

ENCLOSURE

CALCULATION WCG-1-969
QUALIFICATION OF WORST CASES OF THERMALLY
RESTRAINED STRUCTURES - PACKAGE I

VA 10697 (DNE-6-86)

TITLE : Qualification of Worst Cases of Thermally Restrained Structures - Package I				Plant/Unit WBNP / UNIT 1	
PREPARING ORGANIZATION NE, Ebasco Services Inc.		KEY NOUNS (Consult RIMS DESCRIPTORS LIST) CEB, CALC, THERMAL, RESTRAINT, SELECTION, MISC STEEL, STRUCTURAL			
BRANCH/PROJECT IDENTIFIERS WCG-1-969		Each time these calculations are issued, preparers must ensure that the original (RO) RIMS accession number is filled in. Rev (for RIMS' use) RIMS accession number			
APPLICABLE DESIGN DOCUMENT(S) WB-DC-20-21		RO	B18 910907 255		
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Revision 0		R1	R2	R3	Safety-related? Yes <input checked="" type="checkbox"/> No <input type="checkbox"/>
ECN NO. (or indicate Not Applicable) DCN M-16250-A		Statement of Problem.			
Prepared: <i>S. Alan Lin</i>		<p>Perform evaluation for the worst cases of thermally restrained structures for Category I structural steel and Miscellaneous steel population.</p> <p style="font-size: 1.5em; text-align: center; margin-top: 20px;">ORIGINAL</p>			
Checked/Verified: <i>C.Y. Ou</i>					
Reviewed: <i>H. Totunoglu</i>					
Approved: <i>[Signature]</i>					
Date <i>9/5/91</i>					
USE FORM TVA 10534	List all pages added by this revision				
IF MORE SPACE REQUIRED	List all pages deleted by this revision				
	List all pages changed by this revision				
Abstract: These calculations contain an unverified assumption(s) that must be verified later. Yes <input type="checkbox"/> No <input checked="" type="checkbox"/>					
<p>Five (5) of the fifteen (15) worst cases of the thermally restrained structures identified in Calculation WCG-1-790 RO are evaluated and qualified in this calculation. <i>RO 9A 8/29/91</i></p>					
<p>LEGIBILITY EVALUATED and ACCEPTED for issue.</p> <p><i>[Signature]</i> <i>9/5/91</i></p> <p>Signature Date</p>					
[] Microfilm and store calculations in RIMS Service Center			Microfilm and destroy. []		
[X] Microfilm and return calculations to: Calculations Library			Address: WBN N QA BLDG.		

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D. Kilgore

WBN Trailer No. E-22

TVATITLE: *Qualification of Worst Cases of Thermally Restrained Struc. - Package I*

REVISION LOG

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0	Original Issue	<i>9/5/91</i>

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SUBJECT: Qualif. of Worse Cases of Thermally Restrained Structures - Package I

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THIS CALCULATION PACKAGE CONTAINS 257 TOTAL PAGES.

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5. PURPOSE:

The purpose of this calculation is to qualify five (5) worst cases of the thermally restrained structures of category I structural and miscellaneous steel population.

5. APPLICABLE CRITERIA (MARKED "X")

- X 1. DS-C1.7.1 R5 General Anchorage To Concrete
- NR 2. DS-C1.7.3 R0 Application of Baseplate II
- NR 3. DS-C1.8.1 R3 Standard Calculation for Evaluation Type II Embedded Plates
- NR 4. WB-DC-20-1 R6 Concrete Structures-General
- NR 5. WB-DC-20-2 R6 Reactor Building Concrete Structures
- X 6. WB-DC-20-21 R6 Miscellaneous Steel Component for Category I Structures (per DCN's S-14373-A and S-16078-A)
- X 7. DG-C1.6.12 R1 Evaluation of Steel Structures with Thermal Restraint.
- X 8. WB-DC-20-24 R5 Dynamic Earthquake Analysis of Category I Structures and Earth Embankments
- NR 9. WB-DC-40-31.53 R4 Design Criteria for Pipe Whip Restraints, Jet Deflectors and Sleeves
- NR 10. (Specified)

7. APPLICABLE CODES AND STANDARDS (MARKED "X"):

- X 1. AISC Manual of Steel Construction, 7th Edition
- NR 2. AWS D.1.1-83, 1983, Structural Welding Code.
- NR 3. ACI 318-77 Building Code Requirements for Reinforced Concrete
- NR 4. (specified)

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8. ASSUMPTIONS, LITERATURE SEARCHES AND OTHER APPLICABLE BACKGROUND DATA

None

9. APPLICABLE REFERENCES (MARKED "X")

- X 1. Design of Welded Structures by O. W. Blodgett 1966
- NR 2. Formulas for Stress and Strain by R. J. Roark & W.C. Young, 4th Edition
- NR 3. WCG-1-580 R Selection and Grouping of Embedded Plates for Evaluation
- NR 4. WCG-1-256 R0 Concrete Quality Evaluation
- NR 5. N3-PA-34 R0 Non-linear Transient Dynamic Analysis of FW Lines-Piping Analysis Calculation No. N3-PA-34
- NR 6. QIR-CEB-WBN 90757 R0 Active List of Pipe Whip Restraints/Protective Devices (PD's)
- X 7. QIR-CEB-WBN-39-361 R0 Auxiliary Control and Reactor Buildings Acceleration & Response Spectra
- NR 8. WCG-1-370 R0 Generation of OBE 4% Damping ARS for Unit 1 ICS, SCV and SB
- NR 9. WCG-1-377 R0 Auxiliary Control Building 4% Damping Curves and Seismic Forces
- X 11. QIR-EBA-WBN-91-435 R0 Pipe Rupture Loads on Non-Primary Safety Function Structures
- X 12. WCG-1-684 R1 Design Instruction for Structural Adequacy Evaluation of Steel Platforms
- X 13. WCG-1-686 R1 Selection of Misc. and Structural Steel with Thermal Restraint
- X 14. WCG-1-790 R1 Worst Case Selection of Thermally Restrained Structures
- X 15. CSG-91-003 R0 Concrete Anchorage Stiffness for Thermal Evaluation
- X 16. WCG-2-50 R1, Vol. 6 Pressurizer Enclosure Platform @ EL. 783'-8 1/2"
- X 17. WCG-1-825 R2, Field Walkdown Data for the Platform @ EL. 783' of the Reactor Building
- X 18. WCG-1-830 R1, Walkdown of Elevation 713'-0" Platform on Drawing 48N1210-16
- X 19. WCG-1-964 R0, Evaluation of Embedded Plates for Platforms
- X 20. WCG-1-1047 R0, Study of the Effect of Repeated Load and Load Sequence for Thermal Evaluation

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- x 21. Steel Structures, Design and Behavior, Second Edition, by Salmon and Johnson
- x 22. Drawing 48N968 R22
- x 23. Drawing 48N1210-16 R8
- x 24. Drawing 48N967-1 R0
- x 25. ANSYS Revision 4.3A
- NR 26. (specified)

10. APPLICABLE DESIGN INPUT DATA:

See section 11.2

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11.0 CALCULATION

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11.1 Scope

A total of fifteen (15) worst cases of the thermally restrained structures of Category I structural and miscellaneous steel population were identified in Calculation WCG-1-790 (Reference 9.14: Worst Case Selection of the Thermally Restrained Structures). Five (5) of the worst cases are evaluated in this package.

Four (4) ANSYS models, as summarized in Table 11.1.1, are constructed for detailed evaluation. Geometric Configurations for these worst cases, as well as members included in each model, are presented in Figures 11.1.2 through 11.1.5.

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Table 11.1.1 Summary of Models for Worst Cases in this Calculation

Model ID	Worst Case ID*	Restraint Type **	Location	Drawing No.	Elevation	Accident Temp. (°F)
PR9C	1-27 2&3-9(C)	Axial Proximity	Reactor Bldg.	48N968	783'-8 1/2"	327°
AX11	1-11	Axial	Aux. Bldg.	48N1210- 16	713'-0"	210°
PR8G	2&3-8(G)	Proximity	Reactor Bldg.	48N967-1	774'-0"	327°
HD07	5-7	Header	Aux. Bldg.	48N1210- 16	713'-0"	210°

* Worst Case ID as defined in Calculation WCG-1-790 (Worst Case Selection of Thermally Restrained Structures).

** See Figure 11.1.1 for Typical Thermal Restraint; For more detail, see Calculation WCG-1-686 and WCG-1-790.

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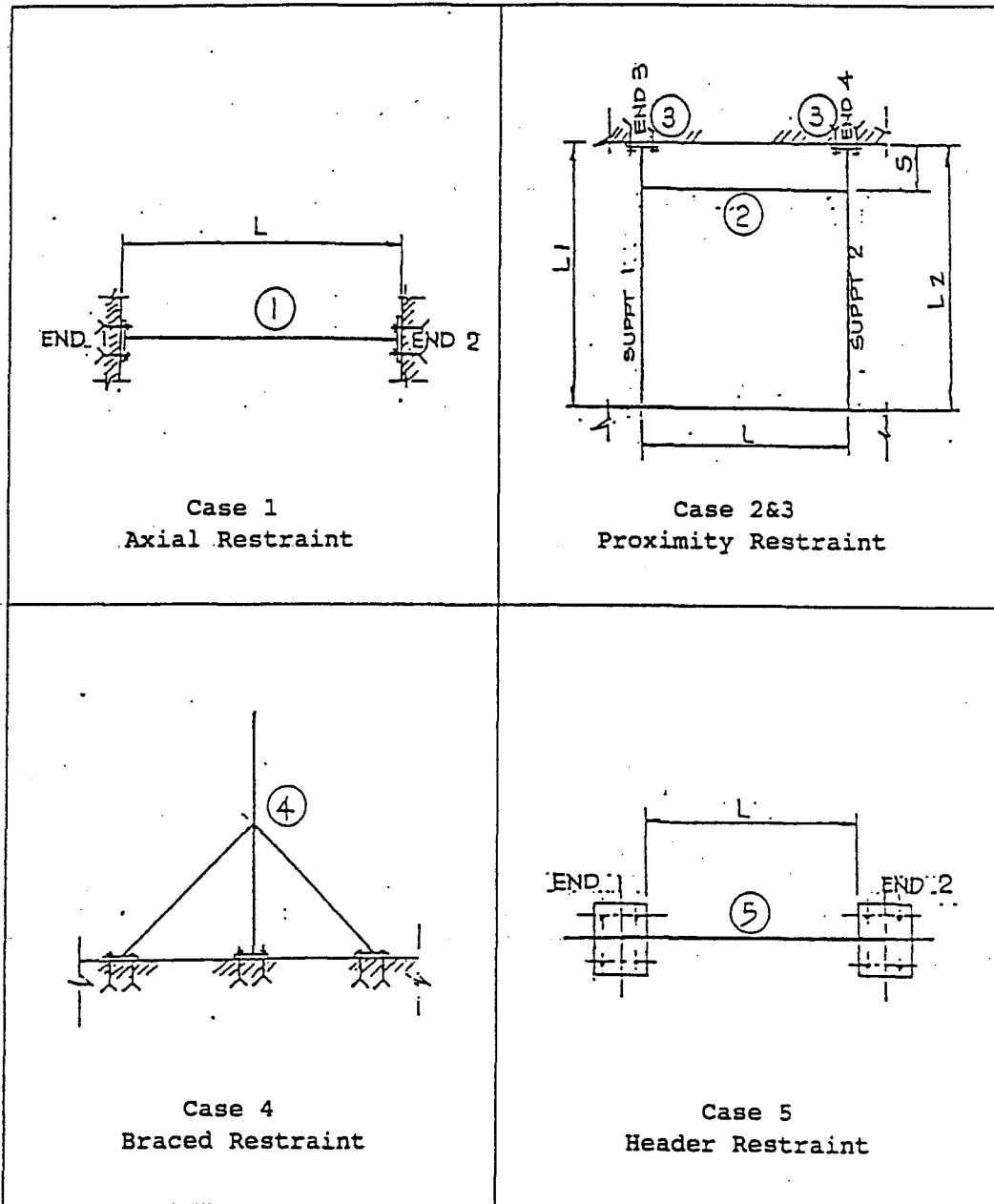
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Figure 11.1.1 Typical Thermal Restraint



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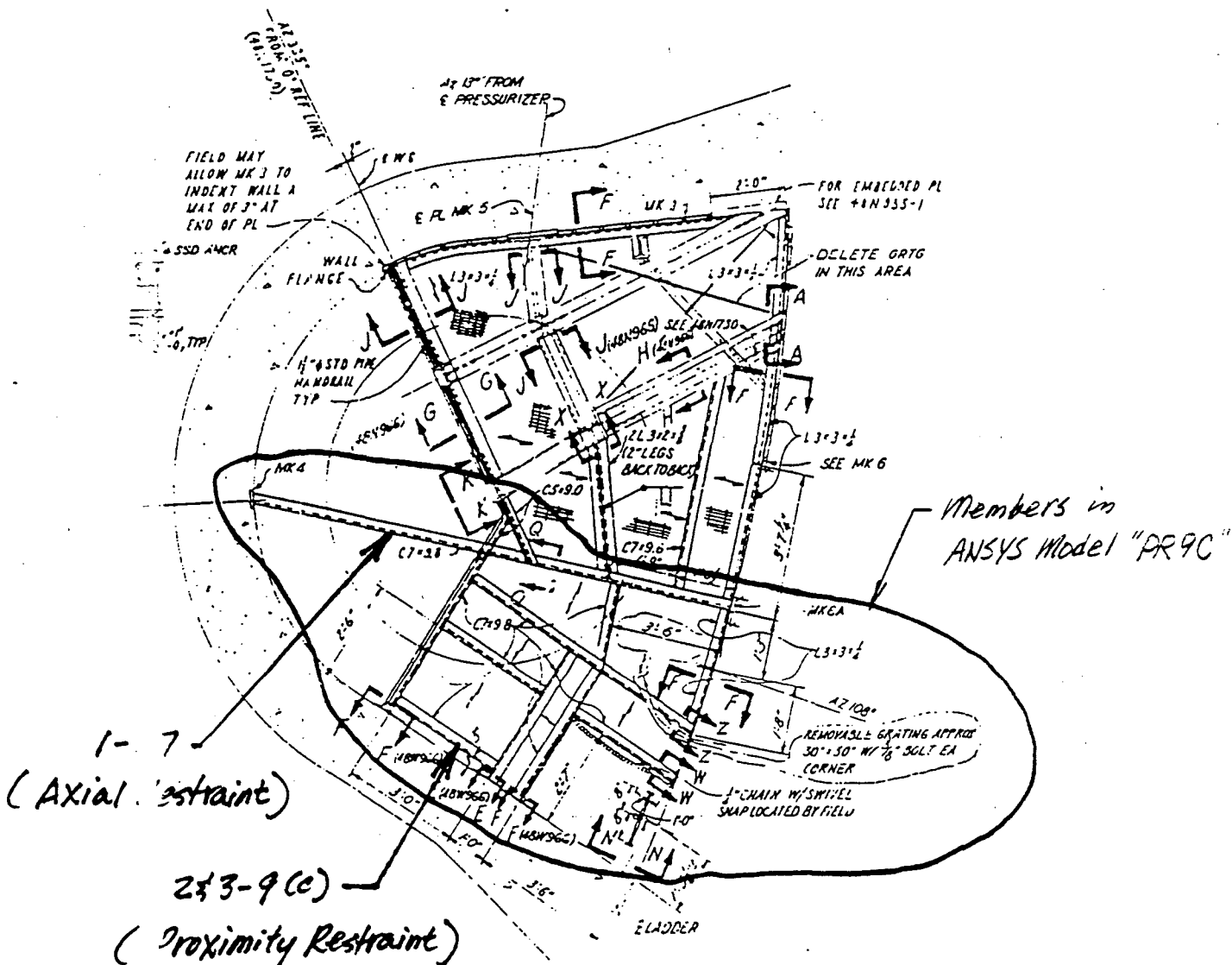
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Figure 11. 2 Worst Cases 1-27 and 2&3-9(C)

(Reference: WCG-1-790)



1-7
(Axial Restraint)

2&3-9(c)
(Proximity Restraint)

Members in
ANSYS Model "PR9C"

PLAN - EL 783'-8 1/2"
T/S 1/2" BELOW EL 783'-8 1/2" - EXCEPT AS NOTED
UNIT 1 ONLY

(DWG 48N968)

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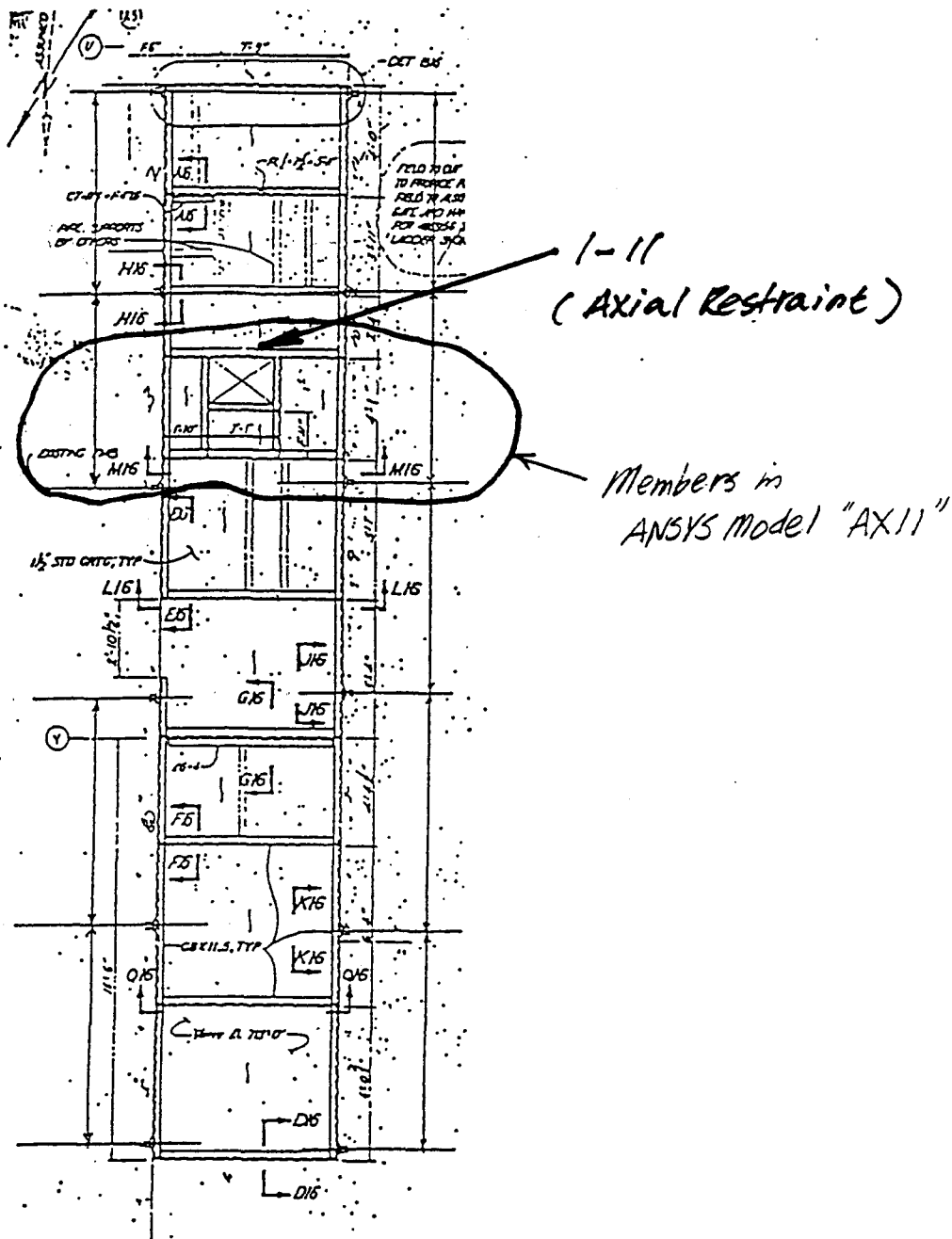
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Figure 11.1.3 Worst Case 1-11

(Reference: WCG-1-790)



PLAN PLATFORM - EL 713'0"
UNIT 1 ONLY
3/8" = 1'-0"
(DWG 48N1210-16)

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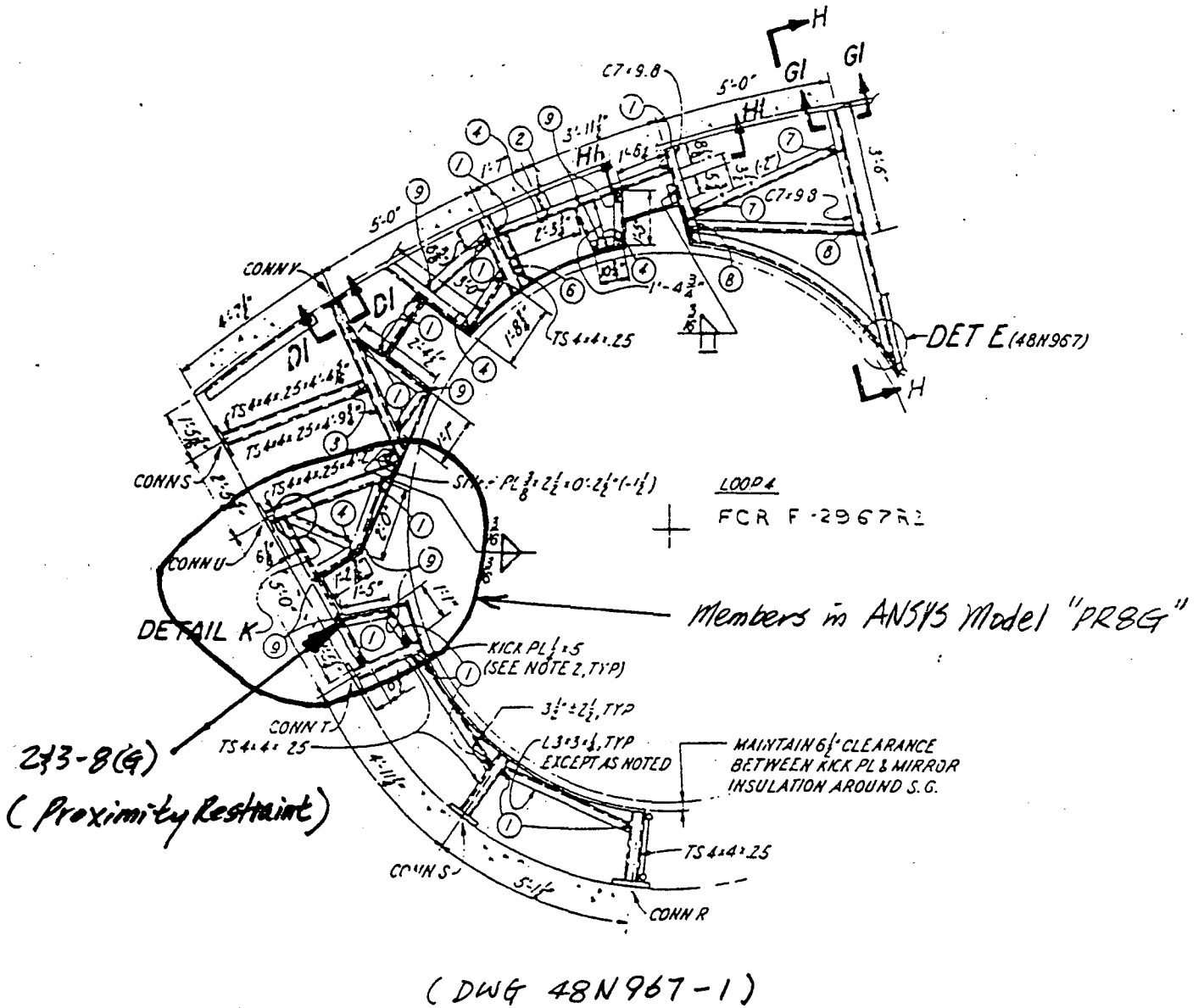
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Figure 11.1.4 Worst Case 2&3-8(G)

(Reference: WCG-1-790)



(DWG 48N967-1)

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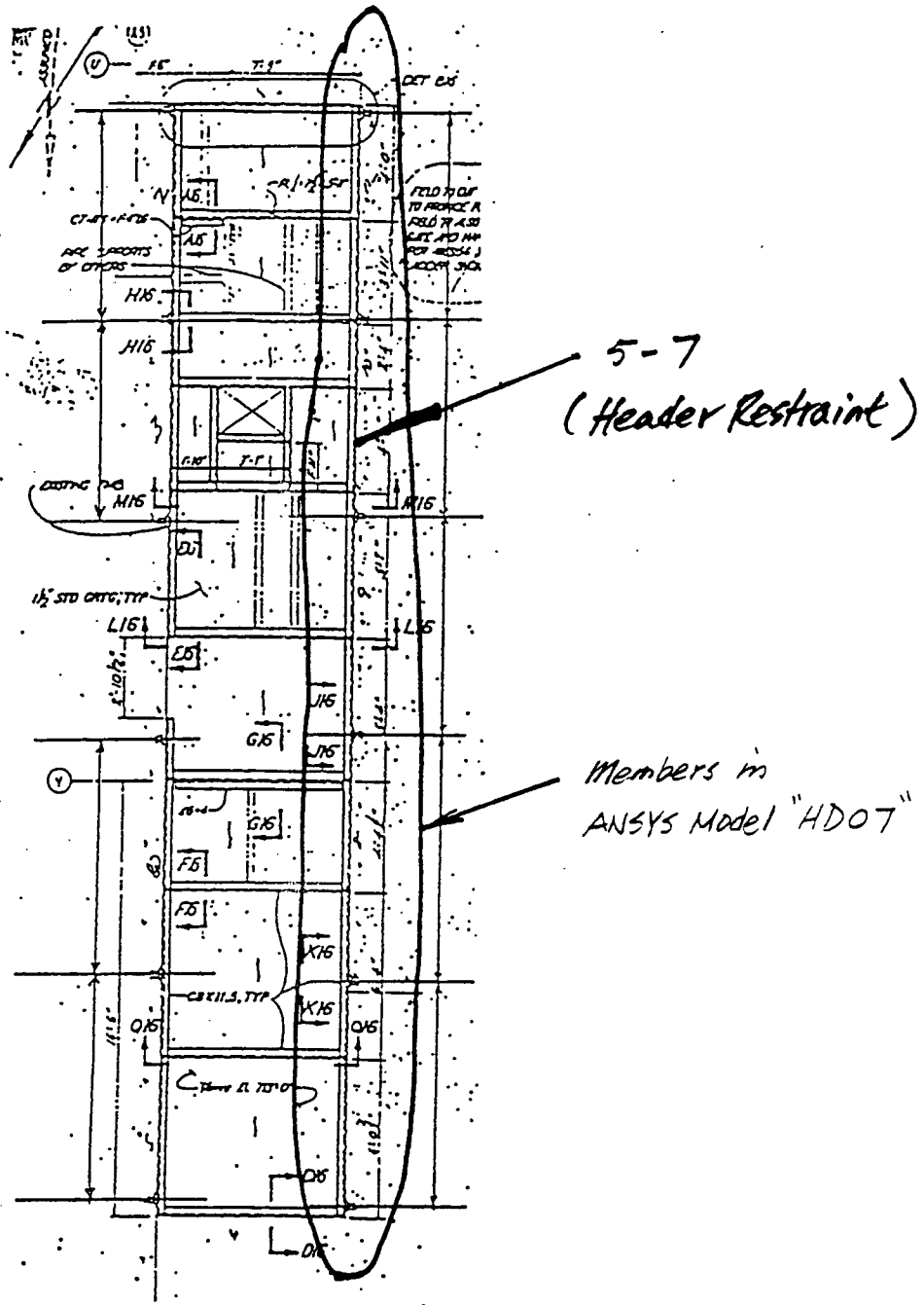
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Figure 11.1.5 Worst Case 5-7

(Reference: WCG-1-790)



PLAN PLATFORM - EL 713'0"
UNIT 1 ONLY
3/8" = 1'-0"

(DWG 48N1210-16)

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11.2 Methodology

Each worst case is evaluated and qualified per Design Criteria WB-DC-20-21, R6 (Miscellaneous Steel component for Category I Structures) and Design Guide DG-C1.6.12 R1 (Evaluation of Steel Structures with Thermal Restraint). The rigorous nonlinear analysis method using the ANSYS computer program is adopted in this evaluation.

Mathematical Model

A total of four (4) mathematical models (PR9C, AX11, PR8G and HD07) are constructed to represent five (5) worst cases. (See Table 11.1.1). The models include worst case members and their adjacent members so that the combination of the thermal load and other applicable loads can be properly addressed. Five types of ANSYS elements are used to generate the models wherever applicable:

- (1) STIF24 - Three dimensional thin-wall plastic beam element; This is a uniaxial element of arbitrary cross-section (open or closed) with tension-compression, bending and ST. Venant torsional capability. The element has plastic capability in the axial direction as well as a user-defined cross-section. This element is used to simulate the members which are loaded beyond yield.
- (2) STIF4 - Three dimensional beam element; This element is also a uniaxial element with tension, compression, torsion, and bending capabilities. This element is used to simulate members which are in the linear elastic range.
- (3) STIF14 - Three dimensional spring-damper element; This element has longitudinal

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- (3) STIF14 - Three dimensional spring-damper element; This element has longitudinal or torsional capability in one, two or three dimensional applications. This element is used to represent the shear stiffness of anchor bolts or rotational stiffness of the baseplate.
- (4) STIF40 - Combination element; This element is a combination of a spring-slider and damper in parallel, coupled to a gap in series. Similar to the use of STIF14, this element is to simulate the nonlinear shear stiffness of anchor bolts and gaps. A bilinear curve of the stiffness can be used and the (1/32") free travel (A8.7, WB-DC-20-21 R6 and C8.0 of DG-C1.6.12 R1) can be modeled utilizing the Gap capacity of this element.
- (5) STIF21 - Generalized mass element; This is a point element having up to six degrees of freedom. This element is used to represent an attachment weight on the member.

With the STIF24 element, the cross-section of members is represented by rectangular segments defined by segment points. The cross-sections of W-shape, Channels, Structural Tubing and Angles are presented in Figures 11.2.1 through 11.2.4, respectively. The stresses and strains, including elastic and plastic strains, at each segment points on the cross-section are calculated in the nonlinear analysis.

The temperature-dependent bilinear stress-strain property for the steel is used in the nonlinear analysis, as shown in Figure 11.2.5. The Modulus of Elasticity (E) decreases as temperature increases. Figure 4 (Variation in Modulus of Elasticity) in DG-C1.6.12, Section 4.2, is used to obtain E_{70} , E_{100} , E_{200} , E_{300} and E_{400} (E at 70°F, 100°F, 200°F, 300°F and 400°F respectively) as ANSYS input. 29,000 ksi is used for E_{70} . 150 ksi is used as the Tangent Modulus (Et), which is obtained from Figure 2.1.1 of "Steel Structures, Design and Behavior", Second Edition, by Salmon and Johnson. Temperature-dependent Coefficients of Thermal Expansion are also considered in the

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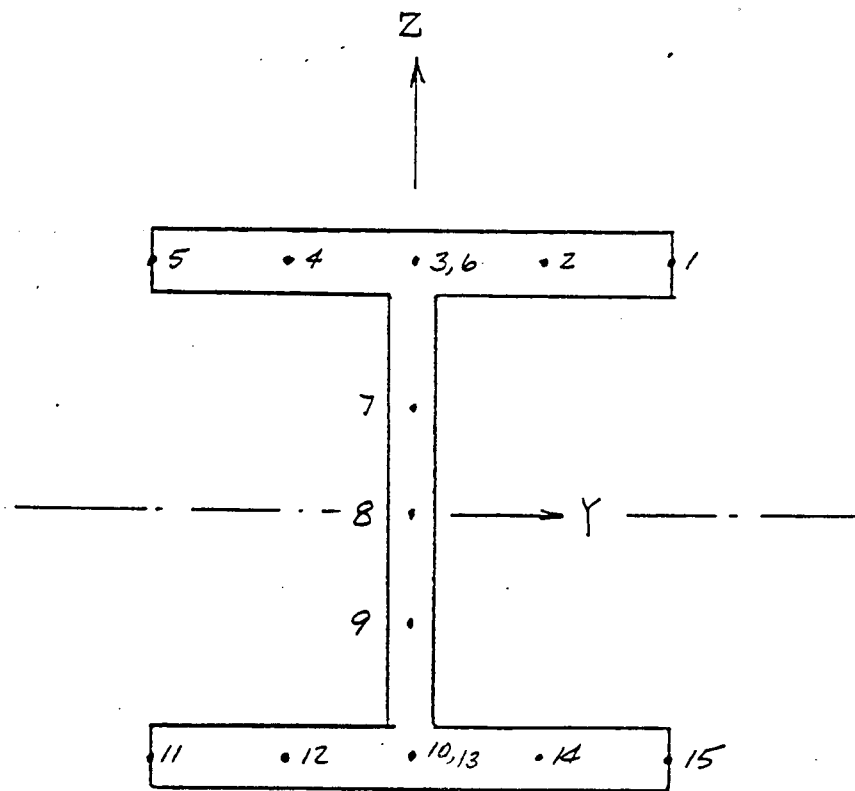
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Figure 11.2.1 Segment Point Definition for Cross-section of W-Shape



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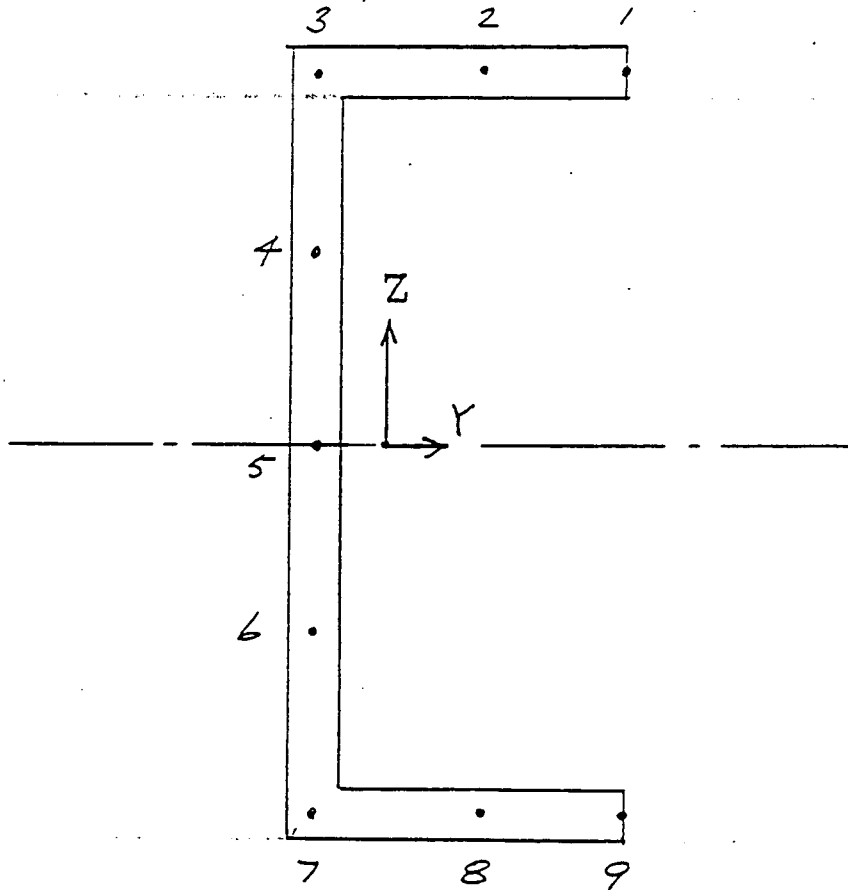
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Figure 11.2.2 Segment Point Definition for Cross-section of Channel



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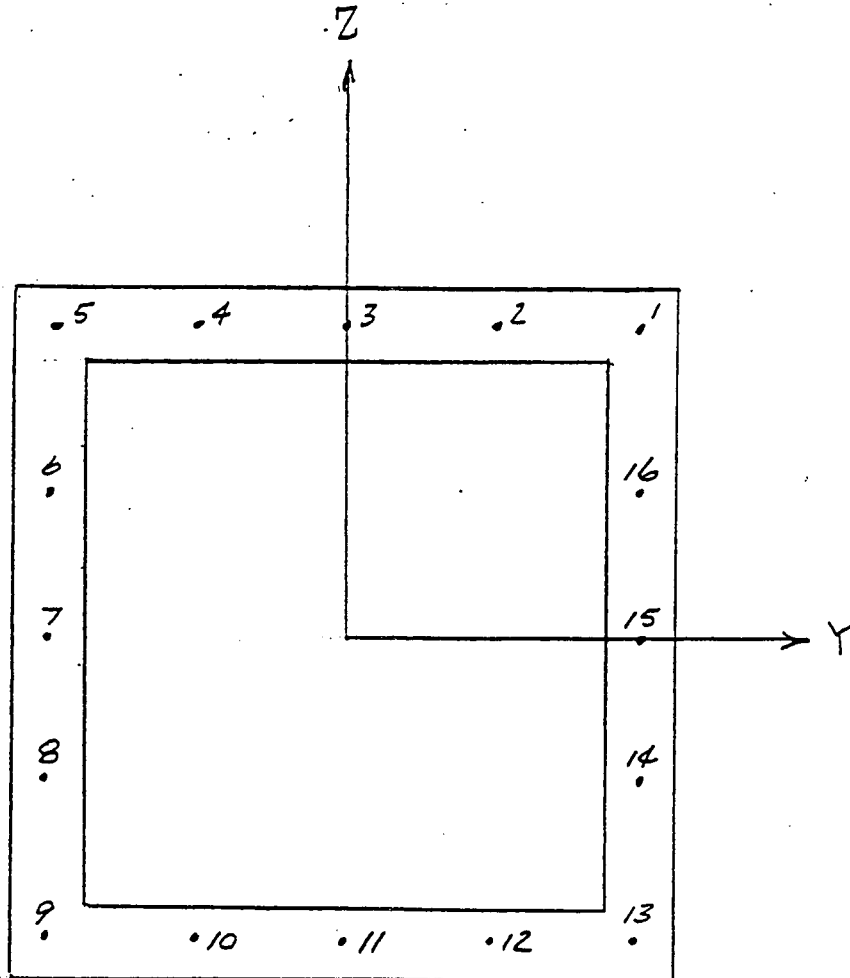
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Figure 11.2.3 Segment Point Definition for Cross-section of Structural Tubing



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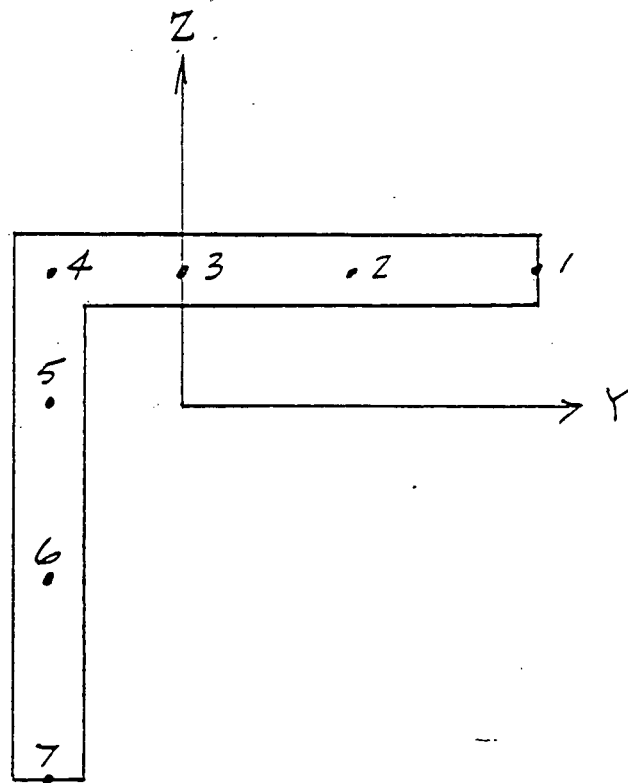
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Figure 11.2.4 Segment Point Definition for Cross-section of Angle



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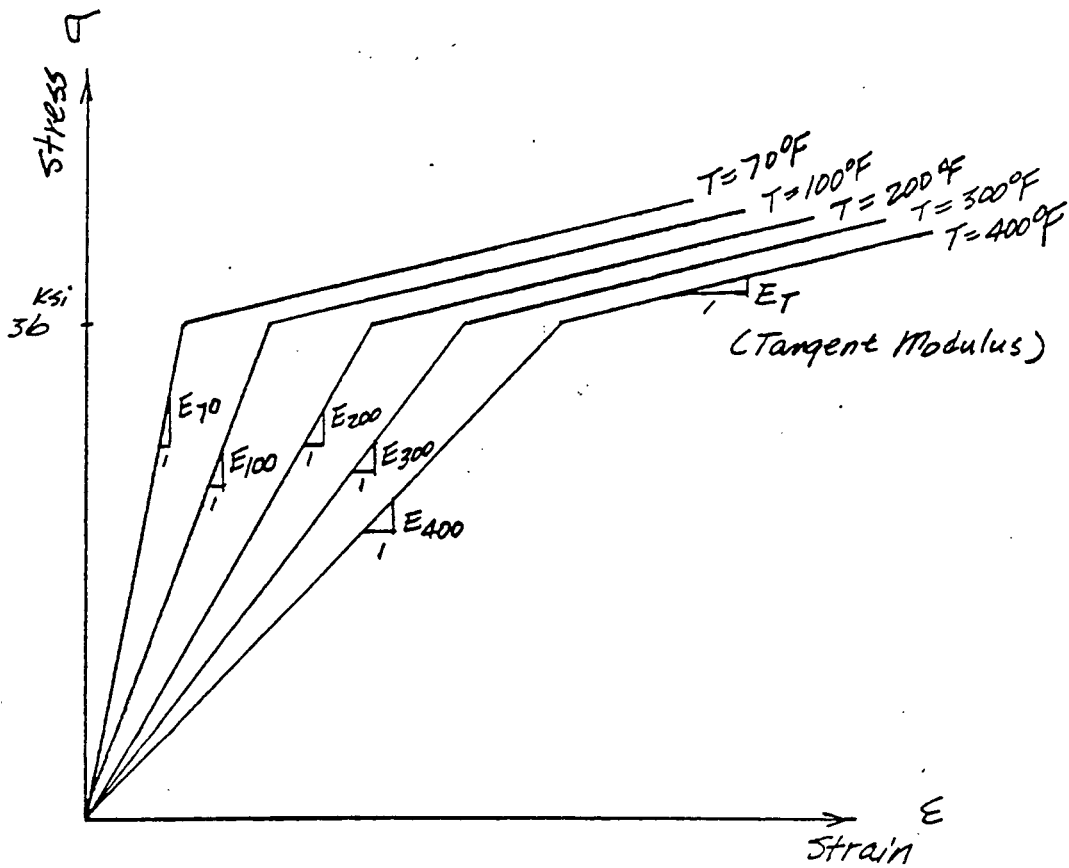
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Figure 11.2.5 Temperature-Dependent Material Properties



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nonlinear analysis. The formula provided in Section 4.3 of the DG-C1.6.12 is used to calculate the Coefficients of Thermal Expansion at 70°F, 100°F, 200°F, 300°F and 400°F respectively as ANSYS input. 6.5×10^{-6} in/in-°F is used for 70°F. The minimum yield strength, 36 ksi, for the ASTM A36 steel is considered in the analysis.

The ANSYS large-deflection solution method is used in the nonlinear analysis so that the stiffness matrix characterizes the deformed structure in the iterative solution process.

Boundary Condition

ANSYS Spring Elements STIF14 or STIF40 are used to model the shear stiffness of concrete anchors. The shear stiffness of anchors are obtained from Calculation CSG-91-003, which provides bilinear curves for various type and size of concrete anchors from TVA test data. Alternatively, 1000 kip/in per bolt is conservatively used as shear stiffness per DS-C1.7.1. STIF14 and STIF40 are also used as rotational spring to model the stiffness of the baseplate. The rotation stiffness of baseplate is obtained by making a computer run using BASEPLATE II program. The rotational stiffness is the moment required to create a unit rotation of the baseplate. Wherever applicable, an one thirty-second inch (1/32") is imposed at supports for the free travel per Section C8.0 of DG-C1.6.12.

Loading Condition

The loading combinations for misc. steel components for Category I structures are defined in Sections 3.2 and A4.3 of WB-DC-20-21. For the Operating Temperature conditions, T_o can be assumed to zero in the loading combinations (1a) through (5a) if T_o is less than 120°F per A8.1.2 of WB-DC-20-21. No evaluations are required for the worst cases in this package since the T_o 's are less than 120°F for all five cases.

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For Accident Temperature conditions, the most critical loading combination (8) is considered in the evaluation of the worst cases of thermally restrained structures. Dead Loads (D) and Live Loads (L) are considered per Sections 3.1.1 and 3.1.2 of WB-DC-20-21. Equivalent densities for all members are calculated to include effects of the dead load and live loads. The Equivalent Static approach is used to address the SSE loads (E') by applying Accelerations (G loads) to the structures (using the ANSYS ACEL command). In general, the peak G of the Acceleration Response Spectrum with the MMF (Multi-Mode Magnification Factor) equals to 1.5 are used unless lower G's can be justified by appropriate references. 1.2 is used for the MMF for Conduit SSE loads (per WCG-1-684). The G-loads can be obtained from QIR-CEB-WBN-89-361 (Reference 9.7) and the appropriate damping values to use are contained in WB-DC-20-24 (Dynamic Earthquake Analysis of Category I Structures and Earth Embankments). Pipe Reactions (Ra) are obtained from applicable QIR's. Thermal Loads (Ta) are obtained from environmental drawings as documented in Calculation WCG-1-686 (Reference 9.13) and WCG-1-790 (Reference 9.14). The ambient temperature is taken as 70°F. The differential between ambient and accident temperature are used for evaluation. The Pressure Equivalent Static Loads (Pa) is not considered because it is insignificant compared with Thermal Load. Pipe Rupture Loads (Yr, Yj and Ym) are not included per QIR EBA-WBN-91-435 R0 (Pipe Rupture evaluations do not consider platform and miscellaneous steel structures as essential (primary) safety targets).

An conservative loading sequence is used in the nonlinear analysis: All loads, except thermal load (Ta), are first applied on the computer model. Incremental temperature steps are then applied starting at 70°F (Ambient) up to Ta. This is the most critical loading sequence in terms of the Ductility Ratio results used for member qualification. The Ductility Ratio will be smaller if the SSE loads are applied AFTER the thermal loads. However, the final displacement results for both loading sequences are nearly identical. Parametric studies were performed (documented in Calc. WCG-1-1047) on two worst case structures (Worst Case 1-27 and 1-28) and the results indicated the differences are less than 1% comparing the maximum displacements on members with thermal restraint).

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Member Qualification

The Non-linear Acceptance Criteria applicable to the non-linear analysis, specified in Appendix D of DG-C1.6.12 R1, is used for the qualification of members. Primary members are evaluated in accordance with the displacement based Ductility Ratios.

Connection and Anchorage Qualification

Connections and Concrete anchorage are evaluated using the Acceptance Criteria in Appendix C of DG-C1.6.12 R1. BASEPLATE II and CONAN computer programs are used to for the evaluations of concrete anchors as well as concrete pullout capacity.

Modification

DCN's will be issued for modifications if member, connection or anchorage is not accepted per WB-DC-20-21. The modified structure will be re-analyzed using Acceleration Response Spectrum B+C per WB-DC-20-24 R5. The overall behavior of the structure will be examined and the modified components will be re-evaluated.

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11.3 Qualification of Worst Cases 1-27 and 2&3-9(C)

11.3.1 Mathematical Model (PR9C)

The ANSYS model PR9C is shown in Figures 11.3.1.1 and 11.3.1.1A. This model contains two worst cases: one with axial restraint (1-27) and the second with proximity restraint (2&3-9(C)). The model includes the worst case members and their adjacent members so that the combination of the thermal load and non-thermal loads can be properly addressed. ANSYS STIF24 (3-D thin-wall plastic beam element) elements are used to simulate the worst case members, e.g. elements between node 8 and 10 for the axially restrained member, elements between 2 and 5 for the proximity member, elements between nodes 1 and 9, nodes 4 and 6 for the supporting members of the proximity member. ANSYS STIF4 (beam element) elements are used to model the members adjacent to the worst case members. A column at node 32 along global Z direction is modeled with a STIF14 spring element (element 163). A beam element (element 161) is also attached at node 32 to represent the axial restraint along the line of nodes 33 and 32. Two members near nodes 25 and 26 are not included in the model because they have negligible effect on the worst case member (1-27) since the most critical part of the axially restrained member is the unbraced segment between nodes 8 and 9. ANSYS STIF21 mass elements are used to simulate hand rails at nodes 95 and 96, which are located above nodes 28 and 29 respectively. These two mass elements are connected to nodes 28 and 29 with rigid links. A small bore pipe located below node 30 is also modeled with a STIF21 mass element and connected with a rigid link to node 30. Pinned connections at nodes 2, 5 and 9 are modelled by the ANSYS CP commands to de-couple the moments between nodes 2 and 102, between nodes 5 and 105, and between nodes 9 and 109. Temperature dependent material properties, as described in Section 11.2, are used.

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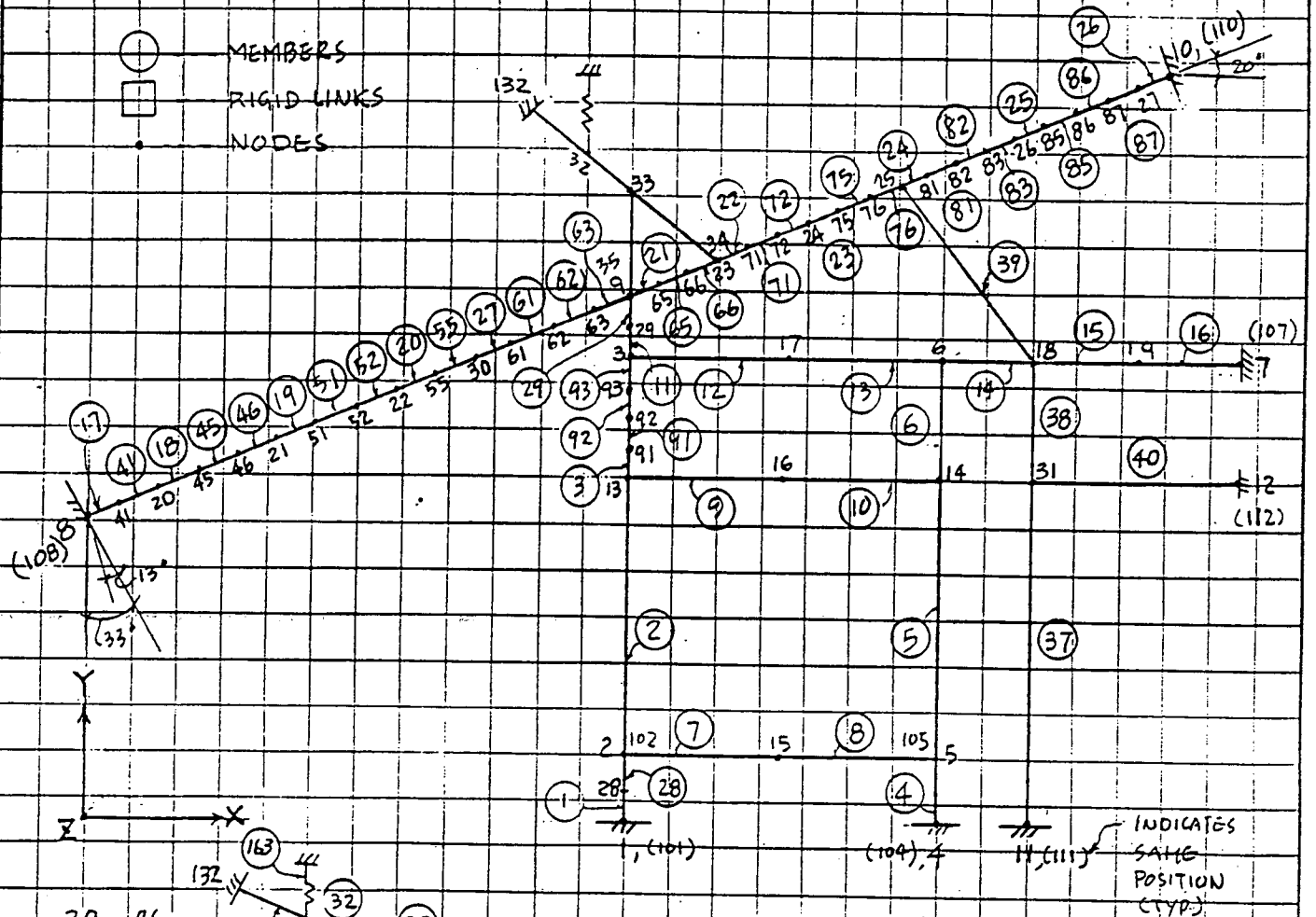
SUBJECT QUALIF. OF WORST CASES OF THERM. REST. STRUCTURES - PACKAGE I

Figure 11.3.1.1 ANSYS Model for Worst Cases 1-27 and 2&3-9(C)

PRQC ANSYS MODEL

LEGENDS

-  MEMBERS
-  RIGID LINKS
-  NODES



PLAN VIEW (NTS)

INDICATES SAME POSITION (TYP)

RIGID LINKS

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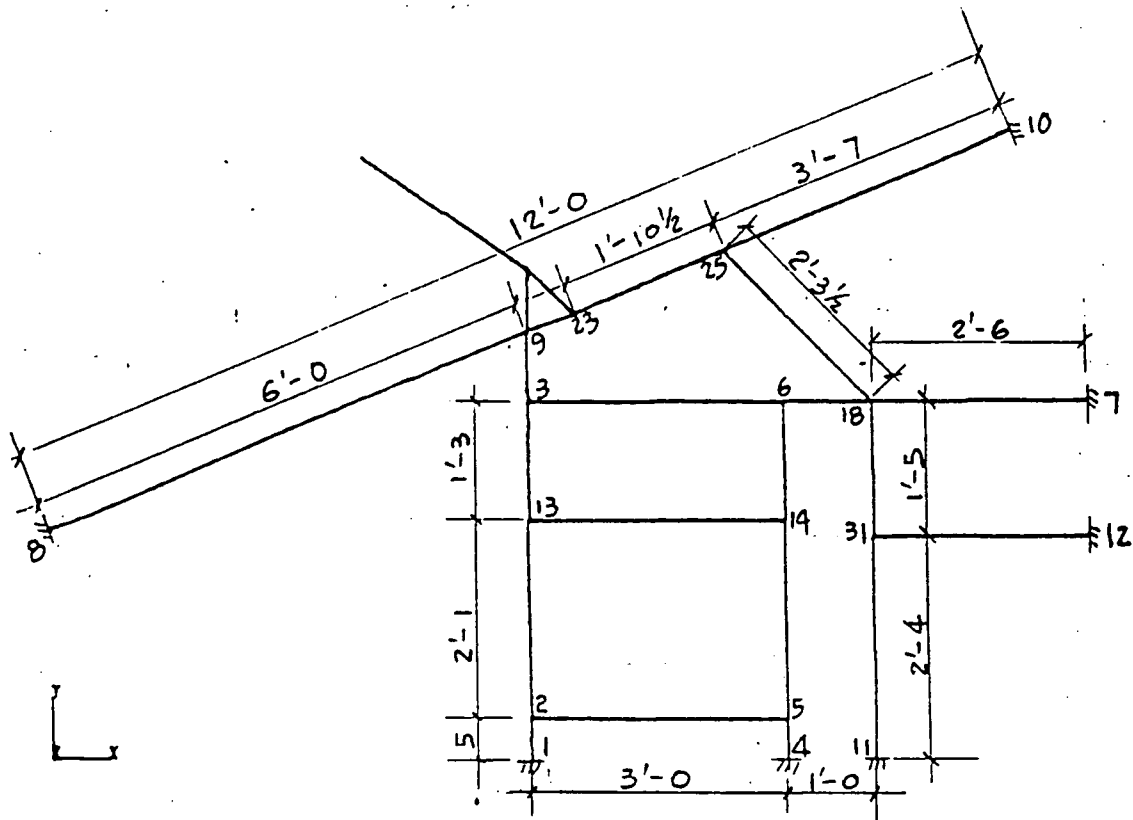
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Figure 11.3.1.1A ANSYS Model for Worst Cases 1-27 and 2&3-9(C)
(Model Dimension)



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11.3.1 Mathematical Model (Cont'd)Boundary Condition

ANSYS STIF14 spring elements are used to simulate the shear stiffness of concrete anchors and the rotational stiffness of the baseplate and concrete anchorage. Shear springs are specified at all support nodes except nodes 1 and 4. Rotational springs are used at all support nodes. Nodes 101, 104, 107, 108, 110, 111 and 112, defined at the same locations of nodes 1, 4, 7, 8, 10, 11 and 12 respectively, are the support nodes for the spring elements. 4000 kip/in (1000*4) obtained from DS-C1.7.1 is used as the shear stiffness for each spring. The rotational stiffness is calculated from the output of BASEPLATE II runs. A total of five (5) baseplate models were generated as shown in Figures 11.3.1.2 through 11.3.1.6. The input files for the baseplate runs to calculate rotational stiffness are provided in Tables 11.3.1.1 through 11.3.1.5.

To represent the actual orientation of the baseplate and concrete anchorage, Nodes 8 and 108 in the ANSYS model are rotated 33°. Similarly, Nodes 10 and 110 are rotated 20°. A displacement of 1/32" is specified at nodes 12, 7, 10 in +X direction in nodal coordinate system. A displacement of 1/32" is specified at node 8 in -X direction in nodal coordinate system. A displacement of 1/32" is specified at node 11 in -Y direction. A displacement of 0.03" is specified at nodes 1 and 4 in -X and +X directions respectively. This approach has the same effect of using a Gap Element at support to allow free movement before load starts to transfer. Studies are performed in the evaluations of Worst Case 1-11 (Section 11.4) and Worst Case 2&3-8(G) (Section 11.5) to demonstrate that the analysis results are identical whether the (1/32") free travel is simulated by imposed displacement or by gap element. For details, see Sections 11.4 and 11.5.

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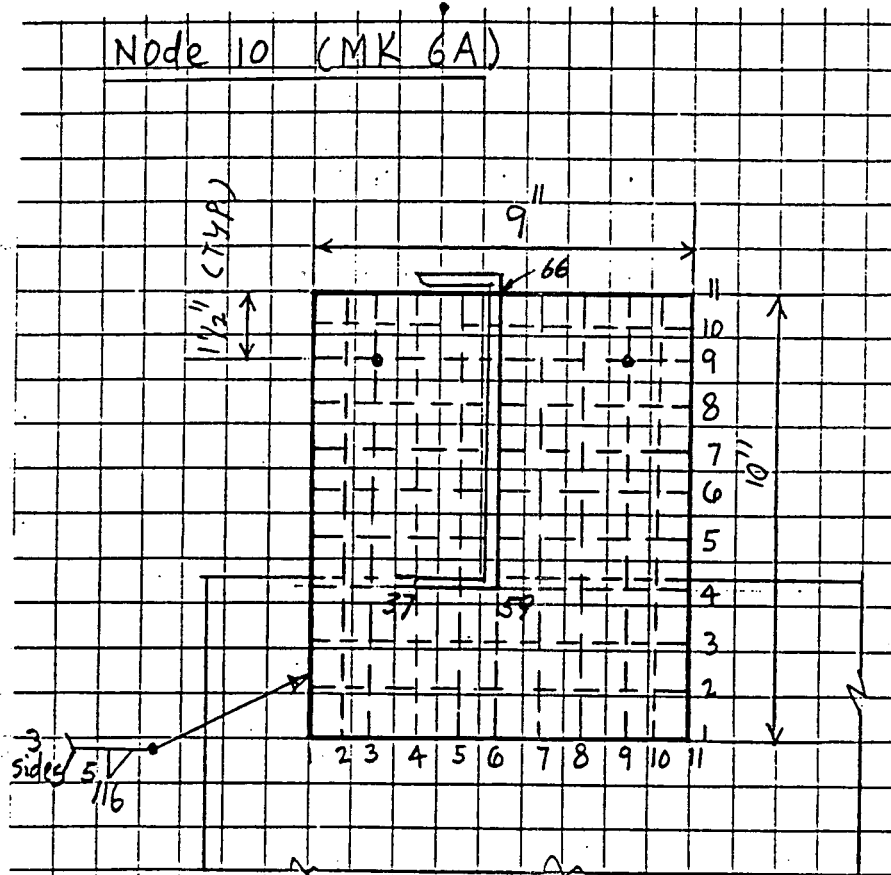
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Figure 11.3.1.4 BASEPLATE II Model for Nodes 10 of ANSYS Model

(Ref = WCG-1-B25R2)



Base Size: (MK 2B) = $\frac{3}{4}$ " x 9" x 0'-10" LG.

