2	
Progression Factor	1.32	1.43		1.04	0.88			1.00			1.00	
ncremental Delay, d2	0.0	0.1		0.0	0.1			0.1			0.0	
Delay (s)	4.2	6.3		3.4	3.5			53.5			53.2	
evel of Service	Α	Α		Α	Α			D			D	
Approach Delay (s)		6.3			3.5			53.5			53.2	
Approach LOS		Α			Α			D			D	
ntersection Summary												
HCM 2000 Control Delay			6.6	Н	CM 2000	I evel of	Service		A			
CM 2000 Volume to Capaci	ty ratio		0.34		OIII 2000	2010101	0011100					
Actuated Cycle Length (s)	ty ratio		120.0	S	um of lost	time (s)			20.4			
ntersection Capacity Utilizati	on		38.7%			of Service			A			
Analysis Period (min)	•		15									
: Critical Lane Group												

,	٠	-	*	1	•	•	1	†	1	/	ļ	4	
Movement E	BL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	٦	†		*	†		7	† 1>		7	† ⊅		
	09	700	52	27	406	88	52	253	41	81	160	91	
Future Volume (veh/h) 1	09	700	52	27	406	88	52	253	41	81	160	91	
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0	
	.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00	
	.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Work Zone On Approach		No	4070	4000	No	1000		No		1070	No	4000	
	370	1870 1000	1870 74	1870	1870 580	1870 126	1870 74	1870 361	1870 59	1870	1870 229	1870 130	
	56 .91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	116 0.91	0.91	0.91	
Percent Heavy Veh, %	2	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	
	111	2135	158	104	657	142	149	426	69	142	336	184	
	.14	0.21	0.21	0.01	0.07	0.07	0.05	0.14	0.14	0.04	0.15	0.15	
	81	3354	248	1781	2905	629	1781	3062	496	1781	2217	1212	
	56	530	544	39	354	352	74	208	212	116	182	177	
Grp Sat Flow(s), veh/h/ln17		1777	1826	1781	1777	1757	1781	1777	1781	1781	1777	1652	
	4.6	31.3	31.4	2.1	23.7	23.8	4.5	13.7	13.9	3.2	11.6	12.2	
	4.6	31.3	31.4	2.1	23.7	23.8	4.5	13.7	13.9	3.2	11.6	12.2	
	.00	01.0	0.14	1.00	20.1	0.36	1.00	10.1	0.28	1.00	11.0	0.73	
	311	1131	1162	104	402	397	149	247	248	142	269	250	
	.19	0.47	0.47	0.37	0.88	0.89	0.50	0.84	0.86	0.81	0.68	0.71	
	311	1131	1162	268	752	744	208	410	411	214	410	381	
HCM Platoon Ratio 0.	.33	0.33	0.33	0.33	0.33	0.33	1.00	1.00	1.00	1.00	1.00	1.00	
	.90	0.90	0.90	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Uniform Delay (d), s/veh 3	1.1	29.6	29.6	40.5	53.9	54.0	47.7	50.4	50.5	55.6	48.1	48.4	
	0.0	1.3	1.2	0.8	23.3	24.0	1.0	3.5	4.4	7.9	1.1	1.4	
Initial Q Delay(d3),s/veh (0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
%ile BackOfQ(50%),veh/lri		15.2	15.6	0.9	13.9	13.8	2.0	6.3	6.5	3.8	5.2	5.1	
Unsig. Movement Delay, s.						ma a		=0.0	=		49.2	10.0	
	1.2	30.9	30.8	41.3	77.2	78.0	48.6	53.8	54.8	63.4		49.8	
LnGrp LOS	С	C	С	D	E 745	Е	D	D 104	D	Е	D	D	
Approach Vol, veh/h		1230			745			494			475		
Approach Delay, s/veh Approach LOS		30.9 C			75.7 E			53.5 D			52.9 D		
••											U		
Timer - Assigned Phs	1	2	3	4	5	6	7	8					
Phs Duration (G+Y+Rc), 5		32.3	10.5	22.0	6.0	81.6	9.0	23.5					
Change Period (Y+Rc), \$ 5		* 5.2	5.3	* 5.3	3.0	* 5.2	3.0	5.3					
Max Green Setting (Gmax)		* 51	10.0	* 28	14.0	* 52	10.0	27.7					
Max Q Clear Time (g_c+l1		25.8	5.2	15.9	4.1	33.4	6.5	14.2					
Green Ext Time (p_c), s	0.0	1.3	0.0	0.7	0.0	2.1	0.0	1.2					
Intersection Summary													
HCM 6th Ctrl Delay			49.6										
HCM 6th LOS			D										
Notes													
* HCM 6th computational e	engir	ne requi	ires equ	ıal clea	rance ti	mes for	the ph	ases cri	ossing t	he barr	ier.		
Culebra Arterial Study 7:15		10/15/	2020 2	20 444	Culatia	- 0	ision.						Synchro 10 Rep

	٠	-	•	1	•	•	4	Ť	1	1	Ţ	4	
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
ane Configurations	7	†		1	11			4			4		
raffic Volume (veh/h)	2	791	29	32	489	4	7	- 1	21	2	5	2	
uture Volume (veh/h)	2	791	29	32	489	4	7	1	21	2	5	2	
nitial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0	
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Vork Zone On Approac	h	No			No			No			No		
ldj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	
ldj Flow Rate, veh/h	3	1143	42	46	706	6	10	1	30	3	7	3	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2	
Cap, veh/h	666	2853	105	481	3009	26	49	6	48	47	47	17	
Arrive On Green	0.00	1.00	1.00	0.04	1.00	1.00	0.04	0.04	0.04	0.04	0.04	0.04	
Sat Flow, veh/h	1781	3496	128	1781	3611	31	282	139	1149	232	1118	405	
Grp Volume(v), veh/h	3	581	604	46	347	365	41	0	0	13	0	0	
Grp Sat Flow(s), veh/h/lr	1781	1777	1847	1781	1777	1865	1570	0	0	1754	0	0	
Serve(g_s), s	0.0	0.0	0.0	0.5	0.0	0.0	1.8	0.0	0.0	0.0	0.0	0.0	
Cycle Q Clear(g_c), s	0.0	0.0	0.0	0.5	0.0	0.0	3.0	0.0	0.0	0.8	0.0	0.0	
Prop In Lane	1.00		0.07	1.00		0.02	0.24		0.73	0.23		0.23	
ane Grp Cap(c), veh/h	666	1450	1508	481	1481	1554	103	0	0	110	0	0	
//C Ratio(X)	0.00	0.40	0.40	0.10	0.23	0.23	0.40	0.00	0.00	0.12	0.00	0.00	
Avail Cap(c_a), veh/h	820	1450	1508	603	1481	1554	280	0	0	301	0	0	
ICM Platoon Ratio	2.00	2.00	2.00	2.00	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00	
Jpstream Filter(I)	1.00	1.00	1.00	0.92	0.92	0.92	1.00	0.00	0.00	1.00	0.00	0.00	
Jniform Delay (d), s/veh		0.0	0.0	1.6	0.0	0.0	56.5	0.0	0.0	55.5	0.0	0.0	
ncr Delay (d2), s/veh	0.0	0.8	0.8	0.0	0.3	0.3	0.9	0.0	0.0	0.2	0.0	0.0	
nitial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
6ile BackOfQ(50%),veh		0.3	0.3	0.1	0.1	0.1	1.2	0.0	0.0	0.4	0.0	0.0	
Jnsig. Movement Delay													
.nGrp Delay(d),s/veh	2.0	0.8	0.8	1.7	0.3	0.3	57.5	0.0	0.0	55.7	0.0	0.0	
nGrp LOS	A	A	A	A	A	A	E	A	A	E	A	A	
Approach Vol, veh/h		1188			758			41			13		
Approach Delay, s/veh		0.8			0.4			57.5			55.7		
Approach LOS		Α			Α			Е			Е		
imer - Assigned Phs	- 1	2		4	5	6		8					
hs Duration (G+Y+Rc)	c4.7	105.1		10.2	6.8	103.0		10.2					
		*5.1		* 5.2	* 4.4	* 5.1		* 5.2					
Change Period (Y+Rc), Max Green Setting (Gm		* 76		* 19	* 11	* 76		* 19					
Max Green Setting (Gm Max Q Clear Time (g_c		2.0		5.0	2.5	2.0		2.8					
Green Ext Time (p_c), s		1.3		0.0	0.0	2.4		0.0					
0 - 7-	0.0	1.0		0.0	0.0	2.7		0.0					
ntersection Summary HCM 6th Ctrl Delay			22										
HCM 6th LOS			2.2 A										
Votes													
Jser approved pedestri HCM 6th computation								ases cr	ossing :	the barr	ier.		

-	~ `	¥	•	←	4	1	
Movement E	BT E	BR 1	WBL	WBT	NBL	NBR	
Lane Configurations	13		7	44	N/		
Traffic Volume (veh/h) 7	38	37	23	435	39	14	
uture Volume (veh/h) 7	38	37	23	435	39	14	
nitial Q (Qb), veh	0	0	0	0	0	0	
Ped-Bike Adj(A_pbT)	- 1	.00	1.00		1.00	1.00	
Parking Bus, Adj 1.	.00 1	.00	1.00	1.00	1.00	1.00	
Nork Zone On Approach	No			No	No		
Adj Sat Flow, veh/h/ln 18			1870	1870	1870	1870	
	78	54	34	635	57	20	
			0.89	0.89		0.89	
Percent Heavy Veh, %	2	2	2	2	2	2	
		132	406	2975	97	34	
			0.07	1.00	0.08	0.08	
			1781			443	
		576	34	635	78	0	
Grp Sat Flow(s),veh/h/ln17		839 1		1777	1728	0	
		3.1	0.4	0.0	5.2	0.0	
		3.1	0.4	0.0	5.2	0.0	
Prop In Lane			1.00		0.73	0.26	
Lane Grp Cap(c), veh/h 13				2975	133	0	
			0.08	0.21	0.59	0.00	
	162 14		547	2975	386	0	
			2.00	2.00	1.00	1.00	
			0.94	0.94	1.00	0.00	
Uniform Delay (d), s/veh 12		2.4	4.7	0.0	53.5	0.0	
		8.0	0.0	0.2	1.5	0.0	
Initial Q Delay(d3),s/veh (0.0	0.0	0.0	0.0	0.0	
%ile BackOfQ(50%),veh/lff		0.5	0.1	0.1	2.3	0.0	
Unsig. Movement Delay, s.							
LnGrp Delay(d),s/veh 13		3.2	4.7	0.2	55.0	0.0	
LnGrp LOS	В	В	A	A	E	A	
	32			669	78		
	3.3			0.4	55.0		
Approach LOS	В			Α	Е		
Timer - Assigned Phs		2		4	5	6	
Phs Duration (G+Y+Rc), s	10	15.5		14.5	8.5	97.1	
Change Period (Y+Rc), s		5.1		* 5.2	* 4.4	* 5.1	
Max Green Setting (Gmax), s *	83		* 27	* 14	* 65	
Max Q Clear Time (g_c+l1		2.0		7.2	2.4	25.1	
Green Ext Time (p_c), s		2.1		0.1	0.0	3.5	
u = 7:							
Intersection Summary			10.4				
HCM 6th Ctrl Delay							
HCM 6th LOS			В				
Notes							
* HCM 6th computational e	engine i	require	es equ	ual clear	rance ti	mes fo	r the phases crossing the barrier.
							•
	_	_	_				

Movement Sell EBT EBR WBL WBT WBR NBL NBT NBR SBL SBT SBR		٠	→	*	1	←	*	4	†	1	/	Ţ	4	
Traffic Volume (wehlh) 54 597 121 95 314 27 122 371 158 28 204 21	Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Traffic Volume (wehh) 54 597 121 95 314 27 122 371 158 28 204 21 withure Volume (wehh) 54 597 121 95 314 27 122 371 158 28 204 21 millad (Qlob), weh 100 100 100 100 100 100 100 100 100 10	Lane Configurations	75	♠ ₽		75	Αħ		7	44	7	7	Αt		
Future Volume (wehh) 64 597 121 95 314 27 122 371 158 28 204 21 initials (20b), well and (20b) w		54		121	95		27						21	
Peach Black AdjiA, pb.17 1.00	Future Volume (veh/h)	54	597	121	95	314	27	122	371	158	28	204	21	
Peed-Bike AdjiA, pbT) 1 00	Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0	
Parking Bus, Anj 100 1,00 1,00 1,00 1,00 1,00 1,00 1,00 1,00 1,00 1,00 1,00 Non X		1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00	
Adj. Sat Flow, veshintin 1870 1870 1870 1870 1856		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Add Staf Flow, vehinhin 1870 1870 1865 1856		h	No			No			No			No		
Adj Flow Rate, wehin 79 672 177 139 459 39 178 542 231 41 298 31 Peakeh Hour Factor 0.89 0.89 0.89 0.89 0.89 0.89 0.89 0.89			1870	1870	1856	1856	1856	1856	1856	1856	1856	1856	1856	
Peak Hour Factor		79	872	177		459	39	178	542	231	41	298	31	
Percent Heavy Veh, % 2 2 2 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3		0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	
Cap, wehh 459 1282 260 247 1503 127 395 1110 489 236 788 82 Armive On Green 0.01 0.14 0.10 0.64 0.40 0.31 0.31 0.31 0.31 0.32 0.25 25 Sal Flow, wehth 78 25 55.3 187 78 3280 178 7826 1872 176 3293 333 333 333 338 52 10.5 186 25 137 182 52 318 182 185 185 1767 1783 1876 1783 1872 1787 1893 1192 278 178 1783 1805 1787 1783 1875 1783 1805 1787 1783 1875 1783 1805 1787 1783 1875 1783 1805 1805 1787 1783 1805 1805 1805 1805 1805 1805 1805 1803		2	2	2	3	3	3	3	3	3	3	3	3	
Arrive On Green 0.01 0.14 0.14 0.06 0.46 0.46 0.09 0.31 0.31 0.03 0.25 0.25 SET Now, vehih 178 12943 597 1767 3290 279 1767 3526 1572 1767 3225 333 Cip Volume(y), vehih 178 12943 597 1767 329 279 1767 3526 1572 1767 3225 333 Cip Volume(y), vehih 178 1763 1767 1763 1767 1763 1767 1763 1767 1763 1767 1763 1767 1763 1767 1763 1767 1763 1767 1763 1767 1763 1767 1763 1767 1763 1767 1763 1769 1769 126 126 126 126 126 126 126 126 126 126		459		260	247	1503	127	395	1110	495	236	798	82	
Sign Sign		0.01	0.14					0.09		0.31			0.25	
Sign Sign													333	
Sgr Sel Flow(s), webhint 78 1 777 783 1767 1763 1805 1767 1763 1767 1763 1796 1796 1796 1796 1796 1796 1796 1796														
2 Senet(g, s), s														
Cycle o Ciserio (c.)s. 2 9 33.7 33.8 52.2 10.5 10.6 8.7 14.9 14.2 2.1 9.1 9.3 Prop in Lane 100 104 10.0 10.5 10.0 10.9 10.0 10.9 10.0 10.9 10.0 10.9 10.0 10.9 10.0 10.9 10.0 10.9 10.0 10.9 10.0 10.9 10.0 10.9 10.0 10.9 10.0 10.9 10.0 10.9 10.0 10.9 10.0 10.9 10.0 10.9 10.0 10.9 10.0 10.0 10.9 10.0														
Prop In Lane 67 Cap(c), which 459 774 788 247 805 825 305 1110 495 236 436 444 (CRatic)() 0.17 0.68 0.68 0.56 0.30 0.31 0.45 0.49 0.47 0.17 0.37 0.38 (Avail Cap(c.g.), which 549 774 788 328 805 825 305 1110 495 236 436 444 (CRatic)() 0.17 0.68 0.68 0.56 0.30 0.31 0.45 0.49 0.47 0.17 0.37 0.38 (Avail Cap(c.g.), which 549 774 788 328 805 825 506 1110 495 348 436 444 (CRatic)() 0.17 0.33 0.33 0.33 0.33 0.30 1.00 1.00 1.00														
Lame Cgr Capic), with 1459			55.1			10.0			17.0			J. I		
\(\text{VIC Ratio(X)} \) 0.17 \ 0.68 \ 0.68 \ 0.56 \ 0.30 \ 0.31 \ 0.45 \ 0.49 \ 0.47 \ 0.17 \ 0.37 \ 0.38 \ \\ value(Taple, g), webh 549 \ 774 \ 78 \ 83 \ 28 \ 85 \ 25 \ 56 \ 17 \ 83 \ 28 \ 85 \ 25 \ 85 \ 313 \ 320 \ 33 \ 0.03 \ 30.33 \ 0.03 \ 1.00 \ 1.			77/			805			1110			436		
Avail Cap(-i, a), wehit 549 774 768 288 805 825 506 1110 495 348 436 444 444 144 144 144 144 144 144 144 14														
HCM Platon Ratio														
Upstream Filter(1)														
Uniform Delay (d), a vien 18 2 43.4 43.4 23.0 20.6 20.6 28.5 33.3 33.0 32.7 37.4 37.5 Incro Delay (d2), a vien 0 1 4.4 4.5 0.7 1.0 1.0 0.3 0.1 0.3 0.1 2.4 2.4 Initial O Delay (d3), a vien 10 1.4 4.5 0.7 1.0 1.0 0.3 0.1 0.3 0.1 2.4 2.4 Initial O Delay (d3), a vien 10 1.7 1.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Initial O Delay (d3), a vien 10 1.7 1.0 0.3 0.1 2.4 2.4 Initial O Delay (d3), a vien 10 1.7 1.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Initial O Delay (d3), a vien 10 1.7 1.0 0.0 0.0 0.0 0.0 0.0 0.0 Initial O Delay (d3), a vien 10 1.0 0.0 0.0 0.0 0.0 0.0 0.0 Initial O Delay (d3), a vien 10 1.0 0.0 0.0 0.0 0.0 0.0 Initial O Delay (d3), a vien 10 1.0 0.0 0.0 0.0 0.0 Initial O Delay (d3), a vien 10 1.0 0.0 0.0 0.0 Initial O Delay (d3), a vien 10 1.0 0.0 0.0 0.0 Initial O Delay (d3), a vien 10 0.0 0.0 0.0 Initial O Delay (d3), a vien 10 0.0 0.0 0.0 Initial O Delay (d3), a vien 10 0.0 0.0 0.0 Initial O Delay (d3), a vien 10 0.0 0.0 0.0 Initial O Delay (d3), a vien 10 0.0 0.0 0.0 Initial O Delay (d3), a vien 10 0.0 0.0 0.0 Initial O Delay (d3), a vien 10 0.0 0.0 0.0 Initial O Delay (d3), a vien 10 0.0 0.0 0.0 Initial O Delay (d3), a vien 10 0.0 0.0 0.0 Initial O Delay (d3), a vien 10 0.0 0.0 0.0 Initial O Delay (d3), a vien 10 0.0 0.0 0.0 Initial O Delay (d3), a vien 10 0.0 0.0 0.0 Initial O Delay (d3), a vien 10 0.0 0.0 0.0 Initial O Delay (d3), a vien 10 0.0 0.0 0.0 Initial O Delay (d3), a vien 10 0.0 0.0 0.0 0.0 Initial O Delay (d3), a vien 10 0.0 0.0 0.0 0.0 Initial O Delay (d3), a vien 10 0.0 0.0 0.0 0.0 0.0 Initial O Delay (d3), a vien 10 0.0 0.0 0.0 0.0 0.0 Initial O Delay (d3), a vien 10 0.0														
nor Delay (d2), siveh														
Initial C Delay(d3) Siveh 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0														
Algorithms														
Unsign Movement Delay, sveh Unsign Movement Delay, sveh Unsign Delay Movement Delay, sveh 18.2 47.9 47.9 23.8 21.5 21.5 28.8 33.4 33.3 32.8 39.8 39.9 Unsign Long Delay Long Delay Long Delay De														
LnGrp_Delay(d)sivh 18.2 47.9 47.9 23.8 21.5 21.5 28.8 33.4 33.3 32.8 39.8 39.9 LnGrp_Delay(d)sivh 18.2 8 D D C C C C C C C D D D Approach Vol., veh h 112.8 63.7 951 370 370 Approach Delay, sivh 45.8 22.0 32.5 39.1 D D C C C C C C D D D D D D D D D D D				10.5	2.1	7.7	4.0	0.1	0.7	5.5	0.5	7.2	7.7	
LnCrip LOS				/7 Q	23.8	21.5	21.5	28.8	33.4	33.3	32.8	30.8	30.0	
Approach Vol, wehh 1128 637 951 370 Approach Delay, sive 45.8 22.0 32.5 39.1 Approach LOS C C D D Illiner - Assigned Phs 1 2 3 4 5 6 7 8 The Duration (GY+RC), \$7.4 43.1 11.5 580 15.5 35.0 90 60.5 Change Period (Y+RC), \$7.4 4.5.3 *4.4 *5.7 *4.8 *5.3 *4.4 *5.7 Max Green Setting (finantit) is *38 *13 *39 *18 *30 *11 *42 Max GC Loer Time (g c-14), \$8 *16.9 7.2 35.8 10.7 11.3 4.9 12.6 Terene Ext Time (g c-1), \$0.0 1.5 0.0 1.3 0.0 0.6 0.0 0.9 Intersection Summary HCM 6th Crit Delay 36.0 HCM 6th LOS D														
Approach LoS														
Approach LOS														
Timer - Assigned Phs 1 2 3 4 5 6 7 8 Phs Duration (G+Y+Rc), s74 43.1 11.5 8s0 15.5 35.0 9.0 60.5 Change Period (Y+Rc), s44 - 5.1 44 - 5.7 *4.8 *5.3 *4.4 *5.7 Nax Green Setting (GmaX)(s *38 *13 *39 *18 *30 *11 *42 Nax Q Clear Time (g, c+1)(s, 16.9 7.2 3s.8 10.7 11.3 4.9 12.6 Green Ext Time (g, c), s 0.0 1.5 0.0 1.3 0.0 0.6 0.0 0.9 Intersection Summary HOM 6th Crif Delay 36.0 D Notes														
Pits Duration (G-Y-Rc), s74 43,1 11.5 58.0 15.5 35.0 9.0 60.5 Change Period (Y-Rc), s74.4 *5.7 *4.8 *5.3 *4.4 *5.7 *4.8 *5.3 *4.4 *5.7 *4.8 *6.2 *5.7 *4.8 *6.2 *5.7 *4.8 *6.2 *5.7 *4.8 *6.2 *5.7 *4.8 *6.2 *5.7 *4.8 *6.2 *5.7 *4.8 *6.2 *5.7 *6.2 *5.2 *5.2 *5.2 *5.2 *5.2 *5.2 *5.2 *5	Approach LOS		U			U			U			U		
Change Period (Y-RQ, \$4.4 * 5.3 * 4.4 * 5.7 * 4.8 * 5.3 * 4.4 * 5.7 * 4.8 * 5.3 * 4.4 * 5.7 * 4.8 * 5.3 * 4.4 * 5.7 * 4.8 * 5.3 * 4.4 * 5.7 * 4.8 * 5.3 * 4.4 * 5.7 * 4.8 * 5.3 * 4.4 * 5.7 * 4.8 * 5.3 * 5.8 * 5.	Timer - Assigned Phs	- 1	2	3	4	5	6	7	8					
Max Green Setting (Gmatt)1s * 38 * 13 * 39 * 18 * 30 * 11 * 42 Max Q Clear Time (g. c+14).s * 16.9 * 7.2 * 35.8 * 10.7 * 11.3 * 4.9 * 12.6 Green Ext Time (g. c+14).s * 15 * 00 * 1.3 * 00 * 0.6 * 0.0 * 0.9 Intersection Summary HOM 6th CNI Delay 36.0 ONOTE: The Company of th	Phs Duration (G+Y+Rc)	, s7.4	43.1	11.5	58.0	15.5	35.0	9.0	60.5					
Max Q Clear Time (g.c+1)4, b. 16,9 7,2 35,8 10,7 11,3 4,9 12,6 Green Ext Time (g.c), s 0.0 1.5 0.0 1.3 0.0 0.6 0.0 0.9 Intersection Summary HCM 6th Crit Delay 36,0 D	Change Period (Y+Rc).	\$ 4.4			* 5.7	* 4.8	* 5.3	* 4.4	* 5.7					
Green Ext Time (p_0, s 0.0 1.5 0.0 1.3 0.0 0.6 0.0 0.9 Intersection Summary 4-CM 6th Ctrl Delay 36.0 1-CM 6th LDS D Notes	Max Green Setting (Gm	ax)1\$	* 38	* 13	* 39	* 18	* 30	* 11	* 42					
Intersection Summary HCM 6th Crif Delay 36.0 HCM 6th LOS D Notes	Max Q Clear Time (g_c-	+114.1s	16.9	7.2	35.8	10.7	11.3	4.9	12.6					
HCM 6th Crit Delay 46.0 D CHCM 6th LOS D Votes	Green Ext Time (p_c), s	0.0	1.5	0.0	1.3	0.0	0.6	0.0	0.9					
HCM 6th Crit Delay 46.0 D CHCM 6th LOS D Votes	Intersection Summary													
HCM 6th LOS D Notes				36.0										
Notes														
				_										
THUM bith computational engine requires equal clearance times for the phases crossing the barrier.		d and			al ala									
	HCM 6th computation	al engir	ne requ	ires equ	ial clea	rance ti	mes to	the ph	ases cr	ossing 1	he barr	ier.		

Movement Lane Configurations Traffic Volume (vph) Future Volume (vph) Ideal Flow (vphpl)	EBL 69	EBT ↑ p	EBR	WBL	WBT							
Traffic Volume (vph) Future Volume (vph)					WDI	WBR	NBL	NBT	NBR	SBL	SBT	SB
Future Volume (vph)					479			4			4	
		748	0	5	450	38	5	5	5	23	5	4
Ideal Flow (vehal)	69	748	0	5	450	38	5	5	5	23	5	4
	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	190
Total Lost time (s)	4.4	5.2			5.2			5.2			5.2	
Lane Util. Factor	1.00	0.95			0.95			1.00			1.00	
Frt	1.00	1.00			0.99			0.95			0.92	
Flt Protected	0.95	1.00			1.00			0.98			0.98	
Satd. Flow (prot)	1770	3539			3496			1750			1683	
Flt Permitted	0.33	1.00			0.94			0.92			0.89	
Satd. Flow (perm)	612	3539			3300			1644			1515	
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.9
Growth Factor (vph)	130%	130%	130%	130%	130%	130%	130%	130%	130%	130%	130%	130
Adj. Flow (vph)	100	1080	0	7	650	55	7	7	7	33	7	-
RTOR Reduction (vph)	0	0	0	0	4	0	0	6	0	0	54	
Lane Group Flow (vph)	100	1080	0	0	708	0	0	15	0	0	47	
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2
Turn Type	pm+pt	NA		pm+pt	NA		Perm	NA		Perm	NA	
Protected Phases	1	6 26		5	2 22			4 24			8 28	
Permitted Phases	6 26			2 22			4 24			8 28		
Actuated Green, G (s)	86.3	85.5			63.9			13.7			13.7	
Effective Green, g (s)	86.3	85.5			63.9			13.7			13.7	
Actuated g/C Ratio	0.72	0.71			0.53			0.11			0.11	
Clearance Time (s)	4.4											
Vehicle Extension (s)	1.0											
Lane Grp Cap (vph)	606	2521			1757			187			172	
v/s Ratio Prot	0.02	c0.31										
v/s Ratio Perm	0.10				0.21			0.01			c0.03	
v/c Ratio	0.17	0.43			0.40			0.08			0.27	
Uniform Delay, d1	9.1	7.1			16.7			47.5			48.6	
Progression Factor	1.51	1.51			1.10			1.00			1.00	
Incremental Delay, d2	0.0	0.1			0.2			0.1			0.3	
Delay (s)	13.8 B	10.9			18.5 B			47.6 D			48.9	
Level of Service	В	В									D	
Approach Delay (s) Approach LOS		11.1 B			18.5 B			47.6 D			48.9 D	
Intersection Summary												
HCM 2000 Control Delay			16.0	Н	CM 2000	Level of S	Service		В			
HCM 2000 Volume to Capac	ity ratio		0.43									
Actuated Cycle Length (s)			120.0		um of lost				25.2			
Intersection Capacity Utilizat Analysis Period (min)	ion		64.8%	IC	U Level o	f Service			С			
			15									

WBT ↑↑ 387 387 1900 4.4 0.95 1.00 1.00 3539 0.86 6130% NA 7 9 11 70.3 56.5 0.47	WBR 75 3611 1900 4.0 8.8 8.5 1.00 2787 0.86 6.0 546 2% Free 120.0 1.00 120.0 1.00 120.0 1.00 120.0 1.00 120.0 1.00 120.0 1.00 1.0	SBL 436 436 436 436 1.00 0.95 4997 0.86 659 2% Prot 12 10 12.7 0.11	5 5 1900 0.86 130% 8 0 0 2%		
387 387 1900 4.4 0.95 1.00 3539 1.00 3539 0.86 130% 585 2% NA 7 9 11 70.3 56.5 0.47	361 361 1900 4.0 0.88 0.85 1.00 2787 1.00 2787 0.86 0 130% 546 0 546 2% Free	436 436 1990 5.6 0.94 1.00 0.95 4997 0.86 130% 659 1 12 10 12.7 12.7 0.11	5 1900 0.86 130% 8 0		
387 1900 4.4 0.95 1.00 1.00 3539 0.86 130% 585 0 585 2% NA 7 9 11 70.3 56.5 0.47	361 1900 4.0 0.88 0.85 1.00 2787 1.00 2787 0.86 130% 546 2% Free 120.0 1.00	436 1900 5.6 0.94 1.00 0.95 4997 0.95 4997 0.86 130% 659 1 666 2% Prot 12.10	5 1900 0.86 130% 8 0		
1900 4.4 0.95 1.00 3539 1.00 3539 0.86 130% 585 0 585 2% NA 7 9 11 70.3 56.5 0.47	1900 4.0 0.88 0.85 1.00 2787 1.00 2787 0.86 130% 546 0 546 2% Free 120.0 120.0	1900 5.6 0.94 1.00 0.95 4997 0.95 4997 0.86 130% 659 Prot 12 10 12.7 0.11	0.86 130% 8 0		
4.4 0.95 1.00 3539 1.00 3539 0.86 130% 585 0 585 2% NA 7 9 11 70.3 56.5 0.47	4.0 0.88 0.85 1.00 2787 1.00 2787 0.86 130% 546 0 546 2% Free 120.0 120.0	5.6 0.94 1.00 0.95 4997 0.95 4997 0.86 130% 659 1 6666 2% Prot 12 10 12.7 12.7 0.11	0.86 130% 8 0		
0.95 1.00 1.00 3539 1.00 3539 0.86 130% 585 0 585 2% NA 7 9 11 70.3 56.5 0.47	0.88 0.85 1.00 2787 1.00 2787 0.86 130% 546 0 546 2% Free 120.0 1.00	0.94 1.00 0.95 4997 0.86 130% 659 1 666 2% Prot 12.10 12.7 0.11	130% 8 0		
1.00 1.00 3539 1.00 3539 0.86 130% 585 0 585 2% NA 7 9 11 70.3 56.5 0.47	0.85 1.00 2787 1.00 2787 1.00 2787 0.86 130% 546 0 546 2% Free 120.0 120.0	1.00 0.95 4997 0.95 4997 0.86 130% 659 1 666 2% Prot 12 10 12.7 12.7 0.11	130% 8 0		
1.00 3539 1.00 3539 0.86 130% 585 0 585 2% NA 7 9 11 70.3 56.5 0.47	1.00 2787 1.00 2787 0.86 130% 546 0 546 2% Free 120.0 1.00	0.95 4997 0.95 4997 0.86 130% 659 1 666 2% Prot 12 10 12.7 12.7 0.11	130% 8 0		
3539 1.00 3539 0.86 130% 585 0 585 2% NA 7 9 11 70.3 56.5 0.47	2787 1.00 2787 0.86 130% 546 0 546 2% Free Free 120.0 1.00	4997 0.95 4997 0.86 130% 659 1 666 2% Prot 12 10 12.7 12.7 0.11	130% 8 0		
1.00 3539 0.86 130% 585 0 585 2% NA 7 9 11 70.3 56.5 0.47	1.00 2787 0.86 130% 546 0 546 2% Free 120.0 1.00	0.95 4997 0.86 130% 659 1 666 2% Prot 12 10 12.7 12.7 0.11	130% 8 0		
3539 0.86 130% 585 0 585 2% NA 7 9 11 70.3 56.5 0.47	2787 0.86 130% 546 0 546 2% Free 120.0 1.00	4997 0.86 130% 659 1 666 2% Prot 12 10 12.7 12.7 0.11	130% 8 0		
0.86 130% 585 0 585 2% NA 7 9 11 70.3 56.5 0.47	0.86 130% 546 0 546 2% Free 120.0 120.0	0.86 130% 659 1 666 2% Prot 12 10 12.7 12.7 0.11	130% 8 0		
130% 585 0 585 2% NA 7 9 11 70.3 56.5 0.47	130% 546 0 546 2% Free 120.0 120.0 1.00	130% 659 1 666 2% Prot 12 10 12.7 12.7 0.11	130% 8 0		
585 0 585 2% NA 7 9 11 70.3 56.5 0.47	546 0 546 2% Free 120.0 120.0 1.00	659 1 666 2% Prot 12 10 12.7 12.7 0.11	8 0 0		
0 585 2% NA 7 9 11 70.3 56.5 0.47	0 546 2% Free 120.0 120.0 1.00	1 666 2% Prot 12 10 12.7 12.7 0.11	0		
585 2% NA 7 9 11 70.3 56.5 0.47	546 2% Free 120.0 120.0 1.00	666 2% Prot 12 10 12.7 12.7 0.11	0		
2% NA 7 9 11 70.3 56.5 0.47	2% Free Free 120.0 120.0 1.00	2% Prot 12 10 12.7 12.7 0.11			
NA 7 9 11 70.3 56.5 0.47	Free 120.0 120.0 1.00	Prot 12 10 12.7 12.7 0.11	2%		
7 9 11 70.3 56.5 0.47	Free 120.0 120.0 1.00	12.10 12.7 12.7 0.11			
70.3 56.5 0.47	120.0 120.0 1.00	12.7 12.7 0.11			
56.5 0.47 1666	120.0 120.0 1.00	12.7 0.11			
56.5 0.47 1666	120.0	12.7 0.11			
0.47	1.00	0.11			
1666					
	2787				
	2787				
	2787				
c0.17		528			
		c0.13			
	c0.20				
0.35	0.20	1.26			
20.1	0.0	53.6			
0.55	1.00	1.00			
0.0	0.1	132.3			
	Α				
A		F			
	H	ICM 2000	Level of Service	E	
	10	CU Level o	of Service	В	
15					
	77.4 0.86 120.0 55.5%	11.1 0.1 B A 5.8 A 77.4 H 0.86 120.0 S 55.5% IO	11.1 0.1 186.0 B A F 5.8 186.0 A F 77.4 HCM 2000 0.86 120.0 Sum of lost 55.5% ICU Level c	11.1 0.1 186.0 B A F 5.8 186.0 A F 77.4 HCM 2000 Level of Service 0.86 120.0 Sum of lost time (s) 55.5% ICU Level of Service	11.1 0.1 1860 B A F 58 1860 A F 77.4 HCM 2000 Level of Service E 0.86 120.0 Sum of lost time (s) 41.6 55.5% ICU Level of Service B

