

RECEIVED  
NRC

### Washington Public Power Supply System

P.O. Box 968 3000 George Washington Way Richland, Washington 99352 (509) 372-5000

1983 OCT 31 PM 11 24

Docket No. 50-397

REGION V

October 26, 1983  
G02-83-973

Mr. J. B. Martin  
Regional Administrator  
U.S. Nuclear Regulatory Commission  
Region V  
1450 Maria Lane, Suite 210  
Walnut Creek, CA 94596

Subject : NUCLEAR PROJECT 2  
NRC INSPECTION REPORT 50-397/83-38 NOTICE OF VIOLATION

Reference : (a) Meeting with NRC on October 14, 1983, in  
Bethesda, Maryland

Attachments 1 and 2 are transmitted herewith in response to a request made by Mr. R. T. Dodds at reference (a).

Attachment 1 is a response to NRC Inspection Item 83-38/02, Quality Class I - As-Built Program, provided to the Resident Inspector at WNP-2.

Attachment 2 contains representative calculations, including some worst case conditions, performed by Stone and Webster during their performance of a third-party assessment of QCI and QCII/Seismic I as-builts for WNP-2.

If you have any questions or desire further information, please contact Hugh Crisp at (509) 377-2522, extension 4661.

  
C. S. Carlisle - 982A  
Program Director, WNP-2

HAC/f1

Attachments: 1 and 2

cc: Mr. R. Auluck, NRC, Bethesda  
Mr. R. T. Dodds, NRC RV  
Mr. R. F. Heishman, NRC I&E  
Mr. A. D. Toth, NRC Resident, WNP-2

8311080336 831026  
PDR ADOCK 05000397  
G PDR

11 IE 01

NRC REPORT NO. B3-38/02 SUBJECT CATEGORY Quality Class I - As-Built ProgramDATE 9/21/83TYPE UnresolvedGeneral Problem

The Quality Class I As-Built Program does not successfully provide identification of all deficiencies.

Statement of Root Cause

The as-built program deficiencies identified by the NRC CAT Inspection cannot be attributed to one specific root cause. Evaluation of the as-built program has resulted in the identification of five separate areas (causes) which collectively contributed to the as-built program anomalies. The subject areas are discussed below.

Specific Problem(s)

- 1) Measurement and recording errors were made in identifying potential hardware deficiencies during the constructors walk down inspection and preparation of as-built drawings.
- 2) Interpretation errors were made in identifying potential hardware deficiencies during the constructors comparison of the QCI hanger as-built deviations from the design using the allowable construction tolerances in BRI Drawing H-501.

Corrective Action to Prevent Recurrence

The contract specifications was revised to clarify as-built program requirements and provide more concise as-built measurement tolerances. Two PED's were issued to revise the H-501 Drawing in order to clarify measurement and tolerance.

Corrective Action for Specific Problem(s)

- 1) A synopsis of corrective action taken is documented in the Supply System "As-Building - Quality Class I Pipe Support" Summary.
- 2) Resolution of this concern is summarized in the Supply System "As-Building - Quality Class I Pipe Support" Summary.

Supporting Documents

PED 215-H-W851  
215-H-W941  
215-H-W949

As-Building - QCI  
Pipe Support Summary  
Part A

As-Building - QCI  
Pipe Support Summary  
Part B

NRC REPORT NO. 83-38/02

3) Pipe supports categorized as Quality Class II, Seismic Category I were excluded from the project as-built program.

4) Clarification of ASME-NR/AISC Code Jurisdictional Boundaries.

5) Pipe whip restraints were excluded from the as-built program.

6) This inspection item and the NRC CAT Report 83-29 identified specific deficiencies associated with a sample of thirty Bechtel large bore and Gilbert Commonwealth small bore pipe supports.

3) This concern was essentially resolved by NRC Inspection Item 83-05/05.

4) This concern is covered by NRC Inspection Item 82-18/02.

5) BRTI will perform this function as part of their final pipe break and missile hazards insitu walkdowns.

6) Each of the NRC CAT identified supports were evaluated on a case by case basis. In addition the supply system performed a sample reverification of an additional 72 hangers. A summary of this program is attached.

NRC Inspection Item  
83-05/05  
As-Built Ins - QCI  
Pipe Support Summary  
Part C.

NRC Inspection Item  
82-18/02  
As-Built Ins - QCI  
Pipe Support Summary  
Part D.

As-Built Ins - QCI  
Pipe Support Summary  
Part E.

As-Built Ins - QCI  
Large bore As-Built  
Program Evaluations  
(with attachments)

NRC REPORT NO. 83-38/02

7) As noted in this inspection item the Supply System assigned an independent third party to assess the project as-built program.

7) The third party review determined that the project QCI as-built program is acceptable and meets the needs of the project. A copy of the Stone & Webster Evaluation Report is attached.

Stone & Webster -  
Engineering Inspections  
& Evaluation of QCI  
Pipe Supports & Small  
Bore Piping.

Report No.	83-38/02
Paragraph	5.b
Report Date	8-30-83

Type	Unresolved
------	------------

For Further Information:	
H. Boarder	x 6840
D. Cosgrove	x 6826

Responsibility	Bechtel Engineering/ BRI Engineering
----------------	---

## ITEM DESCRIPTION

5.b Mechanical Construction - (2.) Quality Class I - As-Built Program Does Not Successfully Provide Identification of Discrepancies

The adequacy and accuracy of the piping/support as-built program had been challenged by NRC regional inspectors, and licensee corrective actions taken as discussed in NRC inspection report number 50-397/83-05 paragraphs 5, 6.a, 6.d and 6.e. The licensee's actions had included identification, training, testing, and certification of personnel authorized to perform field as-built drawing updating; checking of results of individual as-built engineers; audits of program performance; and feedback to management and individuals. Work done prior to the updated program was sampled for each individual who did such work, and re-performance of the as-builts was scheduled for cases where the sample showed unacceptable discrepancies. Reasonable management attention appeared to have been applied to assure adequate performance accuracy by the as-built engineers. Audit results for program implementation did not reveal significant errors.

In Table III-2 and III-5 of inspection report 50-397/83-29 the NRC CAT inspectors identified several discrepancies between the as-installed piping supports versus the latest as-built drawings. For thirty Bechtel large bore and Gilbert-Commonwealth small bore pipe supports checked: five incorrect dimensions (large bore as-builts RHR-415, RHR-563, LPCS-28; and small bore as-builts SLC-4474-11 and DO-2533-2); two cases of undersize large bore support material (RCIC-21 and RCIC-952N); four undersize large bore support welds (RHR-465 and LPCS-28); two large bore support concrete anchor problems (HPCS-16 and RHR-563); and an incorrect material piece heat number on a large bore support (RCIC-952N). For piping isometrics, five of 89 dimensions appeared to have been incorrectly shown on the as-built drawings.

## ITEM STATUS

As of 9-21-83:

Corrective action for this inspection item encompasses three programs.

- o Evaluation of the as-built program to determine the causes of the deficiencies and provide a generic fix to prevent recurrence.
  - o Implementation of an as-built reverification program to evaluate both the NRC CAT inspection sample and an extended sample reinspection performed by the Supply System.
  - o Assignment of a third party group of engineers to perform an independent assessment of the as-built program.
1. The evaluation of the as-built program identified five general causes (areas) for the deficiencies identified by the CAT Inspection. A synopsis of this evaluation and corrective action taken is documented in the Supply System "As-Building - Quality Class I Pipe Support" Summary.
  2. The as-built reverification program has been completed. A summary of this program and the corrective actions taken is documented in the Supply System "As-Building - Quality Class I Large Bore As-Built Program Evaluation" Report.

Report No.	83-38/02
Paragraph	5.b
Report Date	8-30-83

## ITEM DESCRIPTION

The CAT findings show some weakness in performance by Bechtel and Gilbert-Commonwealth field engineering personnel, in spite of Bechtel management efforts to obtain accurate work. Additional management action appears necessary to ascertain sufficient accuracy of prior and future as-built drawings. Such action appears to have been promptly initiated by the Supply System to resolve this matter, including assignment of a third party group of engineers to assess the as-built situation independently of Bechtel. (Unresolved items 397/83-38-02)

See Attachment 1 "Tables III-2 and III-5 of NRC Report 83-29.

## ITEM STATUS

3. The third-party review determined that the project QCI as-built program is acceptable and meets the needs of the Project. This review is documented in the Stone & Webster "Engineering Inspection and Evaluation of QC-I Pipe Supports and Small Bore Piping" Report.

TABLE III-2

ISOMETRIC DISCREPANCIES

<u>Isometric Dwg. No.</u>	<u>Discrepancy</u>
1. 80/G 213, Rev. 7	<p>Distance between support MS-40-2 and connection for the safety-relief valve -</p> <p>Actual: 7" Design Dwg.: 2' - 8 3/16" As-Built Inspection: Did not check this dimension.</p> <p>[Item 1001 was initiated on this issue]</p>
2. 4PCS-633-112, Rev. 9	<p>Distance between supports 4PCS-18 and 4PCS-19 -</p> <p>Actual: 30" Design Dwg.: 15 3/8" As-Built Inspection: 21 3/4"</p>
3. SLC-046-16.25, Rev. 7	<p>Valve SLC-V-12 (Item No. 12) has no valve operator orientation marked on the isometric drawing. The operator is installed in the vertical direction. This discrepancy was not identified in the "as-built" program. [Item 6070 was initiated on this issue]</p>
4. 80/G-218, Rev. 7	<p>Distance between support 80-9 and attachment 803-9 -</p> <p>Actual: 5' - 0" Design Dwg.: 5' - 0" As-Built Inspection: 7' - 3"</p>
5. MS-4448-4, Rev. 8	<p>Distance between pipe elbow and support MS-4448-42 (u-bolt) -</p> <p>Actual: 34" (exceeds allowable tolerances) Design Dwg.: 6" As-Built Inspection: 4"</p>

TABLE III-5

QUALITY CLASS I SUPPORT/RESTRAINT AS-BUILT DEVIATIONS

<u>SUPPORT-RESTRAINT NO.</u>	<u>NRC CAT INSPECTOR OBSERVATION</u>
NPCS-16	Richmond insert stud threads not staked
RCIC-21	Clip angle 4x4x3/8", drawing specifies 4x4x1/2"
RCIC-952N	<ol style="list-style-type: none"> <li>1. Tube steel 4x4x1/4", drawing specifies 4x4x.375"</li> <li>2. Wrong heat number etched on tube steel.</li> </ol>
RHR-415	<ol style="list-style-type: none"> <li>1. Vendor welds ground undersize during attachment weld grinding.</li> <li>2. Critical dimension shown on red line as 5" ± .1", actual and original design is 5" ± 0.4".</li> </ol>
RHR-563 ✓	<ol style="list-style-type: none"> <li>1. Cold set dimensions on snubbers were 1" ± 5/8" and 1" ± 3/4". Drawing specifies 1" ± 1/8".</li> <li>2. Two of twelve expansion anchor mounting studs had less than required thread engagement into anchor shell.</li> </ol>
SLC-4475-11	Clearance from pipe to support are 1/8" and 1/32", red line indicates 1/16" and 0".
RHR-465	Lug to pipe weld 3/16", drawing specifies 1/4".
LPCS-903N	Weld details were not specified on the drawing.
LPCS-28	<ol style="list-style-type: none"> <li>1. Undersize welds on washer plates.</li> <li>2. Cold set dimension on snubber as 2' 7 7/8" per design drawing, shown on red line as 2' 10 1/2".</li> </ol>
RXCU-162	Weld details were not specified on the drawing.
DC-2533-2	Hanger location with respect to the pipe riser varied from design location by 2 7/8". The tolerance allowed by drawing H501 for this critical dimension is ±2".
MS-998N	Lug to pipe welds 1/4", drawing specifies 5/16".



AS-BUILDING - QUALITY CLASS I PIPE SUPPORTSDESCRIPTION OF CONCERN

The NRC CAT inspected 30 pipe supports at WNP-2. This inspection identified errors or deficiencies in the previously compiled as-built information made by the constructor in 12 of the 30 pipe supports. Also, an additional or extended sample of 72 pipe supports was inspected by the Project Quality Assurance organization. This review also identified errors or deficiencies in the construction as-building process. The NRC CAT concluded that the constructors "as-built" program, while identifying a number of hardware deficiencies, does not appear to be completely effective in that the NRC CAT findings and site sample findings indicate that additional deficiencies exist, some of which are considered significant to the NRC CAT.

The NRC CAT concerns with the project as-built program covers five general areas:

- A. Measurement and recording errors made in identifying potential hardware deficiencies during the constructors walkdown inspection and preparation of as-built drawings.
- B. Interpretation errors made in identifying potential hardware deficiencies during the constructors comparison of the QC I hanger

as-built deviations from the design using the allowable construction tolerances in Burns and Roe (BRI) drawing H-501.

- C. Exclusion of pipe supports categorized as Supply System Quality Class II, Seismic Category I from the project as-built program.
- D. The location of ASME-NF/AISC code jurisdictional boundaries.
- E. Exclusion of pipe whip restraints from the as-built program.

PROGRAM TO RESOLVE CONCERNS

A. Measurement and Recording Errors

The purpose of as-building pipe supports and restraints is to assure that the "as-built" installations are within the design envelope as required by NRC I & E Bulletin 79-14. This is accomplished by the constructor performing actual in-situ measurements of specific attributes of the installation and reporting the results to the A/E when the as-installed attribute exceeds a specific value (or tolerance). Specific attributes that require inspection and measurement to assure that the installations are within the design envelope include:

- o Pipe run geometry
- o Valve type, size and location
- o Valve operator type and orientation
- o Size, type and location of other concentrated weights

- o Pipe support (hanger) type, location, orientation, size, and general assembly details.

To meet these requirements, it is not required that measurements be taken and/or recorded for all attributes associated with a piping system and its supports, nor is it necessary that all attributes need be measured to the same degree of accuracy. Accordingly, the A/E must specify:

- 1) Those attributes that require in-situ measurement
- 2) Those measurements that require recording
- 3) The measurement accuracy required for each type of attribute
- 4) Deviations from design that are required to be identified to the A/E for reconciliation with the design.

At the time of the NRC CAT inspection, the A/E had specified measurement and submittal requirements. Upon post NRC CAT inspection review of these specification requirements, it was determined that these requirements were causing implementation anomalies. As a result, the specification requirements in these areas were changed to:

- 1) Consolidate all as-built program requirements into one section in the specification (previously the as-built

requirements were scattered throughout many sections of the specification)

- 2) Delineate the deficiency submittal to the A/E requirements in more detail
  
- 3) Better specify measurement tolerances that are consistent with the ability of the constructor to measure the attribute with a high degree of repeatability while assuring the precision required by the A/E for various categories of attributes.

In addition, a detailed engineering evaluation was made of QC I hanger measurement, recording and interpretation errors (See Section B, Interpretation Errors). This evaluation was made by Bechtel Engineering personnel from San Francisco who were independent from the project and experienced in evaluation of as-built deviations. This evaluation consisted of developing criteria consistent with code, specification, and I & E Bulletin 79-14 requirements, determination of the effect of each deviation identified in the NRC CAT and site QA samples on the design on a case-by-case basis, and consideration of the generic impact of the deviations.

This evaluation is attached. The evaluation was reviewed and compiled by Bechtel Engineering at the WNP-2 site and was reviewed and approved by Burns and Roe. The evaluation results can be briefly summarized as follows:

- o None of the deviations (measurement, recording and interpretation) impact the design, function, or operability of the specific hanger
  
- o Some of the items noted in the evaluation are of a concern generically because of their potential impact on the function of the hanger. These items are:
  - Coldset of snubbers - snubber/strut pin-to-pin dimensions
  - Offsets in rods, struts and snubbers
  - Clearances/gaps in box-type hangers.

All of these items of concerns are hanger attributes that are subject to change over time due to hanger set and balancing, ambient temperature changes, thermal shakedown, system operation, etc. Because of this, these items are rechecked by the startup organization, after the constructors as-built program is complete, as part of the WNP-2 Section XI preservice inspection program. The evaluation also identified cases of loose nuts, missing bolting locking devices, and missing washers. These items are not items that need be as-built, but their installation may be critical. This general item is being addressed as part of a project corrective action program dealing with bolting and fasteners.

B. Interpretation Errors

The NRC CAT identified that errors were being made in not identifying hardware discrepancies when the constructor evaluated as-built deficiencies against the allowable construction tolerances in Burns and Roe drawing H-501. To resolve this concern and to minimize the interpretation errors:

- 1) The A/E defined in greater detail the requirements as to which as-built measurements need to be reviewed by constructor against drawing H-501 prior to submittal to the A/E.
- 2) Drawing H-501 was revised by the A/E to clarify specific details that were being misinterpreted.
- 3) The constructor re-reviewed all hanger as-built measurements required by the A/E to be reviewed against H-501 prior to submittal and corrected the errors made during previous reviews. The constructor's field engineers chosen for this task were experienced at making these evaluations and were specifically retrained for this task.
- 4) The process was independently audited by a Bechtel San Francisco Engineering Staff senior engineer experienced in design requirements.

5) An engineering evaluation was made of all interpretation errors identified in the NRC CAT sample. None of the errors were found to impact the design, function or operability of the specific hanger.

C. Exclusion of Pipe Supports Categorized as Supply System Quality Class II, Seismic Category I From the Project As-Built Program

The NRC CAT concern is essentially the same as NRC resolved item 83-05/05. The root issue was whether or not pipe supports on non-safety related piping which would otherwise be classified Non-Seismic Category I, but whose failure could reduce the functioning of items important to safety (Seismic Category I) are to be as-built under the requirements of USNRC I & E Bulletin 79-14. Letter G02-83-622 to NRC Region VI, dated July 15, 1983, transmitted the Supply Systems' position with respect to this issue. The issue was subsequently closed and the position that I & E Bulletin 79-14 does not apply to this category of piping and supports therefore accepted.

D. The Location of ASME-NF/AISC Code Jurisdictional Boundaries

The projects response to this issue is being addressed in responses to NRC Notice of Deviation 83-22 (Item 82-18/02) as contained in the Supply System letter G02-83-701 to the NRC Region V, dated August 5, 1983, and in response to NRC NRR question 110.44.

E. Exclusion of Pipe Whip Restraints From the As-Built Program

Burns and Roe Engineering will (as part of their final pipe break and missile hazards in-situ walkdown) measure, record, and reconcile with the design all as-built locations of pipe whip restraints with respect to the piping. This is being done even though a sampling demonstrated that the locations of pipe whip restraints are located within a reasonable tolerance of their design location, that the restraints are demonstrated to be conservatively designed, and that no code, specification or regulatory requirement exists to require that these items be as-built reconciled with the design.



BURNS AND ROE, INC.  
WPPSS  
NUCLEAR PROJECT  
NO. 2

PROJECT  
ENGINEERING  
DIRECTIVE

PA'D  
NOW-  
PA'D  
TO'D  
NOW-TOD

CODE:	PROJECT ENGINEERING DIRECTIVE														
2   1	2	15	-	H	-	W	18	5	1						
1   2	3	4	5	6	7	8	9	10	11	12	13	14	15		
DATE	8	7	2	2	8	3									
	18	17	18	19	20	21									

REASON FOR P. E. D.:

- To clarify Specification 2808-215 As-Built requirements.
- To provide 215 contractor with concise As-Built Measurement Tolerances.

INFORMATION

SHEET 1 OF 22

COPIES 1 Mahoney

R. M. Boyum, G. Engler, J. Hopkins, J. Telford  
M. Ramchandani, C. Foley (WPPSS), J. KAYAC

REFERENCES

SUBJECT As-Built Spec Revision

LOCATION Various

ENG. SYSTEM 811 47-1-P3/80-P14

SU SYSTEM 970.0-P00/9.0-P45.4

QUALITY CLASS I & II 240-P14

ORIGINATING DOCUMENTS Request from D. Johnson (BPC) to resolve NRC CAT Concerns

TRANSMITTED FOR DOC CLOSED  
AND DRAWING UPDATE ONLY

DESCRIPTION OF WORK:

- Void the following PEDS: 215-H-G633 215-H-M757  
215-H-N959 215-H-M466
- Revise Specification 2808-215, Section 15A as indicated on sheet 4 through sheet 13 of this PED.
- Revise Specification 2808-215, Section 15B as indicated on sheet 14 through sheet 15 of this PED.
- Revise Specification 2808-215, Section 15R as indicated on sheet 16 through sheet 21 of this PED.
- Revise Specification 2808-215, Section 1C as indicated on sheet 22 of this PED.

RECEIVED  
Bechtel Power Corporation

AUG 30 1983

REVIEWED BY S/U

JOB NO. 1463

NOTES

- THIS PED REVISES DIRECTION PREVIOUSLY PROVIDED BY N/A THE FOLLOWING PED(S): \_\_\_\_\_
- THIS PED VOIDS DIRECTION PREVIOUSLY PROVIDED BY (see item 1 of Desc. of Work) THE FOLLOWING PED(S): \_\_\_\_\_
- THIS PED WORK SHOULD BE COORDINATED WITH KNOWN N/A OTHER WORK UNDER THE FOLLOWING PED'S: \_\_\_\_\_
- THIS PED DEPENDS ON THE PRIOR INSTALLATION OF N/A THE FOLLOWING PED'S: \_\_\_\_\_

REVISE:

NONE \_\_\_\_\_  
DRAWINGS \_\_\_\_\_  
SPECIFICATION  \_\_\_\_\_

APPROVALS:

[Signature] 7/26/83  
DISCIPLINE ENGINEER / DATE  
[Signature] 8/2/83  
GROUP SUPERVISOR / DATE  
[Signature] 8/5/83  
PROJECT ENGINEER / DATE





TABLE OF CONTENTS (Continued)

	<u>Page</u>
FIGURES 1-4 SACRIFICIAL SHIELD WALL	15A-136 thru 15A-139
Attachment 3A FIELD ASSEMBLY & TESTING OF CONDENSATE PHASE SEPARATORS	15a-139a
FIGURE 5 CONSTRUCTION TEST PROGRAM	15A-140
ATTACHMENT 4 WELDING PROCEDURE SPECIFICATION No. 82148 UL PT. I REV. No. 3 WESTINGHOUSE	15A-142
ATTACHMENT 5 WELDING PROCEDURE SPECIFICATION No. 82148 UL PT No. 2 REV. II WESTINGHOUSE	15A-157
ATTACHMENT 6 RECOMMENDED ERECTION PROCEDURE FOR WPPSS CONDENSER CONTRACT NO. 8	15A-179
ATTACHMENT 7 VENDOR SPECIAL STORAGE PROCEDURES AND REQUIREMENTS	15A-183
ATTACHMENT 8 INSTALLATION SPECIFICATIONS FOR BWR HIGH-DENSITY SPENT FUEL STORAGE RACKS	15A-190

ATTACHMENT 9 AS-BUILT MEASUREMENT TOLERANCES 15A-219

ATTACHMENT 10 APPROVED METHOD FOR THE AS-BUILT REPRESENTATION OF COMPLEX WELDS 15A-221

ADD →

215

Rev. 3 11-19-79

iii

REF DOC PCN	N/A	REF	N/A	WPPSS NUCLEAR PROJECT NO. 2	
REF SPEC SECTION	15A	PAGE	15A-111	PARA	N/A
REF DWG	N/A	DWG ZONE	BURNS AND ROE, INC.		
SCALE	N/A	DRAWN BY	GEE	DATE	7/21/83
		CHKD BY	WTR	DATE	7/29/83
		APPROV	GEE	DATE	7/21/83
				PED	215-H-WBS1
				SH	4 OF 22
				TITLE	AS-BUILT SPECIFICATION REVISION

3.11 As-Built Drawings Confirmation

3.11.1 General Requirements

Contractor shall provide confirmation to Owner that the component/system installed configuration is in compliance with the Issued for Construction drawings.

Confirmation scope, for purposes of as-building, shall include the following drawings as provided by the Owner:

1. Piping Erection Diagrams (Piping Isometric Drawings).
2. Support Detail Sheets (Hanger Detail Drawings).

Contractor shall confirm the installed configuration for the aforementioned drawings for pipe and pipe supports that fall under the following categories:

1. Quality Class I.
2. Quality Class II within Quality Class I/II Anchor Groups.
3. Quality Class II interfacing with Quality Class I/II Anchor Groups.
4. "Inaccessible" (Piping only).

"Inaccessible" is defined as:

- a) Buried, Embedded or Submerged.
- b) Located in any of the Inaccessible Areas defined under Article 2.0 of WNP-2 Project Instruction PMI 6-6 entitled, "As-Building Program Requirements".

3.11.2.1 Quality Class I Items

Contractor shall provide to the Owner redline mark-ups of the Piping Erection Diagrams listed on the BRI As-Built Scoping List indicating the installed configuration of all items on the drawing regardless of whether or not the installed configuration is within the construction tolerances provided by this Specification.

Contractor shall provide to the Owner confirmation of the installed configuration for the Support Detail Sheets listed on the BRI Hanger and Isometric Final As-Built Tracking List. Confirmation shall indicate that the installed configuration is within the construction tolerances provided by this Specification.

15A-15a

REF DOC PCN	N/A	RFI	N/A	WPPSS NUCLEAR PROJECT NO. 2	
REF SPEC SECTION	15A	PAGE	15A-15a	PARA	3.11
REF DWG	N/A	DWG ZONE		PED 215-H-WB51	SH 5 OF 22
SCALE	N/A	DRAWN BY	GEE	DATE	7/26/83
		CHKD		DATE	7/29/83
				TITLE	AS-BUILT SPECIFICATION REVISION

Contractor shall provide to the Owner confirmation of the installed configuration for the Small Diameter Piping and Pipe Supports listed on the As-Built Isometric Status Tracking List (ISTL). Confirmation shall indicate that the installed configuration is within the construction tolerances provided by this Specification.

The measurement tolerances to be applied to the Contractor's as-built confirmation of the above items is provided in ATTACHMENT 9 entitled, "AS-BUILT MEASUREMENT TOLERANCES" of this Specification Section.

The as-built representation of welds on the Support Detail Sheets shall meet the requirements of ATTACHMENT 10 entitled, "APPROVED METHOD FOR THE AS-BUILT REPRESENTATION OF COMPLEX WELDS" of this Specification Section.

3.11.2.2 Quality Class II Items Within Quality Class I/II Anchor Groups

Contractor shall provide to the Owner redline mark-ups of the Piping Erection Diagrams listed on the BRI Hanger and Isometric Final As-Built Tracking List indicating the installed configuration of all items on the drawings regardless of whether or not the installed configuration is within the construction tolerances provided by this Specification.

Contractor shall provide to the Owner confirmation of the installed configuration for the Support Detail Sheets listed on the BRI Hanger and Isometric Final As-Built Tracking List. Confirmation shall indicate that the installed configuration is within the construction tolerances provided by this Specification.

Contractor shall provide to the Owner confirmation of the installed configuration for the Small Diameter Piping and Pipe Supports listed on the As-Built Isometric Status Tracking List (ISTL). Confirmation shall indicate that the installed configuration is within the construction tolerances provided by this Specification.

The measurement tolerances to be applied to the Contractor's as-built confirmation of the above items is provided in ATTACHMENT 9 entitled, "AS-BUILT MEASUREMENT TOLERANCES" of this Specification Section.

The as-built representation of welds on the Support Detail Sheets shall meet the requirements of ATTACHMENT 10 entitled, "APPROVED METHOD FOR THE AS-BUILT REPRESENTATION OF COMPLEX WELDS" of this Specification Section.

15A-15b

REF DOC PCN	N/A	RFI	N/A	WPPSS NUCLEAR PROJECT NO. 2	
REF SPEC SECTION	15A	PAGE	15A-15b	PARA	3.11
REF DWG	N/A	DWG ZONE		PED 215-H-W851	SHT 6 OF 22
SCALE	N/A	DRAWN BY	EE	DATE	7/26/83
		CHKD BY	WTR	DATE	7/29/83
		APPROV	EE	DATE	7/29/83
				TITLE	AS-BUILT SPECIFICATION REVISION

3.11.2.3 Quality Class II Items Interfacing With Quality Class I/II Anchor Groups

Contractor shall provide to the Owner redline mark-ups of the Piping Erection Diagrams listed on the BRI Hanger and Isometric Final As-Built Tracking List indicating the installed configuration of all items on the drawing regardless of whether or not the installed configuration is within the construction tolerances provided by this Specification.

Contractor shall provide to the Owner confirmation of the pipe support orientation with regard to the piping (i.e.: angle with regard to North-South, East-West or vertical directions) and the pipe support function (i.e.: Spring, Sunbber, X-Y Restraint) for those supports listed on the BRI As-Built Scoping List.

Contractor shall provide to the Owner confirmation of the installed configuration for the Small Diameter Piping and Pipe Supports listed on the As-Built Isometric Status Tracking List (ISTL). Confirmation shall indicate that the installed configuration is within the construction tolerances provided by this Specification.

The measurement tolerances to be applied to the Contractor's as-built confirmation of the above items is provided in ATTACHMENT 9 entitled, "AS-BUILT MEASUREMENT TOLERANCES" of this Specification Section.

3.11.2.4 "Inaccessible" Items

Contractor shall provide to the Owner redline mark-ups of the Large Diameter ( 2½") Piping Erection Diagrams only. This information will not be included in the Owner's As-Built Verification Program unless otherwise required by sub-paragraph 3.11.2.1 and 3.11.2.2 above.

The measurement tolerances to be applied to the Contractor's as-built confirmation of the above items is provided in ATTACHMENT 9 entitled, "AS-BUILT MEASUREMENT TOLERANCES" of this Specification Section.

15A-15c

REF DOC PCN	N/A	RFI	N/A	WPPSS NUCLEAR PROJECT NO. 2	
REF SPEC SECTION	15A	PAGE	15A-15c	PARA	3.11
REF DWG	N/A	DWG ZONE	N/A	PED	215-H-W851
SCALE	N/A	DATE	7/24/83	SHT	7 OF 22
DRAWN BY	EE	TITLE	AS-BUILT		
CHECKED					

ATTACHMENT 9

AS-BUILT MEASUREMENT TOLERANCES

THE FOLLOWING MEASUREMENT TOLERANCES ARE TO BE APPLIED DURING THE CONTRACTOR'S AS-BUILT CONFIRMATION OF THE ITEMS LISTED WHEN REQUIRED BY ARTICLE 3.11 OF THIS SPECIFICATION SECTION.

THE BELOW STATED MEASUREMENT TOLERANCES ARE THOSE REQUIRED BY THE OWNER AND ARE TO BE USED BY THE CONTRACTOR IN DETERMINING ACCEPTABLE MEASUREMENT TECHNIQUES AND THE ACCEPTABILITY OF ANY REMEASUREMENTS. THE TOLERANCES ARE GIVEN IN RECOGNITION THAT AS-BUILDING IS ACCOMPLISHED WITH STANDARD MANUAL MEASUREMENT TOOLS SUCH AS RULES, TAPES, PLUMBBOBS AND THAT MEASUREMENT ACCURACY IS FURTHER LIMITED BY LACK OF ACCESSIBILITY.

1. PIPING ERECTION DIAGRAMS

<u>Characteristic Measured</u>	<u>Tolerance</u>
a) Axial Piping Dimensions	$\pm 2$ inches or $\pm \frac{1}{2}$ the nominal pipe diameter, whichever is less restrictive
b) Pipe Support Locations	Same as above
c) Penetration Clearances	
1. Dimensions less than or equal to 2 inches	$\pm 1/8$ inch
2. Dimensions greater than 2 inches	$\pm 1/4$ inch
d) Valve Stem Orientation	
1. Power operated valves in other than vertical pipe runs	$\pm 6$ degrees
2. All other valves	$\pm 15$ degrees
e) Elevations and dimensions from pipe to building column lines	May be estimated

15A-219

REF DOC PCN	N/A	RFI	N/A	WPPSS NUCLEAR PROJECT NO. 2	
REF SPEC SECTION	15A	PAGE	15A-219	PARA	ATTACH 9
REF DWG	N/A	DWG ZONE	N/A	PED 215-H-WBS1	SHT 8 OF 22
SCALE	N/A	DRAWN BY	GEE	DATE	7/29/83
		CHECK BY	LITEL	DATE	7/29/83
		APPROV	GEE	DATE	7/29/83
				TITLE	AS-BUILT SPECIFICATION REVISION



<u>Characteristic Measured</u>	<u>Tolerance</u>
2. SUPPORT DETAIL SHEETS	
a) Structural Member Dimensions	
1. Dimensions less than or equal to 5 inches	+ 10% of dimension but not Less than $\pm 1/32$ inch
2. Dimensions greater than 5 inches	$\pm 1/2$ inch or $\pm 4\%$ of dimension, whichever is less restrictive
b) Component Hardware Dimensions	
1. Pin-to-pin dimensions for Rod and Spring Supports	+ 12 inches - 3 inches
2. Pin-to-pin dimensions for Snubber cold setting	$\pm 1/8$ inch
3. Pin-to-pin dimensions for Rigid Sway Strut Assemblies	$\pm 1/2$ inch or $\pm 4\%$ of dimension, whichever is less restrictive
c) Clearances/Gaps	
1. Dimensions less than or equal to 5 inches	+ 10% of dimensions but not Less than $\pm 1/32$ inch
2. Dimensions greater than 5 inches	$\pm 1/2$ inch or $\pm 4\%$ of dimension, whichever is less restrictive
d) Structural/Base Plates	
1. Length and Width	$\pm 1/4$ inch
2. Thickness	+ 1/4 inch - 0 inch
3. Eccentricity of attaching member(s)	$\pm 1/4$ inch
e) Base Plate Anchor Bolts	
1. Bolt hole center-to-center dimensions	$\pm 1/4$ inch
2. Bolt hole edge-to-edge of plate	$\pm 1/8$ inch

15A-220

REF DOC PCN	N/A	RFI	N/A	WPPSS NUCLEAR PROJECT NO. 2	
REF SPEC SECTION	ISA	PAGE	ISA-220 PARA	ATTACH 9	
REF DWG	N/A	DWG ZONE	PED 215-H-WBS1		
SCALE	N/A	DRAWN BY	GEE	DATE	7/29/83
		CHKD BY	WTR	DATE	7/24/83
		APPRD	GEE	DATE	7/29/83
TITLE				AS-BUILT	
				SPECIFICATION REVISION	

<u>Characteristic Measured</u>	<u>Tolerance</u>
3. Bolt hole diameter	+ 1/8 inch - 0 inch
4. Bolt washer thickness	+ 1/8 inch - 0 inch
f) Angular Dimensions	± 3 degrees
g) Weld Sizes	+ infinity - 1/32 inch

15A-221

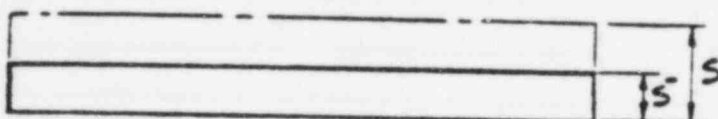
REF DOC PCN	N/A	RFI	N/A	WPPSS NUCLEAR PROJECT NO. 2	
REF SPEC SECTION	N//	PAGE	15A-221	PARA	ATTACH 9
REF DWG	N/A	DWG ZONE		PED 215-H-WBS1	SH-10 OF 22
SCALE		DRAWN BY	GEE	DATE	7/29/83
N/A		CHKD	LITR	DATE	7/29/83
		APPRD	GEE	DATE	7/29/83
				TITLE	AS-BUILT
				SPECIFICATION REVISION	

ATTACHMENT 10  
APPROVED METHOD FOR THE AS-BUILT  
REPRESENTATION OF COMPLEX WELDS

CONSTRUCTED COMPLEX WELDS FOR PIPE SUPPORTS ARE TO BE REPRESENTED ON THE AS-BUILT DRAWINGS IN ACCORDANCE WITH THE FOLLOWING CASES, EXAMPLES, AND NOTES:

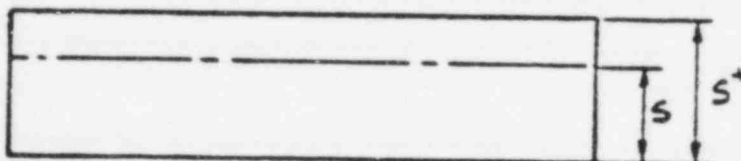
CASE 1 - REGULAR UNIFORM WELD PROFILE

A. WELD LEG LESS THAN DESIGN



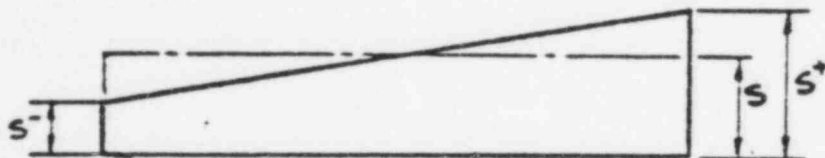
AS-BUILT TO INDICATE  
WELD LEG AS  $S^-$

B. WELD LEG GREATER THAN DESIGN



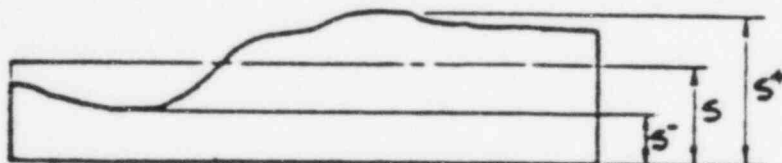
AS-BUILT TO INDICATE  
WELD LEG AS  $S$

CASE 2 - REGULAR TAPERED WELD PROFILE



AS BUILT TO INDICATE  
WELD LEG AS  $\frac{(S^- + S^+)}{2}$   
(SEE NOTE 4)

CASE 3 - IRREGULAR WELD PROFILE

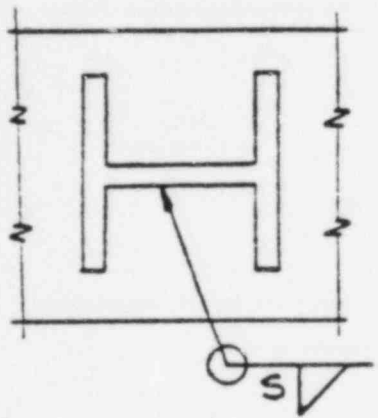


AS-BUILT TO INDICATE  
WELD LEG AS  $S$

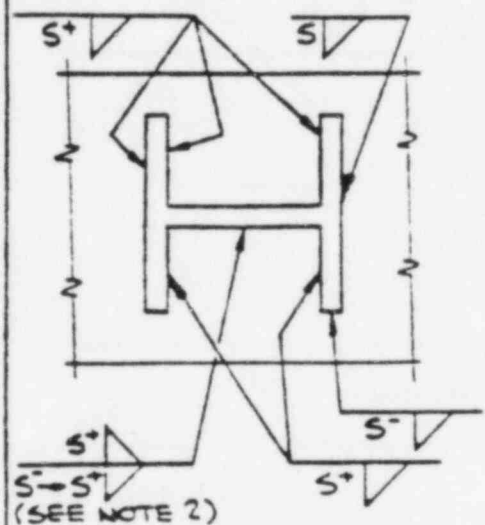
15A-222

REF DOC PCN	N/A	RFI	N/A	WPPSS NUCLEAR PROJECT NO. 2	
REF SPEC SECTION	15A	PAGE	15A-222	PARA	ATTACH. 10
REF DWG	N/A	DWG ZONE		PED 215-H-WBS1	SHT 11 OF 22
SCALE	N/A	DRAWN BY	EE	DATE	7/2/83
		CHKD	1/TB	DATE	7/1/83
		APPROV	EE	DATE	7/1/83
				TITLE	AS-BUILT
				SPECIFICATION	REVISION

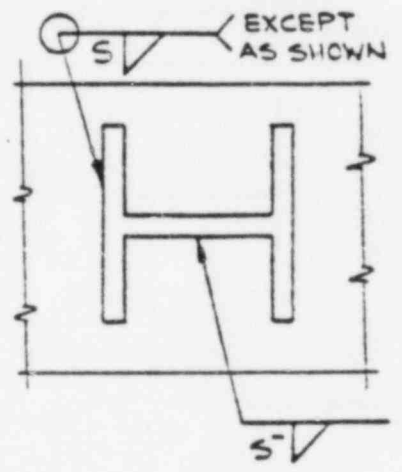
EXAMPLE 1



AS DESIGNED

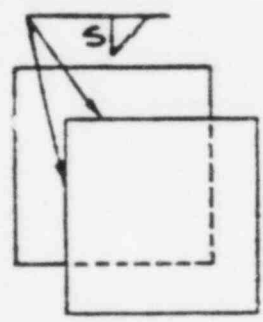


AS CONSTRUCTED

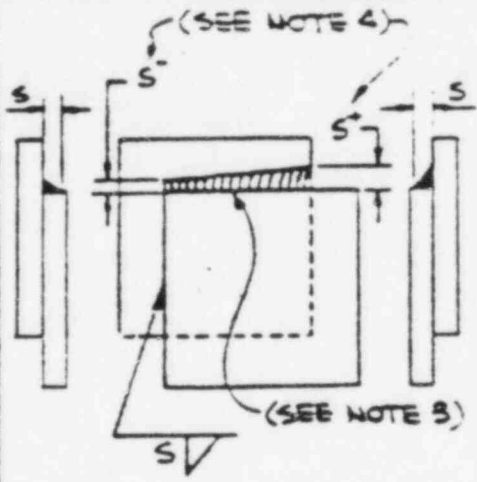


AS-BUILT REPRESENTATION

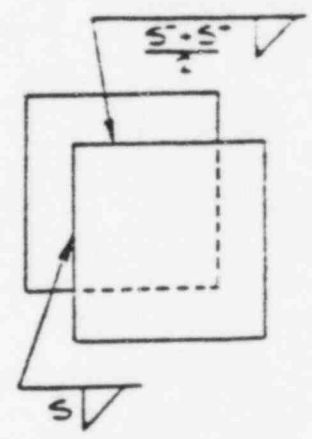
EXAMPLE 2



AS DESIGNED



AS CONSTRUCTED



AS-BUILT REPRESENTATION

15A-223

REF DOC PCN	N/A	RFI	N/A	WPPSS NUCLEAR PROJECT NO. 2	
REF SPEC SECTION	ISA	PAGE	ISA-223	BURNS AND ROE, INC.	
REF DWG	N/A	DWG ZONE	N/A	PED 215-H-W851	SH# 12 OF 22
SCALE	N/A	DRAWN BY	GEE	TITLE AS-BUILT	
		DATE	7/2/88	SPECIFICATION REVISION	
		CHECK BY	JITL	DATE	7/21/87

NOTES:

1. DESIGNATION LEGEND

- S - WELD LEG SIZE AS SPECIFIED ON THE SUPPORT DETAIL SHEET.
- S<sup>+</sup> - WELD LEG SIZE GAUGED AS LARGER THAN SPECIFIED ON THE SUPPORT DETAIL SHEET.
- S<sup>-</sup> - WELD LEG SIZE GAUGED AS SMALLER THAN SPECIFIED ON THE SUPPORT DETAIL SHEET.

2. IN EXAMPLE 1, THE S<sup>-</sup> → S<sup>+</sup> DESIGNATION INDICATES AN IRREGULAR WELD PROFILE AS PER CASE 3.

3. IN EXAMPLE 2, THE WELD REFERENCING THIS NOTE HAS A REGULAR TAPERED WELD PROFILE AS PER CASE 2.

4. IN CASE 2 AND EXAMPLE 2, THE MINIMUM PERMISSIBLE VALUES FOR S<sup>-</sup> AND S<sup>+</sup> ARE AS FOLLOWS:

$$S^- \geq S - 1/16"$$

$$S^+ \geq S + 1/16"$$

15A-224

REF DOC PCN	N/A	RFI	N/A	WPPSS NUCLEAR PROJECT NO. 2	
REF SPEC SECTION	15A	PAGE	15A-224	PARA	ATTACH. 10
REF DWG	N/A	DWG ZONE		PED 215-H-WBS1	SH-13 OF 22
SCALE	N/A	DRAWN BY	EE	DATE	7/22/83
		CHKD BY	WTM	DATE	7/20/83
		APPROV	EE	DATE	7/27/83
				TITLE AS-BUILT	
				SPECIFICATION REVISION	

- f) Applicable reference drawings and revision numbers (e.g.: B&R or Vendor flow diagrams, piping and equipment drawings.)
- g) Identification of all field welds by number.
- h) DELETED
- i) DELETED
- j) Contract limits (as applicable)
- k) Piping Code Group
- l) Quality Class
- m) Seismic Category
- n) Design Pressure
- o) Design Temperature
- p) Operating Temperature
- q) Identification of floor and wall penetrations including size.

r) Identification of all pipe supports by B&R tag number to include location relative to piping.

ADD

215SRP 78

215

15B-6a

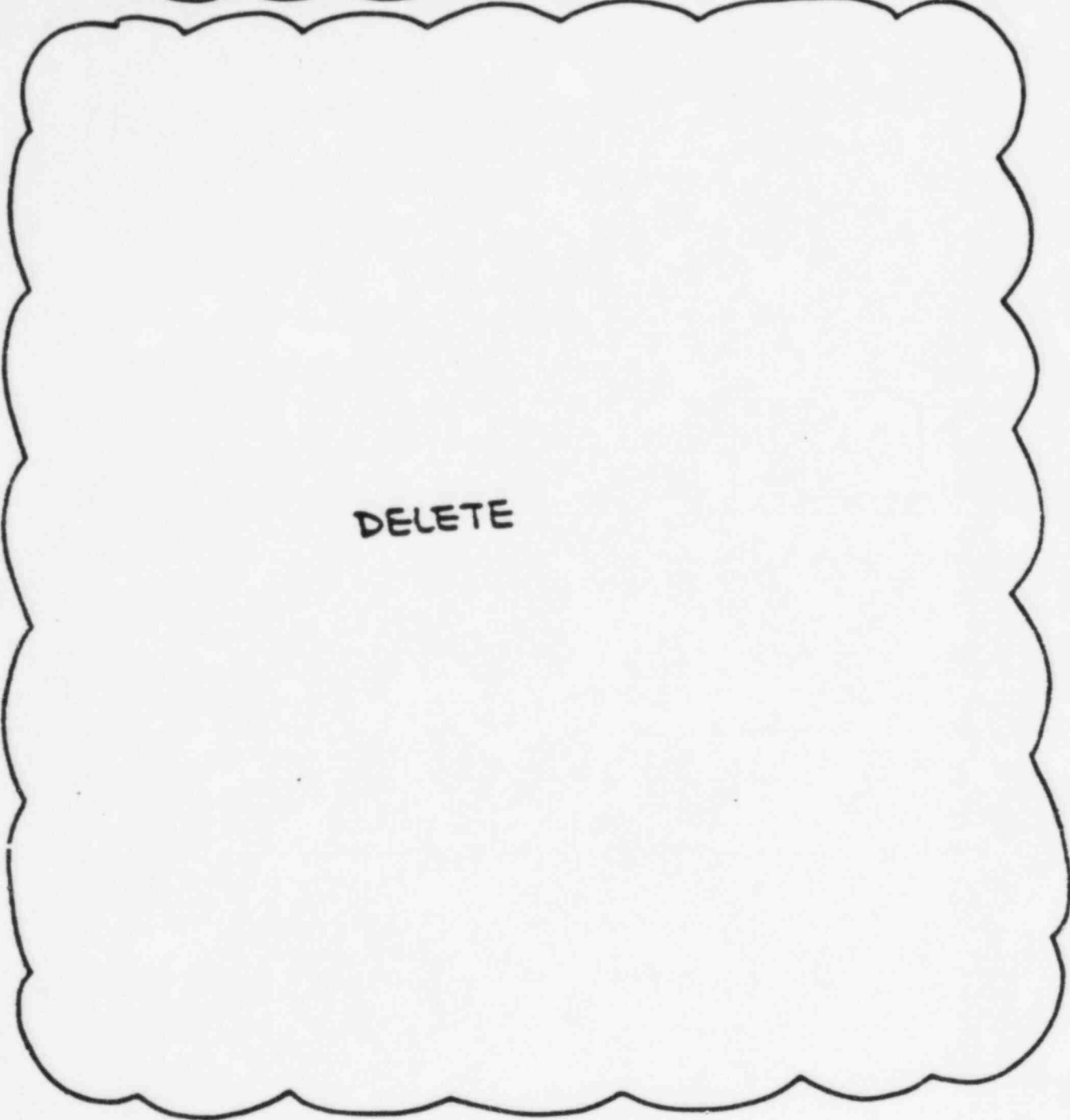
Rev. 2, 10/1/81

REF DOC PCN	N/A	RFI	N/A	WPPSS NUCLEAR PROJECT NO. 2	
REF SPEC SECTION	15B	PAGE	15B-6a	PARA	3.5.1
REF DWG	N/A	DWG ZONE		PED 215-H-WBS1	SHT 14 OF 22
SCALE	N/A	DRAWN BY	GEE	DATE	7/23/81
		CHKD BY	WTM	DATE	7/29/81
		APPRD	[Signature]	DATE	7/29/81
				TITLE AS-BUILT SPECIFICATION REVISION	

3.5.3 "As-Built" Drawing Confirmation

Contractor shall furnish Owner with "As-Built" Piping Erection Diagrams in accordance with the requirements of Section 15A, Article 3.0, Paragraph 3.11.

← ADD



DELETE

#215

15B-8a

Rev.1, 5/14/82

REF DOC PCN	N/A	RFI	N/A	WPPSS NUCLEAR PROJECT NO. 2	
REF SPEC SECTION	15B	PAGE	15B-8a	PARA	3.5.3
REF DWG	N/A	DWG ZONE		PED	215-H-WBS1
SCALE	N/A	DRAWN BY	GE	DATE	7/21/82
		CHKD BY	ITM	DATE	7/21/82
				TITLE	AS-BUILT SPECIFICATION REVISION

SH-15 OF 22

TABLE OF CONTENTS (Cont'd)

DELETE

<del>EXHIBIT 2</del>	<del>Approved Method for the As-Built Representation of Complex Weids</del>	<del>15R-45</del>	
EXHIBIT 3	Installation Instructions for Belleville Spring Washers	15R-48	2
EXHIBIT 4		15R-54	
EXHIBIT 5		15R-57	2

# 215SRP223

115

15R-ii

Rev. 2, 2/11/83

REF DOC PCN	N/A	RFI	N/A	WPPSS NUCLEAR PROJECT NO. 2	
REF SPEC SECTION	15R	PAGE	15R-ii	PARA	N/A
REF DWG	N/A	DWG ZONE		PED 215-H-W851	SH. 16 OF 22
SCALE		DRAWN BY	GEE	LATE?	
		N/A		TITLE AS-BUILT	



... which are Seismic

Quality Class II and G drilled-in concrete anchors which are Seismic Category I shall have their installation verified and documented by Q.C. or at Contractor's option, evaluated and documented by Contractors Engineering, in accordance with Paragraph 3.12.4 (a) above.

*D*  
*H*  
*P*

c) Quality Class II and G, Non Seismic Category I

A minimum of 20% of the Quality Class II and G, Non Seismic Category I, drilled-in concrete anchors shall have their installation verified and documented by QC or, at Contractors option, evaluated and documented by Contractors Engineering, in accordance with paragraph 3.12.4 (a) above.

3.13 "As-Built" Drawings

# 215SRP223

DELETE 2  
REVISE

Contractor shall furnish Owner with "As-Built" ~~Support Detail Sheets~~ Support Detail Sheets in accordance with the requirements of ~~Section 15A, Article 3.0, Paragraph 3.11.~~

DELETE

~~Support Detail Sheets~~

0215

15R-31

Rev. 1, 2/11/83

REF DOC PCN	N/A	RFI	N/A	WPPSS NUCLEAR PROJECT NO. 2	
REF SPEC SECTION	ISR	PAGE	ISR-31	PARA	3.13
REF DWG	N/A	DWG ZONE		PED 215-H-W851	SH: 17 OF 22
SCALE	N/A	DRAWN BY	GEE	DATE	7/22/83
		CHKD BY	WTM	DATE	7/29/83
		APPROV	GEE	DATE	7/21/83
				TITLE	AS-BUILT SPECIFICATION REVISION

~~Contractor may "As-Built" the Large Diameter Support Erection Diagrams in combination with the Large Diameter Piping Erection Diagrams identified in Section 15B, Article 3.0, Paragraph 3.5.3 of this Specification. Under this option, the Contractor is required to meet the "As-Built" requirements of both Specification Sections (i.e.: Section 15B and this Section).~~

~~Dimensions shown on the "As-Built" Drawings shall be accurate to within the tolerances shown below.~~

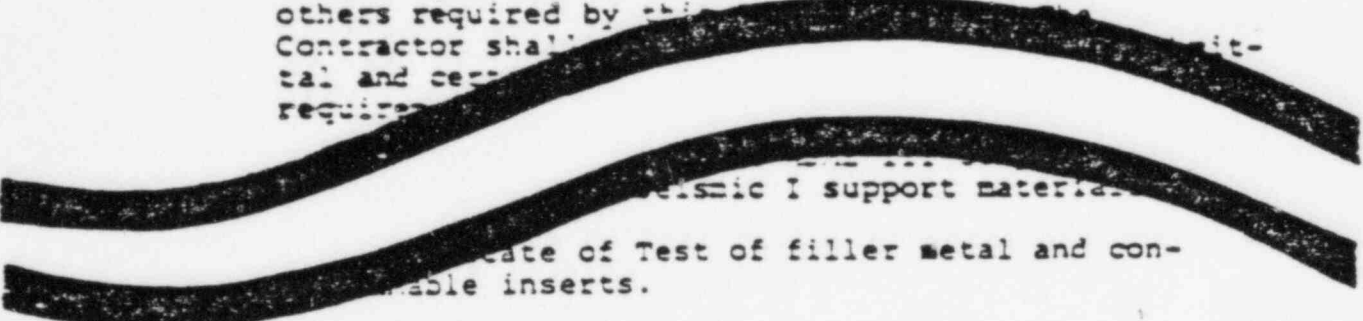
- ~~± 10% For the dimensions up to 5".~~
- ~~± 1/2" For the dimensions 5" and/or larger.~~
- ~~± 1° For the angle measurements.~~

~~The As-Built representation of welds on the Support Detail Sheets shall meet the requirements of Exhibit 2.~~

DELETE →

3.14 Documentation

(a) The Contractor shall submit to the Owner the following documents, as applicable, in addition to others required by this Specification. The Contractor shall submit the following documents, as applicable, in addition to others required by this Specification:



- .4 Weld records.
- .5 Weld repair records.
- .6 Nondestructive test reports.
- .7 Shop test reports.
- .8 Weld maps.
- .9 As-builts.

**215SRP236**

#215

15R-32

Rev. 2, 3/17/83

REF DOC PCN	N/A	RFI	N/A	WPPSS NUCLEAR PROJECT NO. 2	
REF SPEC SECTION	15R	PAGE	15R-32	PARA	3.13
REF DWG	N/A	DWG ZONE		PED 215-H-W851	SHT 18 OF 21
SCALE	N/A	DRAWN BY	GEE	DATE	7/2/83
		CHKD		TITLE	AS-BUILT
				SPECIFICATION REVISION	

DELETED

215SRP1

#215

15R-45

Added, 9/10/82

REF DOC PCN	N/A	REF	N/A	WPPSS NUCLEAR PROJECT NO. 2					
REF SPEC SECTION	15R	PAGE	15R-45	PARA	EXHIBIT 2	BURNS AND ROE, INC.			
REF DWG	N/A	DWG ZONE		PED 215-H-W851	SHT 19 OF 22				
SCALE	N/A	DRAWN BY	GEE	DATE	7/27/83	TITLE AS-BUILT SPECIFICATION REVISION			
		CHKD BY	WTM	DATE	7/29/83	APPRD	GEE	DATE	7/29/83

DELETED

215SRP19!

#215

15R-46

Added 9/10/82

REF DOC PCN	N/A	RFI	N/A	WPPSS NUCLEAR PROJECT NO. 2						
REF SPEC SECTION	15R	PAGE	15R-46	PARA	EXHIBIT 2	BURNS AND ROE, INC.				
REF DWG	N/A	DWG ZONE		PED	215-H-W851	SHT	20 OF 22			
SCALE	N/A	DRAWN BY	EE	DATE	7/2/83	TITLE	AS-BUILT			
		CHECK BY	WJM	DATE	7/29/83	APPROV	EE	DATE	7/29/83	SPECIFICATION REVISION

DELETED

# 215SRP195

#215

15R-47

Added, 9/10/82

REF DOC PCN	N/A	RFI	N/A	WPPSS NUCLEAR PROJECT NO. 2						
REF SPEC SECTION	15R	PAGE	15R-47	PARA	EXHIBIT 2	BURNS AND ROE, INC.				
REF DWG	N/A	DWG ZONE		PED	215-H-WBS1	SHT	21 OF 22			
SCALE	N/A	DRAWN BY	EE	DATE	7/24/83	TITLE	AS-BUILT			
		CHKD BY	WTM	DATE	7/29/83	APPRD	EE	DATE	7/21/83	SPECIFICATION REVISION

DIVISION 1

SECTION 1C

DRAWINGS AND DATA

1.0 CONTRACT DRAWINGS

The drawings listed below form an integral part of the technical specification and Contract Documents;

In addition to drawings listed herein, piping isometric drawings as listed in the Isometric Drawing Control Log (IDCL) and hanger detail drawings as listed in the Hanger Drawing Control Log (HDCL) are included as Contract Drawings under this Specification.

The Contractor shall refer to the DCL issued bi-weekly, the IDCL issued bi-weekly, and the HDCL issued weekly under separate covers and make a part hereof, as the reference for the latest revision of the applicable contract drawings as listed in this section of the contract documents.

The Contractor shall perform work using the latest revision of the contract and information drawings and refer to the DCL, IDCL, or HDCL for identification of changes to the drawings which have been directed by the Owner, but have not been incorporated in the latest revisions of the drawings.

The Contractor shall refer to both the BFI hanger and Isometric Final As-Built Tracking List and the Small Diameter As-Built Isometric Status Tracking List (ISTL), issued periodically under separate covers and make a part hereof, as the reference for the contract drawings to be included in the Contractor's As-Built Verification Program per the requirements Division 15, Section 15A, Article 3.0, Paragraph 3.11 of this Specification.

ADD

Contractor shall revise arrangement drawings dimensionally to suit all equipment purchased by him at no additional cost to Owner. Revised dimensions shall be incorporated on shop drawings and submitted to Owner for approval prior to purchasing any equipment effected by these revisions.

Drawing No.                      Rev.

2507

Interior Details, Sh. 2  
Misc. Interior Details, Sh. 2

#215

1C-1

Rev. 41, 11/8/82

REF DOC PCN	N/A	RFI	N/A	WPPSS NUCLEAR PROJECT NO. 2	
REF SPEC SECTION	IC	PAGE	1C-1	PARA	1.0
REF DWG	N/A	DWG ZONE	N/A	PED 215-H-W851	SHT 22 OF 22
SCALE	N/A	DRAWN BY	GEE	DATE	8/5/83
		CHECK BY	BMS	DATE	8/5/83
		APPROV	GEE	DATE	8/5/83
				TITLE	AS-BUILT SPECIFICATION REVISION

**Washington Public Power Supply System**

P.O. Box 968 3000 George Washington Way Richland, Washington 99352 (509) 372-5000

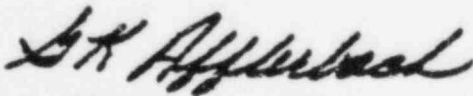
August 30, 1983  
WPBEC-C0500-F-83-3119

Responds to: N/A  
Response required by: N/A

Mr. J. Newgen  
Construction Manager  
Bechtel Power Corporation  
P.O. Box 600, Caller Service  
Richland, WA 99352

Subject: NUCLEAR PROJECT 2, CONTRACT-C0500  
PED TRANSMITTAL  
PED-215-H-W851

The attached Project Engineering Directive (PED) is forwarded to you for "DOCUMENT CLOSEOUT AND DRAWING UPDATE ONLY". Do not implement this PED unless specifically directed by the Supply System Test and Startup Department.



G.K. Afflerbach, (988U)  
Test and Startup Manager

GKA/1h

**RECEIVED**  
BECHTEL POWER CORPORATION

30  
AUG 29 1983

**JOB NO. 14631**

STANG AND ROE, INC.  
 NPPSS  
 NUCLEAR PROJECT  
 NO. 2

PROJECT  
 ENGINEERING  
 DIRECTIVE

NOT PAID

PROJECT ENGINEERING DIRECTIVE														
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15
DATE 07/26/83														
16/17 18/19 20/21														

REVISION FOR P. E. D.:

PROVIDE  $\pm \Delta \Delta$  TOLERANCES  
 FOR LARGE BORE SUPPORT  
 LOCATION DIMENSIONS ON  
 PIPING.

INFORMATION SHEET 1 OF 3  
 COPIES H, J. HOPKINS  
 K. FRINDRICH, M, PIPE STRESS

REFERENCES	
SUBJECT	H-501
LOCATION	VARIOUS
ENG. SYSTEM	VARIOUS
SU SYSTEM	970.0-P00
QUALITY CLASS	I, II & G

ORIGINATING BRI REVIEW OF  
 DOCUMENTS H-501

DESCRIPTION OF WORK:

VOID PED 215-H-P518  
 INFORMATION CONTAINED IN PED 215-H-P518 IS  
 INCORPORATED INTO THIS PED.

REVISE CONTRACT DRAWING H-501 AS SHOWN  
 ON SH 3 OF THIS PED.