2 - $\frac{1}{2}$ " ϕ S.S.D's

Attachment member: C7x9.8

Tensile Stiffness = 400 K/in

Shear Stiffness = 1000 K/in

(Ref: DS-C1.7.1)

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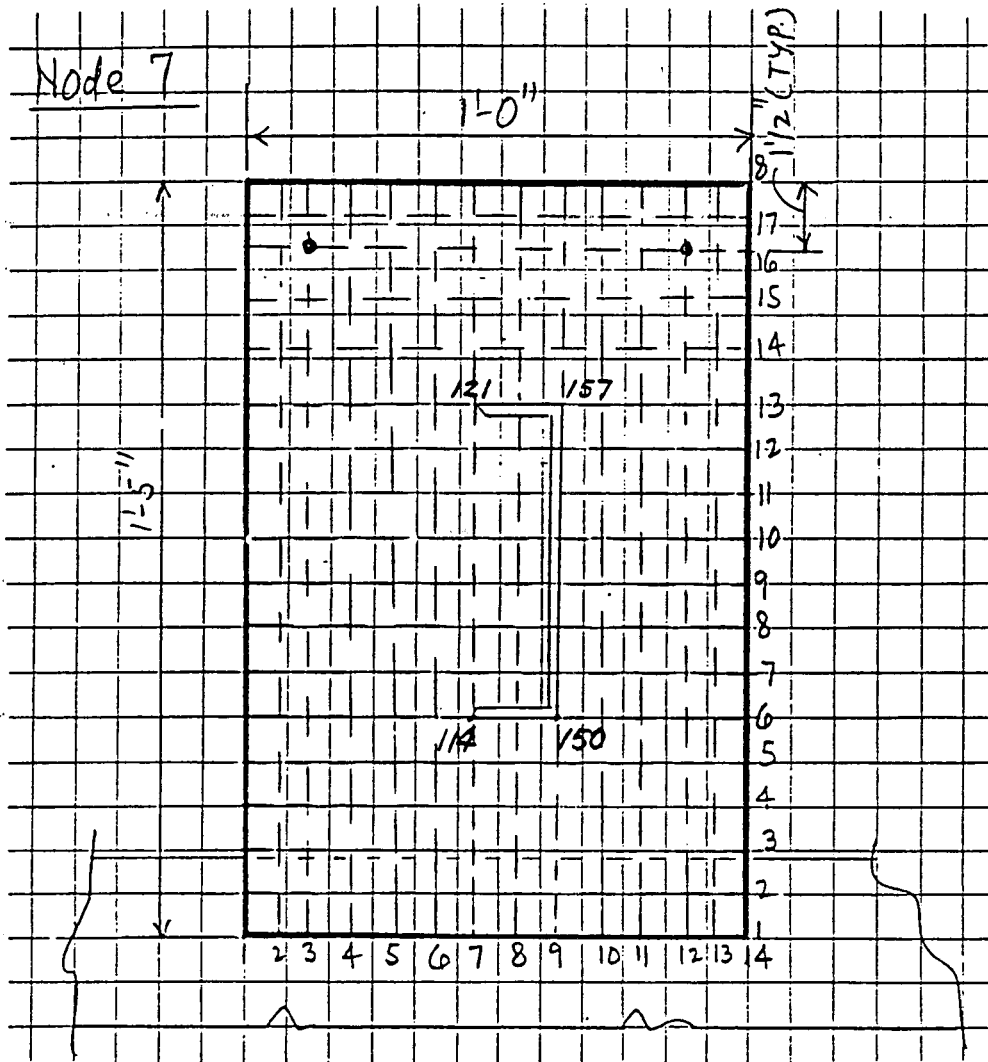
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Figure 11.3.1.5 BASEPLATE II Model for Nodes 7 of ANSYS Model

(Ref: WCG-1-B25 R2)



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Table 11.3.1.1 BASEPLATE II Input for Rot. Stiff. for Node 1, 4 & 11 of ANSYS Model

(Ref: Input = BS4PR9C; Output = BS4PACK)

```
3 BS4PR9C- NODE 4; SURF MOUNT PLATE; 48N968
OUT ,,-1,100/
CON ,,,3000,1.7,1.7/
PLA 11,12,,11,11,1.0,29/
APR 7,2,1,7.0,2.09,.366,.21/
BPR ,,1,.4E6,1.0E6,3000,3300/
BOL 3,3,1,2,2/
BOL 10,3,1,9,2/
BOL 3,10,1,2,9/
BOL 10,10,1,9,9/
END /
ATT 5,3,,3.41,2.00/
JST 5,3,7,2.09,0/
IST 7,3,10,7.0,90/
JST 7,10,5,2.09,180/
END /
END /
POI 5,3,,1.55,3.5/
LOA ,,1,0.0,0.0,0.0,100000.0,100000.0,0/
END /
END OF JOB
```

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Table 11.3.1.2 BASEPLATE II Input for Rot. Stiff. for Node 8 of ANSYS Model

(Ref: Input = BSBPR9C ; Output = BSBP9CR)

```
3 BSBPR9C-NODE 8; EMB PLATE; 48N968.  
OUT ,,-1,100/  
CON ,,,3000,1.7,1.7/  
PLA 10,10,,8,8,.5,29/  
APR 6,2,1,5.75,2.09,.366,.21/  
BPR ,,1 2.46E5,1.0E6,4750,431E/  
BOL 3,3,1,1.5,1.5/  
BOL 8,3,1,6.5,1.5/  
BOL 3,8,1,1.5,6.5/  
BOL 8,8,1,6.5,6.5/  
END /  
ATT 5,4,,3.75,2.25/  
JST 5,4,7,2.09,0/  
IST 5,4,10,5.75,90/  
END /  
END /  
POI 5,4,,.5,3.5/  
LOA ,,1,0.0,0.0,0.0,100000.0,100000.0,0/  
END /  
END OF JOB
```

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Table 11.3.1.3 BASEPLATE II Input for Rot. Stiff. for Node 10 of ANSYS Model

(Ref: Input = BS10PR9 ; Output = BS10PR9R)

```

3 BS10PR9-NODE 10; SURF MOUNT PLATE; 48N968.
OUT ,,-1,100/
CON ,,,3000,1.7,1.7/
PLA 11,11,,9,10,.75,29/
APR 7,2,1,6.63,2.09,.366,.21/
BPR ,,1,-4E6,1.0E6,1550,1400/
BOL 3,9,1,1.5,8.5/
BOL 9,9,1,7.5,8.5/
IWE 1,1,4/
JWE 1,1,11/
IWE 11,1,4/
END /
ATT 4,4,,2.29,3.37/
JST 4,4,6,2.09,0/
IST 6,4,11,6.63,90/
END /
END /
POI 4,4,,1.58,3.5/
LOA ,,1,0.0,0.0,0.0,100000.0,100000.0,0/
END /
END OF JOB
    
```

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Table 11.3.1.4 BASEPLATE II Input for Rot. Stiff. for Node 7 of ANSYS Model

(Ref: Input = BS7PR9C ; Output = BS7P9CR)

```
3 BS7PR9C-NODE 7; SURF MOUNT PLATE; 48N968.
OUT ,,-1,100/
CON ,,,3000,1.7,1.7/
PLA 14,18,,12,17,1.0,29/
APR 7,2,1,7.0,2.09,.366,.21/
BPR ,,1,.4E6,1.0E6,3000,3300/
BOL 3,16,1,1.5,15.5/
BOL 12,16,1,10.5,15.5/
IWE 1,1,3/
JWE 1,1,14/
IWE 14,1,3/
END /
ATT 7,6,,5.41,5.0/
JST 7,6,9,2.09,0/
IST 9,6,13,7.0,90/
JST 9,13,7,2.09,180/
END /
END /
POI 7,6,,1.58,3.5/
LOA ,,1,0.0,0.0,0.0,100000.0,100000.0,0/
END /
END OF JOB
```


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Table 11.3.1.5 BASEPLATE II Input for Rot. Stiff. for Node 12 of ANSYS Model

(Ref: Input = BS12PR9; Output = BS12P9R)

```
3 BS12PR9C-NODE 12; SURF MOUNT PLATE; 48N968.  
OUT ,,-1,100/  
CON ,,,3000;1.7;1.7/  
PLA 13,13,,12,12,0.5,29/  
APR 5,1,1,7.0,2.09,.366,.21/  
BPR ,,1,.4E6,1.0E6,1550,1400/  
BOL 3,11,1,2.0,10.0/  
BOL 11,11,1,10.0,10.0/  
IWE 1,1,2/  
JWE 1,1,13/  
IWE 13,1,2/  
END /  
ATT 5,2,,3.91,1.0/  
JST 5,2,7,2.09,0/  
IST 7,2,9,7.0,90/  
JST 7,9,5,2.09,180/  
END /  
END /  
POI 5,2,,1.58,3.5/  
LOA ,,1,0.0,0.0,0.0,100000.0,100000.0,0/  
END /  
END OF JOB
```

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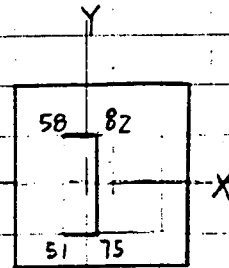
Rotational Stiffness of Base II

REF.

NDDies 194 & 11

FOR M_x

1) $\Delta_{75} = -0.000813'' = \Delta_2$
 $\Delta_{82} = 0.017233'' = \Delta_1$
 $L = 7''$



Base II
 Output File:
 B54P9CR
 (z-displ.)

$$\theta = \tan^{-1} \frac{\Delta_1 + (-\Delta_2)}{L}$$

$$= \tan^{-1} \frac{(0.017233 + 0.000813)}{7}$$

$$\theta = .2076^\circ$$

$$K_{MM} = \frac{M}{\theta} = \frac{100}{.2076 \times \frac{\pi}{180}} = 27600 \frac{\text{K-in}}{\text{Rad.}}$$

FOR M_y

2) $\Delta_{51} = .007229'' = \Delta_1$
 $\Delta_{75} = -.000813'' = \Delta_2$ (z-DISP)
 $L = 2.09''$

$$\theta = \tan^{-1} \frac{\Delta_1 + (-\Delta_2)}{L}$$

$$= \tan^{-1} \frac{(0.007229 + 0.000813)}{2.09}$$

$$= 0.22^\circ$$

$$K_{MM} = \frac{M}{\theta} = \frac{100}{.22 \times \frac{\pi}{180}} = 25989 \frac{\text{K-in}}{\text{Rad.}}$$

3) $\Delta_{58} = 0.025275''$
 $\Delta_{82} = 0.017233''$
 $L = 2.09''$

$$\theta = \tan^{-1} \frac{(0.025275 - 0.017233)}{2.09}$$

$$= .22^\circ$$

$$K_{MM} = \frac{100}{.22 \times \frac{\pi}{180}} = 25989 \text{ K-in / Rad.}$$

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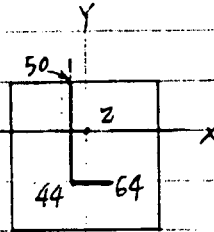
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Rotational Stiffness of Base K

Node 8

For Mx

1) $\Delta_{44} = 0.021104'' = \Delta_2$ (z-displ.)
 $\Delta_{50} = 0.091684'' = \Delta_1$
 $L = 5.75''$



REF.

Base K II
 Output:
 BS8P9CR
 (z-disp.)

$$\theta = \tan^{-1} \frac{\Delta_1 + \Delta_2}{L} = \tan^{-1} \frac{(0.091684 + 0.021104)}{5.75}$$

$$= 0.703^\circ$$

$$K_{MM} = \frac{100}{0.703 \times \frac{\pi}{180}} = 8147 \text{ K-in/Rad.}$$

For My

2) $\Delta_{44} = 0.021104'' = \Delta_1$ (z-disp.)
 $\Delta_{64} = -0.005286'' = \Delta_2$
 $L = 2.09''$

$$\theta = \tan^{-1} \left(\frac{0.021104 + 0.005286}{2.09} \right)$$

$$= 0.723^\circ$$

$$K_{MM} = \frac{100}{0.723 \times \frac{\pi}{180}} = 7920 \text{ K-in/Rad.}$$

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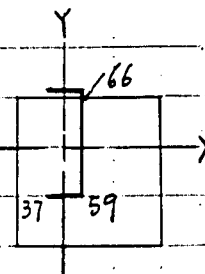
Rotational stiffness of Base K_θ

Ref: Base & II Output, BS10P9R (z-disp.)

Node 10

FOY M_X

1) $\Delta_{59} = -0.003697'' = \Delta_2$ (z-disp.)
 $\Delta_{66} = 0.034592'' = \Delta_1$
 $L = 6.63''$



$\theta = \text{TAN}^{-1} \left(\frac{0.034592 + 0.003697}{6.63} \right)$
 $= 0.33^\circ$

$K_{MM} = \frac{100}{0.33 \times \frac{\pi}{180}} = 17,315 \text{ K-in/radian}$

FOY M_Y

2) $\Delta_{37} = 0.006139'' = \Delta_1$
 $\Delta_{59} = -0.003697'' = \Delta_2$ (z-disp.)
 $L = 2.09''$

$\theta = \text{TAN}^{-1} \left(\frac{0.006139 + 0.003697}{2.09} \right)$
 $= 0.27^\circ$

$K_{MM} = \frac{100}{0.27 \times \frac{\pi}{180}} = 21,249 \text{ K-in/radian}$

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Rotational Stiffness of Base R

Ref:
 Base R II
 output:
 BSTP9CR
 (z-disp.)

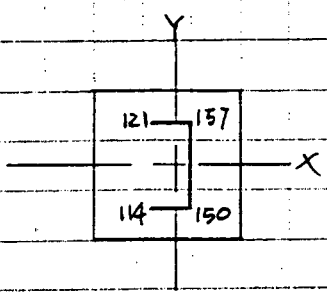
Node 7

FOY Mx

$$\Delta_{157} = .012269'' = \Delta_1 \quad (\text{z-DISP.})$$

$$\Delta_{150} = -.000121'' = \Delta_2$$

$$L = 7''$$



$$\theta = \text{TAN}^{-1} \left(\frac{.012269 + .000121}{7} \right)$$

$$= 0.101^\circ$$

$$K_{MM} = \frac{100}{0.101 \times \frac{\pi}{180}} = 56498 \text{ K-in/radian}$$

FOY My

$$\Delta_{114} = .006187'' = \Delta_1 \quad (\text{z-DISP.})$$

$$\Delta_{150} = -.000121'' = \Delta_2$$

$$L = 2.09''$$

$$\theta = \text{TAN}^{-1} \left(\frac{.006187 + .000121}{2.09} \right)$$

$$= 0.173^\circ$$

$$K_{MM} = \frac{100}{.173 \times \frac{\pi}{180}} = 33132 \text{ K-in/radian}$$

$$\Delta_{121} = 0.018577'' = \Delta_1 \quad \theta = \text{TAN}^{-1} \left(\frac{.018577 - .012269}{2.09} \right)$$

$$\Delta_{157} = 0.012269'' = \Delta_2 = .173^\circ$$

$$L = 2.09'' \quad K_{MM} = \frac{100}{.173 \times \frac{\pi}{180}} = 33132 \text{ K-in/yad.}$$

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Rotational stiffness of Base π		Ref =
<u>Node 12</u>		Base π π
<u>For Mx</u>		Output =
		BS12P9R
		(z-disp)
1) $\Delta_{87} = .030477'' = \Delta_1$		
$\Delta_{80} = -.004893'' = \Delta_2$		
$L = 7''$		
$\theta = \frac{\text{TAN}^{-1} \frac{\Delta_1 + \Delta_2}{L}}{= \text{TAN}^{-1} \left(\frac{.030477 + .004893}{7} \right)}$		
$= .2895^\circ$		
$K_{MM} = \frac{100}{.2895 \times \frac{\pi}{180}} = 19791 \text{ K-in/Rad.}$		
<u>2) For My</u>		
$\Delta_{61} = .046604'' = \Delta_1$		
$\Delta_{87} = .030477'' = \Delta_2$		
$L = 2.09''$		
$\theta = \frac{\text{TAN}^{-1} (.046604 - .030477)}{2.09}$		
$= .442^\circ$		
$K_{MM} = \frac{100}{.442 \times \frac{\pi}{180}} = 12960 \text{ K-in/Rad.}$		
$\Delta_{54} = .011234''$		
$\Delta_{80} = -.004893''$		
$L = 2.09''$		
$\theta = \frac{\text{TAN}^{-1} (.011234 + .004893)}{2.09} = .442^\circ$		
$K_{MM} = 12960 \text{ K-in/Rad.}$		

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11.3.1 Mathematical Model (Cont'd)Loading Condition

Since these two worst case members (1-27 and 2&3-9C) are part of the worst case platform, 48N968-00PF01U1, all loading data are obtained from the platform evaluation calculation, WCG-2-50 Vol. 6, R1 (Reference 9.15). As shown in Table 11.3.1.6, equivalent densities are calculated for each member to consider the dead loads (including self-weight and grating) and the sustained live loads (10 psf per WB-DC-20-21). SSE loads are applied using the equivalent static approach. ANSYS ACEL command is used to apply the G-loads. In accordance with Calculation WCG-2-50 R1, Vol. 6, ZPA (4% damping) are used for members: 0.9G is considered for the Horizontal (N-S and E-W) directions and 0.33G is used for the Vertical direction. Small bore pipes and hand rails are modeled using ANSYS STIF21 Mass elements connected to members with rigid links. The location and the weight of the pipe and hand rails are obtained from the GTSTRUDL input file in WCG-2-50 R1, Vol. 6. 9.38G and 3.38G are applied in the Horizontal and Vertical directions for small bore pipes.

The YZ-SSE load (combination of SSE in Y and Z directions) is more critical than the XZ-SSE load (combination of SSE in X and Z directions). Therefore, YZ-SSE load is specified with DL and SLL followed by incremental temperature steps from 70°F (Ambient temperature) up to 327°F (Accident temperature). However, the XZ-SSE load is still applied in Load Step 1 in the ANSYS analysis and is for reference only. The YZ-SSE load, which supersedes the XZ-SSE load in Load Step 1, is applied in Load Step 2. The temperature loads are then specified in the following load steps from Load Step 3 up to Load Step 16 (final load step).

The ANSYS input files are presented in Section 11.3.2.

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Table 11.3.1.6 Equivalent Densities for Members of ANSYS Model "PR9C"

ANSYS MEMBER	STRUDL MEMBER	BEAM	GRATING			SLL 10PSF			TOTAL
		DENS LB/IN ³	WA LB/FT	WB LB/FT	WG LB/IN	WA LB/FT	WB LB/FT	WL LB/IN	DENS LB/IN ³
1,28	'B6-1'	0.283	11.5	11.5	0.958	0	0	0	0.617
2	'B6-2'	0.283	11.5	11.5	0.958	0	0	0	0.617
3	'B6-3'	0.283	11.5	11.5	0.958	0	0	0	0.617
4	'B2-1'	0.283	6	6	0.500	5	5	0.417	0.602
5	'B2-2'	0.283	6	6	0.500	5	5	0.417	0.602
6	'B2-3'	0.283	6	6	0.500	5	5	0.417	0.602
7	'B3-1'	0.283	15	15	1.250	12.5	12.5	1.042	1.081
8	'B3-2'	0.283	15	15	1.250	12.5	12.5	1.042	1.081
9	'B4-1'	0.283	22.5	22.5	1.875	18.75	18.75	1.563	1.481
10	'B4-2'	0.283	22.5	22.5	1.875	18.75	18.75	1.563	1.481
11,29	'B6-4'	0.283	11.5	11.5	0.958	0	0	0	0.617
12	'B7-5'	0.283	17.325	15	1.347	15	12.5	1.146	1.152
13	'B7-4'	0.283	19.65	17.325	1.541	16	15	1.292	1.270
14	'B7-3'	0.283	14	12.22	1.093	12	10	0.917	0.983
15	'B7-2'	0.283	25	22	1.958	20	18	1.583	1.517
16	'B7-1'	0.283	28	25	2.208	23	20	1.792	1.677
21	'B9-5'	0.283	8.8	7.5	0.679	7.3	6.25	0.565	0.716
22	'B9-4'	0.283	10.7	8.8	0.813	8.9	7.3	0.675	0.801
23	'B9-3'	0.283	11.4	10.7	0.921	9.5	8.9	0.767	0.871
24	'B9-2'	0.283	13.44	11.4	1.035	11.2	9.5	0.863	0.944
25	'B9-1A'	0.283	15	13.44	1.185	12.5	11.2	0.988	1.040
26	'B9-1'	0.283	15	13.44	1.185	12.5	11.2	0.988	1.040
32	'B11-2'	0.283	23.7	23.7	1.975	13.3	13.3	1.108	1.357
33	'B11-1'	0.283	23.7	23.7	1.975	13.3	13.3	1.108	1.357
37	'B1-1'	0.283	17.5	17.5	1.458	5	5	0.417	0.936
37	'B1-2'	0.283	17.5	17.5	1.458	5	5	0.417	0.936
38	'B1-3'	0.283	6	6	0.500	5	5	0.417	0.602
40	'B5-1'	0.283	19	19	1.583	6.25	6.25	0.521	1.016
40	'B5-2'	0.283	19	19	1.583	6.25	6.25	0.521	1.016

NOTES: WA, WB = DISTRIBUTED LOADS AT MEMBER ENDS FROM STRUDL INPUT
 WG, WL = (WA+WB)/24
 DENS = DS + (WG+WL)/A
 DS = STEEL DENSITY=0.283; A=AREA=2.87

* See WCG-2-50, Vol.6, R1, Sht. 35 for STRUDL member plot; Sht. 125-136 for Loading Data.

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11.3.2 ANSYS Input

Two ANSYS input files are presented in this section:

11.3.2.1 "PR9C" - Analysis for the Original structure

11.3.2.2 "PR9CM" - Analysis for the modified structure

Note: The member of the worst case 1-27 is not accepted per DG-C1.6.12 R1 (For details, see Section 11.3.4.1, Member Qualification for worst case 1-27). A slotted hole is installed to release the thermal-induced axial force. The modified structure is re-evaluated with the analysis "PR9CM" and the member is qualified using the results from the new analysis.

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11.3.2.1 ANSYS Input File "PR9C" (Analysis for the Original Structure)

```

1 /PREP7
2 /TITLE,PR9C - EVAL. FOR WORST CASES 1-27 & 2&3-9(C), DWG 48N968, EL. 783'-8 1/2"
3 /COM
4 /COM BRANCH/PROJECT ID = WCG-1-969
5 /COM
6 /COM PREPARED BY: SAL 8/19/91
7 /COM CHECKED BY: C. Y. OU 8/19/91
8 /COM
9 /COM INPUT: PR9C
10 /COM OUTPUT: PR9CR
11 /COM FILE12: PR9CF
12 /COM PLOT: PR9CP
13 /COM
14 C*** ANSYS ANALYSIS OPTIONS AND REFERENCES
15 KAN,0
16 KAY,6,1
17 KNL,1
18 C*** ELEMENT TYPE
19 ET,1,24,,,,,1 * STIF24
20 ET,2,4,,,,,1 * STIF4
21 ET,3,14,,1 * SPRING KX
22 ET,4,14,,2 * SPRING KY
23 ET,5,14,,3 * SPRING KZ
24 ET,6,14,,4 * SPRING ROTX
25 ET,7,14,,5 * SPRING ROTY
26 ET,8,14,,6 * SPRING ROTZ
27 ET,9,21,,,, * 3D MASS ELE.
28 ET,10,14,,0 * SPRING ALONG TWO POINTS
29 C*** MATERIAL PROPERTIES
30 EX,1,29000
31 RP21,1
32 ALPX,1,6.5E-6
33 RP21,1
34 DENS,1,0.283E-3 * STEEL DENSITY - NON-LINEAR
35 DENS,2,0.283E-3 * STEEL DENSITY - LINEAR
36 DENS,3,0.602E-3 * ELE. 4-6
37 DENS,4,1.081E-3 * ELE. 7-8
38 DENS,5,1.481E-3 * ELE. 9-10
39 DENS,6,0.617E-3 * ELE. 1-3, 28, 11,29
40 DENS,7,1.152E-3 * ELE. 12
41 DENS,8,1.270E-3 * ELE. 13
42 DENS,9,0.983E-3 * ELE. 14
43 DENS,10,1.517E-3 * ELE. 15
44 DENS,11,1.677E-3 * ELE. 16
45 DENS,12,0.716E-3 * ELE. 21
46 DENS,13,0.801E-3 * ELE. 22
47 DENS,14,0.871E-3 * ELE. 23
48 DENS,15,0.944E-3 * ELE. 24
49 DENS,16,1.040E-3 * ELE. 25-26
50 DENS,17,0.0 * ZERO DENSITY - RIGID LINK

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11.3.2.1 ANSYS Input File "PR9C" (Continued)

```

51 DENS,18,1.357E-3      * ELE. 32-33
52 DENS,19,0.936E-3      * ELE. 37
53 DENS,20,0.602E-3      * ELE. 38
54 DENS,21,1.016E-3      * ELE. 40
55 C***
56 MPTMP,1,70,100,200,300,400
57 RP21,1
58 MPDATA,EX,1,1,29.0E3,28.9E3,28.3E3,27.9E3,27.4E3
59 RP21,,1
60 MPDATA,ALPX,1,1,6.50E-6,6.50E-6,6.50E-6,6.67E-6,6.86E-6

61 RP21,,1
62 C*** NONLINEAR MATERIAL CONSTANT
63 *CREAT,MNL
64 NL,ARG1,13,10
65 NL,ARG1,19,70,100,200,300,400
66 NL,ARG1,25,36,36.0,36.0,36.0,36.0
67 NL,ARG1,31,150,150,150,150,150
68 *END
69 C***
70 *USE,MNL,1
71 *USE,MNL,3
72 *USE,MNL,4
73 *USE,MNL,6
74 *USE,MNL,12
75 *USE,MNL,13
76 *USE,MNL,14
77 *USE,MNL,15
78 *USE,MNL,16
79 C***
80 C*** CROSS SECTIONAL PROPERTIES
81 C*** CHANNEL C7X9.8 *****
82 CF=2.090
83 TCF=0.366
84 CD=7.00
85 TCW=0.210
86 XBR=(0.541)+0.07069      * SAL 7/10/91
87 C***
88 XO=(TCW/2)-(XBR)
89 CC=CF-(TCW/2)
90 DD=(CD-TCF)/2.
91 QC=CC/2
92 QD=DD/2
93 CC=CC+XO
94 QC=QC+XO
95 *STAT
96 C***
97 R,1,CC,DD,0,QC,DD,TCF      * STIF24 REAL PROP. - C7X9.8
98 RMORE,XO,DD,TCF,XO,QD,TCW
99 RMORE,XO,0,TCW,XO,-QD,TCW
100 RMORE,XO,-DD,TCW,QC,-DD,TCF

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11.3.2.1 ANSYS Input File "PR9C" (Continued)

```

101 RMORE,CC,-DD,TCF
102 C***
103 R,2,2.87,0.968,21.3,7.0,2.09 * C7X9.8
104 RMORE,,0.1
105 R,3,2.64,0.632,8.90,5.0,1.885 * C5X9 - ADDED MEMBERS
106 RMORE,,0.109
107 R,4,4000,5000,5000,20,20 * ANCHOR SHEAR SPRINGS & RIGID BEAM
108 R,5,8147 * SPRING ROTY @NODE 8
109 R,6,17315 * SPRING ROTY @NODE 10
110 R,7,27600 * SPRING ROTX @NODES 1,4,11
111 R,8,56498 * SPRING ROTY @NODE 7
112 R,9,7920 * SPRING ROTZ @NODE 8
113 R,10,21249 * SPRING ROTZ @NODE 10
114 R,11,25989 * SPRING ROTZ @NODES 1,4
115 R,12,33132 * SPRING ROTZ @NODE 7
116 R,13,1590 * VERT. SPR. 'C24' - NODE 32
117 C*** GENERALIZED MASS
118 R,14,.020,.020,.020 * NODE 95 (28)
119 R,15,.031,.031,.031 * NODE 96 (29)
120 MASX=(.035)*9.38
121 MASX=MASX/0.9
122 MASY=MASX
123 MASZ=(.035)*4.38
124 MASZ=MASZ/1.33
125 R,16,MASX,MASY,MASZ * NODE 97 (30)
126 R,17,100,5000,5000 * ANCHOR SHEAR SPRINGS & RIGID BEAM
127 R,18,19791 * SPRING ROTY @NODE 12
128 R,19,12960 * SPRING ROTZ @NODE 12
129 R,20,2885 * SPRING AXIAL @NODE 32
130 R,21,(300*4) * SHEAR SPRING @NODES 1 & 4
131 C***
132 C*** NODAL COORDINATES
133 CSYS,0
134 N,1,67.68
135 N,101,67.68
136 N,2,67.68,5.
137 N,102,67.68,5.
138 N,3,67.68,44.96
139 N,4,103.68
140 N,104,103.68
141 N,5,103.68,5.
142 N,105,103.68,5.
143 N,6,103.68,44.96
144 N,7,145.68,44.96
145 N,107,145.68,44.96
146 N,8,0,29.36
147 N,108,0,29.36
148 N,9,67.68,53.96
149 N,109,67.68,53.96
150 N,10,135.36,78.76

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11.3.2.1 ANSYS Input File "PR9C" (Continued)

```

151 N,110,135.36,78.76
152 N,11,115.68
153 N,111,115.68
154 N,12,145.68,27.96
155 N,112,145.68,27.96
156 N,13,67.68,30.
157 N,14,103.68,30.
158 N,15,85.63,5.
159 N,16,85.63,30.0
160 N,17,85.63,44.96
161 N,18,115.68,44.96
162 N,19,129.18,44.96
163 N,20,5.69,31.2
164 N,21,24.48,38.24
165 N,22,48.92,47.15
166 N,23,74.31,56.03
167 N,24,86.51,60.61
168 N,25,95.44,63.86
169 N,26,117.52,71.9
170 N,27,129.03,76.09
171 N,28,67.68,2.5
172 RP2,67,-3.75,0,29.5
173 N,29,67.68,46.44
174 RP2,67,-3.742,0,32.516
175 N,30,50.986,47.884
176 RP2,67,0.61563,-1.6914,-13.5
177 N,31,115.68,27.96
178 FILL,8,20,1,41,1
179 FILL,20,21,2,45,1
180 FILL,21,22,2,51,1

181 FILL,22,30,1,55,1
182 FILL,30,9,3,61,1
183 FILL,9,23,2,65,1

184 FILL,23,24,2,71,1
185 FILL,24,25,2,75,1
186 FILL,25,26,3,81,1
187 FILL,26,27,3,85,1
188 FILL,13,3,3,91,1
189 N,32,56.11,68.79,5.69 * JOINT '4A' - REF. STRUDL MOEEL
190 DX=(56.11)-67.68
191 DY=(68.79)-61.78
192 N,132,DX+56.11,DY+68.79,5.69
193 N,33,67.68,61.78,5.69 * JOINT '38A'
194 N,34,74.31,56.03,5.69 * JOINT '38B' ABOVE NODE23
195 N,35,67.68,53.96,5.69 * JOINT '39A' ABOVE NODE9
196 N,99,103.68,5.,100
197 N,100,67.68,53.96,100
198 C*** ELEMENT CONNECTIVITIES
199 MAT,6 STYPE,1 SREAL,1 * STIF24
200 EN,1,1,28,100
    
```

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11.3.2.1 ANSYS Input File "PR9C" (Continued)

201	EN,2,2,13,100			
202	EN,3,13,91,100	\$.91,91,92,100	\$.92,92,93,100	\$.93,93,3,100
203	EN,28,28,2,100			
204	EN,11,3,29,100			
205	EN,29,29,109,100			
206	MAT,3 STYPE,1	\$REAL,1	* STIF24	
207	EN,4,4,5,99			
208	EN,5,5,14,99			
209	EN,6,14,6,99			
210	MAT,4 STYPE,1	\$REAL,1	* STIF24	
211	EN,7,102,15,99			
212	EN,8,15,105,99			
213	MAT,1 STYPE,1	\$REAL,1	* STIF24	
214	EN,17,8,41,100	\$.41,41,20,100		
215	EN,18,20,45,100	\$.45,45,46,100	\$.46,46,21,100	
216	EN,19,21,51,100	\$.51,51,52,100	\$.52,52,22,100	
217	EN,20,22,55,100	\$.55,55,30,100		
218	EN,27,30,61,100	\$.61,61,62,100	\$.62,62,63,100	\$.63,63,9,100
219	MAT,12 STYPE,1	\$REAL,1	* STIF24	
220	EN,21,9,65,100	\$.65,65,66,100	\$.66,66,23,100	
221	MAT,13 STYPE,1	\$REAL,1	* STIF24	
222	EN,22,23,71,100	\$.71,71,72,100	\$.72,72,24,100	
223	MAT,14 STYPE,1	\$REAL,1	* STIF24	
224	EN,23,24,75,100	\$.75,75,76,100	\$.76,76,25,100	
225	MAT,15 STYPE,1	\$REAL,1	* STIF24	
226	EN,24,25,81,100	\$.81,81,82,100	\$.82,82,83,100	\$.83,83,26,100
227	MAT,16 STYPE,1	\$REAL,1	* STIF24	
228	EN,25,26,85,100	\$.85,85,86,100	\$.86,86,87,100	\$.87,87,27,100
229	EN,26,27,10,100			
230	MAT,5 STYPE,2	\$REAL,2	* STIF4	
231	EN,9,13,16			
232	EN,10,16,14			
233	MAT,7 STYPE,2	\$REAL,2	* STIF4	
234	EN,12,3,17			
235	MAT,8 STYPE,2	\$REAL,2	* STIF4	
236	EN,13,17,6			
237	MAT,9 STYPE,2	\$REAL,2	* STIF4	
238	EN,14,6,18			
239	MAT,10 STYPE,2	\$REAL,2	* STIF4	
240	EN,15,18,19			
241	MAT,11 STYPE,2	\$REAL,2	* STIF4	
242	EN,16,19,7			
243	MAT,18 STYPE,2	\$REAL,3	* STIF4	
244	EN,32,32,33			
245	EN,33,33,34			
246	MAT,2 STYPE,2	\$REAL,3	* STIF4	
247	EN,34,33,35			
248	MAT,19 STYPE,2	\$REAL,2	* STIF4	
249	EN,37,11,31			
250	MAT,20 STYPE,2	\$REAL,2	* STIF4	

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11.3.2.1 ANSYS Input File "PR9C" (Continued)

```

251 EN,38,31,18
252 MAT,2 $TYPE,2 $REAL,2 * STIF4
253 EN,39,18,25
254 MAT,21 $TYPE,2 $REAL,2 * STIF4
255 EN,40,31,12
256 MAT,17 $TYPE,2 $REAL,4 * RIGID LINK
257 EN,35,35,9
258 EN,36,34,23
259 EN,95,28,95
260 EN,96,29,96
261 EN,97,30,97
262 TYPE,3 $REAL,4 * SPRING X
263 EN,171,111,11
264 TYPE,4 $REAL,4 * SPRING KY
265 EN,142,108,8
266 TYPE,4 $REAL,4 * SPRING KY
267 EN,132,107,7
268 EN,152,110,10
269 EN,182,112,12
270 TYPE,3 $REAL,21 * SPRING KX
271 C*** EN,101,101,1
272 C*** EN,104,4,104
273 TYPE,5 $REAL,13 * SPRING KZ NODE 32
274 EN,163,132,32
275 TYPE,10 $REAL,20 * SPRING AXIAL NODE 32
276 EN,161,132,32
277 TYPE,6 $REAL,7 * SPRING NODES 1,4 RX
278 EN,114,101,1
279 EN,124,104,4
280 EN,174,111,11
281 TYPE,7 $REAL,8 * SPRING NODE 7 RY
282 EN,135,107,7
283 TYPE,7 $REAL,18 * SPRING NODE 12 RY
284 EN,175,112,12
285 TYPE,7 $REAL,5 * SPRING NODE 8 RY
286 EN,145,108,8
287 TYPE,7 $REAL,6 * SPRING NODE 10 RY
288 EN,155,110,10
289 TYPE,8 $REAL,11 * SPRING NODES 1,4 RZ
290 EN,116,101,1
291 EN,126,104,4
292 EN,176,111,11
293 TYPE,8 $REAL,12 * SPRING NODE 7 RZ
294 EN,136,107,7
295 TYPE,8 $REAL,19 * SPRING NODE 12 RZ
296 EN,186,112,12
297 TYPE,8 $REAL,9 * SPRING NODE 8 RZ
298 EN,146,108,8
299 TYPE,8 $REAL,10 * SPRING NODE 10 RZ
300 EN,156,110,10

```

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11.3.2.1 ANSYS Input File "PR9C" (Continued)

```

301 TYPE,9 $REAL,14 * MASS NODE 95 (28)
302 EN,201,95
303 TYPE,9 $REAL,15 * MASS NODE 96 (29)
304 EN,202,96
305 TYPE,9 $REAL,16 * MASS NODE 97 (30)
306 EN,203,97
307 C***
308 /COM
309 /COM MACRO TO SPECIFY ONE SPRING IN Y DIRECTION
310 /COM
311 *CREAT,SPR2
312 PSPRING,ARG1,TRAN,ARG2,ARG3
313 *END
314 C*** *USE,SPR2,1,(200*4),1 * SPRING IN X DIR.
315 C*** *USE,SPR2,4,(200*4),1 * SPRING IN X DIR.
316 C***
317 C*** COUPLED DOFS FOR MEMBER END RELEASES
318 CP,1,UX,102,2
319 CPLGEN,1,UY,UZ,ROTX,ROTY * NODE2
320 CPSGEN,2,3,1,5 * NODE5
321 CPSGEN,2,7,1,5 * NODE9
322 CPLIST,ALL
323 C*** LOCAL COORDINATES SYSTEMS
324 LOCAL,12,0,0,29.36,0,20 $NROT,10,110,100
325 LOCAL,13,0,0,29.36,0,33 $NROT,8,108,100
326 CSYS,0
327 C*** BOUNDARY CONDITIONS
328 D,1,ROTY,,,4,3,UX,UY,UZ * SAL 6/26/91 SAL 7/18/91
329 D,11,ROTY,,,,,UY,UZ
330 D,7,ROTX,,,12,5,UX,UZ
331 D,8,UZ,,,10,2,ROTX,UX
332 D,101,ALL,,,110,3
333 D,108,ALL,,,132,24
334 D,111,ALL,,,112,1
335 C*** ANALYSIS OPTIONS
336 ITER,-20,20
337 TREF,70
338 POSTR,,1,5
339 C*** PLOT GEOMETRY AND B.C.
340 /SHOW
341 /PBC,ALL,1
342 EPLO
343 /PNUM,NODE,1
344 EPLO
345 /PNUM,NODE,-1
346 /PNUM,ELEM,1
347 EPLO
348 /VIEW,,1,1,1
349 EPLO
350 C***

```


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11.3.2.1 ANSYS Input File "PR9C" (Continued)

```
351 C*** DL + LL + SSE XZ-PLANE
352 ACEL,0.9,0,1.33
353 C*** CONCENTRATED LOADS
354 C*** 'DL' + 'SMBOT' + 'SMBDN' + 'HANDDN' + 'HANDOT'
355 C*** TEMPERATURE LOADS
356 *CREAT,MAC
357 TUNIF,ARG1
358 LWRITE
359 *END
360 *USE,MAC,70
361 FDELE,ALL,ALL
362 C*** DL + LL + SSE YZ-PLANE
363 ACEL,0,-0.9,1.33
364 C*** 'DL' + 'SMBCW' + 'HANDDN' + 'HANDCW'
365 *USE,MAC,70
366 C***
367 C*** BOUNDARY CONDITIONS - 2
368 C***
369 *CREAT,MACD
370 TUNIF,ARG1
371 D,1,UX,-ARG2
372 D,4,UX,ARG2
373 LWRITE
374 *END
375 D,11,UY,(-1/32)
376 D,7,UX,(1/32),,12,5
377 D,10,UX,(1/32)
378 C***
379 DD1=(0.03/26)
380 *USE,MACD,80,DD1
381 RP25,,10,DD1
382 *USE,MACD,327,0.03
383 AFWRITE,,1
384 FINISH
385 /INPUT,27
386 FINISH
387 /POST1
388 NLINE,200
389 *CREAT,PLD
390 SET,ARG1
391 PRRFOR
392 PRDISP
393 PLDISP
394 PLDISP,1
395 *END
396 *USE,PLD,1
397 *USE,PLD,2
398 *USE,PLD,28
399 FINISH
400 C***
```

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11.3.2.1 ANSYS Input File "PR9C" (Continued)

```

401 /POST26
402 LINE,200
403 NUMVAR,50
404 TVAR,1
405 ESTR,2,1,53,TEMP
406 C***
407 *CREATE,STCN
408 C*** STORE STRAINS FOR CHANNEL (J NODE)
409 ESTR,13,ARG1,215,EE01
410 ESTR,14,ARG1,216,EP01
411 ESTR,15,ARG1,227,EE03
412 ESTR,16,ARG1,228,EP03
413 ESTR,17,ARG1,251,EE07
414 ESTR,18,ARG1,252,EP07
415 ESTR,19,ARG1,263,EE09
416 ESTR,20,ARG1,264,EP09
417 PRVAR,2,13,14,15,16
418 PRVAR,2,17,18,19,20
419 *END
420 C***
421 *CREATE,STC1
422 C*** STORE STRAINS FOR CHANNEL (I NODE)
423 ESTR,43,ARG1,95,EE01
424 ESTR,44,ARG1,96,EP01
425 ESTR,45,ARG1,107,EE03
426 ESTR,46,ARG1,108,EP03
427 ESTR,47,ARG1,131,EE07
428 ESTR,48,ARG1,132,EP07
429 ESTR,49,ARG1,143,EE09
430 ESTR,50,ARG1,144,EP09
431 PRVAR,2,43,44,45,46
432 PRVAR,2,47,48,49,40
433 *END
434 C***
435 *CREAT,DSP
436 DISP,21,ARG1,UX,UX
437 DISP,22,ARG1,UY,UY
438 DISP,23,ARG1,UZ,UZ
439 PROD,24,21,21,,UXUX
440 PROD,25,22,22,,UYUY
441 PROD,26,23,23,,UZUZ
442 ADD,27,24,25,26,SS
443 SQRT,28,27,,SRSS
444 PRVAR,2,21,22,23,28
445 *END
446 C***
447 *USE,STC1,1
448 *USE,STC1,4
449 C***
450 *USE,STCN,45

```

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11.3.2.1 ANSYS Input File "PR9C" (Continued)

```
451 *USE,STCN,46
452 *USE,STCN,19
453 *USE,STCN,51
454 *USE,STCN,52
455 *USE,DSP,51
456 *USE,DSP,52
457 C***
458 *USE,STCN,28
459 *USE,DSP,2
460 *USE,STCN,4
461 *USE,DSP,5
462 *USE,DSP,1
463 *USE,DSP,4
464 C***
465 *CREAT,RFOR
466 RFORC,31,ARG1,FX,RFX
467 RFORC,32,ARG1,FY,RFY
468 RFORC,33,ARG1,FZ,RFZ
469 RFORC,34,ARG1,MX,RMX
470 RFORC,35,ARG1,MY,RMY
471 RFORC,36,ARG1,MZ,RMZ
472 PRVAR,31,32,33,34,35,36
473 *END
474 *USE,RFOR,8
475 *USE,RFOR,108
476 *USE,RFOR,10
477 *USE,RFOR,110
478 *USE,RFOR,1
479 *USE,RFOR,101
480 *USE,RFOR,4
481 *USE,RFOR,104
482 FINISH
```

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11.3.2.2 ANSYS Input File "PR9CM" (Analysis for Modified structure)

```

1 /PREP7
2 /TITLE,PR9CM - EVAL. FOR WORST CASES 1-27 & 2&3-9(C), DWG 48N968, EL. 783'-8 1/2
3 /COM
4 /COM BRANCH/PROJECT ID = WCG-1-969
5 /COM
6 /COM PREPARED BY: SAL 8/19/91
7 /COM CHECKED BY: C. Y. OU 8/19/91
8 /COM
9 /COM ANALYSIS FOR THE MODIFIED STRUCTURE
10 /COM
11 /COM INPUT: PR9CM
12 /COM OUTPUT: PR9CMR
13 /COM FILE12: PR9CMF
14 /COM PLOT: PR9CMP
15 /COM
16 C*** ANSYS ANALYSIS OPTIONS AND REFERENCES
17 KAN,0
18 KAY,6,1
19 KNL,1
20 C*** ELEMENT TYPE
21 ET,1,24,,,,,1 * STIF24
22 ET,2,4,,,,,1 * STIF4
23 ET,3,14,,1 * SPRING KX
24 ET,4,14,,2 * SPRING KY
25 ET,5,14,,3 * SPRING KZ
26 ET,6,14,,4 * SPRING ROTX
27 ET,7,14,,5 * SPRING ROTY
28 ET,8,14,,6 * SPRING ROTZ
29 ET,9,21,,,, * 3D MASS ELE.
30 ET,10,14,,0 * SPRING ALONG TWO POINTS
31 C*** MATERIAL PROPERTIES
32 EX,1,29000
33 RP21,1
34 ALPX,1,6.5E-6
35 RP21,1
36 DENS,1,0.283E-3 * STEEL DENSITY - NON-LINEAR
37 DENS,2,0.283E-3 * STEEL DENSITY - LINEAR
38 DENS,3,0.602E-3 * ELE. 4-6
39 DENS,4,1.081E-3 * ELE. 7-8
40 DENS,5,1.481E-3 * ELE. 9-10
41 DENS,6,0.617E-3 * ELE. 1-3, 28, 11,29
42 DENS,7,1.152E-3 * ELE. 12
43 DENS,8,1.270E-3 * ELE. 13
44 DENS,9,0.983E-3 * ELE. 14
45 DENS,10,1.517E-3 * ELE. 15
46 DENS,11,1.677E-3 * ELE. 16
47 DENS,12,0.716E-3 * ELE. 21
48 DENS,13,0.801E-3 * ELE. 22
49 DENS,14,0.871E-3 * ELE. 23
50 DENS,15,0.944E-3 * ELE. 24
    
```

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11.3.2.2 ANSYS Input File "PR9CM" (Continued)

```

51 DENS,16,1.040E-3      * ELE. 25-26
52 DENS,17,0.0          * ZERO DENSITY - RIGID LINK
53 DENS,18,1.357E-3     * ELE. 32-33
54 DENS,19,0.936E-3     * ELE. 37
55 DENS,20,0.602E-3     * ELE. 38
56 DENS,21,1.016E-3     * ELE. 40
57 C***
58 MPTEMP,1,70,100,200,300,400
59 RP21,1
60 MPDATA,EX,1,1,29.0E3,28.9E3,28.3E3,27.9E3,27.4E3
61 RP21,,1
62 MPDATA,ALPX,1,1,6.50E-6,6.50E-6,6.50E-6,6.67E-6,6.86E-6
63 RP21,,1
64 C*** NONLINEAR MATERIAL CONSTANT
65 *CREAT,MNL
66 NL,ARG1,13,10
67 NL,ARG1,19,70,100,200,300,400
68 NL,ARG1,25,36,36.0,36.0,36.0,36.0
69 NL,ARG1,31,150,150,150,150,150
70 *END
71 C***
72 *USE,MNL,1
73 *USE,MNL,3
74 *USE,MNL,4
75 *USE,MNL,6
76 *USE,MNL,12
77 *USE,MNL,13
78 *USE,MNL,14
79 *USE,MNL,15
80 *USE,MNL,16
81 C***
82 C*** CROSS SECTIONAL PROPERTIES
83 C*** CHANNEL C7X9.8 *****
84 CF=2.090
85 TCF=0.366
86 CD=7.00
87 TCW=0.210
88 XBR=(0.541)+0.07069      * SAL 7/10/91
89 C***
90 XO=(TCW/2)-(XBR)
91 CC=CF-(TCW/2)
92 DD=(CD-TCF)/2.
93 QC=CC/2
94 QD=DD/2
95 CC=CC+XO
96 QC=QC+XO
97 *STAT
98 C***
99 R,1,CC,DD,0,QC,DD,TCF      * STIF24 REAL PROP. - C7X9.8
100 RMORE,XO,DD,TCF,XO,QD,TCW
    
```

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11.3.2.2 ANSYS Input File "PR9CM" (Continued)

```

101 RMORE,X0,0,TCW,X0,-00,TCW
102 RMORE,X0,-DD,TCW,0C,-DD,TCF
103 RMORE,CC,-DD,TCF
104 C***
105 R,2,2.87,0.968,21.3,7.0,2.09 * C7X9.8
106 RMORE,,0.1
107 R,3,2.64,0.632,8.90,5.0,1.885 * CSX9 - ADDED MEMBERS
108 RMORE,,0.109
109 R,4,4000,5000,5000,20,20 * ANCHOR SHEAR SPRINGS & RIGID BEAM
110 R,5,8147 * SPRING ROTY @NODE 8
111 R,6,17315 * SPRING ROTY @NODE 10
112 R,7,27600 * SPRING ROTX @NODES 1,4,11
113 R,8,56498 * SPRING ROTY @NODE 7
114 R,9,0 * SPRING ROTZ @NODE 8
115 R,10,21249 * SPRING ROTZ @NODE 10
116 R,11,25989 * SPRING ROTZ @NODES 1,4
117 R,12,33132 * SPRING ROTZ @NODE 7
118 R,13,1590 * VERT. SPR. 'C24' - NODE 32
119 C*** GENERALIZED MASS
120 R,14,.020,.020,.020 * NODE 95 (28)
121 R,15,.031,.031,.031 * NODE 96 (29)
122 MASX=(.035)*9.38
123 MASX=MASX/0.9
124 MASY=MASX
125 MASZ=(.035)*4.38
126 MASZ=MASZ/1.33
127 R,16,MASX,MASY,MASZ * NODE 97 (30)
128 R,17,100,5000,5000 * ANCHOR SHEAR SPRINGS & RIGID BEAM
129 R,18,19791 * SPRING ROTY ODE 12
130 R,19,12960 * SPRING ROTZ ODE 12
131 R,20,2885 * SPRING AXIAL ODE 32
132 R,21,(300*4) * SHEAR SPRING ODES 1 & 4
133 C***
134 C*** NODAL COORDINATES
135 CSYS,0
136 N,1,67.68
137 N,101,67.68
138 N,2,67.68,5.
139 N,102,67.68,5.
140 N,3,67.68,44.96
141 N,4,103.68
142 N,104,103.68
143 N,5,103.68,5.
144 N,105,103.68,5.
145 N,6,103.68,44.96
146 N,7,145.68,44.96
147 N,107,145.68,44.96
148 N,8,0,29.36
149 N,108,0,29.36
150 N,9,67.68,53.96
    
```

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11.3.2.2 ANSYS Input File "PR9CM" (Continued)

```

151 N,109,67.68,53.96
152 N,10,135.36,78.76
153 N,110,135.36,78.76
154 N,11,115.68
155 N,111,115.68
156 N,12,145.68,27.96
157 N,112,145.68,27.96
158 N,13,67.68,30.
159 N,14,103.68,30.
160 N,15,85.63,5.
161 N,16,85.63,30.0
162 N,17,85.63,44.96
163 N,18,115.68,44.96
164 N,19,129.18,44.96
165 N,20,5.69,31.2
166 N,21,24.48,38.24
167 N,22,48.92,47.15
168 N,23,74.31,56.03
169 N,24,86.51,60.61
170 N,25,95.44,63.86
171 N,26,117.52,71.9
172 N,27,129.03,76.09
173 N,28,67.68,2.5
174 RP2,67,-3.75,0,29.5
175 N,29,67.68,46.44
176 RP2,67,-3.742,0,32.516
177 N,30,50.986,47.884
178 RP2,67,0.61563,-1.6914,-13.5
179 N,31,115.68,27.96
180 FILL,8,20,1,41,1
181 FILL,20,21,2,45,1
182 FILL,21,22,2,51,1
183 FILL,22,30,1,55,1
184 FILL,30,9,3,61,1
185 FILL,9,23,2,65,1
186 FILL,23,24,2,71,1
187 FILL,24,25,2,75,1
188 FILL,25,26,3,81,1
189 FILL,26,27,3,85,1
190 FILL,13,3,3,91,1
191 N,32,56.11,68.79,5.69 * JOINT '4A' - REF. STRUDL MODEL
192 DX=(56.11)-67.68
193 DY=(68.79)-61.78
194 N,132,DX+56.11,DY+68.79,5.69
195 N,33,67.68,61.78,5.69 * JOINT '38A'
196 N,34,74.31,56.03,5.69 * JOINT '38B' ABOVE NODE23
197 N,35,67.68,53.96,5.69 * JOINT '39A' ABOVE NODE9
198 N,99,103.68,5.,100
199 N,100,67.68,53.96,100
200 C*** ELEMENT CONNECTIVITIES
    
```

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11.3.2.2 ANSYS Input File "PR9CM" (Continued)

201	MAT,6	\$TYPE,1	\$REAL,1	* STIF24		
202	EN,1,1,28,100					
203	EN,2,2,13,100					
204	EN,3,13,91,100		\$,91,91,92,100	\$,92,92,93,100	\$,93,93,3,100	
205	EN,28,28,2,100					
206	EN,11,3,29,100					
207	EN,29,29,109,100					
208	MAT,3	\$TYPE,1	\$REAL,1	* STIF24		
209	EN,4,4,5,99					
210	EN,5,5,14,99					
211	EN,6,14,6,99					
212	MAT,4	\$TYPE,1	\$REAL,1	* STIF24		
213	EN,7,102,15,99					
214	EN,8,15,105,99					
215	MAT,1	\$TYPE,1	\$REAL,1	* STIF24		
216	EN,17,8,41,100		\$,41,41,20,100			
217	EN,18,20,45,100		\$,45,45,46,100	\$,46,46,21,100		
218	EN,19,21,51,100		\$,51,51,52,100	\$,52,52,22,100		
219	EN,20,22,55,100		\$,55,55,30,100			
220	EN,27,30,61,100		\$,61,61,62,100	\$,62,62,63,100	\$,63,63,9,100	
221	MAT,12	\$TYPE,1	\$REAL,1	* STIF24		
222	EN,21,9,65,100		\$,65,65,66,100	\$,66,66,23,100		
223	MAT,13	\$TYPE,1	\$REAL,1	* STIF24		
224	EN,22,23,71,100		\$,71,71,72,100	\$,72,72,24,100		
225	MAT,14	\$TYPE,1	\$REAL,1	* STIF24		
226	EN,23,24,75,100		\$,75,75,76,100	\$,76,76,25,100		
227	MAT,15	\$TYPE,1	\$REAL,1	* STIF24		
228	EN,24,25,81,100		\$,81,81,82,100	\$,82,82,83,100	\$,83,83,26,100	
229	MAT,16	\$TYPE,1	\$REAL,1	* STIF24		
230	EN,25,26,85,100		\$,85,85,86,100	\$,86,86,87,100	\$,87,87,27,100	
231	EN,26,27,10,100					
232	MAT,5	\$TYPE,2	\$REAL,2	* STIF4		
233	EN,9,13,16					
234	EN,10,16,14					
235	MAT,7	\$TYPE,2	\$REAL,2	* STIF4		
236	EN,12,3,17					
237	MAT,8	\$TYPE,2	\$REAL,2	* STIF4		
238	EN,13,17,6					
239	MAT,9	\$TYPE,2	\$REAL,2	* STIF4		
240	EN,14,6,18					
241	MAT,10	\$TYPE,2	\$REAL,2	* STIF4		
242	EN,15,18,19					
243	MAT,11	\$TYPE,2	\$REAL,2	* STIF4		
244	EN,16,19,7					
245	MAT,18	\$TYPE,2	\$REAL,3	* STIF4		
246	EN,32,32,33					
247	EN,33,33,34					
248	MAT,2	\$TYPE,2	\$REAL,3	* STIF4		
249	EN,34,33,35					
250	MAT,19	\$TYPE,2	\$REAL,2	* STIF4		

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11.3.2.2 ANSYS Input File "PR9CM" (Continued)

```

251 EN,37,11,31
252 MAT,20 $TYPE,2 $REAL,2 * STIF4
253 EN,38,31,18
254 MAT,2 $TYPE,2 $REAL,2 * STIF4
255 EN,39,18,25
256 MAT,21 $TYPE,2 $REAL,2 * STIF4
257 EN,40,31,12
258 MAT,17 $TYPE,2 $REAL,4 * RIGID LINK
259 EN,35,35,9
260 EN,36,34,23
261 EN,95,28,95
262 EN,96,29,96
263 EN,97,30,97
264 TYPE,3 $REAL,4 * SPRING X
265 EN,171,111,11
266 TYPE,4 $REAL,4 * SPRING KY
267 EN,142,108,8
268 TYPE,4 $REAL,4 * SPRING KY
269 EN,132,107,7
270 EN,152,110,10
271 EN,182,112,12
272 TYPE,3 $REAL,21 * SPRING KX
273 C*** EN,101,101,1
274 C*** EN,104,4,104
275 TYPE,5 $REAL,13 * SPRING KZ NODE 32
276 EN,163,132,32
277 TYPE,10 $REAL,20 * SPRING AXIAL NODE 32
278 EN,161,132,32
279 TYPE,6 $REAL,7 * SPRING NODES 1,4 RX
280 EN,114,101,1
281 EN,124,104,4
282 EN,174,111,11
283 TYPE,7 $REAL,8 * SPRING NODE 7 RY
284 EN,135,107,7
285 TYPE,7 $REAL,18 * SPRING NODE 12 RY
286 EN,175,112,12
287 TYPE,7 $REAL,5 * SPRING NODE 8 RY
288 EN,145,108,8
289 TYPE,7 $REAL,6 * SPRING NODE 10 RY
290 EN,155,110,10
291 TYPE,8 $REAL,11 * SPRING NODES 1,4 RZ
292 EN,116,101,1
293 EN,126,104,4
294 EN,176,111,11
295 TYPE,8 $REAL,12 * SPRING NODE 7 RZ
296 EN,136,107,7
297 TYPE,8 $REAL,19 * SPRING NODE 12 RZ
298 EN,186,112,12
299 TYPE,8 $REAL,9 * SPRING NODE 8 RZ
300 EN,146,108,8

```

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11.3.2.2 ANSYS Input File "PR9CM" (Continued)

```

301 TYPE,8 $REAL,10 * SPRING NODE 10 RZ
302 EN,156,110,10
303 TYPE,9 $REAL,14 * MASS NODE 95 (28)
304 EN,201,95
305 TYPE,9 $REAL,15 * MASS NODE 96 (29)
306 EN,202,96
307 TYPE,9 $REAL,16 * MASS NODE 97 (30)
308 EN,203,97
309 C***
310 /COM
311 /COM MACRO TO SPECIFY ONE SPRING IN Y DIRECTION
312 /COM
313 *CREAT,SPR2
314 P$PRING,ARG1,TRAN,ARG2,ARG3
315 *END
316 C*** *USE,SPR2,1,(200*4),1 * SPRING IN X DIR.
317 C*** *USE,SPR2,4,(200*4),1 * SPRING IN X DIR.
318 C***
319 C*** COUPLED DOFS FOR MEMBER END RELEASES
320 CP,1,UX,102,2
321 CPLGEN,1,UY,ROT,ROTY * NODE2
322 CPSGEN,2,3,1,5 * NODE5
323 CPSGEN,2,7,1,5 * NODE9
324 CPLIST,ALL
325 C*** LOCAL COORDINATES SYSTEMS
326 LOCAL,12,0,0,29.36,0,20 $NROT,10,110,100
327 LOCAL,13,0,0,29.36,0,33 $NROT,8,108,100
328 CSYS,0
329 C*** BOUNDARY CONDITIONS
330 D,1,ROTY,,,4,3,UX,UY,UZ * SAL 6/26/91 SAL 7/18/91
331 D,11,ROTY,,,,,UY,UZ
332 D,7,ROTX,,,12,5,UX,UZ
333 D,8,UZ,,,,,ROTX
334 D,10,UZ,,,,,ROTX,UX
335 D,101,ALL,,,110,3
336 D,108,ALL,,,132,24
337 D,111,ALL,,,112,1
338 C*** ANALYSIS OPTIONS
339 ITER,-20,20
340 TREF,70
341 POSTR,,1,5
342 C*** PLOT GEOMETRY AND B.C.
343 /SHOW
344 /PBC,ALL,1
345 EPLO
346 /PMUM,NODE,1
347 EPLO
348 /PMUM,NODE,-1
349 /PMUM,ELEM,1
350 EPLO

```

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11.3.2.2 ANSYS Input File "PR9CM" (Continued)

```

351 /VIEW,,1,1,1
352 EPLO
353 C***
354 C*** DL + LL + SSE XZ-PLANE
355 ACEL,0.9,0,1.33
356 C*** CONCENTRATED LOADS
357 C*** 'DL' + 'SMBOT' + 'SMBDN' + 'HANDDN' + 'HANDOT'
358 C*** TEMPERATURE LOADS
359 *CREAT,MAC
360 TUNIF,ARG1
361 LWRITE
362 *END
363 *USE,MAC,70
364 FDELETE,ALL,ALL
365 C*** DL + LL + SSE YZ-PLANE
366 ACEL,0,-0.9,1.33
367 C*** 'DL' + 'SMBCW' + 'HANDDN' + 'HANDCW'
368 *USE,MAC,70
369 C***
370 C*** BOUNDARY CONDITIONS - 2
371 C***
372 *CREAT,MACD
373 TUNIF,ARG1
374 D,1,UX,-ARG2
375 D,4,UX,ARG2
376 LWRITE
377 *END
378 D,11,UY,(-1/32)
379 D,7,UX,(1/32),,12,5
380 D,10,UX,(1/32)
381 C***
382 DD1=(0.03/26)
383 *USE,MACD,80,DD1
384 RP25,,10,DD1
385 *USE,MACD,327,0.03
386 AFWRITE,,1
387 FINISH
388 /INPUT,27
389 FINISH
390 /POST1
391 NLINE,200
392 *CREAT,PLD
393 SET,ARG1
394 PRRFOR
395 PRDISP
396 PLDISP
397 PLDISP,1
398 *END
399 *USE,PLD,1
400 *USE,PLD,2

```

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11.3.2.2 ANSYS Input File "PR9CM" (Continued)

```
401 *USE,PLD,28
402 FINISH
403 C***
404 /POST26
405 LINE,200
406 NUMVAR,50
407 TVAR,1
408 ESTR,2,1,53,TEMP
409 C***
410 *CREATE,STCN
411 C*** STORE STRAINS FOR CHANNEL (J NODE)
412 ESTR,13,ARG1,215,EE01
413 ESTR,14,ARG1,216,EP01
414 ESTR,15,ARG1,227,EE03
415 ESTR,16,ARG1,228,EP03
416 ESTR,17,ARG1,251,EE07
417 ESTR,18,ARG1,252,EP07
418 ESTR,19,ARG1,263,EE09
419 ESTR,20,ARG1,264,EP09
420 PRVAR,2,13,14,15,16
421 PRVAR,2,17,18,19,20
422 *END
423 C***
424 *CREATE,STC1
425 C*** STORE STRAINS FOR CHANNEL (I NODE)
426 ESTR,43,ARG1,95,EE01
427 ESTR,44,ARG1,96,EP01
428 ESTR,45,ARG1,107,EE03
429 ESTR,46,ARG1,108,EP03
430 ESTR,47,ARG1,131,EE07
431 ESTR,48,ARG1,132,EP07
432 ESTR,49,ARG1,143,EE09
433 ESTR,50,ARG1,144,EP09
434 PRVAR,2,43,44,45,46
435 PRVAR,2,47,48,49,40
436 *END
437 C***
438 *CREAT,DSP
439 DISP,21,ARG1,UX,UX
440 DISP,22,ARG1,UY,UY
441 DISP,23,ARG1,UZ,UZ
442 PROD,24,21,21,,UXUX
443 PROD,25,22,22,,UYUY
444 PROD,26,23,23,,UZUZ
445 ADD,27,24,25,26,SS
446 SORT,28,27,,SRSS
447 PRVAR,2,21,22,23,28
448 *END
449 C***
450 *USE,STC1,1
```

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11.3.2.2 ANSYS Input File "PR9CM" (Continued)

```

451 *USE,STC1,4
452 C***
453 *USE,STCN,45
454 *USE,STCN,46
455 *USE,STCN,19
456 *USE,STCN,51
457 *USE,STCN,52
458 *USE,DSP,51
459 *USE,DSP,52
460 C***
461 *USE,STCN,28
462 *USE,DSP,2
463 *USE,STCN,4
464 *USE,DSP,5
465 *USE,DSP,1
466 *USE,DSP,4
467 C***
468 *CREAT,RFOR
469 RFORC,31,ARG1,FX,RFX
470 RFORC,32,ARG1,FY,RFY
471 RFORC,33,ARG1,FZ,RFZ
472 RFORC,34,ARG1,MX,RMX
473 RFORC,35,ARG1,MY,RMY
474 RFORC,36,ARG1,MZ,RMZ
475 PRVAR,31,32,33,34,35,36
476 *END
477 *USE,RFOR,8
478 *USE,RFOR,108
479 *USE,RFOR,10
480 *USE,RFOR,110
481 *USE,RFOR,1
482 *USE,RFOR,101
483 *USE,RFOR,4
484 *USE,RFOR,104
485 FINISH
    