Intersection													
Int Delay, s/veh	0.9												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		ተ ቀር			ተ ቀተ				7			7	
Traffic Vol, veh/h	0	1250	20	0	890	6	0	0	62	0	0	6	
Future Vol, veh/h	0	1250	20	0	890	6	0	0	62	0	0	6	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	
RT Channelized	-		None			None		-	None		-	None	
Storage Length	-		-	-	-	-	-	-	0	-	-	0	
Veh in Median Storage,	# -	0			0			0			0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	89	89	89	89	89	89	89	89	89	89	89	89	
Heavy Vehicles, %	4	4	4	4	4	4	4	4	4	4	4	4	
Mvmt Flow	0	1826	29	0	1300	9	0	0	91	0	0	9	
	lajor1			//ajor2			Minor1			/linor2			
Conflicting Flow All		0	0		- 1	0	-	-	928			655	
Stage 1 Stage 2	- 1	- 1		- 1	- 1		- 1	- 1		- 1	- 1		
Critical Hdwv					- 1				7.18			7.18	
Critical Hdwy Stg 1	- 1	- 1	- 1	- 1	- 1		- 1	- 1	7.10	- 1	- 1	7.10	
Critical Hdwy Stg 2	- 1	- 1	- 1	- 1	- 1	- 1	- 1	- 1	- :	- 1	- 1	- 1	
Follow-up Hdwy	- 1	- 1	- 1	- 1	- 1	- 1		- 1	3.94	- 1	- 1	3.94	
Pot Cap-1 Maneuver	0	- :	- 1	0	- 1	- :	0	0	229	0	0	347	
Stage 1	0			0			0	0	225	0	0	541	
Stage 2	0			0			0	0		0	0		
Platoon blocked. %								•		•			
Mov Cap-1 Maneuver									229			347	
Mov Cap-2 Maneuver			-		-	-	-			-		-	
Stage 1												-	
Stage 2	-	-	-	-	-	-	-	-	-	-	-	-	
Approach	EB			WB			NB			SB			
HCM Control Delay, s	0			0			30.6			15.6			
HCM LOS							D			С			
M		VBI n1	FBT	500	MOT	WDD (DDI - 4						
Minor Lane/Major Mvmt Capacity (veh/h)		229	EBI	EBR -	WBT	WBR S	347						
HCM Lane V/C Ratio		0.395		- 1	- 1		0.025						
HCM Control Delay (s)		30.6			- 1	- 1							
HCM Lane LOS		30.6 D		- 1			15.6 C						
HCM 95th %tile Q(veh)		18	- 1	- 1	- 1	- 1	0.1						
TIOM 55th Attle Q(Ven)		1.0					0.1						

ntersection													
nt Delay, s/veh	1.7												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
ane Configurations		414			414			4					
raffic Vol, veh/h	4	1297	4	2	765	- 1	5	1	7	4	4	4	
uture Vol. veh/h	4	1297	4	2	765	- 1	5	1	7	4	1	4	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	
RT Channelized		-	None			None	-	-	None	-	-	None	
Storage Length			-										
eh in Median Storage.	# -	0	- 1		0			0			0		
Grade, %		0		٠.	0			0			0		
Peak Hour Factor	86	86	86	86	86	86	86	86	86	86	86	86	
leavy Vehicles, %	3	3	3	3	3	3	3	3	3	3	3	3	
Nymt Flow	6	1961	6	3	1156	2	8	2	11	6	2	6	
IVIIIL FIOW	U	1301	U	J	1130		U		- 11	U		U	
Major/Minor N	Major1			Major2			Minor1		,	Minor2			
onflicting Flow All	1158	0	0	1967	0	0	2445	3140	984	1960	3142	579	
Stage 1	1130	-	0	1301	-		1976	1976	304	1163	1163	313	
Stage 2		- 1	- 1	- 1		- 1	469	1164	- :	797	1979		
Critical Hdwy	5.36			5.36			6.46	6.56	7.16	6.46	6.56	7.16	
Critical Hdwy Stg 1	0.00			0.00			7.36	5.56	7.10	7.36	5.56	7.10	
Critical Hdwy Stg 2	- 1	- :	- :	- :	- 1	- :	6.76	5.56	- :	6.76	5.56	- :	
ollow-up Hdwy	3.13	- 1	- 1	3.13	- 1	- 1	3.83	4.03	3.93	3.83	4.03	3.93	
ot Cap-1 Maneuver	325	- 1	- 1	128	- :	- :	33	11	211	66	11	391	
Stage 1	323	- 1	- 1	120		- 1	40	105	211	152	265	351	
Stage 2		_	_	-	_	_	495	265	- :	312	105	_	
Platoon blocked. %		- 1	- 0		- 0	- 0	430	200		312	103		
Nov Cap-1 Maneuver	325			128			27	10	211	53	10	391	
Nov Cap-1 Maneuver	323		- 1	120		- :	27	10	211	53	10	351	
Stage 1	- 1	_	- :	- 1	- 1	_	40	105	- :	152	248	_	
Stage 2	- 3		- 1	- 1	- 1		452	248	- 1	292	105	- 1	
Stage 2			-				402	240		292	105	-	
pproach	EB			WB			NB			SB			
ICM Control Delay, s	0			0.8			156.4			107.3			
ICM LOS	0			0.0			F			F			
linor Lane/Major Mvm	t	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1				
apacity (veh/h)		41	325		-	128			48				
ICM Lane V/C Ratio		0.479	0.019	-		0.024	-	-	0.283				
ICM Control Delay (s)		156.4	16.3	0		33.8	0.7		107.3				
ICM Lane LOS		F	С	Α	-	D	Α		F				
ICM 95th %tile Q(veh)		1.7	0.1	-		0.1	-		- 1				
2011 70110 ((1011)			0.1			0.1							

Intersection														
Int Delay, s/veh	19.2													
Movement	FBI	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR		
Lane Configurations		att.			£141;			4			4			
Traffic Vol, veh/h	0	1328	37	2	815	5	5	5	64	5	5	6		
Future Vol, veh/h	0	1328	37	2	815	5	5	5	64	5	5	6		
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0		
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop		
RT Channelized	-		None		-	None	-		None			None		
Storage Length	-		-		-	-						-		
Veh in Median Storage,	# -	0			0			0			0			
Grade, % Peak Hour Factor	91	91	91	91	91	91	91	91	91	91	91	91		
Heavy Vehicles. %	3	3	3	3	3	3	3	3	3	3	3	3		
Mymt Flow		1897	53		1164	7	7	7	91	7	7	9		
MVIIIL FIOW	U	1031	33	J	1104	- 1	- 1	- 1	31	- 1	- 1	9		
Major/Minor N	lajor1			Major2			Minor1			Minor2				
Conflicting Flow All	1171	0	0	1950	0	0	2399	3101	975	1936	3124	586		
Stage 1	-						1924	1924	-	1174	1174			
Stage 2							475	1177		762	1950	-		
Critical Hdwv	5.36			5.36			6.46	6.56	7.16	6.46	6.56	7.16		
Critical Hdwy Stg 1	-		-		-		7.36	5.56		7.36	5.56	-		
Critical Hdwy Stg 2	-		-		-		6.76	5.56		6.76	5.56	-		
Follow-up Hdwy	3.13	-	-	3.13	-	-	3.83	4.03	3.93	3.83	4.03	3.93		
Pot Cap-1 Maneuver	321		-	131	-		35	- 11	214	69	- 11	387		
Stage 1	-		-	-	-		43	111	-	149	262	-		
Stage 2	-		-	-	-	-	491	261	-	328	108	-		
Platoon blocked, %	204			404	- 1	- 1	14	10	044	16	10	387		
Mov Cap-1 Maneuver Mov Cap-2 Maneuver	321	- 1	- 1	131	- 1	- 1	14	10	214	16	10	387		
Stage 1	- 1				- 1	- 1	43	111	- 1	149	245			
Stage 2	- 1	- 1	- 1	- 1	- 1	- 1	435	244	- 1	176	108			
Olago L							100				100			
Approach	EB			WB			NB			SB				
HCM Control Delay, s	0			0.8		\$	459.5		\$	563.7				
HCM LOS							F			F				
Minor Lane/Major Mvm		NBLn1	EBL	EBT	EBR		WBT	WBR :						
Capacity (veh/h)		64	321		-	131 0.022	-	-	19					
HCM Lane V/C Ratio HCM Control Delay (s)		1.652	- 0	-	- 1	33.1	0.7		1.203					
HCM Control Delay (s)	\$	459.5 F	A	- 1		33.1 D	U.7	•	503.7					
HCM 95th %tile Q(veh)		9.4	0		- 1	0.1	-	- 1	3.2					
Notes														
~: Volume exceeds cap	acity	\$: De	lay exc	eeds 3	00s -	+: Com	putation	Not De	efined	*: All	major v	olume in pla	atoon	

	٦.	EBT						Ť					
Traffic Volume (veh/h) Future Volume (veh/h) Initial Q (Qb), veh Ped-Bike Adj(A_pbT) 1.			EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Future Volume (veh/h) Initial Q (Qb), veh Ped-Bike Adj(A_pbT) 1.		<u>ቀ</u> ቀሴ			<u>ተ</u> ቀሴ		7	•	7	7	7		
Initial Q (Qb), veh Ped-Bike Adj(A_pbT) 1.	69	1100	91	180	700	17	110	168	135	30	132	32	
Ped-Bike Adj(A_pbT) 1.	69	1100	91	180	700	17	110	168	135	30	132	32	
	0	0	0	0	0	0	0	0	0	0	0	0	
Parking Bus, Adi 1.1		4.00	1.00	1.00		1.00	1.00		1.00	1.00		1.00	
	UU	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Work Zone On Approach		No	4050	4050	No	4050	4050	No	4050	4050	No	4050	
Adj Sat Flow, veh/h/ln 18	95	1856 1521	1856 126	1856 249	1856 968	1856 24	1856 152	1856 232	1856	1856 41	1856 183	1856 44	
Adj Flow Rate, veh/h ! Peak Hour Factor 0.		0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	
Peak Hour Factor U.: Percent Heavy Veh. %	3	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	
	3 27	2182	181	307	1110	28	234	266	226	215	208	50	
Cap, veh/h 7: Arrive On Green 0.		0.92	0.92	0.14	0.22	0.22	0.10	0.14	0.14	0.09	0.14	0.14	
Sat Flow, veh/h 17		4767	395	1767	5084	126	1767	1856	1572	1767	1445	348	
	95	1077	570	249	643	349	152	232	187	41	0	227	
Grp Sat Flow(s), ven/n Grp Sat Flow(s), veh/h/ln17		1689	1784	1767	1689	1833	1767	1856	1572	1767	0	1793	
	1.0	9.0	9.0	15.7	22.0	22.1	9.9	14.7	10.3	0.0	0.0	14.9	
	1.0	9.0	9.0	15.7	22.0	22.1	9.9	14.7	10.3	0.0	0.0	14.9	
Prop In Lane 1.		9.0	0.22	1.00	22.0	0.07	1.00	14.7	1.00	1.00	0.0	0.19	
	27	1546	817	307	737	400	234	266	226	215	0	258	
V/C Ratio(X) 0.		0.70	0.70	0.81	0.87	0.87	0.65	0.87	0.83	0.19	0.00	0.88	
	27	1546	817	307	1207	655	307	563	477	220	0.00	453	
HCM Platoon Ratio 2.		2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Upstream Filter(I) 0.		0.88	0.88	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	
Uniform Delay (d), s/veh 9		3.1	3.1	44.9	45.3	45.3	50.3	50.3	27.4	50.0	0.0	50.4	
	1.0	2.3	4.3	14.0	13.4	22.2	1.1	3.5	3.0	0.2	0.0	4.0	
Initial Q Delay(d3),s/veh 0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
%ile BackOfQ(50%).veh/lr0		1.8	2.4	7.9	10.4	12.3	4.4	7.1	4.0	1.1	0.0	6.9	
Unsig. Movement Delay, s/													
	1.6	5.5	7.5	58.9	58.7	67.5	51.4	53.8	30.4	50.1	0.0	54.3	
LnGrp LOS	Α	Α	Α	Е	Е	Е	D	D	С	D	Α	D	
Approach Vol, veh/h		1742			1241			571			268		
Approach Delay, s/veh		6.3			61.2			45.5			53.7		
Approach LOS		Α			E			D			D		
Timer - Assigned Phs	1	2	3	4	5	6	7	8					
Phs Duration (G+Y+Rc), 49	17	31.3	16.2	22.8	21.0	60.0	16.0	22.9					
Change Period (Y+Rc), \$ 5		*5.1	5.7	* 5.6	*42	* 5.1	*42	5.7					
Max Green Setting (Gmax)		* 43	10.8	* 36	* 17	* 37	* 17	30.3					
Max Q Clear Time (g_c+l12		24.1	2.0	16.7	17.7	11.0	11.9	16.9					
Green Ext Time (p_c), s 0		2.1	0.0	0.5	0.0	4.3	0.0	0.4					
Intersection Summary													
HCM 6th Ctrl Delay			33.3										
HCM 6th LOS			C										
Notes													
* HCM 6th computational e	ngin	ne requi	ires equ	ial clea	rance ti	mes for	the ph	ases cr	ossing t	he barr	ier.		

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SB
Lane Configurations		44	17		414					ř	ર્લ	
Traffic Volume (vph)	0	650	665	53	499	0	0	0	0	109	27	3
Future Volume (vph)	0	650	665	53	499	0	0	0	0	109	27	3
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	19
Total Lost time (s)		5.0	5.0		5.0					5.7	5.7	5
Lane Util. Factor		0.95	1.00		0.95					0.95	0.95	1.
Frt		1.00	0.85		1.00					1.00	1.00	0.
Flt Protected		1.00	1.00		1.00					0.95	0.97	1.
Satd. Flow (prot)		3124	1398		3109					1484	1516	13
Flt Permitted		1.00	1.00		0.79					0.95	0.97	1.
Satd. Flow (perm)	0.00	3124	1398	0.05	2473		0.05	0.00	0.00	1484	1516	13
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.
Growth Factor (vph)	130%	130%	130%	130%	130%	130%	130%	130%	130%	130%	130%	130
Adj. Flow (vph)	0	889	910	73	683	0	0	0	0	149	37	5
RTOR Reduction (vph)	0	0 889	502	0	756	0	0	0	0	92	94	3
Lane Group Flow (vph)	4%	889 4%	408 4%	4%	756 4%	4%	4%	4%	4%	92 4%	94 4%	2
Heavy Vehicles (%)	4%					4%	4%	4%	4%			- 4
Turn Type Protected Phases		NA 6	Perm	Prot 5	NA 5.6					Split 8	NA 8	Per
Protected Phases Permitted Phases		ь	6	5	50					8	8	
Permitted Phases Actuated Green, G (s)		35.4	35.4		53.7					14.6	14.6	14
Effective Green, g (s)		35.4	35.4		53.7					14.6	14.6	14
Actuated g/C Ratio		0.42	0.42		0.64					0.17	0.17	0.
Clearance Time (s)		5.0	5.0		0.04					5.7	5.7	U.
Vehicle Extension (s)		1.7	1.7							1.0	1.0	1
Lane Grp Cap (vph)		1316	589		1719					257	263	2
v/s Ratio Prot		0.28	303		c0.10					0.06	0.06	2
v/s Ratio Perm		0.20	c0.29		0.19					0.00	0.00	cD.
v/c Ratio		0.68	0.69		0.13					0.36	0.36	0.
Uniform Delay, d1		19.7	19.9		7.6					30.6	30.6	34
Progression Factor		1.00	1.00		1.67					1.00	1.00	1.
Incremental Delay, d2		2.8	6.6		0.1					0.3	0.3	48
Delay (s)		22.4	26.4		12.7					30.9	30.9	83
Level of Service		С	С		В					С	С	
Approach Delay (s)		24.5			12.7			0.0			69.7	
Approach LOS		С			В			Α			Е	
Intersection Summary												
HCM 2000 Control Delay			31.8	- 11	CM 2000	l aval af (Candaa		С			
HCM 2000 Collifor Delay HCM 2000 Volume to Capacit	tu ratio		0.68	п	CIVI 2000	Level of 3	Service		C			
Actuated Cycle Length (s)	y rauo		84.0	e.	um of lost	time (c)			15.7			
Intersection Capacity Utilization	nn .		100.1%		U Level o				15.7 G			
Analysis Period (min)	J.,		15	IC.	-C LOVE C	JOI 1166			3			

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SI
Lane Configurations	LUL	414	LUIT	1102	†	TTDIT.	*	4	HUIT	ODL	051	
Traffic Volume (vph)	475	285	0	0	130	66	422	9	35	0	0	
Future Volume (vph)	475	285	0	0	130	66	422	9	35	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	19
Total Lost time (s)		4.9			4.7		5.5	5.5				
Lane Util. Factor		0.95			0.95		0.95	0.95				
Frt		1.00			0.95		1.00	0.98				
Flt Protected		0.97			1.00		0.95	0.96				
Satd. Flow (prot)		3432			3361		1681	1662				
FIt Permitted		0.67			1.00		0.95	0.96				
Satd. Flow (perm)		2385			3361		1681	1662				
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.
Growth Factor (vph)	130%	130%	130%	130%	130%	130%	130%	130%	130%	130%	130%	13
Adj. Flow (vph)	643	386	0	0	176	89	571	12	47	0	0	
RTOR Reduction (vph) Lane Group Flow (vph)	0	1029	0	0	60 205	0	0 320	302	0	0	0	
Heavy Vehicles (%)	2%	2%	2%	2%	205	2%	2%	2%	2%	2%	2%	
Turn Type	Prot	NA	270	276	NA	Z76	Split	NA NA	276	276	Z76	_
Protected Phases	1	12			2		3piit 4	4				
Permitted Phases		12					-	-				
Actuated Green, G (s)		44.1			27.3		24.8	24.8				
Effective Green, g (s)		44.1			27.3		24.8	24.8				
Actuated g/C Ratio		0.53			0.33		0.30	0.30				
Clearance Time (s)					4.7		5.5	5.5				
Vehicle Extension (s)					1.3		1.0	1.0				
Lane Grp Cap (vph)		1461			1092		496	490				
v/s Ratio Prot		c0.14			0.06		c0.19	0.18				
v/s Ratio Perm		c0.23										
v/c Ratio		1.82dl			0.19		0.65	0.62				
Uniform Delay, d1		15.0			20.4		25.8	25.5				
Progression Factor		2.09			1.00		1.00	1.00				
Incremental Delay, d2		1.0			0.4		2.2	1.6				
Delay (s) Level of Service		32.5 C			20.8 C		27.9 C	27.1 C				
Approach Delay (s)		32.5			20.8		C	27.5			0.0	
Approach LOS		32.5 C			20.6 C			21.5 C			0.0 A	
•••		C			C			C			Α.	
Intersection Summary												
HCM 2000 Control Delay			29.2	Н	CM 2000	Level of	Service		С			
HCM 2000 Volume to Capac	ity ratio		0.69									
Actuated Cycle Length (s)			84.0 71.1%		um of lost				15.7 C			
Intersection Capacity Utilizati	ion		11.1%	IC	U Level o	of Service			C			
		the second of the		off lane								
Analysis Period (min) dl Defacto Left Lane. Reco												

wsp	
2020 PM Peak Hour	

	•	-	*	1	•	•	1	1	-	-	Ţ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SB
Lane Configurations	36	414			ተተተ	7	36	414	7			
Traffic Volume (vph)	240	1300	0	0	1400	300	886	371	357	0	0	
Future Volume (vph)	240	1300	0	0	1400	300	886	371	357	0	0	
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	200
Total Lost time (s)	5.1	5.1			5.6	5.6	5.1	5.1	5.1			
Lane Util. Factor	0.91	0.91			0.91	1.00	0.91	0.91	1.00			
Frt	1.00	1.00			1.00	0.85	1.00	1.00	0.85			
Flt Protected	0.95	1.00			1.00	1.00	0.95	0.97	1.00			
Satd. Flow (prot)	1712	3601			5406	1683	1712	3508	1683			
FIt Permitted	0.95	1.00			1.00	1.00	0.95	0.97	1.00			
Satd. Flow (perm)	1712	3601			5406	1683	1712	3508	1683			
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.9
Growth Factor (vph)	108%	108%	108%	108%	108%	108%	108%	108%	108%	108%	108%	108
Adj. Flow (vph)	270	1462	0	0	1575	338	997	417	402	0	0	
RTOR Reduction (vph)	0	0	0	0	0	184	0	0	226	0	0	
Lane Group Flow (vph)	243	1490	0	0	1575	154	498	916	176	0	0	
Heavy Vehicles (%)	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	- 1
Turn Type	Split	NA			NA	Perm	Split	NA	Perm			
Protected Phases	1 11	1 11			2		4	4				
Permitted Phases						2			4			
Actuated Green, G (s)	72.9	72.9			30.4	30.4	20.9	20.9	20.9			
Effective Green, g (s)	72.9	72.9			30.4	30.4	20.9	20.9	20.9			
Actuated g/C Ratio	0.52	0.52			0.22	0.22	0.15	0.15	0.15			
Clearance Time (s)					5.6	5.6	5.1	5.1	5.1			
Vehicle Extension (s)					1.0	1.0	1.0	1.0	1.0			
Lane Grp Cap (vph)	891	1875			1173	365	255	523	251			
v/s Ratio Prot	0.14	c0.41			c0.29		c0.29	0.26				
v/s Ratio Perm						0.09			0.10			
v/c Ratio	0.27	0.79			1.34	0.42	1.95	1.88dl	0.70			
Uniform Delay, d1	18.7	27.4			54.8	47.2	59.5	59.5	56.6			
Progression Factor	0.03	0.27			1.16	0.72	1.00	1.00	1.00			
Incremental Delay, d2	0.0	1.5			158.6	0.2	442.8	346.0	7.0			
Delay (s)	0.7	8.9			222.3	34.4	502.4	405.5	63.6			
Level of Service	A	A			F	C	F	F	F			
Approach Delay (s)		7.7			189.1	_		356.4	_		0.0	
Approach LOS		Α			F			F			A	
Intersection Summary												
HCM 2000 Control Delay			187.2	Н	CM 2000	Level of	Service		F			
HCM 2000 Volume to Capac	city ratio		1.18									
Actuated Cycle Length (s)	,		140.0	S	um of lost	time (s)			21.5			
Intersection Capacity Utilizal	tion		105.4%	IC	U Level	of Service			G			
Analysis Period (min)			15									
dl Defacto Left Lane. Rec	ode with 1	though la	ne as a le	eft lane.								
c Critical Lane Group												
Culebra Road Corridor Stud											nchro 10	

	•	-	•	1	•	•	1	†	-	1	↓	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBI
ane Configurations		***	7	7	413					*	413	
Traffic Volume (vph)	0	1052	476	500	1780	0	0	0	0	480	310	21
uture Volume (vph)	0	1052	476	500	1780	0	0	0	0	480	310	21
deal Flow (vphpl)	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	200
Total Lost time (s)		5.7	5.7	5.1	5.1					5.1	5.1	5.
ane Util. Factor		0.91	1.00	0.91	0.91					0.91	0.91	1.0
-rt		1.00	0.85	1.00	1.00					1.00	1.00	0.8
Flt Protected		1.00	1.00	0.95	1.00					0.95	0.98	1.0
Satd. Flow (prot)		5406	1683	1712	3599					1712	3531	168
Flt Permitted		1.00	1.00	0.95	1.00					0.95	0.98	1.0
Satd. Flow (perm)	0.00	5406	1683	1712	3599	0.00	0.00	0.00	0.00	1712	3531	168
Peak-hour factor, PHF	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.9
Growth Factor (vph)	108%	108% 1159	108% 525	108% 551	108% 1962	108%	108%	108%	108%	108% 529	108% 342	1089
Adj. Flow (vph) RTOR Reduction (vph)	0	1159	102	0	1962	0	0	0	0	529	342	19
ane Group Flow (vph)	0	1159	423	496	2017	0	0	0	0	286	585	4
Heavy Vehicles (%)	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	19
Turn Type	170	NA	Perm	Split	NA	170	170	1/0	1/0	Perm	NA.	Perr
Protected Phases		6	Pellii	5 15	5 15					Pellii	8	Pell
Permitted Phases		U	6	3 13	3 13					8	0	
Actuated Green, G (s)		43.3	43.3	56.9	56.9					23.9	23.9	23.
Effective Green, g (s)		43.3	43.3	56.9	56.9					23.9	23.9	23
Actuated g/C Ratio		0.31	0.31	0.41	0.41					0.17	0.17	0.1
Clearance Time (s)		5.7	5.7							5.1	5.1	5.
/ehicle Extension (s)		1.0	1.0							1.0	1.0	1.
ane Grp Cap (vph)		1671	520	695	1462					292	602	28
//s Ratio Prot		0.21		0.29	c0.56							
//s Ratio Perm			c0.25							c0.17	0.17	0.0
//c Ratio		0.69	0.81	0.71	1.38					0.98	0.97	0.1
Jniform Delay, d1		42.5	44.6	34.7	41.5					57.8	57.7	49.
Progression Factor		1.00	1.00	0.19	0.59					1.00	1.00	1.0
ncremental Delay, d2		2.4	13.0	0.3	171.2					46.3	29.4	0.
Delay (s)		44.9	57.6	6.8	195.9					104.1	87.1	49.
_evel of Service		D	Е	Α	F					F	F	- 1
Approach Delay (s)		48.9			158.6			0.0 A			83.4	
Approach LOS		U			F			А			F	
ntersection Summary												
HCM 2000 Control Delay			108.1	Н	CM 2000	Level of S	Service		F			
	ity ratio								01.5			
	ion			IC	U Level	of Service			G			
			10									
HCM 2000 Volume to Capace Actuated Cycle Length (s) Intersection Capacity Utilizat Analysis Period (min) C Critical Lane Group			1.16 140.0 105.4% 15	S	um of los				21.5 G			

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBI
Lane Configurations	36	4	7	36	4	7		^^			个个 ()	
Traffic Volume (veh/h)	260	58	14	101	42	46	81	2300	87	41	1500	14
Future Volume (veh/h)	260	58	14	101	42	46	81	2300	87	41	1500	14
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	
Ped-Bike Adj(A pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.0
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.0
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1885	1885	1885	1885	1885	1885	1885	1885	1885	1885	1885	188
Adj Flow Rate, veh/h	294	65	16	114	47	52	91	2597	98	46	1694	16
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.9
Percent Heavy Veh, %	1	1	1	1	1	1	1	1	1	1	1	
Cap, veh/h	250	92	78	229	88	75	272	3441	129	158	3177	30
Arrive On Green	0.11	0.05	0.05	0.10	0.05	0.05	0.06	1.00	1.00	0.04	1.00	1.0
Sat Flow, veh/h	1795	1885	1598	1795	1885	1598	1795	5091	190	1795	4781	45
Grp Volume(v), veh/h	294	65	16	114	47	52	91	1743	952	46	1215	64
Grp Sat Flow(s),veh/h/ln	1795	1885	1598	1795	1885	1598	1795	1716	1851	1795	1716	180
Q Serve(g_s), s	15.5	4.8	1.2	4.3	3.4	4.5	2.3	0.0	0.0	1.2	0.0	0.
Cycle Q Clear(g_c), s	15.5	4.8	1.2	4.3	3.4	4.5	2.3	0.0	0.0	1.2	0.0	0.
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.10	1.00		0.2
Lane Grp Cap(c), veh/h	250	92	78	229	88	75	272	2319	1251	158	2280	119
V/C Ratio(X)	1.18	0.71	0.21	0.50	0.53	0.70	0.33	0.75	0.76	0.29	0.53	0.5
Avail Cap(c_a), veh/h	250	257	218	229	190	161	346	2319	1251	252	2280	119
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	2.00	2.00	2.00	2.00	2.00	2.0
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	0.77	0.77	0.77	0.85	0.85	0.8
Uniform Delay (d), s/veh	63.8	65.6	50.3	57.8	65.2	65.8	6.7	0.0	0.0	7.1	0.0	0.
Incr Delay (d2), s/veh	112.6	3.7	0.5	0.6	1.9	4.3	0.2	1.8	3.4	0.3	0.8	1.
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.
%ile BackOfQ(50%),veh/ln	16.8	2.4	0.6	3.8	1.7	1.9	0.8	0.6	1.2	0.4	0.2	0.
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	176.3	69.3	50.8	58.5	67.1	70.0	6.9	1.8	3.4	7.4	0.8	1.
LnGrp LOS	F	Е	D	Е	Е	Е	Α	Α	Α	Α	Α	
Approach Vol, veh/h		375			213			2786			1901	
Approach Delay, s/veh		152.4			63.2			2.5			1.2	
Approach LOS		F			E			Α			Α	
Timer - Assigned Phs	- 1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	7.6	99.9	19.7	12.7	9.2	98.3	20.0	12.4				
Change Period (Y+Rc), s	5.0	* 5.3	5.9	* 5.9	5.0	* 5.3	4.5	5.9				
Max Green Setting (Gmax), s	10.0	* 80	10.5	* 19	10.0	* 80	15.5	14.1				
Max Q Clear Time (q. c+l1), s	3.2	2.0	6.3	6.8	4.3	2.0	17.5	6.5				
Green Ext Time (p_c), s	0.0	9.7	0.0	0.1	0.0	4.8	0.0	0.1				
Intersection Summary												
HCM 6th Ctrl Delay			15.1									
HCM 6th LOS			В									
Notes												
Jser approved pedestrian intel HCM 6th computational engin						es crossi	ng the ba	rrier.				
Culebra Road Corridor Study 5	5:00 pm	12/15/202	0 2020 P	M Existin	g Conditio	on				Sy	nchro 10	Repo

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Traffic Volume (vehh) 71 31 79 123 31 84 146 2394 57 75 1500 52 "uture Volume (vehh) 71 31 79 123 31 84 146 2394 57 75 1500 52 "uture Volume (vehh) 71 31 79 123 31 84 146 2394 57 75 1500 52 "uture Volume (vehh) 71 31 79 123 31 84 146 2394 57 75 1500 52 "uture Volume (vehh) 71 31 79 123 31 84 146 2394 57 75 1500 52 "uture Volume (vehh) 71 31 79 123 31 84 146 2394 57 75 1500 52 "uture Volume (vehh) 71 31 79 123 31 84 146 2394 57 75 1500 52 "uture Volume (veh) 71 31 79 123 31 84 146 2394 57 75 1500 52 "uture Volume (veh) 71 31 79 123 31 84 146 2394 57 75 1500 52 "uture Volume (veh) 71 31 79 123 31 84 146 2394 57 75 1500 52 "uture Volume (veh) 71 31 79 123 31 84 146 2394 57 75 1500 52 "uture Volume (veh) 71 31 79 123 31 84 146 2394 57 75 1500 52 "uture Volume (veh) 71 31 79 123 31 84 146 2394 57 75 1500 52 "uture Volume (veh) 71 31 79 123 49 21 120 120 120 120 120 120 120 120 120	Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Traffic Volume (vehh) 71 31 79 123 31 84 146 2394 57 75 1500 52 "uture Volume (vehh) 71 31 79 123 31 84 146 2394 57 75 1500 52 "uture Volume (vehh) 71 31 79 123 31 84 146 2394 57 75 1500 52 "uture Volume (vehh) 71 31 79 123 31 84 146 2394 57 75 1500 52 "uture Volume (vehh) 71 31 79 123 31 84 146 2394 57 75 1500 52 "uture Volume (vehh) 71 31 79 123 31 84 146 2394 57 75 1500 52 "uture Volume (vehh) 71 31 79 123 31 84 146 2394 57 75 1500 52 "uture Volume (veh) 71 31 79 123 31 84 146 2394 57 75 1500 52 "uture Volume (veh) 71 31 79 123 31 84 146 2394 57 75 1500 52 "uture Volume (veh) 71 31 79 123 31 84 146 2394 57 75 1500 52 "uture Volume (veh) 71 31 79 123 31 84 146 2394 57 75 1500 52 "uture Volume (veh) 71 31 79 123 31 84 146 2394 57 75 1500 52 "uture Volume (veh) 71 31 79 123 31 84 146 2394 57 75 1500 52 "uture Volume (veh) 71 31 79 123 49 21 120 120 120 120 120 120 120 120 120	ane Configurations	7	1		, N	λ		ķ	**†		ħ	441		
nitial Q (LQI), veh				79			84		2384	57			52	
Peach Black Adji(A. pbT) 1 0.00	uture Volume (veh/h)	71	31	79	123	31	84	146	2384	57	75	1500	52	
Parking Bus, Agi	nitial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0	
Not Zone On Approach	Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00	
visig Sat Flow, vehinin 1885 18	Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Ling From Rate, weight 7 34 86 135 34 92 160 2608 62 82 1641 57 Reak Hour Factor 0 0.96 </td <td>Vork Zone On Approach</td> <td>ì</td> <td>No</td> <td></td> <td></td> <td>No</td> <td></td> <td></td> <td>No</td> <td></td> <td></td> <td>No</td> <td></td> <td></td>	Vork Zone On Approach	ì	No			No			No			No		
Peak Hour Factor 0 96 0 96 0 96 0 96 0 96 0 96 0 96 0 9	Adj Sat Flow, veh/h/ln	1885	1885	1885	1885	1885	1885	1885	1885	1885	1885	1885	1885	
Percent Heavy Veh, % 1	Adj Flow Rate, veh/h	78	34	86	135	34	92	160	2608	62	82	1641	57	
September 191 84 212 197 80 216 325 3530 83 176 3403 118	Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	
urfixe On Green 0.18 0.18 0.18 0.18 0.18 0.19 0.19 0.10 0.00 0.00 1.00 1.00 staff Flow, weeh 1 275 473 1197 1282 450 1217 1795 5172 122 1795 5107 177 25 Volume(v), wehh 78 0 120 135 0.128 160 1725 945 82 1102 596 1795 1795 1795 1795 1795 1795 1795 1795	Percent Heavy Veh, %													
Sat Flow, weith 1275 473 1197 1282 450 1217 1795 5172 122 1795 5107 177 37 Qriburney), vehith 78 0 120 135 0 126 160 1725 945 82 1102 996 379 Satt Flow(s), vehith 78 0 120 135 0 126 160 1725 945 82 1102 996 379 Satt Flow(s), vehith 178 0 120 135 0 126 160 1725 945 82 1102 996 379 Satt Flow(s), vehith 178 0 120 1282 0 1666 1795 1716 1863 1795 1716 1883 3 25 0 120 120 120 120 120 120 120 120 120 1	Cap, veh/h				197	80		325			176			
Sign Sign	Arrive On Green	0.18	0.18	0.18	0.18	0.18	0.18	0.09	1.00	1.00	0.06	1.00	1.00	
Sin Sas Flow(s) yeth/hint/27	Sat Flow, veh/h	1275	473	1197	1282	450	1217	1795	5172	122	1795	5107	177	
Serve(g, s), s	Grp Volume(v), veh/h	78	0	120	135	0	126	160	1725	945	82	1102	596	
Sycle O Clearing C.J., s. 17.5	Grp Sat Flow(s), veh/h/ln	1275	0	1670	1282	0	1666	1795	1716	1863	1795	1716	1853	
Top In Lane	Serve(g_s), s	8.1	0.0	8.9	14.6	0.0	9.4	4.2	0.0	0.0	2.1	0.0	0.0	
case Gr (Dac)(c), wehh 191 0 296 197 0 295 325 2342 1272 176 2286 1235 (CR Ratio(X) 0.41 0.09 0.41 0.09 0.43 0.49 0.74 0.74 0.74 0.74 0.74 0.74 0.74 0.74 0.49 0.48 vival Capic (a), vesh 191 0 296 197 0 295 438 2342 1272 218 2286 1235 CLO Methor Ball (a) (3), vesh 0.1 0.0 1.00 1.00 1.00 1.00 1.00 1.00 0.00 1.00 2.00	Cycle Q Clear(g_c), s	17.5	0.0	8.9	23.5	0.0	9.4	4.2	0.0	0.0	2.1	0.0	0.0	
\(\text{\(\text{\(\text{\(\text{\(\text{\(\text{\(\text{\(\text{\\ \text{\} \text{\(\text{\\ \text{\\ \text{\\ \text{\\ \text{\\ \text{\\circ}\\ \text{\\ \text	Prop In Lane	1.00		0.72	1.00		0.73	1.00		0.07	1.00		0.10	
	ane Grp Cap(c), veh/h	191	0	296	197	0	295	325	2342	1272	176	2286	1235	
CAM Prison Ratio	//C Ratio(X)		0.00			0.00								
	Avail Cap(c_a), veh/h	191	0	296	197	0	295	438	2342	1272	318	2286	1235	
Inform Delay (d), siveh 59 1 0.0 51.1 61.5 0.0 51.3 6.2 0.0 0.0 6.7 0.0 0.0 cm or Delay (d), siveh 59 1 0.0 0.3 8.0 0.0 1.4 0.2 0.8 1.5 0.6 0.6 1.2 cm or Delay (d), siveh 0.5 0.0 0.3 8.0 0.0 0.4 0.2 0.8 1.5 0.6 0.6 1.2 cm or Delay (d), siveh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.	HCM Platoon Ratio		1.00		1.00									
nor Delay (22), siveh 0 5 00 03 8.0 0.0 0.4 0.2 0.8 16. 0.6 0.6 1.2 initial Q Delay(33) siveh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.	Jpstream Filter(I)	1.00	0.00	1.00	1.00	0.00	1.00	0.39	0.39	0.39	0.88	0.88	0.88	
nisial C Delayrid3, siveh 0, 0 0 0 0, 0 0, 0 0, 0 0, 0 0, 0 0,														
Mile BackOr()5(07), weh/hr.7				0.3	8.0	0.0	0.4	0.2			0.6			
Unity Unit														
ncfp_Delay(d)skyeh				3.8	5.2	0.0	4.0	1.3	0.3	0.6	0.7	0.2	0.4	
### A A A A A A A A A A A A A A A A A A														
Approach Vol. wehh														
A		E		D	E		D	A		A	A		A	
Coproach LOS														
The Francisco Francisco														
Phs Duration (G-Y-K-R), s.9.9 10.1.1 30.0 11.2 98.8 30.0	Approach LOS		D			Е			Α			Α		
Pits Duration (G+Y-Rc), s.8.9 101.1 30.0 11.2 98.8 30.0 hange Period (Y+Rc), s.5.0 1.5.5 1.5.2 5.0 1.5.5 1.5.2 1.0 kar. Green Setting (Gmath, 8 1.8 1.2 1.5.0 1.0 1.0 kar. Green Setting (Gmath, 8 1.8 1.2 1.0 1.0 1.0 1.0 kar. Green Setting (Gmath, 8 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0	Timer - Assigned Phs	- 1	2		4	5	6		8					
Thange Period (Y+Rc); s 5.0 * 5.5 * 5.2 * 5.2 * 5.2 * 5.2 * 5.2 * 5.2 * 5.2 * 5.2 * 5.2 * 5.2 * 5.3 * 5.2 * 5.2 * 5.3 * 5.3		989												
Asa Green Setting (Gnats 8														
Also Ci Clear Time (g. c+14), 8 20 19.5 6.2 2.0 25.5 Tresene Ext Time (p.c.), s 0.0 9.4 0.1 0.0 4.1 0.0 Testesection Summary CLM 6th CIV Delay 6.4 Kotes Asketes Jear approved pedestrian interval to be less than phase max green. HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.														
Arean Ext Time (p_0), s 0,0 9.4 0.1 0.0 4.1 0.0 Intersection Summary ICM 6th Cht Delay 6.4 ICM 6th Cht Delay A A Iddles Jear approved pedestrian interval to be less than phase max green. ICM 6th Cht Demputational engine requires equal clearance times for the phases crossing the barrier.														
Intersection Summary (CM 6th CNI Delay 6.4 (CM 6th LOS A A A A A A A A A A A A A														
ICM 6th ChT Delay 6.4 (CM 6th LOS A A Votos Jeer approved pedestrian interval to be less than phase max green. HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.	4-7-													
HCM 6th LOS A Notes Ster approved pedestrian interval to be less than phase max green. HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.				6.4										
Jser approved pedestrian interval to be less than phase max green. HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.														
Jser approved pedestrian interval to be less than phase max green. HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.														
HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.		n inte	rval to F	ne lese	than rh	ase ma	y oreer	1						
									ases cri	ossing t	he barr	ier.		
Culebra Road Corridor Study 5:00 pm 12/15/2020 2020 PM Existing Condition Synchro 10 Repo														Synchro 10 Repo

