

# RAILROAD BRIDGE MANAGEMENT PROGRAM

# Connecticut Department Of Transportation Office of Rail



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# CONNECTICUT DEPARTMENT OF TRANSPORTATION

# BUREAU OF PUBLIC TRANSPORTATION OFFICE OF RAIL

# <u>CONNECTICUT</u> <u>RAILROAD BRIDGE</u> <u>MANAGEMENT PROGRAM</u>

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### **REVISIONS TO RAILROAD BRIDGE** MANAGEMENT PROGRAM MANUAL

(as of February 6, 2014)

The following are revisions that have been made to the Connecticut Department of Transportation's Office of Rail *Railroad Bridge management program Manual Version 1.0* since issued in February 2012.

<b>CHAPTER</b>	PAGES REVISED	PAGES ADDED	<b>REVISION DATE</b>	<u>COMMENTS</u>
1	1-1		02/06/14	RBIM has been developed
4	4-9		02/06/14	Bridge Strike (Section 4.3 added)
APPENDIX 3		A3A	02/06/14	MNR Bridge Strike procedure. (section added)
APPENDIX 5	A5-1, A5-2, A5-3		02/06/14	Organizational charts for Inspection of railroad bridges (personnel update)
5	A5-4		02/06/14	Organizational chart for Parson Brinckerhoff (removed)
APPENDIX 6	A6-1, A6-2		02/06/14	Organizational charts for Field Verification and Design of Repairs for Railroad bridges (personnel update)
APPENDIX 7		7A	02/06/14	Bridge Inspection Damage Report Form
APPENDIX 10	A10-1		02/06/14	Off-System Contact Numbers
APPENDIX 10		A10-2	02/06/14	MNR Contact Numbers
APPENDIX 10		A10-3	02/06/14	Office of Rail Contact Numbers

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#### **CHAPTER 1: OVERVIEW**

#### 1.1. <u>PURPOSE</u>

The Federal Railroad Administration (FRA) established Railroad Bridge Safety Standards, 49 CFR Parts 213 and 237, that became effective September 13, 2010. These regulations require track owners to adopt and implement a Railroad Bridge Management Program (RBMP). A copy of the Railroad Bridge Safety Standards are attached in Appendix 1.

The purpose of this document is to define the procedures and practices of the Connecticut Department of Transportation (ConnDOT) to:

- Establish and maintain an accurate inventory of railroad bridges owned by ConnDOT including a record of the safe load capacity of each bridge.
- Provide railroad bridge inspection, evaluation, load capacity, and reporting procedures.
- Establish policy for the ConnDOT railroad bridge inspection, evaluation and load rating program.
- Establish standards for the documentation of inspection reports, repairs and modifications of each railroad bridge.
- Define qualifications for persons responsible for executing the RBMP and designate such individuals.
- List the types of railroad bridge inspections and identify the required inspection details.
- Serve as a standard and provide uniformity in the execution of the program by establishing the methods for documenting inspections including a standardization of forms.
- Define program responsibilities for ConnDOT and for liaison with railroads and outside agencies.
- Set guidelines for interpretation and implementation of AREMA and FRA codes and standards.
- Establish formal quality control and quality assurance procedures.

This RBMP is intended to be specific to structures supporting railroad track and is complemented by the ConnDOT "*Railroad Bridge Inspection Manual*" (RBIM). The AREMA "*Bridge Inspection Handbook (2008)*" shall be used as a supplemental reference, providing guidelines for inspectors specific to railroad structures.

The general requirements for the management of bridges carrying highways or pedestrians, or for certain bridges over State routes are defined in the National Bridge Inspection Standards (NBIS) in the Code of Federal Regulations, 23 CFR 650C. The ConnDOT BIM addresses the policy for these bridges that includes a group of railroad bridges that pass over State roadways.

ConnDOT's ownership of railroad structures is not limited to bridges as the rail infrastructure includes numerous retaining walls, culverts not meeting the minimum criteria to be classified as a railroad bridge, and a select group of tunnels. ConnDOT recognizes the critical functions these other types of structures serve in providing a safe railroad network. These other structures are not specifically addressed by the FRA's Bridge Safety Regulations or this RBMP, however, ConnDOT does include the inspection and repair of these structures as part of its overall railroad infrastructure management program.

#### 1.2. RAILROAD BRIDGE MANAGEMENT PROGRAM OBJECTIVES

The objectives of ConnDOT's Railroad Bridge Management Program are:

- To fulfill the requirements of 49 CFR Parts 213 and 237, Bridge Safety Standards.
- To prevent the deterioration of railroad bridges by preserving their capability to safely carry the traffic to be operated over them, and reduce the risk of human casualties, environmental damage, and disruption to the Nation's railroad transportation system that would result from a catastrophic bridge failure.
- To maintain an accurate, up-to-date inventory that records the condition and load capacity of all qualifying railroad structures owned by the State of Connecticut, and to meet the requirements for content of the inventory as defined in §237.33 including but not limited to: bridge identifier; location; configuration; type of construction; number of spans; span length(s); and safe load capacity.
- To establish policy for a railroad bridge inspection program that addresses the requirements for a bridge inspection program as defined in §237.33 including but not limited to: inspection personnel safety considerations; types of inspections; definitions of defect levels and condition codes; method of documenting inspections; and structure type and component nomenclature.
- To define the method of documentation by inspectors; including documentation through the use of standard forms.
- To establish policy for obtaining and maintaining documents pertinent to the management of railroad bridges as defined in §237.33 including but not limited to: record plans; design calculations; and inspection reports.
- To determine the extent of deterioration and initiate routine maintenance and repair work.
- To determine the extent of major deterioration and prioritize the repairs and the capital investment in the rehabilitation or replacement of railroad bridges.

#### 1.3. BACKGROUND

The general requirements for the inspection, evaluation, and load rating of the nation's railroad bridges are defined in the Code of Federal Regulations, 49 CFR Parts 213 and 237. These regulations became effective on September 13, 2010 and each railroad track owner with qualifying structures is required to adopt and implement a Railroad Bridge Management Program (RBMP). The Connecticut rail system consists of over 600 miles of active rail segments that are owned by a combination of the Department of Transportation (ConnDOT), the Department of Energy and Environmental Protection (DEEP), the National Railroad Passenger Corporation (Amtrak) and private carriers. See Connecticut Rail Transportation Ownership and Service Map (Appendix 2). ConnDOT is the railroad track owner as defined in §237.5 for the commuter operations and freight on the New Haven Line and numerous lines or segments of lines supporting freight and tourist operations throughout the state. This RBMP establishes policy for the management of these ConnDOT owned railroad structures. Figure 1 contains a summary of ConnDOT's track ownership.

Rail Line and Location	Owner	Railroad Operator		
NEW HAVEN LINE				
Main Line Greenwich to New Haven	ConnDOT	Metro-North Railroad, Providence and Worcester Railroad Company, CSX Transportation		
New Canaan Branch Stamford to New Canaan	ConnDOT	Metro-North Railroad, CSX Transportation		
Danbury Branch Norwalk to Danbury	ConnDOT	Metro-North Railroad, Providence and Worcester Railroad Company		
Waterbury Branch Milford to Waterbury	ConnDOT	Metro-North Railroad, Providence and Worcester Railroad Company, Springfield Terminal Railway		
	OFF-SYSTEM LIN	ES		
Berkshire Line - North Section New Milford to North Canaan	ConnDOT	Housatonic Railroad Company		
Torrington Branch Line Waterbury to Torrington	ConnDOT	Naugatuck Railroad Company		
Middletown Secondary Line Durham to Middletown	ConnDOT	Providence and Worcester Railroad Company		
Cromwell Industrial Track Middletown	ConnDOT	Providence and Worcester Railroad Company		
Laurel Industrial Track	ConnDOT	Providence and Worcester Railroad Company		
Wethersfield Secondary Line Hartford to Middletown	ConnDOT	Providence and Worcester Railroad Company		
Willimantic Secondary Line Windham to Sprague	ConnDOT	Providence and Worcester Railroad Company		
Plainfield Secondary Line Plainfield to Plainfield	ConnDOT	Providence and Worcester Railroad Company		
Armory Branch Line South Windsor to Enfield	ConnDOT	Central New England Railroad		
Griffin Line Hartford to Windsor	ConnDOT	Central New England Railroad		
Torrington Secondary Waterbury	ConnDOT	Springfield Terminal Railway		

Figure 1. Inventory of ConnDOT owned Railroad Lines (Office of Rail Compilation - July, 2011)

Airline	ConnDOT	Abandoned
East Hampton		
Moosup Valuation Map	ConnDOT	Abandoned
Plainfield		
Vernon	ConnDOT	Abandoned
		(Rails to Trails)
New Britain Secondary Line	ConnDOT	Inactive
New Britain		
Avon Secondary Line	ConnDOT	Inactive
Avon		(Rails to Trails)
Kendall's Industrial Track	ConnDOT	Inactive
Windham		(Rails to Trails)
Farmington Valley Greenway	ConnDOT	Inactive
Farmington		(Rails to Trails)
Valuation Map 56-60/5, 6, 7, 8	ConnDOT	Inactive
Farmington to Canton		(Rails to Trails)
Valuation Map 57-72/54, 56	ConnDOT	Inactive
Winchester		

Figure 1 (cont.). Inventory of ConnDOT owned Railroad Lines (Office of Rail Compilation - July, 2011)

There are eight (8) freight railroad companies operating over tracks owned by ConnDOT. Some control and operate on lines leased by ConnDOT while other railroad companies have trackage rights by agreement to operate over tracks controlled by a competitor. The list of these operating railroads follows:

CSX Transportation (CSX) Providence and Worcester Railroad Company (PWRR) New England Central Railroad (NECR)<sup>1</sup> Connecticut Southern Railroad (CSOR)<sup>1</sup> Housatonic Railroad Company (HRRC) Central New England Railroad (CNZR) Naugatuck Railroad Company (NRR) Springfield Terminal Railway (STRR)

<sup>1</sup> RailAmerica Incorporated is a holding company that owns and operates freight railroads. They have two (2) subsidiaries that operate in Connecticut: CSOR and NECR.

<sup>2</sup> Springfield Terminal Railway is a subsidiary of Pan Am Railways.

The Federal Regulations stipulate that each railroad track owner perform inspections, prepare reports, and determine the safe load capacity for railroad bridges in accordance with their adopted RBMP. The Regulations establish certain minimum criteria that must be included in the RBMP but allow track owners some flexibility to develop a program that is customized to the individual needs of the owner. The regulations encourage owners to inspect, evaluate and load rate bridges using the general provisions of the AREMA "Manual for Railway Engineering" but allows owners the option to establish or adopt other appropriate criteria. In addition, the AREMA "Bridge Inspection Handbook" and AASHTO Manuals, Technical Advisories and AASHTO Specifications, Codes, and Guidelines serve as source material for track owners to conduct operations in compliance with the regulations.

ConnDOT has established this RBMP to meet the requirements of the Federal Regulations. Certain policies and procedures were previously addressed in the BIM, and other practices have been established by ConnDOT Office of Rail policy, written and unwritten, or by historic practices. This Program has been developed to set down the formal ConnDOT policy for complying with FRA Bridge Safety Regulations, and to define the organizational structure and assign responsibilities for implementation of the stated policy.

Prior to the establishment of the Federal Bridge Safety Regulations, ConnDOT's railroad inspection program included routine type inspections on a biennial basis. These routine inspections were often supplemented by annual inspections performed by the operating railroads. CFR §237.101 mandates annual inspection of railroad bridges and ConnDOT has amended their policy to supplement the biennial routine inspections that have been performed historically with an additional biennial program of "verification inspections". The intent of a verification inspection is to visually confirm and/or update the condition of the bridge components identified in the previous Routine or In-Depth Inspection Report. Verification inspections are scheduled on alternating years with the routine inspections.

ConnDOT has entered into agreements with the various railroads that operate on all active lines. These Operating Agreements assign partial responsibility to the Operating Railroad for the maintenance of the rail facilities, including bridges and culverts that are subject to the FRA Bridge Safety Regulations. ConnDOT and the Operating Railroads share information related to the condition of all railroad structures on the given line including but not limited to inspection reports, maintenance memorandums, load ratings and any operating restrictions that may be in place. Where the Operating Agreement assigns responsibility to the railroad for bridge maintenance or related work that is addressed by the FRA Bridge Safety Regulations, it is expected that the work performed by the railroad or by contractor be done in a manner that conforms to the applicable sections of the regulations. It is critical that a policy of open communication and cooperation be maintained between ConnDOT and the Operating Railroads to ensure the resources of both parties are managed in a manner that limits duplication of effort in regards to the management of railroad bridges.

#### 1.4. **DEFINITIONS**

<u>RBMP (Railroad Bridge Management Program)</u>. A program designed to optimize the use of available resources for the inspection, evaluation, load rating, maintenance, rehabilitation, and replacement of railroad bridges.

<u>BIM</u>. ConnDOT Bridge Inspection Manual (September 2001, version 2.1, with interims).

<u>RBIM</u>. ConnDOT Railroad Bridge Inspection Manual (2012).

<u>AASHTO</u>. American Association of State Highway and Transportation Officials, 444 North Capitol Street, N.W., Suite 225, Washington, DC 20001.

<u>AREMA</u>. American Railway Engineering and Maintenance-of-Way Association, 10003 Derekwood Lane, Suite 210, Lanham, MD 20706.

AREMA HANDBOOK. AREMA Bridge Inspection Handbook (2008)

<u>RAILROAD BRIDGE (§237.5)</u>. Any structure with a deck, regardless of length, which supports one or more railroad tracks, or any other undergrade structure with an individual span length of 10 feet or more located at such a depth that it is affected by live loads.

<u>QUALIFYING RAILROAD STRUCTURES</u>. All railroad bridges, as defined by FRA §237.5, and all other undergrade railroad structures (other than pipes) with an individual span length of 5 ft or more, located at such depth that it is affected by live loads, owned by ConnDOT. This term and definition is specific to ConnDOT owned railroad structures and reflects the Department policy of inspecting railroad culverts with individual span lengths in the range of 5 to 10 feet but recognizing these structures are not specifically addressed by the federal bridge regulations.

<u>ACTIVE RAILROAD LINE</u>. A railroad line or segment of a line that is connected to the general railroad system of transportation and over which trains may operate.

<u>INACTIVE or ABANDONED RAILROAD LINE</u>. A former railroad line or segment of a line that is or was connected to the general railroad system of transportation and over which trains do not operate but may in the future.

<u>NBIS (National Bridge Inspection Standards)</u>. Federal regulations establishing requirements for inspection procedures, frequency of inspections, qualifications of personnel, inspection reports, and preparation and maintenance of bridge inventory records. The NBIS apply to all structures defined as bridges located on or over all public roads.

<u>ConnDOT</u>. Connecticut Department of Transportation.

<u>DEPARTMENT</u>. Connecticut Department of Transportation.

FRA. Federal Railroad Administration, U.S. Department of Transportation.

MUTCD. The Manual of Uniform Traffic Control Devices.

#### **CHAPTER 2: CONNDOT RAILROAD BRIDGE INVENTORY**

#### 2.1. BRIDGE INVENTORY

ConnDOT shall maintain an inventory of all qualifying railroad structures. The content of such inventory will include at a minimum, the information to satisfy the requirements of §237.33. This required content includes bridge identifier; location; configuration; type of construction; number of spans; and span lengths. Additional information may be included in the inventory as deemed appropriate by ConnDOT to aid in the management of these structures. This inventory shall be updated as necessary when bridge replacement, rehabilitation or other activities result in changes to the information maintained in the inventory.

See Appendix 3 for the Inventory of Railroad Bridges -Metro-North Railroad Bridges.

See Appendix 4 for the Inventory of Railroad Bridges - Off-System Railroad Bridges.

#### **CHAPTER 3: STAFF QUALIFICATIONS**

#### 3.1. **QUALIFICATIONS**

ConnDOT shall designate and maintain a list of individuals who perform activities critical to the effective management of the railroad bridge infrastructure as required by §237.57. Such lists shall be maintained for the following critical positions: Railroad Bridge Engineer; Railroad Bridge Inspector; and Railroad Bridge Supervisor. ConnDOT shall also maintain a record of each individual's educational or work experience that supports the required competencies for the position. The list shall include both employees of ConnDOT and consulting engineering firms.

<u>Railroad Bridge Engineer (§237.51, §237.111)</u> - A person competent to perform engineering work including: determination of forces and stresses in railroad structures; determination of structural capacity of railroad structures; prescribe inspection procedures for railroad structures; review bridge inspection reports and design repairs or modifications for railroad structures. Railroad Bridge Engineers shall satisfy either of the following educational requirements: possess a degree in engineering from a program accredited by ABET, Inc. or by a foreign organization recognized by ABET, Inc.; or be currently registered as a professional engineer in the State of Connecticut.

<u>Railroad Bridge Inspector (§237.53)</u> - A person technically competent to view, measure, report, and record the condition of a railroad bridge and its individual components. Such competence shall be established based on a combination of education, formal training and work experience.

<u>Railroad Bridge Supervisor (§237.55, §237.111)</u> - A person technically competent to supervise the construction, modification or repair of a railroad bridge in conformance with plans, specifications or instructions applicable to the work to be performed; and to review bridge inspection reports. Such competence shall be established based on a combination of education, formal training and work experience.

ConnDOT has established the following additional criteria for their employees to serve in the roles established and defined by the FRA:

FRA Designation	ConnDOT Classification
Railroad Bridge Engineer	Transportation Supervising Engineer
Railroad Bridge Inspector	Transportation Engineer III
Railroad Bridge Supervisor	Supervisor Rail Officer or Transportation Engineer III

See Appendix 5 for the Organization Charts for Inspection of Railroad Bridges.

See Appendix 6 for the Organization Charts for Field Verification and Design of Repairs for Railroad Bridges.

#### **CHAPTER 4: SAFE LOAD CAPACITY FOR RAILROAD BRIDGES**

#### 4.1. LOAD CAPACITY

ConnDOT shall determine the safe load capacity of all railroad bridges on active railroad lines to satisfy the requirements of §237.71. Load rating is the determination of live load carrying capacity of a bridge using existing bridge plans supplemented by information gathered during indepth field inspections. Engineering judgment is required to incorporate the effect of defects and deterioration in the load rating analysis. For those bridges that do not have calculated capacities on file, ConnDOT shall complete the load rating assessments by no later than March 14, 2016, satisfying the §237.71.e requirement that all capacities be determined within five years of the required date of adoption for the RBMP. ConnDOT has historically performed load rating calculations for the steel, timber and concrete railroad bridges within their railroad system and has updated these ratings regularly to reflect the current condition of the bridges. The federal bridge regulations requires that two types of bridges that have not historically had load ratings performed by ConnDOT now be evaluated: masonry arches and short span deck structures. While both of these structure types are not uncommon in the CT rail infrastructure, ConnDOT has not had a history of any safety issues with these types of structures due to structural capacity, and it is expected that the safe load capacities will exceed the demand by a conservative margin. ConnDOT is in the process of scheduling the evaluation work required for determining the safe load capacity for these outstanding bridges and expects to complete all work prior to the federal deadline.

Load ratings for ConnDOT owned railroad bridges shall be performed in accordance with the provisions set forth is the AREMA "Manual for Railway Engineering" and using the Cooper E80 loading to represent the operating equipment. The Manual recognizes load ratings at two levels, Normal and Maximum. The Normal Rating generally corresponds to the design level of stress, and results in a calculated live load that can safely use the bridge for an indefinite period of time. The Maximum Rating sets the limiting live load which the structure can support at an infrequent interval. The Safe Load Capacity of a bridge shall generally be considered the Normal Rating calculated for the bridge except in special cases where the Railroad Bridge Engineer determines it is appropriate to consider the Maximum Rating as the Safe Load. An example of a condition that might warrant the use of Maximum Rating levels for determining allowable operating loads includes a short to medium duration operational period while a known bridge deficiency is in the process of being repaired, rehabilitated or replaced. In no case shall an operating load producing structural demand exceeding the Maximum Rating of a bridge be allowed to operate over the structure.

The load capacity for each railroad bridge on an active line shall be maintained in the Bridge Files and also be compiled as part of the Summary of Safe Load Capacity of Metro-North Railroad Bridges and the Summary of Safe Load Capacity of Off-System Railroad Bridges maintained in accordance with §237.33. (Appendices 7 and 8) The determination of load capacity shall be made by a Railroad Bridge Engineer using appropriate engineering methods and standards.

Load rating calculations shall be updated whenever a routine, verification or special inspection reveals the condition of the bridge or a bridge component might adversely affect the ability of the bridge to carry the traffic being operated. Bridge conditions that warrant an updated rating include but are not limited to: a reduction in a member's load-carrying strength due to deterioration; a track modification that increases dead load; changes in track geometry; damage to members from collision; and structural modifications.

The load rating calculation for each qualifying railroad structure shall be updated at a maximum interval of ten (10) years and shall be performed in conjunction with the structure's In-Depth Inspection. Updated ratings shall reflect the condition of the bridge as determined by the In-Depth Inspection.

Normal and Maximum ratings shall be determined for the Cooper E 80 load defined by AREMA. The Normal rating level corresponds to the usual design load level, but reflects the existing bridge conditions with regard to age, deterioration and loss of section. This analysis is comparable to that used for design and, therefore, results in an allowable live load that can be carried by the existing structure for its expected service life. The rating is dependent on a specified speed, as impact reductions are allowed for reduced speeds. The Maximum rating is the load level which the structure can support at infrequent intervals, with any applicable speed restrictions. Unlimited usage of the bridge by rail vehicles at the Maximum level will shorten the useful life of the structure.

Generally, the rating factor for a structure is obtained by subtracting the dead load effect on the member from the overall capacity of the member and dividing the results by the effect of the live load and impact induced by rail equipment with known weight and configuration. Allowable stress levels for the Normal and Maximum ratings shall be in accordance with AREMA guidelines. The capacity of a member in relationship to the Cooper E series live load configuration can be obtained by multiplying the rating factor by the 80 kip maximum axle load associated with the Cooper E 80 load used in determining the live load effect. The resulting ratings are considered to represent the "Equivalent Cooper" rating for the member under consideration.

Load Rating calculations shall typically be performed for both the "as-inspected" and "as-built" conditions. The "as-inspected" ratings reflect the current status of the structure and are used for decisions regarding operations, while the "as-built" ratings provide a baseline for assessment of a structure's condition and to assist decision making on bridge repairs, rehabilitation or replacement.

Railroad structures supporting multiple tracks require load ratings to be determined for each track considering the effect of concurrent track loading in accordance with AREMA guidelines.

ConnDOT shall compare the calculated "Equivalent Cooper" Normal Ratings with the live load demand placed on structures of similar configuration by equipment known to operate on the line without restriction. A listing of equipment known to operate on the CT railroad infrastructure and the relative load effects has been compiled to facilitate such comparisons. (Appendix 9) For structures where this demand appears to exceed or is in the range of the Normal Cooper Rating, bridge specific load ratings shall be performed using actual equipment in lieu of the notional Cooper loading. For structures that have calculated Normal Capacity less than the demand from actual equipment operating without restriction, the ConnDOT Railroad Bridge Engineer shall evaluate the structure and determine appropriate actions that may include: scheduling of bridge component repair, modification or replacement; issuing

instructions to the railroads placing operating restrictions necessary to ensure the bridge can safely accommodate traffic until such time when the capacity of the structure is increased to meet the demand; implementing a program of increased monitoring; or in cases where operating loads would exceed Maximum Ratings, restricting operations over the bridge until adequate capacity can be restored.

For railroad bridges on active railroad lines that have not been analyzed for load capacity, ConnDOT shall prioritize the rating of these structures to ensure that all structures have calculated ratings on file by March 14, 2016 to satisfy the requirements of §237.71.e.

See Appendix 7 for the Draft Summary of Safe Load Capacity of Metro-North Railroad Bridges.

See Appendix 8 for the Draft Summary of Safe Load Capacity of Off-System Railroad Bridges.

#### 4.2. PROTECTION OF BRIDGES FROM OVER-WEIGHT AND OVER-DIMENSION LOADS

Railroad equipment operating on ConnDOT owned track is controlled by the written instructions established by the individual railroad assigned responsibility for operation on a given line or segment of a line. Such instructions, typically in the form of a timetable, are developed by the railroad and reviewed by ConnDOT to ensure the equipment allowed to operate does not exceed the safe load or dimensional restrictions associated with all structures on the line. When changes in structural condition affect the capacity of a given line, ConnDOT provides written direction to the operating railroad with any recommended restrictions that may be appropriate to ensure safe operations. The operating railroad is responsible for issuing special instructions that address any restrictions imposed. In general, written instructions shall be sufficient in detail to meet the criteria of §237.73.

Load capacity for structures shall be expressed in terms of the maximum gross weight of rail cars with the minimum car length and axle configuration as established by the Association of American Railroads (AAR) Standards for Freight Cars. The load effects for all equipment not meeting the above standard including all locomotives shall be analyzed on an individual basis. Figure 2 summarizes the gross weight of freight cars allowed to operate on ConnDOT owned rail lines and the allowable clearance envelope for all equipment.

ConnDOT has compiled a listing of all equipment known to operate on the ConnDOT owned rail lines with the equipment configuration provided by the operating railroads. This equipment has been evaluated and compared to the notional Cooper E80 loading to aid decision making related to the safe capacity of bridges. (Appendix 9) For structures that have calculated capacities that are similar in magnitude to the demand from the equipment known to operate on the line, more detailed calculations have been performed using the actual equipment configuration to calculate stress levels in the bridge components. If an individual structure is identified that does not have a Normal Rating that exceeds the demand from normal operations, ConnDOT shall evaluate the condition and make a determination that may include: a decision to allow continued normal operations for a period of time while bridge repair, rehabilitation or replacement work is scheduled and implemented; provide written instructions to the operating railroad restricting certain equipment from operating over the structure; provide written instructions to the operating railroad restricting the authorized speed for certain equipment; or restricting all operations over the given bridge. In no case shall ConnDOT allow operations over a structure that has a calculated Maximum Rating that is less than the demand from equipment allowed to operate over a structure.

OWNER	RAIL LINE	CLEARANCE RESTRICTION	WEIGHT LIMIT (POUNDS)	FREIGHT OPERATOR
ConnDOT	NEW HAVEN LINE		263,000	CSX, PWRR
ConnDOT	NEW CANAAN BRANCH		263,000	CSX
ConnDOT	DANBURY BRANCH	PLATE "C"	263,000	PWRR
ConnDOT	WATERBURY BRANCH - LOWER	PLAIE C	263,000	PWRR
ConnDOT	WATERBURY BRANCH - UPPER		263,000	STRR
ConnDOT	BERKSHIRE LINE - NORTH SECTION	PLATE "F" WITH MAXIMUM HEIGHT OF 17'-8"	286,000	HRRC
ConnDOT	TORRINGTON BRANCH	PLATE "F"	263,000	NRR
ConnDOT	TERRYVILLE SECONDARY - SOUTH SECTION	PLATE "F"	263,000	STRR, NRR
ConnDOT	BRADLEY SPUR	N.A.	286,000	CSOR
ConnDOT	MIDDLETOWN SECONDARY - UPPER	PLATE "F" WITH MAXIMUM HEIGHT OF 17'-2"	263,000	PWRR
ConnDOT	WILLIMANTIC SECONDARY - WEST SECTION	PLATE "F" WITH MAXIMUM HEIGHT OF 19'-2"	263,000	PWRR
ConnDOT	ARMORY (EAST WINDSOR SECONDARY) - NORTH SECTION	N.A.	263,000	CNZR
ConnDOT	GRIFFINS INDUSTRIAL TRACK	PLATE "F"	263,000	CNZR

Figure 2. Allowable Freight Car Gross Weight and Clearance for ConnDOT owned Railroad Lines (September, 2011)

CSX = CSX Transportation PWRR = Providence and Worcester Railroad Company CSOR<sup>1</sup> = Connecticut Southern Railroad HRRC = Housatonic Railroad Company CNZR = Central New England Railroad NRR = Naugatuck Railroad Company STRR<sup>2</sup> = Springfield Terminal Railway

<sup>1</sup> Subsidiary of RailAmerica Incorporated.

<sup>2</sup> Subsidiary of Pan Am Railways.

All railroad structures with structural components extending above a plane established at the top of rail elevation shall have clearance measurements recorded and maintained in the Bridge

Files. For existing structures that have any members within the AREMA clearance envelope for fixed obstructions of railway bridges (Figure 3), detailed measurements of all obstructions within the clearance envelope shall be recorded. For structures that have such obstructions, a detailed geometric assessment shall be made to compare the actual clearance with the AAR Equipment Diagram for Limited Interchange Service - Plate C (Figure 4) or Limited Interchange Service - Plate F (Figure 5) as applicable for the given line, to ensure sufficient clearance is available for operation without restriction. Special instructions shall be issued by ConnDOT to the operating railroads for structures that have insufficient clearance.

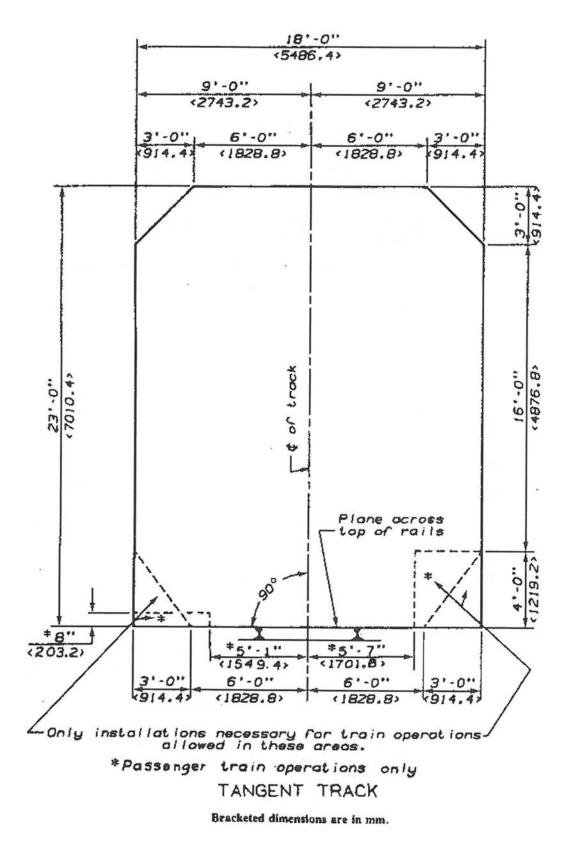
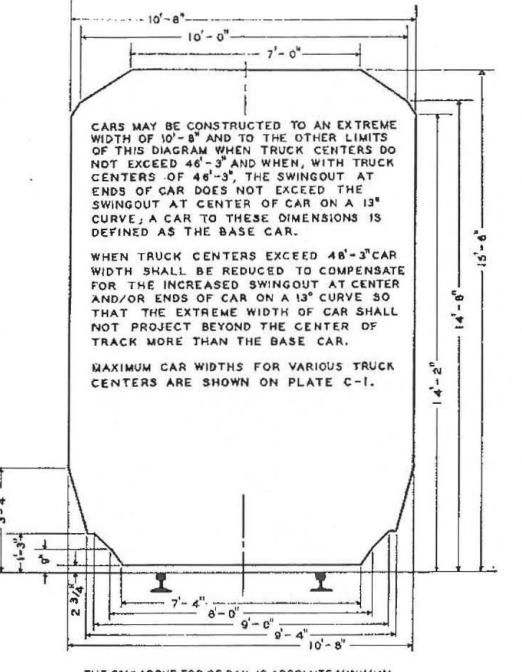


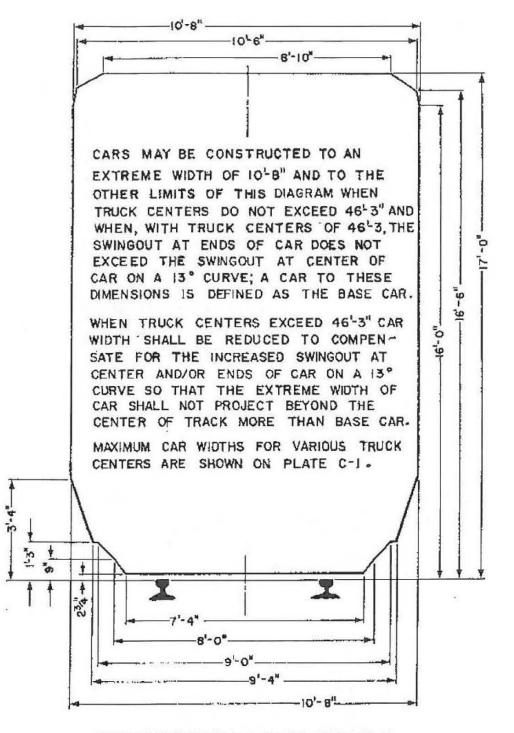
Figure 3. AREMA Figure 28-1-1 General Outline (September, 2011)



THE 23/4" ABOVE TOP OF RAIL IS ABSOLUTE MINIMUM UNDER ANY AND ALL CONDITIONS OF LADING, OPERA-TION, AND MAINTENANCE.

\*THIS DIAGRAM IS THE SAME AS PLATE C OF THE MECHANICAL DIVISION, AAR, AND IS INCLUDED IN THE AREMA MANUAL FOR CONVENIENT REFERENCE. FOR RESTRICTIONS APPLICABLE TO THIS DIAGRAM SEE "RAILWAY LINE" CLEARANCES.

Figure 4. AREMA Figure 28-2-3 Equipment Diagram for Limited Interchange Service - PLATE C (September, 2011)



#### THE 23/4" ABOVE TOP OF RAIL IS ABSOLUTE MINIMUM UNDER ANY AND ALL CONDITIONS OF LADING, OPERA-TION, AND MAINTENANCE.

"THIS DIAGRAM IS THE SAME AS PLATE F OF THE MECHANICAL DIVISION, AAR. AND IS INCLUDED IN A.R.E.A. MANUAL FOR CONVENIENT REFERENCE. FOR RESTRICTIONS APPLICABLE TO THIS DIAGRAM SEE "RAILWAY LINE CLEARANCES."

Figure 5. AREMA Figure 28-2-7 Equipment Diagram for Limited Interchange Service - PLATE F (September, 2011)

Railroad Bridge Inspectors and Railroad Bridge Supervisors are authorized by ConnDOT to authorize or restrict the operation of rail traffic over a bridge based on its immediate condition or state of repair in accordance with §237.53 and §237.55. When the Railroad Bridge Inspector or Railroad Bridge Supervisor is not an employee of the operating railroad, the instruction to restrict operation shall be made through the railroad employee present at the site. The Inspector shall also make immediate notification of such restriction to the supervising ConnDOT Project Manager for action.

See Appendix 10 for a List of Railroad Contact Persons.

#### 4.3 BRIDGE STRIKE - DAMAGE INSPECTIONS

Damage inspections are conducted immediately following any incident that may have an effect on the railroad tracks and/or the structural integrity of a bridge. The inspection is conducted by various Metro North personnel and depending on the seriousness of the incident by ConnDOT on-call Consultant.

The following process shall be followed after a bridge strike (See Appendix - 3):

- 1. The Railroad Traffic Control (RTC) will apply an immediate Speed Restriction and notify the Track and Structures Department to assess the collision damage. Metro North Personnel will perform the following steps:
  - Inspect the damage members to determine the extent and seriousness of the damage. Determine if the railroad track and bridge is safe to carry the railroad traffic, if emergency repairs are needed or if track closure is required.
  - Notify ConnDOT (Office of Rail Design Supervising Engineer) if there is a structural damage requiring the assessment of the On-call Consultant, who will prepare repair plans for the bridge.
- 2. Prepare a Bridge Inspection Damage Report (Use form in Appendix 7A)
  - Describe and document the damage with verbal description and photographs.

Record the name of the owner and the type of vehicle that damaged the bridge. Note the height of the vehicle. Check and record the direction the vehicle was traveling at the time of the accident. Request a copy of the police report or accident report.

- Document any actions, such as speed restrictions or track closures which were taken.
- Record any additional remarks concerning the damage.
- If possible, include a copy of the accident report with the Bridge Inspection Damage report.
- Submit the Bridge Inspection Damage report immediately after the initial inspection.
- **3.** The Bridge Inspection Damaged Reports, field notes, photographs and other pertinent documents will be filed in the Railroad Bridge file, located in the Office of Rail.

#### **CHAPTER 5: RAILROAD BRIDGE INSPECTION**

#### 5.1. BRIDGE INSPECTION

Railroad bridge inspection shall be performed in conformance with the procedures established in ConnDOT's Bridge Inspection Manual (BIM) until such time the Railroad Bridge Inspection Manual (RBIM) is available. AREMA's "Bridge Inspection Handbook" shall be used as a supplement to ConnDOT's inspection manual.

All railroad bridges, as defined by §237.5, on active railroad lines shall be inspected in conformance with the requirements of §237 Subpart E - Bridge Inspection. In addition, those railroad structures that meet ConnDOT's definition for a "Qualifying Railroad Structure" but are not specifically included in the federal definition of a railroad bridge shall be inspected following the same procedures used for federally defined railroad bridges with the following exception: the requirements related to scheduling are considered "suggested" but not "mandatory".

All railroad bridges on active railroad lines shall be inspected at least once each calendar year, with no more than 540 days between successive inspections as required by §237.101.a.

Railroad bridges on inactive or abandoned lines shall not be placed in active service unless an inspection has been performed within the previous 540 days and a Railroad Bridge Engineer has reviewed the inspection report and load rating and determined it is safe to resume service as required by §237.101.d.

Railroad bridges on inactive or abandoned railroad lines shall be inspected in conformance with the requirements of §237 Subpart E - Bridge Inspection with the following exception to §237.101 - Scheduling of Bridge Inspections: Bridge inspections for structures on inactive or abandoned railroad lines shall be inspected at least once every two years or as directed by ConnDOT policy.

For Railroad bridges that cross over public roads and are subject to the requirements of the National Bridge Inspection Standards (NBIS), bridges shall be inspected in conformance with both 23 CFR 650C and 49 CFR Parts 213 and 237.

The general bridge inspection procedures are addressed in detail in Chapter 5 of the BIM and includes detailed discussion on:

- Planning, Scheduling and Safety Considerations
- Types of Inspection
- Record Keeping and Documentation
- Each inspection must be conducted systematically so that all items are inspected with a minimum of duplication or lost motion.

Each inspection must be conducted systematically so that all items are inspected with a minimum of duplication or lost motion.

Some bridges have areas to be inspected that are extremely difficult to reach and may be inaccessible by snooper or bucket truck. These areas may be viewed with binoculars on inspections that are not in-depth. Note on the report that normal access was not possible and record the distance from which the area was viewed. On inspections, or if any defects are found or suspected, closer access should be gained by using rigging, scaffolding, specialized equipment or free-climbing. (See Chapter 6 of the BIM for detailed procedure for inspection of Decks, Superstructure, Substructures, Moveable Bridges, Arches and Culverts.)

#### 5.2. <u>TYPES OF INSPECTION</u>

Routine bridge safety inspections are regularly scheduled bridge safety inspections that are conducted every two years on all railroad bridges. The primary purpose of routine inspections is to identify any critical problems or deficiencies.

Verification inspections are regularly scheduled bridge safety inspections that are conducted every two years on all railroad bridges. These verification inspections are performed on alternating calendar years from the Routine Inspections to satisfy the 49 CFR §237 Subpart E requirement for annual inspection of railroad bridges in service. The primary purpose of Verification Inspection is to confirm the condition of the bridge components as identified in the previous Routine or In-depth Inspection Report. For components where a change in condition is noted, a revision shall be made to the previous Inspection Report. Changes may include further deterioration of a bridge component or may reflect an improved condition based on the performance of maintenance or repair work since the previous inspection. Verification Inspections are not intended to confirm the accuracy of inventory type data including clearance measurements, however if an obvious change has occurred in such data then the change shall be noted and included in the report.

In-Depth inspections, in compliance with current ConnDOT practice, should be conducted on all railroad bridges every 10 years. The biennial routine inspection is not normally conducted in the year that the in-depth inspection is made. An in-depth inspection consists of a "hands-on" examination of all exposed parts of a bridge to assess and record the physical condition of the bridge, to ascertain that the bridge is functioning as shown on the original plans and to ensure that the bridge is adequate to safely carry the intended loads.

Special inspections shall be performed for any railroad structure involved in an event which had the potential to compromise the integrity of the bridge, including but not limited to Damage, flood, fire, earthquake, derailment or vehicular or vessel impact as required by §237.105. ConnDOT shall direct such inspections at the time notification is made by the operating railroad or other that such an event has occurred.

Qualifying Railroad Structures subject to the flow of water shall be inspected for scour at least every 2 years in general conformance with Underwater Inspections as addressed in the BIM or RBIM, as applicable.

All Railroad Structure inspections shall be conducted under the direct supervision of a designated Railroad Bridge Inspector, with support from consulting engineering firms, as required by §237.107.

The Inspection Report for a Railroad Structure shall be submitted to ConnDOT within 30 calendar days of the completion of the inspection and shall be reviewed, revised when appropriate, distributed to the Railroad and filed as complete within 120 calendar days of the completion of the inspection as required by §237.109. An exception to the above schedule is allowed for complex structures including movable bridges where the amount of information and level of detail requires a greater period of time. In all cases, the goal is to issue final inspection reports in an expedient manner.

See Appendix 11 for the Bridge Inspection Schedule for Metro-North Railroad Bridges.

See Appendix 12 for the Bridge Inspection Schedule for Off-System Railroad Bridges.

#### 5.3. BRIDGE INSPECTION RECORDS

ConnDOT shall for each qualifying railroad structure, prepare a "Bridge inspection Report" that includes all information required by §237.109.

See Appendix 13 for Office of Rail Inspection Forms.

See Appendix 14 for an example of a Typical Bridge Inspection Report.

#### 5.4. <u>REVIEW OF BRIDGE INSPECTION REPORTS</u>

After the completion of the Bridge inspection report, the Railroad Bridge Engineer and the Railroad Bridge Supervisor shall review the report as required by §237.111.

From the review of the bridge inspection report the reviewers shall perform the following:

- 1. Determine if the inspection was performed according to schedule for the particular structure.
- 2. Determine if all of the specified procedures for the structure were followed.
- 3. Prescribe any modifications to the specified procedures of inspection schedule for the structure.
- 4. Evaluate whether any items on the report represent a present or potential safety hazard.
- 5. Determine the need for higher-level review.
- 6. Determine if the structure needs to be evaluated for potential repairs or modifications.