```

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11.3.3 Analysis Results

11.3.3.1 Analysis Results for "PR9C" (Analysis for the Original Structure)

The ANSYS output for the analysis "PR9C" is stored in file "PR9CR". The analysis results of the final load step (Load Step 16) are provided in this Section.

The deformed structure is plotted in Figure 11.3.3.1.1. Note that the displacements in this plot are magnified. This member is subjected to weak axis bending mainly due to thermal effect.

The reaction forces and the nodal displacements are provided in Sections 11.3.3.1.1 and 11.3.3.1.2. The maximum displacement for the Axially restrained member occurs at Node 51.

For the worst case 1-27 (Axial Restraint), the stresses and strains for elements 46, 19, 51 and 52 are listed in Section 11.3.3.1.3. These elements are in the neighborhood of node 51 where maximum displacements occurs. For the worst case 2&3-9(C) (Proximity Restraint), the stresses and strains for elements 7, 8, 28 and 4 are also presented in Section 11.3.3.1.3. The following explanations are provided for the labels shown in the stress and strain output.

PT: Segment Point Number, see Figure 11.2.2 for the definition of segment point for cross-section of Channels.

TEMP: Temperature

SIGX: Axial Stress

EP: Axial Elastic Strain

EPPL: Axial Plastic Strain

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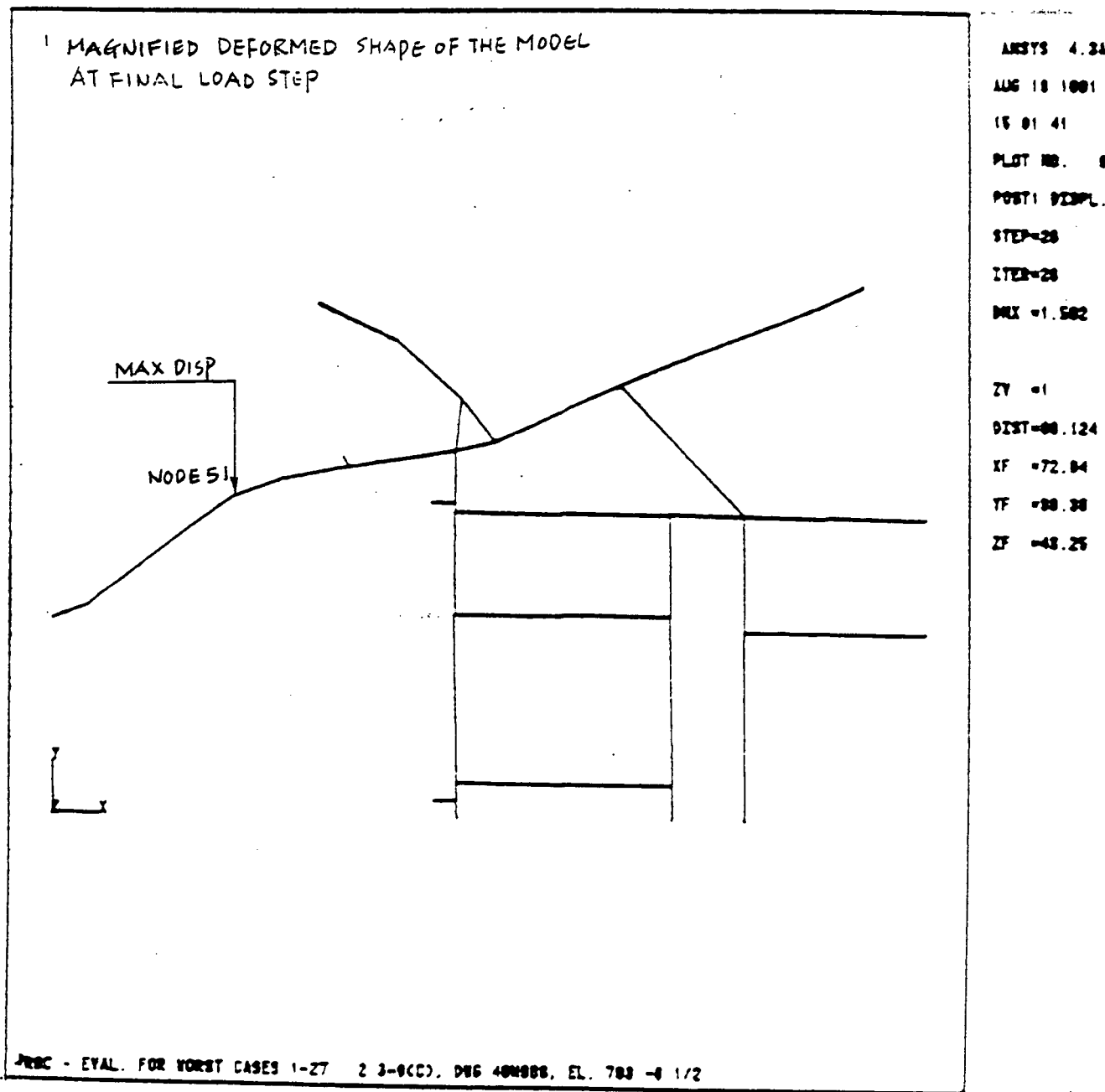
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Figure 11.3.3.1.1 Deformed Shape of the Model for Analysis PR9C



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For worst case 1-27 (Axial Restraint), the strain history at nodes 46, 21, 51, 52, and 22 are presented in Sections 11.3.3.1.4 through 11.3.3.1.8. These nodes are in the neighborhood of node 51 where maximum displacement occurs. The history of both elastic strain and plastic strain at segment points of 1, 3, 7, and 9 (see Figure 11.2.2 for segment point definition) are provided. Labels in the strain history print-out are defined as follows:

- ITER: Cumulative Iteration Number
- TEMP: Temperature
- ARG1EE01: Elastic Strain at Segment Point 1
- ARG1EP01: Plastic Strain at Segment Point 1
- ARG1EE03: Elastic Strain at Segment Point 3
- ARG1EP03: Plastic Strain at Segment Point 3
- ARG1EE07: Elastic Strain at Segment Point 7
- ARG1EP07: Plastic Strain at Segment Point 7
- ARG1EE09: Elastic Strain at Segment Point 9
- ARG1EP09: Plastic Strain at Segment Point 9

The yield temperature is determined where the plastic strain first appear. For example, the yield temperature for node 51 is 260°F (Section 11.3.3.1.6).

To identify the yield displacement of node 51, the displacement history of node 51 is presented in Section 11.3.3.1.9. The labels in the print-out are explained as follows:

- TEMP: Temperature
- UX: Displacement in nodal x direction
- UY: Displacement in nodal y direction
- UZ: Displacement in nodal z direction
- SRSS: Square Root of $(UX^2 + UY^2 + UZ^2)$

The yield displacement and the final displacement are used to determine the Ductility

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Ratio (final displacement/yield displacement).

For worst case 2&3-9(C) (Proximity Restraint), the stress and strain history at nodes 2 and 5 are presented in Sections 11.3.3.1.10 and 11.3.3.1.11 respectively. The displacement history at nodes 2 and 5 are provided in Section 11.3.3.1.12 and 11.3.3.1.13 respectively. No yield occurs in the Proximity member and the Supporting members.

The member of the worst case 1-27 is not accepted per DG-C1.6.12 R1 (For details, see Section 11.3.4.1, Member Qualification for worst case 1-27). A slotted hole is installed to release the thermal-induced axial force. The modified structure is re-evaluated with the analysis "PR9CM" and the member is qualified using the results from the new analysis. The results of the analysis "PR9CM" are presented in Section 11.3.3.2.

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11.3.3.1.1 Reaction Forces

PR9C - EVAL. FOR WORST CASES 1-27 & 283-9(C), DWG 48N968, EL. 783'-8 1/2

***** POST1 REACTION FORCE LISTING *****

LOAD STEP 28 ITERATION= 20 SECTION= 1
TIME= 0.0000 LOAD CASE= 1

THE FOLLOWING X,Y,Z FORCES ARE IN NODAL COORDINATES

NODE	FX	FY	FZ	MX	MY	MZ
1	1.6503752	-4.3482587	.25531173		-.24550348	
4	-.47005047	1.7394106	.22051535		-.26600746	
7	-6.1124883		.41409364	.15686290E-01		
8	50.672930		.40827152	-.79631414		
10	-46.757535		.42557583	-.60811805		
11		-.39898433E-01	.67246683E-01		.54323064E-02	
12	.47668958		.11842651	.57760785E-02		
101	0.0000000	0.0000000	0.0000000	4.1469186	0.0000000	-13.513746
104	0.0000000	0.0000000	0.0000000	3.7209014	0.0000000	-6.1115772
107	0.0000000	-.24104888	0.0000000	0.0000000	7.3102569	4.7319111
108	0.0000000	-11.616362	0.0000000	0.0000000	1.3848897	-14.339904
110	0.0000000	-.75347139	0.0000000	0.0000000	4.9289769	.35916084
111	.19486930	0.0000000	0.0000000	2.2017371	0.0000000	-3.0674356
112	0.0000000	-.29075517	0.0000000	0.0000000	1.8059134	3.3153068
132	-.88400565	.49995323	.11567178E-01	0.0000000	0.0000000	0.0000000
TOTAL	-1.2292163	-15.050431	1.9210084	8.6865872	14.923958	-28.626285

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11.3.3.1.2 Displacements

PR9C - EVAL. FOR WORST CASES 1-27 & 2&3-9(C), DWG 48N968, EL. 783'-8 1/2

***** POST1 NODAL DISPLACEMENT LISTING *****

LOAD STEP 28 ITERATION= 20 SECTION= 1
TIME= 0.0000 LOAD CASE= 1

THE FOLLOWING X,Y,Z DISPLACEMENTS ARE IN NODAL COORDINATES

NODE	UX	UY	UZ	ROTX	ROTY	ROTZ
1	-.30000000E-01	0.0000000	0.0000000	-.15025067E-03	0.0000000	.51997947E-03
2	-.36791175E-01	.90881838E-02	-.81889725E-03	-.17883448E-03	.98458075E-05	.19818545E-02
3	-.10206399	.81683525E-01	-.94132952E-02	-.22907114E-03	.42012134E-04	-.17224160E-02
4	.30000000E-01	0.0000000	0.0000000	-.13481527E-03	0.0000000	.23516015E-03
5	.26147318E-01	.87017696E-02	-.76721987E-03	-.16230317E-03	-.15121071E-04	.13637008E-02
6	-.39888336E-01	.78064588E-01	-.88770415E-02	-.20290884E-03	-.12850342E-03	-.11701424E-02
7	.31250000E-01	.60262219E-04	0.0000000	0.0000000	-.12938966E-03	-.14281997E-03
8	0.0000000	.29040906E-02	0.0000000	0.0000000	-.16998769E-03	.18105940E-02
9	-.70247157E-01	.97986193E-01	-.11010682E-01	-.25038302E-03	.47201120E-03	-.21184229E-01
10	.31250000E-01	.18836785E-03	0.0000000	0.0000000	-.28466514E-03	-.16902482E-04
11	-.48717325E-04	-.31250000E-01	0.0000000	-.79773082E-04	0.0000000	.11802823E-03
12	.31250000E-01	.72688793E-04	0.0000000	0.0000000	-.91249227E-04	-.25581071E-03
13	-.98494325E-01	.54452760E-01	-.60954534E-02	-.22346108E-03	.55706427E-05	.13535926E-02
14	-.34919834E-01	.52124286E-01	-.56870567E-02	-.20782931E-03	-.31502955E-04	.13155114E-02
15	-.54093220E-02	.10781880E-01	-.21081292E-02	-.10997894E-02	.97186143E-07	-.10009503E-04
16	-.66795380E-01	.54129904E-01	-.61014739E-02	-.21603316E-03	-.10443620E-04	-.76413137E-03
17	-.71062680E-01	.77865869E-01	-.99485094E-02	-.20980050E-03	-.32577379E-06	.57327074E-03
18	-.19267263E-01	.48046923E-01	-.66971289E-02	-.22312842E-03	-.23362377E-03	-.19982728E-02
19	.34552322E-02	.20240655E-01	-.33470537E-02	-.13048100E-03	-.24737729E-03	-.19363917E-02
20	-.85014762E-02	.25468829E-01	.57671552E-02	.37743070E-02	.14085812E-02	.17746435E-01
21	-.34491597	.88756364	.23782253E-01	.10974844E-01	.38818479E-02	.42448581E-01
22	-.33923870	.84485813	.60033190E-01	.42279298E-01	.15115796E-01	-.37626270E-01
23	-.22269529E-01	-.33081180E-01	-.14540636E-01	-.30965223E-03	.39826506E-03	-.18665033E-01
24	-.43222354E-01	.34871726E-01	-.19241492E-01	-.12609761E-02	-.42692061E-03	.24784091E-02
25	-.50079112E-01	.85143313E-01	-.17318791E-01	-.28970163E-03	-.29614651E-03	.70033702E-02
26	-.75679907E-02	.48030027E-01	-.11327255E-01	-.20672669E-02	-.12657023E-02	-.44642064E-02
27	.20952777E-01	.10607259E-01	-.46595104E-02	-.16519816E-02	-.11166408E-02	-.18171608E-02
28	-.32480290E-01	.45456279E-02	-.64141351E-03	-.16671220E-03	-.19332989E-03	.14075981E-02
29	-.98810885E-01	.84389977E-01	-.10045051E-01	-.23216657E-03	-.18830098E-03	-.25782027E-02
30	-.31023912	.76533132	.64322340E-01	.47025625E-01	.16567796E-01	-.39133534E-01
31	-.21818803E-01	.18038373E-01	-.34478567E-02	-.16247352E-03	-.12261337E-03	.48003957E-03
32	.20554384	.35033846	-.72745920E-05	-.31638147E-03	.48237515E-03	-.19571970E-01
33	.85924491E-01	.11155758	-.33146764E-02	-.31443469E-03	.48554239E-03	-.19773517E-01
34	-.19981729E-01	-.31337005E-01	-.45097514E-02	-.30973169E-03	.39863366E-03	-.18665762E-01
35	-.67543102E-01	.99384888E-01	-.97993916E-03	-.25044038E-03	.47165636E-03	-.21183423E-01
41	-.10925319E-02	.10914077E-01	.29666006E-02	.19836548E-02	.51559354E-03	.39255430E-02
45	-.12743297	.30722779	.11704922E-01	.63986722E-02	.21131703E-02	.47184794E-01
46	-.23893782	.60427819	.17636339E-01	.86036205E-02	.29704199E-02	.46948039E-01
51	-.46147977	1.1903594	.33884887E-01	.16320544E-01	.60278373E-02	.14252657E-01
52	-.43909492	1.1181509	.45089254E-01	.26938664E-01	.10050178E-01	-.21729873E-01

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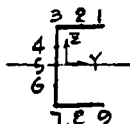
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11.3.3.1.3 Stresses and Strains



EL= 46		NODES= 46		21 MAT= 1		PRESSURES(Z,Y)= 0.0000		0.0000		AVE. TEMP= 327.00	
END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW				
1		327.00	-6.0193	-.0002197	0.0000000	0.0000000	0.0000000				
2		327.00	-15.122	-.0005519	0.0000000	0.0000000	0.0000000				
3		327.00	-24.225	-.0008841	0.0000000	0.0000000	0.0000000				
4		327.00	-23.558	-.0008598	0.0000000	0.0000000	0.0000000				
5		327.00	-22.890	-.0008354	0.0000000	0.0000000	0.0000000				
6		327.00	-22.222	-.0008110	0.0000000	0.0000000	0.0000000				
7		327.00	-21.554	-.0007866	0.0000000	0.0000000	0.0000000				
8		327.00	-12.451	-.0004544	0.0000000	0.0000000	0.0000000				
9		327.00	-3.3478	-.0001222	0.0000000	0.0000000	0.0000000				
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW				
1		327.00	16.826	.0006141	0.0000000	0.0000000	0.0000000				
2		327.00	-10.654	-.0003888	0.0000000	0.0000000	0.0000000				
3		327.00	-30.600	-.0011168	-.0002750	0.0000000	0.0000000				
4		327.00	-30.542	-.0011147	-.0002227	0.0000000	0.0000000				
5		327.00	-30.485	-.0011126	-.0001704	0.0000000	0.0000000				
6		327.00	-30.428	-.0011105	-.0001181	0.0000000	0.0000000				
7		327.00	-30.364	-.0011082	-.0000661	0.0000000	0.0000000				
8		327.00	-4.6943	-.0001713	0.0000000	0.0000000	0.0000000				
9		327.00	22.786	.0008316	0.0000000	0.0000000	0.0000000				
FORCES ON MEMBER AT NODE		46	51.9207	-2.72496	.391963	-.336464	8.49554	10.8017			
		21	-51.9226	2.72060	-.384732	.345567	-11.0930	-29.0130			

EL= 19		NODES= 21		51 MAT= 1		PRESSURES(Z,Y)= 0.0000		0.0000		AVE. TEMP= 327.00	
END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW				
1		327.00	16.800	.0006131	0.0000000	0.0000000	0.0000000				
2		327.00	-10.666	-.0003893	0.0000000	0.0000000	0.0000000				
3		327.00	-30.607	-.0011170	-.0002746	0.0000000	0.0000000				
4		327.00	-30.555	-.0011151	-.0002221	0.0000000	0.0000000				
5		327.00	-30.502	-.0011132	-.0001696	0.0000000	0.0000000				
6		327.00	-30.450	-.0011113	-.0001171	0.0000000	0.0000000				
7		327.00	-30.392	-.0011092	-.0000648	0.0000000	0.0000000				
8		327.00	-4.7014	-.0001716	0.0000000	0.0000000	0.0000000				
9		327.00	22.764	.0008308	0.0000000	0.0000000	0.0000000				
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW				
1		327.00	36.582	.0013351	.0038598	0.0000000	0.0000000				
2		327.00	-7.0483	-.0002572	0.0000000	0.0000000	0.0000000				
3		327.00	-36.659	-.0013379	-.0043715	0.0000000	0.0000000				
4		327.00	-36.643	-.0013373	-.0042653	0.0000000	0.0000000				
5		327.00	-36.627	-.0013368	-.0041591	0.0000000	0.0000000				
6		327.00	-36.611	-.0013362	-.0040529	0.0000000	0.0000000				
7		327.00	-36.595	-.0013356	-.0039468	0.0000000	0.0000000				
8		327.00	4.6530	.0001698	0.0000000	0.0000000	0.0000000				
9		327.00	36.646	.0013375	.0042845	0.0000000	0.0000000				
FORCES ON MEMBER AT NODE		21	51.9683	-1.63280	.367600	-.589604	11.0749	29.0127			
		51	-51.9706	1.62710	-.358224	.601407	-14.2218	-43.1462			

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Client TVA

Project WBNP Unit 1

Subject Qualif. of Worst Cases of Thermally Restrained Structures - Package I

11.3.3.1.3. Stresses and Strains (Cont'd)

EL= 51		NODES= 51		52		MAT= 1		PRESSURES(Z,Y)=		0.0000	0.0000	AVE. TEMP= 327.00
END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW					
1		327.00	36.555	.0013341	.0036820	0.0000000	0.0000000					
2		327.00	-7.3236	-.0002673	0.0000000	0.0000000	0.0000000					
3		327.00	-36.636	-.0013371	-.0042136	0.0000000	0.0000000					
4		327.00	-36.619	-.0013365	-.0041051	0.0000000	0.0000000					
5		327.00	-36.603	-.0013359	-.0039965	0.0000000	0.0000000					
6		327.00	-36.586	-.0013353	-.0038880	0.0000000	0.0000000					
7		327.00	-36.570	-.0013347	-.0037795	0.0000000	0.0000000					
8		327.00	4.6376	.0001693	0.0000000	0.0000000	0.0000000					
9		327.00	36.621	.0013365	.0041161	0.0000000	0.0000000					
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW					
1		327.00	36.172	.0013202	.0011436	0.0000000	0.0000000					
2		327.00	-13.425	-.0004900	0.0000000	0.0000000	0.0000000					
3		327.00	-34.532	-.0012603	-.0021835	0.0000000	0.0000000					
4		327.00	-34.522	-.0012599	-.0020529	0.0000000	0.0000000					
5		327.00	-34.512	-.0012596	-.0019223	0.0000000	0.0000000					
6		327.00	-34.502	-.0012592	-.0017917	0.0000000	0.0000000					
7		327.00	-34.493	-.0012589	-.0016612	0.0000000	0.0000000					
8		327.00	.92414	.0000337	0.0000000	0.0000000	0.0000000					
9		327.00	36.251	.0013230	.0016645	0.0000000	0.0000000					
FORCES ON MEMBER AT NODE		51	51.9947	.407892	.333658	-1.16978	14.5376	43.0244				
		52	-51.9968	-.413593	-.324232	1.18164	-17.3900	-39.4627				

EL= 52		NODES= 52		22		MAT= 1		PRESSURES(Z,Y)=		0.0000	0.0000	AVE. TEMP= 327.00
END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW					
1		327.00	36.152	.0013194	.0010073	0.0000000	0.0000000					
2		327.00	-13.943	-.0005089	0.0000000	0.0000000	0.0000000					
3		327.00	-34.387	-.0012550	-.0020894	0.0000000	0.0000000					
4		327.00	-34.373	-.0012545	-.0019554	0.0000000	0.0000000					
5		327.00	-34.360	-.0012540	-.0018215	0.0000000	0.0000000					
6		327.00	-34.346	-.0012535	-.0016875	0.0000000	0.0000000					
7		327.00	-34.327	-.0012528	-.0015537	0.0000000	0.0000000					
8		327.00	.79340	.0000290	0.0000000	0.0000000	0.0000000					
9		327.00	36.233	.0013224	.0015421	0.0000000	0.0000000					
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW					
1		327.00	8.2020	.0002993	0.0000000	0.0000000	0.0000000					
2		327.00	-13.801	-.0005037	0.0000000	0.0000000	0.0000000					
3		327.00	-29.216	-.0010663	-.0002404	0.0000000	0.0000000					
4		327.00	-29.126	-.0010630	-.0001585	0.0000000	0.0000000					
5		327.00	-28.983	-.0010578	-.0000785	0.0000000	0.0000000					
6		327.00	-28.615	-.0010443	-.0000067	0.0000000	0.0000000					
7		327.00	-26.464	-.0009658	0.0000000	0.0000000	0.0000000					
8		327.00	-4.4610	-.0001628	0.0000000	0.0000000	0.0000000					
9		327.00	17.542	.0006402	0.0000000	0.0000000	0.0000000					
FORCES ON MEMBER AT NODE		52	51.9697	1.73674	.267096	-1.66902	17.8839	39.2223				
		22	-51.9717	-1.74236	-.257590	1.68099	-20.1587	-24.1384				

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 Client TVA
 Project WBNP Unit 1
 Subject Qualif. of Worst Cases of Thermally Restrained Structures - Package I

11.3.3.1.3 Stresses and Strains (Cont'd)

EL= 7 NODES= 102 15 MAT= 4 PRESSURES(Z,Y)= 0.0000 0.0000 AVE. TEMP= 327.00

END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW
1		327.00	-.36908	-.0000135	0.0000000	0.0000000	0.0000000
2		327.00	-.36896	-.0000135	0.0000000	0.0000000	0.0000000
3		327.00	-.36885	-.0000135	0.0000000	0.0000000	0.0000000
4		327.00	-.38607	-.0000141	0.0000000	0.0000000	0.0000000
5		327.00	-.40329	-.0000147	0.0000000	0.0000000	0.0000000
6		327.00	-.42051	-.0000153	0.0000000	0.0000000	0.0000000
7		327.00	-.43774	-.0000160	0.0000000	0.0000000	0.0000000
8		327.00	-.43786	-.0000160	0.0000000	0.0000000	0.0000000
9		327.00	-.43797	-.0000160	0.0000000	0.0000000	0.0000000
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW
1		327.00	.65894E-01	.0000024	0.0000000	0.0000000	0.0000000
2		327.00	-.31412	-.0000115	0.0000000	0.0000000	0.0000000
3		327.00	-.69413	-.0000253	0.0000000	0.0000000	0.0000000
4		327.00	-.64574	-.0000236	0.0000000	0.0000000	0.0000000
5		327.00	-.59736	-.0000218	0.0000000	0.0000000	0.0000000
6		327.00	-.54897	-.0000200	0.0000000	0.0000000	0.0000000
7		327.00	-.50059	-.0000183	0.0000000	0.0000000	0.0000000
8		327.00	-.12058	-.0000044	0.0000000	0.0000000	0.0000000
9		327.00	.25943	.0000095	0.0000000	0.0000000	0.0000000

FORCES ON MEMBER AT NODE 102 1.14800 -.500041E-01 .832016E-01 .923639E-01 -.219073 -.139869E-03
 15 -1.14801 .253117E-03 -.978182E-02 .579293E-04 -.615453 -.450919

EL= 8 NODES= 15 105 MAT= 4 PRESSURES(Z,Y)= 0.0000 0.0000 AVE. TEMP= 327.00

END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW
1		327.00	.65888E-01	.0000024	0.0000000	0.0000000	0.0000000
2		327.00	-.31412	-.0000115	0.0000000	0.0000000	0.0000000
3		327.00	-.69412	-.0000253	0.0000000	0.0000000	0.0000000
4		327.00	-.64574	-.0000236	0.0000000	0.0000000	0.0000000
5		327.00	-.59736	-.0000218	0.0000000	0.0000000	0.0000000
6		327.00	-.54897	-.0000200	0.0000000	0.0000000	0.0000000
7		327.00	-.50059	-.0000183	0.0000000	0.0000000	0.0000000
8		327.00	-.12058	-.0000044	0.0000000	0.0000000	0.0000000
9		327.00	.25943	.0000095	0.0000000	0.0000000	0.0000000
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW
1		327.00	-.42276	-.0000154	0.0000000	0.0000000	0.0000000
2		327.00	-.42283	-.0000154	0.0000000	0.0000000	0.0000000
3		327.00	-.42289	-.0000154	0.0000000	0.0000000	0.0000000
4		327.00	-.41314	-.0000151	0.0000000	0.0000000	0.0000000
5		327.00	-.40339	-.0000147	0.0000000	0.0000000	0.0000000
6		327.00	-.39363	-.0000144	0.0000000	0.0000000	0.0000000
7		327.00	-.38388	-.0000140	0.0000000	0.0000000	0.0000000
8		327.00	-.38381	-.0000140	0.0000000	0.0000000	0.0000000
9		327.00	-.38374	-.0000140	0.0000000	0.0000000	0.0000000

FORCES ON MEMBER AT NODE 15 1.14801 -.367526E-04 .969097E-02 -.252560E-03 .615454 .450915
 105 -1.14800 -.499908E-01 .641383E-01 .931898E-01 -.124068 -.794080E-04

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Client TVA

Project WBNP Unit. 1

Subject Qualif. of Worst Cases of Thermally Restrained Structures - Package I

11.3.3.1.3 Stresses and Strains (Cont'd)

EL= 28 NODES= 28 2 MAT= 6 PRESSURES(Z,Y)= 0.0000 0.0000 AVE. TEMP= 327.00

END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW
1		327.00	-9.8922	-.0003610	0.0000000	0.0000000	0.0000000
2		327.00	-1.9144	-.0000699	0.0000000	0.0000000	0.0000000
3		327.00	6.0633	.0002213	0.0000000	0.0000000	0.0000000
4		327.00	5.8286	.0002127	0.0000000	0.0000000	0.0000000
5		327.00	5.5938	.0002042	0.0000000	0.0000000	0.0000000
6		327.00	5.3591	.0001956	0.0000000	0.0000000	0.0000000
7		327.00	5.1244	.0001870	0.0000000	0.0000000	0.0000000
8		327.00	-2.8533	-.0001041	0.0000000	0.0000000	0.0000000
9		327.00	-10.831	-.0003953	0.0000000	0.0000000	0.0000000
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW
1		327.00	-4.8238	-.0001760	0.0000000	0.0000000	0.0000000
2		327.00	-1.30763	-.0000112	0.0000000	0.0000000	0.0000000
3		327.00	4.2085	.0001536	0.0000000	0.0000000	0.0000000
4		327.00	4.0169	.0001466	0.0000000	0.0000000	0.0000000
5		327.00	3.8252	.0001396	0.0000000	0.0000000	0.0000000
6		327.00	3.6336	.0001326	0.0000000	0.0000000	0.0000000
7		327.00	3.4420	.0001256	0.0000000	0.0000000	0.0000000
8		327.00	-1.0742	-.0000392	0.0000000	0.0000000	0.0000000
9		327.00	-5.5903	-.0002040	0.0000000	0.0000000	0.0000000

FORCES ON MEMBER AT NODE 28 -4.32915 -1.64300 .222196 -.694147E-01 -2.98575 -9.46635
 2 4.32520 1.64301 -.216357 .767642E-01 2.43756 5.35883

EL= 4 NODES= 4 5 MAT= 3 PRESSURES(Z,Y)= 0.0000 0.0000 AVE. TEMP= 327.00

END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW
1		327.00	-7.6779	-.0002809	0.0000000	0.0000000	0.0000000
2		327.00	-2.5474	-.0000930	0.0000000	0.0000000	0.0000000
3		327.00	2.6032	.0000950	0.0000000	0.0000000	0.0000000
4		327.00	2.3107	.0000843	0.0000000	0.0000000	0.0000000
5		327.00	2.0182	.0000737	0.0000000	0.0000000	0.0000000
6		327.00	1.7257	.0000630	0.0000000	0.0000000	0.0000000
7		327.00	1.4332	.0000523	0.0000000	0.0000000	0.0000000
8		327.00	-3.7174	-.0001357	0.0000000	0.0000000	0.0000000
9		327.00	-8.8679	-.0003236	0.0000000	0.0000000	0.0000000
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW
1		327.00	-10.811	-.0003946	0.0000000	0.0000000	0.0000000
2		327.00	-3.6858	-.0001345	0.0000000	0.0000000	0.0000000
3		327.00	3.4396	.0001255	0.0000000	0.0000000	0.0000000
4		327.00	3.2317	.0001179	0.0000000	0.0000000	0.0000000
5		327.00	3.0237	.0001104	0.0000000	0.0000000	0.0000000
6		327.00	2.8158	.0001028	0.0000000	0.0000000	0.0000000
7		327.00	2.6078	.0000952	0.0000000	0.0000000	0.0000000
8		327.00	-4.5177	-.0001649	0.0000000	0.0000000	0.0000000
9		327.00	-11.643	-.0004249	0.0000000	0.0000000	0.0000000

FORCES ON MEMBER AT NODE 4 1.73975 .468684 .220781 .991794E-02 -3.72065 -6.11164
 5 -1.74746 -.468678 -.209388 .442356E-02 2.64522 8.45505

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Client TVA

Project WBNP Unit 1

Subject Qualif. of Worst Cases of Thermally Restrained Structures - Package I

11.3.3.1.4 Strain History at Node 46 (Element 45)
(Segment Points 1 and 3)

PR9C - EVAL. FOR WORST CASES 1-27 & 2&3-9(C), DWG 48N968, EL. 783'-8 1/2

ANSYS POST26 VARIABLE LISTING

ITER	ESTR 1 53	ESTR 45 215	ESTR 45 216	ESTR 45 227	ESTR 45 228
	1 TEMP	ARG1EE01	ARG1EP01	ARG1EE03	ARG1EP03
4.0000	70.0000	.318352E-06	0.00000	-.247614E-05	0.00000
8.0000	70.0000	.239213E-05	0.00000	-.510512E-05	0.00000
12.000	80.0000	.152621E-03	0.00000	.131580E-03	0.00000
15.000	90.0000	.801221E-04	0.00000	.680262E-04	0.00000
18.000	100.000	.743339E-05	0.00000	.447528E-05	0.00000
21.000	110.000	-.654561E-04	0.00000	-.590939E-04	0.00000
24.000	120.000	-.138539E-03	0.00000	-.122685E-03	0.00000
27.000	130.000	-.211804E-03	0.00000	-.186303E-03	0.00000
30.000	140.000	-.285239E-03	0.00000	-.249951E-03	0.00000
33.000	150.000	-.358830E-03	0.00000	-.313636E-03	0.00000
36.000	160.000	-.432557E-03	0.00000	-.377363E-03	0.00000
42.000	170.000	-.506286E-03	0.00000	-.441139E-03	0.00000
49.000	180.000	-.580224E-03	0.00000	-.504895E-03	0.00000
60.000	190.000	-.653596E-03	0.00000	-.567997E-03	0.00000
77.000	200.000	-.726399E-03	0.00000	-.630536E-03	0.00000
96.000	210.000	-.796473E-03	0.00000	-.693752E-03	0.00000
116.00	220.000	-.846424E-03	0.00000	-.761994E-03	0.00000
136.00	230.000	-.894852E-03	0.00000	-.830435E-03	0.00000
156.00	240.000	-.871349E-03	0.00000	-.913477E-03	0.00000
176.00	250.000	-.790157E-03	0.00000	-.100777E-02	0.00000
196.00	260.000	-.694468E-03	0.00000	-.109806E-02	0.00000
216.00	270.000	-.606102E-03	0.00000	-.112815E-02	0.00000
236.00	280.000	-.500020E-03	0.00000	-.112003E-02	0.00000
256.00	290.000	-.394110E-03	0.00000	-.109988E-02	0.00000
276.00	300.000	-.325985E-03	0.00000	-.103733E-02	0.00000
296.00	310.000	-.276695E-03	0.00000	-.959896E-03	0.00000
316.00	320.000	-.240156E-03	0.00000	-.909973E-03	0.00000
336.00	327.000	-.218509E-03	0.00000	-.884242E-03	0.00000

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Client TVA

Project WBNP Unit 1

Subject Qualif. of Worst Cases of Thermally Restrained Structures - Package I

11.3.3.1.4 Strain History at Node 46 (Element 45) (Cont'd)
(Segment Points 7 and 9)

ITER	ESTR 1 53	ESTR 45 251	ESTR 45 252	ESTR 45 263	ESTR 45 264
	1 TEMP	ARG1EE07	ARG1EP07	ARG1EE09	ARG1EP09
4.0000	70.0000	-.794660E-05	0.00000	-.515211E-05	0.00000
8.0000	70.0000	.310424E-06	0.00000	.780767E-05	0.00000
12.000	80.0000	.140412E-03	0.00000	.161453E-03	0.00000
15.000	90.0000	.768373E-04	0.00000	.889332E-04	0.00000
18.000	100.000	.133228E-04	0.00000	.162809E-04	0.00000
21.000	110.000	-.501095E-04	0.00000	-.564717E-04	0.00000
24.000	120.000	-.113458E-03	0.00000	-.129311E-03	0.00000
27.000	130.000	-.176720E-03	0.00000	-.202221E-03	0.00000
30.000	140.000	-.239894E-03	0.00000	-.275183E-03	0.00000
33.000	150.000	-.302979E-03	0.00000	-.348174E-03	0.00000
36.000	160.000	-.365974E-03	0.00000	-.421168E-03	0.00000
42.000	170.000	-.428764E-03	0.00000	-.493911E-03	0.00000
49.000	180.000	-.490894E-03	0.00000	-.566224E-03	0.00000
60.000	190.000	-.552678E-03	0.00000	-.638277E-03	0.00000
77.000	200.000	-.612190E-03	0.00000	-.708054E-03	0.00000
96.000	210.000	-.670488E-03	0.00000	-.773209E-03	0.00000
116.00	220.000	-.732969E-03	0.00000	-.817400E-03	0.00000
136.00	230.000	-.795557E-03	0.00000	-.859974E-03	0.00000
156.00	240.000	-.870914E-03	0.00000	-.828786E-03	0.00000
176.00	250.000	-.955820E-03	0.00000	-.738204E-03	0.00000
196.00	260.000	-.103923E-02	0.00000	-.635640E-03	0.00000
216.00	270.000	-.106121E-02	0.00000	-.539161E-03	0.00000
236.00	280.000	-.104106E-02	0.00000	-.421057E-03	0.00000
256.00	290.000	-.101148E-02	0.00000	-.305704E-03	0.00000
276.00	300.000	-.946434E-03	0.00000	-.235085E-03	0.00000
296.00	310.000	-.864829E-03	0.00000	-.181628E-03	0.00000
316.00	320.000	-.813160E-03	0.00000	-.143343E-03	0.00000
336.00	327.000	-.787023E-03	0.00000	-.121290E-03	0.00000

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Client TVA

Project WBNP Unit 1

Subject Qualif. of Worst Cases of Thermally Restrained Structures - Package I

11.3.3.1.5 Strain History at Node 21 (Element 46)
(Segment Points 1 and 3)

PR9C - EVAL. FOR WORST CASES 1-27 & 2&3-9(C), DWG 48N968, EL. 783'-8 1/2

ANSYS POST26 VARIABLE LISTING

ITER	ESTR 1 53	ESTR 46 215	ESTR 46 216	ESTR 46 227	ESTR 46 228
	1 TEMP	ARG1EE01	ARG1EP01	ARG1EE03	ARG1EP03
4.0000	70.0000	.866323E-05	0.00000	-.574006E-05	0.00000
8.0000	70.0000	.154236E-04	0.00000	-.150152E-04	0.00000
12.000	80.0000	.127651E-03	0.00000	.133402E-03	-0.00000
15.000	90.0000	.734070E-04	0.00000	.636471E-04	0.00000
18.000	100.000	.196337E-04	0.00000	-.634445E-05	0.00000
21.000	110.000	-.336849E-04	0.00000	-.765987E-04	0.00000
24.000	120.000	-.864965E-04	0.00000	-.147135E-03	0.00000
27.000	130.000	-.138743E-03	0.00000	-.217976E-03	0.00000
30.000	140.000	-.190358E-03	0.00000	-.289145E-03	0.00000
33.000	150.000	-.241269E-03	0.00000	-.360668E-03	0.00000
36.000	160.000	-.291392E-03	0.00000	-.432575E-03	0.00000
42.000	170.000	-.339864E-03	0.00000	-.505117E-03	0.00000
49.000	180.000	-.387556E-03	0.00000	-.578160E-03	0.00000
60.000	190.000	-.433152E-03	0.00000	-.650934E-03	0.00000
77.000	200.000	-.476712E-03	0.00000	-.724112E-03	0.00000
96.000	210.000	-.516387E-03	0.00000	-.798881E-03	0.00000
116.00	220.000	-.536114E-03	0.00000	-.878737E-03	0.00000
136.00	230.000	-.552184E-03	0.00000	-.959520E-03	0.00000
156.00	240.000	-.500214E-03	0.00000	-.105381E-02	0.00000
176.00	250.000	-.391657E-03	0.00000	-.115915E-02	0.00000
196.00	260.000	-.264509E-03	0.00000	-.126167E-02	0.00000
216.00	270.000	-.124279E-03	0.00000	-.129758E-02	-.228334E-04
236.00	280.000	.512043E-04	0.00000	-.128545E-02	-.126863E-03
256.00	290.000	.239480E-03	0.00000	-.127622E-02	-.215034E-03
276.00	300.000	.397551E-03	0.00000	-.123246E-02	-.274994E-03
296.00	310.000	.502075E-03	0.00000	-.117406E-02	-.274994E-03
316.00	320.000	.573023E-03	0.00000	-.113591E-02	-.274994E-03
336.00	327.000	.614091E-03	0.00000	-.111678E-02	-.274994E-03

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Subject Qualif. of Worst Cases of Thermally Restrained Structures - Package I

11.3.3.1.5 Strain History at Node 21 (Element 46) (Cont'd)
(Segment Points 7 and 9)

ITER	ESTR 1 53	ESTR 46 251	ESTR 46 252	ESTR 46 263	ESTR 46 264
	1 TEMP	ARG1EE07	ARG1EP07	ARG1EE09	ARG1EP09
4.0000	70.0000	-.104927E-04	0.00000	.391057E-05	0.00000
8.0000	70.0000	-.153523E-05	0.00000	.289036E-04	0.00000
12.000	80.0000	.152224E-03	0.00000	.146472E-03	0.00000
15.000	90.0000	.823654E-04	0.00000	.921252E-04	0.00000
18.000	100.000	.123468E-04	0.00000	.383249E-04	0.00000
21.000	110.000	-.578047E-04	0.00000	-.148909E-04	0.00000
24.000	120.000	-.128102E-03	0.00000	-.674629E-04	0.00000
27.000	130.000	-.198559E-03	0.00000	-.119325E-03	0.00000
30.000	140.000	-.269191E-03	0.00000	-.170404E-03	0.00000
33.000	150.000	-.340018E-03	0.00000	-.220618E-03	0.00000
36.000	160.000	-.411058E-03	0.00000	-.269875E-03	0.00000
42.000	170.000	-.482450E-03	0.00000	-.317197E-03	0.00000
49.000	180.000	-.553433E-03	0.00000	-.362830E-03	0.00000
60.000	190.000	-.624661E-03	0.00000	-.406880E-03	0.00000
77.000	200.000	-.693894E-03	0.00000	-.446493E-03	0.00000
96.000	210.000	-.762054E-03	0.00000	-.479560E-03	0.00000
116.00	220.000	-.834282E-03	0.00000	-.491658E-03	0.00000
136.00	230.000	-.907349E-03	0.00000	-.500013E-03	0.00000
156.00	240.000	-.991745E-03	0.00000	-.438151E-03	0.00000
176.00	250.000	-.108523E-02	0.00000	-.317735E-03	0.00000
196.00	260.000	-.117875E-02	0.00000	-.181592E-03	0.00000
216.00	270.000	-.121942E-02	0.00000	-.232854E-04	0.00000
236.00	280.000	-.124923E-02	0.00000	.214288E-03	0.00000
256.00	290.000	-.126589E-02	-.259908E-04	.438857E-03	0.00000
276.00	300.000	-.123021E-02	-.660775E-04	.608720E-03	0.00000
296.00	310.000	-.116749E-02	-.660775E-04	.717566E-03	0.00000
316.00	320.000	-.112763E-02	-.660775E-04	.790225E-03	0.00000
336.00	327.000	-.110818E-02	-.660775E-04	.831604E-03	0.00000

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Subject Qualif. of Worst Cases of Thermally Restrained Structures - Package I

11.3.3.1.6 Strain History at Node 51 (Element 19)
(Segment Points 1 and 3)

PR9C - EVAL. FOR WORST CASES 1-27 & 283-9(C), DWG 48N968, EL. 783'-8 1/2

ANSYS POST26 VARIABLE LISTING

ITER	ESTR 1 53	ESTR 19 215	ESTR 19 216	ESTR 19 227	ESTR 19 228
	1 TEMP	ARG1EE01	ARG1EP01	ARG1EE03	ARG1EP03
4.0000	70.0000	.178657E-04	0.00000	-.883887E-05	0.00000
8.0000	70.0000	.304412E-04	0.00000	-.266124E-04	0.00000
12.000	80.0000	.132517E-03	0.00000	.123628E-03	0.00000
15.000	90.0000	.838984E-04	0.00000	.520059E-04	0.00000
18.000	100.000	.363913E-04	0.00000	-.200930E-04	0.00000
21.000	110.000	-.100302E-04	0.00000	-.926993E-04	0.00000
24.000	120.000	-.552745E-04	0.00000	-.165848E-03	0.00000
27.000	130.000	-.992408E-04	0.00000	-.239576E-03	0.00000
30.000	140.000	-.141817E-03	0.00000	-.313924E-03	0.00000
33.000	150.000	-.182881E-03	0.00000	-.388937E-03	0.00000
36.000	160.000	-.222294E-03	0.00000	-.464665E-03	0.00000
42.000	170.000	-.258494E-03	0.00000	-.541593E-03	0.00000
49.000	180.000	-.292848E-03	0.00000	-.619622E-03	0.00000
60.000	190.000	-.323484E-03	0.00000	-.697765E-03	0.00000
77.000	200.000	-.350337E-03	0.00000	-.777503E-03	0.00000
96.000	210.000	-.372333E-03	0.00000	-.859814E-03	0.00000
116.00	220.000	-.376752E-03	0.00000	-.946527E-03	0.00000
136.00	230.000	-.375944E-03	0.00000	-.103470E-02	0.00000
156.00	240.000	-.318364E-03	0.00000	-.113274E-02	0.00000
176.00	250.000	-.212070E-03	0.00000	-.123930E-02	0.00000
196.00	<u>260.000</u>	-.915893E-04	0.00000	-.131416E-02	<u>-.532933E-04</u>
216.00	270.000	.207514E-03	0.00000	-.131693E-02	-.555589E-03
236.00	280.000	.682939E-03	0.00000	-.132056E-02	-.121513E-02
256.00	290.000	.115386E-02	0.00000	-.132385E-02	-.181416E-02
276.00	300.000	.131915E-02	.958882E-03	-.132804E-02	-.257453E-02
296.00	310.000	.132639E-02	.227471E-02	-.133229E-02	-.334740E-02
316.00	320.000	.133175E-02	.324905E-02	-.133566E-02	-.395934E-02
336.00	327.000	.133512E-02	.385981E-02	-.133793E-02	-.437147E-02

First Plastic Strain

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11.3.3.1.6 Strain History at Node 51 (Element 19) (Cont'd)
(Segment Points 7 and 9)

ITER	ESTR 1 53	ESTR 19 251	ESTR 19 252	ESTR 19 263	ESTR 19 264
	1 TEMP	ARG1EE07	ARG1EP07	ARG1EE09	ARG1EP09
4.0000	70.0000	-.135284E-04	0.00000	.131762E-04	0.00000
8.0000	70.0000	-.359306E-05	0.00000	.534605E-04	0.00000
12.000	80.0000	.154472E-03	0.00000	.163361E-03	0.00000
15.000	90.0000	.826478E-04	0.00000	.114540E-03	0.00000
18.000	100.000	.104526E-04	0.00000	.669369E-04	0.00000
21.000	110.000	-.620782E-04	0.00000	.205910E-04	0.00000
24.000	120.000	-.134970E-03	0.00000	-.243970E-04	0.00000
27.000	130.000	-.208252E-03	0.00000	-.679170E-04	0.00000
30.000	140.000	-.281954E-03	0.00000	-.109848E-03	0.00000
33.000	150.000	-.356111E-03	0.00000	-.150055E-03	0.00000
36.000	160.000	-.430762E-03	0.00000	-.188391E-03	0.00000
42.000	170.000	-.506288E-03	0.00000	-.223189E-03	0.00000
49.000	180.000	-.581656E-03	0.00000	-.254882E-03	0.00000
60.000	190.000	-.657910E-03	0.00000	-.283629E-03	0.00000
77.000	200.000	-.732480E-03	0.00000	-.305313E-03	0.00000
96.000	210.000	-.805996E-03	0.00000	-.318514E-03	0.00000
116.00	220.000	-.882704E-03	0.00000	-.312930E-03	0.00000
136.00	230.000	-.960798E-03	0.00000	-.302047E-03	0.00000
156.00	240.000	-.104625E-02	0.00000	-.231877E-03	0.00000
176.00	250.000	-.113802E-02	0.00000	-.110781E-03	0.00000
196.00	260.000	-.123338E-02	0.00000	.424902E-04	0.00000
216.00	270.000	-.131573E-02	-.338381E-03	.425918E-03	0.00000
236.00	280.000	-.131922E-02	-.971454E-03	.927960E-03	0.00000
256.00	290.000	-.132217E-02	-.150722E-02	.131468E-02	.147818E-03
276.00	300.000	-.132579E-02	-.216562E-02	.132140E-02	.136778E-02
296.00	310.000	-.132998E-02	-.292693E-02	.132870E-02	.269518E-02
316.00	320.000	-.133333E-02	-.353482E-02	.133409E-02	.367356E-02
336.00	327.000	-.133559E-02	-.394675E-02	.133745E-02	.428453E-02

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Subject Qualif. of Worst Cases of Thermally Restrained Structures - Package I

11.3.3.1.7 Strain History at Node 52 (Element 51)
 (Segment Points 1 and 3)

PR9C - EVAL. FOR WORST CASES 1-27 & 2&3-9(C), DWG 48N968, EL. 783'-8 1/2

ANSYS POST26 VARIABLE LISTING

ITER	ESTR 1 53	ESTR 51 215	ESTR 51 216	ESTR 51 227	ESTR 51 228
	TEMP	ARG1EE01	ARG1EP01	ARG1EE03	ARG1EP03
4.0000	70.0000	.267728E-04	0.00000	-.112554E-04	0.00000
8.0000	70.0000	.425815E-04	0.00000	-.367403E-04	0.00000
12.000	80.0000	.134384E-03	0.00000	.115371E-03	0.00000
15.000	90.0000	.915551E-04	0.00000	.418191E-04	0.00000
18.000	100.000	.500470E-04	0.00000	-.323051E-04	0.00000
21.000	110.000	.981145E-05	0.00000	-.107034E-03	0.00000
24.000	120.000	-.290616E-04	0.00000	-.182402E-03	0.00000
27.000	130.000	-.664734E-04	0.00000	-.258448E-03	0.00000
30.000	140.000	-.102316E-03	0.00000	-.335213E-03	0.00000
33.000	150.000	-.136471E-03	0.00000	-.412741E-03	0.00000
36.000	160.000	-.168807E-03	0.00000	-.491081E-03	0.00000
42.000	170.000	-.197620E-03	0.00000	-.570767E-03	0.00000
49.000	180.000	-.224180E-03	0.00000	-.651928E-03	0.00000
60.000	190.000	-.246386E-03	0.00000	-.733258E-03	0.00000
77.000	200.000	-.264337E-03	0.00000	-.816933E-03	0.00000
96.000	210.000	-.278052E-03	0.00000	-.903602E-03	0.00000
116.00	220.000	-.279644E-03	0.00000	-.992931E-03	0.00000
136.00	230.000	-.276395E-03	0.00000	-.108357E-02	0.00000
156.00	240.000	-.235811E-03	0.00000	-.117765E-02	0.00000
176.00	250.000	-.162585E-03	0.00000	-.127492E-02	0.00000
196.00	260.000	-.855664E-04	0.00000	-.131454E-02	-.121523E-03
216.00	270.000	.163016E-03	0.00000	-.131707E-02	-.581510E-03
236.00	280.000	.535449E-03	0.00000	-.131969E-02	-.105745E-02
256.00	290.000	.911271E-03	0.00000	-.132128E-02	-.148000E-02
276.00	300.000	.131389E-02	.363571E-05	-.131317E-02	-.187076E-02
296.00	310.000	.131715E-02	.596767E-03	-.128070E-02	-.208435E-02
316.00	320.000	.131927E-02	.980976E-03	-.126688E-02	-.217793E-02
336.00	327.000	.132016E-02	.114362E-02	-.126028E-02	-.218345E-02

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11.3.3.1.7 Strain History at Node 52 (Element 51) (Cont'd)

(Segment Points 7 and 9)

ITER	ESTR 1 53	ESTR 51 251	ESTR 51 252	ESTR 51 263	ESTR 51 264
	1 TEMP	ARG1EE07	ARG1EP07	ARG1EE09	ARG1EP09
4.0000	70.0000	-.167413E-04	0.00000	.212869E-04	0.00000
8.0000	70.0000	-.488893E-05	0.00000	.744329E-04	0.00000
12.000	80.0000	-.157506E-03	0.00000	.176519E-03	0.00000
15.000	90.0000	-.836698E-04	0.00000	.133406E-03	0.00000
18.000	100.000	-.940314E-05	0.00000	.917553E-04	0.00000
21.000	110.000	-.652472E-04	0.00000	.515980E-04	0.00000
24.000	120.000	-.140306E-03	0.00000	.130347E-04	0.00000
27.000	130.000	-.215799E-03	0.00000	-.238247E-04	0.00000
30.000	140.000	-.291757E-03	0.00000	-.588604E-04	0.00000
33.000	150.000	-.368212E-03	0.00000	-.919412E-04	0.00000
36.000	160.000	-.445197E-03	0.00000	-.122924E-03	0.00000
42.000	170.000	-.523147E-03	0.00000	-.150000E-03	0.00000
49.000	180.000	-.600962E-03	0.00000	-.173215E-03	0.00000
60.000	190.000	-.679963E-03	0.00000	-.193090E-03	0.00000
77.000	200.000	-.757153E-03	0.00000	-.204557E-03	0.00000
96.000	210.000	-.832767E-03	0.00000	-.207217E-03	0.00000
116.00	220.000	-.909643E-03	0.00000	-.196356E-03	0.00000
136.00	230.000	-.987778E-03	0.00000	-.180599E-03	0.00000
156.00	240.000	-.106649E-02	0.00000	-.124651E-03	0.00000
176.00	250.000	-.114594E-02	0.00000	-.335981E-04	0.00000
196.00	260.000	-.123881E-02	0.00000	.111688E-03	0.00000
216.00	270.000	-.131557E-02	-.308690E-03	.437338E-03	0.00000
236.00	280.000	-.131802E-02	-.754734E-03	.839830E-03	0.00000
256.00	290.000	-.131959E-02	-.114907E-02	.124389E-02	0.00000
276.00	300.000	-.127984E-02	-.143396E-02	.131646E-02	.471194E-03
296.00	310.000	-.127573E-02	-.157322E-02	.131998E-02	.111004E-02
316.00	320.000	-.126529E-02	-.165584E-02	.132214E-02	.150179E-02
336.00	327.000	-.125886E-02	-.166117E-02	.132303E-02	.166446E-02

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11.3.3.1.8 Strain History at Node 22 (Element 52)

(Segment Points 1 and 3)

PR9C - EVAL. FOR WORST CASES 1-27 & 2&3-9(C), DWG 48N968, EL. 783'-8 1/2"

ANSYS POST26 VARIABLE LISTING

ITER	ESTR 1 53	ESTR 52 215	ESTR 52 216	ESTR 52 227	ESTR 52 228
	1 TEMP	ARG1EE01	ARG1EP01	ARG1EE03	ARG1EP03
4.0000	70.0000	.353583E-04	0.00000	-.129496E-04	0.00000
8.0000	70.0000	.522916E-04	0.00000	-.454511E-04	0.00000
12.0000	80.0000	.133984E-03	0.00000	.108460E-03	0.00000
15.0000	90.0000	.971078E-04	0.00000	.329252E-04	0.00000
18.0000	100.000	.611874E-04	0.00000	-.430830E-04	0.00000
21.0000	110.000	.261440E-04	0.00000	-.119596E-03	0.00000
24.0000	120.000	-.798079E-05	0.00000	-.196633E-03	0.00000
27.0000	130.000	-.411417E-04	0.00000	-.274215E-03	0.00000
30.0000	140.000	-.732897E-04	0.00000	-.352365E-03	0.00000
33.0000	150.000	-.104372E-03	0.00000	-.431105E-03	0.00000
36.0000	160.000	-.134330E-03	0.00000	-.510461E-03	0.00000
42.0000	170.000	-.161926E-03	0.00000	-.590807E-03	0.00000
49.0000	180.000	-.187694E-03	0.00000	-.672715E-03	0.00000
60.0000	190.000	-.209692E-03	0.00000	-.754435E-03	0.00000
77.0000	200.000	-.228455E-03	0.00000	-.838722E-03	0.00000
96.0000	210.000	-.245303E-03	0.00000	-.925817E-03	0.00000
116.00	220.000	-.257951E-03	0.00000	-.101295E-02	0.00000
136.00	230.000	-.268116E-03	0.00000	-.110056E-02	0.00000
156.00	240.000	-.265433E-03	0.00000	-.118335E-02	0.00000
176.00	250.000	-.251304E-03	0.00000	-.126214E-02	0.00000
196.00	260.000	-.235837E-03	0.00000	-.130587E-02	-.412738E-04
216.00	270.000	-.237168E-03	0.00000	-.127185E-02	-.165822E-03
236.00	280.000	-.147972E-03	0.00000	-.124962E-02	-.215093E-03
256.00	290.000	.313331E-05	0.00000	-.124502E-02	-.230078E-03
276.00	300.000	.115221E-03	0.00000	-.119310E-02	-.240426E-03
296.00	310.000	.198531E-03	0.00000	-.112775E-02	-.240426E-03
316.00	320.000	.263448E-03	0.00000	-.108744E-02	-.240426E-03
336.00	327.000	.299342E-03	0.00000	-.106628E-02	-.240426E-03

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11.3.3.1.8 Strain History at Node 22 (Element 52) (Cont'd)
(Segment Points 7 and 9)

ITER	ESTR 1 53	ESTR 52 251	ESTR 52 252	ESTR 52 263	ESTR 52 264
	1 TEMP	ARG1EE07	ARG1EP07	ARG1EE09	ARG1EP09
4.0000	70.0000	-.201437E-04	0.00000	.281641E-04	0.00000
8.0000	70.0000	-.563791E-05	0.00000	.921048E-04	0.00000
12.0000	80.0000	.161038E-03	0.00000	.186561E-03	0.00000
15.0000	90.0000	.851330E-04	0.00000	.149316E-03	0.00000
18.0000	100.0000	.893567E-05	0.00000	.113206E-03	0.00000
21.0000	110.0000	-.674924E-04	0.00000	.782473E-04	0.00000
24.0000	120.0000	-.144159E-03	0.00000	.444932E-04	0.00000
27.0000	130.0000	-.221071E-03	0.00000	.120022E-04	0.00000
30.0000	140.0000	-.298238E-03	0.00000	-.191631E-04	0.00000
33.0000	150.0000	-.375669E-03	0.00000	-.489352E-04	0.00000
36.0000	160.0000	-.453373E-03	0.00000	-.772411E-04	0.00000
42.0000	170.0000	-.531621E-03	0.00000	-.102739E-03	0.00000
49.0000	180.0000	-.609477E-03	0.00000	-.124456E-03	0.00000
60.0000	190.0000	-.688394E-03	0.00000	-.143651E-03	0.00000
77.0000	200.0000	-.764871E-03	0.00000	-.154605E-03	0.00000
96.0000	210.0000	-.838681E-03	0.00000	-.158167E-03	0.00000
116.00	220.0000	-.910991E-03	0.00000	-.155996E-03	0.00000
136.00	230.0000	-.983761E-03	0.00000	-.151321E-03	0.00000
156.00	240.0000	-.104865E-02	0.00000	-.130734E-03	0.00000
176.00	250.0000	-.110695E-02	0.00000	-.961134E-04	0.00000
196.00	260.0000	-.116237E-02	0.00000	-.510573E-04	0.00000
216.00	270.0000	-.117676E-02	0.00000	.237444E-04	0.00000
236.00	280.0000	-.116201E-02	0.00000	.154725E-03	0.00000
256.00	290.0000	-.114769E-02	0.00000	.330542E-03	0.00000
276.00	300.0000	-.109860E-02	0.00000	.450143E-03	0.00000
296.00	310.0000	-.102860E-02	0.00000	.538106E-03	0.00000
316.00	320.0000	-.986849E-03	0.00000	.604462E-03	0.00000
336.00	327.0000	-.965833E-03	0.00000	.640214E-03	0.00000

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Client TVA

Project WBNP Unit 1

Subject Qualif. of Worst Cases of Thermally Restrained Structures - Package I

11.3.3.1.9 Displacement History of Node 51

PR9C - EVAL. FOR WORST CASES 1-27 & 2&3-9(C), DWG 48N968, EL. 783'-8 1/2

ANSYS POST26 VARIABLE LISTING

ITER	ESTR 1 53	DISP 51 UX	DISP 51 UY	DISP 51 UZ	OPER 28 SORT
	1 TEMP	51 UX	51 UY	51 UZ	SRSS
4.0000	70.0000	.205782E-02	.513653E-02	.152621E-01	.162343E-01
8.0000	70.0000	-.419198E-02	.113306E-01	.468734E-01	.484053E-01
12.000	80.0000	.689674E-02	.288424E-02	.454990E-01	.461090E-01
15.000	90.0000	.433593E-02	.987720E-02	.458298E-01	.470822E-01
18.000	100.000	.167650E-02	.171305E-01	.461762E-01	.492799E-01
21.000	110.000	-.108071E-02	.246417E-01	.465407E-01	.526728E-01
24.000	120.000	-.394257E-02	.324291E-01	.469235E-01	.571752E-01
27.000	130.000	-.691665E-02	.405127E-01	.473249E-01	.626799E-01
30.000	140.000	-.100113E-01	.489147E-01	.477449E-01	.690829E-01
33.000	150.000	-.132355E-01	.576594E-01	.481837E-01	.762984E-01
36.000	160.000	-.165996E-01	.667737E-01	.486413E-01	.842630E-01
42.000	170.000	-.202389E-01	.766311E-01	.491644E-01	.932688E-01
49.000	180.000	-.241026E-01	.871138E-01	.496258E-01	.103114
60.000	190.000	-.282166E-01	.983087E-01	.502043E-01	.113935
77.000	200.000	-.327135E-01	.110669	.506013E-01	.126009
96.000	210.000	-.377141E-01	.124409	.507433E-01	.139552
116.00	220.000	-.442365E-01	.141794	.509111E-01	.157017
136.00	230.000	-.511398E-01	.160179	.510464E-01	.175722
156.00	240.000	-.629417E-01	.189661	.513081E-01	.206314
176.00	250.000	-.788481E-01	.228478	.515409E-01	.247135
196.00	<u>260.000</u>	-.972529E-01	.273655	.506501E-01	<u>.294806</u>
216.00	270.000	-.135539	.371122	.495398E-01	.398192
236.00	280.000	-.187848	.504164	.497467E-01	.540317
256.00	290.000	-.239528	.635453	.472668E-01	.680741
276.00	300.000	-.307284	.807290	.424716E-01	.864838
296.00	310.000	-.377618	.983632	.387890E-01	1.05434
316.00	320.000	-.430055	1.11346	.357558E-01	1.19416
336.00	327.000	-.461480	1.19036	.338849E-01	<u>1.27713</u>

Yield Displ.

Final Displ.

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Project WBNP Unit 1

Subject Qualif. of Worst Cases of Thermally Restrained Structures -- Package I

11.3.3.1.10 Strain History at Node 2 (Element 28)
(Segment Points 1 and 3)

PR9C - EVAL. FOR WORST CASES 1-27 & 2&3-9(C), DWG 48N968, EL. 783'-8 1/2

ANSYS POST26 VARIABLE LISTING

ITER	ESTR 1 53	ESTR 28 215	ESTR 28 216	ESTR 28 227	ESTR 28 228
	1 TEMP	ARG1EE01	ARG1EP01	ARG1EE03	ARG1EP03
4.0000	70.0000	.126358E-04	0.00000	.231501E-04	0.00000
8.0000	70.0000	.282074E-04	0.00000	.262407E-04	0.00000
12.000	80.0000	.842797E-04	0.00000	.166335E-04	0.00000
15.000	90.0000	.711777E-04	0.00000	.197638E-04	0.00000
18.000	100.000	.582223E-04	0.00000	.230422E-04	0.00000
21.000	110.000	.454067E-04	0.00000	.264621E-04	0.00000
24.000	120.000	.327331E-04	0.00000	.300249E-04	0.00000
27.000	130.000	.202034E-04	0.00000	.337320E-04	0.00000
30.000	140.000	.781967E-05	0.00000	.375849E-04	0.00000
33.000	150.000	-.441581E-05	0.00000	.415852E-04	0.00000
36.000	160.000	-.165007E-04	0.00000	.457346E-04	0.00000
42.000	170.000	-.280059E-04	0.00000	.505317E-04	0.00000
49.000	180.000	-.389695E-04	0.00000	.559947E-04	0.00000
60.000	190.000	-.487900E-04	0.00000	.627303E-04	0.00000
77.000	200.000	-.576493E-04	0.00000	.707554E-04	0.00000
96.000	210.000	-.663898E-04	0.00000	.790809E-04	0.00000
116.00	220.000	-.747512E-04	0.00000	.877232E-04	0.00000
136.00	230.000	-.829416E-04	0.00000	.965189E-04	0.00000
156.00	240.000	-.900031E-04	0.00000	.106098E-03	0.00000
176.00	250.000	-.962415E-04	0.00000	.116188E-03	0.00000
196.00	260.000	-.101878E-03	0.00000	.126503E-03	0.00000
216.00	270.000	-.105902E-03	0.00000	.136529E-03	0.00000
236.00	280.000	-.112825E-03	0.00000	.142326E-03	0.00000
256.00	290.000	-.123040E-03	0.00000	.144571E-03	0.00000
276.00	300.000	-.132936E-03	0.00000	.146379E-03	0.00000
296.00	310.000	-.143865E-03	0.00000	.146863E-03	0.00000
316.00	320.000	-.155175E-03	0.00000	.147988E-03	0.00000
336.00	327.000	-.176050E-03	0.00000	.153595E-03	0.00000

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Project WBNP Unit 1

Subject Qualif. of Worst Cases of Thermally Restrained Structures - Package I

11.3.3.1.10 Strain History at Node 2 (Element 28) (Cont'd)
(Segment Points 7 and 9)

ITER	ESTR 1 53	ESTR 28 251	ESTR 28 252	ESTR 28 263	ESTR 28 264
	1 TEMP	ARG1EE07	ARG1EP07	ARG1EE09	ARG1EP09
4.0000	70.0000	-.106445E-04	0.00000	-.211588E-04	0.00000
8.0000	70.0000	-.489001E-06	0.00000	.147770E-05	0.00000
12.000	80.0000	.408649E-05	0.00000	.717326E-04	0.00000
15.000	90.0000	.288165E-05	0.00000	.542956E-04	0.00000
18.000	100.000	.194347E-05	0.00000	.371235E-04	0.00000
21.000	110.000	.126817E-05	0.00000	.202128E-04	0.00000
24.000	120.000	.860009E-06	0.00000	.356822E-05	0.00000
27.000	130.000	.723384E-06	0.00000	-.128052E-04	0.00000
30.000	140.000	.862830E-06	0.00000	-.289024E-04	0.00000
33.000	150.000	.128306E-05	0.00000	-.447179E-04	0.00000
36.000	160.000	.198900E-05	0.00000	-.602464E-04	0.00000
42.000	170.000	.376870E-05	0.00000	-.747689E-04	0.00000
49.000	180.000	.742664E-05	0.00000	-.875375E-04	0.00000
60.000	190.000	.126276E-04	0.00000	-.988927E-04	0.00000
77.000	200.000	.219258E-04	0.00000	-.106479E-03	0.00000
96.000	210.000	.337828E-04	0.00000	-.111688E-03	0.00000
116.00	220.000	.462950E-04	0.00000	-.116179E-03	0.00000
136.00	230.000	.589481E-04	0.00000	-.120512E-03	0.00000
156.00	240.000	.728097E-04	0.00000	-.123292E-03	0.00000
176.00	250.000	.875194E-04	0.00000	-.124910E-03	0.00000
196.00	260.000	.102399E-03	0.00000	-.125982E-03	0.00000
216.00	270.000	.116896E-03	0.00000	-.125535E-03	0.00000
236.00	280.000	.124434E-03	0.00000	-.130717E-03	0.00000
256.00	290.000	.125952E-03	0.00000	-.141659E-03	0.00000
276.00	300.000	.125872E-03	0.00000	-.153443E-03	0.00000
296.00	310.000	.123422E-03	0.00000	-.167306E-03	0.00000
316.00	320.000	.121724E-03	0.00000	-.181439E-03	0.00000
336.00	327.000	.125619E-03	0.00000	-.204025E-03	0.00000

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Client TVA

Project WBNP Unit 1

Subject Qualif. of Worst Cases of Thermally Restrained Structures - Package I

11.3.3.1.11 Strain History at Node 5 (Element 4)
(Segment Points 1 and 3)

PR9C - EVAL. FOR WORST CASES 1-27 & 2&3-9(C), DWG 48N968, EL. 783'-8 1/2

ANSYS POST26 VARIABLE LISTING

ITER	ESTR 1 53	ESTR 4 95	ESTR 4 96	ESTR 4 107	ESTR 4 108
	1 TEMP	ARG1EE01	ARG1EP01	ARG1EE03	ARG1EP03
4.0000	70.0000	-.153250E-04	0.00000	.253475E-04	0.00000
8.0000	70.0000	.190337E-04	0.00000	.175954E-04	0.00000
12.0000	80.0000	.810100E-04	0.00000	-.522927E-04	0.00000
15.0000	90.0000	.655804E-04	0.00000	-.450494E-04	0.00000
18.0000	100.0000	.501399E-04	0.00000	-.378224E-04	0.00000
21.0000	110.0000	.346912E-04	0.00000	-.306093E-04	0.00000
24.0000	120.0000	.192349E-04	0.00000	-.234106E-04	0.00000
27.0000	130.0000	.377136E-05	0.00000	-.162269E-04	0.00000
30.0000	140.0000	-.116987E-04	0.00000	-.905907E-05	0.00000
33.0000	150.0000	-.271746E-04	0.00000	-.190769E-05	0.00000
36.0000	160.0000	-.426557E-04	0.00000	.522646E-05	0.00000
42.0000	170.0000	-.583013E-04	0.00000	.122903E-04	0.00000
49.0000	180.0000	-.743689E-04	0.00000	.191327E-04	0.00000
60.0000	190.0000	-.906978E-04	0.00000	.259360E-04	0.00000
77.0000	200.0000	-.108182E-03	0.00000	.321893E-04	0.00000
96.0000	210.0000	-.126415E-03	0.00000	.380102E-04	0.00000
116.0000	220.0000	-.144501E-03	0.00000	.437083E-04	0.00000
136.0000	230.0000	-.162541E-03	0.00000	.494048E-04	0.00000
156.0000	240.0000	-.179638E-03	0.00000	.548064E-04	0.00000
176.0000	250.0000	-.195920E-03	0.00000	.599415E-04	0.00000
196.0000	260.0000	-.211388E-03	0.00000	.645806E-04	0.00000
216.0000	270.0000	-.221750E-03	0.00000	.675039E-04	0.00000
236.0000	280.0000	-.227826E-03	0.00000	.701418E-04	0.00000
256.0000	290.0000	-.232711E-03	0.00000	.732635E-04	0.00000
276.0000	300.0000	-.235794E-03	0.00000	.755023E-04	0.00000
296.0000	310.0000	-.237794E-03	0.00000	.778664E-04	0.00000
316.0000	320.0000	-.242681E-03	0.00000	.810392E-04	0.00000
336.0000	327.0000	-.280946E-03	0.00000	.950075E-04	0.00000

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Client: TVA

Project: WBNP Unit 1

Subject: Qualif. of Worst Cases of Thermally Restrained Structures - Package I

11.3.3.1.11 Strain History at Node 5 (Element 4) (Cont'd)
(Segment Points 7 and 9)

ITER	ESTR 1 53	ESTR 4 131	ESTR 4 132	ESTR 4 143
	1 TEMP	ARG1EE07	ARG1EP07	ARG1EE09
4.0000	70.0000	-.411777E-05	0.00000	-.447902E-04
8.0000	70.0000	-.112882E-04	0.00000	-.984989E-05
12.000	80.0000	-.735072E-04	0.00000	.597955E-04
15.000	90.0000	-.687810E-04	0.00000	.418488E-04
18.000	100.000	-.640298E-04	0.00000	.239325E-04
21.000	110.000	-.592565E-04	0.00000	.604400E-05
24.000	120.000	-.544609E-04	0.00000	-.118155E-04
27.000	130.000	-.496428E-04	0.00000	-.296445E-04
30.000	140.000	-.448021E-04	0.00000	-.474417E-04
33.000	150.000	-.399387E-04	0.00000	-.652056E-04
36.000	160.000	-.350524E-04	0.00000	-.829345E-04
42.000	170.000	-.300462E-04	0.00000	-.100638E-03
49.000	180.000	-.247229E-04	0.00000	-.118224E-03
60.000	190.000	-.192481E-04	0.00000	-.135882E-03
77.000	200.000	-.129641E-04	0.00000	-.153336E-03
96.000	210.000	-.613761E-05	0.00000	-.170563E-03
116.00	220.000	.675396E-06	0.00000	-.187534E-03
136.00	230.000	.746666E-05	0.00000	-.204479E-03
156.00	240.000	.140167E-04	0.00000	-.220428E-03
176.00	250.000	.203696E-04	0.00000	-.235491E-03
196.00	260.000	.266231E-04	0.00000	-.249345E-03
216.00	270.000	.313399E-04	0.00000	-.257914E-03
236.00	280.000	.339865E-04	0.00000	-.263982E-03
256.00	290.000	.356888E-04	0.00000	-.270286E-03
276.00	300.000	.369629E-04	0.00000	-.274333E-03
296.00	310.000	.376049E-04	0.00000	-.278056E-03
316.00	320.000	.392181E-04	0.00000	-.284502E-03
336.00	327.000	.523064E-04	0.00000	-.323647E-03

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Client TVA

Project WBNP Unit 1

Subject Qualif. of Worst Cases of Thermally Restrained Structures - Package I

11.3.3.1.12 Displacement History of Node 2

PR9C - EVAL. FOR WORST CASES 1-27 & 2&3-9(C), DWG 48N968, EL. 783'-8 1/2

ANSYS POST26 VARIABLE LISTING

ITER	ESTR 1 53	DISP 2 UX	DISP 2 UY	DISP 2 UZ	OPER 28 SQRT
	1 TEMP	2 UX	2 UY	2 UZ	SRSS
4.0000	70.0000	-.320371E-03	.173329E-04	-.106532E-02	.111259E-02
8.0000	70.0000	.141122E-04	.676097E-04	-.984201E-03	.986622E-03
12.0000	80.0000	-.145581E-03	.481808E-03	-.573448E-03	.763004E-03
15.0000	90.0000	-.165966E-02	.808955E-03	-.691055E-03	.197140E-02
18.0000	100.000	-.317372E-02	.113711E-02	-.805138E-03	.346609E-02
21.0000	110.000	-.468777E-02	.146624E-02	-.915577E-03	.499633E-02
24.0000	120.000	-.620179E-02	.179636E-02	-.102229E-02	.653714E-02
27.0000	130.000	-.771578E-02	.212750E-02	-.112519E-02	.808242E-02
30.0000	140.000	-.922971E-02	.245966E-02	-.122419E-02	.962996E-02
33.0000	150.000	-.107436E-01	.279286E-02	-.131920E-02	.111788E-01
36.0000	160.000	-.122574E-01	.312712E-02	-.141013E-02	.127284E-01
42.0000	170.000	-.137721E-01	.346558E-02	-.148651E-02	.142790E-01
49.0000	180.000	-.152885E-01	.381021E-02	-.152872E-02	.158301E-01
60.0000	190.000	-.168069E-01	.416170E-02	-.155679E-02	.173843E-01
77.0000	200.000	-.183306E-01	.452622E-02	-.150677E-02	.189411E-01
96.0000	210.000	-.198570E-01	.489764E-02	-.139829E-02	.204998E-01
116.00	220.000	-.213812E-01	.527155E-02	-.128022E-02	.220587E-01
136.00	230.000	-.229051E-01	.564618E-02	-.116180E-02	.236193E-01
156.00	240.000	-.244184E-01	.602622E-02	-.103025E-02	.251721E-01
176.00	250.000	-.259226E-01	.641002E-02	-.888985E-03	.267182E-01
196.00	260.000	-.274157E-01	.679529E-02	-.752212E-03	.282553E-01
216.00	270.000	-.288510E-01	.718134E-02	-.620929E-03	.297378E-01
236.00	280.000	-.302474E-01	.754114E-02	-.563860E-03	.311784E-01
256.00	290.000	-.316378E-01	.787733E-02	-.574351E-03	.326088E-01
276.00	300.000	-.330055E-01	.820941E-02	-.623740E-03	.340169E-01
296.00	310.000	-.343650E-01	.853263E-02	-.700607E-03	.354154E-01
316.00	320.000	-.357559E-01	.885797E-02	-.774253E-03	.368449E-01
336.00	327.000	-.367912E-01	.908818E-02	-.818897E-03	.379059E-01

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Project WBNP Unit 1

Subject Qualif. of Worst Cases of Thermally Restrained Structures - Package I

11.3.3.1.13 Displacement History of Node 5

PR9C - EVAL. FOR WORST CASES 1-27 & 2&3-9(C), DWG 48N968, EL. 783'-8 1/2

ANSYS POST26 VARIABLE LISTING

ITER	ESTR 1 53	DISP 5 UX	DISP 5 UY	DISP 5 UZ	OPER 28 SQRT
	1 TEMP	5 UX	5 UY	5 UZ	SRSS
4.0000	70.0000	-.318847E-03	.112461E-05	-.541007E-03	.627975E-03
8.0000	70.0000	.385520E-05	.173282E-04	-.532645E-03	.532941E-03
12.000	80.0000	.237514E-02	.198222E-03	-.390766E-03	.241522E-02
15.000	90.0000	.329823E-02	.542256E-03	-.436933E-03	.337095E-02
18.000	100.000	.422135E-02	.886307E-03	-.482309E-03	.434027E-02
21.000	110.000	.514449E-02	.123038E-02	-.526984E-03	.531576E-02
24.000	120.000	.606766E-02	.157446E-02	-.570942E-03	.629455E-02
27.000	130.000	.699088E-02	.191856E-02	-.614169E-03	.727534E-02
30.000	140.000	.791417E-02	.226269E-02	-.656646E-03	.825742E-02
33.000	150.000	.883752E-02	.260682E-02	-.698358E-03	.924040E-02
36.000	160.000	.976097E-02	.295098E-02	-.739285E-03	.102241E-01
42.000	170.000	.106836E-01	.329513E-02	-.776591E-03	.112071E-01
49.000	180.000	.116044E-01	.363926E-02	-.803799E-03	.121882E-01
60.000	190.000	.125232E-01	.398337E-02	-.827385E-03	.131675E-01
77.000	200.000	.134366E-01	.432735E-02	-.825681E-03	.141404E-01
96.000	210.000	.143472E-01	.467119E-02	-.805774E-03	.151099E-01
116.00	220.000	.152603E-01	.501502E-02	-.783746E-03	.160823E-01
136.00	230.000	.161739E-01	.535885E-02	-.762051E-03	.170556E-01
156.00	240.000	.170994E-01	.570290E-02	-.739105E-03	.180404E-01
176.00	250.000	.180351E-01	.604718E-02	-.714720E-03	.190354E-01
196.00	260.000	.189834E-01	.639164E-02	-.683626E-03	.200422E-01
216.00	270.000	.199968E-01	.673670E-02	-.649580E-03	.211110E-01
236.00	280.000	.210535E-01	.708173E-02	-.648060E-03	.222221E-01
256.00	290.000	.221166E-01	.742651E-02	-.672776E-03	.233399E-01
276.00	300.000	.232053E-01	.777145E-02	-.690642E-03	.244818E-01
296.00	310.000	.243031E-01	.811636E-02	-.722221E-03	.256327E-01
316.00	320.000	.253655E-01	.846097E-02	-.750931E-03	.267500E-01
336.00	327.000	.261473E-01	.870177E-02	-.767220E-03	.275679E-01

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11.3.3.2 Analysis Results for "PR9CM" (Analysis for the Modified Structure)

The ANSYS output for the analysis "PR9CM" is stored in file "PR9CMR". The analysis results of the final load step (Load Step 16) are provided in this Section.

The deformed structure is plotted in Figure 11.3.3.2.1. Note that the displacements in this plot are magnified.

Since the worst case 2&3-9(C) is already qualified and the differences between the results of PR9C and PR9CM for the worst case 2&3-9(C) are insignificant, only the results related to the worst case 1-27 are presented.

The reaction forces and the nodal displacements are provided in Sections 11.3.3.2.1 and 11.3.3.2.2. The maximum displacement for the axially restrained member occurs at Node 52.

The stresses and strains for elements 19, 51, 52 and 20 are listed in Section 11.3.3.2.3. These elements are located around node 52 where maximum displacements occurs. For explanation of labels, see Section 11.3.3.1. The stress and strain results indicate no yield occurs in elements located around node 52. Since there is no yield occurs in elements around node 52, strain and displacement history are not presented.

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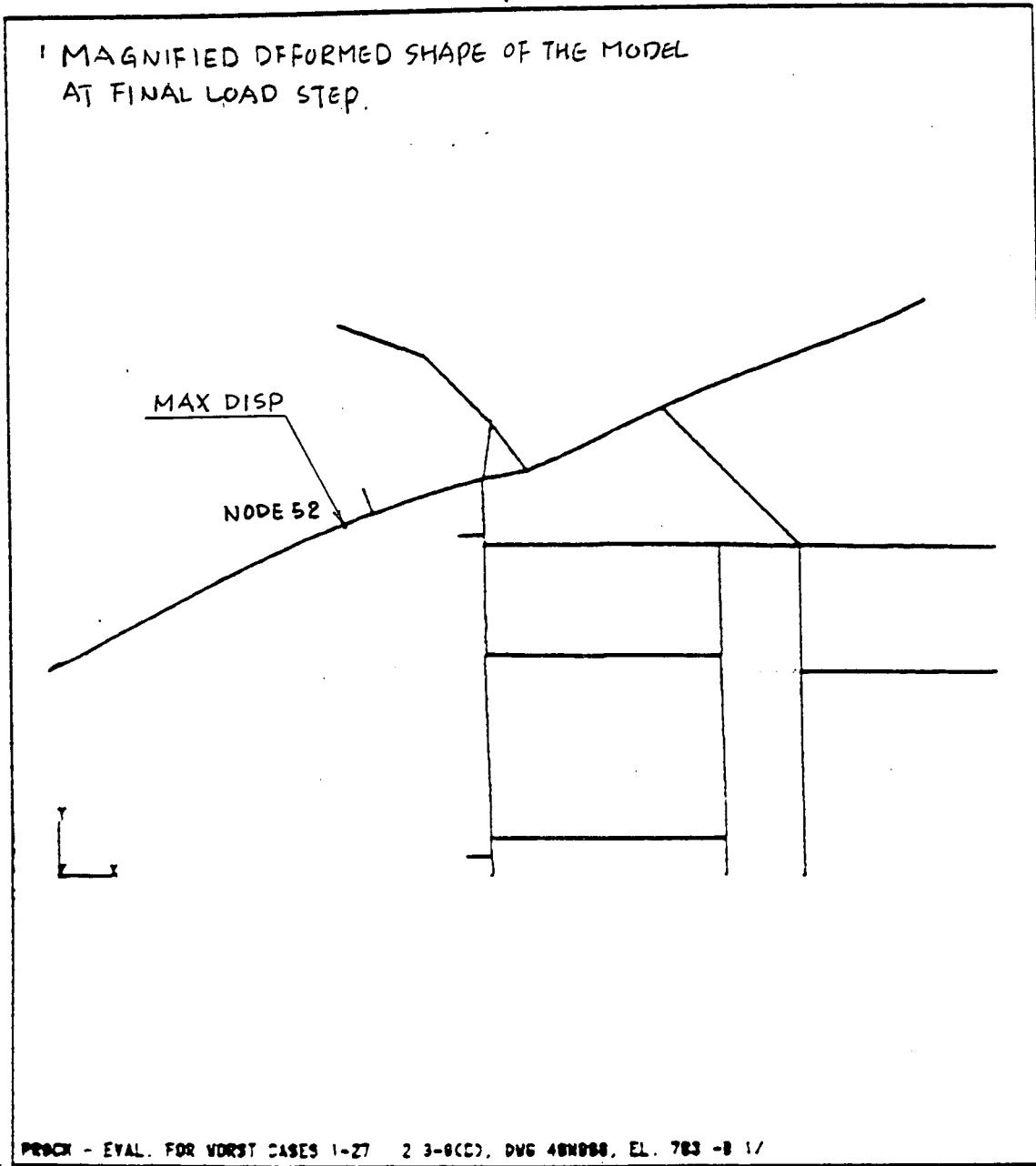
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Figure 11.3.3.2.1 Deformed Shape of the Model for Analysis PR9CM



ANSTS 4.34
AUG 18 1991
15 09 53
PLOT NO. 9
POST1 DISPL.
STEP=28
ITER=28
DIX =1.144

ZY =1
DIST=88.124
XF =72.84
YF =30.38
ZF =43.25

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11.3.3.2.1 Reaction Forces

PR9CM - EVAL. FOR WORST CASES 1-27 & 2&3-9(C), DWG 48N968, EL. 783'-8 1/2

***** POST1 REACTION FORCE LISTING *****

LOAD STEP 28 ITERATION= 20 SECTION= 1
TIME= 0.0000 LOAD CASE= 1

THE FOLLOWING X,Y,Z FORCES ARE IN NODAL COORDINATES

NODE	FX	FY	FZ	HX	HY	HZ
1	1.4087475	5.7494642	1.0539391		-1.2435130	
4	-.46901430	1.7586797	.29376733		-.35193110	
7	-4.7788720		.56925887	.32640895E-01		
8			.17777728	-1.0169051		
10	-7.5205966		.85151312	-1.2579102		
11		.69183167	-.17641853E-01		.96430288E-02	
12	.45629147		.16273704	.11610257E-01		
101	0.0000000	0.0000000	0.0000000	15.165983	0.0000000	-11.928565
104	0.0000000	0.0000000	0.0000000	7.0455124	0.0000000	-4.6363303
107	0.0000000	-.25503964	0.0000000	0.0000000	13.208701	4.8246538
108	0.0000000	-.47898371	0.0000000	0.0000000	.73947305	0.0000000
110	0.0000000	-.82053205	0.0000000	0.0000000	11.675166	11.513603
111	.19417214	0.0000000	0.0000000	3.4942703	0.0000000	-3.0640068
112	0.0000000	-.28835107	0.0000000	0.0000000	3.1376952	3.2805424
132	9.7142126	-5.4361256	-1.1703425	0.0000000	0.0000000	0.0000000
TOTAL	-.99505919	.92094354	1.9210084	23.475202	27.175234	-.10102576E-01

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11.3.3.2 Displacements

PR9CM - EVAL. FOR WORST CASES 1-27 & 283-9(C), DWG 48N968, EL. 783'-8 1/2

***** POST1 NODAL DISPLACEMENT LISTING *****

LOAD STEP 28 ITERATION= 20 SECTION= 1
TIME= 0.0000 LOAD CASE= 1

THE FOLLOWING X,Y,Z DISPLACEMENTS ARE IN NODAL COORDINATES

NODE	UX	UY	UZ	ROTX	ROTY	ROTZ
1	-.30000000E-01	0.0000000	0.0000000	-.54949215E-03	0.0000000	.45898515E-03
2	-.36020310E-01	.84409586E-02	-.30678537E-02	-.65597392E-03	-.21303923E-04	.17648799E-02
3	-.10419775	.75870351E-01	-.25829178E-01	-.81870311E-04	-.12497013E-03	.70184635E-03
4	.30000000E-01	0.0000000	0.0000000	-.25527219E-03	0.0000000	.17839587E-03
5	.27004497E-01	.87009688E-02	-.14793129E-02	-.30992358E-03	-.46305603E-04	.10780986E-02
6	-.40739911E-01	.78061166E-01	-.18462022E-01	-.45119357E-03	-.36010631E-03	-.10881638E-02
7	.31250000E-01	.63759911E-04	0.0000000	0.0000000	-.23379059E-03	-.14561915E-03
8	-.20054021	.11974593E-03	0.0000000	0.0000000	-.90766300E-04	.14199793E-01
9	-.11001204	.91069247E-01	-.22470889E-01	.33651750E-03	.16671418E-02	-.19951888E-01
10	.31250000E-01	.20513301E-03	0.0000000	0.0000000	-.67428044E-03	-.54184213E-03
11	-.48543034E-04	-.31250000E-01	0.0000000	-.12660400E-03	0.0000000	.11789629E-03
12	.31250000E-01	.72087768E-04	0.0000000	0.0000000	-.15854152E-03	-.25312827E-03
13	-.90863042E-01	.50578620E-01	-.20727957E-01	-.56924994E-03	-.23541899E-03	.11362994E-02
14	-.27625622E-01	.52128710E-01	-.11624549E-01	-.44269963E-03	-.27195468E-03	.16874800E-02
15	-.45954008E-02	.10456144E-01	-.35908353E-02	-.14122412E-02	-.30997432E-04	.78674052E-05
16	-.59332081E-01	.49531904E-01	-.16395609E-01	-.50382923E-03	-.25180181E-03	-.64238292E-03
17	-.72557262E-01	.85541605E-01	-.23250711E-01	-.29645844E-03	-.18472032E-03	.19130075E-03
18	-.19774896E-01	.47626461E-01	-.13188847E-01	-.50505135E-03	-.52080912E-03	-.18998331E-02
19	.31764313E-02	.20419981E-01	.62512493E-02	.30366186E-03	-.48216550E-03	-.19334039E-02
20	-.18487420	-.25447189E-01	.84767945E-02	.61075016E-02	.19613538E-02	.13944192E-01
21	-.23863514	.21449039	.39257095E-01	.24596468E-01	.91286523E-02	.94145740E-02
22	-.22332971	.30534081	.69006338E-01	.51833426E-01	.19429273E-01	-.46286656E-02
23	-.57650276E-01	-.41183048E-01	-.30747436E-01	.15466540E-02	.17612469E-02	-.18965868E-01
24	-.61308748E-01	.59318518E-02	-.40073752E-01	-.83803459E-03	-.31689502E-03	.72158039E-02
25	-.66938572E-01	.66478708E-01	-.37707629E-01	-.63894768E-03	-.71713990E-03	.37342272E-02
26	-.20323043E-01	.52703656E-01	-.21838766E-01	-.26171306E-02	-.20280227E-02	-.40313646E-02
27	.14285298E-01	.17050612E-01	-.87978351E-02	.22632530E-02	-.18746571E-02	.27280712E-02
28	-.32192487E-01	.42216820E-02	-.17279044E-02	-.60926603E-03	-.21743604E-03	.12465391E-02
29	-.10521585	.78370355E-01	-.25918369E-01	-.18611565E-04	-.13638785E-03	-.67237960E-03
30	-.21567684	.29523747	.70106002E-01	.54501301E-01	.20414949E-01	-.62179041E-02
31	-.21810712E-01	.1777736E-01	-.61572626E-02	-.32793596E-03	-.22413071E-03	.48116204E-03
32	.23181492	.40562392	.73644596E-03	.60777424E-03	.14934895E-03	-.26682337E-01
33	.67675907E-01	.10119472	-.73290545E-02	.84876442E-03	.52119635E-03	-.23108396E-01
34	.47696137E-01	.50092156E-01	-.20731750E-01	.15458175E-02	.17605936E-02	-.18966673E-01
35	-.10052927	.89053461E-01	-.12447371E-01	.33759623E-03	.16670884E-02	-.19952224E-01
41	-.17662738	-.67095434E-01	.42559136E-02	.30740123E-02	.94399345E-03	.14136387E-01
45	-.20623696	.63387266E-01	.18941640E-01	.12199757E-01	.43245435E-02	.13062039E-01
46	-.22465906	.14467193	.29222743E-01	.18354250E-01	.67089406E-02	.11549000E-01
51	-.24716061	.28172366	.50288548E-01	.33331062E-01	.12417654E-01	.57315582E-02
52	-.24299374	.31473135	.60392389E-01	.42368791E-01	.15833545E-01	.10430536E-02

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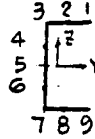
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11.3.3.2.3 Stresses and Strains



EL= 19		NODES= 21		51		MAT= 1		PRESSURES(Z,Y)=		0.0000		0.0000		AVE. TEMP= 327.00	
END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW								
	1	327.00	13.968	.0005098	0.0000000	0.0000000	0.0000000								
	2	327.00	4.0221	.0001468	0.0000000	0.0000000	0.0000000								
	3	327.00	-5.9240	-.0002162	0.0000000	0.0000000	0.0000000								
	4	327.00	-5.5202	-.0002015	0.0000000	0.0000000	0.0000000								
	5	327.00	-5.1165	-.0001867	0.0000000	0.0000000	0.0000000								
	6	327.00	-4.7128	-.0001720	0.0000000	0.0000000	0.0000000								
	7	327.00	-4.3091	-.0001573	0.0000000	0.0000000	0.0000000								
	8	327.00	5.6370	.0002057	0.0000000	0.0000000	0.0000000								
	9	327.00	15.583	.0005687	0.0000000	0.0000000	0.0000000								
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW								
	1	327.00	18.564	.0006775	0.0000000	0.0000000	0.0000000								
	2	327.00	5.3862	.0001966	0.0000000	0.0000000	0.0000000								
	3	327.00	-7.7912	-.0002844	0.0000000	0.0000000	0.0000000								
	4	327.00	-7.2791	-.0002657	0.0000000	0.0000000	0.0000000								
	5	327.00	-6.7670	-.0002470	0.0000000	0.0000000	0.0000000								
	6	327.00	-6.2548	-.0002283	0.0000000	0.0000000	0.0000000								
	7	327.00	-5.7427	-.0002096	0.0000000	0.0000000	0.0000000								
	8	327.00	7.4347	.0002713	0.0000000	0.0000000	0.0000000								
	9	327.00	20.612	.0007523	0.0000000	0.0000000	0.0000000								
FORCES ON MEMBER AT NODE		21		.110649		-.444987		.163763		-.962057		5.13534		11.8020	
		51		-.112841		.439391		-.154293		.973978		-6.51430		-15.6363	

EL= 51		NODES= 51		52		MAT= 1		PRESSURES(Z,Y)=		0.0000		0.0000		AVE. TEMP= 327.00	
END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW								
	1	327.00	18.462	.0006738	0.0000000	0.0000000	0.0000000								
	2	327.00	5.3372	.0001948	0.0000000	0.0000000	0.0000000								
	3	327.00	-7.7875	-.0002842	0.0000000	0.0000000	0.0000000								
	4	327.00	-7.2641	-.0002651	0.0000000	0.0000000	0.0000000								
	5	327.00	-6.7407	-.0002460	0.0000000	0.0000000	0.0000000								
	6	327.00	-6.2173	-.0002269	0.0000000	0.0000000	0.0000000								
	7	327.00	-5.6939	-.0002078	0.0000000	0.0000000	0.0000000								
	8	327.00	7.4308	.0002712	0.0000000	0.0000000	0.0000000								
	9	327.00	20.555	.0007502	0.0000000	0.0000000	0.0000000								
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW								
	1	327.00	22.983	.0008388	0.0000000	0.0000000	0.0000000								
	2	327.00	6.6817	.0002439	0.0000000	0.0000000	0.0000000								
	3	327.00	-9.6194	-.0003511	0.0000000	0.0000000	0.0000000								
	4	327.00	-8.9912	-.0003281	0.0000000	0.0000000	0.0000000								
	5	327.00	-8.3631	-.0003052	0.0000000	0.0000000	0.0000000								
	6	327.00	-7.7349	-.0002823	0.0000000	0.0000000	0.0000000								
	7	327.00	-7.1068	-.0002594	0.0000000	0.0000000	0.0000000								
	8	327.00	9.1943	.0003356	0.0000000	0.0000000	0.0000000								
	9	327.00	25.495	.0009305	0.0000000	0.0000000	0.0000000								
FORCES ON MEMBER AT NODE		51		.114684		-.437429		.158453		-.996065		6.65749		15.5738	
		52		-.116852		.431913		-.148931		1.00805		-7.99017		-19.3429	

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11.3.3.2.3 Stresses and Strains (Cont'd)

EL= 52		NODES= 52		22 MAT= 1		PRESSURES(Z,Y)= 0.0000		0.0000		AVE. TEMP= 327.0	
END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW				
	1	327.00	22.853	.0008340	0.0000000	0.0000000	0.0000000				
	2	327.00	6.6189	.0002416	0.0000000	0.0000000	0.0000000				
	3	327.00	-9.6148	-.0003509	0.0000000	0.0000000	0.0000000				
	4	327.00	-8.9722	-.0003275	0.0000000	0.0000000	0.0000000				
	5	327.00	-8.3295	-.0003040	0.0000000	0.0000000	0.0000000				
	6	327.00	-7.6868	-.0002805	0.0000000	0.0000000	0.0000000				
	7	327.00	-7.0442	-.0002571	0.0000000	0.0000000	0.0000000				
	8	327.00	9.1896	.0003354	0.0000000	0.0000000	0.0000000				
	9	327.00	25.423	.0009279	0.0000000	0.0000000	0.0000000				
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW				
	1	327.00	27.298	.0009963	0.0000000	0.0000000	0.0000000				
	2	327.00	7.9435	.0002899	0.0000000	0.0000000	0.0000000				
	3	327.00	-11.411	-.0004165	0.0000000	0.0000000	0.0000000				
	4	327.00	-10.667	-.0003893	0.0000000	0.0000000	0.0000000				
	5	327.00	-9.9236	-.0003622	0.0000000	0.0000000	0.0000000				
	6	327.00	-9.1798	-.0003350	0.0000000	0.0000000	0.0000000				
	7	327.00	-8.4359	-.0003079	0.0000000	0.0000000	0.0000000				
	8	327.00	10.919	.0003985	0.0000000	0.0000000	0.0000000				
	9	327.00	30.274	.0011049	0.0000000	0.0000000	0.0000000				
FORCES ON MEMBER AT NODE		52	.119094	-.429807	.153188	-1.04404	8.17472	19.2630			
		22	-.121234	.424375	-.143611	1.05610	-9.46152	-22.9663			

EL= 20		NODES= 22		55 MAT= 1		PRESSURES(Z,Y)= 0.0000		0.0000		AVE. TEMP= 327.00	
END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW				
	1	327.00	27.235	.0009940	0.0000000	0.0000000	0.0000000				
	2	327.00	7.9132	.0002888	0.0000000	0.0000000	0.0000000				
	3	327.00	-11.409	-.0004164	0.0000000	0.0000000	0.0000000				
	4	327.00	-10.659	-.0003890	0.0000000	0.0000000	0.0000000				
	5	327.00	-9.9085	-.0003616	0.0000000	0.0000000	0.0000000				
	6	327.00	-9.1584	-.0003342	0.0000000	0.0000000	0.0000000				
	7	327.00	-8.4083	-.0003069	0.0000000	0.0000000	0.0000000				
	8	327.00	10.914	.0003983	0.0000000	0.0000000	0.0000000				
	9	327.00	30.235	.0011035	0.0000000	0.0000000	0.0000000				
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW				
	1	327.00	27.791	.0010143	0.0000000	0.0000000	0.0000000				
	2	327.00	8.0790	.0002949	0.0000000	0.0000000	0.0000000				
	3	327.00	-11.633	-.0004246	0.0000000	0.0000000	0.0000000				
	4	327.00	-10.870	-.0003967	0.0000000	0.0000000	0.0000000				
	5	327.00	-10.108	-.0003689	0.0000000	0.0000000	0.0000000				
	6	327.00	-9.3451	-.0003411	0.0000000	0.0000000	0.0000000				
	7	327.00	-8.5825	-.0003132	0.0000000	0.0000000	0.0000000				
	8	327.00	11.129	.0004062	0.0000000	0.0000000	0.0000000				
	9	327.00	30.841	.0011256	0.0000000	0.0000000	0.0000000				
FORCES ON MEMBER AT NODE		22	.126111	-.422357	.145348	-1.16197	9.54120	22.9273			
		55	-.126373	.421672	-.144135	1.16350	-9.69987	-23.3899			

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11.3.4 Member Qualification

11.3.4.1 Member Qualification for Worst Case 1-27

The member for worst case 1-27 is first evaluated using the results of the analysis "PR9C" for the original structure. The member is not qualified and requires modification. The member is then evaluated using the results of the analysis "PR9CM" for the modified structure.

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	References
<u>Member Qualification for Worst Case 1-27.</u>	
1. Based on the Results from the analysis for the original struct. ('PRQC'), The max displ. for worst case member (Axial Restraint) occurs at Node 51.	Sec 11.3.3.1.2 Displacement
2. Based on the strain History at location around Node 51 = (Nodes 46, 21, 51, 52, 22) Temperature = 260°F when the first plastic strain occurs. (Segment point 3, ARGH EP03) Section 11.3.3.1.6	Sec 11.3.3.1.4 through Sec 11.3.3.1.8
3. The stress displ. (Node 51) @ yield temp (260°F) = 0.294"	Sec 11.3.3.1.9 Displ. hist. of Node 51
Final (Actual) Displ. = 1.277"	
Ductility Ratio, $\mu = \frac{\text{Actual Displ.}}{\text{Yield Displ.}}$ $= \frac{1.277}{0.294}$ $= 4.3 > 3$ N.G.	DG-C1.6.12 R1 D 3.2
⇒ Modification is required for the member with AXIAL Restraint See Section 11.3.7 for Modification Design.	

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	References
<u>Member Qualification for Worst Case 1-27 (cont'd)</u>	
<u>Based on the results from the analysis on the modified struc. (PR9CM):</u>	
<u>1. Max Displ. of the worst case (1-27) member occurs at node 52</u>	<u>Sec 11.3.3.2.2 Displ.</u>
<u>2. No yielding occurs at nodes around Node 52 (Elements 19, 51, 52, 22)</u>	<u>Sec 11.3.3.2.3 stresses & strains</u>
<u>→ Ductility Ratio at Node 52 < 1.</u>	
<u>∴ The Member for Worst case 1-27 is qualified.</u>	

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11.3.4.2 Member Qualification for Worst Case 2&3-9(C)

The members of the worst case 2&3-9(C) are evaluated using the results of the analysis "PR9C" for the original structure (Section 11.3.3.1).

Since the location of worst case 2&3-9(C) is far away from the region which modification is made for worst case 1-27, the members of worst case 2&3-9(C) are not re-evaluated using the results from the analysis "PR9CM" for the modified structure.

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 CLIENT TVA

PROJECT WBNP - Unit 1
 SUBJECT Qual. of Worst Case of Thermally Restrained Struc. - Package I

	<u>References</u>
<u>Member Qualification (Results from PRSC)</u>	
<u>Proximity Member (ELEMENTS 7 & 8)</u>	
1. Max. displ @ Node 15	Sec 11.3.3.1.2 Displ.
2. No yielding occurs @ Node 15	Sec 11.3.3.1.3 Stresses & Strains
3. Ductility Ratio $\mu < 1$ O.K.	DG-C1.6.12 P.1
<u>Supporting Members</u>	
<u>(Check Ductility Ratios @ Nodes 2 & 5)</u>	
1. No yielding occurs @ Nodes 2 & 5	Sec 11.3.3.1.3 Stresses & Strains
2. Ductility Ratio $\mu < 1$ O.K.	
<u>The member with PROXIMITY Restraint</u>	
<u>is Qualified</u>	

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Client TVA

Project WBNP Unit 1

Subject Qualif. of Worst Cases of Thermally Restrained Structures - Package I

11.3.5 Connection and Anchorage Qualification for Worst Case 1-27

11.3.5.1 Connection Qualification for Worst Case 1-27

Weld connections at nodes 8 and 10 of ANSYS model are evaluated using the results of the analysis "PR9CM" for the modified structure (Section 11.3.3.2).

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 PROJECT WBNP - UNIT #1
 SUBJECT QUALIF. OF WORST CASES OF THERM. REST. STRUCT. - PACK. 1

NODE 10 & 110 (FROM PRGCMF DATED 8/19/91)

NODE FORCES (ANSYS OUTPUT)

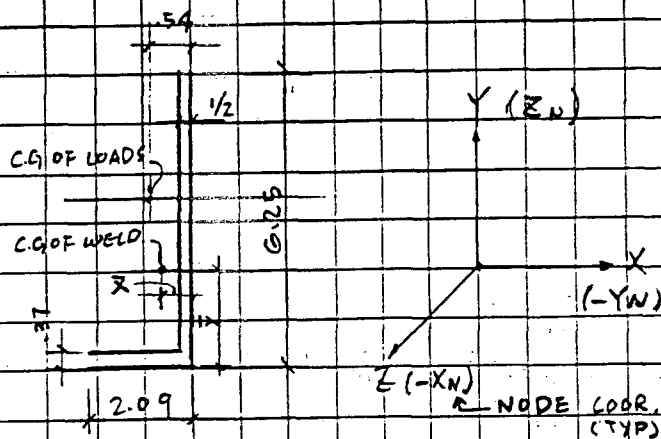
$F_x = -7.52 \text{ K}$ $M_x = -1.26 \text{ ''-K}$
 $F_y = -0.82 \text{ K}$ $M_y = 11.68 \text{ ''-K}$
 $F_z = 0.85 \text{ K}$ $M_z = 11.51 \text{ ''-K}$

LOADS ON PLATE

$F_x = 0.82 \text{ K}$ $M_x = 11.68 \text{ ''-K}$
 $F_y = -0.85 \text{ K}$ $M_y = -11.5 \text{ ''-K}$
 $F_z = -7.52 \text{ K}$ $M_z = -1.26 \text{ ''-K}$

C.G. OF WELDS

WELD SIZE $\frac{3}{16}$ ''



$$\bar{X} = \frac{6.25 \times 0 + 5.88 \times 2 + \frac{2.09^2}{2} + 1.88 \times \left(\frac{1.88}{2} + 0.21 \right)}{6.25 + 5.88 + 2.09 + 1.88}$$

$$= \frac{1.23 + 2.18 + 2.16}{16.1} = 0.35''$$

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BY: SM DATE: 8/20/91 BRANCH PROJECT I.D.: WCG-1-969 SHEET: 111 OF 230
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 CLIENT: TVA
 PROJECT: WBNP - UNIT 1
 SUBJECT: QUALIF OF WORST CASES OF THERM. REST. STRUCTURE - PACKAGE I

NODE

10

$$\bar{y} = \frac{2.09 \times 0 + 1.88 \times 0.37 + \frac{6.25^2}{2} + 5.88 \times \left(\frac{5.88}{2} + 0.37 \right)}{16.10}$$

$$= 2.47''$$

$$e_x = .541 - .35 = .191''$$

$$C_x = 2.47''$$

$$e_y = 3.5 - 2.47 = 1.03'$$

$$C_y = 2.09 - .35 = 1.74''$$

WELD PROPERTIES

$$A_w = 6.25 + 5.88 + 2.09 + 1.88 = 16.1''$$

$$I_x = \frac{6.25^3}{12} + \frac{5.88^3}{12} + 6.25 \times \left(\frac{6.25}{2} - 2.47 \right)^2$$

$$+ 5.88 \times \left(\frac{5.88}{2} - 2.1 \right)^2 + 2.09 \times 2.47^2 + 1.88 \times 2.1^2$$

$$= 20.35 + 16.94 + 2.68 + 4.15 + 12.75 + 8.29$$

$$= 65.16 \text{ IN}^3$$

$$I_y = \frac{2.09^3}{12} + \frac{1.88^3}{12} + 2.09 \times \left(\frac{2.09}{2} - 0.35 \right)^2$$

$$+ 1.88 \times \left(\frac{1.88}{2} - .14 \right)^2 + 6.25 \times .35^2 + 5.88 \times 0.14^2$$

$$= 0.76 + 0.55 + 1.01 + 1.20 + 0.77 + 0.12$$

$$= 4.41 \text{ IN}^3$$

$$J_w = 65.16 + 4.41 = 69.57 \text{ IN}^3$$

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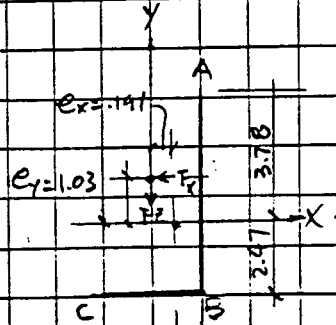
BY SAL DATE 8/20/91 BRANCH PROJECT I.D. : WCG-1-969 SHEET 112 OF 230
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 PROJECT WBNP - UNIT # 1
 SUBJECT QUALIF. OF WORST CASES OF THERM. REST. STRUCT. - PACK. 1

$$S_{x_A} = \frac{65.16}{3.78} = 7.24 \text{ IN}^2$$

$$S_{x_B} = S_{x_C} = \frac{65.16}{2.47} = 26.38 \text{ IN}^2$$

$$S_{y_A} = S_{y_B} = \frac{4.41}{0.35} = 12.60 \text{ IN}^2$$

$$S_{y_C} = \frac{4.41}{1.74} = 2.53 \text{ IN}^2$$



NEGLECT ECCENTRIC MOMENT DUE TO COMP.

$$f_c = \frac{0.82}{16.1} + \frac{11.68}{2638} + \frac{11.51}{2.53} = -5.04 \text{ K/IN}$$

$$M_z = -1.26 + 0.85 \times 1.03 - 0.82 \times 0.191 = -0.54 \text{ IN-K}$$

$$f_x = \frac{0.82}{16.1} + \frac{1.26 \times 2.47}{56.14} = -0.005 \text{ K/IN}$$

$$f_y = \frac{0.85}{16.1} + \frac{1.26 \times 1.74}{56.14} = -0.01 \text{ K/IN}$$

$$f_r = \left(5.04^2 + 0.005^2 + 0.01^2 \right)^{1/2} = 5.04 \text{ K/IN}$$

$$\text{WELD CAPACITY} = 6.19 \text{ K/IN} > 5.04 \text{ K/IN}$$

O.K

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BY SKW DATE 8/20/91 BRANCH PROJECT I.D.: WCG-1-969 SHEET 113 OF 230
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 CLIENT TVA
 PROJECT WBNP - UNIT #1
 SUBJECT QUALIF. OF WORST CASES OF THERMALLY REST. STRUCTURE - PACK. 1

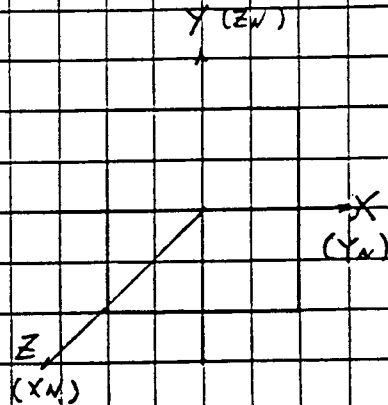
NODE 8 & 108 (RESULTS FROM PR9CMR DATED 8/19/91)

NODE FORCES (FROM ANSYS OUTPUT)

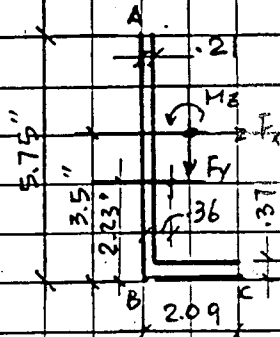
$$\begin{aligned} F_x &= 0.0 \text{ K} & M_x &= -1.02 \text{ ''-K} \\ F_y &= -0.48 \text{ K} & M_y &= 0.74 \text{ ''-K} \\ F_z &= 0.18 \text{ K} & M_z &= 0.0 \text{ ''-K} \end{aligned}$$

LOADS ON BASE PLATES

$$\begin{aligned} F_x &= 0.48 \text{ K} & M_x &= -0.74 \text{ ''-K} \\ F_y &= 0.18 \text{ K} & M_y &= 0.0 \\ F_z &= 0.0 & M_z &= 1.02 \text{ ''-K} \end{aligned}$$



C.G. OF WELDS



$$\begin{aligned} C_x &= 2.23'' \\ C_y &= 2.09 - 0.36 \\ &= 1.73'' \end{aligned}$$

$$\bar{X} = \frac{5.75 \times 0 + 5.38 \times 0.2 + \frac{2.09^2}{2} + (2.09 - 0.21) \left(\frac{1.88}{2} + 0.21 \right)}{5.75 + 5.38 + 2.09 + (2.09 - 0.21)} = 0.36''$$

$$\bar{Y} = \frac{2.09 \times 0 + (2.09 - 0.21) \times 0.37 + \frac{5.75^2}{2} + 5.38 \left(\frac{5.38}{2} + 0.37 \right)}{15.1} = 2.23''$$

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PROJECT WBNP - UNIT 1

SUBJECT QUALIF. OF WORST CASES OF THERMALLY RESTRAINED STRUCT. - PACK. 1

WELD PROPERTIES

$$A_w = 5.75 + 5.38 + 2.09 + (2.09 - 0.21) = 15.1''$$

$$I_x = \frac{5.75^3}{12} + \frac{5.38^3}{12} + 5.75 \times \left(\frac{5.75}{2} - 2.23\right)^2 + 5.38 \left(\frac{5.38}{2} - 1.86\right)^2$$

$$+ 2.09 \times 2.23^2 + (2.09 - 0.21) \times 1.86^2$$

$$= 15.84 + 12.98 + 2.39 + 3.71 + 10.39 + 6.50$$

$$= 51.81 \text{ in}^3$$

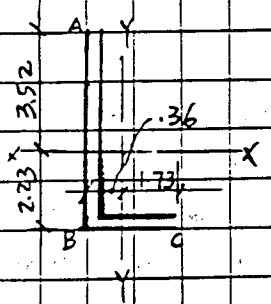
$$I_y = \frac{2.09^3}{12} + \frac{1.88^3}{12} + 2.09 \times \left(\frac{2.09}{2} - 0.36\right)^2$$

$$+ 1.88 \times \left(\frac{1.88}{2} - 0.15\right)^2 + 5.75 \times 0.36^2 + 5.38 \times 0.15^2$$

$$= 0.76 + 0.55 + 0.98 + 1.17 + 0.75 + 0.12$$

$$= 4.33 \text{ in}^3$$

$$J_w = I_x + I_y = 51.81 + 4.33 = 56.14 \text{ in}^3$$



$$S_{x_A} = \frac{51.81}{3.52} = 14.72 \text{ in}^2$$

$$S_{x_B} = \frac{51.81}{2.23} = 23.23 \text{ in}^2 (= S_{x_C})$$

$$S_{y_A} = S_{y_B} = \frac{4.33}{0.36} = 12.03 \text{ in}^2$$

$$S_{y_C} = \frac{4.33}{1.73} = 2.50 \text{ in}^2$$

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 PROJECT WBNP
 SUBJECT QUALIF. OF WORST CASES OF THERM. REST. STRUCT. - PACK. 1

CHECK WELDS NODE B

$$e_x = .541 - .36 = .181$$

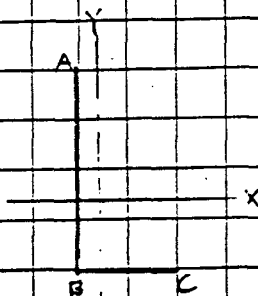
$$e_y = 3.5 - 2.23 = 1.27$$

$$M_z = 1.49 + F_y \cdot e_x + F_x \cdot e_y$$

$$= 1.49 - 0.31 \times 0.181 - 0.51 \times 1.27 = 0.79 \text{ "K}$$

$$f_c = \frac{F_z}{A_w} + \frac{M_x}{S_x} + \frac{M_y}{S_y}$$

$$= 0 + \frac{0.74}{23.23} + 0 = 0.03 \text{ K/IN}$$



$$f_x = \frac{F_x}{A_w} + \frac{M_z \cdot C_x}{J_w}$$

$$= \frac{0.48}{15.1} + \frac{1.02 \times 2.23}{56.14} = 0.07 \text{ K/IN}$$

$$f_y = \frac{F_y}{A_w} + \frac{M_z \cdot C_y}{J_w}$$

$$= \frac{0.18}{15.1} + \frac{1.02 \times 1.73}{56.14} = 0.04 \text{ K/IN}$$

$$f_r = \left(0.03^2 + 0.07^2 + 0.04^2 \right)^{1/2} = 0.09 \text{ K/IN}$$

$$\text{WELD CAPACITY} = .707 \times \frac{2}{3} \times 70 \times \frac{3}{16}$$

$$= 6.19 \text{ K/IN} > 0.09 \text{ K/IN} \quad \text{O.K.}$$

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Client TVA

Project WBNP Unit 1

Subject Qualif. of Worst Cases of Thermally Restrained Structures - Package I

11.3.5.2 Anchorage Qualification for Worst Case 1-27

Concrete anchorages at node 8 and 10 of ANSYS model are evaluated using the the results of the analysis "PR9CM" for the modified structure (Section 11.3.3.2).

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PROJECT WBNP- Unit I
 SUBJECT Qualit. of Worst Cases of Thermally Rest. Struc. - Package I

	References.
<u>SHEAR</u>	
	<u>PG-C1.6.12.R1</u> <u>C3.0</u>
<u>Shear Loading evaluation on Anchor bolts</u> <u>@ Node 8 (welded stud 1/2" φ)</u>	
$U_y = 0.0001" < 0.2 D = 0.2 \times 1/2"$ $= 0.1"$	<u>Sec 11.3.3.2.2</u> <u>Displ.</u>
<u>O.K.</u>	
<u>Shear Loading evaluation on Anchor bolts</u> <u>@ Node 10 (SSD 3/4" φ)</u>	
$U_y = 0.0002" < 0.1 D = 0.1 \times 3/4"$ $= 0.075"$	<u>Sec 11.3.3.2.2</u> <u>Displ.</u>
<u>O.K.</u>	
<u>TENSION</u>	
<u>The Baseplate models used to calculate</u> <u>Rotational Stiffness at Nodes 8 & 10</u> <u>(as shown in Figures 11.3.3 & 11.3.4)</u> <u>are used to perform BASEPLATE II</u> <u>analysis to obtain Tension forces</u> <u>at anchor bolts.</u>	

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 PROJECT: WBNP- UNIT 1
 SUBJECT: Qualif. of worst cases of Therm. Rest. struct. - Package I

	REF.
The BASEPLATE Input & Output are listed	DG-C1.6-12 C.2.3.1
in Tables 11.3.5.2.1 & 11.3.5.2.2 for Nodes B & 10 respectively.	
The forces applied to Base R _e Models are from	
the reactions from the analysis "PRGCM" (for the modified structure)	Sec 11.3.3.2.1 Reaction
<u>Node 8</u> (1 1/2" φ Welded Studs)	
Max. Tension on bolt = 0.06 Kip	Table 11.3.5.2.1
0.06 K < T ₀ = 4.75 K (allowable) D.K.	DS-C1.7.1 Eq. 3.3.4
∴ The Anchorage at Node 8 is Qualified.	
<u>Node 10</u> (1/2" φ SSD's)	
Max. Tension on bolt = 1.148 Kip	Table 11.3.5.2.2
1.148 K < T ₀ = 1.55 K (allowable) D.K.	DS-C1.7.1 Table 4
A CONAN Run is performed to check the concrete pull-out capacity for Node 10 due to G-32 Violation.	WCG-1-805
The CONAN Results indicate no pull-out problem at Node 10.	Table 11.3.5.2.3
∴ The Anchorage at Node 10 is Qualified.	

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Project WBNP Unit 1
 Subject Qualif. of Worst Cases of Thermally Restrained Structures - Package I

Table 11.3.5.2.1 BASEPLATE II Input and Output for Anchorage at Nodes 8

(Ref. Input = BBPR9C ; output = BBPR9CR)