RECEIVED  
 GENERAL POWER CORPORATION

AUG 23 1983

JOB NO. 14631

**REVIEWED BY S/U**

NOTES

- THIS PED REVISES DIRECTION PREVIOUSLY PROVIDED BY THE FOLLOWING PED(S): N/A
- THIS PED VOIDS DIRECTION PREVIOUSLY PROVIDED BY THE FOLLOWING PED(S): PED 215-H-P518
- THIS PED WORK SHOULD BE COORDINATED WITH KNOWN OTHER WORK UNDER THE FOLLOWING PED'S: N/A
- THIS PED DEPENDS ON THE PRIOR INSTALLATION OF THE FOLLOWING PED'S: N/A

REVISE:  
 NONE \_\_\_\_\_  
 DRAWINGS ✓  
 SPECIFICATION \_\_\_\_\_

APPROVALS:  
W. J. Murphy 7/26/83  
 DISCIPLINE ENGINEER DATE  
W. J. Murphy 7/29/83  
 GROUP SUPERVISOR DATE  
J. R. Miller 8/3/83  
 PROJECT ENGINEER DATE



PROJECT ENGINEERING DIRECTIVE

CODE: PROJECT ENGINEERING DIRECTIVE  
215 - H - W941

WNP 2 BURNS & ROE, INC. PAGE 2 OF 3

REFERENCE DRAWINGS

REFERENCE DRAWINGS

DRAWING NO.	SHEET NO.	SUFFIX	REV.
11-501-	01-		02
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-

DRAWING NO.	SHEET NO.	SUFFIX	REV.
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-

22 23 24 25 26 27 28 29 30 31 32 33 34 35 36 37 38 39

22 23 24 25 26 27 28 29 30 31 32 33 34 35 36 37 38 39

REFERENCE SPEC. PARAGRAPHS

SECTION	PARAGRAPH	PAGE	REV.
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-

1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26 27 28 29

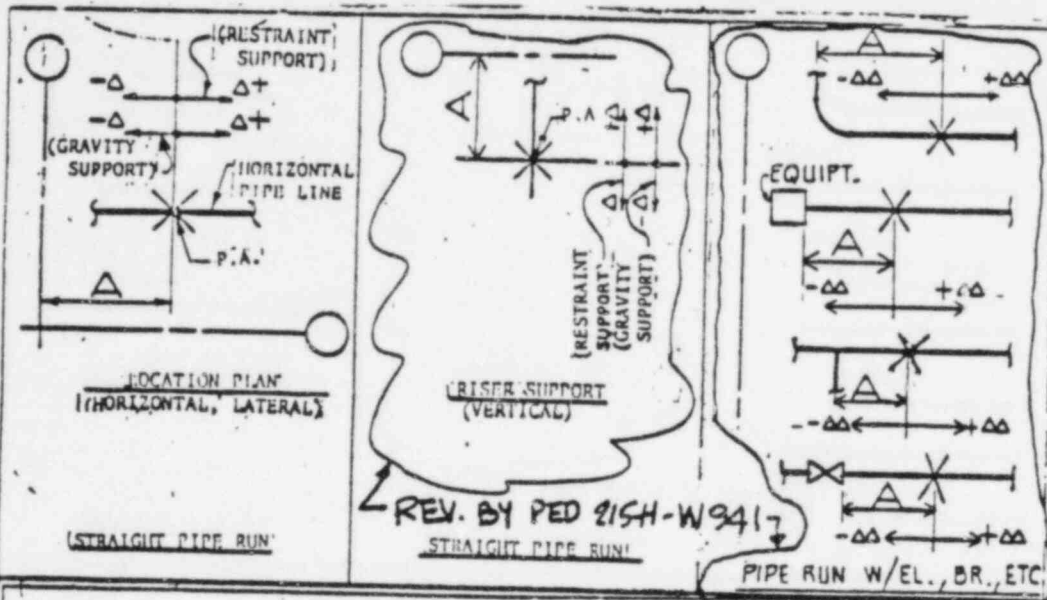


TABLE 22 RELOCATION TOLERANCES OF P. A. ( $\Delta$ ,  $\Delta\Delta$ )

LARGE BORE - HANGER					SMALL BORE - HANGER				
TYPE OF LINE & HGR.	PIPE SIZE	+ $\Delta$	- $\Delta$	+ $\Delta\Delta$	PIPING CLASS	OPERAT. TEMP.	+ $\Delta$	- $\Delta\Delta$	REMARKS
HORIZONTAL (GRAVITY)	< 4"	4"	4"	2"	INSIDE OF CONTAINMENT				
	4" ~ 12"	8	8	4					
	> 12"	18	18	6	NB	ALL	1	1	
HORIZONTAL (RESTRAINT)	< 4"	4	4	2	OUTSIDE OF CONTAINMENT				
	4" ~ 12"	6	6	3					
	> 12"	12	12	6	NC ND B31.1	ALL	2	2	
VERTICAL (GRAVITY)	< 4"	6	6	N/A	OUTSIDE OF CONTAINMENT				
	4" ~ 12"	12	12						
	> 12"	18	18		NB NC ND B31.1	ALL	6	2	
VERTICAL (RESTRAINT)	< 4"	6	6	N/A	NOTE: 1. TOLERANCES SHOWN ABOVE APPLY UNLESS OTHERWISE NOTED ON THE HANGER ISOMETRIC DRAWING. 2. ( $\Delta$ ) AND ( $\Delta\Delta$ ) TOLERANCES ARE NON-ADDITIVE.				
	4" ~ 12"	10	10						
VERTICAL (RESTRAINT)	< 4"	6	6	N/A					
	4" ~ 12"	10	10						
REMARKS									

REVISED BY PED 215-H-W941

REF DOC PCN	RFI	WPPSS NUCLEAR PROJECT NO. 2	
REF SPEC SECTION	PAGE	BURNS AND ROE, INC.	
REF DWG	DWG ZONE	PED 215-H-W941	SHT 3 OF 3
SCALE	DRAWN BY	DATE	TITLE
	WTM	7/27/83	DWG. H-501, SH 1, DETAIL 22
CHKD BY	DATE	APPRD	DATE
	7-27-83	GEE	7/29/83

Washington Public Power Supply System

P.O. Box 968 3000 George Washington Way Richland, Washington 99352 (509) 372-6000

August 23, 1983  
WPBEC-C0500-F-83-3021

Responds to: N/A  
Response required by: N/A

Mr. J. Newgen  
Construction Manager  
Bechtel Power Corporation  
P.O. Box 600, Caller Service  
Richland, WA 99352

Subject: NUCLEAR PROJECT 2, CONTRACT C0500  
PED TRANSMITTAL  
PED-215-H-W941

The attached Project Engineering Directive (PED) 215-H-W941 is forwarded for implementation and you are hereby authorized to assign work as required by this PED. Master Work List (MWL) input is required for all physical work to be performed on this PED. A Startup Work Request (SWR) processed in accordance with PMI 9-1.1 is required prior to commencing physical work on provisionally accepted or turned-over system(s).

The following system package(s) affected by this PED have been provisionally accepted or turned-over as of this transmittal; NONE.

*GK Afflerbach*

G. K. Afflerbach, 927M  
Test and Startup Manager

JOC/1h

RECEIVED  
BECHTEL POWER CORPORATION

AUG 23 1983

JOB NO. 14531

BURNS AND ROE, INC.  
WPPSS  
NUCLEAR PROJECT  
NO. 2

PROJECT  
ENGINEERING  
DIRECTIVE

NON-PAID

CODE:	PROJECT ENGINEERING DIRECTIVE														
2	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	
DATE	0	8	/	0	1	/	8	3							
	X	16	17	X	18	19	X	20	21						

REASON FOR P. E. D.:

TO INCORPORATE COMMENTS  
AND SUGGESTIONS FROM  
REVIEWERS AND USERS OF  
H-501 DWG.

INFORMATION

COPIES H, I. HOPKINS  
K. FRINDRICH, 208

SHEET 1 OF 17

REFERENCES

SUBJECT H-501  
LOCATION VARIOUS  
ENG. SYSTEM VARIOUS  
SU SYSTEM 970.0-HOP  
QUALITY CLASS I

ORIGINATING  
DOCUMENTS

DESCRIPTION OF WORK:

REVISE DWG. H-501 AS SHOWN ON  
SHEETS 3 THRU 16 OF THIS PED.

RECEIVED  
BECHTEL POWER CORPORATION

AUG 22 1983

JOB NO. 14651

REVIEWED BY S/U

NOTES

1. THIS PED REVISES DIRECTION PREVIOUSLY PROVIDED BY \_\_\_\_\_ THE FOLLOWING PED(S): \_\_\_\_\_
2. THIS PED VOIDS DIRECTION PREVIOUSLY PROVIDED BY \_\_\_\_\_ THE FOLLOWING PED(S): \_\_\_\_\_
3. THIS PED WORK SHOULD BE COORDINATED WITH KNOWN OTHER WORK UNDER THE FOLLOWING PED'S: \_\_\_\_\_
4. THIS PED DEPENDS ON THE PRIOR INSTALLATION OF THE FOLLOWING PED'S: \_\_\_\_\_

REVISE:

NONE  
DRAWINGS H-501 SHT. LAND 2  
SPECIFICATION \_\_\_\_\_

APPROVALS:

W. K. Buchner WTM 7-29-83  
DISCIPLINE ENGINEER DATE  
John M. King 8/1/83  
GROUP SUPERVISOR DATE  
OM 8/1/83  
PROJECT ENGINEER DATE

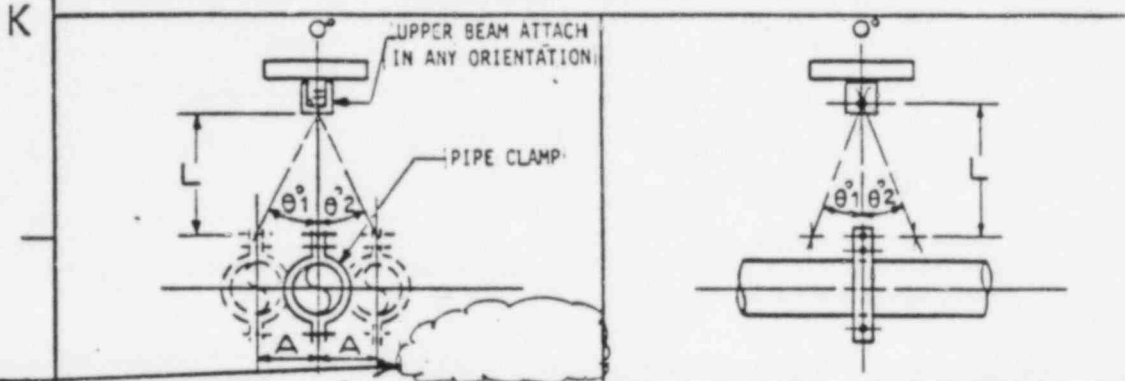


PED-H-949

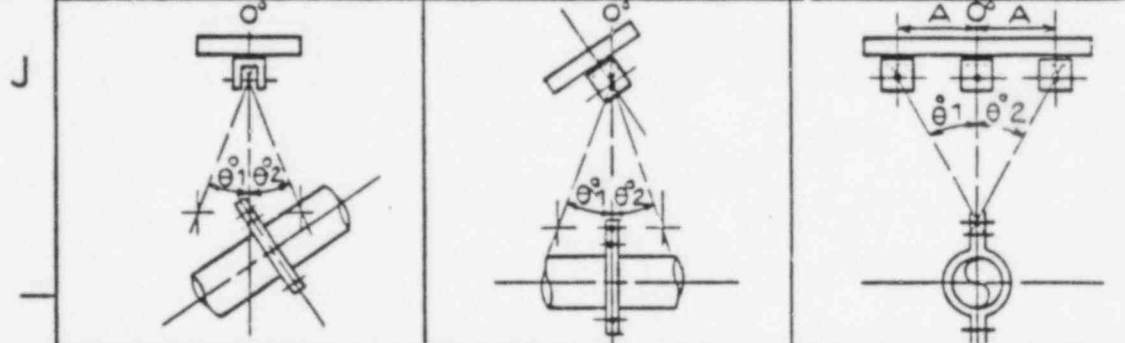
DETAIL #13 TOLERANCE FOR MISALIGNMENT OF P. A. & S.A. FOR ROD HANGERS OR SPRING HGRS.

NOTES:

- (1) THESE TOLERANCES SHALL NOT BE UTILIZED FOR RIGID STRUT OR SNUBBER INSTALLATIONS. THESE TOLERANCES ARE TO BE USED BY ERECTION INSTALLATION OR ENGINEERING FOR THE PROPER FUNCTIONING OF ROD HANGERS. TOLERANCES ARE MEANT TO LIMIT THE MISALIGNMENT TO A MINIMUM.
- (2) TOLERANCES FOR  $\theta_1$ ,  $\theta_2$ , AND A CANNOT BE COMBINED.



PED-H-W949



$\theta_1, \theta_2$ : ANGLE DUE TO THE MISALIGNMENT OR OFF-SETTING OF THE PINS TO THEIR ORIGINAL DESIGN AXI

$A=L \cdot \tan \theta$  ( $\theta = \theta_1$  OR  $\theta_2$ ). TABLE 1 TOLERANCE FOR ANGULARITY OF ROD HANGERS OR SPRING HGRS.

DESIGN OFFSET	DESIGN TOLERANCE	SHOP FAB. TOLERANCE	ERECTION TOL. W/O OFFSET
$\theta_1$ $\pm 4^\circ$ $-0^\circ$	N/A	N/A	$\pm 0.2^\circ$
$\theta_2$ $\pm 4^\circ$ $-0^\circ$	N/A	N/A	$\pm 0.2^\circ$
A	$L \tan \theta$	N/A	$\pm 0.2(A)$

PED-H-W949

EXAMPLE: IF OFFSET =  $2^\circ$  ERECTION TOLERANCES ARE  $\pm 0.2 \times (2^\circ) = \pm 0.4^\circ$   
OR  $(1 \pm 0.2) \times 'A'$  ON 'A' DIMENSION.

PED-H-W949

IF  $\theta = 4^\circ$ , IT ALLOWS  $\frac{13}{16}$ " LATERAL MOVEMENT FOR EVERY FT. OF ROD LENGTH. FOR EXAMPLE ROD LENGTH ON THE HANGER WAS 4'0" THEN THE S.A. POINT OR THE P.A. POINT COULD BE MOVED  $4 \times \frac{13}{16} = 3\frac{1}{4}$ ".

REF DOC PCN	RFI	WPPSS NUCLEAR PROJECT NO. 2
REF SPEC SECTION	PAGE	BURNS AND ROE, INC.
REF DWG H-501 SH 1	DWG ZONE	PED 215-H-W949 SH 3 OF 17
SCALE	DRAWN BY K15 DATE 7-8-83	TITLE H-501
	CHKD BY WTM DATE 7-29-83	

PED-H-  
W 949

DETAIL 2 TOLERANCE FOR MISALIGNMENT OF P.A. & S.A.

FOR STRUTS & SNUBBERS

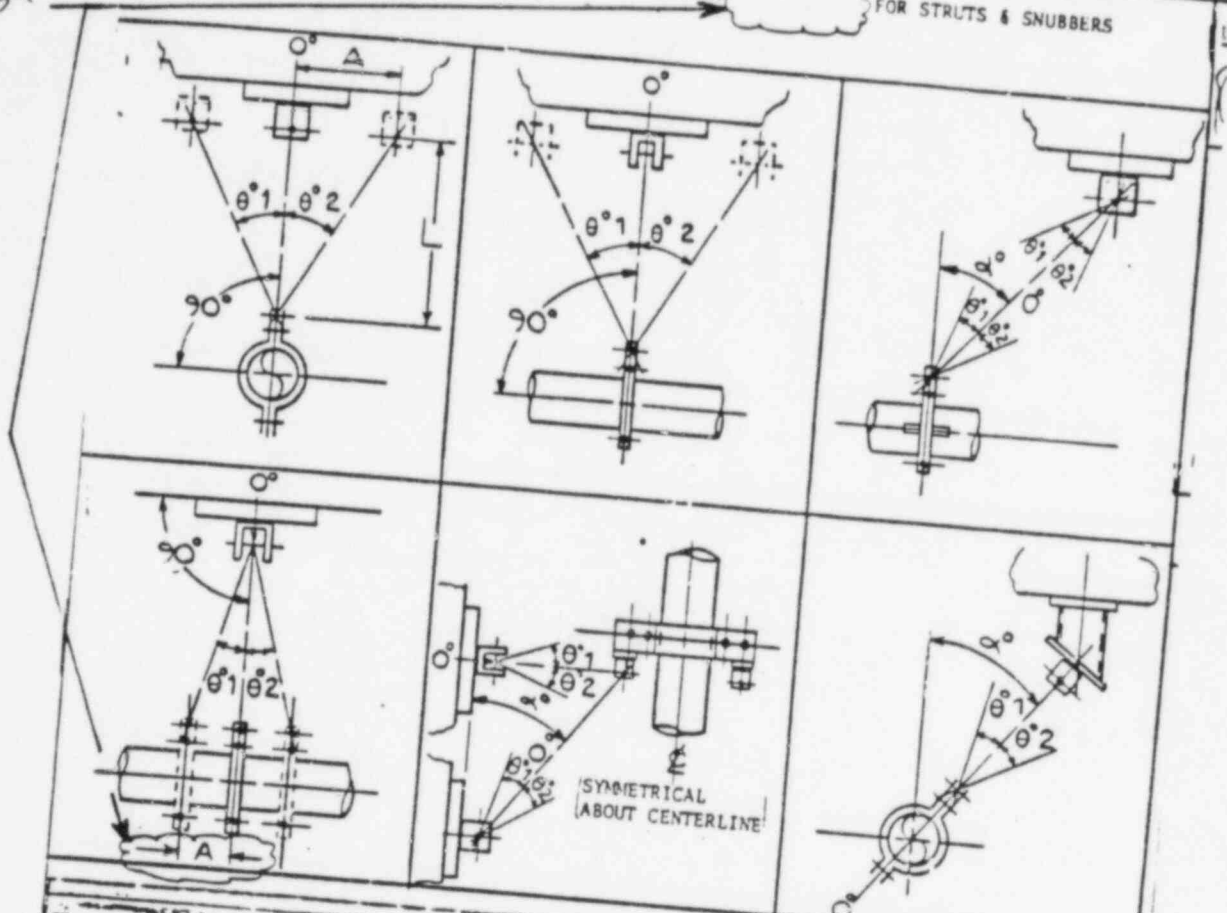


TABLE 2 TOLERANCES FOR ANGULARITY OF SNUBBER & STRUT ASSY'S

DESIGN OFFSET	DESIGN TOLERANCE	SHOP FAB. TOLERANCE	ERECTION		ERECTION NO OFFSET GE SNUBBER ONLY
			OFFSET	NO OFFSET	
$\theta_1$	$\pm 3^\circ$ $-0^\circ$	N/A	N/A	$\pm 0.2\theta_1$	$+4^\circ$ $-0^\circ$
$\theta_2$	$\pm 3^\circ$ $-0^\circ$	N/A	N/A	$\pm 0.2\theta_2$	$+2^\circ$ $-0^\circ$
"A"	$L(\tan \theta)$	N/A	N/A	$\pm 0.2A$	$+2^\circ$ $-0^\circ$ $+A$ $-0$

- NOTE: 1.  $\theta$  = DESIGN ANGLE  
 2. CONSTRUCTION TOLERANCE: IF OFFSET BY DESIGN IS "A", THEN TOLERANCES ON "A" =  $\pm 20\%$  OF A  
 3. A = L (TAN  $\theta$ )  
 4. TOLERANCES FOR  $\theta_1$  &  $\theta_2$  CAN NOT BE COMBINED

PED-H-W 949

OC PCN

RFI

REC SECTION

PAGE

PARA

WPPSS NUCLEAR PROJECT NO. 2

BURNS AND ROE, INC.

YG H-501 SHT 1

OWG ZONE

PED 215-H-W949 SHT 4 OF 17

DRAWN BY *[Signature]* DATE 7.8.83

K12

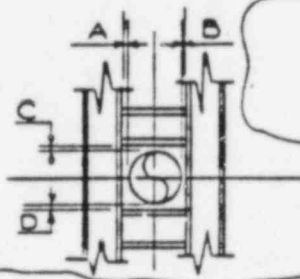
CHKD BY WTH DATE 7-29-83

*[Signature]* DATE 8/1/83

TITLE

H-501

CLEARANCE FOR DUA FRAME HANGER  
 DETAIL # 5 CASE: 1. HORIZONTAL & LATERAL RESTRAINT-RISER  
 2. VERTICAL (SEISMIC LOAD ONLY) & LATERAL RESTRAINT-HORIZONTAL RUN  
 (SEE GENERAL NOTES 7&13)



CASE 1 PLAN VIEW  
 CASE 2 ELEV. VIEW

TABLE 3 TOLERANCES IN (INCHES)

ITEM	OPER. TEMP.	PIPE DIA. (INCH)	ERECT. DIM.	ERECT. TOL.
A OR B	—	ALL	1/16	—
C OR D	—	ALL	1/16	—
A + B OR C + D	$\geq 200^{\circ}\text{F}$	1/2 ~ 12	1/8	+0 -1/16
		14 & OVER	1/8	+1/16 -0
	$< 200^{\circ}\text{F}$	1/2 ~ 12	1/8	+0 -3/32
		14 & OVER	1/8	+0 -3/32

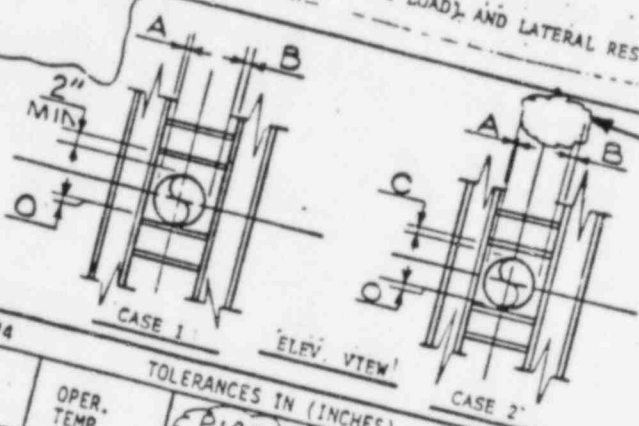
PED-H-W949

REF DOC PCN	RFI	WPPSS NUCLEAR PROJECT NO. 2	
REF SPEC SECTION	PAGE	PARA	BURNS AND ROE, INC.
REF DWG H-501 SHT 1	DWG ZONE	PED 215-H-W949	SHT 5 OF 17
SCALE	DRAWN BY KRB DATE 7-8-83	CHKD BY WTK DATE 7-29-83	TITLE H-501
	APPROVED BY [Signature] DATE 8/1/83		



DETAIL # 4 'CLEARANCE FOR BOX FRAME HANGER'  
 CASE: 1. 'DEAD LOAD' AND 'LATERAL RESTRAINT'  
 2. '(DEAD LOAD+SEISMIC LOAD), AND LATERAL RESTRAINT'

(SEE GENERAL NOTES 7&13)



PED-H-W 949

D-H-949

TABLE #4

ITEM	OPER. TEMP.	TOLERANCES IN (INCHES)		
		PIPE DIA (INCH)	ERECT. DIM.	ERECT. TOL J
A OR B	—	ALL	1/16	—
C	—	ALL	1/16	—
A + B	≥ 200°F	1/2 ~ 12	1/8	+1/32 -1/32
		14 & OVER	1/8	+0 -1/16
	200°F	1/2 ~ 12	1/8	+1/16 -0
		14 & OVER	1/8	+0 -3/32
		1/8	1/8	+0 -3/32

RFI	—	WPPSS NUCLEAR PROJECT NO. 2
PAGE	—	BURNS AND ROE, INC.
HT 1	PARA —	PED 215-H-W949 / SH: 6 OF 17
DATE 7-8-83	DWG ZONE	TITLE H-501
DATE 8/1/83	APPROV J B	

DETAIL 6

CLEARANCE FOR RADIAL STOPS IN PIPE SLEEVES (FLOORS & WALLSLEEVE)

(SEE GENERAL NOTES 7 & 13)

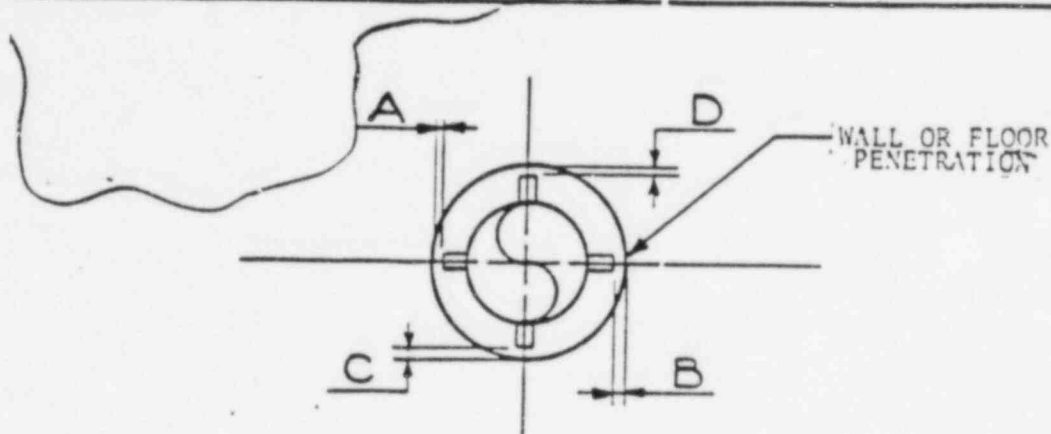


TABLE 6

ITEM	OP. TEMP	PIPE SIZE	ERECT. DIM.	ERECTION TOL.	REMARKS
A	-	ALL	1/16"	-	
B	-	ALL	1/16"	-	
C	-	ALL	1/16"	-	
D	-	ALL	1/16"	-	
A+B OR C+D	≥ 200°F	1/2" ~ 12"	1/8"	+0 -1/16"	
"	< 200°F	14" & OVER	1/8"	+1/16" - 0	
"	< 200°F	1/2" ~ 12"	1/8"	+ 0 - 3/32	
"	< 200°F	14" & OVER	1/8"	+0 -3/32	

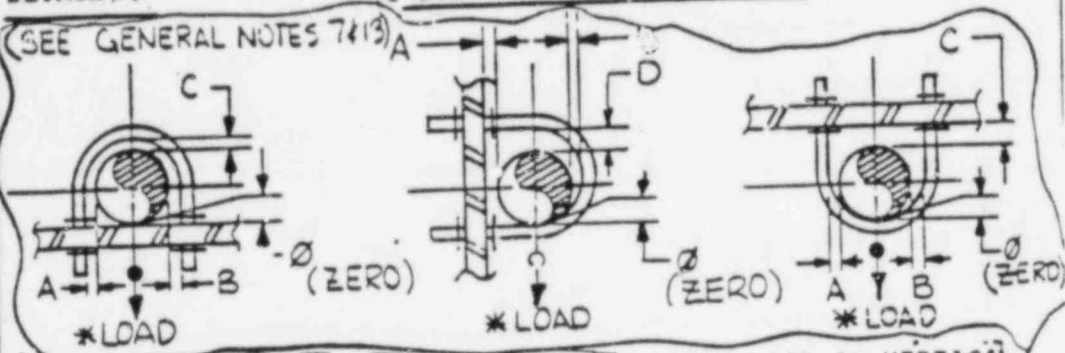
PED-H-W949

REF DOC PCN	RFI	WPPSS NUCLEAR PROJECT NO. 2	
REF SPEC SECTION	PAGE	BURNS AND ROE, INC.	
REF DWG	H-501 SHT 1	DWG ZONE	PED 215-H-W949 SHT 7 OF 19
SCALE	DRAWN BY <i>K. B. ...</i> DATE 7-8-83	CHKD BY <i>WTH</i> DATE 7-29-83	TITLE H-501
REV 1	<i>J4</i> <i>[Signature]</i> DATE 8/1/83		

DETAIL #7:

U-BOLT UNRESTRAINED AXIALLY

(SEE GENERAL NOTES 7 & 13)



CASE 1) DEAD LOAD IN VERTICAL DIRECTION. (USE ONLY FOR SEISMIC LOAD OR AS A GUIDE):

2) LATERAL, RESTRAINT AND DEAD LOAD AND/OR SEISMIC LOAD IN VERTICAL DIRECTION.

3) \*: IF NO LOAD EXISTS, A CLEARANCE MUST BE MAINTAINED. SEE GENERAL NOTE 7.

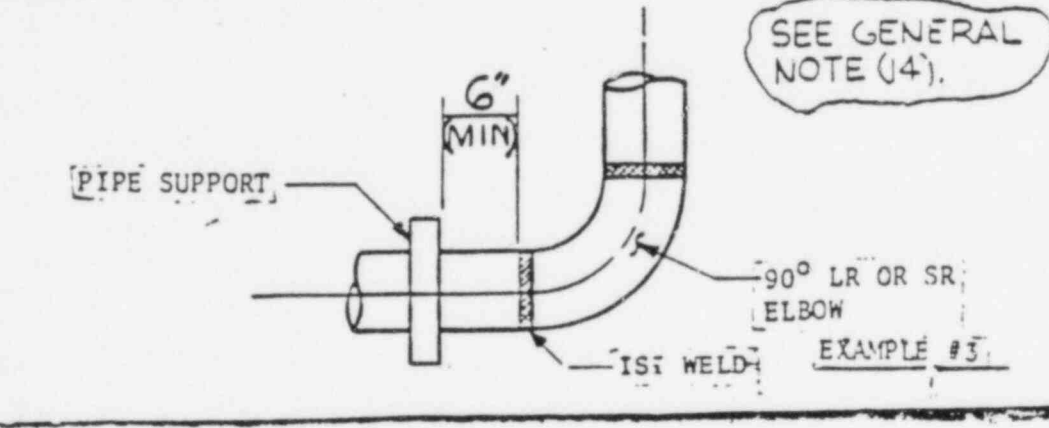
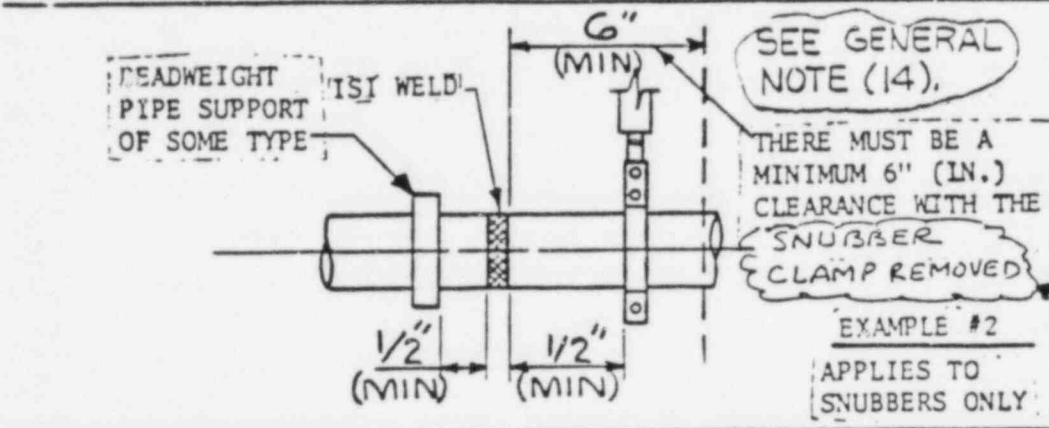
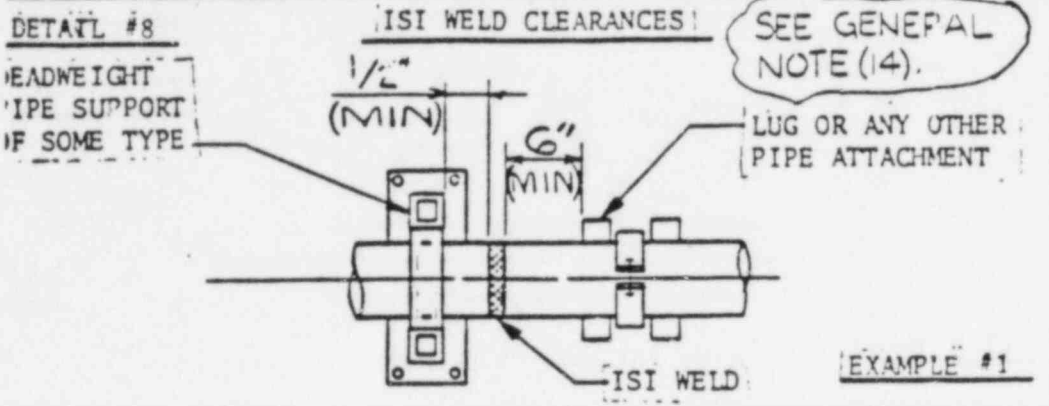
PED-H-  
W949

TABLE 7 TOLERANCES FOR U-BOLT ERECTION

ITEM	CASE	ERECT. DIM.	ERECT. TOL.
A	1 or 2	1/16"	
B	1 or 2	1/16"	
C	1 or 2	1/16"	+1/16 -0
D	1 or 2	1/16"	+1/16 -1/32
A&B	1 or 2	1/8"	+1/32 -1/8

REF DOC PCN	RFI	WPPSS NUCLEAR PROJECT NO. 2
REF SPEC SECTION	PAGE	PARA
REF DWG	H-501 SHT 1	DWG ZONE
SCALE	DRAWN BY: <i>H. W. Anderson</i> DATE: 7-8-83	F15
	CHKD BY: <i>W. T. M.</i> DATE: 7-27-83	TITLE
		H-501

PED 215-H-W949 SHT 8 OF 15

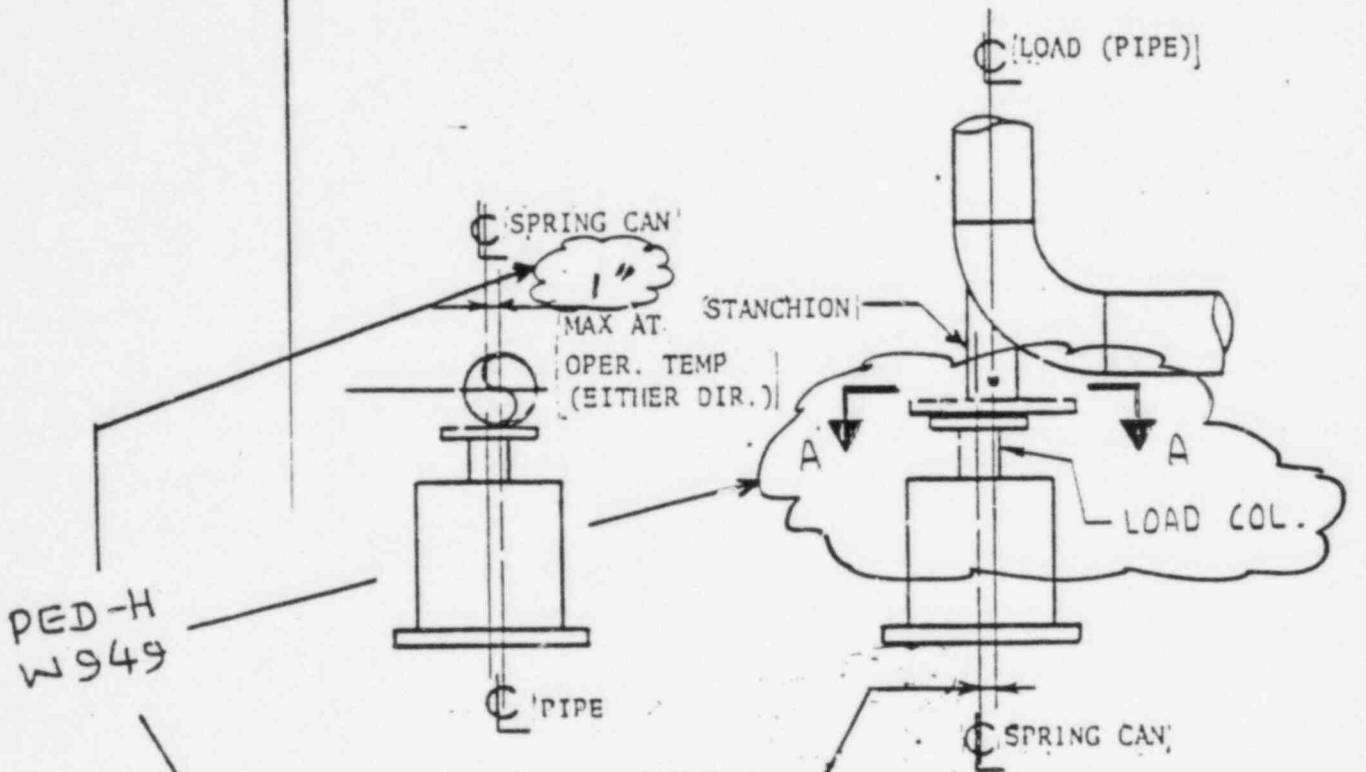


PED-H-W949

REF DOC PCN	RFI	WPPSS NUCLEAR PROJECT NO. 2
REF SPEC SECTION	PAGE	BURNS AND ROE, INC.
REF DWG H-501 SHT 1	DWG ZONE G13	PED 215-H-W949 SHT 9 OF 17
SCALE	DRAWN BY <i>K. Budheiser</i> DATE 7-8-83	TITLE H-501
	CHKD BY WTM DATE 7-29-83	
	APPROVED <i>[Signature]</i> DATE 8/1/83	

DETAIL #10

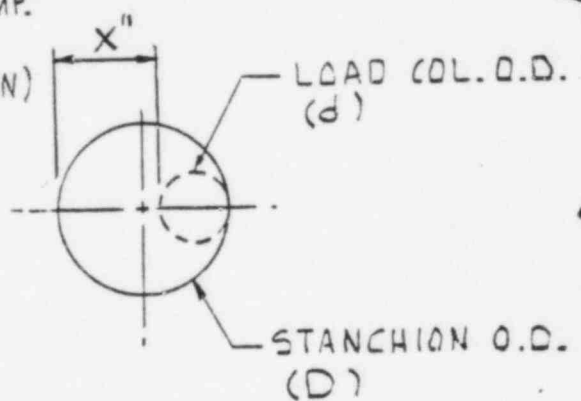
CENTERLINE TOLERANCES FOR INSTALLATION  
OF BOTTOM MOUNTED SPRING CANS



PED-H  
W949

MAX. AT OPER. TEMP.  
= THE LARGER OF  
1/2" (EITHER DIRECTION)  
OR 'X' DIM.

$X' \text{ DIM} = |D - d|$



SECT. A-A  
(FLANGES OMITTED FOR CLARITY)

REF. DOC. PCN		RFI	WPPSS NUCLEAR PROJECT NO. 2	
REF. SPEC. SECTION:		PAGE:	BURNS AND ROE, INC.	
REF. DWG. H-501 SHT. 1		DWG. ZONE	PED 215-H-W949 SHT. 10 OF 17	
SCALE	DRAWN BY: <i>K. Budh...</i> DATE: 7-27-83	G11		TITLE: H-501
	CHKD I.T.M. DATE: 7-29-83	DATE: 8/1/83		

REF DOC PCN  
 REF SPEC SECTION  
 REF DWG H-501 SH T 2  
 SCALE  
 DRAWN BY  
 CHECKED BY  
 DATE 7-29-83  
 DATE 8/1/83  
 WTM  
 DWG ZONE  
 PARA  
 TITLE  
 PED 215-H-W949 SH T 11 OF 17  
 H-501  
 WPPSS NUCLEAR PROJECT NO. 2  
 BURNS AND ROE, INC.

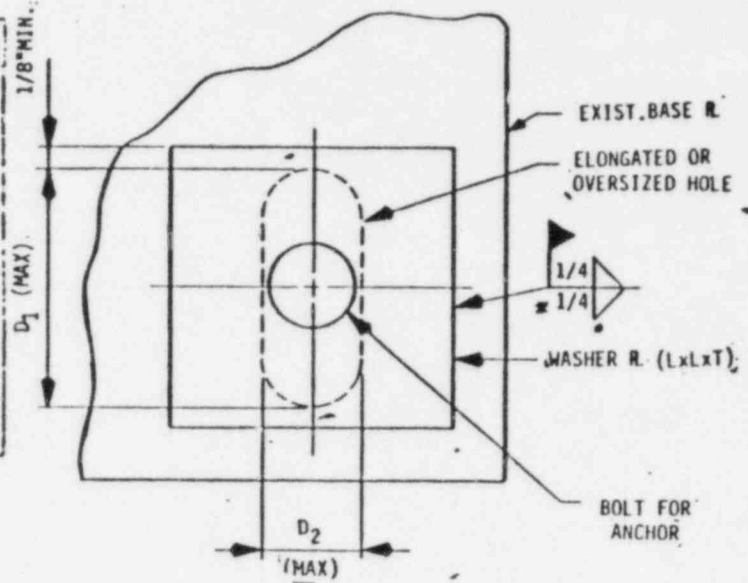
DETAIL 9

WASHER PLATE REQUIREMENTS FOR ENLARGED BOLT HOLES

ANCHOR SIZE	WASHER R.		D1 (MAX) (IN)	D2 (MAX) (IN)
	L (IN)	T (IN)		
1/4" Ø	1	1/4	3/4	7/2
3/8" Ø	1 1/4	1/4	1	5/8
1/2" Ø	1 3/4	1/4	1 1/2	3/4
5/8" Ø	2 1/4	1/4	2	15/16
3/4" Ø	2 1/2	1/4	2 1/4	1 1/8
7/8" Ø	3	1/4	2 3/4	1 5/16
1" Ø	3 3/4	1/2	3"	1 1/2
1 1/4" Ø	4 1/2	1/2	3 3/4	1 7/8

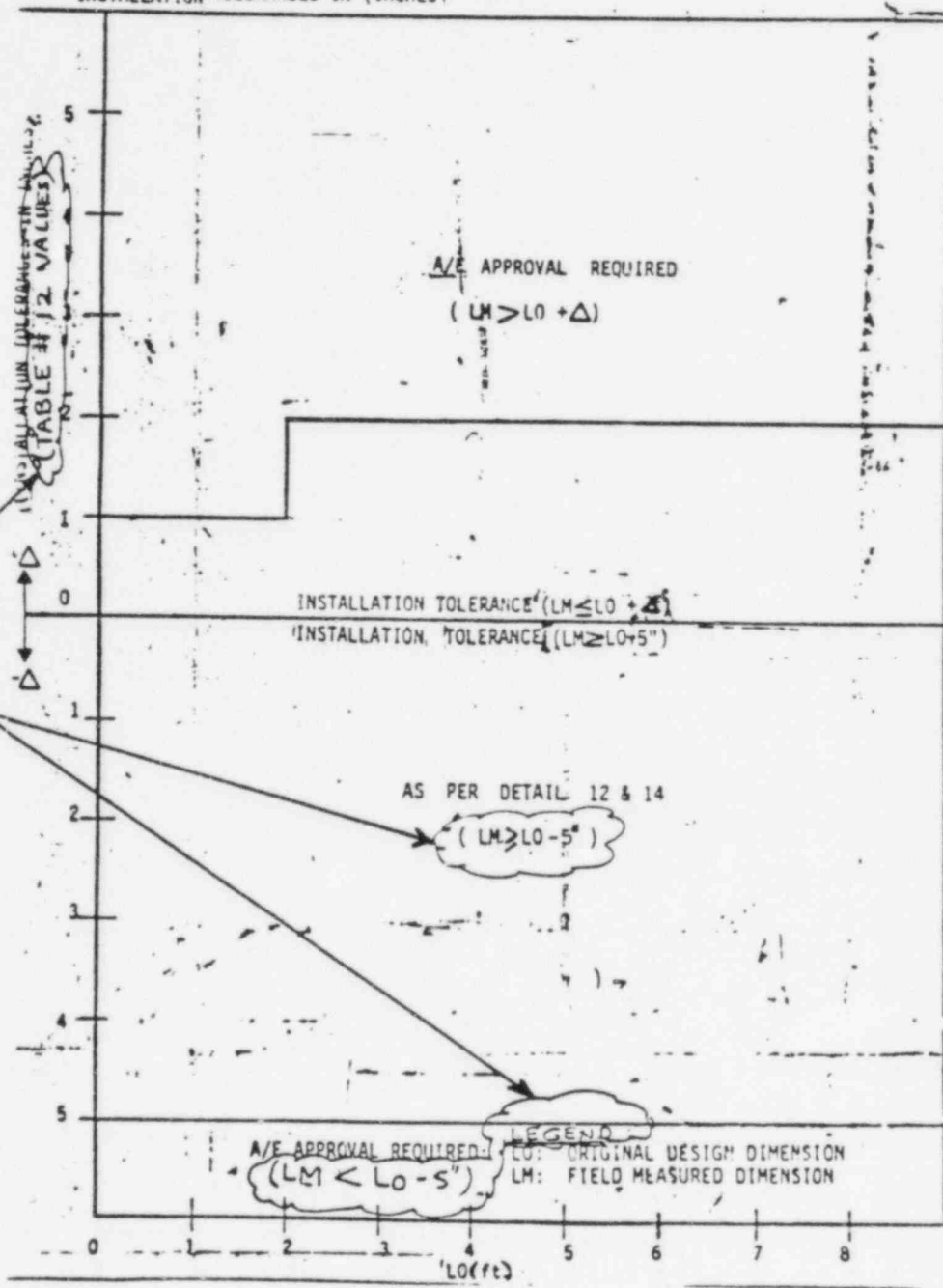
NOTES

- 1) HOLE IN WASHER R. TO BE IN ACCORDANCE WITH DETAIL 17.
- 2) WELDING SHOULD BE DONE ON ANY TWO OPPOSITE SIDES, HOWEVER, FOR 5/8" Ø AND LARGER ANCHORS, IF THE ELONGATION OCCURS TOWARDS THE CORNER OF THE BASE R, A MAXIMUM OVERHANG OF 1/4" IS ACCEPTABLE ON TWO SIDES AND THE WELDING CAN BE DONE ON THE TWO REMAINING ADJACENT SIDES.
- 3) ON INSTALLATION MADE PRIOR TO 6-22-79 A WELD SIZE OF 3/16" IS ACCEPTABLE.



PED-H-  
W949

GRAPH 1 INSTALLATION TOLERANCES AND APPLICATIONS PER DETAIL 12  
 INSTALLATION TOLERANCES IN (INCHES)



PED-H-W949

INSTALLATION TOLERANCES IN (INCHES)  
 TABLE # 12 VALUES

A/E APPROVAL REQUIRED  
 $(LM > LO + \Delta)$

INSTALLATION TOLERANCE  $(LM \leq LO + \Delta)$   
 INSTALLATION TOLERANCE  $(LM \leq LO + 5")$

AS PER DETAIL 12 & 14  
 $(LM \geq LO - 5")$

A/E APPROVAL REQUIRED  
 $(LM < LO - 5")$

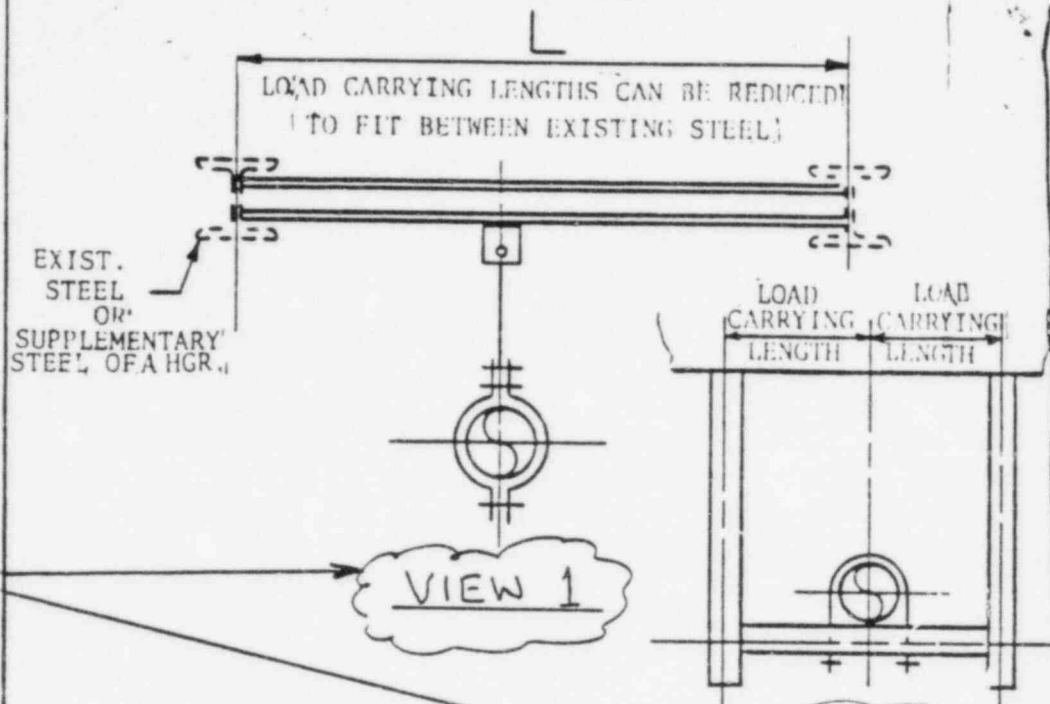
LEGEND  
 LO: ORIGINAL DESIGN DIMENSION  
 LM: FIELD MEASURED DIMENSION

REF DOC PCN	RFI	WPPSS NUCLEAR PROJECT NO. 2	
REF SPEC SECTION	PAGE	PARA	BURNS AND ROE, INC.
REF DWG	H-501 SH1 1	DWG ZONE	PED 215-H-W949 SH1 12 OF 17
SCALE	DRAWN BY	DATE	TITLE
	WTH	7-29-83	H-501
	CHKD BY	DATE	
		8/1/83	

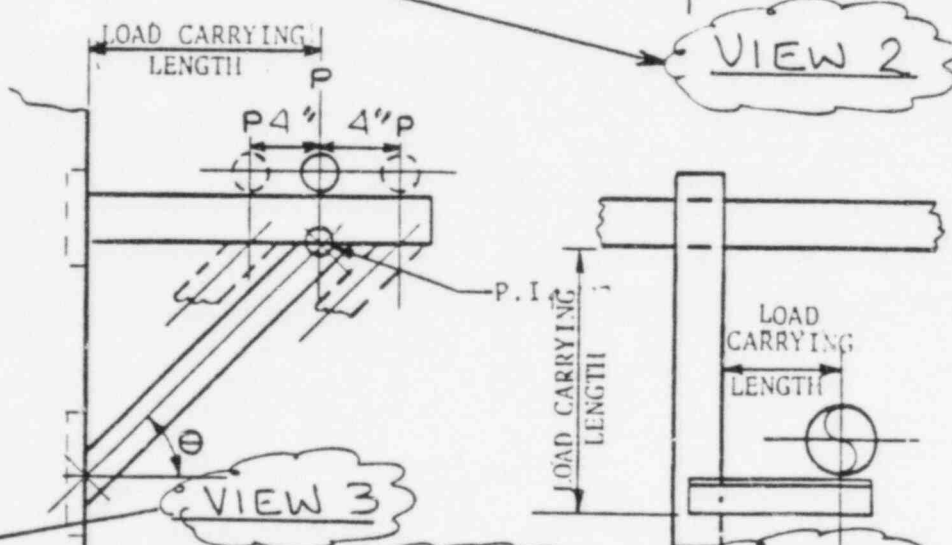
PED-H-  
W949

DETAIL # 14. **CHANGE IN LOAD CARRYING LENGTHS**

NOTE 1) ALLOWABLE MAX. REDUCTION = 5", IF OVER 5" APPROVAL BY A/E REQUIRED.



PED-H-  
W949



PED-H-  
W949

NOTE FOR VIEW 3: CHG = ±4" AS SHOWN

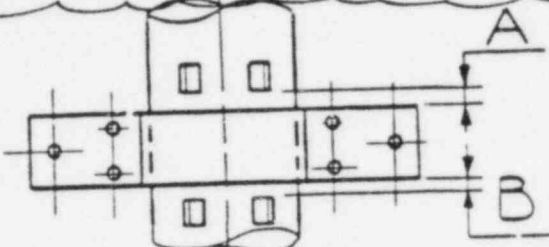
THE RELATIVE POSITION OF THE POINTS OF INTERSECTION (P.I.) MUST REMAIN THE SAME.  
ANGLE  $\theta$  MUST REMAIN WITHIN  $\pm 3^\circ$

REF DOC PCN	RFI	WPPSS NUCLEAR PROJECT NO. 2
REF SPEC SECTION	PAGE	BURNS AND ROE, INC.
REF DWG H-501 SHT 1	DWG ZONE D13	PED 215-H-W949 SHT 13 OF 13
SCALE	DRAWN BY KFB	TITLE H-501
	CHKD BY WTM	
	DATE 7-8-53	
	DATE 7-29-53	
	DATE 8/1/53	



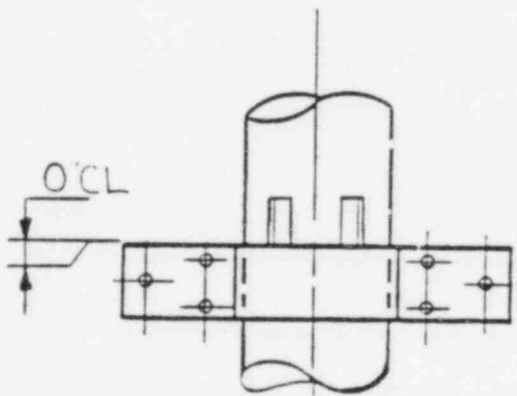
DETAIL #15

NOTE A) FULL CONTACT IS REQUIRED BETWEEN LUGS AND DEAD WEIGHT BEARING SURFACES OF CLAMPS OR STRUCTURES. HOWEVER WHEN MULTIPLE LUGS ARE USED ON ONE SIDE OF THE CLAMP OR STRUCTURE ONE LUG IN FULL CONTACT IS ACCEPTABLE PROVIDED THAT THE REMAINING LUGS ON THE SAME SIDE ARE INSTALLED WITHIN 0.016" OF THE CLAMP OR LOAD BEARING SURFACE.



B) THE GAP BETWEEN LUGS AND BEARING SURFACES SHALL BE SHIMMED AS NEEDED TO MEET CLEARANCES OR FULL CONTACT REQUIREMENTS. CASES WHERE ~~THE~~ REQUIRED A OR B DIMENSION CANNOT BE MET BY SHIMMING SHALL BE REFERRED TO A/E ENGINEERING FOR RESOLUTION. REFER TO GENERAL NOTE 13 FOR SHIMMING.

PED-H-W949



C) ON RISERS WHERE THE LUGS AND CLAMP SUPPORT THERMAL LOAD, DIMENSION "A" = 0 FOR MOVEMENT TOWARD B; DIMENSION "B" = 0 FOR MOVEMENT TOWARD A.

D) THE TOLERANCES SHOWN IN THIS DETAIL APPLY TO ALL AXIALLY LOADED PIPE (INCLUDING HORIZONTALLY LOADED RISER CLAMPS.)

E) SEE GENERAL NOTE 7.

REF DOC PCN	RFI	WPPSS NUCLEAR PROJECT NO. 2	
REF SPEC SECTION	PAGE	PARA	BURNS AND ROE, INC.
REF DWG	H-501 SHT 1	DWG ZONE	PED 215-H-W949 SHT 14 OF 17
SCALE	DRAWN BY <i>kkh</i> DATE 7-8-83	B14	TITLE H-501
	CHKD BY <i>WTM</i> DATE 7-27-83	APPR BY <i>[Signature]</i> DATE 8/1/83	

PED-H-  
W949

TABLE 15, TOLERANCES FOR RISER CLAMP & LUGS

TYPE OF SUPPORT	ITEM	DESIGN CLEAR.	SHOP TOLERANCE	ERECTION TOLERANCE	APPLICABLE NOTES
CASE WHERE DEAD WEIGHT LOAD IS SUPPORTED	A	0	N/A	N/A	A, B D
	B	1/16	N/A	+ 1/16 - 1/32	
CASE WHERE DEAD WEIGHT LOAD IS NOT SUPPORTED	A	1/16	N/A		B C, D
	B	1/16	N/A		
	A+B	1/8	N/A	+0 -1/8	

PED-H-  
W949

REF DOC PCN	RFI	WPPSS NUCLEAR PROJECT NO. 2	
REF SPEC SECTION	PAGE	PARA	BURNS AND ROE, INC.
REF DWG H-501 SHT 1	DWG ZONE	PED 215-H-W949 SHT 15 OF 17	
SCALE	DRAWN BY <i>W. M. T. M.</i> DATE 7-8-83	B 12	TITLE H-501
	CHKD BY WTM DATE 7-29-83	APPROVED <i>[Signature]</i> DATE 8/1/83	

DETAIL 19 SKEWED WELDING

SKewed WELDS WITH INCLUDED ANGLES BETWEEN  $60^{\circ}$  and  $135^{\circ}$  MAY BE CALLED OUT AS FILLET WELDS, WITH SPECIFIED EFFECTIVE THROAT.

A/E USES AWS D1.1 TO DETERMINE EQUIVALENT LEG LENGTHS. (CONDENSED TABLE SHOWN BELOW)

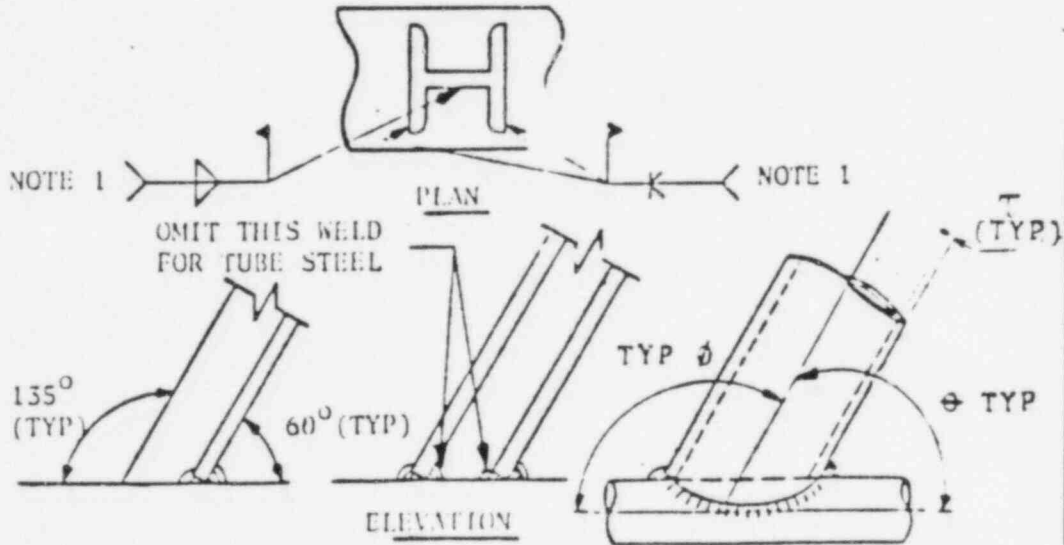
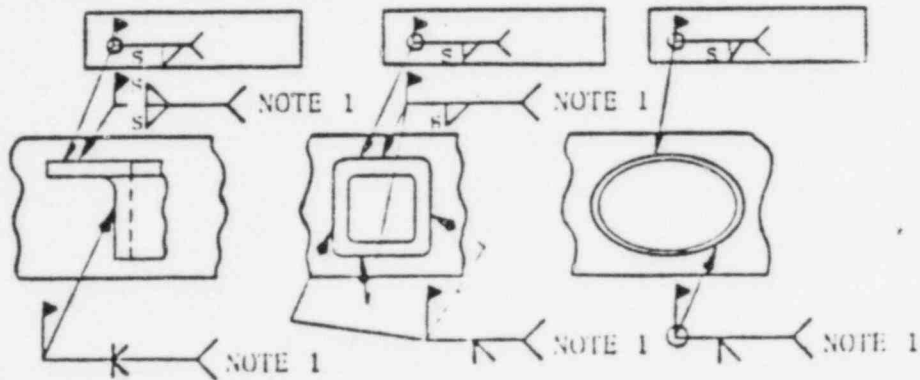
FOR ALL OTHER ANGLES  $<60^{\circ}$  or  $>135^{\circ}$  A GROOVE WELD SHOULD BE MADE AS SHOWN IN EXAMPLES BELOW. THESE GROOVE WELDS MUST HAVE, AS A MINIMUM, THE EFFECTIVE THROAT REQUIRED FOR FILLET WELDS CALLED OUT ON HGR. DETAIL DRAWING.

MIN SLOPE SHOULD BE  $30^{\circ}$  (IF THIS ANGLE IS LESS THAN  $30^{\circ}$  ON AN EXISTING DESIGN SUBMIT TO A/E FOR REVIEW)

PED-H-WS45

NOTE:

- (1) FILLET AND GROOVE WELDS TO BE BLENDED AT A CONVENIENT POINT ON THE RADIUS.



EFFECTIVE THROATS OF FILLET WELDS IN SKEWED T-JOINTS

REF DOC PCN	RFI	WPPSS NUCLEAR PROJECT NO. 2	
REF SPEC SECTION	PAGE	BURNS AND ROE, INC.	
REF DWG	DWG ZONE	PED 215-H-W949 SHT 16 OF 17	
SCALE	DATE	TITLE	
DRAWN BY: <i>W. R. ...</i>	DATE: 7-9-83	H-501	
CHKD BY: WTM	DATE: 7-29-83	DATE: 9/1/83	

GENERAL NOTES;

(1) JURISDICTIONAL BOUNDARIES

Jurisdictional Boundary Piping Supports

(1) Supporting Structure

The supporting structure consists of steel members attached to building concrete, or to building steel members. The supporting structure is to be considered an extension of the building structure for the purpose of supporting a pipe. All Specifications, Codes, and special requirements for safety related building structures shall apply.

K

(12) OVER-WELDING

Extra welds welded beyond the requirement of design/drawing are acceptable, except the angle clips or angle knees used for simply supported beam designs which shall be reviewed by A/E.

(13) SHIMMING

Contractor is authorized to use shims to attain proper clearances as indicated in detail # 25.

PED-H-W949

Where deadweight is to be supported the required metal-to-metal contact should be complete. However the contact within the zone requirements specified in Detail # 25 is permitted provided that 0.000" replaces 0.016" for a "NO-GO" case.

D

(14)

Detail #8 depicts minimum clearance dimensions between pipe support components and ISI welds and is applicable to all supports unless otherwise specified on the individual pipe support detail. Where the pipe support detail specifies or implies a locating dimension for pipe support components with respect to ISI welds that is less than the minimum specified by Detail #8, then that dimension shall be considered a minimum.

C

REF DOC. PCN	RFI	WPPSS NUCLEAR PROJECT NO. 2
REF SPEC SECTION:	PAGE: PARA:	BURNS AND ROE, INC.
REF DWG. H-501 SHT. 1	DWG. ZONE D-1	PED 215-H-W949 SHT. 17 OF 17
SCALE	DRAWN BY: <i>KRBudha</i> DATE: 7-27-83	TITLE H-501
	CHKD BY: <i>WTH</i> DATE: 7-29-83	
	APP: <i>[Signature]</i> DATE: 8/1/83	

# Washington Public Power Supply System

P.O. Box 968 3000 George Washington Way Richland, Washington 99352 (509) 372-5000

August 23 1983  
WPBEC-C0500-F-83-3023

Responds to: N/A  
Response required by: N/A

Mr. J. Newgen  
Construction Manager  
Bechtel Power Corporation  
P.O. Box 600, Caller Service  
Richland, WA 99352

Subject: NUCLEAR PROJECT 2, CONTRACT C0500  
PED TRANSMITTAL  
PED- 215-H-W949

The attached Project Engineering Directive (PED) 215-H-W949 is forwarded for implementation and you are hereby authorized to assign work as required by this PED. Master Work List (MWL) input is required for all physical work to be performed on this PED. A Startup Work Request (SWR) processed in accordance with PMI 9-1.1 is required prior to commencing physical work on provisionally accepted or turned-over system(s).