#### CHAPTER 6: RAILROAD BRIDGE REPAIR AND MODIFICATION

#### 6.1. BRIDGE DESIGN REPAIRS OR MODIFICATIONS

Repairs or modifications of Qualifying Railroad Structures on active railroad lines shall be designed by a Consultant Engineer, under supervision of a Railroad Bridge Engineer and a Railroad Bridge inspector as required by §237.131.

See Appendix 6 for the Organization Chart for Field Verification and Design of Repairs for Railroad Bridges.

Prior to design of repairs, a Consulting Engineering firm shall perform a field verification of prescheduled bridges for potential design of repairs.

Consulting Engineering Firms shall perform field verifications and visual assessments for the structures identified by the Railroad Bridge Engineer. Field work shall concentrate on elements with previously reported condition rating of 5 (fair) or less and suspected to be in need of repair. The result of the field verification will be the collection and documentation of site specific data, including quantities and dimensions that can be used in the development of repair plans as needed.

Repair plans and design calculations shall be prepared by the Consulting Engineering Firm. The repair plans and design calculations shall be reviewed by ConnDOT's Railroad Bridge Engineer. The repair plans shall then be provided to Metro-North Railroad (Track and Structural Department) or of the Operating Railroad for Off-System bridges for use in repairing the specific structure.

Repairs or modifications include the replacement of structural members; strengthening of deteriorated or otherwise deficient members; heat straightening of misaligned members; addition of dead load to structures; attachment of sign supports or other miscellaneous material to structures; and the temporary or permanent modification of the structure configuration resulting in a modified load path. In general, repairs or modifications result in a change in the capacity of individual members or alters the stress in a primary load-carrying component either temporarily or permanently.

The design of structural repairs or modifications shall specify whether any restrictions to the rail traffic operating over such structure are required during the actual repair or modification work. Any restrictions shall be clearly indicated on the repair or modification plans.

#### 6.2. SUPERVISION OF REPAIRS AND MODIFICATIONS

The work to repair or modify a railroad structure as designed in accordance with this section shall only be performed under the immediate supervision of a Railroad Bridge Supervisor as required by §237.133. The Railroad Bridge Supervisor may be supported by qualified staff from Consulting Engineering Firms for the performance of the work. The Railroad Bridge Supervisor shall ensure that any required operating restrictions are in effect while the work is performed.

Welding of any material to bridge structures shall only be done when such work has been designed by a Railroad Bridge Engineer and under the supervision of a Railroad Bridge Supervisor.

The Railroad Bridge Supervisor shall prepare or supervise the preparation of as-built plans representing actual repairs or modifications performed in accordance with this section. The asbuilt plans shall be transmitted to the Office of Rail upon their completion to be filed in the bridge file.

The S-Program is the capital improvement program established by ConnDOT that prioritizes and develops repair plans for bridges on the Metro-North System. This is an annual program that is funded by ConnDOT with the repair work performed by Metro-North Railroad. A similar program for off-system railroad bridges is planned, subject to the availability of funds.

See Appendix 15 for the S-Program Summary.

#### **CHAPTER 7: DOCUMENT MANAGEMENT**

#### 7.1. DOCUMENT MANAGEMENT

ConnDOT shall collect, store and maintain documentation pertinent to the management of all qualifying railroad structures on active, inactive and abandoned lines as required by §237.33.c. Documentation shall be filed in either digital or hard copy format but in either case shall be filed by railroad line, milepost and state bridge number to facilitate retrieval.

Documentation to be maintained in the Railroad Bridge Files shall include:

- Original Bridge Plans, Design Calculations, Shop Drawings and Working Drawings
- Repair and Rehabilitation Plans, Design Calculations and as-built plans
- Related correspondences
- Routine, Verification, Special, In-Depth, and Underwater Inspection Reports
- Load Rating Calculations or alternate rating documentation

Given the age of the railroad infrastructure in Connecticut and the history of owners, it is recognized that original plans and design calculations are not available for many of the structures in the inventory. For such structures, all available information will be maintained in the Railroad Bridge Files.

Bridge Inspection Reports shall be maintained in the Railroad Bridge Files for a minimum period of 2 years from the completion of the inspection as required by §237.109.f.

- In-Depth Inspection Reports shall be maintained until the completion and review of the next In-Depth Inspection Report
- Underwater Inspection Reports shall be maintained until the completion and review of the next Underwater Inspection Report

A key practice is the comparison between previous bridge inspection reports and the actual condition of a bridge. A comparison of successive reports can reveal any accelerating rates of deterioration or degradation of bridge components.

ConnDOT continually makes improvements in the area of secure digital file storage and retrieval systems and recognizes the critical importance of data management as a key element of its overall Railroad Bridge Management Program.

See Appendix 16 for List of Railroad Bridges and Microfilm Availability. See Appendix 17 for Plans and Microfilm availability for Metro-North Bridges.

See Appendix 18 for Map file Legend.

See Appendix 19 for Basement Stored Files by Project and other.

See Appendix 20 for Location of Bridge Plans (From Bridge Safety and Evaluation) in Basement Storage.

#### **CHAPTER 8: QUALITY CONTROL/QUALITY ASSURANCE**

#### 8.1. <u>GENERAL</u>

The effective and efficient management of ConnDOT owned railroad bridges requires dedication on the part of all the individuals who perform the many functions associated with the RBMP. It is critical to maintain the accuracy and consistency of all data used in the program including but not limited to inspection and load rating data.

The Connecticut Department of Transportation Office of Rail has instituted a Railroad Bridge Management Program (RBMP) to meet FRA 49 CFR Parts 213 and 237 requirements. Railroad Bridge inspectors collect inventory and condition information on each bridge for inclusion in the Department's Railroad Bridge Files. The accuracy and consistency of the inspection and documentation are vital to public safety, and also impact programming and funding appropriations. In recognition of the importance of this information, ConnDOT has established quality control and quality assurance procedures for all bridge inspection and load rating work performed for ConnDOT owned structures.

Quality Control and Quality Assurance measures for the inspection and rating of railroad structures are similar to the requirements for highway bridges and the general requirements for Quality Control and Assurance contained in Chapter 4 of the BIM are applicable for railroad structures. These general measures will be complemented with railroad specific quality control and quality assurance measures in the RBIM.

Quality Assurance measures are instituted to monitor the effectiveness and compliance of the overall RBMP program. Periodic audits of the RBMP shall be performed by ConnDOT staff to ensure compliance with the provisions of the program as required by §237.151 and §237.153.

To be effective, quality control/quality assurance procedures must be followed by all personnel and the procedures should be evaluated and updated regularly. The program shall be flexible and shall be updated routinely by memos and directives from the Transportation Principal Engineer of the Office of Rail.

Version: 1.0 - February, 2012

### APPENDIX 1 FEDERAL RAILROAD ADMINISTRATION BRIDGE SAFETY STANDARDS, 49 CFR PARTS 213 AND 237 JULY 15, 2010



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Thursday, July 15, 2010

## Part II

# Department of Transportation

Federal Railroad Administration

49 CFR Parts 213 and 237 Bridge Safety Standards; Final Rule

#### DEPARTMENT OF TRANSPORTATION

#### Federal Railroad Administration

#### 49 CFR Parts 213 and 237

[Docket No. FRA 2009–0014, Notice No. 2] RIN 2130–AC04

#### **Bridge Safety Standards**

**AGENCY:** Federal Railroad Administration (FRA), Department of Transportation (DOT). **ACTION:** Final rule.

**SUMMARY:** FRA is establishing Federal safety requirements for railroad bridges. This final rule requires track owners to implement bridge management programs, which include annual inspections of railroad bridges, and to audit the programs. This final rule also requires track owners to know the safe load capacity of bridges and to conduct special inspections if the weather or other conditions warrant such inspections.

**DATES:** This final rule is effective September 13, 2010.

#### FOR FURTHER INFORMATION CONTACT:

Gordon A. Davids, P.E., Chief Engineer—Structures, Office of Railroad Safety, FRA, 1200 New Jersey Avenue, SE., Washington, DC 20590 (telephone: (202) 493–6320); or Sarah Grimmer Yurasko, Trial Attorney, Office of Chief Counsel, FRA, 1200 New Jersey Avenue, SE., Washington, DC 20950 (telephone: (202) 493–6390).

#### SUPPLEMENTARY INFORMATION:

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#### Background

#### I. The Safety of Railroad Bridges

#### A. General

There are nearly 100,000 railroad bridges in the United States. These

bridges are owned by over 600 different entities. The bridges vary in length, load capacity, design, and construction material. Everything that is shipped or transported via rail likely travels across one or more railroad bridges. Thus, everything from intermodal goods, automobiles, grain, coal, hazardous materials, and passengers is transported on the nation's rail system and therefore across railroad bridges.

The structural integrity of bridges that carry railroad tracks is important to the safety of railroad employees and to the public. The responsibility for the safety of railroad bridges rests with the owner of the track carried by the bridge, together with any other party to whom that responsibility has been assigned by the track owner. The severity of a train accident is usually compounded when a bridge is involved, regardless of the cause of the accident.

Beginning in 1991, FRA conducted a review of the safety of railroad bridges. The review was prompted by the agency's perception that the bridge population was aging, traffic density and loads were increasing on many routes, and the consequences of a bridge failure could be catastrophic. During the past five decades, not one fatality has been caused by the structural failure of a railroad bridge. Train accidents caused by the structural failure of railroad bridges have been extremely rare.

Although the average construction date of railroad bridges predates most highway bridges by several decades, the older railroad bridges were designed to carry heavy steam locomotives. Design factors were generally conservative, and the bridges' functional designs permit repairs and reinforcements when necessary to maintain their viability. Railroad bridges are most often privately, rather than publicly, owned. Their owners seem to recognize the economic consequences of neglecting important maintenance. Private ownership enables the railroads to control the loads that operate over their bridges. Cars and locomotives exceeding the nominal capacity of a bridge are allowed on a bridge only with permission from the responsible bridge engineers, and then only under restrictions and conditions that protect the integrity of the bridge.

Many railroad bridges display superficial signs of deterioration but still retain the capacity to safely carry their loads. Corrosion on a bridge is not a safety issue unless a critical area sees significant loss of material. Routine inspections are prescribed to detect this condition, but determination of its effect requires a detailed inspection and analysis of the bridge. In general, timber bridges continue to function safely, and masonry structures built as early as the 1830s remain functional and safe for their traffic. Of the few train accidents that involved bridges, most have not been caused by structural failure. FRA accident records for the 27 years 1982 through 2008 show 58 train accidents that were caused by the structural failure of railroad bridges. These accidents resulted in nine reportable injuries and a reported \$26,555,878 in damages to railroad facilities, cars and locomotives.

#### B. Guidelines

On April 27, 1995, FRA issued an Interim Statement of Policy on the Safety of Railroad Bridges. Published in the Federal Register at 60 FR 20654, the interim statement included a request for comments to be submitted to FRA during a 60-day period following publication. On August 30, 2000, FRA published a Final Statement of Agency Policy on the Safety of Railroad Bridges ("policy statement"). See 65 FR 52667. With the policy, FRA established criteria for railroads to use to ensure the structural integrity of bridges that carry railroad tracks, which reflected minor changes following public comment on the interim statement. Unlike regulations under which FRA ordinarily issues violations and assesses civil penalties, the policy statement contained guidelines for the proper maintenance of bridge structures and is advisorv in nature.

On October 16, 2008, President Bush signed into law, the Railroad Safety Improvement Act of 2008, Public Law 110-432, Division A ("RSIA"). Section 417 of the RSIA directs FRA to issue regulations requiring railroad track owners to adopt and follow specific procedures to protect the safety of their bridges. Prior to the passage of the RSIA, FRA had already begun work on revising the policy statement. On January 13, 2009, FRA published an amendment to the policy statement by incorporating changes proposed by the Railroad Safety Advisory Committee ("RSAC") on September 10, 2008. RSAC developed a list of essential elements of railroad bridge management programs ("essential elements") which make up the bulk of the amendment. See 74 FR 157. All aspects of the policy statement that are not incorporated into the regulatory text of part 237 are now found in its appendix A.

#### C. Regulatory History

On August 17, 2009, FRA issued a Notice of Proposed Rulemaking (NPRM) as a first step in the agency's promulgation of bridge safety regulations as mandated by the RSIA. See 74 FR 41558. FRA received comments from eight parties, including two professional engineers, the Alaska Railroad Corporation, Maryland Department of Transportation ("Maryland DOT"), Iowa Department of Transportation ("Iowa DOT"), RailAmerica, the American Short Line and Regional Railroad Association (ASLRRA), and the Association of American Railroads (AAR). FRA will address the concerns raised by the comments in the text below.

This final rule is the culmination of FRA's efforts to develop and promulgate bridge safety standards. In the Sectionby-Section Analysis, below, FRA will discuss how the regulatory text addresses each portion of the RSIA.

### II. Railroad Safety Advisory Committee (RSAC) Overview

In March 1996, FRA established RSAC, which provides a forum for developing consensus recommendations to FRA's Administrator on rulemakings and other safety program issues. The RSAC includes representation from all of the industry's major stakeholders, including railroads, labor organizations, suppliers and manufacturers, and other interested parties. A list of RSAC members follows:

American Association of Private Railroad Car Owners (AARPCO);

American Association of State Highway & Transportation Officials (AASHTO);

- American Chemistry Council;
- American Petrochemical Institute;

American Public Transportation Association (APTA);

American Short Line and Regional Railroad Association (ASLRRA);

- American Train Dispatchers Association (ATDA);
- Association of American Railroads (AAR);
- Association of Railway Museums (ARM);
- Association of State Rail Safety Managers (ASRSM);
- Brotherhood of Locomotive Engineers and Trainmen (BLET);
- Brotherhood of Maintenance of Way Employees Division (BMWED);
- Brotherhood of Railroad Signalmen (BRS);

Chlorine Institute;

- Federal Transit Administration (FTA)\*; Fertilizer Institute;
- High Speed Ground Transportation Association (HSGTA);
- Institute of Makers of Explosives;
- International Association of Machinists and Aerospace Workers;
- International Brotherhood of Electrical Workers (IBEW);
- Labor Council for Latin American Advancement (LCLAA)\*;

- League of Railway Industry Women\*; National Association of Railroad
- Passengers (NARP);
- National Association of Railway Business Women\*;
- National Conference of Firemen & Oilers;
- National Railroad Construction and Maintenance Association;
- National Railroad Passenger Corporation (Amtrak);
- National Transportation Safety Board (NTSB)\*;
- Railway Supply Institute (RSI);
- Safe Travel America (STA);
- Secretaria de Comunicaciones y Transporte\*;
- Sheet Metal Workers International Association (SMWIA);
- Tourist Railway Association Inc.;
- Transport Canada\*;
- Transport Workers Union of America (TWU);
- Transportation Communications International Union/BRC (TCIU/BRC);
- Transportation Security Administration (TSA); and
- United Transportation Union (UTU).
- \*Indicates associate, non-voting membership.

When appropriate, FRA assigns a task to RSAC, and after consideration and debate, RSAC may accept or reject the task. If the task is accepted, RSAC establishes a working group that possesses the appropriate expertise and representation of interests to develop recommendations to FRA for action on the task. These recommendations are developed by consensus. A working group may establish one or more task forces to develop facts and options on a particular aspect of a given task. The task force then provides that information to the working group for consideration. If a working group comes to unanimous consensus on recommendations for action, the package is presented to the full RSAC for a vote. If the proposal is accepted by a simple majority of RSAC, the proposal is formally recommended to FRA. FRA then determines what action to take on the recommendation. Because FRA staff plays an active role at the working group level in discussing the issues and options and in drafting the language of the consensus proposal, FRA is often favorably inclined toward the RSAC recommendation.

However, FRA is in no way bound to follow the recommendation, and the agency exercises its independent judgment on whether the recommended rule achieves the agency's regulatory goal, is soundly supported, and is in accordance with policy and legal requirements. Often, FRA varies in some respects from the RSAC recommendation in developing the actual regulatory proposal or final rule. Any such variations would be noted and explained in the rulemaking document issued by FRA. If the working group or RSAC is unable to reach consensus on recommendations for action, FRA moves ahead to resolve the issue through traditional rulemaking proceedings.

#### III. RSAC Railroad Bridge Working Group

RSAC on February 20, 2008, agreed to accept the task of reviewing FRA's railroad bridge safety policies and activities, and to make appropriate recommendations to FRA to improve the bridge safety program. RSAC accordingly established a Railroad Bridge Working Group (Working Group), composed of representatives of the various organizations on the RSAC and including persons with particular expertise in railroad bridge safety and management. The Working Group met on April 24-25, 2008, June 12, 2008, and August 7, 2008. On September 10, 2008, the full RSAC voted on the Working Group's report, Essential **Elements of Railroad Bridge** Management Programs, and recommended that FRA incorporate it into FRA's Statement of Agency Policy on the Safety of Railroad Bridges. The Working Group met again on January 28-29, 2009, and February 23-24, 2009, to recommend rule text to address the RSIA's mandate to FRA in Section 417 to promulgate bridge safety regulations. The Working Group reached consensus on proposed regulatory text which made up most of the provisions of the NPRM.

After the NPRM comment period closed, the Working Group reconvened on December 15, 2009, to review the comments and offer additional advice on how FRA should proceed with the final rule. Due to time constraints, FRA elected to seek advice from the Working Group regarding the public comments and possible revisions to the NPRM rather than asking the group and the full RSAC to formally provide recommendations regarding the final rule.

#### **IV. Response to Public Comment**

As mentioned above, FRA received eight comments to the NPRM. Comments were submitted by a variety of affected parties, including individual professional engineers, the Alaska Railroad Corporation, RailAmerica, two state DOTs, the AAR and the ASLRRA. FRA reviewed the comments with the Working Group and FRA staff also extensively reviewed and evaluated the comments. In this section, FRA will respond to comments regarding the application of the bridge rule, the responsibility for compliance, definitions, adoption of bridge management programs, the definition of a railroad bridge engineer, the determination of bridge load capacities, bridge inspection records, and other general comments. FRA is also responding to some of the smaller concerns within the section-by-section analysis.

# Application

Mr. Wayne Duffet, P.E., commented that FRA proposed that this part apply to tourist railroads because the passengers on those railroads are entitled to the protection afforded by this rule. He observed that, as written, the rule applies to every bridge with a gauge of two feet or more, that handles trains, regardless of whether part of the general railroad system. The comment requests clarification on two points: whether the rule applies to a tourist railroad that is not part of the general railroad system, and whether the rule applies to a two-foot gage bridge within an amusement park.

FRA notes that a "tourist railroad" comes under the uniform FRA definition of the term "railroad" as found at 49 CFR 209.3 and within the meaning of the Federal railroad safety statutes as found at 49 U.S.C. 20102(1)(A). Tourist railroads move passengers by the use of track and equipment that, taken together, would commonly be described as a "railroad," and their operations pose a distinct risk to the safety of the public. "An installation which is not part of the general railroad system of transportation and over which trains are not run by a railroad" refers to tracks located within an industrial operation where rolling equipment is moved only by and for the account of that particular industry. If a railroad as defined in 49 CFR 209.3 operates over a bridge inside such an installation, then this regulation applies to that bridge and to the owner of track on that bridge.

Specifically as to tourist railroad operations, FRA exercises jurisdiction over tourist operations whether or not they are conducted on the "general railroad system of transportation" ("general system"), which is defined as "the network of standard gage track over which goods may be transported throughout the nation." Appendix A to 49 CFR part 209. The only exceptions where FRA typically does not exercise jurisdiction are for tourist operations on track gage that is less than 24 inches and tourist operations that are off of the

general system and are "insular." A tourist operation is considered "insular" if its operations are limited to a separate enclave in such a way that there is no reasonable expectation that the safety of any member of the public-except a business guest, a licensee of the tourist operation or an affiliated entity, or a trespasser-would be affected by the operation. Appendix A to 49 CFR part 209. FRA does, however, exercise limited jurisdiction over tourist railroads that do not operate on the general system, but that are non-insular. Specifically, FRA will consider a railroad to be non-insular if one or more of the following exist on its line: A public highway-rail crossing that is in use; an at-grade rail crossing that is in use; a bridge over a public road or waters used for commercial navigation; or a common corridor with another railroad. Appendix A to 49 CFR part 209. With respect to this rule, FRA is exercising jurisdiction over all tourist and excursion operations regardless of whether they are insular or not.

Maryland DOT requested an explanation of the definition of the "general railroad system of transportation" as it applies to urban rapid transit operations as set forth in the rule. FRA replies that § 237.1(b) is consistent with 49 U.S.C. 20102(1)(B) and 49 CFR 213.3(b)(2), which exempt "track used exclusively for rapid transit operations in urban areas that are not connected with the general system of transportation" from the application of that regulation. If an urban rapid transit system operates over the general system, FRA will exercise jurisdiction over the urban rapid transit operation to the extent that it is connected to the general system. In situations in which an urban rapid transit operation has a minor connection to the general system, i.e., at a highway-rail grade crossing, FRA will exercise limited jurisdiction over the urban rapid transit system and only to the extent necessary to ensure safety at the points of connection for that system, the general system, and the public.

# Responsibility for compliance

AAR noted that there are numerous tracks on railroad bridges that have been leased by their owners to other companies. The proposed bridge rule attempted to account for these historical leases by providing that where an owner of the track over the bridge has assigned responsibility for the track to another company and FRA has been notified pursuant to 49 CFR 213.5(c), additional notification under part 237 for the bridge is not needed. This is because part 237 places responsibility for the bridge with the person to whom

responsibility for the track has been assigned and recognized pursuant to part 213. AAR is concerned that there will be situations where notification pursuant to § 213.5(c) has not taken place, and argues that notification might not have taken place because the lease was entered into before § 213.5 was adopted. AAR explains that there might be other reasons notification did not take place or a railroad might simply be unable to determine whether notification occurred. If it cannot be established that notification did occur, AAR argues that the rule, literally interpreted, might not permit FRA to hold the lessee responsible for compliance even though, as a practical matter, the lessee controls the track and bridge and is performing all functions related to track and bridge safety. AAR suggests FRA address the issue of historical leases by adding regulatory text which states that FRA may hold a lessee of track to which this part applies responsible for compliance with this part where the lessee exercises control over the track.

This provision follows the use of the term "owner of track" in the Track Safety Standards at 49 CFR part 213. FRA believes that it would be confusing and inconsistent for FRA to define an "owner of track" differently in two different parts of the Rail Safety Standards. FRA advises an owner of track to resubmit a notification of assignment if the owner is uncertain whether an assignment has been made. However, assignment does not relieve a track owner of compliance with part 237, as § 237.3(c) states that FRA can always hold the track owner responsible for compliance with the bridge safety standards.

Maryland DOT noted that its state highway administration, and several counties in the state, own and inspect several railroad-carrying bridges. Unstated, but implicit in the comment, is that while the state highway administration owns the bridge, the track is owned by a third party. Maryland DOT states that in accordance with this section, however, the state highway administration would not be responsible for compliance with this rule, since the "track owner" is responsible. In addition, several counties own railroad-carrying bridges as well.

FRA replies that the rule does not alter the financial responsibility of a highway agency that owns, inspects and maintains railroad bridges. The rule does, however, hold the track owner responsible to assure that the inspections and maintenance are performed correctly by qualified and designated persons. The track owner would be permitted to accept work performed by a highway agency provided that it conforms to the requirements of this part. FRA also notes that instances have arisen in which state agencies have performed inspections and evaluations in which a state-owned railroad bridge was found to be seriously deficient, and where the operating railroad was never notified or advised of the problem. FRA accident records include at least one such instance in which the bridge failed under a train, resulting in a catastrophic train accident, an accident which occurred on the Southern Railroad of New Jersey on August 12, 1999. This provision is intended, partly, to prevent such a loss of vital communication among the concerned parties

Maryland DOT also questions whether the track owner could assign responsibility to someone else. If one of these railroads requests the state agency to be the responsible party for the FRA inspection, they would consider refusing the request because they would have to be in compliance with the whole program, which would require a railroad bridge engineer, railroad bridge inspectors and a railroad bridge management program.

FRA responds that, in any case of assignment of responsibility, the assignee must first accept the assignment before it can become effective. See § 237.3(b)(6). The final rule states that the track owner must send a written notification of assignment to FRA at least 30 days in advance of the assignment, and that the notification must include a statement signed by the assignee acknowledging the assignment. A notification that did not include an acknowledging statement would not comply with § 237.3(b)(6), and FRA would disregard the assignment.

# Definitions

FRA received three comments regarding the definition of a railroad bridge. The comments suggested that the definition of a railroad bridge is either not broad enough or too broad and that there is an inconsistency between the definition of a railroad bridge and the Federal Highway Administration's (FHWA) definition of a bridge. FRA intends the explanations in this response to clarify that the definition of a railroad bridge is consistent with long-held industry practice and is neither too broad nor too narrow.

One commenter suggested that the definition of a bridge be changed to "any structure with an open deck." FRA replies that the regulatory definition of a bridge includes open decks, ballast decks, and solid decks. Essentially, a bridge deck is the component of the bridge upon which the track is supported, and which is subject to bending stresses from trains moving over it.

Another comment requests an explanation of an apparent inconsistency between the definition of a railroad bridge in this rule, and the definition of a bridge used by the FHWA, which defines a bridge as a structure with a span length of 20 feet or more. FRA responds that railroad bridges differ greatly from highway bridges in many respects, particularly in regard to the nature of the heavy live load which they support. This definition represents the consensus of all parties in the Working Group and is consistent with long-held railroad industry practice.

A third commenter suggests that the railroad bridge definition is broad and potentially includes types of structures that are affected by track live loads that have not previously been managed as bridges. These structures may include waterfront structures such as piers and wharves, mechanical shop structures including drop tables and inspection pits, as well as scales, large culverts and potentially even various types of retaining walls that have under-grade structural layout features that could be interpreted to be span lengths of 10 feet or more.

FRA replies that piers and wharves, scales, and other structures that carry railroad track and meet the span definition of a bridge are included under this regulation. Retaining walls and other roadbed structures are not included, because they do not carry track on a span over a gap. Additionally, culverts with a span of 10 feet or greater are also subject to this regulation and must be included in track owner's bridge management program.

# Adoption of Bridge Management Programs

Three comments addressed concerns with the adoption of bridge management programs. Maryland DOT asked if the regulations "distinguish between Transit Railroads or short-lines, or rail traffic volume," and requested that FRA define Class I and II carriers and the general railroad system. ASLRRA remarks that some design documents for each bridge might be difficult, if not impossible, to obtain. ASLRRA proposes that all documentation required by the rule be completed no later than five years following the program's adoption. This would allow for the search and retrieval,

or replication, of required documentation over more realistic time frames, as well as the allocation of necessary expense over a longer, and possibly less impacting, period of time. The Alaska Railroad Corporation requests that the bridge management program adoption time be extended to the effective date of the final rule plus one year. The additional time is necessary for inventory and database development of all structures covered by the regulation, as seasonal climatic conditions will potentially make some of these structures on the Alaska Railroad inaccessible until early summer 2010.

With regard to the first concern, FRA replies that the Surface Transportation Board defines the class of railroad at 49 CFR part 1201, based on the carrier's annual operating revenue. This section specifies time periods for program adoption according to the type of railroad, not according to railroad traffic volume or load intensity. By "general railroad system of transportation," FRA refers to the network of standard gage track over which goods may be transported throughout the nation and passengers may travel between cities and within metropolitan and suburban areas. See appendix A to 49 CFR part 209.

Regarding the second comment, ASLRRA's proposal is consistent with the proposed rule. Pursuant to §237.33(c), the program, when adopted by a track owner, need only incorporate a provision to obtain and maintain the design documents of each bridge if available, and to document all repairs, modifications, and inspections of each bridge. There is no deadline for acquisition of these documents. FRA anticipates that the priorities for acquisition of archived bridge design documents would closely follow their usefulness in determining bridge capacities.

To address the Alaska Railroad Corporation's concerns, FRA replies that the bridge inventory need not be complete in all of its details at the time of adoption of a railroad's bridge management program. It is reasonable to expect that an adopted program would specify the format for recording the inventory information, or "bridge list," and that information readily available from existing records, such as valuation maps, could be used to initially populate the data base. After that, additions and refinements to that information would be generated by normal inspection work.

# Railroad Bridge Engineer

AAR noted in its comment that the NPRM reference to the "Accreditation Board for Engineering and Technology (ABET)" is obsolete in that the organization has changed its name to ABET, Inc. AAR further notes that ABET Inc. only accredits engineering education programs in the United States, but mutually recognizes programs accredited by corresponding organizations in other nations. The same commenter notes an ambiguity in the term "licensed scope of practice" as it applies to the professional practice of engineering.

FRA acknowledges the concern regarding ABET, Inc., and has changed the reference in the regulatory text to ABET, Inc., or its successor. FRA did not intend to exclude engineers who received their education in other nations from being recognized as railroad bridge engineers, and has amended the text to specify that, in order to fulfill the educational requirements of this section, a railroad bridge engineer can also have received a degree from a program accredited as a professional engineering curriculum by a foreign organization recognized by ABET, Inc. or its successor. FRA has clarified the ambiguity commented on in the language of the NPRM by stating that a railroad bridge engineer can also be considered to have fulfilled the educational requirements of this section if he or she is currently registered as a professional engineer. FRA notes that state law governing the professional practice of engineering requires that professional engineers limit the subject of their practice to areas in which they are competent.

RailAmerica commented that nothing in this section speaks to the competence of an engineer as a railroad bridge engineer. FRA replies that the determination of the competence of a railroad bridge engineer is left to the track owner. FRA does not intend to engage in qualifying individuals to perform those functions. That determination will have to be made by the track owner after review of the engineer's qualifications and experience in the light of the qualification requirements of this part. The employer or the client of an engineer has always had the prerogative and responsibility to determine the qualifications of that individual, and FRA does not propose to alter that relationship.

#### Determination of Bridge Load Capacities

One commenter remarked on the difficulty of assigning a precise capacity rating to a timber bridge owing to the

wide variations in the properties of timber material and the changes that occur to timber components over time. FRA recognizes that the evaluation of timber trestles is not an exact science. Although theoretical values of safe forces and stresses can be placed on individual timber components, the actual nature of wood varies widely, even within the same species. In addition, timber deteriorates over time and under repeated loads. Some timber bridge components are not easily inspected, especially where faces of the members are hidden by other adjacent or supported members. A load rating on a timber bridge must also account for time and for expected costs to maintain the bridge under its rated traffic. An engineer can raise the capacity of a timber trestle from 263.000- to 286.000pound cars, for instance, but the owner must be advised that increased maintenance costs will probably result, and that a more intensive inspection program must be instituted for that bridge, owing to the more rapid deterioration that will occur.

The same commenter also suggested that a revised rating not be required where an existing, valid rating provides a large margin of capacity above the loads that are actually operated. The rule text has been slightly modified to address that issue with a realistic solution. FRA has revised § 237.71(f) to state that a new bridge load capacity shall be determined, if, in the opinion of the railroad bridge engineer, a bridge inspection reveals that the condition of a bridge or a bridge component might adversely affect the ability of the bridge to carry the traffic being operated. This issue is also addressed further in the section-by-section analysis, below.

The same commenter also noted the difficulty of assigning a precise rating to many older concrete and masonry structures that are not well documented, and of which the internal configuration cannot be easily determined. FRA recognizes that many older concrete and masonry structures are not documented. Especially in the case of reinforced concrete, the configuration of reinforcing steel greatly affects the calculated capacity of the bridge. The analysis of brick and stone arches is possible, but the unknown variables can produce widely differing results. The practice to date in the railroad bridge engineering profession has been to observe these structures for any obvious signs of distress, and to rate them based on their condition at the time of inspection. FRA will accept the reasonable application of present methods for evaluating and managing these structures, because there is not a

history of sudden catastrophic failure, absent sudden damage from severe weather conditions or heavy water flows.

ASLRRA commented that "an individual trained as a bridge supervisor and inspector with many years of experience inspecting a bridge that itself has been in place for many years, is fully qualified to determine whether that bridge has the capacity to carry the loads for which it is rated. Under normal bridge inspection procedures, if the bridge shows signs of problems, a bridge inspector usually 'rates' a bridge each time he inspects it. If problems are encountered, additional steps will be taken to address the problem in accordance with these regulations. Rating an old masonry arch or bridge span may be difficult to do even for a railroad bridge engineer. While a number of bridges have been upgraded on many short lines and capacity rating calculations are available for those bridges, many more have not been upgraded and are performing well." FRA responds that there is a clear distinction between what some consider a "condition rating" ascribed to a bridge by an inspector, and a "capacity rating" which is determined by a qualified engineer. The term "rating" in the context of this rule refers only to a "capacity rating." This rule does not address a "condition rating" to be applied to a bridge.

A bridge inspector or supervisor who is not an engineer can certainly determine by observation and measurement whether the condition and configuration of a bridge corresponds with its state when it was rated by an engineer for capacity. However, if the bridge displays a condition or deterioration that materially affects its capacity, as by increasing the stress intensity in one or more components of the bridge, accurate determination of the revised capacity requires the experience, education and training of a competent railroad bridge engineer. In the same manner, the determination of the capacity of an existing bridge requires that the engineer should consider all available information related to the configuration and condition of the bridge, including all available design and modification documents and current reports of inspections. These determinations of bridge capacity ratings are usually performed in an office environment, and only seldom in the field.

RailAmerica commented that the rule would require bridge ratings to be completed within 5 years of the adoption of a Bridge Management System. This provision would penalize those railroads which have adopted a bridge management program before the final date required in the rule. FRA agrees with this comment. The rule has been modified so that the determinations of load capacity are required within five years of the required date for adoption of the bridge management program, rather than the actual date of adoption if earlier than required.

# Bridge Inspection Records

Several commenters suggested that the interim bridge inspection report be deleted from the rule, or that the time period for its submission be extended. Several also suggested that the time period for submission of the complete inspection report be extended. FRA understands that the regulated community is reluctant to see the imposition of record-keeping requirements that might not correspond with their current practices. However, bridge inspections performed by or for the track owner are a critical function which must be monitored in the enforcement process. Since FRA cannot be present on-site at each bridge inspection, the agency must see a record that shows that the inspection was performed, when and by whom it was performed, and the conditions found in the inspection. If there were no time requirements for recording inspections, it would be impossible for FRA to effectively monitor this vital function.

FRA views the interim report as a management tool in the bridge program audit to show whether bridge inspections are being performed at or near their scheduled frequency, with ample time to permit adjustments as necessary in the inspection program. Most railroad bridge inspection programs at present do not incorporate an interim inspection report. The time between an inspection and the filing of the inspection report is found to vary. An effective bridge management program requires that the person in charge of the program have reasonably current information on the progress of the vital function of bridge inspection. The proposed time frame of 14 days has been extended to 30 days in the final rule because FRA now believes that the 30-day time period is sufficient for effective management by the railroad and effective compliance monitoring by FRA.

Two commenters requested that the time period for submission of the complete inspection report be extended from 45 to 90 days, and one commenter requested 120 days. FRA understands the circumstances in which a consultant is engaged to conduct detailed bridge inspections and evaluations. Some of those evaluations include a considerable amount of engineering work that is performed in an office rather than in the field, and several months are often used in preparing the complete report. The extension of the time period for filing the report is intended to allow the most efficient use of inspection and engineering resources, while still providing effective input for management by the bridge owner and monitoring by FRA.

In light of the reasons given, and discussion at the RSAC Railroad Bridge Working Group, FRA finds that a 120day period for submission of the complete report would be reasonable and effective.

Two commenters noted that the proposed requirement to retain inspection reports until the completion of the next two following inspections of the same type would be burdensome and ineffective in the case of certain special inspections. For instance, if a highway vehicle strike occasions a special inspection, it would have been necessary to retain the records of the special inspection until the bridge had twice again been struck by a highway vehicle and inspected. This is not realistic, so the final rule simply requires that records of inspections be retained for two years following completion of the inspection, and that records of underwater inspections be retained until the completion and review of the next underwater inspection of the same components of the bridge.

Additionally, the final rule also accommodates instances in which a bridge inspection does not encompass the entire bridge. It also includes a clarification that when a complete report is filed before an interim report is due, the interim report is not required.

#### Other Comments

FRA received a number of comments that did not pertain to specific sections of the rule text. FRA will address these concerns below.

Maryland DOT suggested that FRA consider whether it would be beneficial to have the same inspection frequency criteria for all rail and transit lines or whether it is relevant to distinguish between Class I railroads, short lines, and transit lines, or to factor in rail traffic volume in general. Maryland DOT also states that it already has a detailed structural inspection program and database. It recommends that the new regulations not require replacement of existing agency programs, reporting forms, etc., to be in accordance with a national standard. Additionally, Maryland DOT asks whether FRA will compensate state agencies for the cost of overhauling their structural inspection program and database, and for the additional expense of conducting annual rather than biennial inspections. Finally, Maryland DOT asked if any regulations are proposed for tunnel, station or other miscellaneous structural inspections.

With respect to the first question, FRA has not distinguished among railroads of different sizes because the size of the railroad is in no way related to the physical attributes of a bridge and the loads that it carries. As noted above, this rule does not affect transit lines. The only criterion related to inspection frequency in this rule is a minimum of one inspection per year. As this provision is found in the RSIA, FRA has no option in this regard. See Section 417(b)(5), Public Law 110-432, 122 Stat. 4890 (49 U.S.C. 20103, note). With regard to the second concern, the rule does not require replacement of existing programs as long as they comply with the requirements of the rule. In response to the third concern, FRA is not aware of any Congressional appropriation of funds to provide assistance in order for regulated entities to comply with bridge safety regulations and thus FRA will not be providing any funding for that purpose. Finally, tunnels, stations, and other structures were not addressed in the proposed rule and thus are not addressed in this final rule.

Iowa DOT commented on the various types of ownership and maintenance agreements in place between highway agencies and railroads that cross those highways on bridges. Iowa DOT stated that "it would be more logical and provide a more consistent bridge safety program if the responsibility for inspection, load capacity ratings, and other aspects of the bridge safety program were fully retained by the track owner and not by the party that is financially responsible for maintenance. Where no agreement exists there can be a conflict over the responsibilities, therefore having the track owner fully responsible for the bridge safety program aspects would prevent any bridge from 'falling through the cracks' due to that conflict." Iowa DOT would like to see the final rule assign track owners the full responsibility for the bridge safety program, regardless of who is financially responsible for the structure's maintenance. Finally, the comment also states that, although the agency's bridge inspectors are fully qualified to inspect railroad bridges, determine load capacities, etc., they would not have the experience or

knowledge to translate the load capacities into railroad operational terms as required by the rule.

FRA notes that the final rule holds the track owner responsible for compliance, which is consistent with the commenter's request. The regulation does not address the question of financial responsibility or apportionment of expenses for bridge management or maintenance. That issue would continue to be governed by the terms of any agreements between the track owner and bridge owner. The rule does not assign or apportion financial or functional responsibility for inspection or maintenance of railroad bridges. The rule simply holds the track owner responsible for the adequate and safe support of its track on bridges. FRA does not specify who will perform those functions, so long as they are performed correctly by qualified individuals designated by the track owner. That designated individual may accept work performed by others, such as a state agency, if it is acceptable to them and can be adequately verified.

Regarding the last concern, bridge inspectors do not normally calculate the load capacities of a railroad bridge, unless they also happen to be competent railroad bridge engineers. Moreover, an engineer who cannot translate load capacities into railroad operational terms is not qualified to prescribe the loadings for a railroad bridge. The rule places the responsibility upon the track owner to have this done by a designated, competent railroad bridge engineer.

#### V. Section-by-Section Analysis

# Amendment to 49 CFR Part 213, Track Safety Standards

Appendix C to Part 213—Statement of Agency Policy on the Safety of Railroad Bridges

FRA is removing appendix C to part 213, which is FRA's Statement of Agency Policy on the Safety of Railroad Bridges ("policy statement"). As many portions of the text in the policy statement are covered in part 237, it would be redundant and confusing to leave them in the policy statement as currently published in part 213. With regard to the portions of the policy statement that are advisory in nature, FRA is publishing them in a new appendix to part 237, which will be discussed further below.

# Addition of 49 CFR Part 237, Bridge Safety Standards

#### Subpart A—General

This part prescribes minimum safety requirements for the management of railroad bridges that support one or more tracks. Track owners may adopt more stringent standards as long as they are in accordance with this part. FRA notes that it expressed these statements in proposed § 237.1, Scope of part, in the NPRM. See 74 FR 41560, 41573. FRA does not believe it necessary to include these explanatory statements directly in a section of the rule text, however, and is retaining them here instead.

Separately, FRA has removed proposed § 237.3, Preemptive effect. See 74 FR 41573. One commenter questioned whether the provisions in the proposed section were necessary, and whether they were inconsistent with other regulations. This section has been removed; discussion of the federalism implications of the rulemaking is found under Regulatory Impact and Notices, below. The sections in subpart A have been renumbered, accordingly.

# Section 237.1 Application

This rule applies to all owners of track carried on railroad bridges with certain exceptions as outlined or explained in following subsections. As delineated in FRA's Statement of Agency Policy Concerning Enforcement of the Federal Railroad Safety Laws at appendix A of 49 CFR part 209, FRA exercises jurisdiction over tourist. scenic, and excursion railroad operations whether or not they are conducted on the general railroad system. This part applies to both insular and non-insular tourist railroads because the passengers on those railroads are entitled to the protection afforded by this rule. As a matter of policy, FRA does not consider devices that run on rails in amusement parks to be railroads.

Paragraph (b). This part does not apply to bridges on track used exclusively for rapid transit operations in urban areas that are not connected with the general system of transportation. This is in accordance with 49 U.S.C. 20103 and appendix A of 49 CFR part 209.

Paragraph (c). This part does not apply to bridges located in an installation which is not a part of the general railroad system of transportation and over which trains are not operated by a railroad. Section 237.3 Responsibility for Compliance

The responsibility for the safety of trains on any track lies with the owner of that track. Therefore, the track owner is responsible for complying with the bridge safety standards promulgated in this part. If a bridge carries tracks owned by two or more owners, then the track owners can choose to make an assignment of responsibility for compliance with this part. The assignment process, delineated in paragraphs (b) through (d) of this section, is similar to the assignment process detailed in 49 CFR 213.5. However, FRA will hold the track owner or the assignee, or both, responsible for compliance with this part and subject to penalties under § 237.7. FRA intends that the responsibility for compliance with this part will follow, as closely as practicable, the responsibility for compliance with the Federal Track Safety Standards, and that where such responsibility is already established, it would not be necessary for the track owner to file an additional assignment of responsibility. As in part 213, FRA intends that "person" means an entity of any type covered under 1 U.S.C. 1, including but not limited to the following: A railroad; a manager, supervisor, official, or other employee or agent of a railroad; any owner, manufacturer, lessor, or lessee of railroad equipment, track or facilities; any independent contractor providing goods or services to a railroad; any employee of such owner, manufacturer, lessor, lessee, or independent contractor; and anyone held by FRA to be responsible for compliance with this part.