```

1 3 BBPR9C-NODE 8; EMB PLATE; 48N968.
2 OUT ,,-1,100/
3 CON ,,,3000,1.7,1.7/
4 PLA 10,10,,8,8,.5,29/
5 APR 6,2,1,5.75,2.09,.366,.21/
6 BPR ,,1,2.46E5,1.0E6,4750,4318/
7 BOL 3,3,1,1.5,1.5/
8 BOL 8,3,1,6.5,1.5/
9 BOL 3,8,1,1.5,6.5/
10 BOL 8,8,1,6.5,6.5/
11 END /
12 ATT 5,4,,3.75,2.25/
13 JST 5,4,7,2.09,0/
14 IST 5,4,10,5.75,90/
15 END /
16 END /
17 POI 5,4,,.5,3.5/
18 LOA ,,1,479,-178,0,-739,0,1017/
19 END /
20 END OF JOB
21
    
```

Results from Output

```

*****
*
* BBPR9C-NODE 8; EMB PLATE; 48N968.
*
*****
    
```

B O L T L O A D S

I	J					
LINE	LINE	NODE	X-SHEAR	Y-SHEAR	SRSS	Z-TENSION
3	3	23	125.	-49.	134.	60.
8	3	73	130.	-41.	136.	51.
3	8	28	113.	-52.	125.	14.
8	8	78	111.	-37.	117.	0.

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Table 11.3.5.2.2 BASEPLATE II Input and Output for Anchorage at Nodes 10

(Ref: Input = B10PRAC ; Output = B10PACR)

```

1 3 B10PR9C-NODE 10; SURF MOUNT PLATE; 48N968.
2 OUT , , , -1,100/
3 CON , , , ,3000,1.7,1.7/
4 PLA 11,11, ,9,10, .75,29/
5 APR 7,2,1,6.63,2.09, .366, .21/
6 BPR , ,1, .4E6,1.0E6,1550,1400/
7 BOL 3,9,1,1.5,8.5/
8 BOL 9,9,1,7.5,8.5/
9 IWE 1,1,4/
10 JWE 1,1,11/
11 IWE 11,1,4/
12 END /
13 ATT 4,4, ,2.29,3.37/
14 JST 4,4,6,2.09,0/
15 IST 6,4,11,6.63,90/
16 END /
17 END /
18 POI 4,4, ,1.58,3.5/
19 LOA , ,1, -821, -852,0,11675, -11514,1258/
20 END /
21 END OF JOB
    
```

Results from Output

BOLT LOADS						
I	J					
LINE	LINE	NODE	X-SHEAR	Y-SHEAR	SRSS	Z-TENSION
3	9	31	-64.	-28.	70.	636.
9	9	97	-65.	10.	66.	1148.

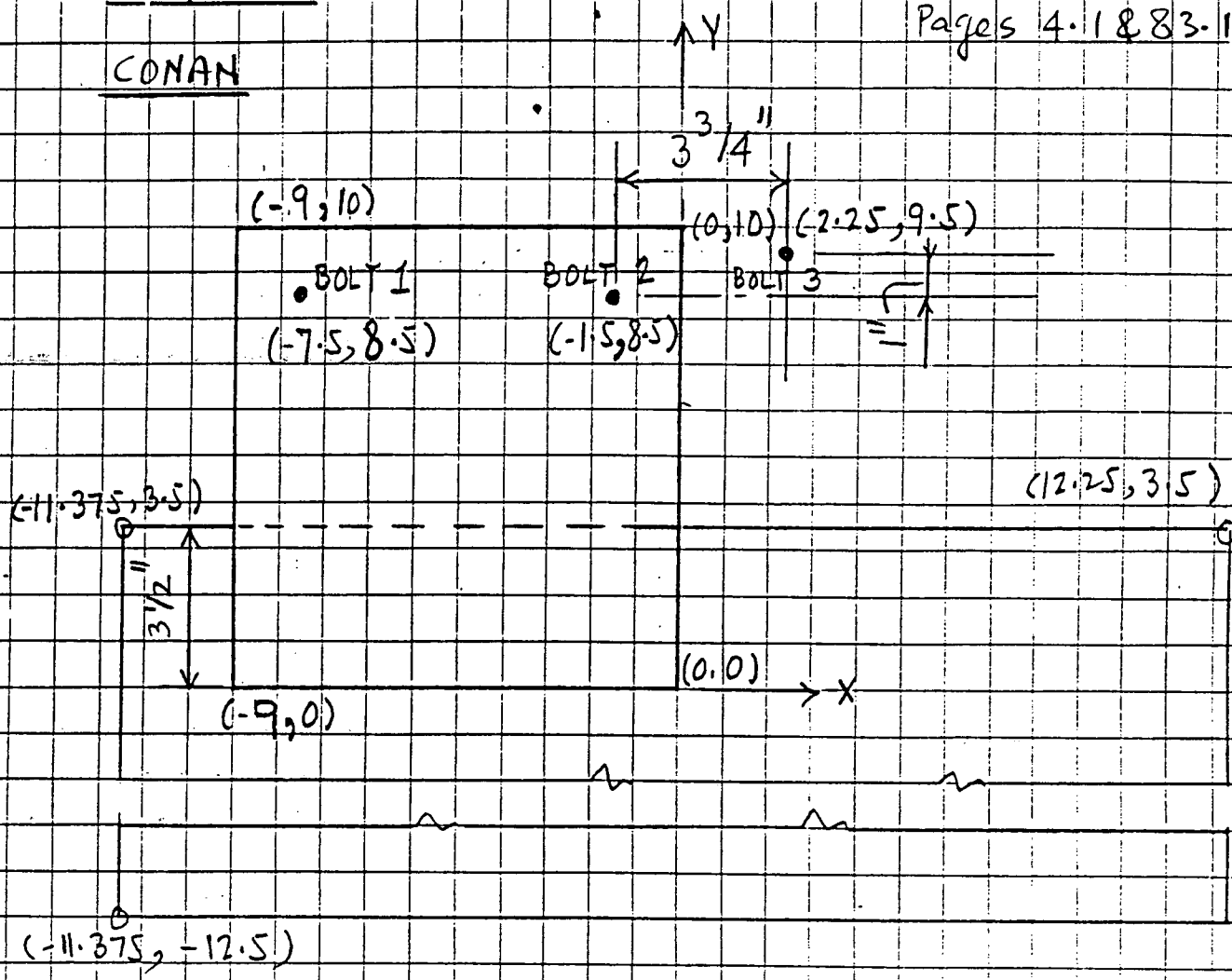
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BY TAK DATE 7-30-91 BRANCH PROJECT I.D. WCG-1-969 SHEET 121 OF 230
 CHKD. BY JOU DATE 8-22-91 OFS NO. NR DEPT. NO. NR
 CLIENT TVA
 PROJECT WBNP- UNIT 1
 SUBJECT Base Plates

REF.
 WCG-1-825
 Pages 4.1 & 83.1

Node 10

CONAN



The input and output of the CONAN analysis
 are shown in Table H.3.5.2.3.

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Client TVA

Project WBNP Unit 1

Subject Qualif. of Worst Cases of Thermally Restrained Structures - Package I

Table 11.3.5.2.3 CONAN Input and Output for Anchorage at Nodes 10

(Ref = Input = C10PR9C ; Output = C10P9CR)

C10PR9C-NODE 10; CONAN FOR EMB. PLATE; 48N968.

BPROP

1,SD,0.5,,,A

BDATA

-7.5,8.5,1,0.636,U

-1.5,8.5,1,1.148,U

2.25,9.5,1,,U

EPDATA

12.25,3.5,-11.38,3.5,-11.38,-12.5,1.5

CONAN OUTPUT REPORT

FOR C10PR9C-NODE 10; CONAN FOR EMB. PLATE; 48N968.

LOAD CASE NO. 1

BOLT	MAX ALLOW TEN LOAD	ALLOWABLE LOAD	APPLIED LOAD	ALLOW/APPLIED LOAD	PULLOUT AREA
1	1.55	1.55	.64	2.44	26.55
2	1.55	1.36	1.15	1.19	20.72
3	1.55	1.55	1.55	1.00	26.55

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11.3.6 Connection and Anchorage Qualification for Worst Case 2&3-9(C)

11.3.6.1 Connection Qualification for Worst Case 2&3-9(C)

Weld connections at nodes 1, 4, 2 and 5 of ANSYS Model are evaluated using the results of the analysis "PR9C" for the original structure (Section 11.3.3.1).

Since the weld connections at nodes 1, 4, 2 and 5 are far away from the region which modification is made for worst case 1-27; these connections are not re-evaluated using the results from the analysis "PR9CM" for the modified structure.

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BY SRL DATE 2/20/91 BRANCH PROJECT I.D.: WCG-1-969 SHEET 124 OF 230
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CLIENT TVA
 PROJECT WBNP - UNIT # 1
 SUBJECT QUALIF. OF WORST CASES OF THERM. REST. STRUCT - PACK. 1

NODE 1, 4, 11 (FROM PRGR DATED 8/19/91)

NODE FORCES (FROM ANSYS OUTPUT)

NODE 1, 101

$F_x = 1.65 \text{ K}$ $M_x = 4.15 \text{ ''-K}$
 $F_y = -4.35 \text{ K}$ $M_y = -0.25 \text{ ''-K}$
 $F_z = 0.26 \text{ K}$ $M_z = -13.51 \text{ ''-K}$

NODE 4, 104

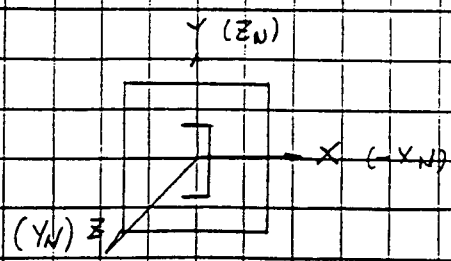
$F_x = -0.47 \text{ K}$ $M_x = 3.72 \text{ ''-K}$
 $F_y = 1.74 \text{ K}$ $M_y = -0.27 \text{ ''-K}$
 $F_z = 0.22$ $M_z = -6.11 \text{ ''-K}$

NODE 11, 111

$F_x = 0.19 \text{ K}$ $M_x = 2.20 \text{ ''-K}$
 $F_y = -0.04 \text{ K}$ $M_y = 0.05 \text{ ''-K}$
 $F_z = 0.07 \text{ K}$ $M_z = -3.07 \text{ ''-K}$

BY COMPARING THE ABOVE FORCES, LOADS ON NODE 1 GOVERNED, SINCE NODE 1, 4 11 HAD SAME WELDING DETAIL AND PROPERTIES AS WELL AS LOADING DT. ONLY WELD AT NODE 1 WOULD BE CHECKED.

LOAD ON PLATES



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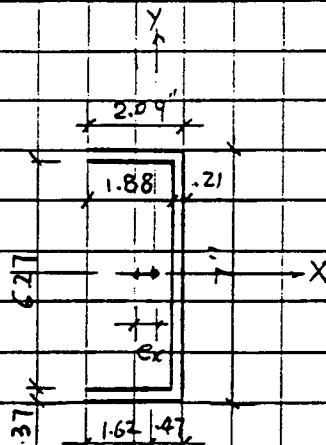
SUBJECT QUALIF. OF WORST CASES OF THERMAL REST. STRUCT. - PACK. 1

NODE 1, 10

LOAD ON PLATE

$$\begin{aligned} F_x &= 11.65 \text{ K} & M_x &= 4.15 \text{ ''-K} \\ F_y &= -0.26 \text{ K} & M_y &= 13.51 \text{ ''-K} \\ F_z &= 4.35 \text{ K} & M_z &= 0.25 \text{ ''-K} \end{aligned}$$

WELD PROPERTIES (size $\frac{3}{16}$)



$$A_w = 2 \times 2.09 + 2 \times 1.88 + 7.0 + 6.27$$

$$= 21.21 \text{ IN}$$

$$\bar{Y} = 3.50 \text{ ''}$$

$$\bar{X} = \frac{2.09 \times \frac{2.09}{2} \times 2 + 1.88 \times \left(\frac{1.88}{2} + 0.21\right) \times 2 + 6.27 \times 0.21}{21.21}$$

$$= \frac{4.37 + 4.32 + 1.32}{21.21} = 0.47 \text{ ''} \quad e_x = 5.41 - 0.47 = 5.07 \text{ ''}$$

$$I_x = \frac{7^3}{12} + \frac{6.27^3}{12} + 2 \times 2.09 \times 3.5^2 + 2 \times 1.88 \times (3.5 - 0.37)^2$$

$$= 28.58 + 20.54 + 51.21 + 36.84$$

$$= 137.17 \text{ IN}^3$$

$$I_y = 2 \times \frac{2.09^3}{12} + 2 \times \frac{1.88^3}{12} + 2 \times 2.09 \times \left(\frac{2.09}{2} - 0.47\right)^2$$

$$+ 2 \times 1.88 \times \left(\frac{1.88}{2} + 0.21 - 0.47\right)^2 + 7 \times 0.47^2 + 6.27 \times 0.26^2$$

$$= 1.52 + 1.11 + 1.38 + 1.74 + 1.55 + 0.42 = 7.72 \text{ IN}^3$$

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$J_w = I_x + I_y = 137.17 + 7.72 = 144.89 \text{ IN}^3$			
$S_x = \frac{137.17}{3.5} = 39.19 \text{ IN}^2$	$C_h = 3.5"$		
$S_y = \frac{7.72}{1.62} = 4.77 \text{ IN}^2$	$C_v = 2.09 - 0.47 = 1.62"$		
$r = \frac{4.35}{21.21} + \frac{4.15}{39.19} + \frac{13.51}{4.77} = 3.14 \text{ K/IN}$			
$r_x = \frac{1.65}{21.21} + \frac{0.25 \times 3.5}{144.89} = 0.08 \text{ K/IN}$			
$r_y = \frac{0.26}{21.21} + \frac{0.25 \times 1.62}{144.89} = 0.05 \text{ K/IN}$			
$r = \left(3.14^2 + 0.08^2 + 0.05^2 \right)^{1/2} = 3.14 \text{ K/IN}$			
WELD CAPACITY = $0.707 \times \frac{2}{3} \times 70 \times \frac{3}{16}$			
$= 6.19 \text{ K/IN} > 3.14 \text{ K/IN} \text{ O.K.}$			

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CHECK CONNECTION @ NODES 2 & 5

LOADS @ NODE 102 & 105 (FROM ANSYS OUTPUT)

NODE 102 (ELEMENT 7)

$$\begin{aligned} F_x &= 1.15 \text{ K} & M_x &= 0.09 \text{ ''-K} \\ F_y &= -0.05 \text{ K} & M_y &= -0.22 \text{ ''-K} \\ F_z &= 0.08 \text{ K} & M_z &= -0.0 \text{ ''-K} \end{aligned}$$

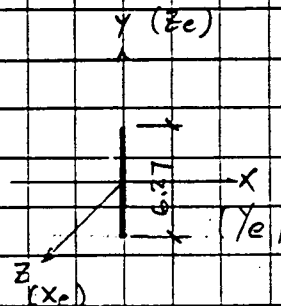
NODE 105 (ELEMENT 15)

$$\begin{aligned} F_x &= -1.15 \text{ K} & M_x &= 0.09 \text{ ''-K} \\ F_y &= -0.05 \text{ K} & M_y &= -0.12 \text{ ''-K} \\ F_z &= 0.06 \text{ K} & M_z &= -0.0 \text{ ''-K} \end{aligned}$$

CHECK CONNECTION FOR NODE 102 WHICH GOVERNS

FORCES ON WELDS

$$\begin{aligned} F_x &= 0.05 \text{ K} & M_x &= 0.22 \text{ ''-K} \\ F_y &= -0.08 \text{ K} & M_y &= 0.0 \\ F_z &= -1.15 \text{ K} & M_z &= -0.09 \text{ ''-K} \end{aligned}$$



WELD PROPERTIES

$$A_w = 6.27 \text{ ''} \quad S_x = \frac{6.27^2}{6} = 6.55 \text{ in}^2 \quad S_y = 0$$

$$J_w = I_x + I_y = \frac{6.27^3}{12} + 0 = 20.54 \text{ in}^3$$

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$$f_t = \frac{F_z}{A_w} + \frac{M_x}{S_x} + \frac{M_y}{S_y} = \frac{1.15}{6.27} - \frac{0.22}{6.55} + 0 = -0.22 \text{ K/IN}$$

$$f_x = \frac{F_x}{A_w} + \frac{M_z C_h}{J_w} = \frac{0.05}{6.27} + \frac{0.09 \times 3.135}{20.54} = 0.02 \text{ K/IN}$$

$$f_y = \frac{F_y}{A_w} + \frac{M_z C_v}{J_w} = \frac{0.08}{6.27} + 0 = 0.01 \text{ K/IN}$$

$$f_r = \left(0.22^2 + 0.02^2 + 0.01^2 \right)^{1/2} = 0.22 \text{ K/IN}$$

$$\text{WELD CAPACITY} = .707 \times \frac{2}{8} \times 70 \times \frac{3}{16}$$

$$= 6.19 \text{ K/IN} > 0.22 \text{ K/IN}$$

O.K.

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11.3.6.2 Anchorage Qualification for Worst Case 2&3-9(C)

Concrete anchorages at nodes 1 and 4 of ANSYS model are evaluated using the results of the analysis "PR9C" for the original structure (Section 11.3.3.1).

Since nodes 1 and 4 are far away from the region which modification is made for worst case 1-27, the anchorage at nodes 1 and 4 are not re-evaluated using the results from the analysis "PR9CM" for the modified structure.

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	References
<u>SHEAR</u>	
Shear evaluation for anchor bolts @ Nodes 1 and 4 (550 $\frac{3}{4}$ " ϕ)	PG-C1.6.12 R1 C.3.0
$u_y = 0.03" < 0.1D = 0.1 \times \frac{3}{4}$ $= 0.075"$	Sec 11.3.3.2 Displ.
O.K.	
<u>TENSION</u>	
The baseplate model used to calculate Rotational stiffness at nodes 1 & 4 (as shown in Figure 11.3.2) is used to perform BASEPLATE II analysis to obtain Tension forces at the anchor bolts.	
The Input and Output of the BASEPLATE II analysis is listed in Table 11.3.6.2.1	

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	Ref.
Tension Forces of Anchor Bolts are obtained from BASEPLAT output as shown in Table 11.3.6.2.1.	DS-C1.6.12 R1 C1.3.1
Nodes 1 & 4 3/4" ϕ SSD's	
MAX tension on anchor = 2428 #	Table 11.3.6.2.1
$T_0 = 3.0^k > 2.428^k$	DS-C1.7.1 Table 4
Bolts are O.K.	
Since there is G-32 Violation at Node 4, a CONAN evaluation is performed to check the pull-out capacity.	
The Input and Output of CONAN run are listed in Table 11.3.6.2.2.	
The CONAN Results indicate no problem on Concrete Pull-out.	Table 11.3.6.2.2
\therefore The Anchorage at Nodes 1 and 4 are qualified.	

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Table 11.3.6.2.1 BASEPLATE II Input and Output for Anchorage at Nodes 1 and 4

(Ref. Input = BIPR9C; Output = BIPR9R)