The following system package(s) affected by this PED have been provisionally accepted or turned-over as of this transmittal; NONE .



G. K. Afflerbach, 927M  
Test and Startup Manager

JOC/1h

BECHTEL POWER CORPORATION  
RICHLAND, WASHINGTON

AUG 23 1983

JOB NO. 1-1-01

Report No.	83-05/05
Paragraph	5
Report Date	3-29-83

Type
Unresolved

For Further Information:	
D. Cosgrove	#4580
H. Boarder	#4589

Responsibility
WPPSS Engineering

ITEM DESCRIPTION

ITEM STATUS

5. Bechtel As-Built Drawing Program

The inspector examined the Bechtel efforts to improve the program for as-built drawing revisions. Weaknesses in this area had been identified in Inspection Report 50-397/83-03 (paragraph 8) and as prior follow-up items 397/80-01-01 and 397/82-24-01.

The Bechtel lead as-built engineer stated that the updating effort would include 2,854 Quality Class I pipe support drawings. This would include detailed field verification of configuration and locations. The governing Bechtel procedure (SWP-P-P-6, Revision 4) excludes Quality Class II, Seismic Category I drawings from this population. This appears to conflict with the WPPSS governing procedure, PMI-6.6, which requires verification of Seismic Category I pipe supports. The WPPSS Project Quality Assurance Manager stated that a proposed revision to the PMI-6.6, to omit Seismic Category I drawings, had already been challenged by the project quality assurance organization. The issue involves the NRC Bulletin 79-14 requirement for as-built verification of Seismic Category I pipe supports. This matter is unresolved. (50-397/83-05-05)

The engineer currently assigned this task displayed a computer printout of the NCR's for each site contractor, and showed evidence of the random selection process which he had used, and his measures to assure proper qualification of personnel performing the technical reviews. He discussed the results which showed that none of the selected 10% sample revealed engineering dispositions which would unacceptably compromise the design. He also described additional reviews of design change documents of various types for the various contractors and reviews of contractors' internal deficiency documents. His files included copies of the various documents selected, and review procedures originally in effect at the time the documents were generated. The sampling process appeared to be reasonable and inclusive of contractors' internal documents. The reverification group is in process of completing a report of this activity. This item is closed.

~~(Class) II/Seismic Category I pipe supports~~ - Exclusion of Quality Class II/Seismic Category I pipe supports from the detailed as-built verification program conducted for Quality Class I supports.

Procedures PMI-6-6 Revision 4 now reflects that final as-built drawings will only be prepared for Quality Class I piping and supports, except where additional scope is defined in each contract specification. Bechtel procedure SWP/P-P-6 Revision 5 describes the Quality Class I as-built program.

The as-built requirements of NRC Bulletin 79-14 are based upon assuring pipe stress analysis accuracy for piping systems having a safety function. In the WNP-2 FSAR such systems were designated Quality Class I/Seismic Category I. The WNP-2 program includes supports for such systems.

There are other piping supports which do not support safety related piping, but whose failures may affect a safety system (i.e. by acting as missiles). In the FSAR these were originally also designated Quality Class I Seismic Category I. It appears that in 1981 the project redefined the criteria for such supports, designating them as Quality Class II/Seismic Category I. For these supports, the degree of deformation prior to collapse, and other functional criteria, are less restrictive than for supports of piping on safety related systems; this is described in the attachment to the July 15, 1983 WPPSS letter to NRC. Accordingly, the rigorous as built verification requirements of Bulletin 79-14 do not apply to the Quality Class II/Seismic Category I supports for purposes of stress analysis of the piping supported by those supports. (However, construction verification of critical portions of such supports may be warranted for purposes other than the specific safety related piping stress considerations of Bulletin 79-14).

This matter is closed with exception of the item opened in paragraph 5.a. (83-38/01)

9. Unresolved Items

Report No.	83-22 (82-18/02)
Paragraph	Appendix A 6d
Report Date	7-5-83

Type
Deviation

For Further Information:
D. Cosgrove Ext 6826
H. Boarder 6840

Responsibility
Supply System Engineering

ITEM DESCRIPTION

ITEM STATUS

See Attachment 1 "Notice of Deviation," Appendix A

(Reclassified) Deviation (50-397/82-18/02) - Jurisdictional  
Boundaries for ASME Section III NF

This item was previously considered closed, as described in NRC Inspection Report 50-397/83-03. The inspector has reconsidered that conclusion after consultation with NRC Construction Appraisal Team members on site this report period.

The Supply System submitted an inspection criteria procedure (QVI-09) to NRC for approval as part of an FSAR amendment regarding compliance with workmanship details of the AWS-D1.1 Welding Code. This document described reverification program criteria to be used for structural steel. In other on-site documentation (e.g., Drawing H-501), the engineer had defined typical boundaries between structural steel (AWS-D1.1) and pipe supports (ASME III NF). The Drawing H-501 Sheet 3 shows various configurations of welded steel shapes upon which piping is supported. These are described as extensions of the building structure and subject to AISC/AWS Codes (and thus QVI-09).

As an example, Drawing H-501 Case VI shows a welded square frame, around a pipe; the pipe rests on the lower member of the frame, which is supported by a vertical tubular steel member (welded to the frame and to an embed plate on the concrete floor). The frame bears on the pressure boundary component, and thus meets the definition of "non-integral support" of ASME Section III NF-1510. Section NF-1515



Report No.	83-22 (82-18/02)
Paragraph	Appendix A; 6d
Report Date	7-5-83

## ITEM DESCRIPTION

## ITEM STATUS

provides the jurisdictional boundary between a building structure and a non-integral support shall be the surface of the building structure; this appears to require design and fabrication of the steel frame and the vertical member in accordance with rules of ASME Section III NF.

The Burns and Roe Resident Group Supervisor for pipe supports stated that all supports had been designed in accordance with rules for ASME Section III NF; he stated that details which H-501 classify as non-NF do not involve any plate and shell structures and thus do not require any nondestructive examination beyond that of the AISC/AWS Code. Traceability requirements appear to be more extensive for ASME NF than AISC/AWS. The departure from the ASME Code appears to be a deviation from FSAR commitments. (Deviation 50-397/82-18-02)

Note: The original NRC Inspection Open Item Status Sheet and the excerpt (para 8e) from NRC Inspection Report 83-03 that previously closed this item are attached for information (Attachment II).

Follow-up (8-30-83) NRC Report 83-38 Paragraph 5.k(2)

Development of QVI-09 and the associated deviations from AWS-D1.1 welding workmanship criteria have been previously addressed by the regional inspectors in NRC inspection reports 50-397/82-18 and 83-03 (NRC open item 397/82-18-02). The matter was closed with the licensee issuance of FSAR amendment number 27 (page 3.8-190), and supporting letters to NRC dated January 17, 1983 (G02-83-007) and March 23, 1983 (G02-83-249), which documented the basis for exceptions to AWS D1.1 criteria.

Report No.	83-22
Paragraph	Appendix A 6d
Report Date	7-5-83

## ITEM DESCRIPTION

The NRC CAT inspectors identified concerns with the implementation of the reduced acceptance criteria for weld quality, as allowed by reverification program procedure QVI-09. The inspectors considered that structural steel at reactor building elevation 444' contained welds with multiple defects, each of which may be acceptable under QVI-09, but as a composite may be "questionable quality". The inspectors noted that such multiple defects may potentially not be acceptable at connections more critical than those observed by the inspectors (if such connections exist). These CAT perceptions have been factored into the current NRC deliberations regarding the FSAR amendment.

## ITEM STATUS

APPENDIX ANOTICE OF DEVIATION

Washington Public Power Supply System  
P. O. Box 968  
Richland, Washington 99352

Docket No. 50-397  
Construction Permit No. CPPR-93

As a result of the inspection conducted on May 1-31, 1983, and in accordance with the NRC Enforcement Policy, 10 CFR Part 2 Appendix C, 47 FR 9887 (March 9, 1982), the following deviation was identified:


Paragraph 3.2.3 of the FSAR states that piping system supports shall be appropriate for the components supported as defined by the ASME Code Section III. The ASME Code Section III NF-1510(d) defines a non-integral pipe support as one which "bears on the pressure boundary component" and NF-1511 states that "the jurisdictional boundary between a building structure and a non-integral support shall be the surface of the building structure."

Contrary to the above, on April 9, 1982, the architect engineer issued drawing H-501, sheet three, which allowed non-integral supports to be excluded from the construction and inspection requirements of ASME Section III, subsection NF even though the non-integral supports extend beyond the building surface structure into the jurisdictional boundary of the ASME Code.

You are hereby requested to submit to this office within thirty days of the date of this Notice, a written statement or explanation regarding the item of deviation, describing corrective action taken, the results achieved (or corrective steps that are planned), and the date when corrective action will be completed.

JUL 5 1983

          
Dated

  
          
R. T. Dodds, Chief  
Reactor Projects Section 1

Report No.	82-18/02
Paragraph	4
Report Date	9-28-82

Type  
Concern

For Further Information:  
D. Cosgrove Ext 4580

Responsibility  
SUPPLY SYSTEM QUALITY  
VERIFICATION PROGRAM  
MANAGER

ITEM DESCRIPTION

ITEM STATUS

Reverification Program

In response to the June 17 NRC inquiry under 10CFR50.54(f), the Supply System, Bechtel, and site contractors have engaged in a reverification program which includes review of records and reinspections of hardware installed prior to July 1980. The Supply System described the policies for conduct of reviews and reinspections in the WPPSS July 17, 1980, reply to the NRC inquiry. One of the policies read "This program will have priority over ongoing work. The project construction work pace will be adjusted accordingly." Another Supply System policy included integration of the reverification effort into the general project completion activities. However, associated with this integration has been a drain of personnel from the reverification effort, and a postponement of reverification activities to support the recent reactor vessel hydrotest. The reverification staff has been reporting this status to the Supply System Management in weekly progress reports. Following the Management meeting, the WPPSS Director of Licensing and Quality stated that this was his first notification of staffing problems with the reverification group, and he indicated that additional support in this area may be forthcoming.

At this time the Supply System appears to be prioritizing the reverification work to support construction completion schedules. A special reverification report was issued for the reactor pressure vessel hydro boundary. This documents the reviews and reinspection performed, the sampling basis, the results, and the evaluation of results. For the 12 systems planned for inclusion in the hydrotest, there were 80 small bore and 90 large bore piping isometric drawings involved. The reverification review and inspection included

As of 10-18-82

The PQA organization has reestablished the priority for completing prepurchase and inactive (PP&IA) reviews. The remaining work has been rescheduled for completion in the fourth quarter of 1982. The assigned staff has been increased to meet this schedule. All other QVP work activity is either complete or on schedule.  
(Re: WPBEC-C0500-F-82-2237)

Programmatic action is in place in the form of FSAR Change Notice #SCN-82-165 for the concern regarding visual weld examination criteria as defined in QVI-09. Resolution of this concern is dependent upon NRC review of the FSAR amendment to be generated from this change notice. No additional site action is required at this time.

Report No.	82-18/02
Paragraph	4
Report Date	9-28-82

14 of the large bore and 6 of the small bore piping drawings. The reverification effort identified only minor discrepancies, none of which appeared to warrant further inspection or increased sample size.

The Supply System has implemented a procedure QVI-09 (Special Structural Steel Reinspection Criteria). This document allows deviations from AWS-D.1 welding visual examination criteria for items within its definition of structural steel. The Supply System stated that an amendment to the final safety analysis report (FSAR) is in process to define the AWS deviation. (WPPSS in-house change notice SCN-82-165, dated August 9-27, 1982). The notice defines structural steel as including radial and structural framing systems, steam tunnel beams and pipe hangers. The procedure QVI-09 also mentions ductwork, stiffeners, cable trays, brackets, and similar components. The Supply System verbally advised the inspector that the pipe support and hanger portions were limited to those parts excluded from ASME Section NF jurisdiction. It is not clear that WPPSS defined exclusions are consistent with the ASME definitions. This matter will be re-examined following NRC review of the amendment. (397/82-18-02)

-17-

This item was encompassed by the WBG/Bechtel procurement and installation documentation review activities and procedures which were conducted November 1981 through September 1982. The procedures WP-787 and WP-782 were apparently revised to reflect consistent requirements for documentation review prior to material release.

This matter is closed.

d. (Closed) Follow-up Item (397/82-12-03) Incorrect Heat Number on Steel Plate

The Bechtel fabrication shop personnel identified a 5/8-inch steel plate which was marked with a heat number associated with a shipment of 1/2-inch thick material.

Bechtel documented this matter on a nonconformance report number 781, which was resolved in September 14, 1982 by return of the material to the vendor.

The identification of this issue at the fabrication shop indicated a multiple breakdown of quality controls with respect to: (1) material issuance from the site storage yard, (2) material receipt inspection at the site storage yard, and (3) the supplier's shipping inspections.

Bechtel has not identified the cause of the wrong identification number, although it appears to have originated at the supplier. The Bechtel purchasing personnel stated that this matter has not been incorporated into the Bechtel audit activities for the vendor involved and that there is no provision for routine feedback of such discrepancies for inclusion into the vendor audit program. (However, the site quality assurance department performs routine trending of nonconformance reports, and a significant item or trend data may be forwarded to the vendor auditing group from this source.)

The Bechtel quality assurance department addressed this matter as an isolated event and indicated that no further action was planned.

No items of noncompliance were identified.

S. e. ~~397/82-12-03~~ Reverification Program Deviations from AWS-D1.1

The quality verification program "Special Structural Steel Reinspection Criteria" procedure QVI-09 allowed deviations from the AWS-D1.1 welding Code, for structural steel and pipe support sections not covered by ASME Section NF. The licensee planned an amendment to the safety analysis report (FSAR) to define the deviations. It was not apparent that the ASME Section NF boundaries had been properly defined for use with the QVI-09 procedure.

-18-

The Engineer (Burns and Roe) had issued drawing H-501, Sheet 3, to provide field guidance for determining ASME Section NF Code jurisdiction boundaries. The defined boundaries appear consistent with ASME Section NF (Part NF-1510).

The inspector reviewed a draft Amendment 27 of the FSAR Table 3.8-9 which described the deviations to be used for accepting completed work. Not all of the deviations were identified. The licensee subsequently clarified the amendment and submitted it to NRC via letter dated January 7, 1983.

Procedure QVI-09 includes an Attachment I which defines the justification for the deviations listed in Attachment II. Attachment II had been approved by the Engineer. However, the Bechtel originated Attachment I had not been endorsed by the Engineer and did not accurately represent the basis for the Engineer's approval. The licensee representative stated that the Engineer has been requested to clearly document the basis for approval. The inspector examined the approval routing sheets for the other QVI series of procedures, and determined that engineering, quality assurance, and test and startup organizations had concurred.

The licensee's proposed departures from the AWS-D1.1 Code appear reasonable, and have been documented in the FSAR. Further licensee action on this matter may be prescribed by the NRC licensing organization, if required.

This matter is closed.

f. (Closed) Unresolved Item (397/82-27-05) - Inspection criteria for scrubber and swav-strut bracket pin connections

Appropriate acceptance criteria had not been provided to the field inspectors to assure end connection configurations which would prevent unacceptable disengagement of self-aligning bearings.

The licensee has now filed a 50.55(e) report with NRC dated December 20, 1982. Burns and Roe engineering direction has been amended by PED-215-H-G758. The licensee had issued direction to Bechtel (December 10, 1982) and to the startup organization (December 9, 1982) to perform reinspections and Bechtel revised the hanger balancing procedure SWP-P-P-12 (revision 2) to require end connection inspections.

The inspector examined these documents and noted that the Bechtel revision allows disengagement of up to 1/2 thickness of the end paddle, whereas the Engineer's direction limits it to 1/3-thickness, as described in the report to NRC. On January 7, 1983, the licensee and Bechtel representatives affirmed that the procedure has not yet been used to accept any hardware, and that a different procedure may be applied.

AS-BUILDING - QUALITY CLASS I LARGE BORE AS-BUILT PROGRAM EVALUATIONBACKGROUND

The NRC Construction Appraisal Team (CAT) review of the WNP-2 as-built program resulted in several findings against the program. One of the areas of concern was that there were too many as-buiting errors (mismeasurements, incorrect weld symbols, and missing data). Because of the NRC CAT findings, the Supply System agreed to extend the sample of hangers reviewed by 72 hangers and report the results to the NRC.

EVALUATION

The results of the as-built reverification for both the NRC sample and the extended sample were evaluated using the criteria shown in Attachment 1. Where it was necessary to confirm the evaluation using the actual design calculation, the A/E made the confirmatory review. Each deviation was evaluated first on a case-by-case basis (see Attachment 2). The deviations were then categorized. The summary of the categorization is shown in Attachment 3. Then, based on the frequency and significance, the deviations were evaluated on a generic basis (Attachment 4). Items of high frequency but low potential design impact are considered acceptable based on a high degree of confidence that the design would not be impacted.



## CONCLUSIONS

None of the errors or omissions identified by the NRC CAT and the Project QA Organization in the extended sample effect the design, safety, or function of the supports reviewed with the exception of a potential thermal interference which is not an error of the as-built program and would have been subsequently identified in the hot pipe interference walkdown. The total number of findings against a sample in excess of 100 hangers is not significant considering that approximately 2500 welds and thousands of measurements were reviewed. Some of the items noted by the NRC CAT and the project extended sample are not part of the as-built program, but are important to the operability of the hangers. All of these items are covered in other existing inspection programs that have not yet been completed for the hangers at issue. There were no areas identified in either the NRC CAT or the extended samples that would require retrofitting the in-situ portion of the as-built program. The as-built review portion of the program is being 100% rereviewed.

ATTACHMENT 1

EVALUATION

CRITERIA

FOR

PIPE SUPPORTS

AS-BUILT DEVIATIONS

By: Bechtel Power Corporation  
San Francisco Power Division  
Plant Design Engineering Staff

RECOMMENDED ACCEPTANCE  
CRITERIA FOR AS-BUILDING

The criteria provided in the following sections shall be used for as-building of pipe support location on the isometric and pipe support drawing. This document shall be used in conjunction with As-Built Program Procedure SWP/P-P-6.

1. Pipe Support Location:

Measured dimension shall be within  $\pm 2"$  or  $1/2$  nominal pipe diameter, whichever is less restrictive. For details, refer to Table 1.

TABLE 1

TOLERANCES ON DIMENSIONS FOR PIPING	
CHARACTERISTICS MEASURED	AS-BUILT MEASUREMENT TOLERANCE
Pipe Routing Dimensions	$\pm 2''$ or $\frac{\text{Pipe Dia}}{2}$ whichever is less restrictive.
Support Locations	$\pm 2''$ or $\frac{\text{pipe dia}}{2}$ whichever is less restrictive.
Penetration Clearances	$\pm 1/8''$ (DIM $\leq 2''$ ) $\pm 1/4''$ (DIM $> 2''$ )
Valve Stem Orientation	+ 6° (Power Operated valves in other than vertical pipe runs) + 15° (Others)
Elevations And Dimensions From Pipe To Building Column Lines	Estimate

2.0 PIPE SUPPORT DETAIL:

2.1 Hanger Assembly and Structural Dimensions:

Hanger assembly and structural dimensions shown in the pipe support detail drawing are categorized in Table 3 and associated tolerances for as-building are shown. Figures 1 to 10 illustrate these dimensions for some typical pipe support details. Any combination of Figure 1 to 10 shall apply to one pipe support detail. Table 2 shall apply when the dimension in the question does not fall in one of the categories shown in Figures 1 to 10.

TABLE 2

ITEM	TOLERANCE
Dimension $\leq$ 5"	$\pm$ 10% of Dimensions
Dimension $>$ 5"	Greater of $\pm$ 1/2" or 4% of Dimension
Angular Dimension	$\pm 3^{\circ}$

TABLE 3

HANGER ASSEMBLY AND STRUCTURAL MEMBER  
DIMENSIONAL TOLERANCES

DIMENSION	TOLERANCE	INSPECTION CATEGORY (See Note 1)	REMARKS
$C_3, C_4$ ( $\leq 5''$ )	$\pm 10\%$ of c dimension	2	
$C_3, C_4$ ( $> 5''$ )	Greater of $+ 1/2''$ and $\pm 4\%$ of c dimension	2	
$C_{min}$ (Rod and Spring hangers)	$+ 12''$ $- 3''$	2	
$C_{min}$ (Snubber and Strut Assemblies)	Not to exceed $PP_{max}$ and not be less than $PP_{min}$ . Where $PP_{max}$ is maximum and $PP_{min}$ is minimum dimension permitted by vendor.	2	Snubber cold setting shall be within $+ 1/8''$ of specified on the draw- ing.
Length of Spring can, $L, L_1, L_2,$ $Q_1, Q_2$	No measurement required.	1	
D & E	Visual Inspection $D \geq E$	1	If condition not met it should be so noted.

- NOTES: 1. For inspection Category #1, the dimensions are not required to be measured and recorded. For inspection Category #2, the dimensions are required to be measured and recorded.

## 2.2 Bill of Materials

2.2.1 Verify that items listed in the Bill of Materials are installed.

2.2.2 Structural shape shall be at least the size specified in the Bill of Materials.

2.2.3 In case of conflict of length dimensions of members between Bill of Materials and drawing, dimensions shown on drawing governs and dimensional requirement of paragraph 2.1 governs.

## 2.3 Clearances:

1) One Side Clearance: Applies when design drawing shows  $1/16$ " gap on only one side of pipe such as gravity support and vertical restraint and other-side (lower) does not have any clearance. If measured clearance is within the range  $1/32$ " to  $1/8$ " (inclusive), it shall be noted as acceptable. The clearance outside this range shall be recorded within  $\pm 1/32$ " tolerance.

2) Two Side Clearance: Applies when the design drawing shows  $1/16$ " gap on both sides of the pipe. If the total clearance is within the range of  $1/16$ " to  $5/32$ " (inclusive), it shall be noted as acceptable. The total clearance outside this range shall be recorded within  $\pm 1/16$ " tolerance.

## 2.4 Weld Size:

The as-building tolerance for weld size is + unlimited and - 0". Excessive and additional welds than specified need not be recorded. The undersize weld shall be recorded by the associated length within  $\pm 10\%$  tolerance.

- 2.5 Threaded components shall be inspected for properly locked, adequately engaged and torqued where required. Threaded engagement shall be considered adequate when the male end breaks the outside plane of the nut.
- 2.6 Spring Hangers, Snubbers and Strut Assemblies
- 2.6.1 Sizes and types shall be checked.
- 2.6.2 Cold setting of the snubber shall be within  $\pm 1/8"$ . The cold load setting shall be within one load division spacing (load setting  $\pm 1/2"$  load division). Marking of the setting should be verified.
- 2.6.3 End connection clearances for snubbers and struts shall be verified.
- 2.7 Anchor Bolt Verification shall be within the tolerances specified in Table 4.

TABLE 4

ITEM	TOLERANCE
Eccentricity	$\pm 1/4"$
Center to center dimension	$\pm 1/4"$
Edge distance (to edge of steel and to edge of concrete)	$\pm 1/8"$
Size (diameter)	+ $1/8"$ - 0"
Thickness of washer	+ $1/8"$ - 0"

Also anchor bolt type, size and orientation of reinforcing washers at anchor bolt shall be verified.



2.8 PLATE:

The width and length of the plate shall be measured within  $\pm 1/4"$ . the thickness of the plate shall be measured within  $+ 1/4" - 0"$ .

2.9 REFERENCE DIMENSION:

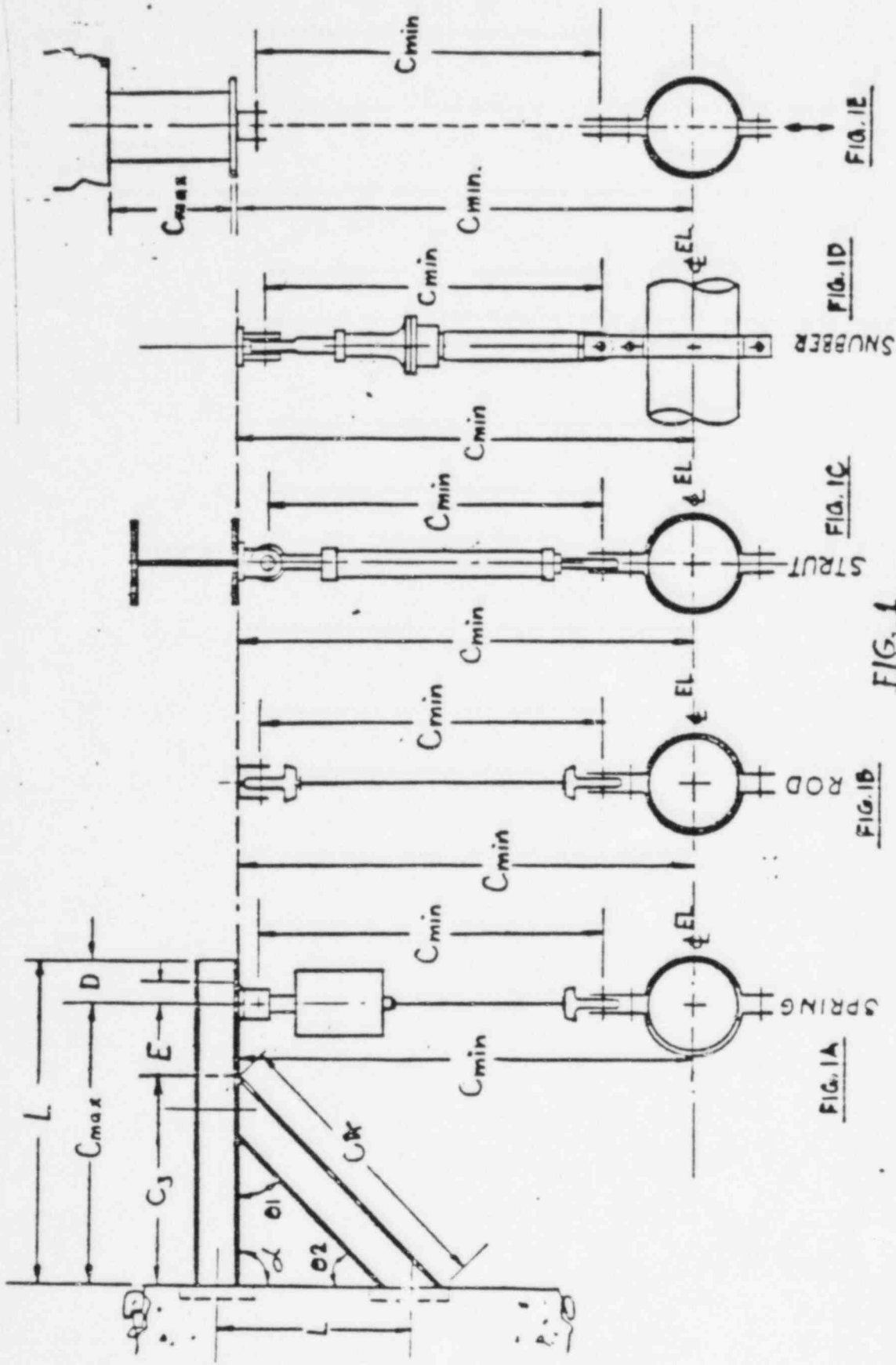
1. The following dimensions are included in pipe support details for reference only. They do not require "as-built" verification and may not be identified as reference.

- a) T.O.S. elevation
- b) B.O.S. elevation
- c) Pipe elevation
- d) B.O.C., T.O.C. or floor elevation
- e) Dimensions from column lines
- f) Azimuths and radial dimensions
- g) Lug elevation
- h) Orientation with respect to north
- i) Location plan

2. The size of existing members need not be verified and are for reference only.

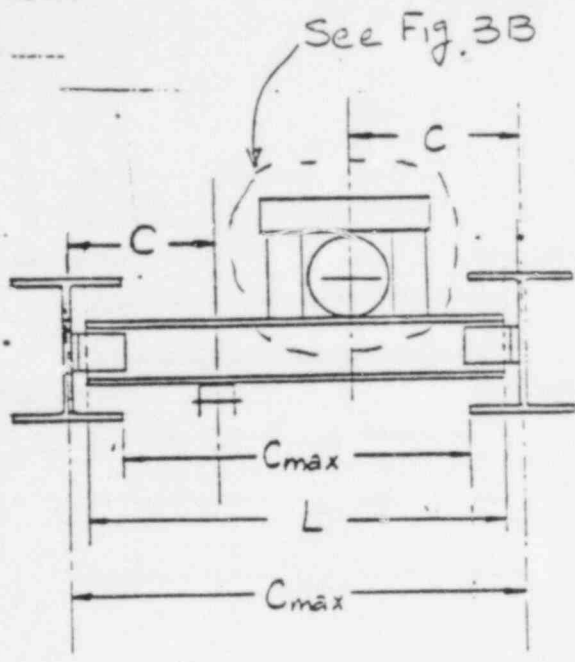
2.10 Shim plates sizes and numbers of shims required to meet specified clearances need not be checked against Bill of Materials.

2.11 The structural orientation recorded within the guide line of Detail #13 of H501, sheet 3 shall be considered acceptable.

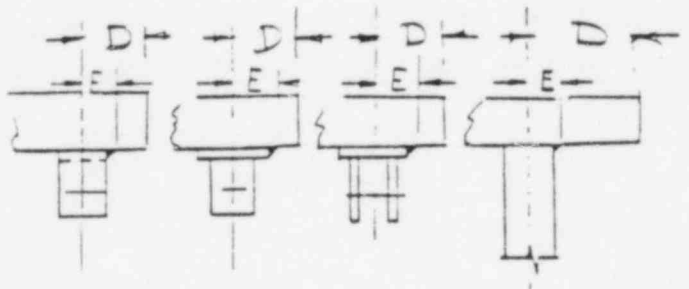
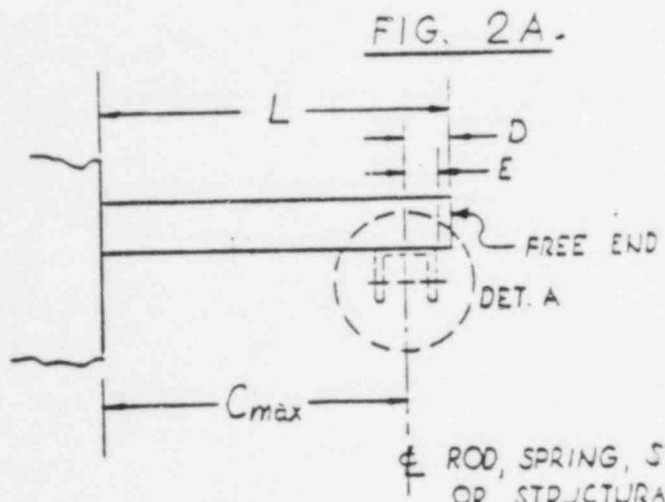


**FIG. 1**

**NOTE:**  $C_{min}$  can be either of the dimensions shown. Only one needs to be verified



NOTE:  
 C<sub>max</sub> can be any of the two dimensions shown. Only one needs to be verified.



DET. A  
 OTHER APPLICABLE CONNECTION TO FIG. 2B & 2C

⌀ ROD, SPRING, STRUT, SNUBBER, OR STRUCTURAL MEMBER

FIG 2B

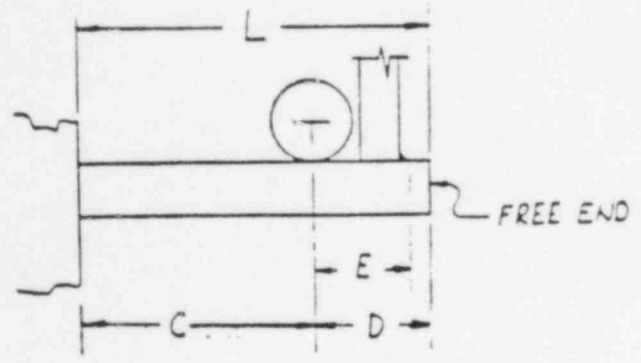


FIG 2C

FIG 2

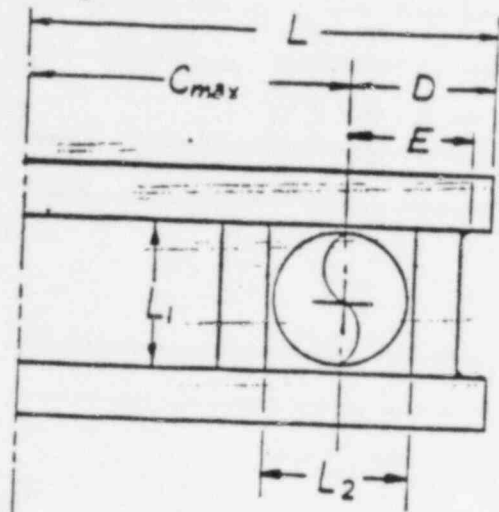


FIG. 3A

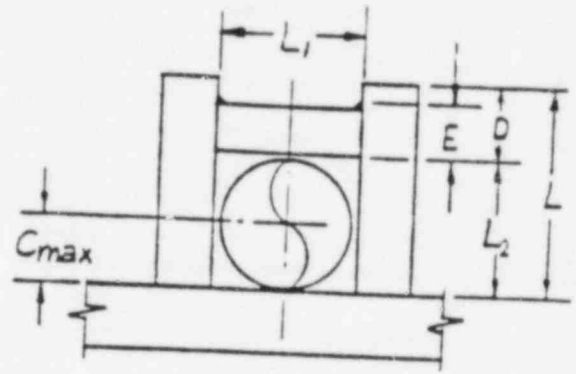


FIG. 3B

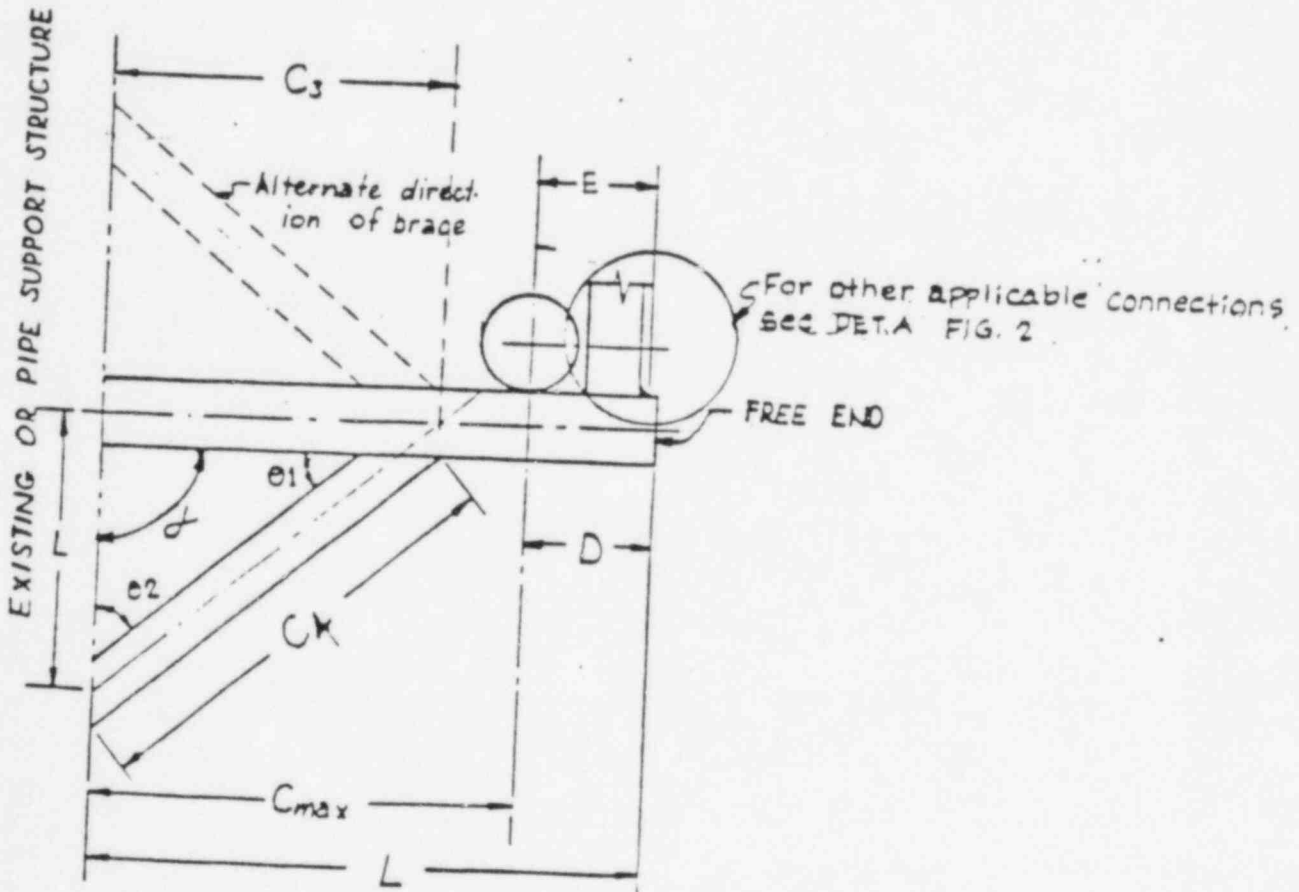


FIG. 3C

FIG. 3

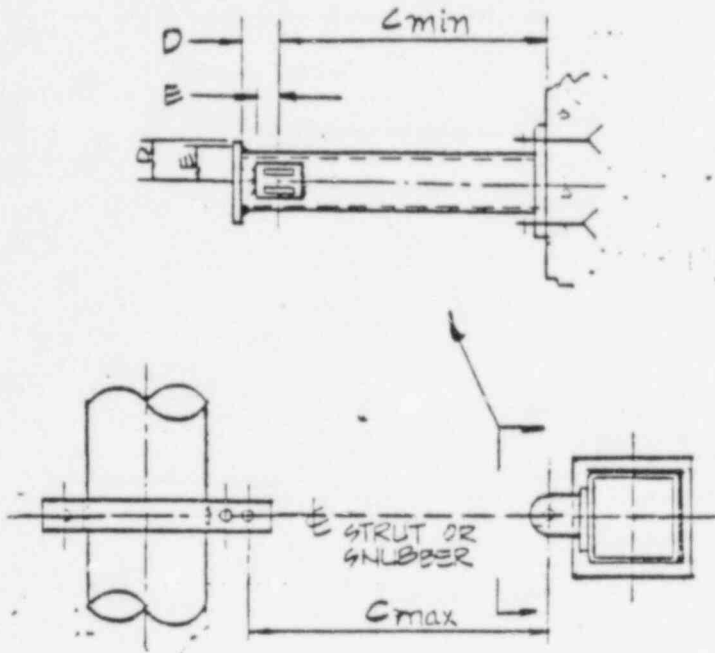


FIG- 4A

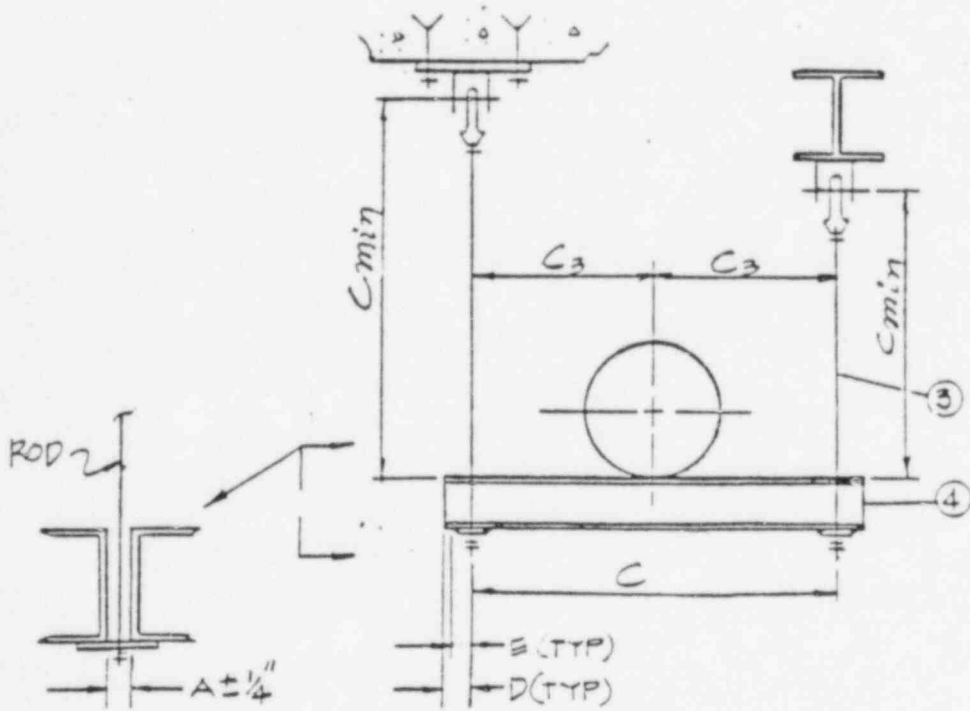


FIG 4B

Fig. 4

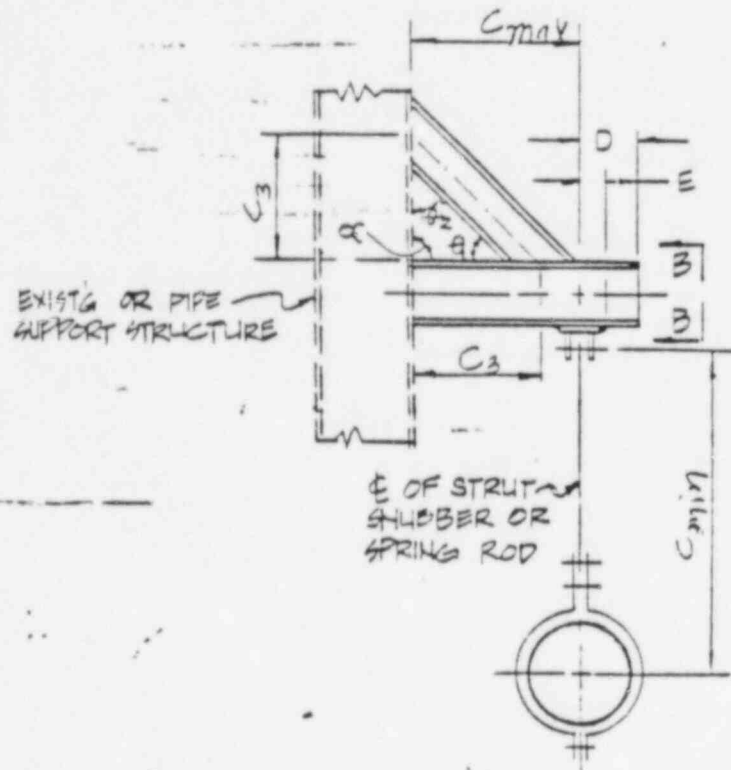


FIG-5A

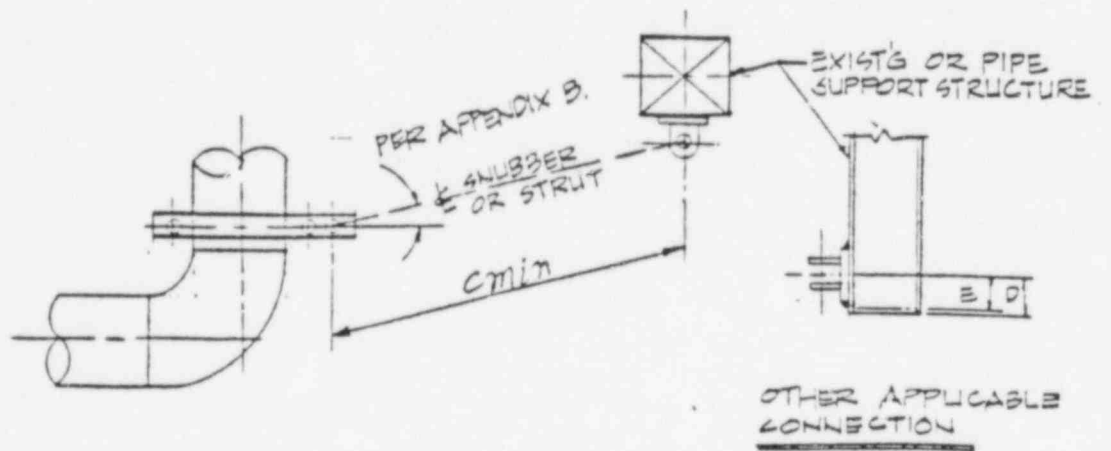


FIG. 5B

Fig. 5

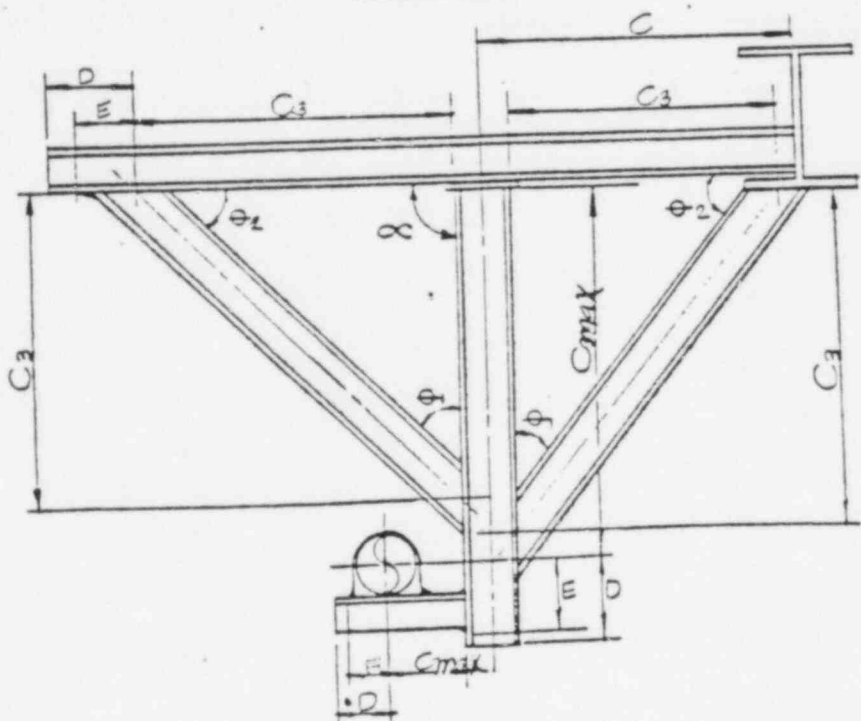
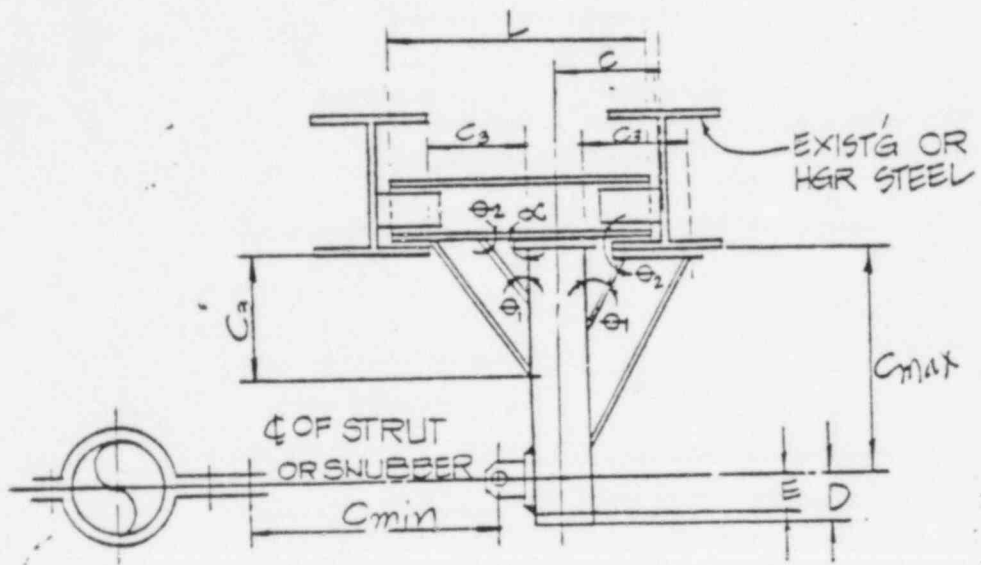


Fig. 6

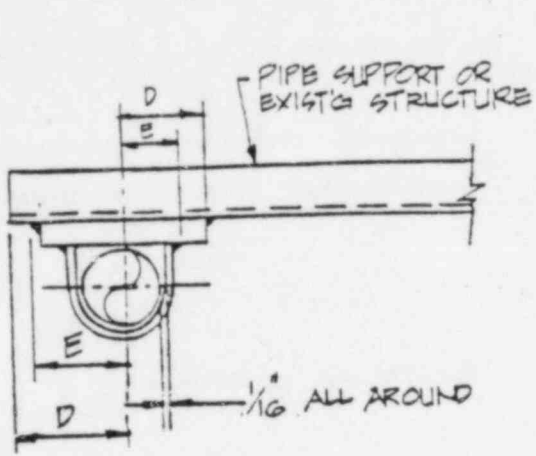


FIG. 7A

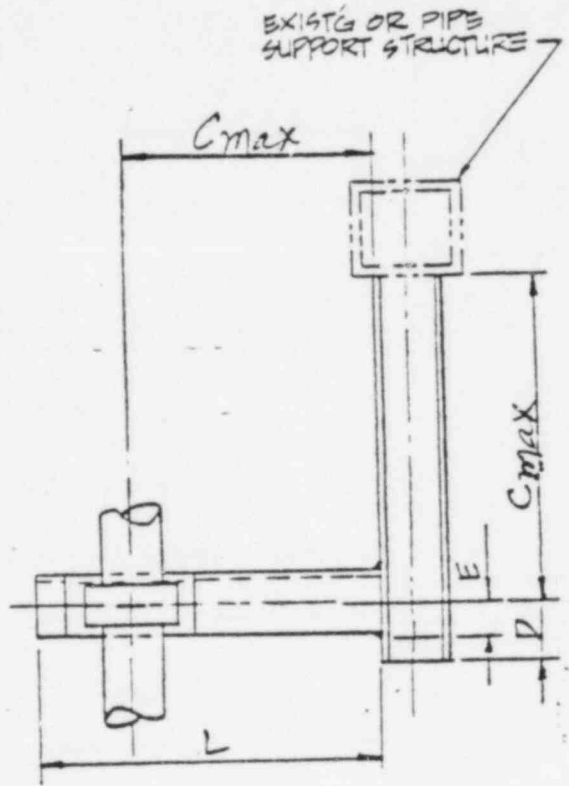


FIG. 7B

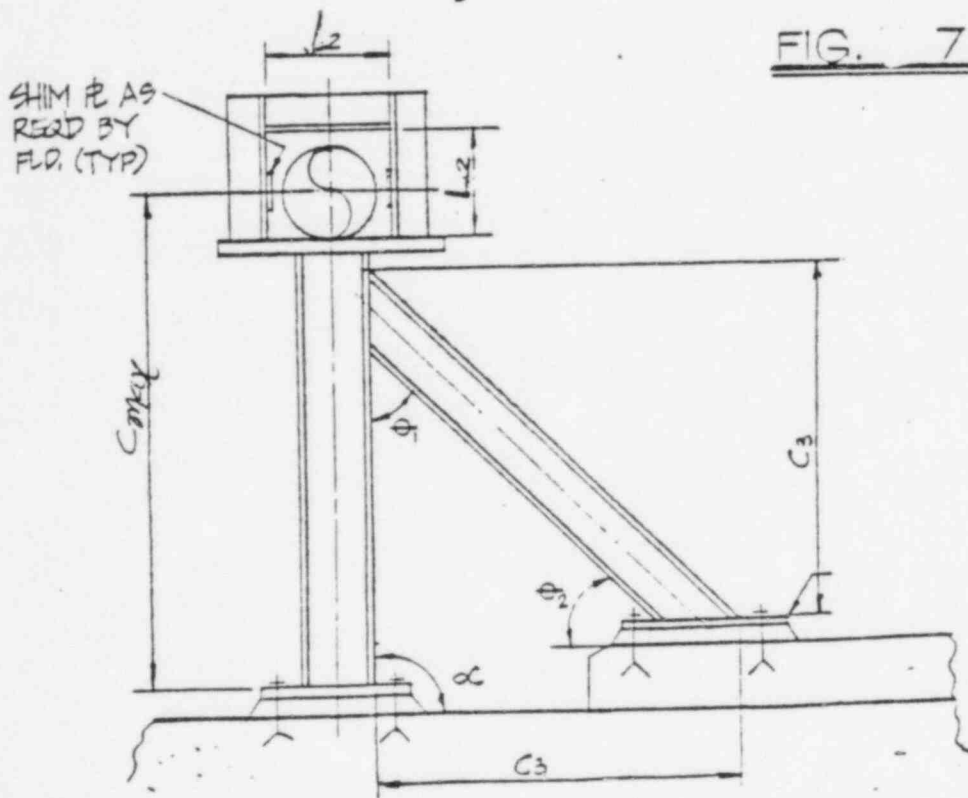


FIG. 7C

Fig. 7



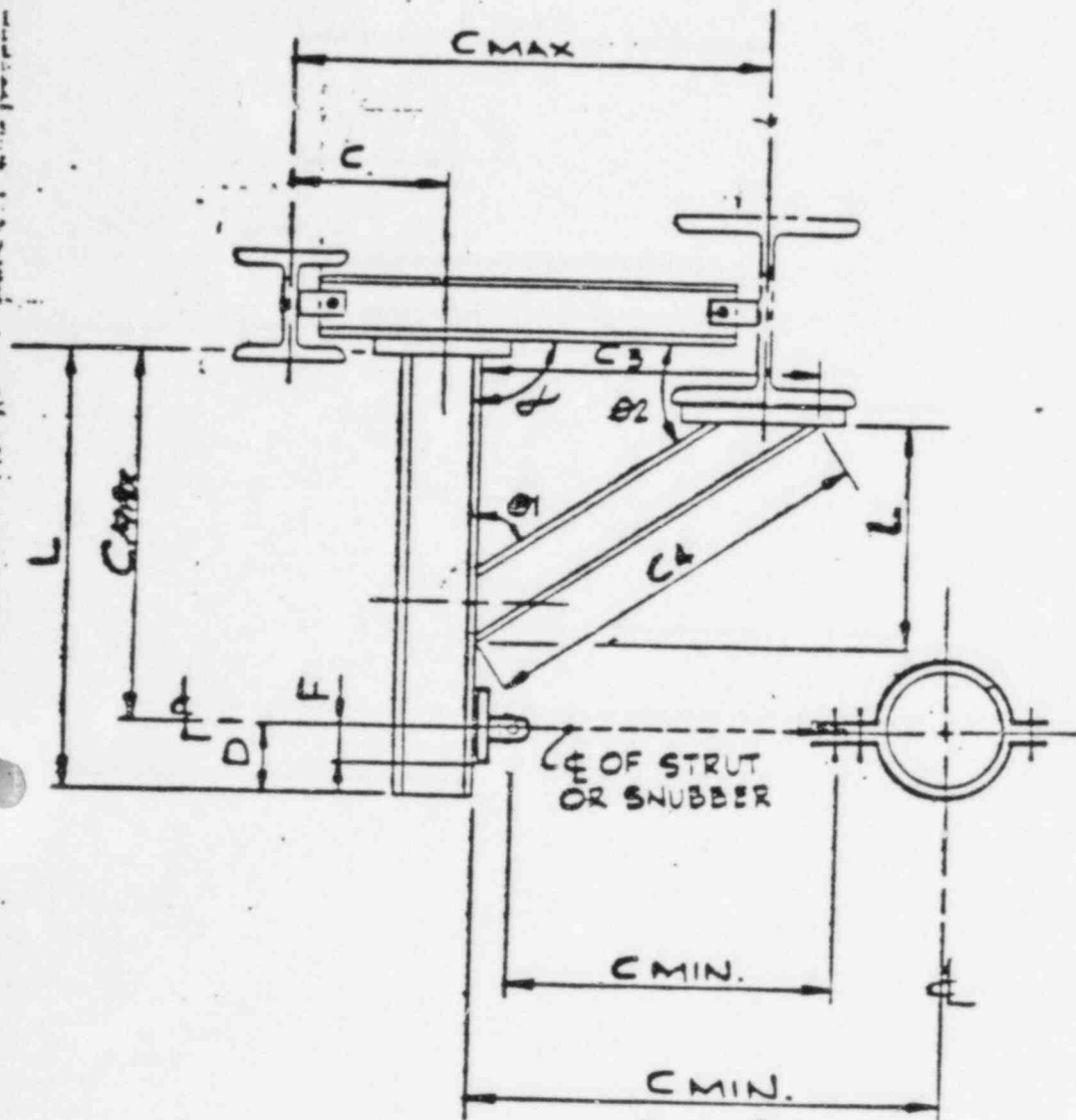


FIG. 8A

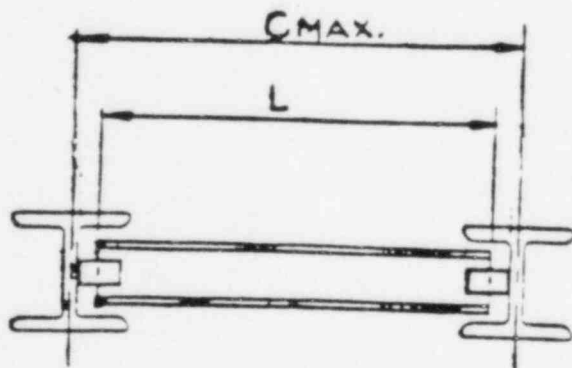


FIG 8B (Fig-8)

Note: For other dimensions, see Figure #1 to #8.