Paragraph (d). As described in 49 CFR part 213, a common carrier by railroad which is directed by the Surface Transportation Board to provide service over the track of another railroad under 49 U.S.C. 11123 is considered the owner of that track for the purposes of the application of this part during the period the directed service order remains in effect. On rare occasions, such as a cessation of service by a railroad, the Surface Transportation Board has directed a railroad other than the track owner to provide service. In such cases, the designated operator shall be considered the owner for purposes of compliance with the bridge safety regulations.

Paragraph (e). This paragraph requires any person, including a state agency, who performs a function on a railroad bridge that is required by this part to perform that function in accordance with this part. Instances have occurred in which state agencies have performed bridge inspections and evaluations in which the bridge was found to be seriously deficient, and where the operating railroad was never notified or advised of the problem. FRA accident records include at least one such instance in which the bridge failed under a train, resulting in a catastrophic train accident. Section 237.109 requires that the track owner keep the bridge inspection reports, and must therefore obtain them from a state agency or any other party that performs bridge inspections in conformance with the requirements of these regulations. This provision will prevent a loss of vital communication among concerned parties.

Paragraph (f). Where an owner of track to which this part applies has previously assigned responsibility for a segment of track to another person as prescribed in 49 CFR 213.5(c), additional notification to FRA is not required.

Paragraph (g). This paragraph provides that FRA reserves the right to reject an assignment of responsibility under § 237.3(b) for cause shown. As stated in paragraph (c) of this section, FRA may hold the track owner or the assignee, or both, responsible for compliance with this part and subject to penalties under § 237.7. Consequently, if FRA rejects an assignment of responsibility, FRA will not consider the rejected assignee responsible for compliance with part 237 pursuant to paragraph (c) of this section.

#### Section 237.5 Definitions

The definitions in this section are only intended to apply to this part, and not to alter the same terminology wherever used outside this part for other purposes.

Bridge modification and bridge repair. "Bridge modification" means a change to the configuration of a railroad bridge that affects the load capacity of the bridge, while "bridge repair" means remediation of damage or deterioration which has affected the structural integrity of a railroad bridge. This part requires that modifications and repairs to bridges be designed by railroad bridge engineers, and the work supervised by designated railroad bridge supervisors. This definition clarifies that minor modifications and repairs, such as replacing a wire rope handrail with one made of pipe, or painting a bridge, do not need to be designed and supervised pursuant to this part. However, this does not exempt the track owner from properly supervising the personal safety of the individuals performing the work

because that issue is addressed in other rules.

*Railroad bridge.* A "railroad bridge" is any structure which spans an opening under the track except for a small culvert, pipe, or other such structure that is located so far below the track that it only carries dead load from soil pressure and is not subjected to measurable bending, tension or compression stresses from passing trains. Unloading pits, track scales, and waterfront structures such as piers and wharves that fall within the definition of a "railroad bridge" are considered bridges for purposes of this part.

FRA does not intend to relieve a railroad from taking any action necessary to protect the safety of trains in the case of any structure, including small culverts, retaining walls, tunnels or overhead structures by providing for their inspection and maintenance, but it exempts them from the specific requirements of this regulation. A structure in a locomotive or car maintenance facility which is used to support cars or locomotives for maintenance is not included in the specific requirements of this regulation.

#### Section 237.7 Penalties

This provision conforms to provisions of the enabling legislation and stated agency policy. Consistent with FRA's Statement of Agency Policy Concerning Enforcement of the Federal Railroad Safety Laws, a penalty may be assessed against an individual only for a willful violation. The Administrator reserves the right to assess a penalty of up to \$100,000 for any violation where circumstances warrant. See 49 CFR part 209, appendix A.

### Section 237.9 Waivers

This section provides that each petition for a waiver under this part shall be filed in the manner and contain the information required by 49 CFR part 211, which prescribes rules of practice that apply to waiver proceedings. The processing of petitions for waiver of safety rules is found at subpart C to part 211.

# Subpart B—Railroad Bridge Safety Assurance

This subpart prescribes minimum requirements for persons responsible for railroad bridges to implement programs to assure the structural integrity of those bridges and to protect the safe operation of trains over those bridges. The responsibility for the safety of a railroad bridge rests with the owner of the track supported by that bridge, who relies upon the work of the engineer who makes the critical decisions regarding the management and use of that bridge.

Section 237.31 Adoption of Bridge Management Programs

Congress mandated that FRA "promulgate a regulation requiring owners of track carried on one or more railroad bridges to adopt a bridge safety management program to prevent the deterioration of railroad bridges and reduce the risk of human casualties, environmental damage, and disruption to the Nation's railroad transportation system that would result from a catastrophic bridge failure." Section 417(a), Public Law 110–432, 122 Stat. 4890 (49 U.S.C. 20103, note). This section requires track owners to adopt a bridge safety management program that prevents the deterioration of railroad bridges by preserving their capability to safely carry the traffic to be operated over them. Class I carriers and owners of track segments which are part of the general railroad system of transportation and which carry more than ten scheduled passengers trains per week shall implement their bridge safety programs no later than March 14, 2011. Class II carriers which carry ten or fewer scheduled passenger trains per week shall implement their bridge safety programs no later than September 13, 2011. All other track owners subject to this part shall implement their bridge safety programs no later than September 13, 2012.

FRA considers this implementation schedule to be realistic and effective, with priorities given to railroads with the highest levels of freight or passenger traffic. The implementation dates apply to the track owner, not to specific track segments. However, it is reasonable to consider that the specific provisions of each program will be implemented in a manner that accords higher priority to individual track segments with high volumes of freight or passenger traffic.

# Section 237.33 Content of Bridge Management Programs

Certain elements of a bridge management program are essential to its effectiveness. Those elements are enumerated in this section. Track owners and individuals responsible for the safety of railroad bridges are encouraged to adapt these elements to the needs of their areas of responsibility, and to adopt additional elements not inconsistent with the requirements of this part.

Paragraph (a). Congress mandated that the new regulations require each track owner to "develop and maintain an accurate inventory of its railroad bridges, which shall identify the location of each bridge, its configuration, type of construction, number of spans, span lengths, and all other information necessary to provide for the safe management of the bridges." Section 417(b)(1), Public Law 110–432, 122 Stat. 4890 (49 U.S.C. 20103, note). This paragraph requires that such an inventory be maintained. An accurate inventory of any property to be managed is essential so that the responsible individuals may schedule and track inspection, maintenance, and repair of the property units.

Paragraph (b). Congress mandated that the new regulations require that the track owner "maintain, and update as appropriate, a record of the safe capacity of each bridge which carries its track and, if available, maintain the original design documents of each bridge and a documentation of all repairs, modifications, and inspections of the bridge." Section 417(b)(3), Public Law 110-432, 122 Stat. 4890 (49 U.S.C. 20103, note). This paragraph requires that a record of the safe load capacity of each bridge be established. The operation of excessively heavy loads over a bridge will seriously shorten a bridge's useful life and will reduce or even eliminate the margin of safety between structural integrity and catastrophic failure. It is essential that the track owner should know that the loads permitted to be operated on a bridge are within the safe limits of the bridge.

Paragraph (c). The track owner must obtain and maintain the design documents of each bridge, if available, and document all repairs, modifications, and inspections of each bridge. The determination of safe load capacity requires knowledge of the configuration of the bridge and the materials of which it is constructed. Although the configuration may be determined by actual measurements of all of the components, that procedure can be tedious and expensive. Good documentation of the design and history of a bridge will facilitate more rapid and accurate determination of bridge capacity when such calculations are needed, as well as determination of the maintenance and service history of a bridge to detect and correct possible deterioration of its components. If the design documents for a bridge cannot be located, the track owner must measure and document the configuration of the bridge in sufficient detail to enable an accurate determination of the safe capacity of the bridge.

Paragraph (d). Bridge inspection is absolutely essential to an effective bridge management program. In this paragraph, FRA requires that the track

owner's bridge management program contain a bridge inspection program. Items (1) through (6) should be addressed in the program to a degree that promotes effective and efficient conduct of the inspection program. With regard to item (1), bridge inspection can present certain risks that are inherent in working at heights and around moving vehicles. A bridge inspection program should at least address the unique hazards associated with the process. With regard to item (2), a bridge inspection program should incorporate standards for the procedures and required details of any different types of inspection that are referenced in the program, such as annual inspections, post-event inspections, rating inspections, and intermediate periodic inspections. A large railroad might find it convenient to describe the standard procedures for various types of inspections in some detail, while a small railroad that normally conducts only annual inspections might describe only that procedure as well as postevent special inspections, and then issue instructions of particular applicability for other types of inspections that occur only infrequently. With regard to items (3) through (6), use of a standard method of describing the condition of components promotes effective and efficient communication between the inspector and those persons who review and evaluate a bridge using information from the inspection.

Subpart C—Qualifications and Designations of Responsible Persons

In subpart C, FRA establishes minimum standards for incorporation in railroad bridge management programs for qualification and designations of persons who perform safety critical functions that affect the integrity and safety of railroad bridges. Many aspects of railroad bridge work differ from other fields of engineering, inspection and maintenance. It is essential that the individuals who are responsible for these safety-critical functions be qualified by education, training and experience to perform them correctly.

# Section 237.51 Railroad Bridge Engineers

This section sets forth the minimum standards that a railroad bridge engineer must meet. Congress directed FRA to "ensure that an engineer who is competent in the field of railroad bridge engineering" is responsible for the development of all inspection procedures, reviews all inspection reports, and determines whether bridges are being inspected according to the

applicable procedures and frequency, and reviews any items noted by an inspector as exceptions. See Section 417(b)(7) of the RSIA. Railroad bridge engineering is based on the same principles of engineering as all other structural engineering work, but the application of many of those principles is unique to this particular field. The live loads carried on railroad bridges are generally much higher than the loads on highway bridges or other transportation structures. Overall configuration and details of construction of railroad bridges differ greatly from other classes of structures, to the extent that dealing with these features requires some experience with them as well as an understanding of the fundamentals of engineering.

FRA understands that not all railroad bridge engineers will be faced with all aspects of railroad bridge engineering. For example, an engineer engaged to prescribe safe loads for short steel spans and timber trestles on a particular railroad might never have to perform a detailed analysis of a large truss bridge. The basic premise is that the engineer be competent to perform the functions that are encompassed by that individual's employment. The determination of qualifications by the track owner includes employment of the engineer by the track owner, and designation of the engineer to exercise the authority called for in this part. By employment, FRA includes both engineers who are employees of the track owner as well as those engaged under a consulting contract.

A railroad bridge engineer must also have either: (1) A degree in engineering granted by a school of engineering with at least one program accredited by ABET, Inc. or its successor organization, as a professional engineering curriculum, or a degree from a program accredited as a professional engineering curriculum by a foreign organization recognized by ABET, Inc. or its successor; or (2) current registration as a professional engineer.

FRA believes that the critical nature of railroad bridge engineering work called for in this rule requires persons to meet a minimal educational or experience standard which is common to the engineering profession and which is necessary for an individual who will perform the functions of an engineer as called for in this rule.

In paragraph (c), FRA states that nothing in this part affects the States' authority to regulate the professional practice of engineering. This section represents a minimum standard to be attained by engineers who perform the functions called for in this regulation. Recognition by FRA as a railroad bridge engineer would not enable a person to provide professional engineering services in violation of a state law or regulation. FRA does not intend to preempt or interfere with any state laws regarding the professional practice of engineering. For example, a person registered as a professional engineer in Maryland could not work as a professional engineer in Virginia under this regulation in violation of Virginia law if such work violated Virginia law regarding the practice of engineering.

Section 237.53 Railroad Bridge Inspectors

In this section, FRA establishes the minimum standards that a railroad bridge inspector must meet. Effective inspection of bridges is essential to preserving their integrity and serviceability. Inspectors must be able to understand and carry out the inspection procedures, including accessing inspection points on a bridge, measuring components and any changes, describing conditions found in a standard, unambiguous manner, and detecting the development of conditions that are critical to the safety of the bridge. It is essential that an inspector who detects a potential hazard to the safe operation of trains be authorized by the track owner to place appropriate restrictions on the operation of railroad traffic, pending review as necessarv by a railroad bridge engineer. An individual who is not competent in railroad bridge work cannot overrule a determination made by a designated bridge inspector, supervisor, or engineer.

# Section 237.55 Railroad Bridge Supervisors

In this section, FRA establishes minimum standards that a railroad bridge supervisor must meet. Individuals who supervise and take responsibility for construction, repair and modification of railroad bridges must be competent to ensure that the work is performed in accordance with valid standards and any specific specifications, plans and instructions applicable to the work to be performed. This provision applies to any such individual, regardless of job title, who directly oversees such work and approves or restricts the movement of railroad traffic during the progress of the work.

# Section 237.57 Designations of Individuals

In the RSIA, Congress mandated that the bridge regulations designate qualified bridge inspectors or maintenance personnel to authorize the operation of trains on bridges following repairs, damage, or indications of potential structural problems. See Section 417(b)(8), Public Law 110-432, 122 Stat 4890 (49 U.S.C. 20103, note). In this section, FRA requires that each track owner designate certain individuals as qualified railroad bridge engineers, inspectors, and supervisors, and provide a recorded basis for each designation in effect. The track owner must record designations of individuals, whether employees, consultants or contractors. If a consultant or contractor has several individuals performing the described functions then one or more individuals should be designated as being responsible to the track owner for the work performed under that engagement, with the others working under the responsible charge of that individual.

# Subpart D-Capacity of Bridges

In subpart D, FRA prescribes minimum standards to be incorporated in railroad bridge management programs to prevent the operation of equipment that could damage a bridge by exceeding safe stress levels in bridge components or by extending beyond the horizontal or vertical clearance limits of the bridge. Protection of bridges and bridge components from overstress is essential to the continued integrity and serviceability of the bridge. It is also essential that equipment or loads that exceed the clearance limits of a bridge not be operated owing to the potential for severe damage to the bridge.

Section 237.71 Determination of Bridge Load Capacities

Paragraph (a). Each track owner must determine the load capacity of each of its railroad bridges. It is essential that the track owner know that loads operated over a bridge do not exceed the safe capacity of that bridge. However, once it is determined that a bridge has adequate capacity to carry the loads being operated, the regulation does not require that the track owner precisely calculate the additional capacity of that bridge, although that could be useful from a planning or economic standpoint.

Paragraph (b). This paragraph requires that the load capacity of each bridge be documented in the track owner's bridge management program, together with the method by which the capacity was determined. Once the load capacity is determined, the value must be recorded in order for it to be useful. Examples of methods of determination could be the original design documents, recalculation, or rating inspection. Paragraph (c). In the RSIA, Congress mandated that a professional engineer competent in the field of railroad bridge engineering, or a qualified person under the supervision of the track owner, determine bridge capacity. See Section 417(b)(2), Public Law 110–432, 122 Stat. 4890 (49 U.S.C. 20103, note). Load capacity determination in most instances requires the education, experience and training of an engineer who is familiar with railroad bridges and the standard practices that are unique to that class of structure.

The present standard references for railroad bridge design and analysis are found in the "Manual for Railway Engineering" of the American Railway Engineering and Maintenance-of-Way Association (AREMA). The chapters in this Manual dealing with Timber, Concrete and Steel structures, and Seismic Design, are under continuous review by committees consisting of leading engineers in the railroad bridge profession, including representatives of FRA. Although bridges exist that were designed using different or earlier references, they can still be evaluated by use of the AREMA Manual.

Paragraph (d). This paragraph permits bridge load capacity to be determined from existing design and modification records of a bridge, provided that the bridge substantially conforms to its records configuration. Determination of bridge load capacity requires information on the configuration of the bridge and the dimensions and material of its component parts. If the bridge is found to conform to the drawings of its original design and modifications, those drawings may serve as the basis for any rating calculation that might be performed, thus simplifying the process. Lacking that prior information, it is necessary that the configuration, dimensions, condition and properties of the bridge and its components be determined by on-site measurement of the bridge as it currently exists.

FRA recognizes that a rigorous, exact method of rating is not practicable with several types of bridges, including some massive concrete or masonry bridges and many timber trestles. The railroad bridge engineer will necessarily use judgment in determining the loads which should be permitted to operate over these bridges, and assuring that adequate inspections are performed so that any developing deterioration or signs of overload are detected before they progress to become a serious problem.

Paragraph (e). In this paragraph, FRA requires a track owner to schedule the evaluation of bridges for which the load capacity has not already been determined. This section provides for a phase-in period for determination of bridge capacities. There is probably not sufficient engineering expertise available in the United States for immediate rating of all unrated railroad bridges. This will provide a reasonable time period for track owners to accomplish this work. It is intended that the unrated bridges be given relative priority for rating, based on the judgment of a railroad bridge engineer. This prioritization can be accomplished either by observation or by evaluation of certain critical members of a bridge, as determined by the engineer using professional judgment.

Paragraph (f). A new capacity must be determined by a railroad bridge engineer when a bridge inspection record reveals that the condition of a bridge or a bridge component might affect the load capacity of the bridge. Accurate determination of current bridge capacity depends on accurate information about the current configuration and condition of the bridge. The railroad bridge engineer might determine that a change in condition or configuration calls for a revised rating calculation.

Paragraph (g). In this paragraph, FRA states that bridge load capacity may be expressed in terms of numerical values related to a standard system of bridge loads, but shall in any case be stated in terms of weight and length of individual or combined cars and locomotives, for the use of transportation personnel. Engineers use standard definitions of loading combinations for design and rating of bridges. Common among these standard definitions is a series of proportional loads known as the Cooper System. The capacity of a bridge and its components can be described in terms of a Cooper Rating, and the effect of rail equipment on a bridge can also be related to a Cooper System value.

Proper application of this system requires a full understanding of its use and limitations. However, the results of its application can be translated into terms of equipment weights and configurations that can be effectively applied by persons who manage regular transportation operations of the railroad. This enables them to determine if a given locomotive, car, or combination can be operated on a bridge with no further consideration, or if the equipment must be evaluated as an exceptional movement.

Paragraph (h). FRA states that bridge load capacity may be expressed in terms of both normal and maximum load conditions. Normal bridge ratings generally define the loads that can be operated on a bridge for an indefinite period without damaging the bridge. In some cases, mostly involving steel or iron bridges, a higher rating, up to a maximum rating, can be given to the bridge to permit the operation of heavier loads on an infrequent basis. These heavier loads should not, in themselves, damage the bridge, but the cumulative effect of the higher resulting stresses in bridge members could cause their eventual deterioration.

Paragraph (h) also states that operation of equipment that produces forces greater than the normal capacity shall be subject to any restrictions or conditions that may be prescribed by a railroad bridge engineer. A railroad bridge engineer can often prescribe compensating conditions that will permit the movement of equipment that is heavier than normal. Examples include speed restrictions to reduce the impact factor of the rolling load, the insertion of lighter-weight spacer cars between the heavier cars in a train, or the installation of temporary bents or other supports under specific points on the bridge.

Section 237.73 Protection of Bridges From Over-Weight and Over-Dimension Loads

Bridges can be seriously damaged by the operation of loads that exceed their capacity. Movement of equipment that exceeds the clear space on a bridge is an obvious safety hazard. In this section, FRA addresses Congress' mandate in the RSIA that the track owner "develop, maintain, and enforce a written procedure that will ensure that its bridges are not loaded beyond their capacities." See Section 417(b)(4), Public Law 110–432, 122 Stat. 4890 (49 U.S.C. 20103, note).

Paragraph (a). In this paragraph, FRA requires that each track owner issue instructions to its personnel who are responsible for the configuration and operation of trains over its bridges to prevent the operation of cars, locomotives and other equipment that would exceed the capacity or dimensions of its bridges. Transportation personnel of a railroad are ultimately responsible for the movement of trains, cars and locomotives. It is essential that they should know and follow any restrictions that are placed on those movements.

Paragraph (b). In this paragraph, FRA states that the instructions regarding weight shall be expressed in terms of maximum equipment weights, and either minimum equipment lengths or axle spacing. Transportation personnel have information on the weights and configuration of cars and locomotives, and they must be able to relate that information to any restrictions placed on the movement of that equipment.

Paragraph (c). In this paragraph, FRA states that the instructions regarding dimensions shall be expressed in terms of feet and inches of cross section and equipment length, in conformance with common railroad industry practice for reporting dimensions of exceptional equipment in interchange in which height above top-of-rail is shown for each cross section measurement, followed by the width of the car or the shipment at that height. In the industry, a standard format exists for the exchange of information on dimensions of railroad equipment. This standard practice is practical, even if it is not intuitive. Use of the industry practice is necessary to avoid error and confusion.

Paragraph (d). In this paragraph, FRA states that the instructions may apply to individual structures or to a defined line segment or groups of line segments where the published capacities and dimensions are within the limits of all structures on the subject line segments. Railroads commonly issue instructions related to equipment weights and dimensions to be effective on line segments of various lengths. It is not necessary that transportation personnel be advised of the capacity of every bridge as long as each bridge in the line segment has the capacity to safely carry the loads permitted on that line.

# Subpart E-Bridge Inspection

In subpart E, FRA establishes minimum standards to be incorporated into railroad bridge management programs to provide for an effective program of bridge inspections.

Bridge inspection is a vital component in any bridge management program. A bridge with undetected or unreported damage or deterioration can present a serious hazard to the safe operation of trains. Bridge inspection and evaluation is a multi-tiered process, unlike many other types of inspection on a railroad. While track, equipment and signal inspectors usually can compare measurements against common standards to determine whether the inspected feature complies with the standards, such is not the case with most bridges. The evaluation of a bridge requires the application of engineering principles by a competent person, who is usually not present during the inspection. It is therefore necessary that an inspection report should show any conditions on the bridge that might lead to a reduction in capacity, initiation of repair work, or a more detailed inspection to further characterize the condition.

Section 237.101 Scheduling of Bridge Inspections

Paragraph (a). In this paragraph, FRA establishes regulations to address Congress' mandate that the track owner "conduct regular comprehensive inspections of each bridge, at least once every year, and maintain records of those inspections that include the date on which the inspection was performed, the precise identification of the bridge inspected, the items inspected, and accurate description of the condition of those items, and a narrative of any inspection item that is found by the inspector to be a potential problem." Section 417(b)(5), Public Law 110-432, 122 Stat. 4890 (49 U.S.C. 20103, note). Annual inspection of bridges has been an industry practice for over a century, and has proven to be an effective tool of bridge management. Even where a bridge sees very low levels of railroad traffic, the potential still exists for damage from external sources or natural deterioration. This paragraph calls for one inspection per calendar year, with not more than 540 days between successive inspections. Both criteria apply. For example, if a bridge is inspected on January 3, 2011, it becomes overdue for inspection on June 27, 2012, 541 days later. If it is inspected on December 18, 2011, it becomes overdue on January 1, 2013, since it was not inspected in calendar year 2012.

One commenter requested that FRA clarify what constitutes a yearly inspection. The commenter asked if this means a "hands-on" type of inspection, or a routine cursory type of inspection. FRA responds that the rule does not prescribe an inspection procedure; that decision is left to the railroad bridge engineer. It is quite likely that the engineer might prescribe varying levels of detail for inspections performed at different periods, depending on the configuration and condition of the bridge.

Paragraph (b). In this paragraph, FRA states that a bridge shall be inspected more frequently than the period referenced in paragraph (a), above, when a railroad bridge engineer determines that such inspection frequency is necessary. The responsibility for adequate inspection remains with the track owner, with the conditions prescribed by a railroad bridge engineer. The inspection regimen for every bridge should be determined from its condition, configuration, environment, and traffic levels.

Paragraph (c). FRA requires that each bridge management program define requirements for the special inspection of a bridge to be performed whenever the bridge is involved in an event which might have compromised the integrity of the bridge, including flood, fire, earthquake, derailment, or other vehicular or vessel impact. It is essential that railroad traffic be protected from possible bridge failure resulting from damage from an event caused by natural or non-railroad agents. The track owner should have in place a means to receive notice of such an event, including weather and earthquakes, and a procedure to conduct an inspection following such an event.

Paragraph (d). In this paragraph, FRA states that any railroad bridge that has not been in railroad service and has not been inspected in accordance with this section within the previous 540 days must be inspected and the inspection report reviewed by a railroad bridge engineer prior to the resumption of railroad service. The inspection frequency requirements of this section do not apply to bridges that are not in railroad service. FRA notes that although inspections are not required on out-of-service railroad bridges, state law regarding responsibility for damage to outside parties that might be caused by the condition of the bridge is not affected. If a bridge not in service has been inspected within the 540 day period, the track owner may accept that inspection and begin railroad service, subject to any determination in that regard by a railroad bridge engineer. The inspection period would date from the last inspection, with no credit for outof-service time.

Section 237.103 Bridge Inspection Procedures

In this section, FRA requires that each bridge management program specify the procedure to be used for inspection of individual bridges or classes and types of bridges. As mandated by the RSIA, FRA states that the bridge inspection procedures must be as specified by a railroad bridge engineer who is designated as responsible for the conduct and review of the inspections. See Section 417(b)(7)(A), Public Law 110-432, 122 Stat 4890 (49 U.S.C. 20103, note). In the RSIA, Congress also mandated that the bridge safety regulations must "ensure that the level of detail and the inspection procedures are appropriate to the configuration of the bridge, conditions found during the previous inspections, and the nature of the railroad traffic moved over the bridge, including car weights, train frequency and lengths, levels of passenger and hazardous materials traffic, and vulnerability of the bridge to damage." Accordingly, FRA requires

that the bridge inspection procedures must ensure that the level of detail and the inspection procedures are appropriate to the configuration of the bridge. Additionally, the bridge inspection procedures must be designed to detect, report and protect deterioration and deficiencies before they present a hazard to safe train operation. The responsibility for adequate inspection remains with the track owner, with the conditions prescribed by a railroad bridge engineer. The inspection regimen for every bridge should be determined from its condition, configuration, environment, and traffic levels. The instructions for bridge inspection may be both general, as by bridge type or line segment; and specific, as needed by particular considerations for an individual bridge.

ASLRRA commented that the rule provides that a railroad bridge engineer must direct programs, review inspections, record procedures, and undertake other similar steps. ASLRRA suggests that this seems to imply the railroad must have a railroad bridge engineer capable of designing a bridge on staff or employed as a consultant each time an inspection is made. ASLRRA contends that a railroad supervisor can implement a program, review the inspection, audit a program, and assess whether a bridge inspection exception needs to go to a railroad bridge engineer for review.

FRA responds that a bridge inspection program can be established by a railroad bridge engineer, either as an employee of or as a consultant to the track owner. The engineer is not required to be on site, or even on the property, during an inspection. A primary purpose of the audit procedure called out below is to permit the railroad bridge engineer to review and monitor the effectiveness of the bridge inspection program that has been conducted under his overall charge.

## Section 237.105 Special Inspections

Paragraph (a). In this paragraph, FRA requires that each bridge management program prescribe a procedure for protection of train operations and for inspection of any bridge that might have been damaged by a natural or accidental event, including flood, fire, earthquake, derailment or vehicular or vessel impact. It is essential that railroad traffic be protected from possible bridge failure caused by damage from an event caused by natural or non-railroad agents. The track owner should have in place a means to receive notice of such an event, including weather conditions and earthquakes, and a procedure to conduct an inspection following such an event.

Paragraph (b). In this paragraph, FRA requires that each bridge management program provide for the detection of scour or deterioration of bridge components that are submerged or subject to water flow. The condition of bridge components located underwater is usually not evident from above. Means to determine their condition might be as simple as using measuring rods from the surface, or might call for periodic or special diving inspection. Advanced technology might also provide devices that can be used to determine underwater conditions.

Maryland DOT requested that FRA provide advice on a required inspection frequency for the underwater inspection, noting that FHWA requires underwater inspections at least once in every five years. FRA responds that the rule does not prescribe a particular frequency for underwater inspections; that decision is left to the railroad bridge engineer, to be based on the particular conditions at each bridge.

Section 237.107 Conduct of Bridge Inspections

In this section, FRA requires that bridge inspections be conducted under the direct supervision of a designated railroad bridge inspector, who shall be responsible for the accuracy of the results and the conformity of the inspection to the bridge management program. Bridge inspections can often require more than one person for safety and efficiency. This provision permits others to assist the designated inspector, who remains responsible for the results of the inspection.

Section 237.109 Bridge Inspection Records

In this section, FRA requires that each track owner to which this part applies keep a record of each inspection required to be performed under this part. A bridge inspection has little value unless it is recorded and reported to the individuals who are responsible for the ultimate determination of the safety of the bridge. Bridge inspectors may use a variety of methods to record their findings as they move about the bridge. These include notebooks, voice recordings, having another individual transcribe notes, and photographs. These notes and other items are usually compiled into a prescribed report format at the end of the day or at the conclusion of the inspection. In paragraph (c), FRA delineates the essential elements that must be addressed and reported in any bridge inspection.

Paragraph (d). In this paragraph, FRA requires that an initial report of each

bridge inspection be placed in the location designated by the bridge management program within 30 calendar days of the completion of the field portion of the inspection. The initial report must include the information delineated in paragraph (c)(1) through (c)(5). The actual conduct of the inspection should be reported and recorded, showing the fact that the bridge was actually inspected on a certain date, the type of inspection performed, by whom it was performed, and whether or not any critical conditions were detected. Inspection and reporting procedures vary widely among different railroads and circumstances. In many cases, especially on larger railroads, an inspector would prepare the report before leaving the bridge. The reports might be forwarded by mail, by electronic means, or by hand delivery. They might be forwarded daily, weekly, or even less frequently. In other circumstances, a consulting engineer might be engaged by a small railroad to inspect all of the bridges on all or part of the line, and the final report might be prepared by the engineering firm after all of the inspections are completed. Similarly, a large railroad might begin a comprehensive inspection and evaluation of a large structure that will take several months to complete.

FRA recognizes the wide range of time periods required for these various inspections and reporting procedures, so this provision was developed as a means for the track owner to track inspection progress, bridge by bridge, with a simple line item showing:

(1) identification of the bridge inspected;

(2) date of completion of the inspection;

(3) identification of the inspector;

(4) type of inspection performed; and (5) indication on the report as to whether any item noted thereon requires expedited or critical review by a railroad bridge engineer, and any restrictions placed at the time of the inspection.

These five items can usually be listed on a single line of a report. The initial report might include all of the bridges inspected by one individual in a week or two. FRA does not anticipate that the initial or summary report include all of the data called for in the bridge management program, together with any narrative descriptions necessary for the correct interpretation of the report. This information would be included in the complete inspection report.

Paragraph (e). In this paragraph, FRA requires that a complete report of each bridge inspection shall be placed in the

location designated in the bridge management program within 120 days of the completion of the field portion of the inspection. A bridge inspection is not complete until the report of the inspection is filed and available to the persons who are responsible for the management of the bridges inspected. This time period does not include the time used by a consultant or in-house engineering group to complete an analysis of the results of the inspection, and it is not expected that the analysis need be completed within that time period. In cases where a detailed analysis is required, FRA intends that the inspection report on which the analysis is based would be separated from the analysis and filed within the required time frame.

Paragraph (f). This paragraph requires that each bridge inspection program specify the retention period and location for bridge inspection records. The retention period must be at least two years from the completion of the inspection. A comparison of successive reports can reveal any accelerating rates of deterioration or degradation of bridge components. Additionally, an audit or review of the effectiveness of a bridge inspection program requires comparison of previous inspection reports with the actual condition of a bridge included in the audit. The practice of comparing previous inspection reports with actual bridge conditions has been followed by FRA for more than a decade when evaluating railroad bridge management programs. It is a valuable factor in determining the effectiveness of a railroad's program.

Section 237.111 Review of Bridge Inspection Reports

The RSIA requires that an engineer who is competent in the field of railroad bridge engineering reviews all inspection reports and determines whether bridges are being inspected according to the applicable procedures and frequencies, and reviews any items noted by an inspector as exceptions. See Section 417(b)(7), Public Law 110-432, 122 Stat. 4890 (49 U.S.C. 20103, note). In this section, FRA requires responsible railroad bridge supervisors and railroad bridge engineers to review bridge inspection reports. Bridge inspection is usually a multi-tiered procedure. The inspector reports on the conditions noted in the inspection, but an engineer will necessarily evaluate those noted conditions and determine what, if any, further action is required.

The regulation does not require that a railroad bridge engineer review every inspection report, so long as the responsible management personnel keep track of the conduct of inspections to see that they are performed in accordance with the schedule and other requirements of this rule and the railroad's program. It should be a simple matter for the inspector to indicate on a report whether or not the report would require higher-level or engineering review. The engineering staff would review the reports that indicate problems or issues for them to resolve. Section 237.153, "Audits of inspections," includes a provision for sampling of routine inspection reports to assure that the inspectors are properly identifying reports that require review.

Subpart F—Repair and Modification of Bridges

In subpart F, FRA establishes minimum standards to be incorporated in railroad bridge management programs to provide for adequate design and effective supervision of those bridge modifications and repairs which will materially modify the capacity of the bridge or the stresses in any primary load-carrying component of the bridge. This section provides for correct design and adequate supervision of repair and modification of bridges where the work could materially affect the capacity of the bridge, or its continued integrity. FRA does not intend that minor repairs that do not affect the capacity of the bridge must be designed by an engineer, but the supervision of that work should be performed by a person who is competent to assure that the work does not inadvertently compromise the integrity of the bridge. For instance, arc welding handrails to the members of a through truss might appear to some to be a minor repair, but it could seriously compromise the structural integrity of the bridge.

# Section 237.131 Design

Design of entire railroad bridges, modifications and repairs which materially modify the capacity of the bridge or the stresses in any primary load-carrying component of the bridge require the intelligent application of the principles of engineering and can be performed only by an engineer with training and experience in the field of railroad bridges. Railroads have typically issued standard instructions for the performance of common maintenance repairs, such as replacement or upgrading of components of timber trestles. This section specifically permits such a practice. For purposes of this part, a primary load-carrying component is a railroad bridge component, the failure of which would immediately compromise the structural integrity of the bridge.

One commenter notes that the proposed rule requires that while all bridge work that eliminates a deteriorated condition requires design by a bridge engineer, for many situations ranging from cracked flange angles to failed timber caps, a simple component change-out is the most effective repair. These types of repairs have historically been performed by bridge forces without the benefit of formal design oversight. The commenter suggested that each track owner should determine what repairs require the oversight of an engineer.

FRA understands this concern, and has modified § 237.131 to read, in part, that "[e]ach repair or modification which materially modifies the capacity of a bridge or the stresses in any primary load-carrying component of a bridge shall be designed by a railroad bridge engineer."

The comment regarding simple component replacement is addressed in the last sentence of the paragraph, which states that designs and procedures for repair or modification of bridges of a common configuration, such as timber trestles, or instructions for in-kind replacement of bridge components, may be issued as a common standard. Although it may be a standard procedure, the standard should be designed and issued by a qualified railroad bridge engineer.

Section 237.133 Supervision of Repairs and Modifications

This section requires that each repair or modification pursuant to this part shall be performed under the immediate supervision of a railroad bridge supervisor as defined in § 237.55 of this part who is designated and authorized by the track owner to supervise the particular work to be performed. Modifications and repairs which materially modify the capacity of the bridge or the stresses in any primary load-carrying component of the bridge must be performed according to the specific or general specifications and instructions issued by a railroad bridge engineer. Particularly when trains are permitted to pass over a bridge which is being repaired or modified, the supervisor at the bridge must be able to make the necessary determination to either permit, restrict or halt train operation depending on the state of the bridge. As this part does not specify the employment relationship between the track owner and the bridge supervisor, the track owner may designate a contractor or a consultant as the bridge supervisor.

One commenter asked if FRA would object to a track owner designating a contractor's foreman as the bridge supervisor qualified to return a bridge to service at the end of each work window. The commenter also stated that small railroads that do not have a bridge engineer may have to designate their engineering consultant as the bridge supervisor whose full-time presence on a job will be expensive and will take money away from repairs. FRA responds that the proposed regulation does not specify the employment relationship between the track owner and a bridge supervisor. A contractor employee or a consultant may be so designated. It is necessary, however, that a qualified individual be responsible for the proper and safe performance of work on a bridge, and that the individual be authorized to perform the actions necessary to fulfill that responsibility.

# Subpart G—Documentation, Records, and Audits of Bridge Management Programs

Documentation is essential to any effective management program. In subpart G, FRA establishes minimum standards to be incorporated in railroad bridge management programs to provide for verification of the effectiveness of the program and the accuracy of the information developed thereby, by the track owner and by FRA to evaluate compliance with this regulation.

#### Section 237.151 Audits; General

In this section, FRA requires that each program adopted to comply with this part include provisions for auditing the effectiveness of the several provisions of that program, including the validity of bridge inspection reports and bridge inventory data, and the correct application of movement restrictions to railroad equipment of exceptional weight or configuration. Effective management of a safety-critical program such as this requires an adequate level of checks to assure that the requisite work is being performed correctly.

#### Section 237.153 Audits of Inspections

FRA has found over the years during which it has conducted evaluations of railroad bridge programs that one of the most important indicators of the effectiveness of a program is a comparison of recent bridge inspection reports against actual conditions found at the subject bridges. This is fundamental to an effective audit of a bridge management program. Therefore, in this section, FRA states that each bridge management program incorporate provisions for an internal audit. Each bridge management program shall incorporate provisions for an internal audit to determine whether the inspection provisions of the program are being followed, and whether the program itself is effectively providing for the continued safety of the subject bridges. Additionally, the inspection audit shall include an evaluation of a representative sampling of bridge inspection reports at the bridges noted on the reports to determine whether the reports accurately describe the condition of the bridge.

# Section 237.155 Documents and Records

In this section, FRA requires each track owner required to implement a bridge management program and keep records under this part to make those program documents and records available for inspection and reproduction by FRA. This section addresses Congress' mandate in the RSIA to establish a program to periodically review bridge inspection and maintenance data from railroad carrier bridge inspectors and FRA bridge experts. See Section 417(d), Public Law 110-432, 122 Stat. 4890 (49 U.S.C. 20103, note). As in the case of all railroad safety regulations, FRA has an enforcement responsibility. FRA will require access to the vital documents and records of the various bridge management programs to enable it to carry out that responsibility.

Paragraphs (a) and (b). In these paragraphs, FRA establishes minimum standards for electronic record-keeping provisions that a track owner may elect to utilize to comply with the recordkeeping provisions of this part. FRA recognizes the growing prevalence of electronic records, and acknowledges the unique challenges that electronic transmission, storage, and retrieval of records can present. To allow for future advances in technology, FRA is establishing electronic record storage provisions in these paragraphs that are technology-neutral.

For purposes of complying with the record-keeping requirements of this part, a track owner may create and maintain any of the required records through electronic transmission, storage, and retrieval, provided that certain conditions are met. Not only must the system used to generate the electronic records meet all of the requirements of this subpart and the records contain all of the information required by this subpart, but the track owner must also: monitor the electronic database through a sufficient number of monitoring indicators to ensure a high degree of the accuracy of the records; train the

employees who use the system on the proper use of the system; and maintain an information technology security program adequate to ensure the integrity of the system, including the prevention of unauthorized access to the program logic or individual records.

Additionally, the integrity of the bridge inspection records must be protected by a security system that incorporates user identity and password, or a comparable method, to establish appropriate levels of program and record data access meeting all of the following standards: no two individuals can have the same electronic identity; a record cannot be deleted or altered by any individual after the record is certified by the employee who created the record; any amendment to the record must either be electronically stored apart from the record it amends, or electronically attached to the record as information without changing the original record; each amendment to a record must uniquely identify the person making the amendment; and the electronic system must provide for the maintenance of inspection records as originally submitted without corruption or loss of data.

Two commenters expressed a general concern that the security provisions of the proposed rule would preclude the modification of permanent bridge records, such as the inventory itself. As FRA responds that was not the intent, the final rule has been modified so that the data security provisions apply only to bridge inspection records.

Appendix A to Part 237—Supplemental Statement of Agency Policy on the Safety of Railroad Bridges

A Statement of Agency Policy on the Safety of Railroad Bridges was originally published by FRA in 2000 as Appendix C of the Federal Track Safety Standards, 49 CFR part 213. With the issuance of 49 CFR part 237, Bridge Safety Standards, certain non-regulatory provisions in that Policy Statement have been incorporated in that regulation. However, FRA has determined that other non-regulatory items are still useful as information and guidance. Those provisions of the Policy Statement are therefore retained and placed in this Appendix in lieu of their former location in the Track Safety Standards.

#### Appendix B to Part 237—Schedule of Civil Penalties

Consistent with FRA's Statement of Agency Policy Concerning Enforcement of the Federal Railroad Safety Laws, a penalty may be assessed against an individual only for a willful violation. The Administrator reserves the right to assess a penalty of up to \$100,000 for any violation where circumstances warrant. See 49 CFR part 209, appendix A

# **VI. Regulatory Impact and Notices**

# A. Executive Order 12866 and DOT Regulatory Policies and Procedures

This final rule has been evaluated in accordance with existing policies and procedures and determined to be nonsignificant under both Executive Order 12866 and DOT policies and procedures. See 44 FR 11034; February 26, 1979. FRA has prepared and placed in the docket a regulatory impact analysis addressing the economic impacts from this final rule.

As part of the regulatory impact analysis FRA has assessed quantitative measurements of the cost and benefit streams expected from the adoption of this final rule. For the 20-year period the estimated quantified costs total \$164.2 million, and have a present value (PV, 7%) of \$84.4 million. For the same period of time the estimated quantified benefits total \$19.4 million and have a PV(7%) of \$9.8 million. These benefits are exclusive of long-term efficiencies to the railroads with respect to conservation of the capital value of the structures in question. Very often targeted repairs or restoration at an early stage in the deterioration of a bridge may significantly extend the useful life of a bridge. The benefits also do not consider the potential for a catastrophic event resulting in a bridge failure and consequent fatalities to railroad personnel, rail passengers, or persons underneath the bridge. Although FRA has verified through its bridge program that most railroads properly manage their bridges most of the time, in the recent past FRA has also determined circumstances-even on Class I railroads—where proper inspections or repairs have been inappropriately deferred. Accordingly, this final rule offers the opportunity to capture and extend the current heightened attention to bridge management achieved through industry and FRA efforts over the past several years.