```

1 3 BIPR9C-NODES 1&4 ; SURF MOUNT PLATE; 48N968.
2 OUT ,,-1,100/
3 COM ,,,3000,1.7,1.7/
4 PLA 11,12,,11,11,1.0,29/
5 APR 7,2,1,7.0,2.09,.366,.21/
6 BPR ,,1,.4E6,1.0E6,3000,3300/
7 BOL 3,3,1,2,2/
8 BOL 10,3,1,9,2/
9 BOL 3,10,1,2,9/
10 BOL 10,10,1,9,9/
11 END /
12 ATT 5,3,,3.41,2.00/
13 JST 5,3,7,2.09,0/
14 IST 7,3,10,7.0,90/
15 JST 7,10,5,2.09,180/
16 END /
17 END /
18 POI 5,3,,1.55,3.5/
19 LOA ,,1,1650,255,4348,4147,13514,246/
20 LOA ,,2,-470,-221,0,3721,6112,266/
21 END /
22 END OF JOB
    
```

BOLT LOADS (for Node 1)

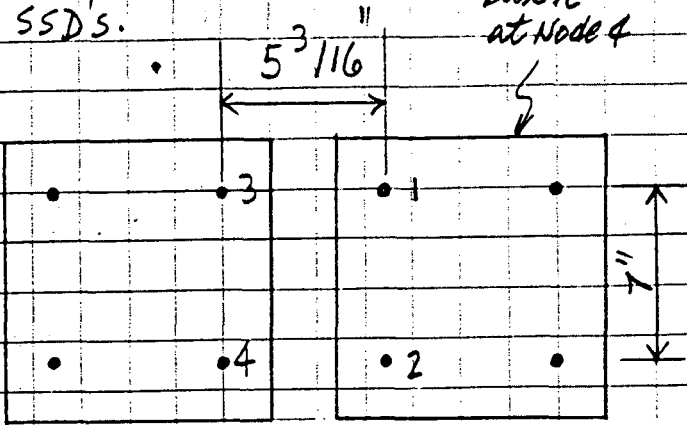
I LINE	J LINE	NODE	X-SHEAR	Y-SHEAR	SRSS	Z-TENSION
3	3	27	420.	62.	425.	1930.
10	3	111	413.	65.	418.	203.
3	10	34	410.	59.	414.	2428.
10	10	118	406.	70.	412.	486.

BOLT LOADS (for Node 4)

I LINE	J LINE	NODE	X-SHEAR	Y-SHEAR	SRSS	Z-TENSION
3	3	27	-105.	-70.	126.	309.
10	3	111	-103.	-41.	111.	0.
3	10	34	-132.	-69.	148.	566.
10	10	118	-131.	-42.	137.	53.

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	REF.
Node 4, PR9C CONAN	WCG-1-825
<p>Anchors - $3/4"$ ϕ SSD's.</p> 	Pages 4.1, 13.1 & 23.1
Load on anchor #1 = 309 #	Tab. 11.3.6.2.1
Load on anchor #2 = 1566 #	

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Table 11.3.6.2.2 CONAN Input and Output for Anchorage at Nodes 4

(Ref: Input = CAPRAC ; Output = CAPRACR)

C4PR9C-NODE 4; CONAN FOR EMB. PLATE; 48N968.

BPROP

1,SD,0.75,,,A

BDATA

0,0,1,,U

5.375,0,1,..309,U

0,7,1,,U

5.375,7,1,..566,U

CONAN OUTPUT REPORT

FOR C4PR9C-NODE 4; CONAN FOR EMB. PLATE; 48N968.

LOAD CASE NO. 1

BOLT	MAX ALLOW TEN LOAD	ALLOWABLE LOAD	APPLIED LOAD	ALLOW/APPLIED LOAD	PULLOUT AREA
1	3.00	3.00	3.00	1.00	49.08
2	3.00	2.51	.31	8.13	38.23
3	3.00	3.00	3.00	1.00	49.08
4	3.00	2.51	.57	4.44	38.23

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By SK Date 8/10/91Sheet 135 of 230Chkd. by JOU Date 8/22/91OFS No. NR Dept. No. NRClient TVAProject WBNP Unit 1Subject Qualif. of Worst Cases of Thermally Restrained Structures - Package I**11.3.7 Modification**

A modification is required for the worst case 1-27 (member with Axially Restraint) per Section 11.3.4.1. The existing design of the member is not accepted because the ductility ratio of the member is greater than allowable (3). A slotted hole connection will be implemented at node 8 to release the axial load due the accident thermal condition per DCN M-16250-A. A second analysis "PR9CM" (See Section 11.3.2.2 for Input listing) is performed using the Response Spectrum Set B+C. The results of this analysis is shown in Section 11.3.3.2. Member, connections and Anchorage are reevaluated and qualified as shown in Sections 11.3.4 and 11.3.5.

In addition, two new STRUDL runs are performed in Calculation WCG-2-50, Vol. 6, R1, (Evaluation of Pressurizer Enclosure Platform @ EL. 783'-8 1/2") to investigate the effect of this modification. The impact of the modification is assessed mainly by comparing results of the analyses that are performed before and after the slotted hole connections incorporated into the computer model. Comparisons are performed for the following:

- * Natural frequencies of the platform
- * Member stress interaction ratios
- * Maximum torsional moments on the members
- * Forces and moments used in steel to steel connection checks
- * Forces and moments used in surface mounted plate checks

The results of the comparison indicate that the differences due to the modification is negligible. For details, see WCG-2-50 R1.

The design of the slotted connection is presented as follows:

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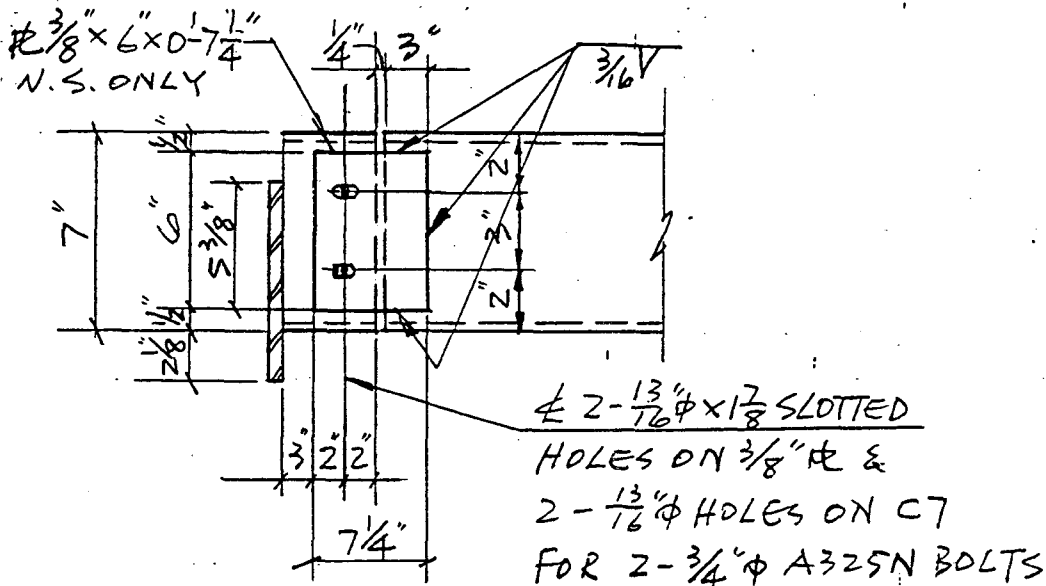
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Design of the slotted Hole Connection @ Node 8

REF

In order to qualify the worst case member 1-27, the connection at Node 8 has to be modified as per sketch shown below:



Design load : $D + L / OBE$

$F_y = 0.104^k$ $F_z = 0.048^k$ $M_x = 1.052^k$

Modification factor :

Ratio = $\frac{\text{Spectra B+C}}{\text{Spectra B}} = \frac{9.58}{3.75} = 2.55$

NOTE: Design load is obtained from calc. WCG-2-50, Vol. 6, R1
Attach. K, Pg. 26

SH. 313
of
WCG-2-50
Vol. 6, R1

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Connection design: Try $\frac{3}{8} \times 6 \times 7\frac{1}{4}$

Torsional constant $J = \frac{1}{3} \times 6 \times 0.375^3 = 0.106 \text{ in}^4$

Shear stress: $M_x' = 1.052 + 0.104 \times 0.541 = 1.11 \text{ K}$

$V_x = \frac{0.048}{0.375 \times 6} = 0.21 \text{ KSI}$

$V_y = \frac{0.104}{0.375 \times 6} + \frac{1.11 \times 0.375}{0.106} = 3.97 \text{ KSI}$

$\therefore V = 2.55 \sqrt{0.21^2 + 3.97^2}$
 $= 10.13 \text{ KSI} < 0.4 \times 36 = 14.4 \text{ KSI} \quad \text{O.K.}$

Check block shear: $\frac{3}{8}$ thick

$F_v = 0.3 F_u = 0.3 \times 58 = 17.4 \text{ KSI}$

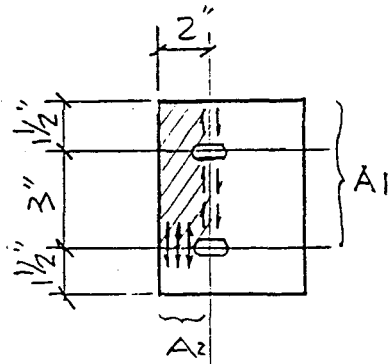
$F_t = 0.5 F_u = 0.5 \times 58 = 29 \text{ KSI}$

$A_1 = 0.375 \times (4.5 - 1.5 \times \frac{13}{16}) = 1.23 \text{ in}^2$

$A_2 = 0.375 \times (2 - 1.875 \times \frac{1}{2}) = 0.4 \text{ in}^2$

$R_y = 17.4 \times 1.23 + 29 \times 0.4$

$= 33 \text{ K} > 2.55 F_y = 2.55 \times 0.104 = 0.27 \text{ K} \quad \text{O.K.}$



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check bending of connection #

$$M_x = 0.104 \times 2.25 = 0.234 \text{ K"}$$

$$M_y = 0.048 \times 2.25 = 0.108 \text{ K"}$$

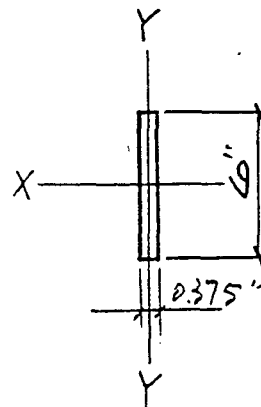
$$S_x = \frac{1}{6} \times 0.375 \times 6^2 = 2.25 \text{ in}^3$$

$$S_y = \frac{1}{6} \times 6 \times 0.375^2 = 0.14 \text{ in}^3$$

$$f_{bx} = \frac{0.234}{2.25} = 0.104 \text{ ksi}$$

$$f_{by} = \frac{0.108}{0.14} = 0.77 \text{ ksi}$$

$$\left(\frac{0.104}{21.6} + \frac{0.77}{27} \right) \times 2.55 = 0.085 < 1 \text{ O.K.}$$



Weld of connection # to C7x9.8

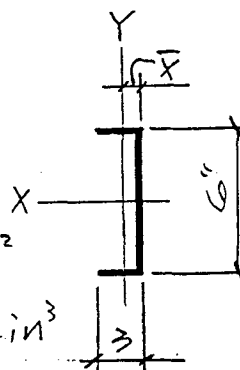
$$A = 3 \times 2 + 6 = 12 \text{ in}^2$$

$$\bar{X} = \frac{1}{12} \times 3 \times 2 \times 1.5 = 0.75 \text{ in}$$

$$I_x = 3 \times 2 \times 3^2 + \frac{1}{12} \times 6^3 = 72 \text{ in}^4$$

$$I_y = 6 \times 0.75^3 + \frac{1}{12} \times 3^3 \times 2 + 2 \times 3 \times 0.75^2 = 10.4 \text{ in}^4$$

$$I_z = 72 + 10.4 = 82.4 \text{ in}^4$$



Forces on weld

$$F_y = 0.104 \text{ K} \quad M_x = 1.052 \quad F_z = 0.048 \text{ K}$$

$$M_y = 0.048 \times (5.25 - 0.75) = 0.216 \text{ K"}$$

$$M_z = 0.104 \times (5.25 - 0.75) = 0.47 \text{ K"}$$

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$$f_{zt} = \frac{0.048}{12} + \frac{1.052 \times 3}{72} + \frac{0.216 \times 2.25}{10.4} = 0.095 \%$$

REF

$$f_{yv} = \frac{0.104}{12} + \frac{0.47 \times 2.25}{82.4} = 0.022 \%$$

$$f_{xv} = \frac{0.47 \times 3}{82.4} = 0.017 \%$$

$$f_r = 2.55 \sqrt{0.095^2 + 0.022^2 + 0.017^2} = 0.252 \%$$

$$W_{req'd} = \frac{0.252}{0.707 \times 21} = 0.017''$$

$$\text{Min. F.W. req'd} = \frac{3}{16}'' > 0.017''$$

\(\therefore\) use $\frac{3}{16}''$ F.W. as shown above

Design of connection bolts: 2- $\frac{3}{4}''$ ϕ A325N

$$f_t = \frac{0.048}{2} + \frac{1.11}{3} = 0.394 \text{ K/bolt}$$

$$v_y = \frac{0.104}{2} = 0.052 \text{ K/Bolt}$$

$$v_x = \frac{0.104 \times (5.25 - 0.75)}{3} = 0.156 \text{ K/bolt}$$

Allow. Single shear = 6.83 K for $t = \frac{3}{16}''$

Allow. Tension = 40 \times 0.3345 = 13.38 K

$$v = \sqrt{0.052^2 + 0.156^2} \times 2.55 = 0.42 \text{ K} < 6.83 \text{ K}$$

$$f_u = \frac{0.42}{0.4418} = 0.95 \text{ KSI}$$

$$F_t = 50 - 1.6 f_u = 50 - 1.6 \times 0.95 = 48.5 \text{ KSI} > 40 \text{ KSI}$$

$$\text{USE } F_t = 40 \text{ KSI} > \frac{0.394}{0.3345} = 1.18 \text{ KSI O.K.}$$

USE 2- $\frac{3}{4}''$ ϕ A325N Bolts

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11.4 Qualification of Worst Case 1-11

11.4.1 Mathematical Model (AX11)

The ANSYS model AX11 is shown in Figure 11.4.1.1. This model contains two members (C8x11.5) with Axial Restraint. These two members are modeled with ANSYS STIF24 elements (3-D thin-wall plastic beam elements) and the interconnecting members are modeled with STIF4 beam elements. STIF21 mass elements are used to simulate the weight of conduits at nodes 303 and 309 which are located under nodes 3 and 9 and connected to nodes 3 and 9 with rigid links. Temperature dependent material properties, as described in Section 11.2, are used.

Boundary Condition

All support nodes (nodes 1, 4, 7 and 10) are modeled as pinned connections. A (1/32") displacement is specified at nodes 4 and 10 in the +X direction and at nodes 1 and 7 in the -X direction at the third load step when temperature is 150°F. This is equivalent to specify Gap elements to allow the 1/32" free movement. A study run is performed by using gap elements (STIF40) at nodes 4 and 10 with gaps equal to 1/16" (1/32" * 2). The final results of the analysis using gap elements are identical to the results of the analysis using imposed displacements. The input file and the results (reactions and displacements at final load step) are provided in Attachment C.

Loading Condition

Equivalent densities are calculated for each member to consider the dead loads (including self-weight and grating) and the sustained lived loads. The 1 1/2 " grating weight is considered 12 psf and sustained live loads, 10 psf, based on WB-DC-20-21, Section 3.1.1. The grating and sustained live loads for members between nodes 1 and 4, and

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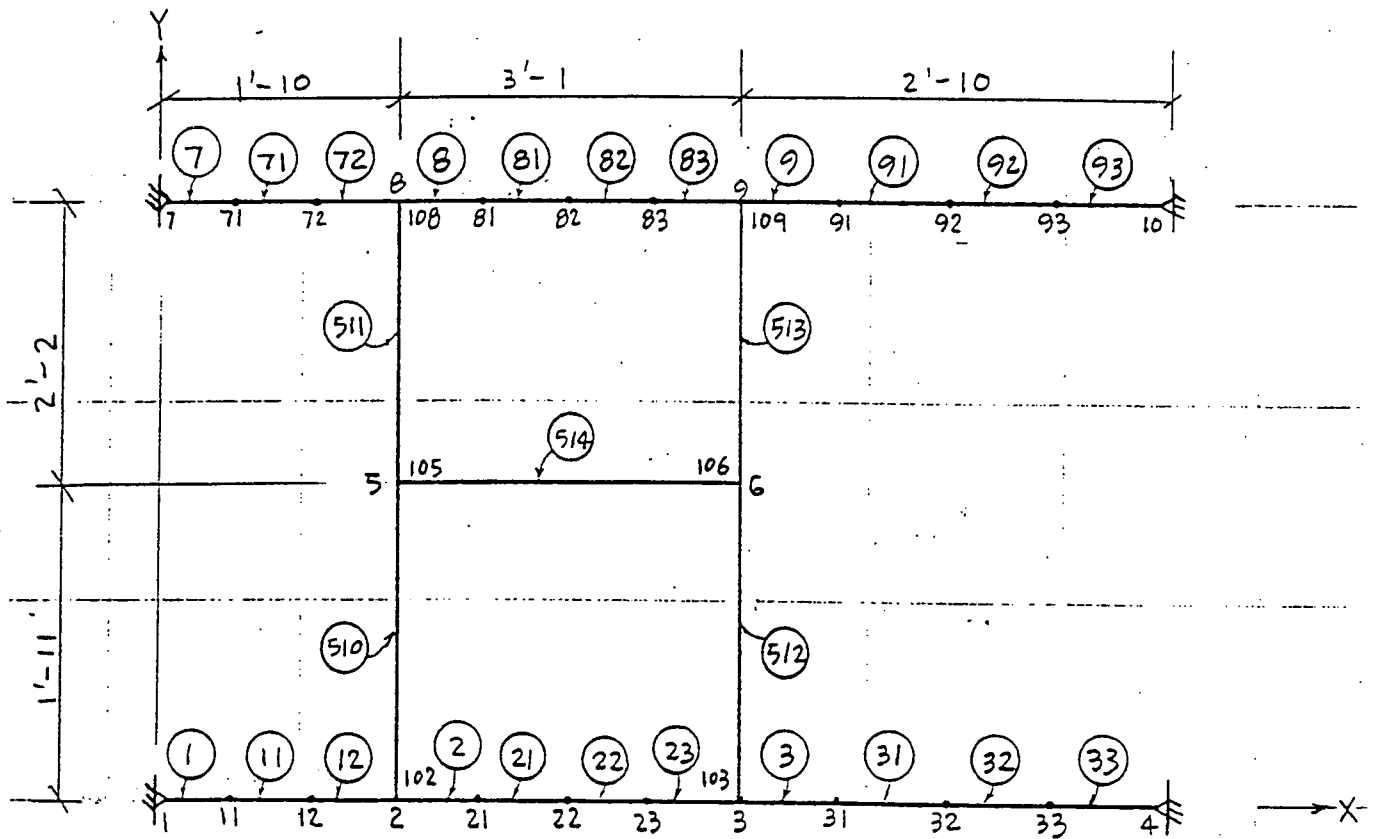
Client TVA

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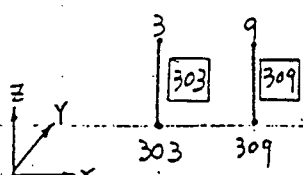
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Figure 11.4.1.1 ANSYS Model for Worst Case 1-11

(MODEL AX11)



PLAN VIEW (NTS)



RIGID LINKS

LEGENDS :

○ -- MEMBER

□ -- RIGID LINK

• -- NODE

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nodes 7 and 10 are calculated as follows:

For the member between nodes 1 and 4:

Grating load per inch, W:

$$(67/2 + 49/2) * 12/144 = 4.833333 \text{ \#/"}$$

$$\text{Equivalent density: } W/A = W/3.38 = 1.429980 \text{ lbs/in}^{**3}$$

$$A = \text{Sectional area of C8X11.5} = 3.38 \text{ IN}^{**2}$$

Sustained live load per inch, W:

$$(67/2 + 49/2) * 10/144 = 4.027777 \text{ \#/"}$$

$$\text{Equivalent density: } W/A = W/3.38 = 1.191650 \text{ lbs/in}^{**3}$$

$$\text{Total density} = 0.283 + 1.43 + 1.192 = 2.905 \text{ lbs/in}^{**3}$$

For the member between nodes 7 and 10:

Grating load: per inch, W:

$$(28/2 + 49/2) * 12/144 = 3.208333 \text{ \#/"}$$

$$\text{Equivalent density: } W/A = W/3.38 = 0.949211 \text{ lbs/in}^{**3}$$

$$A = \text{Sectional area of C8X11.5} = 3.38 \text{ IN}^{**2}$$

Sustained live load per inch, W:

$$(28/2 + 49/2) * 10/144 = 2.673611 \text{ \#/"}$$

$$\text{Equivalent density: } W/A = W/3.38 = 0.791009 \text{ lbs/in}^{**3}$$

$$\text{Total density} = 0.283 + 0.949 + 0.791 = 2.023 \text{ \#/in}^{**3}$$

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Walkdown information documented in Calculation WCG-1-830 is used to calculate attachment loads. There is no handrail attached to this portion of platform. A conduit is located at x=58.5" from node 1 under the channel. The effective conduit span is 7'-7.25" supported by the channel. From WB-DC-40-31.10 Table 3.3-1:

Conduit weight = 2.72 #/’ for 1 1/2" steel conduit.
 Total weight = 2.72*7.604 = 20.68 #s = 0.0207 kips

For the channel between nodes 7 and 10, one (1) 2" and two (2) 1.5" conduits, located at x=56" from node 7, are attached to the bottom of the channel. The conduit contributory span is 23.75". From WB-DC-40-31.10 Table 3.3-1:

Conduit weight = 2.72 #/’ for 1 1/2" steel conduit.
 Conduit weight = 3.65 #/’ for 2" steel conduit.
 Total weight = (2.72 + 2*3.65) * (23.75/12) = 19.83 #s = 0.0198 kips

The conduits are simulated with ANSYS STIF21 mass elements (elements 303 and 309, located at 5" below nodes 3 and 9) in the ANSYS model.

A pipe support is welded to the channel between node 1 and 4. The pipe reaction forces are obtained from QIR CEB-WBN-89-103 R1. The forces and moments are tabulated in the following table and applied at nodes 3 and 23.

ANSYS Node	FX	FY	FZ	MX	MY	MZ
23	-0.003	0	.307	-5.749	0	-0.06
3	-0.003	0	.307	-5.749	0	-0.06

(Units: Kips, Inches)

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SSE loads are considered using the equivalent static approach by applying the peak G of the ARS (4% and 7% damping) with MMF=1.5 for member (per WB-DC-20-21) and MMF=1.2 for conduit supports (per WCG-1-684). The peak G-loads are obtained from QIR-CEB-WBN-89-361 (Ref. 9.7) and presented in the following table:

Direction	NS	EW	Vertical
Peak G	1.40	1.65	0.38

In the ANSYS model, 0.38G is used in the global Z (vertical) direction. 1.65G is conservatively used as the accelerations in both X and Y direction. Since the YZ-SSE (combination of SSE in Y and Z directions) is more critical than the XZ-SSE (combination of SSE in X and Z direction), the YZ-SSE is used to combine with other applicable loadings (DL, SLL and Thermal loads). However, the XZ-SSE is also applied as first load step (superseded by the second load step) in the analysis and is for reference only. The thermal load is applied by specifying incremental temperature steps from 70° up to the accident temperature 210° with an increment of 10° per load step.

The ANSYS input file is presented in Section 11.4.2.

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11.4.2 ANSYS Input

***** ANSYS INPUT DATA LISTING (FILE18) *****

```

1 /PREP7
2 /TITLE, AX11 - EVAL. FOR AXIAL CASE 1-11, DWG. 48N1210-16, EL. 713'-0"
3 /COM
4 /COM BRANCH/PROJECT ID = WCG-1-969
5 /COM
6 /COM PREPARED BY SAL 8/9/91.
7 /COM CHECKED BY J. OU 8/12/91
8 /COM
9 /COM INPUT = AX11
10 /COM OUTPUT = AX11R
11 /COM FILE12 = AX11F
12 /COM PLOT = AX11P
13 /COM
14 C*** ANSYS ANALYSIS OPTIONS AND REFERENCES
15 KAN,0
16 KAY,6,1
17 KNL,1
18 TREF,70
19 C*** ELEMENT TYPE AND MATERIAL PROPERTIES
20 ET,1,4,,,,,1
21 ET,2,24,,,,,1
22 ET,3,21 * 3D MASS ELEMENT
23 C***
24 EX,1,29000
25 RP4,1
26 ALPX,1,6.5E-6
27 RP4,1
28 DENS,1,0.283E-3
29 DENS,2,2.905E-3 * INCLUDE DL+SLL
30 DENS,3,2.002E-3 * INCLUDE DL+SLL
31 DENS,4,0 * RIGID LINK
32 C***
33 MPTEMP,1,70,100,200,300,400
34 RP4,1
35 MPDATA,EX,1,1,29.0E3,28.9E3,28.3E3,27.9E3,27.4E3
36 RP4,,1
37 MPDATA,ALPX,1,1,6.50E-6,6.50E-6,6.50E-6,6.67E-6,6.86E-6 * ALPX=6.29 MU FOR 100F
38 RP4,,1
39 C*** NONLINEAR MATERIAL CONSTANT
40 *CREAT,MNL
41 NL,ARG1,13,10
42 NL,ARG1,19,70,100,200,300,400
43 NL,ARG1,25,36,36.0,36.0,36.0,36.0
44 NL,ARG1,31,150,150,150,150,150
45 *END
46 *USE,MNL,2
47 *USE,MNL,3
48 C*** MEMBER CROSS SECTIONAL PROPERTIES
49 R,1,3.38,1.32,32.6,8.00,2.26, *CBX11.5
50 RMORE,,0.131
    
```

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11.4.2 ANSYS Input (Cont'd)

```

51 C***
52 C*** NONLINEAR CROSS SECTIONAL PROPERTIES
53 C*** CHANNEL CBX11.5 *****
54 CF=2.260
55 TCF=0.390
56 CD=8.00
57 TCW=0.220
58 XBR=(0.571)+0.07695
59 C***
60 XO=(TCW/2)-(XBR)
61 CC=CF-(TCW/2)
62 DD=(CD-TCF)/2.
63 QC=CC/2
64 QD=DD/2
65 CC=CC+XO
66 QC=QC+XO
67 *STAT
68 C***
69 R,2,CC,DD,0,QC,DD,TCF          * STIF24 REAL PROP. - CBX11.5
70 RMORE,XO,DD,TCF,XO,QD,TCW
71 RMORE,XO,0,TCW,XO,-QD,TCW
72 RMORE,XO,-DD,TCW,QC,-DD,TCF
73 RMORE,CC,-DD,TCF
74 C***
75 R3=(.0207*1.2)/1.5
76 R4=(.0198*1.2)/1.5
77 R,3,R3,R3,R3          * CONDUIT
78 R,4,R4,R4,R4          * CONDUIT
79 R,5,100,1000,1000    * RIGID LINK
80 C***
81 C*** NODES AND COORDINATES
82 X1=22   SX2=59   SY1=23   SY2=49
83 N,1,0,0
84 N,101,-1,0
85 N,2,X1,0
86 N,20,X1,0,5
87 N,102,X1,0
88 N,3,X2,0
89 N,103,X2,0
90 N,303,X2,0,-5
91 N,4,93,0
92 N,104,94,0
93 N,5,X1,Y1
94 N,105,X1,Y1
95 N,6,X2,Y1
96 N,106,X2,Y1
97 N,7,0,Y2
98 N,107,-1,Y2
99 N,8,X1,Y2
100 N,80,X1,Y2,5
101 N,108,X1,Y2
102 N,9,X2,Y2
103 N,109,X2,Y2
    
```

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11.4.2 ANSYS Input (Cont'd)

```

104 N,309,X2,Y2,-5
105 N,10,93,Y2
106 N,110,94,Y2
107 FILL,1,2,2,11,1
108 FILL,2,3,3,21,1
109 FILL,3,4,3,31,1
110 FILL,7,8,2,71,1
111 FILL,8,9,3,81,1
112 FILL,9,10,3,91,1
113 C***
114 C*** ELEMENT CONNECTIVITIES
115 C***
116 MAT,2 STYPE,2 SREAL,2
117 EN,1,1,11,20 SEN,11,11,12,20 SEN,12,12,2,20
118 EN,2,2,21,20 SEN,21,21,22,20 SRP2,1,1,1 SEN,23,23,3,20
119 EN,3,3,31,20 SEN,31,31,32,20 SRP2,1,1,1 SEN,33,33,4,20
120 MAT,3
121 EN,7,7,71,80 SEN,71,71,72,80 SEN,72,72,8,80
122 EN,8,8,81,80 SEN,81,81,82,80 SRP2,1,1,1 SEN,83,83,9,80
123 EN,9,9,91,80 SEN,91,91,92,80 SRP2,1,1,1 SEN,93,93,10,80
124 C***
125 MAT,4 STYPE,1 SREAL,5 * RIGID LINK
126 EN,303,3,303
127 EN,309,9,309
128 C***
129 MAT,4 STYPE,3
130 REAL,3 SEN,503,303
131 REAL,4 SEN,509,309
132 C***
133 MAT,1 STYPE,1 SREAL,1
134 E,102,5

135 E,5,108
136 E,103,6
137 E,6,109
138 E,105,106
139 C***
140 C*** COUPLED DOFS FOR MEMBER JOINT RELEASE
141 CP,1,UX,102,2
142 CPLGEN,1,UY,UZ * SETS 2-3 NODE2
143 CPSGEN,2,1,1,3 * SETS 4-6 NODE3
144 CPSGEN,2,3,1,3 * SETS 7-9
145 CPLGEN,9,ROTX * SET 10 NODE5
146 CPSGEN,2,4,1,3 * SETS 11-13 NODE6
147 CPSGEN,2,6,1,3 * SETS 14-16
148 CPLGEN,16,ROTY * SET 17 NODE8
149 CPSGEN,2,1,14,17 * SETS 18-21 NODE9
150 C*** BOUNDARY CONDITIONS
151 *CREAT,BND
152 DDELE,ALL,ALL
153 D,1,UY,,,,UZ
154 D,4,UY,,,,UZ,ROTX
155 D,7,UY,,,,UZ
156 D,10,UY,,,,UZ,ROTX
157 C***
158 DDX=ARG1
    
```

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11.4.2 ANSYS Input (Cont'd)

```

159 D,1,UX,-DDX
160 D,4,UX,DDX
161 D,7,UX,-DDX
162 D,10,UX,DDX
163 *END
164 *USE,BND,0
165 C*** ANALYSIS OPTIONS
166 ITER,-20,20
167 TREF,70
168 POSTR,,2,5
169 C*** PLOTS - GEOMETRY W/B.C.
170 /SHOW
171 /VIEW,,0,0,1
172 /PNUM,NODE,1
173 EPLO
174 /SHRINK,0.3
175 /PNUM,NODE,-1
176 /PNUM,ELEM,1
177 EPLO
178 /PNUM,ELEM
179 /PBC,ALL,1
180 EPLO
181 /VIEW,,1,1,1
182 EPLO
183 C*** LOADING CASES
184 C***
185 C*** PIPE FPL
186 F,3,FX,-.003,,23,20
187 F,3,FZ,-.307,,23,20
188 F,3,MX,5.749,,23,20
189 F,3,MZ,.06,,23,20
190 C***
191 *CREAT,MAC
192 TUNIF,ARG1
193 LWRITE
194 *END
195 C***
196 C*** FOR AX11 AT ELE. 713'-0",
197 C*** PEAK ACCEL.(G): NS=1.4, EW=1.65, VT=0.38
198 C*** FACTOR: MMF=1.5
199 MMF=1.5 $GX=MMF*1.65 $GY=MMF*1.65 $GZ=MMF*0.38
200 C***
201 C*** DL+SSE XZ-PLANE
202 ACEL,-GX,0,GZ+1
203 *USE,MAC,70
204 C***
205 C*** DL+SSE YZ-PLANE
206 ACEL,,-GY,GZ+1
207 *USE,MAC,70
208 C***
209 *USE,BND,(1/32)
210 *USE,MAC,150
211 RP7,,10

```

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11.4.2 ANSYS Input (Cont'd)

```

212 AFWRITE,,1
213 FINISH
214 /INPUT,27
215 FINISH
216 /POST1
217 /SHOW
218 NLINE,200
219 *CREAT,PRF
220 SET,ARG1
221 PRRFOR
222 PRDISP
223 PLDISP
224 PLDISP,1
225 *END
226 *USE,PRF,1
227 *USE,PRF,2
228 *USE,PRF,9
229 /VIEW,,1,1,1
230 PLDISP
231 PLDISP,1
232 FINISH
233 C***
234 /POST26
235 /SHOW
236 NUMVAR,50
237 TVAR,1
238 C***
239 ESTR,2,1,53,TEMP          * TEMPERATURE
240 *CREATE,STRN

241 C*** STORE STRAINS FOR CHANNEL
242 ESTR,3,ARG1,215,EE01
243 ESTR,4,ARG1,216,EPO1
244 ESTR,5,ARG1,227,EE03
245 ESTR,6,ARG1,228,EPO3
246 ESTR,7,ARG1,251,EE07
247 ESTR,8,ARG1,252,EPO7
248 ESTR,9,ARG1,263,EE09
249 ESTR,10,ARG1,264,EPO9
250 PRVAR,2,3,4,5,6
251 PRVAR,2,7,8,9,10
252 *END
253 C***
254 *CREAT,DSP
255 DISP,21,ARG1,UX,UX
256 DISP,22,ARG1,UY,UY
257 DISP,23,ARG1,UZ,UZ
258 PROD,24,21,21,,UXUX
259 PROD,25,22,22,,UYUY
260 PROD,26,23,23,,UZUZ
261 ADD,27,24,25,26,SS
262 SORT,28,27,,SRSS
263 PRVAR,2,21,22,23,28
264 *END

```

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11.4.2 ANSYS Input (Cont'd)

265 *USE,STRN,2
266 *USE,DSP,21
267 *USE,STRN,21
268 *USE,DSP,22
269 *USE,STRN,22
270 *USE,DSP,23
271 *USE,STRN,23
272 *USE,DSP,3

273 *USE,STRN,8
274 *USE,DSP,81
275 *USE,STRN,81
276 *USE,DSP,82
277 *USE,STRN,82
278 *USE,DSP,83
279 *USE,STRN,83
280 *USE,DSP,9
281 FINISH

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11.4.3 Analysis Results

The ANSYS output for the analysis "AX11" is stored in file "AX11R". The analysis results of the final load step (Load Step 9) are provided in this Section.

The deformed structure is plotted in Figure 11.4.3.1. Note that the displacements in this plot are magnified. This member is subjected to weak axis bending mainly due to thermal effect.

The reaction forces and the nodal displacements are provided in Sections 11.4.3.1 and 11.4.3.2 respectively. The maximum displacement for the member between nodes 1 and 4 occurs at Node 23. The maximum displacement for the member between nodes 7 and 10 occurs at Node 83.

The stresses and strains for elements around node 23 (elements 21, 22, 23 and 3) and node 83 (elements 81, 82, 83 and 3) are presented in Section 11.4.3.3. The following explanations are provided for the labels shown in the stress and strain output.

PT: Segment Point Number, see Figure 11.2.2 for the definition of segment point for cross-section of Channels.
TEMP: Temperature
SIGX: Axial Stress
EP: Axial Elastic Strain
EPPL: Axial Plastic Strain

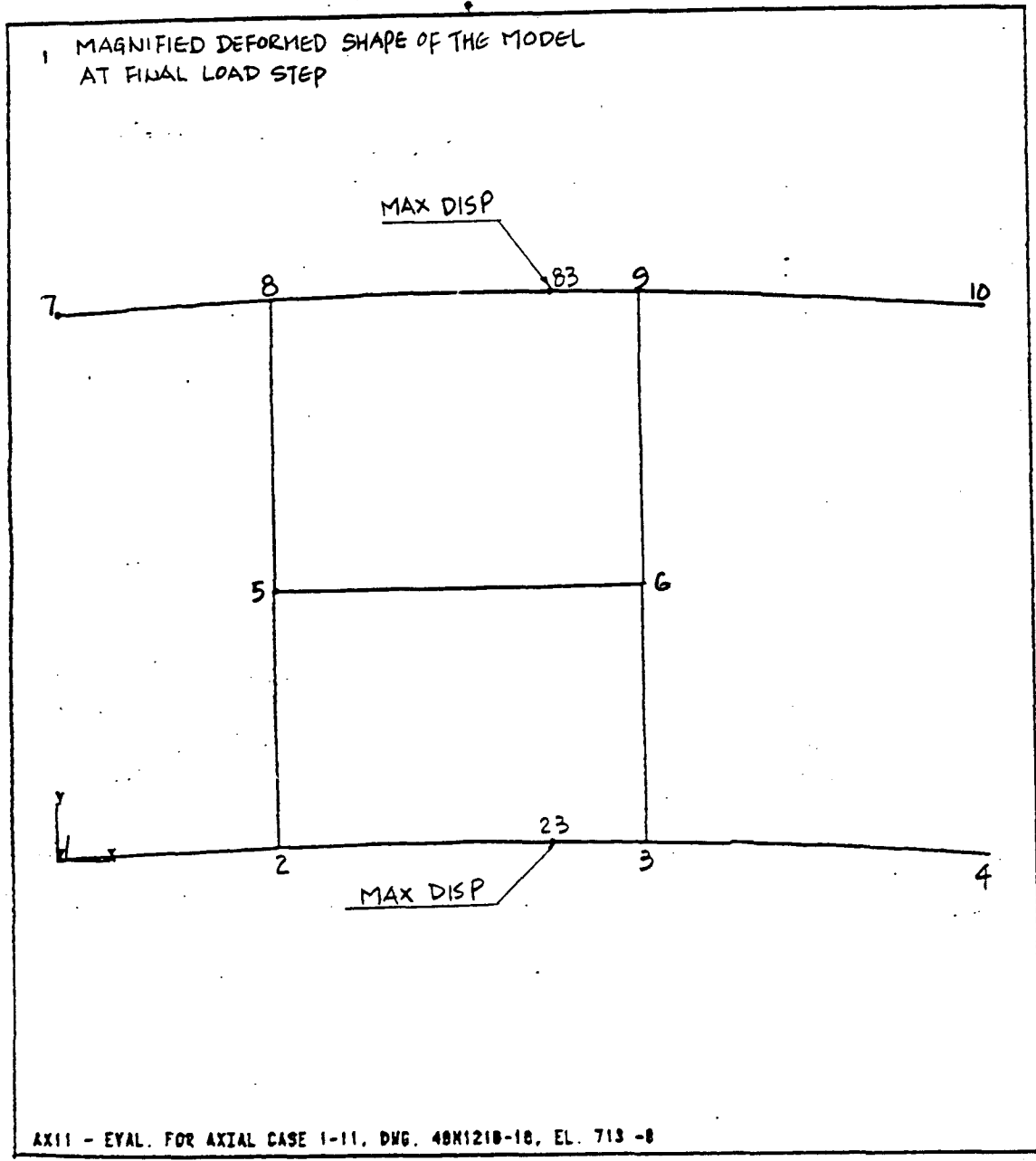
The stress and strain results from output file "AX11R" indicate that no yielding occurs in the structure at the final load step (temperature = 210°F).

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Figure 11.4.3.1 Deformed Shape of the Model AX11 (Worst Case 1-11)



ANSYS - 4.8A
AUG 23 1991
17.17.85
PLOT NO. 9
POST1 DISPL.
STEP=9
ITER=20
DMX =1.899
ZY =1
DIST=51.15
XF =48.5
YF =24.5

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11.4.3.1 Reaction Forces

AX11 - EVAL. FOR AXIAL CASE 1-11, DWG. 48N1210-16, EL. 713'-0"

***** POST1 REACTION FORCE LISTING *****

LOAD STEP 9 ITERATION= 20 SECTION= 1
 TIME= 0.0000 LOAD CASE= 1

THE FOLLOWING X,Y,Z FORCES ARE IN NODAL COORDINATES

NODE	FX	FY	FZ	MX	MY	MZ
1	15.055942	-1.2048128	1.0694251			
4	-15.049941	-1.2947006	1.0960250	-10.275460		
7	16.973767	-.90209277	.55390628			
10	-16.973768	-.78326339	.54928815	.18450515		
TOTAL	.60000012E-02	-4.1848695	3.2686445	-10.090955	0.0000000	0.0000000

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11.4.3.2 Displacements

AX11 - EVAL. FOR AXIAL CASE 1-11, DWG. 48N1210-16, EL. 713'-0"

16.0328 AUG 9, 1991 CP= 27.461

***** POST1 NODAL DISPLACEMENT LISTING *****

LOAD STEP 9 ITERATION= 20 SECTION= 1
TIME= 0.0000 LOAD CASE= 1

THE FOLLOWING X,Y,Z DISPLACEMENTS ARE IN NODAL COORDINATES

NODE	UX	UY	UZ	ROTX	ROTY	ROTZ
1	-.3125000E-01	0.0000000	0.0000000	.33684571	-.91102546E-02	.19805924E-01
2	-.17965737E-01	.36942408	.22212226	.31028067	-.81797670E-02	.14353553E-01
3	.10044532E-01	.49917512	.39290142	.26562040	-.26966772E-02	-.79105298E-02
4	.31250000E-01	0.0000000	0.0000000	0.0000000	.30777397E-02	-.20931454E-01
5	-.21740197E-01	.39116307	.98133023E-01	-.53783054E-02	.81415268E-03	.20584943E-04
6	.13448034E-01	.52037226	.18392250	-.90699911E-02	-.25478704E-03	-.56076173E-05
7	-.31250000E-01	0.0000000	0.0000000	-.97184105E-02	.12664750E-02	.21010004E-01
8	-.18179799E-01	.41571132	-.41611145E-01	-.20798344E-01	.81657336E-03	.14883195E-01
9	.94734879E-02	.54430923	-.51894768E-01	-.19389452E-01	-.25628351E-03	-.84133419E-02
10	.31250000E-01	0.0000000	0.0000000	0.0000000	-.10297768E-02	-.20191537E-01
11	-.27054074E-01	.13541346	.76578161E-01	.32641765	-.91253891E-02	.19066253E-01
12	-.22669099E-01	.26100336	.15165582	.31751752	-.87965475E-02	.17192614E-01
21	-.11500944E-01	.47201014	.30121536	.30421276	-.70674131E-02	.96669410E-02
22	-.45240438E-02	.52779203	.36577292	.30152770	-.57037829E-02	.41168662E-02
23	.27196559E-02	.53102884	.41398440	.30256153	-.42422576E-02	-.18106090E-02
31	.16291243E-01	.43455239	.30992814	.19670309	-.11011267E-02	-.13075533E-01
32	.21907891E-01	.32253946	.21449911	.12998070	.39206043E-03	-.17377166E-01
33	.26835822E-01	.17273634	.11030760	.64649522E-01	.17634889E-02	-.20197763E-01
71	-.27156325E-01	.15223731	-.15762926E-01	-.14148452E-01	.11496564E-02	.20277112E-01
72	-.22848490E-01	.29393062	-.29803172E-01	-.17863352E-01	.99866745E-03	.18171853E-01
81	-.11707885E-01	.52949603	-.51926561E-01	-.22775326E-01	.56389512E-03	.95799491E-02
82	-.47355336E-02	.59074728	-.57244262E-01	-.23244484E-01	.29329253E-03	.35961864E-02
83	.24396829E-02	.59547164	-.57220546E-01	-.22120436E-01	.16378651E-04	-.25698872E-02
91	.15564572E-01	.45228872	-.43634411E-01	-.16223982E-01	-.48742555E-03	-.13122626E-01
92	.21183465E-01	.32406927	-.31806895E-01	-.11862432E-01	-.69624240E-03	-.16870256E-01
93	.26355555E-01	.16922754	-.17057967E-01	-.64233121E-02	-.87773703E-03	-.19323999E-01
102	.17965737E-01	.36942408	.22212226	-.53899115E-02	.81528980E-03	.23234312E-03
103	.10044532E-01	.49917512	.39290142	-.90816001E-02	-.25670448E-03	-.21722434E-03
105	-.21740197E-01	.39116307	.98133023E-01	-.53783054E-02	.23215341E-02	.36211564E-02
106	.13448034E-01	.52037226	.18392250	-.53776337E-02	-.23300584E-02	.33439645E-02
108	-.18179799E-01	.41571132	-.41611145E-01	-.53646544E-02	.81657336E-03	-.21882507E-03
109	.94734879E-02	.54430923	-.51894768E-01	-.90563397E-02	-.25628351E-03	.23366310E-03
303	.28619924E-01	.1.8129076	.56363581	.26562042	-.26966771E-02	-.79105298E-02
309	.10348164E-01	.44727096	-.55755912E-01	-.19389433E-01	-.25628351E-03	-.84133419E-02

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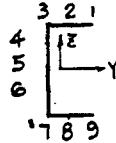
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Project WBNP Unit 1

Subject Qualif. of Worst Cases of Thermally Restrained Structures - Package I

11.4.3.3 Stresses and Strains



EL=	21	NODES=	21	22	MAT=	2	PRESSURES(Z,Y)=	0.0000	0.0000	AVE. TEMP=	210.00
END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW				
	1	210.00	17.516	.0006393	0.0000000	0.0000000	0.0000000				
	2	210.00	.52299	.0000191	0.0000000	0.0000000	0.0000000				
	3	210.00	-16.470	-.0006011	0.0000000	0.0000000	0.0000000				
	4	210.00	-14.729	-.0005376	0.0000000	0.0000000	0.0000000				
	5	210.00	-12.988	-.0004740	0.0000000	0.0000000	0.0000000				
	6	210.00	-11.247	-.0004105	0.0000000	0.0000000	0.0000000				
	7	210.00	-9.5055	-.0003469	0.0000000	0.0000000	0.0000000				
	8	210.00	7.4878	.0002733	0.0000000	0.0000000	0.0000000				
	9	210.00	24.481	.0008935	0.0000000	0.0000000	0.0000000				
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW				
	1	210.00	20.105	.0007338	0.0000000	0.0000000	0.0000000				
	2	210.00	1.0334	.0000377	0.0000000	0.0000000	0.0000000				
	3	210.00	-18.039	-.0006583	0.0000000	0.0000000	0.0000000				
	4	210.00	-16.033	-.0005852	0.0000000	0.0000000	0.0000000				
	5	210.00	-14.028	-.0005120	0.0000000	0.0000000	0.0000000				
	6	210.00	-12.023	-.0004388	0.0000000	0.0000000	0.0000000				
	7	210.00	-10.018	-.0003656	0.0000000	0.0000000	0.0000000				
	8	210.00	9.0543	.0003304	0.0000000	0.0000000	0.0000000				
	9	210.00	28.126	.0010265	0.0000000	0.0000000	0.0000000				
FORCES ON MEMBER AT NODE		21	15.0272	-.422714	.586218	.482175	29.6158	25.5164			
		22	-15.0276	.252196	-.384760	-.211306	-34.1066	-28.6379			

EL=	22	NODES=	22	23	MAT=	2	PRESSURES(Z,Y)=	0.0000	0.0000	AVE. TEMP=	210.00
END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW				
	1	210.00	20.137	.0007349	0.0000000	0.0000000	0.0000000				
	2	210.00	1.0459	.0000382	0.0000000	0.0000000	0.0000000				
	3	210.00	-18.046	-.0006586	0.0000000	0.0000000	0.0000000				
	4	210.00	-16.042	-.0005855	0.0000000	0.0000000	0.0000000				
	5	210.00	-14.038	-.0005123	0.0000000	0.0000000	0.0000000				
	6	210.00	-12.035	-.0004392	0.0000000	0.0000000	0.0000000				
	7	210.00	-10.031	-.0003661	0.0000000	0.0000000	0.0000000				
	8	210.00	9.0605	.0003307	0.0000000	0.0000000	0.0000000				
	9	210.00	28.152	.0010274	0.0000000	0.0000000	0.0000000				
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW				
	1	210.00	20.548	.0007499	0.0000000	0.0000000	0.0000000				
	2	210.00	.97396	.0000355	0.0000000	0.0000000	0.0000000				
	3	210.00	-18.600	-.0006788	0.0000000	0.0000000	0.0000000				
	4	210.00	-16.440	-.0006000	0.0000000	0.0000000	0.0000000				
	5	210.00	-14.279	-.0005211	0.0000000	0.0000000	0.0000000				
	6	210.00	-12.119	-.0004423	0.0000000	0.0000000	0.0000000				
	7	210.00	-9.9591	-.0003635	0.0000000	0.0000000	0.0000000				
	8	210.00	9.6148	.0003509	0.0000000	0.0000000	0.0000000				
	9	210.00	29.189	.0010653	0.0000000	0.0000000	0.0000000				
FORCES ON MEMBER AT NODE		22	15.0287	-.163645	.388542	.719963E-02	34.0796	28.6670			
		23	-15.0281	-.704427E-02	-.187230	.263472	-36.7425	-29.3913			

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11.4.3.3 Stresses and Strains (Cont'd)

EL= 23		NODES= 23		3 MAT= 2		PRESSURES(Z,Y)= 0.0000		0.0000		AVE. TEMP= 210.00	
END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW				
	1	210.00	21.310	.0007777	0.0000000	0.0000000	0.0000000				
	2	210.00	1.2677	.0000463	0.0000000	0.0000000	0.0000000				
	3	210.00	-18.774	-.0006852	0.0000000	0.0000000	0.0000000				
	4	210.00	-16.643	-.0006074	0.0000000	0.0000000	0.0000000				
	5	210.00	-14.513	-.0005297	0.0000000	0.0000000	0.0000000				
	6	210.00	-12.382	-.0004519	0.0000000	0.0000000	0.0000000				
	7	210.00	-10.251	-.0003741	0.0000000	0.0000000	0.0000000				
	8	210.00	9.7912	.0003573	0.0000000	0.0000000	0.0000000				
	9	210.00	29.833	.0010888	0.0000000	0.0000000	0.0000000				
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW				
	1	210.00	20.643	.0007534	0.0000000	0.0000000	0.0000000				
	2	210.00	1.1588	.0000423	0.0000000	0.0000000	0.0000000				
	3	210.00	-18.325	-.0006688	0.0000000	0.0000000	0.0000000				
	4	210.00	-16.279	-.0005941	0.0000000	0.0000000	0.0000000				
	5	210.00	-14.233	-.0005195	0.0000000	0.0000000	0.0000000				
	6	210.00	-12.187	-.0004448	0.0000000	0.0000000	0.0000000				
	7	210.00	-10.141	-.0003701	0.0000000	0.0000000	0.0000000				
	8	210.00	9.3432	.0003410	0.0000000	0.0000000	0.0000000				
	9	210.00	28.827	.0010521	0.0000000	0.0000000	0.0000000				
FORCES ON MEMBER AT NODE		23	15.0240	.342663E-02	-.567804E-01	4.84944	36.2436	30.0942			
		3	-15.0230	-.177704	.254994	-4.58293	-34.8017	-29.2564			

EL= 3		NODES= 3		31 MAT= 2		PRESSURES(Z,Y)= 0.0000		0.0000		AVE. TEMP= 210.00	
END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW				
	1	210.00	22.699	.0008284	0.0000000	0.0000000	0.0000000				
	2	210.00	1.9566	.0000714	0.0000000	0.0000000	0.0000000				
	3	210.00	-18.786	-.0006856	0.0000000	0.0000000	0.0000000				
	4	210.00	-16.828	-.0006142	0.0000000	0.0000000	0.0000000				
	5	210.00	-14.870	-.0005427	0.0000000	0.0000000	0.0000000				
	6	210.00	-12.912	-.0004713	0.0000000	0.0000000	0.0000000				
	7	210.00	-10.954	-.0003998	0.0000000	0.0000000	0.0000000				
	8	210.00	9.7884	.0003572	0.0000000	0.0000000	0.0000000				
	9	210.00	30.531	.0011143	0.0000000	0.0000000	0.0000000				
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW				
	1	210.00	18.672	.0006815	0.0000000	0.0000000	0.0000000				
	2	210.00	1.0963	.0000400	0.0000000	0.0000000	0.0000000				
	3	210.00	-16.479	-.0006014	0.0000000	0.0000000	0.0000000				
	4	210.00	-14.882	-.0005431	0.0000000	0.0000000	0.0000000				
	5	210.00	-13.285	-.0004849	0.0000000	0.0000000	0.0000000				
	6	210.00	-11.688	-.0004266	0.0000000	0.0000000	0.0000000				
	7	210.00	-10.091	-.0003683	0.0000000	0.0000000	0.0000000				
	8	210.00	7.4847	.0002732	0.0000000	0.0000000	0.0000000				
	9	210.00	25.060	.0009146	0.0000000	0.0000000	0.0000000				
FORCES ON MEMBER AT NODE		3	15.0476	.474734	-.635374	9.68766	33.3027	31.1464			
		31	-15.0464	-.644287	.808791	-9.45449	-27.1650	-26.3906			

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11.4.3.3 Stresses and Strains (Cont'd)

EL= 81		NODES= 81	82	MAT= 3	PRESSURES(Z,Y)= 0.0000			0.0000	AVE. TEMP= 210.00	
END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW			
	1	210.00	20.997	.0007663	0.0000000	0.0000000	0.0000000			
	2	210.00	2.6979	.0000985	0.0000000	0.0000000	0.0000000			
	3	210.00	-15.601	-.0005694	0.0000000	0.0000000	0.0000000			
	4	210.00	-14.908	-.0005441	0.0000000	0.0000000	0.0000000			
	5	210.00	-14.214	-.0005188	0.0000000	0.0000000	0.0000000			
	6	210.00	-13.521	-.0004934	0.0000000	0.0000000	0.0000000			
	7	210.00	-12.827	-.0004681	0.0000000	0.0000000	0.0000000			
	8	210.00	5.4725	.0001997	0.0000000	0.0000000	0.0000000			
	9	210.00	23.772	.0008676	0.0000000	0.0000000	0.0000000			
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW			
	1	210.00	22.926	.0008367	0.0000000	0.0000000	0.0000000			
	2	210.00	3.2464	.0001185	0.0000000	0.0000000	0.0000000			
	3	210.00	-16.433	-.0005998	0.0000000	0.0000000	0.0000000			
	4	210.00	-15.669	-.0005719	0.0000000	0.0000000	0.0000000			
	5	210.00	-14.905	-.0005440	0.0000000	0.0000000	0.0000000			
	6	210.00	-14.141	-.0005161	0.0000000	0.0000000	0.0000000			
	7	210.00	-13.377	-.0004882	0.0000000	0.0000000	0.0000000			
	8	210.00	6.3027	.0002300	0.0000000	0.0000000	0.0000000			
	9	210.00	25.983	.0009483	0.0000000	0.0000000	0.0000000			
FORCES ON MEMBER AT NODE				81	16.9465	-.301981	.176434	.122813	11.7983	27.4776
				82	-16.9475	.146192	-.825526E-01	.341433E-02	-12.9961	-29.5504

EL= 82		NODES= 82	83	MAT= 3	PRESSURES(Z,Y)= 0.0000			0.0000	AVE. TEMP= 210.00	
END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW			
	1	210.00	22.921	.0008365	0.0000000	0.0000000	0.0000000			
	2	210.00	3.2438	.0001184	0.0000000	0.0000000	0.0000000			
	3	210.00	-16.433	-.0005998	0.0000000	0.0000000	0.0000000			
	4	210.00	-15.669	-.0005718	0.0000000	0.0000000	0.0000000			
	5	210.00	-14.904	-.0005439	0.0000000	0.0000000	0.0000000			
	6	210.00	-14.139	-.0005160	0.0000000	0.0000000	0.0000000			
	7	210.00	-13.375	-.0004881	0.0000000	0.0000000	0.0000000			
	8	210.00	6.3021	.0002300	0.0000000	0.0000000	0.0000000			
	9	210.00	25.979	.0009481	0.0000000	0.0000000	0.0000000			
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW			
	1	210.00	22.565	.0008236	0.0000000	0.0000000	0.0000000			
	2	210.00	3.1035	.0001133	0.0000000	0.0000000	0.0000000			
	3	210.00	-16.358	-.0005970	0.0000000	0.0000000	0.0000000			
	4	210.00	-15.577	-.0005685	0.0000000	0.0000000	0.0000000			
	5	210.00	-14.796	-.0005400	0.0000000	0.0000000	0.0000000			
	6	210.00	-14.015	-.0005115	0.0000000	0.0000000	0.0000000			
	7	210.00	-13.234	-.0004830	0.0000000	0.0000000	0.0000000			
	8	210.00	6.2275	.0002273	0.0000000	0.0000000	0.0000000			
	9	210.00	25.689	.0009376	0.0000000	0.0000000	0.0000000			
FORCES ON MEMBER AT NODE				82	16.9481	-.429682E-01	.771563E-01	.803695E-01	13.0047	29.5462
				83	-16.9482	-.112794	.167759E-01	.206665	-13.2840	-29.2233

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11.4.3.3 Stresses and Strains (Cont'd)

EL= 83		NODES= 83		9 MAT= 3		PRESSURES(Z,Y)= 0.0000		0.0000		AVE. TEMP= 210.00	
END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW				
	1	210.00	22.534	.0008224	0.0000000	0.0000000	0.0000000				
	2	210.00	3.0889	.0001127	0.0000000	0.0000000	0.0000000				
	3	210.00	-16.356	-.0005969	0.0000000	0.0000000	0.0000000				
	4	210.00	-15.572	-.0005683	0.0000000	0.0000000	0.0000000				
	5	210.00	-14.788	-.0005397	0.0000000	0.0000000	0.0000000				
	6	210.00	-14.003	-.0005111	0.0000000	0.0000000	0.0000000				
	7	210.00	-13.219	-.0004825	0.0000000	0.0000000	0.0000000				
	8	210.00	6.2258	.0002272	0.0000000	0.0000000	0.0000000				
	9	210.00	25.671	.0009369	0.0000000	0.0000000	0.0000000				
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW				
	1	210.00	19.907	.0007265	0.0000000	0.0000000	0.0000000				
	2	210.00	2.2645	.0000826	0.0000000	0.0000000	0.0000000				
	3	210.00	-15.378	-.0005612	0.0000000	0.0000000	0.0000000				
	4	210.00	-14.632	-.0005340	0.0000000	0.0000000	0.0000000				
	5	210.00	-13.886	-.0005068	0.0000000	0.0000000	0.0000000				
	6	210.00	-13.139	-.0004795	0.0000000	0.0000000	0.0000000				
	7	210.00	-12.393	-.0004523	0.0000000	0.0000000	0.0000000				
	8	210.00	5.2498	.0001916	0.0000000	0.0000000	0.0000000				
	9	210.00	22.893	.0008355	0.0000000	0.0000000	0.0000000				
FORCES ON MEMBER AT NODE			83	16.9472	.214776	-.225714E-01	-.285140	13.3388	29.1978		
			9	-16.9463	-.370355	.116802	.411837	-12.6942	-26.4916		

EL= 9		NODES= 9		91 MAT= 3		PRESSURES(Z,Y)= 0.0000		0.0000		AVE. TEMP= 210.00	
END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW				
	1	210.00	19.853	.0007246	0.0000000	0.0000000	0.0000000				
	2	210.00	2.2357	.0000816	0.0000000	0.0000000	0.0000000				
	3	210.00	-15.382	-.0005614	0.0000000	0.0000000	0.0000000				
	4	210.00	-14.631	-.0005340	0.0000000	0.0000000	0.0000000				
	5	210.00	-13.880	-.0005066	0.0000000	0.0000000	0.0000000				
	6	210.00	-13.129	-.0004792	0.0000000	0.0000000	0.0000000				
	7	210.00	-12.378	-.0004518	0.0000000	0.0000000	0.0000000				
	8	210.00	5.2395	.0001912	0.0000000	0.0000000	0.0000000				
	9	210.00	22.857	.0008342	0.0000000	0.0000000	0.0000000				
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW				
	1	210.00	16.049	.0005857	0.0000000	0.0000000	0.0000000				
	2	210.00	1.1282	.0000412	0.0000000	0.0000000	0.0000000				
	3	210.00	-13.792	-.0005034	0.0000000	0.0000000	0.0000000				
	4	210.00	-13.161	-.0004803	0.0000000	0.0000000	0.0000000				
	5	210.00	-12.530	-.0004573	0.0000000	0.0000000	0.0000000				
	6	210.00	-11.899	-.0004343	0.0000000	0.0000000	0.0000000				
	7	210.00	-11.268	-.0004112	0.0000000	0.0000000	0.0000000				
	8	210.00	3.6529	.0001333	0.0000000	0.0000000	0.0000000				
	9	210.00	18.573	.0006779	0.0000000	0.0000000	0.0000000				
FORCES ON MEMBER AT NODE			9	16.9703	.405132	-.196161	-.381048	12.7728	26.4542		
			91	-16.9687	-.547835	.283171	.498036	-10.7357	-22.4041		

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11.4.4 Member Qualification

As presented in Section 11.4.3, the maximum displacement for the member between nodes 1 and 4 occurs at Node 23. The maximum displacement for the member between nodes 7 and 10 occurs at Node 83. The stress and strain results of the analysis indicate that no yielding occurs in both members at the final temperature step (temperature=210°F). Therefore, the ductility ratio at both nodes 23 and 83 are less than 1. Note that the ductility ratio is defined as the ratio of the final displacement to the yield displacement.

Per DG-C1.6.12, Appendix D, the members of worst case 1-11 are qualified.

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11.4.5 Connection and Anchorage Qualification

The connections at support nodes 1, 4, 7 and 10 are evaluated. The connection information is obtained from Drawing 48N1210-16 and the walkdown package (Calc. WCG-1-830). The evaluation of connections is documented in sheets 161 through 166.

Based on the analysis results presented in Section 11.4.3, the anchorage, to which the supporting channels are welded, is not affected due to thermal effect (The only force transferred to the anchorage due to thermal effect is compression force). Therefore, no further evaluation is required for the anchorage.

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 PROJECT: WBMP- UNIT 1
 SUBJECT: Qualif. of worst cases of Therm. Rest. Struct. Package I

REF.

AXII - DWG. 48N1210-16

Connection of Bm to base plate

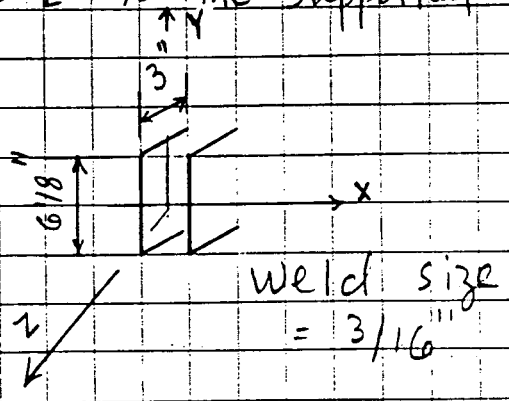
Nodes 1, 4, 7, 10

use loads for node 4, because it is the max^m.

- $F_x = 15.05 \text{ K}$ (compression)
- $F_y = 1.3 \text{ K}$
- $F_z = 1.1 \text{ K}$
- $M_x = 10.28 \text{ in-K}$
- $M_y = 0$
- $M_z = 0$

Output:
AXIIR

Connection of clip LS to the supported Bm (C8x11.5):



Forces on weld group

- $F_x = 15.05 \text{ K}$
- $F_y = 1.1 \text{ K}$
- $F_z = 1.3 \text{ K}$
- $M_x = 10.28 \text{ in-K}$
- $M_y = 0$
- $M_z = 0$

Weld properties

$A_w = 2 \times (3 + 3 + 6.125) = 24.25 \text{ in.}$
 $S_x = 2 \times (3 \times 6.125 + 6.125^2 / 16) = 49.26 \text{ in}^2$

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AX II - cont'd. REF.

$$f_t = \frac{F_z}{A_w} + \frac{M_x}{S_x}$$

$$= \frac{1.3}{24.25} + \frac{10.28}{47.26} = 0.054 + 0.209$$

$$= 0.263 \text{ K/in}$$

$$f_x = \frac{15.05}{24.25} = 0.62 \text{ K/in}$$

$$f_y = \frac{1.1}{24.25} = 0.045 \text{ K/in}$$

$$f_r = (f_t^2 + f_x^2 + f_y^2)^{1/2}$$

$$= (.263^2 + .62^2 + .045^2)^{1/2}$$

$$= 0.67 \text{ K/in}$$

Allowable stress on weld

$$= \frac{2}{3} \times 70 = 46.67 \text{ KSI}$$

Allowable load on weld

$$= 0.707 \times 46.67 \times \frac{3}{16} = 6.19 \text{ K/in}$$

> 0.67 K/in OK

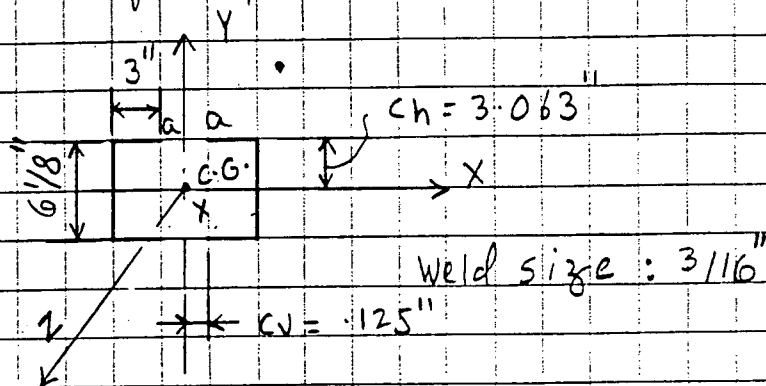
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AX II - cont'd.

REF.

Connection of clip L^s to Connected Bm:



FORCES on weld group:

$$F_x = 1.3K$$

$$F_y = 1.1K$$

$$F_z = 15.05K \text{ (compression - neglected)}$$

$$M_x = 0$$

$$M_y = 0$$

$$M_z = 10.28 \text{ in-K}$$

Weld properties

$$A_w = 2 \times (3 + 6 \cdot 125) = 24.25 \text{ in.}$$

$$I_w = 2 \times \left\{ \frac{(2 \times 3 + 6 \cdot 125)^3}{12} - \frac{3^2 (3 + 6 \cdot 125)^2}{(2 \times 3 + 6 \cdot 125)} \right\}$$

$$= 173.5 \text{ in}^3$$

C.G. of the weld group is at x.

Max^m stresses in welds are at pts a

$$Ch = 3.063, CV = .125$$

$$S_{th} = \frac{M_z \times Ch}{I_w} = \frac{10.28 \times 3.063}{173.5} = .18 \text{ K/in}$$

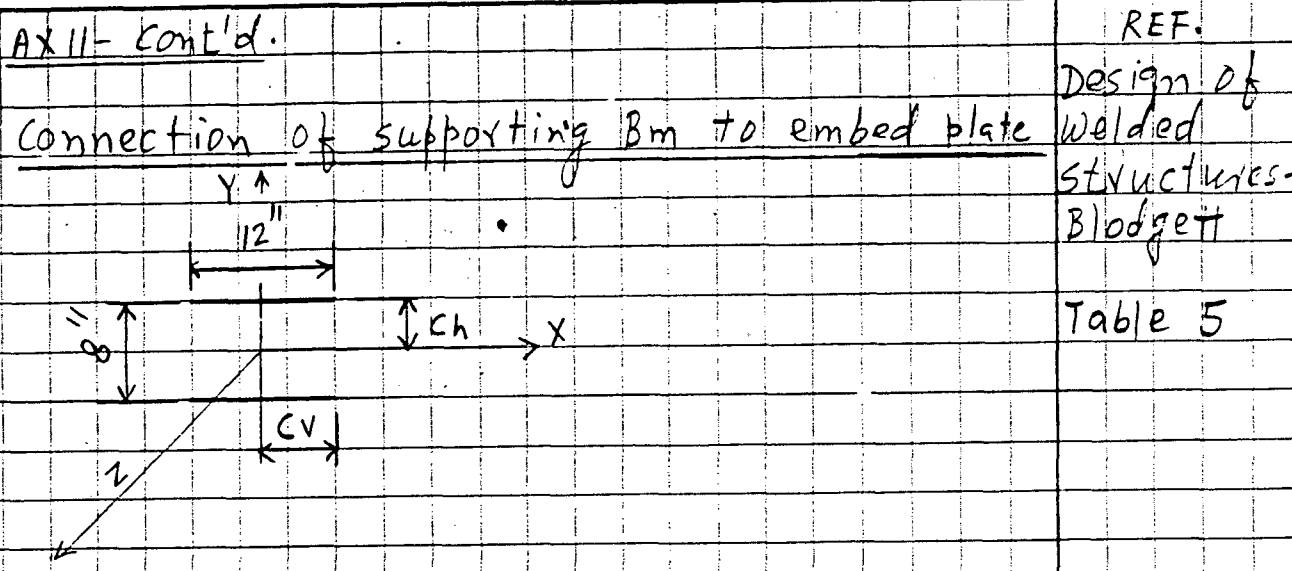
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AXII - Conl'd.	REF.
$f_{tN} = \frac{M_z \times C_V}{J_W} = \frac{10.28 \times 125}{173.5} = .007 \text{ K/in}$	Design of Welded Structures - Blodgett
$f_{sh} = \frac{F_x}{A_W} = \frac{1.3}{24.25} = .054 \text{ K/in}$	Table 5.
$f_{sV} = \frac{F_y}{A_W} = \frac{1.1}{24.25} = .045 \text{ K/in}$	
$f_r = \left\{ (f_{tN} + f_{sh})^2 + (f_{tV} + f_{sV})^2 \right\}^{1/2}$ $= \left\{ (.007 + .054)^2 + (.007 + .045)^2 \right\}^{1/2}$ $= .24 \text{ K/in}$	
Allowable load on weld	
$= .707 \times 46.67 \times 3/16 = 6.19 \text{ K/in}$ 7.24 K/in OK	

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REF.
 Design of
 Welded
 Structures -
 Blodgett
 Table 5

Forces on weld group

$F_x = 1.3K$
 $F_y = 1.1K$
 $F_z = 15.05K$ (compression - neglected)

$M_x = 0$
 $M_y = 0$
 $M_z = 10.28 \text{ in K}$

Weld Properties

$A_w = 2 \times 12 = 24 \text{ in}$
 $I_w = \frac{12 \times (3 \times 8^2 + 12^2)}{6} = 672 \text{ in}^3$
 $c_h = 4 \text{ in}, c_v = 6 \text{ in}$

$f_{sh} = \frac{M_z \times c_h}{I_w} = \frac{10.28 \times 4}{672} = .06 \text{ K/in}$

$f_{tv} = \frac{M_z \times c_v}{I_w} = \frac{10.28 \times 6}{672} = .092 \text{ K/in}$

$f_{sh} = \frac{F_x}{A_w} = \frac{1.3}{24} = .054 \text{ K/in}$

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AXII - Cont'd.	REF.
$SSV = \frac{F_y}{A_w} = \frac{1.1}{24} = .046 \text{ K/in}$	
$S_x = \left\{ (S_{Eh} + S_{Sh})^2 + (S_{Ev} + S_{Sv})^2 \right\}^{1/2}$ $= \left\{ (.06 + .054)^2 + (.072 + .046)^2 \right\}^{1/2}$ $= .118 \text{ K/in}$	
<p>Allowable load on weld</p> $= .707 \times 46.67 \times 3/16 = 6.19 \text{ K/in}$ <p style="text-align: right;">$> .118 \text{ K/in} \quad \text{OK}$</p>	

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11.5 Qualification of Worst Case 2&3-8(G)

11.5.1 Mathematical Model (PR8G)

The ANSYS model PR8G is shown in Figure 11.5.1.1. This model contains members for a member (Angle 3x3x1/4) with Proximity Restraint, two supporting members (structural tubing TS 4x4x1/4) and other interconnecting members. ANSYS STIF24 elements (3-D thin-wall plastic beam elements) are used to simulate the proximity member and the supporting members. The interconnecting members are modeled with STIF4 beam elements. Temperature dependent material properties, as described in Section 11.2, are used.

Boundary Condition

ANSYS STIF14 spring elements and STIF40 combination elements are used to simulate the shear stiffness of concrete anchors and the rotational stiffness of the baseplate and concrete anchorage at support nodes 1 and 9. Nodes 101 and 109 are the support nodes for the spring elements. 1600 kip/in (400*4) obtained from CSG-91-0003 is used as the shear stiffness for each spring to represent the 1" ϕ Wedge Bolts. The rotational stiffness is calculated from the output of BASEPLATE II runs. A total of two (2) baseplate models were generated as shown in Figures 11.5.1.2 and 11.5.1.3. The input files for the baseplate runs to calculate rotational stiffness are provided in Tables 11.5.1.1 and 11.5.1.2.

A displacement of 1/32" is specified at node 101 in +Y direction and at node 109 in -Y direction. This approach has the same effect of using a Gap Element at support to allow free movement before load starts to transfer. A study run is performed by specifying a gap of 1/32" for the combination elements (STIF40) at both nodes 1 and 9. The final results of the analysis using gap elements are identical to the results of the analysis using imposed displacements. The input file and the results (reactions and displacements at

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final load step) are provided in Attachment D.

Loading Condition

As shown in Sheets 178 and 179, equivalent densities are calculated for the proximity member and the supporting members to consider the dead loads (including self-weight and grating) and the sustained live loads (10 psf per WB-DC-20-21). There is no attachment on the members per walkdown information shown in Attachment E. The SSE loads are applied using the equivalent static approach. The peak G's of the ARS, obtained from QIR-CEB-WBN-89-361 (Ref. 9.7) as shown in the following table, are applied with MMF=1.5.

Direction	NS	EW	Vertical
Peak G	3.5	3.25	1.4

In the ANSYS model, 1.4G is used in the global Z (vertical) direction and 3.5G is conservatively used as the accelerations in both X and Y direction. Since the YZ-SSE (combination of SSE in Y and Z directions) is more critical than the XZ-SSE (combination of SSE in X and Z direction), the YZ-SSE is used to combine with other applicable loadings (DL, SLL and Thermal loads). The thermal load is applied by specifying incremental temperature steps from 70° up to the accident temperature 327° with an increment of 10° per load step.

The ANSYS input file is presented in Section 11.5.2.

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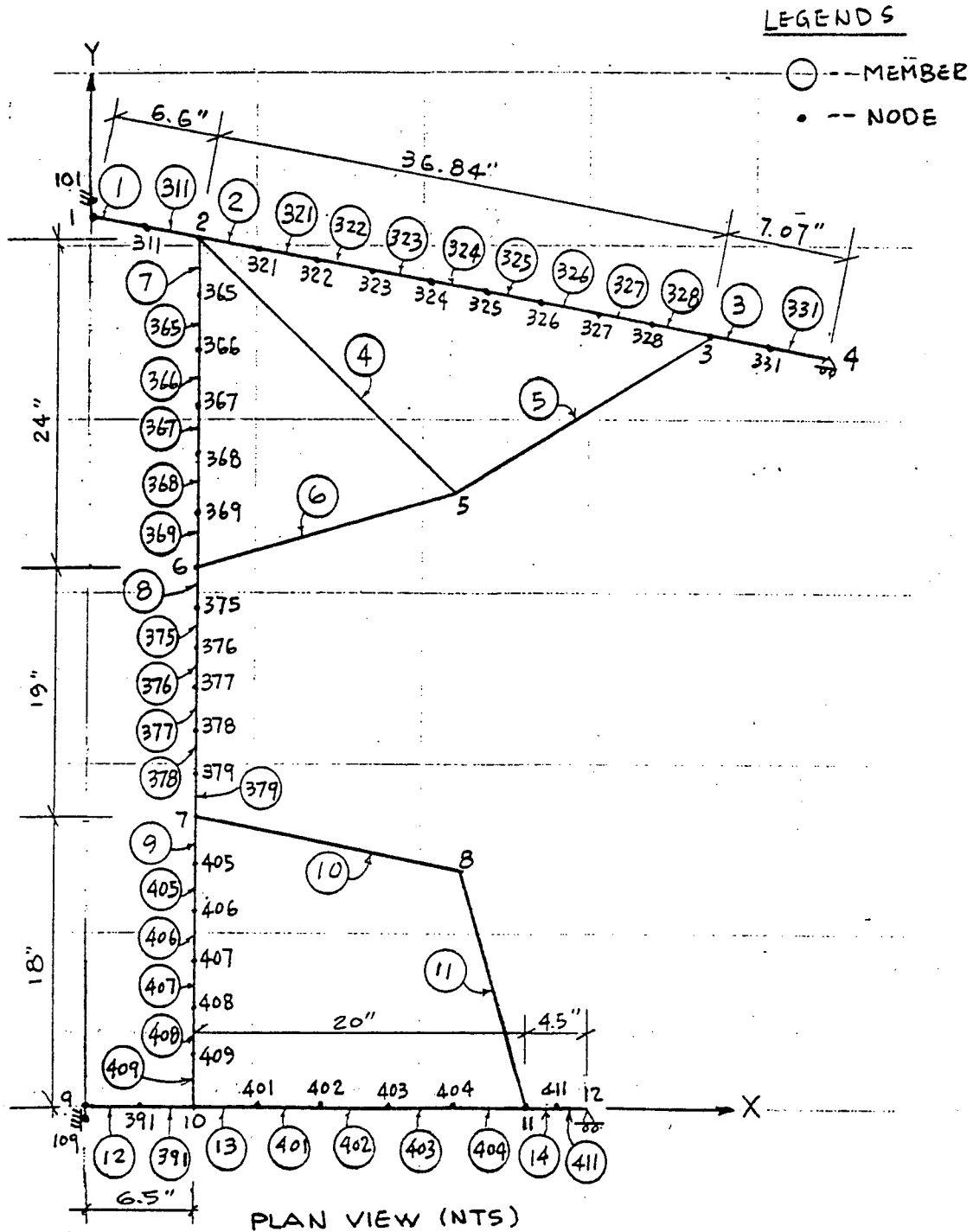
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Figure 11.5.1.1 ANSYS Model for Worst Case 2&3-8(G)



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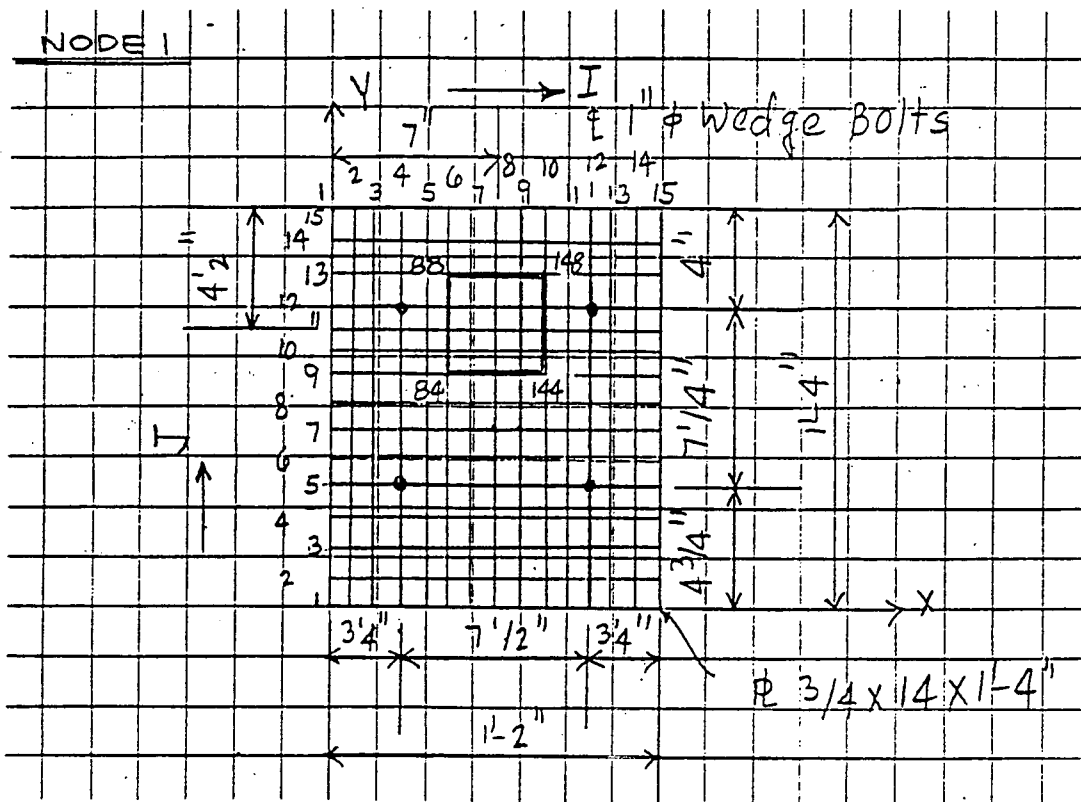
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Figure 11.5.1.2 BASEPLATE II Model for Nodes 1 of ANSYS Model



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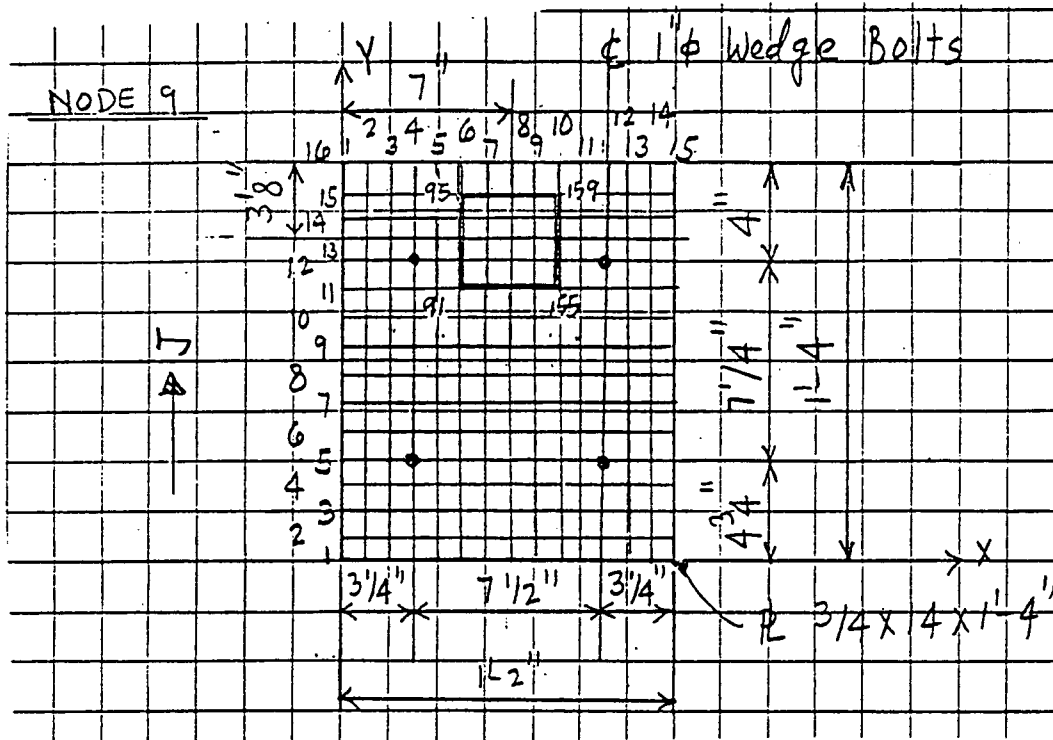
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Figure 11.5.1.3 BASEPLATE II Model for Nodes 9 of ANSYS Model



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Table 11.5.1.1 BASEPLATE II Input for Rot. Stiff. for Node 1 of ANSYS Model

(Ref = Input = BS1PR8G ; Output = BS1PBGR)