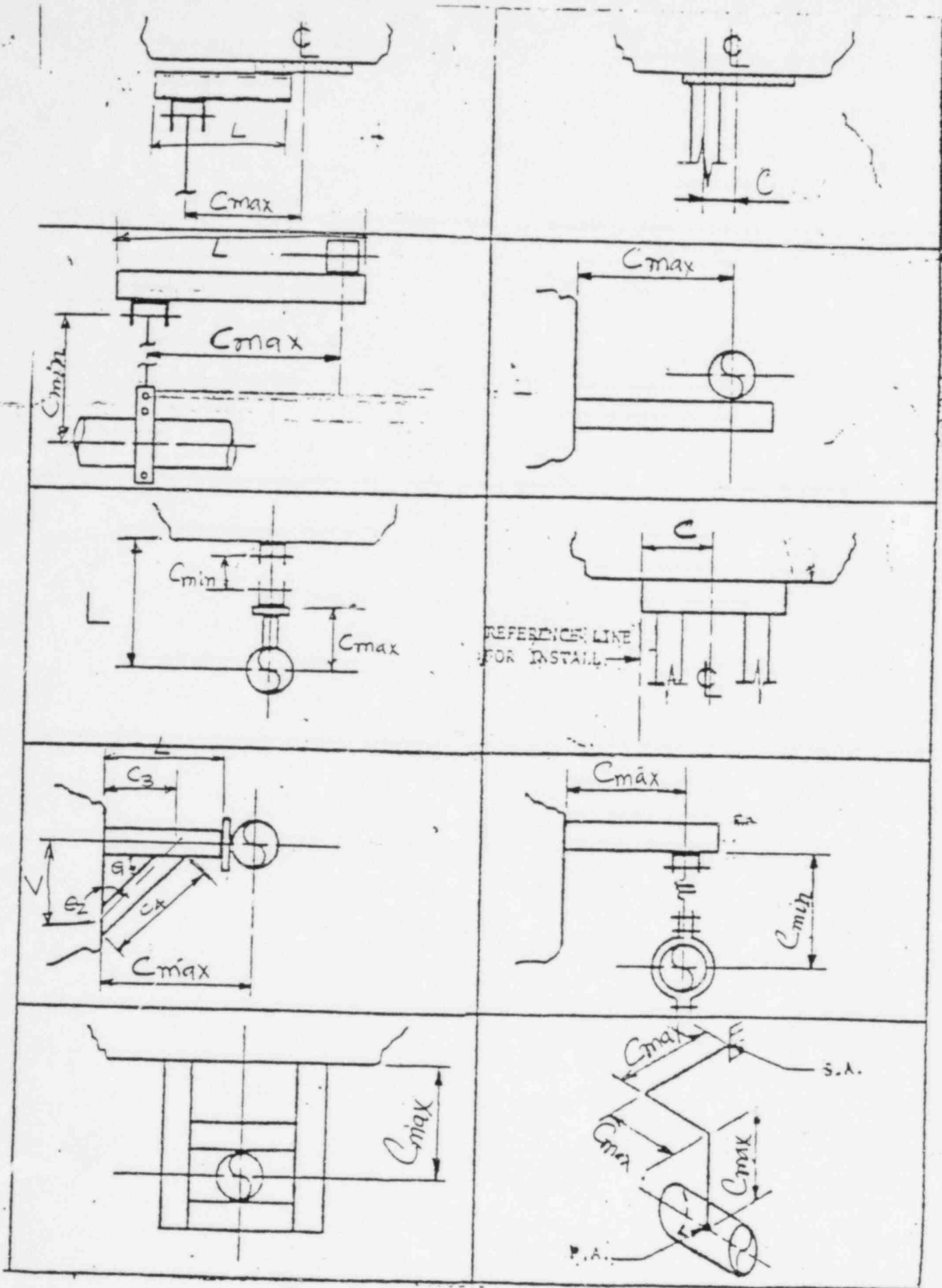


Fig-9

Note: For other dimensions, see Figure # 1 to 8

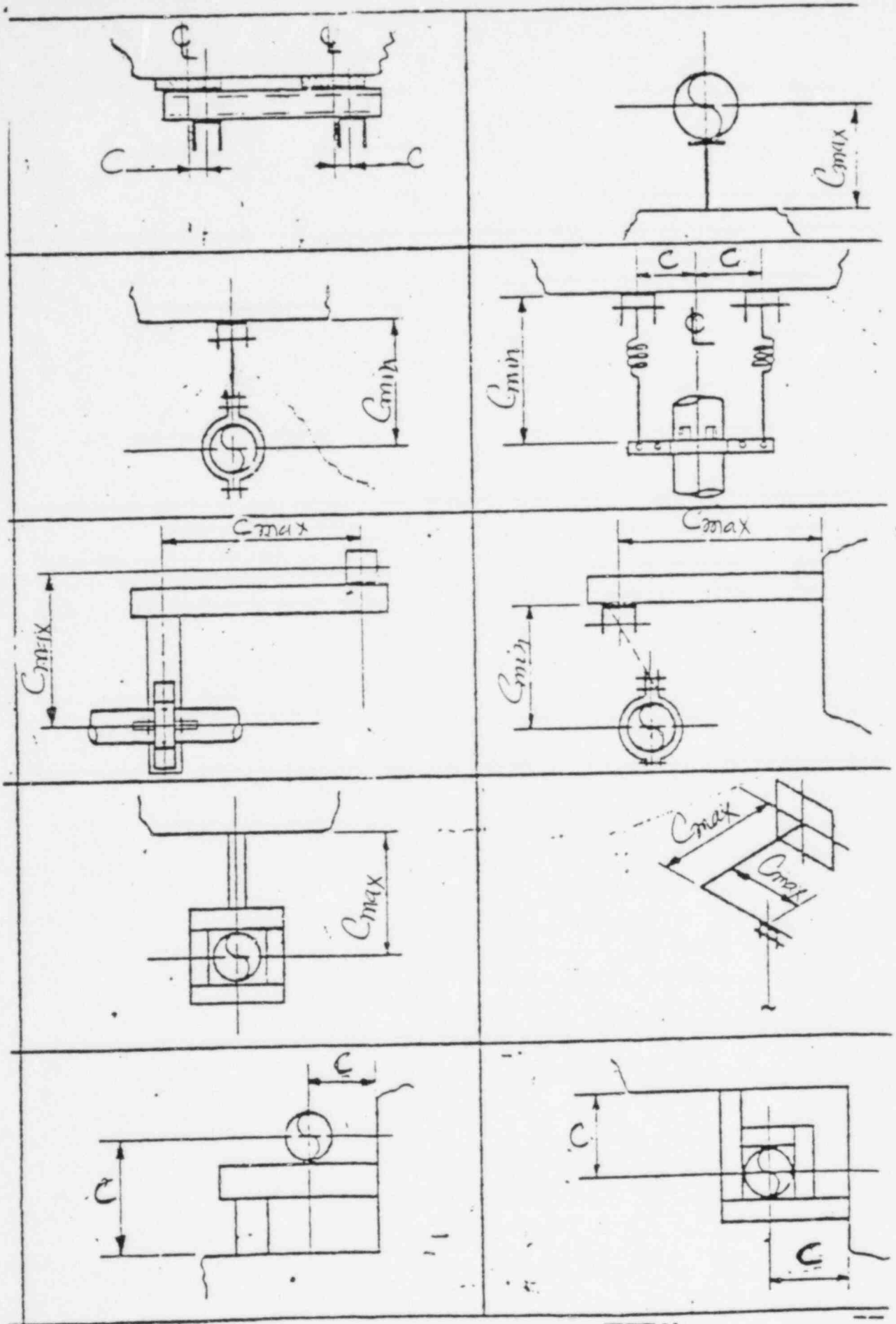


FIG-10 17

CASE - CASE EVALUATION OF NRC CAT SAMPLE DEVIATIONS

ATTACHMENT 2  
PAGE 1 OF 8

HANGER NO.	DEVIATIONS	REASON FOR DEVIATION	ENGINEERING EVALUATION	CATEGORY
DO-2533-2	Hanger location with respect to the pipe riser varied from design location to 2-7/8". The tolerance allowed by drawing H501 for this critical dimension is $\pm 2"$ .	Reviewer problem - Delta delta misunderstanding.	No Design Impact per Engineering Evaluation.	M1
HPCS-16	Richmond insert stud threads not staked.	Not as-builder problem.	No Design Impact per Engineering Evaluation.	D2
LPCS-28	1. Undersize welds on washer plates. 2. Cold set dimension on snubber as 2' 7-7/8" per design drawing, shown on redline as 2' 10-1/2".		No Design Impact per Engineering Evaluation. No Design Impact per Engineering Evaluation.	W1 M2
LPCS-903N	Weld details were not specified on the drawing.	As-builder did not check to see if every joint had weld symbol. As-builder checked everything on drawing - weld symbol not shown on drawing - appears only on FRPED. Weld is OK.	No Design Impact per Engineering Evaluation.	W3
MS-998N	Lug to pipe welds 1/4", drawing specified 5/16".	Not as-builder error - Hanger was not as-built at time of NRC walkdown.	No Design Impact per Engineering Evaluation.	W1
RCIC-21	Clip angle 4x4x3/8", drawing specifies 4x4x1/2"		No Design Impact per Engineering Evaluation.	M7
RCIC-952N	1. Tube steel 4x4x1/4", drawing specifies 4x4x.375" 2. Wrong heat number etched on tube steel.	Heat number belongs to previously attached piece that was removed.	No Design Impact per Engineering Evaluation.	M7

CASE -CASE EVALUATION OF NRC CAT SAMPLE DEVIATIONS

ATTACHMENT 2  
PAGE 2 OF 8

HANGER NO.	DEVIATIONS	REASON FOR DEVIATION	ENGINEERING EVALUATION	CATEGORY
RHR-415	<ol style="list-style-type: none"> <li>1. Vendor welds ground undersize during attachment weld grinding.</li> <li>2. Critical dimension shown on redline as 5' 1/4", actual and original design is 5' 7-3/4".</li> </ol>	Mismeasurement - to wrong point (top of pipe instead of plate.)	<p>No Design Impact per Engineering Evaluation.</p> <p>No Design Impact per Engineering Evaluation.</p>	<p>W1</p> <p>M1</p>
RHR-465	Lug to pipe weld 3/16", drawing specifies 1/4".	1/32 concavity in weld.	No Design Impact per Engineering Evaluation.	W1
RHR-563	<ol style="list-style-type: none"> <li>1. Cold set dimensions on snubbers were 1" 1-5/8" and 1' 1-3/4'. Drawing specifies a 1" = 1/8".</li> <li>2. Two of twelve expansion anchor mounting studs had less than required thread engagement into anchor shell.</li> </ol>		<p>No Design Impact per Engineering Evaluation.</p> <p>No Design Impact per Engineering Evaluation.</p>	<p>M2</p> <p>D5</p>
RWCU-162	Weld details were not specified on the drawing.	This is a QC-II hanger.	No Design Impact per Engineering Evaluation.	W3
SLC-4475-11	Clearance from pipe to support are 1/8" and 1/32", redline indicates 1/16" and 0".		No Design Impact per Engineering Evaluation.	M5

CASE-BY-CASE EVALUATION OF EXTENDED SAMPLE DEVIATIONS

ATTACHMENT 2  
PAGE 3 OF 8

HANGER NO.	DEVIATIONS	REASON FOR DEVIATION	ENGINEERING EVALUATION	CATEGORY
MSLC-11	1. As-builder - fillet weld instead of flare bevel. 2. Cir. was 1/16" and 1/32" instead of recorded 1/8" and 1/16".	Misunderstanding of weld symbol.	No Design Impact per Engineering Evaluation.	W3 M5
RHR-298	Gusset plate's welding less than design 5" 7" 9" and 9" actual versus 11-1/16" shown.		No Design Impact per Engineering Evaluation.	W2
RHR-319	A flare V weld was recorded as a flare bevel.		No Design Impact per Engineering Evaluation.	W3
RHR-408	1/4" fillet weld is actually 3/16" x 1/4".		No Design Impact per Engineering Evaluation.	W1
RHR-520	2' 7-7/8" recorded as 2' 8-1/2".	Not as-builder error (W851).	N/A	M1
RHR-609	Distance between welds is 4-1/4" instead of 4".	Not as-builder error (W851).	N/A	M1
RHR-920N	Thermal exp. interference with RHR 924N.	Not as-builder error (WNP-2-075)	Hanger Redesign Required for either RHR-920N or RHR-924N	D1
RHR-924N	1. See RHR 920N. 2. Under welding of flare bevel weld. 3. Unrecorded gap between lug and top of clamp.	Not as-builder error  This is an extra weld not specified on design.	See RHR-920N  No Design Impact per Engineering Evaluation. " " " "	D1 W1 M5
RHR-925N	Missing lock washer on Richmond insert.		No Design Impact per Engineering Evaluation.	D2
RHR-946N	P-P is 1' 7-3/8" instead of 1' 7-11/16" (critical dimension is OK).		No Design Impact per Engineering Evaluation.	M2

CASE-BY CASE EVALUATION OF EXTENDED SAMPLE DEVIATIONS

ATTACHMENT 2  
PAGE 4 OF 8

HANGER NO.	DEVIATIONS	REASON FOR DEVIATION	ENGINEERING EVALUATION	CATEGORY
MSLC-32	Three way rest. clr. E-1/32" versus 1/16", W-3/32" versus 1/32", N-1/16" versus 1/32", S-OK - 1-lug 1/64" clr.		No Design Impact per Engineering Evaluation.	M5
FPC-908N	1. Shim t = 1/32" versus 1/16". 2. Lug → clamp clr.		No Design Impact per Engineering Evaluation. " "	M1 M5
LPCS-45	1. Undersized fillet weld 1/4" x 3/16" and 1/8" x 3/16" versus 1/4". 2. Measurement 2' 1-3/4" versus 2' 0-7/8".	Not as-builder error (W851).	No Design Impact per Engineering Evaluation. ↓	W1 M1
HPCS-909N	1. Weld called inaccessible was 1/8" undersized. 2. One side of all around 5/16" fillet is 1/4" x 5/16". 3. Rear bracket missing spacer washers.	Not as-builder error - spacer not required.	No Design Impact per Engineering Evaluation. ↓ N/A	W1 W1 D3
HPCS-925N	7/8" clamp bolt instead of 1".	Not as-builder error - Reinspector error.	N/A	M6
MS-42	Flare bevel weld undersized by 1/8".		No Design Impact per Engineering Evaluation.	W1
MS-68	1. Strut interference with insulation. 2. Power Piping strut used instead of NPS. 3. P-P dimension not on drawing or verified. 4. Dimension 3' 7-1/4" versus 3'11". 5. Offset 1 3/4" versus 1". 6. Undersized welds on wrong side of stiffner plate.		No Design Impact per Engineering Evaluation. ↓	D1 D4 M3 M1 M4 W1

CASE-BY CASE EVALUATION OF EXTENDED SAMPLE DEVIATIONS

ATTACHMENT 2  
PAGE 5 OF 8

HANGER NO.	DEVIATIONS	REASON FOR DEVIATION	ENGINEERING EVALUATION	CATEGORY
MS-170	Fillet weld is 1/4" x 5/16" versus 5/16".		No Design Impact per Engineering Evaluation.	W1
MS-171	Under filled flare bevel.		No Design Impact per Engineering Evaluation.	W1
MS-1000N	1. There are 4 item #15 versus 2 recorded. 2. Critical dimension not measured.	Original Bill of Material was wrong. Redundant dimension.	No Design Impact per Engineering Evaluation.	D4 M1
MS-1011S	1. Snubber 11-9/16" versus 11-3/4". 2. Dimension 5-3/16" versus 5".	Not as-builder error (WB51).	No Design Impact per Engineering Evaluation.	M2 M1
MS-1013S	1. Snubber dimension 11-3/16" versus 11-9/16" and 1' 0-5/16" versus 1' 0-5/8". 2. Base plate dimension 5-1/4" versus 5", 6-3/4" versus 7", 7-1/4" versus 7", 4-3/4" versus 5".	Not as-builder error (WB51).	No Design Impact per Engineering Evaluation. ↓	M2 M1
MS-295	Spring Rod with 2-3/4" thread instead of 6" thread.		No Design Impact per Engineering Evaluation.	M1
RHR-17	Welded beam attachment is 3/4" - 5/8" was specified on drawing.		No Design Impact per Engineering Evaluation.	D4
RHR-96	Weld is 3/16" x 1/4" instead of 5/16" fillet.		No Design Impact per Engineering Evaluation.	W1
RHR-212	1. 8' 10-5/16" versus 8' 9-1/4". 2. Missed weld length 3. Clr. 1" specified - 1/2" exists 4. 1'-0" versus 8-15/16" 5. 3/16" fillet all around specified versus seal welded on 2 of sides. 6. 3/16" weld missing on bottom of attachment 7. 5-1/2" versus 7".	Not as-builder error (WB51).	N/A No Design Impact per Engineering Evaluation. ↓	M1 W2 M5 M1 W4 W4 M1



CASE-BASE EVALUATION OF EXTENDED SAMPLE DEVIATIONS

HANGER NO.	DEVIATIONS	REASON FOR DEVIATION	ENGINEERING EVALUATION	CATEGORY
RHR-245	Eight bolt base plate has one stud bolt instead of match bolt.	Not as-builter error - allowable substitution.	N/A	
RHR-548	Extra weld on plate washer.	Not as-builter error (WB51)	N/A	
RHR-554	Strut p-p 1' 9-3/4" and 1' 10-7/8" versus 1' 11-1/2" each.		No Design Impact per Engineering Evaluation.	M3
RHR-928N	As-builter misidentified spring size.	Spring was poorly marked.	No Design Impact per Engineering Evaluation.	D4
RHR-SB-31	1. 1' 8-1/2" versus 1' 10" 2. Snubber p-p 2' 3-1/8" (actual) versus 2' 4" (as-built) 2' 7" (specified).		No Design Impact per Engineering Evaluation.	M1
MSRV-2A-5	Snubber p-p 1' 8-1/8" versus 1' 7-7/8".		No Design Impact per Engineering Evaluation.	M2
MSRV-2C-9	1. Flare bevel 1/16" under fill. 2. Oversize reinforcement plate 12-1/4" x 13-3/8" versus 11-1/4" x 13-1/4".		No Design Impact per Engineering Evaluation.	M2
LPCS-46	Missing weld not noted.		No Design Impact per Engineering Evaluation.	W1
LPCS-901N	Stiffener weld and plate shorter.		No Design Impact per Engineering Evaluation.	M1
RCIC-15	Stud used instead of bolt.		No Design Impact per Engineering Evaluation.	W4
RCIC-26	Snubber p-p 2' 9-13/16" versus 2' 6".	Not as-builter error per PED-215-H-E854.	N/A	W2
			No Design Impact per Engineering Evaluation.	M2

CASE- ASE EVALUATION OF EXTENDED SAMPLE DEVIATIONS

HANGER NO.	DEVIATIONS	REASON FOR DEVIATION	ENGINEERING EVALUATION	CATEGORY
RCIC-41	Clearance dev.		No Design Impact per Engineering Evaluation.	M5
RCIC-107	Weld marked TYP which was not. Other weld was specified OK.		No Design Impact per Engineering Evaluation.	W3
RCIC-121	1. Weld L 7-1/2" versus 7-3/4". 2. Flare bevel with 1/4" fillet cap is 3/16" x 1/4".	Not as-builder error (WB51).	No Design Impact per Engineering Evaluation.	W2 W1
RCIC-904N	1. 3' 0-5/8" versus 3' 0". 2. Missing weld not reported. 3. 1/4" fillet actually 3/16" x 1/4". 4. 2' 11-5/16" versus 3' 0".	Not as-builder error (WB51).  Not as-builder error (WB51).	No Design Impact per Engineering Evaluation.  ↓	M2 W4 W1 M1
RCIC-911N	1" clamp bolt used instead of 1-1/8".	Not as-builder error - Reinspector error.	N/A	M6
HPCS-12	1. Bill of Material for sway struct called out rear bracket but M-146 was installed but not specified on as-built. 2. 5" offset not reported.		No Design Impact per Engineering Evaluation.  ↓	D4 M4
HPCS-17	1. Portion of all around 1/4" fillet is 3/16" x 5/16" and 3/16" x 1/4" (skewed weld). 2. Washer plate is welded on two sides instead of four.		No Design Impact per Engineering Evaluation.  ↓	W1 W2
VR-2	1/4" all around fillet weld is 3/16" x 1/4" on one side.		No Design Impact per Engineering Evaluation.	W1

CASE-BY CASE EVALUATION OF EXTENDED SAMPLE DEVIATIONS

HANGER NO.	DEVIATIONS	REASON FOR DEVIATION	ENGINEERING EVALUATION	CATEGORY
RCIC-127	Undersized weld.		No Design Impact per Engineering Evaluation.	NI
RHR-462	Hanger offset missed 4" -15' and 4" -45'.		No Design Impact per Engineering Evaluation.	MA

FREQUENCY SUMMARY OF CATEGORIZED DEFICIENCIES IN ATTACHMENT 2

	NRC	Extended	
<u>Welding</u>			
W1 Undersize	4	16	
W2 Underlength		5	
W3 Symbol	2	3	
W4 Missing	<u>        </u>	<u>    4    </u>	
TOTAL	6	28	
<u>Measurements</u>			
M1 Dimensions	2	15	
M2 Snubber P-P	2	7	
M3 Strut P-P		2	
M4 Offset		3	
M5 Clearance	1	6	
M6 Clamp Bolt Size		2	
M7 Steel Size	<u>    2    </u>	<u>    0    </u>	
TOTAL	7	35	
<u>Details</u>			
D1 Thermal Interference		3	
D2 Locking Device	1	1	
D3 Spacer Washer		1	
D4 Component Identification		5	
D5 Thread Engagement	<u>    1    </u>	<u>    0    </u>	
TOTAL	2	10	

GENERIC EVALUATION OF DEVIATIONSFINDINGS

The errors reported by the re-as-built team fell into three general categories: welding, measurements, and miscellaneous details (See Attachment 1). There were 34 welding discrepancies noted of which six were from the NRC sample and 28 from the extended sample. Undersized welding not being measured and/or recorded was the most prevalent discrepancy with 20 findings. There was a total of 42 measurement errors reported, seven from the NRC sample and 35 from the extended sample. The remaining miscellaneous details, such as thermal interferences, locking devices, and component manufacturer that were missed during the original as-built numbered 12. Two of these items were from the NRC sample, 10 from the extended sample.

Many of the items, in both the NRC sample and the extended sample, that were originally thought to be errors turned out not to be as-building errors per se. With the development of a more definitive set of measurement tolerances and acknowledgement that other existing inspection programs existed to reconcile certain deficiencies, most of the findings either fall within acceptable measurement tolerances or the deviations would be picked up by one or more other inspection programs. The following is a detailed summary of each category of deficiencies and a brief discussion on the findings in that category.

W1 - Undersized Weld: The NRC CAT identified MS-998N as deficient. However, the hanger was not as-built when the NRC CAT inspection was made. The hanger has subsequently been as-built and the undersized weld identified by the CAT was also recorded by the as-builder. The remainder of the undersized welds represent a small percentage of the total number of welds (19 of approximately 2,500, < 1%). None of the 19 undersized welds affected the design in accordance with an engineering evaluation.

W2 - Underlength Weld: The potential design impact of a weld that is shorter than designed is similar to that for a weld that is undersized. For two of the welds, the deviation was less than the measurement tolerance of PED-215-H-W851 and therefore is not really an as-builder error. In another case, the weld was called deficient because the weld of a web stiffener did not run from flange to flange. It is standard practice to clip the corners of stiffeners resulting in a shorter weld even though the hanger detail did not call for it to be clipped. None of these deficiencies impacted the design.

W3 - Welding Symbol: Welding symbol errors cover two types of errors. Welds that are improperly specified, i.e., calling a flare bevel weld a fillet weld, are usually very obvious to the designer, and as such, result in a request by the AE for a re-as-built of that item. The other error is welds that are missing any weld symbol, and as such, appear not to be welded on the as-built when reviewed by the AE. In nearly all cases, this would be conservative.

W4 - Missing Welds: There was a total of four welds specified that were not made and not identified as missing by the as-builder. In all cases, an engineering evaluation of the detail indicated that the existing hanger meets code requirements. Further, four welds out of the total population in over 100 hangers with approximately 2,500 welds is a small error rate.

M1 - Dimensioning Errors: Nine of the 17 reported dimensioning errors are acceptable as-is because they fall within the WNP-2 measurement tolerance (PED-215-H-W851). The remaining eight dimensioning errors were minor and acceptable as they do not impact the design of the hangers.

M2 - Snubber Pin-to-Pin Measurement: The snubber pin-to-pin measurement errors are not significant for two reasons. First, the measurement tolerance of + 1/8" is quite tight considering that pipes move due to ambient temperature changes and as a result of the hanger setting and balancing. Either of these can result in a hanger that is acceptable one day and unacceptable the next. Further, startup has a preservice inspection procedure (SLT-S-303) which reinspects all snubber supports prior to operation of the system and the pin-to-pin dimension is one of the items checked.

M3 - Strut Pin-to-Pin Measurements: Only two strut measurement errors were noted, and in one of the cases, the error was that the dimension was not verified. Strut pin-to-pin dimensions are change due to hanger

setting and balancing and are not critical as long as they are within the manufacturers limits on the load capacity data sheets(s). Further, many of the struts, approximately 80%, will be reinspected during startup's preservice inspection of supports (SLT-S-303).

M4 - Offset Measurement: In all cases, the offsets were acceptable as-is by an engineering review. Hanger offset measurements are another item that is reviewed by the startup preservice inspection procedure (SLT-S-303), and as such, most offsets will receive a second review. In two of the three offset measurement errors, offsets existed that were not reported by the as-builder.

M5 - Clearance Measurements: All seven of the clearance measurement problems were acceptable from an engineering review. Box restraint clearances like snubber settings can change from day-to-day due to ambient temperature changes and can be affected by the hanger setting and balancing. Hanger clearances are reinspected after the line has been balanced and most will be inspected again when startup does their preservice inspection under SLT-S-303.

M6 - Clamp Bolt Size: Both of the bolt size errors reported were later found not to be as-builder errors, but rather errors made during the as-built reinspection. The original as-built is correct. No deficiencies were found.



M7 - Steel Sizing Errors: The two cases where the steel thicknesses were not identified by the as-builders appear to be isolated cases as the extended sample did not identify a single additional case. These two cases represent a very small error rate among the hundreds of pieces of steel covered by the sample of over 100 supports. In both cases, the existing condition was acceptable based on an engineering evaluation.

D1 - Thermal Interference: The three thermal expansion interferences identified in the extended sample do not belong as as-building errors. The requirement to look for thermal interferences has been removed from the as-built program. These items will be covered by both the Hot Pipe Interference Walkdown (BRI Project Instruction WNP-2-075) and startup preservice inspection (SLT-S-303).

D2,3,5 - Locking Devices, Spacer Washers, and Threaded Engagement: These items are not the responsibility of the as-built program with the exception of noting double nuts as a locking device. These deviations would be covered by several other programs. The hanger set and balance team insures that there is full threaded engagement and a locking device for all adjusted hangers. Startup's preservice inspection program (SLT-S-303) covers these items. Also, other sampling programs resulting from the generic problem identified by the NRC CAT concerning nuts and bolts will address these items.

D4 - Component Identification: There were five components that were not properly identified: a strut manufacturer, a spring can size, a

rear bracket for a strut the size of a welded beam attachment (a size larger than specified was installed), and the quantity of PSA-35 weld adapters (the original bill of materials was wrong). This represents a small error rate. Further, the spring size error would be readily identified by the hanger set and balance program. Both the spring can and the strut could also be identified during the preservice inspection (SLT-S-303). These errors are random and limited and were found not to impact the design or function of the hangers.

ENGINEERING INSPECTION AND EVALUATION OF  
QC I PIPE SUPPORTS AND SMALL BORE PIPING

WNP-#2

Prepared For

WASHINGTON PUBLIC POWER SUPPLY SYSTEM

September 15, 1983 (Revision 1)

~~September 17, 1983~~

*J. Varoe* *J.C. REV. 1*  
Project Engineer

*J. L. Cook* *J.C. REV. 1*  
Manager, Engineering  
Mechanics Division

*E. H. Lewis* *J.C. REV. 1*  
Supervisor, Engineering  
Assurance Division

*D. Davis* *J.C. REV. 1*  
Assistant Engineering  
Manager

Copyright 1983

Stone & Webster Engineering Corporation  
Denver, Colorado 80217



## REASON FOR CHANGES

### Revision 1

Changes from the initial report (dated September 1, 1983) have been made and are identified with a vertical line in the right hand margin.

The affected page/section/paragraph and the reason for each change is listed below:

1. Page 6, deleted the first item (bullet) under paragraph 3.5-C.  
Reason for deletion:  
During the final stages of the SWEC pipe support inspections, clearance was obtained for access to the Diesel Generator Rooms. Access to these rooms had been severely restricted due to ongoing major construction. During the inspection in these rooms, the inspection team noted a deviation on pipe support SW-1048-22 and turned it over to the evaluation group. The evaluation group determined that the sketch made by the inspection team did not contain sufficient detail to adequately assess the condition so a reinspection was recommended. The SWEC team made two attempts to obtain clearance for access to the Diesel Generator Rooms to reinspect this support but were unsuccessful because of newly instituted security clearance requirements. By this time schedule requirements dictated that the final report be prepared and issued, so this item was evaluated to the best of SWEC's ability as sketched on the inspection form and included in the final report.  
  
Subsequent to the preparation and submittal of the final report to the Supply System, SWEC engineers were able to obtain access to the Diesel Generator Rooms and conducted two inspections of support SW-1048-22. These inspections indicated that the information on the inspection form was incomplete and that the condition assumed in the report did not exist.
2. Page 7, second paragraph of section 3.7. Added clarifying sentences.  
Reason for change:  
Expanded the text of the report to more accurately explain the results of Attachment E.
3. Attachment E "Engineering Assurance Evaluation", page 2, under "Observation & \*Disposition":
  - Changed the disposition of paragraph 3  
Reason for change:  
The only instances SWEC observed the starting of as-builts before construction completion was when temporary items were involved (paragraph 4.2 of Bechtel procedure 14631/RL-1.00). Providing this procedure is rigorously implemented, proper control of this work approach can be achieved.
  - Replaced the fifth paragraph.  
Reason for change:  
Developing an overall audit program at this point in the construction schedule would probably not yield any benefits. However, the current monitoring program must continue to be scheduled and performed.



TABLE OF CONTENTS

<u>SECTION</u>	<u>TITLE</u>	<u>PAGE</u>
	REASON FOR CHANGES	i
	TABLE OF CONTENTS	1
1.0	INTRODUCTION	2
2.0	CONCLUSIONS	3
3.0	SWEC PROGRAM	4
3.1	Sample	4
3.2	Inspection	5
3.3	Data Analysis	5
3.4	Evaluations	5
3.5	Results	5
3.6	Review of SWEC Activities	7
3.7	Review of As-Built Program	7

ATTACHMENTS

A	Sample
B	Procedure WRO-01
C	Inspection Results
D	Trending Results
E	Engineering Assurance Evaluation



## 1.0 INTRODUCTION

In May and June 1983, a NRC Construction Assessment Team (CAT) reviewed the Quality Class I as-built program for WNP-2 as planned and implemented by the contractor for the Washington Public Power Supply System (Supply System). During this review, discrepancies between the as-built documentation and as-installed pipe support configurations were noted by the CAT Team. The CAT Team also noted that previous reviews of the as-built supports by both Project QA (PQA) and Bechtel QA (BQA) had indicated different rates of deviation. These three reviews were based on a relatively small sample of the pipe support population. As a result, the Supply System contracted with Stone & Webster Engineering Corporation (SWEC) to perform an independent third-party review of the QC I as-built program. This review included a physical inspection of a relatively large cross-section of the QC I pipe support and small bore piping population, the analysis of deviations, and an engineering evaluation of the significance of the deviations. In addition, a review of the as-built program was conducted to ascertain compliance with the requirements of I&E Bulletin 79-14.



## 2.0 CONCLUSIONS

- The present QC I as-built program in use at WNP-2 is acceptable and meets the needs of the project.
- The implementing procedures for the as-built program are satisfactory and meet ASME III and NRC I&E Bulletin No. 79-14 requirements.
- Deviations found during the SWEC review, were determined not to have a significant effect on the structural integrity of the support.
- Several isolated items found during the SWEC review may require further action by the Supply System. These are described in section 3.5 C of this report.



### 3.0 SWEC PROGRAM

SWEC performed a third-party review consisting of three parts:

1. Sample selection, physical inspection, and analysis of deviations for a significant sample of the hanger population.

Sections 3.1, 3.2 and 3.3 of this report provide details of the inspections performed including sample selection and data analysis.

2. Engineering evaluation of deviations found during the physical inspection.

Section 3.4 of this report provide details of the evaluations performed.

3. A review of the as-built program.

Section 3.7 of this report describes the review performed and assesses the adequacy of the as-built program relative to regulatory requirements.

SWEC's work was limited to 1) comparing as-installed conditions to as-built drawings, and evaluating any deviations, and 2) reviewing the contractor implementation of the QC I as-built program. It was not within SWEC's scope of work to re-perform work done by others or to review the basis of the original design. Large bore QC I piping was not included in SWEC's Scope of Work. Also, since SWEC used as-built drawings for their inspections, it was assumed that any deviations previously found by Bechtel in the as-built program will be reconciled by others.

#### 3.1 Sample Selection

Supports and piping to be inspected were initially selected at random from the Bechtel computer listing of as-built drawings by startup system. This list contains 2,270 QC I large bore pipe supports, 733 small bore QC I piping isometrics (with the associated supports), and 269 unique QC I small bore supports.

A sample of 15 percent of the population of large and small bore QC I pipe supports, small bore piping, and small bore unique supports was selected. This sample size greatly exceeds the sample sizes established in MIL-STD-105D for a 95 percent confidence level. For example, from a population of 2270 large bore supports, a sample of 340 supports was inspected to achieve 15 percent while MIL-STD-105D at a confidence level of 95 percent requires a sample of 125. On this basis, there is greater than 95 percent confidence level that the results of the inspection and evaluation of the sample can also be applied to the uninspected population.

A 15 percent sample consists of 340 large bore supports, 110 small bore piping isometrics, 184 small bore standard supports, and 55 unique small bore supports. The initial random sample was adjusted to assure a representative cross section of various support types and sizes which exist in the plant. The final sample and a breakdown by type, system, etc., is shown in Attachment A.





### 3.2 Inspection

Prior to commencement of the inspection phase, SWEC personnel were trained in weld measurement and the inspection procedures to be used, and were familiarized with the plant and the history of the WNP-2 as-built program.

Inspections were performed and data were collected according to Project Procedure WRO-01 - Procedure for Engineering and Evaluation of Pipe Supports (Attachment B).

Each pipe support was inspected for 17 attributes. The attributes selected for pipe support evaluation were primarily those associated with the structural integrity of the support. The 13 attributes selected for the small bore isometric evaluation were those considered to be associated with the structural integrity of the piping system and the configuration of the piping system. The results of the inspections by attribute are shown in Attachment C.

### 3.3 Data Analysis

The disposition of the attributes on each inspection form were inputted to SWEC's Pipe Hanger Information System Program (PHIS) and sorted by attribute, system and support type. From this information, the number and types of various deviations were compiled and results analyzed. Data results are shown in Attachment D.

### 3.4 Evaluations

If during the inspection an attribute was determined to be outside the tolerances established in Attachment B, it was marked as a deviation and evaluated. If the engineer on the inspection team could determine that the deviation did not affect the structural integrity of the support, it was documented on the inspection form as acceptable. If the inspection team could not determine disposition of the evaluation, it was referred to an Evaluation Group. The Evaluation Group then reviewed the deviation against the criteria in Section 3.1.5.1 of Attachment B. The effect of a deviation on the structural integrity of supports which could be accepted with calculations was so documented.

The basis for acceptance of the effect of a deviation on the structural integrity of a support was either by referencing existing design calculations or by performing calculations based on existing load data provided by Burns & Roe Inc., or Gilbert Commonwealth. Generic acceptance of the effects of deviations was also used where the Supply System has shown that those deviations will systematically be remedied. The details of these types of bases for acceptance are shown in Attachment B, Section 3.1.5.1.

### 3.5 Results

The results of the inspection phase are shown in Attachment C. They are presented in a tabular form indicating the number and type of deviations found.

- A. During the inspection process, 141 supports were found to have fillet weld deviations. These supports contained a total of 1150 fillet welds. There were 233 of these fillet welds that were not in conformance with the as-built documentation; 204 of which, were considered critical to the structural integrity of the support.



From the information shown in Attachment C, a trend can be seen in the area of weld measurement on the as-built drawings. A further sort of weld deviations is presented in Attachment D. In the most common weld deviations, one or both legs of the fillet weld were 1/16" less than specified on the drawing. Review of stress levels in these welds indicated that they were within allowable limits.

It would be highly reasonable (because of the large sample size) to expect a similar proportion of weld deviations to be found in the uninspected population. Since the sample size was greater than that which would yield a 95 percent level of confidence (discussed in section 3.1) it would be reasonable to expect a similar proportion of the weld deviations in the uninspected population and for them be evaluated and dispositioned (structurally adequate) with the same rate of success as in the inspected population. On this basis, SWEC is more than 95 percent confident that no deviations in fillet welds, which could adversely affect the structural adequacy of the supports, will be found in the uninspected portion of the population. Therefore, SWEC does not recommend that any further weld inspections be conducted.

- B. Many deviations were found in is U-bolt side clearances for small bore supports. The supply system has an ongoing program to cover these clearance problems; therefore, they were not further evaluated by SWEC.
- C. The following items were observed during the inspection process which may require further action by the Supply System:

- Another condition observed by SWEC, was that insulation around small bore supports and pipes restricts thermal expansion pipe movement in the axial direction. It is recommended that the Supply System develop a program to account for this condition.
- It was observed that the stiffener plates located on building steel for supports RCIC-16 and RCIC-19 were distorted. This condition is not in the scope of the As-built Program but is being investigated by Bechtel.
- During the review of the inspection of a small bore piping isometric, SWEC discovered that a deviation was noted by a Bechtel as-builder but was not followed up with a Request For Information (RFI) or Project Engineering Directive (PED). Bechtel is currently reviewing the as-built drawings for this condition.
- Where deviations were found such as missing parts, excessive gaps, snubber settings, etc., the Supply System procedure SLT S303.0 was referenced for acceptance. SLT S303.0 references ASME XI which would exclude small bore supports for pipes one inch and below. It is recommended that this procedure be revised to include QC I pipe supports for pipes of 1 inch and 3/4 inch normal size.



Of the 689 large or small bore pipe support or small bore piping isometrics inspected, 311 had deviations requiring evaluations. 78 were evaluated by the inspection teams. Their effect on the structural integrity of pipe supports and piping were all determined acceptable. 233 required more detailed analysis by the evaluation group. The effect of these deviations were also found acceptable to the criteria shown in Attachment B, Section 3.1.5.1, and the section 3.5 C of this report. Where deviations were observed that could affect the structural adequacy of the support, they were analyzed and found to have sufficient design margin so that the structural integrity of the support was not compromised.

### 3.6 Review of SWEC Activities

The Engineering Assurance Division\* performed an audit of the SWEC project activities to determine if the procedure in use adequately describes and controls the work, and if the work was being accomplished according to the procedure. Work performed by the field inspection teams was observed and the subsequent documentation by these teams as well as the Evaluation Group was reviewed. The project procedure, Attachment B, and the project's compliance to it were found to be satisfactory.

### 3.7 Review of As-Built Program

An evaluation of the WNP-2 as-built program (as it applies to the QC I piping and associated pipe supports) was performed by SWEC's Engineering Assurance Division\*. The intent of this evaluation was to assess the as-built program's compliance to ASME III and NRC I&E Bulletin No. 79-14 and its' ability to ensure that the engineering analysis applies to the actual as-installed configuration (members, location, welds) of the piping system. The evaluation was conducted by reviewing the applicable procedures, holding discussions with appropriate personnel from the Supply System (the Owner), Burns & Roe, Inc. (the A/E), the Bechtel Power Corp. (the Constructor) and others involved in the program, and by reviewing as-built documentation. The review concentrated on pipe supports and the construction program for identifying the as-built configuration to the A/E.

The results of this evaluation indicate that the written program and procedures are satisfactory and comply with ASME III and NRC I&E Bulletin No. 79-14 requirements. However, some procedural changes could be made to better describe interfaces between affected groups, reflect actual practices, and improve effectiveness. A detailed list of the items evaluated, associated observations, and dispositions are described in Attachment E. The dispositions regarding changes to procedures are suggestions that in SWEC's judgement could improve program effectiveness. A detailed list of the items evaluated and associated observations are presented in Attachment E.

\*The SWEC Engineering Assurance (EA) Division is a Division in the Engineering Department and is responsible for administering the SWEC Standard Nuclear Quality Assurance Program as it applies to engineering and design activities.



TABLE IA - LARGE BORE SUPPORT SAMPLE

<u>SUPPORT</u>	<u>REV.</u>	<u>SUPPORT</u>	<u>REV.</u>	<u>SUPPORT</u>	<u>REV.</u>
CAC 86	2	CAC 10	2	EDR 493	4FO
CAC 87	1	CAC 108	1	EDR 503	2
CAC 9	1	CAC 109	1	EDR 903N	1
CAC 902N	2FO	CAC 111	3FO	EDR 904N	3FO
COND 591	4FO	CAC 116	3	EDR 906N	3FO
COND 886	2	CAC 117	4FO	FUR 475	1
CRD 904S	0	CAC 16	3	FDR 900N	2FO
CRD 905S	0	CAC 18	4FO	FDR 901N	2
DE 1	4FO	CAC 19	2	FDR 902N	2FO
DE 2	5FO	CAC 20	2	FPC 209	1
DE 3	3FO	CAC 21	3FO	FPC 210	3F1
DE 35	2F1	CAC 23	2	FPC 223	3
DE 36	3FO	CAC 49	2	FPC 224	5FO
DE 4	6FO	CAC 5	2	FPC 225	3FO
DE 40	2FO	CAC 50	4FO	FPC 228	2
DE 41	2FO	CAC 6	2FO	FPC 229	6FO
DE 42	3FO	CAC 78	1	FPC 41	5FO
DE 5	4FO	CAC 8	1	FPC 42	3FO
DE 68	4F1	CAC 80	1FO	FPC 43	4
DE 904N	1FO	CAC 81	1	FPC 59	4FO
DE 906N	1FO	CAC 82	2FO	FPC 60	2FO
DU 905N	1FO	CAC 83	2FO	FPC 64	2FO
DU 908N	1FO	CAC 84	1	FPC 906N	1FO



TABLE 1A - LARGE BORE SUPPORT SAMPLE (CONTINUED)

<u>SUPPORT</u>	<u>REV.</u>	<u>SUPPORT</u>	<u>REV.</u>	<u>SUPPORT</u>	<u>REV.</u>
FPC 909N	1	LPCS 905N	2FO	MS 265	2FO
FPC 919N	1FO	LPCS 907N	3	MS 289	3FO
HPCS 1	3	LPCS 908N	2FO	MS 290	3FO
HPCS 16	5	LPCS 909N	2FO	MS 305	3FO
HPCS 17	5FO	MS 1C-1PS	2FO	MS 306	2FO
HPCS 18	4	MS 1003N	2FO	MS 316	2FO
HPCS 31	7FO	MS 101	3FO	MS 321	2
HPCS 32	6FO	MS 115	4FO	MS 324	3FO
HPCS 38	4	MS 117	5FO	MS 325	1
HPCS 44	5FO	MS 119	5FO	MS 328	2FO
HPCS 7	4FO	MS 120	6FO	MS 342	2FO
HPCS 907N	3FO	MS 121	3	MS 45	5
HPCS 908N	4FO	MS 157	5FO	MS 47	6FO
HPCS 910N	3FO	MS 163	6FO	MS 993N	0
HPCS 911N	3FO	MS 168	5	MS 996N	1FO
HPCS 916N	2	MS 170	6FO	MS 997N	1FO
LPCS 11	3FO	MS 171	4FO	MS 998N	1FO
LPCS 13	4FO	MS 260	2FO	MSLC 10	1
LPCS 38	3	MS 261	1	MSLC 11	2FO
LPCS 39	4FO	MS 274	1	MSLC 12	1
LPCS 57	4FO	MS 275	2FO	MSLC 13	1
LPCS 61	4FO	MS 277	3FO	MSLC 25	2
LPCS 904N	4FO	MS 278	2FO	MSLC 26	2FO



TABLE 1A - LARGE BORE SUPPORT SAMPLE (CONTINUED)

<u>SUPPORT</u>	<u>REV.</u>	<u>SUPPORT</u>	<u>REV.</u>	<u>SUPPORT</u>	<u>REV.</u>
MSLC 27	2	RCIC 8	2	MSRV 48-7	3FO
MSLC 28	1	RCIC 80	3FO	MSRV 48-8	3FO
MSLC 29	2FO	RCIC 82	4FO	MSRV 50-2	4FO
MSLC 31	1	RCIC 83	4FO	MSRV 50-3	2FO
MSLC 32	2FO	RCIC 86	4	MSRV 50-4	3FO
MSLC 34	2FO	RCIC 88	2	MSRV 50-5	3FO
MSLC 35	1	RCIC 91	2	MSRV 50-1	2FO
MSLC 39	2FO	RCIC 93	2	MSRV 50-2	3FO
MSLC 9	1	RCIC 95	3FO	MSRV 50-3	2FO
MSRV 20-5	2FO	RCIC 952N	2F1	RCIC 10-10	4
MSRV 20-6	3FO	RCIC 954N	3FO	RCIC 10	3FO
MSRV 20-7	3FO	RCIC 969S	0	RCIC 11	4
MSRV 3A-6	4	RCIC 970S	1FO	RCIC 12	4FO
MSRV 3B-5	3FO	RFW 151	4	RCIC 14	4
MSRV 3B-6	3FO	RFW 152	2	RCIC 15	0
MSRV 3D-8PS	1FO	RFW 153	5	RCIC 16	3FO
MSRV 4A-10	2	RFW 162	5FO	RCIC 17	4FO
MSRV 4A-4	3FO	RFW 164	2	RCIC 19	4FO
MSRV 4A-5	2FO	RFW 177	4FO	RCIC 5	3FO
MSRV 4A-6	3FO	RFW 179	3	RCIC 6	3FO
MSRV 4A-9	2FO	RFW 182	7FO	RCIC 7	5FO
MSRV 40-10	3FO	RFW 942N	0	RCIC 74	2FO
MSRV 40-6	0FO	RHR SA-50	3FO	RCIC 79	4FO



TABLE IA - LARGE BORE SUPPORT SAMPLE (CONTINUED)

<u>SUPPORT</u>	<u>REV.</u>	<u>SUPPORT</u>	<u>REV.</u>	<u>SUPPORT</u>	<u>REV.</u>
KHK SA-53	3FO	KHR 370	4FO	KHR 983N	3FO
KHR SA-54	2FO	KHR 408	3FO	KXC HA-1	1FO
KHK SA-56	RFO	KHR 412	3FO	KXC MB-1	0
KHK SA-58	3FO	RHR 414	3FO	RRC RA-1	2FO
KHK SA-59	3FO	KHR 415	5FO	KRC Sc-11	2FO
KHR 10	4FO	RHR 416	3FO	KXC SB-13	1
RHR 11	4FO	KHR 425	3FO	RXC 10-14	1
RHR 135	5F1	RHR 428	RFO	KXC 30-15	2FO
RHR 136	5	RHR 431	4FO	KXC SB-16	2FO
KHR 138	3FO	KHR 582	4FO	RXC 1	2
KHR 17	3FO	KHR 583	3FO	KXC 1C-1	3FO
RHR 170	2	RHR 584	5FO	KXC 1C-13	3FO
KHR 171	6	KHR 585	4FO	KXC 1C-14	2
RHR 172	2FO	RHR 586	4FO	KXC 1C-15	2
KHR 173	3FO	KHR 66	3	RRC 1C-6PS	3FO
KHR 18	4	KHR 67	4	RRC 1C-9CUN	0
KHR 186	4FO	KHR 68	4	KXC 2	4FO
KHR 19	5FO	KHR 70	3	KXC 3	2
KHR 370	3FO	KHR 77	5FO	RRC 4	2FO
KHR 371	4FO	KHR 79	4FO	RWCU 1C-1	2
RHX 372	3FO	KHR 8	5FO	KWCU 1C-1PS	2
RHR 373	3FO	RHR 81	2	KWCU 1C-11	2FO
KHR 374	2	KHR 9	5	RWCU 1C-12	3FO



TABLE IA - LARGE BORE SUPPORT SAMPLE (CONTINUED)

<u>SUPPORT</u>	<u>REV.</u>	<u>SUPPORT</u>	<u>REV.</u>	<u>SUPPORT</u>	<u>REV.</u>
RWCU 1C-2	2	SW 100	1	SW 411	2
RWCU 1C-2PS	3FO	SW 107	1	SW 413	3
RWCU 1C-4	2	SW 108	1	SW 43	1
RWCU 1C-7	3FO	SW 109	3FO	SW 44	2FO
RWCU 1C-7PS	1	SW 110	2FO	SW 40	3FO
RWCU 139	3FO	SW 111	2	SW 47	2FO
RWCU 140	2FO	SW 112	4FO	SW 48	2FO
RWCU 141	3	SW 138	1	SW 49	2FO
RWCU 145	3FO	SW 25	2	SW 50	2FO
RWCU 140	1	SW 26	2	SW 51	1
SGT 20	1	SW 30	1	SW 905N	1
SGT 900N	1FO	SW 346	2	SW 900N	1
SGT 901N	1FO	SW 304	2	VR 1	3
SLC 901N	1	SW 305	1	VR 3	4FO
SLC 903N	2F1	SW 401	2	VR 4	3FO
SLC 904N	2FO	SW 402	2	VR 5	2FO
SLC 905N	2FO	SW 403	2	VR 900N	1
SW 100	1	SW 404	2	VR 902N	2FO
SW 101	3FO	SW 400	2		
SW 102	1	SW 407	2		
SW 103	1	SW 408	2		
SW 104	1	SW 409	3		
SW 105	1	SW 410	2		





TABLE IB - SMALL BORE ISOMETRIC SAMPLE

<u>ISOMETRIC</u>		<u>REV.</u>	<u>ISOMETRIC</u>		<u>REV.</u>	<u>ISOMETRIC</u>		<u>REV.</u>
UU 2532	3	4	CAC 2759	5	9	MS 2614	1	7
DU 2533	1	6FO	CAS 3085	2	9FO	MS 2615	1	7
DU 2708	1	7FO	CAS 3086	3	10FO	MS 2617	1	7
UG 2710	3	6F1	CAS 3088	3	9FO	MSLC 2826	1	2
UG 2711	1	5FO	CIA 4101	1	4	MSLC 2826	2	4FO
DU 2715	3	4	CIA 4115	1	5	RCC 2470	2	1
DSA 2536	1	6FO	COND 4631	3	6	RCC 2474	2	0
DSA 2729	2	7FO	COND 4631	4	5	RCIC 1477	4	2
USA 2732	1	3FO	COND 4631	5	5	RCIC 1482	2	9
DW 1154	7	12	COND 4631	6	5	RCIC 1483	2	3
FUR 2222	2	7	DCW 2518	1	6FO	RCIC 1486	3	5FO
FPC 2934	1	4FO	DCW 2520	1	4	RCIC 2558	1	6
FPC 4444	2	5	DCW 2722	1	7FO	RCIC 2560	1	8
HPCS 1458	1	8	DCW 2724	1	5FO	RCIC 2560	8	8FO
HPCS 1458	3	7	DCW 4329	1	5FO	RCIC 2560	16	6
HPCS 1459	2	8FO	DCW 4603	1	3	RCIC 2560	16	5
HPCS 1645	1	3	DCW 4640	1	2FO	RCIC 4529	1	3
LPCS 1627	1	11	DE 2840	1	4FO	RHR 1968	1	12FO
LPCS 2565	1	13FO	DU 2525	3	6FO	RHR 1968	6	9FO
LPCS 3077	1	10FO	DU 2526	3	7FO	RHR 2018	1	5FO
MD 1288	2	8	DU 2527	1	5FO	RHR 2020	1	4
MS 1293	1	6	DU 2528	1	5FO	RHR 2104	1	10
MS 1425	1	8	DU 2531	3	5FO	RHR 2264	1	10



TABLE 1B - SMALL BORE ISOMETRIC SAMPLE (CONTINUED)

<u>ISOMETRIC</u>		<u>REV.</u>	<u>ISOMETRIC</u>		<u>REV.</u>
RHR 2287	2	4	SW 1038	4	9
RHR 2289	2	5FO	SW 1048	2	5FO
RHR 2578	1	4FO	SW 1520	17B	4
RHR <del>4434</del>	1	8FO	SW 1529	4	0
RHR 4515	1	5	SW 1532	3	4
RHR 4525	1	7	SW 2515	1	1
RRC 1330	1	8	SW 2521	1	4
RRC 1337	1	6FO	SW 2598	1	0
RRC 1337	3	4	SW 2700	2	7
RRC 1549	1	8	SW 2717	1	1FO
RRC 1551	4	8	SW 2723	1	5
RRC 1552	6	12	SW 4222	4	4
RRC 1940	1	7	SW <del>4441</del>	1	7FO
RRC 4300	3	7	SW 4504	1	2
RWCU 3084	1	5	SW 4565	1	1
SGT <del>4449</del>	2	3	SW 4018	1	1
SGT <del>4449</del>	3	3	SW 4041	1	2
SLC <del>4450</del>	2	5	SW 4042	1	1
SW 1006	2	7			
SW 1032	2	10FO			
SW 1034	3	4			
SW 1035	5	4			
SW 1038	1	5			



TABLE IC - SMALL BORE SUPPORT SAMPLE

<u>SUPPORT</u>	<u>REV.</u>	<u>SUPPORT</u>	<u>REV.</u>	<u>SUPPORT</u>	<u>REV.</u>
AB 284-7	06	UCW 4603-13	3	DU 2710-33	0F1
CAC 2759-51	9	UCW 4640-11	2F0	DU 2710-34	0F1
CAC 2759-52	9	DE 2840-12	3F0	DU 2710-35	0F1
CAS 3085-21	9F0	DE 2840-13	3F0	DU 2710-36	0F1
CAS 3085-22	9F0	UE 2840-14	3F0	DSA 2530-13	1F0
CAS 3085-25	9F0	UE 2840-15	3F0	DSA 2530-13A	1F0
CAS 3086-32	10F0	DE 2840-16	4F0	DSA 2530-13F	1F0
CAS 3088-32	8	DE 2840-17	3F0	USA 2729-21	7F0
CIA 4101-11	4	DU 2526-31	7F0	USA 2729-22	7F0
CIA 4101-12	4	DU 2526-32	7F0	USA 2729-23	7F0
CIA 4101-13	4	DU 2526-33	7F0	USA 2729-24	7F0
CIA 4101-14	4	DU 2526-34	7F0	USA 2729-25	7F0
CIA 4115-11	0	DU 2526-35	7F0	USA 2729-26	7F0
DCW 2513-11	1	DU 2533-11	0F0	DW 1154-71	
UCW 2518-11A	0F0	DU 2533-12	0F0	DW 1154-72	
DCW 2518-11B	0F0	DU 2533-13	0F0	FPC 2934-11	4F0
DCW 2518-12	0F0	DU 2533-14	0F0	FPC 2934-12	4F0
UCW 2520-11	4	DU 2533-15	0F0	FPC <del>4444</del> -21	5
UCW 2724-11	5F0	DU 2708-11	7F0	FPC <del>4444</del> -23	5
DCW 2727-11C	2F0	DU 2708-12	7F0	FPC <del>4444</del> -24	5
UCW 4329-11	5F0	DU 2708-13	7F0	FPC <del>4444</del> -25	5
DCW 4603-11	3	DU 2710-31	0F1	FPC <del>4444</del> -26	5
UCW 4603-12	3	DU 2710-32	0F1	MPLS 1456-12	8



TABLE IC - SMALL BORE SUPPORT SAMPLE (CONTINUED)

<u>SUPPORT</u>	<u>REV.</u>	<u>SUPPORT</u>	<u>REV.</u>	<u>SUPPORT</u>	<u>REV.</u>
MPCS 1458-13	8	MSLC 2826-23	4FO	RMR 1988-11	12FO
MPCS 1459-21	8FO	RCIC 1477-41	2	RMR 1988-01	9FO
MPCS 1459-22	8FO	RCIC 1482-21	9	RMR 1988-02	9FO
MPCS 1459-23	8FO	RCIC 1482-22	9	RMR 1988-03	9FO
MPCS 1459-24	8FO	RCIC 1486-31	5FO	RMR 1988-04	9FO
MPCS 1459-25	8FO	RCIC 2568-12	6	RMR 1988-05	9FO
LPCS 1627-11A	11	RCIC 2560-12	4	RMR 2018-11	5FO
LPCS 1627-11B	13FO	RCIC 2560-13	4	RMR 2018-12	5FO
LPCS 2565-11A	13FO	RCIC 2560-14	4	RMR 2020-13	3
LPCS 2565-11B	11	RCIC 2560-101	6	RMR 2020-15	3
LPCS 2565-11C	13FO	RCIC 2560-102	6	RMR 2020-15A	4
LPCS 2565-12A	13FO	RCIC 2560-103	6	RMR 2104-11	10
LPCS 2565-12B	11	RCIC 2560-104	6	RMR 2104-12	10
LPCS 3077-11	10FO	RCIC 2560-105	6	RMR 2107-11C	10
MD 1288-21	6	RCIC 2560-106	6	RMR 2107-11D	10
MS 1293-13	6	RCIC 2560-107	6	RMR 2204-11	10
MS 1425-15A	4	RCIC 2560-108	6	RMR 2204-12A	10
MS 1425-15B	4	RCIC 2560-181	5	RMR 2204-12B	10
MS 1425-15C	4	RCIC 2560-182	5	RMR 2287-21	4
MS 1425-16	4	RCIC 2560-81	8FO	RMR 2289-21	4FO
MSLC 2826-11	2	RCIC 2560-82	8FO	RMR 2578-11A	4FO
MSLC 2826-21	4FO	RCIC 2560-83	8FO	RMR 2578-11B	4FO
MSLC 2826-22	4FO	RCIC 2560-84	8FO	RMR 2578-12A	4FO



TABLE IC - SMALL BORE SUPPORT SAMPLE (CONTINUED)

<u>SUPPORT</u>	<u>REV.</u>	<u>SUPPORT</u>	<u>REV.</u>
RHR 2578-12b	4FO	SW 1032-22	10FO
RHR 4434-11A	8FO	SW 1032-23	10FO
RHR 4434-12	8FO	SW 1035-51	4
RHR 4515-11	5	SW 1038-1	9
RHR 4515-12	5	SW 1038-2	0
RRC 1330-11b	8	SW 1038-5	9
RRC 1330-12	8	SW 1048-22	5FO
RRC 1337-14	5FO	SW 1048-23	5FO
RRC 1337-15A	5FO	SW 2515-11	1
RRC 1337-15b	5FO	SW 2515-12	1
RRC 1549-13	8	SW 2521-11	4
RRC 1551-41	8	SW 2521-12	4
RRC 1551-43	8	SW 2717-11	1FO
RRC 1552-03A	12	SW 2723-11	5
RRC 1552-03b	12	SW 4441-11	2FO
RRC 1552-04	12	SW 4441-12	7FO
RRC 1552-05	12	SW 4441-13	7FO
RRC 4300-33A	7	SW 4441-14	7FO
RRC 4300-33b	7	SW 4441-15	2FO
RRC 4300-34	7	SW 4441-16	2FO
RRC 4300-35	7	SW 4504-11	2
SGT 4449-31	3	SW 4505-11	1
SW 1032-21	10FO	SW 4505-12	1



TABLE ID - SMALL BORE UNIQUE SUPPORT SAMPLE

<u>SUPPORT</u>	<u>REV.</u>	<u>SUPPORT</u>	<u>REV.</u>	<u>SUPPORT</u>	<u>REV.</u>
CRD 1000-15	1FO	MS <del>4448-2</del>	1FO	SLC <del>4452-34</del>	0
CRD 1001-15	0	MSLC 2820-12	0	SLC 4452-71A	1FO
CRD 1001-16	0	MSLC 2820-14	1FO	SLC <del>4452-71B</del>	1FO
CRD 9145	0	MSLC 2820-22	0	SLC 4452-73	1FO
HY 4230-15	0	MSLC 2820-23	1FO	SLC <del>4453-45</del>	1
HY 4230-16	0	MSLC 2820-41	0	SLC <del>4453-65</del>	1F1
HY 4234-16	2FO	MSLC 2820-43	1FO	SLC 4453-66	7
HY 4234-17	1	MSLC 2820-45	0	SLC <del>4453-67</del>	0
HY 4236-15	2FO	MSLC 2821-22	0	SLC <del>4475-110</del>	0
HY 4236-16	1	MSLC 2821-39	0		
HY 4236-17	2FO	MSLC 2821-41	0		
HY 4237-110	10FO	MSLC 2822-33	0		
HY 4237-17	1	MSLC 2823-23	0		
MS 1308-11	1FO	MSLC 2823-32	0		
MS 2019-311	1FO	MSLC 2823-34	0		
MS 2019-314	1FO	MSLC 2823-41	0		
MS 2019-320	1FO	MSLC 2823-42	0		
MS 2019-420	0	MSLC 2823-43	0		
MS 2019-43	1	RRC 1819-21	0		
MS <del>4448-12</del>	0	KXC 1946-32	0		
MS <del>4448-13</del>	0	SLC <del>4452-11</del>	1FO		
MS <del>4448-31</del>	1FO	SLC <del>4452-12</del>	1FO		
MS <del>4448-41</del>	0	SLC <del>4452-31</del>	1FO		



TABLE IIA LARGE BORE SUPPORT SAMPLE - DISTRIBUTION

DISTRIBUTION OF SAMPLE BY SYSTEM

<u>SYSTEM DESIGNATION</u> (Description)	<u>TOTAL HANGER</u> (Per Bechtel Data Base, 8-1-83)	<u>15% SAMPLE</u>	<u>HANGERS AUDITED</u>
CAC (Containment Atmos. Control)	102	15	27
CEP (Containment Purge Exhaust)	6	1	0
COND (Condensate)	27	4	2
CPR (Cond. Filter Demineralizer System)	2	1	0
CRD (Control Rod Drive)	0*	0	2
CSP (Containment Purge Supply)	3	1	0
DCW (Diesel Cooling Water)	4	1	0
DE (Diesel Exhaust & Intake Air)	65	10	13
DO (Diesel Fuel Oil)	6	1	2
EDR (Equipment Drain Radioactive)	6	1	5
FDR (Floor Drain Radioactive)	5	1	4
FPC (Fuel Pool Coolant)	45	7	16
HPCS (High Pressure Core Spray)	60	9	14
HY (Hydraulic)	2	1	0
LPCS (Low Pressure Core Spray)	44	7	11
MD (Miscellaneous Drains)	2	1	0
MS (Main Steam)	255	38	36
MSRV (Main Steam Relief Valve)	147	22	23
MSLC (Main Steam Valve Leakage Control)	41	6	15

\*No supports listed on large bore data base. These supports are listed on small bore data base.



TABLE IIA CONTINUED

<u>SYSTEM DESIGNATION</u> (Description)	<u>TOTAL HANGER</u> (Per Bechtel Data Base, 8-1-83)	<u>15% SAMPLE</u>	<u>HANGERS AUDITED</u>
RCIC (Reactor Core Isol. Coolant)	172	26	27
RFW (Reactor Feed Water)	41	6	9
RHR (Residual Heat Removal)	615	92	48
RRC (Reactor Recirculation)	82	12	18
RWCU (Reactor Water Clean-up)	31	5	14
SGT (Stand-by Gas Treatment)	24	4	3
SLC (Stand-by Liquid Control)	7	1	4
SW (Service Water)	453	68	41
VR (Radioactive Vent)	16	2	6
RCC (Reactor Closed Cool. Water)	<u>1</u>	<u>1</u>	<u>0</u>
TOTAL	2264	344	340





TABLE IIB LARGE BORE SUPPORT SAMPLE - DISTRIBUTION

DISTRIBUTION OF SAMPLE BY SUPPORT TYPE

<u>SUPPORT TYPE</u>	<u>TOTAL NUMBER OF TYPE</u>	<u>15% SAMPLE</u>	<u>HANGERS AUDITED</u>
RIGID	1066	160	136
SNUBBER	526	79	87
SPRING	426	64	72
STRUT	<u>246</u>	<u>37</u>	<u>45</u>
TOTAL	2264	340	340

TABLE IIC LARGE BORE SUPPORT SAMPLE - DISTRIBUTION

DISTRIBUTION OF SAMPLE BY ASME CODE GROUPS

<u>CODE GROUP</u>	<u>TOTAL NUMBER</u>	<u>15% SAMPLE</u>	<u>HANGERS AUDITED</u>
A	391	59	73
B, C, D	<u>1873</u>	<u>281</u>	<u>267</u>
TOTAL	2264	340	340



TABLE IID SMALL BORE ISOMETRIC SAMPLE - DISTRIBUTION

DISTRIBUTION OF SAMPLE BY SYSTEM

<u>SYSTEM DESIGNATION</u> (Description)	<u>TOTAL ISOMETRICS</u> (Per Bechtel Data Base, 8-1-83)	<u>15% SAMPLE</u>	<u>ISOs AUDITED</u>
CAC (Containment Atmos. Control)	10	2	1
CAS (Control Air System)	14	2	3
CIA (Containment Instr. Air)	71	10	2
COND (Condensate)	10	2	4
CSP (Containment Purge Supply)	2	1	0
DCW (Diesel Cooling Water)	34	5	7
DE (Diesel Exhaust & Intake Air)	8	1	1
DO (Diesel Fuel Oil)	77	11	11
DSA (Diesel Starting Air)	21	3	3
DW (De-mineralizer Water)	1	1	1
FDR (Floor Drain Radioactive)	6	1	1
FPC (Fuel Pool Coolant)	4	1	2
HPCS (High Pressure Core Spray)	11	2	4
HY (Hydraulic)	8	1	0
LPCS (Low Pressure Core Spray)	16	2	3
MD (Miscellaneous Drains)	8	1	1
MS (Main Steam)	31	5	5
MSLC (Main Steam Valve Leakage Control)	13	2	2
RCC (Reactor Closed Cooling Water)	4	1	2
RCIC (Reactor Core Isol. Coolant)	65	10	10



TABLE IID CONTINUED

<u>SYSTEM DESIGNATION</u> (Description)	<u>TOTAL ISOMETRICS</u> (Per Bechtel Data Base, 8-1-83)	<u>15% SAMPLE</u>	<u>ISO'S AUDITED</u>
RHR (Residual Heat Removal)	88	12	12
RRC (Reactor Recirculation)	47	7	8
RWCU (Reactor Water Clean-up)	1	1	1
SA (Service Air)	2	1	0
SGT (Stand-by Gas Treatment)	6	1	2
SLC (Stand-by Liquid Control)	2	1	1
SW (Service Water)	<u>161</u>	<u>23</u>	<u>23</u>
TOTAL	721	110	110



TABLE IIE SMALL BORE ISOMETRIC SAMPLE - DISTRIBUTION

DISTRIBUTION OF SAMPLE BY SYSTEM

<u>CODE GROUP</u>	<u>TOTAL NUMBER</u>	<u>15% SAMPLE</u>	<u>ISOs AUDITED</u>
A	50	8	10
B, C, D	<u>671</u>	<u>101</u>	<u>100</u>
TOTAL	721	109	110

TABLE IIF SMALL BORE UNIQUE SUPPORT SAMPLE - DISTRIBUTION

DISTRIBUTION OF SAMPLE BY SYSTEM

<u>SYSTEM DESIGNATION</u> (Description)	<u>TOTAL HANGER</u> (Per Bechtel Data Base, 8-1-83)	<u>15% SAMPLE</u>	<u>HANGERS AUDITED</u>
CRD (Control Rod Drive)	32	5	4
HY (Hydraulic)	52	8	9
MS (Main Steam)	64	10	11
MSLC (Main Stm. Valve Leakage Control)	103	16	17
RRC (Reactor Recirculation)	9	2	2
SLC (Stand-by Liquid Control)	<u>71</u>	<u>11</u>	<u>12</u>
TOTAL	331	52	55



TABLE IIG SMALL BORE UNIQUE SUPPORT SAMPLE - DISTRIBUTION

DISTRIBUTION OF SAMPLE BY ASME CODE GROUPS

<u>CODE GROUP</u>	<u>TOTAL NUMBER</u>	<u>15% SAMPLE</u>	<u>HANGERS AUDITED</u>
A	139	21	18
B, C, D	<u>192</u>	<u>29</u>	<u>37</u>
TOTAL	331	51	55

TABLE IIH SMALL BORE UNIQUE SUPPORT SAMPLE - DISTRIBUTION

DISTRIBUTION OF SAMPLE BY SUPPORT TYPE

<u>SUPPORT TYPE</u>	<u>NO. SAMPLE</u>	<u>15% SAMPLE</u>	<u>HANGERS AUDITED</u>
RIGID	258	39	40
SNUBBER	62	10	10
SPRING	<u>11</u>	<u>2</u>	<u>5</u>
TOTAL	331	51	55



PROCEDURE FOR  
ENGINEERING INSPECTION AND EVALUATION  
OF PIPE SUPPORTS

PROJECT INSTRUCTION NO. 1

J.O. No. 14420.03

WNP-2

WASHINGTON PUBLIC POWER SUPPLY SYSTEM  
WRO-01

APPROVALS:

DATE:

*D. Varas*

PROJECT ENGINEER

*8/31/83*

*J. L. Cook*

DOC EMD MANAGER

*8/31/83*



TABLE OF CONTENTS

	Page
1.0 PURPOSE	3
2.0 INTRODUCTION	3
2.1 DEFINITIONS	3
2.2 BACKGROUND	3
2.3 LIMITATIONS	3
3.0 GENERAL PROCEDURE	3
3.1 SAMPLE SIZE AND TRENDING OF RESULTS	4
3.2 ENGINEERING REVIEW CHECKLIST	4
3.3 COMPUTERIZED DATA MANAGEMENT	4
3.4 DOCUMENTATION AND FINAL REPORT	4
4.0 ORGANIZATION	4
5.0 DETAILED PROCEDURE	5
5.1 SCOPE	5
5.2 DATE COLLECTION	5
5.3 TRAINING	6
5.4 INSPECTION	6
5.5 DEVIATION EVALUATION	7
5.6 DOCUMENTATION OF REVIEW	8
5.7 COMPUTERIZED DATA MANAGEMENT	8
5.8 SAMPLE SIZE AND TRENDING	8
5.9 DOCUMENTATION	9

ATTACHMENTS

- A Support Checklist
- B Isometric Checklist
- C Inspection and Evaluation Guideline - Activity Sequence
- D Support Inspection Guideline
- E Isometric Inspection Guideline
- F Calculation Form



## 1.0 PURPOSE

This document defines the scope, organization, and procedures to be employed by SWEC in performing WRO-01, Engineering Inspection and Evaluation Quality Class I (QCI) of Pipe Supports and Small Bore Piping.