# B. Regulatory Flexibility Act and Executive Order 13272; Final Regulatory Flexibility Assessment

The Regulatory Flexibility Act (5 U.S.C. 601 *et seq.*) and Executive Order 13272 require a review of proposed and final rules to assess their impacts on small entities. An agency must prepare an initial regulatory flexibility analysis (IRFA) unless it determines and certifies that a rule, if promulgated, would not have a significant impact on a substantial number of small entities. During the NPRM stage, FRA had not determined whether the proposed rule would have a significant economic impact on a substantial number of small entities. Therefore, FRA published an IRFA to aid the public in commenting on the potential small business impacts of the proposals in the NPRM. All interested parties were invited to submit data and information regarding the potential economic impact that would result from adoption of the proposals in the NPRM.

The Regulatory Flexibility Act also requires an agency to conduct a final regulatory flexibility assessment (FRFA) unless it determines and certifies that a rule is not expected to have a significant impact on a substantial number of small entities. FRA is not able to certify that the final rule will not have a significant economic impact on a substantial number of small entities due to insufficient information. FRA did not receive many comments, or data from commenters, on the IRFA, and the information that was received was not sufficient to make a determination. Thus, FRA is publishing this FRFA and will issue a small entity guidance document soon.

FRA estimates, primarily based on two facts, that approximately 70 percent of the total cost of this rulemaking (see regulatory impact analysis (RIA)) will be borne by small entities. First, larger railroads generally have more comprehensive bridge management programs and more frequent bridge inspections. Second, since FRA's RIA is an overall industry analysis, it is not immediately obvious that the incremental cost burden on small railroads is proportionally larger than for larger entities. This is because more small railroads will have to increase inspection frequency and enhance their management programs. It should be noted that the bridge populations of typical small railroads are less complex than those of larger railroads.

Below, FRA provides the rationale it used for assessing what impacts would be borne by small entities. FRA considered all comments received in the public comment process when making a determination in the FRFA.

This FRFA was developed in accordance with the Regulatory Flexibility Act.

(1) A Succinct Statement of the Need for and Objectives of the Rule

As discussed in Section I of the preamble to this rule, the structural integrity of bridges that carry railroad tracks is important because the severity of a train accident is usually compounded when a bridge is involved, regardless of the cause of the accident. In 2000, FRA published a final statement of agency policy for the safety of railroad bridges, establishing criteria to ensure the structural integrity of bridges that carry railroad tracks. The Rail Safety Improvement Act of 2008 (RSIA) directs FRA to issue, by October 16, 2009, regulations requiring railroad track owners to adopt and follow specific procedures to protect the safety of their bridges.

There are more than 100,000 railroad bridges in the United States. Federal regulations offer the benefit of uniformity that would allow railroads that operate in more than one State to develop and implement a single management program that would apply to all of its railroad bridges, supporting one or more tracks, rather than several programs tailored to meet the different requirements of each different State or local jurisdiction.

FRA is issuing this rule to promulgate minimum bridge safety standards as mandated by RSIA, Section 417, Public Law 110–432, 122 Stat. 4890 (49 U.S.C. 20103, note).

(2) A Summary of the Significant Issues Raised by the Public Comments in Response to the IRFA, a Summary of the Assessment of the Agency of Such Issues, and a Statement of Any Changes Made to the Proposed Rule as a Result of Such Comments

No comments were received that directly addressed the IRFA. However, a few comments did address items of cost used in the RIA, which are related to the IRFA for the NPRM.

# (a) Security of Records

In 49 CFR 237.155, FRA proposed numerous recordkeeping requirements primarily dealing with security. The recordkeeping requirements in the proposed rule assumed that the documents would be kept electronically. One commenter noted that not all documents for small railroads would be maintained that way. Thus, the final rule has a minor revision that accommodates bridge inspection records that are not electronic. The impact of this minor change will not cause any cost calculation changes.

# (b) Bridge Inspection Cost

One commenter did not agree with the average bridge inspection cost that the FRA used in its RIA. More specifically, this commenter mentioned that \$750 for the average cost of a bridge inspection is not realistic. This commenter also opined that the actual cost is more excessive (in the range of \$4,000 to \$5,000 per bridge) for a bridge that was inspected on a 2-year cycle.

FRA disagrees with this commenter and believes that the cost used in the RIA for the NPRM is appropriate, given its understanding and interpretation of the regulatory requirements. In response, FRA emphasizes that its cost estimate is an average that includes lower cost inspections, such as that of a wood trestle bridge over a small stream, which would be less than the average cost. In addition, this commenter was basing the higher cost estimate on a more expensive, hands-on detailed bridge inspection process required on a 2-year frequency for highway bridges by FHWA. Finally, this commenter was providing comments related to experiences with inspecting a population of large highway bridges. For these reasons, FRA has not modified its cost estimate for bridge inspections.

(3) A Description and an Estimate of the Number of Small Entities to Which the Rule Will Apply or an Explanation of Why No Such Estimate Is Available

The "universe" of the entities to be considered generally includes only those small entities that are reasonably expected to be directly regulated by this action. Two types of small entities are potentially affected by this rulemaking: (1) railroads that own track supported by a bridge, and (2) governmental jurisdictions of small communities that own railroad bridges.

"Small entity" is defined in 5 U.S.C. 601 as having the same meaning as "small business concern" under Section 3 of the Small Business Act. This includes any small business concern that is independently owned and operated, and is not dominant in its field of operation. Section 601(4) includes nonprofit enterprises that are independently owned and operated, and are not dominant in their field of operations within the definition of "small entities." Additionally, 5 U.S.C. 601(5) defines "small entities" as governments of cities, counties, towns, townships, villages, school districts, or special districts with populations less than 50,000.

The U.S. Small Business Administration (SBA) stipulates "size standards" for small entities. It provides that the largest a for-profit railroad business firm may be (and still classify as a "small entity") is 1,500 employees for "line-haul operating" railroads, and 500 employees for "shortline operating" railroads.<sup>1</sup>

SBA size standards may be altered by Federal agencies in consultation with SBA and in conjunction with public comment. Pursuant to the authority provided to it by SBA, FRA has published a final policy, which formally establishes small entities as railroads that meet the line haulage revenue requirements of a Class III railroad.<sup>2</sup> Currently, the revenue requirements are \$20 million or less in annual operating revenue, adjusted annually for inflation. The \$20 million limit (adjusted annually for inflation) is based on the Surface Transportation Board's threshold of a Class III railroad carrier, which is adjusted by applying the railroad revenue deflator adjustment.<sup>3</sup> The same dollar limit on revenues is established to determine whether a railroad shipper or contractor is a small entity. FRA proposed to use this definition for the rulemaking in the NPRM and received no comments on that proposal. FRA is using this definition for the final rule.

# (a) Governmental Jurisdictions of Small Communities

Small entities that are classified as governmental jurisdictions of small communities may also be affected by this rulemaking. As stated above, and defined by SBA, this term refers to the governments of cities, counties, towns, townships, villages, school districts, or special districts with populations of less than 50,000. The potential impact of this rulemaking to these entities is related to their ownership of a bridge, and possibly the track supported by the bridge as well. Such bridges are usually built by communities, with railroad collaboration, to achieve highway-rail grade separation. FRA does not have information regarding the number of small communities that own such bridges and received no additional information during the comment process of the NPRM.

In some cases, however, the government entity and the railroad apportion ownership, expenses, and maintenance responsibility according to the provisions of an order from the State regulatory agency that governs highway and railroad crossing improvements. It is most common for the railroad to retain the responsibility for the actual inspection and management of the bridge. To the extent that agreements which require cost-sharing and existing bridge management programs would have to be enhanced to meet the final regulation, there may be some burden passed on to small government jurisdictions; however, such burden is not expected to be substantial. To the extent that any burden does result, it is possible that insurance premiums could be adjusted to reflect the risk reduction, resulting in some level of savings in addition to the cost of the program enhancement. This would, of course, be in addition to safety benefits related to fewer accidents.

Accordingly, FRA cannot accurately assess the number of governmental jurisdictions of small communities that would be directly impacted by this regulation and what the impact would be to them. FRA requested comment from affected governmental jurisdictions as to the impact the proposed rule would have on them during the NPRM comment process. The comments received during the public comment period of the NPRM did not provide any additional data or information on this issue.

# (b) Railroads

There are approximately 687 small railroads meeting the definition of "small entity" as described above. FRA estimates that approximately 95 percent of these small entities, or approximately 653, own track supported by a bridge. Because the final rule would apply to all of these small railroads, FRA has concluded that a substantial number of such entities would be impacted. Note, however, that approximately 125 of these railroads are subsidiaries of large shortline holding companies with the expertise and resources comparable to larger railroads. In the IRFA for the NPRM, FRA estimated a smaller number of subsidiaries, but since then has gained more accurate information as to the best estimate of how many small railroads are subsidiaries of larger corporations. In addition, absent this rulemaking, most railroads that own track supported by bridges, including many of the railroads identified as small entities, would to some extent voluntarily incur the expense associated with implementation of the bridge management programs in accordance with the requirements imposed by FRA to address the risk associated with structural failure of a bridge. In fact, the ASLRRA, which represents most of the small railroads impacted by this rulemaking, has developed a model bridge management program intended to keep bridge and culvert infrastructure safe and structurally sound. Member railroads are expected to take the

generic plan and customize it to meet their specific circumstances and the requirements in this rule. Such initiative would minimize the program development cost. Nevertheless, program implementation costs may be substantial for those small railroads that do not currently have bridge management programs, and do not inspect railroad bridges regularly.

While FRA does recognize that some small railroads do not currently have bridge management programs, FRA believes that many railroads have already made (or are making) the transition to track structures and bridges capable of handling 286,000-pound cars in line with the general movement in the industry toward these heavier freight cars. To protect such investments, which are usually quite significant, railroads are already implementing bridge management programs.

For example, in 2005, the Texas Transportation Institute reported that 42 percent of the shortline railroad miles that were operated in Texas that year had already been upgraded, 9 percent would not need an upgrade, and 47 percent needed upgrading if they wanted to transport any type of 286,000pound shipments.<sup>4</sup> In addition, the results of a 1998–1999 survey conducted by ASLRRA indicated that 41 percent of respondent shortline railroads could handle 286,000-pound rail cars and 87 percent of the respondent shortline railroads indicated that they would need to accommodate 286,000-pound railcars in the future.<sup>5</sup>

In addition, at least one Class I railroad has arranged for shortline and regional railroads that connect with it to send participants to several multiday bridge inspection classes this year.

In general, implementation of the final rule will likely significantly burden only a small portion of the small railroads potentially affected. FRA invited commenters to submit information that might assist us in assessing the cost impacts on small railroads of the proposals during the comment process of the NPRM; however, very little comment was received on this matter, and comments received were not sufficient to allow us to make a determination.

<sup>&</sup>lt;sup>1</sup> "Table of Size Standards," U.S. Small Business Administration, January 31, 1996, 13 CFR Part 121. *See also* NAICS Codes 482111 and 482112.

<sup>&</sup>lt;sup>2</sup> See 68 FR 24891 (May 9, 2003).

<sup>&</sup>lt;sup>3</sup> For further information on the calculation of the specific dollar limit, please see 49 CFR Part 1201.

<sup>&</sup>lt;sup>4</sup> Jeffrey E. Warner and Manuel Solari Terra, "Assessment of Texas Short Line Railroads," Texas Transportation Institute (November 15, 2005).

<sup>&</sup>lt;sup>5</sup> The 10-Year Needs of Short Line and Regional Railroads, Standing Committee on Rail Transportation, American Association of State Highway and Transportation Officials, Washington, DC (December 1999). This report was based on a survey conducted by the ASLRRA in 1998 and 1999, with data from 1997.

(4) A Description of the Projected Reporting, Recordkeeping, and Other Compliance Requirements of the Rule, Including an Estimate of the Classes of Small Entities That Will Be Subject to the Requirement and the Type of Professional Skills Necessary for Preparation of the Report or Record

The impacts from this rulemaking will primarily result from complying with the requirements for the adoption of bridge management programs. The final rule provides affected entities 6- to 24-month periods of time in which to adopt such programs. Class III railroads will have the full 24-month period from the effective date of the final rule, unless they have more than 10 scheduled passenger trains per week operating anywhere on their system, in which case they would have only 6 months.

# (a) Recordkeeping Requirements of § 237.33

The requirements in § 237.33 stipulate that each bridge management program includes an accurate inventory of railroad bridges; a record of the safe load capacity of each bridge; a provision to obtain and maintain the design documents of each bridge if available, and to document all repairs, modifications, and inspections of each bridge; and a bridge inspection program covering the method of documenting inspections, including standard forms and formats.

FRA believes that most railroads, regardless of size, already maintain an accurate inventory of their railroad bridges, records of the safe load capacity of their bridges, and design documents to the extent they are available. Likewise, because it is good business practice to do so, most railroads maintain documents related to all repairs, modifications, and inspections of bridges. The States of Ohio, Michigan, and New York have existing bridge regulations requiring railroads to maintain bridge inventories and inspect bridges annually. There are approximately 100 small railroads that operate in those States. However, some railroads may not include in their documentation some of the particular data items specified in this rule. Thus these requirements will impose a nominal additional recordkeeping burden on some small railroads.

As noted above, not all small railroads have inspection programs. ASLRRA, however, has developed a model program for its members, thus minimizing the burden associated with the development of such plans. FRA estimates that the burden for individual railroad customization of the program would range from \$570, for the smaller Class III railroads, to \$3,000 for the larger Class III railroads. Costs associated with maintenance, modifications, and updates to bridge management plans will average approximately 15 percent of the initial development cost, or between \$85 and \$450, annually. Therefore, this reporting requirement will have minimal impact on small entities.

Determination of bridge load capacity will be made by a bridge engineer. The engineer is determined by the track owner to be competent to perform the functions necessary for the determination of load capacity. Bridge inspection procedures would be specified by a railroad bridge engineer who is designated as responsible for the conduct and review of the inspections.

# (b) Bridge Inspections

Bridge management programs will be required to contain bridge inspection programs. Subpart E requires calendar year inspections of bridges according to specified procedures, as well as special inspection of bridges that might be damaged by a natural or accidental event. This subpart also specifies that bridge inspections must be conducted under the direct supervision of a designated bridge inspector. The inspector is deemed technically competent to view, measure, report, and record the condition of a railroad bridge and its individual components. FRA expects there will be a significant increase in the number of bridge inspections conducted by small railroads or their contractors or consulting engineers. FRA requested comments and input regarding the extent to which Class III railroads already conduct annual inspection of bridges and the extent to which they would have to conduct additional bridge inspections. FRA did not receive any comments or information related to this request.

Most small railroads do not have bridge engineers or inspectors on staff. They contract out bridge inspections. A typical contract is for the inspection of most (if not all) the bridges the railroad owns, with delivery of a final report addressing the state of all bridges. Interim reports are provided to the railroad, or the responsible railroad bridge engineer, to record the fact that a certain bridge has actually been inspected and whether or not any significant deficiencies were noted. Some States provide shortline railroads funding via grants and loans for infrastructure improvements including bridge rehabilitation, track maintenance, and bridge inspection. For instance, the Tennessee Department of Transportation provides significant grants for such projects to most of the 20 Class III railroads in the State.<sup>6</sup> The Pennsylvania Department of Transportation administers a matching grant program to support freight railroad maintenance and construction costs.

FRA believes that small railroads own, or would otherwise be responsible for inspecting, approximately 20,000 bridges. FRA estimates that the average cost per bridge inspection is \$750, and that approximately 10,000 bridges are being inspected less frequently than once a year, while 5,000 are not inspected at all. Most small railroads may own track supported by several bridges, especially in some areas where the terrain requires such structures. FRA requested comment regarding the level of cost burden that the annual inspection would impose. The cost for this requirement was the largest cost in FRA's RIA. FRA believes that, of the railroads which do not presently inspect their bridges on an annual basis, most are small railroads.

## (c) Determination of Bridge Load Capacities

Subpart D requires the determination of bridge load capacities. FRA believes that railroad bridge owners are generally aware of bridge load capacities. Nevertheless, it is likely that some railroads will have to take action to verify this information in order to develop the type of documentation required by this subpart. Bridge load capacity information is vital to ensuring that safe capacity is not exceeded. Small railroads affected by this requirement will likely have a consulting engineer perform such calculations. Most of the bridges that do not already have load capacities calculated are smaller, less complex structures.

(d) Repair and Modification of Bridges

Subpart F prescribes minimum standards for bridge modification and repair that will materially modify the capacity of a bridge or the stresses in any primary load carrying component of the bridge. Modifications and repairs to bridges (except for minor modifications and repairs) will have to be designed by railroad bridge engineers, and the work will have to be supervised by designated bridge supervisors. Small railroads will generally contract out such modifications and repairs. As common

<sup>&</sup>lt;sup>6</sup>U.S. General Accounting Office, "Railroad Bridges and Tunnels, Federal Role in Providing Safety Oversight and Freight Infrastructure Investment Could Be Better Targeted," August 2007, (GAO–07–770).

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practice, consulting engineers meet the design and supervision requirements of this rule, and competent contractor employees may be designated to perform the immediate supervision of much of the modification and repair work.

# (e) Audits

Each program will have to include provisions for auditing the effectiveness of several provisions of the program, including the validity of bridge inspection reports and bridge inventory data, and the correct application of movement restrictions to railroad equipment of exceptional weight or configuration. FRA anticipates that Class III railroad audits will generally be performed by a company official following guidance in the ASLRRA model program and without assistance from an external financial or engineering auditor. In general, FRA anticipates that the audit process will be simpler and consume fewer resources for small railroads than for larger railroads. This is because, by the nature of their operations, shortlines will probably have smaller and less complex bridge populations.

(5) A Description of the Steps the Agency Has Taken To Minimize the Significant Adverse Economic Impact On Small Entities Consistent With the Objectives of Applicable Statutes, Including a Statement of Factual, Policy, and Legal Reasons for Selecting the Alternative Adopted in the Final Rule, and Why Each of the Other Significant Alternatives to the Rule Considered by the Agency Was Rejected

In § 237.31, FRA sets the schedule for railroads to adopt bridge safety

management programs. In consideration of the impact on small railroads that may not already have such programs, this schedule generally provides small railroads with an additional 18 months more than Class I carriers, and an additional 12 months more than Class II carriers, to adopt these programs.

FRA has identified no additional, significant alternative to this final rule that satisfies the mandate of the RSIA or meets the agency's objective in promulgating this rule, and that would minimize the economic impact of the rulemaking on small entities. As in all aspects of this rulemaking, FRA requested comments on this finding of no significant alternative related to small entities. No comments were received relative to the question of what alternatives could be provided to small entities.

The process by which this final rule was developed provided outreach to small entities. As noted in Section III of this final rule, this rule was developed in consultation with industry representatives through RSAC, which includes small railroad representatives. On December 10, 2008, RSAC referred to the Working Group, established in March 2008, the task of developing a draft rule requiring the owners of track carried on one or more railroad bridges to adopt a bridge safety management program to reduce the risk of human casualties, environmental damage, and disruption to the Nation's railroad transportation system resulting from catastrophic bridge failure. The Working Group met twice, on January 28-29, 2009, and February 23-24, 2009. Small railroad representatives participated in both meetings and raised issues of

concern to small railroads. Of specific concern to small railroads that own several bridges and contract out the inspection of these bridges, was the ability to continue to enter into such contractual agreements structured such that final inspection reports are submitted as part of a single report at the completion of the contract, which could span several months. After the comment period for the NPRM closed, FRA held a 1-day meeting for the Working Group to review the comments to the docket. This meeting was held in Washington, DC, on December 15, 2009. At this meeting all comments were reviewed and the Working Group provided FRA with pertinent input on potential issues. This final rule takes into account the comments and input provided by the Working Group.

# C. Paperwork Reduction Act

The information collection requirements in this final rule have been submitted for approval to the Office of Management and Budget (OMB) under the Paperwork Reduction Act of 1995, 44 U.S.C. 3501 *et seq.* The sections that contain the new information collection requirements and the estimated time to fulfill each requirement are as follows:

CFR Section	Respondent universe	Total annual responses	Average time per response	Total an- nual bur- den hours
237.3:				
Notifications to FRA of Assignment of Bridge Respon- sibility.	693 Railroads	15 notifications	90 minutes	22.5
Signed Statement by Assignee Concerning Bridge Re- sponsibility.	693 Railroads	15 signed statements	30 minutes	7.5
237.9: Waivers—Petitions	693 Railroads	12 petitions	4 hours	48
237.31 and 237.33: Development/Adoption of Bridge Management Program.	693 Railroads	693 plans	Varies	20,100
237.57: Designation of Qualified Individuals	693 Railroads	200 designations	30 minutes	100
237.71: Determination of Bridge Load Capacities	693 Railroads	2,000 determinations	8 hours	16,000
237.73: Issuance of Instructions to Railroad Personnel by Track Owner.	693 Railroads	2,000 instructions	2 hours	4,000
237.105:		7 500	10 50 1	00 750
Special Bridge Inspections and Reports/Records	693 Railroads	7,500 inspections and reports/records.	12.50 hours	93,750
Special Underwater Inspections	693 Railroads	50 inspections and re- ports/records.	40 hours	2,000
237.107 and 237.109:		-		
Nationwide Annual Bridge Inspections—Reports	693 Railroads	18,000 inspections and reports.	4 hours	72,000
Records	693 Railroads	18,000 records	1 hour	18,000

CFR Section	Respondent universe	Total annual responses	Average time per response	Total an- nual bur- den hours
Report of Deficient Condition on a Bridge (New from NPRM).	693 Railroads	50 reports	30 minutes	25
237.111:				
Review of Bridge Inspection Reports by Railroad Bridge Engineers.	693 Railroads	2,000 inspection report reviews.	30 minutes	1,000
Prescription of Bridge Inspection Procedure Modifica- tions After Review.	693 Railroads	200 inspection proce- dure modifications.	30 minutes	100
237.131:				
Design of Bridge Modifications or Bridge Repairs	693 Railroads	1,250 designs	16 hours	20,000
Bridge Modification Repair Reviews/Supervisory Efforts	693 Railroads	1,250 bridge modifica- tion repair reviews.	1.50 hours	1,875
Common Standard Designed by Railroad Bridge Engi- neer (New from NPRM).	693 Railroads	50 standards	24 hours	1,200
237.153: Audits of Inspections	693 Railroads	693 inspection audits	80 hours/24 hours/6 hours.	5,470
237.155—Documents and Records:				
Establishment of Railroad Monitoring and Information Technology Security Systems for Electronic Record- keeping.	693 Railroads	5 systems	80 hours	400
Employees Trained in System	693 Railroads	100 employees	8 hours	800

All estimates include the time for reviewing instructions; searching existing data sources; gathering or maintaining the needed data; and reviewing the information. For information or a copy of the paperwork package submitted to OMB, contact Mr. Robert Brogan at 202–493–6292 or Ms. Kimberly Toone at 202–493–6132, or via e-mail at the following respective addresses: *Robert.Brogan@dot.gov*; or *Kimberly.Toone@dot.gov*.

Organizations and individuals desiring to submit comments on the collection of information requirements should direct them to the Office of Management and Budget, Office of Information and Regulatory Affairs, Washington, DC 20503, Attention: FRA Desk Officer. Comments may also be sent via e-mail to the Office of Management and Budget at the following address:

 $oira\_submissions@omb.eop.gov.$ 

OMB is required to make a decision concerning the collection of information requirements contained in this final rule between 30 and 60 days after publication of this document in the **Federal Register**. Therefore, a comment to OMB is best assured of having its full effect if OMB receives it within 30 days of publication.

FRA cannot impose a penalty on persons for violating information collection requirements that do not display a current OMB control number, if required. FRA intends to obtain current OMB control numbers for any new information collection requirements resulting from this rulemaking action prior to the effective date of this final rule. The OMB control number, when assigned, will be announced by separate notice in the **Federal Register**.

# D. Environmental Impact

FRA has evaluated this final rule in accordance with its "Procedures for Considering Environmental Impacts" (FRA's Procedures) (64 FR 28545, May 26, 1999) as required by the National Environmental Policy Act (42 U.S.C. 4321 et seq.), other environmental statutes, Executive Orders, and related regulatory requirements. FRA has determined that this action is not a major FRA action (requiring the preparation of an environmental impact statement or environmental assessment) because it is categorically excluded from detailed environmental review pursuant to section 4(c)(20) of FRA's Procedures. 64 FR 28547, May 26, 1999. In accordance with section 4(c) and (e) of FRA's Procedures, the agency has further concluded that no extraordinary circumstances exist with respect to this final rule that might trigger the need for a more detailed environmental review. As a result, FRA finds that this final rule is not a major Federal action significantly affecting the quality of the human environment.

#### E. Federalism Implications

Executive Order 13132, "Federalism" (64 FR 43255, Aug. 10, 1999), requires FRA to develop an accountable process to ensure "meaningful and timely input by State and local officials in the development of regulatory policies that have federalism implications." "Policies that have federalism implications" are defined in the Executive Order to include regulations that have "substantial direct effects on the States, on the relationship between the national government and the States, or on the distribution of power and responsibilities among the various levels of government." Under Executive Order 13132, the agency may not issue a regulation with federalism implications that imposes substantial direct compliance costs and that is not required by statute, unless the Federal government provides the funds necessary to pay the direct compliance costs incurred by State and local governments or the agency consults with State and local government officials early in the process of developing the regulation. Where a regulation has federalism implications and preempts State law, the agency seeks to consult with State and local officials in the process of developing the regulation.

FRA has analyzed this final rule in accordance with the principles and criteria contained in Executive Order 13132. This final rule will not have a substantial direct effect on the States, on the relationship between the Federal government and the States, or on the distribution of power and responsibilities among the various levels of government. FRA has also determined that this final rule will not impose substantial direct compliance costs on State and local governments. Therefore, the consultation and funding requirements of Executive Order 13132 do not apply.

Moreover, FRA notes that RSAC, which provided advice regarding this final rule, has as permanent members, two organizations representing State and local interests: AASHTO and ASRSM. Both of these State organizations concurred with the RSAC recommendation made in this rulemaking. RSAC regularly provides recommendations to the Administrator of FRA for solutions to regulatory issues that reflect significant input from its State members. To date, FRA has received no indication of concerns about the federalism implications of this rulemaking from these representatives or from any other representatives of State government.

However, this final rule could have preemptive effect by operation of law under a provision of the former Federal Railroad Safety Act of 1970 (former FRSA), 49 U.S.C 20106 (Sec. 20106). The former FRSA provides that States may not adopt or continue in effect any law, regulation, or order related to railroad safety or security that covers the subject matter of a regulation prescribed or order issued by the Secretary of Transportation (with respect to railroad safety matters) or the Secretary of Homeland Security (with respect to railroad security matters), except when the State law, regulation, or order qualifies under the "local safety or security hazard" exception to Section 20106.

In sum, FRA has analyzed this final rule in accordance with the principles and criteria contained in Executive Order 13132. As explained above, FRA has determined that this final rule has no federalism implications, other than the possible preemption of State laws under the former FRSA. Accordingly, FRA has determined that preparation of a federalism summary impact statement for this final rule is not required.

# F. Unfunded Mandates Reform Act of 1995

Pursuant to Section 201 of the Unfunded Mandates Reform Act of 1995 (Pub. L. 104-4, 2 U.S.C. 1531), each Federal agency "shall, unless otherwise prohibited by law, assess the effects of Federal regulatory actions on State, local, and tribal governments, and the private sector (other than to the extent that such regulations incorporate requirements specifically set forth in law)." Section 202 of the Act (2 U.S.C. 1532) further requires that "before promulgating any general notice of proposed rulemaking that is likely to result in the promulgation of any rule that includes any Federal mandate that may result in the expenditure by State, local, and tribal governments, in the aggregate, or by the private sector, of \$100,000,000 or more (adjusted annually for inflation) (currently \$140,800,000) in any 1 year, and before promulgating any final rule for which a general notice of proposed rulemaking was published, the agency shall prepare

a written statement" detailing the effect on State, local, and tribal governments and the private sector. This final rule will not result in the expenditure, in the aggregate, of \$140,800,000 or more in any one year, and thus preparation of such a statement is not required.

## G. Energy Impact

Executive Order 13211 requires Federal agencies to prepare a Statement of Energy Effects for any "significant energy action." See 66 FR 28355, May 22, 2001. Under the Executive Order a "significant energy action" is defined as any action by an agency that promulgates or is expected to lead to the promulgation of a final rule or regulation, including notices of inquiry, advance notices of proposed rulemaking, and notices of proposed rulemaking: (1)(i) That is a significant regulatory action under Executive Order 12866 or any successor order, and (ii) is likely to have a significant adverse effect on the supply, distribution, or use of energy; or (2) that is designated by the Administrator of the Office of Information and Regulatory Affairs as a significant energy action. FRA has evaluated this final rule in accordance with Executive Order 13211. FRA has determined that this final rule is not likely to have a significant adverse effect on the supply, distribution, or use of energy. Consequently, FRA has determined that this final rule is not a "significant energy action" within the meaning of the Executive Order.

#### H. Privacy Act Statement

Anyone is able to search the electronic form of all comments received into any of DOT's dockets by the name of the individual submitting the comment (or signing the comment, if submitted on behalf of an association, business, labor union, etc). You may review DOT's complete Privacy Act Statement published in the **Federal Register** on April 11, 2000 (Volume 65, Number 70, Pages 19477–78), or you may visit *http://DocketsInfo.dot.gov.* 

# List of Subjects

#### 49 CFR Part 213

Penalties, Railroad safety, Reporting and recordkeeping requirements.

#### 49 CFR Part 237

Penalties, Railroad safety, Bridge safety, Reporting and recordkeeping requirements.

## The Rule

■ In consideration of the foregoing, FRA amends chapter II, subtitle B, of title 49, Code of Federal Regulations by removing appendix C to part 213 and adding part 237 as follows:

# PART 213-[AMENDED]

■ 1. The authority citation for part 213 continues to read as follows:

Authority: 49 U.S.C. 20102–20114 and 20142; 28 U.S.C. 2461, note; and 49 CFR 1.49.

#### Appendix C to Part 213—[Removed]

■ 2. In part 213, remove appendix C.

■ 3. Add part 237 to read as follows:

# PART 237—BRIDGE SAFETY STANDARDS

#### Subpart A—General

- Sec.
- 237.1 Application.
- 237.3 Responsibility for compliance.
- 237.5 Definitions.
- 237.7 Penalties.
- 237.9 Waivers.

#### Subpart B—Railroad Bridge Safety Assurance

- 237.31 Adoption of bridge management programs.
- 237.33 Content of bridge management programs.

#### Subpart C—Qualifications and Designations of Responsible Persons

- 237.51 Railroad bridge engineers.
- 237.53 Railroad bridge inspectors.
- 237.55 Railroad bridge supervisors.
- 237.57 Designation of individuals.

#### Subpart D—Capacity of Bridges

- 237.71 Determination of bridge load capacities.
- 237.73 Protection of bridges from overweight and over-dimension loads.

#### Subpart E—Bridge Inspection

- 237.101 Scheduling of bridge inspections.
- 237.103 Bridge inspection procedures.
- 237.105 Special inspections.
- 237.107 Conduct of bridge inspections.
- 237.109 Bridge inspection records.
- 237.111 Review of bridge inspection reports.

#### Subpart F—Repair and Modification of Bridges

- 237.131 Design.
- 237.133 Supervision of repairs and modifications.

# Subpart G—Documentation, Records, and Audits of Bridge Management Programs

- 237.151 Audits; general.
- 237.153 Audits of inspections.
- 237.155 Documents and records.
- Appendix A—Supplemental Statement of Agency Policy on the Safety of Railroad Bridges
- Appendix B—Schedule of Civil Penalties

Authority: 49 U.S.C. 20102–20114; P.L. 110–432, division A, section 417; 28 U.S.C. 2461, note; and 49 CFR 1.49.

#### Subpart A—General

### §237.1 Application.

(a) Except as provided in paragraphs (b) or (c) of this section, this part applies to all owners of railroad track with a gage of two feet or more and which is supported by a bridge.

(b) This part does not apply to bridges on track used exclusively for rapid transit operations in an urban area that are not connected with the general railroad system of transportation.

(c) This part does not apply to bridges located within an installation which is not part of the general railroad system of transportation and over which trains are not operated by a railroad.

#### §237.3 Responsibility for compliance.

(a) Except as provided in paragraph (b) of this section, an owner of track to which this part applies is responsible for compliance.

(b) If an owner of track to which this part applies assigns responsibility for the bridges that carry the track to another person (by lease or otherwise), written notification of the assignment shall be provided to the appropriate FRA Regional Office at least 30 days in advance of the assignment. The notification may be made by any party to that assignment, but shall be in writing and include the following—

(1) The name and address of the track owner;(2) The name and address of the

person to whom responsibility is assigned (assignee);

(3) A statement of the exact relationship between the track owner and the assignee;

(4) A precise identification of the track segment and the individual bridges in the assignment;

(5) A statement as to the competence and ability of the assignee to carry out the bridge safety duties of the track owner under this part; and

(6) A statement signed by the assignee acknowledging the assignment to him of responsibility for purposes of compliance with this part.

(c) The Administrator may hold the track owner or the assignee, or both, responsible for compliance with this part and subject to penalties under § 237.7.

(d) A common carrier by railroad which is directed by the Surface Transportation Board to provide service over the track of another railroad under 49 U.S.C. 11123 is considered the owner of that track for the purposes of the application of this part during the period the directed service order remains in effect.

(e) When any person, including a contractor for a railroad or track owner,

performs any function required by this part, that person is required to perform that function in accordance with this part.

(f) Where an owner of track to which this part applies has previously assigned responsibility for a segment of track to another person as prescribed in 49 CFR 213.5(c), additional notification to FRA is not required.

(g) FRA reserves the right to reject an assignment of responsibility under § 237.3(b) for cause shown.

# §237.5 Definitions.

For the purposes of this part— *Bridge modification* means a change to the configuration of a railroad bridge that affects the load capacity of the bridge.

*Bridge repair* means remediation of damage or deterioration which has affected the structural integrity of a railroad bridge.

Railroad bridge means any structure with a deck, regardless of length, which supports one or more railroad tracks, or any other undergrade structure with an individual span length of 10 feet or more located at such a depth that it is affected by live loads.

*Track owner* means a person responsible for compliance in accordance with § 237.3.

#### §237.7 Penalties.

(a) Any person who violates any requirement of this part or causes the violation of any such requirement is subject to a civil penalty of at least \$650 and not more than \$25,000 per violation, except that: Penalties may be assessed against individuals only for willful violations, and, where a grossly negligent violation or a pattern of repeated violations has created an imminent hazard of death or injury to persons, or has caused death or injury, a penalty not to exceed \$100,000 per violation may be assessed. "Person" means an entity of any type covered under 1 U.S.C. 1, including but not limited to the following: A railroad; a manager, supervisor, official, or other employee or agent of a railroad; any owner, manufacturer, lessor, or lessee of railroad equipment, track, or facilities; any independent contractor providing goods or services to a railroad; any employee of such owner, manufacturer, lessor, lessee, or independent contractor; and anyone held by the Administrator of the Federal Railroad Administration to be responsible under §237.3(d). Each day a violation continues shall constitute a separate offense. See Appendix B to this part for a statement of agency civil penalty policy.

(b) Any person who knowingly and willfully falsifies a record or report required by this part may be subject to criminal penalties under 49 U.S.C. 21311.

# §237.9 Waivers.

(a) Any person subject to a requirement of this part may petition the Administrator for a waiver of compliance with such requirement. The filing of such a petition does not affect that person's responsibility for compliance with that requirement while the petition is being considered.

(b) Each petition for waiver must be filed in the manner and contain the information required by part 211 of this chapter.

(c) If the Administrator finds that a waiver of compliance is in the public interest and is consistent with railroad safety, the Administrator may grant the waiver subject to any conditions the Administrator deems necessary. If a waiver is granted, the Administrator publishes a notice in the Federal Register containing the reasons for granting the waiver.

## Subpart B—Railroad Bridge Safety Assurance

# §237.31 Adoption of bridge management programs.

Each track owner shall adopt a bridge safety management program to prevent the deterioration of railroad bridges by preserving their capability to safely carry the traffic to be operated over them, and reduce the risk of human casualties, environmental damage, and disruption to the Nation's railroad transportation system that would result from a catastrophic bridge failure, not later than the dates in the following schedule:

(a) March 14, 2011: Class I carriers; (b) March 14, 2011: Owners of track segments which are part of the general railroad system of transportation and which carry more than ten scheduled passenger trains per week;

(c) September 13, 2011: Class II carriers to which paragraph (b) of this section does not apply; and

(d) September 13, 2012: All other track owners subject to this part and not described paragraphs (a) through (c) of this section.

# §237.33 Content of bridge management programs.

Each bridge management program adopted in compliance with this part shall include, as a minimum, the following:

(a) An accurate inventory of railroad bridges, which shall include a unique identifier for each bridge, its location, configuration, type of construction, number of spans, span lengths, and all other information necessary to provide for the management of bridge safety;

(b) A record of the safe load capacity of each bridge;

(c) A provision to obtain and maintain the design documents of each bridge if available, and to document all repairs, modifications, and inspections of each bridge; and

(d) A bridge inspection program covering as a minimum:

(1) Inspection personnel safety considerations;

(2) Types of inspection including required detail;

(3) Definitions of defect levels along with associated condition codes if condition codes are used;

(4) The method of documenting inspections including standard forms or formats;

(5) Structure type and component nomenclature; and

(6) Numbering or identification protocol for substructure units, spans, and individual components.

# Subpart C—Qualifications and Designations of Responsible Persons

# §237.51 Railroad bridge engineers.

(a) A railroad bridge engineer shall be a person who is determined by the track owner to be competent to perform the following functions as they apply to the particular engineering work to be performed:

(1) Determine the forces and stresses in railroad bridges and bridge components;

(2) Prescribe safe loading conditions for railroad bridges;

(3) Prescribe inspection and

maintenance procedures for railroad bridges; and

(4) Design repairs and modifications to railroad bridges.

(b) The educational qualifications of a railroad bridge engineer shall include either:

(1) A degree in engineering granted by a school of engineering with at least one program accredited by ABET, Inc. or its successor organization as a professional engineering curriculum, or a degree from a program accredited as a professional engineering curriculum by a foreign organization recognized by ABET, Inc. or its successor; or

(2) Current registration as a professional engineer.

(c) Nothing in this part affects the States' authority to regulate the professional practice of engineering.

# §237.53 Railroad bridge inspectors.

A railroad bridge inspector shall be a person who is determined by the track

owner to be technically competent to view, measure, report and record the condition of a railroad bridge and its individual components which that person is designated to inspect. An inspector shall be designated to authorize or restrict the operation of railroad traffic over a bridge according to its immediate condition or state of repair.

# §237.55 Railroad bridge supervisors.

A railroad bridge supervisor shall be a person, regardless of position title, who is determined by the track owner to be technically competent to supervise the construction, modification or repair of a railroad bridge in conformance with common or particular specifications, plans and instructions applicable to the work to be performed, and to authorize or restrict the operation of railroad traffic over a bridge according to its immediate condition or state of repair.

# §237.57 Designations of individuals.

Each track owner shall designate those individuals qualified as railroad bridge engineers, railroad bridge inspectors and railroad bridge supervisors. Each individual designation shall include the basis for the designation in effect and shall be recorded.

#### Subpart D—Capacity of Bridges

# §237.71 Determination of bridge load capacities.

(a) Each track owner shall determine the load capacity of each of its railroad bridges. The load capacity need not be the ultimate or maximum load capacity, but must be a safe load capacity.

(b) The load capacity of each bridge shall be documented in the track owner's bridge management program, together with the method by which the capacity was determined.

(c) The determination of load capacity shall be made by a railroad bridge engineer using appropriate engineering methods and standards that are particularly applicable to railroad bridges.

(d) Bridge load capacity may be determined from existing design and modification records of a bridge, provided that the bridge substantially conforms to its recorded configuration. Otherwise, the load capacity of a bridge shall be determined by measurement and calculation of the properties of its individual components, or other methods as determined by a railroad bridge engineer.

(e) If a track owner has a group of bridges for which the load capacity has not already been determined, the owner shall schedule the evaluation of those bridges according to their relative priority, as established by a railroad bridge engineer. The initial determination of load capacity shall be completed not later than five years following the required date for adoption of the track owner's bridge management program in conformance with § 237.31.

(f) Where a bridge inspection reveals that, in the determination of the railroad bridge engineer, the condition of a bridge or a bridge component might adversely affect the ability of the bridge to carry the traffic being operated, a new capacity shall be determined.

(g) Bridge load capacity may be expressed in terms of numerical values related to a standard system of bridge loads, but shall in any case be stated in terms of weight and length of individual or combined cars and locomotives, for the use of transportation personnel.

(h) Bridge load capacity may be expressed in terms of both normal and maximum load conditions. Operation of equipment that produces forces greater than the normal capacity shall be subject to any restrictions or conditions that may be prescribed by a railroad bridge engineer.

# §237.73 Protection of bridges from overweight and over-dimension loads.

(a) Each track owner shall issue instructions to the personnel who are responsible for the configuration and operation of trains over its bridges to prevent the operation of cars, locomotives and other equipment that would exceed the capacity or dimensions of its bridges.

(b) The instructions regarding weight shall be expressed in terms of maximum equipment weights, and either minimum equipment lengths or axle spacing.

(c) The instructions regarding dimensions shall be expressed in terms of feet and inches of cross section and equipment length, in conformance with common railroad industry practice for reporting dimensions of exceptional equipment in interchange in which height above top-of-rail is shown for each cross section measurement, followed by the width of the car of the shipment at that height.

(d) The instructions may apply to individual structures, or to a defined line segment or group(s) of line segments where the published capacities and dimensions are within the limits of all structures on the subject line segments.

### Subpart E—Bridge Inspection

# §237.101 Scheduling of bridge inspections.

(a) Each bridge management program shall include a provision for scheduling an inspection for each bridge in railroad service at least once in each calendar year, with not more than 540 days between any successive inspections.

(b) A bridge shall be inspected more frequently than provided for in the bridge management program when a railroad bridge engineer determines that such inspection frequency is necessary considering conditions noted on prior inspections, the type and configuration of the bridge, and the weight and frequency of traffic carried on the bridge.