	1	*	1	-	/	↓		
Movement	WBL	WBR	NBT	NBR	SBL	SBT		
Lane Configurations	7	7	ተተተ	7	7	ተተተ		
Traffic Volume (veh/h)	257	114	2100	301	206	1304		
Future Volume (veh/h)	257	114	2100	301	206	1304		
nitial Q (Qb), veh	0	0	0	0	0	0		
Ped-Bike Adj(A_pbT)	1.00	1.00		1.00	1.00			
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00		
Nork Zone On Approac			No			No		
Adj Sat Flow, veh/h/ln	1885	1885	1885	1885	1885	1885		
Adj Flow Rate, veh/h Peak Hour Factor	275 0.98	122	2250 0.98	322 0.98	0.98	1397		
	0.98	0.98	0.98	0.98	0.98	0.98		
Percent Heavy Veh, %	299	266	3302	1025	241	3864		
Cap, veh/h Arrive On Green	0.17	0.17	0.85	0.85	0.14	1.00		
Sat Flow, veh/h	1795	1598	5316	1598	1795			
Grp Volume(v), veh/h	275	122	2250	322	221	1397		
Grp Sat Flow(s), veh/h/li		1598	1716	1598	1795	1716		
2 Serve(q.s), s	21.1	9.6	21.4	5.7	8.0	0.0		
Cycle Q Clear(q_c), s	21.1	9.6	21.4	5.7	8.0	0.0		
Prop In Lane	1.00	1.00	21.7	1.00	1.00	0.0		
ane Grp Cap(c), veh/h		266	3302	1025	241	3864		
//C Ratio(X)	0.92	0.46	0.68	0.31	0.92	0.36		
Avail Cap(c_a), veh/h	527	469	3302	1025	314	3864		
-ICM Platoon Ratio	1.00	1.00	1.33	1.33	2.00	2.00		
Jpstream Filter(I)	1.00	1.00	0.72	0.72	0.85	0.85		
Jniform Delay (d), s/vel	n 57.4	52.7	5.2	4.1	28.5	0.0		
ncr Delay (d2), s/veh	7.0	0.5	0.8	0.6	20.7	0.2		
nitial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0		
%ile BackOfQ(50%),veh	n/1H0.2	3.9	4.1	1.6	8.3	0.1		
Jnsig. Movement Delay								
_nGrp Delay(d),s/veh		53.1	6.1	4.7	49.2	0.2		
_nGrp LOS	E	D	A	A	D	A		
Approach Vol, veh/h	397		2572			1618		
Approach Delay, s/veh	61.0		5.9			6.9		
Approach LOS	Е		Α			Α		
Timer - Assigned Phs	- 1	2				6	8	
Phs Duration (G+Y+Rc)	, \$5.3	95.5				110.8	29.2	
Change Period (Y+Rc),	s* 5.2	* 5.7				5.7	5.9	
Max Green Setting (Gm		* 67				87.3	41.1	
Max Q Clear Time (g_c		23.4				2.0	23.1	
Green Ext Time (p_c), s	0.0	8.4				3.8	0.2	
ntersection Summary								
HCM 6th Ctrl Delay			11.0					
HCM 6th LOS			В					
Notes								
HCM 6th computation	al engi	ne requ	ires equ	ual clea	rance ti	mes for	he phases crossing the barrier.	
Culebra Road Corridor	Study 5	5:00 pm	12/15/	2020 20	020 PM	Fxisting	Condition	Synchro 10 Repo

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SB
Lane Configurations		4	7		4		*	11		*	ተ ቀሴ	
Traffic Volume (vph)	91	5	72	17	12	39	150	2400	18	90	1520	7
Future Volume (vph)	91	5	72	17	12	39	150	2400	18	90	1520	7
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	190
Total Lost time (s)		5.2	5.2		5.0		5.0	6.8		5.0	5.7	
Lane Util. Factor		1.00	1.00		1.00		1.00	0.91		1.00	0.91	
Frt		1.00	0.85		0.92		1.00	1.00		1.00	0.99	
Flt Protected		0.95	1.00		0.99		0.95	1.00		0.95	1.00	
Satd. Flow (prot)		1796	1599		1714		1787	5130		1787	5102	
Flt Permitted		0.63	1.00		0.90		0.10	1.00		0.04	1.00	
Satd. Flow (perm)		1187	1599		1563		191	5130		75	5102	
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.9
Growth Factor (vph)	105%	105%	105%	105%	105%	105%	105%	105%	105%	105%	105%	105
Adj. Flow (vph)	101	6	80	19	13	43	166	2653	20	99	1680	7
RTOR Reduction (vph)	0	0	71	0	35	0	0	1	0	0	3	
Lane Group Flow (vph)	0	107	9	0	40	0	166	2672	0	99	1754	
Heavy Vehicles (%)	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	- 1
Turn Type	Perm	NA	Perm	Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4		4	8			2			6		
Actuated Green, G (s)		15.6	15.6		15.8		107.9	99.1		108.0	99.7	
Effective Green, g (s)		15.6	15.6		15.8		107.9	99.1		108.0	99.7	
Actuated g/C Ratio		0.11	0.11		0.11		0.77	0.71		0.77	0.71	
Clearance Time (s)		5.2	5.2		5.0		5.0	6.8		5.0	5.7	
Vehicle Extension (s)		1.0	1.0		1.0		1.0	1.0		1.0	1.0	
Lane Grp Cap (vph)		132	178		176		247	3631		159	3633	
v/s Ratio Prot							c0.04	c0.52		0.04	0.34	
v/s Ratio Perm		c0.09	0.01		0.03		0.47			0.44		
v/c Ratio		0.81	0.05		0.22		0.67	0.74		0.62	0.48	
Uniform Delay, d1		60.8	55.6		56.5		7.8	12.5		29.4	8.8	
Progression Factor		1.00	1.00		1.00		1.56	1.49		1.35	0.84	
Incremental Delay, d2		28.7	0.0		0.2		3.9	0.9		5.0	0.4	
Delay (s)		89.5	55.6		56.8		16.1	19.5		44.6	7.9	
Level of Service		F	Е		Е		В	В		D	Α	
Approach Delay (s)		75.0			56.8			19.3			9.8	
Approach LOS		Е			Е			В			Α	
Intersection Summary												
HCM 2000 Control Delay			18.4	Н	CM 2000	Level of	Service		В			
HCM 2000 Volume to Capacit	ty ratio		0.74									
Actuated Cycle Length (s)			140.0	Sı	um of lost	time (s)			17.0			
Intersection Capacity Utilization	on		80.7%	IC	U Level o	of Service			D			
Analysis Period (min)			15									
c Critical Lane Group												

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EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBI
	4	7	- 10	î,		- 1	 ቀተር		- 1	 ቀተር	
41	2	70	19	4	28	144	2291	21	27	1500	7
41	2	70	19	4	28	144	2291	21	27	1500	7
1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	190
	5.9	5.9	5.9	5.9		5.0	5.1		5.0	5.5	
	1.00	1.00	1.00	1.00		1.00	0.91		1.00	0.91	
	1.00	0.85	1.00	0.87		1.00	1.00		1.00	0.99	
	0.95	1.00	0.95	1.00		0.95	1.00		0.95	1.00	
	1795	1599	1787	1632		1787	5128		1787	5101	
	0.71	1.00	0.95	1.00		0.10	1.00		0.04	1.00	
	1336	1599	1787	1632		196	5128		78	5101	
0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.9
105%	105%	105%	105%	105%	105%	105%	105%	105%	105%	105%	105
44	2	75	20	4	30	154	2455	22	29	1607	7
0	0	70	0	29	0	0	0	0	0	3	
0	46	5	20	5	0	154	2478	0	29	1679	
1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	19
Perm	NA	Perm	Split	NA		pm+pt	NA		pm+pt	NA	
	4		3	3			2		1	6	
4		4				2			6		
	8.7	8.7	5.1	5.1		109.3	101.7		98.6	96.0	
	8.7	8.7	5.1	5.1		109.3	101.7		98.6	96.0	
		0.06		0.04						0.69	
	5.9	5.9	5.9	5.9		5.0	5.1		5.0	5.5	
	1.0	1.0	1.0	1.0		1.0	1.0		1.0	1.0	
	83	99	65	59		242	3725		86	3497	
			c0.01	0.00		c0.04	c0.48		0.01	0.33	
	c0.03	0.00				0.46			0.23		
	0.55	0.05	0.31	0.09		0.64	0.67		0.34	0.48	
	63.8	61.8	65.7	65.2		8.3	10.1		10.2	10.3	
	1.00	1.00	1.00	1.00		3.02	0.41		1.75	0.70	
	4.5	0.1	1.0	0.2		2.8	0.7		0.8	0.4	
	68.2	61.8	66.7	65.4		27.8	4.8		18.6	7.6	
	Е	E	Е	Е		C	Α		В	A	
	64.3			65.9			6.1			7.8	
	E			Е			Α			A	
		9.0		CM 2000	Laural	Camilas		A			
		0.66	п	CIVI 2000	Level 01	Service		А			
								22.3			
ratio			0		Care (a)						
		140.0		um of lost							
ratio				um of lost U Level o		8		22.3 C			
	41 41 1900 0.98 105% 44 0 0 1%	4 11 2 1900 1900 1900 1900 1900 1900 1900	41	44 7 7 7 7 7 7 7 7 7	14	14	14	Bell Bell	EBL EBT EBR WBL WBT WBR NBL NBT NBR NBT NBT NBR NBT NBT NBR NBT BBL EBT EBR WBL WBT WBR NBL NBT NBR SBL WBT WBR NBL NBT NBR SBL MBT MBR NBL NBT NBR SBL MBT MBR MBL MBT EBL EBT EBR WBL WBT WBR NBL NBT NBR SBL SBT MBL NBT		
	•	-	•	1	—	•	4	1	1	1	Į.
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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
ane Configurations	- 1	ቀ ቀሴ		*	ቀ ቀሴ			4		*	4
Fraffic Volume (vph)	183	1271	18	31	1752	386	10	7	18	250	18
uture Volume (vph)	183	1271	18	31	1752	386	10	7	18	250	18
deal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.2	5.7		5.2	5.7			5.9		5.9	5.9
ane Util. Factor	1.00	0.91		1.00	0.91			1.00		0.95	0.95
rt	1.00	1.00		1.00	0.97			0.93		1.00	0.91
Flt Protected	0.95	1.00		0.95	1.00			0.99		0.95	0.98
Satd. Flow (prot)	1787	5125		1787	4997			1727		1698	1605
Flt Permitted	0.95	1.00		0.95	1.00			0.99		0.95	0.98
Satd. Flow (perm)	1787	5125		1787	4997			1727		1698	1605
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Growth Factor (vph)	105%	105%	105%	105%	105%	105%	105%	105%	105%	105%	105%
Adj. Flow (vph)	207	1435	20	35	1978	436	11	8	20	282	20
RTOR Reduction (vph)	0	1	0	0	21	0	0	19	0	0	36
ane Group Flow (vph)	207	1454	0	35	2393	0	0	20	0	217	168
Heavy Vehicles (%)	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%
Turn Type	Prot	NA		Prot	NA		Split	NA		Split	NA
Protected Phases	1	6		5	2		4	4		3	3
Permitted Phases											
Actuated Green, G (s)	19.9	89.4		5.5	75.0			5.3		17.1	17.1
Effective Green, g (s)	19.9	89.4		5.5	75.0			5.3		17.1	17.1
Actuated g/C Ratio	0.14	0.64		0.04	0.54			0.04		0.12	0.12
Clearance Time (s)	5.2	5.7		5.2	5.7			5.9		5.9	5.9
Vehicle Extension (s)	1.0	1.0		1.0	1.0			1.0		1.0	1.0
ane Grp Cap (vph)	254	3272		70	2676			65		207	196
//s Ratio Prot	c0.12	0.28		0.02	c0.48			c0.01		c0.13	0.10
//s Ratio Perm											
//c Ratio	0.81	0.44		0.50	0.89			0.30		1.05	0.86
Jniform Delay, d1	58.3	12.8		65.9	29.0			65.6		61.4	60.3
Progression Factor	0.80	1.07		0.96	0.53			1.00		1.00	1.00
ncremental Delay, d2	11.2	0.3		1.4	3.7			1.0		75.8	28.0
Delay (s)	57.7	14.0		64.9	19.1			66.5		137.3	88.3
_evel of Service	E	В		Е	В			Е		F	F
Approach Delay (s)		19.4			19.7			66.5			113.5
Approach LOS		В			В			Е			F
ntersection Summary											
			28.7	н	CM 2000	Level of	Service				
	city ratio		0.88	- 11	J.M 2000	23101011	OUI VICE		0		
	,			S	um of lost	time (s)			22.7		
	tion										
					70 201011	J. 00. 1.00					
ntersection Summary HCM 2000 Control Delay HCM 2000 Volume to Capac Actuated Cycle Length (s) ntersection Capacity Utilizat Analysis Period (min)	,	В	28.7 0.88 140.0 87.0%	S	CM 2000 um of lost	time (s)		E	C 22.7 E		-

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SE
Lane Configurations		*			*			414	7	36	ĵ.	
Traffic Volume (vph)	21	950	413	335	1488	25	786	15	574	9	35	
Future Volume (vph)	21	950	413	335	1488	25	786	15	574	9	35	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	19
Total Lost time (s)	5.2	6.2		5.2	5.8		6.1	6.1	6.1	5.9	5.9	
Lane Util. Factor	1.00	0.91		1.00	0.91		0.91	0.91	1.00	1.00	1.00	
Frt	1.00	0.95		1.00	1.00		1.00	1.00	0.85	1.00	0.96	
Flt Protected	0.95	1.00		0.95	1.00		0.95	0.95	1.00	0.95	1.00	
Satd. Flow (prot)	1787	4902		1787	5123		1626	3266	1599	1787	1813	
FIt Permitted	0.13	1.00		0.07	1.00		0.95	0.95	1.00	0.95	1.00	
Satd. Flow (perm)	248	4902		140	5123		1626	3266	1599	1787	1813	
Peak-hour factor, PHF	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.
Growth Factor (vph)	105%	105%	105%	105%	105%	105%	105%	105%	105%	105%	105%	10
Adj. Flow (vph)	22	1018	442	359	1594	27	842	16	615	10	38	
RTOR Reduction (vph)	0	54	0	0	1	0	0	0	381	0	9	
Lane Group Flow (vph)	23	1407	0	359	1620	0	421	437	234	10	41	
Heavy Vehicles (%)	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	
Turn Type	pm+pt	NA		pm+pt	NA		Split	NA	Perm	Split	NA	
Protected Phases	1	6		5	2		4	4		3	3	
Permitted Phases	6			2					4			
Actuated Green, G (s)	51.1	48.3		80.5	72.5		35.7	35.7	35.7	6.0	6.0	
Effective Green, g (s)	51.1	48.3		80.5	72.5		35.7	35.7	35.7	6.0	6.0	
Actuated g/C Ratio	0.37	0.34		0.58	0.52		0.26	0.26	0.26	0.04	0.04	
Clearance Time (s)	5.2	6.2		5.2	5.8		6.1	6.1	6.1	5.9	5.9	
Vehicle Extension (s)	1.0	1.0		1.0	1.0		1.0	1.0	1.0	1.0	1.0	
Lane Grp Cap (vph)	121	1691		393	2652		414	832	407	76	77	
v/s Ratio Prot	0.00	0.29		c0.17	0.32		c0.26	0.13		0.01	c0.02	
v/s Ratio Perm	0.07			c0.35					0.15			
v/c Ratio	0.19	0.83		0.91	0.61		1.02	0.97dl	0.58	0.13	0.54	
Uniform Delay, d1	28.6	42.1		43.5	23.8		52.1	44.9	45.5	64.5	65.6	
Progression Factor	1.07	1.33		0.85	0.39		1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	0.3	4.6		13.8	0.5		48.6	0.3	1.2	0.3	3.6	
Delay (s)	31.0	60.8		50.8	9.7		100.7	45.1	46.8	64.8	69.2	
Level of Service	С	E		D	Α		F	D	D	Е	E	
Approach Delay (s)	-	60.3			17.2			61.7	_	_	68.5	
Approach LOS		E			В			E			E	
ntersection Summary												
HCM 2000 Control Delay			43.7	Н	CM 2000	Level of	Service		D			
HCM 2000 Volume to Capa	acity ratio		0.95									
Actuated Cycle Length (s)			140.0		um of lost				23.4			
Intersection Capacity Utiliza Analysis Period (min)	ation		92.6% 15	IC	CU Level of	of Service			F			
Defacto Left Lane. Rec Critical Lane Group	code with 1	though la		eft lane.								
Culebra Road Corridor Stud	dv 5:00 nm	12/15/202	on 2020 E	OM Evictin	a Conditi	20				0,	nchro 10	Don

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	Ĭ	† 1>		7	***			ની	7		4		
Traffic Volume (veh/h)	- 1	1583	27	348	2350	0	64	5	195	3	5	5	
Future Volume (veh/h)	- 1	1583	27	348	2350	0	64	5	195	3	5	5	
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0	
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Work Zone On Approach	1	No			No			No			No		
Adj Sat Flow, veh/h/ln	1885	1885	1885	1885	1885	0	1885	1885	1885	1885	1885	1885	
Adj Flow Rate, veh/h	- 1	1714	29	377	2544	0	69	5	211	3	5	5	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	
Percent Heavy Veh, %	1	1	1	1	1	0	- 1	1	1	- 1	1	1	
Cap, veh/h	117	2343	40	325	3859	0	238	16	220	65	104	86	
Arrive On Green	0.00	0.65	0.65	0.10	0.75	0.00	0.14	0.14	0.14	0.14	0.14	0.14	
Sat Flow, veh/h	1795	3604	61	1795	5316	0	1363	113	1598	245	754	625	
Grp Volume(v), veh/h	- 1	850	893	377	2544	0	74	0	211	13	0	0	
Grp Sat Flow(s),veh/h/ln	1795	1791	1874	1795	1716	0	1476	0	1598	1624	0	0	
Q Serve(q_s), s	0.0	44.3	44.6	14.1	34.2	0.0	4.1	0.0	18.4	0.0	0.0	0.0	
Cycle Q Clear(g_c), s	0.0	44.3	44.6	14.1	34.2	0.0	6.0	0.0	18.4	0.9	0.0	0.0	
Prop In Lane	1.00		0.03	1.00		0.00	0.93		1.00	0.23		0.38	
Lane Grp Cap(c), veh/h	117	1164	1218	325	3859	0	253	0	220	256	0	0	
	0.01	0.73	0.73	1.16	0.66	0.00	0.29	0.00	0.96	0.05	0.00	0.00	
Avail Cap(c_a), veh/h	296	1164	1218	325	3859	0	253	0	220	256	0	0	
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Upstream Filter(I)	0.77	0.77	0.77	1.00	1.00	0.00	1.00	0.00	1.00	1.00	0.00	0.00	
Uniform Delay (d), s/veh	10.0	16.3	16.4	38.6	8.7	0.0	54.5	0.0	59.9	52.4	0.0	0.0	
Incr Delay (d2), s/veh	0.0	3.1	3.0	101.1	0.9	0.0	0.2	0.0	48.4	0.0	0.0	0.0	
Initial Q Delay(d3),s/veh		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
%ile BackOfQ(50%),veh.		17.3	18.2	16.4	10.7	0.0	2.4	0.0	10.4	0.4	0.0	0.0	
Unsig. Movement Delay,													
	10.0	19.5	19.4	139.8	9.6	0.0	54.7	0.0	108.3	52.4	0.0	0.0	
LnGrp LOS	В	В	В	F	A	A	D	A	F	D	A	A	
Approach Vol, veh/h		1744			2921			285			13		
Approach Delay, s/veh		19.4			26.4			94.4			52.4		
Approach LOS		В			С			F			D		
Timer - Assigned Phs	- 1	2		4	5	6		8					
Phs Duration (G+Y+Rc),	s5.0	110.0		25.0	19.0	96.0		25.0					
Change Period (Y+Rc),		* 5		* 5.7	* 4.9	* 5		* 5.7					
Max Green Setting (Gma		* 91		* 19	* 14	* 91		* 19					
Max Q Clear Time (g. c+	112.Gs	36.2		20.4	16.1	46.6		2.9					
Green Ext Time (p_c), s		10.7		0.0	0.0	4.0		0.0					
Intersection Summary													
HCM 6th Ctrl Delay			27.9										
HCM 6th LOS			С										
Notes													
User approved pedestria * HCM 6th computationa								ases cr	ossing t	he barr	ier.		
Culebra Road Corridor S													Synchro 10 Rep

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Movement	•	-	7	1	—	•	1	†	/	/	↓	4
	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SB
Lane Configurations	Jan.	414			** 1		100	414	7			
Traffic Volume (vph)	900	655	0	0	1250	120	568	475	276	0	0	
Future Volume (vph)	900	655	0	0	1250	120	568	475	276	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	190
Total Lost time (s)	4.5	4.5			4.9		5.7	5.7	5.7			
Lane Util. Factor	0.91	0.91			0.91		0.91	0.91	1.00			
Frt	1.00	1.00			0.99		1.00	1.00	0.85			
Flt Protected	0.95	0.98			1.00		0.95	0.98	1.00			
Satd. Flow (prot)	1626	3360			5068		1626	3369	1599			
Flt Permitted	0.95	0.59			1.00		0.95	0.98	1.00			
Satd. Flow (perm)	1626	2005			5068		1626	3369	1599			
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.9
Growth Factor (vph)	105%	105%	105%	105%	105%	105%	105%	105%	105%	105%	105%	105
Adj. Flow (vph)	974	709	0	0	1353	130	615	514	299	0	0	
RTOR Reduction (vph)	0	0	0	0	8	0	0	0	202	0	0	
Lane Group Flow (vph)	545	1138	0	0	1475	0	369	760	97	0	0	
Heavy Vehicles (%)	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	- 1
Turn Type	Prot	NA			NA		Split	NA	Perm			
Protected Phases	1 11	1 2 11			2		4 14	4 14				
Permitted Phases									4 14			
Actuated Green, G (s)	59.0	92.1			33.1		33.3	33.3	33.3			
Effective Green, g (s)	59.0	92.1			33.1		33.3	33.3	33.3			
Actuated g/C Ratio	0.42	0.66			0.24		0.24	0.24	0.24			
Clearance Time (s)					4.9							
Vehicle Extension (s)					1.0							
Lane Gro Cap (vph)	685	1890			1198		386	801	380			
v/s Ratio Prot	c0.34	0.25			c0.29		c0.23	0.23				
v/s Ratio Perm		0.14							0.06			
v/c Ratio	0.80	0.60			1.23		0.96	0.95	0.26			
Uniform Delay, d1	35.3	13.6			53.5		52.6	52.5	43.3			
Progression Factor	0.23	0.19			0.82		1.00	1.00	1.00			
Incremental Delay, d2	2.5	0.2			109.9		34.0	19.9	0.1			
Delay (s)	10.4	2.7			153.9		86.6	72.4	43.4			
Level of Service	В	Α			F		F	Е	D			
Approach Delay (s)		5.2			153.9			70.0			0.0	
Approach LOS		Α			F			E			Α	
Intersection Summary												
HCM 2000 Control Delay			73.4	Н	CM 2000	Level of	Service		Е			
HCM 2000 Volume to Capaci	ity ratio		1.11									
Actuated Cycle Length (s)	,		140.0	S	um of lost	time (s)			32.5			
	on		145.1%		U Level				Н			
Intersection Capacity Utilizati			15									

2 333 72 2 333 72 0 1900 1900 1 6.1 6.6 5 1.00 0.91 0 0.85 1.00 0 1.00 0.95 4 1599 1626 0 1.00 0.95 4 1599 1626 0 1.00 0.95 4 1599 1626 0 1.05% 105% 105% 0 360 78 0 360 78	7 33 72 37 33 37 2 37 30 190 190 190 190 190 190 190 190 190 19	78 2 78 2 78 2 900 190 99 900 99 900 98 97 0.9
2 333 72 2 333 72 2 1900 1900 1 6.1 6.6 5 1.00 0.91 0 0.85 1.00 0 1.00 0.95 4 1599 1626 0 1.00 0.95 4 1599 1626 0 1.00 0.95 4 1599 1626 0 1.05 105% 105% 0 360 78	33 72 37 33 72 37 30 1900 1900 1900 1:1 6.6 6 00 0.91 0.9 55 1.00 0.9 00 0.95 1.0 109 1626 338 00 0.95 1.0 109 1626 338 00 0.95 1.0 109 1626 338 00 0.95 1.0 109 1626 338 109 1626 338	78 278 278 278 278 278 278 278 278 278 2
2 333 72 2 333 72 2 1900 1900 1 6.1 6.6 5 1.00 0.91 0 0.85 1.00 0 1.00 0.95 4 1599 1626 0 1.00 0.95 4 1599 1626 0 1.00 0.95 4 1599 1626 0 1.05 105% 105% 0 360 78	33 72 37 33 72 37 30 1900 1900 1900 1:1 6.6 6 00 0.91 0.9 55 1.00 0.9 00 0.95 1.0 109 1626 338 00 0.95 1.0 109 1626 338 00 0.95 1.0 109 1626 338 00 0.95 1.0 109 1626 338 109 1626 338	78 278 278 278 278 278 278 278 278 278 2
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5750: Loop 410 SBI		иоы									02/0	
	•	\rightarrow	*	1	—	•	1	†	/	-	Ţ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	S
Lane Configurations		ተ ተተ	7	- 1	414					7	414	
Traffic Volume (vph)	0	1383	473	450	1380	0	0	0	0	173	655	8
Future Volume (vph)	0	1383	473	450	1380	0	0	0	0	173	655	8
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	19
Total Lost time (s)		4.9	4.9	4.8	4.8					5.9	5.9	
ane Util, Factor		0.91	1.00	0.91	0.91					0.91	0.91	1
rt		1.00	0.85	1.00	1.00					1.00	1.00	0
It Protected		1.00	1.00	0.95	1.00					0.95	1.00	1
Satd. Flow (prot)		5136	1599	1626	3418					1626	3419	1
It Permitted		1.00	1.00	0.95	0.72					0.95	1.00	1
Satd. Flow (perm)		5136	1599	1626	2478					1626	3419	15
Peak-hour factor, PHF	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0
Growth Factor (vph)	105%	105%	105%	105%	105%	105%	105%	105%	105%	105%	105%	10
Adj. Flow (vph)	0	1482	507	482	1479	0	0	0	0	185	702	10
RTOR Reduction (vph)	0	0	108	0	0	0	0	0	0	0	0	
ane Group Flow (vph)	0	1482	399	434	1527	0	0	0	0	166	721	
Heavy Vehicles (%)	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	
Furn Type	1/0	NA	Perm	Prot	NA.	1/0	170	1/0	1/0	Split	NA.	Pe
Protected Phases		6	reiiii	5 15	5 6 15					8 18	8 18	F
Permitted Phases		0	6	5 15	5015					0 10	0 10	8
		42.1	42.1	54.3	96.4					28.0	28.0	2
Actuated Green, G (s)		42.1	42.1		96.4					28.0		
Effective Green, g (s)		0.30	0.30	54.3 0.39	0.69						28.0	2
Actuated g/C Ratio Clearance Time (s)		4.9	4.9	0.39	0.69					0.20	0.20	0
		1.0	1.0									
/ehicle Extension (s)		1544	480	630	2070					325	683	- ;
ane Grp Cap (vph)		c0.29	480	0.27	c0.29					0.10	0.21	
		CU.29		0.27						0.10	0.21	
/s Ratio Perm		0.96	0.25	0.69	0.22					0.51	1.06	c0
/c Ratio		48.1	45.6	35.8	13.8					49.9	56.0	5
Jniform Delay, d1												
Progression Factor		1.25	1.46	0.33	0.30					1.00	1.00	1
ncremental Delay, d2		7.1	6.1	0.2	0.1 4.2					0.6	50.1	59
Delay (s)		67.2 E	72.5 E	12.2						50.5	106.1 F	64
_evel of Service			E	В	Α					D		
Approach Delay (s)		68.6			6.0			0.0			372.6	
Approach LOS		Е			Α			Α			F	
ntersection Summary												
HCM 2000 Control Delay			141.8	H	ICM 2000	Level of S	Service		F			
HCM 2000 Volume to Capaci	ty ratio		1.34									
Actuated Cycle Length (s)			140.0		ium of lost				32.5			
ntersection Capacity Utilizati Analysis Period (min)	on		145.1% 15	I	CU Level	of Service			Н			
Analysis Period (min) c Critical Lane Group			15									
Culebra Road Corridor Study WSP - San Antonio	5:00 pm	12/15/202	0 2020 F	M Existin	ng Conditi	on				S	nchro 10	Rep

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EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SB
*	† †	17	*	**		*	†	7	7	A	
	1000			980	200	131			186		11
103	1000	11	8	980	200	131	203	25	186	13	11
1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	190
4.4	5.3	5.3	4.4	4.9		5.2	5.2	5.2	5.1	5.1	
1.00	0.95	1.00	1.00	0.95		1.00	1.00	1.00	1.00	1.00	
1.00	1.00	0.85	1.00	0.97		1.00	1.00	0.85	1.00	0.87	
0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	
1752		1568	1752	3416		1752	1845	1568	1752	1596	
0.06	1.00	1.00	0.15	1.00		0.95	1.00	1.00	0.95	1.00	
108	3505	1568	268	3416		1752	1845	1568	1752	1596	
0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	3.0
106%	106%	106%	106%	106%	106%	106%	106%	106%	106%	106%	106
127	1233	14	10	1208	247	161	250	31	229	16	14
0	0	7	0	9	0	0	0	26	0	121	
127	1233	7	10	1446	0	161	250	5	229	38	
3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3
pm+pt	NA	Perm	pm+pt	NA		Split	NA	Perm	Split	NA	
1	6		5	2		4	4		3	3	
		6						4			
78.6	73.2	73.2	65.5	64.5		24.0	24.0	24.0	21.8	21.8	
78.6	73.2	73.2	65.5	64.5		24.0	24.0	24.0	21.8	21.8	
0.56	0.52	0.52	0.47	0.46		0.17	0.17	0.17	0.16	0.16	
1.0		2.0						2.0			
		819						268			
	0.35			c0.42		0.09	c0.14		c0.13	0.02	
E		В	С			D		D	E		
	C			D			E			É	
		41.3	Н	CM 2000	Level of	Service		D			
ty ratio		0.86									
		140.0	Sı	um of lost	time (s)			20.0			
y rado					of Service			D			
on		80.1%	IC								
		80.1% 15	IC	O Level (// CO! 1100						
	103 103 1900 4.4 1.00 1.00 0.95 1752 0.06 108 0.86 106% 127 3% pm+pt 1 6 78.6	103 1000 1900 1900 1900 1900 1900 1900 1	103 1000 11 1900 11 1900 1900 1900 1900 1900	103 1000 11 8 1900 190 190 190 190 190 190 190 190 190	103 1000 11 8 890 1900 1910 1900 1910 1900 1910 190	103 1000	103 1000 11 8 990 200 131 1900 1900 1900 1900 1900 1900 1900 144 5.3 5.3 4.4 4.9 5.2 1.00 0.95 1.00 1.00 0.95 1.00 1.05 1.00 1.00 0.95 1.00 0.95 1.00 1.00 0.95 1.00 0.95 1.00 1.00 0.95 1.00 0.95 1.00 1.00 0.95 1.00 0.95 1.00 1.00 0.95 1.00 0.95 1.00 1.00 0.95 1.00 0.95 1.00 1.00 0.95 1.00 0.95 1.00 1.00 0.95 1.00 0.95 1.00 1.00 0.95 1.00 0.95 1.00 1.00 0.95 1.00 0.95 1.00 1.00 0.95 1.00 0.95 1.00 1.00 0.95 1.00 0.95 1.00 1.00 0.95 1.00 0.95 1.00 1.00 0.95 1.00 0.95 1.00 1.00 0.95 1.00 0.95 1.00 1.00 0.95 1.00 0.95 1.00 0.05 1.00 0.95 1.00 1.00 0.05 1.00 0.95 1.00 1.00 0.05 1.00 0.95 1.00 1.00 0.05 1.00 0.95 1.00 0.05 1.00 0.05 0.86	103 1000	1003 1000	103 1000	103 1000

	٠	-	•	*	-	4		
Movement	EBL	EBT	WBT	WBR	SBL	SBR		
ane Configurations		414	ΦÞ			7"		
Fraffic Volume (vph)	14	920	1100	61	123	107		
uture Volume (vph)	14	920	1100	61	123	107		
deal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Total Lost time (s)		5.3	5.3		5.0	5.0		
ane Util. Factor		0.95	0.95		1.00	1.00		
rt		1.00	0.99		1.00	0.85		
It Protected		1.00	1.00		0.95	1.00		
Satd. Flow (prot)		3502	3477		1752	1568		
It Permitted		0.91	1.00		0.95	1.00		
Satd. Flow (perm)		3192	3477		1752	1568		
Peak-hour factor, PHF	0.86	0.86	0.86	0.86	0.86	0.86		
Growth Factor (vph)	106%	106%	106%	106%	106%	106%		
Adj. Flow (vph)	17	1134	1356	75	152	132		
RTOR Reduction (vph)	0	0	2	0	0	28		
ane Group Flow (vph)	0	1151	1429	0	152	104		
leavy Vehicles (%)	3%	3%	3%	3%	3%	3%		
Furn Type	Perm	NA	NA		Prot	Perm		
Protected Phases		6 26	2 22		8 28			
Permitted Phases	6 26					8 28		
Actuated Green, G (s)		98.5	98.5		20.9	20.9		
Effective Green, g (s)		98.5	98.5		20.9	20.9		
Actuated g/C Ratio		0.70	0.70		0.15	0.15		
Clearance Time (s)								
/ehicle Extension (s)								
ane Grp Cap (vph)		2245	2446		261	234		
/s Ratio Prot		22.10	c0.41		c0.09	201		
//s Ratio Perm		0.36	00.11		00.00	0.07		
/c Ratio		0.51	0.58		0.58	0.44		
Jniform Delay, d1		9.6	10.4		55.5	54.3		
Progression Factor		0.68	1.51		1.00	1.00		
ncremental Delay, d2		0.1	0.2		2.1	0.5		
Delay (s)		6.6	16.0		57.6	54.7		
evel of Service		Α.	В		57.0	D D		
Approach Delay (s)		6.6	16.0		56.3	D		
Approach LOS		Α.	B		50.5 E			
ntersection Summary								
HCM 2000 Control Delay			16.2	H	CM 2000	Level of Service	В	
HCM 2000 Volume to Capac	ity ratio		0.58					
Actuated Cycle Length (s)			140.0		um of lost		20.6	
ntersection Capacity Utilizat	ion		53.3%	IC	U Level o	of Service	A	
Analysis Period (min)			15					
c Critical Lane Group								

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	Ĭ	† 1>		٦	44			4			4		
Traffic Volume (veh/h)	62	1000	18	23	1000	98	- 11	25	18	144	48	98	
Future Volume (veh/h)	62	1000	18	23	1000	98	11	25	18	144	48	98	
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0	
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Work Zone On Approach		No			No			No			No		
Adj Sat Flow, veh/h/ln		1885	1885	1885	1885	1885	1885	1885	1885	1885	1885	1885	
Adj Flow Rate, veh/h	68	1094	20	25	1094	107	12	27	20	158	52	107	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	
Percent Heavy Veh, %	1	1	1	1	1	1	1	1	1	1	1	1	
Cap, veh/h	259	1264	23	615	2036	199	88	189	123	214	58	116	
Arrive On Green	0.01	0.12	0.12	0.60	1.00	1.00	0.22	0.22	0.22	0.22	0.22	0.22	
	1795		66	1795	3296	322	233	853	557	762	264	523	
Grp Volume(v), veh/h	68	544	570	25	594	607	59	0	0	317	0	0	
Grp Sat Flow(s),veh/h/ln	1795		1873	1795	1791	1827	1643	0	0	1549	0	0	
Q Serve(g_s), s	3.2	35.8	35.9	0.0	0.0	0.0	0.0	0.0	0.0	20.8	0.0	0.0	
Cycle Q Clear(g_c), s	3.2	35.8	35.9	0.0	0.0	0.0	3.2	0.0	0.0	24.0	0.0	0.0	
Prop In Lane	1.00		0.04	1.00		0.18	0.20		0.34	0.50		0.34	
Lane Grp Cap(c), veh/h		629	658	615	1106	1128	400	0	0	388	0	0	
V/C Ratio(X)	0.26	0.87	0.87	0.04	0.54	0.54	0.15	0.00	0.00	0.82	0.00	0.00	
Avail Cap(c_a), veh/h	348	849	888	615	1106	1128	551	0	0	529	0	0	
HCM Platoon Ratio	0.33	0.33	0.33	2.00	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00	
Upstream Filter(I)	0.84	0.84	0.84	1.00	1.00	1.00	1.00	0.00	0.00	1.00	0.00	0.00	
Uniform Delay (d), s/veh		50.3	50.3	15.7	0.0	0.0	37.6	0.0	0.0	45.4	0.0	0.0	
Incr Delay (d2), s/veh	0.2	12.7	12.3	0.0	1.9	1.8	0.1	0.0	0.0	5.1	0.0	0.0	
Initial Q Delay(d3),s/veh		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
%ile BackOfQ(50%),veh		19.4	20.2	0.3	0.6	0.6	1.4	0.0	0.0	9.7	0.0	0.0	
Unsig. Movement Delay			00 =										
	28.8	63.0	62.5	15.7	1.9	1.8	37.7	0.0	0.0	50.6	0.0	0.0	
LnGrp LOS	С	Е	E	В	A	A	D	A	A	D	A	A	
Approach Vol, veh/h		1182			1226			59			317		
Approach Delay, s/veh		60.8			2.1			37.7			50.6		
Approach LOS		Е			Α			D			D		
Timer - Assigned Phs	- 1	2		4	5	6		8					
Phs Duration (G+Y+Rc).	s9 N	79.2		31.8	41.0	47.3		31.8					
Change Period (Y+Rc),		* 5.1		* 5.2	* 5.1	* 5.1		* 5.2					
Max Green Setting (Gma		* 57		* 38	* 11	* 57		* 38					
Max Q Clear Time (q. c+		2.0		5.2	2.0	37.9		26.0					
Green Ext Time (p c), s		5.5		0.1	0.0	4.3		0.6					
0-7													
Intersection Summary HCM 6th Ctrl Delay			33.3										
HCM 6th LOS			33.3 C										
Notes													
* HCM 6th computationa	ıl engii	ne requ	ires eq	ual clea	rance ti	mes for	the ph	ases cr	ossing t	he barr	ier.		
Culebra Road Corridor S													Synchro 10 Re