## 2.0 INTRODUCTION

### 2.1 DEFINITIONS

SWEC - Stone & Webster Engineering Corporation

The Supply System - Washington Public Power Supply System

Data Package - All information associate with the inspection of an individual pipe support including as-built drawing and checklist.

Final Data Package - All information associated with the inspection and evaluation of an individual pipe support or small bore pipe including signed checklist together with as-built drawings, attachments, and calculations as required.

### 2.2 BACKGROUND

In May and June of 1983, the NRC Construction Assessment Team (CAT) reviewed the Supply System pipe support as-built program, as planned and implemented by the Supply System contractor. During this review, discrepancies between as-built documentation and as-installed support configurations were discovered. As a result, an independent third party review is being made. This procedure addresses that portion of the third party review concerning engineering inspection and evaluation of pipe supports.

### 2.3 LIMITATIONS

It is not in SWEC's scope of work to reperform design work done by others, or to review the basis of the original design. SWEC work will be limited to comparing as-installed conditions to the appropriate as-built drawings. The following describes the procedure for our review and the steps to be taken if discrepancies are identified.

## 3.0 GENERAL PROCEDURE

SWEC will perform and document an engineering inspection and qualification of a representative sample of the QC 1 pipe supports installed on the Supply System - Unit #2 project. The review will include on-site inspection of the pipe supports by experienced engineers and/or designers. The installation will be evaluated against the design documents of record (as-builts, specifications, etc.) using a checklist of pertinent engineering attributes as a guide. Deviations from the design documents will be noted in the checklist and will be evaluated based on engineering judgment and/or calculations.





Deviations that are found to be unacceptable (i.e., prevent the support from serving its intended function) will be documented in the checklist and made known to the Supply System in accordance with 5.5.3.

### 3.1 SAMPLE SIZE AND TRENDING OF RESULTS

The subject pipe supports will be divided into population groups based upon support type and pipe size. The results of the inspection for each of these populations will be evaluated periodically to determine trends.

The objective of the sampling and trending program is to identify as rapidly as possible any areas of genuine engineering concern. By using this type of approach, SWEC can facilitate the initiation of required rework by the Supply System on a timetable compatible with the fuel load schedule.

### 3.2 ENGINEERING REVIEW CHECKLIST

The engineering review checklist in Attachments A and B is designed to be a one-page (two-sided) document that addresses pertinent engineering concerns, provides traceability, and documents the review. It also documents deviations identified and the disposition of those deviations, together with identifying the engineer or designer that performed the review.

### 3.3 COMPUTERIZED DATA MANAGEMENT

SWEC will use PIPE HANGER INFORMATION SYSTEM (PHIS) (IS-202) program for data management to track the progress and status of these pipe support tasks.

### 3.4 DOCUMENTATION AND FINAL REPORT

The original data packages will be maintained on file by SWEC. The computerized progress reports and data will be maintained by SWEC and provided to the Supply System upon request. Copies of all final data packages and calculations produced will be turned over to the Supply System at the completion of the task.

SWEC will provide a final report which will state the findings, and delineate corrective actions taken during SWEC's execution of this task.

### 4.0 ORGANIZATION

SWEC will implement, control, and monitor the activities required to review the QC I pipe supports which have been as-built under the Bechtel as-built program. This task will be performed under the direction of the SWEC Project Manager located at the SWEC Richland, Washington office and the SWEC Project Team located at the site.



On-site inspection and evaluation of the pipe support installation will be performed by a Project Team reporting to the SWEC Project Engineer at the WNP-2 site. The Project Team will identify acceptable supports, document and evaluate deviations, track daily progress and update the computerized database.

Inspection will be performed by teams consisting of two engineers or one engineer and one designer. Where convenient one engineer or one designer may perform an inspection. The Inspection Team will evaluate deviations as discussed in Section 5.5.1. The balance of the deviations as discussed in 5.5.2 will be evaluated by an Evaluation Group. Computer tracking and trend analysis will be a function of the Evaluation group.

The Project Engineer will work closely with the Supply System to ensure that the goals of the review program are met satisfactorily.

## 5.0 DETAILED PROCEDURE

### 5.1 SCOPE

SWEC will perform a detailed inspection of 15% of the QC 1 large bore pipe supports, together with 15% of the QC 1 small bore piping and associated supports, including small bore unique supports. No large bore piping will be inspected.

There are 2270 large bore QC 1 pipe supports on this project. 15% equals 340 supports. There are 733 small bore QC 1 piping isometrics on this project. 15% equals 110 isometrics. There are 268 small bore unique supports. 15% equals 55 supports.

The supports and piping to be inspected shall be randomly selected from the Bechtel computer listing of as-built drawings by start-up system. The initial random selection will be adjusted to assure a sampling of support type and size.

### 5.2 DATA COLLECTION

A separate data package will be assembled for each pipe support or pipe that must be inspected. This package will consist of:

1. An engineering review checklist (Attachment A or B).
2. A copy of the as-built pipe support sketch or isometric drawing (the as-built document is a marked-up issue of the issued for construction document).

SWEC will use Bechtel-controlled files to determine the latest document of record. Copies of required documents will be requested from Site Document Control.



### 5.3 TRAINING

Shown below is the training required for all personnel assigned to this task. A training record will be maintained in a site located job book:

1. "History of the WNP-2 Pipe Support program", and a Summary of the Bechtel as-built program, given by Mr. L. Cantin of Bechtel Construction.
2. "Weld Measurement", given by Mr. P.J. Inserra of the Supply System.
3. "Implementation of the SWEC Checklist, Tolerances and Inspection Guidelines", given by Mr. Paul Hector of SWEC.
4. Plant familiarization tour, escorted by Mr. L. Goering of Bechtel Construction.

### 5.4 SUPPORT INSPECTION

Each inspection team will perform on-site inspections of assigned pipe supports and isometrics and will review each installation against the support design documents of record. Primarily those dimensions pertaining to the structural adequacy of the support will be checked. Each checklist attribute will be checked under the appropriate column at the time of the inspection, indicating whether the attribute is not applicable (NA), is acceptable (A), or is a deviation (D).

A deviation is an attribute which falls outside the tolerances established in Attachments D and E.

Any deviation will be noted in the Comments column of the checklist and/or in other appropriate documents attached to the checklist in the data package.

If the deviation is noted on other than the checklist, the document on which it is noted will be cited in the Comments column of the checklist, and that document will be affixed to the check lists.

A member of the inspection team will complete and sign the checklist in accordance with Section 5.5.1 of this procedure.

Attachment A	Support Checklist
Attachment B	Isometric Checklist
Attachment C	Inspection and Evaluation Guideline - Activity Sequence
Attachment D	Support Inspection Guideline
Attachment E	Isometric Inspection Guideline



## 5.5 DEVIATION EVALUATION

The cited deviations will fall into two groups: those which can be evaluated immediately by the inspection team and those which will require a more detailed evaluation by the Evaluation Group.

When the evaluation of a deviation is based on the support calculations or support loads, the evaluator must first judge the adequacy of the loads since they are based on pre-as-built construction documents. The adequacy of the support loads will be judged by a review of the as-built isometrics and applicable support drawings considering piping configuration, support location, and support type. If a judgment cannot be made on the support load without a computer pipe stress analysis, then notification will be made to the Supply System through the Project Engineer. A deviation can be accepted by referencing a Burns and Roe (B&R) calculation or performing a detailed calculation using B&R supplied loading data. Allowable stresses, loads, loading combinations, etc., are those currently used for the WNP-2 Project.

The effect of a deviation on the structural integrity of a support or pipe may be judged acceptable if that deviation will be remedied programmatically or referenced to a document which covers it on a generic level. Types of these deviations and their basis for acceptance are listed below.

- o AWS/AISC Minimum Fillet Weld Criteria - WPPSS  
Interoffice memorandum No. SS2-PE-83-138 dated 5/26/83 from P.J. Inserra to R.T. Johnson titled "Fillet Welds Not Meeting AWS D1.1 or AISC Minimum Size Requirements to Avoid Weld Cracking - NRC Open item 79-06-01".
- o Spring and Snubber Settings - WPPSS Procedures
  - 1) "Adjutment and Balancing of Components Supports" No. SLT-S305.0
  - 2) "Visual Examination of Component Supports" No. SLT-S303.0
- o Missing Parts, Snubber Paddle End Conenction Interferences and Close Clearance Excessive Gaps - WPPSS Procedure  
"Visual Examination of Component Supports", No. SLT-S303.0
- o Small Bore Support Generic Details - Where it is not specifically called out to refer to a small bore standard detail it is assumed that the small bore standards in GC-1000-1 apply based on RFI No. C0500-H-2939.

### 5.5.1 Deviations Evaluated Immediately

The inspection team may judge the effect of a deviation as acceptable from an engineering standpoint either on the basis of a cursory review of the pipe support calculation, or by comparison with another calculation or design standard.



These judgments and the basis thereof must be stated clearly in the "deviations evaluated as acceptable by inspection team" section of the checklist.

The inspection team may also judge that a deviation requires rework of the support to its existing design. If such is the case, refer to Section 5.5.3 of this procedure.

An Engineer or Designer may sign and date the checklist if no deviations are noted. If deviations are noted, an Engineer must sign the checklist.

#### 5.5.2 Deviations Requiring Further Evaluation

The deviations listed in the "Deviations Require Further Evaluation" section of the checklist will be evaluated by a detailed review of the calculation and/or use of an alternative calculation. It may also be referenced to a document which covers that item on a general basis. This will be done by the Evaluation Group, which is separate from the inspection team.

Alternative calculations which SWEC performs will be prepared on a Standard Form, Attachment F, which will become part of the final data package. The calculation will indicate the objective and conclusions and include necessary detailed calculations performed together with applicable references. These calculations will be signed by the preparer and reviewer. The reviewer shall also perform an independent review.

If an evaluation requires an extensive manual or computerized analysis, the Supply System will be notified through the Project Engineer and guidance requested.

After the Evaluation Group has completed its evaluation, it will complete and sign that section of the checklist, noting next to each deviation whether it is acceptable or requires rework. In addition it will refer to the calculation that substantiates this position. For deviations that require rework, the Evaluation Group will refer to Section 5.5.3 of this procedure.

#### 5.5.3 Support Modification Request Preparation

Deviations that were evaluated in Sections 5.5.1 or 5.5.2 of this procedure as requiring rework shall be made known to the Supply System through the Project Engineer.

#### 5.6 DOCUMENTATION OF REVIEW

The completed checklist will be filed with the document data package, which will be kept at the SWEC site offices.



A copy of each final data package and calculations (if prepared), will be submitted to the Supply System at the completion of SWEC's effort.

The results of the inspection and engineering review will be coded on the PHIS database by the Evaluation Group.

#### 5.7 COMPUTERIZED DATA MANAGEMENT

A data entry coordinator will update the database daily to incorporate the results of the previous day's inspection and/or calculations.

#### 5.8 SAMPLE SIZE AND TRENDING

##### 5.8.1 Sample Size

It is SWEC's position that 15 percent is an adequate sample size if the results of this program indicate that the quality of the installations are good. Further sampling will only be done with the express agreement of the Supply System.

Various sizes and types of supports from each system shall be adequately sampled.

##### 5.8.2 Trending

The subject pipe supports will be divided into various population groups based upon attributes such as support type and support size. The results of the inspection for each of these populations will be evaluated periodically to determine trends.

#### 5.9 DOCUMENTATION

A copy of all final data packages and applicable calculations will be transmitted to the Supply System. The original document will be maintained in a file by SWEC. The computerized progress reports and data base will be maintained by SWEC and provided to the Supply System at its request. A copy of all separate calculations produced to substantiate actions taken will be turned over to the Supply System at the completion of the task.

All judgments rendered and calculations performed will be submitted to the Supply System after the completion of this scope of work. All of these items will be identified and referenced in the final report.



ATTACHMENT A - SUPPORT CHECKLIST

Revision 3

HANGER DRAWING NO.	REV.	INSPECTION TEAM	LEGEND: A - ACCEPTABLE D - DEVIATION EXISTS N/A - NOT APPLICABLE
ISOMETRIC NO.	REV.	CHANGE DOCUMENT	
CHECKLIST ITEMS		STATUS	COMMENTS
		N/A   A   D	

- |   |  |  |  |  |
|---|--|--|--|--|
| 1. General                                |  |  |  |  |
| A. Support Location                       |  |  |  |  |
| B. Support Orientation                    |  |  |  |  |
| C. Catalog Items                          |  |  |  |  |
| D. Close Clearance Gaps                   |  |  |  |  |
| 2. Support Structure                      |  |  |  |  |
| A. Critical Dimensions                    |  |  |  |  |
| B. Member Sizes, Structural Plates        |  |  |  |  |
| 3. Struts and Snubbers                    |  |  |  |  |
| A. Pin to Pin Dimensions, Snubber Setting |  |  |  |  |
| B. Paddle-Pin Assembly Connections        |  |  |  |  |
| 4. Baseplates                             |  |  |  |  |
| A. Plate & Gusset Sizes                   |  |  |  |  |
| B. Bolt Size & Type                       |  |  |  |  |
| C. Bolt Hole Spacing                      |  |  |  |  |
| D. Attachment Location                    |  |  |  |  |
| E. Bolt Spacing to Adjacent Inserts       |  |  |  |  |
| 5. Lugs - Bearing Surface                 |  |  |  |  |
| 6. Welding                                |  |  |  |  |
| A. Size, Length, Quality                  |  |  |  |  |
| B. Symbols                                |  |  |  |  |
| 7. Miscellaneous (Specify)                |  |  |  |  |
|   |  |  |  |  |
|   |  |  |  |  |

---

Notes:

---

Results of Evaluation:

\_\_\_\_\_ No deviations noted.

\_\_\_\_\_ Deviations evaluated as acceptable by inspection team.

\_\_\_\_\_ Deviations require further evaluation.

\_\_\_\_\_  
Signature

\_\_\_\_\_  
Date

Disposition of deviations subject to further evaluation:

\_\_\_\_\_  
Signature

\_\_\_\_\_  
Date





ATTACHMENT B - ISOMETRIC CHECKLIST

Revision B

ISOMETRIC NO.	REV.	INSPECTION TEAM	LEGEND: A - ACCEPTABLE D - DEVIATION EXLSTS N/A - NOT APPLICABLE
CHANGE DOCUMENT			

CHECKLIST ITEMS	STATUS			COMMENTS
	N/A	A	D	
<b>1. General</b>				
A. Pipe Size				
B. Piping Location				
C. Piping Dimensions				
D. Wall Penetrations/Clearances				
<b>2. Fittings &amp; Components</b>				
A. Elbows				
B. Tees				
C. Valves				
D. Reducers				
E. Flanges				
F. Couplings				
G. Other Equipment				
<b>3. Fillet Weld Size Socket Connections</b>				
<b>4. Miscellaneous (Specify)</b>				

NOTE: Support data verified on support review checklists



ATTACHMENT B - ISOMETRIC CHECKLIST (continued)

---

Notes:

---

Results of Evaluation:

\_\_\_\_\_ No deviations noted.

\_\_\_\_\_ Deviations evaluated as acceptable by inspection team.

\_\_\_\_\_ Deviations require further evaluation.

\_\_\_\_\_ Signature

\_\_\_\_\_ Date

Disposition of deviations subject to further evaluation:

\_\_\_\_\_ Signature

\_\_\_\_\_ Date



ATTACHMENT C

INSPECTION AND EVALUATION GUIDELINE - ACTIVITY SEQUENCE

INSPECTION SEQUENCE

1. A support or isometric is assigned to an inspection team and recorded in the activity log.
2. The inspection team compares the 'as installed' (field) condition with the as-built revision of the sketch.
3. Deviations outside the SWEC tolerances are clearly detailed on the checklist, a mark-up of the sketch, or an added sketch as required.
4. Those deviations which can be judged acceptable by the inspector with a minimum of evaluation are listed with a brief explanation of the basis for judgement under 'deviations evaluated as acceptable by inspection team'.
5. Those deviations which require more extensive analysis to evaluate are listed under 'deviations subject to further evaluation'.
6. The checklist is signed and dated by the engineer/inspector and submitted to the inspection task engineer.
7. After review by the inspection task engineer, the checklist information is recorded in the PHIS database and refiled.

EVALUATION SEQUENCE

1. The log is reviewed for supports or isometrics requiring further evaluation. The data packages are pulled from the files and a cursory review is performed to determine if Burns and Roe calculations are required.
2. Data packages are signed out to an analyst and log and data base are updated.
3. Following an analysis the evaluations are routed to a reviewer.
4. Reviewed evaluations are separated into categories based on their final disposition. The log and data base are then updated and the data packages returned to files.



SUPPORT INSPECTION GUIDELINECHECKLIST  
ITEM

## ATTRIBUTE DESCRIPTION/TOLERANCE

- 1A SUPPORT LOCATION  
Hanger location to be checked with respect to work points on the piping using the as-built isometric as a reference.  
Tolerance: larger of  $\frac{\text{pipe O.D.}}{2}$  or 2"
- 1B SUPPORT ORIENTATION  
Compare the as-installed with that indicated on the as-built.  
Tolerance:  $\pm 5^\circ$
- 1C CATALOG ITEMS (Including snubber and strut sizes).  
Ensure that installed items match catalog, data and bill of materials.
- 1D Close clearance gaps (in the restrained direction)  
Measure gaps between restraining members or clamps and pipe surface/lugs.  
Tolerance: 1. Where individual gaps =  $\frac{1''}{16}$  on the drawings, then  

$$\frac{1''}{32} \leq \text{total gap} \leq \frac{5''}{32}$$
Where total gap = sum of gaps in any restrained direction  
2. Deadweight restraint - 0 gap specified.  
No tolerance  
3. Other gaps:  $\pm \frac{1''}{32}$
- 2A CRITICAL DIMENSIONS  
Structural dimensions (not to include ref. dims, or dims)  
Tolerance: Dims  $< 5''$  -  $\pm 10\%$   
Dims  $> 5''$  - larger of  $\frac{1''}{2}$  or 4%
- 2B MEMBER SIZES, STRUCTURAL PLATES  
Not to include Base R's, gusset R's or lugs.  
Plate tolerance: Thickness  $+\frac{1''}{4}$ , -0". Cut dimensions  $+\frac{1''}{4}$   
Member sizes outside dims, nominal -0  
Wall thickness -0, +no limit



SUPPORT INSPECTION GUIDELINE

CHECKLIST ITEM	ATTRIBUTE DESCRIPTION/TOLERANCE
3A	STRUT/SNUBBER PIN TO PIN, SNUBBER SETTINGS Tolerance: Pin to pin - +3", -no limit Snubber cold set $\pm \frac{1"}{4}$
3B	PADDLE-PIN CONNECTIONS Inspection for compliance with 215 spec. Section 15R, exhibit 5.*
4A	BASEPLATE AND GUSSETT SIZES Tolerance: Same as 2B
4B	BOLT SIZE AND TYPE Verify visible characteristics.
4C	BOLT HOLE SPACING Tolerance: Spacing $\pm \frac{1"}{4}$ Edge distance $\pm \frac{1"}{8}$
4D	ATTACHMENT LOCATION Tolerance: $\pm \frac{1"}{4}$
4E	SPACING TO ADJACENT INSERTS Min. spacing = $10 \frac{1}{2}$ x dia. of largest bolt Min. edge distance = $5 \frac{1}{4}$ x bolt dia.
5	LUGS, BEARING SURFACE Tolerance: Lug dims: Thickness $+ \frac{1"}{4}, - 0"$ Cut sizes $\pm \frac{1"}{8}$

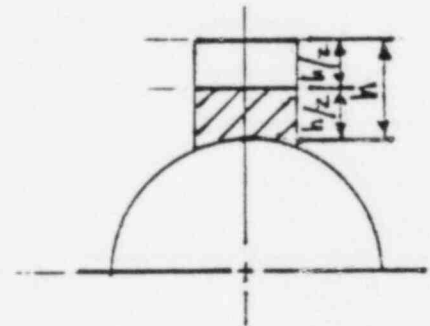


Figure A

Bearing surface: Min. of point contact between lug and restraint required within shaded area of figure A.

\*215 specification Sect. 15R - Procurement, Fabrication, and Erection of Pipe Supports W.O. 2808 Washington Public Power Supply System WPPS Nuclear Project No. 2 approval date Nov. 12, 1980.



SUPPORT INSPECTION GUIDELINE

CHECKLIST ITEM	ATTRIBUTE DESCRIPTION/TOLERANCE
6A & 6B	WELDING SIZE AND LENGTH, SYMBOLS
	Tolerance weld size +no limit, -0
	Weld length +no limit -10%

NOTE: Symbols and measurement criteria per 'as-built program' presentation by L. Cantin and printed handout from that presentation.

- 7 MISCELLANEOUS  
Include any deviations from the as-built not directly addressed in other attribute categories.



ATTACHMENT E

ISOMETRIC INSPECTION GUIDELINE

CHECKLIST  
ITEM

ATTRIBUTE DESCRIPTION/TOLERANCE

- 1A PIPE SIZE - OUTSIDE DIAMETER  
Tolerance: Use nominal dimension to confirm pipe size.
- 1B PIPING LOCATION  
Report only gross location discrepancies.
- 1C PIPING FABRICATION DIMENSIONS  
Tolerance:  $\pm 2''$
- 1D WALL PENETRATIONS/CLEARANCES  
Measure pipe O.D. to penetration I.D.  
Tolerance:  $\pm \frac{1}{8}''$  (dim.  $\leq 2''$ )  
  
 $\pm \frac{1}{4}''$  (dim.  $> 2''$ )
- 2A ELBOWS  
Check type - butt weld, socket, bend, threaded  
Check radius - short, long, 5D, and arc length  
(NOTE: Radius of pipe bends are 5 times the normal pipe diameter unless otherwise noted.)
- 2B TEES  
Check type - butt weld, socket, threaded, etc., as indicated on Bill of Material.
- 2C VALVES: CHECK THE FOLLOWING ITEMS:  
  
1. Type (gate, globe, check) and pressure rating.  
2. Operator type (manual, air, motor) and orientation.  
3. End to end dimension and type (socket, butt, threaded).
- 2D REDUCERS  
Check type (concentric vs. eccentric) and dims. (length).
- 2E FLANGES  
Check type and rating.
- 2F COUPLINGS  
Check type and rating.
- 2G OTHER EQUIPMENT (STRAINERS ETC.)  
Check to match equipment vs. Bill of Material.



ATTACHMENT E (CONT'D)

ISOMETRIC INSPECTION GUIDELINE

CHECKLIST  
ITEM

ATTRIBUTE DESCRIPTION/TOLERANCE

- |   |   |
|---|---|
| 3 | FILLET WELD SIZE - SOCKET CONNECTIONS<br>Check fillet size vs. piping spec.                       |
| 4 | MISCELLANEOUS<br>List deviations and concerns not identified by previous<br>attribute categories. |





ATTACHMENT F - CALCULATION FORM

EVALUATION OF SUPPORT NO. \_\_\_\_\_ REV. \_\_\_\_\_  
ISOMETRIC NO. \_\_\_\_\_ REV. \_\_\_\_\_

REFERENCES

PREPARER: \_\_\_\_\_ DATE: \_\_\_\_\_

REVIEWER: \_\_\_\_\_ DATE: \_\_\_\_\_

LARGE BORE SUPPORTS

Number of Large Bore Supports this Report - 340  
Number of Large Bore Supports with Deviations - 157

1	CHECKLIST ITEMS	NUMBER OF DEVIATIONS
1.	General	
	A. Support Location	7
	B. Support Orientation	2
	C. Catalog Items	17
	D. Close Clearance Gaps	7
2.	Support Structure	
	A. Critical Dimensions	23
	B. Member Sizes, Structural Plates	10
3.	Struts and Snubbers	
	A. Pin to Pin Dimensions, Snubber Setting	2
	B. Paddle-Pin Assembly Connections	6
4.	Baseplates	
	A. Plate & Gusset Sizes	3
	B. Bolt Size & Type	3
	C. Bolt Hole Spacing	15
	D. Attachment Location	17
	E. Bolt Spacing to Adjacent Inserts	5
5.	Lugs - Bearing Surface	4
6.	Welding	
	A. Size, Length, Quality	92
	B. Symbols	20
7.	Miscellaneous (Specify)	21



SMALL BORE PIPING ISOMETRICS

Number of Small Bore Isometrics this Report - 110  
Number of Small Bore Isometrics with Deviations - 26

<u>CHECKLIST ITEMS</u>	<u>NUMBER OF DEVIATIONS</u>
1. General	
A. Pipe Size	0
B. Piping Location	0
C. Piping Dimensions	3
D. Wall Penetrations/Clearances	1
2. Fittings & Components	
A. Elbows	1
B. Tees	0
C. Valves	4
D. Reducers	1
E. Flanges	3
F. Couplings	0
G. Other Equipment	3
3. Fillet Weld Size Socket Connections	10
4. Miscellaneous (Specify)	7



SMALL BORE SUPPORTS

Number of Small Bore Supports this Report - 184  
Number of Small Bore Supports with Deviations - 88

<u>CHECKLIST ITEMS</u>	<u>NUMBER OF DEVIATIONS</u>
1. General	
A. Support Location	1
B. Support Orientation	1
C. Catalog Items	15
D. Close Clearance Gaps	8
2. Support Structure	
A. Critical Dimensions	15
B. Member Sizes, Structural Plates	9
3. Struts and Snubbers	
A. Pin to Pin Dimensions, Snubber Setting	0
B. Paddle-Pin Assembly Connections	0
4. Baseplates	
A. Plate & Gusset Sizes	5
B. Bolt Size & Type	0
C. Bolt Hole Spacing	29
D. Attachment Location	19
E. Bolt Spacing to Adjacent Inserts	5
5. Lugs - Bearing Surface	2
6. Welding	
A. Size, Length, Quality	41
B. Symbols	5
7. Miscellaneous (Specify)	8



SMALL BORE UNIQUE SUPPORTS

Number of Small Bore Unique Supports This Report - 55  
Number of Small Bore Unique Supports with Deviations - 40

<u>CHECKLIST ITEMS</u>	<u>NUMBER OF DEVIATIONS</u>
<u>1. General</u>	
A. Support Location	1
B. Support Orientation	0
C. Catalog Items	4
D. Close Clearance Gaps	4
<u>2. Support Structure</u>	
A. Critical Dimensions	13
B. Member Sizes, Structural Plates	4
<u>3. Struts and Snubbers</u>	
A. Pin to Pin Dimensions, Snubber Setting	2
B. Paddle-Pin Assembly Connections	6
<u>4. Baseplates</u>	
A. Plate & Gusset Sizes	4
B. Bolt Size & Type	0
C. Bolt Hole Spacing	9
D. Attachment Location	4
E. Bolt Spacing to Adjacent Inserts	2
<u>5. Lugs - Bearing Surface</u>	0
<u>6. Welding</u>	
A. Size, Length, Quality	12
B. Symbols	2
<u>7. Miscellaneous</u>	9



TABULATION OF WELD DEVIATIONS

Total Supports <sup>1</sup>	-	141
Approximate No. of Welds	-	1150
No. of Weld Deviations	-	233
No. of Weld Deviations on Primary Load Path	-	204
<u>Deviation Type</u>		
Fillet Weld Leg Size	-	192
Excessive Root Gap	-	8
Configuration (missing or short lengths)	-	33

NOTES: 1 Total supports include large bore, small bore and small bore unique supports that had one or more weld deviations.



ENGINEERING ASSURANCE EVALUATION OF THE AS-BUILT  
PROGRAM FOR WASHINGTON PUBLIC POWER SUPPLY SYSTEM WNP UNIT 2

<u>CRITERIA EVALUATED</u>	<u>OBSERVATION &amp; *DISPOSITION</u>
1. Is there a written program or procedure that clearly describes the WPPSS As-Built program?	1. The procedures that describe the program are: <ul style="list-style-type: none"><li>• The Supply System (the Utility) Project Instruction, PMI-6-6, "As-Building Program Requirements"</li><li>• Burns &amp; Roe, Inc. (BRI) (the A/E) Project Instruction, WNP-2-065, Rev. 0, "Final As-Built Verification for Quality Class I Piping and Pipe Supports"</li><li>• Bechtel Power Corporation (BPC) (the Constructor) Quality Control Instruction, QC1-R1-1.00, Rev. 2 "Field Engineering As-Built Drawings"</li><li>• BPC Specific Work Plan/Procedure, SWP/P-P-6, "As-Building Program".</li></ul> *Continue Compliance.
2. Do the procedures specify what piping systems are applicable to this program?	2. The list of applicable piping systems was prepared by BRI and approved by the Supply System. However, this list has been superseded by addenda to the appropriate specifications. This serves as a cross-check for engineering as the Construction procedures apply to "all Quality Class I systems". *Continue Compliance.
3. Do the procedures identify what constitutes a design "package", i.e., specifications, diagrams, calculations, drawings, and how the latest issue (including all design change systems) is identified?	3. The procedures require evaluation against the latest design document. This is accomplished by BPC using the latest known drawing that was used to construct the support, and documents any changes to it during the walkdown. BRI uses the calculation, the latest drawings and changes, and any differences noted by Construction. *Continue Compliance.
4. Do the procedures identify which components of the piping system are to be included? Are all supports included or only seismic?	4. The procedures do identify which components are to be included in the program. However, to make the most efficient use of the time available, this review concentrated on pipe supports as they appeared to be of the most concern to the CAT. It should be noted that this program deals with Quality Class 1 systems only. *Continue Compliance.



CRITERIA EVALUATED

5. Do the procedures specify the responsibilities of, and the interface between, the Utility, the A/E, and the Constructor?

OBSERVATION &  
\*DISPOSITION

5. The Supply System procedure delegates the responsibility for the performance of this task to the A/E and the Constructor.

The BRI procedure describes the documentation flow and the tasks performed by the A/E. Interface with the Supply System and the Constructor however, are discussed only minimally.

\*Compliance with ASME III requirements could be more readily demonstrated if the procedures were revised to show more interface with the engineers.

Although the BPC QC procedures adequately describe the documentation required to show that construction is complete, they allow the process to continue if construction is incomplete as long as it is documented.

\*Continue implementing the procedures for tracking support drawings with temporary items until final installation.

The BPC field walk procedures concentrate on obtaining all possible measurements of any given support, and document how discrepancies are reported to the A/E. They do not however, refer to any engineering requirements and do not provide for ample interface with the A/E.

\*Compliance with ASME III requirements could be more readily demonstrated if the procedures were revised to show more interface with the engineers.

Although a QA Master Audit Plan identifies continuous monitoring of the as-built program, the monitoring primarily focuses on a second check of red line drawings. A more comprehensive audit program that evaluates the adequacy and determines the effectiveness of the overall as-built program might have identified problems and obtained corrective measures earlier in the program (i.e. consistent application of tolerances used during inspection).

\*Continue the monitoring program. Should the Supply System perform future as-built efforts, expand the monitoring activities to include comprehensive audits of all facets and interfaces that comprise the entire as-built program.





CRITERIA EVALUATED

OBSERVATION &  
\*DISPOSITION

6. Do the procedures identify all of the parameters/criteria which are necessary to ensure that the plant is in agreement with the design; are reasonable tolerances included; is there a checklist to convey the principle points to the people performing the inspection?
6. Although both the A/E and the Constructor's procedures contain checklists, the direction to the persons doing the field walk is to review all dimensions as opposed to identifying the critical dimensions. All differences are noted by red-lining the drawings. Construction tolerances are applied by the system engineer to determine whether the noted differences are acceptable or if they need to be evaluated by BRI.  
\*Should the Supply System perform future as-built efforts, it would be beneficial to identify the critical parameters.
- There were inadequate tolerances established for the as-built inspection to determine whether the noted measurements were within reasonable limits for repeatability. Procedures have now been revised to include this tolerance.  
\*Continue Compliance.
7. Does the procedure require documentation of the as-built inspection?
7. The procedure requires the as-built inspection to be documented on a checklist and the completed checklist is filed in the documentation center to identify the status of each support.  
\*Continue Compliance.
8. Are there requirements for documentation of discrepancies?
8. The as-built program contains requirements for documenting discrepancies. The as-built inspectors record all differences between the dimensions they obtain and what the drawing calls for by red-lining the drawing.
- The Constructor's system engineers cross out all discrepancies which are within construction tolerances or which are accounted for in other design documents.
- All other discrepancies are reported on an RFI to the A/E for evaluation.  
\*Continue Compliance.



CRITERIA EVALUATED

OBSERVATION &  
\*DISPOSITION

- |  |  |
|--|--|
| 9. Are there requirements for documentation of the engineering evaluation to determine whether the design documents or the hardware should be modified to resolve the discrepancies? | 9. The Request for Information (RFI) form is the documentation of the discrepancy and of the engineering evaluation. In addition the Project Engineering Directive (PED) is issued to show any additional work that may be required.<br>*Continue Compliance.  |
| 10. Is there a system to control re-entry into a piping system after the as-built process is started?  | 10. The BPC procedure governs rework during the as-built process and start-up procedures govern afterwards.<br>*Continue Compliance.   |
| 11. Is there a means of identifying description of changes and approval signature for revisions to procedures?   | 11. Each of the procedures that has been revised used margin indicators to show current changes and had approval signatures for the revisions.<br>*Continue Compliance.  |
| 12. What is the basis for the As-Built effort?   | 12. Review of the procedures noted in item 1 above, discussions with responsible personnel, and review of documentation packages for completed work, such as SLC-903N and FPC-211, indicated that the principle thrust of the as-built program apparently was to resolve a previous NRC observation by shoring up the construction QA effort to ensure that the construction was in compliance with applicable drawings.<br>The comparison of the as-built condition with the design analysis was also a function of the program.<br>*Continue Compliance. |



# STONE & WEBSTER ENGINEERING CORPORATION



1200 JADWIN AVENUE, SUITE 455  
RICHLAND, WASHINGTON 99352

BOSTON  
NEW YORK  
CHERRY HILL, N.J.  
DENVER  
CHICAGO  
HOUSTON  
PORTLAND, OREGON  
WASHINGTON, D.C.

Mr. Hugh A. Crisp  
Manager of Construction, WNP-2  
Washington Public Power Supply System  
3000 George Washington Way  
Richland, Washington 99352

October 18, 1983  
SW-WPPSS-045L-RH  
J.O. No. 14420.03  
14420.04

Dear Mr. Crisp:

QCI AND QCII/SCI PIPE SUPPORTS -  
WASHINGTON PUBLIC POWER SUPPLY SYSTEM  
MEETING WITH THE NRC AND STONE & WEBSTER  
ENGINEERING CORPORATION

During a meeting held on October 14, 1983, between the Nuclear Regulatory Commission (NRC), Washington Public Power Supply System (Supply System), and Stone & Webster Engineering Corporation (SWEC), the NRC requested additional information from the Supply System on QCI and QCII/SCI Pipe Supports. This information was with respect to the third-party review performed by SWEC. The request was for examples of SWEC inspection checklists and evaluations representing types of deviations and "minimum margin" conditions between the effects of deviations and project allowables.

Attachment A is a list of QCI and QCII/SCI Pipe Supports which describes these examples.

Attachment B includes examples of checklists and evaluations of deviations.

If you have any questions or need additional information, please do not hesitate to contact me at (509) 943-8392.

Very truly yours,

*R. K. Westfahl* by *A. Dennis* (per phone approval)  
R.K. Westfahl  
Senior Project Manager

RKW:akd

cc: W.K. Stockdale      WPPSS  
T. Bostrom              Bechtel

WPPSS WNP-2

SWEC's ENGINEERING INSPECTION AND EVALUATION OF PIPE SUPPORTS

Supplemental Information to the Final Report

<u>QCI Support</u>	<u>Description</u>
CAC-116	- Typical weld deviation
DE-906N	- Typical baseplate deviation - Least margin for anchor bolts
DO-908N	- Typical close clearance gap deviation
COND-591	- Typical catalog deviation
FPC-43	- Typical member size deviation
LPCS-908N	- Typical weld deviation - Least margin for welds
MSLC-31	- Typical critical dimension deviation
RCIC-15	- Typical anchor bolt deviation
RFW-177	- Typical critical dimension deviation
RWCU-1C-2PS	- Typical catalog, weld and critical dimension deviations
SGT-901N	- Typical baseplate deviation
VR-5	- Typical weld root gap deviation
<u>QCII/SCI Support</u>	<u>Description</u>
TSW-355	- Support location, bolt spacing, attachment location, weld size and length deviations - Least margin, anchor bolts

SWEC ENGINEERING REVIEW CHECKLIST

ATTACHMENT B  
 SW-WPPSS-045L-RH  
 TOTAL NO. OF PAGES - 87

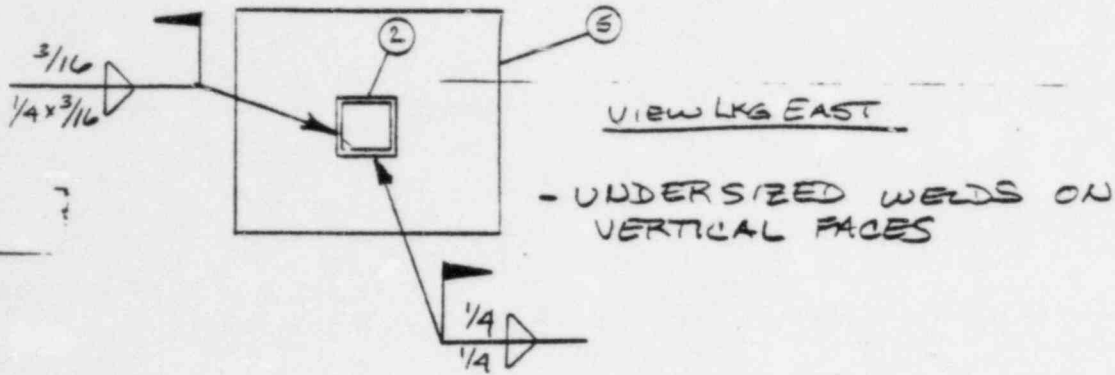
DRAWING NO. <b>CAC-116</b>		REV. <b>3</b>	INSPECTION TEAM <b>CARTER</b> <b>H DIEZ</b>
ISOMETRIC NO. REV. <b>9PΦ</b>	CHANGE DOCUMENT		
<b>CAC-628-5,8</b>			

A - ACCEPTABLE  
 D - DEVIATION EXISTS  
 N/A - NOT APPLICABLE

CHECKLIST ITEMS	STATUS			COMMENTS
	N/A	A	D	
<b>1. General</b>				
A. Support Location		✓		
B. Support Orientation		✓		
C. Catalog Items	✓			
D. Close Clearance Gaps		✓		
<b>2. Support Structure</b>				
A. Critical Dimensions		✓		
B. Member sizes, structural plates		✓		
<b>3. Struts and Snubbers</b>				
A. Pin to Pin Dims., Snubber Setting	✓			
B. Paddle - Pin Assembly Connections	✓			
<b>4. Baseplates</b>				
A. Plate & Gusset sizes		✓		
B. Bolt size & type		✓		
C. Bolt hole spacing		✓		
D. Attachment location		✓		
E. Bolt spacing to adjacent inserts		✓		
5. Lugs - Bearing Surface	✓			
<b>6. Welding</b>				
A. Size, length, quality			✓	
B. Symbols		✓		
7. Miscellaneous (specify)	✓			

SWEC ENGINEERING REVIEW CHECKLIST

Notes:



Results of Evaluation:

N/A No deviations noted.

N/A Deviations evaluated as acceptable by inspection team.

-1- Deviations require further evaluation: 6A

Thomas Carter 30 JUL 83  
Signature Date

Disposition of deviations subject to further evaluation:

ABOVE DEVIATION IS FOUND TO BE ACCEPTABLE.

SEE ATTACHED EVALUATION SHEET.

426

E. de Guzman 8/9/83  
Signature Date

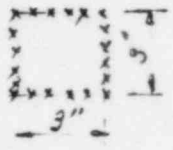
EVALUATION OF SUPPORT NO. CAC-116 REV. 3  
ISOMETRIC NO. CAC-628-5.8 REV. 9FD

NOTES: 1) BURNS & ROE DWGS. # M-200 SHT 169 HAS BEEN REVIEWED FOR DISCREPANCIES BETWEEN AS-ANALYZED & AS-BUILT PIPING CONFIGURATIONS. IT IS JUDGED THAT ANY SUCH DISCREPANCIES DO NOT SIGNIFICANTLY AFFECT CALCULATED SUPPORT LOADS.

MODIFIED LOAD

$$F = (100 + 1947 + 1315) 1.25 = 4203 \#$$

ANALYSIS / CONCLUSION



$$f = \frac{4203}{12} = 350$$

$$w = \frac{350}{18,000 \times .707} = 0.028$$

← CONSERVATIVE

MIN. REQUIRED (PER AISC) = 3/16"

∴ DEVIATION 6A IS ACCEPTABLE.

REFERENCES

- ① ISO. CAC-628-5.8 REV. 9FD
- ② ISO. M200, SHT. 169, REV. 4
- ③ HANGER DWG. CAC-116 REV. 3

PREPARED: E. de Guzman 8/9/83  
REVIEWED: G. Johnson 8/27/83

SWEC ENGINEERING REVIEW CHECKLIST

HANGER DRAWING NO. <b>DE-906N</b>	REV. <b>1FO</b>	INSPECTION TEAM <b>DAVIS EWART</b>	LEGEND: A - ACCEPTABLE D - DEVIATION EXISTS N/A - NOT APPLICABLE
ISOMETRIC NO. <b>REV. 3FO</b>	CHANGE DOCUMENT		
<b>DE-061-1.1S</b>			

CHECKLIST ITEMS	STATUS			COMMENTS
	N/A	A	D	
<b>1. General</b>				
A. Support Location		✓		
B. Support Orientation		✓		
C. Catalog Items	✓			
D. Close Clearance Gaps	✓			
<b>2. Support Structure</b>				
A. Critical Dimensions		✓		
B. Member sizes, structural plates		✓		
<b>3. Struts and Snubbers</b>				
A. Pin to Pin Dims., Snubber Setting	✓			
B. Rod-to-Pin Assembly Connections	✓			
<b>4. Baseplates</b>				
A. Plate & Gusset sizes		✓		
B. Bolt size & type		✓		
C. Bolt hole spacing			✓	I SEE NOTE-4
D. Attachment location		✓		
E. Bolt spacing to adjacent inserts			✓	I SEE NOTE-3
5. Lugs - Bearing Surface	✓			
<b>6. Welding</b>				
A. Size, length, quality			✓	SEE NOTE-5, 6, 7
B. Symbols			✓	
7. Miscellaneous (specify)				
<del>GAPS (SLIPPERY PIPES)</del>	✓			



SWEC ENGINEERING REVIEW CHECKLIST

Notes: 1. VOID

2. ITEM 15 BILL OF MATERIALS ATTACHMENT LOCATION ON BASE PLATE IS 1" WEST OF DESIGNATED POSITION ON AS-BUILT ISO. (RESTRAINT)
3. 7" DIMENSION SPECIFIED <sup>(edge to bolt)</sup> BETWEEN BASE PLATES DENOTED ON BILL OF MATERIALS AS ITEMS NO. - 7 OF AS-BUILT (Pg 3 of 6) IS INCORRECT. THE DIMENSION BETWEEN ADJACENT BOLT INSERTS WAS MEASURED AS 7". THE AS-BUILT ISO. SPECIFIES THE DIMENSION AS 9 3/4" (7" + 2 3/4"). INSERT ALLOWABLES SHOULD BE ASSESSED

Results of Evaluation:

N/A No deviations noted.  
 \_\_\_\_\_ Deviations evaluated as acceptable by inspection team.

5 Deviations require further evaluation.

R. Ewart 8-8-83  
 Signature Date

Disposition of deviations subject to further evaluation:

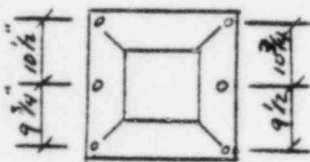
SEE ATTACHED SHTS. FOR ANALYSIS OF DEVIATIONS

494

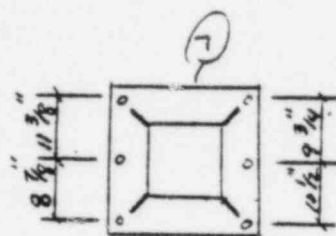
JOE STOWS 8-31-83  
 Signature Date  
S. Collins 8-21-83

ATTACHMENT TO THE  
SWEC ENGINEERING CHECK LIST

4. Pg 3 of 6 As-built Dwg. BASE plate UPPER right. DEVIATION BOLT HOLE SPACING

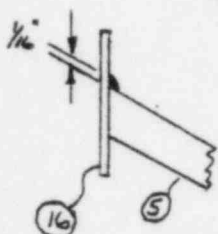


DIM.  
SHOWN ON  
AS-BUILT



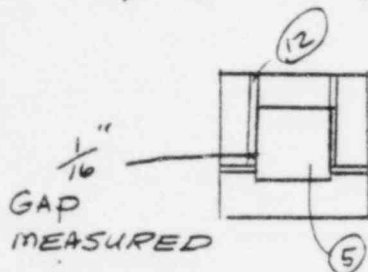
DIMENSIONS  
MEASURED IN  
FIELD

5. Pg-4 of 6 SECTION C-C, ITEM-5, skewed upper left. AS-BUILT ISO. SPECIFIES A  $1/16$ " THICKNESS FOR A PENETRATION WELD. THE WELD IS NOT MEASURABLE IN THE FIELD (ITS FLUSH).



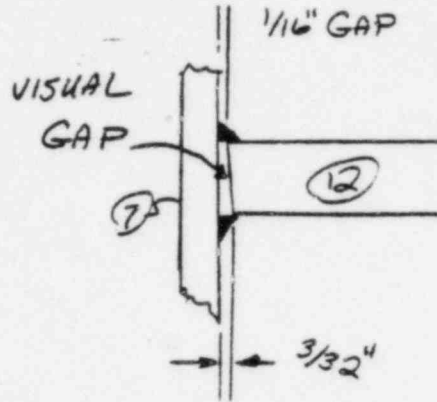
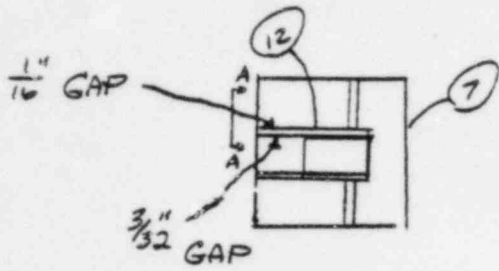
"AS-BUILT" (SHOWS  $1/16$ " THICKNESS)

6. A  $1/16$ " GAP WAS MEASURED BETWEEN BILL OF MATERIAL ITEMS 5 & 12 THIS GAP CAUSES THE EFFECTIVE THROAT OF THE TO BE REDUCED. (Pg 4 of 6 SECTION D-D)



ATTACHMENT TO THE SWEC  
ENGINEERING CHECK LIST

7. A  $\frac{3}{32}$ " GAP WAS MEASURED ON THE  
BOTTOM OF ITEM-12. A  $\frac{1}{32}$  inch GAP  
WAS MEASURED BETWEEN STIFFNER  
PLATE AND BASE PLATE. LOCATION  
TOP OF STIFFNER PLATE.



THE EFFECTIVE  
THROAT OF  
THE  $\frac{5}{16}$ " FILLETS  
SHOULD BE  
REDUCED TO  
ACCOUNT FOR  
THE GAP

SECTION A-A

EVALUATION OF SUPPORT NO. DE-906N REV. 1F  
 ISOMETRIC NO. DE-061-1.15 REV. 3F

- NOTES: 1) Burns & Roe Drawing #M-200 Sheet N/A has been reviewed for discrepancies between as-analyzed and as-built piping configurations. It is judged that any such discrepancies do not significantly affect calculated support loads.
- 2) PIPE STRESS ISOMETRIC IS NOT AVAILABLE. PER AS-BUILT ISO, NO SIGNIFICANT CHANGE HAS BEEN NOTED BETWEEN AS-BUILT AND "ISSUED-FOR-CONSTRUCTION" STATUS. A FACTOR OF SAFETY OF 1.25 IS USED ON FINAL LOADS.
- 3) CRITICAL DEVIATIONS ARE CONSIDERED ONLY. THESE ARE THE WELD SIZE DEVIATIONS @ THE STIFFENERS. TO ANALYZE THESE, LOADS AT THE BASEPLATE ARE DETERMINED FROM A SIMPLE STRENGTH ANALYSIS. PLATE STRESSES AND BOLT LOADS ARE THEN CHECKED PER STARDYNE ANALYSIS BY ELIMINATING THE STIFFENER IN QUESTION. (SEE NOTE 7, ON CHECKLIST, SHT 2)

ATTRIBUTE 4.C) (NOTE 4, CHECKLIST, SHT 2) BOLT SPACING.

UNDER ASSUMPTION THAT ORIGINAL DESIGN OF BOLTS DETERMINED THAT ALL BOLTS (1"Ø) WERE ACCEPTABLE BELOW ALLOWABLES AND THAT APPROVAL PER PED 215-14-M294 FROM AS-BUILT DWG. SHT 3 OF 6 ACCEPTS SPACING OF 9 1/2", IT CAN BE ASSUMED THAT THE ACTUAL LOADS ARE SIGNIFICANTLY LOW ENOUGH TO ALLOW 9 3/4" SPACING. MIN. SPACING FOR 1" BOLTS IS RECOMMENDED @ 10" BECAUSE STIFFENERS ON BASEPLATE WILL TAKE THE LOADS MORE DIRECTLY TO THE CORNER BOLTS THE FOUR INSIDE BOLTS WILL CARRY MUCH LESS ACTUAL LOAD THAN THE CORNER BOLTS. ∴ DEVIATION ACCEPTABLE (BOLT SPACING PER REF 4). ALSO REFER TO STARDYNE RUN TO SEE ACTUAL INTERACTION ON THE BOLTS (MAX. INTERACTION = 0.66)

REFERENCES

- 1) PIPE SUPPORT DWG. DE-906N, REV. 1F
- 2) PIPING ISOMETRIC DE-061-1.15, REV. 3F
- 3) B&R STATUS AS-BUILT SUPPORT LOAD CALC. B.42.152
- 4) B&R DESIGN GUIDE M1409.

PREPARER: JOE STOLUS DATE: 8-31-83

REVIEWER: [Signature] DATE: 8-31-83

EVALUATION OF SUPPORT NO. DE-906N REV. 1FD  
 ISOMETRIC NO. DE-061-1.15 REV. 3FD

ATTRIBUTE 4E) (NOTE 3, CHECKLIST, SHT. 1) BOLT SPACING

DEVIATION PRIMARILY DRAFTING ERROR. ORIGINAL "ISSUED FOR CONSTRUCTION" DWG SHOWS 7" BOLT SPACING (SEE DWG. SHT. 2 OF 6). SINCE THERE IS NO DEVIATION FROM ORIGINAL DESIGN, BY ENGINEERING JUDGEMENT, THIS DEVIATION IS ACCEPTABLE. (ALSO SEE STRESSLINE RUN FOR BOLT INTERACTION; I.E. MAX INTERACTION = .66)

ATTRIBUTE 6A) (NOTES 5 AND 6, CHECKLIST, SHT. 2) WELD DEVIATIONS

NOTE 5) DEVIATION PRIMARILY DRAFTING ERROR. ORIGINAL "ISSUED FOR CONSTRUCTION" DWG. SHOWS BEVEL END PREPARATION REQ'D. (SEE DWG. SHT. 2 OF 6, SECT. B-B). SINCE END PREPARATION CANNOT BE VERIFIED, DEVIATION IS ASSUMED ACCEPTABLE.

NOTE 6) ORIGINAL "ISSUED FOR CONSTRUCTION" DWG. CALLS FOR <sup>6 (DWG. SHT 2 OF 6)</sup> 1/4" FILLETS. 1/16" GAP WILL LOWER EFFECTIVE WELD LEG FROM 5/16" (PER AS-BUILT) TO 1/4" LEG. BY ENGINEERING JUDGEMENT, THIS DEVIATION IS ACCEPTABLE.

REFERENCES

PREPARER: JOE STUWS DATE: 8-31-83

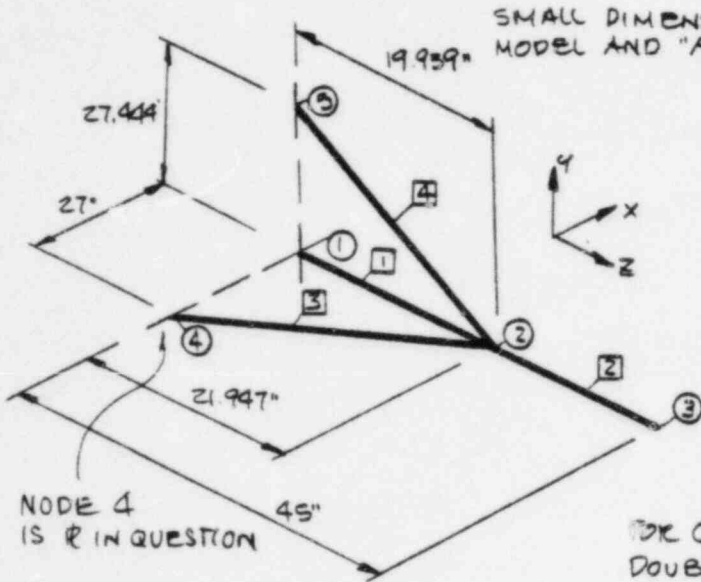
REVIEWER: [Signature] DATE: 8-31-83

EVALUATION OF SUPPORT NO. DE-906N REV. 1F  
 ISOMETRIC NO. DE-061-1.15 REV. 3F

ATTRIBUTE 6A) (NOTE 7, CHECKLIST SHT. 2) WELD DEVIATION

WELD ANALYSIS - INSTEAD OF CHECKING WELOSIZE DETERMINE IP STRESSES AND BOLT LOADS W/OUT THE STIFFENER IN QUESTION.

DETERMINE LOADS ON BASEPLATE - GENERATE STEUDL MODEL (SEE ATTACH. 1.0)



SMALL DIMENSIONAL DIFFERENCES MAY EXIST BETWEEN MODEL AND "AS-BUILT". THESE ARE ASSUMED INSIGNIFICANT.

MEMBER PROPERTIES

TS 10x10x1/2 (MEM 1 2) (REF 5)  
 TS 8x8x1/2 (MEM 3 4) (REF 5)

CONSTANTS

E = 29,000,000 PSI  
 G = 11,200,000 PSI  
 POISSON'S = 0.3  
 } REF 6

MAX LOADS (DESIGN) (REF 5)

FOR CONSERVATIVE ANALYSIS MOMENTS ARE DOUBLED.

	FX	Fy	Fz	(FT-LBS) Mx	(FT-LBS) My	(FT-LBS) Mz
DEADWT.	116	-982	-118	-494	439	-253
NORMAL	+1815	+1206	+1148	+12514	+13066	+32530
	+1925 ①	-2188	+1030	-156096	+162060 ②	-393396
	-1706 ②				-151524 ①	

① MOMENTS IN IN-LBS

REFERENCES

- 5.) S & W "STEUDL-SW USERS MANUAL" STEUDL, ST-316 VERSION 03 LEVEL 01.
- 6.) AISC "STEEL CONSTRUCTION MANUAL" 8TH EDITION.

PREPARER: Joe Stoker DATE: 8-31-83  
 REVIEWER: S. G. Little DATE: 8-31-83

EVALUATION OF SUPPORT NO. DE-906N REV. 1F  
 ISOMETRIC NO. DE-061-1.15 REV. 3F

GENERATION OF STARDYNE MODEL (BASEPLATE IS LOCATED @ STRUDL NODE 4)

LOAD RESULTS FROM STRUDL (APPLY A MODIFIED LOAD FACTOR OF 1.25 TO STRUDL LOADS; STARDYNE 'X' = STRUDL '4'; STARDYNE 'Y' = STRUDL 'X'; STARDYNE 'Z' = STRUDL 'Z')

LOAD CASES 1 AND 2 (STARDYNE) (PER STRUDL RESULTS - LOADING 1)

	<u>CASE 1</u>	<u>CASE 2</u>	
$F_x = 1.25(4839) = 6049$ LBS	+6074	-6074	LOADS OF 6074* IS CONSERVATIVE
$F_y = 1.25(8653) = 10817$ LBS	+10817	+10567	
$F_z = 1.25(8531) = 10664$ LBS	+10664	+10664	(SHEAR LOAD DISCREPANCY NEGUGIBLE)
$M_x = 1.25(2315) = 2894$ IN LBS	+2894	-2894	
$M_y = 1.25(55724) = 69654$ IN LBS	+69654	+69654	
$M_z = 1.25(13135) = 16419$ IN LBS	-16419	-16419	

LOAD CASES 3 & 4 (STARDYNE)

	<u>CASE 3</u>	<u>CASE 4</u>	<u>CONCRETE ALLOW.</u>
$F_x = 1.25(5117) = 6397$ LBS	+6397	-6397	ASSUME COMPRESSIVE STRENGTH = 3500 PSI
$F_y = 1.25(1063) = 1330$ LBS	+1330	-1330	
$F_z = 1.25(1870) = 2338$ LBS	+2338	+2338	
$M_x = 1.25(19274) = 24093$ IN LBS	-24093	+24093	
$M_y = 1.25(57188) = 71485$ IN LBS	+71485	+71485	
$M_z = 1.25(13060) = 16327$ IN LBS	-16327	-16327	

BOLT PROPERTIES 1" H.S.K.