(c) Each bridge management program shall define requirements for the special inspection of a bridge to be performed whenever the bridge is involved in an event which might have compromised the integrity of the bridge, including but not limited to a flood, fire, earthquake, derailment or vehicular or vessel impact.

(d) Any railroad bridge that has not been in railroad service and has not been inspected in accordance with this section within the previous 540 days shall be inspected and the inspection report reviewed by a railroad bridge engineer prior to the resumption of railroad service.

# §237.103 Bridge inspection procedures.

(a) Each bridge management program shall specify the procedure to be used for inspection of individual bridges or classes and types of bridges.

(b) The bridge inspection procedures shall be as specified by a railroad bridge engineer who is designated as responsible for the conduct and review of the inspections. The inspection procedures shall incorporate the methods, means of access, and level of detail to be recorded for the various components of that bridge or class of bridges.

(c) The bridge inspection procedures shall ensure that the level of detail and the inspection procedures are appropriate to: the configuration of the bridge; conditions found during previous inspections; the nature of the railroad traffic moved over the bridge (including equipment weights, train frequency and length, levels of passenger and hazardous materials traffic); and vulnerability of the bridge to damage.

(d) The bridge inspection procedures shall be designed to detect, report and protect deterioration and deficiencies before they present a hazard to safe train operation.

# §237.105 Special inspections.

(a) Each bridge management program shall prescribe a procedure for protection of train operations and for inspection of any bridge that might have been damaged by a natural or accidental event, including but not limited to a flood, fire, earthquake, derailment or vehicular or vessel impact.

(b) Each bridge management program shall provide for the detection of scour or deterioration of bridge components that are submerged, or that are subject to water flow.

# §237.107 Conduct of bridge inspections.

Bridge inspections shall be conducted under the direct supervision of a designated railroad bridge inspector, who shall be responsible for the accuracy of the results and the conformity of the inspection to the bridge management program.

# §237.109 Bridge inspection records.

(a) Each track owner to which this part applies shall keep a record of each inspection required to be performed on those bridges under this part.

(b) Each record of an inspection under the bridge management program prescribed in this part shall be prepared from notes taken on the day(s) the inspection is made, supplemented with sketches and photographs as needed. Such record will be dated with the date(s) the physical inspection takes place and the date the record is created, and it will be signed or otherwise certified by the person making the inspection.

(c) Each bridge management program shall specify that every bridge inspection report shall include, as a minimum, the following information:

(1) A precise identification of the bridge inspected;

(2) The date on which the physical inspection was completed;

(3) The identification and written or electronic signature of the inspector;

(4) The type of inspection performed, in conformance with the definitions of inspection types in the bridge management program;

(5) An indication on the report as to whether any item noted thereon requires expedited or critical review by a railroad bridge engineer, and any restrictions placed at the time of the inspection;

(6) The condition of components inspected, which may be in a condition reporting format prescribed in the bridge management program, together with any narrative descriptions necessary for the correct interpretation of the report; and

(7) When an inspection does not encompass the entire bridge, the portions of the bridge which were inspected shall be identified in the report.

(d) An initial report of each bridge inspection shall be placed in the location designated in the bridge management program within 30 calendar days of the completion of the inspection unless the complete inspection report is filed first. The initial report shall include the information required by paragraphs (c)(1) through (c)(5) of this section.

(e) A complete report of each bridge inspection, including as a minimum the information required in paragraphs (c)(1) through (c)(6) of this section, shall be placed in the location designated in the bridge management program within 120 calendar days of the completion of the inspection.

(f) Each bridge inspection program shall specify the retention period and location for bridge inspection records. The retention period shall be no less than two years following the completion of the inspection. Records of underwater inspections shall be retained until the completion and review of the next underwater inspection of the bridge.

(g) If a bridge inspector, supervisor, or engineer discovers a deficient condition on a bridge that affects the immediate safety of train operations, that person shall report the condition as promptly as possible to the person who controls the operation of trains on the bridge in order to protect the safety of train operations.

# §237.111 Review of bridge inspection reports.

Bridge inspection reports shall be reviewed by railroad bridge supervisors and railroad bridge engineers to:

(a) Determine whether inspections have been performed in accordance with the prescribed schedule and specified procedures;

(b) Evaluate whether any items on the report represent a present or potential hazard to safety;

(c) Prescribe any modifications to the inspection procedures or frequency for that particular bridge;

(d) Schedule any repairs or modifications to the bridge required to maintain its structural integrity; and

(e) Determine the need for further higher-level review.

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# Subpart F—Repair and Modification of Bridges

# §237.131 Design.

Each repair or modification which materially modifies the capacity of a bridge or the stresses in any primary load-carrying component of a bridge shall be designed by a railroad bridge engineer. The design shall specify the manner in which railroad traffic or other live loads may be permitted on the bridge while it is being modified or repaired. Designs and procedures for repair or modification of bridges of a common configuration, such as timber trestles, or instructions for in-kind replacement of bridge components, may be issued as a common standard. Where the common standard addresses procedures and methods that could materially modify the capacity of a bridge or the stresses in any primary load-carrying component of a bridge, the standard shall be designed and issued by a qualified railroad bridge engineer.

# §237.133 Supervision of repairs and modifications.

Each repair or modification pursuant to this part shall be performed under the immediate supervision of a railroad bridge supervisor as defined in § 237.55 of this part who is designated and authorized by the track owner to supervise the particular work to be performed. The railroad bridge supervisor shall ensure that railroad traffic or other live loads permitted on the bridge under repair or modification are in conformity with the specifications in the design.

# Subpart G—Documentation, Records, and Audits of Bridge Management Programs

#### §237.151 Audits; general.

Each program adopted to comply with this part shall include provisions for auditing the effectiveness of the several provisions of that program, including the validity of bridge inspection reports and bridge inventory data, and the correct application of movement restrictions to railroad equipment of exceptional weight or configuration.

## §237.153 Audits of inspections.

(a) Each bridge management program shall incorporate provisions for an internal audit to determine whether the inspection provisions of the program are being followed, and whether the program itself is effectively providing for the continued safety of the subject bridges.

(b) The inspection audit shall include an evaluation of a representative sampling of bridge inspection reports at the bridges noted on the reports to determine whether the reports accurately describe the condition of the bridge.

# §237.155 Documents and records.

Each track owner required to implement a bridge management program and keep records under this part shall make those program documents and records available for inspection and reproduction by the Federal Railroad Administration.

(a) *Electronic recordkeeping; general.* For purposes of compliance with the recordkeeping requirements of this part, a track owner may create and maintain any of the records required by this part through electronic transmission, storage, and retrieval provided that all of the following conditions are met:

(1) The system used to generate the electronic record meets all requirements of this subpart;

(2) The electronically generated record contains the information required by this part;

(3) The track owner monitors its electronic records database through sufficient number of monitoring indicators to ensure a high degree of accuracy of these records;

(4) The track owner shall train its employees who use the system on the proper use of the electronic recordkeeping system; and

(5) The track owner maintains an information technology security program adequate to ensure the integrity of the system, including the prevention of unauthorized access to the program logic or individual records.

(b) *System security.* The integrity of the bridge inspection records must be protected by a security system that incorporates a user identity and password, or a comparable method, to establish appropriate levels of program and record data access meeting all of the following standards:

(1) No two individuals have the same electronic identity;

(2) A record cannot be deleted or altered by any individual after the record is certified by the employee who created the record:

(3) Any amendment to a record is either—

(i) Electronically stored apart from the record that it amends; or

(ii) Electronically attached to the record as information without changing the original record;

(4) Each amendment to a record uniquely identifies the person making the amendment; and

(5) The electronic system provides for the maintenance of inspection records as originally submitted without corruption or loss of data.

# Appendix A to Part 237—Supplemental Statement of Agency Policy on the Safety of Railroad Bridges

A Statement of Agency Policy on the Safety of Railroad Bridges was originally published by FRA in 2000 as Appendix C of the Federal Track Safety Standards, 49 CFR Part 213. With the promulgation of 49 CFR Part 237, Bridge Safety Standards, many of the nonregulatory provisions in that Policy Statement have been incorporated into the bridge safety standards in this part.

However, FRA has determined that other non-regulatory items are still useful as information and guidance for track owners. Those provisions of the Policy Statement are therefore retained and placed in this Appendix in lieu of their former location in the Track Safety Standards.

#### General

1. The structural integrity of bridges that carry railroad tracks is important to the safety of railroad employees and to the public. The responsibility for the safety of railroad bridges is specified in § 237.3, "Responsibility for compliance."

2. The capacity of a bridge to safely support its traffic can be determined only by intelligent application of engineering principles and the law of physics. Track owners should use those principles to assess the integrity of railroad bridges.

3. The long term ability of a structure to perform its function is an economic issue beyond the intent of this policy. In assessing a bridge's structural condition, FRA focuses on the present safety of the structure, rather than its appearance or long term usefulness.

4. FRA inspectors conduct regular evaluations of railroad bridge inspection and management practices. The objective of these evaluations is to document the practices of the evaluated railroad, to disclose any program weaknesses that could affect the safety of the public or railroad employees, and to assure compliance with the terms of this regulation. If the evaluation discloses problems, FRA seeks a cooperative resolution. If safety is jeopardized by a track owner's failure to resolve a bridge problem, FRA will use appropriate measures, including assessing civil penalties and issuance of emergency orders, to protect the safety of railroad employees and the public.

5. This policy statement addresses the integrity of bridges that carry railroad tracks. It does not address the integrity of other types of structures on railroad property (i.e., tunnels, highway bridges over railroads, or other structures on or over the right-of-way).

6. The guidelines published in this statement are advisory. They do not have the force of regulations or orders, which FRA may enforce using civil penalties or other means. The guidelines supplement the requirements of part 237 and are retained for information and guidance.

#### Guidelines

1. Responsibility for safety of railroad bridges.

(a) The responsibility for the safety of railroad bridges is specified in § 237.3.

(b) The track owner should maintain current information regarding loads that may be operated over the bridge, either from its own engineering evaluations or as provided by a competent engineer representing the track owner. Information on permissible loads may be communicated by the track owner either in terms of specific car and locomotive configurations and weights, or as values representing a standard railroad bridge rating reference system. The most common standard bridge rating reference system incorporated in the Manual for Railway Engineering of the American Railway Engineering and Maintenance-of-Way Association is the dimensional and proportional load configuration devised by Theodore Cooper. Other reference systems may be used where convenient, provided their effects can be defined in terms of shear, bending and pier reactions as necessary for a comprehensive evaluation and statement of the capacity of a bridge.

(c) The owner of the track on a bridge should advise other railroads operating on that track of the maximum loads permitted on the bridge stated in terms of car and locomotive configurations and weights. No railroad should operate a load which exceeds those limits without specific authority from, and in accordance with restrictions placed by, the track owner.

2. Capacity of railroad bridges.

(a) The safe capacity of bridges should be determined pursuant to § 237.71.

(b) Proper analysis of a bridge requires knowledge of the actual dimensions, materials and properties of the structural members of the bridge, their condition, and the stresses imposed in those members by the service loads.

(c) The factors which were used for the design of a bridge can generally be used to determine and rate the load capacity of a bridge provided:

(i) The condition of the bridge has not changed significantly; and

(ii) The stresses resulting from the service loads can be correlated to the stresses for which the bridge was designed or rated.

3. Railroad bridge loads.

(a) Control of loads is governed by §237.73.

(b) Authority for exceptions. Equipment exceeding the nominal weight restriction on a bridge should be operated only under conditions determined by a competent railroad bridge engineer who has properly analyzed the stresses resulting from the proposed loads and has determined that the proposed operation can be conducted safely without damaging the bridge.

(c) Operating conditions. Operating conditions for exceptional loads may include speed restrictions, restriction of traffic from adjacent multiple tracks, and weight limitations on adjacent cars in the same train.

4. Railroad bridge records.

(a) The organization responsible for the safety of a bridge should keep design, construction, maintenance and repair records readily accessible to permit the determination of safe loads. Having design or rating drawings and calculations that conform to the actual structure greatly simplifies the process of making accurate determinations of safe bridge loads. This provision is governed by § 237.33.

(b) Organizations acquiring railroad property should obtain original or usable copies of all bridge records and drawings, and protect or maintain knowledge of the location of the original records.

5. Specifications for design and rating of railroad bridges.

(a) The recommended specifications for the design and rating of bridges are those found in the Manual for Railway Engineering published by the American Railway Engineering and Maintenance-of-Way Association. These specifications incorporate recognized principles of structural design and analysis to provide for the safe and economic utilization of railroad bridges during their expected useful lives. These specifications are continually reviewed and revised by committees of competent engineers. Other specifications for design and rating, however, have been successfully used by some railroads and may continue to be suitable.

(b) A bridge can be rated for capacity according to current specifications regardless of the specification to which it was originally designed.

6. Periodic inspections of railroad bridges. (a) Periodic bridge inspections by competent inspectors are necessary to determine whether a structure conforms to its design or rating condition and, if not, the degree of nonconformity. See § 237.101. Section 237.101(a) calls for every railroad bridge to be inspected at least once in each calendar year. Deterioration or damage may occur during the course of a year regardless of the level of traffic that passes over a bridge. Inspections at more frequent intervals may be required by the nature or condition of a structure or intensive traffic levels.

7. Underwater inspections of railroad bridges.

(a) Inspections of bridges should include measuring and recording the condition of substructure support at locations subject to erosion from moving water.

(b) Stream beds often are not visible to the inspector. Indirect measurements by sounding, probing, or any other appropriate means are necessary in these cases. A series of records of these readings will provide the best information in the event unexpected changes suddenly occur. Where such indirect measurements do not provide the necessary assurance of foundation integrity, diving inspections should be performed as prescribed by a competent engineer.

8. Seismic considerations.

(a) Owners of bridges should be aware of the risks posed by earthquakes in the areas in which their bridges are located. Precautions should be taken to protect the safety of trains and the public following an earthquake.

(b) Contingency plans for seismic events should be prepared in advance, taking into account the potential for seismic activity in an area.

(c) The predicted attenuation of ground motion varies considerably within the United States. Local ground motion attenuation values and the magnitude of an earthquake both influence the extent of the area affected by an earthquake. Regions with low frequency of seismic events produce less data

from which to predict attenuation factors. That uncertainty should be considered when designating the area in which precautions should be taken following the first notice of an earthquake. In fact, earthquakes in such regions might propagate their effects over much wider areas than earthquakes of the same magnitude occurring in regions with frequent seismic activity.

9. Special inspections of railroad bridges. Requirements for special inspections of railroad bridges are found in § 237.105.

10. Railroad bridge inspection records.

(a) The requirements for recording and reporting bridge inspections are found in § 237.109.

(b) Information from bridge inspection reports should be incorporated into a bridge management program to ensure that exceptions on the reports are corrected or accounted for. A series of inspection reports prepared over time should be maintained so as to provide a valuable record of trends and rates of degradation of bridge components. The reports should be structured to promote comprehensive inspections and effective communication between an inspector and an engineer who performs an analysis of a bridge.

(c) An inspection report should be comprehensible to a competent person without interpretation by the reporting inspector.

11. Railroad bridge inspectors and engineers.

(a) Bridge inspections should be performed by technicians whose training and experience enable them to detect and record indications of distress on a bridge. Inspectors should provide accurate measurements and other information about the condition of the bridge in enough detail so that an engineer can make a proper evaluation of the safety of the bridge. Qualifications of personnel are addressed in subpart C to part 237.

(b) Accurate information about the condition of a bridge should be evaluated by an engineer who is competent to determine the capacity of the bridge. The inspector and the evaluator often are not the same individual; therefore, the quality of the bridge evaluation depends on the quality of the communication between them. Review of inspection reports is addressed in §237.111.

12. Scheduling inspections.

(a) A bridge management program should include a means to ensure that each bridge under the program is inspected at the frequency prescribed for that bridge by a competent engineer. Scheduling of bridge inspections is addressed in § 237.101.

(b) Bridge inspections should be scheduled from an accurate bridge inventory list that includes the due date of the next inspection.

13. Special considerations for railroad bridges.

Railroad bridges differ from other types of bridges in the types of loads they carry, in their modes of failure and indications of distress, and in their construction details and components. Proper inspection and analysis of railroad bridges require familiarity with the loads, details and indications of distress that are unique to this class of structure. Particular care should be taken that modifications to railroad bridges, including

retrofits for protection against the effects of earthquakes, are suitable for the structure to which they are to be applied. Modifications should not adversely affect the serviceability of neither the bridge nor its accessibility for periodic or special inspection.

14. Railroad implementation of bridge safety programs.

FRA recommends that each track owner or other entity which is responsible for the integrity of bridges which support its track should comply with the intent of this regulation by adopting and implementing an effective and comprehensive program to ensure the safety of its bridges. The bridge safety program should incorporate the following essential elements, applied according to the configuration of the railroad and its bridges. The basis of the program should be in one comprehensive and coherent document which is available to all railroad personnel and other persons who are responsible for the application of any portion of the program. The program should include:

(a) Clearly defined roles and responsibilities of all persons who are designated or authorized to make determinations regarding the integrity of the track owner's bridges. The designations may be made by position or by individual;

(b) Provisions for a complete inventory of bridges that carry the owner's track, to include the following information on each bridge:

(1) A unique identifier, such as milepost location and a subdivision code;

(2) The location of the bridge by nearest town or station, and geographic coordinates;

(3) The name of the geographic features crossed by the bridge;

(4) The number of tracks on the bridge;

(5) The number of spans in the bridge;

(6) The lengths of the spans:

(7) Types of construction of:

(i) Substructure;

(ii) Superstructure; and

(iii) Deck;

(8) Overall length of the bridge;

(9) Dates of: (i) Construction;

(ii) Major renovation; and

(iii) Strengthening; and

(10) Identification of entities responsible

for maintenance of the bridge or its different components.

(c) Known capacity of its bridges as determined by rating by competent railroad bridge engineer or by design documents;

(d) Procedures for the control of movement of high, wide or heavy loads exceeding the nominal capacity of bridges;

(e) Instructions for the maintenance of permanent records of design, construction, modification, and repair;

(f) Railroad-specific procedures and standards for design and rating of bridges; (g) Detailed bridge inspection policy,

including:

(1) Inspector qualifications; including: (i) Bridge experience or appropriate educational training;

(iii) Training on Railroad Workplace Safety; and

(ii) Training on bridge inspection

(2) Type and frequency of inspection; including:

(i) Periodic (at least annually);

- (ii) Underwater;
- (iii) Special;

procedures; and

(iv) Seismic; and

(v) Cursory inspections of overhead bridges that are not the responsibility of the railroad;

(3) Inspection schedule for each bridge;

(4) Documentation of inspections; including:

(i) Date;

(ii) Name of inspector;

(iii) Reporting Format; and

(iv) Coherence of information;

(5) Inspection Report Review Process;

(6) Record retention; and

(7) Tracking of critical deficiencies to resolution; and

(h) Provide for the protection of train operations following an inspection, noting a critical deficiency, repair, modification or adverse event and should include:

(1) A listing of qualifications of personnel permitted to authorize train operations following an adverse event; and

(2) Detailed internal program audit

procedures to ensure compliance with the provisions of the program.

#### Appendix B to Part 237—Schedule of **Civil Penalties**

APPENDIX B TO PART 237—SCHEDULE OF CIVIL PENALTIES<sup>1</sup>

Section <sup>2</sup>	Violation	Willful violation							
Subpart B—Railroad Bridge Safety Assurance									
237.31 Adoption of bridge management program	\$9,500	\$17,000							
(a) Inventory of railroad bridges	2,500	5,000							
(b) Record of safe load capacity	5,500	10,000							
(c) Provision to obtain and maintain:									
(i) Design documents	5,500	10,000							
(ii) Documentation of repairs and modifications	2,500	5,000							
(iii) Inspection reports	2,500	5,000							
(d) Bridge inspection program content	2,500	5,000							
Subpart C—Qualification and Designation of Responsible Persons		1							
237.51 Railroad bridge engineers:									

Lerier riamead brage engineerer		1
(a) Competency	5,500	10,000
(b) Educational qualification	2,500	5,000
237.53 Railroad bridge inspectors	5,500	10,000
237.55 Railroad bridge supervisors	5,500	10,000
237.57 Designation of individuals	2,500	5,000
-	L	4

# Subpart D—Capacity of Bridges

237.71 Determination of bridge load capacities:		
(a) Safe load capacity	5,500	10,000
(b) Load capacity documented	5,500	10,000
(c) Load capacity determined by a railroad bridge engineer	5,500	10,000
(d) Method of load capacity determination	2,500	5,000
(e) Prioritization of load capacity determination	2,500	5,000
(f) New load capacity determined due to change in condition	2,500	5,000
(g) Load capacity stated in terms of weight and length of equipment	2,500	5,000
(h) Restriction on operations by railroad bridge engineer	5,500	10,000
237.73 Protection of bridges from over-weight and over-dimension equipment:		
(a) Instructions issued	5,500	10,000
(b) Weight instructions	2,500	5,000

# APPENDIX B TO PART 237—SCHEDULE OF CIVIL PENALTIES 1—Continued

Section <sup>2</sup>	Violation	Willful violatior
(c) Dimensional instructions	2,500	5,000
(d) Incorrect instructions issued	2,500	5,000
Subpart E—Bridge Inspection		
237.101 Scheduling of bridge inspections:		
(a) Scheduling:		
(i) Failure to inspect	9,500	17,000
(ii) Inspection within calendar year	2,500	5,000
(iii) Inspection frequency exceeding 540 days	2,500	5,000
(b) Increased inspection frequency	5,500	10,000
(c) Special inspections	2,500	5.000
(d) Resumption of railroad operations prior to inspection & review	9,500	17,000
237.103 Bridge inspection procedures	2,500	5,000
237.105 Special inspections:	2,000	0,000
(a) Procedures to protect train operations and requiring special inspections	2,500	5.000
(b) Provision for the detection of scour or underwater deterioration	2,500	5.000
237.107 Conduct of bridge inspections	5,500	10,000
237.109 Bridge inspection records:	0,000	10,000
(a) Record of inspection	2,500	5,000
(b) Inspection record:	2,000	0,000
(i) Certification and date	2,500	5.000
	2,500	17,000
(c) Inspection record information	2,500	5,000
(d) Initial report within 30 days	2,500	5.000
	2,500	5,000
(e) Final inspection report within 120 calendar days	2,500	5,000
(f) Retention	,	- ,
(g) Prompt reporting of dangerous conditions	5,500	10,000
237.111 Review of bridge inspection reports.	0.500	5.00
(a) Review by railroad bridge engineers and supervisors	2,500	5,000
(b) Appropriate action concerning present or potential safety hazards	5,500	10,000
(c) Modification of inspection frequency or procedures	2,500	5,000
(d) Scheduling remedial action	2,500	5,000
(e) Higher-level review	2,500	5,000
Subpart F—Repair and Modification of Bridges		
237.131 Design	5,500	10.000
237.133 Supervision of repairs and modifications	5,500	10,000

#### Subpart G—Documentation, Records and Audits of Bridge Management Programs

237.151 Audits; general	2,500 2,500	5,000 5,000
237.155 Documents and records:		
(a) Electronic recordkeeping, general	2,500	5,000
(b) System security	2,500	5,000

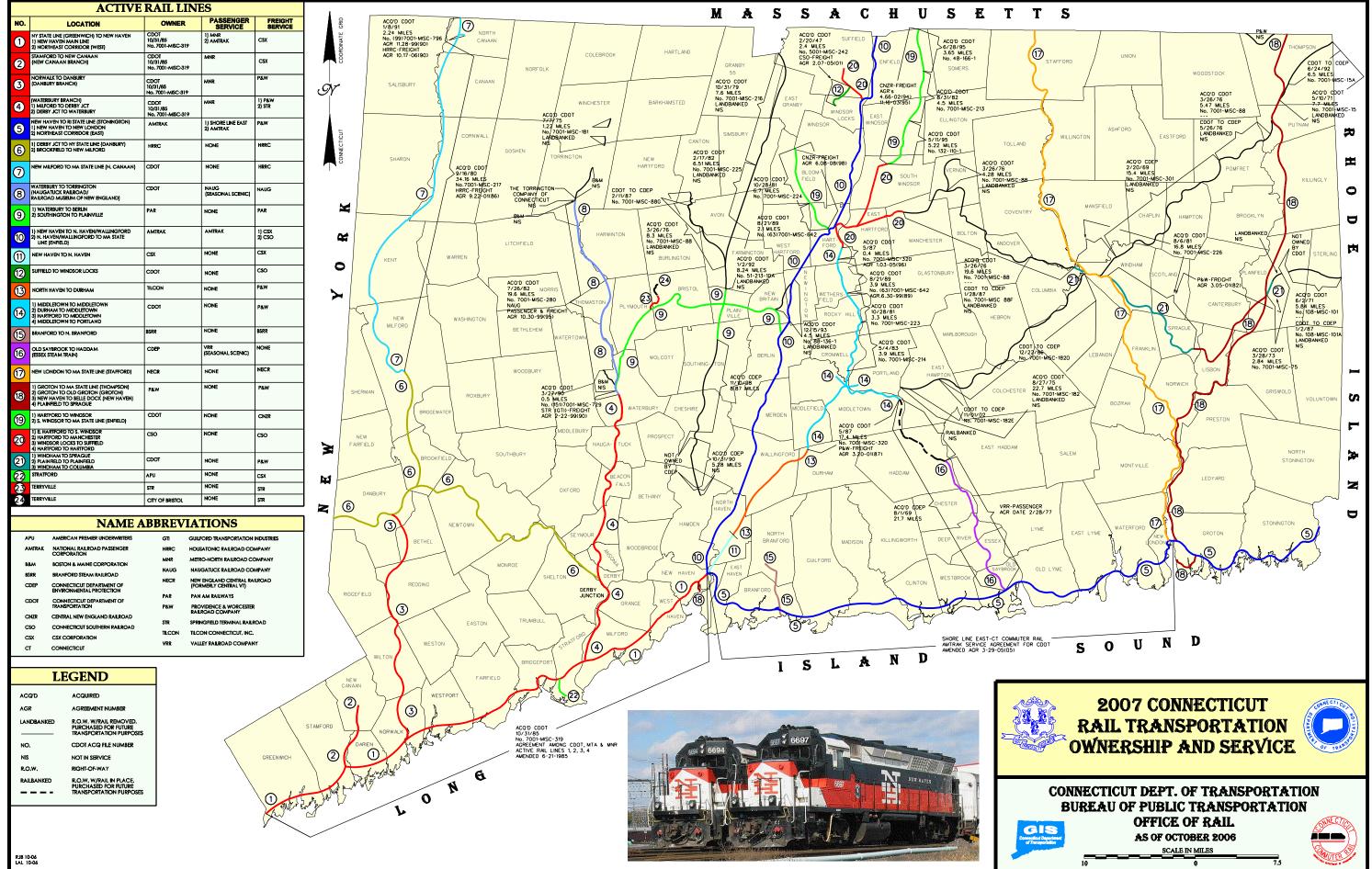
<sup>1</sup>A penalty may be assessed against an individual only for a willful violation. The Administrator reserves the right to assess a penalty of up to \$100,000 for any violation where circumstances warrant. See 49 CFR part 209, appendix A. <sup>2</sup>The penalty schedule uses section numbers from 49 CFR part 237. If more than one item is listed as a type of violation of a given section, each item is also designated by a "penalty code," which is used to facilitate assessment of civil penalties, and which may or may not correspond to any subsection designation(s). For convenience, penalty citations will cite the CFR section and the penalty code, if any. FRA reserves the right, should litigation become necessary, to substitute in its complaint the CFR citation in place of the combined CFR and penalty code citation, characterized as a structure of the combined CFR and penalty code citation. should they differ.

Issued in Washington, DC, on July 7, 2010. Joseph C. Szabo, Administrator, Federal Railroad Administration. [FR Doc. 2010–16929 Filed 7–14–10; 8:45 am]

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Version: 1.0 - February, 2012

# APPENDIX 2 2007 CONNECTICUT RAIL TRANSPORTATION OWNERSHIP AND SERVICE MAP



Version: 1.0 - February, 2012

# APPENDIX 3 INVENTORY OF RAILROAD BRIDGES METRO-NORTH RAILROAD BRIDGES

# State of Connecticut-Department of Transportation-Office of Rails Inventory of Metro-North Railroad Bridges 06-13-11

Track Chart Name	Town	Bridge Type	Bridge No.	Location	Mile Point	Deck Area (sqft)	Bridge Structural Type	Deck Type	No. of Tracks		Length of Max Span	Total Length of Bridge	Year Built	Year Rehabilitated
DANBURY	NORWALK	U	04134R	MARSHALL STREET	0.11	530	DG	OD	1	1	49	53	1895	1962
DANBURY	NORWALK	U	08200R	ANN STREET	0.19	2052	DG	OD	3	1	49	55	1895	
DANBURY	NORWALK	U	08225R	REED STREET	0.36	2816	PC	BD	4	1	64	70	2009	
DANBURY	NORWALK	U	08201R	NORWALK RIVER - STEEL GIRDER	1.56	2614	TG	BD	1	2	64	130	UK	1990
DANBURY	NORWALK	U	08202R	NORWALK RIVER - STEEL GIRDER	3.20	2117	TG	OD	1	2	79	150	1905	
DANBURY	WILTON	U	08203R	BROOK - MULTIBEAM DECK	5.12	144	DG	OD	1	1	11	15	1919	
DANBURY	WILTON	U	08204R	STREAM - MULTIBEAM DECK	6.43	360	DG	OD	1	1	35	45	1904	1956
DANBURY	WILTON	U	08205R	NORWALK RIVER - STEEL GIRDER	6.64	495	DG	OD	1	1	46	55	1919	
DANBURY	WILTON	U	08206R	NORWALK RIVER - STEEL GIRDER	8.70	510	DG	OD	1	1	54	60	1896	1956
DANBURY	WILTON	U	08207R	NORWALK RIVER - STEEL GIRDER	9.42	672	DG	OD	1	2	39	86	1904	
DANBURY	WILTON	U	08208R	BROOK - CULVERT MASONRY	9.91	252	SB	BD	1	1	6	12	UK	
DANBURY	WILTON	U	08209R	OLD MILL ROAD	11.01	320	DG	OD	1	1	28	32	1909	
DANBURY	WILTON	U	08210R	NORWALK RIVER - STEEL GIRDER	11.55	1570	DG	OD	1	3	50	161	1896	
DANBURY	WILTON	U	08211R	FACTORY POND	12.17	588	DG	OD	1	1	43	49	1904	1985
DANBURY	REDDING	U	08212R	BRANCHVILLE BROOK	12.83	420	СВ	BD	2	1	6	12	UK	
DANBURY	REDDING	U	08213R	OLD REDDING ROAD	14.16	208	DG	OD	1	1	22	26	1904	1940
DANBURY	REDDING	U	08214R	SIMPAUG TURNPIKE	14.80	199	BA	CS	1	1	14	14	1893	1998
DANBURY	REDDING	U	08215R	UMPAWAUG POND BROOK - STEEL GIRDER	16.41	248	DG	OD	1	1	24	29	1904	1987
DANBURY	REDDING	U	08216R	SAUGATUCK RIVER - STEEL GIRDER	17.09	392	DG	OD	1	1	43	49	1904	1956
DANBURY	BETHEL	U	01020R	GRASSY PLAIN ROAD (ROUTE 53)	19.64	468	DG	OD	1	1	36	41	1909	2004
DANBURY	BETHEL	U	08224R	SYMPAUG BROOK - DOUBLE BARREL MASONRY CULVE	19.79	1036	SB	BD	1	2	3	20	UK	
DANBURY	BETHEL	U	08217R	BROOK - CONCRETE	19.99	390	SB	BD	1	1	4	13	UK	
DANBURY	BETHEL	U	08218R	SYMPAUG BROOK - MULTIBEAM DECK	21.41	216	DG	OD	1	1	18	22	1919	
DANBURY	DANBURY	U	08219R	SYMPAUG BROOK - CONCRETE SLAB	21.52	216	SB	BD	1	1	16	18	1936	
DANBURY	DANBURY	U	08220R	STILL RIVER - STEEL GIRDER	22.39	2520	TG	BD	1	2	67	140	1975	
DANBURY	DANBURY	U	05100R	STILL RIVER - CONCRETE CULVERT	22.94	1680	СВ	BD	1	2	22	46	1975	
DANBURY	DANBURY	U	08223R	STILL RIVER - CONCRETE CULVERT	23.18	8820	СВ	BD	1	2	20	42	1976	
DANBURY	DANBURY	U	04290R	STILL RIVER - CONCRETE CULVERT	23.42	2537	СВ	BD	2	2	20	43	1976	
MAINLINE	GREENWICH	U	03943R	NORTH WATER STREET	26.10	2068	TG	BD	4	1	44	50	1892	2004
MAINLINE	GREENWICH	U	03945R	HAMILTON AVENUE	26.79	1296	TG	BD	4	1	27	33	1893	2004
MAINLINE	GREENWICH	PT	08000R	PEDESTRIAN UNDERPASS AT HAMILTON AVENUE	26.79	684	CA	BD	4	1	8	8	1935	
MAINLINE	GREENWICH	U	03947R	FIELD POINT ROAD	27.77	2160	SA	BD	4	1	40	40	1895	
MAINLINE	GREENWICH	U	03673R	ARCH STREET (SR 742)	28.06	5304	DG	BD	4	1	100	115	1895	2005
MAINLINE	GREENWICH	PO	08002R	PEDESTRIAN OVERPASS FOOTBRIDGE	28.16	385	TT	CS	0	1	61	77	2005	
MAINLINE	GREENWICH	U	03946R	STEAMBOAT ROAD	28.22	3068	DG	OD	4	1	53	59	1895	1940

# State of Connecticut-Department of Transportation-Office of Rails Inventory of Metro-North Railroad Bridges 06-13-11

Track Chart Name	Town	Bridge Type	Bridge No.	Location	Mile Point	Deck Area (sqft)	Bridge Structural Type	Deck Type	No. of Tracks	No. of Spans	Length of Max Span	Total Length of Bridge	Year Built	Year Rehabilitated
MAINLINE	GREENWICH	U	08003R	DAVIS AVENUE #2	28.48	1824	DG	OD	4	1	33	38	1894	
MAINLINE	GREENWICH	U	08004R	INDIAN HARBOR (DAVIS MILL POND)	28.68	3400	DG	OD	4	1	63	68	1895	1943
MAINLINE	GREENWICH	U	08005R	SACHEM ROAD	29.50	2240	DG	OD	4	1	26	32	1893	1936
MAINLINE	GREENWICH	U	08006R	LUKES CROSSING (SOUND SHORE DRIVE)	29.68	1672	DG	OD	4	1	35	38	1893	
MAINLINE	GREENWICH	М	08008R	MIANUS RIVER (COS COB BRIDGE) - MOVABLE	29.90	58262	DT	OD	4	12	122	1089	1904	1989
MAINLINE	GREENWICH	U	08009R	ARCH STREET #2 - MASONRY ARCH	31.03	1330	SA	BD	4	1	14	14	1895	
MAINLINE	GREENWICH	U	08010R	STREAM - MASONRY CULVERT	31.12	1120	SA	BD	4	1	10	10	1890	
MAINLINE	GREENWICH	U	03948R	SOUND BEACH AVENUE	31.29	3416	MB	BD	4	2	27	56	1894	1929
MAINLINE	GREENWICH	U	03955R	TOMAC AVENUE	31.62	1800	TG	OD	4	1	32	36	1895	1943
MAINLINE	STAMFORD	U	03680R	GREENWICH AVENUE	32.81	3465	TG	OD	4	3	40	63	1896	1944
MAINLINE	STAMFORD	U	08011R	RIPPOWAM RIVER - TRUSS	32.85	7280	DT	OD	4	1	120	130	1896	1988
MAINLINE	STAMFORD	U	03683R	WASHINGTON BOULEVARD (STATE ROUTE 493)	32.97	7700	TG	BD	5	2	59	127	1898	2003
MAINLINE	STAMFORD	U	08012R	ATLANTIC STREET	33.19	4580	TG	OD	5	3	44	71	1896	
MAINLINE	STAMFORD	U	03678R	CANAL STREET	33.41	5850	DG	OD	7	1	60	65	1896	
MAINLINE	STAMFORD	U	03686R	ELM STREET	33.75	6144	DG	OD	7	3	37	64	1896	
MAINLINE	STAMFORD	U	02237R	EAST MAIN STREET (U.S. ROUTE 1)	34.17	6825	TG	OD	5	3	71	105	1896	1931
MAINLINE	STAMFORD	U	08013R	HAMILTON AVENUE	34.72	3060	TG	OD	5	2	34	51	1896	1958
MAINLINE	STAMFORD	U	08014R	NOROTON RIVER - MASONRY ARCH	35.58	2646	MP/CA/BA	BD	4	1	27	27	1894	2003
MAINLINE	DARIEN	PO	08015R	NOROTON STATION PEDESTRIAN OVERPASS	36.24	525	TT	CS	0	1	74	75	1972	
MAINLINE	DARIEN	U	08016R	STONY BROOK - MASONRY CULVERT	37.16	812	CA/SA	BD	4	1	10	10	1893	1955
MAINLINE	DARIEN	U	08017R	CUMMINGS BROOK - MASONRY CULVERT	37.36	360	SA	BD	4	1	6	6	1893	
MAINLINE	DARIEN	U	04142R	LEROY AVENUE #1	37.59	1616	DG	OD	4	1	27	32	1894	1993
MAINLINE	DARIEN	U	00316R	BOSTON POST ROAD (U.S. ROUTE 1)	37.82	3840	DG	BD	4	3	56	113	1894	2011
MAINLINE	DARIEN	U	08018R	GOODWIVES RIVER - CONCRETE DECK	38.00	1144	SB	BD	4	1	15	22	1894	1946
MAINLINE	DARIEN	U	08019R	STREAM	38.36									
MAINLINE	DARIEN	U	08020R	TOKENEKE BROOK - 2 STONE CULVERTS	38.53	720	CA	BD	4	1	4	4	2008	2008
MAINLINE	DARIEN	U	08021R	TOKENEKE BROOK #2 - MASONRY CULVERT	38.68	600	SA	BD	4	1	6	6	1893	
MAINLINE	DARIEN	U	04271R	RAYMOND STREET	38.93	1392	DG	OD	4	1	25	30	1894	
MAINLINE	DARIEN	U	08022R	FIVE MILE RIVER - BRICK ARCH	39.07	2795	SA/BA	BD	4	1	26	26	1893	
MAINLINE	NORWALK	U	08023R	DRY STREAM - MASONRY CULVERT	39.08	384	SA	BD	4	1	10	10	1893	
MAINLINE	NORWALK	U	04132R	ROWAYTON AVENUE	39.11	1778	MB	BD	4	1	29	34	1893	2011
MAINLINE	NORWALK	U	08024R	FARM CREEK	39.55									
MAINLINE	NORWALK	U	08025R	FRANKLIN STREET (SPRING STREET)	40.89	3000	TG	OD	4	1	46	52	1896	
MAINLINE	NORWALK	PT	08026R	STATION UNDERPASS	41.02	500	SB	BD	4	1	10	10	1895	1994
MAINLINE	NORWALK	U	08027R	MONROE STREET	41.12	7040	TG	BD	6	3	51	78	1895	2011

# State of Connecticut-Department of Transportation-Office of Rails Inventory of Metro-North Railroad Bridges 06-13-11

Track Chart Name	Town	Bridge Type	Bridge No.	Location	Mile Point	Deck Area (sqft)	Bridge Structural Type	Deck Type	No. of Tracks		Length of Max Span	Total Length of Bridge	Year Built	Year Rehabilitated
MAINLINE	NORWALK	U	03693R	WASHINGTON & MAIN STREETS	41.28	7008	TT	OD	4	1	146	155	1896	1999
MAINLINE	NORWALK	М	04288R	NORTH WATER STREET & NORWALK RIVER - MOVABLE	41.51	33840	DT	OD	4	4	198	564	1896	1985
MAINLINE	NORWALK	U	04131R	FORT POINT STREET	41.79	2000	DG	BD	4	1	36	43	1890	1941
MAINLINE	NORWALK	U	08028R	OSBORNE AVENUE	41.96	2064	TG	OD	4	1	37	43	1894	
MAINLINE	NORWALK	U	03691R	EAST AVENUE	42.14	1968	TG	OD	4	1	39	41	1905	
MAINLINE	NORWALK	U	08029R	BROOK - MASONRY CULVERT	42.26	480	SA/BA	BD	4	1	5	5	UK	1890
MAINLINE	NORWALK	U	03692R	STRAWBERRY HILL AVENUE	42.37	1767	DG	BD	4	1	32	38	1890	1940
MAINLINE	WESTPORT	U	08103R	STREAM - STONE CULVERT	43.50	UK	SB	BD	4	0	3	UK	1890	
MAINLINE	WESTPORT	U	08290R	INDIAN RIVER - TWO CONCRETE PIPES 7' DIAMETER	43.80	3233	CA	BD	4	2	7	27	1997	
MAINLINE	WESTPORT	U	01348R	SAUGATUCK AVENUE (ROUTE 136)	43.97	3024	TG	BD	5	1	41	48	1888	1940
MAINLINE	WESTPORT	U	08030R	SAUGATUCK AVE SIDING										
MAINLINE	WESTPORT	PT	08105R	NEW STATION UNDERPASS	44.12	1021	СВ	BD	4	1	11	13	2007	
MAINLINE	WESTPORT	PT	08031R	STATION UNDERPASS	44.20	785	СВ	BD	4	1	10	10	1907	
MAINLINE	WESTPORT	М	08032R	FERRY LANE & SAUGATUCK RIVER - MOVABLE	44.32	25190	DG	OD	4	6	92	460	1904	1990
MAINLINE	WESTPORT	U	03963R	COMPO ROAD	44.70	2200	TG	BD	4	1	39	44	1891	1940
MAINLINE	WESTPORT	U	08033R	SHERWOOD MILL POND - CONCRETE CULVERT	45.75	336	CA	BD	4	1	6	6	1951	
MAINLINE	WESTPORT	U	08034R	MUDDY BROOK - CONCRETE CULVERT	46.11	436	CA	BD	4	1	8	8	1951	
MAINLINE	WESTPORT	U	08035R	NEW CREEK ROAD	47.15	1954	DG	OD	4	1	32	36	1891	1934
MAINLINE	WESTPORT	U	08036R	GREENS FARMS BROOK - METAL PIPE CULVERT	47.29	600	MP	CS/BD	4	1	5	5	UK	2002
MAINLINE	WESTPORT	U	08037R	MAPLE LANE	47.44	2100	DG	BD	4	1	39	45	1889	1941
MAINLINE	WESTPORT	U	08038R	SASCO RIVER - MULTIBEAM DECK	48.29	2000	SB	BD	4	1	34	40	1940	
MAINLINE	FAIRFIELD	U	08039R	WESTWAY ROAD	48.65	2697	DG	BD	4	1	48	58	1890	1940*
MAINLINE	FAIRFIELD	U	08040R	MASONRY CULVERT	48.80	240	CA	BD	4	1	4	4	1890	
MAINLINE	FAIRFIELD	U	04200R	CENTER STREET	48.81	1925	DG	BD	4	1	33	43	1890	1940
MAINLINE	FAIRFIELD	U	08041R	SPRUCE STREET	48.91	1584	DG	BD	4	1	32	38	1890	1940
MAINLINE	FAIRFIELD	U	08042R	OLD POST ROAD	49.01	1700	DG	BD	4	1	33	38	1890	1940
MAINLINE	FAIRFIELD	U	08043R	MILL RIVER - MULTIBEAM DECK	49.66	3744	DG	BD	4	1	74	78	1891	1941
MAINLINE	FAIRFIELD	U	08044R	NORTH PINE CREEK ROAD	50.02	1680	DG	BD	4	1	27	33	1890	1941
MAINLINE	FAIRFIELD	U	04197R	MILL PLAIN ROAD	50.29	1968	DG	BD	4	1	38	44	1890	1941
MAINLINE	FAIRFIELD	U	04198R	ROUND HILL ROAD	50.90	1650	DG	BD	4	1	33	38	1890	1940
MAINLINE	FAIRFIELD	U	01344R	NORTH BENSON ROAD (ROUTE 135)	51.12	1724	DG	BD	4	1	32	36	1890	1940*
MAINLINE	FAIRFIELD	U	08045R	FAIRFIELD CREEK - MULTIBEAM DECK	51.68	960	DG	OD	4	1	14	18	1890	1921
MAINLINE	FAIRFIELD	U	08046R	CULVERT	52.29									
MAINLINE	FAIRFIELD	U	08047R	ASH CREEK - MULTIBEAM DECK	53.00	1900	IB	BD	4	1	31	38	1890	1940
MAINLINE	BRIDGEPORT	U	03635R	FAIRFIELD AVENUE (ROUTE 130)	53.42	5724	TG	BD	4	3	58	106	1895	*