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Movement	FBI	EBT	WBT	WBR	SBL	SBR		
Lane Configurations	*	44	† 12	TTDIX	'V	ODIT		
Traffic Volume (vph)	60	1125	1100	39	22	76		
Future Volume (vph)	60	1125	1100	39	22	76		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Total Lost time (s)	4.4	5.1	5.1	1300	5.2	1300		
Lane Util. Factor	1.00	0.95	0.95		1.00			
Earle Uill. Factor	1.00	1.00	0.99		0.89			
Fit Protected	0.95	1.00	1.00		0.03			
Satd, Flow (prot)	1787	3574	3556		1665			
Flt Permitted	0.15	1.00	1.00		0.99			
Satd. Flow (perm)	275	3574	3556		1665			
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91		
Growth Factor (vph)	105%	105%	105%	105%	105%	105%		
Adj. Flow (vph)	69	1298	1269	45	25	88		
RTOR Reduction (vph)	0	0	2	0	75	0		
Lane Group Flow (vph)	69	1298	1312	0	38	0		
Heavy Vehicles (%)	1%	1%	1%	1%	1%	1%		
Turn Type	pm+pt	NA	NA		Prot			
Protected Phases	1	6 26	2 22		8 28			
Permitted Phases	6 26							
Actuated Green, G (s)	81.9	81.2	73.8		18.2			
Effective Green, g (s)	81.9	81.2	73.8		18.2			
Actuated g/C Ratio	0.68	0.68	0.61		0.15			
Clearance Time (s)	4.4							
Vehicle Extension (s)	1.0							
Lane Grp Cap (vph)	225	2418	2186		252			
v/s Ratio Prot	0.01	c0.36	c0.37		c0.02			
v/s Ratio Perm	0.20	00.00	00.07		00.02			
v/c Ratio	0.20	0.54	0.60		0.15			
Uniform Delay, d1	19.3	9.9	14.1		44.2			
Progression Factor	1.00	1.00	1.12		1.00			
Incremental Delay, d2	0.3	0.1	0.3		0.1			
Delay (s)	19.6	10.0	16.1		44.3			
Level of Service	15.0 B	Α	В		44.3 D			
Approach Delay (s)	ь	10.5	16.1		44.3			
Approach LOS		10.5 B	10.1 B		44.3 D			
Approach LOS		D	D		U			
Intersection Summary								
HCM 2000 Control Delay			14.5	H	CM 2000	Level of Service	В	
HCM 2000 Volume to Capa	city ratio		0.52					
Actuated Cycle Length (s)			120.0		um of lost		25.0	
Intersection Capacity Utiliza	tion		59.0%	IC	U Level o	of Service	В	
Analysis Period (min)			15					
c Critical Lane Group								
Culebra Road Corridor Stud	lv 5:00 pm	12/15/202	20 2020 P	'M Existin	a Conditio	on		Synchro 10 Rep

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Movement	EBT	EBR	WBL	WBT	NBL	NBR		
Lane Configurations	† ‡		7	44	¥/f			
Traffic Volume (vph)	1120	34	74	1300	44	51		
Future Volume (vph)	1120	34	74	1300	44	51		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Total Lost time (s)	5.4		5.4	5.4	5.2			
Lane Util. Factor	0.95		1.00	0.95	1.00			
Frt	1.00		1.00	1.00	0.93			
Flt Protected	1.00		0.95	1.00	0.98			
Satd. Flow (prot)	3559		1787	3574	1705			
Flt Permitted	1.00		0.19	1.00	0.98			
Satd. Flow (perm)	3559		356	3574	1705			
Peak-hour factor, PHF	0.98	0.98	0.98	0.98	0.98	0.98		
Growth Factor (vph)	105%	105%	105%	105%	105%	105%		
Adj. Flow (vph)	1200	36	79	1393	47	55		
RTOR Reduction (vph)	1	0	0	0	40	0		
Lane Group Flow (vph)	1235	0	79	1393	62	0		
Heavy Vehicles (%)	1%	1%	1%	1%	1%	1%		
Turn Type	NA		Perm	NA	Prot			
Protected Phases	6 26			2 22	4 24			
Permitted Phases			2 22					
Actuated Green, G (s)	85.7		85.7	85.7	13.1			
Effective Green, g (s)	85.7		85.7	85.7	13.1			
Actuated g/C Ratio	0.71		0.71	0.71	0.11			
Clearance Time (s)								
Vehicle Extension (s)								
Lane Grp Cap (vph)	2541		254	2552	186			
v/s Ratio Prot	0.35			c0.39	c0.04			
v/s Ratio Perm			0.22					
v/c Ratio	0.49		0.31	0.55	0.33			
Uniform Delay, d1	7.5		6.3	8.0	49.4			
Progression Factor	0.47		1.37	1.33	1.00			
Incremental Delay, d2	0.1		0.2	0.1	0.4			
Delay (s)	3.6		8.8	10.7	49.8			
Level of Service	A		Α	В	D			
Approach Delay (s)	3.6 A			10.6 B	49.8 D			
Approach LOS	А			В	D			
Intersection Summary								
HCM 2000 Control Delay			8.9	H	CM 2000	Level of Service	A	
HCM 2000 Volume to Capa	acity ratio		0.52					
Actuated Cycle Length (s)			120.0		um of lost		21.2	
Intersection Capacity Utiliza	ation		61.2%	IC	U Level o	of Service	В	
Analysis Period (min)			15					
c Critical Lane Group								
Culebra Road Corridor Stud								

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SB
Lane Configurations	-	个个 ()		-	titi		-	1	7	-	↑ ₽	
Traffic Volume (vph)	39	1500	167	160	1850	34	190	176	220	49	182	8
Future Volume (vph)	39	1500	167	160	1850	34	190	176	220	49	182	- 1
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	19
Total Lost time (s)	5.0	4.4		5.0	13.4		4.4	6.0	6.0	4.4	6.4	
Lane Util. Factor	1.00	0.91		1.00	0.86		1.00	1.00	1.00	1.00	0.95	
Frt	1.00	0.98		1.00	1.00		1.00	1.00	0.85	1.00	0.95	
Fit Protected	0.95	1.00		0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd, Flow (prot)	1787	5058		1787	6454		1787	1881	1599	1787	3410	
FIt Permitted	0.95	1.00		0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (perm)	1787	5058		1787	6454		1787	1881	1599	1787	3410	
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.
Growth Factor (vph)	105%	105%	105%	105%	105%	105%	105%	105%	105%	105%	105%	105
Adj. Flow (vph)	45	1731	193	185	2135	39	219	203	254	57	210	100
RTOR Reduction (vph)	0	11	0	0	2100	0	0	0	198	0	46	
Lane Group Flow (vph)	45	1913	0	185	2172	0	219	203	56	57	257	
Heavy Vehicles (%)	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	-
Turn Type	Prot	NA.	170	Prot	NA.	170	Prot	NA.	Perm	Prot	NA.	
Protected Phases	1	6		5	2		7	4	reiiii	3	8	
Permitted Phases	- '	0		5	2		- /	4	4	3	0	
Actuated Green, G (s)	5.4	54.7		13.0	35.3		19.7	26.4	26.4	6.1	12.4	
Effective Green, g (s)	5.4	54.7		13.0	35.3		19.7	26.4	26.4	6.1	12.4	
Actuated g/C Ratio	0.05	0.46		0.11	0.29		0.16	0.22	0.22	0.05	0.10	
Clearance Time (s)	5.0	4.4		5.0	13.4		4.4	6.0	6.0	4.4	6.4	
Vehicle Extension (s)	0.5	0.5		0.5	1.0		0.5	1.0	1.0	0.5	1.0	
	80	2305		193	1898		293	413	351	90	352	
Lane Grp Cap (vph) v/s Ratio Prot	0.03							0.11	351	0.03	c0.08	
v/s Ratio Prot v/s Ratio Perm	0.03	c0.38		c0.10	c0.34		c0.12	0.11	0.03	0.03	CU.U8	
	0.50	0.00		0.00			0.75	0.40		0.00	0.70	
v/c Ratio	0.56	0.83		0.96	1.14		0.75	0.49	0.16	0.63	0.73	
Uniform Delay, d1	56.1	28.6		53.2			47.8	40.9	37.8	55.9	52.2	
Progression Factor	1.50	0.18		0.93	1.28 70.7		1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	0.5	0.3 5.5		46.5	124.8		8.8	0.3 41.3	0.1 37.9	10.2	6.6 58.8	
Delay (s)	84.7 F			95.9 F	124.8 F		56.6 F	41.3 D	37.9 D	66.1	58.8 E	
Level of Service	F	A		F			E		D	E		
Approach Delay (s)		7.3			122.6			45.0			59.9	
Approach LOS		Α			F			D			Е	
Intersection Summary												
HCM 2000 Control Delay			66.3	Н	CM 2000	Level of	Service		Е			
HCM 2000 Volume to Capacit	y ratio		1.07									
Actuated Cycle Length (s)			120.0		um of lost				41.6			
Intersection Capacity Utilization	n		79.5%	IC	U Level o	of Service			D			
Analysis Period (min) Critical Lane Group			15									

Culebra Road Corridor Study 5:00 pm 12/15/2020 2020 PM Existing Condition WSP - San Antonio

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Movement E	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		^ ^		*	^ ^			4			4		
Traffic Volume (veh/h)	49	1550	51	26	2000	24	93	21	16	31	55	78	
Future Volume (veh/h)	49	1550	51	26	2000	24	93	21	16	31	55	78	
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0	
	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00	
	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Work Zone On Approach		No	4000	4000	No	1000	1000	No	1000	1000	No	4000	
	870	1870 1678	1870	1870	1870 2165	1870	1870	1870	1870	1870 34	1870	1870 84	
Adj Flow Rate, veh/h Peak Hour Factor (53	0.97	55 0.97	0.97	0.97	26 0.97	0.97	0.97	0.97	0.97	0.97	0.97	
Percent Heavy Veh, %	2.91	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	
	223	3600	118	206	3657	44	160	35	20	71	100	121	
	0.01	0.23	0.23	0.03	1.00	1.00	0.15	0.15	0.15	0.15	0.15	0.15	
	781	5078	166	1781	5201	62	733	238	133	237	675	814	
Grp Volume(v), veh/h	53	1125	608	28	1416	775	141	0	0	178	0/3	014	
Grp Sat Flow(s), veh/h/ln1		1702	1840	1781	1702	1859	1104	0	0	1726	0	0	
Q Serve(q_s), s	1.0	34.1	34.1	0.5	0.0	0.0	3.9	0.0	0.0	0.0	0.0	0.0	
Cycle Q Clear(q_c), s	1.0	34.1	34.1	0.5	0.0	0.0	15.5	0.0	0.0	11.7	0.0	0.0	
	1.00	07.1	0.09	1.00	0.0	0.03	0.72	0.0	0.12	0.19	0.0	0.47	
Lane Grp Cap(c), veh/h		2413	1305	206	2394	1307	215	0	0.12	292	0	0.11	
	0.24	0.47	0.47	0.14	0.59	0.59	0.65	0.00	0.00	0.61	0.00	0.00	
	337	2413	1305	330	2394	1307	402	0	0	516	0	0	
	0.33	0.33	0.33	2.00	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00	
Upstream Filter(I) 1	1.00	1.00	1.00	0.32	0.32	0.32	1.00	0.00	0.00	1.00	0.00	0.00	
Uniform Delay (d), s/veh	4.8	26.4	26.4	9.8	0.0	0.0	50.4	0.0	0.0	48.5	0.0	0.0	
Incr Delay (d2), s/veh	0.2	0.6	1.2	0.0	0.3	0.6	1.3	0.0	0.0	0.8	0.0	0.0	
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
%ile BackOfQ(50%),veh/li		15.7	17.2	0.2	0.1	0.2	4.2	0.0	0.0	5.1	0.0	0.0	
Unsig. Movement Delay, :													
LnGrp Delay(d),s/veh	5.0	27.1	27.6	9.9	0.3	0.6	51.7	0.0	0.0	49.3	0.0	0.0	
LnGrp LOS	A	С	С	A	A	A	D	A	A	D	A	A	
Approach Vol, veh/h		1786			2219			141			178		
Approach Delay, s/veh		26.6			0.6			51.7			49.3		
Approach LOS		С			Α			D			D		
Timer - Assigned Phs	- 1	2		4	5	6		8					
Phs Duration (G+Y+Rc), s		89.3		23.4	6.6	90.0		23.4					
Change Period (Y+Rc), \$		4.9		* 5.6	* 4.8	4.9		* 5.6					
Max Green Setting (Gmax		60.1		* 34	* 10	60.1		* 34					
Max Q Clear Time (g_c+l		2.0		17.5	2.5	36.1		13.7					
Green Ext Time (p_c), s	0.0	6.9		0.3	0.0	4.5		0.4					
Intersection Summary													
HCM 6th Ctrl Delay			15.0										
HCM 6th LOS			В										
Notes													
Jser approved pedestriar	inte	rval to h	ne less t	than nh	ase ma	y oreen							
* HCM 6th computational								ases cri	ossing t	he barr	ier.		
Culebra Road Corridor St	udu s	5-00 nm	12/15/	2020.20	120 PM	Eviction	a Condi	ition					Synchro 10 Rep

Synchro 10 Report Page 4

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Movement	EBL	EBT	EBR	₩BL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SB
Lane Configurations	*	444	LUIT	*	*****	TIDIX	NDL	4	INDIX	ODL	4	00
Traffic Volume (vph)	9	1650	39	56	2050	4	61	0	54	1	5	
Future Volume (vph)	9	1650	39	56	2050	4	61	0	54	1	5	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	190
Total Lost time (s)	4.8	5.6		4.8	5.6			5.0			5.0	
Lane Util. Factor	1.00	0.91		1.00	0.91			1.00			1.00	
Frt	1.00	1.00		1.00	1.00			0.94			0.98	
Fit Protected	0.95	1.00		0.95	1.00			0.97			0.99	
Satd. Flow (prot)	1770	5067		1770	5084			1700			1820	
Flt Permitted	0.05	1.00		0.08	1.00			0.83			1.00	
Satd. Flow (perm)	90	5067		146	5084			1448			1831	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.9
Growth Factor (vph)	105%	105%	105%	105%	105%	105%	105%	105%	105%	105%	105%	105
Adj. Flow (vph)	10	1883	45	64	2340	5	70	0	62	1	6	
RTOR Reduction (vph)	0	1	0	0	0	0	0	103	0	0	1	
Lane Group Flow (vph)	10	1927	0	64	2345	0	0	29	0	0	7	
Turn Type	pm+pt	NA		pm+pt	NA		Perm	NA		Perm	NA	
Protected Phases	1	6		5	2			4			3	
Permitted Phases	6			2			4			3		
Actuated Green, G (s)	84.9	84.2		90.9	87.2			10.5			1.2	
Effective Green, g (s)	84.9	84.2		90.9	87.2			10.5			1.2	
Actuated g/C Ratio	0.71	0.70		0.76	0.73			0.09			0.01	
Clearance Time (s)	4.8	5.6		4.8	5.6			5.0			5.0	
Vehicle Extension (s)	0.5	1.0		1.0	1.0			1.0			1.0	
Lane Grp Cap (vph)	73	3555		160	3694			126			18	
v/s Ratio Prot	0.00	0.38		c0.01 0.29	c0.46			c0.02			c0.00	
v/s Ratio Perm v/c Ratio	0.10	0.54		0.40	0.63			0.23			0.39	
Vic Ratio Uniform Delay, d1	7.2	8.6		6.3	8.3			51.0			59.0	
Progression Factor	0.68	1.26		1.71	0.78			1.00			1.00	
Incremental Delay, d2	0.00	0.4		0.5	0.70			0.3			5.0	
Delay (s)	5.1	11.3		11.3	7.2			51.3			64.0	
Level of Service	Α.	B		В	7.2 A			D D			E	
Approach Delay (s)		11.2			7.3			51.3			64.0	
Approach LOS		В			A			D			E	
Intersection Summary												
HCM 2000 Control Delay			10.4	H	CM 2000	Level of	Service		В			
HCM 2000 Volume to Capaci	ity ratio		0.60									
Actuated Cycle Length (s)			120.0		um of lost				20.4			
Intersection Capacity Utilizati	on		71.4%	IC	CU Level of	of Service			С			
Analysis Period (min)			15									

Intersection							
nt Delay, s/veh	0.6						
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
ane Configurations	† ⊅		*	^		7	
Fraffic Vol, veh/h	1480	12	55	1300	0	18	
Future Vol, veh/h	1480	12	55	1300	0	18	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control		Free			Stop	Stop	
RT Channelized	-			None	-	None	
Storage Length	-	-	160	0	- 0	0	
Veh in Median Storage, Grade. %	# 0	- :	- 1	0	0	- 1	
eak Hour Factor	94		94	94	94	94	
Heavy Vehicles, %	94		1	94	1	1	
Mymt Flow	2047			1798	0	25	
		-			-		
Major/Minor N	lajor1	1	Major2		Minor1		
Conflicting Flow All	0		2064	0	-	1032	
Stage 1	-		-	-		-	
Stage 2	-		-		-		
Critical Hdwy	-		4.12			6.92	
Critical Hdwy Stg 1	-						
Critical Hdwy Stg 2	-		-				
ollow-up Hdwy	-	-		-		3.31	
Pot Cap-1 Maneuver	-					232	
Stage 1	- 1	-		-	0		
Stage 2 Platoon blocked. %	- 1				U		
Mov Cap-1 Maneuver	- :		271	- 1		232	
Mov Cap-2 Maneuver			211			202	
Stage 1							
Stage 2							
Approach	EB		WB		NB		
HCM Control Delay, s	0		0.9		22.4		
HCM LOS					С		
Minor Lane/Major Mvmt	- 1	NBLn1	EBT	EBR	WBL	WBT	
Capacity (veh/h)		232					
HCM Lane V/C Ratio		0.107	-		0.281		
HCM Control Delay (s)		22.4	-		23.4	-	
HCM Lane LOS		C			C	-	
HCM 95th %tile Q(veh)		0.4	-		1.1	-	

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58: Culebra Rd	& Ca	mino	Ros	а				02
Intersection								
Int Delay, s/veh	9							
**	ODI	000	051	OFT	ABACT	MANO		
Movement	SBL	SBR	SEL	SET		NWR		
Lane Configurations	Y	404	04	414	†	106		
Traffic Vol, veh/h	11	134	61	1155	1400			
Future Vol, veh/h	11	134	61	1155	1400	106		
Conflicting Peds, #/hr	0				0	Free		
Sign Control RT Channelized	Stop	Stop	Free	Free	Free	None		
Storage Length	0	None -	- 1	None -	- 1	140116		
Storage Lengtn Veh in Median Storage				0	0	-		
ven in median Storage Grade. %	t, # U	- 1	- 1	0	0	- 1		
Peak Hour Factor	94	94	94	94	94	94		
Heavy Vehicles, %	1	1	1	94	1	1		
Mymt Flow	15	185	84	1597	1936	147		
	10	.00	- 04	.001	.550	.71		
Matan Million	W		Mark and		1-1-0			
	Minor2		Major1		Major2			
Conflicting Flow All	2977		2083	0		0		
Stage 1	2010 967	- :	- 1	- 1	- 1			
Stage 2 Critical Hdwy	6.82	6.92	4.12			-		
Critical Hdwy Stg 1	5.82	0.92	4.12		- 1			
Critical Hdwy Stg 2	5.82	- 1	- 1	- 1	- 1	- 1		
Follow-up Hdwy	3.51	3.31	2.21	- 1		- 1		
Pot Cap-1 Maneuver	~ 11	228	266					
Stage 1	91	220	200			- 1		
Stage 2	332							
Platoon blocked. %	002							
Mov Cap-1 Maneuver	0	228	266					
Mov Cap-2 Maneuver	0							
Stage 1	0	-		-				
Stage 2	332					-		
Approach	SB		SE		NW			
HCM Control Delay, s	77.3		11.9		0			
HCM LOS	F				-			
Minor Lane/Major Mvm	nt	NWT	NWR	SEL	SET	SBLn1		
Capacity (veh/h)				266		228		
HCM Lane V/C Ratio				0.317		0.88		
HCM Control Delay (s)				24.7	11.2	77.3		
HCM Lane LOS				С	В	F		
HCM 95th %tile Q(veh))			1.3	-	7.1		
Notes ~: Volume exceeds cap	nacity	\$. D.	lav ave	eeds 30	Me	T. Come	utation Not Defined *: All major volume in platoon	

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	ř	† 1>		Ĭ	† 1>		Ž	†	7		4	7	
Traffic Volume (veh/h)	135	1160	12	18	1285	171	4	5	17	82	6	113	
Future Volume (veh/h)	135	1160	12	18	1285	171	4	5	17	82	6	113	
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0	
	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00	
	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Work Zone On Approach		No			No			No			No		
	984	1984	1984	1984	1984	1984	1984	1984	1984	1984	1984	1984	
Adj Flow Rate, veh/h	179	1539	16	24	1705	227	5	7	23	109	8	150	
		0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	
Percent Heavy Veh, %	251	2912	30	315	1 2535	330	83	204	173	189	10	185	
Cap, veh/h Arrive On Green	0.04	0.76	0.75	0.03	0.76	0.74	0.10	0.10	0.10	0.12	0.10	0.11	
	890	3823	40	1890	3354	437	1303	1984	1682	1350	99	1682	
Grp Volume(v), veh/h	179	758	797	24	942	990	5	7	23	117	99	150	
Grp Sat Flow(s), veh/h/ln1		1885	1977	1890	1885	1906	1303	1984	1682	1449	0	1682	
Q Serve(q_s), s	3.1	22.4	22.5	0.4	34.1	37.3	0.5	0.4	1.7	10.6	0.0	12.2	
Cycle Q Clear(q_c), s	3.1	22.4	22.5	0.4	34.1	37.3	11.6	0.4	1.7	11.0	0.0	12.2	
	1.00	22.7	0.02	1.00	04.1	0.23	1.00	0.4	1.00	0.93	0.0	1.00	
	251	1436	1506	315	1425	1440	83	204	173	216	0	185	
	0.71	0.53	0.53	0.08	0.66	0.69	0.06	0.03	0.13	0.54	0.00	0.81	
	452	1436	1506	406	1425	1440	175	344	292	322	0.00	304	
	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	
Uniform Delay (d), s/veh	20.4	6.6	6.7	5.5	8.3	8.9	66.9	56.5	57.1	60.7	0.0	60.9	
Incr Delay (d2), s/veh	1.4	1.4	1.3	0.0	2.4	2.7	0.1	0.0	0.1	0.8	0.0	3.2	
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
%ile BackOfQ(50%),veh/l		7.8	8.2	0.1	12.2	13.6	0.2	0.2	0.7	4.0	0.0	5.4	
Unsig. Movement Delay,													
	21.8	8.0	8.0	5.6	10.8	11.6	67.0	56.6	57.2	61.5	0.0	64.1	
LnGrp LOS	С	A	A	A	В	В	E	E	E	E	A	E	
Approach Vol, veh/h		1734			1956			35			267		
Approach Delay, s/veh		9.4			11.1			58.5			63.0		
Approach LOS		Α			В			Е			Е		
Timer - Assigned Phs	- 1	2		4	5	6		8					
Phs Duration (G+Y+Rc),	\$0.1	109.8		20.1	9.2	110.7		20.1					
Change Period (Y+Rc), s		* 6.1		* 5.7	5.0	* 6.1		* 5.7					
Max Green Setting (Gma		* 79		* 24	11.0	* 88		* 24					
Max Q Clear Time (g_c+l		39.3		13.6	2.4	24.5		14.2					
Green Ext Time (p_c), s	0.1	1.7		0.0	0.0	1.2		0.2					
Intersection Summary													
HCM 6th Ctrl Delay			14.3										
HCM 6th LOS			В										
Notes													
User approved pedestrial * HCM 6th computational								ases cr	ossing t	he barr	ier.		
Culebra Road Corridor Si													Synchro 10 Rep

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Delay, s/veh										
Bell										
Inter Configurations	Delay, s/veh	2.4								
raffic Vol., viehrh	ement	EBL			WBR		SBR			
uture Vol. weith 11 1245 1500 27 7 10 ornificing Peds, #hr 0 <td></td> <td>7</td> <td>*</td> <td></td> <td></td> <td>Y</td> <td></td> <td></td> <td></td> <td></td>		7	*			Y				
onflicting Peds, #thr 0										
Sign Confrol Free Free Free Stop Stop										
TÜ Channelized										
Dronge Length										
thin Median Storage, # . 0 0 0 - 0 - 1										
rade, %										
ask Hour Factor 96 96 96 96 96 98 98 98 98 98 98 98 98 98 98 98 98 98										
agor/Minor Major1 Major2 Minor2 minor2 minoriting Flow MI 208 0 0 2923 1034 Stage 1 - 2050 Stage 2 - 873 - 10164 Holly Stage 1 - 5.52 - 10164 Holly Stage 2 - 5.52 - 10164 Holly Stage 2 - 5.52 - 10164 Holly Stage 2 - 5.52 - 10164 Holly Stage 2 - 5.52 - 10164 Holly Stage 2 - 5.52 - 10164 Holly Stage 3 - 5.52 - 10164 Holly Stage 3 - 5.52 - 10164 Holly Stage 3 - 5.52 - 10164 Holly Stage 3 - 5.52 - 10164 Holly Stage 3 - 5.52 - 10164 Holly Stage 3 - 5.52 - 10164 Holly Stage 3 - 5.52 - 10164 Holly Stage 3 - 5.52 - 10164 Holly Stage 3 - 5.52 - 10164 Holly Stage 3 - 5.52 - 10164 Holly Stage 3 - 5.52 - 5.										
Identify Identify										
ontlicting Flow All 2088 0 - 0 2923 1034 Stage 1 2050 - Stage 2 873 - 7716at Heavy 4,12 6,82 6,92 rinical Heavy 4,12 6,82 6,92 rinical Heavy 81g 2 5,82 7716at Heavy 81g 2 5,82 - 7716at Heavy 81g 2 3,51 3,31 - 7716 Heavy 81g 2 12 231 stage 1 6 6 - 7716 Heavy 81g 2 3,71 - 7716 Heavy 81g 2 3,										
ontlicting Flow All 2088 0 - 0 2923 1034 Stage 1 2050 - Stage 2 873 - 7716at Heavy 4,12 6,82 6,92 rinical Heavy 4,12 6,82 6,92 rinical Heavy 81g 2 5,82 7716at Heavy 81g 2 5,82 - 7716at Heavy 81g 2 3,51 3,31 - 7716 Heavy 81g 2 12 231 stage 1 6 6 - 7716 Heavy 81g 2 3,71 - 7716 Heavy 81g 2 3,	or/Minor	Major1		Maior2	_1	Minor2				
Siage 1							1034			
Stage 2		2000								
ritical Holdy 4.12 - 6.82 6.92 ritical Holdy Sig 2 - 5.82 - ritical Holdy Sig 2 - 5.82 - ritical Holdy Sig 2 - 5.82 - ritical Holdy Sig 2 - 3.51 3.31 Clockp-1 Maneuver 270 - 12 231 Slage 1 - 66 - 8 Slage 2 - 371 - 371 Jatono Diccked, % 11 231 ov Cap-1 Maneuver - 11 231 ov Cap-2 Maneuver - 11 - 31 Slage 1 - 81 - 81 Slage 2 - 371 - 371 Slage 2 - 371 - 81 MC Ontrol Delay, s 0.2 0 \$376.1										
Inical Holy Stg 1		4.12				6.82	6.92			
Illow-up Howy						5.82				
at Cap-1 Meneuver 270 - 12 231 Stage 1 - - 66 - Stage 2 - - 371 - atton blocked, % - - - ov Cap-2 Maneuver - - 11 231 ov Cap-2 Maneuver - - 81 - - 81 - Stage 2 - - 371 procech EB WB SB OK Control Delay, s 0.2 0 \$376.1		-				5.82	-			
Stage 2	ow-up Hdwy	2.21		-		3.51	3.31			
Stage 2	Cap-1 Maneuver	270				12	231			
Internal Dicked, %	Stage 1	-					-			
Nov Cap-2 Maneuver 270 - 11 231						371				
Mov Cap 2 Maneuver - - 11 Stage 1 - - 81 - Stage 2 - - 371 - opproach EB WB SB CM Control Delay, s 02 0 \$376.1										
Slage 1										
Stage 2 371										
pproach										
CM Control Delay, s 0.2 0 \$376.1	Stage 2				-	3/1				
CM Control Delay, s 0.2 0 \$376.1				IA/O		00				
Om EGG F		0.2		U	3					
	11 200					г				
inor Lane/Major Mvmt EBL EBT WBT WBR SBLn1	or Lane/Major Mar	mt	ERI	ERT	WRT	WRP	SRI n1			
apacity (veh/h) 270 25		III.		LDI		NON				
CM Lane V/C Ratio 0.055 0.921										
CM Control Delay (s) 19.1 \$ 376.1		1		- 1						
CM Lane LOS C F		,								
CM 95th %tile Q(veh) 0.2 2.8		1)		-		-				
otes	,	,								
		an a site	¢. D	lau a	anda M	20-	Car	estation Nat Defice 4	A. All maior values in al-1-1-	
Volume exceeds capacity \$: Delay exceeds 300s +: Computation Not Defined *: All major volume in platoon	olume exceeds ca	pacity	\$: D6	elay exc	eeas 30	JUS	+: Comp	outation Not Defined	:: All major volume in platoon	

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	-	_	*	*			.,		- /		*	_
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SE
Lane Configurations	3	44	7		† \$			ર્ન	7	٦	.	
Traffic Volume (vph)	58	1135	180	230	1067	14	309	15	338	25	23	
Future Volume (vph)	58	1135	180	230	1067	14	309	15	338	25	23	
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	20
Total Lost time (s)	4.7	4.0	5.3	4.0	4.0		4.0	5.1	4.4	5.1	5.1	
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95		0.95	0.95	1.00	1.00	1.00	1.
Fit	1.00	1.00	0.85	1.00	1.00		1.00	1.00	0.85	1.00	1.00	0.
Fit Protected	0.95	1.00	1.00	0.95	1.00		0.95	0.96	1.00	0.95	1.00	1.
Satd. Flow (prot)	1881	3762	1683	1881	3755		1787 0.95	1799	1683	1881 0.95	1980	1.
Flt Permitted	0.11	1.00	1.00		1.00			0.96	1.00			
Satd. Flow (perm)	211	3762	1683	116	3755	0.00	1787	1799	1683	1881	1980	16
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.
Growth Factor (vph)	130%	130%	130%	130%	130%	130%	130%	130%	130%	130%	130%	130
Adj. Flow (vph)	82	1604	254	325	1508	20	437	21	478	35	32	
RTOR Reduction (vph)	0	0	50	0	0	0	0	0	240	0	0	
Lane Group Flow (vph)	82	1604	204	325	1528	0	227	231	238	35	33	
Heavy Vehicles (%)	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	_
Turn Type	pm+pt	NA	Perm	pm+pt	NA		Split	NA	Perm	Split	NA	Pe
Protected Phases	1	6		5	2		4	4		3	3	
Permitted Phases	6		6	2					4			
Actuated Green, G (s)	69.4	63.2	63.2	92.7	81.8		24.3	24.3	24.3	6.8	6.8	6
Effective Green, g (s)	69.4	65.6	64.3	93.4	83.8		25.4	24.3	25.0	6.8	6.8	- 6
Actuated g/C Ratio	0.50	0.47	0.46	0.67	0.60		0.18	0.17	0.18	0.05	0.05	0.
Clearance Time (s)	4.7	6.4	6.4	4.7	6.0		5.1	5.1	5.1	5.1	5.1	
Vehicle Extension (s)	1.0	1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0	1.0	_
Lane Grp Cap (vph)	178	1762	772	393	2247		324	312	300	91	96	
v/s Ratio Prot	0.02	c0.43		c0.15	0.41		0.13	0.13		c0.02	0.02	
v/s Ratio Perm	0.21		0.12	0.40					c0.14			0.
v/c Ratio	0.46	0.91	0.26	0.83	0.68		0.70	0.74	0.79	0.38	0.34	0.
Uniform Delay, d1	21.2	34.5	23.3	44.8	19.0		53.7	54.9	55.0	64.6	64.4	63
Progression Factor	1.62	1.34	1.86	1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.
Incremental Delay, d2	0.4	5.6	0.5	12.7	1.7		5.5	8.0	12.6	1.0	0.8	(
Delay (s)	34.6	51.6	43.9	57.5	20.7		59.2	62.9	67.6	65.6	65.2	63
Level of Service	С	D	D	E	С		E	E	Е	E	E	
Approach Delay (s)		49.9			27.2			64.4			64.4	
Approach LOS		D			С			E			E	
Intersection Summary												
HCM 2000 Control Delay			44.5	ш	CM 2000	Laval of	Service		D			
HCM 2000 Control Delay HCM 2000 Volume to Capac	rity ratio		0.85	П	5WI 2000	FEAGI OI	DEI VICE		U			
Actuated Cycle Length (s)	ony rano		140.0	e.	um of lost	time (c)			18.9			
	tion		83.1%		U Level o				10.9 E			
	UUII		15	IC	O Level C	JI SEI VICE						
Intersection Capacity Utilizat Analysis Period (min)												

	•	-	•	F	•	•	•	1	1	-	-	1
Movement	FBI	FBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBI	S
Lane Configurations	36	44			ă	Ŷβ		36	ĵ.		76	
Fraffic Volume (vph)	308	1170	19	2	22	1099	221	19	3	12	141	
uture Volume (vph)	308	1170	19	2	22	1099	221	19	3	12	141	
deal Flow (vphpl)	2000	2000	1900	2000	1900	2000	2000	1900	1900	1900	2000	19
otal Lost time (s)	4.0	4.0			4.9	4.0		5.4	5.4		4.2	
ane Util. Factor	1.00	0.95			1.00	0.95		1.00	1.00		1.00	- 1
rt	1.00	1.00			1.00	0.97		1.00	0.88		1.00	(
It Protected	0.95	1.00			0.95	1.00		0.95	1.00		0.95	- 1
Satd. Flow (prot)	1881	3753			1787	3668		1787	1655		1881	1
It Permitted	0.06	1.00			0.14	1.00		0.21	1.00		0.74	- 1
Satd. Flow (perm)	114	3753			263	3668		400	1655		1474	1
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0
Growth Factor (vph)	130%	130%	130%	130%	130%	130%	130%	130%	130%	130%	130%	13
Adj. Flow (vph)	421	1601	26	3	30	1504	302	26	4	16	193	
RTOR Reduction (vph)	0	1	0	0	0	12	0	0	14	0	0	
ane Group Flow (vph)	421	1626	0	0	33	1794	0	26	6	0	193	
leavy Vehicles (%)	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	
Turn Type	pm+pt	NA		pm+pt	pm+pt	NA		Perm	NA		Perm	
Protected Phases	1	6		5	5	2			4			
Permitted Phases	6			2	2			4			8	
ctuated Green, G (s)	94.7	87.4			66.8	64.4		18.8	18.8		18.8	1
Effective Green, g (s)	95.6	89.5			66.8	66.5		18.8	18.8		20.0	- 1
Actuated g/C Ratio	0.76	0.72			0.53	0.53		0.15	0.15		0.16	(
Clearance Time (s)	4.9	6.1			4.9	6.1		5.4	5.4		5.4	
/ehicle Extension (s)	0.5	1.0			0.5	1.0		1.0	1.0		1.0	
ane Grp Cap (vph)	458	2687			169	1951		60	248		235	
/s Ratio Prot	c0.19	0.43			0.00	0.49			0.00			(
/s Ratio Perm	c0.51				0.10			0.06			c0.13	
/c Ratio	0.92	0.61			0.20	0.92		0.43	0.03		0.82	(
Jniform Delay, d1	41.6	8.9			13.9	26.8		48.3	45.3		50.8	4
Progression Factor	1.00	1.00			1.00	1.00		1.00	1.00		1.00	1
ncremental Delay, d2	23.0	1.0			0.2	8.5		1.8	0.0		19.2	
Delay (s)	64.6	9.9			14.1	35.3		50.1	45.3		70.0	4
evel of Service	Е	Α			В	D		D	D		Е	
Approach Delay (s)		21.2				35.0			48.0			5
Approach LOS		С				С			D			
ntersection Summary												
ICM 2000 Control Delay			31.1	Н	CM 2000	Level of	Service		С			
ICM 2000 Volume to Capac	ity ratio		0.93									
Actuated Cycle Length (s)			125.0	S	um of lost	time (s)			14.3			
	ion		99.0%	IC	U Level	of Service			F			
ntersection Capacity Utilizat			15									
ntersection Capacity Utilizat unalysis Period (min)												