TENSILE STIFF = 1057000 LB/IN } REF. 7  
 SHEAR STIFF = 500000 LB/IN }

TENSION ALLOW. = 8740 LBS } REF. 4  
 SHEAR ALLOW. = 6890 LBS }

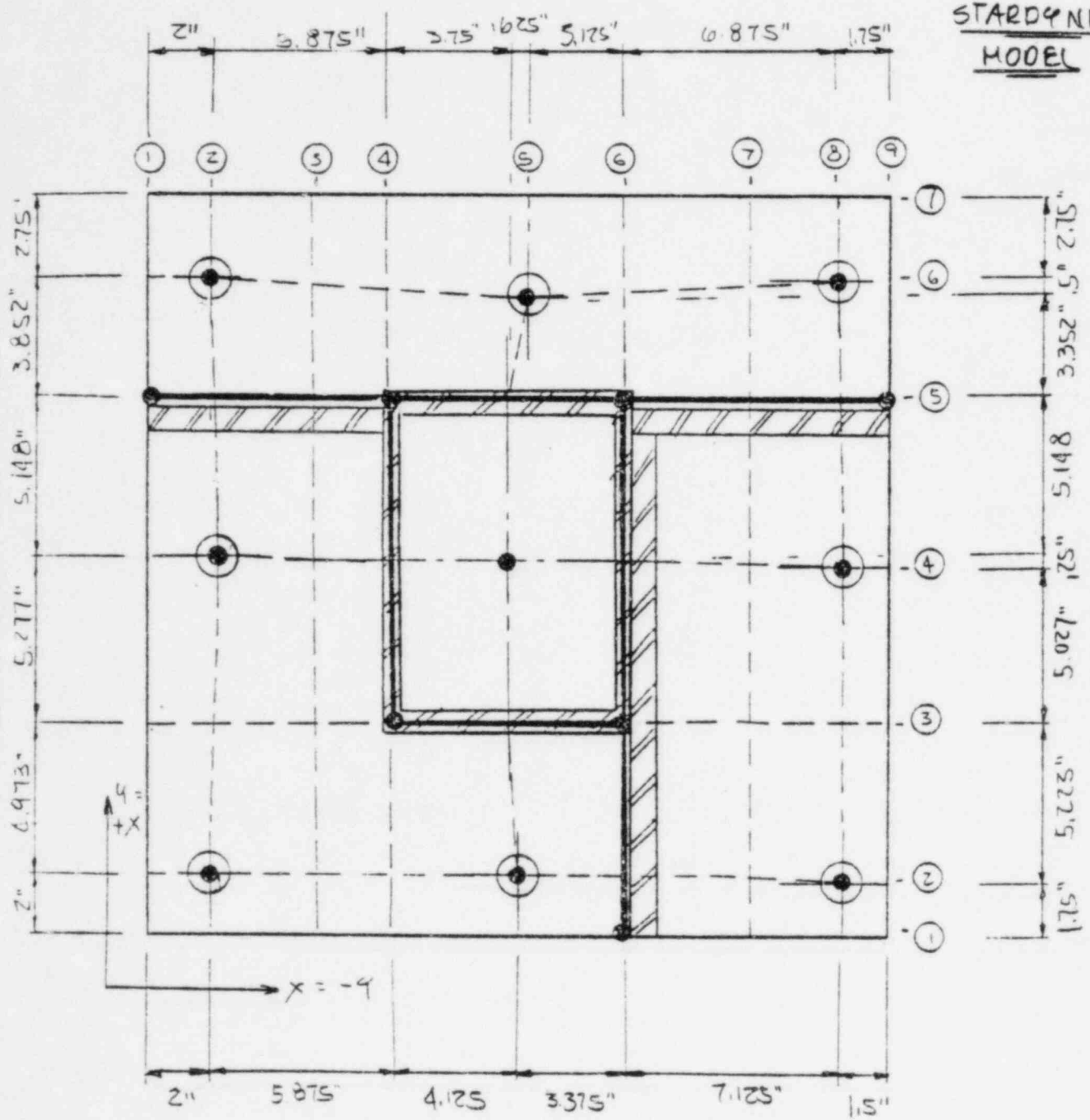
REFERENCES

- 7) B&R "BOOK OF TYPICAL BASEPLATES" (REV. 1); APRIL, 1982
- 8) CONTROL DATA CORP. "BASEPLATE II USERS MANUAL" REV. 1.05, STARDYNE COMPUTER PROGRAM ST-330, VERSION 03 LEVEL 00.

PREPARER: JOE STOKER DATE: 8-31-83  
 REVIEWER: [Signature] DATE: 8-31-83

EVALUATION OF SUPPORT NO. DE-906N REV. 1FD  
 ISOMETRIC NO. DE-061-1.16 REV. 3FD

STARDYNE  
MODEL



PREPARER: Joe Stouls DATE: 8-31-83  
 REVIEWER: A. J. G. [Signature] DATE: 8-31-83



EVALUATION OF SUPPORT NO. DE-306N R.V. 1F0  
 ISOMETRIC NO. DE-061-1.15 REV. 3F0

STAR DYNE RESULTS

MAX BASEPLATE STRESS = 4399 PSI

(REF STAR DYNE RUN, LOAD CASE 1)

MAX. ALLOWABLE = 27000 PSI (REF 4) > 4399 PSI ∴ O.K.

MAX. BOLT LOAD INTERACTION = 0.66 < 1.0 ∴ O.K. (INTERACTION PER REF 4)

BY ENGINEERING, DUE TO AMOUNT OF CONSERVATISM IN USING DOUBLE MOMENTS (NORMAL CONDITION) AND 1.25 FACTOR OF SAFETY, THE FAULTED CONDITION WILL ALSO BE ACCEPTABLE.

BY OBSERVATION OF NEW STRESSES, OBTAINED BY ELIMINATING DEVIATE STIFFENER, ON THE BASE PLATE, THE OTHER STIFFENERS AND THEIR WELDS WILL STILL BE O.K. (MAX STRESS = 4399 PSI)

REFERENCES

PREPARER: JOE STOLIN DATE: 8-31-83

REVIEWER: [Signature] DATE: 8-31-83

# STRUDL RESULTS

type sr      rname  
 units in lb deg  
 constants  
 e 22000000. all  
 p 11200000. all  
 poisson .3 all  
 joint-coordinates  
 1 0.0 0.0 0.0 s  
 2 0.0 0.0 21.95  
 3 0.0 0.0 45.0  
 4 -27.0 0.0 0.0 s  
 5 0.0 27.45 0.0 s  
 member incidences  
 1 1 2  
 2 2 3  
 3 4 2  
 4 5 2  
 member end joint size end 6.5  
 member properties prismatic  
 1 2 table sttubel 110x60.9'  
 3 4 table sttubel 18x47.35'  
 loading 1 faulted  
 joint load  
 3 force x -1925. y -2188. z 1030.  
 3 moment x -156096. y -151524. z -393396.  
 loading 2 faulted 2  
 joint load  
 3 force x -1705. y -2189. z 1030.  
 3 moment x -156096. y 162060. z -393396.  
 stiffness analysis reduce

SUPPORT NO. PE-900 N  
 ATTACH. PG. 1

\*\*\*\*\*  
 results of latest analyses  
 \*\*\*\*\*

problem - mess title - analysis of de-906n

active units inch lb deg deaf sec

active structure type space frame

active coordinate axes x y z

loading - 1 faulted

member forces

member	joint	axial	shear x	shear y	shear z	torsional	bending x	bending y	bending z
1	1	-10042.5	4927.1	4927.1	643.8	134812.9	52366.9	-1715.6	
1	2	10042.5	-4927.1	-4927.1	-643.8	-134812.9	-52366.9	109822.5	
2	2	-1030.0	2188.0	2188.0	-1925.0	393395.9	195895.2	-105662.6	
2	3	1030.0	-2188.0	-2188.0	1925.0	-393395.9	-195895.2	156095.9	
3	4	12095.2	-4838.2	-4838.2	1160.8	34952.6	-2314.9	-45342.2	
3	2	-12095.2	4838.2	4838.2	-1160.8	-34952.6	2314.9	45342.2	
4	5	-1338.3	1687.4	1687.4	6084.0	16789.5	-46613.6	13036.1	
4	2	1338.3	-1687.4	-1687.4	-6084.0	-16789.5	46613.6	-13036.1	

SUPPORT DE-906N  
 ATTACH PG. 2

resultant joint loads - supports

joint	x force	y force	z force	x moment	y moment	z moment
1	-843.8	4927.1	-10042.5	1715.6	52366.9	134812.9
4	8652.9	-4638.2	8530.4	55723.3	-2314.9	-13134.3
5	-6084.0	2099.0	482.1	-13036.1	-42223.9	-25920.1

loading - 2 faulted 2

member forces

member	axial	shear y	shear z	torsional	bending y	bending z
1	-109.7	5435.4	-5684.5	143761.9	3770.7	7585.5
1	107.7	-5435.4	5684.5	-143761.9	121003.7	111742.9
2	-1030.0	2188.0	-1705.0	393395.9	-122759.7	-105662.6
2	1030.0	-2188.0	1705.0	-393395.9	162059.9	156095.9
3	-354.6	-5117.0	-2121.4	36135.4	19273.5	-46208.1
3	354.6	5117.0	2121.4	-36135.4	54542.5	-131845.9
4	-866.5	1908.5	5042.5	36548.3	-47000.5	18027.2
4	866.5	-1908.5	-5042.5	-36548.3	-190229.2	-47050.3

resultant joint loads - supports

joint	x force	y force	z force	x moment	y moment	z moment
1	5684.5	5435.4	-109.7	-7585.5	3770.7	143761.9
4	1063.1	-5117.0	-1859.7	57187.3	19273.3	-13060.1
5	-5042.5	1868.6	949.4	-18027.2	-57897.3	-13882.5

SUPPORT NO. DE-906N  
ATTACH. R. 3

502C

QUADRILATERAL PLATE  
MAXIMUM PRINCIPAL STRESS SUMMARY

SUPPORT DE-906N  
ATTACHMENT PG. 4

ELEM	+ Z FACE SIGMA PRINC	/	ELEM	- Z FACE SIGMA PRINC
15	3705.	/	5	4399.
5	3010.	/	11	3899.

14	3400.	/	15	3424.
11	3438.	/	14	3251.
17	3047.	/	17	3111.
23	2098.	/	23	2373.
10	1967.	/	10	1055.
10	1892.	/	25	1045.
4	1754.	/	13	1605.
25	1601.	/	29	1604.

MAX. COMPRESSION SUMMARY

NODE	Z-FORCE
61	-331.
54	-213.
62	-153.
60	-141.
1	-135.
58	-88.
55	-78.
59	-65.
2	-65.
63	-44.

502 D

B O L T   L O A D S

I LINE	J LINE	NUDE	X-SHEAR	Y-SHEAR	SRSS	Z-TENSION
2	2	9	563.	1500.	1654.	802.
2	4	11	783.	1578.	1762.	5549.
2	6	13	958.	1581.	1848.	2772.
5	2	30	549.	1377.	1483.	2782.
5	6	34	960.	1577.	1678.	1709.
8	2	51	518.	1090.	1207.	200.
8	4	53	773.	1119.	1360.	154.
8	6	55	969.	1135.	1493.	0.

B O L T   I N T E R A C T I O N   E Q U A T I O N S

EQUATION=((TENSION/TALLOW)\*\* 1.000+(SHEAR/VALLOW)\*\* 1.000)\*\* 1.000

NUDE	TENSION ALLOW	SHEAR ALLOW	TENSION RATIO	SHEAR RATIO	INTERACTION FACTOR	SAFETY FACTOR
9	8740.	6890.	.099	.241	.339	2.95
11	8740.	6890.	.406	.256	.602	1.51
13	8740.	6890.	.317	.268	.585	1.71
30	8740.	6890.	.318	.215	.534	1.87
34	8740.	6890.	.196	.244	.439	2.28
51	8740.	6890.	.030	.175	.206	4.86
53	8740.	6890.	.018	.197	.215	4.05
55	8740.	6890.	0.000	.217	.217	4.02

\*\*\*\*\*  
\* E N U   L O A D   C A S E   1 \*  
\*\*\*\*\*

UNILATERAL PLATE  
MAXIMUM PRINCIPAL STRESS SUMMARY

+ Z FACE		/	- Z FACE	
ELEM	SIGMA PRINC	/	ELEM	SIGMA PRINC
15	3463.	/	5	3868.
5	3375.	/	11	3650.
14	3259.	/	15	3588.
11	3039.	/	14	3438.
17	2698.	/	17	2917.
23	1843.	/	23	2174.
4	1829.	/	25	2015.
10	1808.	/	13	1754.
10	1790.	/	10	1707.
25	1407.	/	29	1508.

MAX. COMPRESSION SUMMARY

NODE	Z-FORCE
01	-286.
02	-147.
1	-137.
54	-125.
00	-120.
58	-94.
55	-92.
2	-67.
59	-60.
14	-53.

B O L T   L O A D S

4

SUPPORT NO. DR-906N  
ATTACHMENT PG. 7

<u>I</u>	<u>J</u>	<u>NUDE</u>	<u>X-SHEAR</u>	<u>Y-SHEAR</u>	<u>SRSS</u>	<u>Z-TENSION</u>
<u>LINE</u>	<u>LINE</u>					
2	2	9	-683.	1452.	1700.	875.
2	4	11	-767.	1491.	1677.	3429.
2	6	13	-604.	1504.	1625.	2571.
3	2	30	-423.	1348.	1634.	2984.
5	6	34	-609.	1345.	1477.	1565.
8	2	51	-930.	1130.	1467.	541.
8	4	53	-761.	1140.	1375.	210.
8	6	55	-590.	1147.	1290.	0.

B O L T   I N T E R A C T I O N   E Q U A T I O N S

$$EQUATION = ((TENSION/TALLOW)** 1.000 + (SHEAR/VALLOW)** 1.000)** 1.000$$

<u>NUDE</u>	<u>TENSION</u>	<u>SHEAR</u>	<u>TENSION</u>	<u>SHEAR</u>	<u>INTERACTION</u>	<u>SAFETY</u>
	<u>ALLOW</u>	<u>ALLOW</u>	<u>RATIO</u>	<u>RATIO</u>	<u>FACTOR</u>	<u>FACTOR</u>
9	8740.	6890.	.100	.247	.347	2.86
11	8740.	6890.	.392	.243	.636	1.57
13	8740.	6890.	.294	.236	.530	1.89
30	8740.	6890.	.341	.237	.579	1.73
34	8740.	6890.	.179	.214	.393	2.54
51	8740.	6890.	.039	.213	.252	3.91
53	8740.	6890.	.024	.200	.224	4.47
55	8740.	6890.	0.000	.187	.187	5.34

\*\*\*\*\*  
\*   E N D   L O A D   C A S E   2   \*  
\*\*\*\*\*



QUADRILATERAL PLATE  
 MAXIMUM PRINCIPAL STRESS SUMMARY

SUPPORT DE-906N 5  
 ATTACHMENT P. 8

+ Z FACE		- Z FACE	
ELEM	SIGMA PRINC	ELEM	SIGMA PRINC
15	2292.	5	2143.
14	2180.	15	2033.
5	1833.	14	1997.
11	1794.	11	1790.
17	1472.	17	1378.
10	1131.	47	1109.
25	1071.	10	909.
47	1029.	4	930.
10	1007.	13	907.
41	997.	25	899.

MAX. COMPRESSION SUMMARY

NODE	Z-FORCE
54	-1490.
01	-045.
47	-439.
55	-435.
53	-224.
02	-205.
48	-170.
00	-140.
1	-79.
58	-40.

502 H

B O L T L O A D S

SUPPORT NO. DE-906N  
ATTACHMENT PG. 5

<u>I</u> <u>LINE</u>	<u>J</u> <u>LINE</u>	<u>NODE</u>	<u>X-SHEAR</u>	<u>Y-SHEAR</u>	<u>SRSS</u>	<u>Z-TENSION</u>
2	2	9	626.	349.	716.	543.
2	4	11	824.	342.	892.	2012.
2	6	13	962.	339.	1020.	1436.
5	2	30	653.	171.	655.	1624.
5	6	34	959.	168.	974.	575.
8	2	51	620.	-21.	621.	136.
8	4	53	812.	-15.	812.	0.
8	6	55	961.	-3.	961.	0.

B O L T I N T E R A C T I O N E Q U A T I O N S

$$\text{EQUATION} = ((\text{TENSION/TALLOW})^{**} 1.000 + (\text{SHEAR/VALLOW})^{**} 1.000)^{**} 1.000$$

<u>NODE</u>	<u>TENSION</u> <u>ALLOW</u>	<u>SHEAR</u> <u>ALLOW</u>	<u>TENSION</u> <u>RATIO</u>	<u>SHEAR</u> <u>RATIO</u>	<u>INTERACTION</u> <u>FACTOR</u>	<u>SAFETY</u> <u>FACTOR</u>
9	8740.	6890.	.062	.104	.166	6.02
11	8740.	6890.	.230	.130	.360	2.76
13	8740.	6890.	.164	.148	.312	3.20
30	8740.	6890.	.186	.095	.281	3.56
34	8740.	6890.	.066	.141	.207	4.83
51	8740.	6890.	.016	.090	.106	4.47
53	8740.	6890.	0.000	.118	.118	8.48
55	8740.	6890.	0.000	.139	.139	7.17

\*\*\*\*\*  
\* E N D L O A D C A S E 3 \*  
\*\*\*\*\*

QUADRILATERAL PLATE  
 MAXIMUM PRINCIPAL STRESS SUMMARY

SUPPORT DE-906N  
 ATTACHMENT PZ. 10

+ Z FACE		- Z FACE	
ELEM	SIGMA PRINC	ELEM	SIGMA PRINC
5	2951.	5	2817.
11	2609.	11	2672.
17	2107.	17	2226.
15	1650.	15	1923.
23	1508.	14	1564.
14	1326.	23	1554.
0	1018.	31	1411.
29	1008.	32	1185.
31	985.	29	1062.
32	959.	16	1040.

MAX. COMPRESSION SUMMARY

NODE	Z-FORCE
54	-862.
01	-455.
44	-422.
37	-310.
53	-299.
45	-236.
30	-150.
43	-134.
00	-129.
62	-122.

502 J

B O L T   L O A D S

8

SUPPORT NO. DE-906N

ATTACHMENT R. 11

<u>I</u>	<u>J</u>	<u>NODE</u>	<u>X-SHEAR</u>	<u>Y-SHEAR</u>	<u>SRSS</u>	<u>Z-TENSION</u>
<u>LINE</u>	<u>LINE</u>	<u>NODE</u>	<u>X-SHEAR</u>	<u>Y-SHEAR</u>	<u>SRSS</u>	<u>Z-TENSION</u>
2	2	9	-891.	-72.	894.	318.
2	4	11	-809.	-62.	812.	1927.
2	6	13	-693.	-50.	694.	1918.
5	2	30	-907.	-165.	922.	389.
5	6	34	-704.	-170.	724.	1139.
8	2	51	-875.	-259.	932.	0.
8	4	53	-604.	-273.	850.	0.
8	6	55	-694.	-278.	748.	0.

B O L T   I N T E R A C T I O N   E Q U A T I O N S

EQUATION=((TENSION/TALLOW)\*\* 1.000+(SHEAR/VALLOW)\*\* 1.000)\*\* 1.000

<u>NODE</u>	<u>TENSION</u>	<u>SHEAR</u>	<u>TENSION</u>	<u>SHEAR</u>	<u>INTERACTION</u>	<u>SAFETY</u>
	<u>ALLOW</u>	<u>ALLOW</u>	<u>RATIO</u>	<u>RATIO</u>	<u>FACTOR</u>	<u>FACTOR</u>
9	8740.	6890.	.036	.130	.166	6.02
11	8740.	6890.	.220	.118	.338	2.96
13	8740.	6890.	.219	.101	.320	3.12
30	8740.	6890.	.044	.134	.178	5.61
34	8740.	6890.	.130	.105	.235	4.25
51	8740.	6890.	0.000	.135	.135	7.39
53	8740.	6890.	0.000	.123	.123	8.11
55	8740.	6890.	0.000	.109	.109	9.21

\*\*\*\*\*  
 \* E N D   L O A D   C A S E   4 \*  
 \*\*\*\*\*

502K

SWEC ENGINEERING REVIEW CHECKLIST

HANGER DRAWING NO. <b>DO-908 N</b>	REV. <b>1FO</b>	INSPECTION TEAM <b>D. McDONELL C. HARRIS</b>	LEGEND: A - ACCEPTABLE D - DEVIATION EXISTS N/A - NOT APPLICABLE
ISOMETRIC NO. REV. <b>DO-448-1.8 (6)</b>	CHANGE DOCUMENT		

CHECKLIST ITEMS	STATUS			COMMENTS
	N/A	A	D	
<b>1. General</b>				
A. Support Location		✓		
B. Support Orientation		✓		
C. Catalog Items <i>(11 BOLT)</i>		✓		
D. Close Clearance Gaps			✓	<i>SEE NOTE 1</i>
<b>2. Support Structure</b>				
A. Critical Dimensions		✓		
B. Member sizes, structural plates		✓		
<b>3. Struts and Snubbers</b>				
A. Pin to Pin Dims., Snubber Setting	✓			
B. Paddle - Pin Assembly Connections	✓			
<b>4. Baseplates</b>				
A. Plate & Gusset sizes	✓			
B. Bolt size & type	✓			
C. Bolt hole spacing	✓			
D. Attachment location	✓			
E. Bolt spacing to adjacent inserts	✓			
<b>5. Lugs - Bearing Surface</b>		✓		
<b>6. Welding</b>				
A. Size, length, quality		✓		
B. Symbols		✓		
<b>7. Miscellaneous (specify)</b>	✓			

SWEC ENGINEERING REVIEW CHECKLIST

Notes:

NOTE 1: A  $\frac{1}{16}$ " clearance gap was measured between the top lug of the pipe and the stop construct. This gap is to be zero, therefore, further work is necessary for this deviation.

Results of Evaluation:

       No deviations noted.

-0- Deviations evaluated as acceptable by inspection team.

(1) Deviations require further evaluation.

10

518

Ronald M. Randall 8/15/83  
Signature Date

Disposition of deviations subject to further evaluation:

SUPPORT ID-908N IS A DEAD WEIGHT SUPPORT AND NO GAP ALLOWED IN A VERTICAL DIRECTION, THEREFORE DEVIATION DESCRIBED IN NOTE 1 ABOVE REQUIRES INSTALLATION OF  $\frac{1}{16}$ " SHIM PLATE BETWEEN THE LUG AND SUPPORTING PLATE ITEM # 8.

ACCEPTED, BASED ON "VISUAL EXAMINATION OF COMPONENT SUPPORTS" PRGPM SLT-5303, SECTION 4.2.2-8.

W. F. Polunsky 08-22-83  
Signature Date

REVIEWER R. P. B. 8-26-85

HANGER DRAWING NO. <i>CND-591</i>	REV. <i>2/70</i>	INSPECTION TEAM <i>A. DeLonge B. Kunkin</i>	LEGEND: A - ACCEPTABLE D - DEVIATION EXISTS N/A - NOT APPLICABLE
ISOMETRIC NO. REV. <i>CND-351-10-15 Not App</i>	CHANGE DOCUMENT <i>PEP-215-H - 12/12</i>		

CHECKLIST ITEMS	STATUS			COMMENTS
	N/A	A	D	
1. General				
A. Support Location		<input checked="" type="checkbox"/>		<i>See PEP-215-H</i>
B. Support Orientation		<input checked="" type="checkbox"/>		
C. Catalog Items			<input checked="" type="checkbox"/>	<i>See Deviations PEP</i>
D. Close Clearance Gaps		<input checked="" type="checkbox"/>		
2. Support Structure				
A. Critical Dimensions		<input checked="" type="checkbox"/>		
B. Member sizes, structural plates		<input checked="" type="checkbox"/>		
3. Struts and Snubbers				
A. Pin to Pin Dims., Snubber Setting		<input checked="" type="checkbox"/>		
B. Paddle - Pin Assembly Connections		<input checked="" type="checkbox"/>		
4. Baseplates				
A. Plate & Gusset sizes		<input checked="" type="checkbox"/>		
B. Bolt size & type		<input checked="" type="checkbox"/>		
C. Bolt hole spacing		<input checked="" type="checkbox"/>		
D. Attachment location		<input checked="" type="checkbox"/>		
E. Bolt spacing to adjacent inserts		<input checked="" type="checkbox"/>		
5. Lugs - Bearing Surface		<input checked="" type="checkbox"/>		
6. Welding				
A. Size, length, quality				
B. Symbols				
7. Miscellaneous (specify)				

SWEC ENGINEERING REVIEW CHECKLIST

2/4

Notes:

See attached pages showing Deviations (1) & (2)

Results of Evaluation:

         No deviations noted.

  2   Deviations evaluated as acceptable by inspection team.

  2   Deviations require further evaluation.

See attached pages showing Deviations (1) & (2)

          
Signature          Date 3/22/23

Disposition of deviations subject to further evaluation:

ALL DEVIATIONS (10) ACCEPTABLE PER ATTACHED SHEETS.

461

REVIEWER

Ben F. Burns (ERC EDC) 3/22/23  
Signature          Date 3/22/23

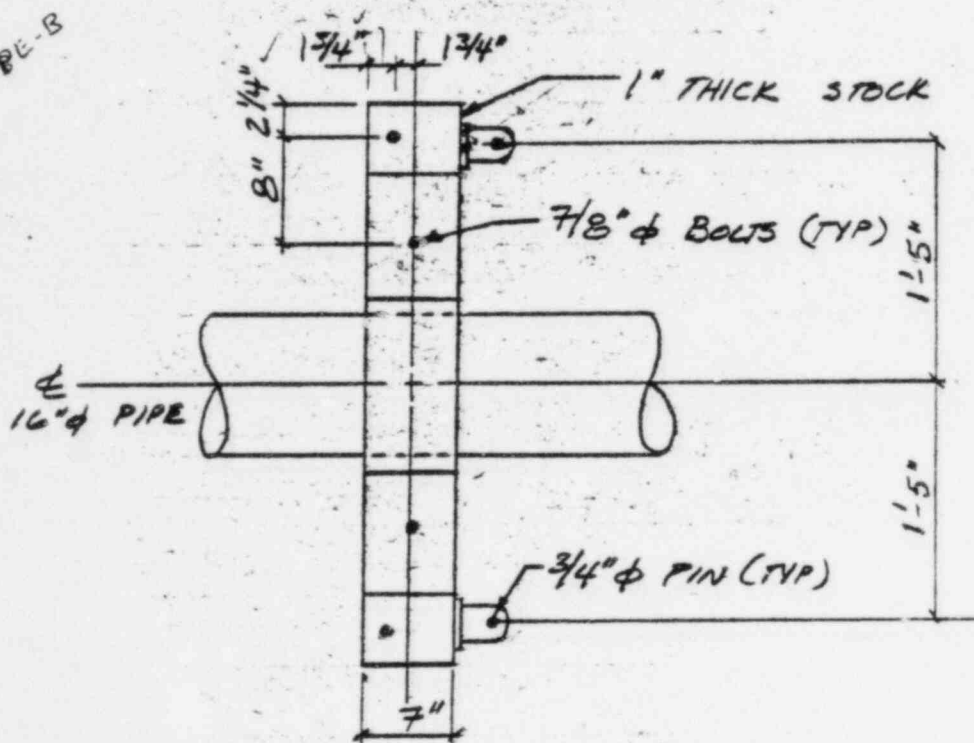


ATTACHMENT 'I' SWEC ENG.  
REVIEW CHECKLIST  
HANGER COND-591

3/4

( DEVIATION (1) )

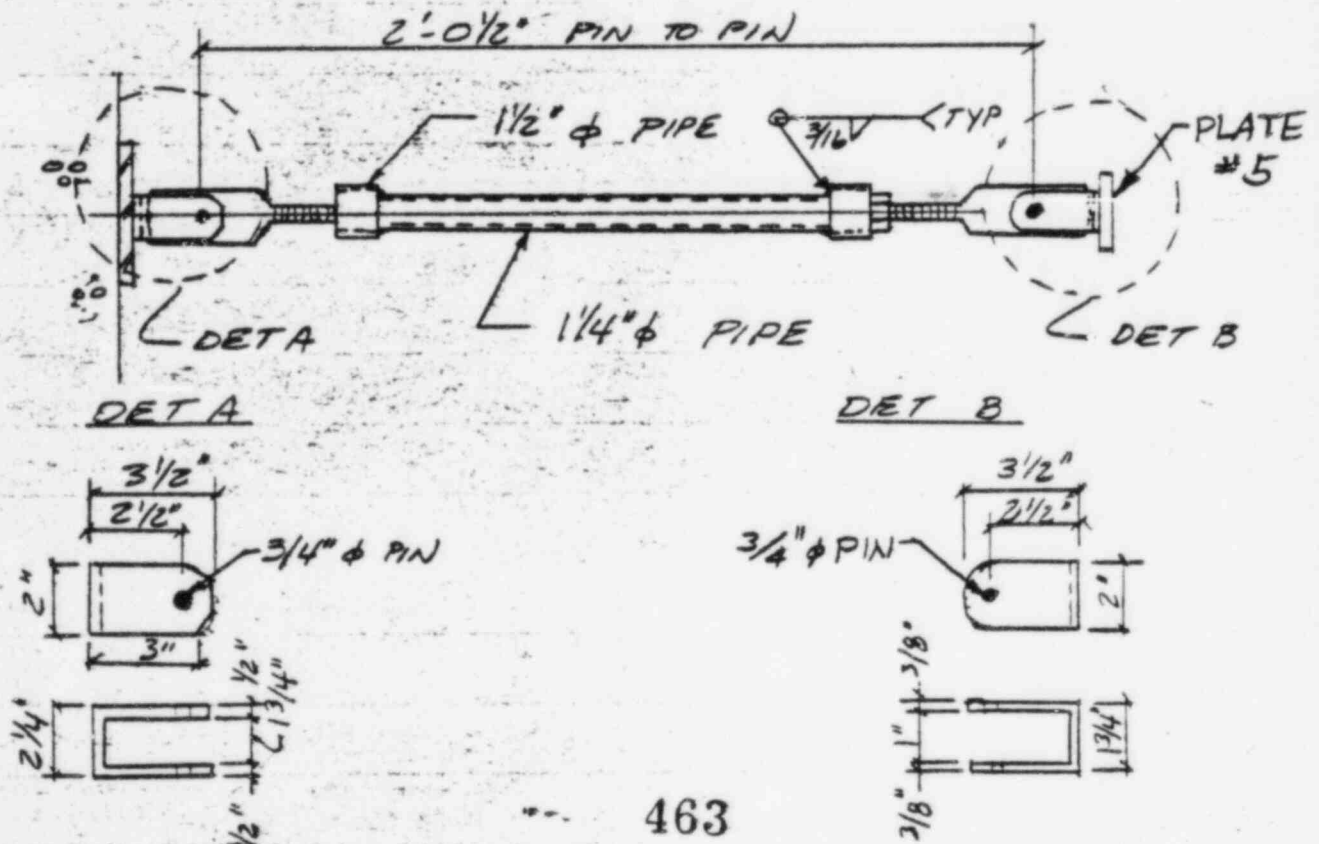
The 16"  $\phi$  riser clamp stock size called out on the as-built drawing does not agree with the clamp stock size measured. The as-built calls for a 16"  $\phi$  Special Riser Clamp Fig. 135A Stock Size = 1 1/4" x 7" w/ 1 1/4"  $\phi$  BOLTS. The clamp dimensions measured in the field are shown below:



We were unable to find this clamp in any available catalog so that we could not verify these dimensions. The observed stock size was 1" x 7" instead of 1 1/4" x 7".

DEVIATION (2)

The strut size is called out on the as-built drawing as, Rigid Strut Assembly NPS Fig. SRS Size 10 x 10 1/4" Lg., Pin  $\phi = 3/4"$ , Pipe  $\phi = 1 1/2"$  Nom., 1"  $\phi$  Rod End w/ Power Piping Size 1 1/2" HS-142 Beam Attachment. The strut measured in the field does not agree w/ this description & is shown below:



463

The strut assembly is a Size 8 instead of 10. The rear bracket (DET A) is correct. The front bracket (DET B) does not match the description given on the as-built (it has a 3/4"  $\phi$  pin instead of 1 1/4"  $\phi$  as indicated). We could not find this bracket in any available catalogs?

EVALUATION OF SUPPORT NO. COND-591 REV. 4FO  
 ISOMETRIC NO. COND-351-10.15H REV. \_\_\_\_\_  
 (NOT AVAIL.)

NOTES: 1) Burns & Roe Drawing #M-200 Sheet N/A has been reviewed for discrepancies between as-analyzed and as-built piping configurations. It is judged that any such discrepancies do not significantly affect calculated support loads. AS-BUILT ISO. NOT AVAILABLE (8-12-83) SO THEREFORE IT IS ASSUMED ACCEPTABLE AT THIS TIME.

DEV.-1 THE .16"  $\phi$  RISER CLAMP STOCK SIZE CALLED OUT ON THE AS-BUILT DRAWING DOES NOT AGREE WITH THE CLAMP STOCK SIZE MEASURED. HOWEVER, THE SPECIAL CLAMP FIG-135A SPECIFIED ON THE AS-BUILT DWG. IS IN AGREEMENT WITH THE CLAMP STOCK SIZE MEASURED IN THE FIELD. (1" X 7") THE BAR STOCK SIZE ON THE AS-BUILT RESTRAINT DWG. SHOULD BE DELETED SINCE IT IS NOT CORRECT. THE BOLT SIZE CORRESPONDING TO THE BAR STOCK SIZE (1" X 7") IS 1/8"  $\phi$  BOLTS. THE SIZE BOLTS MEASURED IN THE FIELD ARE 3/4"  $\phi$ .

THE BAR STOCK SPECIFIED FOR THE 16"  $\phi$  PIPE BY FIG-135A HAS A RATED CAPACITY OF 9500#. THE BOLT HAS A RATED CAPACITY OF 6100#. THEREFORE DEVIATION #1 IS ACCEPTABLE SINCE THE MAXIMUM DESIGN LOAD IS 4292#.

REFERENCES

PREPARER: A. D. Curtis DATE: 8-13-83  
 REVIEWER: Bruce Bahr DATE: 8-13-83

EVALUATION OF SUPPORT NO. COND-591 REV. 4FO  
 ISOMETRIC NO. COND-351-10.15H REV.         
 (NOT AVAILABLE)

DEV.-2

THE STRUT SIZE SPECIFIED ON THE AS-BUILT ISO. IS SIZE 10. THE STRUT SIZE IN THE FIELD IS SIZE-8. THE MAXIMUM LOAD PER STRUT IS DESIGNATED AS 4960#. THE MAXIMUM DESIGN LOAD PER STRUT IS 2146# WHICH IS LESS THAN 50% OF THE RATED CAPACITY OF THE STRUT. THE 1/4" FILLET WELD CALLED OUT ON THE STRUT (NOT ASSEMBLY) IS 3/16". THE UNDERSIZED WELD IS ACCEPTABLE SINCE IT REDUCES THE CAPACITY BY 25% OF THE WELD) COUPLED WITH THE FACT THAT THE DESIGN LOAD IS LESS THAN 50% OF THE ALLOWABLE

THE BRACKETS ARE SPECIFIED CORRECTLY BY THE AS-BUILT RESTRAINT DWG.

REFERENCES

PREPARER: R.D. Ewart DATE: 8-13-83  
 REVIEWER: Ben S. Bull DATE: 8-13-83

BURNS AND ROE, INC.  
Headquarters Office - Onaluta, Mo.

W.O. No. 2900-76 CRS Book No. 8.15-4 Page No. \_\_\_\_\_  
 Drawing No. COND-591 Calc. No. 8.15.189 Sheet 3 Cont. on Sheet 4  
 BY A. Dancer C. Malini Approved \_\_\_\_\_  
 Title APSS NP2 PIPE SUPPORT REVIEW/REDESIGN PS MARK NO. COND-591

LOADING CONDITIONS: Thermal Movement to 0.005 -0.053 ✓  
 $\Delta x = 0.0"$ ,  $\Delta y = 0.002"$ ,  $\Delta z = -0.057"$

3  
K. Hays  
12/14/62  
H. Calbra

LOAD TYPE	POUNDS			POUNDS-FT.		
	Fx	Fy	Fz	Mx	My	Mz
Thermal	-2277 <del>-269</del>					
Dead Wt.	408 <del>300</del>					
Misc.						
DYNAMIC LOADS	Normal	<del>±1942</del> <del>±1530</del>				
	Emerg.	<del>±1942</del> <del>±1530</del>				
	Faulted	<del>±3884</del> <del>±3560</del>				
TOTAL LOADS	Normal	<del>+1830</del> <sup>+2350</sup> <del>1499</del> <del>-3811</del>				
	Emerg.	<del>+1830</del> <sup>+2350</sup> <del>1499</del> <del>-3811</del>				
	Faulted	<del>+4292</del> <del>+3360</del> <del>-2760</del> <sup>-2476</sup>				
ORIGINAL SUPPORT DESIGN LOADS	Normal					
	Emerg.					
	Faulted					

Loads taken from Calc No. S.14.64 Brev. Book No 8.14.64

6

INSTRUCTIONS FOR SELECTION OF SPECIAL RISER CLAMPS

From Chart A, locate the point determined by the intersection of the total load coordinate and the pipe size curve. Project this point horizontally until the "D" dimension coordinate is reached. The correct stock size is shown on the curve above this point.

The bolt diameter is obtained by taking the size listed below the total load. Use the next largest bolt size if the load falls between two sizes.

To describe the riser clamp for a Bill of Material, list the following:

- a. Type A or B (Use type B for 40,000# load or larger)
- b. Pipe O.D.
- c. Rod to rod dimension "C-C"
- d. Stock size (ASME SA-36 material)
- e. Bolt diameter

ASME SA-36 (ASTM A-36) carbon steel IS shall be used for 750°F max.

Typical example: Total load: 16,500#

Pipe O.D.: 20"

Rod to rod dimension (C-C): 36"

Dimension "D" =  $\frac{36-20}{2} = 8$

From Chart A: Design is type A

Stock size - 1 1/2" x 9"

Bolt size ' 1 1/2"

DO NOT USE ON ABOVE 1111/1 HANGERS

REV. 7

A6-2  
7/9/82

17

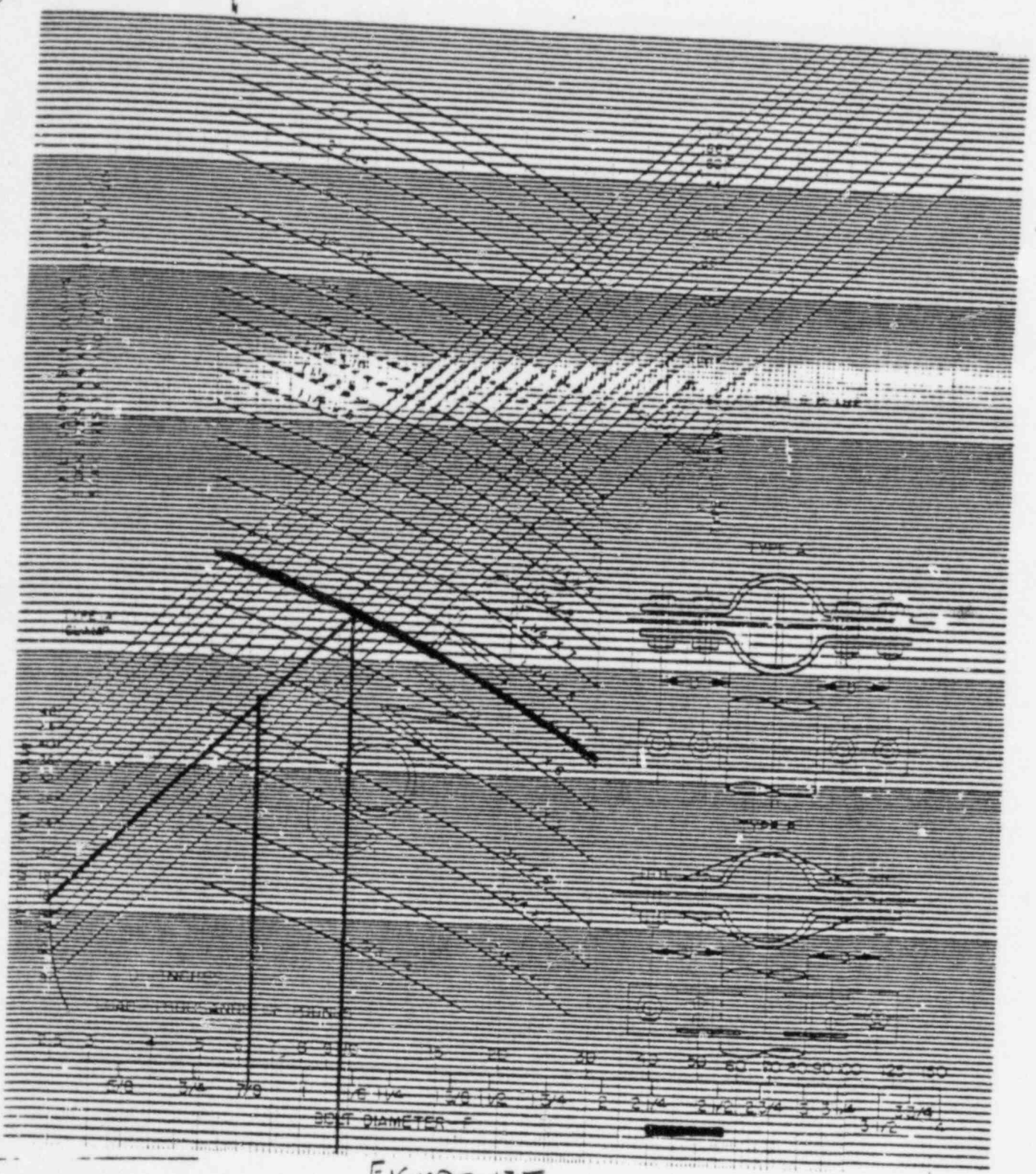
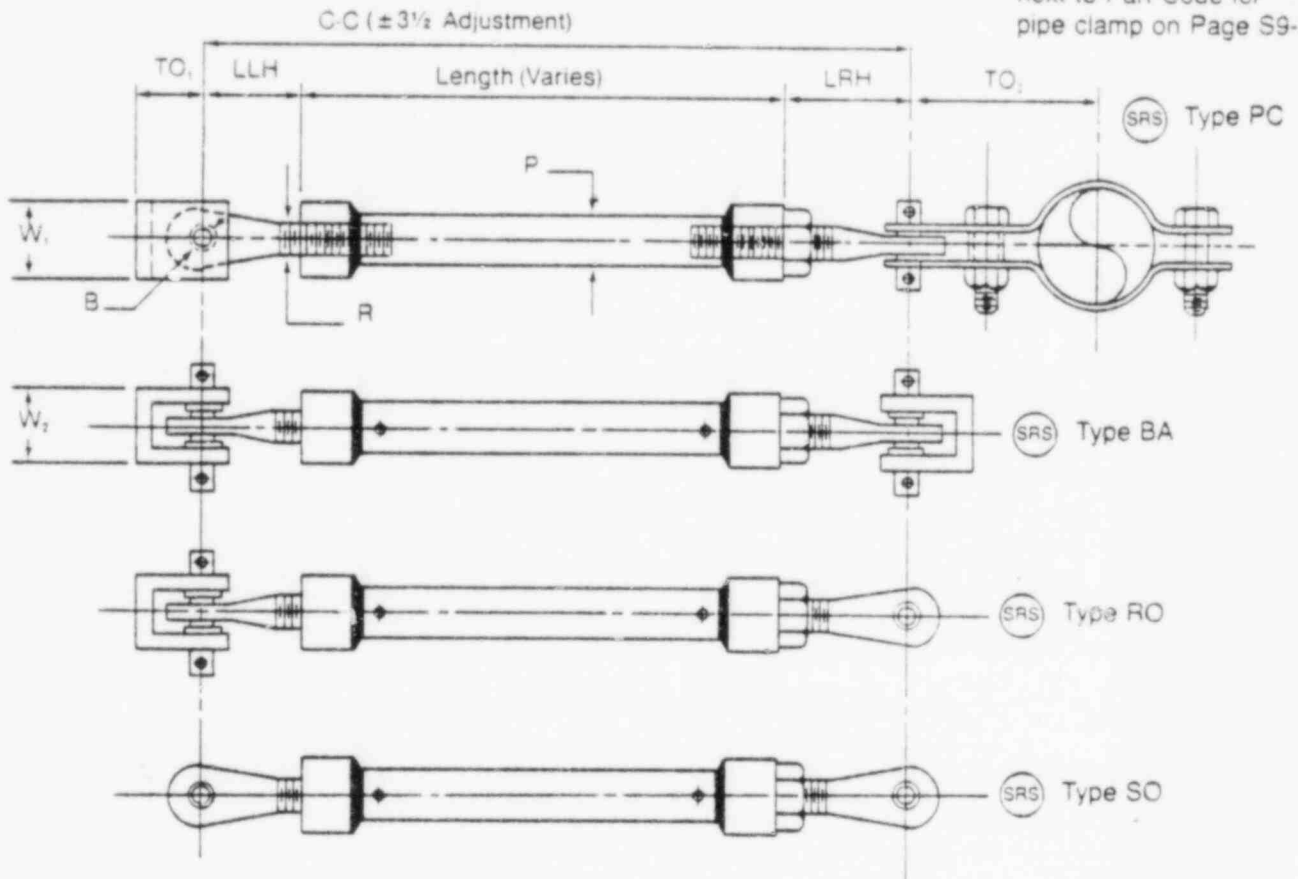


FIGURE 135  
CHART A

FOR SPECIAL RISER CLAMPS

617

Dimension TO<sub>2</sub> found next to Part Code for pipe clamp on Page S9-S17



425E

SWAY STRUT SIZE NO	R	B	REAR BRACKET		P	LLH	LRH	MINIMUM C TO C		MAX. LOAD*	SWAY STRUT SIZE NO		
			BASE SIZE					TAKE OUT	NOMINAL PIPE SIZE			WITH ADJUSTMENT	NO ADJUSTMENT
			W <sub>1</sub>	W <sub>2</sub>									
06	3/4	1/2	1 1/4	2	1 1/2	1"	4 1/4	5 1/4	19 1/4	15 1/4	2700	06	
08	1	3/4	1 1/2	3	2	1 1/2"	5 1/4	6 1/4	21 1/2	18	4960	08	
10	1 1/4	1	2 1/4	3 1/2	2 1/2	1 1/2"	6 1/4	7 1/4	24 1/2	21 1/2	8000	10	
12	1 1/2	1	3	4 1/2	3 1/4	2"	7 1/4	9 1/4	28 1/2	24 1/2	11600	12	
14	1 1/2	1	3	4 1/2	3 1/4	2"	7 1/4	9 1/4	28 1/2	24 1/2	15700	14	
20	2 1/2	1 1/2	6	7	4	3"	11 1/4	13 1/4	38	34 1/2	33500	20	
24	3	1 1/2	10	10	5 1/2	4"	12 1/4	15 1/4	41 1/2	37 1/2	50600	24	
36	4 1/2	3	12 1/2	12 1/2	8	6"	16 1/4	21	54 1/2	51	120000	36	

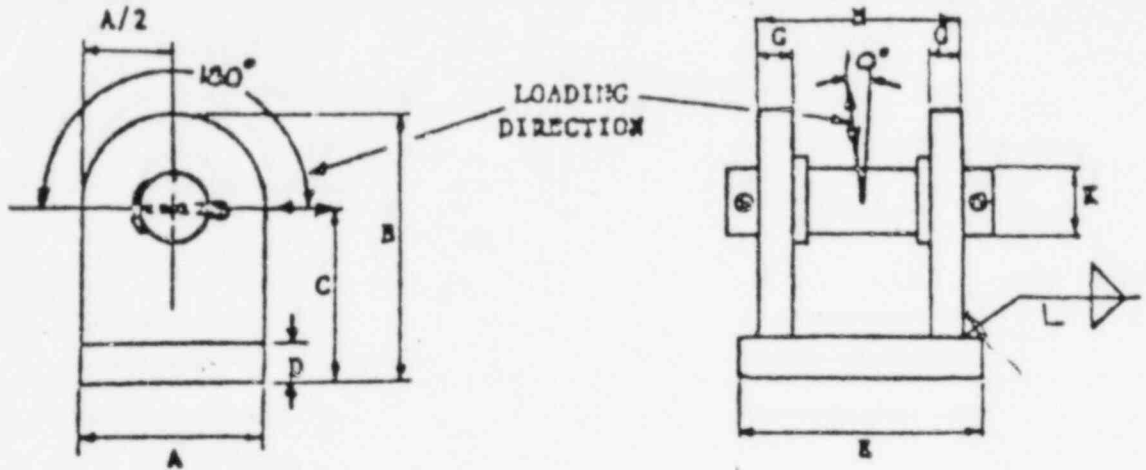
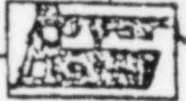
\*Maximum Load at Minimum C to C





# POWER PIPING COMPANY

819 BEAVER AVENUE  
PITTSBURGH, PA. 15223



SIZE	A	B	C	E	G	N	K	D	L
15 1 1/2	2.0	3.625	2.50	2.1875	.375	1.6875	.75	.375	1/2
20 2	2.5	3.625	2.50	2.5625	.500	1.9375	1.00	.375	1/2
25 2 1/2	3.0	4.250	2.75	3.2500	.750	2.6562	1.00	.500	1/2
40 4	5.0	6.250	4.00	4.6250	1.000	3.8750	1.50	.750	3/4
50 5	6.5	8.000	5.00	6.0000	1.250	5.0000	2.25	1.000	3/4
60 6	7.5	9.750	6.00	6.7500	1.250	5.6250	3.00	1.250	3/4
80 8	8.5	11.750	7.50	10.0000	1.500	6.6250	3.50	1.500	1/2

## US-142 BEAM ATTACHMENT

For use with Figure 350 Rigid Sway Strut or Hydraulic Snubber

Pin Material:  
A-108 or A-193-B7 per M-300

470

CATALOG DATA:  
Subject to change  
without notice

SWEC ENGINEERING REVIEW CHECKLIST

HANGER DRAWING NO.  <div style="font-size: 2em; font-weight: bold; text-align: center;">FPC-43</div>	REV.  <div style="font-size: 2em; font-weight: bold; text-align: center;">4</div>	INSPECTION TEAM JACOB MENDEZ	LEGEND: A - ACCEPTABLE D - DEVIATION EXISTS N/A - NOT APPLICABLE
ISOMETRIC NO. REV. <i>6FO</i> FPC-603-10.12	CHANGE DOCUMENT PED-219-W-K234 PED-219-H-MZBI		

CHECKLIST ITEMS	STATUS			COMMENTS
	N/A	A	D	
<b>1. General</b>				
A. Support Location		✓		
B. Support Orientation		✓		
C. Catalog Items		✓		
D. Close Clearance Gaps	✓			
<b>2. Support Structure</b>				
A. Critical Dimensions		✓		
B. Member sizes, structural plates				✓ <i>W6x25 ITEM #2 SHOULD BE W6x20 SEE NOTE 1</i>
<b>3. Struts and Snubbers</b>				
A. Pin to Pin Dims., Snubber Setting		✓		
B. Paddle - Pin Assembly Connections		✓		
<b>4. Baseplates</b>				
A. Plate & Gusset sizes	✓			
B. Bolt size & type	✓			
C. Bolt hole spacing	✓			
D. Attachment location	✓			
E. Bolt spacing to adjacent inserts	✓			
5. Lugs - Bearing Surface	✓			
<b>6. Welding</b>				
A. Size, length, quality		✓		
B. Symbols		✓		
7. Miscellaneous (specify)	✓			



EVALUATION OF SUPPORT NO. FPC-43 REV. 4  
 ISOMETRIC NO. FPC-605-10.12 REV. 6 FO

Change in SECTION PROPERTIES FOR member 2 by using W6x20 ( $A=5.88 \text{ in}^2$   $S=13.4 \text{ in}^3$  Ref. 1) instead of W6x25 ( $A=7.35 \text{ in}^2$   $S=16.7 \text{ in}^3$  Ref 1) has insignificant effect due to max force & moment values acting @ this member:

Max shear force & bending moment for member 2:

$$F = 3824 \times 1.25 = 4780 \#$$

$$M = 4780 \times 15 = 71700 \# \cdot \text{in}$$

REF. 2

$$S = \frac{71700}{13.4} = 5351 \text{ psi} < F_{AL}$$

Therefore deviation 2B can be judged as acceptable.

#### REFERENCES

- 1) AISC "STEEL CONSTRUCTION MANUAL"
- 2) SUPPORT DWG FPC-43 Rev. 4
- 3) PIPING ISO FPC-605-10.12 Rev. 6 FO

PREPARER: S. J. [Signature]

DATE: 8/13/83

REVIEWER: E. de Guzman

DATE: 8/15/83

SWEC ENGINEERING REVIEW CHECKLIST

HANGER DRAWING NO. <b>LPCS - 908N</b>	REV. <b>1</b>	INSPECTION TEAM <b>W. Pokryshevsky J. MIKA</b>	LEGEND: A - ACCEPTABLE D - DEVIATION EXISTS N/A - NOT APPLICABLE
ISOMETRIC NO. REV. <b>LPCS-756-19.21 10FO</b>	CHANGE DOCUMENT		

CHECKLIST ITEMS	STATUS			COMMENTS
	N/A	A	D	
<b>1. General</b>				
A. Support Location		✓		
B. Support Orientation		✓		
C. Catalog Items		✓		
D. Close Clearance Gaps		✓		
<b>2. Support Structure</b>				
A. Critical Dimensions		✓		
B. Member sizes, structural plates		✓		
<b>3. Struts and Snubbers</b>				
A. Pin to Pin Dims., Snubber Setting		✓		
B. Paddle - Pin Assembly Connections		✓		
<b>4. Baseplates</b>				
A. Plate & Gusset sizes		✓		
B. Bolt size & type		✓		
C. Bolt hole spacing		✓		
D. Attachment location		✓		
E. Bolt spacing to adjacent inserts		✓		
<b>5. Lugs - Bearing Surface</b>		✓		
<b>6. Welding</b>				
A. Size, length, quality			✓	<b>SEE NOTE 1</b>
B. Symbols			✓	
<b>7. Miscellaneous (specify)</b>				
	✓			

SWEC ENGINEERING REVIEW CHECKLIST

Notes:

1. WELD SIZES DEVIATED SHOWN ON DWG

- a) WELD SIZE BETWEEN ITEM 6 & EXIST. STEEL (LEFT) IS  $5/16"$  VS.  $3/8"$ , ALSO 1 EXTRA WELD  $5/16"$  ON RIGHT SIDE
- b) BETWEEN GUSSET 2 ON RIGHT SIDE OF SUBROOF & ITEM # 6 -  $1/4"$  VS.  $5/16"$  (FAR SIDE)

Results of Evaluation:

N/A No deviations noted.

N/A Deviations evaluated as acceptable by inspection team.

-1- Deviations require further evaluation.  
(ITEM 6A)

Joseph O. M. Jha  
W. P. K. Sheeney  
Signature

7-20-83  
07-20-83  
Date

Disposition of deviations subject to further evaluation:

- 1. A) SEE ATTACHED EVALUATION;
- B) WELDS MEET MIN. WELD SIZE REQUIREMENT. O.K.

661

JOE STOKES  
Signature

8-23-83  
Date

(Reviewer) Craigley 8/23/83

EVALUATION OF SUPPORT NO. LPCS-908N REV. 1 (2FO)  
 ISOMETRIC NO. LPCS-756-19.21 REV. 10FO

NOTES: 1) Burns & Roe Drawing #M-200 Sheet 1 has been reviewed for discrepancies between as-analyzed and as-built piping configurations. It is judged that any such discrepancies do not significantly affect calculated support loads.

MAX. DESIGN LOADS (SEE REF. 3)

NORMAL

$F_x' = 1.25 [(1583)^2 + (4713)^2]^{1/2} = 6215 \text{ LBS}$  (1.25 IS A MODIFIED LOAD FACTOR)

EMERGENCY

$F_x' = 1.25 [(2065)^2 + (6149)^2]^{1/2} = 8108 \text{ LBS}$

FAULTED

$F_x' = 1.25 [(5257)^2 + (15657)^2]^{1/2} = 20645 \text{ LBS}$

RESOLVE FRAME FOR REACTIONS @ WELDS

RESOLVE LOADS TO PT. A:

NORMAL

$F_{x'A} = 6215 \text{ LBS}$

$M_{z'A} = (26.88)(6215) = 167060 \text{ IN LBS}$

EMERGENCY

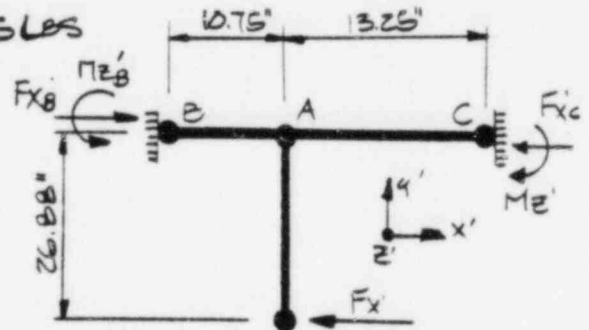
$F_{x'A} = 8108 \text{ LBS}$

$M_{z'A} = (26.88)(8108) = 217943 \text{ IN LBS}$

FAULTED

$F_{x'A} = 20645 \text{ LBS}$

$M_{z'A} = 26.88(20645) = 554937 \text{ IN LBS}$



RESOLVE LOADS TO PTS B & C (REF 5)

FAULTED CONDITION

$F_{x'B} = F_{z'B} = 20645 \text{ LBS}$

REFERENCES

- 1.) PIPE SUPPORT DWG. LPCS-908N REV. 2FO
- 2.) PIPING ISO LPCS-756-19.21 REV. 10FO
- 3.) B&R SUPPORT REVIEW/REDESIGN CALC 8.15.1128 REV 2.
- 4.) AISC "STEEL CONSTRUCTION MANUAL" 8<sup>TH</sup> ED.

5.) BLODGETT "MANUAL OF STEEL CONSTRUCTION"

(CONT'D, SHT. 2)

PREPARER:

JOE STOKES

DATE: 8-23-83

REVIEWER:

Craig Gray

DATE: 8/23/83

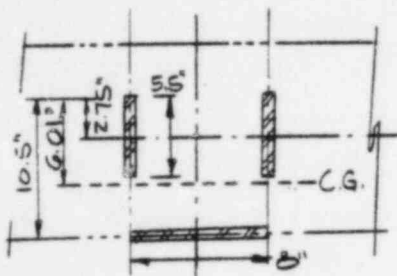
EVALUATION OF SUPPORT NO. LPCS-908N REV. 1 (ZFD)  
 ISOMETRIC NO. LPCS-756-19.21 REV. 10 F0

FAULTED REACTIONS (CONTINUED)

$$F_{yB} = \frac{6(554937)(10.75)(13.25)}{(24)^3} = 34307 \text{ LBS}$$

$$M_{EB} = \frac{(554937)(13.25)}{(24)^2} [24 - 3(10.75)] = 105316 \text{ IN LBS}$$

CONSERVATIVELY, USE  $M_{EB} = 196161 \text{ IN LBS}$  AS DETERMINED PER STRUHL MODEL, REF 3, SHT 6

WELD ANALYSIS @ PT. BWELD GEOMETRY (ASSUME WELD THICKNESS AS UNITY)

$$A_W = 2(5.5)(1) + 8(1) \text{ IN}$$

$$\bar{y} = \frac{2(5.5)(1)(2.75) + 10.5(8)(1)}{19}$$

$$= 6.01 \text{ IN}$$

$$I_z = \left[ \frac{(5.5)^3(1)}{12} + 5.5(3.27)^2(1) \right](2) + 8(1)(4.49)^2 = 306.6 \text{ IN}^3$$

$$I_x = (5.5)(40)^2(2) + (8)^3(1/12) = 2187 \text{ IN}^3$$

$$S_{WZ} = \frac{306.6}{6.01}$$

$$= 51.02 \text{ IN}^2$$

WELD FORCES - FAULTED

$$F_x = \frac{F_x}{A_W} = \frac{20645}{19} = 1087 \text{ LBS/IN}$$

$$F_y = \frac{F_y}{A_W} + \frac{M_z}{S_{Wz}} = \frac{34307}{19} + \frac{196161}{51.02} = 5651 \text{ LBS/IN}$$

$$\text{RESULTANT } F_r = (F_x^2 + F_y^2)^{1/2} = 5754 \text{ LBS/IN}$$

$$\text{LEG SIZE REQ'D} = \frac{5754}{0.7071(28000)} = 0.291 \text{ IN} < 0.3125 \text{ IN. O.K. (REF. 6)}$$

REFERENCES

(SEE SHT 1)

6) ASME III, SUBSECTION NF, APPENDIX F.

7) AISC STEEL CONST. MANUAL 7TH EDIT.

PREPARER:

Joe Stow

DATE: 8-23-83

REVIEWER:

Craig Gray

DATE: 8/23/83



EVALUATION OF SUPPORT NO. LPCS-908N REV. 1 (ZFD)  
 ISOMETRIC NO. LPCS-756-19.21 REV. 1 (DFD)

WELD FORCES (NORMAL-UPSET) APPLY RATIO (EMERG/FAULTED) LOAD

RESULTANT =  $(8108/20645)(5754) = 2260 \text{ LB/IN}$

LEG SIZE REQ'D =  $2260/12711 (18000) = 0.178 \text{ IN}$  C.O. SIZE IN  $\therefore$  O.K.  
 (REF. 7)

$\therefore$  WELD DEVIATION IS ACCEPTABLE

REFERENCES

(SEE SHTS. 1 & 2)

PREPARER: JOE STOKER DATE: 8-23-83

REVIEWER: Pringle DATE: 8/23/83

SWEC ENGINEERING REVIEW CHECKLIST

HANGER DRAWING NO. MSLC - 31	REV. 1	INSPECTION TEAM W. POKRYSHEVSKY R. EWART	LEGEND: A - ACCEPTABLE D - DEVIATION EXISTS N/A - NOT APPLICABLE
---------------------------------	-----------	--	---

ISOMETRIC NO. REV. MSLC - 084 - 7.10 GPO	CHANGE DOCUMENT PED-215-MN167		
---	----------------------------------	--	--

CHECKLIST ITEMS	STATUS			COMMENTS
	N/A	A	D	

1. General

A. Support Location		✓		
B. Support Orientation		✓		
C. Catalog Items		✓		
D. Close Clearance Gaps		✓		

2. Support Structure

A. Critical Dimensions			✓	DIMENSION WALL TO Q PIPE 5 1/4" OFF
B. Member sizes, structural plates		✓		

3. Struts and Snubbers

A. Pin to Pin Dims., Snubber Setting	✓			
B. Paddle - Pin Assembly Connections	✓			

4. Baseplates

A. Plate & Gusset sizes	✓			
B. Bolt size & type	✓			
C. Bolt hole spacing	✓			
D. Attachment location	✓			
E. Bolt spacing to adjacent inserts	✓			

5. Lugs - Bearing Surface

	✓			
--	---	--	--	--

6. Welding

A. Size, length, quality			✓	SEE NOTE 2
B. Symbols		✓		

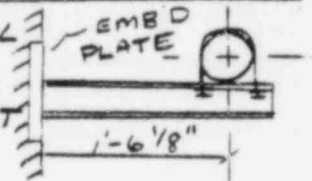
7. Miscellaneous (specify)

	✓			
--	---	--	--	--

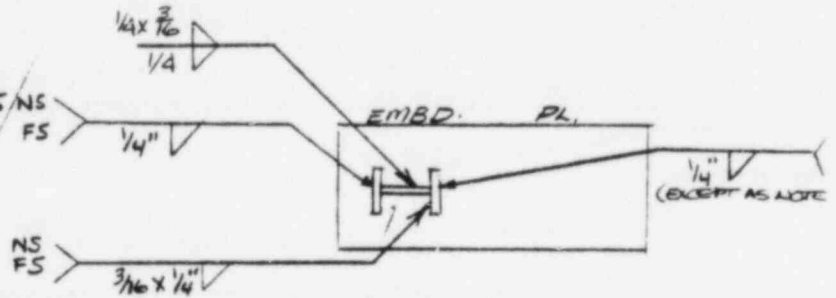
SWEC ENGINEERING REVIEW CHECKLIST

1. ITEM 2A. THE DIMENSION FROM THE WALL TO THE  $\phi$  OF THE PIPE WAS SPECIFIED ON THE AS-BUILT ISO. AS 1'-0<sup>13/16</sup>"

Notes:



2. ITEM 6A. SUPPORT DWG. REQUIRES 1/4" FILLET ALL AROUND W 4 X 13



Results of Evaluation:

N/A No deviations noted.