Track Chart Name	Town	Bridge Type	Bridge No.	Location	Mile Point	Deck Area (sqft)	Bridge Structural Type	Deck Type	No. of Tracks		Length of Max Span	Total Length of Bridge	Year Built	Year Rehabilitated
MAINLINE	BRIDGEPORT	U	08048R	BOSTWICK AVENUE	53.60	3200	TG	OD	4	3	40	67	1900	1952
MAINLINE	BRIDGEPORT	U	08049R	HANCOCK AVENUE	53.72	3400	TG	OD	4	3	40	68	1900	
MAINLINE	BRIDGEPORT	U	08050R	HOWARD AVENUE	53.84	3264	DG	OD	4	3	40	68	1900	
MAINLINE	BRIDGEPORT	U	08051R	WORDIN AVENUE	54.07	3564	DG	OD	4	3	51	81	1900	
MAINLINE	BRIDGEPORT	U	08052R	IRANISTAN AVENUE	54.22	3496	DG	OD	4	3	51	76	1900	
MAINLINE	BRIDGEPORT	U	08053R	SOUTH AVENUE	54.44	4224	TG	OD	4	3	46	88	1900	*
MAINLINE	BRIDGEPORT	U	03638R	PARK AVENUE	54.58	4752	TG	OD	4	3	57	99	1900	1931
MAINLINE	BRIDGEPORT	U	08054R	MYRTLE AVENUE	54.70	2928	TG	OD	4	3	33	62	1900	
MAINLINE	BRIDGEPORT	U	08055R	WARREN STREET	54.77	2436	DG	OD	4	3	31	58	1900	
MAINLINE	BRIDGEPORT	U	08056R	LAFAYETTE STREET	54.84	2436	DG	OD	4	3	32	58	1900	
MAINLINE	BRIDGEPORT	U	08057R	BROAD STREET	54.93	2990	DG	OD	4	3	31	65	1900	1996
MAINLINE	BRIDGEPORT	U	08058R	MAIN STREET #1	54.98	3400	DG	OD	4	3	35	68	1900	1968*
MAINLINE	BRIDGEPORT	U	08059R	HOUSATONIC CROSSING	55.13	6090	TT	BD	4	1	77	101	1903	
MAINLINE	BRIDGEPORT	PO	08107R	SOUTH PEDESTRIAN OVERPASS	ESTRIAN OVERPASS 55.35			N/A	0	16	120	695	2008	
MAINLINE	BRIDGEPORT	U	08060R	UNION STREET			TG	OD	4	1	34	40	1903	1996
MAINLINE	BRIDGEPORT	PO	08108R	NORTH PEDESTRIAN OVERPASS	55.41	5211	TT	N/A	0	5	140	605	2008	
MAINLINE	BRIDGEPORT	U	08061R	BRIDGEPORT HARBOR - REINFORCED CONCRETE SLAB	55.41	51308	CS	CS	4	35	25	690	1955	1998
MAINLINE	BRIDGEPORT	PT	08104R	BRIDGEPORT HARBOR - PEDESTRIAN TUNNEL	55.50	1078	СВ	BD	0	1	75	77	1973	
MAINLINE	BRIDGEPORT	U	08062R	STATION VIADUCT	55.51	22500	DG	BD	4	7	60	360	1998	
MAINLINE	BRIDGEPORT	U	03636R	STRATFORD AVENUE (ROUTE 130)	55.61	9110	TG	BD	4	3	74	119	1998	
MAINLINE	BRIDGEPORT	U	08063R	VIADUCT - PARKING LOT NORTH	55.69	70860	СВ	BD	4	25	59	1028	1998	
MAINLINE	BRIDGEPORT	М	08064R	PEQUONNOCK RIVER - MOVABLE	55.90	32941	TG	BD	4	5	138	462	1998	
MAINLINE	BRIDGEPORT	U	08065R	VIADUCT	55.91	12875	DG	BD	4	4	50	190	1998	
MAINLINE	BRIDGEPORT	U	08066R	PULASKI STREET	55.95	4410	DG	BD	4	1	61	63	1998	
MAINLINE	BRIDGEPORT	U	08067R	VIADUCT	55.97	5735	DG	BD	4	2	45	83	1998	
MAINLINE	BRIDGEPORT	U	08068R	NOBLE AVENUE	55.98	5717	TG	BD	4	1	76	78	1998	
MAINLINE	BRIDGEPORT	U	08101R	VIADUCT	55.99	7788	DG	BD	4	2	66	125	1998	
MAINLINE	BRIDGEPORT	U	08102R	CLARENCE STREET	56.00	4173	TG	BD	4	1	62	68	1998	
MAINLINE	BRIDGEPORT	U	08069R	KOSSUTH STREET	56.10	2402	DG	BD	4	1	47	52	1998	
MAINLINE	BRIDGEPORT	U	03639R	EAST MAIN STREET (ROUTE 127)	56.20	3400	TG	OD	4	3	41	69	1902	1963
MAINLINE	BRIDGEPORT	U	08070R	PEMBROKE STREET	56.35	3100	TG	OD	4	3	33	62	1902	1950
MAINLINE	BRIDGEPORT	U	08071R	HALLETT STREET	56.46	2950	TG	OD	4	3	30	59	1902	1964
MAINLINE	BRIDGEPORT	U	08072R	YELLOW MILL POND - BRICK ARCH	56.68	1056	BA	BD	4	1	16	16	1890	1902
MAINLINE	BRIDGEPORT	U	08073R	YELLOW MILL POND SIDING	56.68									
MAINLINE	BRIDGEPORT	U	08074R	SEAVIEW AVENUE	56.77	3534	TG	OD	5	3	30	55	1902	1948

Track Chart Name	Town	Bridge Type	Bridge No.	Location	Mile Point	Deck Area (sqft)	Bridge Structural Type	Deck Type	No. of Tracks		Length of Max Span	Total Length of Bridge	Year Built	Year Rehabilitated
MAINLINE	BRIDGEPORT	U	08075R	BISHOP AVENUE	57.46	4320	TG	OD	6	3	31	60	1896	1969
MAINLINE	BRIDGEPORT	U	08076R	BRUCE BROOK - CONCRETE PIPE 9' DIAMETER	57.54	3380	CA	BD	6	2	9	26	UK	1984
MAINLINE	STRATFORD	U	08077R	BRUCE AVENUE	57.62	2573	TG	OD	6	1	33	38	1906	1944
MAINLINE	STRATFORD	U	08078R	WEST BROAD STREET	58.72	1919	TG	OD	4	1	34	39	1893	1966
MAINLINE	STRATFORD	U	08079R	KING STREET	58.88	1850	TG	OD	4	1	35	39	1893	
MAINLINE	STRATFORD	U	01318R	MAIN STREET (ROUTE 113)	59.01	3692	TG	OD	4	3	40	71	1893	1935
MAINLINE	STRATFORD	U	01312R	EAST MAIN STREET (ROUTE 110)	59.96	3308	TG	OD	5	1	43	49	1905	1966
MAINLINE	STRATFORD	М	08080R	HOUSATONIC RIVER (DEVON BRIDGE)	60.42	76433	TT	OD	4	7	223	1069	1905	1990
MAINLINE	MILFORD	U	08081R	BEAVER CREEK - MASONRY ARCH	61.62	384	SA	BD	4	1	8	10	1893	
MAINLINE	MILFORD	U	08082R	BEARDSLEY AVENUE	62.94	1850	DG	OD	3	1	33	37	1893	1999
MAINLINE	MILFORD	U	03640R	HIGH STREET	63.27	3135	TG	BD	3	1	48	53	1894	2006
MAINLINE	MILFORD	U	03644R	RIVER STREET	63.44	3840	TG	BD	3	3	52	72	1894	2006
MAINLINE	MILFORD	U	08083R	WEPAWAUG RIVER & PROSPECT STREET	63.53	6500	DG	BD	3	2	94	130	1893	1999
MAINLINE	MILFORD	U	08084R	GULF STREET	63.83	900	DG	OD	3	1	26	30	1894	
MAINLINE	MILFORD	PT	08085R	PEDESTRIAN UNDERPASS AT GULF STREET	63.84	549	CA/PT	BD	3	1	8	8	1960	
MAINLINE	MILFORD	U	08086R	INDIAN RIVER - MULTIBEAM DECK	64.59	1748	DG	OD	3	1	33	38	1880	1987
MAINLINE	MILFORD	U	08087R	OLD GATE LANE	64.74	1600	DG	BD	3	2	39	85	1893	2006
MAINLINE	MILFORD	U	08088R	QUIRK POND BROOK - MASONRY CULVERT	66.29	600	SA	BD	4	1	6	6	1880	1893
MAINLINE	MILFORD	U	08090R	DEPOT ROAD	66.66	971	СВ	BD	4	1	10	10	2000	
MAINLINE	ORANGE	U	08091R	OYSTER RIVER - MASONRY CULVERT	67.50	1360	SA	BD	4	1	8	8	1893	1990
MAINLINE	WEST HAVEN	U	08092R	STREAM - STONE/CONCRETE CULVERT	67.98	540	CA/MP/SA	BD	4	1	6	6	1880	
MAINLINE	WEST HAVEN	U	08093R	MORGAN LANE	68.11	1550	DG	OD	4	1	28	33	1917	1987
MAINLINE	WEST HAVEN	U	08094R	CULVERT	68.14									
MAINLINE	WEST HAVEN	U	08095R	CULVERT- NEW IN 1995	68.89	336	CA	BD	4	1	4	4	1995	
MAINLINE	WEST HAVEN	U	08096R	COVE RIVER - MASONRY CULVERT	69.19	1985	MP/SA	BD	4	1	8	8	1893	
MAINLINE	WEST HAVEN	U	01403R	SAW MILL ROAD (ROUTE 162)	69.66	3185	DG	BD	4	1	61	67	1962	
MAINLINE	WEST HAVEN	U	08097R	CAMPBELL AVENUE	70.19	3050	DG	OD	4	3	36	61	1894	1966
MAINLINE	WEST HAVEN	U	08098R	WASHINGTON AVENUE	70.36	2900	TG	OD	4	3	33	58	1894	1940
MAINLINE	WEST HAVEN	U	08099R	WEST RIVER - CONCRETE BOX	71.26	7192	СВ	BD	4	4	30	131	1930	1998
MAINLINE	NEW HAVEN	PT	08100R	STATION UNDERPASS	72.28	2880	MB/CB	BD	8	2	16	34	1929	1988
MAINLINE	NEW HAVEN	PO	08106R	STATE STREET STATION OVERPASS	72.80	1413	TT	N/A	N/A	1	108	109	2000	
NEW CANAAN	STAMFORD	U	08150R	VIADUCT ROAD	2.81	1885	TG	BD	1	1	82	89	1996	
NEW CANAAN	STAMFORD	U	08151R	NOROTON RIVER - MULTIBEAM DECK	3.77	296	DG	OD	1	1	31	37	1907	1957
NEW CANAAN	NEW CANAAN	U	00710R	MERRITT PARKWAY (ROUTE 15)	5.76	1008	CA	BD	1	2	32	77	1937	
NEW CANAAN	NEW CANAAN	U	01302R	OLD STAMFORD ROAD (ROUTE 106)	6.16	795	TG	OD	1	1	50	55	1906	

Track Chart Name	Town	Bridge Type	Bridge No.	Location	Mile Point	Deck Area (sqft)	Bridge Structural Type	Deck Type	No. of Tracks		Length of Max Span	Total Length of Bridge	Year Built	Year Rehabilitated
NEW CANAAN	NEW CANAAN	U	08154R	WATERWAY - MASONRY CULVERT	7.00	787	SA	BD	1	1	6	6	1900	
WATERBURY	MILFORD	U	08250R	BROOK	3.12									
WATERBURY	MILFORD	U	08251R	CULVERT	3.30									
WATERBURY	MILFORD	U	08252R	UNNAMED STREAM - CONCRETE & RAIL DECK	3.80	160	SB	CS	1	1	4	7	1900	
WATERBURY	MILFORD	U	08253R	BROOK - MASONRY CULVERT	4.14	552	SA	BD	1	1	7	7	1900	
WATERBURY	MILFORD	U	08254R	BEARDS FARMWAY	4.55	528	DG	OD	1	1	20	24	1917	1944
WATERBURY	MILFORD	U	08255R	FARMWAY CROSSING	4.88	572	СВ	CS	1	1	18	22	1944	1983
WATERBURY	MILFORD	U	08256R	DRAINAGE PIPE - ACC METAL PIPE	4.90	1025	MP	BD	1	1	5	5	UK	
WATERBURY	MILFORD	U	08257R	GOLF CROSSING - REINFORCED CONCRETE SLAB	5.01	442	CS	CS	1	1	14	17	1944	1982
WATERBURY	ORANGE	U	08258R	DAVIS BROOK - 2 CONCRETE CULVERTS	7.02	1696	CA	BD	1	2	6	16	1995	
WATERBURY	ORANGE	U	08260R	TWO MILE BROOK (TURKEY BROOK) - CONCRETE CULV	7.36	560	BA/CA/SA	BD	1	1	8	8	1901	
WATERBURY	DERBY	U	08261R	STREAM - UNNAMED STREAM	7.92	660	SB	BD	1	1	3	3	UK	
WATERBURY	DERBY	U	08262R	NAUGATUCK RIVER - STEEL TRUSS	8.62	3948	TT	OD	1	3	107	330	1903	1965
WATERBURY	DERBY	U	08263R	NAUGATUCK RIVER FLOOD PLAIN	8.68	4680	PG	BD	1	10	30	246	1965	
WATERBURY	ANSONIA	U	08264R	NAUGATUCK RIVER - STEEL TRUSS	10.30	9827	TT	OD	1	2	155	317	1903	1956
WATERBURY	ANSONIA	U	08266R	RACEWAY FARREL COMPANY - MASONRY ARCH	10.85	768	SA	BD	1	1	14	14	1900	
WATERBURY	ANSONIA	U	08267R	SPILLWAY	11.36	2366	TG	OD	1	2	45	94	1904	2003
WATERBURY	SEYMOUR	U	08268R	CANAL - STEEL GIRDER	12.57	1170	TG	OD	1	2	43	95	1904	1956
WATERBURY	SEYMOUR	U	08269R	MUD BROOK - MASONRY CULVERT	13.20	396	SA	BD	1	1	6	6	1904	
WATERBURY	SEYMOUR	U	01321R	MAIN STREET (ROUTES 115 & 313)	14.29	1664	TG	OD	1	2	48	71	1906	
WATERBURY	SEYMOUR	PO	08270R	JAMES STREET FOOTBRIDGE	14.45	608	TI	N/A	0	6	42	76	1906	2001
WATERBURY	SEYMOUR	U	01063R	DERBY AVENUE (ROUTE 67)	14.61	754	TG	OD	1	1	82	89	1906	1999
WATERBURY	SEYMOUR	U	08272R	NAUGATUCK RIVER - MULTIBEAM DECK	14.64	2763	DG	OD	1	5	71	307	1903	1956
WATERBURY	BEACON FALLS	U	08274R	OLD PINE'S BRIDGE ROAD	16.79	150	DG	OD	1	1	18	23	1905	1952
WATERBURY	BEACON FALLS	U	08275R	HEMP SWAMP BROOK - MASONRY CULVERT	16.81	1140	SA	BD	1	1	12	12	1900	
WATERBURY	BEACON FALLS	U	08276R	SPRUCE BROOK - MULTIBEAM DECK	19.21	180	DG	OD	1	1	17	23	1900	1947
WATERBURY	NAUGATUCK	U	08277R	SUGAR BUSH BROOK - MULTIBEAM DECK	20.11	180	DG	OD	1	1	18	23	1905	1940
WATERBURY	NAUGATUCK	U	08278R	US RUBBER COMPANY - PEDESTRIAN TUNNEL	21.22	336	CA	BD	2	1	8	8	1930	
WATERBURY	NAUGATUCK	U	08279R	PRIVATE ROAD AT UNIROYAL	21.36	504	DG	OD	2	1	19	22	1910	1960
WATERBURY	NAUGATUCK	U	08280R	LONGMEADOW POND BROOK - CONCRETE ARCH	21.48	6240	CA	BD	2	2	19	48	1906	2008
WATERBURY	NAUGATUCK	U	08281R	OVERFLOW - CONCRETE SLAB	21.65	5440	CA	BD	2	1	10	14	1906	
WATERBURY	NAUGATUCK	U	04224R	MAPLE STREET	21.74	972	TG	OD	1	1	64	69	1913	
WATERBURY	NAUGATUCK	U	08282R	CANAL	22.11									
WATERBURY	NAUGATUCK	U	08283R	HOP BROOK - MULTIBEAM DECK	22.42	359	DG	OD	1	1	43	48	1900	
WATERBURY	NAUGATUCK	U	08284R	BRIDGE STREET (GENERAL PULASKI WALK)	22.74	754	TG	OD	1	3	41	58	1906	

Track Chart Name	Town	Bridge Type	Bridge No.	Location	Mile Point	Deck Area (sqft)	Bridge Structural Type	Deck Type	No. of Tracks		Length of Max Span	Total Length of Bridge	Year Built	Year Rehabilitated
WATERBURY	WATERBURY	U	03723R	BRISTOL STREET #1	24.10	611	TG	OD	1	1	45	50	1904	1918
WATERBURY	WATERBURY	U	08285R	STREAM - STONE/CONCRETE CULVERT	24.34	1320	SA	BD	1	1	8	8	1905	
WATERBURY	WATERBURY	U	04232R	WASHINGTON AVENUE #1	26.18	2144	TG	OD	1	1	61	67	1906	
WATERBURY	WATERBURY	U	04235R	BANK STREET #1	26.35	2054	TG	OD	2	1	77	82	1910	
WATERBURY	WATERBURY	U	08286R	NAUGATUCK RIVER - STEEL GIRDER	26.42	3435	DG	OD	2	2	114	229	1907	1956
WATERBURY	WATERBURY	U	08287R	JACKSON STREET	26.50	2074	TG	OD	2	2	53	78	1910	
WATERBURY	WATERBURY	U	08288R	GAS CONDUIT	26.64	UK	CA	BD	14	UK	7	7	1900	

TOTAL NUMBER OF STRUCTURES: 216

#### KEY TO ABBREVIATIONS

#### BRIDGE TYPE

U = UNDERGRADE BRIDGE M = MOVABLE BRIDGE PT = PEDESTRIAN TUNNEL

PO = PEDESTRIAN OVERPASS

BRIDGE STRUCTURAL TYPE TT = THROUGH TRUSS DT = DECK TRUSS TG = THROUGH GIRDER DG = DECK GIRDER SA = STONE ARCH BA = BRICK ARCH MA = METAL/STEEL ARCH CA = CONCRETE ARCH PC = PRESTRESSED CONCRETE SLABS PG = PRESTRESSED GIRDER TI = TIMBER TRESTLE ST = STEEL TRESTLE MP = METAL/STEEL PIPE CULVERT CP = CONCRETE PIPE CULVERT CB = CONCRETE BOX CULVERT SB = STONE BOX CULVERT PP = PLASTIC PIPE CULVERT SR = CONCRETE ENCASED STOCK RAILS MB = MULTI-BEAM

#### DECK TYPE

OD = OPEN DECK DF = DIRECT FIXATION SP = STEEL PLATE IP = IRON PLATE CS = CONCRETE SLAB SS = STONE SLAB TD = TIMBER/PLANK BD = BALLASTED DECK \* = CURRENTLY UNDER CO

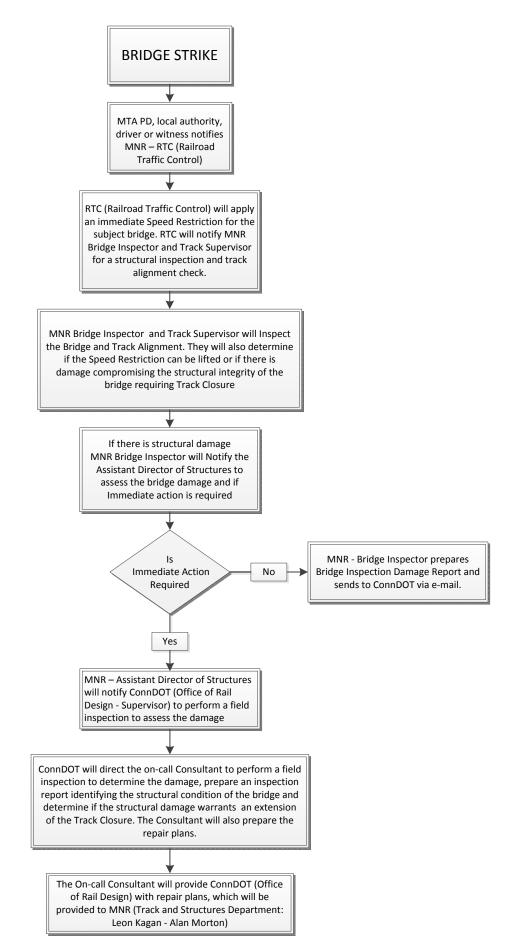
#### \* = CURRENTLY UNDER CONSTRUCTION UK = UNKNOWN

Version: 1.1 - February, 2014

#### APPENDIX 3A 2014 OFFICE OF RAIL – METRO NORTH BRIDGE STRIKE PROCEDURE

3A

#### OFFICE OF RAIL - MNR RAILROAD BRIDGE STRIKE PROCEDURE



Version: 1.0 - February, 2012

### APPENDIX 4 INVENTORY OF RAILROAD BRIDGES OFF-SYSTEM RAILROAD BRIDGES

#### State of Connecticut - Department of Transportation - Office of Rail Inventory List of Off-System Railroad Bridges 6/09/2011

		Г — Г			ist of OII-System Raint										
Bridge No.	M.P.	Railroad	Track Chart Name	Town	Location	Deck Area (sqft)	Bridge Type	Bridge Structural Type	Deck Type	No. of Tracks	No. Of Spans	Length of max. Span (ft)	Total Length of Bridge (ft)	Year Built	Year Rehabilitated
09901R	30.39	ABANDONED	AIRLINE VL MAP 54-64/29	FAST HAMPTON	MINE BROOK-STONE CULV.	3667	U	SA	CS	0	1	19	19	1910	
09902R			AIRLINE VL MAP 54-64/30		PINE BROOK	76	<u> </u>	DG	CD	0	1	18	24	1910	
09903R			AIRLINE VL MAP 54-64/31		MUDDY GUTTER BROOK	1919	<u> </u>	SA	CS	0	1	19	19	1900	
09904R			AIRLINE VL MAP 54-64/32		MAIN STREET	86	U			0	0			1900	
09905R			AIRLINE VL MAP 54-64/32		BROOK	760	U	DG	OD	0	2	45	76	1900	
09908R			MOOSUP V MAP 54-67/13	PLAINFIELD	MOOSUP RIVER	1447	U	DG	OD	0	4	79	201	1920	
09909R			MOOSUP V MAP 54-67/15	PLAINFIELD	HORSE BROOK		U	SB	CD	0	1	3	3	1900	
09101R			TORRINGTON SEC.	WATERBURY	CONC. BOX/PIPE CULVERT	840	U	CB	CS	2	1	6	6	1930	1960
00600R				WATERBURY	WEST MAIN STREET (SR 847)	4929	U	TG	SP	1	3	68	106	1906	
09100R			TORRINGTON SEC.	WATERBURY	CRANE STREET	2883	U	DG	OD	4	2	36	95	1907	
04234R			TORRINGTON SEC.	WATERBURY	FREIGHT STREET	6717	U	DG	OD	4	4	36	95	1907	
09814R			E. LONGMEADOW SEC.(ARMORY LINE)			221	U	DG	OD	1	1	22	26	1917	
09815R			E. LONGMEADOW SEC.(ARMORY LINE)		KETCH BROOK	281	U	CA	CS	1	1	14	14	1910	
09816R			, <i>, , , , , , , , , , , , , , , , , , </i>		BROAD BROOK AND PRIV. RD.	880	U	DG	OD	1	2	40	88	1910	
09817R				EAST WINDSOR	CULVERT	605	U	SA	CS	0	1	9	15	UK	
09818R				ENFIELD	SCANTIC RIVER	2340	U	DG	OD	1	2	71	180	1914	
09819R			· · · · · · · · · · · · · · · · · · ·	ENFIELD	WATER STREET	1020	U	TG	OD	1	1	63	68	1914	
09820R				ENFIELD	TERRY BRK-2 CULVERTS	1615	U	CP	SS	1	2	8	19	UK	
09821R			, <i>, , , , , , , , , , , , , , , , , , </i>	ENFIELD	STREAM	208	<u> </u>	SR	CS	1	1	6	13	UK	
09823R		CENTRAL NEW ENGLAND	, <i>, , , , , , , , , , , , , , , , , , </i>	HARTFORD	CULVERT		U	CP	CS	1	1	43	48	1940	
09824R		CENTRAL NEW ENGLAND		HARTFORD	STREAM	330	<u> </u>	SA	CS	1	1	10	10	1910	
09825R		CENTRAL NEW ENGLAND			STREAM (WASH BROOK)	280	U	SR	CS	1	2	6	14	1911	
09200R					STREAM - STONE CULVERT	516	U	SA/MA	SS	1	1	5	8	1900	
09201R			,	NEW MILFORD	BROOK - STONE CULVERT	252	U	SB	SS	1	1	4	7	1900	
09202R					BROOK - CULVERT	140	U	SB	SS	1	1	4	11	1900	
09203R			,		BROOK - CULVERT	287	U	SB	SS	1	1	10	10	1900	
09204R			· · · · · · · · · · · · · · · · · · ·	KENT	SWAMP (CULVERT)	254	U	DG	SP	1	2	5.5	14.5	UK	
09204R			· · · · · · · · · · · · · · · · · · ·	KENT	COBBLE BROOK	634	U	SA	SS	1	1	16	16	1900	
09207R			,	KENT	MAUWEE BROOK	159	<u> </u>	SR	CS	1	1	6	9	1900	
09208R			,	KENT	KENT FALLS BROOK-CULVERT	776	<u> </u>	SA	SS	1	1	16	16	1900	
09209R			NEW MILFORD SEC.(BERKSHIRE LINE)	KENT	CULVERT-DBL 4' CULVERTS		<u> </u>	CP	CS	1	1	4	13	1900	
09210R			,	CORNWALL	DEEP BROOK	221	U	SR	CS	1	1	10	13	1900	
09211R					MILLARD BROOK	221	U	SR	CS	1	1	10	13	1900	
				CORNWALL	GUNN BROOK	175	U	SA	SS	1	1	10	14	1900	1940
09213R			,	CORNWALL	BONNEY BROOK	1147	U	CS	CS	2	2	19	39	1960	
09214R				CORNWALL	FURNACE BROOK	910	<u> </u>	DG	OD	1	1	31	35	1893	
09215R			· · · · · · ·	CORNWALL	BROOK	225	U	SR	CS	1	1	6	13	1900	
09217R			,	CORNWALL	MILL BROOK	310	<u> </u>	DG	OD	1	1	25	31	1900	
09218R			· · · · · · · · · · · · · · · · · · ·	CORNWALL	STREAM	210	<u> </u>	DG	OD	1	1	16	21	1910	
09219R				CANAAN	CHILD POND BROOK	332	<u> </u>	SA	SS	1	1	6	11	1910	
09220R			· · · · · · · · · · · · · · · · · · ·	CANAAN	BROOK - STONE SLAB	414	<u> </u>	SB	SS	1	1	3	3	1910	
09221R			,		WATER STREET	320	<u> </u>	DG	OD	1	1	26	32	1913	
09222R			,		BRANCH HOUSATONIC RIVER	580	U	DG	OD	1	1	52	58	1890	1951
09223R			,		HOLLENBECK RIVER	1073	U	DG	CD	1	6	12	75	1910	
09224R			,		BLACKBERRY RIVER	1264	U	DG	OD	1	2	47	98	1900	1950
09225R			,		NORTH BLACKBERRY RIVER	315	U	SR	CS	2	1	5	10	1910	
04242R			NEW BRITAIN SEC.		ROUTE 9 (BUSWAY)	13754	U	TG	SP	1	5	122	598	1976	
09826R			NEW BRITAIN SEC.	NEW BRITAIN	CEMETERY UNDERPASS(BW)	645	U	SR	CS	1	1	10	14	1910	
09827R			NEW BRITAIN SEC.	NEW BRITAIN	PIPER BROOK (BUSWAY)	1782	U	CB	CS	0	1	10	14	1950	
09916R			VAL. MAP 57-72/54		STILL RIVER	281	U	DG	OD	0	1	30	36	1907	
09104R			TORRINGTON SEC.	WATERBURY	MAHAN CANAL	260	U	DG	OD	1	1	21	26	1902	
09105R			TORRINGTON SEC.	WATERBURY	BROOK	340	U	CB	CS	1	1	5	5	1950	
09106R			TORRINGTON SEC.	WATERBURY	HANCOCK BROOK	1714	U	TG	OD	1	2	53	115	1902	
09107R			TORRINGTON SEC.	WATERBURY	AMERICAN PIN FOOTBRIDGE	1166	 Z	TT	TD	0	1	67	67	1935	
09108R			TORRINGTON SEC.	WATERBURY	NAUGATUCK RIVER	2924	 U	DG	OD	1	4	56	238	1907	
09109R			TORRINGTON SEC.	WATERTOWN	CULVERT	582	U	MP	CS	1	1	97	97	1950	
001001	0.00					002	<u> </u>			•	· · ·		01		1

#### State of Connecticut - Department of Transportation - Office of Rail Inventory List of Off-System Railroad Bridges 6/09/2011

									1		r			r	
Bridge No.	M.P.	Railroad	Track Chart Name	Town	Location	Deck Area (sqft)	Bridge Type	Bridge Structural Type	Deck Type	No. of Tracks	No. Of Spans	Length of max. Span (ft)	Total Length of Bridge (ft)	Year Built	Year Rehabilitated
09110R	5.41	NAUGATUCK	TORRINGTON SEC.	WATERTOWN	NAUGATUCK RIVER	3624	U	DG	OD	1	5	65	302	1907	
09111R		NAUGATUCK	TORRINGTON SEC.	THOMASTON	BROOK	534	U	CP	CS	1	1	6	6	1950	
09112R		NAUGATUCK	TORRINGTON SEC.	THOMASTON	RAILROAD STREET ANNEX	322	U	DG	OD	1	1	18	23	1958	
09113R		NAUGATUCK	TORRINGTON SEC.	THOMASTON	BROOK - CULVERT	393	U	CP	CS	1	1	5.5	6	1958	
09114R		NAUGATUCK	TORRINGTON SEC.	THOMASTON	NAUGATUCK RIVER DAM SP.	2718	U	TG	SP	1	1	100	106	1958	
09115R		NAUGATUCK	TORRINGTON SEC.	THOMASTON	DAM CONTROL	3880	U	CP	CS	1	1	10	10	1958	
		NAUGATUCK	TORRINGTON SEC.	LITCHFIELD	BROOK - CULVERT-CONC. PIPE	460	Ŭ	CP	CS	1	1	6	6	1960	
09117R		NAUGATUCK	TORRINGTON SEC.	LITCHFIELD	CULVERT	1130	Ŭ	CB	CS	1	1	9	11	1958	
09118R		NAUGATUCK	TORRINGTON SEC.	LITCHFIELD	BROOK	855	U	CP	CS	1	1	5	5	1960	
09119R		NAUGATUCK	TORRINGTON SEC.	LITCHFIELD	CULVERT		U	CP	CS	1	1	3	3	1950	
09120R		NAUGATUCK	TORRINGTON SEC.	LITCHFIELD	SPRUCE BROOK	765	U	DG	CS	1	1	39	45	1958	
		NAUGATUCK	TORRINGTON SEC.	TORRINGTON	BROOK - CULVERT	290	U	CS	CS	1	1	5	10	1950	
09122R		NAUGATUCK	TORRINGTON SEC.	TORRINGTON	GULF STREAM	478	U	DG	OD	1	1	27	33	1907	
09123R		NAUGATUCK	TORRINGTON SEC.	TORRINGTON	SUMMER STREET	1971	U	TG	OD	1	1	39	45	1903	1914
09124R	19.04	NAUGATUCK	TORRINGTON SEC.	TORRINGTON	PASSWAY	623	U	DG	OD	1	1	18	22	1899	
09125R		NAUGATUCK	TORRINGTON SEC.	TORRINGTON	NAUGATUCK RIVER	2128	U	DG	OD	1	1	69	152	1907	1917
09000R		PROV & WORCESTER	CROMWELL IND. TRACK	MIDDLETOWN	SEBETHE RIVER	2483	U	TG	OD	1	4	63	191	1910	
00863R		PROV & WORCESTER	CROMWELL IND. TRACK	MIDDLETOWN	HARTFORD AVENUE	1093	U	TG	CS	1	1	66	71	1925	
09001R		PROV & WORCESTER	EAST BERLIN IND. TRACK	MIDDLETOWN	COGINCHAUG RIVER	1030	U	DG	OD	1	2	46	103	1926	
09002R		PROV & WORCESTER	LAUREL IND. TRACK	MIDDLETOWN	GREEN STREET	500	U	DG	OD	1	1	22	22	1911	
09003R		PROV & WORCESTER	LAUREL IND. TRACK	MIDDLETOWN	SUMNER BROOK	1518	U	TG	OD	1	2	51	115	1911	
09004R		PROV & WORCESTER	LAUREL IND. TRACK	MIDDLETOWN	EASTERN DRIVE	230	U	DG	OD	1	1	22	27	1911	
09005R		PROV & WORCESTER	LAUREL IND. TRACK	MIDDLETOWN	RESERVOIR BROOK	306	U	DG	OD	1	1	29	34	1904	
09006R		PROV & WORCESTER	LAUREL IND. TRACK	MIDDLETOWN	INDIAN HILL BROOK	320	U	DG	OD	1	1	27	32	1911	
09007R		PROV & WORCESTER	LAUREL IND. TRACK	MIDDLETOWN	FOREST CREEK	246	U	CS	CS	1	2	5	12	1910	
09008R		PROV & WORCESTER	LAUREL IND. TRACK	MIDDLETOWN	ASYLUM CREEK	280	U	DG	OD	1	1	23	28	1904	
09009R		PROV & WORCESTER	LAUREL IND. TRACK	MIDDLETOWN	MAROMAS CREEK	240	U	DG	OD	1	1	17	24	1904	
09010R		PROV & WORCESTER	MIDDLETOWN SEC.	MIDDLEFIELD	CATTLE PASS	150	U	DG	OD	1	1	11	15	1917	
01373R		PROV & WORCESTER	MIDDLETOWN SEC.	MIDDLEFIELD	RTE 147 (BAILEYVILLE ROAD)	880	U	DG	OD	1	1	81	88	1998	
09011R	17.86	PROV & WORCESTER	MIDDLETOWN SEC.	MIDDLEFIELD	COGINCHAUG RIVER	682	U	DG	OD	1	1	52	62	1898	
09012R	18.76	PROV & WORCESTER	MIDDLETOWN SEC.	MIDDLEFIELD	FARMWAY	390	U	DT	OD	1	1	113	135	1898	
09013R	18.77	PROV & WORCESTER	MIDDLETOWN SEC.	MIDDLEFIELD	COGINCHAUG RIVER	1243	U								
09014R	19.93	PROV & WORCESTER	MIDDLETOWN SEC.	MIDDLETOWN	COGINCHAUG RIVER	1419	U	DT	OD	1	1	118	129	1898	
09015R	20.58	PROV & WORCESTER	MIDDLETOWN SEC.	MIDDLETOWN	UNKNOWN STREAM	108	U	PP	SS	1	2	3	8	1900	
00522R	21.18	PROV & WORCESTER	MIDDLETOWN SEC.	MIDDLETOWN	ROUTE 66	4400	U	TT	CS	1	1	200	204	1996	
00639R	0.19	PROV & WORCESTER	PORTLAND IND. TRK.	MIDDLETOWN	RT. 9 & CONNECTICUT RIVER	19520	М	TG/TT	OD	1	5	210	1199	1910	1999
00479R	3.20	PROV & WORCESTER	WETHERSFIELD SEC.	HARTFORD	AIRPORT ROAD (SR 530)	1037	U	TG	CS	1	1	70	78	1959	
09833R	3.80	PROV & WORCESTER	WETHERSFIELD SEC.	HARTFORD	FOLLY BROOK-CULVERT	1980	U	MP	SS	1	1	12	12	UK	
09832R	5.20	PROV & WORCESTER	WETHERSFIELD SEC.	WETHERSFIELD	BEAVER BROOK-CULVERT		U	MP	CS	1	1				
09831R	7.10	PROV & WORCESTER	WETHERSFIELD SEC.	ROCKY HILL	GOFF'S BROOK	600	U	DG	OD	1	1	54	60	1909	
09830R		PROV & WORCESTER	WETHERSFIELD SEC.	ROCKY HILL	EVAN'S ROAD	139	U	DG	OD	1	1	16	19	1950	
09836R		PROV & WORCESTER	WETHERSFIELD SEC.	ROCKY HILL	DIVIDEND BROOK	864	U	MP	SS	1	1	9	9	1990	
09829R		PROV & WORCESTER	WETHERSFIELD SEC.	CROMWELL	NOOKS HILL ROAD	319	U	DG	OD	1	1	23.5	29	1910	
09828R		PROV & WORCESTER	WETHERSFIELD SEC.	CROMWELL	SOUTH STREET	308	U	DG	OD	1	1	22	28	1910	
09834R		PROV & WORCESTER	WETHERSFIELD SEC.	HARTFORD	THRASHER'S CROSSING	594	U	DG	OD	2	1	22.9	27	1911	
09835R		PROV & WORCESTER	WETHERSFIELD SEC.	CROMWELL	FIRE ACCESS ROAD	190	U	DG	OD	0	1	13	19	1910	
09301R		PROV & WORCESTER	WILLIMANTIC BRANCH	SPRAGUE	WALDO BROOK	680	U	CA	CS	1	1	27	35	1906	
09302R		PROV & WORCESTER	WILLIMANTIC BRANCH	SCOTLAND	MERRICK BROOK	600	U	DG	OD	1	1	55	60	1901	
09303R		PROV & WORCESTER	WILLIMANTIC BRANCH	WINDHAM	FROG BROOK	120	U	SA	CS	1	1			1954	
09304R		PROV & WORCESTER	WILLIMANTIC BRANCH	WINDHAM	SHETUCKET RIVER	2450	U	DG	OD	1	4	58	245	1907	
09305R		PROV & WORCESTER	WILLIMANTIC BRANCH	WINDHAM	PLAINS ROAD	836	U	DG	OD	2	1	33	38	1911	
01849R		PROV & WORCESTER	WILLIMANTIC BRANCH	WINDHAM	ABANDONED RD/PEDESTRIAN	1571	U	TG	OD	2	1	50	52	1892	
09306R		PROV & WORCESTER	WILLIMANTIC BRANCH	WINDHAM	WILLIMANTIC RIVER	5427	U	DG	OD	2	3	103	257	1904	
09307R		PROV & WORCESTER	WILLIMANTIC BRANCH	WINDHAM	STATION FOOTBRIDGE	2388	Z	TT	CD	0	5	156	640	1910	
09800R		RAILS TO TRAILS*	AVON SECONDARY	FARMINGTON	SCOTT SWAMP BROOK	750	U	SA	CS	1	1	10	10	1900	
09802R	32.00	RAILS TO TRAILS*	AVON SECONDARY	FARMINGTON	FARMINGTON RIVER	3960	U	DG	OD	1	4	112	396	1956	
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#### State of Connecticut - Department of Transportation - Office of Rail Inventory List of Off-System Railroad Bridges 6/09/2011