Lang® configurations Traffic Volume (vph) 2 Future Volume (vph) 2 Ideal Flow (vphp) 3 Ideal Flow (vphp) 4	95 95 96 97 97 98 98 98 98 98 98	
Landighordigurations Traffice Volume (vph) Future Volume (vph) Gleaf Flow (vphp) Zotal Lost time (o) Lane (bit. Factor Fit Protected Stad. Flow (prot) Fit Permitted Stad. Flow (prot) Fit Permitted Stad. Flow (prot) Fit Permitted Stad. Flow (prot) Add, Flow (vph) Add, Flow (vph) Lane Group Flow (vph) Lane Gr	36 36 36 00 95 95 97 23 0	
Traffic Volume (vph) Future Volume (vph) Ideal Flow (vphpl) Ideal Flow (vphpl) Ideal Flow (vphpl) Ideal Flow (vphpl) If Protected Satd. Flow (prot) If Permitted Satd. Flow (prot) If Permitted Satd. Flow (prot) If Permitted Satd. Flow (vph) Ideal Flow (vph) Ideal Flow (vph) Ideal Flow (vph) Ideal Flow (vph) Ideal Flow (vph) Ideal Flow (vph) Im Type Protected Phases	95 95 96 97 97 90 90	
Future Volume (vph) (dale Flow (vphp)) 2 (dale Flow (vphp)) 2 (for flow flow flow flow flow flow flow flow	95 % 23 0	
Total Lost time (s) Lane UBL Factor Fri Fit Fit Protected Satd. Flow (prot) Fit Permitted Satd. Flow (prot) Feak-hour factor. PHF 0 Growth Factor (pyh) 13 Adj. Flow (pyh) Lane Group Flow (yph) Lane Group Flow (yph) Lane William (Pheavy Vehicles (%) Turn Type Protected Pheases	95 9% 23 0	
Lane Ull. Factor Fit Fit Fit Fit Fit Protected Sald. Flow (prot) Fit Permitted Sald. Flow (prot) Fit Permitted Sald. Flow (prot) Growth Factor, (PhH 0 Growth Factor, (PhH) 13 Adj. Flow (Pph) 13 RTOR Reduction (vph) Lane Group Flow (vph) Heavy Vehicles (%) Turn Type Protected Phases	0% 23 0 0	
Fri El Polacidad Sald, Flow (prot) File Pomission Sald, Flow (perm) Feak-hour factor, PHF Orgowin Factor (php) 3Adj, Flow (pph) Sald, Flow (pph) Lane Group Flow (pph) Lane Group Flow (pph) Turn Type Protected Phases	0% 23 0 0	
Fill Protected Satid. Flow (prot) Fill Permitted Satid. Flow (perm) Peak-hour factor, PHF Growth Factor, (vph) 43, Flow (vph) 13 Add. Flow (vph) 15 RTOR Reduction (vph) Lane Group Flow (vph) Heavy Vehicles (%) Turn Type Protected Phases	0% 23 0 0	
Satd. Flow (prot) Filt Permitted Satd. Flow (perm) Peak-hour factor, PHF 0 Growth Factor (vph) 13 Adj. Flow (vph) 4 RTOR Reduction (vph) Heavy Vehicles (%) Turn Type Protected Phases	0% 23 0 0	
Fil Permitted Satd. Flow (perm) Peak-hour factor, PHF 0 Growth Factor (vph) 13 Ad, Flow (vph) 5 RTOR Reduction (vph) Heavy Vehicles (Vp) Turn Type Protected Phases	0% 23 0 0	
Satd. Flow (perm) Peak-hour factor, PHF 0 Growth Factor (vph) 13 Adj. Flow (vph) 3 RTOR Reduction (vph) Lane Group Flow (vph) Heavy Vehicles (%) Turn Type Protected Phases	0% 23 0 0	
Peak-hour factor, PHF 0 Growth Factor (vph) 13 Adj. Flow (vph) 3 RTOR Reduction (vph) Lane Group Flow (vph) Heavy Vehicles (%) Turn Type Protected Phases	0% 23 0 0	
Growth Factor (vph) 13 Adj. Flow (vph) 3 RTOR Reduction (vph) Lane Group Flow (vph) Heavy Vehicles (%) Turn Type Protected Phases	0% 23 0 0	
Adj. Flow (vph) RTOR Reduction (vph) Lane Group Flow (vph) Heavy Vehicles (%) Turn Type Protected Phases	23 0 0	
RTOR Reduction (vph) Lane Group Flow (vph) Heavy Vehicles (%) Turn Type Protected Phases	0	
Lane Group Flow (vph) Heavy Vehicles (%) Turn Type Protected Phases	0	
Heavy Vehicles (%) Turn Type Protected Phases		
Turn Type Protected Phases	1%	
Protected Phases		
Actuated Green, G (s)		
Effective Green, g (s)		
Actuated g/C Ratio		
Clearance Time (s)		
Vehicle Extension (s)		
Lane Grp Cap (vph)		
v/s Ratio Prot		
v/s Ratio Perm		
v/c Ratio		
Uniform Delay, d1		
Progression Factor		
Incremental Delay, d2		
Delay (s)		
Level of Service		
Approach Delay (s)		
Approach LOS		
Intersection Summary		

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Movement	FBI	EBT	WBT	WBR	SBL	SBR		
	EDL	<u>†</u>	↑ ₽	WDR	ODL.	JDK 7		
Lane Configurations Traffic Volume (vph)	129	TT 1195	1226	178	125	93		
Future Volume (vph)	129	1195	1226	178	125	93		
	2000	2000	2000	2000	2000	2000		
Ideal Flow (vphpl)	4.0	4.0	4.0	2000	4.0	4.0		
Total Lost time (s) Lane Util. Factor	1.00	0.95	0.95		1.00	1.00		
Lane Util. Factor Frt	1.00	1.00	0.95		1.00	0.85		
FIt Protected	0.95	1.00	1.00		0.95	1.00		
Fit Protected Satd. Flow (prot)	1881	3762	3691		1881	1683		
Sato. Flow (prot) Flt Permitted	0.05	1.00	1.00		0.95	1.00		
Satd. Flow (perm)	103	3762	3691		1881	1683		
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95		
Growth Factor (vph)	130%	130%	130%	130%	130%	130%		
Adj. Flow (vph)	177	1635	1678	244	171	127		
RTOR Reduction (vph)	0	0	6	0	0	0		
Lane Group Flow (vph)	177	1635	1916	0	171	127		
Heavy Vehicles (%)	1%	1%	1%	1%	1%	1%		
Turn Type	pm+pt	NA	NA		Prot	pt+ov		
Protected Phases	1	6	2		8	18		
Permitted Phases	6					6		
Actuated Green, G (s)	104.8	104.8	88.9		14.6	124.4		
Effective Green, g (s)	105.8	106.4	90.5		15.6	125.4		
Actuated g/C Ratio	0.81	0.82	0.70		0.12	0.96		
Clearance Time (s)	5.0	5.6	5.6		5.0			
Vehicle Extension (s)	0.5	1.0	1.0		1.0			
Lane Grp Cap (vph)	246	3079	2569		225	1623		
v/s Ratio Prot	c0.07	0.43	c0.52		c0.09	0.02		
v/s Ratio Perm	0.52					0.06		
v/c Ratio	0.72	0.53	0.75		0.76	0.08		
Uniform Delay, d1	36.1	3.8	12.5		55.4	0.1		
Progression Factor	1.00	1.00	1.00		1.00	1.00		
Incremental Delay, d2	8.1	0.7	2.0		12.7	0.0		
Delay (s)	44.2	4.4	14.5		68.1	0.1		
Level of Service	D	Α	В		Е	Α		
Approach Delay (s)		8.3	14.5		39.1			
Approach LOS		Α	В		D			
Intersection Summary								
			13.5	ш	CM 2000	Level of Service	В В	
	itu ratio			- 11	CIVI 2000	Level of Service	, ,	
	aty ratio			e.	um of loc	t time (e)	12.0	
	tion							
	ioi1			IC	O LEVEL	UI USI VIUS	U	
			10					
HCM 2000 Control Delay HCM 2000 Volume to Capace Actuated Cycle Length (s) Intersection Capacity Utilizat Analysis Period (min) C Critical Lane Group			13.5 0.74 130.0 76.2% 15	S	um of los		12.0 D	

	•	→	*	1	←	•	1	1	-	-	Ţ	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SB
Lane Configurations	*	†î»		*	44	7	7	ĵ,		*	1>	
Traffic Volume (vph)	98	1120	33	35	1380	114	34	5	40	57	10	13
Future Volume (vph)	98	1120	33	35	1380	114	34	5	40	57	10	13
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	190
Total Lost time (s)	4.5	5.8		4.5	5.8	5.8	5.4	5.4		5.4	5.4	
Lane Util. Factor	1.00	0.95		1.00	0.95	1.00	1.00	1.00		1.00	1.00	
Frt	1.00	1.00		1.00	1.00	0.85	1.00	0.87		1.00	0.86	
Flt Protected	0.95	1.00		0.95	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1787	3559		1787	3574	1599	1787	1631		1787	1618	
Flt Permitted	0.05	1.00		0.12	1.00	1.00	0.38	1.00		0.72	1.00	
Satd. Flow (perm)	95	3559		219	3574	1599	724	1631		1349	1618	
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.9
Growth Factor (vph)	130%	130%	130%	130%	130%	130%	130%	130%	130%	130%	130%	130
Adj. Flow (vph)	131	1501	44	47	1849	153	46	7	54	76	13	18
RTOR Reduction (vph)	0	1	0	0	0	35	0	50	0	0	159	
Lane Group Flow (vph)	131	1544	0	47	1849	118	46	11	0	76	38	
Heavy Vehicles (%)	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	- 1
Turn Type	pm+pt	NA		pm+pt	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	1	6		5	2			4			3	
Permitted Phases	6			2		2	4			3		
Actuated Green, G (s)	87.8	87.8		90.8	89.5	89.5	10.4	10.4		10.2	10.2	
Effective Green, g (s)	87.8	87.8		90.8	89.5	89.5	10.4	10.4		10.2	10.2	
Actuated g/C Ratio	0.63	0.63		0.65	0.64	0.64	0.07	0.07		0.07	0.07	
Clearance Time (s)	4.5	5.8		4.5	5.8	5.8	5.4	5.4		5.4	5.4	
Vehicle Extension (s)	0.5	0.5		1.0	1.0	1.0	0.5	0.5		0.5	0.5	
Lane Grp Cap (vph)	165	2232		259	2284	1022	53	121		98	117	
v/s Ratio Prot	0.05	c0.43		0.01	c0.52			0.01			0.02	
v/s Ratio Perm	0.44			0.10		0.07	c0.06			c0.06		
v/c Ratio	0.79	0.69		0.18	0.81	0.12	0.87	0.09		0.78	0.32	
Uniform Delay, d1	37.3	17.2		22.5	18.9	9.8	64.1	60.4		63.8	61.6	
Progression Factor	0.84	1.55		1.47	1.41	2.04	1.00	1.00		1.00	1.00	
Incremental Delay, d2	18.9	1.6		0.1	1.4	0.1	74.2	0.1		28.7	0.6	
Delay (s)	50.4	28.1		33.1	28.0	20.2	138.4	60.5		92.4	62.2	
Level of Service	D	С		С	С	С	F	Е		F	Е	
Approach Delay (s)		29.9			27.6			94.0			70.6	
Approach LOS		С			С			F			Е	
Intersection Summary												
HCM 2000 Control Delay			33.1	ш	CM 2000	Level of	Consiso C		C			
HCM 2000 Collino Delay HCM 2000 Volume to Capac	ity ratio		0.81	- 11	CIVI 2000	Level UI	DEI VICE		U			
Actuated Cycle Length (s)	aty ratio		140.0	c.	um of lost	time (c)			21.1			
Intersection Capacity Utilizat	ion		90.9%			of Service			21.1 E			
mioreoction Gapacity Utilizat	ioit		15	IC	O LEVEL	or our vice			2			
Analysis Period (min)			13									

		٠	-	*	1	•	•	1	†	-	-	1
Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	S
Lane Configurations		Ä	44	7	100	† }		37	† ‡		16	-
Traffic Volume (vph)	7	150	520	485	223	690	109	720	1000	114	148	5
Future Volume (vph)	7	150	520	485	223	690	109	720	1000	114	148	ŧ
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	19
Total Lost time (s)		4.4	5.7	5.7	4.8	5.7		4.9	5.8		4.7	
Lane Util. Factor		1.00	0.95	1.00	1.00	0.95		0.97	0.95		0.97	0
rt		1.00	1.00	0.85	1.00	0.98		1.00	0.98		1.00	0
It Protected		0.95	1.00	1.00	0.95	1.00		0.95	1.00		0.95	- 1
Satd. Flow (prot)		1787	3574	1599	1787	3501		3467	3519		3467	3
It Permitted		0.26	1.00	1.00	0.95	1.00		0.95	1.00		0.95	1
Satd. Flow (perm)		482	3574	1599	1787	3501		3467	3519		3467	3
Peak-hour factor, PHF	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0
Growth Factor (vph)	130%	130%	130%	130%	130%	130%	130%	130%	130%	130%	130%	13
\dj. Flow (vph)	9	197	683	637	293	906	143	945	1313	150	194	
RTOR Reduction (vph)	0	0	0	398	0	9	0	0	6	0	0	
ane Group Flow (vph)	0	206	683	239	293	1040	0	945	1457	0	194	- 1
leavy Vehicles (%)	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	
Turn Type	custom	Prot	NA	Perm	Prot	NA		Prot	NA		Prot	
Protected Phases		1	6		5	2		7	4		3	
Permitted Phases	- 1			6								
Actuated Green, G (s)		15.6	29.3	29.3	22.2	36.3		39.4	56.9		10.6	2
Effective Green, g (s)		15.6	29.3	29.3	22.2	36.3		39.4	56.9		10.6	2
Actuated g/C Ratio		0.11	0.21	0.21	0.16	0.26		0.28	0.41		0.08	(
Clearance Time (s)		4.4	5.7	5.7	4.8	5.7		4.9	5.8		4.7	
/ehicle Extension (s)		1.0	1.0	1.0	1.0	1.0		1.0	1.0		1.0	
ane Grp Cap (vph)		53	747	334	283	907		975	1430		262	
/s Ratio Prot			0.19		0.16	c0.30		c0.27	c0.41		0.06	cC
/s Ratio Perm		c0.43		0.15								
/c Ratio		3.89	0.91	0.72	1.04	1.15		0.97	1.02		0.74	1
Jniform Delay, d1		62.2	54.1	51.5	58.9	51.9		49.7	41.5		63.4	5
Progression Factor		1.06	0.68	1.59	1.00	1.00		1.08	1.27		1.00	1
ncremental Delay, d2		1333.7	14.5	9.8	63.0	79.0		19.2	26.8		9.4	8
Delay (s)		1399.6	51.5	91.7	121.9	130.9		72.9	79.6		72.8	13
evel of Service		F	D	F	F	F		Е	Е		Е	
Approach Delay (s)			250.3			128.9			77.0			12
Approach LOS			F			F			Е			
ntersection Summary												
ICM 2000 Control Delay			138.0	Н	CM 2000	Level of	Service		F			
HCM 2000 Volume to Capac	city ratio		1.45									
Actuated Cycle Length (s)			140.0		um of lost				21.0			
	tion		122.1%	IC	U Level	of Service			Н			
ntersection Capacity Utilizat Analysis Period (min)			15									

Movement SBR Lat	
Lad Dorfgurations Tartifits (volume (ph) 77 Future Volume (ph) 77 Future Volume (ph) 77 Future Volume (ph) 990 Total Lus time (s) 990 Lane Util, Factor Fit Froid Lus time (s) 1900 Till Fermitted Statt Flow (prot) 1900 Till Permitted Statt Flow (prot) 1909 Flow Hour (prot) 1909 Flow Hour (prot) 1909 Growth Factor (ph) 1909 Growth Factor (ph) 1910 Lane Group Flow (ph) 191 Lane Group Flow (ph) 0 Lane Group Flow (ph) 0 Lane Group Flow (ph) 195 Froid Cade Phases Protected Phases Actuated Green, G (s) 195 Actuated (g) C Ratio Clearance Time (s) 1909 Vehicle Extension (s) 1909 Vehicle Extension (s) 1909 Vehicle Extension (s) 1909 Ver Ratio Perm Ver Ratio P	
Traffic Volume (vph) 77 detail Flow (vphp) 77 detail Flow (vphp) 1900 Total Lost time (s) Lame UBL, Factor Fit Protected Staft Flow (prot) Ri Permitted Staft Flow (prot) 1019 RTOR Radicution (vph) 101 RTOR Radicution (vph) 0 Leavy Verbides (%) 1% Turn Type Protected Phases Actuated Green, G (s) Efficience Green, G (s) Eff	
Future Volume (vph) 77 dealer Flow (vphp) 1990 Total Lost time (s) Lane UBI, Factor Fit Fremitted Statt Flow (prm) Fit Permitted Statt Flow (prm) Feak-hour factor, PHF 0.99 Footh Factor (vph) 130% Adj. Flow (vph) 101 TOR Reduction (vph) 0 Lane Group Flow (vph) 101 Lane Group Flow (vph) 0 Lane Group Flow (vph) 1% Lane Statt Flow (prm) Fremitted Phases Future Type Frotected Phases Actuated Green, G (s) Actuated (g): Ratio Clearance Time (s) Vehicle Extension (s) Lane Group (ph) Vis Ratio Prot Vis R	
Total Lost time (s) Lance UME, Factor Fit Fit Fit Protested Satut. Flow (prot) Fit Premitted Satut. Flow (perm) Flow (perm) Flow (perm) Flow (perm) Satut. Flow (
Lane UBI. Factor Fit Protected Sald Flow (prot) Fit Permitted Sald Flow (prot) Fit Permitted Sald Flow (prot) Fit Permitted Sald Flow (prot) Fleak-hour factor, PHF 0.99 Peak-hour factor (vph) 100's FLOR Reduction (vph) 101 FLOR Reduction (vph) 0 Leavy Vehicles (%) 1% Turn Type Protected Phases Actuated Green, G (s) Effective Green, G (s) Effectiv	
"Il Protected Sald Flow (prot) "Il Permitted Sald Flow (prot) "El Permitted Sald Flow (perm) **beak-hour factor (prh)	
Sald Flow (prot)	
### Permitted ##	
Sald Flow (perm) Peak-hour factor, PHF 3.99 Srowth Factor (vph) 101 107 Reduction (vph) 101 TOR Reduction (vph) 0 ane Group Flow (vph) 104 Turn Type Protected Phases Permitted Pha	
Peak-hour factor, PHF 0.99 7 orwh Factor (php) 130% 4di, Flow (pph) 101 17 CIR Reduction (pph) 100 17 CIR Reduction (pph) 0 18 deprive (pph) 100 18	
30wth Factor (vph) 130% 44, 150w (vph) 101 17 CR Reduction (vph) 101 17 CR Reduction (vph) 0 0 0 0 0 0 0 0 0	
Adj. Flow (rph) 101 TGR Reduction (vph) 0 ane Group Flow (vph) 0 teary Verticles (%) 1% Unit Type "rotected Phases "chated Creen, G (s) Chatalage (g) CRatio Electrical (g) CRa	
Aug. Flow (yeh) 101 TOR Reduction (yeh) 0 anne Group Flow (yeh) 0 tenny Vehicles (%) 1% Turn Type Volucted Phases Vermitted Phases Vermitted Phases Vehicle Extension (s) Authority (C) Ratio Distraction (s) Authority (s	
ane Group Flow (oph) Greaty Vehicles (%) Furn Type F	
leany Vehicles (%) 1% Upun Type Protected Phases Actuated Green, G (s) Effective Green, g (s) Actuated g C Ratio Learnance Time (s) Learnance Time (s) S Ratio Port S Ratio Port S Ratio Port S Ratio Port C Ratio Linition Delay, cd Torgression Factor normental Delay, cd Delay (s) Learnance Teator Linition Delay, cd Delay (s) Learnance Teator Learnance	
Jum Type Profected Phases Profected Phases Profected Phases Actuated Green, G (s) Effective Green, g (s) Actuated Q C Ratio Bearance Time (s) Vehicle Extension (s) As Ratio Prot As Ratio Prot As Ratio Prot As Ratio Prot Juliorom Delay, d1 Progression Factor Incremental Delay, d2 Delay (s) Level of Service Approach Delay (s)	
Protected Phases Permitted Phases Actuated Green, G (s) Effictive Green, g (s) Actuated g (C Ratio Actuated g (C Ratio Actuated g (C Ratio Actuated g (S Ratio Actuated g (
Permitted Phases Authated Green, g (s) Effective Green, g (s) Authated Q P. Ratio Bearrince Time (s) Vehicle Extension (s) Als Ratio Prot Als Ratio Prot Als Ratio Prot Als Ratio Prot Als Ratio Prot Als Ratio Prot Als Ratio Prot Als Ratio Prot Als Ratio Prot Als Ratio Prot Als Ratio Prot Als Ratio Prot Als Ratio Prot Als Ratio Prot Als Ratio Prot Als Ratio Prot Als Ratio Prot Als Ratio Als Rati	
Actuated Green, G (s) Effictive Green, g (s) Actuated g (C Ratio Jearance Time (s) Vehicle Extension (s) ane Gip Gap (vph) Is Ratio Prut Is Ratio Prut Is Ratio Prut Vehicle Extension (s) and Gip Gap (vph) Is Ratio Prut Vehicle Extension (s) Individual Company Vehicle Extension (s) Individual Company Vehicle Extension (s) Vehi	
Effective Green, g (s) Actualed g (c) Ratio Dearrance Time (s) White Extension (s) White Extension (s) White Ratio Prot Wis Ratio Prot Wis Ratio Prot Wis Ratio Prot Progression Factor noriemental Delay, d1 Progression Factor service and the Ratio Delay (s) White Ratio Prot Progression Factor service and Delay, d2 Delay (s) White Ratio Prot Progression Factor service Ratio Rat	
Actuated QC Ratio Delarance Time (s) (Arbide Extension (s)	
Diserance Time (s) Archide Extension (s) Lane Grp Cap (vph) Is Ratio Prot Is Ratio Prom Is Ratio Prom Is Ratio Prom Inform Delay, c11 **Torgression Factor noremental Delay, d2 Jelay (s) Level of Service Approach Delay (s)	
Vehide Extension (s) Lame Gry Cap (cpt) Vis Ratio Per Vis	
Lame Grp (Cap (rph) vis Ratio Prot vis Ratio Prom vic Ratio vic Ratio nitrom Delay, d1 Progression Factor noncemental Delay, d2 Delay (c) Level of Service Approach Delay (s)	
vis Ratio Prot. vis Ratio Prem vic Ratio vic Ratio Progression Factor Incremental Delay, d2 Delay (s) Level of Service Approach Delay (s)	
vls Ratio Perm vlc Ratio linitism Delay, d1 Progression Factor incremental Delay, d2 Delay (3 Level of Service Approach Delay (5)	
vic Ratio Inflorm Delay, d1 Progression Factor Incremental Delay, d2 Delay (s) Level of Service Approach Delay (s)	
Uniform Delay, d1 **Progression Factor **reamental Delay, d2 **Delay (s) **Delay (s) **Provided Service **Approach Delay (s)	
Progression Factor noremental Delay, d2 Jelay (s) Level of Service Approach Delay (s)	
ncremental Delay, d2 Delay (s) Sevel of Senvice Approach Delay (s)	
Delay (s) evel of Service hpproach Delay (s)	
Level of Service Approach Delay (s)	
Approach Delay (s)	
Approach LOS	
Intersection Summary	

	Dr &	Cule	bra F	Rd					
tersection									
nt Delay, s/veh	0.5								
lovement	EBT	EBR	WBL	WBT	NBL	NBR			
ane Configurations	ተ ቀተ»			ተተጉ	٦	7			
raffic Vol, veh/h	1200	21	23	1500	9	12			
uture Vol, veh/h	1200	21	23	1500	9	12			
onflicting Peds, #/hr	0	0	0	0	0	0			
gn Control	Free		Free	Free	Stop	Stop			
T Channelized	-	None	-	None		None			
torage Length				-	0	0			
eh in Median Storage Grade. %	e, # 0 0	- 1	- 1	0	0	- :			
rade, % eak Hour Factor	98	98	98	98	98	98			
eavy Vehicles, %	98	98	98	98	98	98			
fumt Flow	1592	28	31	1990	12	16			
VIIICI IOW	1032	20	JI	1000	12	10			
ajor/Minor I	Major1		Major2		Minor1				
onflicting Flow All	0	0	1620	0		810			
Stage 1	-	-	.020	-	1606	-			
Stage 2					858	-			
itical Hdwy			5.34		5.74	7.14			
ritical Hdwy Stg 1	-	-	-	-	6.64	-			
critical Hdwy Stg 2	-		-		6.04	-			
ollow-up Hdwy	-	-	3.12	-	3.82	3.92			
ot Cap-1 Maneuver	-	-	194	-	52	277			
Stage 1	-		-		102	-			
Stage 2	-		-		340	-			
latoon blocked, %			404			077			
lov Cap-1 Maneuver lov Cap-2 Maneuver	- 1	- 1	194	- 1	52 86	277			
Stage 1		- 1	-	- 1	102	-			
Stage 2	- 1	- 1	- 1	- :	340				
Glaye 2					J4U				
pproach	EB		WB		NB				
CM Control Delay, s	0		0.4		33.7				
	- 0		0.4		33.7 D				
CM LOS									
ICM LOS							WBT		
	rt I	NBLn1	NBLn2	EBT	EBR	WBL			
HCM LOS Minor Lane/Major Mvm Capacity (veh/h)	nt I	NBLn1 86	NBLn2 277	EBT -	EBR -	194	-		
Minor Lane/Major Mvm	nt I	86 0.139	277 0.057		-	194 0.157			
finor Lane/Major Mvm apacity (veh/h) ICM Lane V/C Ratio ICM Control Delay (s)		86 0.139 53.5	277 0.057 18.8	-	-	194 0.157 27	0		
inor Lane/Major Mvm apacity (veh/h) CM Lane V/C Ratio		86 0.139	277 0.057	- :	-	194 0.157			

Intersection									
Int Delay, s/veh	14.5								
Movement	NBT	NBR	SBL	SBT	SWL	SWR			
Lane Configurations	ተ ቀሰ			417	¥				
Traffic Vol, veh/h	1980	53	13	1300	29	16			
Future Vol, veh/h	1980	53	13	1300	29	16			
Conflicting Peds, #/hr	0	0	0	0	0	0			
Sign Control	Free	Free	Free	Free	Stop	Stop			
RT Channelized		None		None		None			
Storage Length	-		-	-	0	-			
Veh in Median Storage		-	-	0	0	-			
Grade, %	0	-	-	0	0	-			
Peak Hour Factor	96	96	96	96	96	96			
Heavy Vehicles, %	1	1	- 1	1	- 1	1			
Mvmt Flow	2681	72	18	1760	39	22			
	Major1		Major2		Minor1	4077			
Conflicting Flow All Stage 1	0	0	2753	0	3457 2717	1377			
	- 1		- 1		740	- 1			
Stage 2	-								
Critical Hdwy	-		5.32		5.72	7.12			
Critical Hdwy Stg 1		-	-		6.62	-			
Critical Hdwy Stg 2 Follow-up Hdwy			3.11		3.81	3.91			
Pot Cap-1 Maneuver	- 1		52	- 1	~ 15	117			
Stage 1	- 1	- :	52	- :	~ 19	117			
Stage 1					395				
Platoon blocked. %	- 0	- 1		- 1	333				
Mov Cap-1 Maneuver			52	- 1	0	117			
Mov Cap-1 Maneuver	- 1	- :	- 52	- :	0	1111			
Stage 1	- 1	- :	- 1	- 1	~ 19	- :			
Stage 2	- 0	- 1	- 1	- 1	- 13				
Approach	NB		SB		SW				
HCM Control Delay, s	0		35.1		65.2				
HCM LOS					F				
Minor Lane/Major Mvm	nt	NBT	NBR	SBL	SBTS	WLn1			
Capacity (veh/h)		-	-	52	-	117			
HCM Lane V/C Ratio		-		0.339		0.521			
HCM Control Delay (s)				106.2	34.4	65.2			
HCM Lane LOS			-	F	D	F			
HCM 95th %tile Q(veh)	-	-	1.2	-	2.4			
Notes									
~: Volume exceeds cap	pacity	\$: De	elay exc	eeds 30	00s -	+: Comp	outation Not Defined	*: All major volume in platoon	
			,					,	

	٠	•	•	1	ţ	4	
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
ane Configurations		7	100	ተ ተተ	个个 ()		
raffic Volume (veh/h)	97	51	100	1750	1083	137	
uture Volume (veh/h)	97	51	100	1750	1083	137	
nitial Q (Qb), veh	0	0	0	0	0	0	
Ped-Bike Adj(A_pbT)	1.00	1.00	1.00			1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	
Vork Zone On Approach	No			No	No		
ldj Sat Flow, veh/h/ln	1885	1885	1885	1885	1885	1885	
Adj Flow Rate, veh/h	127	67	131	2298	1422	180	
Peak Hour Factor	0.99	0.99	0.99	0.99	0.99	0.99	
Percent Heavy Veh, %	1	1	1	1	1	1	
Cap, veh/h	151	135	350	4286	3549	449	
Arrive On Green	0.08	0.08	0.06	1.00	1.00	1.00	
Sat Flow, veh/h	1795	1598	1795	5316	4795	585	
Grp Volume(v), veh/h	127	67	131	2298	1055	547	
Grp Sat Flow(s),veh/h/ln	1795	1598	1795	1716	1716	1780	
Serve(g_s), s	9.8	5.6	2.2	0.0	0.0	0.0	
Cycle Q Clear(g_c), s	9.8	5.6	2.2	0.0	0.0	0.0	
Prop In Lane	1.00	1.00	1.00	4000	0000	0.33	
ane Grp Cap(c), veh/h	151 0.84	135 0.50	350 0.37	4286 0.54	2632 0.40	1365 0.40	
//C Ratio(X)	245	218	425	4286	2632	1365	
Avail Cap(c_a), veh/h HCM Platoon Ratio	1.00	1.00	2.00	2.00	2.00	2.00	
Jpstream Filter(I)	1.00	1.00	0.69	0.69	0.09	0.09	
Jniform Delay (d), s/veh	63.2	61.3	2.6	0.09	0.09	0.09	
ncr Delay (d2), s/veh	6.5	1.1	0.2	0.3	0.0	0.1	
nitial Q Delav(d3).s/veh	0.0	0.0	0.0	0.0	0.0	0.0	
6ile BackOfQ(50%), veh/ln	4.7	5.0	0.5	0.1	0.0	0.0	
Jnsig. Movement Delay, s/veh							
nGrp Delay(d),s/veh	69.6	62.3	2.8	0.3	0.0	0.1	
nGrp LOS	Е	Е	A	Α	Α	Α	
Approach Vol, veh/h	194			2429	1602		
Approach Delay, s/veh	67.1			0.5	0.1		
Approach LOS	Е			Α	Α		
Times Assissed Dhe		2		4	5	6	
imer - Assigned Phs		122.3		17.7			
Phs Duration (G+Y+Rc), s		5.7		5.9	9.2 5.0	113.1 * 5.7	
Change Period (Y+Rc), s		109.3		19.1	10.0	* 95	
Max Green Setting (Gmax), s Max Q Clear Time (q_c+l1), s		2.0		11.8	4.2	2.0	
Green Ext Time (p_c), s		3.7		0.1	0.0	1.7	
u = /-		0.1		0.1	0.0		
ntersection Summary HCM 6th Ctrl Delay			3.4				
ICM 6th LOS			3.4 A				
lotes							
Jser approved pedestrian inter HCM 6th computational engir						ses crossi	ng the barrier.

Intersection Int Delay, sheh 3.2 Movement EBL EBT WBT WBR SBL SBR Lane Configurations 3.4 ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑										
Movement EBL EBT WBT WBR SBL SBR	Intersection									
Lane Configurations	Int Delay, s/veh	3.2								
Traffic Vol. vehh 1 4 1451 2197 18 8 10 Conflicing Peds, #thr 0 0 0 0 0 0 0 Conflicing Peds, #thr 0 0 0 0 0 0 0 Sign Control Major 1 None None None Storage Langth - 0 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0	Movement	EBL	EBT	WBT	WBR	SBL	SBR			
Traffic Vol. vehh 1 4 1451 2197 18 8 10 Conflicing Peds, #thr 0 0 0 0 0 0 0 Conflicing Peds, #thr 0 0 0 0 0 0 0 Sign Control Major 1 None None None Storage Langth - 0 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0	Lane Configurations		4412	** 12		¥.				
Conflicting Pests, #thr 0 0 0 0 0 0 0 0 Sygn Control Free Free Free Stop Stop RT Channelized None None None None Stop Stop RT Channelized None None None None Stop Stop Storage Length	Traffic Vol, veh/h	14		2197	18	8				
Sign Control Free Free Free Stop Stop										
RT Chamelized None None None None Storage Length										
Storage Length -		Free								
Veh in Median Storage, # 0 0 0 0 - 0 - Grade, % 0 0 0 0 - 0 - Peak Hour Factor										
Grade, %	Storage Length									
Peak Hori Factor 98 98 98 98 98 98 98 98 98 98 98 98 98										
Heavy Vehicles, %										
Major/Minor Major1 Major2 Minor2										
Conflicting Flow All 2938 0 - 0 3734 1469 Stage 1 - 29266 - Stage 2 - 0 808 - Citical Hebry Stg 1 - 662 - Citical Hebry Stg 1 - 662 - Citical Hebry Stg 2 - 662 - Citical Hebry Stg 2 - 662 - Citical Hebry Stg 2 - 662 - Citical Hebry Stg 2 - 662 - Citical Hebry Stg 3 - 7 - 7 - Citical Hebry Stg 4 - 7 - 7 - Citical Hebry Stg 5 - 7 - 7 - Citical Hebry Stg 6 - 7 - 7 - Citical Hebry Stg 7 - 7 - 7 - Citical Hebry Stg 7 - 7 - 7 - Citical Hebry Stg 7 - 7 - 7 - Citical Hebry Stg 7 - 7 - 7 - Citical Hebry Stg 7 - Citical Hebry Stg 7 - Citical Hebry Stg 7 - Citical Hebr				2914			13			
Conflicting Flow All 2938 0 - 0 3734 1469 Stage 1 - 29266 - Stage 2 - 808 - Citical Hebry Stg 1 - 662 - Citical Hebry Stg 1 - 662 - Citical Hebry Stg 2 - 602 - Follow-up Hebry 3.11 - 3.81 3.91 Felt Cap-1 Maneuver 4210 101 Stage 1 - 14 - Stage 2 - 364 - Felt Cap-1 Maneuver 4210 101 Mov Cap-2 Maneuver 4210 101 Mov Cap-2 Maneuver 4210 101 Mov Cap-2 Maneuver 4210 101 Mov Cap-2 Maneuver 4310 101 Mov Cap-2 Maneuver 4410 101 Mov Cap-2 Maneuver 4510 5 Stage 1 - 14 - Stage 2 - 364 - Stage 2 - 364 - Stage 1 - 14 - Stage 2 - 364 - Stage 2 - 364 - Stage 2 - 364 - Stage 3 - 364 - Stage 4 - 364 - Stage 5 - 364 - Stage 6 - 364 - Stage 6 - 364 - Stage 7 - 364 - Stage 7 - 364 - Stage 8 - 364 - Stage 8 - 364 - Stage 8 - 364 - Stage 9 - 364 - Stage										
Conflicting Flow All 2938 0 0 3734 1469 Stage 1 - 2926 - Stage 2 - 808 - Stage 2 - 808 - Stage 2 - 808 - Stage 2 - 808 - Stage 2 - 808 - Stage 2 - 808 - Stage 3 - 8572 7.12 Citical Hefwy Stg 1 - 862 - Stage 4 - 802 - Stage 4 - 910 - Stage 1 - 14 - Stage 2 - 364 - Stage 2 - 364 - Stage 2 - 1010 - Stage 1 - 101 - Stage 2 - 364 - Stage 2 - 364 - Stage 2 - 364 - Stage 2 - 364 - Stage 2 - 364 - Stage 3 - 364 - Stage 2 - 364 - Stage 2 - 364 - Stage 2 - 364 - Stage 2 - 364 - Stage 2 - 364 - Stage 2 - 364 - Stage 3 - 364 - Stage 2 - 364 - Stage 3 - 364 - Stage 3 - 364 - Stage 3 - 364 - Stage 4 - 364 - Stage 4 - 364 - Stage 4 - 364 - Stage 5 - 364 - Stage	Major/Minor	Maior1		Maior2	1	Minor2				
Stage 1							1469			
Critical Helay 5.32 - 5.72 7.12 Critical Helay 5.32 - 6.62 Critical Helay Sig 2 - 7.00 Critical Helay Sig 2 - 7.00 Critical Helay Sig 2 - 7.00 Critical Helay Sig 2 - 7.00 Critical Helay Sig 2 - 7.00 Critical Helay Sig 2 - 7.00 Critical Helay Sig 2 - 7.00 Critical Helay Sig 2 - 7.00 Critical Helay Sig 2 - 7.00 Critical Helay Sig 2 - 7.00 Critical Helay Sig 3 - 7.00 Critical Helay Sig 3 - 7.00 Critical Helay Sig 3 - 7.00 Critical Helay Sig 3 - 7.00 Critical Helay Sig 3 - 7.00 Critical Helay Sig 3 - 7.00 Critical Helay Sig 3 - 7.00 Critical Helay Sig 3 - 7.00 Critical Helay Sig 3 - 7.00 Critical Helay Sig 3 - 7.00 Critical Helay Sig 3 - 7.00 Critical Helay Sig 3 - 7.00 Critical Helay Sig 3 - 7.00 Critical Helay Sig 4 - 7.00		-			-					
Critical Halwy Stg 1	Stage 2			-	-	808	-			
Critical Havy Stig 2 6.02 - Follow-up Havy 3 1.1 - 3.81 3.91 Pot Cap-1 Maneuver 42 1.0 101 Stage 1 - 14		5.32					7.12			
Follow-up Hayy 3.11 3.81 3.91 Pet Cap-I Maneuver 42 10 101 Stage 1 - 14 - 3684 - Platon blocked, % 16 101 Mov Cap-1 Maneuver 42 10 101 Mov Cap-1 Maneuver 42 10 101 Mov Cap-1 Maneuver 42 10 - 101 Stage 1 - 14 - Stage 2 - 364 - 10 - 10 101 Stage 2 - 364 - 10 - 10 101 Approach EB WB SB HCM Control Delay, s 1.4 0 \$542.6 HCM LoS F HCM Lors BE WB SB SB HCM Control Delay, s 1.4 0 \$542.6 HCM LoS F HCM Cap-1 Maneuver 42 20 HCM Cap-1 Maneuver 43 1194 HCM Cap-1 Maneuver 44 1194 HCM Control Delay (s) 14.6 3 0 \$42.6 HCM LoS F HCM Los Stage 1										
Pot Cap- Maneuver 4210 101 Stage 1 14 Stage 2 364 Plation blocked, % Mov Cap- Maneuver 4210 101 Mov Cap- Maneuver 4210 101 Mov Cap- Maneuver 4210 101 Mov Cap- Maneuver 410 101 Stage 1 - 14 Stage 2 - 364 Approach EB WB SB HCM Control Delay, s 1.4 0 \$542.6 HCM Control Delay, s 1.4 0 \$542.6 HCM Cape Maneuver 5										
Stage 1										
Stage 2										
Pilaton Bocked, %										
Mov Cap-1 Maneuver 42						304				
Mov Cap 2 Maneuver 10 - Stage 1 14 - Stage 2 364 - Stage 2		42				~ 10	101			
Stage 2						~ 10				
Approach EB WB SB HCM Control Delay, s 1.4 0 \$542.6 HCM LOS F Minor Lane/Major Mvmt EBL EBT WBT WBR SBLn1 Capacity (vehrh) 42 20 HCM Lane V/C Ratio 0.442 1.194 HCM Control Delay (s) 146.3 0 - \$542.6 HCM Lane LOS F A - F HCM Stin Walle Q(vehr) 1.6 3.2 Notes	Stage 1					14				
HCM Control Delay, s 1.4 0 \$542.6 F HCM LOS F Minor LaneMajor Mymt EBL EBT WBT WBR SBLn1 Capacity (vehih) 42 - 20 HCM Lane WC Ratio 0.442 - 1.194 HCM Control Delay (s) 16.3 0 . \$542.6 HCM Lane LOS F A - F HCM S5th %Mile Q(veh) 1.6 - 3.2 Noises	Stage 2	-	-	-	-	364	-			
HCM Control Delay, s 1.4 0 \$542.6 F HCM LOS F Minor LaneMajor Mymt EBL EBT WBT WBR SBLn1 Capacity (vehih) 42 - 20 HCM Lane WC Ratio 0.442 - 1.194 HCM Control Delay (s) 16.3 0 . \$542.6 HCM Lane LOS F A - F HCM S5th %Mile Q(veh) 1.6 - 3.2 Noises										
HCM LOS F Minor Lane/Major Mymt EBL EBT WBT WBR SBLn1	Approach	EB		WB		SB				
Minor Lane/Major Mvmt EBL EBT WBT WBR SBLn1		1.4		0	\$					
Capacity (vehh) 42 - - 20 HCM Lane VIC Ratio 0.442 - - 1.194 HCM Control Delay (s) 146.3 0 - 542.6 HCM Lane LOS F A - F HCM S5fir Willie Q(veh) 1.6 - - 3.2 Notes	HCM LOS					F				
Capacity (vehh) 42 - - 20 HCM Lane VIC Ratio 0.442 - - 1.194 HCM Control Delay (s) 146.3 0 - 542.6 HCM Lane LOS F A - F HCM S5fir Willie Q(veh) 1.6 - - 3.2 Notes										
HCM Lane V/C Ratio 0.442 1.194 HCM Cartion Delay (s) 146.3 0 - \$.542.6 HCM Lane LOS F A - F HCM Stiff Nation Q(veh) 1.6 - 3.2 Notes		ıt				WBR:				
HCM Control Delay (s) 146.3 0 - \$542.6 HCM Lane LOS F A - F HCM 5th %Bile Q(veh) 1.6 3.2 Notes					-					
HCM Lane LOS F A F HCM 95th Wille Q(veh) 1.6 3.2 Notes					-					
HCM 95th %tile Q(veh) 1.6 3.2 Notes										
Notes						-				
			1.6	-		-	3.2			
-: Volume exceeds capacity \$: Delay exceeds 300s +: Computation Not Defined *: All major volume in platoon										
	~: Volume exceeds ca	oacity	\$: De	elay exc	eeds 30	JUS ·	+: Com	outation Not Defined	*: All major volume in platoor	1