Ø Deviations evaluated as acceptable by inspection team.

2 DEVIATIONS REQUIRE FURTHER EVALUATION (ITEM 2A, 6A)

Ronnie D. Curant 07-22-83  
Signature Date  
W. T. Stupskobey 07-22-83  
Signature Date

DISPOSITION OF DEVIATIONS SUBJECT TO FURTHER EVALUATION:

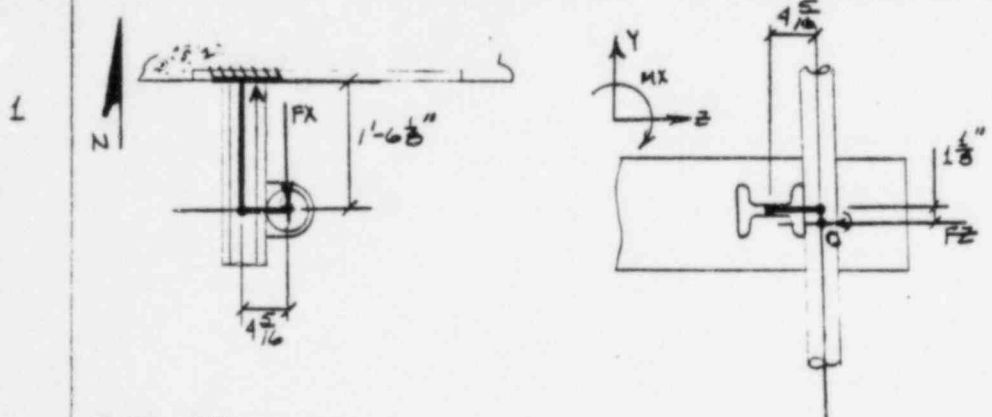
DEVIATIONS CONSIDERED ACCEPTABLE

Thomas J. Carter 19 AUG 83  
Signature Date  
Ronald H. McRae 8/19/83  
Signature Date

EVALUATION OF SUPPORT NO. MSLC-31 REV. 1  
 ISOMETRIC NO. MSLC-084-7.10 REV. 6FD

NOTES: 1) \* Burns & Roe Drawing #M-200 Sheet        has been reviewed for discrepancies between as-analyzed and as-built piping configurations. It is judged that any such discrepancies do not significantly affect calculated support loads.  
 \* No M-200 ISO IS AVAILABLE - MSLC-084-7.10 ISOU 6 FD HAS BEEN FIELD VERIFIED & NO DISCREPANCIES WERE NOTED WHICH WOULD AFFECT THE ANALYSIS OF THIS SUPPORT.

REF ANALYSIS OF DEVIATIONS NOTED AS ITEMS 2A & 6A:



2 @ Pt "O":  
 $F_x = 210 \text{ lbs}$   
 $F_z = 200 \text{ lbs}$

@ Pt "A":  
 $F_x = 210 \text{ lbs}$   
 $F_z = 200 \text{ lbs}$   
 $M_x = 200(1.125) = 225 \text{ in-lbs}$   
 $M_y = 200(18.125) + 210(4.3125) = 4,531 \text{ in-lbs}$

\* DUE TO RELATIVELY LOW LOADS COMPARED TO RELATIVELY HIGH SECTION PROPERTIES OF EXISTING WELD, BY ENGINEERING JUDGEMENT THIS WELD & STRUCTURAL MEMBER HAVE ADEQUATE STRENGTH FOR THE APPLIED LOADING.

REFERENCES

- 1) AISC. 8th Ed
- 2) BURNS & ROE CALC B.16.7553, DATED 12/3/82, SHEET 3

PREPARER: Thomas J. Carter DATE: 19 Aug 83  
 REVIEWER: Ronald J. McPherson DATE: 19 Aug 83

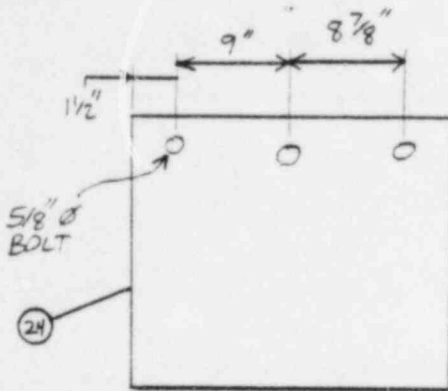
SWEL ENGINEERING REVIEW CHECKLIST

HANGER DRAWING NO. <b>RCIC-15</b>	REV. <b>6</b>	INSPECTION TEAM <b>FISCHER + McDONELL</b>	LEGEND: A - ACCEPTABLE D - DEVIATION EXISTS N/A - NOT APPLICABLE
ISOMETRIC NO. REV. <b>RCIC-659-7.10 6FO</b>	CHANGE DOCUMENT		

CHECKLIST ITEMS	STATUS			COMMENTS
	N/A	A	D	
<b>1. General</b>				
A. Support Location		✓		
B. Support Orientation		✓		
C. Catalog Items	✓			
D. Close Clearance Gaps		✓		
<b>2. Support Structure</b>				
A. Critical Dimensions		✓		
B. Member sizes, structural plates		✓		
<b>3. Struts and Snubbers</b>				
A. Pin to Pin Dims., Snubber Setting	✓			
B. Paddle - Pin Assembly Connections - -	✓			
<b>4. Baseplates</b>				
A. Plate & Gusset sizes		✓		
B. Bolt size & type			✓	ONE BOLT IS A 5/8"Ø, NOT A 3/4"Ø (SEE BACK)
C. Bolt hole spacing			✓	SEE NOTES SECTION ON BACK
D. Attachment location		✓		
E. Bolt spacing to adjacent inserts		✓		
<b>5. Lugs - Bearing Surface</b>				
		✓		
<b>6. Welding</b>				
A. Size, length, quality		✓		
B. Symbols		✓		
<b>7. Miscellaneous (specify)</b>				
	✓			

SWEC ENGINEERING REVIEW CHECKLIST

Notes:



ALL OTHER BOLT TO BOLT SPACINGS ARE WITHIN 1/4" OF DIMENSIONS SHOWN IN SECT. B-B OF HGR DWG AND ALL OTHER EDGE DISTANCES ARE WITHIN 1/8".

PARTIAL SECT. 'B-B'

Results of Evaluation:

- \_\_\_\_\_ No deviations noted.
- \_\_\_\_\_ Deviations evaluated as acceptable by inspection team.

2 Deviations require further evaluation. 4B+C

Bruce Fischer      7/27/83  
Signature                      Date

Disposition of deviations subject to further evaluation:

ALL DEVIATIONS SEE ATTACHED SHEETS FOR ANALYSIS

868

DE STOICA      8-26-83  
Signature                      Date

R. McCall      7/26/83

EVALUATION OF SUPPORT NO. RCIC-15 REV. 6  
 ISOMETRIC NO. RCIC-659-7.15 REV. GFØ

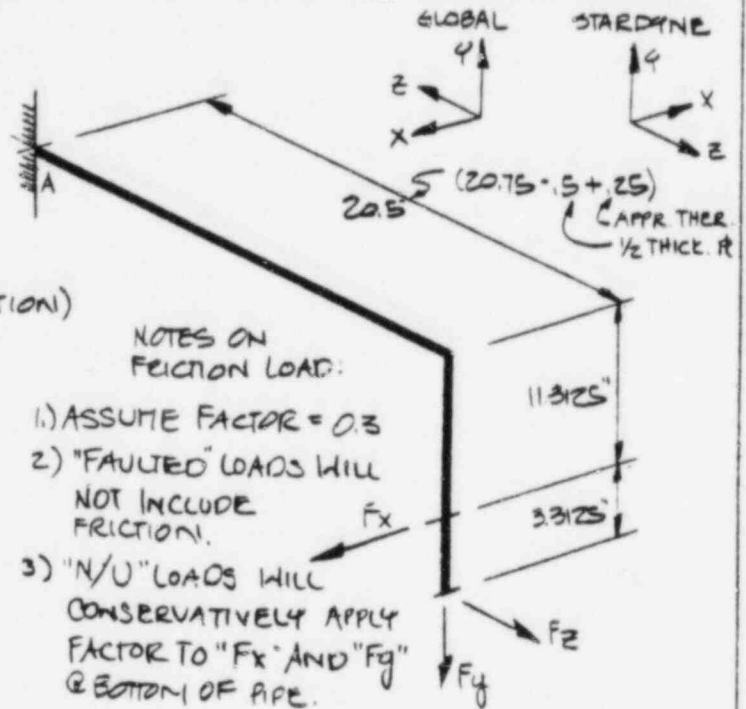
NOTES: 1) Burns & Roe Drawing #M-200 Sheet 115 has been reviewed for discrepancies between as-analyzed and as-built piping configurations. It is judged that any such discrepancies do not significantly affect calculated support loads.

DERIVATION OF DESIGN LOADS  
(REF 3)

	<u>F<sub>x</sub></u>	<u>F<sub>y</sub></u>	<u>F<sub>z</sub> (FRICTION)</u>
DWT	-3	-776	
THERM	+1282	-39	
SEIS-"N/U"	+107	+607	
SEIS-"F"	+214	+1214	
TOTAL "N/U"	+1386	+0	+416
	-110	-1422	+460
TOTAL "F"	+211	+1214	0
	-217	-1990	0

NOTES ON FRICTION LOAD:

- 1.) ASSUME FACTOR = 0.3
- 2.) "FAULTED" LOADS WILL NOT INCLUDE FRICTION.
- 3.) "N/U" LOADS WILL CONSERVATIVELY APPLY FACTOR TO "F<sub>x</sub>" AND "F<sub>y</sub>" @ BOTTOM OF PIPE.



RESOLVED LOADS

STARDYNE CASE	<u>F<sub>x</sub></u>	<u>F<sub>y</sub></u>	<u>F<sub>z</sub></u>	<u>M<sub>x</sub></u>	<u>M<sub>y</sub></u>	<u>M<sub>z</sub></u>	<u>MAX REACTIONS @ P. A</u>
(N/U) 1	+1386	-1422	+843	-16823	+28415	+15680	<u>MOMENT EQUATIONS</u> $M_x = 20.5(+F_y) + 14.625(+F_z)$ $M_y = 20.5(+F_x)$ $M_z = 11.3125(+F_x)$
(N/U) 2	-110	-1422	+460	-22424	-2255	-1244	
(F) 3	-217	-1990	Ø	-40795	-4449	-2455	
(F) 4	+211	-1990	Ø	-40795	+4326	+2387	

REFERENCES

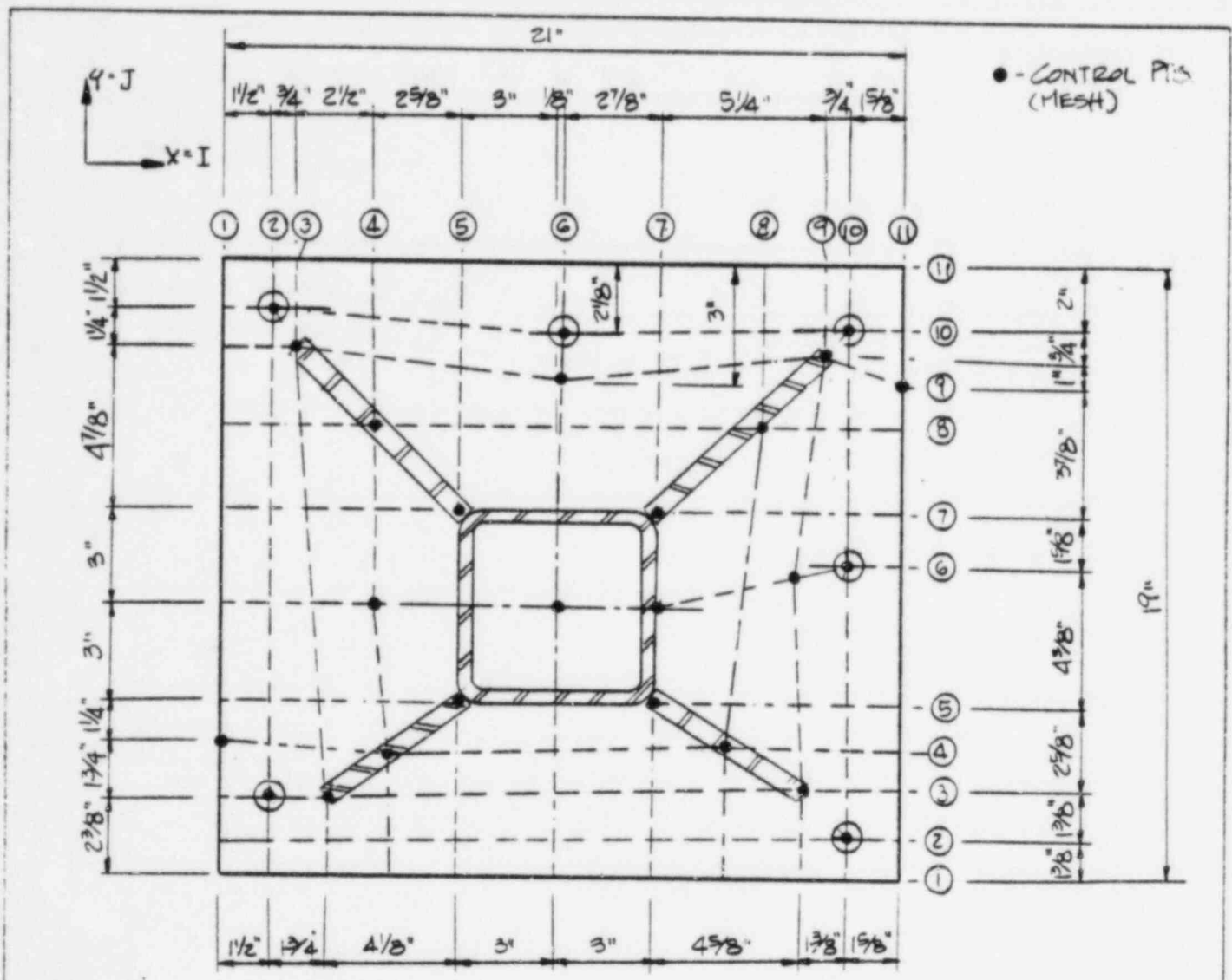
- 1.) PIPE SUPPORT DWA RCIC-15 REV. 6
- 2.) PIPING ISOMETRIC RCIC-659-7.15 REV. GFØ
- 3.) B&R SUPPORT REVIEW/REDESIGN CALC. 8.15. 2139 REV. 3
- 4.) B&R DESIGN GUIDE M409
- 5.) B&R "BOOK OF TYPICAL BASE PLATES (REV. 1)" APPEN. C
- 6.) AISC "STEEL CONSTRUCTION MANUAL" 8TH EDITION

PREPARER: JOE STOICA DATE: 8-24-83

REVIEWER: R. McQuill DATE: 8/26/83

(CONT'D ON SHT. 3)

EVALUATION OF SUPPORT NO. RCIC-15 REV. 6  
 ISOMETRIC NO. RCIC-659-7.10 REV. 6F0



STARDAGNE MODEL AND INPUT (SEE ATTACHMENT 1.0)

BOLT PROPERTIES

3/4" φ HILTI DROP-INS (REF 5)  
 TENSILE STIFF. = 980000 LB/IN  
 SHEAR STIFF. = 302000 LB/IN  
 TENSION ALLOW. = 4060 LBS  
 SHEAR ALLOW = 4420 LBS  
 (REF 4)

PREPARER: DE STOLWA DATE: 3-24-83  
 REVIEWER: [Signature] DATE: 7-26-83



EVALUATION OF SUPPORT NO. RCIC-15 REV. 6  
 ISOMETRIC NO. RCIC-659-7.15 REV. 6FD

BOLT PROPERTIES (CONT'D)5/8"  $\phi$  HILT DROP-IN BOLT

TENSILE STIFF = 840000 LBS/IN  
 SHEAR STIFF = 250000 LBS/IN  
 (REF. 5)

TENSION ALLOWABLE = 2510 LBS  
 SHEAR ALLOWABLE = 3070 LBS  
 (REF. 4)

ASSUME CONCRETE COMPRESSIVE STRENGTH = 3500 PSI

CONCRETE MODULUS OF ELASTICITY  $E_c = 57000 \sqrt{3500} = 3372166$  PSI (REF. 7)

RESULTS OF STARDYNE RUNATTRIBUTE 4.B.) UNDERSIZED BOLT

MAX. INTERACTION FOR 5/8"  $\phi$  BOLT = 0.619  $\leq$  1.0  $\therefore$  O.K.

(REF STARDYNE OUTPUT NODE 21, CASE 1, AND REF. 4)

$\therefore$  5/8"  $\phi$  BOLT IS O.K., DEVIATION ACCEPTABLE

ATTRIBUTE 4.C) BOLT HOLE SPACING

BY ENGINEERING JUDGEMENT, BECAUSE OF VERY LOW BOLT LOADS  
 BOLT SPACING DEVIATION IS ACCEPTABLE. MINIMUM SPACING  
 REQUIREMENT IS STILL MET  $\therefore$  DEVIATION IS ACCEPTABLE.  
 (REF STARDYNE OUTPUT)

REFERENCES

- 7.) STARDYNE COMPUTER PROGRAM ST-330 VERSION  $\phi$ 3 LEVEL 00 W/  
 CONTROL DATA CORP. "BASEPLATE II USER MANUAL" REVISION 1.05, 1982.

PREPARER: JOE STICKS DATE: 8-26-83

REVIEWER: ALMORWELL DATE: 8/26/83

EVALUATION OF SUPPORT

RCIC-15

ATTACH 1.0

PE

\*\*\*\*\*

Preprocessor input data cards

\*

	1	2	3	4	5	6	7
1234567890	234567890	234567890	234567890	234567890	234567890	234567890	234567
3 title is baseplate anal. support rcic-15 rev. 6. iso rcic-659-7.10 rev.6f0							
out	1*	0	0*	1	0*	3	0*
con	0*	0	0*	0.000*	3500.000*	0.000*	0.000*
pla	11*	11	0*	21.000*	19.000*	1.000*	0.000*
apr	0*	0	1*	6.000*	6.000*	.375*	.375*
ber	0*	0	1*	980.e3*	302.e3*	4060.*	4420.*
ber	0*	0	2*	840.e3*	250.e3*	2510.*	3070.*
bol	2*	3	1*	1.500*	2.375*	0.000*	0.000*
bol	10*	2	1*	19.375*	1.375*	0.000*	0.000*
bol	10*	6	1*	19.375*	9.750*	0.000*	0.000*
bol	2*	10	2*	1.500*	17.500*	0.000*	0.000*
bol	6*	10	1*	10.500*	16.875*	0.000*	0.000*
bol	10*	10	1*	19.375*	17.000*	0.000*	0.000*
con	1*	4	0*	0.000*	4.500*		
con	3*	3	0*	3.250*	2.375*		
con	5*	5	0*	7.375*	5.375*		
con	7*	5	0*	13.375*	5.375*		
con	9*	3	0*	18.000*	2.750*		
con	4*	6	0*	4.750*	8.375*		
con	7*	6	0*	13.375*	8.375*		
con	5*	7	0*	7.375*	11.375*		
con	7*	7	0*	0.000*	0.000*		
con	3*	9	0*	2.250*	16.250*		
con	6*	9	0*	10.500*	16.000*		
con	9*	9	0*	18.625*	16.250*		
con	11*	9	0*	21.000*	15.250*		
con	4*	4	0*	5.313*	3.875*		
con	8*	4	0*	15.688*	4.063*		
con	4*	8	0*	4.813*	13.813*		
con	8*	8	0*	16.000*	13.813*		
con	9*	6	0*	17.750*	9.375*		
end							
tub	6*	6	1*	10.375*	8.375*	0.000*	0.000*
sti	3*	3	2*	3.000*	0.000*	0.000*	.750*
sti	9*	3	2*	4.000*	0.000*	0.000*	.750*
sti	3*	9	2*	-4.000*	0.000*	0.000*	.750*
sti	9*	9	2*	-3.000*	0.000*	0.000*	.750*
end							
poi	6*	6	0*	0.000*	0.000*	0.000*	
loa	0*	0	1*	1386.*	-1422.*	843.*	-14823.*
loa	0*	0	2*	-110.*	-1422.*	420.*	-22424.*
loa	0*	0	3*	-217.*	-1990.*	0.*	-40795.*
loa	0*	0	4*	211.*	-1990.*	0.*	-40795.*
md							

"end". of j

\*\*\*\*\*  
 \* baseplate anal. support rcic-15 rev. 6, iso rcic-659-7.10 rev.6f0  
 \*  
 \*\*\*\*\*

input parameters  
 -----

plate thickness ..... 1.000  
 plate x-dimension ..... 21.000  
           y-dimension ..... 19.000  
 modulus of elasticity ... 28.e6  
 concrete modulus ..... 3.37e6  
 conc. comp. strength .... 3500.

bolt locations  
 -----

bolt	i	j	star node	x-coord.	y-coord.
1	2	3	14	1.500	2.375
2	10	2	101	19.375	1.375
3	10	6	105	19.375	9.750
4	2	10	21	1.500	17.500
5	6	10	45	10.500	16.875
6	10	10	109	19.375	17.000

bolt properties  
 -----

bolt	stiffness (lb/in.)		allowable force (lb)	
	tension	shear	tension	shear
1	980.e3	302.e3	4060.	4420.
2	980.e3	302.e3	4060.	4420.
3	980.e3	302.e3	4060.	4420.
4	840.e3	250.e3	2510.	3070.
5	980.e3	302.e3	4060.	4420.
6	980.e3	302.e3	4060.	4420.

\*\*\*\*\*

\* baseplate anal. support rcic-15 rev. 6, iso rcic-659-7.10 rev.6f0

\*\*\*\*\*

attachment data

attachment number 1

type .....	tub	property number ....	1
d dimension .....	6.000	i line number .....	6
b dimension .....	6.000	j line number .....	6
height .....	0.000	x-coordinate .....	10.375
thickness (t1) ..	.375	y-coordinate .....	8.375
thickness (t2) ..	.375	orientation angle ..	0.0

load conditions

load case no. 1

star node	122
force fx	1386.
force fy	-1422.
force fz	843.
moment mx	-16823.
moment my	23413.
moment mz	15680.

load case no. 2

star node	122
force fx	-110.
force fy	-1422.
force fz	460.
moment mx	-22424.
moment my	-2255.
moment mz	-1244.

\*\*\*\*\*  
 \* baseplate anal. support rcic-15 rev. 6, iso rcic-659-7.10 rev.6f0  
 \*  
 \*\*\*\*\*

load case no. 3

star node 122  
 force fx -217.  
 force fy -1990.  
 force fz 0.  
 moment mx -40795.  
 moment my -4449.  
 moment mz -2455.

load case no. 4

star node 122  
 force fx 211.  
 force fy -1990.  
 force fz 0.  
 moment mx -40795.  
 moment my 4326.  
 moment mz 2387.

baseplate connectivity  
 -----

element number	ja node	jb node	jc node	jd node	element number	ja node	jb node	jc node	jd node	n
1	1	12	13	2	2	2	13	14		
3	3	14	15	4	4	4	15	16		
5	5	16	17	6	6	6	17	18		
7	7	18	19	8	8	8	19	20		
9	9	20	21	10	10	10	21	22		
11	12	23	24	13	12	13	24	25		
13	14	25	26	15	14	15	26	27		
15	16	27	28	17	16	17	28	29		
17	18	29	30	19	18	19	30	31		
19	20	31	32	21	20	21	32	33		
21	22	34	35	24	22	24	35	36		
23	25	36	37	26	24	26	37	38		
25	27	38	39	28	26	28	39	40		
27	29	40	41	30	28	30	41	42		
29	31	42	43	32	30	32	43	44		
31	34	45	46	35	32	35	46	47		
33	36	47	48	37	34	37	48	49		
35	38	49	50	39	36	39	50	51		

baseplate anal. support rcic-15 rev. 6, iso rcic-659-7.10 rev.6f0

baseplate connectivity

element number	Ja node	Jb node	Jc node	Jd node	element number	Ja node	Jb node	Jc node	Jd node
37	40	51	52	41	38	41	52	53	42
39	42	53	54	43	40	43	54	55	44
41	45	56	57	46	42	46	57	58	47
43	47	58	59	48	44	48	59	60	49
45	49	60	61	50	45	50	61	62	51
47	51	62	63	52	48	52	63	64	53
49	53	64	65	54	50	54	65	66	55
51	56	67	68	57	52	57	68	69	58
53	58	69	70	59	54	59	70	71	60
55	60	71	72	61	56	61	72	73	62
57	62	73	74	63	58	63	74	75	64
59	64	75	76	65	60	65	76	77	66
61	67	78	79	68	62	68	79	80	69
63	69	80	81	70	64	70	81	82	71
65	71	82	83	72	66	72	83	84	73
67	73	84	85	74	68	74	85	86	75
69	75	86	87	76	70	76	87	88	77
71	78	89	90	79	72	79	90	91	80
73	80	91	92	81	74	81	92	93	82
75	82	93	94	83	76	83	94	95	84
77	84	95	96	85	78	85	96	97	86
79	86	97	98	87	80	87	98	99	88
81	89	100	101	90	82	90	101	102	91
83	91	102	103	92	84	92	103	104	93
85	93	104	105	94	86	94	105	106	95
87	95	106	107	96	88	96	107	108	97
89	97	108	109	98	90	98	109	110	99
91	100	111	112	101	92	101	112	113	102
93	102	113	114	103	94	103	114	115	104
95	104	115	116	105	96	105	116	117	106
97	106	117	118	107	98	107	118	119	108
99	108	119	120	109	100	109	120	121	110

OUT 1,  $\phi$ ,  $\phi$ ,  $\phi$ , -1, 3/  
 CON , , , 35 $\phi$  $\phi$ /  
 PLA .11, 11, , 21., 19., 1./  
 APR , , 1, 6., 6., .375, .375/  
 BPR , , 1, 98 $\phi$  $\phi$  $\phi$  $\phi$ , 3 $\phi$ 2 $\phi$  $\phi$  $\phi$ , 4 $\phi$ 6 $\phi$ , 442 $\phi$ /  
 BPR , , 2, 84 $\phi$  $\phi$  $\phi$  $\phi$ , 25 $\phi$  $\phi$  $\phi$  $\phi$ , 251 $\phi$ , 3 $\phi$ 7 $\phi$ /  
 BOL 2, 3, 1, 1.5, 2.375/  
 BOL 1 $\phi$ , 2, 1, 19.375, 1.375/  
 BOL 1 $\phi$ , 6, 1, 19.375, 9.75/  
 BOL 2, 1 $\phi$ , 2, 1.5, 17.5/  
 BOL 6.1 $\phi$ , 1, 1 $\phi$ .5, 16.875/  
 BOL 1 $\phi$ , 1 $\phi$ , 1, 19.375, 17. $\phi$ /  
 CON 1, 4, , 0., 4.5/  
 CON 3, 3, , 3.25, 2.375/  
 CON 5, 5, , 7.375, 5.375/  
 CON 7, 5, , 13.375, 5.375/  
 CON 9, 3, , 18., 2.75/  
 CON 4, 6, , 4.75, 8.<sup>3</sup>/<sub>4</sub>75/  
 CON 7, 6, , 13.375, 8.75/  
 CON 5, 7, , 7.375, 11.75/  
 CON 7, 7, , 13.375, 11.75/  
 CON 2, 9, , 2.25, 16.25/  
 CON 6, 9, , 1 $\phi$ .5, 16./  
 CON 9, 9, , 18.625, 16.25/

CON 11, 9, , 21. , 15.25/  
 CON 4, 4, , 5.313, 4.063/  
 CON 8, 4, , 15.688, 4.063/  
 CON 4, 8, , 4.813, 14./  
 CON 8, 8, , 16. , 15.813/  
 CON 9, 6, , 17.75, 9.375/  
 END /  
 TUB 6, 6, 1, 10.375, 8.75/  
 STI 3, 3, 2, 3.0, , , .75/  
 STI 9, 3, 2, 4.0, , , .75/  
 STI 3, 9, 2, -4.0, , , .75/  
 STI 9, 9, 2, -3.0, , , .75/  
 END /  
 POI 6, 6/  
 LOA , , 1, 1386, -1422, 843, -16823, +28413 +15680/  
 LOA , , 2, -110, -1422, 460, -22424, -2255, -1244/  
 LOA , , 3, -217, -1990, 0, -40795, -4449, -2455/  
 LOA , , 4, +211, -1990, 0, -40795, +4326, +2387/  
 END /  
 END OF JOB



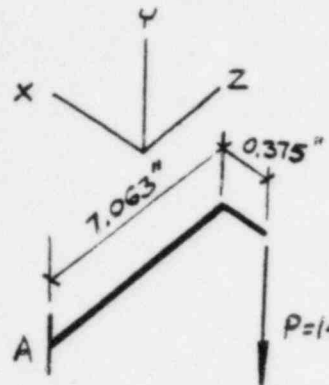
HANGER DRAWING NO. <b>RFW-177</b>	REV. <b>4FO</b>	INSPECTION TEAM <b>S JACOB J MENDEZ</b>	LEGEND: A - ACCEPTABLE D - DEVIATION EXISTS N/A - NOT APPLICABLE
ISOMETRIC NO. REV. <b>RFW-438-1.2</b>	CHANGE DOCUMENT <b>POD 215-H-668</b>		

CHECKLIST ITEMS	STATUS			COMMENTS
	N/A	A	D	
<b>1. General</b>				
A. Support Location	✓			AS BUILT ISO NOT AVAILABLE
B. Support Orientation		✓		
C. Catalog Items		✓		
D. Close Clearance Gaps	✓			
<b>2. Support Structure</b>				
A. Critical Dimensions			✓	SEE NOTE 1
B. Member sizes, structural plates		✓		
<b>3. Struts and Snubbers</b>				
A. Pin to Pin Dims., Snubber Setting	✓			
B. Paddle - Pin Assembly Connections	✓			
<b>4. Baseplates</b>				
A. Plate & Gusset sizes		✓		
B. Bolt size & type		✓		
C. Bolt hole spacing		✓		
D. Attachment location		✓		
E. Bolt spacing to adjacent inserts		✓		
5. Lugs - Bearing Surface	✓			
<b>6. Welding</b>				
A. Size, length, quality		✓		
B. Symbols		✓		
7. Miscellaneous (specify)	✓			



EVALUATION OF SUPPORT NO. RFW-177 REV. 4FO  
 ISOMETRIC NO. RFW-438-1.2 REV. \_\_\_\_\_

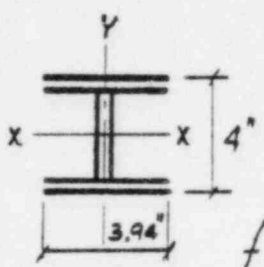
EVALUATION OF DEVIATION - ATTRIBUTE "2A"



FORCES & MOMENTS @ PT. A  
 (LOAD FROM CALC 8.14.14BB)  
 $F_x = 0$   
 $F_y = 1198 \times 1.25$  (LOAD FACTOR)  $= 1498$ "  
 $F_z = 0$   
 $M_x = 1498 \times 7.063 = 10577$  in-lb  
 $M_y = 0$   
 $M_z = 1498 \times 0.375 = 562$  in-lb

DUE TO INSIGNIFICANT MOMENT  $M_z$  AND RELATIVELY BIG SECTION PROPERTIES FOR MEMBER M14x3 SHEAR STRESS IN EXISTING MEMBER CONSIDERED AS NEGLIGIBLE BY ENGINEERING JUDGEMENT.  
 THEREFORE DEVIATION REQUIRES ONLY WELD SIZE CHECK AT POINT OF CONNECTION OF ITEM #4 & BASE PLATE, ITEM #1 AND CHECK ON BASE PLATE.

CHECK ON WELD @ PT. A (REF. 2)



AS-BUILT WELD IS 1/4 ALL AROUND.  
 FOR WELD TREATED AS LINE-SECTION PROPERTIES:  
 $A = 23.76$   $S_{wx} = 36.85$   $J_w = 94.1$   $C_y = 2.0$

FORCE ON WELD

$$f = \sqrt{\left(\frac{M_x}{S_{wx}}\right)^2 + \left(\frac{F_y}{A} + \frac{M_z \cdot C_y}{J_w}\right)^2} = \sqrt{\left(\frac{10577}{36.85}\right)^2 + \left(\frac{1498}{23.76} + \frac{562 \times 2.0}{94.1}\right)^2}$$

$$= 297 \text{ psi}$$

$$t = \frac{f}{0.707 \times S_y} = \frac{297}{0.707 \times 18000} = 0.0023 < 1/4 \text{ O.K.}$$

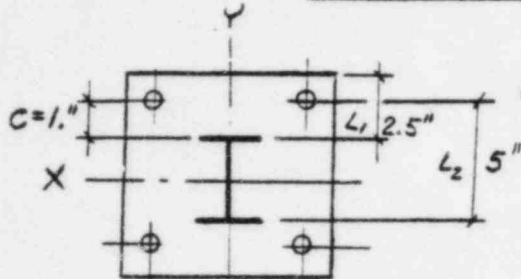
REFERENCES

1. CALC. NO. 8.16.658 & 8.14.14BB
2. BLODGETT "DESIGN OF WELDED STRUCTURES"
3. AISC MANUAL - 8TH EDITION.

PREPARER: W. Pokyshewsky DATE: 08-24-83  
 REVIEWER: Bruce Fischer DATE: 8/25/83

EVALUATION OF SUPPORT NO. RFW-177 REV. 4FO  
 ISOMETRIC NO. RFW-438-1.2 REV. \_\_\_\_\_

(CONT. FROM PG 1 OF 2)

CHECK ON BASE PLATE & ANCHOR BOLTS

BASE PLATE  $\frac{1}{2}'' \times 9'' \times 9''$   
 ANCHOR BOLTS  $\frac{1}{2}''$  HVY. HEX. MACH. BOLTS  
 $T_a = 2110$   $S_a = 1960$  (HILTI DROP-IN)

$$\frac{L_1}{t} = \frac{2.5}{0.5} = 5.0 > 2;$$

$$S_x = \frac{M_z \cdot y_i}{4(x^2 + y^2)} = \frac{562 \times 3}{4(3^2 + 3^2)} = 23 \#$$

$$S_y = \frac{M_z \cdot x_i}{4(x^2 + y^2)} + \frac{F_y}{4} = \frac{562 \times 3}{4(3^2 + 3^2)} + \frac{1498}{4} = 398 \#$$

$$S = \sqrt{S_x^2 + S_y^2} = \sqrt{23^2 + 398^2} = 399 \#$$

$$\frac{S}{S_a} = \frac{399}{1960} = 0.203; \quad P_{max} = LFM(P_{mx}) \quad \frac{L_2}{t} = \frac{5.0}{0.5} = 10.0$$

$$P_{mx} = \frac{M_x \cdot L_2}{2L_2^2} = \frac{10577 \times 5}{(2) \times 5^2} = 1058 \#;$$

LFM = 1

$$P_{max} = 1058 \# \quad P_{AL} = T_a [1 - (S/S_a)^{5/3}]^{3/5} = 2020 \#$$

$$\frac{P_{max}}{P_{AL}} = \frac{1058}{2020} = 0.52 < 1.0 \rightarrow \text{calculate } T_{eq} = \left( \frac{6M_y}{b_{eff} \times G_{AL}} \right)^{1/2}$$

$$M_x = T \times C \times N; \quad T = \frac{M_x \cdot L_2}{2 \times L_2^2} = \frac{10577 \times 5}{2 \times 5^2} = 1058; \quad C = 1.0''; \quad N = 2$$

$$M_x = 1058 \times 1.0 \times 2 = 2116 \text{ in-}\# \quad b_{eff} = 0.66 \times 5 + 4.0 = 7.3$$

$$G_{AL} = 0.75 S_v = 27000 \text{ [REF 2]} \quad T_{eq} = \left( \frac{6 \times 2116}{7.3 \times 27000} \right)^{1/2} = 0.25'' < \frac{1}{2}''$$

REFERENCES

BASE PLATE IS O.K.

1. STRUCTURAL EVALUATION PROGRAM (SD-STEP) -  
 PROCEDURE FOR EVALUATION OF BASE PLATE.

S&amp;W NO. SDM-79-2Y

2. ASME III, SUBSECTION NF, APP F

PREPARER: W. Polysky DATE: 08-24-83REVIEWER: P. M. Fisher DATE: 8/25/83

SWEC ENGINEERING REVIEW CHECKLIST

HANGER DRAWING NO.	REV.	INSPECTION TEAM	LEGEND:
RWCR - 1C-2PS - <del>3FO</del>	3FO	W. POKRYSHESKY R. EWART	A - ACCEPTABLE D - DEVIATION EXISTS N/A - NOT APPLICABLE
ISOMETRIC NO. REV.	CHANGE DOCUMENT		
EWCR-212-37H			

CHECKLIST ITEMS	STATUS			COMMENTS
	N/A	A	D	
<b>1. General</b>				
A. Support Location		✓		
B. Support Orientation		✓		
C. Catalog Items			✓	6" PIPE CLAMP IN FIELD NOT AS NOTED IN BILL OF MATERIAL
D. Close Clearance Gaps	✓			
<b>2. Support Structure</b>				
A. Critical Dimensions			✓	↓ DISTANCE W/ 14x14 TO ↓ STRUT 1'-4 3/4"
B. Member sizes, structural plates		✓		
<b>3. Struts and Snubbers</b>				
A. Pin to Pin Dims., Snubber Setting		✓		
B. Paddle - Pin Assembly Connections		✓		
<b>4. Baseplates N/A</b>				
A. Plate & Gusset sizes	✓			
B. Bolt size & type	✓			
C. Bolt hole spacing	✓			
D. Attachment location	✓			
E. Bolt spacing to adjacent inserts	✓			
5. Lugs - Bearing Surface N/A	✓			
<b>6. Welding</b>				
A. Size, length, quality			✓	5/16" Fillet Specified 1/4" MEASURED
B. Symbols		✓		
7. Miscellaneous (specify)			✓	ITEM #3 SHOWN ON BILL OF MATERIAL BUT NOT CALLED OUT ON SKETCH

SWEC ENGINEERING REVIEW CHECKLIST

- Notes: 1. CRITICAL DIMENSION DISCREPANCY SEE  
Pg 2 OF 2
2. FILLET WELD SIZE DISCREPANCY  
SEE PAGE 2 OF 2
3. PIPE CLAMP BAR STOCK  
DISCREPANCY SEE PAGE 2 OF 2

Results of Evaluation:

- N/A No deviations noted.
- Ø Deviations evaluated as acceptable by inspection team.

3 Deviations require further evaluation.

R. D. East 7-23-83  
Signature Date  
W. Toky

Disposition of deviations subject to further evaluation:

ABOVE DEVIATIONS ARE FOUND TO BE ACCEPTABLE.  
SEE ATTACHED EVALUATION SHEET.

1154

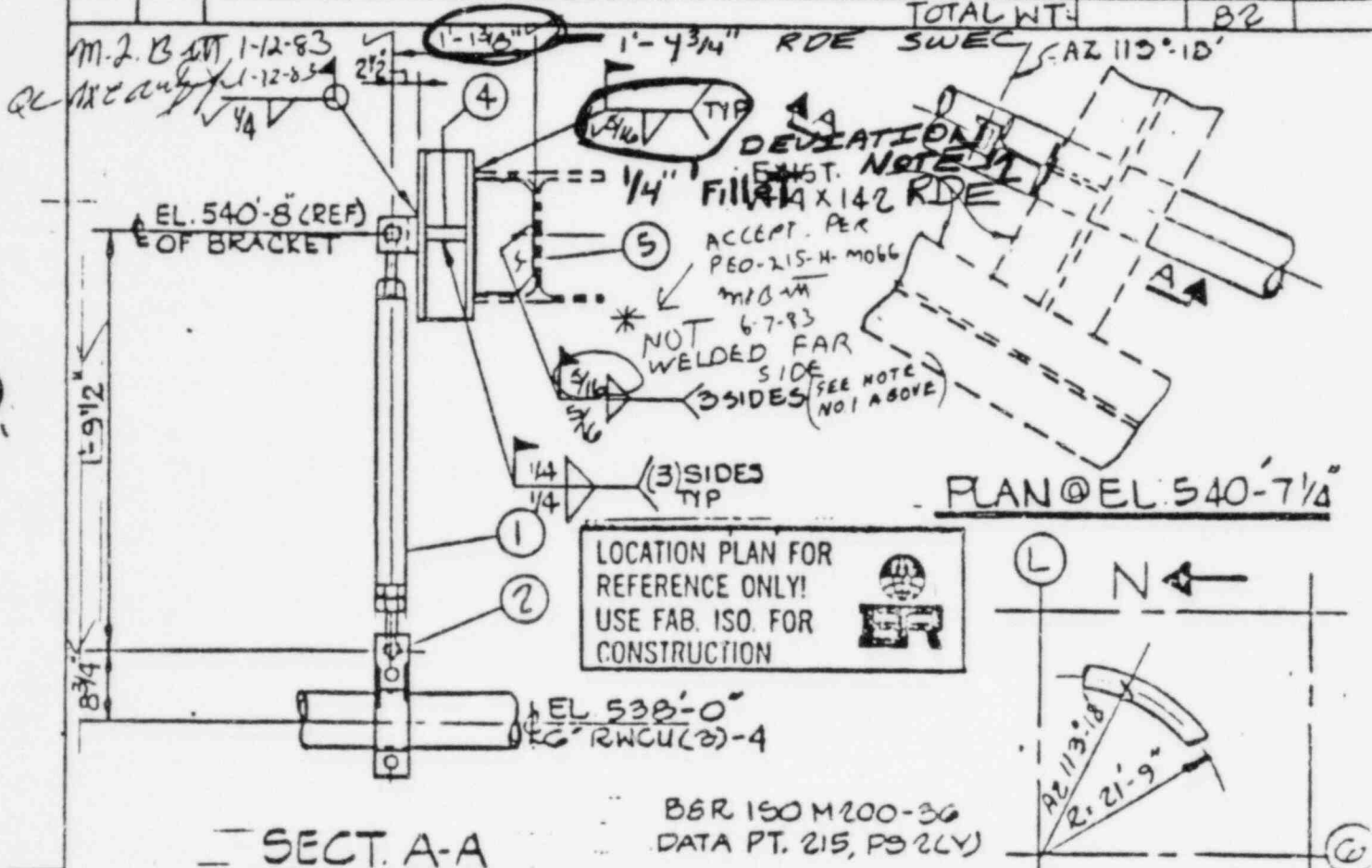
E. de Guzman 8/5/83  
Signature Date

# ATTACHMENT TO CHECKLIST

NOTE #1 FOR HGR Results of Review by Mike Dean of B & R Inc. ACCEPT AS IS ANOPEO-215-H-M066 J. Ingleson Feb 2/83.  
 RWCW-1C-2PS

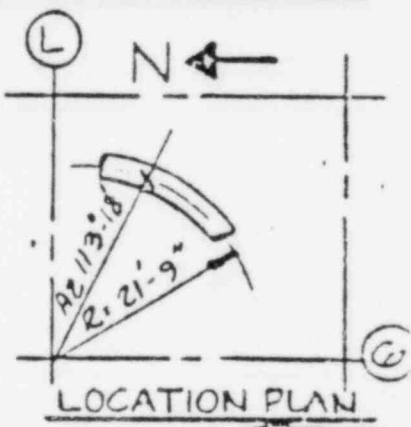
\* THIS STIFFENER IS WELDED ON THE WEST SIDE ONLY. THE EAST SIDE IS NOT WELDED AND IS NOW INACCESSIBLE FOR WELDING DUE TO ANOTHER STIFFENER LOCATED APPROX. 1' TO

NO.	QTY.	DESCRIPTION	SIZE-DESCRIPTION	THE EAST. 2" x 1/2"	ASTM	WT.
1	1	RIGID SWAY BRUT	SIZE 15 FIG. 250 X 0-8"			8
2	1	6" Ø PIPE CLAMP	M-202			21
3	1	WG X 25 X 1'-6" LG (FOR TO SHIT)			A-36	38
4	2	STIFF. R	1/4" x 1 7/8" x 0-5 1/16"		A-36	2
5	1	STIFF R	1/2" x 7 3/8" x 1'-0 9/16"		A-36	13
DEVIATION - NOTE 1						
TOTAL WT:						82



**SECT. A-A**

LOCATION PLAN FOR REFERENCE ONLY!  
 USE FAB. ISO. FOR CONSTRUCTION



B&R 150 M200-36  
 DATA PT. 215, P9 (LY)

1155

THERMAL MVTS.			ZONE: R-57	OP LD: -122#	HYDRO LD: -122#
PIPE	STRUCT.	NET	THERMAL LD: VERT: +730#	H-S: —	E-W: —
DX: .976'N	.039'S	1.015'N	SEISMIC LD: VERT: 3100#	H-S: —	E-W: —
DY: .453"UP	.453"UP	.000"UP	CODE/CLASS III/I	GROUP: I	O.A. LEVEL: I
DE: .558'E	.101'E	.457"E	PAINT: III	PIPE CALC. B.14.135	STL CALC. B.16.240#

2	REV'D BY B&R & IN ACCORDANCE WITH BGR STATUS AS-BUILT	Fig 82	TG	DATE	APVD
REV	REVISION	DATE	APVD		
OWN	CHKD	SCALE	NTS		

PIPING REACTOR WATER SYSTEM CLEAN UP  
 REF. DWG. 150. RWCU-812-57H PIPING: M-713  
 WASHINGTON PUBLIC POWER SUPPLY SYSTEM  
 HANFORD NO. 2 004.1 104  
 MARK NO. RWCU-1C-2PS L10-97

EDINACCORD ICE 1/21/83 SNP-P.P.6

EVALUATION OF SUPPORT NO. RWCU-10-2PS REV. 3FO  
 ISOMETRIC NO. RWCU-812-3.7 REV. 6

NOTES: 1) BURNS & ROE DWGS. # M-200 SHT # 36 & 36A HAS BEEN REVIEWED FOR DISCREPANCIES BETWEEN AS-ANALYZED & AS-BUILT PIPING CONFIGURATIONS. IT IS JUDGED THAT ANY SUCH DISCREPANCIES DO NOT SIGNIFICANTLY AFFECT CALCULATED SUPPORT LOADS.

MODIFIED LOAD

$$F_+ = (730 + 2291 - 122) \times 1.25 = 3624 \# \uparrow$$

$$F_- = (122 + 2291) \times 1.25 = 3016 \# \downarrow$$

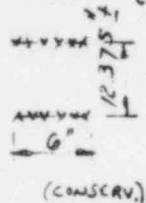
ANALYSIS / CONCLUSION

① PIPE CLAMP (DEVIATION 1C);  
 FROM REF. # 4, FOR 6"  $\phi$  PIPE CLAMP, STOCK SIZE =  $\frac{1}{2} \times 2 \frac{1}{2}$ "  
 ALLOW. = 4960 #

RATIOING,  $P = 4960 \times \frac{2}{2.5} = 3968 \#$  APPROX.  $> 3624 \#$ , OK  
 (FOR  $\frac{1}{2} \times 2 \frac{1}{2}$ " )

② CRITICAL DIMENSIONS (DEVIATION 2A);  
 NEW DIMENSION ( $1' - 4 \frac{3}{4}$ ") WILL NOT AFFECT STRUCTURAL INTEGRITY OF PIPE SUPPORT, HENCE OK.

③  $\frac{1}{4}$ " FILLET (DEVIATION 6A);



$A = 12$ "

$S_w = 6 \times 12.375 = 74.25$

$f_1 = \frac{3624}{12} = 302$

$f_2 = \frac{32163}{74.25} = 433$

$P = 3624 \#$

$M = 3624 \times (6.375 + 2.5) = 32.163 \text{ ''-}\#$

$f_R = 528 \# / \text{''}$

$q = \frac{528}{\frac{1}{4} \times 0.707} = 2987 \text{ PSI}$ , SMALL, HENCE OK

REFERENCES:

- ① Iso. RWCU-812-3.7 REV. 6.
- ② Iso. M200, SHT 36 REV. 7 and SHT. 36A REV. 0
- ③ HANGER DWG. RWCU-10-2PS REV. 3FO
- ④ NPS CATALOG 1981, P. 921.

PREPARER: E. de Guzman 8/5/83

REVIEWER: Joseph D. Miska 8-8-83



BURNS AND ROE, INC.

Headquarters Office - Camden, N.J.

W.O. No. 3900-75 Date 11/24/91 Book No. R.16.30A Page No. \_\_\_\_\_  
 Drawing No. PWCU-10-2PS Calc. No. R.16.3409 Sheet 3 Cont. on Sheet 4  
 By R. Bradlow Checked R. Za Approved \_\_\_\_\_  
 Title POSS NP2 PIPE SUPPORT REVIEW/REDESIGN PS MARK NO. PWCU-10-2PS

① CALC. 8.14.135, REV. 6; BK 8.14.135 ✓

LOADING CONDITIONS:

REV. 1 BY J. Baird 7/8/92  
 CRO: [Signature] 11/2/92

LOAD TYPE	POUNDS (1)			POUNDS-FT.		
	Fx	Fy	Fz	Mx	My	Mz
Thermal		730 <del>+500</del>				
Dead Wt.		122 <del>###</del>				
Misc.		—				
DYNAMIC LOADS	Normal	1048 <del>+1500</del>				
	Energ.	1263 <del>+1500</del>				
	Faulted	2291 <del>+3000</del>				
TOTAL LOADS	Normal	<del>+2000</del> 1900 -1000	-926			
	Energ.	<del>+2000</del> <del>-1000</del>	+2115 -1141			
	Faulted	<del>+3000</del>	-2413 -2169			
ORIGINAL SUPPORT DESIGN LOADS	Normal	+2000 -1000				
	Energ.	+2000 -1000				
	Faulted	±3000				

① THERMAL MOVEMENTS 1157  
 $\Delta_x = 0.0710$   $\Delta_z = 0.0000$   $\Delta_y = .0579$

SWEC ENGINEERING REVIEW CHECKLIST

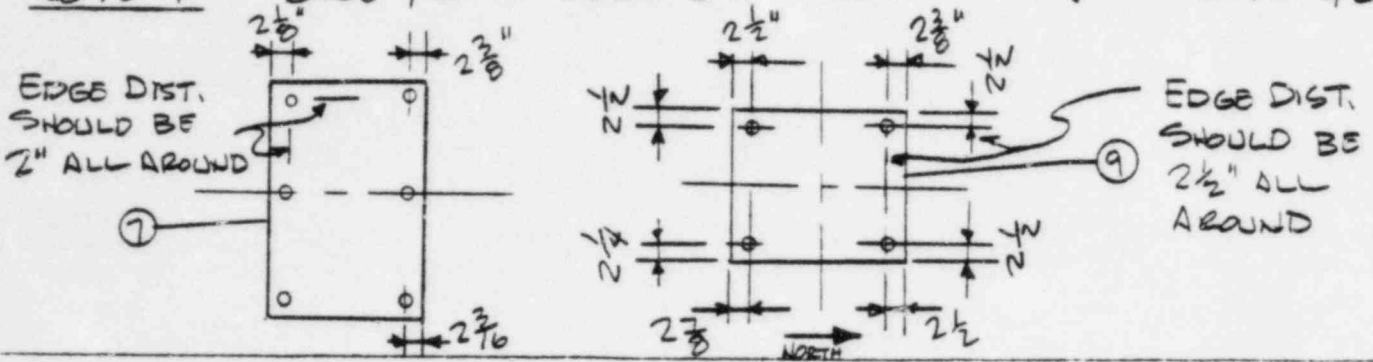
HANGER DRAWING NO. <b>SGT 901N</b>	REV. <b>1 FO</b>	INSPECTION TEAM <b>S JACOB J MENDEZ</b>	LEGEND: A - ACCEPTABLE D - DEVIATION EXISTS N/A - NOT APPLICABLE
ISOMETRIC NO. REV. <b>9 FO</b>	CHANGE DOCUMENT <b>PEO-215-H-P876</b>		
<b>SGT-62.3-4.7</b>			

CHECKLIST ITEMS	STATUS			COMMENTS
	N/A	A	D	
<b>1. General</b>				
A. Support Location		✓		
B. Support Orientation		✓		
C. Catalog Items		✓		
D. Close Clearance Gaps	✓			
<b>2. Support Structure</b>				
A. Critical Dimensions		✓		
B. Member sizes, structural plates		✓		
<b>3. Struts and Snubbers</b>				
A. Pin to Pin Dims., Snubber Setting		✓		
B. Rod-to-Pin Assembly Connections		✓		
<b>4. Baseplates</b>				
A. Plate & Gusset sizes		✓		
B. Bolt size & type		✓		
C. Bolt hole spacing			✓	SEE NOTE 1
D. Attachment location			✓	SEE NOTE 2
E. Bo spacing to adjacent inserts		✓		
5. Lugs - Bearing Surface	✓			
<b>6. Welding</b>				
A. Size, length, quality		✓		
B. Symbols		✓		
7. Miscellaneous (specify)	✓			

SWEC ENGINEERING REVIEW CHECKLIST

Notes:

NOTE 1: BASE PLATE EDGE DISTANCES VARY (ATTRIBUTE 4C)



Results of Evaluation:

\_\_\_\_\_ No deviations noted.

0 \_\_\_\_\_ Deviations evaluated as acceptable by inspection team.

2 \_\_\_\_\_ Deviations require further evaluation.

**ATTRIBUTES 4C & 4D**

Steve P. Joub      8-8-83  
 Signature                      Date

Disposition of deviations subject to further evaluation:

*see attached pages. Evaluations 4C & 4D judged acceptable. Load Factor is not applied.*

i177

E. de Guzman      8/22/83  
 Signature                      Date  
 Reviewer                      8/26/83

CALCULATION SHEET

▲ 5010.55 ATTACHMENT TO SWEC REVIEW CHECKLIST FOR HGR SGT 901N REV 1FO

CALCULATION IDENTIFICATION NUMBER				SHT. 1 OF 1 PAGE _____
J.O. OR W.O. NO.	DIVISION & GROUP	CALCULATION NO.	OPTIONAL TASK CODE	

NOTE 2: ATTRIBUTE 4 D

ATTACHMENT OF TUBES TO BASE PLATES IS AS INDICATED

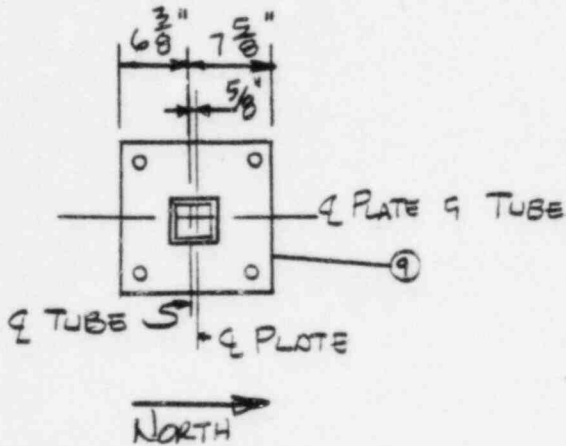
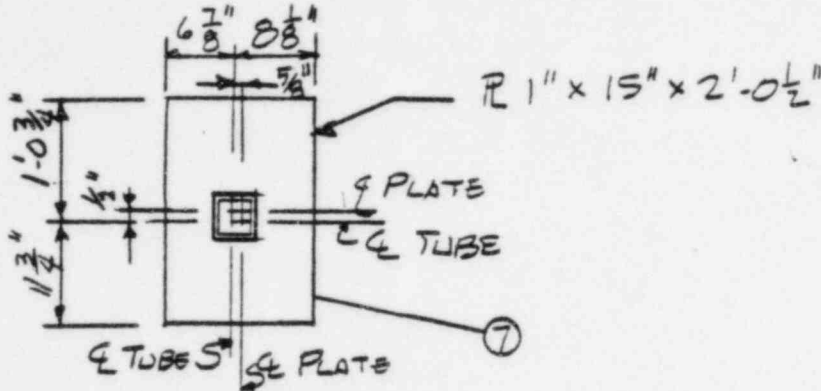
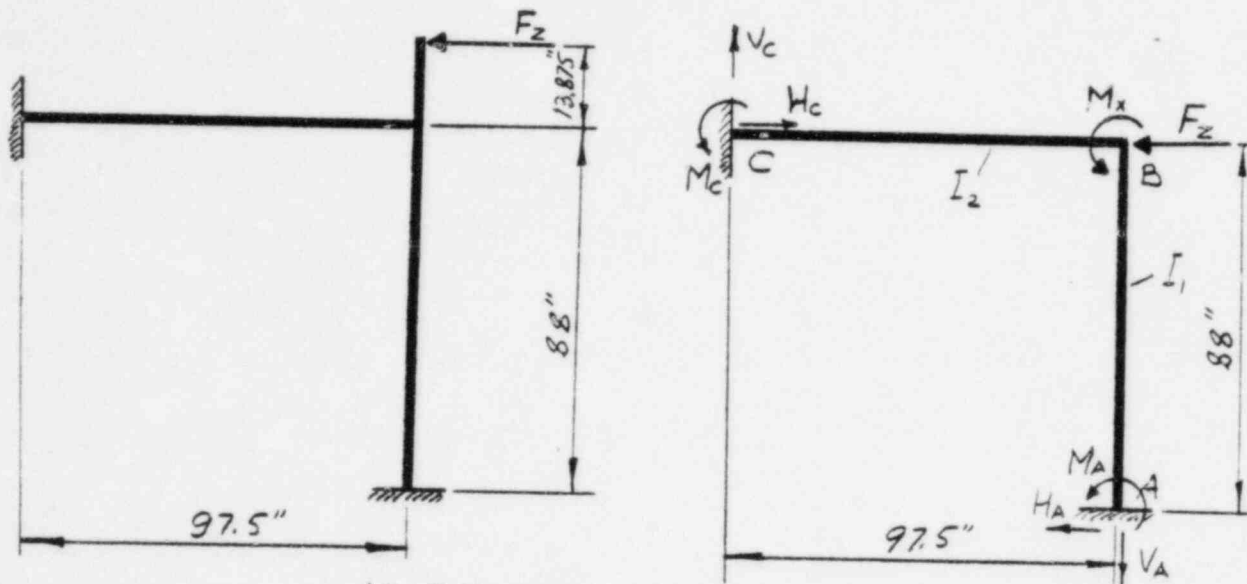


PLATE ⑨ ATTACHMENT DEVIATIONS WERE PREVIOUSLY ACCEPTED PER H-501 WHICH ALLOWS ECCENTRICITY UP TO 1"

SWEC ALLOWANCES ARE  $\pm 1/4$ "

EVALUATION OF SUPPORT NO. SGT-901N REV. 1FO  
 ISOMETRIC NO. SGT-62.3-4.7 REV. 9FO

CHECK LOADS @ Base A (REF 1)



MODEL FOR ANALYSIS

REACTIONS @ nodes A & C

1) FROM MOMENT Mx

$$K = \frac{I_2}{I_1} \times \frac{h}{L} = \frac{h}{L} = \frac{88}{97.5} = .9 \quad (I_1 = I_2 \text{ Ref. 2}) \quad N = K + 1 = .9 + 1 = 1.9$$

$$F_z = 8617 \# (\text{Ref. 3}) \quad M_x = F_z \times 13.875 = 119,561 \#-in$$

$$M_{B1} = \frac{M}{N} = \frac{119561}{1.9} = 62,927 \#-in \quad M_{B2} = \frac{M \times K}{N} = \frac{119561 \times (.9)}{1.9} = 56,634 \#-in$$

$$M_A = \frac{M_{B1}}{2} = \frac{62927}{2} = 31464 \#-in \quad M_C = \frac{M_{B2}}{2} = \frac{56634}{2} = 28317 \#-in$$

$$V_A = V_C = \frac{3M_C}{L} = \frac{3 \times 28317}{97.5} = 871 \# \quad H_A = H_C = \frac{3M_A}{h} = \frac{3 \times 31464}{88} = 1073 \#$$

REFERENCES

1. A. Kleinlogel "Rigid frame formulas"
2. AISC "STEEL CONSTRUCTION MANUAL"
3. B & R PIPE SUPPORT LOADS.
4. SWEC, SD-STEP-4, Rev. 1, nov 1979
5. SUPPORT DWG SGT-901N Rev 1FO
6. PIPING ISO SGT-62.3-4.7 Rev 9FO
7. SPEC CHANGE FCD CO 206-H-0145
8. ASME III, SECTION "F", app. 1

PREPARER: G. J. O'Neil DATE: 8/22/83  
 REVIEWER: E. de Guzman DATE: 9/22/83

EVALUATION OF SUPPORT NO. SGT-901NREV. 1FOISOMETRIC NO. SGT-62.3-4.7REV. 9FO2) FROM FORCE F<sub>2</sub>

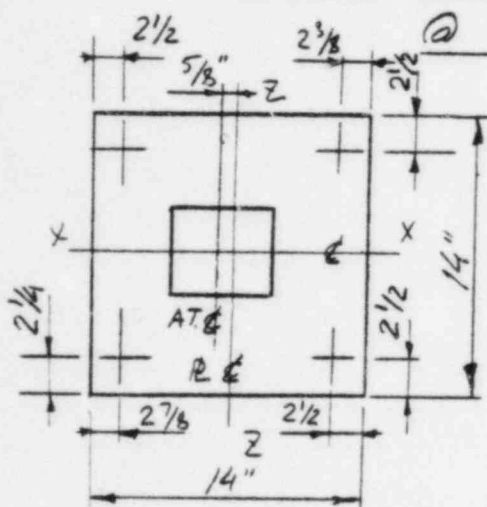
$$H_C = F_2 = 8617\# \quad H_A = 0 \quad V_A = V_C = 0 \quad M_A = M_C = 0$$

3) TOTAL REACTIONS @ NODES 'A' & 'C'

$$H_A = 1073\# \quad H_C = 1073 + 8617 = 9690\#$$

$$V_A = 871\# \quad V_C = 871\#$$

$$M_A = 31464\#-IN \quad M_C = 28317\#-IN$$

CHECK BASE PLATE & ANCHOR BOLTS (REF. 4)

$$F_x = 0 \quad F_y = 871\# \quad F_z = 1073\#$$

$$M_x = 31464\#-IN \quad M_y = M_z = 0$$

$$P_T = \frac{F_y}{N} = \frac{871}{4} = 218\# \quad LFT = 1.0$$

$$P_{m_x} = \frac{M_x}{N_i \times l_2} = \frac{31464}{2 \times 7} = 2247\# \quad LFM = 1.0$$

$$P_{max} = P_T + P_{m_x} = 218 + 2247 = 2465\# < 8740\# \text{ (REF. 7)}$$

$$*S = \frac{F_z}{N} = \frac{1073}{4} = 268\# < 6090\# \text{ (REF. 7)}$$

$$\left(\frac{2465}{8740}\right)^{5/3} + \left(\frac{268}{6090}\right)^{5/3} = .127 < 1.0 \quad \therefore \text{O.K.}$$

$$b_{eff} = 0.66 \times 7 + 5 = 9.62\text{IN} \quad M = P_{max} \times l_1 \times N = 2465 \times 4.5 \times 2 = 22185\#-IN$$

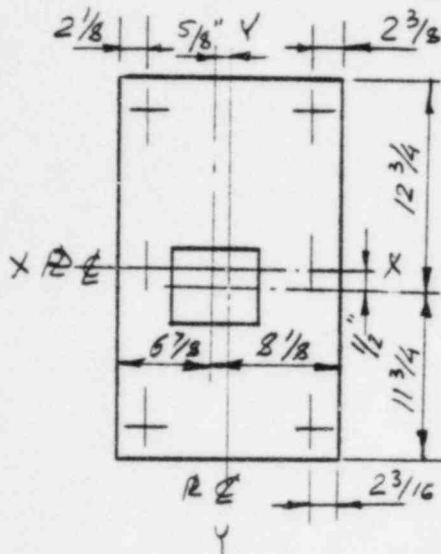
$$T_{req'd} = \left(\frac{6M}{b_{eff} \times G_{AL}}\right)^{1/2} = \left(\frac{6 \times 22185}{9.62 \times 27000}\right)^{1/2} = 0.72\text{IN} < 1.0 \quad \therefore \text{O.K.}$$

REFERENCES (see p. 1)

REF. 8

PREPARER: S. JohnstonDATE: 8/22/83REVIEWER: E. de GuayDATE: 9/22/73

+ Neglect 5/8" eccentricity due to small loads values

EVALUATION OF SUPPORT NO. SGT-901NREV. 1FOISOMETRIC NO. SGT-62.3-4.7REV. 9FOCHECK BASE PLATE ANCHOR BOLTS@ NODE 'C' (REF. 4)

$$F_x = 0 \quad F_y = 871 \# \quad F_z = 9690 \#$$

$$M_x = 28,317 \#-IN \quad M_y = M_z = 0$$

$$P_T = \frac{F_z}{N} = \frac{9690}{6} = 1615 \#$$

$$P_{mx} = \frac{M_x}{N \cdot e_z} = \frac{28317}{2 \times 12.25} = 1156 \# \quad (\text{conservative direction})$$

$$\frac{L_z}{e} = \frac{13.25}{1} = 13.25 \quad \frac{L_y}{e} = \frac{8.25}{1} = 8.25$$

$$L_{FM} = 1.4 \quad L_{FT} = 1.3$$

$$P_{max} = 1.4 \times 1156 + 1.3 \times 1615 = 3718 \# < 8740 \# (\text{REF. 7})$$

$$* S = \frac{F_y}{N} = \frac{871}{6} = 145 \# < 6090 \# (\text{REF. 7})$$

$$\left( \frac{3718}{8740} \right)^{5/3} + \left( \frac{145}{6090} \right)^{5/2} = .24 < 1.0 \quad \therefore \text{O.K.}$$

$$k_{eff} = 0.66 \times 12.25 + 5 = 13.1 \text{ IN} \quad P = P_T + P_{mx} = 1615 + 1156 = 2771 \#$$

$$M = P \times C_x \times N = 2771 \times 7.25 \times 2 = 40180 \#-IN$$

$$T_{req'd} = \left( \frac{6M}{k_{eff} \times \sigma_{AL}} \right)^{1/2} = \left( \frac{6 \times 40180}{13.10 \times 27000} \right)^{1/2} = .83 \text{ IN} < 1.0 \quad \therefore \text{O.K.}$$

REF 8

REFERENCES

See p. 1

PREPARER: S. J. J. J.DATE: 8/22/83REVIEWER: E. de GuzmanDATE: 8/22/83\* Neglect  $5/8$ " eccentricity due to small loads values

SWEC ENGINEERING REVIEW CHECKLIST

1/2

HANGER DRAWING NO. <b>VR-5</b>	REV. <b>2FO</b>	INSPECTION TEAM <b>JOE STOKES BRUCE BRUNSDON</b>	LEGEND: A - ACCEPTABLE D - DEVIATION EXISTS N/A - NOT APPLICABLE
ISOMETRIC NO. REV. <b>VR-666-1.3 5FO</b>	CHANGE DOCUMENT		

CHECKLIST ITEMS	STATUS			COMMENTS
	N/A	A	D	
<b>1. General</b>				
A. Support Location		✓		
B. Support Orientation		✓		
C. Catalog Items		✓		
D. Close Clearance Gaps		✓		
<b>2. Support Structure</b>				
A. Critical Dimensions		✓		
B. Member sizes, structural plates		✓		
<b>3. Struts and Snubbers</b>				
A. Pin to Pin Dims., Snubber Setting	✓			
B. Paddle - Pin Assembly Connections	✓			
<b>4. Baseplates (PLATE WELDED TO EMBEDDED PLATE)</b>				
A. Plate & Gusset sizes		✓		
B. Bolt size & type	✓			
C. Bolt hole spacing	✓			
D. Attachment location				✓ SEE NOTE 1 AND SKETCH ON SH. 2 OF 2
E. Bolt spacing to adjacent inserts	✓			
<b>5. Lugs - Bearing Surface</b>				
		✓		
<b>6. Welding</b>				
A. Size, length, quality				✓ SEE NOTE 2 #3 ON BACK
B. Symbols		✓		
<b>7. Miscellaneous (specify)</b>				
	✓			

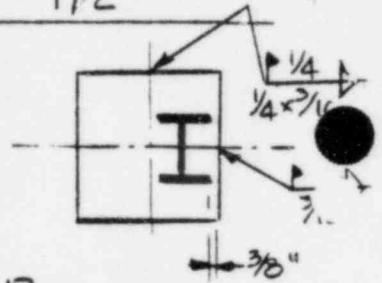
1282



Notes: 1.) ATTACHMENT IS OFFSET ON PLATE AS SHOWN:

2.) WELD OF ITEM ① TO EMBED. # IS AS SHOWN: THIS VIOLATES MIN. WELD REQUIREMENTS OF CODES.