Bridge No.	M.P.	Railroad	Track Chart Name	Town	Location	Deck Area (sqft)	Bridge Type	Bridge Structural Type	Deck Type	No. of Tracks	No. Of Spans	Length of max. Span (ft)	Total Length of Bridge (ft)	Year Built	Year Rehabilitated
00430R	32.25	RAILS TO TRAILS*	AVON SECONDARY	FARMINGTON	FARMINGTON AVENUE	1098	U	DG	CS	0	1	55	60	1928	1954
09803R	34.05	RAILS TO TRAILS*	AVON SECONDARY	AVON	THOMPSON BROOK	465	U	CA	CS	0	1	15	15	1900	
01624R	36.65	RAILS TO TRAILS*	AVON SECONDARY	AVON	ARCH ROAD	1299	U	TG	SP	0	1	63	71	1896	
09804R	37.79	RAILS TO TRAILS*	AVON SECONDARY	AVON	NOD BROOK	845	U	SA	SS	0	1	10	10	1900	
09805R	37.81	RAILS TO TRAILS*	AVON SECONDARY	AVON	ROSEWOOD ROAD	516	U	SA	SS	0	1	12	12	1900	
09806R	41.38	RAILS TO TRAILS*	AVON SECONDARY	SIMSBURY	HOP BROOK	336	U	DG	OD	1	1	34	37	1940	
09807R	44.91	RAILS TO TRAILS*	AVON SECONDARY	SIMSBURY	ROBERT'S CATTLE PASS	150	U	SA	SS	0	1	5	5	1920	
09808R	46.25	RAILS TO TRAILS*	AVON SECONDARY	EAST GRANBY	FARMINGTON CANAL		U								
09809R	46.44	RAILS TO TRAILS*	AVON SECONDARY	EAST GRANBY	SALMON RIVER	3270	U	DG	TD	0	2	68	268	1950	
09810R	49.46	RAILS TO TRAILS*	AVON SECONDARY	EAST GRANBY	GRIFFIN'S BROOK	280	U	DG	OD	0	3	9	28	1950	
09811R	50.06	RAILS TO TRAILS*	AVON SECONDARY	SUFFIELD	MUDDY RIVER-CONC. ARCH	1464	U	MP	SS	0	1	6	24		
		RAILS TO TRAILS*	AVON SECONDARY	SUFFIELD	FARMINGTON CANAL	340	U	SR	CS	0	1	8	17	1910	
09813R	51.27	RAILS TO TRAILS*	AVON SECONDARY	SUFFIELD	STREAM	211	U	DG	TD	0	3	10	27	1950	
09911R	32.81	RAILS TO TRAILS*	FARMINGTON VALLEY GREENWAY	FARMINGTON	HYDE BROOK-STONE ARCH		U								
09300R	1.10	RAILS TO TRAILS*	KENDALL'S IND. TRACK	WINDHAM	NATCHAUG RIVER	2471	U	DG	CD	0	2	84	174	1905	
09912R	33.09	RAILS TO TRAILS*	VAL MAP 56-60/4	FARMINGTON	OLD BROOK-ARCH CULVERT	704	U	SA	SS	0	1	16	16	1900	
09921R		RAILS TO TRAILS*	VAL MAP 56-60/6	BURLINGTON	BARNES BROOK - DEP		U	DG	OD	0	1	43	48		
09922R	6.14	RAILS TO TRAILS*	VAL MAP 56-60/7	BURLINGTON	CEDAR BROOK - DEP		U	DG	OD	0	1	34	40		
09923R	6.37	RAILS TO TRAILS*	VAL MAP 56-60/7	BURLINGTON	BROOK - DEP		U	DG	OD	0	1	13	17		
09913R		RAILS TO TRAILS*	VAL MAP 56-60/8	CANTON	FARMINGTON RIVER	7716	U	TT	CD	0	1	175	643	1910	
09914R	33.19	RAILS TO TRAILS*	VAL MAP 56-60/8	CANTON	FARMINGTON CANAL		U								

133 TOTAL STRUCTURES

#### BRIDGE TYPE

U = UNDERGRADE BRIDGE M = MOVABLE BRIDGE PT = PEDESTRIAN TUNNEL PO = PEDESTRIAN OVERPASS

#### KEY TO ABBREVIATIONS

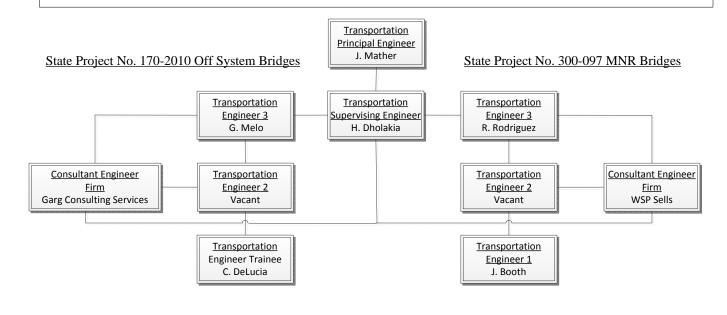
BRIDGE STRUCTURAL TYPE	DECK TYPE
TT = THROUGH TRUSS	OD = OPEN DECK
DT = DECK TRUSS	DF = DIRECT FIXATION
TG = THROUGH GIRDER	SP = STEEL PLATE
DG = DECK GIRDER	IP = IRON PLATE
SA = STONE ARCH	CS = CONCRETE SLAB
BA = BRICK ARCH	SS = STONE SLAB
MA = METAL/STEEL ARCH	TD = TIMBER/PLANK
CA = CONCRETE ARCH	BD = BALLASTED DECK
PC = PRESTRESSED CONCRET	E SLABS
PG = PRESTRESSED GIRDER	* = CURRENTLY UNDER CONSTRUCTION
TI = TIMBER TRESTLE	UK = UNKNOWN
ST = STEEL TRESTLE	
MP = METAL/STEEL PIPE CULVE	RT
CP = CONCRETE PIPE CULVER	Г
CB = CONCRETE BOX CULVERT	-
SB = STONE BOX CULVERT	
PP = PLASTIC PIPE CULVERT	
SR = CONCRETE ENCASED STC	OCK RAILS
MB = MULTI-BEAM	

Version: 1.0 - February, 2012

### APPENDIX 5 ORGANIZATION CHARTS FOR INSPECTION OF RAILROAD BRIDGES

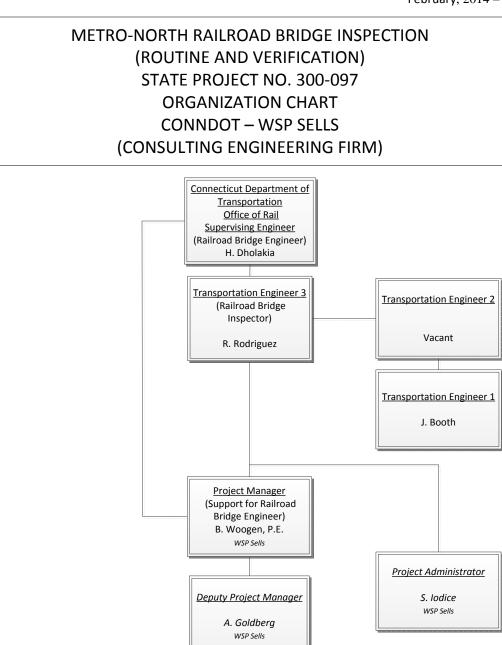
Revision 1.1 (2/6/14)

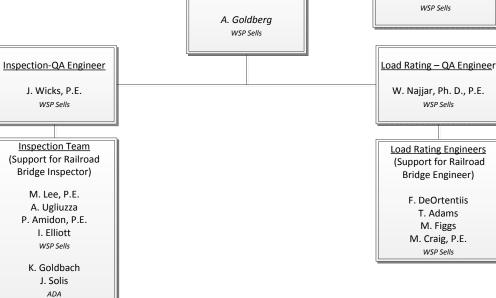
#### ORGANIZATION CHART FOR INSPECTION OF RAILROAD BRIDGES



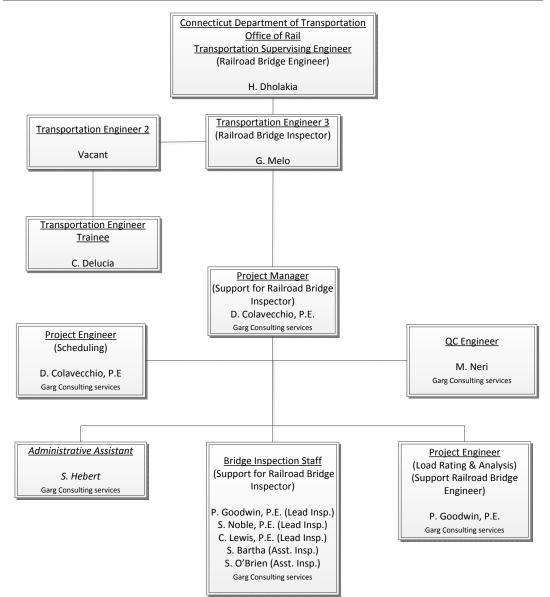
FRA Designation	CDOT Office of Rail Designation
Railroad Bridge Engineer –	Transportation Supervising Engineer with support from Consultant Engineering Firm
Railroad Bridge Inspector –	Transportation Engineer 3 with support from Consultant Engineering Firm
Railroad Bridge Supervisor –	Transportation Engineer 3 with support from Consultant Engineering Firm

See Page A5-2 Detailed Organization Charts for Inspection (Routine and Verification) of Metro-North Railroad Bridges See Page A5-3 Detailed Organization Charts for Routine and Verification Inspections of Off-System Bridges



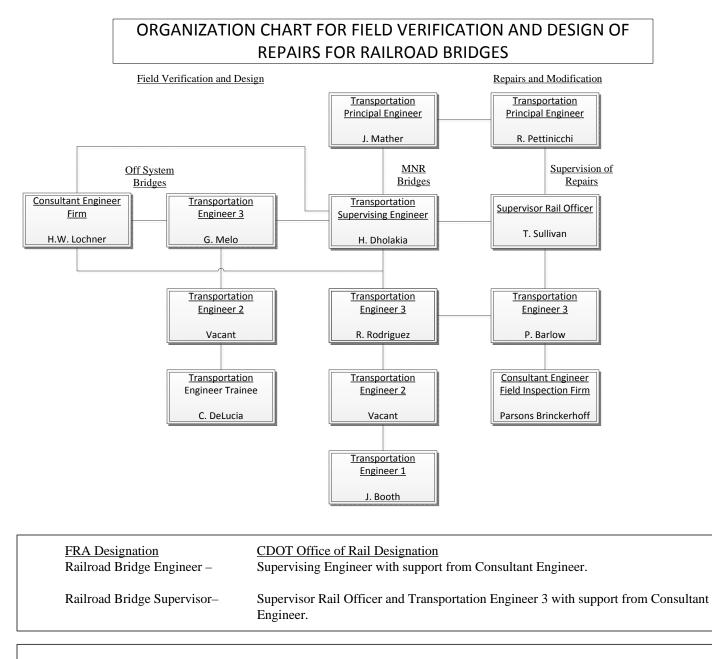


#### OFF SYSTEM ROUTINE AND VERIFICATION INSPECTIONS STATE PROJECT NO. 170-2010 ORGANIZATION CHART CONNDOT – GARG CONSULTING SERVICES, INC. (CONSULTING ENGINEERING FIRM)



Version: 1.0 - February, 2012

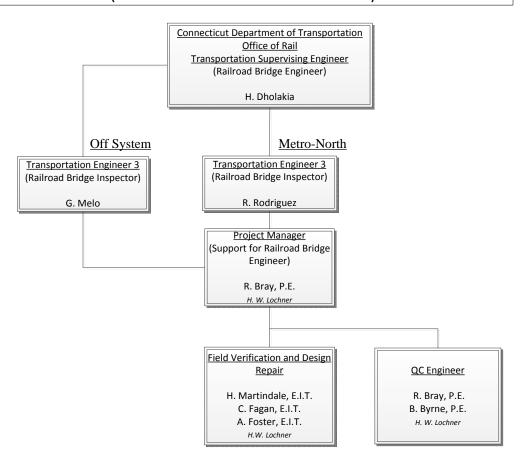
### APPENDIX 6 ORGANIZATION CHARTS FOR FIELD VERIFICATION AND DESIGN OF REPAIRS FOR RAILROAD BRIDGES

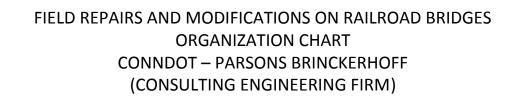


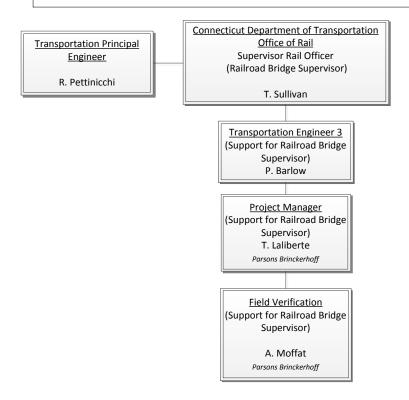
See Page A6-2 - Detail Organization Chart for Field Verification and Design Repairs of Railroad Bridges

See Page A6-3 – Detail Organization Chart for Field Repairs and Modification of Railroad Bridges

#### FIELD VERIFICATION AND DESIGN OF REPAIRS FOR RAILROAD BRIDGES STATE PROJECT NO. 300-0175 ORGANIZATION CHART CONNDOT – H.W. LOCHNER, INC (CONSULTING ENGINEERING FIRM)







Version: 1.0 - February, 2012

### APPENDIX 7 DRAFT SUMMARY OF SAFE LOAD CAPACITY OF METRO-NORTH RAILROAD BRIDGES

# State of Connecticut-Department of Transportation-Office of Rail

## Draft Safe Load Assesment List of Metro-North Railroad Bridge - As of 6/13/11

						Overall Cond.	E80 C	010 New cooper	Co	ontrolling Merr	ber & Location			Demand for Permitted Car	Does Normal Rating	Act	ating for tual	Axle I
Track Chart Name	Town	Bridge Type	Bridge No.	Location	Mile Point	Rating Current		ting Max	Member	Moment or Shear	Track No./Span No.	Rating Span Length (ft.)	Time Table max Equip Load Ibs	from	(Capacity) meets Demand?	Norm	pment Max	for Ac Equip (Kij
DANBURY	NORWALK	U	04134R	MARSHALL STREET	0.11	5	68	86	G1 & G2	Moment	1/1	46.75	263,000					-
DANBURY	NORWALK	U	08200R	ANN STREET	0.19	5	62	78	G4	Moment	2/1	49.58	263,000					-
DANBURY	NORWALK	U	08225R	REED STREET	0.36							64.00	263,000					
DANBURY	NORWALK	U	08201R	NORWALK RIVER - STEEL GIRDER	1.56	7	76	102	G2	Moment	1/1	64.00	263,000					
DANBURY	NORWALK	U	08202R	NORWALK RIVER - STEEL GIRDER	3.20	5	74	96	G1	Moment	1/1	60.40	263,000					-
DANBURY	WILTON	U	08203R	BROOK - MULTIBEAM DECK	5.12	6	124	151	G5	Moment	1/1	11.42	263,000					-
DANBURY	WILTON	U	08204R	STREAM - MULTIBEAM DECK	6.43	4	64	80	G2 & G3	Moment	1/1	35.00	263,000					
DANBURY	WILTON	U	08205R	NORWALK RIVER - STEEL GIRDER	6.64	5	63	79	G1 & G4	Moment	1/1	46.00	263,000					
DANBURY	WILTON	U	08206R	NORWALK RIVER - STEEL GIRDER	8.70	4	69	87	G1 & G2	Moment	1/1	54.19	263,000					
DANBURY	WILTON	U	08207R	NORWALK RIVER - STEEL GIRDER	9.42	5	76	99	G1 & G2	Moment	1/1& 2	40.68	263,000					_
DANBURY	WILTON	U		BROOK - CULVERT MASONRY	9.91	5			N/A	N/A	N/A	N/A	263,000					
DANBURY	WILTON	U		OLD MILL ROAD	11.01	4	53	67	G8	Moment	1/1	27.75	263,000				<u> </u>	-
DANBURY	WILTON	U		NORWALK RIVER - STEEL GIRDER	11.55	5	69	87	G1	Moment	1/1	50.15	263,000				<u> </u>	-
DANBURY	WILTON	U		FACTORY POND	12.17	5	60	77	??	Moment	1/1	33.42	263,000					
DANBURY	REDDING	U		BRANCHVILLE BROOK	12.83													
DANBURY	REDDING	U		OLD REDDING ROAD	14.16	6	93	118	G1-G8	Moment	1/1	21.00	263,000					_
DANBURY	REDDING	U		SIMPAUG TURNPIKE	14.80	6			N/A	N/A	N/A	N/A	263,000				<u> </u>	
DANBURY	REDDING	U		UMPAWAUG POND BROOK - STEEL GIRDER	16.41	5	91	115	G1	Moment	1/1	23.42	263,000					
DANBURY	REDDING			SAUGATUCK RIVER - STEEL GIRDER	17.09	5	76	96	G1	Moment	1/1	43.40	263,000					
DANBURY	BETHEL	U		GRASSY PLAIN ROAD (ROUTE 53)	19.64	5	209	309	G1-G6	Moment	1/1	36.20	263,000					
DANBURY	BETHEL	U		SYMPAUG BROOK - DOUBLE BARREL MASONRY CUL	_													
DANBURY	BETHEL	U		BROOK - CONCRETE	19.99	4			N/A	N/A	N/A	N/A	263,000					_
DANBURY	BETHEL	U		SYMPAUG BROOK - MULTIBEAM DECK	21.41	4	68	94	G1	Moment	1/1	20.00	263,000					
DANBURY	DANBURY	U		SYMPAUG BROOK - CONCRETE SLAB	21.52	5	91	123	G1	Moment	1/1	16.33	263,000					
DANBURY	DANBURY	U		STILL RIVER - STEEL GIRDER	22.39	7	87	140	G1 & G2	Moment	1/1	67.00	263,000					
DANBURY	DANBURY	U		STILL RIVER - CONCRETE CULVERT	22.94	7	99	129	Concrete Culvert	Shear	1/1	22.00	263,000					
DANBURY	DANBURY	U		STILL RIVER - CONCRETE CULVERT	23.18	7	114	148	Concrete Culvert	Moment	1/1	20.00	263,000				<u> </u>	
DANBURY	DANBURY			STILL RIVER - CONCRETE CULVERT	23.42	6	86	112	Concrete Culvert	Moment	1/1	20.00	263,000				<u> </u>	_
MAINLINE	GREENWICH			NORTH WATER STREET	26.10	6	93	140	G2-G17	Moment	T1-T4/1	44.00	263,000					_
MAINLINE	GREENWICH			HAMILTON AVENUE	26.79	7	135	208	G2-G17	Moment	T1-T4/1	27.00	263,000					
	GREENWICH			PEDESTRIAN UNDERPASS AT HAMILTON AVENUE	26.79	6			N/A	N/A	N/A	N/A	263,000					
MAINLINE	GREENWICH			FIELD POINT ROAD	27.77	6			N/A	N/A	N/A	N/A	263,000					
	GREENWICH			ARCH STREET (SR 742)	28.06	7	95	113	G3	Moment	4/1	100.00	263,000					
MAINLINE	GREENWICH	7		PEDESTRIAN OVERPASS FOOTBRIDGE	28.16	8			N/A	N/A	N/A	N/A	263,000				<u> </u>	
MAINLINE	GREENWICH			STEAMBOAT ROAD	28.22		55	69	G1	Moment	3/1	53.47	263,000				<u> </u>	
MAINLINE	GREENWICH			DAVIS AVENUE #2	28.48	4	53	67	G5	Moment	2/1	32.50	263,000				<u> </u>	
MAINLINE	GREENWICH			INDIAN HARBOR (DAVIS MILL POND)	28.68		56	71	G1-G8	Moment	T1-T4/1	62.25	263,000					
	GREENWICH				29.50	5	66	96	G1-68		3/1		-				<u> </u>	
MAINLINE MAINLINE	GREENWICH			SACHEM ROAD LUKES CROSSING (SOUND SHORE DRIVE)	29.50		65	82	G2 G1-G8	Moment Moment	T1-T4/1	25.92 34.33	263,000 263,000				<u> </u>	
MAINLINE MAINLINE	GREENWICH			MIANUS RIVER (COS COB BRIDGE) - MOVABLE	29.08	4	00	02	91-00	Moment	1 1-14/1	54.55	263,000					
					31.03	E			N/A	N/A	N/A	N/A						4
	GREENWICH			ARCH STREET #2 - MASONRY ARCH		5 F							263,000				<u> </u>	
MAINLINE	GREENWICH	U	UOUTUR	STREAM - MASONRY CULVERT	31.12	5			N/A	N/A	N/A	N/A	263,000	l			<u> </u>	

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e load Actual pment (ip)	Meets Safe Load capacity?	Comments

Track Chart	_	Bridge	Bridge		Mile	Overall Cond. Rating	E80 (	010 New Cooper Iting	C	ontrolling Men	nber & Location			Demand for Permitted Car		Load ra Act Equip		Axle I
Name	Town	Туре	No.	Location	Point	Current		Max	Member	Moment or Shear	Track No./Span No.	Rating Span Length (ft.)	Time Table max Equip Load Ibs		(Capacity) meets Demand?	Norm	Мах	Equipı (Kij
MAINLINE	GREENWICH	U	03948R	SOUND BEACH AVENUE	31.29	4	74	122	Pier Cap	Shear	3/	27.00	263,000					1
MAINLINE	GREENWICH	U	03955R	TOMAC AVENUE	31.62	5	57	73	G2-G4	Moment	T1-T4/1	31.96	263,000					
MAINLINE	STAMFORD	U	03680R	GREENWICH AVENUE	32.81	4	61	79	G2-G4	Shear	T1-T4/2	39.81	263,000					
MAINLINE	STAMFORD	U	08011R	RIPPOWAM RIVER - TRUSS	32.85	5	66	102	T1 & T 8/U3 & U4	Axial Comp. & Moment	T3 &T4/1	120.00	263,000					
MAINLINE	STAMFORD	U	03683R	WASHINGTON BOULEVARD (STATE ROUTE 493)	32.97	7	85	142	G3 & G6	Moment	T2 & T3/S1&S2	59.42	263,000					
MAINLINE	STAMFORD	U	08012R	ATLANTIC STREET	33.19	4	49	75	G5	Shear	T2 & T4/2	58.02	263,000					
MAINLINE	STAMFORD	U	03678R	CANAL STREET	33.41	5	67	85	G3, G13, G14	Moment	T5 & T6/1	62.75	263,000					
MAINLINE	STAMFORD	U	03686R	ELM STREET	33.75	4	60	88	G5, G7, G9, G11, G13	Moment & Shear	T3, T1, T2, T4, T6/2	36.60	263,000					
MAINLINE	STAMFORD	U	02237R	EAST MAIN STREET (U.S. ROUTE 1)	34.17	3	48	65	G3 & G5	Shear	T1-T4/2	69.60	263,000					
MAINLINE	STAMFORD	U	08013R	HAMILTON AVENUE	34.72	4	48	73	G3-G5	Shear	T1-T4/1	36.67	263,000					
MAINLINE	STAMFORD	U	08014R	NOROTON RIVER - MASONRY ARCH	35.58	5			N/A	N/A	N/A	N/A	263,000					
MAINLINE	DARIEN	Z	08015R	NOROTON STATION PEDESTRIAN OVERPASS	36.24	5			N/A	N/A	N/A	N/A	263,000					
MAINLINE	DARIEN	U	08016R	STONY BROOK - MASONRY CULVERT	37.16	5			N/A	N/A	N/A	N/A	263,000					
MAINLINE	DARIEN	U	08017R	CUMMINGS BROOK - MASONRY CULVERT	37.36	5			N/A	N/A	N/A	N/A	263,000					
MAINLINE	DARIEN	U	04142R	LEROY AVENUE #1	37.59	6	54	68	G8	Moment	4/1	27.00	263,000					1
MAINLINE	DARIEN	U	00316R	BOSTON POST ROAD (U.S. ROUTE 1)	37.82	4	56	73	New Bridge	New Bridge	New Bridge	New Bridge	263,000					1
MAINLINE	DARIEN	U	08018R	GOODWIVES RIVER - CONCRETE DECK	38.00	7	76	126	B1 & B8	Moment	T3 & T4/1	16.31	263,000					1
MAINLINE	DARIEN	U	08019R	STREAM	38.36													
MAINLINE	DARIEN	U	08020R	TOKENEKE BROOK - 2 STONE CULVERTS	38.53	8			N/A	N/A	N/A	N/A	263,000					
MAINLINE	DARIEN	U	08021R	TOKENEKE BROOK #2 - MASONRY CULVERT	38.68	5			N/A	N/A	N/A	N/A	263,000					
MAINLINE	DARIEN	U	04271R	RAYMOND STREET	38.93	5	54	70	G5 & G6	Moment	2/1	25.21	263,000					
MAINLINE	DARIEN	U	08022R	FIVE MILE RIVER - BRICK ARCH	39.07	5			N/A	N/A	N/A	N/A	263,000					
MAINLINE	NORWALK	U	08023R	DRY STREAM - MASONRY CULVERT	39.08	5			N/A	N/A	N/A	N/A	263,000					1
MAINLINE	NORWALK	U	04132R	ROWAYTON AVENUE	39.11	4	45	59	New Bridge	New Bridge	New Bridge	New Bridge	263,000					
MAINLINE	NORWALK	U	08024R	FARM CREEK	39.55													
MAINLINE	NORWALK	U	08025R	FRANKLIN STREET (SPRING STREET)	40.89	5	62	90	G3-G5	Shear	T1-T4/1	45.33	263,000					
MAINLINE	NORWALK	U	08026R	STATION UNDERPASS	41.02	6			N/A	N/A	N/A	N/A	263,000					
MAINLINE	NORWALK	U	08027R	MONROE STREET	41.12	5	134	171	New Bridge	New Bridge	New Bridge	New Bridge	263,000					
MAINLINE	NORWALK	U	03693R	WASHINGTON & MAIN STREETS	41.28	4	55	86	Center Truss L7-L8	Axial Tension	N/A	112.00	263,000					1
MAINLINE	NORWALK	М	04288R	NORTH WATER STREET & NORWALK RIVER - MOVABL	41.51	5	48	73					263,000					
MAINLINE	NORWALK	U		FORT POINT STREET	41.79	5	60	79	End Floorbeams	Moment	T1-T4/1	12.52	263,000					
MAINLINE	NORWALK	U	08028R	OSBORNE AVENUE	41.96	5	52	67	G2-G4	Moment	T1-T4/1	37.33	263,000					
MAINLINE	NORWALK	U	03691R	EAST AVENUE	42.14	4	50	73	FB1-FB3	Moment	T1-T4/1	12.00	263,000					-
MAINLINE	NORWALK	U		BROOK - MASONRY CULVERT	42.26	5			N/A	N/A	N/A	N/A	263,000					-
MAINLINE	NORWALK	U		STRAWBERRY HILL AVENUE	42.37	6	67	88	G8	Moment	4/1	32.50	263,000					
MAINLINE	WESTPORT	U		STREAM - STONE CULVERT	43.50													
MAINLINE	WESTPORT	U		INDIAN RIVER - TWO CONCRETE PIPES 7' DIAMETER	43.80	7			N/A	N/A	N/A	N/A	263,000					
MAINLINE	WESTPORT	U		SAUGATUCK AVENUE (ROUTE 136)	43.97	5	69	91	G3-G10	Moment	T1-T4/1	40.00	263,000					+
MAINLINE	WESTPORT	U		SAUGATUCK AVE SIDING	43.97													
MAINLINE	WESTPORT	U		NEW STATION UNDERPASS	44.12	7			N/A	N/A	N/A	N/A	263,000					
MAINLINE	WESTPORT	U		STATION UNDERPASS - CONCRETE CULVERT	44.20	6			N/A	N/A	N/A	N/A	263,000					+
MAINLINE	WESTPORT	M		FERRY LANE & SAUGATUCK RIVER - MOVABLE	44.32	5	66	100	G8	Moment	4/3	96.38	263,000					+
			000321		ע.טב						4/3	90.00	200,000					

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e load Actual pment (ip)	Meets Safe Load capacity?	Comments

Track Chart		Bridge	Bridge		Mile	Overall Cond. Rating	E80 (	010 New Cooper tting	Co	ontrolling Mem	ber & Location			Demand for Permitted Car	Does Normal Rating	Act	ating for tual pment	Axle Ic
Name	Town	Туре	No.	Location	Point	Current		Max	Member	Moment or Shear	Track No./Span No.	Rating Span Length (ft.)	Time Table max Equip Load Ibs		(Capacity) meets Demand?	Norm	Max	– Equipm (Kip
MAINLINE	WESTPORT	U	03963R	COMPO ROAD	44.70	6	68	89	G1-G8 & East FB	Moment	1/1	38.50	263,000					
MAINLINE	WESTPORT	U	08033R	SHERWOOD MILL POND - CONCRETE CULVERT	45.75	5			N/A	N/A	N/A	N/A	263,000					
MAINLINE	WESTPORT	U	08034R	MUDDY BROOK - CONCRETE CULVERT	46.11	6			N/A	N/A	N/A	N/A	263,000					
MAINLINE	WESTPORT	U	08035R	NEW CREEK ROAD	47.15	5	60	87	G2, G3, G5-G8	Moment	4/1	31.75	263,000					
MAINLINE	WESTPORT	U	08036R	GREENS FARMS BROOK - METAL PIPE CULVERT	47.29	6			N/A	N/A	N/A	N/A	263,000					
MAINLINE	WESTPORT	U	08037R	MAPLE LANE	47.44	5	53	83	G1-G3	Moment	T1 & T3/1	39.04	263,000					
MAINLINE	WESTPORT	U	08038R	SASCO RIVER - MULTIBEAM DECK	48.29	6	67	113	G2-G31	Moment	T1-T4/1	34.00	263,000				<u> </u>	
MAINLINE	FAIRFIELD	U	08039R	WESTWAY ROAD	48.65	4			New Bridge	New Bridge	New Bridge	New Bridge	263,000				<u> </u>	
MAINLINE	FAIRFIELD	U	08040R	MASONRY CULVERT	48.80													
MAINLINE	FAIRFIELD	U	04200R	CENTER STREET	48.81	6	63	94	FB1 @ G7 & G8	Moment	4/1	10.50	263,000				<b></b>	
MAINLINE	FAIRFIELD	U	08041R	SPRUCE STREET	48.91	5	72	107	FB18 @ G1 & G2	Moment	3/1	10.50	263,000				<b></b>	
MAINLINE	FAIRFIELD	U	08042R	OLD POST ROAD	49.01	5	67	101	FB1 @ G1 & G2	Moment	3/1	10.50	263,000				<u> </u>	
MAINLINE	FAIRFIELD	U	08043R	MILL RIVER - MULTIBEAM DECK	49.66	5	64	81	G3 & G4	Moment	1/1	74.33	263,000				<b></b>	
MAINLINE	FAIRFIELD	U	08044R	NORTH PINE CREEK ROAD	50.02	5	58	86	FB1 & FB14 @ G5/G6		2/1	10.50	263,000				ļ	
MAINLINE	FAIRFIELD	U	04197R	MILL PLAIN ROAD	50.29	5	65	99	End FB2 & FB23 @ G5 & G6	Shear	2/1	10.50	263,000					
MAINLINE	FAIRFIELD	U	04198R	ROUND HILL ROAD	50.90	5	59	89	FB19 @ G1 & G2	Moment	3/1	10.50	263,000					
MAINLINE	FAIRFIELD	U	01344R	NORTH BENSON ROAD (ROUTE 135)	51.12	4	46	72	New Bridge	New Bridge	New Bridge	New Bridge	263,000					
MAINLINE	FAIRFIELD	U	08045R	FAIRFIELD CREEK - MULTIBEAM DECK	51.68	5	97	141	G1-G24	Moment	T1-T4/1	13.92	263,000					
MAINLINE	FAIRFIELD	U	08046R	CULVERT	52.29													
MAINLINE	FAIRFIELD	U	08047R	ASH CREEK - MULTIBEAM DECK	53.00	6	72	118	G25	Moment	4/1	34.00	263,000					
MAINLINE	BRIDGEPORT	U	03635R	FAIRFIELD AVENUE (ROUTE 130)	53.42	4	47	72	New Bridge	New Bridge	New Bridge	New Bridge	263,000					
MAINLINE	BRIDGEPORT	U	08048R	BOSTWICK AVENUE	53.60	5	54	70	G2-G4	Moment	T1-T4/2	40.00	263,000					
MAINLINE	BRIDGEPORT	U	08049R	HANCOCK AVENUE	53.72	5	68	88	G3 & G4	Moment	T1, T2, T4/2	40.00	263,000					
MAINLINE	BRIDGEPORT	U	08050R	HOWARD AVENUE	53.84	5	60	105	G7	Shear	4/2	40.00	263,000					
MAINLINE	BRIDGEPORT	U	08051R	WORDIN AVENUE	54.07	5	59	87	G1	Moment	3/2	51.17	263,000					
MAINLINE	BRIDGEPORT	U	08052R	IRANISTAN AVENUE	54.22	5	64	93	G6	Moment	2/2	43.65	263,000					
MAINLINE	BRIDGEPORT	U	08053R	SOUTH AVENUE	54.44	4	73	95	New Bridge	New Bridge	New Bridge	New Bridge	263,000					
MAINLINE	BRIDGEPORT	U	03638R	PARK AVENUE	54.58	5	64	83	G3	Moment	T1 & T2/2	56.61	263,000				<u> </u>	
MAINLINE	BRIDGEPORT	U	08054R	MYRTLE AVENUE	54.70	5	68	92	G2-G4	Shear	T1-T4/2	32.91	263,000				<u> </u>	
MAINLINE	BRIDGEPORT	U	08055R	WARREN STREET	54.77	5	71	103	C1-C8	Axial & Bending	T1-T4(Piers 1 &2)/2	30.73	263,000				ļ	
MAINLINE	BRIDGEPORT	U	08056R	LAFAYETTE STREET	54.84	5	74	129	G1, G3, G5, G7	Shear	T1-T4/2	30.29	263,000				ļ	
MAINLINE	BRIDGEPORT	U		BROAD STREET	54.93	5	67	106	G4	Shear	1/2	30.67	263,000				<b></b>	
MAINLINE	BRIDGEPORT	U	08058R	MAIN STREET #1	54.98	4	52	68	New Bridge	New Bridge	New Bridge	New Bridge	263,000				<b></b>	
MAINLINE	BRIDGEPORT	U	08059R	HOUSATONIC CROSSING	55.13	5	72	114	South End FB supported by west truss	Moment	T1 & T3/1	18.17/4.70	263,000					
MAINLINE	BRIDGEPORT	Z	08107R	SOUTH PEDESTRIAN OVERPASS	55.35	7			N/A	N/A	N/A	N/A	263,000					
MAINLINE	BRIDGEPORT	U	08060R	UNION STREET	55.40	4	67	99	G6	Moment	T2 & T4/1	33.75	263,000					
MAINLINE	BRIDGEPORT	Z	08108R	NORTH PEDESTRIAN OVERPASS	55.41	8			N/A	N/A	N/A	N/A	263,000					1
MAINLINE	BRIDGEPORT	U	08061R	BRIDGEPORT HARBOR - REINFORCED CONCRETE SLA	55.41	5	44	73	Span 5/Pier 5	Moment	All/5	25.00	263,000					1
MAINLINE	BRIDGEPORT		08104R	BRIDGEPORT HARBOR - PEDESTRIAN TUNNEL	55.50	7			N/A	N/A	N/A	N/A	263,000					
MAINLINE	BRIDGEPORT	U	08062R	STATION VIADUCT	55.51	6	85	111	G1 & G4	Moment	T1 & T3/7	38.33	263,000					
MAINLINE	BRIDGEPORT	U	03636R	STRATFORD AVENUE (ROUTE 130)	55.61	7	75	125	G2	Moment	T1 - T4/2	73.50	263,000					1

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e load Actual pment Kip)	Meets Safe Load capacity?	Comments

Track Chart	Town	Bridge	Bridge	e Location	Mile	Overall Cond. Rating	E80	010 New Cooper ating	С	ontrolling Men	nber & Location			Demand for Permitted Car	Does Normal Rating		ating for tual	Axle I for Ac
Name		Туре	No.		Point	Current			Member	Moment or Shear	Track No./Span No.	Rating Span Length (ft.)	Time Table max Equip Load lbs		(Capacity) meets Demand?	Norm	Max	– Equipi (Kij
MAINLINE	BRIDGEPORT	U	08063R	VIADUCT - PARKING LOT NORTH	55.69	7	81	117	G1-G8	Moment	T1-T4/22	43.29 (G2, G3, G6, G7)/43.45 (G1, G4, G5, G8)	263,000					
MAINLINE	BRIDGEPORT	М	08064R	PEQUONNOCK RIVER - MOVABLE	55.90	6	85	125	FB7 (No. & So.)	Moment	T1-T4/3	12.80	263,000					
MAINLINE	BRIDGEPORT	U	08065R	VIADUCT	55.91	7	77	124	G6	Moment	2/4	45.49	263,000					
MAINLINE	BRIDGEPORT	U	08066R	PULASKI STREET	55.95	7	73	94	G1, G4, G5, G8	Moment	T1-T4/1	61.70	263,000					
MAINLINE	BRIDGEPORT	U	08067R	VIADUCT	55.97	7	75	122	G1	Moment	3/1	59.45	263,000					
MAINLINE	BRIDGEPORT	U	08068R	NOBLE AVENUE	55.98	7	74	129					263,000					
MAINLINE	BRIDGEPORT	U	08101R	VIADUCT	55.99	7	71	84	G8	Moment	4/2	68.27	263,000					
MAINLINE	BRIDGEPORT	U	08102R	CLARENCE STREET	56.00	7	78	126	FB0	Moment	T1 & T3/1	35.25	263,000					
MAINLINE	BRIDGEPORT	U	08069R	KOSSUTH STREET	56.10	7	103	157	G2, G3, G4	Moment	T1-T4/1	46.55	263,000					
MAINLINE	BRIDGEPORT	U	03639R	EAST MAIN STREET (ROUTE 127)	56.20	5	78	100	G1, G3, G5	Shear (G1 & G5)/Moment (G3)	T1-T4/2	40.92	263,000					
MAINLINE	BRIDGEPORT	U	08070R	PEMBROKE STREET	56.35	4	72	92	G3	Moment	T1 & T2/2	32.52	263,000					
MAINLINE	BRIDGEPORT	U	08071R	HALLETT STREET	56.46	4	57	87	G3	Shear	T1 & T2/2	30.24	263,000					
MAINLINE	BRIDGEPORT	U	08072R	YELLOW MILL POND - BRICK ARCH	56.68	5			N/A	N/A	N/A	N/A	263,000					
MAINLINE	BRIDGEPORT	U	08073R	YELLOW MILL POND SIDING	56.68													
MAINLINE	BRIDGEPORT	U	08074R	SEAVIEW AVENUE	56.77	4	55	98	G3 & G4	Shear	T1, T2, T4/2	30.04	263,000					
MAINLINE	BRIDGEPORT	U	08075R	BISHOP AVENUE	57.46	5	64	80	FB1	Moment	1/2	12.00	263,000					
MAINLINE	BRIDGEPORT	U	08076R	BRUCE BROOK - CONCRETE PIPE 9' DIAMETER	57.54	5			N/A	N/A	N/A	N/A	263,000					
MAINLINE	STRATFORD	U	08077R	BRUCE AVENUE	57.62	5	65	84	G5	Moment	T2 & T4/1	32.50	263,000					
MAINLINE	STRATFORD	U	08078R	WEST BROAD STREET	58.72	4	70	89	G1 & G5	Moment	T3 & T4/1	33.42	263,000					
MAINLINE	STRATFORD	U	08079R	KING STREET	58.88	5	74	96	G2 & G4	Shear	T1-T4/1	32.53	263,000					
MAINLINE	STRATFORD	U	01318R	MAIN STREET (ROUTE 113)	59.01	4	56	72	G4	Moment	T2 & T4/2	40.00	263,000					
MAINLINE	STRATFORD	U	01312R	EAST MAIN STREET (ROUTE 110)	59.96	4	60	91	G2 & G4	Shear	T1-T4/1	42.44	263,000					
MAINLINE	STRATFORD	М	08080R	HOUSATONIC RIVER (DEVON BRIDGE)	60.42	4	50	74					263,000					
MAINLINE	MILFORD	U	08081R	BEAVER CREEK - MASONRY ARCH	61.62	5			N/A	N/A	N/A	N/A	263,000					
MAINLINE	MILFORD	U	08082R	BEARDSLEY AVENUE	62.94	5	68	88	G3, G7, G8	Moment	T1 & T4/1	32.58	263,000					
MAINLINE	MILFORD	U	03640R	HIGH STREET	63.27	8	80	124	G2, G3, G4	Moment	T1-T4/1	47.50	263,000					
MAINLINE	MILFORD	U	03644R	RIVER STREET	63.44	7	86	134	G2, G3, G4	Moment	T1-T4/2	52.40	263,000					
MAINLINE	MILFORD	U	08083R	WEPAWAUG RIVER & PROSPECT STREET	63.53	7	88	134	G1	Moment	1/2	31.83	263,000					
MAINLINE	MILFORD	U	08084R	GULF STREET	63.83	5	58	72	G7	Moment	T4/1	25.67	263,000					
MAINLINE	MILFORD	U	08085R	PEDESTRIAN UNDERPASS AT GULF STREET	63.84	6			N/A	N/A	N/A	N/A	263,000					
MAINLINE	MILFORD	U	08086R	INDIAN RIVER - MULTIBEAM DECK	64.59	4	88	127	G3-G8	Moment	T1-T4/1	32.75	263,000					
MAINLINE	MILFORD	U	08087R	OLD GATE LANE	64.74	5	102	155	G2-G19	Moment	T1-T4/S1 & S2	39.37	263,000					
MAINLINE	MILFORD	U	08088R	QUIRK POND BROOK - MASONRY CULVERT	66.29	5			N/A	N/A	N/A	N/A	263,000					
MAINLINE	MILFORD	U	08090R	DEPOT ROAD	66.66	7	80	134	N/A (LR on file)	N/A (LR on file	e)N/A (LR on file)	N/A (LR on file)	263,000					
MAINLINE	ORANGE	U	08091R	OYSTER RIVER - MASONRY CULVERT	67.50	6			N/A	N/A	N/A	N/A	263,000					
MAINLINE	WEST HAVEN	U	08092R	STREAM - STONE/CONCRETE CULVERT	67.98	6			N/A	N/A	N/A	N/A	263,000					
MAINLINE	WEST HAVEN	U	08093R	MORGAN LANE	68.11	6	85	107	G1-G8	Moment	T1-T4/1	27.50	263,000					
MAINLINE	WEST HAVEN	U	08094R	CULVERT	68.14													
MAINLINE	WEST HAVEN	U	08095R	CULVERT- NEW IN 1995	68.89													
MAINLINE	WEST HAVEN	U	08096R	COVE RIVER - MASONRY CULVERT	69.19	6			N/A	N/A	N/A	N/A	263,000					
MAINLINE	WEST HAVEN	U	01403R	SAW MILL ROAD (ROUTE 162)	69.66	5	65	120	G1 & G8	Shear	T1-T4/1	61.00	263,000					
MAINLINE	WEST HAVEN	U	08097R	CAMPBELL AVENUE	70.19	4	54	67	G1-G6	Moment	T1-T3/2	36.00	263,000					