0: Pipers Cree	к St.	(NB)	& Cı	uebra	Rd				
tersection									
t Delay, s/veh	17.5								
lovement	EBT	EBR	WBL	WBT	NBL	NBR			
ane Configurations	^ 1			411	Y				
raffic Vol, veh/h	1382	69	58	2215	33	40			
uture Vol, veh/h	1382	69	58	2215	33	40			
onflicting Peds, #/hr	0	0	0	0	0	0			
ign Control T Channelized	Free	Free	Free	Free	Stop	Stop			
torage Length	- 1	None	- 1	IVOILE	0	None			
eh in Median Storage.		- 1		0	0				
rade. %	,# 0	- 1	- 1	0	0	- 1			
eak Hour Factor	98	98	98	98	98	98			
eavy Vehicles, %	1	1	1	1	1	1			
lvmt Flow	1833	92	77	2938	44	53			
lajor/Minor N	Major1		Major2	_1	Minor1				
onflicting Flow All	0	0		0	3208	963			
Stage 1	-	-	1020	-	1879	300			
Stage 2					1329				
itical Hdwy			5.32		5.72	7.12			
itical Hdwy Stg 1	-	-	-		6.62				
ritical Hdwy Stg 2		-			6.02				
ollow-up Hdwy		-	3.11		3.81	3.91			
ot Cap-1 Maneuver		-	138		~ 20	221			
Stage 1	-		-		69				
Stage 2		-			191				
latoon blocked, %	-								
lov Cap-1 Maneuver	-	-	138		~ 20	221			
lov Cap-2 Maneuver				- 1	~ 20				
Stage 1 Stage 2	- 1	- :	- 1	- :	191	- :			
Stage 2	- 1				191				
			14/0		ND				
pproach	EB 0		1.5	0	NB 862.2				
CM Control Delay, s CM LOS	0		1.5	\$	862.2 F				
JWI LOG									
inne I ann Maine Mean		NBLn1	EDT	EBR	WDI	WBT			
linor Lane/Major Mvm apacity (veh/h)		40	EBT	LDK	WBL 138	WDI			
CM Lane V/C Ratio		2.421		- :	0.558				
CM Control Delay (s)	9	862.2	- 1	- :	59.8	0			
CM Lane LOS	,	F			55.0 F	A			
CM 95th %tile Q(veh)		10.5			2.8	-			
otes									
Volume exceeds cap	a aib.	ê. De	lass assa	d- 20	20-		putation Not Defined	*: All major volume in platoon	
voiume exceeds cap	acity	Ş. De	iay exc	eeds 30	JUS -	+. Com	putation Not Delined	. All major volume in piatoon	

latera esta e									
Intersection Int Delay, s/veh	9.3								
**		E0.	I I I I I I	MDE	OP	005			
Movement	EBL		WBT	WBR	SBL	SBR			
Lane Configurations		444		49	Y	57			
Traffic Vol, veh/h Future Vol. veh/h	46	1400 1400		49	8	57			
Conflicting Peds, #/hr	46	1400	2244	49	0	0			
Sign Control	Free		Free	Free	Stop	Stop			
RT Channelized		None	1100	None	- Otop	None			
Storage Length	210				0				
Veh in Median Storage.	# -	0	0		0				
Grade, %	-	0	0		0				
Peak Hour Factor	98	98	98	98	98	98			
Heavy Vehicles, %	1	1	1	1	1	1			
Mvmt Flow	61	1857	2977	65	11	76			
Major/Minor N	Najor1		Major2		Minor2				
Conflicting Flow All	3042	0			3875	1521			
Stage 1	-								
Stage 2	-				865				
Critical Hdwy	5.32				5.72	7.12			
Critical Hdwy Stg 1	-	-	-	-		-			
Critical Hdwy Stg 2	-				6.02				
Follow-up Hdwy	3.11	-	-	-		3.91			
Pot Cap-1 Maneuver	~ 37				~ 8	93			
Stage 1	-	-	-	-	12 340	-			
Stage 2 Platoon blocked, %		- 1	- 1	- 1	340				
Mov Cap-1 Maneuver	~ 37				0	93			
Mov Cap-1 Maneuver	~ 31	- 1	- 1	- 1	0	93			
Stage 1	- 1	- :	- 1	- 1	0	- :			
Stage 2					340				
Approach	FB		WB		SB				
HCM Control Delay, s			0		155.5				
HCM LOS	17.0		U		F				
Minor Lane/Major Mvm		EBL	EBT	WBT	WBR:	SRI n1			
Capacity (veh/h)		~ 37	-		TIDIC.				
HCM Lane V/C Ratio		1.649	- 1			0.927			
HCM Control Delay (s)	5	554.2				155.5			
HCM Lane LOS		F				F			
HCM 95th %tile Q(veh)		6.5	-		-	5.3			
Notes									
~: Volume exceeds cap	noitu	¢. D.	elay exc	oods 2	200	ı: Com	putation Not Defined	*: All major volume in platoon	
voiume exceeds cap	acity	⊅. ⊔€	sidy exc	eeus 3	008	com	pulation NOT Delined	. All major volume in piatoon	

Intersection														
nt Delay, s/veh	1.2													
**		EDT	500	14.001	1400.00		NIDI	NOT		0.01	007	000		
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR		
ane Configurations	-	4	47	47	4			41	4	04	41	44		
Fraffic Vol, veh/h Future Vol. veh/h	3	5	17	47 47	5	8	46 46	1900	4	21	1400	11		
Conflicting Peds, #/hr	0	0	0	47	0	0	46	1900	0	0	1400	- 11		
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free		
RT Channelized	ошр	Olup -	None	Stup	otup	None	1166	1166	None	1166	1166	None		
Storage Length	- 1		NOHE			INUITE	- 1		IVUIIC -			None		
/eh in Median Storage		0			0			0			0			
Grade %		0			0			0			0			
Peak Hour Factor	98	98	98	98	98	98	98	98	98	98	98	98		
leavy Vehicles, %	1	1	1	1	1	1	1	1	1	1	1	1		
Mvmt Flow	4	7	23	62	7	- 11	61	2520	5	28	1857	15		
Major/Minor I	Minor2			Minor1			Major1		1	Major2				
Conflicting Flow All	3055	4568	936	3447	4573	1263	1872	0	0	2525	0	0		
Stage 1	1921	1921		2645	2645							-		
Stage 2	1134	2647		802	1928							-		
Critical Hdwy	6.42	6.52	7.12	6.42	6.52	7.12	5.32			5.32		-		
Critical Hdwy Stg 1	7.32	5.52	-	7.32	5.52		-	-	-	-	-	-		
Critical Hdwy Stg 2	6.72	5.52		6.72	5.52							-		
Follow-up Hdwy	3.81	4.01	3.91	3.81	4.01	3.91	3.11	-	-	3.11		-		
Pot Cap-1 Maneuver	14	~ 1	230	~ 8	~ 1	139	147			68		-		
Stage 1	195	114 48	-	~ 13	48	-	-	-	-	-		-		
Stage 2	195	48		314	114				- 1					
Platoon blocked, %		~ 1	230		~ 1	139	147			68				
Mov Cap-1 Maneuver Mov Cap-2 Maneuver		~ 1	230		~1	139	147			80		-		
Stage 1	44	114	- :	~ 13	48	- :	- 1	- 1	- 1	- 1	- :	- 1		
Stage 2	155	48		267	114		- 1				- 1			
Olage 2	100	70		201	114									
Approach	EB			WB			NB			SB				
HCM Control Delay, s							1.1			1.3				
HCM LOS														
Minor Lane/Major Mvm	rt	NBL	NBT	NBR I	EBLn1\	VBLn1	SBL	SBT	SBR					
Capacity (veh/h)		147					68							
HCM Lane V/C Ratio		0.415		-	-		0.41	-	-					
HCM Control Delay (s)		45.9	0			-	90.7	0	-					
TOW CONTROL Delay (S)		Е	Α	-	-		F	Α	-					
		1.8	-	-	-		1.6	-	-					
HCM Lane LOS														
HCM Lane LOS HCM 95th %tile Q(veh)														
HCM Control Delay (s) HCM Lane LOS HCM 95th %tile Q(veh) Notes ~: Volume exceeds cap	acity	\$: De	elay exc	eeds 30	00s	+: Comp	outation	Not De	fined	*: All	major v	olume in	platoon	

	۶	→	•	1	•	•	1	1	1	-	↓	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SB
Lane Configurations	×	^^		×	11		¥	1	7	,	4	
Traffic Volume (veh/h)	204	1181	27	60	1800	59	46	26	89	136	114	43
Future Volume (veh/h)	204	1181	27	60	1800	59	46	26	89	136	114	43
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.0
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.0
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1885	1885	1885	1885	1885	1885	1885	1885	1885	1885	1885	188
Adj Flow Rate, veh/h	279	1616	37	82	2463	81	63	36	122	186	156	59
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.9
Percent Heavy Veh, %	1	1	1	1	1	1	1	1	1	1	1	
Cap, veh/h	265	3332	76	256	2910	95	165	123	104	302	244	20
Arrive On Green	0.11	0.64	0.64	0.03	0.57	0.57	0.04	0.06	0.06	0.11	0.13	0.1
Sat Flow, veh/h	1795	5176	119	1795	5119	167	1795	1885	1598	1795	1885	159
Grp Volume(v), veh/h	279	1071	582	82	1646	898	63	36	122	186	156	59
Grp Sat Flow(s),veh/h/ln	1795	1716	1864	1795	1716	1855	1795	1885	1598	1795	1885	159
Q Serve(g_s), s	15.0	22.6	22.6	2.7	55.7	56.7	4.5	2.5	9.1	13.2	11.0	18
Cycle Q Clear(g_c), s	15.0	22.6	22.6	2.7	55.7	56.7	4.5	2.5	9.1	13.2	11.0	18
Prop In Lane	1.00		0.06	1.00		0.09	1.00		1.00	1.00		1.0
Lane Grp Cap(c), veh/h	265	2208	1200	256	1951	1055	165	123	104	302	244	20
V/C Ratio(X)	1.05	0.49	0.49	0.32	0.84	0.85	0.38	0.29	1.17	0.62	0.64	2.8
Avail Cap(c_a), veh/h	265	2208	1200	326	1961	1060	216	123	104	302	244	20
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.0
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.0
Uniform Delay (d), s/veh	47.7	12.9	12.9	12.4	25.0	25.3	57.8	62.4	65.4	51.8	57.8	60
Incr Delay (d2), s/veh	70.4	0.8	1.4	0.3	4.7	8.7	0.5	0.5	142.8	2.8	4.3	856
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.
%ile BackOfQ(50%),veh/ln	14.0	8.2	9.2	1.0	22.1	25.5	2.1	1.2	7.8	6.1	5.5	56
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	118.2	13.7	14.3	12.7	29.7	33.9	58.3	62.9	208.3	54.6	62.1	917
LnGrp LOS	F	В	В	В	C	C	E	E	F	D	E	
Approach Vol, veh/h		1932			2626			221			936	
Approach Delay, s/veh		29.0			30.6			141.8			603.1	
Approach LOS		С			С			F			F	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	20.0	85.0	20.0	15.0	9.5	95.5	11.0	24.0				
Change Period (Y+Rc), s	5.0	* 5.4	5.0	5.9	5.0	5.4	5.0	* 5.9				
Max Green Setting (Gmax), s	15.0	* 80	15.0	9.1	10.0	84.6	10.0	* 15				
Max Q Clear Time (q. c+l1), s	17.0	58.7	15.2	11.1	4.7	24.6	6.5	20.1				
Green Ext Time (p_c), s	0.0	7.4	0.0	0.0	0.0	3.9	0.0	0.0				
Intersection Summary												
HCM 6th Ctrl Delay			128.1									
HCM 6th LOS			F									
Notes												
User approved pedestrian inter * HCM 6th computational engir						es crossi	ng the ba	rrier.				
Culebra Road Corridor Study 5	-00 nm	12/15/202	n 2020 D	M Evictin	a Conditio					0,	nchro 10	Dane

Delay, siveh	97: Avenue G	x Culi	ерга і	κα			
Delay, siveh 1.2 Delay, siveh 1.2 Delay, siveh 1.2 Del							
Stage 1	ntersection						
ane Configurations	Int Delay, s/veh	1.2					
affic Vol. vehinh	Movement	EBT	EBR	WBL	WBT		NBR
uture Vol. verbin 750 3 5 991 54 12 ontological public services of the Control Free Free Free Free Stop Stop T Channelized None None None I Channelized None None None None None None None None	Lane Configurations	†î>		٦	^	Y	
onflicting Peds, #thr 0	Traffic Vol, veh/h						
gn Control Free Free Free Free Stop Stop (Tchannelizad None No	Future Vol, veh/h						
T.Channelizard None							
Drage Length - 120							
bih in Median Storage, # 0 - 0 0 - and thour Factor		-					
rade, % 0 - 0 0 - ask Hour Factor 94 94 94 94 94 94 94 94 94 94 94 94 94		. # 0					
ask Hour Factor 94 94 94 94 94 94 94 94 94 94 94 94 94							
early Vehicles, % 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2							
Algorithmor Major1 Major2 Minor1							
ontlicting Flow All 0 0 1041 0 1739 521 Slage 1 - 1039 - 1039 - 1039 - 1039 - 1039 - - 1039 - - 1039 - - 1039 - - 1039 - - 1039 - - - 700 -<	WIVING FIOW	1037	4	- /	13/1	15	17
ontlicting Flow All 0 0 1041 0 1739 521 Slage 1 - 1039 - 1039 - 1039 - 1039 - 1039 - - 1039 - - 1039 - - 1039 - - 1039 - - 1039 - - - 700 -<							
Stage 1							
Stage 2							
Intical Holwy Stg 2							-
rifical Halvy Stg 1							
Infinized Holdy Stig 2							
200							
at Cap-1 Maneuver 664 78 500 Stage 1 - 302							
Slage							
Slage 2							
atton blocked, %				- 1			- 1
ov Cap-1 Maneuver 664 77 500 ov Cap-2 Maneuver - 198 - Stage 1 - 302 - Stage 2 - 449 - opproach EB WB NB CM Control Delay, s 0 0.1 32.1 CM Los D D - inor Lane/Major Mvmt NBLn1 EBT EBR WBT apachy (welhh) 222 664 - CM Lane V/C Ratio 0.411 - 0.01 - CM Control Delay (s) 32.1 - 10.5 - CM Control Delay (s) 3.2 - 10.5 - S D - B - - - S M - - <td></td> <td></td> <td></td> <td></td> <td></td> <td>704</td> <td></td>						704	
ov Cap-Z Maneuver - 198 Slage 1 - 302 Slage 2 - 449 oprosech EB WB NB CM Control Delay, s 0 0.1 32.1 CM LOS D D Image: Control Delay (s) 0.0 on Lose Vice Ratio 0.411 - 0.01 - CM Lane LOS 0.411 - 0.01 - CM Spith %sile Q(veh) 1.9 - 0 - ulebra Road Corridor Study 5:00 pm 12/15/2020 2020 PM Existing Condition - - -				664		77	500
Slage 1	Mov Cap-2 Maneuver						
Stage 2							
Deproach EB WB NB Deproach EB WB NB Deproach EB WB NB Deproach							
CM Control Delay, s 0 0.1 32.1 D INDEX. ST. ST. ST. ST. ST. ST. ST. ST. ST. ST							
CM Control Delay, s 0 0.1 32.1 D INDEX. ST. ST. ST. ST. ST. ST. ST. ST. ST. ST	Approach	En		WD		ND	
CM LOS D inor Lane/Major Mvmt NBLn1 EBT EBR WBL WBT apacity (vehh)h 222 - 664 - CM Lane V/C Rato 0.411 - 0.01 - CM Control Delay (s) 32.1 - 10.5 - CM Control Delay (s) 32.1 - 10.5 - CM Control Delay (s) 1.9 - 8 - CM 95th %dile O(veh) 1.9 - 0 -							
inor Lane/Major Mvmt NBLn1 EBT EBR WBL WBT apacity (vehit) 222 - 664 - OM Lane VIC Ratio 0.411 - 0.01 - OM Control Delay (s) 32.1 - 10.5 - OM Lane LOS D - B - OM 99th %sile Q(veh) 1.9 - 0 -		0		0.1			
apacity (vehit) 222 - 664 - ML ane VIC Ratio 0.411 - 0.01 - 0.01 CM Control Delay (s) 32.1 - 10.5 - CM Lane LOS D - B - CM 95th %dile Q(veh) 1.9 - 0 -	HOW LUS					U	
apacity (vehit) 222 - 664 - ML ane VIC Ratio 0.411 - 0.01 - 0.01 CM Control Delay (s) 32.1 - 10.5 - CM Lane LOS D - B - CM 95th %dile Q(veh) 1.9 - 0 -							
ONL Lane VIC Ratio 0.411 - 0.01 - 0.00 - 0.0		nt					
CM Control Delay (s) 32.1 - 10.5 - CM Lane LOS D - B - CM 195th %sile Q(veh) 1.9 - 0 -							
ON Lane LOS							
OM 95th %dile Q(veh) 1.9 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 -)					
ulebra Road Corridor Study 5:00 pm 12/15/2020 2020 PM Existing Condition		.)					
	HOW SOUL WILLE CHICK	1)	1.9			U	
/SP - San Antonio		r Study	5:00 pm	12/15/	2020 20	020 PM	Existin
	WSP - San Antonio						

Internetion									
Intersection Int Delay, s/veh	14.6								
**									
Movement	EBL	EBT	WBT	WBR	SBL	SBR			
Lane Configurations	18	41	† 1>	121	42	38			
Traffic Vol, veh/h Future Vol. veh/h	18	874 874	900	121	42	38			
Conflicting Peds, #/hr	18	0 0	900	121	42	38			
Sign Control		Free	Free	Free	Stop	Stop			
RT Channelized		None	1100	None	- Otop	None			
Storage Length				-	0	-			
Veh in Median Storage,	# -	0	0		0				
Grade, %		0	0		0				
Peak Hour Factor	94	94	94	94	94	94			
Heavy Vehicles, %	2	2	2	2	2	2			
Mvmt Flow	25	1209	1245	167	58	53			
Major/Minor N	lajor1	- 1	Major2	- 1	Minor2				
Conflicting Flow All	1412	0	-		1984	706			
Stage 1									
Stage 2	-		-		655				
Critical Hdwy	4.14				6.84	6.94			
Critical Hdwy Stg 1	-	-	-	-		-			
Critical Hdwy Stg 2	-		-		5.84	-			
Follow-up Hdwy	2.22		-	-	3.52	3.32			
Pot Cap-1 Maneuver	479				~ 53	378			
Stage 1	-		-	-	212 479	-			
Stage 2 Platoon blocked. %		- 1	- 1	- 1	4/9				
Mov Cap-1 Maneuver	479				~ 45	378			
Mov Cap-1 Maneuver	4/9	- :	- 1	- 1	~ 45	3/0			
Stage 1					178				
Stage 2					479				
Approach	FB		WB		SB				
HCM Control Delay, s	1.2		0	S	349.8				
HCM LOS	1.2		0	Ψ	545.0 F				
Minor Lane/Major Mvmt		EBL	EBT	WBT	WBR:	CDI n1			
Capacity (veh/h)		479	EDI.	1101	WDR				
HCM Lane V/C Ratio		0.052	- 1	- :		1.437			
HCM Control Delay (s)		12.9	1	- 1		349.8			
HCM Lane LOS		В	A			F			
HCM 95th %tile Q(veh)		0.2	-		-	8.9			
. ,									
Notes	anit.	£. D.	lass as	Of	20-		udelies Net Defer	t. All major volume is alatere	
~: Volume exceeds cap	acity	2: De	nay exc	eeds 30	JUS ·	+. Com	outation Not Defined	*: All major volume in platoon	

	٠	-	•	•	•	•	4	Ť	/	/	ţ	✓	
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	N.	^	7	ň	ħβ		16	↑ ↑		77	ħβ		
Traffic Volume (veh/h)	74	600	303	200	555	165	329	495	209	290	567	108	
Future Volume (veh/h)	74	600	303	200	555	165	329	495	209	290	567	108	
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0	
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Work Zone On Approach	1	No			No			No			No		
	1885	1885	1885	1885	1885	1885	1885	1885	1885	1885	1885	1885	
Adj Flow Rate, veh/h	98	796	0	265	736	0	436	657	0	385	752	0	
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	
Percent Heavy Veh. %	1	1	- 1	1	1	1	- 1	- 1	1	- 1	1	1	
Cap, veh/h	317	1091		355	1319		497	891		448	840		
Arrive On Green	0.05	0.30	0.00	0.12	0.37	0.00	0.14	0.25	0.00	0.13	0.23	0.00	
Sat Flow, veh/h	1795	3582	1598	1795	3676	0	3483	3676	0	3483	3676	0	
Grp Volume(v), veh/h	98	796	0	265	736	0	436	657	0	385	752	0	
Grp Sat Flow(s),veh/h/ln	1795	1791	1598	1795	1791	0	1742	1791	0	1742	1791	0	
Q Serve(q s), s	3.7	19.9	0.0	9.7	16.3	0.0	12.3	16.9	0.0	10.8	20.3	0.0	
Cycle Q Clear(q_c), s	3.7	19.9	0.0	9.7	16.3	0.0	12.3	16.9	0.0	10.8	20.3	0.0	
Prop In Lane	1.00		1.00	1.00		0.00	1.00		0.00	1.00		0.00	
Lane Grp Cap(c), veh/h	317	1091		355	1319		497	891		448	840		
	0.31	0.73		0.75	0.56		0.88	0.74		0.86	0.90		
Avail Cap(c_a), veh/h	499	1091		441	1319		505	1006		470	971		
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Upstream Filter(I)	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00	
Uniform Delay (d), s/veh		31.1	0.0	22.1	25.1	0.0	42.0	34.6	0.0	42.7	37.1	0.0	
Incr Delay (d2), s/veh	0.2	4.3	0.0	3.8	1.7	0.0	15.1	2.0	0.0	13.5	9.0	0.0	
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
%ile BackOfQ(50%), vehi		8.8	0.0	4.1	6.9	0.0	6.3	7.5	0.0	5.5	9.8	0.0	
Unsig. Movement Delay,	s/veh												
LnGrp Delay(d),s/veh	22.6	35.4	0.0	25.9	26.8	0.0	57.1	36.6	0.0	56.2	46.1	0.0	
LnGrp LOS	С	D		С	С		Е	D		Е	D		
Approach Vol, veh/h		894	Α		1001	Α		1093	Α		1137	Α	
Approach Delay, s/veh		34.0			26.6			44.8			49.5		
Approach LOS		С			C			D			D		
Times Andrew d Die		^	^		-	^	-						
Timer - Assigned Phs	1	2	3	4	5	6	7	8					
Phs Duration (G+Y+Rc),		42.0	17.4	30.8	16.2	35.7	18.8	29.4					
Change Period (Y+Rc),		* 5.2	4.5	* 5.9	4.5	* 5.2	4.5	* 5.9					
Max Green Setting (Gma		* 23	13.5	* 28	16.5	* 22	14.5	* 27					
Max Q Clear Time (g_c+		18.3	12.8	18.9	11.7	21.9	14.3	22.3					
Green Ext Time (p_c), s	0.0	0.9	0.0	1.4	0.1	0.0	0.0	1.1					
Intersection Summary													
HCM 6th Ctrl Delay			39.3										
HCM 6th LOS			D										
Notes													
User approved pedestria	n inte	rval to h	ne lese	than rh	ase ma	y oreer	1						
* HCM 6th computationa								ases cr	nssina	he harr	ier		
Unsignalized Delay for [1												intercor	rtion delay
anaignamed Delay IOI [I	101 L	-DIX, 41	טויי, טט	11/10/07	www	noill to	arcuidill	110 01 (11	io appii	JOINT UE	iuy allu	111101300	stion dolay.

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EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SE
,	† 1,		À	† 13			4			413	
	720	1	5	1029	6	6	0	4		5	
		1900			1900	1900		1900	1900		19
											0.
											130
		6%			6%			6%			- 6
						Perm			Perm		
	6 26			2 22			4 24			8 28	
						4 24			8 28		
	0.71			0.70			0.09			0.09	
							120			236	
	0.31			c0.44							
A			В								
	A			В			E			E	
			_	_					_		
		10.1	Н	CM 2000	Level of	Service		В			
ity ratio		0.58									
		140.0	S	um of lost	time (s)			27.6			
		54.9%		U Level o				A			
on		15									
	35 35 35 36 1900 4.4 1.00 0.95 1703 0.12 211 0.89 130% 51 6% 60 102.7 102.7 102.7 102.7 102.7 102.7 102.7 102.7 102.7 102.7 103.8 10	35 720 1990 1990 1990 144 5.8 1.00 0.95 1.00 0.95 1.00 0.95 1.00 1703 3405 0.12 1.00 2.11 3405 0.89 0.89 1.0	36 720 1 35 720 1 35 720 1 1900 1900 1900 1900 4.4 5.8 1.00 0.95 1.00 1.00 0.95 1.00 0.12 100 0.1703 3405 0.12 100 0.11 3405 0.89 0.89 0.89 0.89 0.89 0.89 130% 130% 130% 130% 51 1052 1 0 0 0 0 51 1053 0 6% 6% 6% 6% 6% 6% 6% 6% 6% 102.7 99.3 0.73 0.71 4.4 1.0 191 2415 0.010 0.31 0.19 0.27 0.44 1.0 0.99 0.77 0.45 0.38 0.40 0.38 0.40 0.77 3.5 0.37 A	35 720 1 5 35 720 1 5 35 720 1 5 35 720 1 5 35 720 1 5 35 720 1 5 35 720 1 5 36 720 1 5	36 720 1 5 1029 35 720 1 5 1029 35 720 1 5 1029 19900 19900 19900 19900 1900 1900 100 0.55 0.00 0.00 0.00 0.00 0.00 0.00 0	35 720 1 5 1029 6 35 720 1 5 1029 6 350 720 1 5 1029 6 350 720 1 5 1029 6 1900 1900 1900 1900 1900 1900 1900 1900	36	35 720 1 5 1029 6 6 6 0 35 720 1 5 1029 6 6 6 0 19900 19900 1990 1990 1990 1990 1990	35 720 1 5 1029 6 6 0 4 1900 1900 1900 1900 1900 1900 1900 1900	35 720 1 5 1029 6 6 6 0 4 22 35 720 1 5 1029 6 6 6 0 4 22 19900 1990 1990 1990 1990 1990 1990 199	35 720

Internation						
Intersection	1.2					
Int Delay, s/veh						
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	† ₽		ী	**		7
Traffic Vol, veh/h	930	150	71	921	0	73
Future Vol, veh/h	930	150	71	921	0	73
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None		Stop
Storage Length			160	-	-	0
Veh in Median Storage,			-	0	0	
Grade, %	92	92	92	92	92	92
Peak Hour Factor		92	92	92	92	92
Heavy Vehicles, % Mvmt Flow	1314	212	100	1301	0	103
MVIIIL FIOW	1314	212	100	1301	U	103
	Major1		Major2		Minor1	
Conflicting Flow All	0	0		0	-	763
Stage 1	-		-			
Stage 2	-		-	-	-	-
Critical Hdwy	-		4.12			6.92
Critical Hdwy Stg 1	-		-	-	-	-
Critical Hdwy Stg 2						
Follow-up Hdwy			2.21			3.31
Pot Cap-1 Maneuver			438	-	0	349
Stage 1	-		-	-	0	-
Stage 2				-	0	
Platoon blocked, %	-	-	438	-		349
Mov Cap-1 Maneuver		-				
Mov Cap-2 Maneuver	-	-	-	-	-	
Stage 1			-	-	-	
Stage 2	-	-	-		-	
Approach	EB		WB		NB	
HCM Control Delay, s	0		1.1		19.6	
HCM LOS					С	
Minor Lane/Major Mvmt		NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		349	LUI	LDIX	438	******
HCM Lane V/C Ratio		0.296	- 1		0.229	- 1
HCM Control Delay (s)		19.6			15.6	
HCM Lane LOS		C			C	
HCM 95th %tile Q(veh)		1.2			0.9	

137: Culebra Rd	IV								02/03/20
Intersection									
Int Delay, s/veh	2.9								
Movement	EBL	EBT	WBT	WBR	SBL	SBR			
Lane Configurations	EDL	414	†1 >	WDIN	N/	SBIN			
Traffic Vol. veh/h	37	950	924	32	31	46			
Future Vol. veh/h	37	950	924	32	31	46			
Conflicting Peds. #/hr	0	0	0.0	0	0	0			
Sign Control	Free	Free	Free	Free	Stop	Stop			
RT Channelized	-	None				None			
Storage Length	-				0				
Veh in Median Storage,	# -	0	0		0				
Grade, %	-	0	0	-	0	-			
Peak Hour Factor	94	94	94	94	94	94			
Heavy Vehicles, %	0	0	0	0	0	0			
Mvmt Flow	51	1314	1278	44	43	64			
Major/Minor M	lajor1	- 1	Major2	- 1	Minor2				
	1322	0		0	2059	661			
Stage 1	-								
Stage 2	-				759				
Critical Hdwy	4.1				6.8	6.9			
Critical Hdwy Stg 1	-		-	-	5.8	-			
Critical Hdwy Stg 2	-				5.8				
Follow-up Hdwy	2.2		-	-	3.5	3.3			
Pot Cap-1 Maneuver	529				49	410			
Stage 1	-	-	-	-	223	-			
Stage 2	-				428				
Platoon blocked, %	529	-	-	-		410			
Mov Cap-1 Maneuver	529	- 1	- 1		~ 32	410			
Mov Cap-2 Maneuver Stage 1		- :	- 1	- 1	143	- 1			
Stage 2	- 1	- 1	- 1		428				
Staye 2					420				
Approach	FB		WB		SB				
HCM Control Delay, s	2.5		0		44.5				
HCM LOS	2.0		U		44.5				
110111 200									
Minor Lane/Major Mymt		EBL	EBT	WBT	WBR:	CDI n1			
Capacity (veh/h)		529	EDI.	WDI	WDR	193			
HCM Lane V/C Ratio		0.097	- 1	- 1		0.552			
HCM Control Delay (s)		12.5	2.1	- 1	- 1	44.5			
HCM Lane LOS		12.5 B	Α.			E			
HCM 95th %tile Q(veh)		0.3	-		-	2.9			
		-							
Notes	a alte	6. 0	Jan a	O	20-	Carr	entation Nat Deferred	t. All maior values in alterna	
~: Volume exceeds capa	acity	\$: De	elay exc	eeds 30	JUS ·	+: Com	outation Not Defined	*: All major volume in platoon	

	Ave 8	& Cul	epra	Kd			
rsection							
Delay, s/veh	2.7						
vement	EBT	EBR	WBL	WBT	NBL	NBR	
e Configurations	† 1>			414	¥		
fic Vol, veh/h	920	19	36	930	33	31	
ure Vol, veh/h	920	19	36	930	33	31	
flicting Peds, #/hr	0	_ 0	0	0	0	0	
Changelined	Free	Free	Free	Free	Stop	Stop	
Channelized rage Length		None		None -	0	None	
in Median Storage		- 1	- 1	0	0	- 1	
de. %	0	- 0	- 1	0	0	- 1	
k Hour Factor	91	91	91	91	91	91	
vy Vehicles, %	1	1	- 1	1	- 1	1	
nt Flow	1314	27	51	1329	47	44	
or/Minor	Major1		Major2	1	Minor1		
flicting Flow All	0	0	1341	0	2095	671	·
Stage 1					1328		
Stage 2					767		
cal Hdwy			4.12		6.82	6.92	
cal Hdwy Stg 1	-	-	-	-	5.82	-	
cal Hdwy Stg 2 ow-up Hdwy	- 1	- 1	2.21	- :	3.51	3.31	
Cap-1 Maneuver		- 1	515		~ 45	401	
Stage 1	- 1	- :	010	- 1	214	401	
Stage 2					421		
oon blocked, %							
Cap-1 Maneuver	-	-	515	-	~ 28	401	
Cap-2 Maneuver	-	-	-	-	120	-	
Stage 1	-	-	-	-	214	-	
Stage 2					262		
oach	EB		WB		NB		
	0		2.6		43.2		
A Control Delay, s					Е		
A Control Delay, s							
I Control Delay, s I LOS						WBT	
I Control Delay, s I LOS or Lane/Major Mvn	it I	NBLn1	EBT	EBR	WBL		
I Control Delay, s I LOS I Lane/Major Mvn acity (veh/h)	it I	182	-	-	515	-	
M Control Delay, s M LOS or Lane/Major Mvn acity (veh/h) M Lane V/C Ratio		182 0.502	- 1	EBR -	515 0.1	- :	
M Control Delay, s M LOS or Lane/Major Mvn acity (veh/h) M Lane V/C Ratio M Control Delay (s)		182 0.502 43.2			515 0.1 12.8	2.2	
M Control Delay, s M LOS or Lane/Major Mvn acity (veh/h) M Lane V/C Ratio M Control Delay (s) M Lane LOS		182 0.502 43.2 E	- 1	-	515 0.1 12.8 B	- :	
M Control Delay, s M LOS or Lane/Major Mvn Pacity (veh/h) M Lane V/C Ratio M Control Delay (s) M Lane LOS M 95th %tile Q(veh		182 0.502 43.2			515 0.1 12.8	2.2	
M Control Delay, s M LOS or Lane/Major Mvn acity (veh/h) M Lane V/C Ratio M Control Delay (s) M Lane LOS M 95th %tile Q(veh as)	182 0.502 43.2 E 2.5	-	-	515 0.1 12.8 B 0.3	2.2 A	
Control Delay, s LOS Lane/Major Mvn city (veh/h) Lane V/C Ratio Control Delay (s) Lane LOS 95th %tile Q(veh)	182 0.502 43.2 E 2.5	-		515 0.1 12.8 B 0.3	2.2 A	putation Not Defined *: All major volume in plator

Interception						
Intersection Int Delay, s/veh	1					
**		505	MIDI	WD=	ND	NDC
Movement	EBT	EBR	WBL	WBT	NBL	NBR
ane Configurations	↑↑ >	27	9	1120	18	12
ramic voi, ven/n future Vol. veh/h	910	27	9	1120	18	12
onflicting Peds, #/hr	910	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized		None			- Otop	None
torage Length		-		-	0	-
eh in Median Storage,	# 0			0	0	
rade, %	0		-	0	0	-
Peak Hour Factor	94	94	94	94	94	94
eavy Vehicles, %	- 1	1	- 1	1	- 1	1
/lvmt Flow	1259	37	12	1549	25	17
Major/Minor M	lajor1	1	Major2	1	Minor1	
Conflicting Flow All	0		1296	0	2077	648
Stage 1	-	-	-		1278	-
Stage 2	-	-	-	-	799	-
Critical Hdwy	-		4.12		6.82	6.92
Critical Hdwy Stg 1	-	-	-	-		-
Critical Hdwy Stg 2	-	-		-	5.82	-
ollow-up Hdwy	-	-			3.51	3.31
Pot Cap-1 Maneuver	-		536	-	47	415
Stage 1 Stage 2			-	- 1	227 406	-
Platoon blocked, %	- 1	- 1		- 1	400	-
Mov Cap-1 Maneuver	-		536	- :	39	415
ov Cap-1 Maneuver		- 1	330		141	410
Stage 1	-		-		227	
Stage 2					341	
proach	EB		WB		NB	
CM Control Delay, s	0		1		28.8	
ICM Control Delay, s	U		- 1		28.8 D	
JIII EOO					U	
ines Lene (Meios 24		NDI	EDT	EDD	MDI	WDT
inor Lane/Major Mvmt		NBLn1	EBT	EBR .	WBL	WBT
Capacity (veh/h) HCM Lane V/C Ratio		0.216			536 0.023	- 1
HCM Control Delay (s)		28.8	- 1	- 1	11.9	0.9
ICM Lane LOS		20.0 D	- 1		11.9 B	0.9 A
ICM 95th %tile Q(veh)		0.8		- :	0.1	Α.
2501 70010 Q(F011)		0.0			0.1	