3.) WELD BETWEEN BEAM ATTACHMENT AND PL. x 13 IS INADEQUATE. 1/8" GAP EXISTS BETWEEN THE TWO. WELDS ARE UNDERSIZED AND UNDERCUT. REFER TO ATTACHED SKETCH FOR DETAIL.



Results of Evaluation:

\_\_\_\_\_ No deviations noted.

\_\_\_\_\_ Deviations evaluated as acceptable by inspection team.

\_\_\_\_\_  
Signature

\_\_\_\_\_  
Date

Deviations require further evaluation:

SEE NOTES ABOVE

1283

DISPOSITION OF DEVIATIONS:

ANALYZED AND ACCEPTABLE

Ben S. Bul  
Signature  
Joe Stokes  
Signature  
Joseph D. m. lra  
Signature

7-21-83

7-21-83

Date

8-8-83

CALCULATION SHEET

▲ 5010.65 ATTACHMENT TO SWEC REVIEW CHECKLIST FOR HGR VR-9 REV 2FO

CALCULATION IDENTIFICATION NUMBER				SHT 1 OF 1 PAGE _____
J.O. OR W.O. NO.	DIVISION & GROUP	CALCULATION NO.	OPTIONAL TASK CODE	

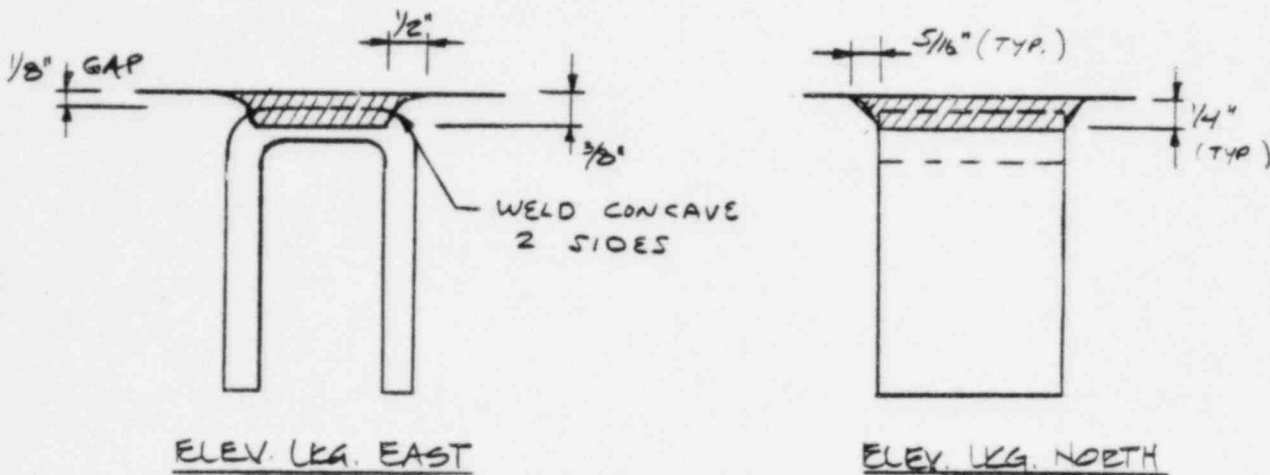
SWEC ENGRG REVIEW CHECKLIST

HANGER DWG. NO. YR-902N REV. 2FO

ISOMETRIC NO. YR-666-1.3 REV. 5FO

INSPECTION TEAM: JOE STOKES  
BRUCE BRUNSDON

WELD PROFILE - CONNECTION BETWEEN BEAM ATTACH.  
AND M4X13.



EVALUATION OF SUPPORT NO. VR-5 REV. 2 FO  
 ISOMETRIC NO. VR-666-1.3 REV. 5 FO

NOTES: 1) BURNS & ROE DWGS. # M-200 SHT. 122 HAS BEEN REVIEWED FOR DISCREPANCIES BETWEEN AS-ANALYZED & AS-BUILT\* PIPING CONFIGURATIONS. IT IS JUDGED THAT ANY SUCH DISCREPANCIES DO NOT SIGNIFICANTLY AFFECT CALCULATED SUPPORT LOADS.

\* AS-BUILT NOT AVAIL. COMPARED AS-ANALYZED TO MOST RECENT ISO. (I.E. REV. 4)

CHECK ON WELD BETWEEN M4X13 & BEAM ATT.

$$\text{MAX. LOAD} = (2320. \#) (1.25) = 2900. \# = F_3 \quad (\text{REF. 1})$$

↳ MODIFIED LOAD FACTOR

$$f_w (\text{WELD FORCE}) = F_3 / A_w$$

$$A_w = 2 (1\frac{3}{8} \text{''}) = 2.75 \text{''}$$

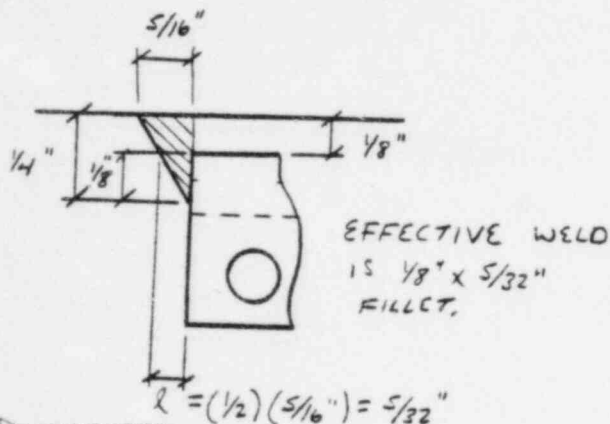
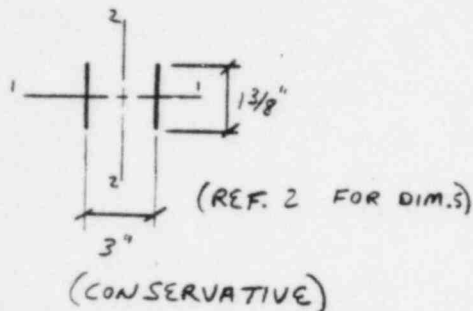
$$f_w = (2900 \#) / (2.75 \text{''}) = 1055. \# / \text{IN}$$

$$f_{wA} (\text{WELD ALLOW.}) = 18,000. \text{ PSI} \quad (\text{REF. 3})$$

$$t_r (\text{REQ'D THROAT}) = f_w / f_{wA} = .059 \text{''}$$

$$t (\text{ACTUAL THROAT}) = \left( \frac{5}{32} \text{''} \right) \cos \left[ \arctan \left( \frac{5/32 \text{''}}{1/8 \text{''}} \right) \right] = .098 \text{''}$$

$t > t_r$ , SO WELD IS ACCEPTABLE



REFERENCES

- (1) B&R CALC. NO. 8.16.1367
- (2) POWER PIPING CATALOG
- (3) ASME III SUB. NF
- (4) DESIGN OF WELDED STRUCTURES - BY O.W. BLODGETT - 1976

(5) LETTER FROM H.A. CRISP TO

J.F. NEWGEN, DATED JUNE PREPARER: Joseph D. Miba 8-8-83

7, 1983 - NO. WPBEC-CO500-F-83-2153.

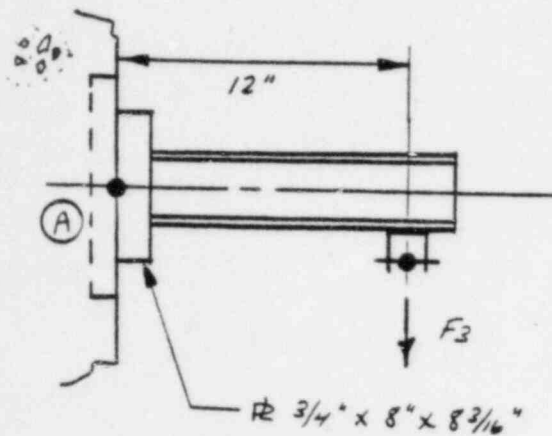
REVIEWER: Bruce Fischer 8-8-83

(6) HANGER DWG.

1285

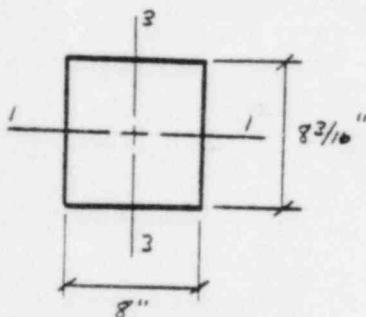
EVALUATION OF SUPPORT NO. VR-5 REV. 2 FO  
 ISOHERIC NO. VR-666-1.3 REV. SFO

RESOLVE LOADS TO EMBED. PLATE



PT. A MAX. LOADS: (REF. P. 1)  
 $F_3 = 4493. \#$   
 $M_1 = (12") (F_3) = 53,916. \text{ IN} \#$

CHECK ON WELD BETWEEN EMB. PLATE & #3/4 x 8 x 8 3/16



3/16" FILLETS  
 (CONSERV.)

WELD PROPERTIES: (REF. 4)

$$A_w = 2(8" + 8^{3/16} ") = 32.38"$$

$$S_w = (8")(8^{3/16} ") + (8^{3/16} ")^2 / 3 = 87.85 \text{ IN}^2$$

FORCE ON WELD: (REF. 4)

$$f_w = [(M_1 / S_w)^2 + (F_3 / A_w)^2]^{1/2}$$

$$f_w = 629. \# / \text{IN}$$

REQUIRED WELD SIZE (W): (REF. 3)

$$W = f_w / (.707) (18,000. \text{ PSI}) = .049" < 3/16"$$

SO ACCEPTABLE

CHECK ON ATTACHMENT LOCATION:

DUE TO THE LOADS IMPOSED ON #3/4" x 8" x 8 3/16", THE OFFSET OF THE ATTACHMENT WILL HAVE A NEGLIGIBLE EFFECT AND SO IS JUDGED ACCEPTABLE.

NOTE:

MIN. WELD VIOLATIONS  
 ACCEPTABLE PER REF. 5.

PREPARED: Joseph O. M'Jag 8-8-83

REVIEWER: Bruce Fischer 8-8-83

SUPPORT CHECKLIST

HANGER DRAWING NO. <b>TSW-355</b>	REV. <b>1</b>	INSPECTION TEAM <b>J. STOKES S. JACOB</b>	LEGEND: A - ACCEPTABLE D - DEVIATION EXISTS N/A - NOT APPLICABLE
--------------------------------------	------------------	--	---

ISOMETRIC NO. <b>TSW-251-15.19</b>	REV. <b>5</b>	CHANGE DOCUMENT	
---------------------------------------	------------------	-----------------	--

CHECKLIST ITEMS	STATUS			COMMENTS
	N/A	A	D	

1. General

A. Support Location			<input checked="" type="checkbox"/>	NOTE 1 ON BACK
B. Support Orientation			<input checked="" type="checkbox"/>	
C. Catalog Items	<input checked="" type="checkbox"/>			
D. Close Clearance Gaps			<input checked="" type="checkbox"/>	

2. Support Structure

A. Critical Dimensions			<input checked="" type="checkbox"/>	
B. Member Sizes, Structural Plates			<input checked="" type="checkbox"/>	

3. Struts and Snubbers

A. Pin to Pin Dimensions, Snubber Setting	<input checked="" type="checkbox"/>			
B. Paddle-Pin Assembly Connections	<input checked="" type="checkbox"/>			

4. Baseplates

A. Plate & Gusset Sizes			<input checked="" type="checkbox"/>	
B. Bolt Size & Type			<input checked="" type="checkbox"/>	
C. Bolt Hole Spacing			<input checked="" type="checkbox"/>	NOTE 3; ATTACHED
D. Attachment Location			<input checked="" type="checkbox"/>	NOTES 4, 5; ATTACHED
E. Bolt Spacing to Adjacent Inserts			<input checked="" type="checkbox"/>	

5. Lugs - Bearing Surface

	<input checked="" type="checkbox"/>			
--	-------------------------------------	--	--	--

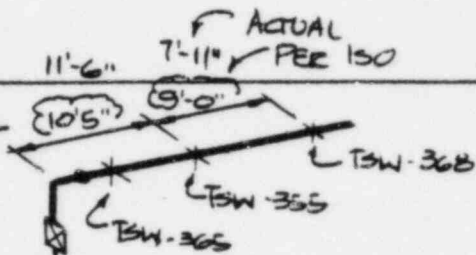
6. Welding

A. Size, Length, Quality			<input checked="" type="checkbox"/>	NOTES 4, 5, 6, 7; ATTACHED
B. Symbols			<input checked="" type="checkbox"/>	NOTES 6, 7; ATTACHED

7. Miscellaneous (Specify)

INSULATION INTERFERENCES			<input checked="" type="checkbox"/>	NOTE 2, ON BACK U 6308
--------------------------	--	--	-------------------------------------	---------------------------

Notes: 1) PER ATTRIBUTE 1A) SEE ISOMETRIC  
DIMENSIONS SHOW  $\pm 1'-1"$  IN  
OFFSETTING DEVIATIONS



- 2) BECAUSE OF INSULATION, THE FOLLOW-  
ING ITEMS COULDN'T BE VERIFIED:
- PADS WELDED TO PIPE AND THEIR WELDS
  - WELDS BETWEEN PADS AND TSG X G  $\times 3/8$
  - LENGTHS OF MEMBERS 6 & 7 (TSG X G  $\times 3/8$ )

Results of Evaluation:

No deviations noted.

Deviations evaluated as acceptable by inspection team.

ATTRIBUTE 7) ITEMS NOT ABLE TO BE VERIFIED

11 Deviations require further evaluation.

ATTRIBUTES 1A (1); 4C (1); 4D (2); 6A (4); 6B (3) SEE NOTES ABOVE  
AND ON ATTACHED SHEETS

JOE STOKES  
Signature

09-08-83  
Date

Disposition of deviations subject to further evaluation:

1A - DUE TO THE PIPING CONFIGURATION THE LOCATION DEVIATION  
IS JUDGED TO HAVE A NEGLIGIBLE EFFECT ON THE SUPPORT  
LOADS OR THE PIPE STRESS.

4C }  
4D } - JUDGED TO BE ACCEPTABLE AS PER  
6A } ATTACHED EVALUATION  
6B }

0 0309

Bruce Fischer  
Signature

9/21/83  
Date

REVIEWER

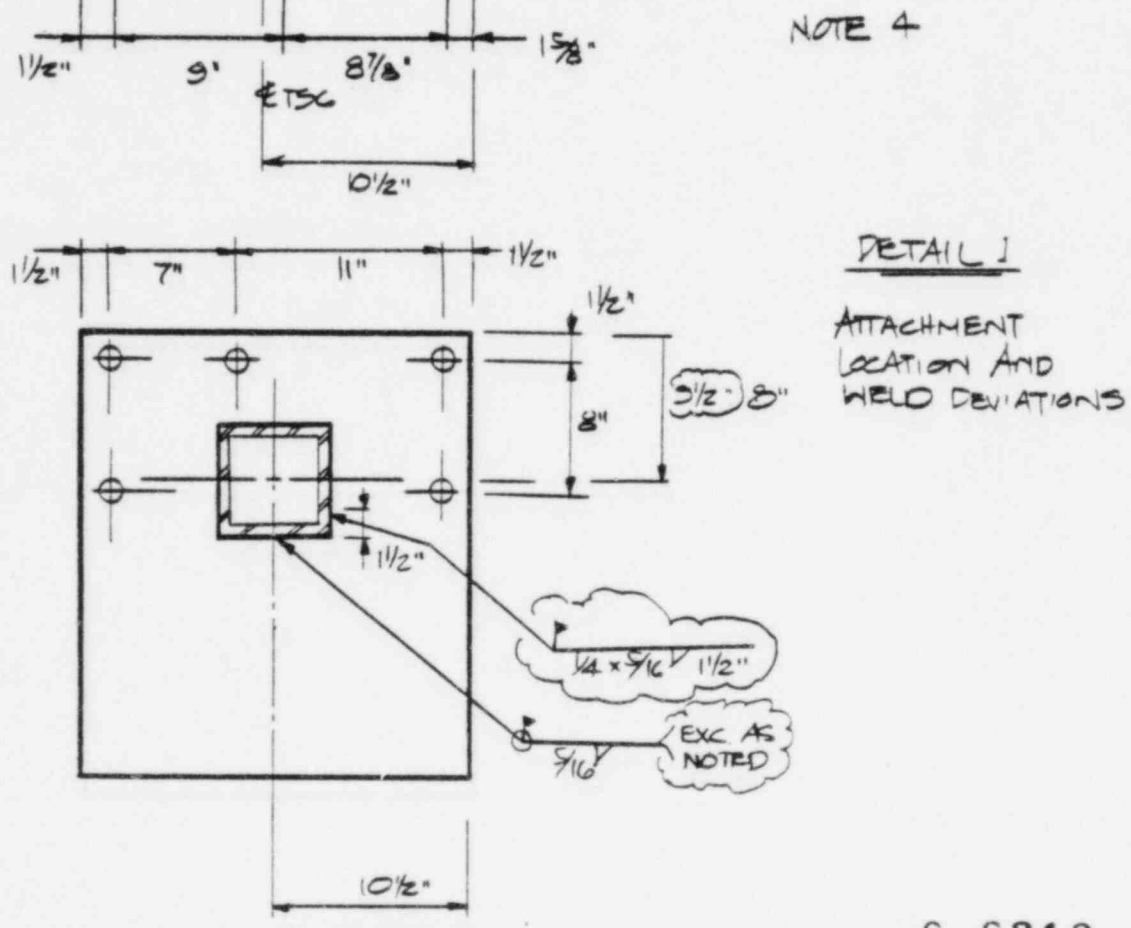
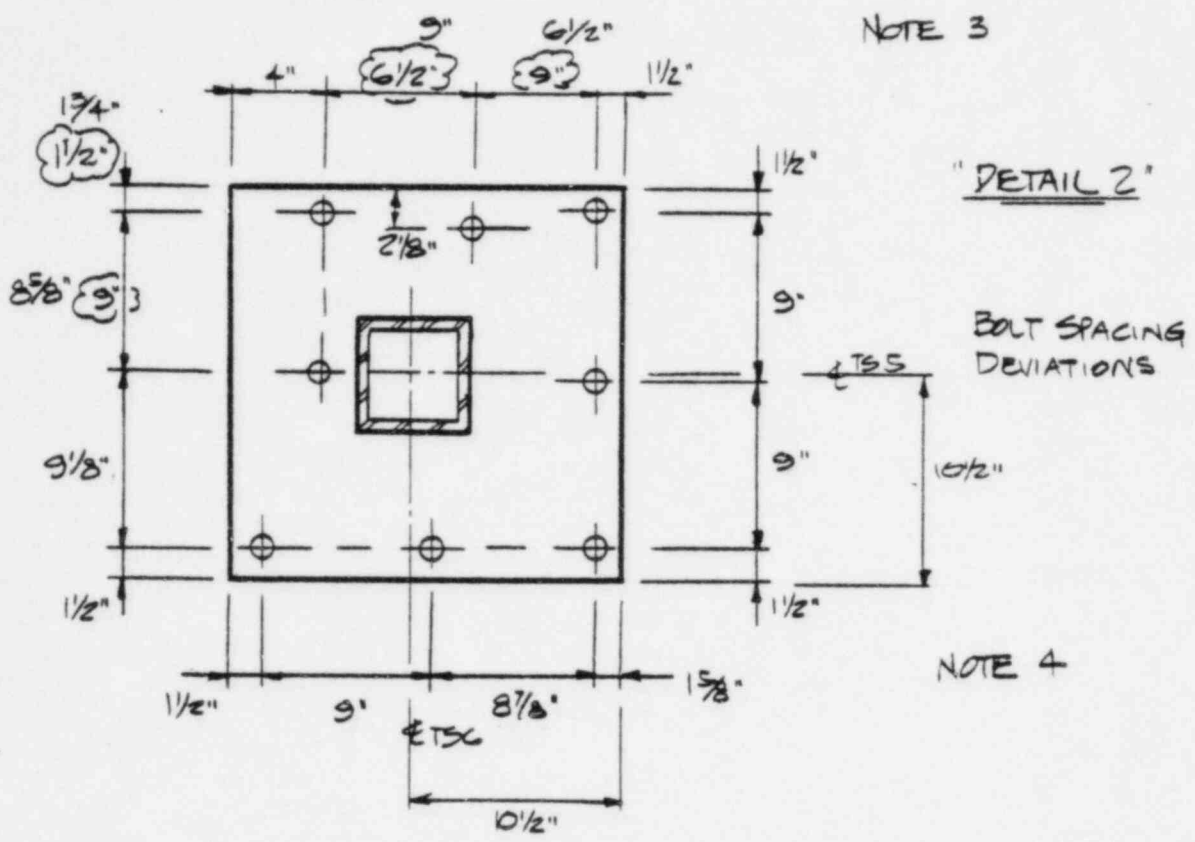
DATE

STONE & WEBSTER ENGINEERING CORPORATION  
 CALCULATION SHEET

▲ 5010.65

CALCULATION IDENTIFICATION NUMBER				
J.O. OR W.O. NO.	DIVISION & GROUP	CALCULATION NO.	OPTIONAL TASK CODE	PAGE ____

1  
2  
3  
4  
5  
6  
7  
8  
9  
10  
11  
12  
13  
14  
15  
16  
17  
18  
19  
20  
21  
22  
23  
24  
25  
26  
27  
28  
29  
30  
31  
32  
33  
34  
35  
36  
37  
38  
39  
40  
41  
42  
43  
44  
45  
46



STONE & WEBSTER ENGINEERING CORPORATION  
 CALCULATION SHEET

▲ 5010.65

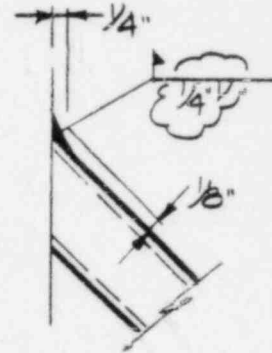
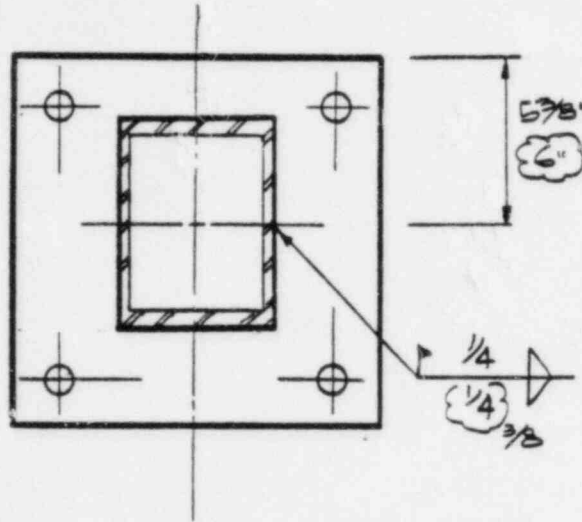
CALCULATION IDENTIFICATION NUMBER				PAGE ____
J.O. OR W.O. NO.	DIVISION & GROUP	CALCULATION NO.	OPTIONAL TASK CODE	

1  
2  
3  
4  
5  
6  
7  
8  
9  
10  
11  
12  
13  
14  
15  
16  
17  
18  
19  
20  
21  
22  
23  
24  
25  
26  
27  
28  
29  
30  
31  
32  
33  
34  
35  
36  
37  
38  
39  
40  
41  
42  
43  
44  
45  
46

NOTE 5

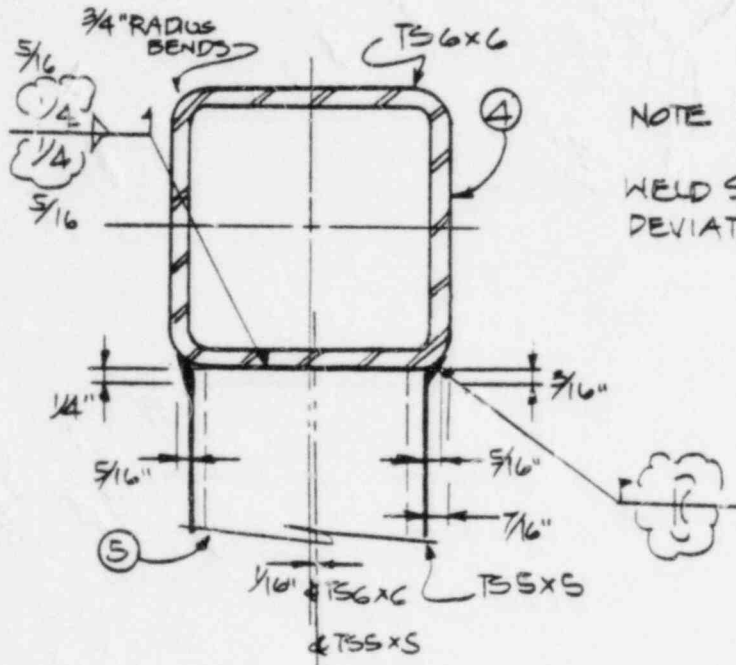
"DETAIL 14"

ATTACHMENT LOCATION  
 AND WELD DEVIATIONS



NOTE 6

WELD SIZE AND SYMBOL  
 DEVIATION



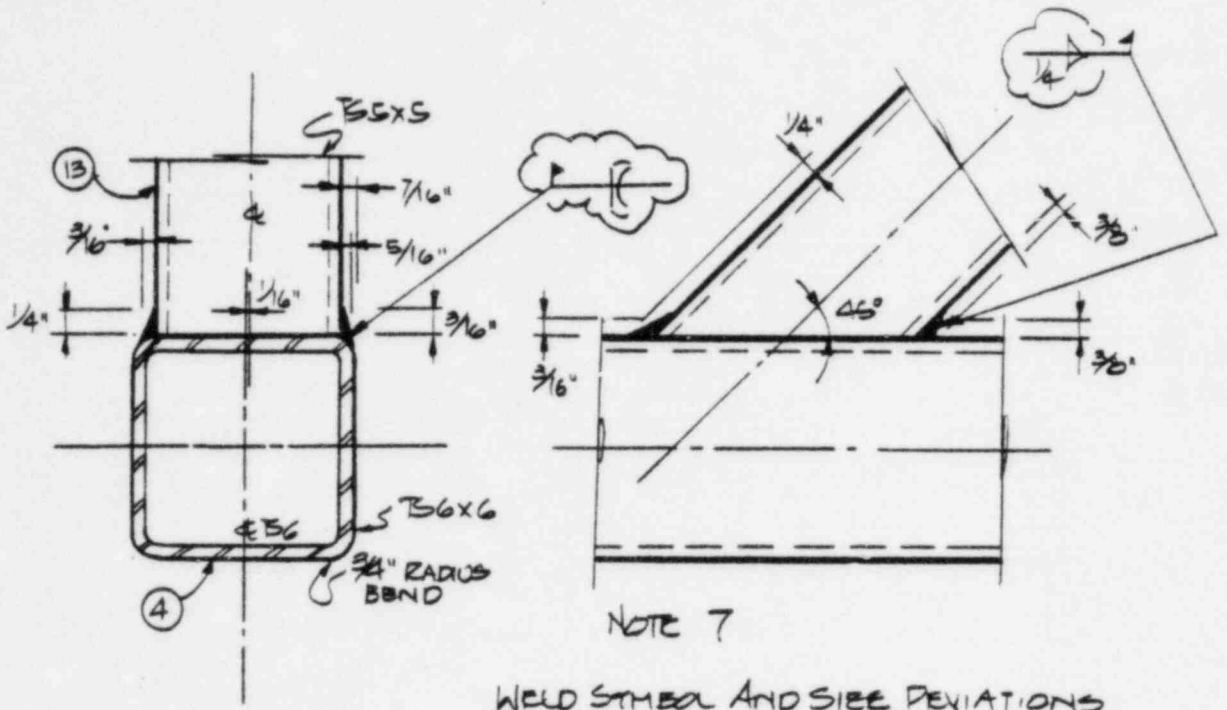


STONE & WEBSTER ENGINEERING CORPORATION  
 CALCULATION SHEET

▲ 5010.65

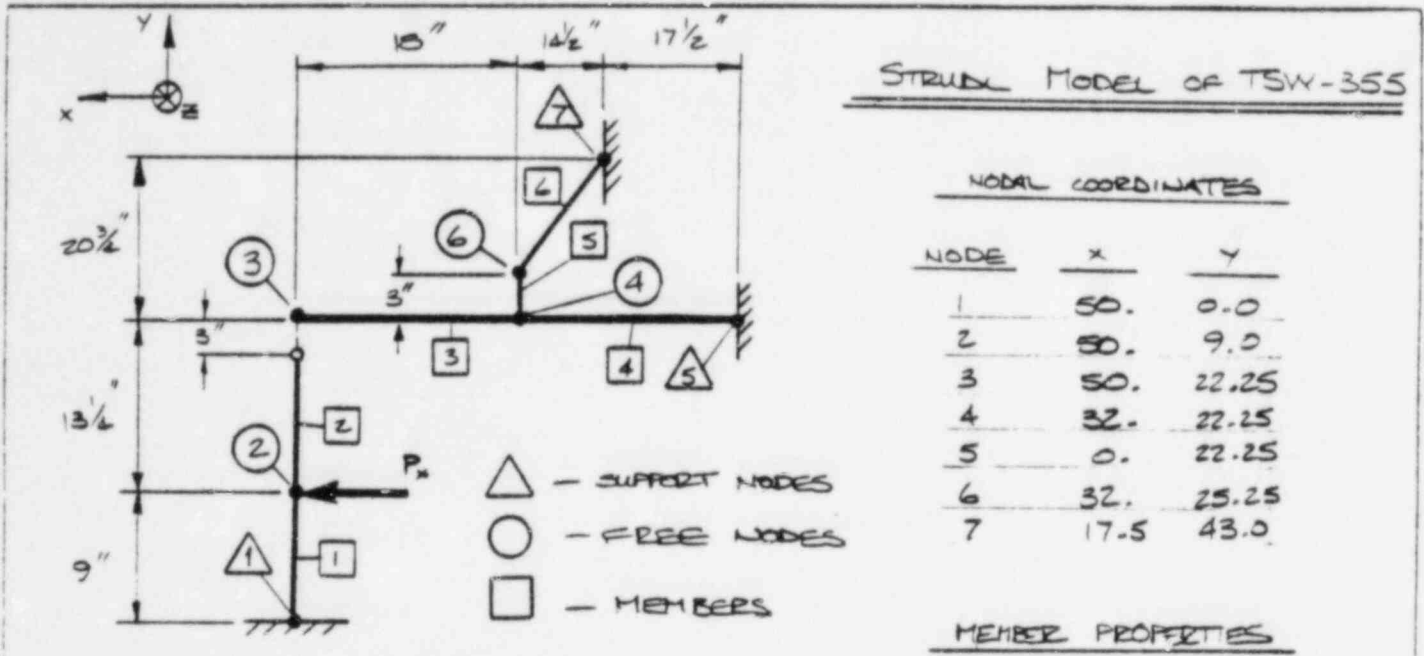
CALCULATION IDENTIFICATION NUMBER				PAGE ____
J.O. OR W.O. NO.	DIVISION & GROUP	CALCULATION NO.	OPTIONAL TASK CODE	

1  
2  
3  
4  
5  
6  
7  
8  
9  
10  
11  
12  
13  
14  
15  
16  
17  
18  
19  
20  
21  
22  
23  
24  
25  
26  
27  
28  
29  
30  
31  
32  
33  
34  
35  
36  
37  
38  
39  
40  
41  
42  
43  
44  
45  
46



0 0312

EVALUATION OF SUPPORT NO. TSW - 355 REV. 1  
 ISOMETRIC NO. TSW-251-15.19 REV. 5



NOTE: FIELD VERIFIED DIMENSIONS USED IN STRUT MODEL.

APPLIED LOAD

PER REF. 1, PG. 3

$P_x = \pm 30,322 \text{ LB.}$

REFERENCES

- 1) BURNS AND ROE CALC. NO. 8.16.3278 REV. 0
- 2) JWEC DOCUMENT SD-STEP 4, REV. 1
- 3) HFR DWG TSW-355 REV. 1
- 4) BURNS AND ROE DESIGN GUIDE, APPENDIX 2
- 5) BLODGETT, 'DESIGN OF WELDED STRUCTURES', 8<sup>th</sup> ED.
- 6) ASME III, SUBSECTION NF AND APPENDIX F
- 7) AISC MANUAL OF STEEL CONSTRUCTION, 8<sup>th</sup> ED.

PREPARER: Bruce Fischer DATE: 9/21/83

REVIEWER: B. J. Bahr DATE: 9/21/83

STRIDL INPUT

TYPE SPACE FRAME

UNITS IN LC DEG

CONSTANTS

E 2000000.0, ALL

V 1120000.0, ALL

POISSONS RZ ALL

JOINT COORDINATES

1 20.0 9.00 3

2 20.0 9.00

3 20.0 22.25

4 22.0 22.25

5 0.0 22.25 3

6 22.0 22.25

7 17.0 22.00 3

MEMBER INCIDENCES

1 1 2

2 2 2

3 3 4

4 4 2

5 4 6 4 DUMMY MEMBER

6 6 7

MEMBER 2 END JOINT SIZE END 3.0

MEMBER PROPERTIES PRISMATIC

1 2 6 TABLE \*SITUBEL\* \*12X21.94\*

3 4 TABLE \*SITUBEL\* \*16X27.04\*

5 AX 200. AY 500. AZ 500. EX 1000. EY 1000. EZ 1000.

LOADING 1 \*FAULTED\*

JOINT LOADS

1 2 FORCE A 30322.

0 0314

\*\*\*\*\*  
 \*RESULTS OF LATEST ANALYSES\*  
 \*\*\*\*\*

PROBLEM - MWP-2 TITLE - SIMUL ANALYSIS OF ISM-325

ACTIVE UNITS INCH LB DEG DEGF SEC

ACTIVE STRUCTURE TYPE SPACE FRAME

ACTIVE COORDINATE AXES X Y Z

LOADING - 1 FAULTED

MEMBER FORCES

MEMBER	JOINT	AXIAL	SHEAR X	SHEAR Y	SHEAR Z	TORSIONAL	MOMENT BENDING X	MOMENT BENDING Y	MOMENT BENDING Z
1	1	-3348.73364	20475.2500	0.0	0.0	0.0	0.0	0.0	103809.375
1	2	3348.73364	-20475.2500	0.0	0.0	0.0	0.0	0.0	80467.8750
2	2	-3348.73364	9846.74609	0.0	0.0	0.0	0.0	0.0	-80467.8750
2	3	3348.73364	-9846.74609	0.0	0.0	0.0	0.0	0.0	-50001.5234
3	3	-9846.74609	3348.73364	0.0	0.0	0.0	0.0	0.0	-50001.5234
3	4	9846.74609	-3348.73364	0.0	0.0	0.0	0.0	0.0	-10275.6856
4	4	-6621.67187	58.3300629	0.0	0.0	0.0	0.0	0.0	-1409.71973
4	5	6621.67187	-58.3300629	0.0	0.0	0.0	0.0	0.0	-456.842524
5	4	-3290.40326	3025.07373	0.0	0.0	0.0	0.0	0.0	-11685.4042
5	6	3290.40326	-3025.07373	0.0	0.0	0.0	0.0	0.0	2610.16188
6	6	-4402.01923	261.094482	0.0	0.0	0.0	0.0	0.0	2610.16188
6	7	4402.01923	-261.094482	0.0	0.0	0.0	0.0	0.0	3374.02563

RESULTANT JOINT LOADS - SUPPORTS

JOINT	A FORCE	Y FORCE	Z FORCE	X MOMENT	Y MOMENT	Z MOMENT
1	-20475.2500	-3348.73364	0.0	0.0	0.0	103809.375
5	6621.67187	58.3300629	0.0	0.0	0.0	456.842524
7	-3025.07373	3290.40326	0.0	0.0	0.0	-3374.02563

EVALUATION OF SUPPORT NO. TSW-355 REV. 1  
 ISOMETRIC NO. TSW-251-15.19 REV. 5

REF.

2

EVALUATION OF DEVIATION 4C:



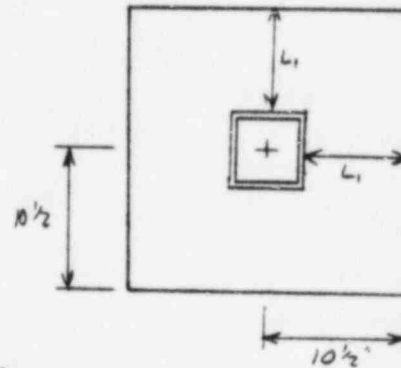
3

$$L_1 = 10\frac{1}{2} - \frac{5}{2} = 8 \text{ in}$$

$$t = 1 \text{ in}$$

$$\frac{L_1}{t} = 8 > 2$$

∴ RIGID SOLUTION UNACCEPTABLE



LOADS (SEE PG. 3 - SUPPORT LOADS, JOINT 1)

THE SIGNS ON THE LOADS HAVE BEEN REVERSED SO THAT THEY NOW REFLECT THE LOADS ON THE BASE PLATE. (NOTE: UNI-DIRECTIONAL LOADING)

$$F_x = 20475 \text{ lb}$$

$$M_x = 0 \text{ in-lb}$$

$$F_y = 3349 \text{ lb}$$

$$M_y = 0 \text{ in-lb}$$

$$F_z = 0 \text{ lb}$$

$$M_z = -103,809 \text{ in-lb}$$

REFERENCES

PREPARER: Bruce Fischer DATE: 7/21/83

REVIEWER: Lu E. Bahr DATE: 7/21/83

0 0316

EVALUATION OF SUPPORT NO. TSW-355 REV. 1  
 ISOMETRIC NO. TSW-251-15,19 REV. 5

REF.

COMPUTE SHEAR/BOLT

Z

$$S = \frac{F_x}{N} = \frac{20475}{8} = 2559 \text{ lb}$$

COMPUTE TENSION/BOLT

DUE TO THE DIRECTION OF THE BENDING MOMENT, ONLY THE BOLTS ON THE RIGHT SIDE OF THE PLATE GET ANY TENSION LOAD FROM THE MOMENT.

$$l_2 = 13 \text{ in}$$

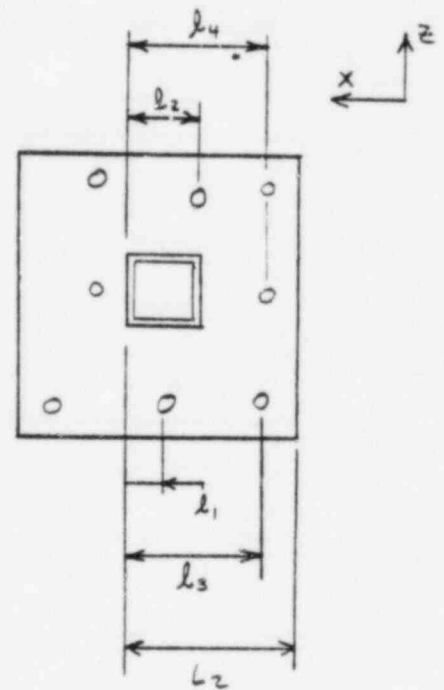
$$l_1 = 2.5 \text{ in}$$

$$l_2 = 5 \text{ in}$$

$$l_3 = 11.375 \text{ in}$$

$$l_4 = 11.5 \text{ in}$$

NOTE: ALL DIMENSIONS VERIFIED IN FIELD.



$$P_{Mz} = \frac{M_z l_4}{n_1 l_1^2 + n_2 l_2^2 + n_3 l_3^2 + n_4 l_4^2}$$

$$= \frac{103,809 (11.5)}{(2.5)^2 + (5)^2 + (11.375)^2 + 2(11.5)^2}$$

$$= 2808 \text{ lb}$$

REFERENCES

H&R. DWGS. TSW-355, REV. 1

PREPARER: Bruce Fisher DATE: 9/21/83  
 REVIEWER: Ben P. Bahr DATE: 9/21/83

0 0317

EVALUATION OF SUPPORT NO. TSW-355 REV. 1  
 ISOMETRIC NO. TSW-251-15.19 REV. 5

REF.

2

$$P_T = \frac{F_y}{N} = \frac{3347}{8} = 419 \text{ lb}$$

COMPUTE LOAD FACTORS

$$\frac{L_2}{E} = \frac{13}{1} = 13 \Rightarrow LFM = 1.35$$

$$\frac{L_1}{E} = 8 \Rightarrow LFT = 1.65$$

COMPUTE MAXIMUM TENSION/BOLT

$$\begin{aligned} P_{MAX} &= LFM(P_{H2}) + LFT(P_T) \\ &= 1.35(2808) + 1.65(419) \\ &= 4482 \text{ lb} \end{aligned}$$

BOLT ALLOWABLES ( $\frac{3}{4}$ "  $\emptyset$ , HDI BOLTS)

$$\begin{aligned} T_A &= 6040 \text{ lb} \\ S_A &= 5870 \text{ lb} \end{aligned}$$

BOLT INTERACTION

$$I = \frac{P_{MAX}}{T_A} + \frac{S}{S_A} = \frac{4482}{6040} + \frac{2559}{5870} = 1.18 > 1.0 \therefore \text{DO FINITE ELEMENT ANALYSIS OF B.E.}$$

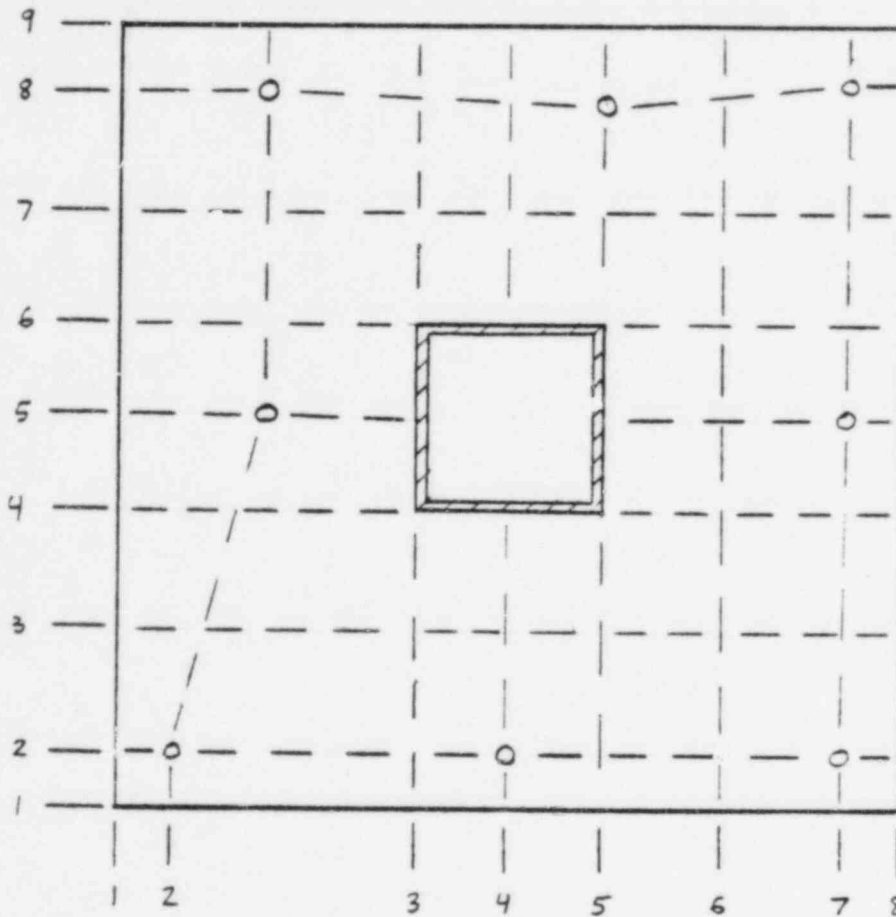
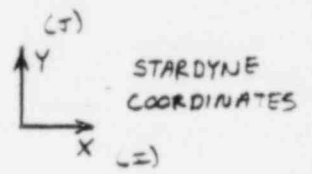
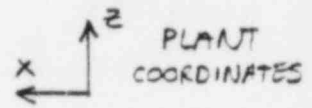
3+4

REFERENCES

PREPARER: Bruce Fischer DATE: 9/21/83  
 REVIEWER: Don S. Eubank DATE: 9/21/83

0 C318

STARDYNE MODEL



Bruce Fischer 9/13/83

0 0319



83/09/21. 11.43.33.

VERSION 1.0 (FEB.82 RELEASE)

PA

\*\*\*\*\*  
 \*  
 \* PREPROCESSOR INPUT DATA CARDS  
 \*  
 \*\*\*\*\*

CAKU	1	2	3	4	5	6	7
NO	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890
1	3 BASEPLT ANAL FOR HGR TSW-355, BAF						
2	OUT 0,0,0,-1,1,0/						
3	LON ,,, ,3500/						
4	PLA 8,9,,21,21,1./						
5	APR ,,1,5,5,.375,.375/						
6	BPR ,,1,980000,302000,0040,5870/						
7	BOL 2,2,1,1.5,1.5/						
8	BOL 4,2,1,10.5,1.5/						
9	BOL 7,2,1,19.375,1.5/						
10	BOL 2,5,1,4.,10.025/						
11	BOL 7,5,1,19.5,10.5/						
12	BOL 2,8,1,4.,19.25/						
13	BOL 5,8,1,13.,18.875/						
14	BOL 7,8,1,19.5,19.5/						
15	END /						
16	TUB 4,5,1,10.5,10.5/						
17	END /						
18	PUI 4,5/						
19	LUA ,,1,-20475,,3349,, -103809,/						
20	END /						
21	END OF JOB						

END OF INPUT DATA

Bruce Fisher 9/21/83

0 0320

ELEM	SIGMA	ELEM	SIGMA	ELEM	SIGMA	ELEM	SIGMA
1	371.	2	658	3	404.	4	191.
5	392.	6	382.	7	617.	8	203.
9	2287.	10	2976.	11	5427.	12	8190.
13	8849.	14	6611.	15	3094.	16	1849.
17	2215.	18	3066.	19	2925.	20	248.
21	248.	22	3316.	23	3331.	24	1870.
25	1270.	26	3987.	27	6008.	28	458.
29	401.	30	7712.	31	4603.	32	1937.
33	4246.	34	5908.	35	12533.	36	14808.
37	14793.	38	12652.	39	4603.	40	2106.
41	2422.	42	2935.	43	4940.	44	4339.
45	4473.	46	5219.	47	2904.	48	2125.
49	760.	50	1229.	51	2454.	52	1960.
53	2361.	54	3289.	55	1579.	56	1062.

QUADRILATERAL PLATE  
PRINCIPAL STRESSES

ELEMENT - Z FACE

ELEM	SIGMA	ELEM	SIGMA	ELEM	SIGMA	ELEM	SIGMA
1	891.	2	445.	3	748.	4	1136.
5	1237.	6	951.	7	555.	8	209.
9	1694.	10	2284.	11	4792.	12	6568.
13	7192.	14	5755.	15	2378.	16	1529.
17	2320.	18	2633.	19	1836.	20	140.
21	140.	22	1932.	23	2804.	24	1796.
25	1606.	26	3441.	27	5707.	28	243.
29	244.	30	6714.	31	3105.	32	2169.
33	3907.	34	5740.	35	11294.	36	13304.
37	13325.	38	11163.	39	4641.	40	1030.
41	1466.	42	2993.	43	5565.	44	3503.
45	3770.	46	5908.	47	3159.	48	1105.
49	576.	50	2004.	51	4252.	52	1137.
53	1453.	54	4523.	55	2388.	56	640.

MAX. COMPRESSION SUMMARY

NUDE	Z-FORCE
14	-1796.
15	-1649.
13	-1409.
16	-857.
12	-640.
19	-327.
17	-237.
27	-193.
63	-189.
64	-158.

Bruce Fischer 9/21/83

B U L T L O A D S

I LINE	J LINE	NODE	X-SHEAR	Y-SHEAR	SRSS	Z-TENSION
2	2	11	-2516.	-57.	2517.	0.
2	5	14	-2632.	-9.	2632.	0.
2	8	17	-2569.	11.	2569.	0.
4	2	29	-2531.	0.	2531.	1714.
5	8	44	-2500.	-1.	2500.	2688.
7	2	50	-2517.	59.	2518.	1260.
7	5	59	-2604.	17.	2604.	5011.
7	8	62	-2540.	-27.	2540.	821.

B O L T I N T E R A C T I O N E Q U A T I O N S

$$EQUATION = ((TENSION/TALLOW)** 1.000 + (SHEAR/VALLOW)** 1.000)** 1.000$$

NODE	TENSION ALLOW	SHEAR ALLOW	TENSION RATIO	SHEAR RATIO	INTERACTION FACTOR	SAFETY FACTOR
11	6040.	5870.	0.000	.429	.429	2.33
14	6040.	5870.	0.000	.448	.448	2.23
17	6040.	5870.	0.000	.438	.438	2.29
29	6040.	5870.	.284	.431	.715	1.40
44	6040.	5870.	.445	.437	.882	1.13
50	6040.	5870.	.209	.429	.638	1.57
59	6040.	5870.	.830	.444	1.273	.79
62	6040.	5870.	.136	.433	.569	1.76

\*\*\*\*\*  
 \* E N D L O A D C A S E 1 \*  
 \*\*\*\*\*

Bruce Fisher 9/21/83

0 0322

EVALUATION OF SUPPORT NO. TSW-355 REV. 1  
 ISOMETRIC NO. TSW-251-15,19 REV. 5

REF.

CHECK BOLTS \*

THE CRITICAL BOLT IS AT STARDYNE NODE 59.

$$\left. \begin{array}{l} \text{SHEAR RATIO } \frac{s}{s_A} = .444 \\ P_{\text{MAX}} = 5011 \text{ lb} \end{array} \right\} \text{ (SEE PG. 10)}$$

4  $T_A = 6040 \text{ lb}$

2 
$$P_{\text{AL}} = T_A \left[ 1 - \left( \frac{s}{s_A} \right)^{5/3} \right]^{3/5} = 6040 \left[ 1 - (.444)^{5/3} \right]^{3/5}$$

$$= 5048 \text{ lb} > 5011 \text{ lb}$$

∴ BOLTS ARE ADEQUATE

CHECK PLATE

7  $\sigma_{\text{ALL}} = .75 S_y = 27,000 \text{ lb/in}^2$

$$\sigma_{\text{MAX}} = 14,808 \text{ lb/in}^2 < \sigma_{\text{ALL}} \text{ (SEE PG. 9)}$$

∴ PLATE IS ADEQUATE

\* INTERACTION EXPONENT OF  $5/3$  USED PER REF. 2

REFERENCES

PREPARER: Bruce Fischer DATE: 9/21/83  
 REVIEWER: Ed S. Buh DATE: 9/21/83

0 0323

EVALUATION OF SUPPORT NO. TSW-355 REV. 1  
ISOMETRIC NO. TSW-251-15.19 REV. 5

REF

## EVALUATION OF DEVIATION 4D, NOTE 4:

BY REVIEW OF THE LOADS APPLIED TO THE PLATE (SEE PG. 3 - SUPPORT LOADS, JOINT 5), AND THE PLATE DETAIL, THE ATTACHMENT LOCATION DEVIATION IS JUDGED TO BE ACCEPTABLE.

## EVALUATION OF DEVIATION 4D, NOTE 5:

BY REVIEW OF THE LOADS APPLIED TO THE PLATE (SEE PG. 3 - SUPPORT LOADS, JOINT 7), AND THE PLATE DETAIL, THE ATTACHMENT LOCATION DEVIATION IS JUDGED TO BE ACCEPTABLE.

REFERENCES

PREPARER: Bruce Fischer DATE: 9/21/83  
REVIEWER: Gu P. Buh DATE: 9/21/85

0 0324

EVALUATION OF SUPPORT NO. TSW-355 REV. 1  
ISOMETRIC NO. TSW-251-15.19 REV. 5

REF.

EVALUATION OF DEVIATION 6A, NOTE 4:

BY REVIEW OF THE LOADS APPLIED TO THE WELD (SEE PG. 3 - SUPPORT LOAD, JOINT 5), THE WELD PROVIDED IS JUDGED TO BE ADEQUATE.

NOTE: THE AISC MINIMUM WELD REQUIREMENT CAN BE WAIVED AS PER WPPSS DOCUMENT WPBEC-C0500-F-83-2153.

EVALUATION OF DEVIATION 6A, NOTE 5:

BY REVIEW OF THE LOADS APPLIED TO THE WELD (SEE PG. 3 - SUPPORT LOAD, JOINT 7), THE WELD IS JUDGED TO BE ADEQUATE (ONLY VERTICAL WELDS CONSIDERED).

NOTE: THE AISC MINIMUM WELD REQUIREMENT CAN BE WAIVED AS PER WPPSS DOCUMENT WPBEC-C0500-F-83-2153.

REFERENCES

PREPARER: Bruce Fische DATE: 9/21/83  
REVIEWER: Br S. Bub DATE: 9/21/83

EVALUATION OF SUPPORT NO. TSW-355 REV. 1  
 ISOMETRIC NO. TSW-251-15.19 REV. 5

REF.

EVALUATION OF DEVIATIONS 6A+6B, NOTE 6:

LOADS AND WELD PROPERTIES IN LOCAL STRUDL COORDINATES.

LOADS (SEE PG. 3 - MEMBER 2, JOINT 3)

$$\begin{array}{ll} F_x = 3349 \text{ lb} & M_x = 0 \text{ in-lb} \\ F_y = 9847 \text{ lb} & M_y = 0 \text{ in-lb} \\ F_z = 0 \text{ lb} & M_z = 50,002 \text{ in-lb} \end{array}$$

WELD PROPERTIES

$$\begin{array}{l} L_w = 4(5) = 20 \text{ in} \\ S_w = 5(5) + \frac{5^2}{3} = 33.33 \text{ in}^2 \end{array}$$

REQUIRED WELD SIZE

$$f_x = \frac{F_x}{L_w} + \frac{M_z}{S_w} = \frac{3349}{20} + \frac{50,002}{33.33} = 1668 \text{ lb/in}$$

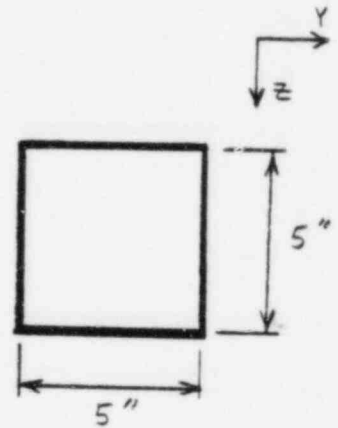
$$f_y = \frac{F_y}{L_w} = \frac{9847}{20} = 492 \text{ lb/in}$$

$$f = \sqrt{f_x^2 + f_y^2} = \sqrt{1668^2 + 492^2} = 1739 \text{ lb/in}$$

$$N/U \text{ ALLOWABLE WELD STRESS} = 18,000 \text{ lb/in}^2$$

$$WREQ = \frac{f}{.707(18,000)} = \frac{1739}{.707(18,000)} = .14 \text{ in} < \text{tall for } \frac{3}{16} - \frac{5}{16} \text{ fillet}$$

∴ WELD PROVIDED IS ADEQUATE



REFERENCES

PREPARER: Bruce Fischer DATE: 9/21/83  
 REVIEWER: Ben S. Oak DATE: 9/21/83

0 0326

EVALUATION OF SUPPORT NO. TSW-355 REV. 1  
ISOMETRIC NO. TSW-251-15.19 REV. 5

REF.

EVALUATION OF DEVIATIONS 6A + 6B, NOTE 7:

BY REVIEW OF THE LOADS APPLIED TO THE WELD  
(SEE PG. 3 - MEMBER 5, JOINT 6), THE WELD IS JUDGED  
TO BE ADEQUATE.

REFERENCESPREPARER: Bruce Fische DATE: 9/21/83  
REVIEWER: Ben S. Baker DATE: 9/21/83

0 0327