```
3 BS1PR8G-NODE 1; SURF. MOUNT. PLATE; 48W967-1
OUT ,,-1,100/
CON ,,,3000,1.7,1.7/
PLA 15,15,,14,16,.75,29/
APR 4,4,1,4,4,.25,.25/
BPR ,,1,.4E6,1.0E6/
BOL 4,5,1,3.25,4.75/
BOL 12,5,1,10.75,4.75/
BOL 4,12,1,3.25,12/
BOL 12,12,1,10.75,12/
END /
TUB 8,11,1,7,11.5/
END /
POI 8,11/
LOA ,,1,0,0,0,100000,100000,0/
END /
END OF JOB
```

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Table 11.5.1.2 BASEPLATE II Input for Rot. Stiff. for Node 9 of ANSYS Model

(Ref = Input: BS9PRBG ; Output = BS9P8GR)

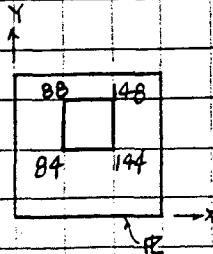
```
3 BS9PRBG-NODE 9; SURF. MOUNT. PLATE; 48W967-1
OUT ,,, -1,100/
COM ,,,, 3000,1.7,1.7/
PLA 15,16,,14,16,.75,29/
APR 4,4,1,4,4,.25,.25/
BPR ,,1, .4E6,1.0E6,5200,6700/
BOL 4,5,1,3.25,4.75/
BOL 12,5,1,10.75,4.75/
BOL 4,12,1,3.25,12/
BOL 12,12,1,10.75,12/
END /
TUB 8,13,1,7,12.88/
END /
POI 8,13/
LOA ,,1,0,0,0,100000,100000,0/
END /
END OF JOB
```

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PR 24 B-8 (G) . . Node 1
Rotational Stiffness

FOR Mx



REF.
 OUTPUT:
 BSIP8GR

1) $\Delta_{88} = .048550'' = \Delta_1$ (z-DISP)
 $\Delta_{84} = .015980 = \Delta_2$

2) $\Delta_{148} = .031597'' = \Delta_1$ (z-DISP)
 $\Delta_{144} = -.000972'' = \Delta_2$

$L = 4''$

1) $\theta = \text{TAN}^{-1} \frac{\Delta_1 - \Delta_2}{L}$
 $= \text{TAN}^{-1} \frac{.048550 - .015980}{4} = 0.00814232$ Rad.
 $= .4665^\circ$

$K_{MM} = \frac{M}{\theta} = \frac{100}{.4665 \times \frac{\pi}{180}} = 12282 \frac{\text{K-in}}{\text{Rad.}}$

2) $\theta = \text{TAN}^{-1} \frac{\Delta_1 + (-\Delta_2)}{L}$
 $= \text{TAN}^{-1} \frac{.031597 + .000972}{4} = 0.00814267$ Rad.
 $= .4665^\circ$

$K_{MM} = 12282 \frac{\text{K-in}}{\text{Rad.}}$

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	Joint 1 Cont'd.	REF.
	For MY	
1)	$\Delta 84 = .015980'' = \Delta 1$ (Z DISP) $\Delta 144 = -.000972'' = \Delta 2$	
2)	$\Delta 88 = .048550'' = \Delta 1$ $\Delta 148 = .031597'' = \Delta 2$	
	$L = 4''$	
1)	$\theta = \text{TAN}^{-1} \frac{\Delta 1 + (-\Delta 2)}{4}$ $= \text{TAN}^{-1} \frac{.015980 + .000972}{4} = .004238 \text{ Rad.}$ $= .2428^\circ$	
2)	$\theta = \text{TAN}^{-1} \frac{\Delta 1 - \Delta 2}{4}$ $= \text{TAN}^{-1} \frac{.048550 - .031597}{4} = .004238 \text{ Rad.}$ $= .2428^\circ$	
	$KMM = \frac{M}{\theta} = \frac{100}{.2428 \times \frac{\pi}{180}} = 23598 \frac{\text{K-in}}{\text{Rad.}}$	

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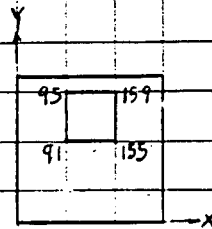
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PR 2 & 3-8 (G), Joint 9
Rotational Stiffness:

For M_x



REF.

OUTPUT:
BS9P8GR

$$1) \quad \Delta 91 = .024949'' = \Delta_2$$

$$\Delta 95 = .078465'' = \Delta_1 \quad (Z-DISP)$$

$$2) \quad \Delta 155 = .004915'' = \Delta_2$$

$$\Delta 159 = .058430'' = \Delta_1$$

$$L = 4''$$

$$1) \quad \theta = \text{TAN}^{-1} \frac{\Delta_1 - \Delta_2}{L}$$

$$= \text{TAN}^{-1} \frac{.078465 - .024949}{4} = .013378 \text{ Rad.}$$

$$= .7665^\circ$$

$$2) \quad \theta = \text{TAN}^{-1} \frac{\Delta_1 - \Delta_2}{L}$$

$$= \text{TAN}^{-1} \frac{.058430 - .004915}{4} = .013378 \text{ Rad.}$$

$$= .7665^\circ$$

$$K_{MM} = \frac{M}{\theta} = \frac{100}{.7665 \times \frac{\pi}{180}} = \frac{7475 \text{ K-in}}{\text{Rad}}$$

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Joint 9 Cont'd.

For My:

1) $\Delta_{91} = .024949'' = \Delta_1$ (Z-DISP)
 $\Delta_{155} = .004915'' = \Delta_2$

2) $\Delta_{95} = .078465'' = \Delta_1$
 $\Delta_{159} = .058430'' = \Delta_2$

$L = 4''$

1) $\theta = \text{TAN}^{-1} \frac{\Delta_1 - \Delta_2}{L}$
 $= \text{TAN}^{-1} \frac{.024949 - .004915}{4} = .005088 \text{ Rad.}$
 $= .287^\circ$

2) $\theta = \text{TAN}^{-1} \frac{\Delta_1 - \Delta_2}{L}$
 $= \text{TAN}^{-1} \frac{.078465 - .058430}{4} = .005088 \text{ Rad.}$
 $= .287^\circ$

$K_{MM} = \frac{M}{\theta} = \frac{100}{.287 \times \frac{\pi}{180}} = 19965 \frac{\text{K-in}}{\text{Rad.}}$

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	REF.
<u>Calculation of density :</u>	WB-DC-20-21
<u>PRSG : 48N967-1</u>	SECT. 3.1.1
<u>Proximity members :</u>	& 3.1.2
<u>Loads :</u>	
LL = 10 psf	
Grating = 12 "	
22 psf = $\frac{22}{144}$ psi	
Contributory load width on Bm	
= $\frac{4'-7\frac{1}{2}''}{2} \times 12 = 27.75''$	
For 1" strip of width :	
Load = $\frac{22}{144} \times \frac{27.75 \times 1}{1000} = 0.00424$ kip	
Vol. of Bm 13x3x1/4 for 1" width = $A \times l$	
= 1.44x1	
= 1.44 in ³	
∴ Density = Load / Vol. = $\frac{0.00424}{1.44}$	
= 0.00295 K/in ³	
<u>Supporting members :</u>	
<u>Longer member</u>	
Contributory load width on Bm = $\frac{2'-5\frac{5}{8}''}{2}$	
= 1.234 "	
= 14.813	

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For 1" strip of width:

$$\text{Load from floor} = \frac{22}{144} \times \frac{14.813 \times 1}{1000} = .00226 \text{ K}$$

$$\text{Wt. of TS } 4 \times 4 \times .25 = \frac{12.02}{12} \times \frac{1}{1000} = .00100$$

$$\text{Volume of TS } 4 \times 4 \times .25 = 3.54 \times 1 = 3.54 \text{ in}^3$$

$$\therefore \text{Density} = \frac{.00326}{3.54} = .00092 \text{ K/in}^3$$

Shorter member:

$$\text{Contributory load width on Bm} = \frac{1'-7\frac{1}{4}''}{2} = 0.802' = 9.625''$$

For 1" strip of width:

$$\text{Load from floor} = \frac{22}{144} \times \frac{9.625 \times 1}{1000} = .00147$$

$$\text{Wt. of TS } 4 \times 4 \times .25 = \frac{12.02}{12} \times \frac{1}{1000} = .00100$$

$$\text{Density} = \frac{.00247}{3.54} = 0.0007 \text{ K/in}^3$$

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11.5.2 ANSYS Input

***** ANSYS INPUT DATA LISTING (FILE18) *****

```

1 /PREP7
2 /TITLE, PRBG - EVAL. FOR PROXIMITY CASE 8(G), DWG 48N967-1, EL. 774'-0", LOOP 4
3 /COM
4 /COM BRANCH/PROJECT ID : WCG-1-969
5 /COM
6 /COM PREPARED BY : SAL 8/15/91
7 /COM CHECKED BY J. OU 8/15/91
8 /COM
9 /COM INPUT: PRBG
10 /COM OUTPUT: PR8GR
11 /COM FILE12: PR8GF
12 /COM PLOT: PR8GP
13 /COM
14 C*** ANSYS ANALYSIS OPTIONS AND REFERENCES
15 KAN,0
16 KAY,6,1
17 KNL,1
18 TREF,70
19 C*** ELEMENT TYPE AND MATERIAL PROPERTIES
20 ET,1,24,,,,,1
21 ET,2,4,,,,,1
22 ET,3,14,,1
23 ET,4,40,,,2 * DOF=Y
24 C*** MATERIAL PROPERTIES
25 EX,1,29000
26 RP3,1
27 ALPX,1,6.5E-6
28 RP3,1
29 C***
30 DENS,1,0.92E-3
31 DENS,2,0.70E-3
32 DENS,3,(0.283E-3+2.95E-3)
33 DENS,4,0.283E-3
34 C***
35 MPTEMP,1,70,100,200,300,400
36 RP4,1
37 MPDATA,EX,1,1,29.0E3,28.9E3,28.3E3,27.9E3,27.4E3
38 RP4,,1
39 MPDATA,ALPX,1,1,6.50E-6,6.50E-6,6.50E-6,6.67E-6,6.86E-6 * ALPX=6.29 MU FOR 100F
40 RP4,,1
41 C*** NONLINEAR MATERIAL CONSTANT
42 *CREAT,MNL
43 NL,ARG1,13,10
44 NL,ARG1,19,70,100,200,300,400
45 NL,ARG1,25,36,36.0,36.0,36.0,36.0
46 NL,ARG1,31,150,150,150,150,150
47 *END
48 *USE,MNL,1
49 *USE,MNL,2
50 *USE,MNL,3
    
```

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11.5.2 ANSYS Input (Cont'd)

```

51 C*** MEMBER CROSS SECTIONAL PROPERTIES
52 R,1,3.54,8.0,8.0,4.0,4.0      * TS4X4X.25
53 R,2,1.44,1.24,1.24,3,3      * L3X3X.25
54 RMORE,,0.03
55 C***
56 K1=(400*4)      * SAL 7/31/91
57 C*** GAP=(1/32)
58 GAP=0      * SAL 7/24/91
59 FSL=(20.8*4)
60 R,8,K1,,,GAP,FSL      * NODE 1
61 GAP=0
62 R,5,K1,,,GAP,FSL      * NODE 9
63 C***
64 BF=4      * WIDTH
65 D=4      * DEPTH
66 TW=0.25      * WALL THICKNESS
67 C***
68 AA=(BF-TW)/2
69 BB=(D-TW)/2
70 QA=AA/2
71 QB=BB/2
72 *STAT
73 R,3,AA,BB,0,QA,BB,TW      *STIF24 REAL PROP. - TS
74 RMORE,0,BB,TW,-QA,BB,TW
75 RMORE,-AA,BB,TW,-AA,QB,TW
76 RMORE,-AA,0,TW,-AA,-QB,TW
77 RMORE,-AA,-BB,TW,-QA,-BB,TW
78 RMORE,0,-BB,TW,QA,-BB,TW
79 RMORE,AA,-BB,TW,AA,-QB,TW
80 RMORE,AA,0,TW,AA,QB,TW
81 RMORE,AA,BB,TW
82 C***
83 *CREATE,ANGL
84 D=ARG2      * D=DEPTH      *
85 AF=ARG3      * AF=FLANGE      * ANGLE L D X AF X TAW
86 TAW=ARG4      * TAW=THICKNESS      *
87 XBR=ARG5      * XBAR=CENTROID X FROM AISC
88 YBR=ARG6      * YBAR=CENTROID Y
89 C***      *
90 XX=AF-(TAW/2)      * *4 *3 *2 *1      ARG7=0 OR DEFAULT CASE
91 YY=D-(TAW/2)      *      *      *      *      FOR ANGLE SHOWN LEFT
92 X0=XBR-(TAW/2)      * *5 CG(XBAR,YBAR)      WHERE
93 Y0=YBR-(TAW/2)      *      *      *      *      FLANGE IS ON RIGHT OF
94 XX=XX-X0      * *6      *      *      WEB
95 YY=YY-Y0      *
96 MX=XX/2      * *7
97 MY=YY/2      *
98 *IF,ARG7,LE,1,:RAN
99 X=-XX      SX0=-X0      SMX=-MX      * CHN SIGN FOR FLANGE ON LEFT OF WEB
100 :RAN
    
```

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11.5.2 ANSYS Input (Cont'd)

```

101 R,ARG1,XX,YO,0,MX,YO,TAW
102 RMORE,0,YO,TAW,-XO,YO,TAW
103 RMORE,-XO,0,TAW,-XO,-MY,TAW
104 RMORE,-XO,-YY,TAW
105 *END
106 *USE,ANGL,4,3,3,0.25,0.842,0.842,1 * L3X3X1/4 LEFT FLANGE
107 C*** NODAL COORDINATES
108 N,1,0,62
109 N,101,0,64 $NDD=300
110 N,2,6.5,61 $FILL,1,2,1,NDD+11,1
111 N,102,6.5,61
112 N,202,6.5,61
113 N,3,43,56 $FILL,2,3,8,NDD+21,1
114 N,103,43,56
115 N,4,50,55 $FILL,3,4,1,NDD+31,1
116 N,5,22,38
117 N,6,6.5,37
118 N,106,6.5,37
119 N,7,6.5,18
120 N,8,25,12
121 N,108,25,12
122 N,208,25,12
123 N,9,0,0 $FILL,2,6,5,NDD+65,1
124 N,109,0,-2 $FILL,6,7,5,NDD+75,1
125 N,10,6.5 $FILL,9,10,1,NDD+91,1 $FILL,7,10,5,NDD+105,1
126 N,11,26.5 $FILL,10,11,4,NDD+101,1
127 N,12,31 $FILL,11,12,1,NDD+111,1
128 N,99,6.5,61,100
129 N,100,6.5,0,100
130 C*** ELEMENT CONNECTIVITIES
131 MAT,1 STYPE,1 $REAL,3 * TS4X4X.25
132 EN,1,1,311,99 $,311,311,2,99
133 EN,2,2,321,99 $,321,321,322,99 $SRP7,1,1,1 $EN,328,328,3,99
134 EN,3,3,331,99 $,331,331,4,99
135 MAT,2
136 EN,12,9,391,100 $,391,391,10,100
137 EN,13,10,401,100 $,401,401,402,100 $SRP3,1,1,1 $EN,404,404,11,100
138 EN,14,11,411,100 $,411,411,12,100
139 C***
140 MAT,4 STYPE,2 $REAL,2 * L3X3X.25
141 EN,4,2,5
142 EN,5,3,5
143 EN,6,6,5
144 C***
145 MAT,3 STYPE,1 $REAL,4 * L3X3X.25
146 EN,7,2,365,100 $,365,365,366,100 $SRP4,1,1,1 $EN,369,369,6,100
147 EN,8,6,375,100 $,375,375,376,100 $SRP4,1,1,1 $EN,379,379,7,100
148 EN,9,7,405,100 $,405,405,406,100 $SRP4,1,1,1 $EN,409,409,10,100
149 C***
150 MAT,4 STYPE,2 $REAL,2 * L3X3X.25
    
```

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11.5.2 ANSYS Input (Cont'd)

```

151 EN,10,7,8
152 EN,11,11,8
153 C***
154 C*** SPRING (STIF40)
155 TYPE,4 $REAL,8 * SPRING Y
156 EN,15,1,101
157 REAL,5
158 EN,16,109,9
159 C***
160 C*** BOUNDARY CONDITIONS
161 C***
162 D,1,UX,,,9,8,UZ,ROTX * SAL 7/12/91
163 D,4,UY,,,12,8 * SAL 7/15/91
164 D,101,ALL
165 D,109,ALL
166 C***
167 C*** SPECIFY ROTATIONAL SPRINGS
168 PSPRING,1,ROT,12282,,2 * ROTY
169 PSPRING,1,ROT,23598,,,2 * ROTZ
170 PSPRING,9,ROT,7475,,2 * ROTY
171 PSPRING,9,ROT,19965,,,2 * ROTZ
172 C***
173 C*** ANALYSIS OPTIONS
174 ITER,-20,20
175 TREF,70
176 POSTR,,1,5
177 C*** PLOT GEOMETRY AND BOUNDARY CONDITIONS
178 /SHOW
179 /PNUM,NODE,1
180 EPLO
181 /SHRINK,0.3
182 /PNUM,NODE,-1
183 /PNUM,ELEM,1
184 EPLO
185 /PBC,ALL,1
186 /PNUM,ELEM,-1
187 EPLO
188 C*** LOADING CASES
189 C*** TEMPERATURE LOADS *** *
190 *CREAT,MAC
191 TUNIF,ARG1
192 LWRITE
193 *END
194 C***
195 C*** PEAK ACCEL.(G): NS=3.5 EW=3.25 VT=1.4
196 C*** FACTOR: MMF=1.5
197 MMF=1.5 SGX=MMF*3.5 SGY=MMF*3.5 SGZ=MMF*1.4
198 C***
199 C*** DL+SSE XZ-PLANE
200 C*** ACEL,-GX,0,GZ+1

```


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11.5.2 ANSYS Input (Cont'd)

```

201 C*** *USE,MAC,70
202 C***
203 C*** DL+SSE YZ-PLANE
204 ACEL,,-GY,GZ+1 * SAL 7/15/91
205 *USE,MAC,70
206 D,101,UY,(1/32) * SAL 7/24/91
207 D,109,UY,(-1/32)
208 *USE,MAC,80
209 RP25,,10
210 *USE,MAC,327
211 C***
212 AFWRITE,,1
213 FINISH
214 /INPUT,27
215 FINISH
216 /POST1
217 /SHOW
218 NLINE,200
219 *CREAT,PLD
220 SET,ARG1
221 PRRFOR
222 PRDISP
223 PLDISP
224 PLDISP,1
225 *END
226 *USE,PLD,1
227 *USE,PLD,27
228 FINISH
229 C***
230 /POST26
231 LINE,200
232 NUMVAR,50
233 TVAR,1
234 ESTR,2,1,53,TEMP
235 *CREATE,STRN
236 ESTR,3,ARG1,215,EE01
237 ESTR,4,ARG1,216,EP01
238 ESTR,5,ARG1,239,EE05
239 ESTR,6,ARG1,240,EPO5
240 ESTR,7,ARG1,263,EE09
241 ESTR,8,ARG1,264,EPO9
242 ESTR,9,ARG1,287,EE13
243 ESTR,10,ARG1,288,EP13
244 PRVAR,2,3,4,5,6
245 PRVAR,2,7,8,9,10
246 *END
247 C***
248 *CREAT,DSP
249 DISP,21,ARG1,UX,UX
250 DISP,22,ARG1,UY,UY
    
```

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11.5.2 ANSYS Input (Cont'd)

```

251 DISP,23,ARG1,UZ,UZ
252 PROD,24,21,21,,UXUX
253 PROD,25,22,22,,UYUY
254 PROD,26,23,23,,UZUZ
255 ADD,27,24,25,26,SS
256 SQRT,28,27,,,SRSS
257 PRVAR,2,21,22,23,28
258 *END
259 C***
260 *USE,STRN,311
261 *USE,DSP,2
262 *USE,STRN,391
263 *USE,DSP,10
264 C***
265 *USE,DSP,1
266 *USE,DSP,9
267 C***
268 *USE,STRN,7
269 *USE,DSP,365
270 *USE,STRN,365
271 *USE,DSP,366
272 *USE,STRN,366
273 *USE,DSP,367
274 *USE,STRN,367
275 *USE,DSP,368
276 *USE,STRN,368
277 *USE,DSP,369
278 *USE,STRN,369
279 *USE,DSP,6
280 *USE,STRN,8
281 *USE,DSP,375
282 C***
283 *CREAT,RFOR
284 RFORC,31,ARG1,FX,RFX
285 RFORC,32,ARG1,FY,RFY
286 RFORC,33,ARG1,FZ,RFZ
287 RFORC,34,ARG1,MX,RMX
288 RFORC,35,ARG1,MY,RMY
289 RFORC,36,ARG1,MZ,RMZ
290 PRVAR,31,32,33,34,35,36
291 *END
292 *USE,RFOR,1
293 *USE,RFOR,101
294 *USE,RFOR,413
295 *USE,RFOR,9
296 *USE,RFOR,109
297 *USE,RFOR,415
298 C***
299 *CREAT,ST40
300 ESTR,41,ARG1,3,STR1
    
```

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11.5.2 ANSYS Input (Cont'd)

301 ESTR,42,ARG1,4,STR2
302 ESTR,43,ARG1,5,UI
303 ESTR,44,ARG1,6,UJ
304 ESTR,45,ARG1,7,SLIDE
305 ESTR,46,ARG1,8,STAT
306 ESTR,47,ARG1,9,OSTA
307 ESTR,48,ARG1,10,KCON
308 PRVAR,2,41,42,43,44
309 PRVAT,2,45,46,47,48
310 *END
311 *USE,ST40,15
312 *USE,ST40,16
313 FINISH

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11.5.3 Analysis Results

The ANSYS output for the analysis "PR8G" is stored in file "PR8GR". The analysis results of the final load step (Load Step 27) are provided in this Section.

The deformed structure is plotted in Figure 11.5.3.1. Note that the displacements in this plot are magnified.

The reaction forces and the nodal displacements are provided in Sections 11.5.3.1 and 11.5.3.2 respectively.

The stresses and strains for elements at the center portion of the proximity member (elements 368, 369, 8, 375, 376, 377, 378 and 9), elements at the connections of the proximity member and the supporting members (elements 7, 409, 311 and 391) are presented in Section 11.5.3.3. The following explanations are provided for the labels shown in the stress and strain output.

PT: Segment Point Number, see Figure 11.2.4 and Figure 11.2.3 for the definition of segment points for cross-section of Angles and Structural Tubings respectively.

TEMP: Temperature

SIGX: Axial Stress

EP: Axial Elastic Strain

EPPL: Axial Plastic Strain

The stress and strain results from output file "PR8GR" indicate that no yielding occurs in the structure at the final load step (temperature = 327°F).

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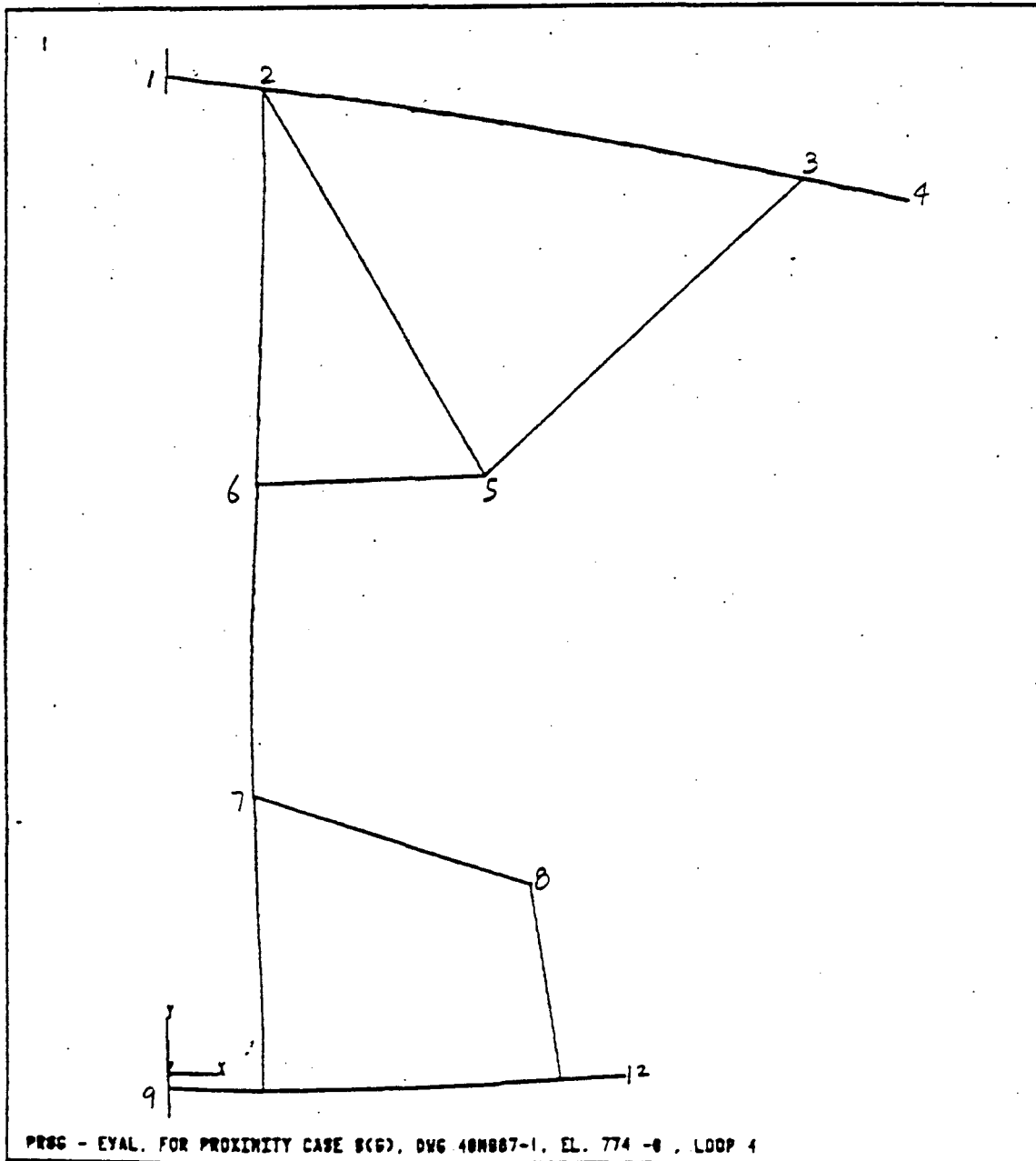
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Figure 11.5.3.1 Deformed Shape of the Model PR8G (Worst Case 2&3-8(G))



ANSYS 4.3A
AUG 15 1991
18 28 42
PLOT NO. 8
POST1 DISPL.
STEP=27
ITER=28
DMX = .14487

ZY = 1
DIST=30.3
XF = 25
YF = 31
ZF = 60

PR8G - EVAL. FOR PROXIMITY CASE 8(G), DWG 40887-1, EL. 774 -8, LOOP 4

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11.5.3.1 Reaction Forces

PRBG - EVAL. FOR PROXIMITY CASE 8(G), DWG 48N967-1, EL. 774'-0", LOOP 4

***** POST1 REACTION FORCE LISTING *****

LOAD STEP 27 ITERATION= 20 SECTION= 1
 TIME= 0.0000 LOAD CASE= 1

THE FOLLOWING X,Y,Z FORCES ARE IN MODAL COORDINATES

NODE	FX	FY	FZ	MX	MY	MZ
1	.39634196		1.0985287	-9.2391064		
4		-1.4231010				
9	-.39634196		.70198716	4.0701865		
12		2.5637629				
101	0.0000000	-13.576338	0.0000000	0.0000000	0.0000000	0.0000000
109	0.0000000	9.3864149	0.0000000	0.0000000	0.0000000	0.0000000
412					0.0000000	0.0000000
413					0.0000000	-35.983550
414					0.0000000	-7.6299577
415					0.0000000	8.5199924
TOTAL	.59577232E-09	-3.0492608	1.8005159	-5.1689198	-25.803545	-27.463557

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11.5.3.2 Displacements

PR8G - EVAL. FOR PROXIMITY CASE 8(G), DWG 48M967-1, EL. 774'-0", LOOP 4

***** POST1 NODAL DISPLACEMENT LISTING *****

LOAD STEP 27 ITERATION= 20 SECTION= 1
TIME= 0.0000 LOAD CASE= 1

THE FOLLOWING X,Y,Z DISPLACEMENTS ARE IN NODAL COORDINATES

NODE	UX	UY	UZ	ROTX	ROTY	ROTZ
1	0.0000000	.39735211E-01	0.0000000	0.0000000	.14796928E-02	.15248559E-02
2	.12908086E-01	.48526651E-01	-.11163992E-01	.36137257E-03	.18613742E-02	.13109438E-02
3	.74055557E-01	.15404434E-01	-.99324985E-01	.48770534E-03	.25527004E-02	-.18518054E-02
4	.84423773E-01	0.0000000	-.11772844	.48770812E-03	.25552903E-02	-.19936275E-02
5	.25123251E-01	-.18843432E-02	-.53884352E-01	.29865523E-03	.25305078E-02	-.93839907E-03
6	-.33016237E-02	.14584376E-01	-.13689831E-01	-.39732926E-03	.26325591E-02	-.14685135E-02
7	-.14880760E-01	-.12577200E-01	-.39494171E-02	.16766035E-03	.18514510E-02	.13475793E-02
8	.27394063E-01	.87897620E-02	-.35361194E-01	-.50237181E-03	.17421180E-02	.13634989E-02
9	0.0000000	-.37116509E-01	0.0000000	0.0000000	.10207301E-02	-.42674643E-03
10	.11480626E-01	-.38852437E-01	-.71859161E-02	-.18942612E-03	.11666963E-02	.16937171E-03
11	.46909213E-01	-.99846820E-02	-.32497120E-01	-.30593186E-03	.13054160E-02	.21426074E-02
12	.54828151E-01	0.0000000	-.38385101E-01	-.30579107E-03	.13059292E-02	.22512665E-02
101	0.0000000	.31250000E-01	0.0000000	0.0000000	0.0000000	0.0000000
109	0.0000000	-.31250000E-01	0.0000000	0.0000000	0.0000000	0.0000000
311	.64810458E-02	.44305174E-01	-.52265068E-02	.18479263E-03	.16941518E-02	.17165543E-02
321	.20646289E-01	.51718618E-01	-.19303362E-01	.38848481E-03	.20372141E-02	.74979192E-03
322	.28083961E-01	-.52711397E-01	-.28105478E-01	.41134889E-03	.21812414E-02	.22919839E-03
323	.35244084E-01	.51680230E-01	-.37444992E-01	.43037910E-03	.22964827E-02	-.24575007E-03
324	.42152762E-01	.48820990E-01	-.47209073E-01	.44599391E-03	.23859625E-02	-.66996523E-03
325	.48839218E-01	.44350185E-01	-.57297412E-01	.45861539E-03	.24527039E-02	-.10383571E-02
326	.55335780E-01	.38504974E-01	-.67622210E-01	.46866869E-03	.24997285E-02	-.13458333E-02
327	.61677869E-01	.31543169E-01	-.78108178E-01	.47658148E-03	.25300568E-02	-.15872992E-02
328	.67903961E-01	.23743262E-01	-.88692530E-01	.48278335E-03	.25467079E-02	-.17576569E-02
331	.79257584E-01	.78269974E-02	-.10852473	.48775575E-03	.25549665E-02	-.19576238E-02
365	.15448238E-01	.42894097E-01	-.13626651E-01	.39741692E-03	.31027066E-02	.42249356E-03
366	.14950167E-01	.37250625E-01	-.15682506E-01	.30609863E-03	.39589856E-02	-.33805953E-03
367	.12027602E-01	.31597790E-01	-.16880058E-01	.13408415E-03	.43826839E-02	-.93842648E-03
368	.74202639E-02	.25936167E-01	-.16959449E-01	-.71563787E-04	.43283124E-02	-.13469647E-02
369	.19871410E-02	.20265392E-01	-.15854246E-01	-.26402008E-03	.37556682E-02	.15332264E-02
375	-.85428536E-02	.10076225E-01	-.12666846E-01	-.57537445E-03	.33956418E-02	-.14644299E-02
376	-.13244398E-01	.55608949E-02	-.10947024E-01	-.67182795E-03	.38022357E-02	-.12886460E-02
377	-.16840257E-01	.10376654E-02	-.88228851E-02	-.66537303E-03	.38485915E-02	-.92931888E-03
378	-.18732628E-01	-.34937652E-02	-.66553873E-02	-.53574851E-03	.35351892E-02	-.37593278E-03
379	-.18295885E-01	-.80327760E-02	-.48695603E-02	-.26393645E-03	.28666475E-02	.38055440E-03
391	.57403588E-02	-.38469318E-01	-.34743046E-02	-.94937338E-04	.11085244E-02	-.33551520E-03

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401	.18570879E-01	-.36779838E-01	-.11978972E-01	-.21241367E-03	.12226025E-02	.84132108E-03
402	.25658979E-01	-.32301187E-01	-.16963066E-01	-.23556191E-03	.12622611E-02	.13709242E-02
403	.32744463E-01	-.25979460E-01	-.22077421E-01	-.25886611E-03	.12878392E-02	.17618811E-02
404	.39827616E-01	-.18362825E-01	-.27269923E-01	-.28232333E-03	.13015025E-02	.20178820E-02
405	-.10631398E-01	-.16944178E-01	-.51555064E-02	.31905819E-03	.24671846E-02	.17345572E-02
406	-.53709348E-02	-.21314482E-01	-.64280665E-02	.36174752E-03	.27714307E-02	.18735131E-02
407	.16384911E-03	-.25689379E-01	-.74725787E-02	.31339070E-03	.27742568E-02	.17745845E-02
408	.52636700E-02	-.30070219E-01	-.80530705E-02	.19249892E-03	.24920788E-02	.14491328E-02
409	.92504153E-02	-.34457897E-01	-.79947316E-02	.18292050E-04	.19471157E-02	.90959985E-03
411	.50868817E-01	-.50535082E-02	-.35440848E-01	-.30582626E-03	.13058650E-02	.22242103E-02
412				0.0000000	0.0000000	0.0000000
413				0.0000000	0.0000000	0.0000000
414				0.0000000	0.0000000	0.0000000
415				0.0000000	0.0000000	0.0000000

EBASCO SERVICES INCORPORATED

BRANCH/PROJECT ID: WCG-1-969

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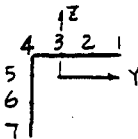
OFS No. NR Dept. No. NR

Client TVA

Project WBNP Unit 1

Subject Qualif. of Worst Cases of Thermally Restrained Structures - Package I

11.5.3.3 Stresses and Strains



PRBG - EVAL. FOR PROXIMITY CASE 8(G), DWG 48W967-1, EL. 774'-0", LOOP 4

***** ELEMENT STRESSES ***** TIME = 0.00000 LOAD STEP= 27 ITERATION= 20 CUM. ITER.= 81

EL= 368		NODES= 368 369		MAT= 3		PRESSURES(Z,Y)= 0.0000		0.0000		AVE. TEMP= 327.00	
END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW				
1		327.00	-5.9924	-.0002187	0.0000000	0.0000000	0.0000000				
2		327.00	-8.2365	-.0003006	0.0000000	0.0000000	0.0000000				
3		327.00	-10.481	-.0003825	0.0000000	0.0000000	0.0000000				
4		327.00	-11.972	-.0004369	0.0000000	0.0000000	0.0000000				
5		327.00	-10.962	-.0004001	0.0000000	0.0000000	0.0000000				
6		327.00	-9.4435	-.0003447	0.0000000	0.0000000	0.0000000				
7		327.00	-7.9246	-.0002892	0.0000000	0.0000000	0.0000000				
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW				
1		327.00	-9.2488	-.0003375	0.0000000	0.0000000	0.0000000				
2		327.00	-9.7400	-.0003555	0.0000000	0.0000000	0.0000000				
3		327.00	-10.231	-.0003734	0.0000000	0.0000000	0.0000000				
4		327.00	-10.557	-.0003853	0.0000000	0.0000000	0.0000000				
5		327.00	-9.7264	-.0003550	0.0000000	0.0000000	0.0000000				
6		327.00	-8.4758	-.0003093	0.0000000	0.0000000	0.0000000				
7		327.00	-7.2251	-.0002637	0.0000000	0.0000000	0.0000000				
FORCES ON MEMBER AT NODE		368	13.6061	.502022	-.143542	-.245830E-01	1.22088	.941094			
		369	-13.5085	-.461356	.184345	.658175E-01	-.565105	.985662			

EL= 369		NODES= 369		MAT= 3		PRESSURES(Z,Y)= 0.0000		0.0000		AVE. TEMP= 327.00	
END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW				
1		327.00	-9.2508	-.0003376	0.0000000	0.0000000	0.0000000				
2		327.00	-9.7406	-.0003555	0.0000000	0.0000000	0.0000000				
3		327.00	-10.230	-.0003734	0.0000000	0.0000000	0.0000000				
4		327.00	-10.556	-.0003853	0.0000000	0.0000000	0.0000000				
5		327.00	-9.7256	-.0003549	0.0000000	0.0000000	0.0000000				
6		327.00	-8.4760	-.0003093	0.0000000	0.0000000	0.0000000				
7		327.00	-7.2264	-.0002637	0.0000000	0.0000000	0.0000000				
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW				
1		327.00	-12.688	-.0004631	0.0000000	0.0000000	0.0000000				
2		327.00	-11.223	-.0004096	0.0000000	0.0000000	0.0000000				
3		327.00	-9.7588	-.0003562	0.0000000	0.0000000	0.0000000				
4		327.00	-8.7858	-.0003206	0.0000000	0.0000000	0.0000000				
5		327.00	-8.3549	-.0003049	0.0000000	0.0000000	0.0000000				
6		327.00	-7.7066	-.0002813	0.0000000	0.0000000	0.0000000				
7		327.00	-7.0583	-.0002576	0.0000000	0.0000000	0.0000000				
FORCES ON MEMBER AT NODE		369	13.5086	.459281	-.186900	-.680038E-01	.564340	.986030			
		6	-13.4109	-.418595	.227652	.109259	.264764	2.74178			

ERASCO SERVICES INCORPORATED

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Client TVA

Project WBNP Unit 1

Subject Qualif. of Worst Cases of Thermally Restrained Structures - Package I

11.5.3.3 Stresses and Strains (Cont'd)

EL= 8		NODES= 6		375 MAT= 3		PRESSURES(Z,Y)= 0.0000		0.0000		AVE. TEMP= 327.00	
END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW				
	1	327.00	-9.1538	-.0003341	0.0000000	0.0000000	0.0000000				
	2	327.00	-9.8823	-.0003607	0.0000000	0.0000000	0.0000000				
	3	327.00	-10.611	-.0003873	0.0000000	0.0000000	0.0000000				
	4	327.00	-11.095	-.0004049	0.0000000	0.0000000	0.0000000				
	5	327.00	-9.7735	-.0003567	0.0000000	0.0000000	0.0000000				
	6	327.00	-7.7846	-.0002841	0.0000000	0.0000000	0.0000000				
	7	327.00	-5.7957	-.0002115	0.0000000	0.0000000	0.0000000				
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW				
	1	327.00	-11.753	-.0004289	0.0000000	0.0000000	0.0000000				
	2	327.00	-10.933	-.0003990	0.0000000	0.0000000	0.0000000				
	3	327.00	-10.113	-.0003691	0.0000000	0.0000000	0.0000000				
	4	327.00	-9.5676	-.0003492	0.0000000	0.0000000	0.0000000				
	5	327.00	-8.6870	-.0003170	0.0000000	0.0000000	0.0000000				
	6	327.00	-7.3616	-.0002687	0.0000000	0.0000000	0.0000000				
	7	327.00	-6.0363	-.0002203	0.0000000	0.0000000	0.0000000				
FORCES ON MEMBER AT NODE				6	13.3471	.378896	-.210537	.923629E-01	.881651	-1.63554	
				375	-13.2698	-.346670	.242787	-.596861E-01	-.163888	2.78436	

EL= 375		NODES= 375		376 MAT= 3		PRESSURES(Z,Y)= 0.0000		0.0000		AVE. TEMP= 327.00	
END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW				
	1	327.00	-11.749	-.0004288	0.0000000	0.0000000	0.0000000				
	2	327.00	-10.932	-.0003990	0.0000000	0.0000000	0.0000000				
	3	327.00	-10.114	-.0003691	0.0000000	0.0000000	0.0000000				
	4	327.00	-9.5712	-.0003493	0.0000000	0.0000000	0.0000000				
	5	327.00	-8.6888	-.0003171	0.0000000	0.0000000	0.0000000				
	6	327.00	-7.3609	-.0002686	0.0000000	0.0000000	0.0000000				
	7	327.00	-6.0330	-.0002202	0.0000000	0.0000000	0.0000000				
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW				
	1	327.00	-14.447	-.0005272	0.0000000	0.0000000	0.0000000				
	2	327.00	-11.964	-.0004366	0.0000000	0.0000000	0.0000000				
	3	327.00	-9.4818	-.0003461	0.0000000	0.0000000	0.0000000				
	4	327.00	-7.8323	-.0002858	0.0000000	0.0000000	0.0000000				
	5	327.00	-7.5246	-.0002746	0.0000000	0.0000000	0.0000000				
	6	327.00	-7.0616	-.0002577	0.0000000	0.0000000	0.0000000				
	7	327.00	-6.5986	-.0002408	0.0000000	0.0000000	0.0000000				
FORCES ON MEMBER AT NODE				375	13.2698	.343911	-.243050	.568193E-01	.165583	-2.78427	
				376	-13.1926	-.311721	.275316	-.241788E-01	.655162	3.82235	

EBASCO SERVICES INCORPORATED

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 Client TVA
 Project WBNP Unit 1
 Subject Qualif. of Worst Cases of Thermally Restrained Structures - Package I

11.5.3.3 Stresses and Strains (Cont'd)

EL= 376 NODES= 376 377 MAT= 3 PRESSURES(Z,Y)= 0.0000 0.0000 AVE. TEMP= 327.00

END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW
1		327.00	-14.445	-.0005272	0.0000000	0.0000000	0.0000000
2		327.00	-11.964	-.0004366	0.0000000	0.0000000	0.0000000
3		327.00	-9.4829	-.0003461	0.0000000	0.0000000	0.0000000
4		327.00	-7.8343	-.0002859	0.0000000	0.0000000	0.0000000
5		327.00	-7.5256	-.0002747	0.0000000	0.0000000	0.0000000
6		327.00	-7.0611	-.0002577	0.0000000	0.0000000	0.0000000
7		327.00	-6.5966	-.0002408	0.0000000	0.0000000	0.0000000
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW
1		327.00	-17.222	-.0006285	0.0000000	0.0000000	0.0000000
2		327.00	-12.972	-.0004734	0.0000000	0.0000000	0.0000000
3		327.00	-8.7230	-.0003184	0.0000000	0.0000000	0.0000000
4		327.00	-5.8994	-.0002153	0.0000000	0.0000000	0.0000000
5		327.00	-6.2923	-.0002296	0.0000000	0.0000000	0.0000000
6		327.00	-6.8835	-.0002512	0.0000000	0.0000000	0.0000000
7		327.00	-7.4747	-.0002728	0.0000000	0.0000000	0.0000000

FORCES ON MEMBER AT NODE 376 13.1927 .308092 -.273192 .209023E-01 -.654213 -3.82255
 377 -13.1154 -.275931 .305478 .117088E-01 1.57044 4.74726

EL= 377 NODES= 377 378 MAT= 3 PRESSURES(Z,Y)= 0.0000 0.0000 AVE. TEMP= 327.00

END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW
1		327.00	-17.223	-.0006286	0.0000000	0.0000000	0.0000000
2		327.00	-12.973	-.0004735	0.0000000	0.0000000	0.0000000
3		327.00	-8.7226	-.0003183	0.0000000	0.0000000	0.0000000
4		327.00	-5.8984	-.0002153	0.0000000	0.0000000	0.0000000
5		327.00	-6.2919	-.0002296	0.0000000	0.0000000	0.0000000
6		327.00	-6.8840	-.0002512	0.0000000	0.0000000	0.0000000
7		327.00	-7.4760	-.0002728	0.0000000	0.0000000	0.0000000
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW
1		327.00	-20.059	-.0007321	0.0000000	0.0000000	0.0000000
2		327.00	-13.951	-.0005092	0.0000000	0.0000000	0.0000000
3		327.00	-7.8435	-.0002863	0.0000000	0.0000000	0.0000000
4		327.00	-3.7847	-.0001381	0.0000000	0.0000000	0.0000000
5		327.00	-4.9985	-.0001824	0.0000000	0.0000000	0.0000000
6		327.00	-6.8252	-.0002491	0.0000000	0.0000000	0.0000000
7		327.00	-8.6518	-.0003158	0.0000000	0.0000000	0.0000000

FORCES ON MEMBER AT NODE 377 13.1156 .271517 -.300677 -.149678E-01 -1.57098 -4.74715
 378 -13.0383 -.239378 .332986 .475572E-01 2.57428 5.55607

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 Chkd. by JOU Date 8/24/91 OFS No. NR Dept. No. NR
 Client TVA
 Project WBNP Unit 1
 Subject Qualif. of Worst Cases of Thermally Restrained Structures - Package I

11.5.3.3 Stresses and Strains (Cont'd)

EL= 378		NODES= 378		379		MAT= 3		PRESSURES(Z,Y)=		0.0000		0.0000		AVE. TEMP= 327.00	
END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW								
1		327.00	-20.064	-.0007323	0.0000000	0.0000000	0.0000000								
2		327.00	-13.952	-.0005092	0.0000000	0.0000000	0.0000000								
3		327.00	-7.8407	-.0002862	0.0000000	0.0000000	0.0000000								
4		327.00	-3.7796	-.0001379	0.0000000	0.0000000	0.0000000								
5		327.00	-4.9963	-.0001823	0.0000000	0.0000000	0.0000000								
6		327.00	-6.8274	-.0002492	0.0000000	0.0000000	0.0000000								
7		327.00	-8.6585	-.0003160	0.0000000	0.0000000	0.0000000								
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW								
1		327.00	-22.939	-.0008372	0.0000000	0.0000000	0.0000000								
2		327.00	-14.895	-.0005436	0.0000000	0.0000000	0.0000000								
3		327.00	-6.8513	-.0002500	0.0000000	0.0000000	0.0000000								
4		327.00	-1.5061	-.0000550	0.0000000	0.0000000	0.0000000								
5		327.00	-3.6531	-.0001333	0.0000000	0.0000000	0.0000000								
6		327.00	-6.8841	-.0002512	0.0000000	0.0000000	0.0000000								
7		327.00	-10.115	-.0003692	0.0000000	0.0000000	0.0000000								
FORCES ON MEMBER AT NODE		378	13.0386	.234266	-.325240	-.503804E-01	-2.57690	-5.55496							
		379	-12.9613	-.202140	.357581	.829562E-01	3.65803	6.24594							

EL= 9		NODES= 7		405		MAT= 3		PRESSURES(Z,Y)=		0.0000		0.0000		AVE. TEMP= 327.00	
END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW								
1		327.00	-17.168	-.0006266	0.0000000	0.0000000	0.0000000								
2		327.00	-12.099	-.0004416	0.0000000	0.0000000	0.0000000								
3		327.00	-7.0300	-.0002566	0.0000000	0.0000000	0.0000000								
4		327.00	-3.6618	-.0001336	0.0000000	0.0000000	0.0000000								
5		327.00	-5.0464	-.0001842	0.0000000	0.0000000	0.0000000								
6		327.00	-7.1300	-.0002602	0.0000000	0.0000000	0.0000000								
7		327.00	-9.2136	-.0003363	0.0000000	0.0000000	0.0000000								
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW								
1		327.00	-12.908	-.0004711	0.0000000	0.0000000	0.0000000								
2		327.00	-10.329	-.0003770	0.0000000	0.0000000	0.0000000								
3		327.00	-7.7507	-.0002829	0.0000000	0.0000000	0.0000000								
4		327.00	-6.0371	-.0002203	0.0000000	0.0000000	0.0000000								
5		327.00	-6.6560	-.0002429	0.0000000	0.0000000	0.0000000								
6		327.00	-7.5874	-.0002769	0.0000000	0.0000000	0.0000000								
7		327.00	-8.5187	-.0003109	0.0000000	0.0000000	0.0000000								
FORCES ON MEMBER AT NODE		7	12.1127	-.563493	.409272	.801966E-01	-2.32056	-3.87408							
		405	-12.0395	.593929	-.378551	-.493348E-01	1.13883	2.13795							

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 Client TVA
 Project WBNP Unit 1
 Subject Qualif. of Worst Cases of Thermally Restrained Structures - Package I

11.5.3.3 Stresses and Strains (Cont'd)

EL= 7		NODES= 2		365		MAT= 3		PRESSURES(Z,Y)=		0.0000		0.0000		AVE. TEMP= 327.00	
END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW								
1		327.00	4.7572	.0001736	0.0000000	0.0000000	0.0000000								
2		327.00	-2.2015	-.0000803	0.0000000	0.0000000	0.0000000								
3		327.00	-9.1602	-.0003343	0.0000000	0.0000000	0.0000000								
4		327.00	-13.784	-.0005031	0.0000000	0.0000000	0.0000000								
5		327.00	-14.371	-.0005245	0.0000000	0.0000000	0.0000000								
6		327.00	-15.254	-.0005567	0.0000000	0.0000000	0.0000000								
7		327.00	-16.137	-.0005890	0.0000000	0.0000000	0.0000000								
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW								
1		327.00	2.4548	.0000896	0.0000000	0.0000000	0.0000000								
2		327.00	-3.6960	-.0001349	0.0000000	0.0000000	0.0000000								
3		327.00	-9.8467	-.0003594	0.0000000	0.0000000	0.0000000								
4		327.00	-13.934	-.0005085	0.0000000	0.0000000	0.0000000								
5		327.00	-13.767	-.0005024	0.0000000	0.0000000	0.0000000								
6		327.00	-13.515	-.0004933	0.0000000	0.0000000	0.0000000								
7		327.00	-13.264	-.0004841	0.0000000	0.0000000	0.0000000								
FORCES ON MEMBER AT NODE		2	13.9972	.650539	.469652E-01	.118505	1.97115	10.1769							
		365	-13.8996	-.609908	-.602864E-02	-.773061E-01	-2.07714	-7.65597							

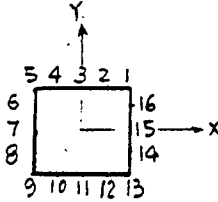
EL= 409		NODES= 409		10		MAT= 3		PRESSURES(Z,Y)=		0.0000		0.0000		AVE. TEMP= 327.00	
END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW								
1		327.00	3.2195	.0001175	0.0000000	0.0000000	0.0000000								
2		327.00	-3.1194	-.0001138	0.0000000	0.0000000	0.0000000								
3		327.00	-9.4583	-.0003452	0.0000000	0.0000000	0.0000000								
4		327.00	-13.670	-.0004989	0.0000000	0.0000000	0.0000000								
5		327.00	-12.398	-.0004525	0.0000000	0.0000000	0.0000000								
6		327.00	-10.482	-.0003826	0.0000000	0.0000000	0.0000000								
7		327.00	-8.5664	-.0003126	0.0000000	0.0000000	0.0000000								
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW								
1		327.00	6.9271	.0002528	0.0000000	0.0000000	0.0000000								
2		327.00	-1.3151	-.0000480	0.0000000	0.0000000	0.0000000								
3		327.00	-9.5573	-.0003488	0.0000000	0.0000000	0.0000000								
4		327.00	-15.034	-.0005487	0.0000000	0.0000000	0.0000000								
5		327.00	-13.615	-.0004969	0.0000000	0.0000000	0.0000000								
6		327.00	-11.479	-.0004189	0.0000000	0.0000000	0.0000000								
7		327.00	-9.3426	-.0003410	0.0000000	0.0000000	0.0000000								
FORCES ON MEMBER AT NODE		409	11.7472	.712716	.245011	.666244E-01	2.67808	5.74040							
		10	-11.6740	-.743188	-.214377	-.975226E-01	-3.36716	-7.92425							

EBASCO SERVICES INCORPORATED

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 Client TVA
 Project WBNP Unit 1
 Subject Qualif. of Worst Cases of Thermally Restrained Structures - Package I

11.5.3.3 Stresses and Strains (Cont'd)



EL= 311		NODES= 311		2 MAT= 1		PRESSURES(Z,Y)=		0.0000		0.0000		AVE. TEMP= 327.00	
END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW						
1		327.00	4.3959	.0001604	0.0000000	0.0000000	0.0000000						
2		327.00	3.5589	.0001299	0.0000000	0.0000000	0.0000000						
3		327.00	2.7220	.0000993	0.0000000	0.0000000	0.0000000						
4		327.00	1.8851	.0000688	0.0000000	0.0000000	0.0000000						
5		327.00	1.0481	.0000383	0.0000000	0.0000000	0.0000000						
6		327.00	-.63618	-.0000232	0.0000000	0.0000000	0.0000000						
7		327.00	-2.3205	-.0000847	0.0000000	0.0000000	0.0000000						
8		327.00	-4.0048	-.0001462	0.0000000	0.0000000	0.0000000						
9		327.00	-5.6891	-.0002076	0.0000000	0.0000000	0.0000000						
10		327.00	-4.8521	-.0001771	0.0000000	0.0000000	0.0000000						
11		327.00	-4.0152	-.0001465	0.0000000	0.0000000	0.0000000						
12		327.00	-3.1782	-.0001160	0.0000000	0.0000000	0.0000000						
13		327.00	-2.3413	-.0000854	0.0000000	0.0000000	0.0000000						
14		327.00	-.65698	-.0000240	0.0000000	0.0000000	0.0000000						
15		327.00	1.0273	.0000375	0.0000000	0.0000000	0.0000000						
16		327.00	2.7116	.0000990	0.0000000	0.0000000	0.0000000						
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW						
1		327.00	12.973	.0004735	0.0000000	0.0000000	0.0000000						
2		327.00	7.4803	.0002730	0.0000000	0.0000000	0.0000000						
3		327.00	1.9877	.0000725	0.0000000	0.0000000	0.0000000						
4		327.00	-3.5049	-.0001279	0.0000000	0.0000000	0.0000000						
5		327.00	-8.9975	-.0003284	0.0000000	0.0000000	0.0000000						
6		327.00	-10.313	-.0003764	0.0000000	0.0000000	0.0000000						
7		327.00	-11.629	-.0004244	0.0000000	0.0000000	0.0000000						
8		327.00	-12.945	-.0004725	0.0000000	0.0000000	0.0000000						
9		327.00	-14.261	-.0005205	0.0000000	0.0000000	0.0000000						
10		327.00	-8.7687	-.0003200	0.0000000	0.0000000	0.0000000						
11		327.00	-3.2761	-.0001196	0.0000000	0.0000000	0.0000000						
12		327.00	2.2165	.0000809	0.0000000	0.0000000	0.0000000						
13		327.00	7.7091	.0002814	0.0000000	0.0000000	0.0000000						
14		327.00	9.0251	.0003294	0.0000000	0.0000000	0.0000000						
15		327.00	10.341	.0003774	0.0000000	0.0000000	0.0000000						
16		327.00	11.657	.0004254	0.0000000	0.0000000	0.0000000						
FORCES ON MEMBER AT NODE		311	2.42467	-13.3031	1.06774	-6.34308	-15.7902	7.84635					
		2	-2.41577	13.2442	-1.03256	6.34308	12.3371	-51.4932					

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11.5.3.3 Stresses and Strains (Cont'd)

EL= 391		NODES= 391		10 MAT= 2		PRESSURES(Z,Y)= 0.0000		0.0000		AVE. TEMP= 327.00	
END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW				
	1	327.00	-3.4490	-.0001259	0.0000000	0.0000000	0.0000000				
	2	327.00	-1.0966	-.0000400	0.0000000	0.0000000	0.0000000				
	3	327.00	1.2559	.0000458	0.0000000	0.0000000	0.0000000				
	4	327.00	3.6083	.0001317	0.0000000	0.0000000	0.0000000				
	5	327.00	5.9608	.0002175	0.0000000	0.0000000	0.0000000				
	6	327.00	5.3859	.0001966	0.0000000	0.0000000	0.0000000				
	7	327.00	4.8111	.0001756	0.0000000	0.0000000	0.0000000				
	8	327.00	4.2362	.0001546	0.0000000	0.0000000	0.0000000				
	9	327.00	3.6613	.0001336	0.0000000	0.0000000	0.0000000				
	10	327.00	1.3089	.0000478	0.0000000	0.0000000	0.0000000				
	11	327.00	-1.0436	-.0000381	0.0000000	0.0000000	0.0000000				
	12	327.00	-3.3960	-.0001239	0.0000000	0.0000000	0.0000000				
	13	327.00	-5.7485	-.0002098	0.0000000	0.0000000	0.0000000				
	14	327.00	-5.1736	-.0001888	0.0000000	0.0000000	0.0000000				
	15	327.00	-4.5988	-.0001678	0.0000000	0.0000000	0.0000000				
	16	327.00	-4.0239	-.0001469	0.0000000	0.0000000	0.0000000				
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW				
	1	327.00	-10.463	-.0003819	0.0000000	0.0000000	0.0000000				
	2	327.00	-4.8336	-.0001764	0.0000000	0.0000000	0.0000000				
	3	327.00	.79603	.0000291	0.0000000	0.0000000	0.0000000				
	4	327.00	6.4257	.0002345	0.0000000	0.0000000	0.0000000				
	5	327.00	12.055	.0004400	0.0000000	0.0000000	0.0000000				
	6	327.00	11.710	.0004274	0.0000000	0.0000000	0.0000000				
	7	327.00	11.365	.0004148	0.0000000	0.0000000	0.0000000				
	8	327.00	11.021	.0004022	0.0000000	0.0000000	0.0000000				
	9	327.00	10.676	.0003896	0.0000000	0.0000000	0.0000000				
	10	327.00	5.0459	.0001842	0.0000000	0.0000000	0.0000000				
	11	327.00	-5.58375	-.0000213	0.0000000	0.0000000	0.0000000				
	12	327.00	-6.2134	-.0002268	0.0000000	0.0000000	0.0000000				
	13	327.00	-11.843	-.0004322	0.0000000	0.0000000	0.0000000				
	14	327.00	-11.498	-.0004196	0.0000000	0.0000000	0.0000000				
	15	327.00	-11.153	-.0004070	0.0000000	0.0000000	0.0000000				
	16	327.00	-10.808	-.0003945	0.0000000	0.0000000	0.0000000				
FORCES ON MEMBER AT NODE		391	-.398057	9.43107	.676431	4.06414	-5.38930	-22.0543			
		10	.398032	-9.47586	-.649990	-4.06414	3.23387	52.7780			

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11.5.4 Member Qualification

As presented in Section 11.5.3, the stress and strain results of the analysis indicate that no yielding occurs in all elements at the final temperature step (temperature=327°F). Therefore, the ductility ratio at any node of the proximity member or the supporting members is less than 1. Note that the ductility ratio is defined as the ratio of the final displacement to the yield displacement.

Per DG-C1.6.12, Appendix D, the members of worst case 2&3-8(G) are qualified.

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11.5.5 Connection and Anchorage Qualification for Worst Case 2&3-8(G)

11.5.5.1 Connection Qualification for Worst Case 2&3-8(G)

The connections at nodes 2 and 10, as well as the connections at support nodes 1 and 9, are evaluated. The evaluation of connections are documented in the next four sheets.

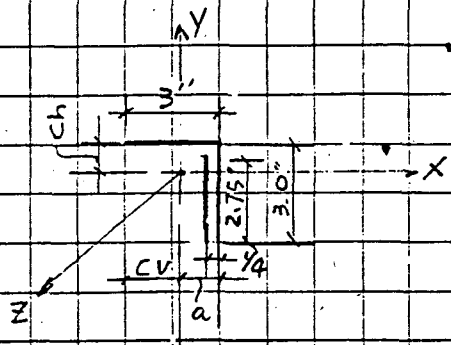
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		REF.
<u>PRBG-48N967-1</u>		
LOADS FROM ANSYS OUTPUT - DATED 8-15-91		File: PRBGR
<u>LOADS</u>		
<u>NODE 2 (ELEMENT 7)</u>		
$F_x = 14.00^k$	$M_x = 0.12''-k$	
$F_y = 0.65^k$	$M_y = 1.97''-k$	
$F_z = 0.05^k$	$M_z = 10.18''-k$	
<u>NODE 10 (ELEMENT 409)</u>		
$F_x = -11.67^k$	$M_x = 0.10''-k$	
$F_y = 0.74^k$	$M_y = -3.37''-k$	
$F_z = -0.21^k$	$M_z = -7.92''-k$	
<u>FORCES ON WELDS</u>		
<u>NODE 2</u>		
$F_x = -0.65^k$	$M_x = -1.97''-k$	
$F_y = -0.05^k$	$M_y = -10.18''-k$	
$F_z = -14.00^k$	$M_z = -0.12''-k$	
<u>NODE 10</u>		
$F_x = -0.74^k$	$M_x = 3.37''-k$	
$F_y = 0.21^k$	$M_y = 7.92''-k$	
$F_z = 11.67^k$	$M_z = -0.10''-k$	

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WELD SIZE = $3/16$ "

REF-

File:
PR86R

$$a = \frac{1 \times 3 \times 0 + 1 \times 2.75 \times 0.25 + 1 \times 3 \times 1.5}{3 + 2.75 + 3} = 0.594''$$

$$C_v = 3 - 0.594 = 2.406''$$

$$b = \frac{1 \times 3 \times 1.5 + 1 \times 2.75 \times \frac{2.75}{2} + 1 \times 3 \times 3}{3 + 2.75 + 3} = 1.975''$$

$$C_h = 3 - 1.975 = 1.025''$$

WELD PROPERTIES

$$A_w = 3 + 3 + 2.75 = 8.75 \text{ IN}^2$$

$$I_x = \frac{3^3}{12} + \frac{2.75^3}{12} + 3 \times (1.975 - 1.5)^2 + 2.75 \times \left(1.975 - \frac{2.75}{2}\right)^2 + 3 \times 1.025^2$$

$$= 2.25 + 1.73 + 0.68 + 0.99 + 3.15 = 8.80 \text{ IN}^3$$

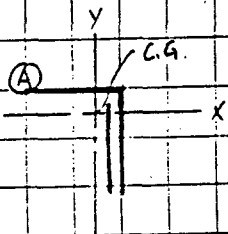
$$I_y = \frac{3^3}{12} + 3 \times 0.594^2 + 2.75 \times (0.594 - 0.25)^2 + 3 \times (1.5 - 0.594)^2$$

$$= 2.25 + 1.06 + 0.33 + 0.33 + 2.46 = 6.1 \text{ IN}^3$$

$$J_w = I_x + I_y = 8.8 + 6.1 = 14.9 \text{ IN}^3$$

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CRITICAL STRESS AT PT. (A)

$$S_{xA} = \frac{I_x}{C_x} = \frac{8.80}{1.025} = 8.59 \text{ in}^2$$

$$S_{yA} = \frac{I_y}{C_y} = \frac{6.10}{2.406} = 2.54 \text{ in}^2$$

NODE 2

$$f_t = \frac{F_z}{A_w} + \frac{M_x}{S_x} + \frac{M_y}{S_y} = \frac{14.00}{8.75} + \frac{1.97}{8.59} + \frac{10.18}{2.54} = 1.6 + 0.23 + 4.0 = 5.84 \text{ k/in}$$

$$f_x = \frac{F_x}{A_w} + \frac{M_z \cdot C_h}{J_w} = \frac{0.65}{8.75} + \frac{0.12 \times 1.025}{14.9} = 0.08 \text{ k/in}$$

$$f_y = \frac{F_y}{A_w} + \frac{M_z \cdot C_v}{J_w} = \frac{0.05}{8.75} + \frac{0.12 \times 2.406}{14.9} = 0.03 \text{ k/in}$$

$$f_r = \sqrt{(5.84)^2 + (-0.08)^2 + (-0.03)^2} = 5.84 \text{ k/in}$$

$$\text{WELD CAPACITY} = 0.707 \times \frac{2}{3} \times 70 \times \frac{3}{16} = 6.19 \text{ k/in} > 5.84 \text{ k/in} \text{ O.K.}$$

NODE 10

$$f_t = \frac{11.67}{8.75} + \frac{3.37}{8.59} + \frac{7.92}{2.54} = 1.33 + 0.39 + 3.12 = 4.84$$

$$f_x = \frac{0.74}{8.75} + \frac{0.10 \times 1.025}{14.9} = 0.09$$

$$f_y = \frac{0.21}{8.75} + \frac{0.10 \times 2.406}{14.9} = 0.01$$

$$f_r = \sqrt{(4.84)^2 + (-0.09)^2 + (-0.01)^2} = 4.84 \text{ k/in} < 6.19 \text{ k/in} \text{ O.K.}$$

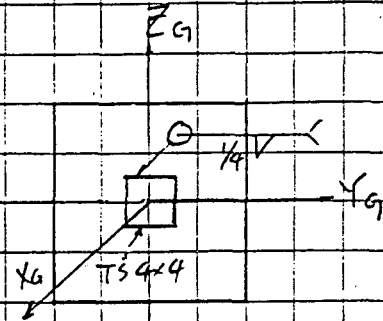
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PRG - 48N967-1

CHECK WELDS BETWEEN
 T54x4 & BASE PL

LOADS FROM ANSYS OUTPUT PRGR
 DATED 8/15/91

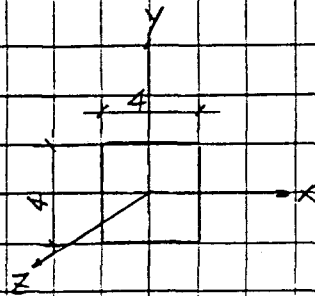


LOADS AT NODE 1, 101, 412, & 413

$$\begin{aligned} F_x &= -0.40 \text{ K} & M_x &= 9.24 \\ F_y &= -3.58 \text{ K} & M_y &= 18.7 \\ F_z &= -1.10 \text{ K} & M_z &= 35.98 \end{aligned}$$

FORCES ON WELDS

$$\begin{aligned} F_x &= 13.58 \text{ K} & M_x &= 18.7 \text{ ''-K} \\ F_y &= -1.10 \text{ K} & M_y &= 35.98 \text{ ''-K} \\ F_z &= -0.40 \text{ K} & M_z &= 9.24 \text{ ''-K} \end{aligned}$$



WELD SIZE $\frac{1}{4}$

$$\begin{aligned} A_w &= 4 \times 4 = 16 \text{ IN}^2 \\ S_x &= S_y = 4 \times 4 + \frac{4^2}{3} = 21.33 \text{ IN}^3 \\ J_w &= (4+4)^3 / 6 = 85.33 \text{ IN}^4 \end{aligned}$$

$$f_t = \frac{0.4}{16} + \frac{18.7}{21.33} + \frac{35.98}{21.33} = 2.51 \text{ K/IN}$$

$$f_x = \frac{13.58}{16} + \frac{9.24 \times 2}{85.33} = 1.07 \text{ K/IN}$$

$$f_y = \frac{-1.10}{16} + \frac{9.24 \times 2}{85.33} = 0.15 \text{ K/IN}$$

$$f_r = (2.51^2 + 1.07^2 + 0.15^2)^{1/2} = 2.73 \text{ K/IN}$$

$$\text{WELD CAPACITY} = 0.707 \times \sqrt{3} \times 70 \times \frac{1}{4} = 8.25 \text{ K/IN} > 2.73 \text{ K/IN} \text{ O.K.}$$

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11.5.5.2 Anchorage Qualification for Worst Case 2&3-8(G)

For shear loading, the 1" ϕ Wedge Bolts are accepted by comparing the displacement at the anchorage to the displacement allowable per Design Guide DG-C1.6.12 R1. The displacement allowable is 0.2D (0.2 * bolt diameter) for ductile anchors (The Wedge Bolt is ductile in shear per DG-C1.6.12 R1). As shown in Section 11.5.3.2, the maximum Y-displacement at support occurs at node 1 (0.0397").

$$0.0397" < 0.2D = 0.2 * 1" = 0.2" \quad \text{O.K.}$$

A Baseplate II analysis is performed using the results at node 1 obtain the tension in the anchor bolts. Note that the reaction at node 1 envelop the reactions at node 9. The input and output of the Baseplate II analysis are presented in Table 11.5.5.2.1. The maximum tension is 4.689 kips. The tension allowable is obtained from Table 5 of DS-C1.7.1 for the 1" ϕ Wedge Bolts.

$$4.689 \text{ kips} < \text{Tension Allowable} = 5.20 \text{ kips} \quad \text{O.K.}$$

No G-32 violation is identified for the anchorages at nodes 1 and 9 per Calc. WCG-1-790 R0, Section 11.6.

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Table 11.5.5.2.1 BASEPLATE II Input and Output for Anchorage at Nodes 1

(Ref: Input = B1PR8G ; Output = B1PR8GR)