158: NW 28th S		Jaiobi				
Interception						
Intersection Int Delay, s/veh	0.2					
Movement		EBR	WBL	WBT	NBL	NBR
Lane Configurations	† ⊅			41	Y	
Traffic Vol, veh/h	800	2	1	1050	9	9
Future Vol, veh/h	800	2	1	1050	9	9
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free		Free	Free	Stop	Stop
RT Channelized		140110	-			None
Storage Length		-	-	-	0	-
Veh in Median Storage,		-	-	0	0	
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	1130	3	1	1484	13	13
Major/Minor N	Major1	,	Major2		Minor1	
Conflicting Flow All	0	0	1133	0	1876	567
Stage 1	-	U	1100	-		307
Stage 2		- :	- 1	- 1	744	
Critical Hdwy	- :			- 1	6.84	6.94
	- 1	- 1	4.14	- 1	5.84	
Critical Hdwy Stg 1 Critical Hdwy Stg 2		- 1	- 1			
	- 1	- 1		- 1	3.52	3.32
Follow-up Hdwy		- 1	612			3.32 467
Pot Cap-1 Maneuver Stage 1	- 1	- 1	612	- 1	63 270	467
		-	-		431	
Stage 2 Platoon blocked. %		-			431	
		-	040		00	407
Mov Cap-1 Maneuver	-		612		62	467
Mov Cap-2 Maneuver	-		-	-	178	-
Stage 1		-			270	-
Stage 2		-			427	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		20.5	
HCM LOS	U		U		20.0	
1101111200					ŭ	
Minor Lane/Major Mvm	t I	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		258	-	-	612	-
HCM Lane V/C Ratio		0.099	-	-	0.002	-
HCM Control Delay (s)		20.5	-	-	10.9	0
HCM Lane LOS		С	-	-	В	Α
HCM 95th %tile Q(veh)		0.3	-		0	
Culebra Road Corridor WSP - San Antonio	Study	5:00 pm	12/15/	2020 2	020 PM	Existin

	•	-	*	1	•		1	1	1	1	↓	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SB
Lane Configurations	- 1	† ‡		- 7	44			4			ર્ન	
Traffic Volume (veh/h)	7	920	21	13	949	95	7	1	11	31	3	
Future Volume (veh/h)	7	920	21	13	949	95	7	- 1	11	31	3	
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1885	1885	1885	1885	1885	1885	1885	1885	1885	1885	1885	
Adj Flow Rate, veh/h	10	1259	29	18	1299	130	10	1	15	42	4	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.
Percent Heavy Veh, %	1	1	1	1	1	1	1	1	1	1	1	
Cap, veh/h	355	3109	72	435	2857	285	63	11	44	122	6	
Arrive On Green	1.00	1.00	1.00	0.87	0.87	0.87	0.05	0.05	0.05	0.05	0.05	0.
Sat Flow, veh/h	378	3579	82	432	3289	328 724	462	251	973	1421 46	135	
Grp Volume(v), veh/h	10	630	658	18	705		26					
Grp Sat Flow(s),veh/h/ln	378	1791	1870	432	1791	1826	1686	0	0	1557 1.6	0	
Q Serve(g_s), s	0.3	0.0	0.0	0.7	10.2	10.3	1.8	0.0	0.0	3.3	0.0	(
Cycle Q Clear(g_c), s Prop In Lane	1.00	0.0	0.04	1.00	10.2	0.18	0.38	0.0	0.58	0.91	0.0	0.
Lane Grp Cap(c), veh/h	355	1556	1625	435	1556	1586	118	0	0.30	128	0	U.
V/C Ratio(X)	0.03	0.40	0.41	0.04	0.45	0.46	0.22	0.00	0.00	0.36	0.00	0.
Avail Cap(c_a), veh/h	355	1556	1625	435	1556	1586	302	0.00	0.00	300	0.00	0.
HCM Platoon Ratio	2.00	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.0
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	0.00	1.00	0.00	0.
Uniform Delay (d), s/veh	0.5	0.0	0.0	1.1	1.7	1.7	55.5	0.0	0.0	56.2	0.00	(
Incr Delay (d2), s/veh	0.1	0.8	0.8	0.2	1.0	0.9	0.3	0.0	0.0	0.6	0.0	Ċ
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	(
%ile BackOfQ(50%),veh/ln	0.0	0.3	0.3	0.0	1.6	1.7	0.8	0.0	0.0	1.4	0.0	Ċ
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	0.7	0.8	0.8	1.3	2.7	2.7	55.9	0.0	0.0	56.8	0.0	(
LnGrp LOS	Α	Α	Α	Α	Α	Α	E	Α	Α	E	Α	
Approach Vol, veh/h		1298			1447			26			46	
Approach Delay, s/veh		0.8			2.6			55.9			56.8	
Approach LOS		Α			Α			Е			Е	
Timer - Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		109.3		10.7		109.3		10.7				
Change Period (Y+Rc), s		* 5.1		* 5.2		* 5.1		* 5.2				
Max Green Setting (Gmax), s		* 90		* 20		* 90		* 20				
Max Q Clear Time (g_c+l1), s		12.7		3.8		12.3		5.3				
Green Ext Time (p_c), s		6.3		0.0		7.8		0.0				
Intersection Summary												
HCM 6th Ctrl Delay			3.2									
HCM 6th LOS			Α									
Notes												
* HCM 6th computational engin	e requir	es equal	clearance	times for	the phas	es crossi	ng the ba	rrier.				

	٠	→	•	1	—	•	4	Ť	1	/	Ţ	4	
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	N.	† 1>		ň	↑ ↑		Ŋ	↑ ↑		ň	↑ ↑		
Traffic Volume (veh/h)	111	740	77	86	898	97	105	242	61	142	368	127	
Future Volume (veh/h)	111	740	77	86	898	97	105	242	61	142	368	127	
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0	
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Work Zone On Approach		No			No			No			No		
Adj Sat Flow, veh/h/ln	1885	1885	1885	1885	1885	1885	1885	1885	1885	1885	1885	1885	
Adj Flow Rate, veh/h	147	982	102	114	1191	129	139	321	81	188	488	168	
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	
Percent Heavy Veh, %	1	1	1	1	- 1	1	1	1	1	1	1	- 1	
Cap, veh/h	243	1861	193	292	1827	197	202	479	119	270	452	155	
	0.02	0.19	0.19	0.01	0.18	0.18	0.08	0.17	0.17	0.08	0.17	0.17	
Sat Flow, veh/h	1795	3275	340	1795	3260	352	1795	2842	707	1795	2619	896	
Grp Volume(v), veh/h	147	537	547	114	653	667	139	201	201	188	333	323	
Grp Sat Flow(s),veh/h/ln	1795	1791	1824	1795	1791	1822	1795	1791	1758	1795	1791	1724	
Q Serve(a s), s	4.1	32.4	32.5	3.2	40.5	40.7	7.6	12.6	12.9	10.0	20.7	20.7	
Cycle Q Clear(q_c), s	4.1	32.4	32.5	3.2	40.5	40.7	7.6	12.6	12.9	10.0	20.7	20.7	
	1.00		0.19	1.00		0.19	1.00	.=	0.40	1.00		0.52	
Lane Grp Cap(c), veh/h	243	1018	1037	292	1004	1021	202	302	296	270	309	297	
	0.60	0.53	0.53	0.39	0.65	0.65	0.69	0.66	0.68	0.70	1.08	1.09	
Avail Cap(c_a), veh/h	332	1018	1037	396	1004	1021	255	354	347	270	309	297	
	0.33	0.33	0.33	0.33	0.33	0.33	1.00	1.00	1.00	1.00	1.00	1.00	
Upstream Filter(I)	0.87	0.87	0.87	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Uniform Delay (d), s/veh		34.2	34.2	15.9	38.0	38.1	38.5	46.7	46.9	39.0	49.7	49.7	
Incr Delay (d2), s/veh	0.8	1.7	1.7	0.3	3.3	3.3	3.2	2.4	2.9	6.5	73.3	77.4	
Initial Q Delay(d3).s/veh		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
%ile BackOfQ(50%), veh/		16.0	16.3	1.3	20.3	20.7	3.5	5.8	5.9	5.1	15.5	15.3	
Unsig. Movement Delay,													
	22.3	35.9	35.9	16.2	41.3	41.4	41.7	49.1	49.8	45.4	122.9	127.0	
LnGrp LOS	С	D	D	В	D	D	D	D	D	D	F	F	
Approach Vol, veh/h	_	1231		_	1434	_		541	_	_	844		
Approach Delay, s/veh		34.3			39.3			47.4			107.2		
Approach LOS		C			D			D			F		
••			_			_	-				_		
Timer - Assigned Phs	c0 1	72.4	13.0	25.5	8.1	73.4	12.5	26.0					
Phs Duration (G+Y+Rc),		*5.2	3.0	5.3	3.0	*5.2	12.5	5.3					
Change Period (Y+Rc), s		* 5.2	10.0	23.7	12.0	* 58	13.0	20.7					
Max Green Setting (Gma		42.7	12.0	14.9	5.2	34.5		20.7					
Max Q Clear Time (g_c+ Green Ext Time (p_c), s		2.7	0.0	0.6	0.0	2.2	9.6	0.0					
u = 7:	0.0		0.0	0.0	0.0		0.0	0.0					
Intersection Summary HCM 6th Ctrl Delay			53.0										
HCM 6th LOS			D										
Notes													
Jser approved pedestria	n inte	rval to b	e less	than ph	ase ma	x greer	1.						
* HCM 6th computationa	l engir	ne requ	ires equ	ual clea	rance ti	mes for	the ph	ases cri	ossing t	he barr	ier.		

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
ane Configurations	7	† 1>		*	**			4			4		
Traffic Volume (veh/h)	4	885	32	42	1105	3	30	5	27	5	5	10	
uture Volume (veh/h)	4	885	32	42	1105	3	30	5	27	5	5	10	
nitial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0	
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00	4.00	1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Nork Zone On Approac		No	4070	4070	No	4070	4070	No	4070	4070	No	4070	
Adj Sat Flow, veh/h/ln	1870	1870 1186	1870 43	1870 56	1870 1481	1870	1870	1870	1870 36	1870	1870	1870	
Adj Flow Rate, veh/h Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	
	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	
Percent Heavy Veh, % Cap. veh/h	306	2760	100	362	2932	8	89	15	46	54	44	56	
Arrive On Green	0.00	0.53	0.53	0.02	0.81	0.81	0.07	0.07	0.07	0.07	0.07	0.07	
Sat Flow, veh/h	1781	3498	127	1781	3636	10	663	223	679	242	653	831	
Grp Volume(v), veh/h	5	602	627	56	724	761	83	0	0/3	27	000	031	
Grp Sat Flow(s), veh/h/lr	_	1777	1848	1781	1777	1869	1565	0	0	1727	0	0	
Q Serve(g_s), s	0.1	24.8	24.8	0.7	16.0	16.0	4.4	0.0	0.0	0.0	0.0	0.0	
Cycle Q Clear(g_c), s	0.1	24.8	24.8	0.7	16.0	16.0	6.2	0.0	0.0	1.8	0.0	0.0	
Prop In Lane	1.00	21.0	0.07	1.00	10.0	0.01	0.48	0.0	0.43	0.26	0.0	0.48	
ane Grp Cap(c), veh/h		1402	1458	362	1433	1507	150	0	0	154	0	0	
//C Ratio(X)	0.02	0.43	0.43	0.15	0.51	0.51	0.55	0.00	0.00	0.18	0.00	0.00	
Avail Cap(c_a), veh/h	456	1402	1458	481	1433	1507	284	0	0	295	0	0	
HCM Platoon Ratio	0.67	0.67	0.67	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Jpstream Filter(I)	1.00	1.00	1.00	0.38	0.38	0.38	1.00	0.00	0.00	1.00	0.00	0.00	
Jniform Delay (d), s/vel	3.4	11.8	11.8	5.2	3.8	3.8	55.0	0.0	0.0	53.0	0.0	0.0	
ncr Delay (d2), s/veh	0.0	1.0	0.9	0.0	0.5	0.5	1.2	0.0	0.0	0.2	0.0	0.0	
nitial Q Delay(d3),s/veh		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
%ile BackOfQ(50%),veh		10.9	11.3	0.2	4.0	4.2	2.5	0.0	0.0	0.8	0.0	0.0	
Jnsig. Movement Delay													
_nGrp Delay(d),s/veh	3.4	12.8	12.7	5.2	4.3	4.3	56.2	0.0	0.0	53.2	0.0	0.0	
_nGrp LOS	A	В	В	A	A	A	E	A	A	D	A	A	
Approach Vol, veh/h		1234			1541			83			27		
Approach Delay, s/veh		12.7			4.3			56.2			53.2		
Approach LOS		В			Α			Е			D		
Timer - Assigned Phs	- 1	2		4	5	6		8					
Phs Duration (G+Y+Rc)	. s4.9	101.9		13.3	6.9	99.8		13.3					
Change Period (Y+Rc),		* 5.1		* 5.2	* 4.4	* 5.1		* 5.2					
Max Green Setting (Gm		* 76		* 19	* 11	* 76		* 19					
Max Q Clear Time (g_c-		18.0		8.2	2.7	26.8		3.8					
Green Ext Time (p_c), s	0.0	3.4		0.1	0.0	2.6		0.0					
ntersection Summary													
HCM 6th Ctrl Delay			9.9										
HCM 6th LOS			Α										
Notes													
Jser approved pedestri HCM 6th computation								ases cro	ossing t	he barr	ier.		

ne Configurations		→	*	•	—	4	1
Lane Configurations ↑↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑	Movement	EBT	EBR	WBL	WBT	NBL	NBR
Traffic Volume (vehh) 815 37 24 1064 42 25 initial of (20b), veh Per-Elike Alfi (AphT) 10 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0							
nitial Q (Db), veh D 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Traffic Volume (veh/h)		37	24			25
Peed-Bike AdjiA, pbT)	Future Volume (veh/h)	815	37	24	1064	42	25
Parking Bus, Agj 100 100 100 100 100 100 No Novic Zone On Approach No No No No No No No No No No No No No		0			0		
Mork Zone On Approach No	Ped-Bike Adj(A_pbT)						
Adj Sal Flow, wehrhin 1885 1885 1885 1885 1885 1885 1885 188			1.00	1.00			1.00
Name							
Peak Hour Factor							
Percent Heavy Veh., % 1 1 1 1 1 1 1 1 2 2 2 2 2 2 2 2 2 2 2							
Cap, wehln 1283 37 143 1549 514 305 Arrive On Green 0.36 0.36 0.07 0.87 0.48 0.48 Salt Flow, wehln 3385 157 1795 3676 1067 633 33 357 377 717 179 1791 1718 0 Sapp Salt Flow(s), wehln 1791 1857 1795 1791 1718 0 0 Sapp Salt Flow(s), wehln 1791 1857 1795 1791 1718 0 0 Sapp Salt Flow(s), wehln 1791 1857 1795 1791 1718 0 0 Sycle Q Clearing Co, s. 377 37.7 1.3 42.3 3.6 0.0 0 <t< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td></t<>							
Arrive On Green 0.36 0.36 0.07 0.87 0.48 0.48 5 Sat Flow, yesh 3.85 157 1795 3676 1067 633 Sat Flow, yesh 3.85 157 1795 3676 1067 633 Sat Flow, yesh 3.85 157 1795 3676 1067 633 Sat Flow, yesh 3.85 157 1795 3676 1067 633 Sat Flow, yesh 3.85 157 1795 3791 1718 0 Sarevig, s), s 377 377 1.3 42.3 3.6 0.0 Tyopic O Clearing, s), s 377 377 1.3 42.3 3.6 0.0 Tyopic O Clearing, s), s 377 377 1.3 42.3 3.6 0.0 Tyopic O Clearing, s), s 377 377 1.3 42.3 3.6 0.0 Tyopic O Clearing, s), s 377 377 1.3 42.3 3.6 0.0 Tyopic O Clearing, s 377 377 1.3 42.3 3.6 0.0 Tyopic O Clearing, s 377 377 1.3 42.3 3.6 0.0 Tyopic O Clearing, s 377 377 1.3 42.3 3.6 0.0 Tyopic O Clearing, s 370 377 377 1.3 42.3 3.6 0.0 Tyopic O Clearing, s 370 377 377 1.3 42.3 3.6 0.0 Tyopic O Clearing, s 377 377 1.3 42.3 3.6 0.0 Tyopic O Clearing, s 370 377 377 1.3 42.3 3.6 0.0 Tyopic O Clearing, s 370 377 377 1.3 42.3 3.6 0.0 Tyopic O Clearing, s 370 377 377 1.3 42.3 3.6 0.0 Tyopic O Clearing, s 370 377 377 1.3 42.3 3.6 0.0 Tyopic O Clearing, s 370 377 377 1.3 42.3 3.6 0.0 Tyopic O Clearing, s 370 377 377 1.3 42.3 3.6 0.0 Tyopic O Clearing, s 42 4 2.5 2.7 47 4 17 0.0 Tyopic O Clearing, s 42 4 2.3 27.3 11.3 17.3 0.0 Tyopic O Clearing, s 42 4 2.3 27.3 11.3 17.3 0.0 Tyopic O Clearing, s 42 4 2.3 27.3 11.3 17.3 0.0 Tyopic O Clearing, s 42 4 11.5 17.3 Tyopic O Clearing, s 42 4 5 6 Tyopic O Clearing, s 42 5 6 Tyopic O Clearing, s 42 5 6 Tyopic O Clearing, s 43 5 6 Tyopic O Clearing, s 43 5 6 Tyopic O Clearing, s 44 5 6 Tyopic O Clearing, s 44 5 6 Tyopic O Clearing, s 44 5 6 Tyopic O Clearing, s 44 5 6 Tyopic O Clearing, s 44 5 6 Tyopic O Clearing, s 44 5 6 Tyopic O Clearing, s 44 5 6 Tyopic O Clearing, s 44 5 6 Tyopic O Clearing, s 44 5 6 Tyopic O Clearing, s 44 5 6 Tyopic O Clearing, s 44 5 6 Tyopic O Clearing, s 44 5 6 Tyopic O Clearing, s 44 5 6 Tyopic O Clearing, s							
Salt Flow, vehih 3885 157 1795 3676 1067 633 Grip Volume(v), vehih 931 613 34 1503 95 0 Grip Salt Flow(s), vehih 931 613 34 1503 95 0 Grip Salt Flow(s), vehih 1671 91 1887 1795 1791 1718 0 3 Serve(s), s 37.7 37.7 1.3 42.3 3.6 0.0 Voyele O'Geierg (c), s 37.7 37.7 1.3 42.3 3.6 0.0 Voyele O'Geierg (c), s 37.7 37.7 1.3 42.3 3.6 0.0 VIC Ratio(X) 0.91 1.3 12.3 36 0.0 VIC Ratio(X) 0.91 0.91 0.91 0.92 0.02 0.02 0.02 VIC Ratio(X) 0.91 0.91 0.91 0.24 0.97 0.11 0.00 Vic Ratio(X) 0.91 0.91 0.91 0.24 0.97 0.11 0.00 Vic Ratio(X) 0.91 0.91 0.91 0.26 0.97 0.11 0.00 Vic Ratio(X) 0.91 0.91 0.91 0.02 0.00 1.00 1.00 Undrom Deley (d), siveh 365 36.5 27.2 7.4 17.1 0.0 Incifor Deley (d), siveh 365 36.5 27.2 7.4 17.1 0.0 Incifor Deley (d), siveh 365 36.5 0.1 3.8 0.3 0.0 Initial O Delay(d3), siveh 0.0 0.0 0.0 0.0 0.0 0.0 Insigh Movement Delay, siveh 1.0 5.8 0.8 5.0 1 3.8 0.3 0.0 Incifor Delay (d2), siveh 36.5 1.5 0.0 1.3 0.0 Incifor Delay (d3), siveh 0.0 5.8 0.8 5.0 1 3.8 0.3 0.0 Incifor Delay (d3), siveh 0.0 0.8 0.8 5.0 1 3.8 0.3 0.0 Incifor Delay (d3), siveh 0.0 0.0 0.0 0.0 0.0 0.0 Approach Delay, siveh 42.4 42.3 27.3 11.3 17.3 0.0 Incifor Delay (d3), siveh 10.9 1.8 B Intimer - Assigned Phs 2 4 5 6 Phis Durston (G+Y-Rc), s 57.0 63.0 8.5 48.5 Change Period (Y-Rc), s 57.0 63.0							
Carp Volume(v) vehit 691 613 34 1503 95 0							
Sgr. Sat Flow(s) web/hint / 191 1857 1795 1718 0 0 0 0 0 0 0 0 0							
2 Serve(g. s), s							
Cycle C (Delarig c.), s. 37.7 37.7 1.3 42.3 3.6 0.0 Prop In Lane 0.8 1.00 1.02 0.2 0.37 Lane Grp Cap(c), vehh 64.8 672 143 1549 827 0 V/C Ratio(y), vehh 699 1044 286 247.4 827 0 HOM Patron Ratio 10 10 20 20 1.00 1.00 Justream Filter II 0.85 0.85 0.47 0.47 1.00 0.00 Juniform Delay (d.), sveh 0.5 3.6 0.1 3.3 3.0 0.0 Infor Delay (d.), sveh 0.0 5.0 2.0 0.0 0.0 0.0 Junig, Movement Delay, sveh 0.0 5.0 0.0 <t< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td></t<>							
Prop In Lane							
Lame Grg Cap(c), wehh 648 672 143 1549 827 0 WCR Ratic(X) 0.91 0.91 0.24 0.97 0.11 0.00 WArall Cap(c), a), wehh 689 1004 286 2474 827 0 HCM Platon Ratio 1.00 1.00 2.00 2.00 1.00 1.00 1.00 Lipstream Flater(1) 0.85 0.85 0.47 0.47 1.00 0.00 Linforn Delay (d), sveh 36.5 \$8.5 0.47 0.47 1.00 0.00 Linforn Delay (d), sveh 36.5 \$8.5 0.1 3.8 0.3 0.0 Initial O Delayd3), sveh 0.0 0.0 0.0 0.0 0.0 0.0 Lings, Movement Delay, sveh 1.00 0.0 0.0 0.0 0.0 0.0 Lings, Movement Delay, sveh 1.00 0.0 0.0 0.0 0.0 0.0 Lings, Movement Delay, sveh 1.00 0.0 0.0 0.0 0.0 0.0 Lings, Movement Delay, sveh 1.00 0.0 0.0 0.0 0.0 0.0 0.0 Lings, Movement Delay, sveh 1.00 0.0 0.0 0.0 0.0 0.0 0.0 Lings, Movement Delay, sveh 1.00 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.							
\(\text{VIC Ratio(X)} \) 0.91 0.91 0.94 0.97 0.11 0.00 \\ \text{VAWAIC Capic a.} \) veh b 69 1004 286 2474 827 0 \\ \text{LCM Platon Ratio} \) 1.00 1.00 2.00 2.00 1.00 1.00 \\ \text{LOM Platon Ratio} \) 1.00 1.00 2.00 2.00 1.00 1.00 \\ \text{Lom Data Platon Ratio} \) 1.00 1.00 2.00 2.00 1.00 1.00 \\ \text{Lom Data Platon Ratio} \) 1.00 1.00 2.00 2.00 1.00 0.00 \\ \text{Lom Data Platon Ratio} \) 1.00 1.00 0.00 \\ \text{Lom Data Platon Ratio} \) 1.00 0.00 0.00 \\ \text{Lom Data Platon Ratio} \) 1.00 0.00 0.00 \\ \text{Lom Data Platon Ratio} \) 1.00 0.00 0.00 \\ \text{Lom Data Platon Ratio} \) 1.00 0.00 0.00 0.00 \\ \text{Lom Data Platon Ratio} \) 1.00 0.00 0.00 0.00 \\ \text{Long Data Platon Ratio} \) 1.00 0.00 0.00 0.00 \\ \text{Long Data Platon Ratio} \) 1.00 0.00 0.00 \\ \text{Long Data Platon Ratio} \) 1.00 0.00 0.00 \\ Long Data Platon Ratio		648	672		1549	827	0
HCM Pelson Ratio			0.91	0.24	0.97	0.11	0.00
Ujstream, Filer(I)	Avail Cap(c_a), veh/h	969	1004	286	2474	827	0
Uniform Delay (d), s/weh 36, 5 36, 5 27, 2 7, 4 17, 1 0, 0	HCM Platoon Ratio	1.00	1.00	2.00	2.00	1.00	1.00
Incr Delay (2D, s/weh 6.0 5.8 0.1 3.8 0.3 0.0			0.85	0.47	0.47	1.00	0.00
Initial O Delayrid3) siveh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.							
Name Name	Incr Delay (d2), s/veh						
Unsig, Movement Delay, siveh LinGrip Delay(s) sheh 42.4 42.3 27.3 11.3 17.3 0.0 LinGrip Delay(s) sheh 42.4 42.3 27.3 11.3 17.3 0.0 LinGrip Delay Sheh 42.4 42.3 27.3 11.5 15.7 9 Approach Delay, siveh 42.4 11.6 17.3 Approach Delay, siveh 42.4 11.6 17.3 Approach Delay, siveh 42.4 11.6 17.3 Approach Delay, siveh 42.4 11.6 17.3 B B B IIIImer - Assigned Phs 2 4 5 6 Approach Delay, siveh 42.5 63.0 8.5 48.5 Approach Delay, siveh 42.5 11.6 17.3 Approach Delay, siveh 42.5 63.0 8.5 48.5 Approach Delay, siveh 43.5 63.0 8.5 48.5 Approach Delay 43.5 63.3 8.7 44 5.1 Ama Crear Fire (g. c), siveh 43.5 63.3 8.7 7 Approach Ama Crear Fire (g. c), siveh 43.5 63.3 8.7 7 Approach Approach Approach Approach Approach Approach Approach Approach Approach Approach Approach Approach Approach Approach Approach Approach Approach Approach B Approach Approach B B B B B B B B B B B B B B B B B B B							
LinGrip Delay(d)s/weh				0.5	3.9	1.5	0.0
LnCip LOS				27.2	44.0	47.0	0.0
Agrorach Vol. wehh 1204 1537 95							
Approach Delay, sveh. 42.4			U	Ü			A
Approach LOS							
Timer - Assigned Phs 2							
Phs Durston (G+Y-Rc), s 57.0 63.0 85 48.5 Change Period (Y-Rc), s 75.1 *5.2 *4.4 *5.1 Max Green Setting (Gmax), s *83 *27 *14 *5.5 Max G Cleer Time (g c-H), s 44.3 5.5 3.3 93.7 Green Ext Time (g c-H), s 44.3 5.5 3.3 93.7 Green Ext Time (g c-H), s 44.3 5.5 3.3 93.7 Green Ext Time (g c-H), s 44.3 5.5 3.3 93.7 Green Ext Time (g c-H), s 44.3 5.7 0.1 0.0 3.7 Green Ext Time (g c-H), s 44.3 5.7 0.1 0.0 3.7 Green Ext Time (g c-H), s 44.3 5.7 0.1 0.0 3.7 Green Ext Time (g c-H), s 44.3 5.7 0.1 0.0 5.7 Green Ext Time (g c-H), s 44.3 5.7 Green Ext Time (g c-H), s 44.3 5.7 Green Ext Time (g c-H), s 44.3 5.7 Green Ext Time (g c-H), s 44.3 5.7 Green Ext Time (g c-H), s 44.3 5.7 Green Ext Time (g c-H), s 44.3 5.7 Green Ext Time (g c-H), s 44.3 5.7 Green Ext Time (g c-H), s 44.3 5.7 Green Ext Time (g c-H), s 44.3 5.7 Green Ext Time (g c-H), s 44.3 5.7 Green Ext Time (g c-H), s 44.3 5.7 Green Ext Time (g c-H), s 44.3 5.7 Green Ext Time (g c-H), s 44.3 5.7 Green Ext Time (g c-H), s 44.3 5.7 Green Ext Time (g c-H), s 44.3 5.7 Green Ext Time (g c-H), s 44.3 5.7 Green Ext Time (g c-H), s 44.3 5.5 Green Ext	••	U					
Change Period (Y+Rc), s	Timer - Assigned Phs						
Max Grean Setting (Gmax), s *83 *27 *14 *65 Max Q Clear Time (g_c+1), s 44, 3 5.6 a, 3 39.7 Green Ext Time (g_c), s 6.7 0.1 0.0 3.7 Intersection Summary HCM 6th Crt Delay 24.9 HCM 6th LOS C Notes HCM 6th Corporational engine requires equal clearance times for the phases crossing Culebra Road Corridor Study 5:00 pm 12/15/2020 2020 PM Existing Condition	Phs Duration (G+Y+Rc),	s					
Max Q Clear Time (g.c+l1), s							
Green Ext Time (p_c), s 6.7 0.1 0.0 3.7 Intersection Summary I-CM 6th Crt Delay 24.9 I-CM 6th LOS C Notes HCM 6th Corp and a computational engine requires equal clearance times for the phases crossing Lower Road Corridor Study 5:00 pm 12/15/2020 2020 PM Existing Condition							
Intersection Summary HCM 6th Chft Delay 24.9 HCM 6th LOS C Notes HCM 6th computational engine requires equal clearance times for the phases crossing		l1), s					
HCM 6th Chil Delay HCM 6th LOS C Notes HCM 6th computational engine requires equal clearance times for the phases crossing Culebra Road Corridor Study 5:00 pm 12/15/2020 2020 PM Existing Condition	Green Ext Time (p_c), s		6.7		0.1	0.0	3.7
HCM 6th Chil Delay HCM 6th LOS C Notes HCM 6th computational engine requires equal clearance times for the phases crossing Culebra Road Corridor Study 5:00 pm 12/15/2020 2020 PM Existing Condition	Intersection Summary						
HCM 6th LOS C Colors HCM 6th computational engine requires equal clearance times for the phases crossing Liberta Road Corridor Study 5:00 pm 12/15/2020 2020 PM Existing Condition	HCM 6th Ctrl Delay			24.9			
HCM 6th computational engine requires equal clearance times for the phases crossing Culebra Road Corridor Study 5:00 pm 12/15/2020 2020 PM Existing Condition	HCM 6th LOS						
HCM 6th computational engine requires equal clearance times for the phases crossing Culebra Road Corridor Study 5:00 pm 12/15/2020 2020 PM Existing Condition	Natas						
Culebra Road Corridor Study 5:00 pm 12/15/2020 2020 PM Existing Condition		l one:	00 100	iron o =	ial alca	ranca ti	mon fe
	now otn computational	engir	ne requ	ıres equ	ial ciea	ance t	mes to
WSP - San Antonio							

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR		
Lane Configurations	Ĭ	† ↑		Ĭ	† 1>		Ž	44	7	٦	† 1>			
Traffic Volume (veh/h)	74	600	172	207	779	54	240	452	165	34	494	60		
Future Volume (veh/h)	74	600	172	207	779	54	240	452	165	34	494	60		
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0		
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00		
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
Work Zone On Approach	ì	No			No			No			No			
	1885	1885	1885	1885	1885	1885	1885	1885	1885	1885	1885	1885		
Adj Flow Rate, veh/h	100	812	233	280	1055	73	325	612	223	46	669	81		
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96		
Percent Heavy Veh, %	1	1	1	1	1	1	1	1	1	1	1	1		
Cap, veh/h	237	1018	292	315	1467	101	350	1164	519	236	609	74		
Arrive On Green	0.05	0.37	0.37	0.11	0.43	0.43	0.16	0.33	0.33	0.03	0.19	0.19		
Sat Flow, veh/h	1795	2746	788	1795	3399	235	1795	3582	1598	1795	3217	389		
Grp Volume(v), veh/h	100	529	516	280	556	572	325	612	223	46	372	378		
Grp Sat Flow(s),veh/h/ln	1795	1791	1743	1795	1791	1843	1795	1791	1598	1795	1791	1815		
Q Serve(g_s), s	4.1	31.7	31.7	11.1	30.7	30.7	17.3	16.7	13.1	2.5	22.7	22.7		
Cycle Q Clear(g_c), s	4.1	31.7	31.7	11.1	30.7	30.7	17.3	16.7	13.1	2.5	22.7	22.7		
Prop In Lane	1.00		0.45	1.00		0.13	1.00		1.00	1.00		0.21		
Lane Grp Cap(c), veh/h	237	664	646	315	773	795	350	1164	519	236	339	343		
V/C Ratio(X)	0.42	0.80	0.80	0.89	0.72	0.72	0.93	0.53	0.43	0.20	1.10	1.10		
Avail Cap(c_a), veh/h	322	664	646	381	773	795	407	1164	519	342	339	343		
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
Upstream Filter(I)	0.66	0.66	0.66	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
Uniform Delay (d), s/veh	24.2	33.7	33.7	25.6	28.1	28.1	33.8	33.0	31.8	37.6	48.7	48.7		
Incr Delay (d2), s/veh	0.3	6.6	6.7	17.2	5.7	5.6	23.9	0.2	0.2	0.1	78.0	78.4		
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		
%ile BackOfQ(50%),veh		14.5	14.1	5.9	13.8	14.2	9.5	7.3	5.0	1.1	17.4	17.7		
Unsig. Movement Delay														
LnGrp Delay(d),s/veh	24.5	40.3	40.5	42.8	33.8	33.7	57.7	33.2	32.0	37.8	126.6	127.0		
LnGrp LOS	С	D	D	D	С	С	E	С	С	D	F	F		
Approach Vol, veh/h		1145			1408			1160			796			
Approach Delay, s/veh		39.0			35.6			39.8			121.7			
Approach LOS		D			D			D			F			
Timer - Assigned Phs	1	2	3	4	5	6	7	8						
Phs Duration (G+Y+Rc).	s7.9	44.3	17.6	50.2	24.2	28.0	10.3	57.5						
Change Period (Y+Rc),	\$ 4.4	* 5.3	* 4.4	* 5.7	* 4.8	* 5.3	* 4.4	* 5.7						
Max Green Setting (Gma	až)1\$	* 36	* 18	* 36	* 23	* 23	* 12	* 43						
Max Q Clear Time (q. c+	114.5s	18.7	13.1	33.7	19.3	24.7	6.1	32.7						
Green Ext Time (p_c), s		1.6	0.1	1.0	0.1	0.0	0.0	1.9						
Intersection Summary														
HCM 6th Ctrl Delay			52.7											
HCM 6th LOS			D											
Notes														
User approved pedestria * HCM 6th computationa								ases cr	ossing t	he barr	ier.			
Culebra Road Corridor S	V d C	.00	10/15/	2020 20	200 DM	Cuintin	- C	alaa.					Synchro 1	0 Dan

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SE
Lane Configurations		↑ ₽			414			4			4	
Traffic Volume (vph)	93	800	5	5	1025	49	5	5	5	57	5	- 1
Future Volume (vph)	93	800	5	5	1025	49	5	5	5	57	5	1
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	19
Total Lost time (s)	4.4	5.2			5.2			5.2			5.2	
Lane Util. Factor	1.00	0.95			0.95			1.00			1.00	
Frt	1.00	1.00			0.99			0.95			0.91	
Flt Protected	0.95	1.00			1.00			0.98			0.99	
Satd. Flow (prot)	1787	3571			3549			1767			1684	
FIt Permitted	0.06	1.00			0.95			0.91			0.89	
Satd. Flow (perm)	113	3571			3369			1635			1526	
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.
Growth Factor (vph)	130%	130%	130%	130%	130%	130%	130%	130%	130%	130%	130%	130
Adj. Flow (vph)	130	1118	7	7	1433	68	7	7	7	80	7	1
RTOR Reduction (vph)	0	0	0	0	3	0	0	5	0	0	23	
Lane Group Flow (vph)	130	1125	0	0	1505	0	0	16	0	0	247	
Heavy Vehicles (%)	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	_
Turn Type	pm+pt	NA		pm+pt	NA		Perm	NA		Perm	NA	
Protected Phases	1	6 26		5	2 22			4 24			8 28	
Permitted Phases	6 26			2 22			4 24			8 28		
Actuated Green, G (s)	73.4	73.4			62.1			25.8			25.8	
Effective Green, g (s)	73.4	73.4			62.1			25.8			25.8	
Actuated g/C Ratio	0.61	0.61			0.52			0.22			0.22	
Clearance Time (s)	4.4											
Vehicle Extension (s)	1.0											
Lane Grp Cap (vph)	165	2184			1743			351			328	
v/s Ratio Prot	c0.05	0.31										
v/s Ratio Perm	0.44				c0.45			0.01			c0.16	
v/c Ratio	0.79	0.51			0.86			0.04			0.75	
Uniform Delay, d1	44.2	13.2			25.3			37.3			44.1	
Progression Factor	0.70	1.44			0.77			1.00			1.00	
Incremental Delay, d2	18.8	0.2			3.9			0.0			8.4	
Delay (s)	49.8	19.2			23.2			37.3			52.6	
Level of Service	D	В			С			D			D	
Approach Delay (s)		22.4			23.2			37.3			52.6	
Approach LOS		С			С			D			D	
Intersection Summary												
HCM 2000 Control Delay			25.6	- 11	CM 2000	I avail of	Camilan		С			
	it		0.83	н	OWI 2000	Level Of	OEI VICE		C			
HCM 2000 Volume to Capac	aty ratio		120.0	C		dina (a)			25.2			
Actuated Cycle Length (s) Intersection Capacity Utilizat	ion		99.8%		um of lost				25.2 F			
	IIOII		99.8%	IC	O LEVEL	JI SETVICE			۲			
Analysis Period (min)			15									