```

1 3 B1PR8G-NODE 1; SURF. MOUNT. PLATE; 48W967-1
2 OUT ,,-1,100/
3 CON ,,,3000,1.7,1.7/
4 PLA 15,15,,14,16,.75,29/
5 APR 4,4,1,4,4,.25,.25/
6 BPR ,,1,.4E6,1.0E6/
7 BOL 4,5,1,3.25,4.75/
8 BOL 12,5,1,10.75,4.75/
9 BOL 4,12,1,3.25,12/
10 BOL 12,12,1,10.75,12/
11 END /
12 TUB 8,11,1,7,11.5/
13 END /
14 POI 8,11/
15 LOA ,,1,13580,-1100,0,18170,35980,9240/
16 END /
17 END OF JOB
    
```

```

*****
*
*   B1PR8G-NODE 1; SURF. MOUNT. PLATE; 48W967-1
*
*****
    
```

B O L T L O A D S

I LINE	J LINE	NODE	X-SHEAR	Y-SHEAR	SRSS	Z-TENSION
4	5	50	2234.	842.	2387.	818.
12	5	170	2239.	-1385.	2633.	0.
4	12	57	4554.	795.	4623.	4689.
12	12	177	4553.	-1352.	4750.	353.

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11.6 Qualification of Worst Case 5-7

11.6.1 Mathematical Model (HD07)

The ANSYS model HD07 is shown in Figure 11.6.1.1. This model considers the continuous member (C8x11.5) with Header Restraints. The member is welded to embedded plates at six (6) locations. ANSYS STIF24 elements (3-D thin-wall plastic beam elements) are used to simulate this member. Temperature dependent material properties, as described in Section 11.2, are used.

Boundary Condition

The modeling of boundary condition is based on information obtained from WCG-1-830 (Walkdown) and WCG-1-964 (Evaluation of Embedded Plates for Platforms). Support nodes 1 and 6 are modeled as pinned connection and support nodes 2 through 5 are moment connection. ANSYS STIF14 elements (Spring elements) are specified at nodes 1 through 6 to simulate the shear stiffness of the concrete anchor bolts in X direction. The anchorage information is obtained from Drawings 48N1219-2 and 48N1219-3 (Plate MK 8W). There are four (4) 5/8" ϕ Welded Studs at support nodes 1 and 6; Six (6) 5/8" Weld Studs at support nodes 2 through 5. The shear stiffness of the anchor bolts are obtained from Calculation CSG-91-0003. 600 k/in per Weld Stud is considered as the shear stiffness for the STIF14 spring elements. Nodes 101 through 106 are defined at the same locations of nodes 1 through 6 as the support nodes for the spring elements.

Loading Condition

There is no attachment on the member per calc. WCG-1-830 (Walkdown package). SSE loads are applied using the equivalent static approach. Since the member (welded to embedded plate) is against the concrete wall, the XZ-SSE (combination of SSE loads of

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X and Z (vertical) directions) is considered as the SSE load in the model. Dead weight (DL) of the member is combined with the XZ-SSE loads followed by incremental temperature steps from 70°F (Ambient Temperature) to 210°F (Accident Temperature).

The ANSYS input file is presented in Section 11.6.2.

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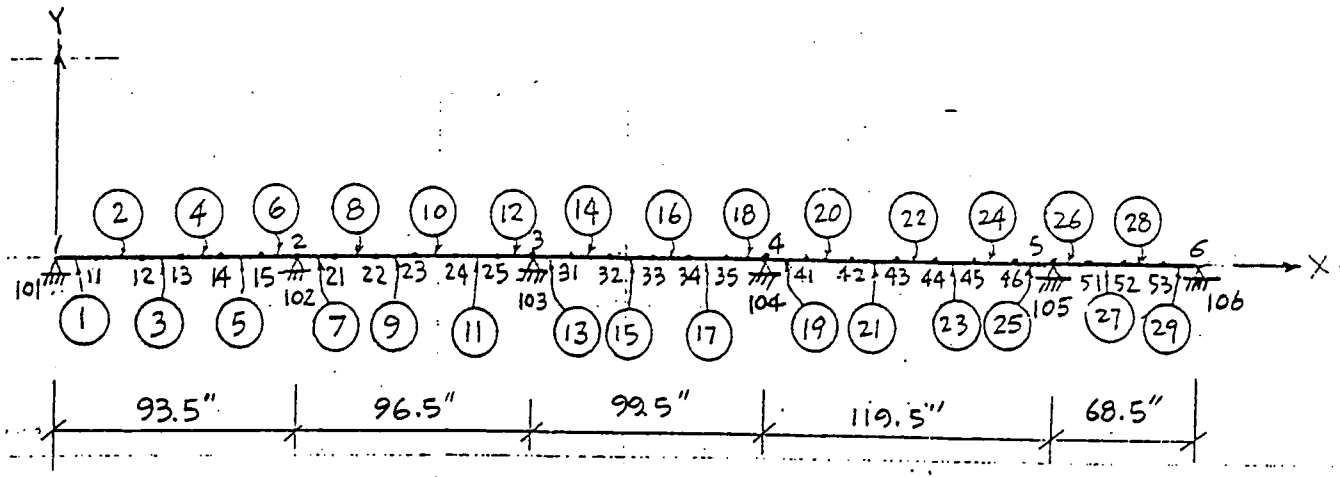
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Figure 11.6.1.1 ANSYS Model for Worst Case 5-7



PLAN VIEW

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11.6.2 ANSYS Input

***** ANSYS INPUT DATA LISTING (FILE18) *****

```

1 /PREP7
2 /TITLE,HD07 - EVAL. FOR HEADER RESTRAINT CASE 5-7, 48N1210-16, EL. 713'-0"
3 /COM
4 /COM BRANCH/PROJECT ID = WCG-1-969
5 /COM
6 /COM PREPARED BY SAL 8/26/91
7 /COM CHECKED BY J. OU 8/26/91
8 /COM
9 /COM INPUT = HD07
10 /COM OUTPUT = HD07R
11 /COM FILE12 = HD07F
12 /COM PLOT = HD07P
13 /COM
14 C*** ANSYS ANALYSIS OPTIONS AND REFERENCES TEMPERATURE
15 KAN,0
16 TREF,70
17 KAY,6,1
18 KNL,1
19 C***
20 C*** ELEMENT TYPE AND MATERIAL PROPERTIES
21 ET,1,24,,,,,1
22 C*** ET,1,4,,,,,1
23 ET,2,14,,1
24 EX,1,29000
25 ALPX,1,6.5E-6
26 DENS,1,1.531E-3 * CBX11.5
27 MPTEMP,1,70,100,200,300,400
28 MPDATA,EX,1,1,29.0E3,28.9E3,28.3E3,27.9E3,27.4E3
29 MPDATA,ALPX,1,1,6.50E-6,6.50E-6,6.50E-6,6.67E-6,6.86E-6
30 NL,1,13,10
31 NL,1,19,70,100,200,300,400
32 NL,1,25,36,36.0,36.0,36.0,36.0
33 NL,1,31,150,150,150,150,150
34 C***
35 C*** MEMBER CROSS SECTIONAL PROPERTIES
36 C*** R,1,3.38,1.32,32.6,8.00,2.26, * CBX11.5
37 C***
38 C*** CHANNEL REAL CONSTRAINTS
39 C***
40 *CREAT,RCHA
41 CF=ARG1
42 TCF=ARG2
43 CD=ARG3
44 TCW=ARG4
45 XBR=ARG5
46 XO=(TCW/2)-(XBR)
47 CC=CF-(TCW/2)
48 DD=(CD-TCF)/2.
49 QC=CC/2
50 QD=DD/2
    
```

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11.6.2 ANSYS Input (Cont'd)

```

51 CC=CC+X0
52 QC=QC+X0
53 *STAT
54 C***
55 R,ARG6,CC,DD,0,QC,DD,TCF
56 RMORE,X0,DD,TCF,X0,QD,TCW
57 RMORE,X0,0,TCW,X0,-QD,TCW
58 RMORE,X0,-DD,TCW,QC,-DD,TCF
59 RMORE,CC,-DD,TCF
60 *END
61 *USE,RCHA,2.26,0.39,8,0.22,0.571,1 * C8X11.5
62 C***
63 R,2,(600*6) * SPRING STIFFNESS FOR 6 ANCHOR BOLTS
64 R,3,(600*4) * SPRING STIFFNESS FOR 4 ANCHOR BOLTS
65 C***
66 C*** NODES AND COORDINATES
67 C***
68 N,1,0,0
69 N,2,93.50,0.00
70 N,3,190.00,0.00
71 N,4,289.50,0.00
72 N,5,409.00,0.00
73 N,6,477.50,0.00
74 N,501,0,0,100
75 NGEN,2,100,1,6,1
76 C***
77 FILL,1,2,5,11,1
78 FILL,2,3,5,21,1
79 FILL,3,4,5,31,1
80 FILL,4,5,6,41,1
81 FILL,5,6,3,51,1
82 C***
83 C*** ELEMENT CONNECTIVITIES
84 MAT,1 $TYPE,1
85 REAL,1
86 E,1,11,501 $E,11,12,501 $RP4,1,1 $E,15,2,501
87 E,2,21,501 $E,21,22,501 $RP4,1,1 $E,25,3,501
88 E,3,31,501 $E,31,32,501 $RP4,1,1 $E,35,4,501
89 E,4,41,501 $E,41,42,501 $RP5,1,1 $E,46,5,501
90 E,5,51,501 $E,51,52,501 $RP2,1,1 $E,53,6,501
91 C***
92 TYPE,2
93 REAL,3
94 E,101,1
95 REAL,2
96 E,102,2
97 RP4,1,1
98 REAL,3
99 E,106,6
100 C***
    
```

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11.6.2 ANSYS Input (Cont'd)

```

101 C*** BOUNDARY CONDITIONS
102 NSEL,,101,106
103 D,ALL,ALL
104 NALL
105 D,1,UY,,,6,5,UZ
106 D,2,UY,,,5,1,UZ,ROTX,ROTY,ROTZ
107 C***
108 C*** PLOTS - GEOMETRY W/B.C.
109 /SHOW
110 /PNUM,NODE,1
111 EPLO
112 /SHRINK,0.3
113 /PNUM,ELEM,1
114 EPLO
115 /PBC,ALL,1
116 EPLO
117 C*** OPERATION OPTIONS
118 ITER,-20,20
119 POSTR,,1,5
120 C***
121 C*** LOADING CASES
122 *CREATE,MTEM
123 TUNIF,ARG1
124 LWRITE
125 *END
126 C***
127 C***
128 C*** SEISMIC 4% PEAK SSE ACC.(G): NS=1.4, EW=1.65, VT=0.38
129 C*** FACTOR: MMF=1.5
130 MMF=1.5 $GX=MMF*1.4 $GY=MMF*1.65 $GZ=MMF*0.38
131 C*** DL+SSE-XZ
132 ACEL,GX,0,GZ+1
133 C***
134 *USE,MTEM,70
135 *USE,MTEM,140
136 RPB,,10
137 AFWRITE,,1
138 FINISH
139 /INPUT,27
140 FINISH
141 /POST1
142 *CREAT,MAC
143 SET,ARG1
144 PRRFOR
145 PRDISP
146 PLDISP
147 PLDISP,1
148 *END
149 /PBC,TDIS,1
150 /USER
151 /FOCUS,,234.75,,50
152 /DIST,,290
153 *USE,MAC,1
154 *USE,MAC,2
155 *USE,MAC,9
156 FINISH

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11.6.3 Analysis Results

The ANSYS output for the analysis "HD07" is stored in file "HD07R". The analysis results of the final load step (Load Step 9) are provided in this Section.

The deformed structure is plotted in Figure 11.6.3.1. Note that the displacements in this plot are magnified. The maximum lateral displacement occurs at node 12.

The reaction forces and the nodal displacements are provided in Sections 11.6.3.1 and 11.6.3.2 respectively. Due to thermal expansion, nodes 1, 2 and 3 move in -X direction and nodes 4, 5, and 6 move in +X direction, as shown in the displacement output in Section 11.6.3.2. The maximum X-displacement occurs at nodes 1 and secondly, node 6.

The stresses and strains for elements near nodes 1 and 6 (elements 1 through 4 and 26 through 29) are presented in Section 11.6.3.3. These elements also include the elements (elements 2 and 3) containing the node (21) with maximum lateral displacement. The following explanations are provided for the labels shown in the stress and strain output.

- PT: Segment Point Number, see Figure 11.2.2 for the definition of segment point for cross-section of Channels.
- TEMP: Temperature
- SIGX: Axial Stress
- EP: Axial Elastic Strain
- EPPL: Axial Plastic Strain

The stress and strain results from output file "HD07R" indicate that no yielding occurs in the structure at the final load step (temperature = 210°F).

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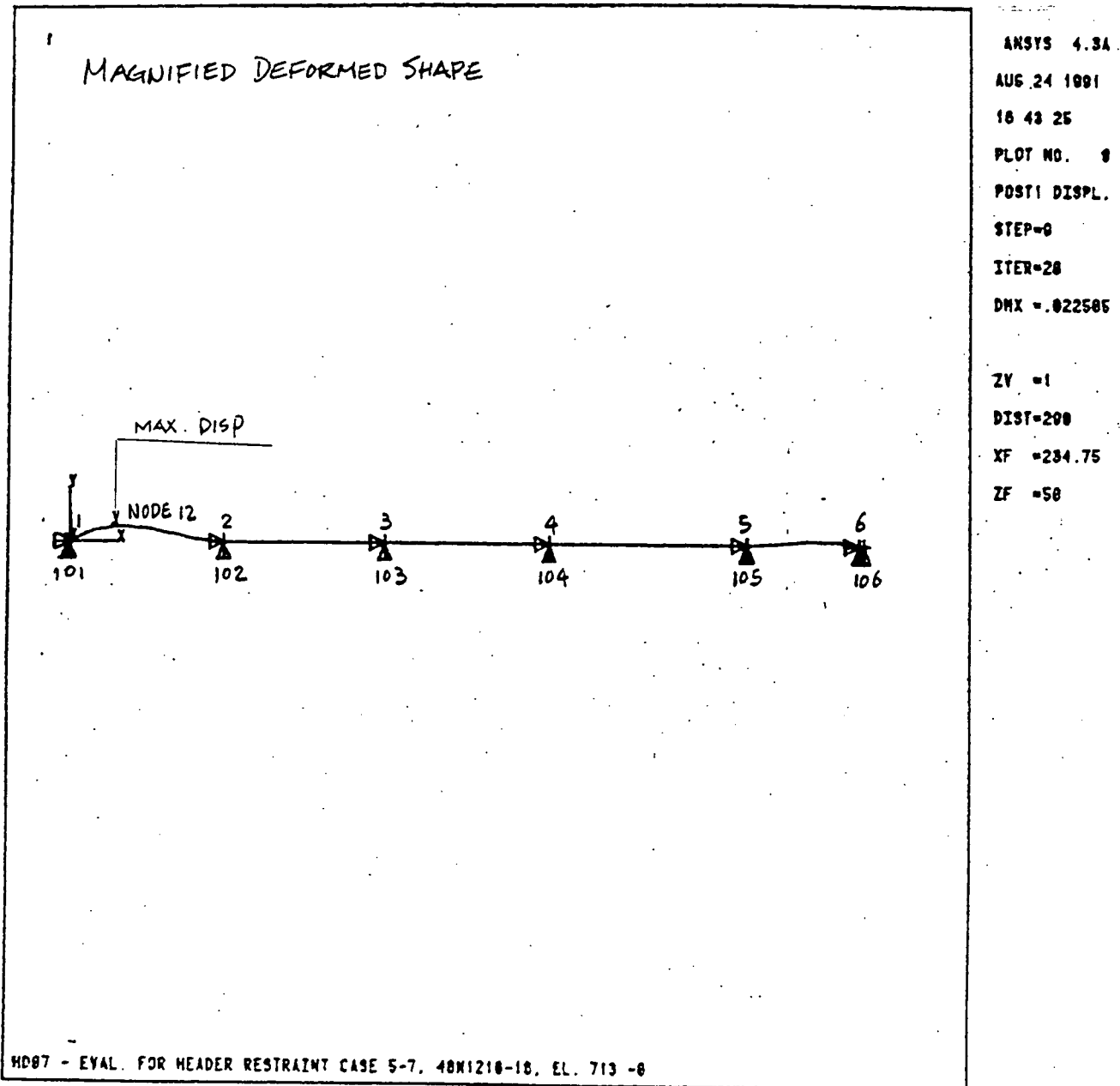
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Figure 11.6.3.1 Deformed Shape of the Model HD07 (Worst Case 5-7)



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11.6.3.1 Reaction Forces

HD07 - EVAL. FOR HEADER RESTRAINT CASE 5-7, 48N1210-16, EL. 713'-0"

***** POST1 REACTION FORCE LISTING *****

LOAD STEP 9 ITERATION= 20 SECTION= 1
 TIME= 0.0000 LOAD CASE= 1

THE FOLLOWING X,Y,Z FORCES ARE IN NODAL COORDINATES

NODE	FX	FY	FZ	MX	MY	MZ
1		.12179357	.29571828			
2		-.12179227	.84606433	.34736335E-01	.44170549	4.7244443
3		.37460977E-06	.78940432	.13486441E-03	-.45424018	-.29826161
4		.58948527E-05	.88201529	.48876782E-03	-3.2479480	.62498699E-01
5		-.11646035	.80115569	.75929137E-02	7.5494518	-1.5600647
6		.11645278	.23199001			
101	64.507925	0.0000000	0.0000000	0.0000000	0.0000000	0.0000000
102	19.219873	0.0000000	0.0000000	0.0000000	0.0000000	0.0000000
103	3.8091038	0.0000000	0.0000000	0.0000000	0.0000000	0.0000000
104	-1.6655594	0.0000000	0.0000000	0.0000000	0.0000000	0.0000000
105	-21.147825	0.0000000	0.0000000	0.0000000	0.0000000	0.0000000
106	-59.578720	0.0000000	0.0000000	0.0000000	0.0000000	0.0000000
TOTAL	5.1447966	.42188475E-12	3.8463479	.42952880E-01	4.2889691	2.9286167

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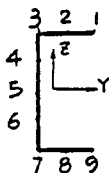
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11.6.3.3 Stresses and Strains



HD07 - EVAL. FOR HEADER RESTRAINT CASE 5-7, 48N1210-16, EL. 713'-0"

***** ELEMENT STRESSES ***** TIME = 0.00000 LOAD STEP= 9 ITERATION= 20 CUM. ITER.= 40

EL=	1	NODES=	1 11	MAT=	1	PRESSURES(Z,Y)=	0.0000	0.0000	AVE. TEMP=	210.00
END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW			
	1	210.00	-14.292	-.0005057	0.0000000	0.0000000	0.0000000			
	2	210.00	-17.598	-.0006227	0.0000000	0.0000000	0.0000000			
	3	210.00	-20.903	-.0007397	0.0000000	0.0000000	0.0000000			
	4	210.00	-20.903	-.0007397	0.0000000	0.0000000	0.0000000			
	5	210.00	-20.903	-.0007397	0.0000000	0.0000000	0.0000000			
	6	210.00	-20.903	-.0007397	0.0000000	0.0000000	0.0000000			
	7	210.00	-20.903	-.0007397	0.0000000	0.0000000	0.0000000			
	8	210.00	-17.598	-.0006227	0.0000000	0.0000000	0.0000000			
	9	210.00	-14.292	-.0005057	0.0000000	0.0000000	0.0000000			
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW			
	1	210.00	-12.821	-.0004537	0.0000000	0.0000000	0.0000000			
	2	210.00	-17.403	-.0006158	0.0000000	0.0000000	0.0000000			
	3	210.00	-21.984	-.0007779	0.0000000	0.0000000	0.0000000			
	4	210.00	-21.738	-.0007692	0.0000000	0.0000000	0.0000000			
	5	210.00	-21.492	-.0007605	0.0000000	0.0000000	0.0000000			
	6	210.00	-21.246	-.0007518	0.0000000	0.0000000	0.0000000			
	7	210.00	-20.999	-.0007431	0.0000000	0.0000000	0.0000000			
	8	210.00	-16.418	-.0005810	0.0000000	0.0000000	0.0000000			
	9	210.00	-11.836	-.0004188	0.0000000	0.0000000	0.0000000			
FORCES ON MEMBER AT NODE										
		1	64.5077	-.123927	.331511	.420218	-.444028E-11	4.96381		
		11	-64.3399	.121906	-.205902	-.260999	-4.18734	-6.87926		

EL=	2	NODES=	11 12	MAT=	1	PRESSURES(Z,Y)=	0.0000	0.0000	AVE. TEMP=	210.00
END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW			
	1	210.00	-12.807	-.0004532	0.0000000	0.0000000	0.0000000			
	2	210.00	-17.398	-.0006156	0.0000000	0.0000000	0.0000000			
	3	210.00	-21.988	-.0007781	0.0000000	0.0000000	0.0000000			
	4	210.00	-21.742	-.0007694	0.0000000	0.0000000	0.0000000			
	5	210.00	-21.496	-.0007607	0.0000000	0.0000000	0.0000000			
	6	210.00	-21.250	-.0007520	0.0000000	0.0000000	0.0000000			
	7	210.00	-21.005	-.0007433	0.0000000	0.0000000	0.0000000			
	8	210.00	-16.414	-.0005808	0.0000000	0.0000000	0.0000000			
	9	210.00	-11.824	-.0004184	0.0000000	0.0000000	0.0000000			
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW			
	1	210.00	-13.293	-.0004704	0.0000000	0.0000000	0.0000000			
	2	210.00	-17.686	-.0006258	0.0000000	0.0000000	0.0000000			
	3	210.00	-22.080	-.0007813	0.0000000	0.0000000	0.0000000			
	4	210.00	-21.714	-.0007684	0.0000000	0.0000000	0.0000000			
	5	210.00	-21.348	-.0007554	0.0000000	0.0000000	0.0000000			
	6	210.00	-20.981	-.0007424	0.0000000	0.0000000	0.0000000			
	7	210.00	-20.615	-.0007295	0.0000000	0.0000000	0.0000000			
	8	210.00	-16.222	-.0005740	0.0000000	0.0000000	0.0000000			
	9	210.00	-11.829	-.0004186	0.0000000	0.0000000	0.0000000			
FORCES ON MEMBER AT NODE										
		11	64.3401	.179490E-01	.194094	.237244	4.18116	6.89262		
		12	-64.1722	-.200048E-01	-.685223E-01	-.780714E-01	-6.22738	-6.59689		

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11.6.3.3 Stresses and Strains (Cont'd)

EL=	3	NODES=	12	13	MAT=	1	PRESSURES(Z,Y)=	0.0000	0.0000	AVE. TEMP=	210.00
END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW				
	1	210.00	-13.288	-.0004702	0.0000000	0.0000000	0.0000000				
	2	210.00	-17.685	-.0006258	0.0000000	0.0000000	0.0000000				
	3	210.00	-22.081	-.0007814	0.0000000	0.0000000	0.0000000				
	4	210.00	-21.715	-.0007684	0.0000000	0.0000000	0.0000000				
	5	210.00	-21.349	-.0007555	0.0000000	0.0000000	0.0000000				
	6	210.00	-20.983	-.0007425	0.0000000	0.0000000	0.0000000				
	7	210.00	-20.617	-.0007295	0.0000000	0.0000000	0.0000000				
	8	210.00	-16.220	-.0005740	0.0000000	0.0000000	0.0000000				
	9	210.00	-11.824	-.0004184	0.0000000	0.0000000	0.0000000				
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW				
	1	210.00	-15.632	-.0005532	0.0000000	0.0000000	0.0000000				
	2	210.00	-18.422	-.0006519	0.0000000	0.0000000	0.0000000				
	3	210.00	-21.213	-.0007506	0.0000000	0.0000000	0.0000000				
	4	210.00	-20.854	-.0007379	0.0000000	0.0000000	0.0000000				
	5	210.00	-20.495	-.0007252	0.0000000	0.0000000	0.0000000				
	6	210.00	-20.136	-.0007125	0.0000000	0.0000000	0.0000000				
	7	210.00	-19.778	-.0006999	0.0000000	0.0000000	0.0000000				
	8	210.00	-16.988	-.0006011	0.0000000	0.0000000	0.0000000				
	9	210.00	-14.197	-.0005024	0.0000000	0.0000000	0.0000000				
FORCES ON MEMBER AT NODE			12	64.1720	.153895	.547295E-01	.479504E-01	6.22600	6.60192		
			13	-64.0042	-.155702	.708043E-01	.111175	-6.10075	-4.18964		

EL=	4	NODES=	13	14	MAT=	1	PRESSURES(Z,Y)=	0.0000	0.0000	AVE. TEMP=	210.00
END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW				
	1	210.00	-15.642	-.0005535	0.0000000	0.0000000	0.0000000				
	2	210.00	-18.425	-.0006520	0.0000000	0.0000000	0.0000000				
	3	210.00	-21.209	-.0007505	0.0000000	0.0000000	0.0000000				
	4	210.00	-20.850	-.0007378	0.0000000	0.0000000	0.0000000				
	5	210.00	-20.492	-.0007251	0.0000000	0.0000000	0.0000000				
	6	210.00	-20.133	-.0007124	0.0000000	0.0000000	0.0000000				
	7	210.00	-19.775	-.0006997	0.0000000	0.0000000	0.0000000				
	8	210.00	-16.991	-.0006012	0.0000000	0.0000000	0.0000000				
	9	210.00	-14.207	-.0005027	0.0000000	0.0000000	0.0000000				
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW				
	1	210.00	-19.070	-.0006748	0.0000000	0.0000000	0.0000000				
	2	210.00	-19.354	-.0006849	0.0000000	0.0000000	0.0000000				
	3	210.00	-19.639	-.0006949	0.0000000	0.0000000	0.0000000				
	4	210.00	-19.415	-.0006870	0.0000000	0.0000000	0.0000000				
	5	210.00	-19.191	-.0006791	0.0000000	0.0000000	0.0000000				
	6	210.00	-18.967	-.0006712	0.0000000	0.0000000	0.0000000				
	7	210.00	-18.743	-.0006632	0.0000000	0.0000000	0.0000000				
	8	210.00	-18.459	-.0006532	0.0000000	0.0000000	0.0000000				
	9	210.00	-18.175	-.0006431	0.0000000	0.0000000	0.0000000				
FORCES ON MEMBER AT NODE			13	64.0039	.240140	-.843182E-01	-.136517	6.09955	4.17999		
			14	-63.8360	-.241526	.209820	.295601	-3.80773	-4.27007		

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11.6.3.3 Stresses and Strains (Cont'd)

EL= 26 NODES= 5 51 MAT= 1 PRESSURES(Z,Y)= 0.0000 0.0000 AVE. TEMP= 210.00

END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW		
1		210.00	-20.940	-.0007410	0.0000000	0.0000000	0.0000000		
2		210.00	-18.754	-.0006636	0.0000000	0.0000000	0.0000000		
3		210.00	-16.568	-.0005863	0.0000000	0.0000000	0.0000000		
4		210.00	-16.736	-.0005922	0.0000000	0.0000000	0.0000000		
5		210.00	-16.905	-.0005982	0.0000000	0.0000000	0.0000000		
6		210.00	-17.073	-.0006041	0.0000000	0.0000000	0.0000000		
7		210.00	-17.241	-.0006101	0.0000000	0.0000000	0.0000000		
8		210.00	-19.428	-.0006875	0.0000000	0.0000000	0.0000000		
9		210.00	-21.614	-.0007648	0.0000000	0.0000000	0.0000000		
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW		
1		210.00	-18.972	-.0006713	0.0000000	0.0000000	0.0000000		
2		210.00	-18.413	-.0006515	0.0000000	0.0000000	0.0000000		
3		210.00	-17.853	-.0006318	0.0000000	0.0000000	0.0000000		
4		210.00	-17.759	-.0006284	0.0000000	0.0000000	0.0000000		
5		210.00	-17.664	-.0006250	0.0000000	0.0000000	0.0000000		
6		210.00	-17.569	-.0006217	0.0000000	0.0000000	0.0000000		
7		210.00	-17.474	-.0006183	0.0000000	0.0000000	0.0000000		
8		210.00	-18.033	-.0006381	0.0000000	0.0000000	0.0000000		
9		210.00	-18.592	-.0006579	0.0000000	0.0000000	0.0000000		
FORCES ON MEMBER AT NODE			5	60.3167	-.142852	.330573	.426373	-2.86551	-3.28261
			51	-60.1322	.142445	-.192595	-.251474	-1.61412	.839759

EL= 27 NODES= 51 52 MAT= 1 PRESSURES(Z,Y)= 0.0000 0.0000 AVE. TEMP= 210.00

END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW		
1		210.00	-18.969	-.0006712	0.0000000	0.0000000	0.0000000		
2		210.00	-18.413	-.0006516	0.0000000	0.0000000	0.0000000		
3		210.00	-17.858	-.0006319	0.0000000	0.0000000	0.0000000		
4		210.00	-17.761	-.0006285	0.0000000	0.0000000	0.0000000		
5		210.00	-17.665	-.0006251	0.0000000	0.0000000	0.0000000		
6		210.00	-17.569	-.0006217	0.0000000	0.0000000	0.0000000		
7		210.00	-17.473	-.0006183	0.0000000	0.0000000	0.0000000		
8		210.00	-18.029	-.0006380	0.0000000	0.0000000	0.0000000		
9		210.00	-18.584	-.0006576	0.0000000	0.0000000	0.0000000		
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW		
1		210.00	-16.409	-.0005806	0.0000000	0.0000000	0.0000000		
2		210.00	-17.679	-.0006256	0.0000000	0.0000000	0.0000000		
3		210.00	-18.949	-.0006705	0.0000000	0.0000000	0.0000000		
4		210.00	-18.737	-.0006630	0.0000000	0.0000000	0.0000000		
5		210.00	-18.524	-.0006555	0.0000000	0.0000000	0.0000000		
6		210.00	-18.312	-.0006480	0.0000000	0.0000000	0.0000000		
7		210.00	-18.099	-.0006404	0.0000000	0.0000000	0.0000000		
8		210.00	-16.829	-.0005955	0.0000000	0.0000000	0.0000000		
9		210.00	-15.558	-.0005505	0.0000000	0.0000000	0.0000000		
FORCES ON MEMBER AT NODE			51	60.1321	-.160568	.184761	.241351	1.63305	-.834136
			52	-59.9476	.159630	-.468074E-01	-.664829E-01	-3.61585	-1.90756

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11.6.3.3 Stresses and Strains (Cont'd)

EL= 28		NODES= 52 53		MAT= 1		PRESSURES(Z,Y)= 0.0000		0.0000		AVE. TEMP= 210.00	
END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW				
1		210.00	-16.406	-.0005805	0.0000000	0.0000000	0.0000000				
2		210.00	-17.678	-.0006256	0.0000000	0.0000000	0.0000000				
3		210.00	-18.951	-.0006706	0.0000000	0.0000000	0.0000000				
4		210.00	-18.738	-.0006631	0.0000000	0.0000000	0.0000000				
5		210.00	-18.525	-.0006555	0.0000000	0.0000000	0.0000000				
6		210.00	-18.312	-.0006480	0.0000000	0.0000000	0.0000000				
7		210.00	-18.100	-.0006405	0.0000000	0.0000000	0.0000000				
8		210.00	-16.827	-.0005954	0.0000000	0.0000000	0.0000000				
9		210.00	-15.555	-.0005504	0.0000000	0.0000000	0.0000000				
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW				
1		210.00	-14.242	-.0005040	0.0000000	0.0000000	0.0000000				
2		210.00	-16.878	-.0005972	0.0000000	0.0000000	0.0000000				
3		210.00	-19.513	-.0006905	0.0000000	0.0000000	0.0000000				
4		210.00	-19.333	-.0006841	0.0000000	0.0000000	0.0000000				
5		210.00	-19.152	-.0006777	0.0000000	0.0000000	0.0000000				
6		210.00	-18.972	-.0006713	0.0000000	0.0000000	0.0000000				
7		210.00	-18.791	-.0006649	0.0000000	0.0000000	0.0000000				
8		210.00	-16.156	-.0005717	0.0000000	0.0000000	0.0000000				
9		210.00	-13.520	-.0004784	0.0000000	0.0000000	0.0000000				
FORCES ON MEMBER AT NODE		52	59.9477	-.119982	.368976E-01	.510854E-01	3.61809	1.91056			
		53	-59.7632	.119052	.101025	.123744	-3.06900	-3.95729			

EL= 29		NODES= 53		MAT= 1		PRESSURES(Z,Y)= 0.0000		0.0000		AVE. TEMP= 210.00	
END I	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW				
1		210.00	-14.248	-.0005042	0.0000000	0.0000000	0.0000000				
2		210.00	-16.879	-.0005973	0.0000000	0.0000000	0.0000000				
3		210.00	-19.511	-.0006904	0.0000000	0.0000000	0.0000000				
4		210.00	-19.331	-.0006840	0.0000000	0.0000000	0.0000000				
5		210.00	-19.150	-.0006776	0.0000000	0.0000000	0.0000000				
6		210.00	-18.970	-.0006713	0.0000000	0.0000000	0.0000000				
7		210.00	-18.790	-.0006649	0.0000000	0.0000000	0.0000000				
8		210.00	-16.158	-.0005718	0.0000000	0.0000000	0.0000000				
9		210.00	-13.526	-.0004786	0.0000000	0.0000000	0.0000000				
END J	PT	TEMP	SIGX	EP	EPPL	EPCR	EPSW				
1		210.00	-13.200	-.0004671	0.0000000	0.0000000	0.0000000				
2		210.00	-16.253	-.0005751	0.0000000	0.0000000	0.0000000				
3		210.00	-19.306	-.0006832	0.0000000	0.0000000	0.0000000				
4		210.00	-19.306	-.0006832	0.0000000	0.0000000	0.0000000				
5		210.00	-19.306	-.0006832	0.0000000	0.0000000	0.0000000				
6		210.00	-19.306	-.0006832	0.0000000	0.0000000	0.0000000				
7		210.00	-19.306	-.0006832	0.0000000	0.0000000	0.0000000				
8		210.00	-16.253	-.0005751	0.0000000	0.0000000	0.0000000				
9		210.00	-13.200	-.0004671	0.0000000	0.0000000	0.0000000				
FORCES ON MEMBER AT NODE		53	59.7633	-.371698E-01	-.110195	-.139681	3.06781	3.95161			
		6	-59.5788	.367480E-01	.248090	.314475	-.771304E-12	-4.58453			

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11.6.4 Member Qualification

As presented in Section 11.6.3, the maximum lateral displacement for the member occurs at Node 12 and the stress and strain results of the analysis indicate that no yielding occurs in all elements at the final temperature step (temperature=210°F). Therefore, the ductility ratio at nodes 12 is less than 1. Note that the ductility ratio is defined as the ratio of the final displacement to the yield displacement.

Per DG-C1.6.12, Appendix D, the member of worst case 5-7 is qualified.

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11.6.5 Connection and Anchorage Qualification for Worst Case 5-7

11.6.5.1 Connection Qualification for Worst Case 5-7

The weld connections at support nodes 1 through 6 are evaluated. The reaction forces documented in Section 11.6.3.1 are considered in the evaluation. The connection information is obtained from Drawing 48N1210-16 and WCG-1-830 R1 (walkdown package).

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CHECK HD07 - CONNECTIONS

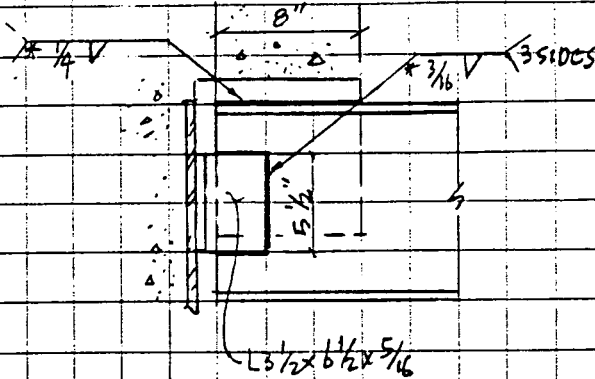
REF.
WCG-1-830

FROM ANSYS OUTPUT HD07R DATED 8/26/91

MAX LOADS AT NODE 1

NODE 1 (FROM ANSYS)

$F_x = 64.51 \text{ K}$
 $F_y = 0.12 \text{ K}$
 $F_z = 0.30 \text{ K}$



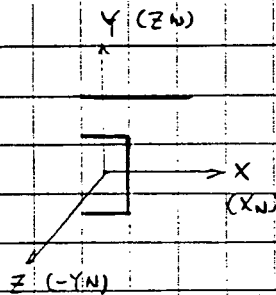
Output
File =
HD07R

FORCES ON WELD GROUP

$F_x = -64.51 \text{ K}$
 $F_y = -0.30 \text{ K}$
 $F_z = 0.12 \text{ K}$

* WELD SIZE PER
DWG 48N1210-16
SECT D16-D16

AXIAL FORCE WILL TRANSFER
THROUGH WELDING BETWEEN EMBEDDED
R AND CHANNEL AND WELD BETWEEN
CLIP ANGLE AND CHANNEL TO BOTH
SIDES OF WALLS. FOR SIMPLICITY
JUST CONSIDERS WELD SIZE $1/4"$ &
LENGTH $B"$ ONLY



$$F_t = (64.51^2 + 0.3^2 + 0.12^2)^{1/2} = 64.51 / 8 = 8.06 \text{ K}/\text{IN}$$

$$\text{WELD CAPACITY: } 0.707 \times \frac{2}{3} \times 70 \times \frac{1}{4} = 8.25 \text{ K}/\text{IN} > 8.06 \text{ K}/\text{IN}$$

O.K.

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<u>HD 07- Connection</u>	REF. DG-CI.6.12 C4.0
check the connection for max ^m loads as per Ansys Output.	Walkdown Package
<u>Nodes 5, 105 (Max My)</u>	TI2007/ESI-5
$F_x = -21.15 \text{ Kilps}$	$M_x = 0.08 \text{ ''-k}$
$F_y = -0.12$	$M_y = 7.55 \text{ ''-k}$
$F_z = 0.80 \text{ K}$	$M_z = -1.56 \text{ ''-k}$
<p style="text-align: center;">↑ Y (Z_N) 12'' ← NODE COOR. 12'' → X (X_N) ↓ Z (Z_N)</p> <p style="text-align: center;">FORCES ON WELD GROUP:</p> <p style="text-align: center;">Size of Weld = 3/16''</p>	
$F_x = 21.15 \text{ K}$	$M_x = -0.08 \text{ ''-k}$
$F_y = -0.80 \text{ K}$	$M_y = 1.56 \text{ ''-k}$
$F_z = -0.12 \text{ K}$	$M_z = 7.55 \text{ ''-k}$
<u>Weld properties :</u>	
$A_w = 24 \text{ in}$	
$S_x = 12 \times 12 = 144 \text{ in}^2$	
$S_y = \frac{12^2}{3} = 48 \text{ in}^2$	
$I_w = \frac{12 \times (3 \times 12^2 + 12^2)}{6} = 1152 \text{ in}^3$	
<u>Weld stresses</u>	
$S_t = \frac{S_z}{A_w} + \frac{M_x}{S_x} + \frac{M_y}{S_y}$	

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$$f_t = \frac{-0.12}{24.0} - \frac{0.08}{144.0} - \frac{1.56}{48.0} = -0.04 \text{ k/in}$$

$$V_x = \frac{F_x}{A_w} + \frac{M_z \times C_h}{J_w}$$

$$= \frac{21.15}{24.0} + \frac{7.55 \times 6}{1152}$$

$$= 0.88 + 0.04 = 0.92 \text{ k/in}$$

$$V_y = \frac{F_y}{A_w} + \frac{M_z \times C_v}{J_w}$$

$$= \frac{0.38}{24} + \frac{7.55 \times 6}{1152}$$

$$= -0.02 + 0.04 = 0.02 \text{ k/in}$$

$$f_r = \left(f_t^2 + V_x^2 + V_y^2 \right)^{1/2}$$

$$= \left(0.04^2 + 0.92^2 + 0.02^2 \right)^{1/2}$$

$$= 0.92 \text{ k/in}$$

Weld Capacity

$$= 0.707 \times \frac{2}{3} \times 70 \times 3/16$$

$$= 6.19 \text{ k/in} > 0.92 \text{ k/in OK}$$

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11.6.5.2 Anchorage Qualification for Worst Case 5-7

For shear loading, the 5/8" ϕ Welded Studs are accepted by comparing the displacement at the anchorage to the displacement allowable per Design Guide DG-C1.6.12 R1. The displacement allowable is 0.2D (0.2 * bolt diameter) for ductile anchors. As shown in Section 11.6.3.2, the maximum X-displacement occurs at node 1 (0.027").

$$0.027" < 0.2D = 0.2 * 5/8" = 0.125" \quad \text{O.K.}$$

A BASEPLATE II model is constructed to evaluate the tension on the Weld Studs. As shown in Figure 11.6.5.2.1, the same model used in Calc. WCG-1-964 (Evaluation of Embedded Plates for Platforms) is used. The reaction forces at node 2, where the maximum moment occurs, are considered. The input and Output of the Baseplate II runs are presented in Table 11.6.5.2.1. The results of analysis indicate that the tension in the bolts are nearly zero since the moment is only 4 inch-kip. The Tension allowable is 7.429 Kip per DS-C1.7.1.

Therefore, the anchorage for worst case 5-7 are qualified.

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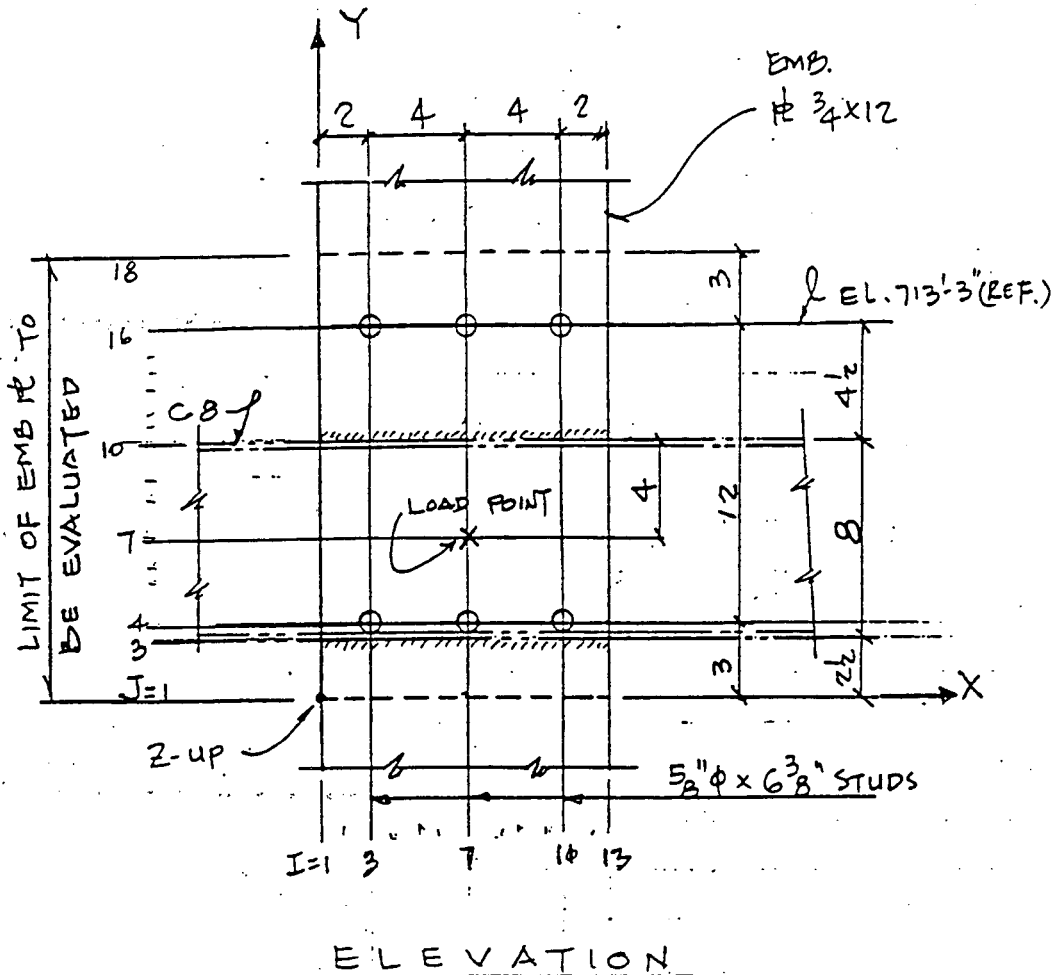
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Figure 11.6.5.2.1 BASEPLATE II Model for Anchorage at Node 2



STIFFNESS OF STUDS: $A = 0.307 \text{ in}^2$ $T_0 = 0.55 \times 44 \times 0.307 = 7.429 \text{ K}$

$$k_t = \frac{1}{\frac{0.015}{7.429} + \frac{6.375}{0.307 \times 29000}} = 365.6 \text{ K/in}$$

$K_s = 1000 \text{ K/in}$

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Table 11.6.5.2.1 BASEPLATE II Input and Output for Anchorage at Nodes 2

(Ref: Input = B2HDØ7 ; Output = B2HDØ7R)

```

1 3 B2HD07 - NODE 2, EL. 713' DWG 48N1210-16
2 OUT ,,,-1,,3/
3 COM,,,,3000/
4 PLA 13,18, ,12.,18.,.75,29/
5 BPR , ,1,365.6E3,1000E3/
6 BOL 3,4,1,2.0,3.0/
7 BOL 3,16,1,2.0,15.0/
8 BOL 7,4,1,6.0,3.0/
9 BOL 7,16,1,6.0,15.0/
10 BOL 11,4,1,10.0,3.0/
11 BOL 11,16,1,10.,15.0/
12 END /
13 ATT 1,3,,0,2.5/
14 JST 1,3,13,12.0,0/
15 IST 7,3,10,8.0,90/
16 JST 7,10,1,6.0,180/
17 JST 7,10,13,6.0,0/
18 END /
19 END /
20 POI 1,3,,6.0,4.0/
21 LOA ,,1,19.2,-0.85,0,0.034,-4.72,-0.44/
22 END /
23 END OF JOB
24 END OF FILE
    
```

BOLT LOADS

I LINE	J LINE	NODE	X-SHEAR	Y-SHEAR	SRSS	Z-TENSION
3	4	40	4.	-1.	4.	0.
3	16	52	2.	-1.	2.	0.
7	4	112	4.	-0.	4.	0.
7	16	124	2.	-0.	2.	0.
11	4	184	4.	0.	4.	0.
11	16	196	2.	1.	2.	0.

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12. SUMMARY OF CALCULATION RESULTS

Worst Case ID	Member Ductility Ratio	Connection	Anchorage	Remarks
1-27	$\mu = 4.3$ (Original Structure) $\mu < 1.0$ (After Modification)	Adequate	Adequate	Modification* Required
2&3-9(C)	$\mu < 1.0$	Adequate	Adequate	
1-11	$\mu < 1.0$	Adequate	Adequate	
2&3-8(G)	$\mu < 1.0$	Adequate	Adequate	
5-7	$\mu < 1.0$	Adequate	Adequate	

* Modification for worst case member 1-27 is shown in Section 11.3.7. A slotted hole connection is implemented per DCN M-16250-A.

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13. CONCLUSION

The thermal worst cases 1-27, 2&3-9(C), 1-11, 2&3-8(G) and 5-7 are evaluated and only one modification for worst case 1-27 is required (for detail, see Section 11.3.7). The evaluation results indicate that, after the modification is implemented, all the members, connections and anchorages for these five case of thermally restrained structure meet the requirement of the design criteria as well as all the applicable codes.

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14. ATTACHMENT

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Attachment "A" Verification Work Sheet	1
Attachment "B" Microfiche Listing for Output Files	1
Attachment "C" The Input and Results for the Analysis to Compare the use of Gap Elements and Imposed Displacement for Worst Case 1-11	7
Attachment "D" The Input and Results for the Analysis to Compare the use of Gap Elements and Imposed Displacement for Worst Case 2&3-8(G).	7
Attachment "E" Walkdown Package for Worst Case 2&3-8(G)	10
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ATTACHMENT A
VERIFICATION WORKSHEET
FOR CALCULATIONS AND DRAWINGSTask No. PWL = CATitle: QUALIF. OF WORST CASES OF THERMALLY RESTRAINED STRUCTURES - PACKAGE 1Document No. WCG-1-969Revision 0

QUESTIONS TO BE ADDRESSED

1	Were the inputs correctly selected at the correct revision level, and incorporated into the design?
2	Are the assumptions necessary to perform the design activity adequately described and reasonable? Where necessary, are assumptions identified for subsequent reverification after the detailed design activities are completed?
3a	Are the appropriate quality requirements specified?
3b	Are the appropriate quality assurance requirements specified?
4	Are the applicable codes, standards, and regulatory requirements including issue and addenda properly identified, and are their requirements for design met?
5	Have applicable construction and operating experience been considered?
6	Have the design interface requirements been satisfied?
7	Was an appropriate design method used?
8	Is the output reasonable compared to inputs?
9	Are the specified parts, equipment, and processes suitable for the required application? Are all applicable construction specifications referenced on the drawing(s)?
10	Are the specified materials compatible with each other and with the design environmental conditions to which the materials will be exposed?
11	Have adequate maintenance features and requirements been specified?
12	Are accessibility and other design provisions adequate for performing needed maintenance and repair?
13	Has adequate accessibility been provided to perform the inservice inspection expected to be required during the plantlife?
14	Has the design properly considered radiation exposure to the public and plant personnel?
15	Is the acceptance criteria incorporated in the design document sufficient to allow verification that design requirements have been satisfactorily accomplished?
16	Have adequate preoperational and subsequent periodic test requirements been appropriately specified?
17	Are adequate handling, storage, cleaning and shipping requirements specified?
18	Are adequate identification requirements specified?
19	Are requirements for record preparation, review, approval, retention, etc., adequately specified?

For Calculations, only questions 1, 2, 4, 6, 7, 8 and 15 are considered applicable.

Independent Verifier

C.Y. OU / Chaojung Ou
Print/Sign8/30/91

Date

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ATTACHMENT B:

MICROFICHE LISTING FOR OUTPUT FILES

Worst Case ID	Output File Name	Microfiche Number
1-27 and 2&3-9(C)	BS4P9CR	TVA-F-G099738
	BS8P9CR	
	BS10P9R	
	BS7P9CR	
	BS12P9R	
1-27 and 2&3-9(C)	B8PR9CR	TVA-F-G099736
	B10P9CR	
	C10P9CR	
1-27 and 2&3-9(C)	B1PR9CR	TVA-F-G099734
	C4PR9CR	
1-27 and 2&3-9(C)	PR9CR	TVA-F-G099732
	PR9CMR	TVA-F-G099730
1-11	AX11R	TVA-F-G099728
2&3-8(G)	BS1P8GR	TVA-F-G099726
	BS9P8GR	
2&3-8(G)	B1PR8GR	TVA-F-G099724
	PR8GR	
5-7	HDO7R	TVA-F-G099720
	B2HDO7R	TVA-F-G099722

***** ANSYS INPUT DATA LISTING (FILE18) *****

```

1 /PREP7
2 /TITLE, AX11G - EVAL. FOR AXIAL CASE 1-11, DWG. 48N1210-16, EL. 713'-0"
3 /COM
4 /COM BRANCH/PROJECT ID = WCG-1-969
5 /COM
6 /COM PREPARED BY SAL 8/13/91
7 /COM CHECKED BY J. OU 8/13/91
8 /COM
9 /COM FOR COMPARISON BETWEEN USING GAP ELEM. AND IMPOSED DISP.
10 /COM (USE STIF40 / GAP AT SUPPORTS)
11 /COM
12 /COM INPUT = AX11G
13 /COM OUTPUT = AX11GR
14 /COM FILE12 = AX11GF
15 /COM PLOT = AX11GP
16 /COM
17 C*** ANSYS ANALYSIS OPTIONS AND REFERENCES
18 KAN,0
19 KAY,6,1
20 KNL,1
21 TREF,70
22 C*** ELEMENT TYPE AND MATERIAL PROPERTIES
23 ET,1,4,,,,,1
24 ET,2,24,,,,,1
25 ET,3,21 * 3D MASS ELEMENT
26 ET,4,40,,,1
27 C***
28 EX,1,29000
29 RP4,1
30 ALPX,1,6.5E-6
31 RP4,1
32 DENS,1,0.283E-3
33 DENS,2,2.905E-3 * INCLUDE DL+SLL
34 DENS,3,2.002E-3 * INCLUDE DL+SLL
35 DENS,4,0 * RIGID LINK
36 C***
37 MPTEMP,1,70,100,200,300,400
38 RP4,1
39 MPDATA,EX,1,1,29.0E3,28.9E3,28.3E3,27.9E3,27.4E3
40 RP4,,1
41 MPDATA,ALPX,1,1,6.50E-6,6.50E-6,6.50E-6,6.67E-6,6.86E-6 * ALPX=6.29 MU FOR 100F
42 RP4,,1
43 C*** NONLINEAR MATERIAL CONSTANT
44 *CREAT,MNL
45 NL,ARG1,13,10
46 NL,ARG1,19,70,100,200,300,400
47 NL,ARG1,25,36,36.0,36.0,36.0,36.0
48 NL,ARG1,31,150,150,150,150,150
49 *END
50 *USE,MNL,2
51 *USE,MNL,3
52 C*** MEMBER CROSS SECTIONAL PROPERTIES
53 R,1,3.38,1.32,32.6,8.00,2.26, *CBX11.5
54 RMORE,,0.131
55 C***
56 C*** NONLINEAR CROSS SECTIONAL PROPERTIES
57 C*** CHANNEL CBX11.5 *****
58 CF=2.260
59 TCF=0.390
60 CD=8.00

```

ATTACHMENT	C
SHEET	1 OF 7

***** ANSYS INPUT DATA LISTING (FILE18) *****

```

61 TCW=0.220 -
62 XBR=(0.571)+0.07695
63 C***
64 XO=(TCW/2)-(XBR)
65 CC=CF-(TCW/2)
66 DD=(CD-TCF)/2.
67 QC=CC/2

```

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```

68 QD=DD/2
69 CC=CC+X0
70 QC=QC+X0
71 *STAT
72 C***
73 R,2,CC,DD,0,QC,DD,TCF          * STIF24 REAL PROP. - CBX11.5
74 RMORE,X0,DD,TCF,X0,QD,TCW
75 RMORE,X0,0,TCW,X0,-QD,TCW
76 RMORE,X0,-DD,TCW,QC,-DD,TCF
77 RMORE,CC,-DD,TCF
78 C***
79 R3=(.0207*1.2)/1.5
80 R4=(.0198*1.2)/1.5
81 R,3,R3,R3,R3                  * CONDUIT
82 R,4,R4,R4,R4                  * CONDUIT
83 R,5,100,1000,1000            * RIGID LINK
84 C***
85 K1=10000000 * SIMULATE A FIXED SUPPORT
86 GAP=(1/16)
87 FSL=0
88 R,7,K1,,,GAP,FSL            * COMB. ELEMENT
89 C***
90 C*** NODES AND COORDINATES
91 X1=22 SX2=59 SY1=23 SY2=49
92 N,1,0,0
93 N,101,-1,0
94 N,2,X1,0
95 N,20,X1,0,5
96 N,102,X1,0
97 N,3,X2,0
98 N,103,X2,0
99 N,303,X2,0,-5
100 N,4,93,0
101 N,104,94,0
102 N,5,X1,Y1
103 N,105,X1,Y1
104 N,6,X2,Y1
105 N,106,X2,Y1
106 N,7,0,Y2
107 N,107,-1,Y2
108 N,8,X1,Y2
109 N,80,X1,Y2,5
110 N,108,X1,Y2
111 N,9,X2,Y2
112 N,109,X2,Y2
113 N,309,X2,Y2,-5
114 N,10,93,Y2
115 N,110,94,Y2
116 FILL,1,2,2,11,1
117 FILL,2,3,3,21,1
118 FILL,3,4,3,31,1
119 FILL,7,8,2,71,1
120 FILL,8,9,3,81,1

```

***** ANSYS INPUT DATA LISTING (FILE18) *****

```

121 FILL,9,10,3,91,1
122 C***
123 C*** ELEMENT CONNECTIVITIES
124 C***
125 MAT,2 $TYPE,2 $REAL,2
126 EN,1,1,11,20 $EN,11,11,12,20 $EN,12,12,2,20
127 EN,2,2,21,20 $EN,21,21,22,20 $RP2,1,1,1 $EN,23,23,3,20
128 EN,3,3,31,20 $EN,31,31,32,20 $RP2,1,1,1 $EN,33,33,4,20
129 MAT,3
130 EN,7,7,71,80 $EN,71,71,72,80 $EN,72,72,8,80
131 EN,8,8,81,80 $EN,81,81,82,80 $RP2,1,1,1 $EN,83,83,9,80
132 EN,9,9,91,80 $EN,91,91,92,80 $RP2,1,1,1 $EN,93,93,10,80
133 C***
134 MAT,4 $TYPE,1 $REAL,5 * RIGID LINK
135 EN,303,3,303
136 EN,309,9,309
137 C***
138 MAT,4 $TYPE,3

```

ATTACHMENT	C
SHEET	3 OF 7

```

139 REAL,3 SEN,503,303
140 REAL,4 SEN,509,309
141 C***
142 MAT,1 STYPE,1 SREAL,1
143 E,102,5
144 E,5,108
145 E,103,6
146 E,6,109
147 E,105,106
148 C*** COMBINATION ELEMENTS
149 TYPE,4 SREAL,7
150 EN,104,4,104
151 EN,110,10,110
152 C***
153 C*** COUPLED DOFS FOR MEMBER JOINT RELEASE
154 CP,1,UX,102,2
155 CPLGEN,1,UY,UZ * SETS 2-3 NODE2
156 CPSGEN,2,1,1,3 * SETS 4-6 NODE3
157 CPSGEN,2,3,1,3 * SETS 7-9
158 CPLGEN,9,ROTX * SET 10 NODE5
159 CPSGEN,2,4,1,3 * SETS 11-13 NODE6
160 CPSGEN,2,6,1,3 * SETS 14-16
161 CPLGEN,16,ROTY * SET 17 NODE8
162 CPSGEN,2,1,14,17 * SETS 18-21 NODE9
163 C*** BOUNDARY CONDITIONS
164 *CREAT,BND
165 DDELE,ALL,ALL
166 D,1,UY,,,,UZ
167 D,4,UY,,,,UZ,ROTX
168 D,7,UY,,,,UZ
169 D,10,UY,,,,UZ,ROTX
170 D,104,ALL
171 D,110,ALL
172 C***
173 DDX=ARG1
174 D,1,UX,-DDX
175 C*** D,4,UX,DDX
176 D,7,UX,-DDX
177 C*** D,10,UX,DDX
178 *END
179 *USE,BND,0
180 C*** ANALYSIS OPTIONS

```

***** ANSYS INPUT DATA LISTING (FILE18) *****

```

181 ITER,-20,20
182 TREF,70
183 POSTR,,2,5
184 C*** PLOTS - GEOMETRY W/B.C.
185 /SHOW
186 /VIEW,,0,0,1
187 /PNUM,NODE,1
188 EPLO
189 /SHRINK,0.3
190 /PNUM,NODE,-1
191 /PNUM,ELEM,1
192 EPLO
193 /PNUM,ELEM
194 /PBC,ALL,1
195 EPLO
196 /VIEW,,1,1,1
197 EPLO
198 C*** LOADING CASES
199 C***
200 C*** PIPE FPL
201 F,3,FX,-.U03,,23,20
202 F,3,FZ,-.307,,23,20
203 F,3,MX,5.749,,23,20
204 F,3,MZ,.06,,23,20
205 C***
206 *CREAT,MAC
207 TUNIF,ARG1
208 LWRITE
209 *END

```

```

210 C***
211 C*** FOR AX11 AT ELE, 713'-0",
212 C*** PEAK ACCEL.(G): NS=1.4, EW=1.65, VT=0.38
213 C*** FACTOR: MMF=1.5
214 MMF=1.5 $GX=MMF*1.65 $GY=MMF*1.65 $GZ=MMF*0.38
215 C***
216 C*** DL+SSE XZ-PLANE
217 ACEL, -GX, 0, GZ+1
218 *USE, MAC, 70
219 C***
220 C*** DL+SSE YZ-PLANE
221 ACEL, , -GY, GZ+1
222 *USE, MAC, 70
223 RP15, , 10
224 C***
225 C*** *USE, BND, (1/32)
226 C*** *USE, MAC, 150
227 C*** RP7, , 10
228 AFWRITE, , 1
229 FINISH
230 /INPUT, 27
231 FINISH
232 /POST1
233 /SHOW
234 NLINE, 200
235 *CREAT, PRF
236 SET, ARG1
237 PRRFOR
238 PRDISP
239 PLDISP
240 PLDISP, 1

```

ATTACHMENT	C
SHEET	4 OF 7

***** ANSYS INPUT DATA LISTING (FILE18) *****

```

241 *END
242 *USE, PRF, 1
243 *USE, PRF, 16
244 /VIEW, , 1, 1, 1
245 PLDISP
246 PLDISP, 1
247 FINISH
248 C***
249 /POST26
250 /SHOW
251 NUMVAR, 50
252 TVAR, 1
253 C***
254 ESTR, 2, 1, 53, TEMP          * TEMPERATURE
255 *CREATE, STRN
256 C*** STORE STRAINS FOR CHANNEL
257 ESTR, 3, ARG1, 215, EE01
258 ESTR, 4, ARG1, 216, EP01
259 ESTR, 5, ARG1, 227, EE03
260 ESTR, 6, ARG1, 228, EP03
261 ESTR, 7, ARG1, 251, EE07
262 ESTR, 8, ARG1, 252, EP07
263 ESTR, 9, ARG1, 263, EE09
264 ESTR, 10, ARG1, 264, EP09
265 PRVAR, 2, 3, 4, 5, 6
266 PRVAR, 2, 7, 8, 9, 10
267 *END
268 C***
269 *CREAT, DSP
270 DISP, 21, ARG1, UX, UX
271 DISP, 22, ARG1, UY, UY
272 DISP, 23, ARG1, UZ, UZ
273 PROD, 24, 21, 21, , UXUX
274 PROD, 25, 22, 22, , UYUY
275 PROD, 26, 23, 23, , UZUZ
276 ADD, 27, 24, 25, 26, SS
277 SQRT, 28, 27, , , SRSS
278 PRVAR, 2, 21, 22, 23, 28
279 *END
280 *USE, STRN, 2

```

281 *USE,DSP,21
 282 *USE,STRN,21
 283 *USE,DSP,22
 284 *USE,STRN,22
 285 *USE,DSP,23
 286 *USE,STRN,23
 287 *USE,DSP,3
 288 *USE,STRN,8
 289 *USE,DSP,81
 290 *USE,STRN,81
 291 *USE,DSP,82
 292 *USE,STRN,82
 293 *USE,DSP,83
 294 *USE,STRN,83
 295 *USE,DSP,9
 296 /COM ST1F40
 297 *CREAT,ST40
 298 ESTR,41,ARG1,3,STR1
 299 ESTR,42,ARG1,4,STR2
 300 ESTR,43,ARG1,5,UI

BRANCH PROJECT I.D.

WCG-1-969

ATTACHMENT	C
SHEET	5 OF 7

***** ANSYS INPUT DATA LISTING (FILE18) *****

301 ESTR,44,ARG1,6,UJ
 302 ESTR,45,ARG1,7,SLIDE
 303 ESTR,46,ARG1,8,STAT
 304 ESTR,47,ARG1,9,OSTA
 305 ESTR,48,ARG1,10,KCON
 306 PRVAR,2,41,42,43,44
 307 PRVAT,2,45,46,47,48
 308 *END
 309 *USE,ST40,104
 310 *USE,ST40,110
 311 FINISH

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 FOR SUPPORT CALL CDC PHONE TWX

TITLE

12.6392 AUG 27,1991 CP= .485

***** ANSYS ANALYSIS DEFINITION (PREP7) *****

NEW TITLE= AX11G - EVAL. FOR AXIAL CASE 1-11, DWG. 48N1210-16, EL. 713'-0"

BRANCH/PROJECT ID = WCG-1-969

PREPARED BY SAL 8/13/91
 CHECKED BY J. OU 8/13/91

FOR COMPARISON BETWEEN USING GAP ELEM. AND IMPOSED DISP.
 (USE STIF40 / GAP AT SUPPORTS)

INPUT = AX11G
 OUTPUT = AX11GR
 FILE12 = AX11GF
 PLOT = AX11GP

C*** ANSYS ANALYSIS OPTIONS AND REFERENCES

ANALYSIS TYPE= 0 (STATIC ANALYSIS)

LARGE DEFLECTION SOLUTION (KAY(6)=1)

NON-LINEAR ANALYSIS - SUPPLY NON-LINEAR PROPERTIES

REFERENCE TEMPERATURE= 70.000 (TUNIF= 70.000)

C*** ELEMENT TYPE AND MATERIAL PROPERTIES

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AX11G - EVAL. FOR AXIAL CASE 1-11, DWG. 48N1210-16, EL. 713'-0"

12.6508 AUG 27, 1991 CP= 36.067

***** POST1 REACTION FORCE LISTING *****

LOAD STEP 16 ITERATION= 20 SECTION= 1
TIME= 0.0000 LOAD CASE= 1

ATTACHMENT	C
SHEET	6 OF 7

THE FOLLOWING X,Y,Z FORCES ARE IN NODAL COORDINATES

NODE	FX	FY	FZ	MX	MY	MZ
1	15.055076	-1.2048088	1.0694231			
4		-1.2946975	1.0960269	-10.275500		
7	16.972626	-.90209648	.55390644			
10		-.78326678	.54928806	.18449400		
104	-15.049075	0.0000000	0.0000000	0.0000000	0.0000000	0.0000000
110	-16.972627	0.0000000	0.0000000	0.0000000	0.0000000	0.0000000
TOTAL	.60000189E-02	-4.1848695	3.2686445	-10.091006	0.0000000	0.0000000

PRINT NODAL DISPLACEMENTS

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AX11G - EVAL. FOR AXIAL CASE 1-11, DWG. 48N1210-16, EL. 713'-0"

12.6508 AUG 27, 1991 CP= 36.078

***** POST1 NODAL DISPLACEMENT LISTING *****

LOAD STEP 16 ITERATION= 20 SECTION= 1
TIME= 0.0000 LOAD CASE= 1

THE FOLLOWING X,Y,Z DISPLACEMENTS ARE IN NODAL COORDINATES

-NODE	UX	UY	UZ	ROTX	ROTY	ROTZ
1	0.0000000	0.0000000	0.0000000	.33684710	-.91101187E-02	.19805316E-01
2	.13284711E-01	.36941251	.22211774	.31028197	-.81796650E-02	.14353111E-01
3	.41295400E-01	.49915962	.39289535	.26562120	-.26967710E-02	-.79102870E-02
4	.62501505E-01	0.0000000	0.0000000	0.0000000	.30774802E-02	-.20930898E-01
5	.95104981E-02	.39115152	.98131041E-01	-.53781952E-02	.81412958E-03	.20582988E-04
6	.44698745E-01	.52035678	.18391982	-.90698438E-02	-.25478280E-03	-.56095217E-05
7	0.0000000	0.0000000	0.0000000	-.97179568E-02	.12664394E-02	.21009426E-01
8	.13070695E-01	.41569979	-.41610257E-01	-.20797859E-01	.81655014E-03	.14882771E-01
9	.40724503E-01	.54429379	-.51893616E-01	-.19389094E-01	-.25627922E-03	-.84131043E-02
10	.62501697E-01	0.0000000	0.0000000	0.0000000	-.10297491E-02	-.20190966E-01
11	.41960892E-02	.13540926	.76576526E-01	.32641901	-.91252533E-02	.19065666E-01
12	.85812173E-02	.26099522	.15165265	.31751885	-.87964233E-02	.17192083E-01
21	.19749639E-01	.47199529	.30120954	.30421399	-.70673511E-02	.96666456E-02
22	.26726643E-01	.52777533	.36576637	.30152883	-.57037697E-02	.41167441E-02
23	.33970430E-01	.53101189	.41397775	.30256251	-.42422974E-02	-.18105459E-02
31	.47542223E-01	.43453958	.30992312	.19670367	-.11012726E-02	-.13075150E-01
32	.53159016E-01	.32253036	.21449552	.12998108	.39187086E-03	-.17376679E-01
33	.58087126E-01	.17273163	.11030570	.64649701E-01	.17632624E-02	-.20197217E-01
71	.40938538E-02	.15223313	-.15762598E-01	-.14147984E-01	.11496242E-02	.20276553E-01
72	.84018558E-02	.29392251	-.29802543E-01	-.17862873E-01	.99863940E-03	.18171346E-01
81	.19542765E-01	.52948121	-.51925428E-01	-.22774845E-01	.56387845E-03	.95796673E-02
82	.26515243E-01	.59073063	-.57243000E-01	-.23244024E-01	.29328285E-03	.35960744E-02
83	.33690575E-01	.59545478	-.57219278E-01	-.22120017E-01	.16376038E-04	-.25698196E-02
91	.46815721E-01	.45227589	-.43633459E-01	-.16223693E-01	-.48741525E-03	-.13122252E-01
92	.52434774E-01	.32406009	-.31806213E-01	-.11862234E-01	-.69622625E-03	-.16869776E-01
93	.57607050E-01	.16922275	-.17057607E-01	-.64232105E-02	-.87771515E-03	-.19323451E-01
102	.13284711E-01	.36941251	.22211774	-.53898013E-02	.81526660E-03	.23232819E-03
103	.41295400E-01	.49915962	.39289535	-.90814528E-02	-.25670009E-03	-.21721329E-03
104	0.0000000	0.0000000	0.0000000	0.0000000	0.0000000	0.0000000
105	.95104981E-02	.39115152	.98131041E-01	-.53781952E-02	-.23215148E-02	.36210506E-02
106	.44698745E-01	.52035678	.18391982	-.53775235E-02	-.23300390E-02	.33438588E-02
108	.13070695E-01	.41569979	-.41610257E-01	-.53645441E-02	.81655014E-03	-.21881236E-03
109	.40724503E-01	.54429379	-.51893616E-01	-.90561924E-02	-.25627922E-03	.23364654E-03
110	0.0000000	0.0000000	0.0000000	0.0000000	0.0000000	0.0000000
303	.59871108E-01	1.8128959	.56363078	.26562121	-.26967709E-02	-.79102869E-02

309 .41599177E-01 .44725731 -.55754795E-01 -.19389076E-01 -.25627922E-03 -.84131043E-02

MAXIMUMS
 NODE 10 303 303 1 11 7
 VALUE .62501697E-01 1.8128959 .56363078 .33684710 -.91252533E-02 .21009426E-01

ATTACHMENT	C
SHEET	7 OF 7

PRODUCE DISPLACEMENT PLOT, KUND= 0

CUMULATIVE PLOT NUMBER 7 WRITTEN TO FILE FILE33_DAT - RASTER MODE.
 PLOT TITLE= AX11G - EVAL. FOR AXIAL CASE 1-11, DWG. 48N1210-16, EL. 713'-0"

PRODUCE DISPLACEMENT PLOT, KUND= 1

CUMULATIVE PLOT NUMBER 8 WRITTEN TO FILE FILE33_DAT - RASTER MODE.
 PLOT TITLE= AX11G - EVAL. FOR AXIAL CASE 1-11, DWG. 48N1210-16, EL. 713'-0"

VIEW POINT FOR WINDOW 1 1.0000 1.0000 1.0000

PRODUCE DISPLACEMENT PLOT, KUND= 0

CUMULATIVE PLOT NUMBER 9 WRITTEN TO FILE FILE33_DAT - RASTER MODE.
 PLOT TITLE= AX11G - EVAL. FOR AXIAL CASE 1-11, DWG. 48N1210-16, EL. 713'-0"

PRODUCE DISPLACEMENT PLOT, KUND= 1

CUMULATIVE PLOT NUMBER 10 WRITTEN TO FILE FILE33_DAT - RASTER MODE.
 PLOT TITLE= AX11G - EVAL. FOR AXIAL CASE 1-11, DWG. 48N1210-16, EL. 713'-0"

***** ROUTINE COMPLETED ***** CP = 36.292

C***

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AX11G - EVAL. FOR AXIAL CASE 1-11, DWG. 48N1210-16, EL. 713'-0" 12.6511 AUG 27,1991 CP= 36.294

***** GENERAL GRAPH POSTPROCESSOR (POST26) *****

ALL POST26 SPECIFICATIONS ARE RESET TO INITIAL DEFAULTS

/SHOW SWITCH PLOTS TO FILE FILE33_DAT - RASTER MODE.

MAXIMUM NUMBER OF VARIABLES= 50

USE NCUMIT AS TIME VARIABLE

C***

VARIABLE 2 IS ELEMENT 1 ITEM 53 NAME= 1 TEMP

START COPY FROM INPUT TO STRN EXT=

C*** STORE STRAINS FOR CHANNEL

END FILE CREATE

C***

START COPY FROM INPUT TO DSP EXT=

END FILE CREATE

VARIABLE 3 IS ELEMENT 2 ITEM 215 NAME= ARG1 EE01

VARIABLE 4 IS ELEMENT 2 ITEM 216 NAME= ARG1 EP01

WCG-1-969

***** ANSYS INPUT DATA LISTING (FILE18) *****

```

1 /PREP7
2 /TITLE, PR8G1 - EVAL. FOR PROXIMITY CASE 8(G), DWG 48N967-1, EL. 774'-0", LOOP 4
3 /COM
4 /COM BRANCH/PROJECT ID : WCG-1-969
5 /COM
6 /COM PREPARED BY : SAL      8/15/91
7 /COM CHECKED BY J. OU      8/15/91
8 /COM
9 /COM FOR COMPARISON BETW. USING GAP ELEMENT AND IMPOSED DISP.
10 /COM USE GAPS ON BOTH NODE 1 AND 9
11 /COM ONE LOAD STEP BEFORE GAPS CLOSE
12 /COM
13 /COM INPUT: PR8G1
14 /COM OUTPUT: PR8G1R
15 /COM FILE12: PR8G1F
16 /COM PLOT: PR8G1P
17 /COM
18 C*** ANSYS ANALYSIS OPTIONS AND REFERENCES
19 KAN,0
20 KAY,6,1
21 KNL,1
22 TREF,70
23 C*** ELEMENT TYPE AND MATERIAL PROPERTIES
24 ET,1,24,,,,,1
25 ET,2,4,,,,,1
26 ET,3,14,,1
27 ET,4,40,,,2 * DOF=Y
28 C*** MATERIAL PROPERTIES
29 EX,1,29000
30 RP3,1
31 ALPX,1,6.5E-6
32 RP3,1
33 C***
34 DENS,1,0.92E-3
35 DENS,2,0.70E-3
36 DENS,3,(0.283E-3+2.95E-3)
37 DENS,4,0.283E-3
38 C***
39 MPTEMP,1,70,100,200,300,400
40 RP4,1
41 MPDATA,EX,1,1,29.0E3,28.9E3,28.3E3,27.9E3,27.4E3
42 RP4,,1
43 MPDATA,ALPX,1,1,6.50E-6,6.50E-6,6.50E-6,6.67E-6,6.86E-6 * ALPX=6.29 MU FOR 100F
44 RP4,,1
45 C*** NONLINEAR MATERIAL CONSTANT
46 *CREAT,MNL
47 NL,ARG1,13,10
48 NL,ARG1,19,70,100,200,300,400
49 NL,ARG1,25,36,36.0,36.0,36.0,36.0
50 NL,ARG1,31,150,150,150,150,150
51 *END
52 *USE,MNL,1
53 *USE,MNL,2
54 *USE,MNL,3
55 C*** MEMBER CROSS SECTIONAL PROPERTIES
56 R,1,3.54,8.0,8.0,4.0,4.0 * TS4X4X.25
57 R,2,1.44,1.24,1.24,3,3 * L3X3X.25
58 RMORE,,0.03
59 C***
60 K1=(400*4) * SAL 7/31/91

```

ATTACHMENT	<u>D</u>
SHEET	<u>1</u> OF <u>7</u>

***** ANSYS INPUT DATA LISTING (FILE18) *****

```

61 C*** GAP=(1/32)
62 GAP=(1/32) * SAL 8/14/91
63 FSL=(20.8*4)
64 R,8,K1,,,GAP,FSL * NODE 1
65 C*** GAP=0
66 R,5,K1,,,GAP,FSL * NODE 9
67 C***

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BRANCH PROJECT I.D.

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ATTACHMENT	<u>D</u>
SHEET	<u>2</u> OF <u>7</u>

```

68 BF=4          * WIDTH
69 D=4          * DEPTH
70 TW=0.25      * WALL THICKNESS
71 C***
72 AA=(BF-TW)/2
73 BB=(D-TW)/2
74 QA=AA/2
75 QB=BB/2
76 *STAT
77 R,3,AA,BB,0,QA,BB,TW      *STIF24 REAL PROP. - TS
78 RMORE,0,BB,TW,-QA,BB,TW
79 RMORE,-AA,BB,TW,-AA,QB,TW
80 RMORE,-AA,0,TW,-AA,-QB,TW
81 RMORE,-AA,-BB,TW,-QA,-BB,TW
82 RMORE,0,-BB,TW,QA,-BB,TW
83 RMORE,AA,-BB,TW,AA,-QB,TW
84 RMORE,AA,0,TW,AA,QB,TW
85 RMORE,AA,BB,TW
86 C***
87 *CREATE,ANGL
88 D=ARG2          * D=DEPTH
89 AF=ARG3          * AF=FLANGE      * ANGLE L D X AF X TAW
90 TAW=ARG4          * TAW=THICKNESS
91 XBR=ARG5          * XBAR=CENTROID X FROM AISC
92 YBR=ARG6          * YBAR=CENTROID Y
93 C***
94 XX=AF-(TAW/2)    *
95 YY=D-(TAW/2)    *
96 XO=XBR-(TAW/2)  *
97 YO=YBR-(TAW/2)  *
98 XX=XX-XO        *
99 YY=YY-YO        *
100 MX=XX/2        *
101 MY=YY/2        *
102 *IF,ARG7,LE,1,:RAN
103 X=-XX $XO=-XO $MX=-MX * CHN SIGN FOR FLANGE ON LEFT OF WEB
104 :RAN
105 R,ARG1,XX,YO,0,MX,YO,TAW
106 RMORE,0,YO,TAW,-XO,YO,TAW
107 RMORE,-XO,0,TAW,-XO,-MY,TAW
108 RMORE,-XO,-YY,TAW
109 *END
110 *USE,ANGL,4,3,3,0.25,0.842,0.842,1 * L3X3X1/4 LEFT FLANGE
111 C*** NODAL COORDINATES
112 N,1,0,62
113 N,101,0,64 $NDD=300
114 N,2,6.5,61 $FILL,1,2,1,NDD+11,1
115 N,102,6.5,61
116 N,202,6.5,61
117 N,3,43,56 $FILL,2,3,8,NDD+21,1
118 N,103,43,56
119 N,4,50,55 $FILL,3,4,1,NDD+31,1
120 N,5,22,38

```

***** ANSYS INPUT DATA LISTING (FILE18) *****

```

121 N,6,6.5,37
122 N,106,6.5,37
123 N,7,6.5,18
124 N,8,25,12
125 N,108,25,12
126 N,208,25,12
127 N,9,0,0 $FILL,2,6,5,NDD+65,1
128 N,109,0,-2 $FILL,6,7,5,NDD+75,1
129 N,10,6.5 $FILL,9,10,1,NDD+91,1 $FILL,7,10,5,NDD+105,1
130 N,11,26.5 - $FILL,10,11,4,NDD+101,1
131 N,12,31 $FILL,11,12,1,NDD+111,1
132 N,99,6.5,61,100
133 N,100,6.5,0,100
134 C*** ELEMENT CONNECTIVITIES
135 MAT,1 $TYPE,1 $REAL,3 * TS4X4X.25
136 EN,1,1,311,99 $,311,311,2,99
137 EN,2,2,321,99 $,321,321,322,99 $SRP7,1,1,1 $EN,328,328,3,99
138 EN,3,3,331,99 $,331,331,4,99

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BRANCH PROJECT I.D.

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```

139 MAT,2
140 EN,12,9,391,100 $,391,391,10,100
141 EN,13,10,401,100 $,401,401,402,100 SRP3,1,1,1 SEN,404,404,11,100
142 EN,14,11,411,100 $,411,411,12,100
143 C***
144 MAT,4 $TYPE,2 $REAL,2 * L3X3X.25
145 EN,4,2,5
146 EN,5,3,5
147 EN,6,6,5
148 C***
149 MAT,3 $TYPE,1 $REAL,4 * L3X3X.25
150 EN,7,2,365,100 $,365,365,366,100 SRP4,1,1,1 SEN,369,369,6,100
151 EN,8,6,375,100 $,375,375,376,100 SRP4,1,1,1 SEN,379,379,7,100
152 EN,9,7,405,100 $,405,405,406,100 SRP4,1,1,1 SEN,409,409,10,100
153 C***
154 MAT,4 $TYPE,2 $REAL,2 * L3X3X.25
155 EN,10,7,8
156 EN,11,11,8
157 C***
158 C*** SPRING (STIF40)
159 TYPE,4 $REAL,8 * SPRING Y
160 EN,15,1,101
161 REAL,5
162 EN,16,109,9
163 C***
164 C*** BOUNDARY CONDITIONS
165 C***
166 D,1,UX,,,9,8,UZ,ROTX * SAL 7/12/91
167 D,4,UY,,,12,8 * SAL 7/15/91
168 D,101,ALL
169 D,109,ALL
170 C***
171 C*** SPECIFY ROTATIONAL SPRINGS
172 PSPRING,1,ROT,12282,,2 * ROTY
173 PSPRING,1,ROT,23598,,,2 * ROTZ
174 PSPRING,9,ROT,7475,,2 * ROTY
175 PSPRING,9,ROT,19965,,,2 * ROTZ
176 C***
177 C*** ANALYSIS OPTIONS
178 ITER,-20,20
179 TREF,70
180 POSTR,,1,5

```

ATTACHMENT	<u>D</u>
SHEET	<u>3</u> OF <u>7</u>

***** ANSYS INPUT DATA LISTING (FILE18) *****

```

181 C*** PLOT GEOMETRY AND BOUNDARY CONDITIONS
182 /SHOW
183 /PNUM,NODE,1
184 EPLO
185 /SHRINK,0.3
186 /PNUM,NODE,-1
187 /PNUM,ELEM,1
188 EPLO
189 /PBC,ALL,1
190 /PNUM,ELEM,-1
191 EPLO
192 C*** LOADING CASES
193 C*** TEMPERATURE LOADS *** *
194 *CREAT,MAC
195 TUNIF,ARG1
196 LWRITE
197 *END
198 C***
199 C*** PEAK ACCEL.(G): NS=3.5 EW=3.25 VT=1.4
200 C*** FACTOR: MMF=1.5
201 MMF=1.5 $GX=MMF*3.5 $GY=MMF*3.5 $GZ=MMF*1.4
202 C***
203 C*** DL+SSE XZ-PLANE
204 C*** ACEL,-GX,0,GZ+1
205 C*** *USE,MAC,70
206 C***
207 C*** DL+SSE YZ-PLANE
208 ACEL,-GY,GZ+1 * SAL 7/15/91
209 C*** *USE,MAC,70

```

```

210 C*** D,101,UY,(1/32) * SAL 7/24/91
211 C*** D,109,UY,(-1/32)
212 C***
213 *USE,MAC,210
214 RP12,,10
215 *USE,MAC,327
216 C***
217 AFWRITE,,1
218 FINISH
219 /INPUT,27
220 FINISH
221 /POST1
222 /SHOW
223 NLINE,200
224 *CREAT,PLD
225 SET,ARG1
226 PRRFOR
227 PRDISP
228 PLDISP
229 PLDISP,1
230 *END
231 *USE,PLD,1
232 *USE,PLD,13
233 FINISH
234 C***
235 /POST26
236 LINE,200
237 NUMVAR,50
238 TVAP,1
239 ESTR,2,1,53,TEMP
240 *CREATE,STRN

```

BRANCH PROJECT I.D.

WCG-1-969

ATTACHMENT	<u>D</u>
SHEET	<u>4</u> OF <u>7</u>

***** ANSYS INPUT DATA LISTING (FILE18) *****

```

241 ESTR,3,ARG1,215,EE01
242 ESTR,4,ARG1,216,EP01
243 ESTR,5,ARG1,239,EE05
244 ESTR,6,ARG1,240,EP05
245 ESTR,7,ARG1,263,EE09
246 ESTR,8,ARG1,264,EP09
247 ESTR,9,ARG1,287,EE13
248 ESTR,10,ARG1,288,EP13
249 PRVAR,2,3,4,5,6
250 PRVAR,2,7,8,9,10
251 *END
252 C***
253 *CREAT,DSP
254 DISP,21,ARG1,UX,UX
255 DISP,22,ARG1,UY,UY
256 DISP,23,ARG1,UZ,UZ
257 PROD,24,21,21,,UXUX
258 PROD,25,22,22,,UYUY
259 PROD,26,23,23,,UZUZ
260 ADC,27,24,25,26,SS
261 SQRT,28,27,,SRSS
262 PRVAR,2,21,22,23,28
263 *END
264 C***
265 *USE,STRN,311
266 *USE,DSP,2
267 *USE,STRN,391
268 *USE,DSP,10
269 C***
270 *USE,DSP,1
271 *USE,DSP,9
272 C***
273 *USE,STRN,7
274 *USE,DSP,365
275 *USE,STRN,365
276 *USE,DSP,366
277 *USE,STRN,366
278 *USE,DSP,367
279 *USE,STRN,367
280 *USE,DSP,368

```

281 *USE,STRN,368
282 *USE,DSP,369
283 *USE,STRN,369
284 *USE,DSP,6
285 *USE,STRN,8
286 *USE,DSP,375
287 C***
288 *CREAT,RFOR
289 RFORC,31,ARG1,FX,RFX
290 RFORC,32,ARG1,FY,RFY
291 RFORC,33,ARG1,FZ,RFZ
292 RFORC,34,ARG1,MX,RMX
293 RFORC,35,ARG1,MY,RMY
294 RFORC,36,ARG1,MZ,RMZ
295 PRVAR,31,32,33,34,35,36
296 *END
297 *USE,RFOR,1
298 *USE,RFOR,101
299 *USE,RFOR,413
300 *USE,RFOR,9

ATTACHMENT	<u>D</u>
SHEET	<u>5</u> OF <u>7</u>

***** ANSYS INPUT DATA LISTING (FILE18) *****

301 *USE,RFOR,109
302 *USE,RFOR,415
303 C***
304 *CREAT,ST40
305 ESTR,41,ARG1,3,STR1
306 ESTR,42,ARG1,4,STR2
307 ESTR,43,ARG1,5,UI
308 ESTR,44,ARG1,6,UJ
309 ESTR,45,ARG1,7,SLIDE
310 ESTR,46,ARG1,8,STAT
311 ESTR,47,ARG1,9,OSTA
312 ESTR,48,ARG1,10,KCON
313 PRVAR,2,41,42,43,44
314 PRVAT,2,45,46,47,48
315 *END
316 *USE,ST40,15
317 *USE,ST40,16
318 FINISH

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TITLE 12.6814 AUG 27,1991 CP= .494

***** ANSYS ANALYSIS DEFINITION (PREP7) *****

NEW TITLE= PR8G1 - EVAL. FOR PROXIMITY CASE 8(G), DWG 48N967-1, EL. 774'-0", LOOP

BRANCH/PROJECT ID : WCG-1-969

PREPARED BY : SAL 8/15/91
CHECKED BY J. OU 8/15/91

FOR COMPARISON BETW. USING GAP ELEMENT AND IMPOSED DISP.
USE GAPS ON BOTH NODE 1 AND 9
ONE LOAD STEP BEFORE GAPS CLOSE

INPUT: PR8G1
OUTPUT: PR8G1R
FILE12: PR8G1F
PLOT: PR8G1P -

C*** ANSYS ANALYSIS OPTIONS AND REFERENCES

ANALYSIS TYPE= 0 (STATIC ANALYSIS)
LARGE DEFLECTION SOLUTION (KAY(6)=1)

BRANCH PROJECT I.D.

WCG-1-969

ATTACHMENT	D
SHEET	6 OF 7

FOR LOAD STEP= 13 ITERATION= 20 SECTION= 1
 TIME= 0.0000 LOAD CASE= 1
 TITLE= PR8G1 - EVAL. FOR PROXIMITY CASE 8(G), DWG 48N967-1, EL. 774'-0", LOOP

PRINT REACTION FORCES PER NODE

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PR8G1 - EVAL. FOR PROXIMITY CASE 8(G), DWG 48N967-1, EL. 774'-0", LOOP 12.6931 AUG 27,1991 CP= 35.549

***** POST1 REACTION FORCE LISTING *****

LOAD STEP 13 ITERATION= 20 SECTION= 1
 TIME= 0.0000 LOAD CASE= 1

THE FOLLOWING X,Y,Z FORCES ARE IN NODAL COORDINATES

NODE	FX	FY	FZ	MX	MY	MZ
1	.39634196		1.0985287	-9.2391064		
4		-1.4231010				
9	-.39634196		.70198716	4.0701865		
12		2.5637629				
101	0.0000000	-13.576338	0.0000000	0.0000000	0.0000000	0.0000000
109	0.0000000	9.3864149	0.0000000	0.0000000	0.0000000	0.0000000
412				0.0000000	-18.173587	0.0000000
413				0.0000000	0.0000000	-35.983550
414				0.0000000	-7.6299577	0.0000000
415				0.0000000	0.0000000	8.5199924
TOTAL	.57180216E-09	-3.0492608	1.8005159	-5.1689198	-25.803545	-27.463557

PRINT NODAL DISPLACEMENTS

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PR8G1 - EVAL. FOR PROXIMITY CASE 8(G), DWG 48N967-1, EL. 774'-0", LOOP 12.6931 AUG 27,1991 CP= 35.563

***** POST1 NODAL DISPLACEMENT LISTING *****

LOAD STEP 13 ITERATION= 20 SECTION= 1
 TIME= 0.0000 LOAD CASE= 1

THE FOLLOWING X,Y,Z DISPLACEMENTS ARE IN NODAL COORDINATES

NODE	UX	UY	UZ	ROTX	ROTY	ROTZ
1	0.0000000	.39735211E-01	0.0000000	0.0000000	.14796928E-02	.15248559E-02
2	.12908086E-01	.48526651E-01	-.11163992E-01	.36137257E-03	.18613742E-02	.13109438E-02
3	.74055557E-01	.15404434E-01	-.99324985E-01	.48770534E-03	.25527004E-02	-.18518054E-02
4	.84423773E-01	0.0000000	-.11772844	.48770812E-03	.25552903E-02	-.19936275E-02
5	.25123251E-01	-.18843432E-02	-.53884352E-01	.29865523E-03	.25305078E-02	-.93839907E-03
6	-.33016237E-02	.14584376E-01	-.13689831E-01	-.39732926E-03	.26325591E-02	-.14685135E-02
7	-.14880760E-01	-.12577200E-01	-.39494171E-02	.16766035E-03	.18514510E-02	.13475793E-02
8	.27394063E-01	.87897620E-02	-.35361194E-01	-.50237181E-03	.17421180E-02	.13634989E-02
9	0.0000000	-.37116509E-01	0.0000000	0.0000000	.10207301E-02	-.42674643E-03
10	.11480626E-01	-.38852437E-01	-.71859161E-02	-.18942612E-03	.11666963E-02	.16937171E-03
11	.46909213E-01	-.99846820E-02	-.32497120E-01	-.30593186E-03	.13054160E-02	.21426074E-02
12	.54828151E-01	0.0000000	-.38385101E-01	-.30579107E-03	.13059292E-02	.22512665E-02
101	0.0000000	0.0000000	0.0000000	0.0000000	0.0000000	0.0000000
109	0.0000000	0.0000000	0.0000000	0.0000000	0.0000000	0.0000000
311	.64810458E-02	.44305174E-01	-.52265068E-02	.18479263E-03	.16941518E-02	.17165543E-02
321	.20646289E-01	.51718618E-01	-.19303362E-01	.38848481E-03	.20372141E-02	.74979192E-03
322	.28083961E-01	.52711397E-01	-.28105478E-01	.41134889E-03	.21812414E-02	.22919839E-03
323	.35244084E-01	.51680230E-01	-.37444992E-01	.43037910E-03	.22964827E-02	-.24575007E-03
324	.42152762E-01	.48820990E-01	-.47209073E-01	.44599391E-03	.23859625E-02	-.66996523E-03
325	.48839218E-01	.44350185E-01	-.57297412E-01	.45861539E-03	.24527039E-02	-.10383571E-02
326	.55335780E-01	.38504974E-01	-.67622210E-01	.46866869E-03	.24997285E-02	-.13458333E-02
327	.61677869E-01	.31543169E-01	-.78108178E-01	.47658148E-03	.25300568E-02	-.15872992E-02
328	.67903961E-01	.23743262E-01	-.88692530E-01	.48278335E-03	.25467079E-02	-.17576569E-02
331	.79257584E-01	.78269974E-02	-.10852473	.48775575E-03	.25549665E-02	-.19576238E-02

365	.15448238E-01	.42894097E-01	-.13626651E-01	.39741692E-03	.31027066E-02	.42249356E-03
366	.14950167E-01	.37250625E-01	-.15682506E-01	.30609863E-03	.39589856E-02	-.33805953E-03
367	.12027602E-01	.31597790E-01	-.16880058E-01	.13408415E-03	.43826839E-02	-.93842648E-03
368	.74202639E-02	.25936167E-01	-.16959449E-01	.71563787E-04	.43283124E-02	-.13469647E-02
369	.19871410E-02	.20265392E-01	-.15854246E-01	-.26402008E-03	.37556682E-02	-.15332264E-02
375	-.85428536E-02	.10076225E-01	-.12666846E-01	-.57537445E-03	.33956418E-02	-.14644299E-02
376	-.13244398E-01	.55608949E-02	-.10947024E-01	-.67182795E-03	.38022357E-02	-.12886460E-02
377	-.16840257E-01	.10376654E-02	-.88228851E-02	-.66537303E-03	.38485915E-02	-.92931888E-03
378	-.18732628E-01	-.34937652E-02	-.66553873E-02	-.53574851E-03	.35351892E-02	-.37593278E-03
379	-.18295885E-01	-.80327760E-02	-.48695603E-02	-.26393645E-03	.28666475E-02	.38055440E-03
391	.57403588E-02	-.38469318E-01	-.34743046E-02	-.94937338E-04	.11085244E-02	-.33551520E-03
401	.18570879E-01	-.36779838E-01	-.11978972E-01	-.21241367E-03	.12226025E-02	.84132108E-03
402	.25658979E-01	-.32301187E-01	-.16963066E-01	-.23556191E-03	.12622611E-02	.13709242E-02
403	.32744463E-01	-.25979460E-01	-.22077421E-01	-.25886611E-03	.12878392E-02	.17618811E-02
404	.39827616E-01	-.18362825E-01	-.27269923E-01	-.28232333E-03	.13015025E-02	.20178820E-02
405	-.10631398E-01	-.16944178E-01	-.51555064E-02	.31905819E-03	.24671846E-02	.17345572E-02
406	-.53709348E-02	-.21314482E-01	-.64280665E-02	.36174752E-03	.27714307E-02	.18735131E-02
407	.16384911E-03	-.25689379E-01	-.74725787E-02	.31339070E-03	.27742568E-02	.17745845E-02
408	.52636700E-02	-.30070219E-01	-.80530705E-02	.19249892E-03	.24920788E-02	.14491328E-02
409	.92504153E-02	-.34457897E-01	-.79947316E-02	.18292050E-04	.19471157E-02	.90959985E-03
411	.50868817E-01	-.50535082E-02	-.35440848E-01	-.30582626E-03	.13058650E-02	.22242103E-02
412				0.0000000	0.0000000	0.0000000
413				0.0000000	0.0000000	0.0000000
414				0.0000000	0.0000000	0.0000000
415				0.0000000	0.0000000	0.0000000

ATTACHMENT D
SHEET 7 OF 7

MAXIMUMS	4	322	4	376	367	12
NODE						
VALUE	.84423773E-01	.52711397E-01	-.11772844	-.67182795E-03	.43826839E-02	.22512665E-02

PRODUCE DISPLACEMENT PLOT, KUND= 0

CUMULATIVE PLOT NUMBER 6 WRITTEN TO FILE FILE33_DAT - RASTER MODE.
PLOT TITLE= PR8G1 - EVAL. FOR PROXIMITY CASE 8(G), DWG 48N967-1, EL. 774'-0", LOOP

PRODUCE DISPLACEMENT PLOT, KUND= 1

CUMULATIVE PLOT NUMBER 7 WRITTEN TO FILE FILE33_DAT - RASTER MODE.
PLOT TITLE= PR8G1 - EVAL. FOR PROXIMITY CASE 8(G), DWG 48N967-1, EL. 774'-0", LOOP

***** ROUTINE COMPLETED ***** CP = 35.715

C***

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PR8G1 - EVAL. FOR PROXIMITY CASE 8(G), DWG 48N967-1, EL. 774'-0", LOOP 12.6933 AUG 27,1991 CP= 35.717

***** GENERAL GRAPH POSTPROCESSOR (POST26) *****

ALL POST26 SPECIFICATIONS ARE RESET TO INITIAL DEFAULTS

NUMBER OF LINES PER PAGE SET TO200

MAXIMUM NUMBER OF VARIABLES= 50

USE NCUMIT AS TIME VARIABLE

VARIABLE 2 IS ELEMENT 1 ITEM 53 NAME= 1 TEMP

START COPY FROM INPUT TO STRN EXT=

END FILE CREATE

C***

START COPY FROM INPUT TO DSP EXT=

ATTACHMENT E
SHEET 1 OF 10

APPENDIX B
Page 1 of 1

WALKDOWN PACKAGE COVERSHEET

WALKDOWN PACKAGE NO. TI-2007/ESI-TH1

Page 1 of 10

WATTS BAR UNIT - 0, 1
MAIN STEEL/PLATFORMS/MISCELLANEOUS STEEL
ENGINEERING WALKDOWN

Building: REACTOR Drawing No. 48H967-1 Rev. 0

Elevation/Location: 774'-0" / LOOP # 4

Walkdown Team: J. Tash Date 7/8/91

Walkdown Team: Z. Meyers Date 7/8/91

Accepted by
Walkdown
Team
Coordinator: Z. Meyers Date 7/8/91

Revisions To Walkdowns (Sign and Date)

	1	2	3	4
Walkdown by				
Walkdown Verified by				
Walkdown Team Coordinator				

APPENDIX C

Page 1 of 9

WALKDOWN CHECKLIST
CIVIL CALCULATIONS PROGRAM (STEEL)

ATTACHMENT E
OF 10

Walkdown Package No. TH

Page 2 of 10

Member No. 1 THRU 8

Description of Attribute	Within Established Criteria (Yes/No/NA)	If No, describe deviation	Comments
A. Member Configuration 1. Location	NA	J.T. 7/8/91	
2. Orientation			
3. Size			
4. Sweep			
5. Copes			
6. Notches & Cuts			

Preparer: J. Tash Date: 7/8/91
 Verifier: J. Mayne Date: 7/8/91

BRANCH PROJECT I.D.

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 Page 15 of 40

ATTACHMENT E
 SHEET 3 OF 10

APPENDIX C
 Page 2 of 9

TI-2007/ESI-
 Walkdown Package No. TH1
 Page 3 of 10
 Member No. 1 THRU 8

Description of Attribute	Within Established Criteria (Yes/No/NA)	If No, describe deviation	Comments
7. Flame Cut Edges	NA ^{J.T.} 7/8/91		
8. Other Attributes			
B. Connections (Bottom or left connection) 1. Identification of connection	—	—	}
2. Size			
3. Alterations	y		

Preparer: J. T. Tash Date: 7/8/91
 Verifier: J. Mejjac Date: 7/8/91

BRANCH PROJECT I.D.

WCG-1-969

WBK

TI-2007

Revision 0

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ATTACHMENT E
SHEET 4 OF 10

APPENDIX C
Page 3 of 9

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Walkdown Package No. 141
Page 4 of 10
Member No. 1 THRU 8

Description of Attribute	Within Established Criteria (Yes/No/NA)	If No, describe deviation	Comments
4. Bolting			
a. Type	NA	J.T. 7/8/91	
b. Size			
c. Number of bolts			
d. Bolt Spacing			
e. Edge Distance			
f. Locking Devices			
g. Nut Engagement			
h. Washers			
i. Bolt Tightness			
5. Welding			
a. Weld Existence and Length			
b. Weld Size			
c. Unspecified Welds			

Preparer: J. Teak Date: 7/8/91
 Verifier: J. Mayhew Date: 7/8/91

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ATTACHMENT E
 SHEET 5 OF 10

APPENDIX C
 Page 4 of 9

TI-2007/ESI-
 Walkdown Package No. TH1
 Page 5 of 10
 Member No. 1 THRU 8

Description of Attribute	Within Established Criteria (Yes/NO/NA)	If No, describe deviation	Comments
6. Expansion Anchor Plate Assemblies NOTE Attach a sketch similar to Appendix G and provide all applicable info.	J.T. 7/8/91 NA		
7. Embedded Plate			
8. Clearance at End of Member			
B. Connections(cont'd) (Top or Right Connections)			
1. Identification of Connection	_____	_____	
2. Size			
3. Alterations	▼		

Preparer: J. Tash Date: 7/8/91
 Verifier: D. Meppa Date: 7/8/91

ATTACHMENT E
 SHEET 6 OF 10

APPENDIX C

Page 5 of 9

TI-2007/ESI-

Walkdown Package No. TH3

Page 6 of 10

Member No. 1 THRU 8

Description of Attribute	Within Established Criteria (Yes/No/NA)	If No, describe deviation.	Comments
4. Bolting	NA J.T. 7/8/91		
a. Type			
b. Size			
c. Number of Bolts			
d. Bolt Spacing			
e. Edge Distance			
f. Locking Devices			
g. Nut Engagement			
h. Washers			
i. Tightness			
5. Welding	Y		
a. Weld Existence and Length			
b. Weld Size			
c. Unspecified Welds			

Preparer: J. Tash
 Verifier: J. Mayone

Date: 7/8/91
 Date: 7/8/91

BRANCH PROJECT I.D.

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WBH
 TI-2007
 Revision 0
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ATTACHMENT E
 SHEET 7 OF 10

APPENDIX C
 Page 6 of 9

TI-2007/ESI-
 Walkdown Package No. TH
 Page 7 of 10
 Member No. 1 THRU 2

Description of Attribute	Within Established Criteria (Yes/No/NA)	If No, describe deviation	Comments
6. Expansion Anchor Plate Assemblies NOTE Attach a sketch similar to Appendix G and provide all applicable info.	NA J.T. 7/8/91		
7. Embedded Plate			
8. Clearance at End of Member			
C. Miscellaneous Details 1. Location			
2. Size			

Preparer: J. Tash Date: 7/8/91
 Verifier: J. Meyer Date: 7/8/91

Walkdown Package No. TI-2007/ESI-741
Page 8 of 10
Member No: 1 THRU 8

D. Attachments

ATTACHMENT E
SHEET 8 OF 10

Type of Support	Tag Number	Location from Bottom of col. or left end	Stiffeners (Yes/No)	Insulation (Yes/No) Thickness	Attachment Point on Deam	Comments
1. Large Bore Pipe Supports (Larger than 2" nominal diameter)	NA ^{J.T. 7/18/91}	NA ^{J.T. 7/18/91}	NA ^{J.T. 7/18/91}	NA ^{J.T. 7/18/91}	NA ^{J.T. 7/18/91}	
2. Small Bore Pipe Supports (2" nominal diameter and under)						
3. HVAC Supports						
4. Cable Tray Supports						

BRANCH PROJECT I.D.
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PBN
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Page 20 of 4

Preparer: J. Trach Date: 7/18/91
Verifier: J. Meppa Date: 7/31/91

Walkdown Package No. TI-2007/ESI-THZ
Page 9 of 10
Member No. 1 THRU 8

D. Attachments (continued)

Type of Support	Tag Number	Location from Bottom of col. or left end	Stiffeners (Yes/No)	Insulation (Yes/No) Thickness	Attachment Point on Beam	Comments
5. Equipment Supports	NA ^{J.T.} 7/8/91	NA ^{J.T.} 7/8/91	NA ^{J.T.} 7/8/91	NA ^{J.T.} 7/8/91	NA ^{J.T.} 7/8/91	
6. Conduit Supports						
7. Instrument Line Supports						
8. Other Supports						

Preparer: J. Trach Date: 7/8/91
Verifier: J. Mayers Date: 7/8/91

ATTACHMENT E
SHEET 9 OF 10

BRANCH PROJECT I.D.

WCG-1-969

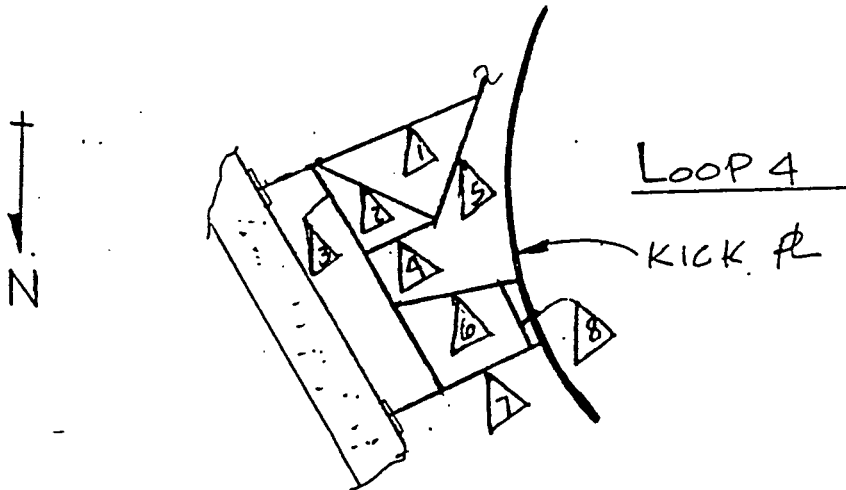
ATTACHMENT	<u>E</u>	
SHEET	<u>10</u>	OF <u>10</u>

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APPENDIX C
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Walkdown Package No. II-2007/ESI-7
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 Member No. 1 THRU 8

CONTINUATION SHEET



PARTIAL PLAN AI-EL774'-0"
 (REF DWG. 48 N 967-1, REV. 1)

NOTE:

"NO DISCREPANCY OF CONFIGURATION IS FOUND BETWEEN
 DRAWING AND INSTALLED CONDITION"

Preparer: J. Tach Date: 7/8/91
 Verifier: J. Meyers Date: 7/8/91