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Movement	EBL	EBT	WBT	WBR	SBL	SBR		
Lane Configurations		44	44	77	220			
Traffic Volume (vph)	0	810	998	694	584	25		
Future Volume (vph)	0	810	998	694	584	25		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Total Lost time (s)		13.8	4.4	4.0	5.6			
Lane Util. Factor		0.95	0.95	0.88	0.94			
Frt		1.00	1.00	0.85	0.99			
Flt Protected		1.00	1.00	1.00	0.95			
Satd. Flow (prot)		3574	3574	2814	5031			
Flt Permitted		1.00	1.00	1.00	0.95			
Satd. Flow (perm)		3574	3574	2814	5031			
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96		
Growth Factor (vph)	130%	130%	130%	130%	130%	130%		
Adj. Flow (vph)	0	1097	1351	940	791	34		
RTOR Reduction (vph)	0	0	0	0	4	0		
Lane Group Flow (vph)	0	1097	1351	940	821	0		
Heavy Vehicles (%)	1%	1%	1%	1%	1%	1%		
Turn Type		NA	NA	Free	Prot			
Protected Phases		11	7911		12 10			
Permitted Phases				Free				
Actuated Green, G (s)		27.2	63.7	120.0	16.1			
Effective Green, g (s)		27.2	49.9	120.0	16.1			
Actuated g/C Ratio		0.23	0.42	1.00	0.13			
Clearance Time (s)		13.8						
Vehicle Extension (s)		0.2						
Lane Grp Cap (vph)		810	1486	2814	674			
v/s Ratio Prot		c0.31	c0.38		c0.16			
v/s Ratio Perm				c0.33				
v/c Ratio		1.35	0.91	0.33	1.22			
Uniform Delay, d1		46.4	32.9 0.64	1.00	51.9 1.00			
Progression Factor		1.35	0.64	0.0	111.1			
Incremental Delay, d2 Delay (s)		227.7	22.1	0.0	163.0			
Level of Service		221.1 F	22.1 C	Ο.0	103.U			
Approach Delay (s)		227.7	13.0	А	163.0			
Approach LOS		221.1 F	13.0 B		103.0 F			
••		F	ь		F			
Intersection Summary								
HCM 2000 Control Delay			98.3	H	ICM 2000	Level of Service	F	
HCM 2000 Volume to Capac	city ratio		1.07					
Actuated Cycle Length (s)			120.0		um of lost		41.6	
Intersection Capacity Utilizat	tion		60.4%	I	CU Level	of Service	В	
Analysis Period (min)			15					
c Critical Lane Group								
Culebra Road Corridor Stud	5.00	40/45/00/	20.0000.0	M.F	0 171			Synchro 10 Rep

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None 99 9 2 35 Major 0	- 0 - 0 9 99 2 2 2 0 2081	None 99 2 13 N	99 2 0	0 0 99 2	None 0 - - 99 2	99	0 0 0 99 2	None 0 - - 99	
99 9 2 35 Major 0	- 0 - 0 9 99 2 2 0 2081	99 2 13	99 2 0	0 0 99 2	99	99	0 0 99 2	99	
99 9 2 35 Major 0	- 0 - 0 9 99 2 2 0 2081	99 2 13	99 2 0	0 0 99 2	99	99	0 0 99 2	99	
99 9 2 35 Major 0 -	- 0 9 99 2 2 0 2081 2	99 2 13 M	99 2 0	99 2	99 2	99	99 2	99	
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Internaction													
Intersection Int Delay, s/veh	17.6												
**		COT		MOI	WOT	WDD	NDI	NOT	NDD	ODI	ODT	000	
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations Traffic Vol. veh/h	9	1400	4	18	445	6	8	4	7	5	↔ 5	5	
Future Vol. veh/h	9	1400	4	18	1670 1670	6	8	1	7	5	5	5	
Conflicting Peds. #/hr	0	1400	0	18	16/0	0	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	
RT Channelized	1100	1100	None	1100	1100	None	Otop	Otop -	None	Otop	Olop -	None	
Storage Length		- 1	NUILE	- 1	- 1	IVUILE	- 1		IVUILE	- 1	- :	None	
Veh in Median Storage	# -	0			0			0			0		
Grade. %		0			0			0			0	-	
Peak Hour Factor	93	93	93	93	93	93	93	93	93	93	93	93	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	13	1957	6	25	2334	8	11	1	10	7	7	7	
Major/Minor 1	Najor1			Major2			Minor1		,	/linor2			
Conflicting Flow All	2342	0	0	1963	0	0	2973	4378	982	3197	4377	1171	
Stage 1	2072	-	-	1300	-	-	1986	1986	302	2388	2388	1171	
Stage 2		- 1	- 1				987	2392		809	1989		
Critical Hdwv	5.34			5.34			6.44	6.54	7.14	6.44	6.54	7.14	
Critical Hdwy Stg 1	0.01			0.01			7.34	5.54		7.34	5.54	-	
Critical Hdwy Stg 2		-					6.74	5.54		6.74	5.54	-	
Follow-up Hdwy	3.12	-		3.12			3.82	4.02	3.92	3.82	4.02	3.92	
Pot Cap-1 Maneuver	84	-		131			15	2	213	- 11	~ 2	159	
Stage 1		-	-		-		39	105	-	20	65	-	
Stage 2		-					240	65		309	105	-	
Platoon blocked, %		-	-			-							
Mov Cap-1 Maneuver	84	-	-	131				2	213	~ 5	~ 2	159	
Mov Cap-2 Maneuver		-	-		-		-	2	-	~ 5	~ 2	-	
Stage 1		-			-	-	39	105	-	20	65	-	
Stage 2	-				-		205	65		291	105	-	
Approach	EB			WB			NB			SB			
HCM Control Delay, s	0.4			0.4					\$ 3	601.3			
HCM LOS										F			
Minor Lane/Major Mvm	t	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR S					
Capacity (veh/h)			84		-	131	-	-	4				
HCM Castel Dalay (a)			0.15 55.3	-		0.192	-		5.242				
HCM Control Delay (s) HCM Lane LOS			55.3 F	0 A	- 1	38.9 F	0 A	\$3	3601.3 F				
HCM Lane LOS HCM 95th %tile Q(veh)		- 1	0.5	A	- 1	0.7	Α -	- 1	4.1				
, , , , , , , , , , , , , , , , , , , ,		_	0.0	_	_	0.1	_	_	7.1				
Notes													
~: Volume exceeds car	acity	\$: De	elay exc	eeds 3	UUS ·	+: Com	putation	Not De	enned	: All	major v	oiume ir	n platoon

Intersection Int Delay, s/veh	6.6												
**		COT	500	14.001		was	. In	HOT	LIDD	001	007	000	
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		1320	41		441	-	5	↔ 5	56	4	↔ 5	9	
Traffic Vol, veh/h Future Vol. veh/h	5	1320	41 41	5	1725 1725	7	5		56	4	5	9	
Conflicting Peds. #/hr	5	1320	41	0	1/25	0	0	5	00	0	0	0	
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	
RT Channelized	1166	1166	None	1166	1166	None	Olup	ошр	None	Olup	Olup	None	
Storage Length		_	140116			IVOIIG		-	140116			HONG	
Veh in Median Storage		0	- :	- 1	0	- 1	- :	0	- 1		0	- 1	
Grade. %	,# -	0	- 0		0			0			0		
Peak Hour Factor	99	99	99	99	99	99	99	99	99	99	99	99	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	
Mymt Flow	7	1733	54	7	2265	9	7	7	74	5	7	12	
												-	
Major/Minor N	Major1			//ajor2			Minor1			Minor2			
Conflicting Flow All	2274	0	0	1787	0	0	2698	4062	894	2995	4085	1137	
Stage 1	-						1774	1774		2284	2284		
Stage 2	-				-		924	2288		711	1801	-	
Critical Hdwy	5.34			5.34			6.44	6.54	7.14	6.44	6.54	7.14	
Critical Hdwy Stg 1		-	-			-	7.34	5.54	-	7.34	5.54	-	
Critical Hdwy Stg 2							6.74	5.54		6.74	5.54		
Follow-up Hdwy	3.12			3.12			3.82	4.02	3.92	3.82	4.02	3.92	
Pot Cap-1 Maneuver	91			160			23	~ 3	244	15	~ 2	168	
Stage 1							56	134		24	74		
Stage 2	-				-	-	262	73		355	130		
Platoon blocked, %		-	-		-	-							
Mov Cap-1 Maneuver	91	-	-	160		-	-	0	244	-	0	168	
Mov Cap-2 Maneuver	-		-	-	-	-	-	0	-	-	0	-	
Stage 1							56	0		24	74		
Stage 2							222	73			0		
Approach	EB			WB			NB			SB			
HCM Control Delay, s	15.3			0.1									
HCM LOS										-			
Minor Lane/Major Mvm	. ,	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR :	CDI n1				
Capacity (veh/h)		- CLIII	91	-	LDR.	160		TIDIC .	- LITE				
HCM Lane V/C Ratio			0.072	-		0.041		-					
			47.6	15.7	-	28.5	0	-					
HCM Control Delay (s)			Е	С	-	D	A	-					
		-	0.2	-	-	0.1	-						
HCM Lane LOS													
HCM Control Delay (s) HCM Lane LOS HCM 95th %tile Q(veh) Notes													

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	٦	^^		٦	** 1,		Ĭ	†	7	ľ	1		
Traffic Volume (veh/h)	87	1000	200	164	1390	46	192	270	175	54	233	75	
Future Volume (veh/h)	87	1000	200	164	1390	46	192	270	175	54	233	75	
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0	
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Work Zone On Approac		No			No			No			No		
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	
Adj Flow Rate, veh/h	119	1368	274	224	1902	63	263	369	239	74	319	103	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2	
Cap, veh/h	186	1832	367	303	2339	77	280	525	445	225	274	89	
Arrive On Green	0.11	0.86	0.86	0.08	0.46	0.46	0.12	0.28	0.28	0.05	0.20	0.20	
Sat Flow, veh/h	1781	4267	854	1781	5076	168	1781	1870	1585	1781	1354	437	
Grp Volume(v), veh/h	119	1091	551	224	1275	690	263	369	239	74	0	422	
Grp Sat Flow(s).veh/h/lr	1781	1702	1717	1781	1702	1840	1781	1870	1585	1781	0	1792	
Q Serve(g_s), s	4.5	15.1	15.2	8.2	38.7	38.9	13.7	21.2	15.3	3.9	0.0	24.3	
Cycle Q Clear(g, c), s	4.5	15.1	15.2	8.2	38.7	38.9	13.7	21.2	15.3	3.9	0.0	24.3	
Prop In Lane	1.00		0.50	1.00		0.09	1.00		1.00	1.00		0.24	
Lane Grp Cap(c), veh/h	186	1462	737	303	1568	848	280	525	445	225	0	363	
V/C Ratio(X)	0.64	0.75	0.75	0.74	0.81	0.81	0.94	0.70	0.54	0.33	0.00	1.16	
Avail Cap(c a), veh/h	251	1462	737	328	1568	848	280	525	445	305	0	363	
HCM Platoon Ratio	2.00	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Upstream Filter(I)	0.87	0.87	0.87	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	
Uniform Delay (d), s/veh		5.9	5.9	19.6	27.9	27.9	33.3	38.7	36.6	36.1	0.0	47.9	
Incr Delay (d2), s/veh	1.2	3.1	6.0	6.6	4.7	8.5	37.6	3.6	0.7	0.3	0.0	99.5	
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
%ile BackOfQ(50%),veh		2.9	3.5	3.7	16.0	18.2	8.8	10.2	6.0	1.7	0.0	20.7	
Unsig. Movement Delay													
	25.8	9.0	11.9	26.2	32.6	36.4	70.9	42.3	37.3	36.4	0.0	147.4	
LnGrp LOS	С	A	В	С	С	D	Е	D	D	D	Α	F	
Approach Vol, veh/h		1761			2189			871			496		
Approach Delay, s/veh		11.0			33.1			49.5			130.8		
Approach LOS		В			С			D			F		
Timer - Assigned Phs	- 1	2	3	4	5	6	7	8					
Phs Duration (G+Y+Rc)	. \$0.6	60.4	9.6	39.4	14.4	56.6	19.0	30.0					
Change Period (Y+Rc),		* 5.1	* 4.2	* 5.7	* 4.2	* 5.1	* 4.2	5.7					
Max Green Setting (Gm	ax)1\$	* 51	* 11	* 28	* 12	* 50	* 15	24.3					
Max Q Clear Time (g. c-		40.9	5.9	23.2	10.2	17.2	15.7	26.3					
Green Ext Time (p_c), s	0.0	4.1	0.0	0.6	0.0	4.5	0.0	0.0					
Intersection Summary													
HCM 6th Ctrl Delay			37.6										
HCM 6th LOS			D										
Notes													
User approved pedestria * HCM 6th computations								ases cr	ossing t	he barr	ier.		
Culebra Road Corridor S	Study 5	:-00 nm	12/15/	2020.2	120 PM	Fyistin	a Condi	ition					Synchro 10 Re

	•	335-11	,		+	4	_	†	<i>></i>	_	11	
		\rightarrow	*	•	- C	_	7			*	+	*
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SI
Lane Configurations		44	7		414					7	ન	
Traffic Volume (vph)	0	630	590	53	1080	0	0	0	0	51	14	5
Future Volume (vph)	0	630	590	53	1080	0	0	0	0	51	14	5
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	19
Total Lost time (s)		5.0	5.0		5.0					5.7	5.7	
ane Util. Factor		0.95	1.00		0.95					0.95	0.95	1
rt		1.00	0.85		1.00					1.00	1.00	0
It Protected		1.00	1.00		1.00					0.95	0.97	_ 1
Satd. Flow (prot)		3185	1425		3178					1513	1549	14
It Permitted		1.00	1.00		0.79					0.95	0.97	1
Satd. Flow (perm)		3185	1425	0.00	2526		0.00	0.00		1513	1549	14
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0
Growth Factor (vph)	130%	130%	130%	130%	130%	130%	130%	130%	130%	130%	130%	13
Adj. Flow (vph)	0	844	791	71	1447	0	0	0	0	68	19	- (
RTOR Reduction (vph)	0	0 844	537 254	0	0 1518	0	0	0	0	0 43	0 44	
ane Group Flow (vph)												
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	-
Turn Type		NA	Perm	Prot	NA					Split	NA	Pe
Protected Phases		6		5	5 6					8	8	
Permitted Phases		07.0	6		46.0					22.3	00.0	
Actuated Green, G (s)		27.0	27.0								22.3	2
Effective Green, g (s)		27.0	27.0		46.0					22.3	22.3	2
Actuated g/C Ratio Clearance Time (s)		0.32 5.0	0.32 5.0		0.55					0.27 5.7	0.27 5.7	0
/ehicle Extension (s)		1.7	1.7							1.0	1.0	
ane Grp Cap (vph)		1023	458		1530					401	411	- 3
ane Grp Cap (vpn)		0.26	458		c0.22					0.03	0.03	
//s Ratio Prot		0.20	0.18		c0.22					0.03	0.03	c0
//s Ratio Perm		0.83	0.16		0.99					0.11	0.11	1
Uniform Delay, d1		26.3	23.5		18.8					23.3	23.3	3
Progression Factor		1.00	1.00		1.36					1.00	1.00	1
ncremental Delay, d2		7.6	4.8		17.0					0.0	0.0	19
Delay (s)		33.9	28.3		42.7					23.4	23.4	22
evel of Service		C	20.5		72.7 D					20.4 C	20.4 C	- 22
Approach Delay (s)		31.2	U		42.7			0.0		U	203.6	
Approach LOS		C			72.7 D			Α.			200.0	
••								- "				
ntersection Summary												
ICM 2000 Control Delay			69.6	Н	CM 2000	Level of	Service		Е			
HCM 2000 Volume to Capac	ity ratio		1.12									
Actuated Cycle Length (s)			84.0		um of lost				15.7			
ntersection Capacity Utilizat Analysis Period (min)	ion		116.2%	IC	U Level o	of Service			Н			
			15									
c Critical Lane Group			15									
Culebra Road Corridor Study												

Lane Configurations Traffic Volume (vph) 4 Tuture Volume (vph) 4 Ideal Flow (vphp) 19 Total Lost time (s) Lane Ubil. Factor Fit Fit Prolected Satd. Flow (prot) Fit Permitted Satd. Flow (prot) Fit Permitted Satd. Flow (prot)	BL 150 150 100	230 230 1900 4.7 0.95	0 0 1900	0 0	WBT ↑↑ 351	WBR 88	NBL 781	NBT	NBR	SBL	SBT	SE
Traffic Volume (vph) 4 Italian Volume (vph) 4 Ideal Flow (vphpl) 9 Total Lost time (s) Lane Uill. Factor Fit Protected Stud. Flow (prot) Fit Permitted Stud. Flow (perm) Peak-hour factor, PHF 0.	50	230 230 1900 4.7	0		351	00						
Future Volume (vph) 4 Ideal Flow (vphpl) 19 Total Lost time (s) Lane URI. Factor Frt FR Protected Satd. Flow (prot) FR Permitted Satd. Flow (prot) Pask-hour factor, PHF 0.	50	230 1900 4.7	0			00	704					
Ideal Flow (yphpl) 19 Total Lost time (s) Lane Util. Factor Frt Fit Protected Satd. Flow (prot) Fit Permitted Satd. Flow (perm) Peak-hour factor, PHF 0.		1900 4.7		0			/81	16	35	0	0	
Total Lost time (s) Lane Util. Factor Frt Fit Protected Satd. Flow (prot) Fit Permitted Satd. Flow (perm) Peak-hour factor, PHF 0.	100	4.7	1900		351	88	781	16	35	0	0	
Lane Util. Factor Frt Fit Protected Satd. Flow (prot) Fit Permitted Satd. Flow (perm) Peak-hour factor, PHF 0.				1900	1900	1900	1900	1900	1900	1900	1900	19
Frt Fit Protected Satd. Flow (prot) Fit Permitted Satd. Flow (perm) Peak-hour factor, PHF 0.					4.7		5.5	5.5				
Fit Protected Satd. Flow (prot) Fit Permitted Satd. Flow (perm) Peak-hour factor, PHF 0.					0.95		0.95	0.95				
Satd. Flow (prot) Fit Permitted Satd. Flow (perm) Peak-hour factor, PHF 0.		1.00			0.97		1.00	0.99				
Flt Permitted Satd. Flow (perm) Peak-hour factor, PHF 0.		0.97			1.00		0.95	0.96				
Satd. Flow (perm) Peak-hour factor, PHF 0.		3426			3432		1681	1674				
Peak-hour factor, PHF 0.		0.57			1.00		0.95	0.96				
		2017			3432		1681	1674				
		0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.
	0%	130%	130%	130%	130%	130%	130%	130%	130%	130%	130%	13
	91	302	0	0	461	116	1026	21	46	0	0	
RTOR Reduction (vph)	0	0	0	0	26	0	0	4	0	0	0	
Lane Group Flow (vph)	0	893	0	0	551	0	544	545	0	0	0	
	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	:
	rot	NA			NA		Split	NA				
Protected Phases	1	2 1			2		4	4				
Permitted Phases												
Actuated Green, G (s)		36.4			27.3		32.5	32.5				
Effective Green, g (s)		36.4			27.3		32.5	32.5				
Actuated g/C Ratio		0.43			0.33		0.39	0.39				
Clearance Time (s)					4.7		5.5	5.5				
Vehicle Extension (s)					1.3		1.0	1.0				
Lane Grp Cap (vph)		1026			1115		650	647				
v/s Ratio Prot		c0.09			0.16		0.32	c0.33				
v/s Ratio Perm		c0.28										
v/c Ratio		3.03dl			0.49		0.84	0.84				
Uniform Delay, d1		21.7			22.8		23.3	23.4				
Progression Factor		1.60			1.00		1.00	1.00				
Incremental Delay, d2		5.1			1.6		8.8	9.4				
Delay (s)		39.7			24.4		32.2	32.8				
Level of Service		D			С		С	С				
Approach Delay (s)		39.7			24.4 C			32.5			0.0 A	
Approach LOS		D			C			C			А	
Intersection Summary												
HCM 2000 Control Delay			33.2	Н	CM 2000	Level of S	Service		С			
HCM 2000 Volume to Capacity rat	io		0.86									
Actuated Cycle Length (s)			84.0		um of lost				15.7			
Intersection Capacity Utilization			91.1%	IC	U Level o	of Service			F			
Analysis Period (min)			15									
dl Defacto Left Lane. Recode wi	ith 1	though la	ne as a le	eft lane.								
c Critical Lane Group												



NTRODUCTIO

STUDY GOAL!

CORRIDOR

APPENDIX D

Multimodal Corridor Transportation Study

Appendix D: Culebra Road – Traffic Volume Projecting Methodology Memo (February 2, 2021)

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APPENDIX

APPENDIX D

TO: Bianca Thorpe, P.E., Capital Programs Manager

FROM: Kerri Collins, PE, PTOE, LEED AP; Michael Trueblood, PE, PTOE; Jemal Ali, PMP

SUBJECT: Task 3: Culebra Road - Traffic Volume Projection Methodology Memo

DATE: February 2, 2021

This technical memorandum outlines our proposed methodology for the development of traffic projections as part of the Culebra Road Corridor Traffic Study located in San Antonio, Texas. Below is a map of the project showing the limits of the corridor at Loop 1604 and IH-10 (**Figure 1**).



Figure 1 – Study Area and Project Limits

The methodology was developed to forecast the existing traffic volumes to the future design year and to adjust 2020 traffic counts for pre-pandemic conditions. The projections will be used in the traffic analysis and evaluation of the proposed project options.

The traffic volumes will be developed to support analysis for the following years.

- Base Year 2020; and
- Future Analyses Year 2045

The following sources of data are used for the development of growth rates and adjustment factors:

- $\bullet \quad$ Traffic counts prior to 2020 provided from the City of San Antonio;
- Project collected traffic counts conducted in December 2020 during COVID-19 pandemic;

- Historical count data obtained from the TxDOT Statewide Traffic Analysis and Reporting System (STARS II) Database or the TxDOT Statewide Planning Map; and
- Alamo Area Metropolitan Planning Organization (AAMPO) travel demand model (TDM) year 2017, 2025, 2035 and 2045 forecast traffic volumes.

Note that the AAMPO travel demand model is used to develop traffic growth rates.

PROPOSED METHODOLOGY

The following steps were used for developing traffic projections. The first step of the methodology includes developing a COVID Factor (CF) to adjust the traffic volumes collected during the pandemic to better represent typical conditions by applying a growth rate to historical data to forecast normal 2020 traffic patterns. The second step is to apply TDM-derived growth factors to the adjusted 2020 counts. The steps to develop traffic volumes and forecasts are as follows:

NORMALIZE 2020 BASE YEAR VOLUMES

Base year traffic volumes for the corridor will be normalized to pre-COVID conditions using a combination of counts collected in December 2020 adjusted with a COVID Factor using counts from previous years adjusted based upon a calculated growth rate to year 2020.

- Pre-pandemic TMC Counts Apply the growth rate from the AAMPO model to traffic counts conducted prior to 2020 to develop 2020 traffic volumes. Perform a reasonableness check.
- Counts Collected During Pandemic the following process is used to develop a Covid Factor (CF)
 - Compare the adjusted 2020 pre-COVID TMC (developed in 1) with the similar count location of 2020 counts to develop a COVID Factor (four locations identified)
 - Compare 2020 projected STARS II count locations to 2020 ATR counts recorded at the same location to develop a COVID Factor (CF) and apply to the latest COVID 2020 counts
 - Compare and develop the most appropriate COVID Factor from the above two methods.
 - Conduct reasonableness check to make sure no turning movement volumes are increased where it is not reasonable, i.e., where development is not expected to increase, where roadway does not continue for a long distance, etc.

ANNUAL GROWTH FACTOR

The AAMPO TDM will be used to develop annual growth factors based on volume projections directly from the AAMPO travel demand forecasts for 2017, 2025, 2035 and 2045 models. The forecasts will be used to develop the annual growth factors for the corridor; a process that includes a review of the consistency and reasonableness of the growth trends from the model relative to historic trends.

2045 FUTURE VOLUMES

AAMPO TDM growth rates will be applied to the 2020 revised traffic volumes to develop the future 2045 traffic volumes

VOLUME PROJECTION CALCULATION

For the purpose of developing annual growth rates and projected traffic volumes, , the study corridor was divided into six segments shown in Figure 2 and listed below based on the corridor characteristics such as land use, density, roadway configuration and connections with intersecting roads.

- 1. Segment 1: Loop 1604 to Tezel/Grissom
- 2. Segment 2: Tezel/Grissom to Micron Dr/Pipers Ln;
- 3. Segment 3: Micron Dr/Pipers Ln to IH 410
- 4. Segment 4: IH 10 to Callaghan Road,
- 5. Segment 5: Callaghan Road to Wilson Boulevard; and
- 6. Segment 6: Wilson Boulevard to IH-10



Figure 2 - Corridor Segments

APPLICATION OF THE AAMPO GROWTH RATE

Traffic volume outputs for the 2017, 2025, 2035 and 2045 horizon years were extracted from the AAMPO TDM for the roadway links along the Culebra Road corridor study area. The growth rates were calculated for each specific link in the network and aggregated into six segments using a volume weighted average as shown in **Table 1**. **Figure 3** shows a graph of how growth rates vary from link to link for different future year volumes. As shown in the table, the growth rate for each segment varies, thus the weighted average in the segment is used for each segment rate calculation. For the short-term future (2017 – 2025), the TDM annual growth rate for segment 2 and Segment 3 are 0.76%, -0.65%, respectively, which is very low. The 2017 TDM volumes for Segment 3 are higher compared with 2025 which results a negative growth rate, between Potranoc Rd to IH 410 as an example. To be conservative, a 1.00% minimum growth rate is assumed for all corridor segments. **Table 2** summarizes the average growth rates by segment.

APPENDIX D

APPENDIX D

C	B1 -	Link from		Traffic Vo	olumes		Anr	ual Growth R	ates
Segment	No.	LINK Trom	2017	2025	2035	2045	2017-2025	2025-2035	2035-204
	1	LP1604 SBFR	49603	61345	75053	87226	2.69%	2.04%	1.51%
	2	LP1604 NBFR	36353	41435	48806	55812	1.65%	1.65%	1.35%
	3	Rogers Rd	36353	41435	48806	55812	1.65%	1.65%	1.35%
펀	4	Mountain View	36353	41435	48806	55812	1.65%	1.65%	1.35%
Segment 1	5	Les Harrison Dr	26012	29043	34301	40309	1.39%	1.68%	1.63%
E E	6	Cliffbrier Dr	25213	26978	31032	37009	0.85%	1.41%	1.78%
Se	7	Nueces Canyon/Village Park	25213	26978	31032	37009	0.85%	1.41%	1.78%
	8	Coppertree Blvd	25213	26978	31032	37009	0.85%	1.41%	1.78%
	9	Walmart/Commercial Properties	25213	26978	31032	37009	0.85%	1.41%	1.78%
	10	Camino Rosa	25213	26978	31032	37009	0.85%	1.41%	1.78%
	11	Grissom Rd/Tezel Rd	42485	45225	50121	56540	0.78%	1.03%	1.21%
	12	Arcadia Creek	42982	46184	53315	63389	0.90%	1.45%	1.75%
	13	Westover Hills Blvd/Ensenada	37236	38683	46517	52216	0.48%	1.86%	1.16%
7	14	Giffin Park Dr	37236	38683	46517	52216	0.48%	1.86%	1.16%
ent	15	Ansley Bend Dr/Timber Path	37236	38683	46517	52216	0.48%	1.86%	1.16%
Segment 2	16	Old Grissom Rd	46795	50162	61301	66050	0.87%	2.03%	0.75%
S	17	Village Pkwy/Grissom Pass	46795	50162	61301	66050	0.87%	2.03%	0.75%
	18	Grissom Gate	46795	50162	61301	66050	0.87%	2.03%	0.75%
	19	Rim Rock Trail/Commercial Property	44503	47543	57417	60393	0.83%	1.90%	0.51%
	20	Timber View Dr	44503	47543	57417	60393	0.83%	1.90%	0.51%
	21	Micron Dr/Pipers Ln	41756	43711	53489	55628	0.57%	2.04%	0.39%
	22	Reed Rd	48273	46511	55385	58881	-0.46%	1.76%	0.61%
	23	Pipers Creek St (Both T Intersections)	49694	47654	56259	60038	-0.52%	1.67%	0.65%
Segment 3	24	Joe Newton St	49694	47654	56259	60038	-0.52%	1.67%	0.65%
mer	25	Ingram Rd	45122	43616	51456	55620	-0.42%	1.67%	0.78%
egi	26	Van Ness	45122	43616	51456	55620	-0.42%	1.67%	0.78%
0,	27	Potranco Rd	58569	43878	51340	56858	-3.55%	1.58%	1.03%
	28	IH410 SBFR	49765	48963	55791	60508	-0.20%	1.31%	0.81%
	29	IH410 NBFR	27691	28960	32519	34984	0.56%	1.17%	0.73%
	30	Commercial Properties/Baseball Field	22758	26555	29624	31877	1.95%	1.10%	0.74%
Segment 4	31	Avenue G	22758	26555	29624	31877	1.95%	1.10%	0.74%
ner	32	Alamo Downs Pkwy	26338	30353	33962	36119	1.79%	1.13%	0.62%
egr	33	Fairgrounds Pkwy	26208	30952	34128	36276	2.10%	0.98%	0.61%
51	34	Oakhill-Tom Slick	25239	29954	32608	35144	2.16%	0.85%	0.75%

gment 5 gment 6		Callaghan to Wilson Wilson to IH 10	372520 327482	407818 370491	448538 407080	483835 466913	1.14% 1.55%	0.96% 0.95%	0.76% 1.38%
gment 4		IH 410 to Callaghan	123301	144369	159946	171293	1.99%	1.03%	0.69%
gment 3		Micron Dr/Pipers Ln to IH 410	415686	394563	463954	498175	-0.65%	1.63%	0.71%
gment 2		Grissom Rd/Tezel Rd to Micron Dr/Pipers Ln	426566	453030	541724	595513	0.76%	1.80%	0.95%
gment 1		LP 1604 to Grissom Rd/Tezel Rd	310739	349583	410932	480016	1.48%	1.63%	1.57%
	58	IH10 EBFR	30014	32863	36324	41784	1.14%	1.01%	1.41%
	57	N Navidad Street	50165	55325	61166	69739	1.23%	1.01%	1.32%
Š	56	Zarzamora	47929	52997	58475	66872	1.26%	0.99%	1.35%
Segment 6	55	Elmendorf	40736	46702	51583	59297	1.72%	1.00%	1.40%
ent	54	N Hamilton	40736	46702	51583	59297	1.72%	1.00%	1.40%
9	53	19th St	40587	46229	50543	57808	1.64%	0.90%	1.35%
	52	Neff Rd	40055	46350	50387	57821	1.84%	0.84%	1.39%
	51	Wilson	37260	43323	47019	54295	1.90%	0.82%	1.45%
	50	Bandera Rd	23102	24835	27791	31769	0.91%	1.13%	1.35%
	49	NW 28th St	23102	24835	27791	31769	0.91%	1.13%	1.35%
	48	General McMullen Dr	23513	25246	28188	32246	0.89%	1.11%	1.35%
	47	N San Felipe Ave	26405	29801	32011	34475	1.52%	0.72%	0.74%
	46	Camino Santa Maria	23135	25096	27185	29247	1.02%	0.80%	0.73%
	45	Memorial St/Rattler Dr	23603	26308	28661	30520	1.37%	0.86%	0.63%
Seg	44	NW 34th St	24635	26871	29655	31431	1.09%	0.99%	0.58%
Segment 5	43	Esmeralda Dr/NW 36th St	24635	26871	29655	31431	1.09%	0.99%	0.58%
ť	42	38th St	21904	22093	24153	27461	0.11%	0.90%	1.29%
	41	Alicia Ave	24027	25245	27456	29595	0.62%	0.84%	0.75%
	40	Hortencia Ave	20820	23472	26026	26943	1.51%	1.04%	0.45%
	38 39	Benrus	21876	24528	26696	28910	1.71%	0.94%	0.72%
	38	El Centro	24452	24528	26921	28910	1.44%	0.91%	0.45%
	36 37	Cantebury Mira Vista	23065 24452	25721 26914	28448 29453	29649 30819	1.37% 1.21%	1.01% 0.91%	0.41%
	35	Callaghan	23065	25721	28448	29649	1.37%	1.01%	0.41%

APPENDIX D

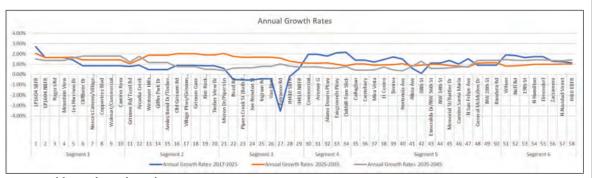


Figure 3 - Culebra Road Annual Growth Rates

Table 2: Annual Growth Rate Summary - from AAMPO TDM

Horizon Years	AA	MPO TDM Grow	th Rate		Adjusted Growth F	late*
norizon rears	2017-2025	2025-2035	2035-2045	2017-2025	2025-2035	2035-2045
Segment 1	1.48%	1.63%	1.57%	1.48%	1.63%	1.57%
Segment 2	0.76%	1.80%	0.95%	1.00%	1.80%	1.00%
Segment 3	-0.65%	1.63%	0.71%	1.00%	1.63%	1.00%
Segment 4	1.99%	1.03%	0.69%	1.99%	1.03%	1.00%
Segment 5	1.14%	0.96%	0.76%	1.14%	1.00%	1.00%
Segment 6	1.55%	0.95%	1.38%	1.55%	1.00%	1.38%
Overall Corridor	0.88%	1.38%	1.03%	1.00%	1.38%	1.03%

^{*} Assuming minimum 1% annual growth rate.

Rates are calculated from the 2017, 2025, 2035 and 2045 TDM annual traffic volumes.



RECOMMENDED GROWTH RATE

The growth rates that will be used to project pre-COVID counts to 2020 analyses year and 2020 adjusted traffic volumes to 2045 are summarized in **Table 3**. The future 2020 to 2045 annual growth rate is calculated from the AAMPO TDM annual rates assuming a minimum of 1.00% growth rate between 2017 to 2045 shown in Table 2.

Table 3: Recommended Annual Growth Rates by Forecast Year

Locations	Annual Gro	wth Rates
Locations	2015 - 2020	2020 - 2045
1 - Segment 1	1.50%	1.60%
2 - Segment 2	1.00%	1.30%
3 - Segment 3	1.00%	1.25%
4 – Segment 4	2.00%	1.20%
5 - Segment 5	1.15%	1.05%
6 - Segment 6	1.55%	1.25%

Corrections to traffic turn movement counts (TMCs) taken in 2020 during COVID 19 were developed using two steps as summarized below:

- a. Pre-COVID Counts Apply annual growth rate developed in Table 3 to traffic counts
- conducted prior to 2020 (Pre-COVID) to develop adjusted 2020 traffic volumes.

 b. COVID Factor via TMCs Compare adjusted 2020 (Pre-COVID) volumes with the actual 2020 counts taken during COVID-19 to develop COVID Factor. The results of this evaluation at four locations are shown in Table 4. As shown, the TMC based COVID Factor varies between 1.23 and 1.39 along the corridor with an average value of 1.31.

Table 4: COVID Factor (CF) from Pre-COVID TMC Projections vs Actual Counts During COVID-19

	Pre-		Intersection	ı Volume	
TMC Counts Intersection Locations	COVID Count Year	Pre-COVID Count	2020 Adjusted Count	2020 Actual Count	COVID Factor (CF)
Culebra @ Grissom Rd/Tezel Rd	2015	19,100	20,600	14,800	1.39
Culebra @ Ingram Rd	2015	16,200	17,000	13,400	1.27
Culebra @ Callaghan	2017	16,100	16,500	12,300	1.34
Culebra @ Zarzamora	2017	14,300	15,000	12,200	1.23
Overall Ave	rage	•	69,100	52,700	1.31

Note: Volumes are sum of Intersection TMC from 7AM to 9AM and 4PM to 6PM.

APPENDIX D





c. COVID Factor via ATRs - Compare the adjusted 2020 Pre-COVID TXDOT STARS II counts with a similar count location as actual 2020 COVID counts to develop COVID Factor. The results of this evaluation at three locations are shown in Table 5. As shown in Table 5, the ATR-based COVID Factor varies between 1.02 to 1.19 along the corridor with an average value of 1.10.

Table 5: COVID Factor (CF) from Pre-COVID ATR Projections vs Actual Counts During COVID-19

			ATR 24 HI	R Volumes	
ATR Counts Locations	STAR II Count Year	STARR II Total Volume	2020 Projected Volume	2020 Existing Volume	COVID Factor (CF)
ATR 1 - Between LP 1604 and Rogers Rd	2019	41,600	42,300	35,500	1.19
ATR 2 - Between Ingram Rd and Potranco Rd	2019	42,500	43,000	39,200	1.10
ATR 3 - Between N Hamilton Ave and Elmendorf Street	2019	35,400	35,900	35,100	1.02
Overall			121,200	109,800	1.10

There are significant differences between COVID Factor estimates derived by the above two approaches. It is our recommendation to use the COVID factor estimate from Pre COVID counts shown in Table 4, where the peak hour volumes are considered. This correction recognizes that the previous level of travel activity may never return due to the drawnts; negativity in the use of commercial office and retuil report in the account.

- correction recognizes that the previous level of travel activity may never return due to the dramatic reduction in the use of commercial office and retail space in the economy.

 d. Develop Adjusted 2020 Traffic Volumes The 2020 base year traffic volumes for the study were calculated by applying the annual growth rates to Pre-COVID AM and PM peak hour traffic counts, or by applying the COVID Factor (CF) to the 2020 COVID, AM and PM peak hour traffic counts.
- 2. Step 4: Develop Future Traffic Volumes The future projected traffic volumes are calculated by applying the annual growth rates to adjusted 2020 daily, AM and PM peak hour volumes.

CONCLUSIONS

Based on available data and analyses results pertaining to corrections to counts for the COVID-19 pandemic and future growth rates, it is recommended that:

- $1. \ \ \, traffic counts taken in 2020 should be factored by applying a COVID Factor (CF) of 1.30 to account the impact of COVID pandemic on those counts$
- 2. the growth rates shown in Table 3 be applied to the 2020 base year traffic volumes to generate the proposed future traffic volumes for the 2045 future analysis year.

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