### Page 4 of 6

e load Actual pment (ip)	Meets Safe Load capacity?	Comments

MAINLINE WE	Town EST HAVEN EST HAVEN EW HAVEN EW HAVEN	Bridge Type U U	Bridge No. 08098R	Location	Mile Point	Rating		+1na			ber & Location	n 	Time Tekk	Demand for Permitted Ca	Rating	Load rating for Actual Equipment		Axle I
MAINLINE WE MAINLINE NE MAINLINE NE NEW CANAAN ST	EST HAVEN EW HAVEN EW HAVEN	_	08098R		Point	Current		ting Max	Member	Moment or Shear	Track No./Span No.	Rating Span Length (ft.)	Time Table max Equip Load Ibs		(Capacity) meets Demand?	Norm	Max	for Ac Equip (Ki
MAINLINE NE MAINLINE NE NEW CANAAN ST	EW HAVEN EW HAVEN	_		WASHINGTON AVENUE	70.36	4	54	80	G4	Moment	T2 & T4/2	33.00	263,000					1
MAINLINE NE	EW HAVEN		08099R	WEST RIVER - CONCRETE BOX	71.26	7	113	188	Box Girder	Moment	All/4	30.00	263,000					
NEW CANAAN ST		U	08100R	STATION UNDERPASS	72.28	6	111	193	??	??	??	??	263,000					
	TAMFORD	Z	08106R	STATE STREET STATION OVERPASS	72.80	7			N/A	N/A	N/A	N/A	263,000					
NEW CANAANST,		U	08150R	VIADUCT ROAD	2.81	7	103	156	G2 (East Girder)	Moment	1/1	82.02	263,000					
. I	TAMFORD	U	08151R	NOROTON RIVER - MULTIBEAM DECK	3.77	5	69	87	G1	Moment	1/1	31.00	263,000					
NEW CANAANNE	EW CANAAN	U	00710R	MERRITT PARKWAY (ROUTE 15)	5.76	4	73	90	Horizontal legs of the frame	Moment	1/1 & 2	35.50	263,000					
NEW CANAANNE	EW CANAAN	U	01302R	OLD STAMFORD ROAD (ROUTE 106)	6.16	5	98	125	G1 & G2	Moment	1/1	50.00	263,000					
NEW CANAANNE	EW CANAAN	U	08154R	WATERWAY - MASONRY CULVERT	7.00	4			N/A	N/A	N/A	N/A	263,000					
WATERBURY MIL	LFORD	U	08250R	BROOK	3.12													
WATERBURY MIL	LFORD	U	08251R	CULVERT	3.30													
WATERBURY MIL	LFORD	U	08252R	UNNAMED STREAM - CONCRETE & RAIL DECK	3.80	5			N/A	N/A	N/A	N/A	263,000					
WATERBURY MIL	LFORD	U	08253R	BROOK - MASONRY CULVERT	4.14	6			N/A	N/A	N/A	N/A	263,000					
WATERBURY MIL	LFORD	U	08254R	BEARDS FARMWAY	4.55	6	121	176	G1-G4	Moment	1/1	20.00	263,000					
WATERBURY MIL	LFORD	U	08255R	FARMWAY CROSSING	4.88	5	81	136	Precast Beam	Moment	1/1	17.42	263,000					
WATERBURY MIL	LFORD	U	08256R	DRAINAGE PIPE - ACC METAL PIPE	4.90													
WATERBURY MIL	LFORD	U	08257R	GOLF CROSSING - REINFORCED CONCRETE SLAB	5.01	6	63	105	B4	Moment	1/1	12.92	263,000					
WATERBURY OR	RANGE	U	08258R	DAVIS BROOK - 2 CONCRETE CULVERTS	7.02	7			N/A	N/A	N/A	N/A	263,000					
WATERBURY OR	RANGE	U	08260R	TWO MILE BROOK (TURKEY BROOK) - CONCRETE CUL	7.36	5			N/A	N/A	N/A	N/A	263,000					
WATERBURY DE	ERBY	U	08261R	STREAM - UNNAMED STREAM	7.92	4			N/A	N/A	N/A	N/A	263,000					
WATERBURY DE	ERBY	U	08262R	NAUGATUCK RIVER - STEEL TRUSS	8.62	4	74	109	FB1-FB11	Moment	Active Track/Spans1-3	16.50	263,000					
WATERBURY DE	ERBY	U	08263R	NAUGATUCK RIVER FLOOD PLAIN	8.68	5			N/A	N/A	N/A	N/A	263,000					
WATERBURY AN	NSONIA	U	08264R	NAUGATUCK RIVER - STEEL TRUSS	10.30	5	102	162	S1 @ FB1 & FB3/S2 @ FB2 & FB3	Moment	1/1	9.66	263,000					
WATERBURY AN	NSONIA	U	08266R	RACEWAY FARREL COMPANY - MASONRY ARCH	10.85	5			N/A	N/A	N/A	N/A	263,000					
WATERBURY AN	NSONIA	U	08267R	SPILLWAY	11.36	5	68	100	G1	Moment	1/2	43.19	263,000					
WATERBURY SE	EYMOUR	U	08268R	CANAL - STEEL GIRDER	12.57	4	89	132	G1	Moment	1/1	43.17	263,000					
WATERBURY SE	EYMOUR	U	08269R	MUD BROOK - MASONRY CULVERT	13.20	5			N/A	N/A	N/A	N/A	263,000					
WATERBURY SE	EYMOUR	U	01321R	MAIN STREET (ROUTES 115 & 313)	14.29	5	75	96	S1 & S2	Shear	1/2	47.42	263,000					
WATERBURY SE	EYMOUR	Z	08270R	JAMES STREET FOOTBRIDGE	14.45	6			N/A	N/A	N/A	N/A	263,000					
WATERBURY SE	EYMOUR	U	01063R	DERBY AVENUE (ROUTE 67)	14.61	7	137	234	S2 @ FB1 & No. Abutment	Shear	1/1	78.84	263,000					
WATERBURY SE	EYMOUR	U	08272R	NAUGATUCK RIVER - MULTIBEAM DECK	14.64	4	71	92	G2	Moment	1/5	47.50	263,000					
WATERBURY BE	EACON FALLS	U	08274R	OLD PINE'S BRIDGE ROAD	16.79	4	97	123	G1A, G1B, G2A, G2B	Moment	1/1	18.00	263,000					
WATERBURY BE	EACON FALLS	U	08275R	HEMP SWAMP BROOK - MASONRY CULVERT	16.81	5			N/A	N/A	N/A	N/A	263,000					
WATERBURY BE	EACON FALLS	U	08276R	SPRUCE BROOK - MULTIBEAM DECK	19.21	4	78	118	G1 & G2	Shear	1/1	17.08	263,000					
WATERBURY NA	AUGATUCK	U	08277R	SUGAR BUSH BROOK - MULTIBEAM DECK	20.11	5	69	85	G1-G4	Moment	1/1	18.15	263,000					
WATERBURY NA	AUGATUCK	U	08278R	US RUBBER COMPANY - PEDESTRIAN TUNNEL	21.22	6			N/A	N/A	N/A	N/A	263,000					
WATERBURY NA	AUGATUCK	U	08279R	PRIVATE ROAD AT UNIROYAL	21.36	4	79	115	G8-G10	Moment	2/1	19.92	263,000					
WATERBURY NA	AUGATUCK	U	08280R	LONGMEADOW POND BROOK - CONCRETE ARCH	21.48	7			N/A	N/A	N/A	N/A	263,000					
WATERBURY NA	AUGATUCK	U	08281R	OVERFLOW - CONCRETE SLAB	21.65	6			N/A	N/A	N/A	N/A	263,000					1
WATERBURY NA	AUGATUCK	U	04224R	MAPLE STREET	21.74	4	77	99	G1	Moment	1/1	63.50	263,000					1
WATERBURY NA	AUGATUCK	U	08282R	CANAL	22.11													
WATERBURY NA	AUGATUCK	U	08283R	HOP BROOK - MULTIBEAM DECK	22.42	4	77	127	G1	Shear	1/1	43.00	263,000					
WATERBURY NA	AUGATUCK	U		BRIDGE STREET (GENERAL PULASKI WALK)	22.74	4	91	134	G1	Moment	1/2	40.68	263,000		1			1

Appendix 7 Draft Safe Load Assesment List of MNR Bridges - 6-13-11-revised.xls

# State of Connecticut-Department of Transportation-Office of Rail

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e load Actual pment (ip)	Meets Safe Load capacity?	Comments

## State of Connecticut-Department of Transportation-Office of Rail

## Draft Safe Load Assesment List of Metro-North Railroad Bridge - As of 6/13/11

						Overall	2008-2010 New E80 Cooper		Controlling Member & Location					Demand for	Does Normal	Load rating for Actual				
Track Chart To Name	Town	· · ·	Bridge No.	Location	Mile	Cond. Rating		ting		Moment or	Track	Rating Span	Time Table	Permitted Car		Equip		Axle load for Actual	Meets Safe Load capacity?	Comments
		Туре	NO.		Point	Current	Norm	Max	Member		No./Span No.		max Equip Load lbs	Graph/Table (cooper)	meets Demand?	Norm	Max	Equipment (Kip)		
WATERBURY	WATERBURY	U	03723R	BRISTOL STREET #1	24.10	5	78	100	G1	Moment	1/1	42.36	263,000							
WATERBURY	WATERBURY	U	08285R	STREAM - STONE/CONCRETE CULVERT	24.34	5			N/A	N/A	N/A	N/A	263,000							
WATERBURY	WATERBURY	U	04232R	WASHINGTON AVENUE #1	26.18	5	74	109	G1	Moment	1/1	60.33	263,000							
WATERBURY	WATERBURY	U	04235R	BANK STREET #1	26.35	4	92	143	S3 & S4	Moment	1/1	74.00	263,000							
WATERBURY	WATERBURY	U	08286R	NAUGATUCK RIVER - STEEL GIRDER	26.42	6	62	81	G4	Moment	1/1	111.80	263,000							
WATERBURY	WATERBURY	U	08287R	JACKSON STREET	26.50	4	86	110	S3 & S4	Moment	1/1	24.17	263,000							
WATERBURY	WATERBURY	U	08288R	GAS CONDUIT	26.64															

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Version: 1.1 - February, 2014

#### APPENDIX 7A METRO NORTH RAILROAD BRIDGE INSPECTION DAMAGE REPORT

	Track & Structu	TH RAILROAD	
	BRIDGE INSPECTIO	N DAMAGE REPORT	
LINE: BRIDGE TYPE: NUMBER OF SPANS:	CROSSING: DECK TYPE: NUMBER OF TRACKS:	MNR BRIDGE NUMBER: TOWN: LEDGTH:	014
TRACK ALIGNMENT:			OH:
DATE OF LAST INSPECTION: HIGHWAY MINIMUM CLEARANCE:		DATE OF THIS INSPECTION: HIGHWAY LOAD LIMIT:	TONS
B.I.N. :		HIGHWAT LOAD LIMIT:	10113
	location of damage, measuren	nents of damaged components, )	
Action Under Load	g_,		
Track On Bridge			
Walkways			
Signage			
Abutments			
Wingwalls			
Barrel			
Parapet walls			
Piers			
Columns			
Cross girders			
Bracing			
Girders			
Floorbeams			
Stringers Laterals			
Lateral Plates			
Rivets or Bolts			
Bearings			
Stiffeners			
Date of Accident: _ Time of Accident: _ Police Case Number: _ Vehicle on scene: _ Vehicle on scene: _ VEHICLE INFORMATION Owner / Operator: _ Address: _ Plate Number: _ US D.O.T. Number: _ Vehicle Height: DESCRIPTION OF ACCIDENT			Time on Accident: ST: OT:
IMPACT ON TRAIN SERVICE			
Duration of interruption:			
Restrictions placed:			
Duration of restriction:			
RECOMMENDED ACTION			
BRIDGE INSPECTOR/ SUPERVISOR	-	REPORT #	

Version: 1.0 - February, 2012

## APPENDIX 8 DRAFT SUMMARY OF SAFE LOAD CAPACITY OF OFF-SYSTEM RAILROAD BRIDGES

## State of Connecticut - Department of Transportation - Office of Rail

## Draft Safe Load Assesment list of Off-system Railroad Bridges - 6/13/11

						Overall	2008-20	10 New							
BRIDGE	M.P.	RAILROAD	TRACK CHART NAME	TOWN	LOCATION	Cond.	E80 C	ooper		Controllin	g Member &	Locations		Demand for	Does N
NO.	IVI.F.	RAILROAD		TOWN	LOCATION	Rating	Rat	ing	Member	Moment or	Track	Rating Span	Time Table Max Equip.	Permitted Car from Graph/Table	(Capa
						Current	Normal	Max	Member	Shear	No./Span No.	Length (ft.)	Load (lbs)	(Cooper)	
00430R		RAILS TO TRAILS*	AVON SECONDARY	FARMINGTON	FARMINGTON AVENUE	-									
00479R		PROV & WORCESTER	WETHERSFIELD SEC.	HARTFORD	AIRPORT ROAD (SR 530)	6	78	144	G1 & G2	Shear	1/1	70.00	263,000	-	
00522R		PROV & WORCESTER	MIDDLETOWN SEC.	MIDDLETOWN	ROUTE 66	7	83	113	All FB's	Moment	1/1	19.85	263,000	-	
00600R		BOSTON & MAINE	TORRINGTON SEC.	WATERBURY	WEST MAIN STREET (SR 847)	3	62	92					263,000	-	
00639R		PROV & WORCESTER	PORTLAND IND. TRK.	MIDDLETOWN	RT. 9 & CONNECTICUT RIVER		50	75					263,000		
00863R		PROV & WORCESTER		MIDDLETOWN	HARTFORD AVENUE	4	71	118	G2	Moment	1/1	65.67	263,000		
01373R		PROV & WORCESTER	MIDDLETOWN SEC.	MIDDLEFIELD	RTE 147 (BAILEYVILLE ROAD)	5	77	117	G1 & G2	Moment	1/1	80.50	263,000		
01624R		RAILS TO TRAILS*	AVON SECONDARY	AVON	ARCH ROAD	_									<u> </u>
01849R		PROV & WORCESTER	WILLIMANTIC BRANCH	WINDHAM	ABANDONED RD/PEDESTRIAN	5	63	93	G2/FB's 1-5	Moment	1/1	48.60/12.60	263,000		
04234R		BOSTON & MAINE	TORRINGTON SEC.	WATERBURY	FREIGHT STREET	5	63	118					263,000		
04242R		INACTIVE	NEW BRITAIN SEC.	NEW BRITAIN	ROUTE 9 (BUSWAY)	6			N/A	N/A	N/A	N/A	263,000		
09000R		PROV & WORCESTER	CROMWELL IND. TRACK	MIDDLETOWN	SEBETHE RIVER	4	69	102	G1 & G2	Moment	1/1	24.10	263,000		
09001R		PROV & WORCESTER		MIDDLETOWN	COGINCHAUG RIVER	4	59	88	G1 & G2	Moment	1/1	46.17	263,000		
09002R		PROV & WORCESTER	LAUREL IND. TRACK	MIDDLETOWN	GREEN STREET	5	89	131	G3, G4, G5, & G6		1/1	??	263,000		
09003R		PROV & WORCESTER	LAUREL IND. TRACK	MIDDLETOWN	SUMNER BROOK	4	70	107	G1	Moment	1/2	51.20	263,000		
09004R		PROV & WORCESTER	LAUREL IND. TRACK	MIDDLETOWN	EASTERN DRIVE	5	86	126	G1-G6	Moment	1/1	21.66	263,000		
09005R		PROV & WORCESTER	LAUREL IND. TRACK	MIDDLETOWN	RESERVOIR BROOK	5	72	126	G2	Shear	1/1	28.70	263,000		
09006R	2.32	PROV & WORCESTER	LAUREL IND. TRACK	MIDDLETOWN	INDIAN HILL BROOK	4	67	99	G1	Moment	1/1	27.33	263,000		
09007R	2.97	PROV & WORCESTER	LAUREL IND. TRACK	MIDDLETOWN	FOREST CREEK	4	168	247	Concrete Encased Girder	Moment	1/?	??	263,000		
09008R	3.23	PROV & WORCESTER	LAUREL IND. TRACK	MIDDLETOWN	ASYLUM CREEK	4	84	126	Gilder G1	Shear	1/1	22.50	263,000		
09009R		PROV & WORCESTER		MIDDLETOWN	MAROMAS CREEK	5	92	139	G2	Shear	1/1	18.50	263,000		
09010R		PROV & WORCESTER	MIDDLETOWN SEC.	MIDDLEFIELD	CATTLE PASS	5	68	100	G1 & G2	Moment	1/1	10.50	263,000		
09011R		PROV & WORCESTER	MIDDLETOWN SEC.	MIDDLEFIELD	COGINCHAUG RIVER	5	75	117	G1 & G2	Shear	1/1	52.00	263,000		
09012R		PROV & WORCESTER	MIDDLETOWN SEC.	MIDDLEFIELD	FARMWAY	4	69	104	M11 & M12/T1 & T2	Moment	1/2	113.25	263,000		
09014R		PROV & WORCESTER	MIDDLETOWN SEC.	MIDDLETOWN	COGINCHAUG RIVER	4	68	104	L12 & M11 (T1 & T2	Moment	1/1	118.12	263,000		
09015R		PROV & WORCESTER	MIDDLETOWN SEC.	MIDDLETOWN	UNKNOWN STREAM	7			N/A	N/A	N/A	N/A	263,000		
09100R		BOSTON & MAINE	TORRINGTON SEC.	WATERBURY	CRANE STREET	4	96	141	11/74	110/73	11/7	19/73	263,000		
09101R		BOSTON & MAINE	TORRINGTON SEC.	WATERBURY	CONC. BOX/PIPE CULVERT	5			N/A	N/A	N/A	N/A	263,000		
09104R		NAUGATUCK	TORRINGTON SEC.	WATERBURY	MAHAN CANAL	6	85	137	G1 & G2	Shear	1/1	20.40	263,000		
09105R		NAUGATUCK	TORRINGTON SEC.	WATERBURY	BROOK	0	00	137	010.02	Sileai	1/1	20.40	203,000		
09106R		NAUGATUCK	TORRINGTON SEC.	WATERBURY	HANCOCK BROOK	4	77	116	G2	Moment	1/1 & 2	53.33	263,000		
09100R				WATERBURY	AMERICAN PIN FOOTBRIDGE	6			N/A	N/A	N/A	N/A	263,000		
		NAUGATUCK NAUGATUCK	TORRINGTON SEC.			-									
09108R		NAUGATUCK	TORRINGTON SEC.	WATERBURY		4	90	135	G1 & G2	Moment	1/1-4	56.00	263,000		
09109R			TORRINGTON SEC.	WATERTOWN			00	00	01.8.00	Managart	1/0	05.05	263,000		
09110R		NAUGATUCK	TORRINGTON SEC.	WATERTOWN	NAUGATUCK RIVER	5	62	93	G1 & G2	Moment	1/3	65.25	263,000		<u> </u>
09111R		NAUGATUCK	TORRINGTON SEC.	THOMASTON	BROOK	6	00	400	01.0.00	N.4	4/4	10.00	263,000	-	
09112R		NAUGATUCK	TORRINGTON SEC.	THOMASTON	RAILROAD STREET ANNEX	6	89	132	G1 & G2	Moment	1/1	18.00	263,000	-	
09113R		NAUGATUCK	TORRINGTON SEC.	THOMASTON	BROOK - CULVERT	5			N/A	N/A	N/A	N/A	263,000	-	
		NAUGATUCK	TORRINGTON SEC.	THOMASTON	NAUGATUCK RIVER DAM SP.	6	65	100	FB0-FB28	Moment	1/1	20.42	263,000	-	
09115R		NAUGATUCK	TORRINGTON SEC.	THOMASTON	DAM CONTROL	7			N/A	N/A	N/A	N/A	263,000		-
		NAUGATUCK	TORRINGTON SEC.	LITCHFIELD	BROOK - CULVERT-CONC. PIF				N/A	N/A	N/A	N/A	263,000		<u> </u>
09117R		NAUGATUCK	TORRINGTON SEC.	LITCHFIELD	CULVERT	5			N/A	N/A	N/A	N/A	263,000		<u> </u>
09118R		NAUGATUCK	TORRINGTON SEC.	LITCHFIELD	BROOK	4			N/A	N/A	N/A	N/A	263,000		L
09119R		NAUGATUCK	TORRINGTON SEC.	LITCHFIELD	CULVERT										
09120R		NAUGATUCK	TORRINGTON SEC.	LITCHFIELD	SPRUCE BROOK	6	86	136	G1 & G2	Moment	1/1	39.00	263,000		
		NAUGATUCK	TORRINGTON SEC.	TORRINGTON	BROOK - CULVERT	3			N/A	N/A	N/A	N/A	263,000		
09122R	18.37	NAUGATUCK	TORRINGTON SEC.	TORRINGTON	GULF STREAM	5	81	119	G1	Moment	1/1	27.10	263,000		

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s Normal Rating apacity) meets	Load rating Equip	g for Actual oment	Actual Equipment	Meets Safe Load capacity?	Comments
Demand?	Normal	Max	(kip)		

## State of Connecticut - Department of Transportation - Office of Rail

## Draft Safe Load Assesment list of Off-system Railroad Bridges - 6/13/11

BRIDGE						Overall Cond.	2008-20 E80 C			Controllin	g Member & I	Locations		Demand for	Does No
NO.	M.P.	RAILROAD	TRACK CHART NAME	TOWN	LOCATION	Rating	Rat	ing	Member	Moment or Shear	Track No./Span No.	Rating Span Length (ft.)	Time Table Max Equip.	Permitted Car from Graph/Table	(Capac Des No Des
004000	40.00			TODDINOTON		Current	Normal		00				Load (lbs)	(Cooper)	
09123R		NAUGATUCK	TORRINGTON SEC.	TORRINGTON	SUMMER STREET	4	99	147	G3	Moment	1/1	37.10	263,000		
09124R 09125R		NAUGATUCK NAUGATUCK	TORRINGTON SEC.	TORRINGTON TORRINGTON	PASSWAY NAUGATUCK RIVER	5	78 65	132 98	G3 & G4 G1 & G2	Shear Moment	1/1 1/3	17.50 46.50	263,000 263,000		
09125R		HOUSATONIC	NEW MILFORD SEC.	NEW MILFORD	STREAM - STONE CULVERT	3			N/A	N/A	N/A	40.50 N/A	286,000		
09200R		HOUSATONIC	NEW MILFORD SEC.	NEW MILFORD	BROOK - STONE CULVERT	6			N/A N/A	N/A	N/A	N/A N/A	286,000	-	+
09201R 09202R		HOUSATONIC	NEW MILFORD SEC.	NEW MILFORD	BROOK - CULVERT	5			N/A N/A	N/A	N/A N/A	N/A N/A	286,000		+
09202R		HOUSATONIC	NEW MILFORD SEC.	NEW MILFORD	BROOK - CULVERT	6			N/A N/A	N/A	N/A N/A	N/A N/A	286,000		
09203R		HOUSATONIC	NEW MILFORD SEC.	KENT	SWAMP (CULVERT)	4			N/A N/A	N/A	N/A N/A	N/A N/A	286,000		
09204R		HOUSATONIC	NEW MILFORD SEC.	KENT	COBBLE BROOK	3			N/A N/A	N/A	N/A	N/A N/A	286,000		
09200R		HOUSATONIC	NEW MILFORD SEC.	KENT	MAUWEE BROOK	5			N/A	N/A	N/A	N/A	286,000		
09207R		HOUSATONIC	NEW MILFORD SEC.	KENT	KENT FALLS BROOK-CULVER	-			N/A N/A	N/A	N/A	N/A	286,000		
09209R		HOUSATONIC	NEW MILFORD SEC.	KENT	CULVERT-DBL 4' CULVERTS	0			11/74	IN/A	IN/A	IN/A	200,000		-
092091 09210R		HOUSATONIC	NEW MILFORD SEC.	CORNWALL	DEEP BROOK	5			N/A	N/A	N/A	N/A	286,000		-
09210R		HOUSATONIC	NEW MILFORD SEC.	CORNWALL	MILLARD BROOK	6			N/A	N/A	N/A	N/A	286,000		+
09211R		HOUSATONIC	NEW MILFORD SEC.	CORNWALL	GUNN BROOK	4			N/A N/A	N/A	N/A	N/A	286,000		+
09212R		HOUSATONIC	NEW MILFORD SEC.	CORNWALL	BONNEY BROOK	6			N/A N/A	N/A N/A	N/A N/A	N/A N/A	286,000		
09213R		HOUSATONIC	NEW MILFORD SEC.	CORNWALL	FURNACE BROOK	4	70	105	G2	Moment	1/1	29.29	286,000		
09214R		HOUSATONIC	NEW MILFORD SEC.	CORNWALL	BROOK	5			N/A	N/A	N/A	N/A	286,000		
09215R		HOUSATONIC	NEW MILFORD SEC.	CORNWALL	MILL BROOK	5	93	137	G3 & G4	Moment	1/1	23.50	286,000		
09217R 09218R		HOUSATONIC	NEW MILFORD SEC.	CORNWALL	STREAM	6	93	137	S1-S6	Moment	1/1	15.50	286,000		
09218R		HOUSATONIC	NEW MILFORD SEC.	CANAAN	CHILD POND BROOK	5			N/A	N/A	N/A	N/A	286,000		
09219R 09220R		HOUSATONIC	NEW MILFORD SEC.	CANAAN	BROOK - STONE SLAB	5			N/A N/A	N/A	N/A	N/A N/A	286,000		
09220R		HOUSATONIC	NEW MILFORD SEC.	CANAAN	WATER STREET	5	84	124	G1	Moment	1/1	26.08	286,000		
09221R		HOUSATONIC	NEW MILFORD SEC.	CANAAN	BRANCH HOUSATONIC RIVER		121	179	All Girders	Moment	1/1	51.50	286,000		
09222R		HOUSATONIC	NEW MILFORD SEC.	CANAAN	HOLLENBECK RIVER	4	34	45	All Gilders	Moment	1/1	51.50	286,000		+
09223R		HOUSATONIC	NEW MILFORD SEC.		BLACKBERRY RIVER	5	88	131	G1	Shear	1/1	45.40	286,000		+
09224R 09225R		HOUSATONIC	NEW MILFORD SEC.		NORTH BLACKBERRY RIVER	6			N/A	N/A	N/A	43.40 N/A	286,000		
09223R		RAILS TO TRAILS*	KENDALL'S IND. TRACK	WINDHAM	NATCHAUG RIVER	0			11/74	IN/A	IN/A	IN/A	200,000		-
09300R		PROV & WORCESTER	WILLIMANTIC BRANCH	SPRAGUE	WALDO BROOK	5			N/A	N/A	N/A	N/A	263,000		-
09302R		PROV & WORCESTER	WILLIMANTIC BRANCH	SCOTLAND	MERRICK BROOK	6	69	115	G1 & G2	Moment	1/1	54.58	263,000		+
09302R		PROV & WORCESTER	WILLIMANTIC BRANCH	WINDHAM	FROG BROOK	6			N/A	N/A	N/A	N/A	263,000		
09304R		PROV & WORCESTER	WILLIMANTIC BRANCH	WINDHAM	SHETUCKET RIVER	5	107	160	G1 & G2	Moment	1/All	57.83	263,000		+
09305R		PROV & WORCESTER	WILLIMANTIC BRANCH	WINDHAM	PLAINS ROAD	5	76	114	G1 & G2	Moment	1/1	33.71	263,000		
09306R		PROV & WORCESTER	WILLIMANTIC BRANCH	WINDHAM	WILLIMANTIC RIVER	5	70	105	010.02	WOMEN	17.1	55.71	263,000		
09307R		PROV & WORCESTER	WILLIMANTIC BRANCH	WINDHAM	STATION FOOTBRIDGE	4			N/A	N/A	N/A	N/A	263,000		
09800R		RAILS TO TRAILS*	AVON SECONDARY	FARMINGTON	SCOTT SWAMP BROOK					11// (	10// (	10/7	200,000		
09802R		RAILS TO TRAILS*	AVON SECONDARY	FARMINGTON	FARMINGTON RIVER										
09803R		RAILS TO TRAILS*	AVON SECONDARY	AVON	THOMPSON BROOK										
		RAILS TO TRAILS*	AVON SECONDARY		NOD BROOK										
		RAILS TO TRAILS*	AVON SECONDARY	AVON	ROSEWOOD ROAD										
		RAILS TO TRAILS*	AVON SECONDARY	SIMSBURY	HOP BROOK										
		RAILS TO TRAILS	AVON SECONDARY	SIMSBURY	ROBERT'S CATTLE PASS										
		RAILS TO TRAILS	AVON SECONDARY	EAST GRANBY	FARMINGTON CANAL										
09809R		RAILS TO TRAILS*	AVON SECONDARY	EAST GRANBY	SALMON RIVER										
		RAILS TO TRAILS	AVON SECONDART	EAST GRANBY	GRIFFIN'S BROOK										
		RAILS TO TRAILS	AVON SECONDART	SUFFIELD	MUDDY RIVER-CONC. ARCH										
09811R		RAILS TO TRAILS	AVON SECONDARY	SUFFIELD	FARMINGTON CANAL										
09812R		RAILS TO TRAILS*	AVON SECONDARY AVON SECONDARY	SUFFIELD	STREAM										
09013R	51.27	INALS TO TRAILS	AVON SECONDART		OTALAW										

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es Normal Rating Capacity) meets Demand?	Equip	g for Actual oment	Actual Equipment	Meets Safe Load capacity?	Comments
Demand?	Normal	Max	(kip)		
	-				

## State of Connecticut - Department of Transportation - Office of Rail

## Draft Safe Load Assesment list of Off-system Railroad Bridges - 6/13/11

						Overall	2008-20	10 New												
BRIDGE	M.P.	RAILROAD	TRACK CHART NAME	TOWN	LOCATION	Cond.	E80 Co	-		Controlling	Member &	Locations		Demand for	Does Normal Rating	Load ratin	g for Actual	Axle load for	Manta Cafa Lagad	
NO.		KAIEKOAD		10111	LOOATION	Rating	Rati	ng	Member	Moment or	Track	Rating Span	Time Table Max Equip.	Permitted Car from Graph/Table	(Capacity) meets	Equi	pment	Actual Equipment	Meets Safe Load capacity?	Comments
						Current	Normal	Max	Member	Shear	No./Span No.	Length (ft.)	Load (lbs)	(Cooper)	Demand?	Normal	Max	(kip)		
09814R		CENTRAL NEW ENGLAND		SOUTH WINDSOR	DRY BROOK	5	84	124	G1 - G8	Moment	1/1	22.10	263,000							
09815R	12.26	CENTRAL NEW ENGLAND	E. LONGMEADOW SEC.	EAST WINDSOR		4			N/A	N/A	N/A	N/A	263,000							
09816R	15.81	CENTRAL NEW ENGLAND	E. LONGMEADOW SEC.	EAST WINDSOR	BROAD BROOK AND PRIV. RD	5	80	119	G1	Moment	1/1&2	40.17	263,000							
09817R	15.91	CENTRAL NEW ENGLAND	E. LONGMEADOW SEC.	EAST WINDSOR	CULVERT	4			N/A	N/A	N/A	N/A	263,000							
09818R	19.44	CENTRAL NEW ENGLAND	E. LONGMEADOW SEC.	ENFIELD	SCANTIC RIVER	5	N/A	N/A	N/A	N/A	N/A	N/A	263,000							
09819R		CENTRAL NEW ENGLAND			WATER STREET	4	88	133	G2	Moment	1/1	63.17	263,000							
09820R		CENTRAL NEW ENGLAND		ENFIELD	TERRY BRK-2 CULVERTS	5			N/A	N/A	N/A	N/A	263,000							
09821R		CENTRAL NEW ENGLAND			STREAM	4			N/A	N/A	N/A	N/A	263,000							
09823R		CENTRAL NEW ENGLAND			CULVERT															
09824R		CENTRAL NEW ENGLAND		HARTFORD	STREAM	5			N/A	N/A	N/A	N/A	263,000							
09825R	6.81	CENTRAL NEW ENGLAND	GRIFFIN'S INDUST. TRK.	BLOOMFIELD	STREAM (WASH BROOK)	4			N/A	N/A	N/A	N/A	263,000							
09826R		INACTIVE	NEW BRITAIN SEC.	NEW BRITAIN	CEMETERY UNDERPASS(BW)	4			N/A	N/A	N/A	N/A	263,000							
09827R	5.82	INACTIVE	NEW BRITAIN SEC.	NEW BRITAIN	PIPER BROOK (BUSWAY)	7			N/A	N/A	N/A	N/A	263,000							
09828R	14.30	PROV & WORCESTER	WETHERSFIELD SEC.	CROMWELL	SOUTH STREET	5	78	115	G6	Moment	1/1	22.00	263,000							
09829R		PROV & WORCESTER	WETHERSFIELD SEC.	CROMWELL	NOOKS HILL ROAD	6	85	125	S1-S6	Moment	1/1	22.21	263,000							
09830R	9.60	PROV & WORCESTER	WETHERSFIELD SEC.		EVAN'S ROAD	5	97	142	G1, G2, G4	Moment	1/1	15.50	263,000							
09831R		PROV & WORCESTER	WETHERSFIELD SEC.	ROCKY HILL	GOFF'S BROOK	6			N/A	N/A	N/A	N/A	263,000							
09832R		PROV & WORCESTER	WETHERSFIELD SEC.		BEAVER BROOK-CULVERT															
09833R		PROV & WORCESTER	WETHERSFIELD SEC.	HARTFORD	FOLLY BROOK-CULVERT	4			N/A	N/A	N/A	N/A	263,000							
09834R		PROV & WORCESTER	WETHERSFIELD SEC.	HARTFORD	THRASHER'S CROSSING	5	86	128	G1-G6	Moment	1/1	21.91	263,000							
09835R	35.71	PROV & WORCESTER	WETHERSFIELD SEC.	CROMWELL	FIRE ACCESS ROAD	5	97	142	G1-G4	Moment	1/1	13.20	263,000							
09836R	10.50	PROV & WORCESTER	WETHERSFIELD SEC.	ROCKY HILL	DIVIDEND BROOK	6			N/A	N/A	N/A	N/A	263,000							
09901R	30.39	ABANDONED	AIRLINE VL MAP 54-64/29	EAST HAMPTON	MINE BROOK-STONE CULV.	5			N/A	N/A	N/A	N/A	263,000							
09902R	31.53	ABANDONED	AIRLINE VL MAP 54-64/30	EAST HAMPTON	PINE BROOK	6														
09903R	32.20	ABANDONED	AIRLINE VL MAP 54-64/31	EAST HAMPTON	MUDDY GUTTER BROOK	6			N/A	N/A	N/A	N/A	263,000							
09904R	33.09	ABANDONED	AIRLINE VL MAP 54-64/32	EAST HAMPTON	MAIN STREET	7			N/A	N/A	N/A	N/A	263,000							
09905R	33.17	ABANDONED	AIRLINE VL MAP 54-64/32	EAST HAMPTON	BROOK	5			N/A	N/A	N/A	N/A	263,000							
09908R	32.54		MOOSUP V MAP 54-67/13	PLAINFIELD	MOOSUP RIVER	2			N/A	N/A	N/A	N/A	263,000							
09909R	31.53	ABANDONED	MOOSUP V MAP 54-67/15	PLAINFIELD	HORSE BROOK															
09911R	32.81	RAILS TO TRAILS*	FARMINGTON VALLEY GRE	FARMINGTON	HYDE BROOK-STONE ARCH															
09912R	33.09	RAILS TO TRAILS*	VAL MAP 56-60/4	FARMINGTON	OLD BROOK-ARCH CULVERT															
09913R	33.17	RAILS TO TRAILS*	VAL MAP 56-60/8	CANTON	FARMINGTON RIVER															
09914R	33.19	RAILS TO TRAILS*	VAL MAP 56-60/8	CANTON	FARMINGTON CANAL															
09916R	53.61	INACTIVE	VAL. MAP 57-72/54	WINCHESTER	STILL RIVER	5			N/A	N/A	N/A	N/A	263,000							
09921R	5.61	RAILS TO TRAILS*	VAL MAP 56-60/6	BURLINGTON	BARNES BROOK - DEP															
09922R	6.14	RAILS TO TRAILS*	VAL MAP 56-60/7	BURLINGTON	CEDAR BROOK - DEP															
09923R	6.37	RAILS TO TRAILS*	VAL MAP 56-60/7	BURLINGTON	BROOK - DEP															

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Version: 1.0 - February, 2012

## APPENDIX 9 EQUIVALENT COOPER LOAD CHARTS, SEPTEMBER 2011



# Connecticut Railroad Equipment Equivalent Cooper Load Charts

Prepared for:

**Connecticut Department of Transportation Office of Rail** 



September, 2011

Prepared by:



H.W. Lochner, Inc 2110 Silas Deane Highway Rocky Hill, CT 06067



## **CONNDOT OFFICE OF RAIL** CONNECTICUT RAILROAD EQUIPMENT • EQUIVALENT COOPER LOAD CHARTS

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CONNDOT OFFICE OF RAIL

CONNECTICUT RAILROAD EQUIPMENT • EQUIVALENT COOPER LOAD CHARTS

## **EXECUTIVE SUMMARY**

As requested by the Connecticut Department of Transportation, H.W. Lochner has developed Axle Loading Diagrams for actual equipment known to operate over the railroad bridges in Connecticut; and for Association of American Railroads (AAR) freight car standard configurations for unrestricted interchange (See Appendix for AAR standards). The AAR freight car is the standard used by ConnDOT for classifying the live load capacity of rail lines in the state and the most conservative (highest load effect) axle configurations were used to develop load diagrams for freight cars loaded to gross weights of 220 kips, 263 kips and 286 kips. Load charts were developed for the railroad equipment with moments and shear effects for span lengths of 5' to 100' equated to the theoretical Cooper live load series as defined by the American Railway Engineering and Maintenance-of-Way Association (AREMA).

These charts were developed using the load generating capabilities of the STAAD (<u>ST</u>ructural <u>A</u>nalysis <u>And D</u>esign) computer program to develop influence lines for each type of equipment, as well as for the Cooper E 80 loading. From these charts, the maximum moments and shears were obtained for the various span lengths. The equivalent Cooper E rating for each type of equipment was then calculated by using the following equation:

Equivalent Cooper E Rating (equipment) = [(M or V) (equipment) ÷ (M or V) (Cooper E80)] × 80,

where M or V correspond to the maximum moment or shear effect produced by the particular load for the various span lengths analyzed.

Based on these calculations, the Axle Load Diagrams showing Span vs. Equivalent Cooper E Rating were plotted for the following equipment:

- Alco RS-3 Locomotive
- Amtrak AMD-103(Genesis) Locomotive
- Bombardier Acela Power Car
- Bombardier HHP-8 Locomotive
- Brookville BL20GH Locomotive
- Electric Motive Division AEM-7 Locomotive
- Electric Motive Division FL9m Locomotive
- Electric Motive Division FL9AC Locomotive
- Electric Motive Division GP8 Locomotive
- Electric Motive Division GP9 Locomotive
- Electric Motive Division GP35 Locomotive
- Electric Motive Division GP40 Locomotive
- Electric Motive Division GP40-2 Locomotive
- General Electric P32AC (Genesis) Locomotive
- General Electric P40DC/P42DC (Genesis) Locomotive
- General Electric U23-B Locomotive
- General Electric B40-8 Locomotive
- Metro-North M-2 Coaches
- Metro-North M-4 Coaches
- Metro-North M-6 Coaches
- Metro-North M-8 Coaches
- 220 Kip Hopper Car
- 220 Kip AAR Freight Car
- 263 Kip AAR Freight Car
- 286 Kip AAR Freight Car
- 263 Kip Triple Hopper Ballast Service Car
- 286 Kip Triple Hopper Ballast Service Car



Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts



It is the intent that these charts provide a means of screening bridges that have moderate *Cooper load* ratings in order to determine whether additional analysis is required.

The following should be considered when using these charts.

- 1. They were developed based on the analysis of simple spans. As such, they are strictly applicable to simple span longitudinal girder bridges, or to simple span stringers which are part of a girder (or truss) / floorbeam / stringer structural system.
- 2. They may be used for an approximate evaluation of simple span girders which are part of a girder / floorbeam / stringer structural system. Engineering judgment is required to determine the applicability of the charts under these conditions.
- 3. They cannot be used under any circumstances for the evaluation of floorbeams.
- 4. Variations to the impact factor and centrifugal forces produced by the different types of equipment analyzed were not considered in the development of these charts.
- 5. The charts only consider the coupling of similar equipment. There may be cases where the coupling of different types of equipment may produce higher equivalent Cooper E ratings than those shown.



#### CONNECTICUT RAILROAD EQUIPMENT TABLE A EQUIVALENT COOPER LOADING SUMMARY

				PL 0	English			T	in the second			Thus Fig. 7	Late Devi	
-		PER E80	May Cha	FL9m Single		TINC	Mary Cha	Two FL9m		TINC	May 61	Two FL9m Bac		TINC
span			Max. Shear			TING		Max. Moment	"E" RA		Max. Shear	Max. Moment		TING
(Feet)	(KIP)	(FT KIP)	(NIP)	(FT KJP)	SHEAR	MOMENT	(KIP)	(FT KIP)	SHEAR	MOMENT	(K.1P)	(FT KIP)	SHEAR	MOMENT
5	80	100	56	71	E 56	E 56	56	71	E 56	E 56	56	70	E 56	E 56
10	120 160	22.5 500	93	141 2.52	E 50	E 50 E 40	93	141 252	E 50	E 50	90	141 2.52	E 50	E 50
20	200	82.5	112	462	E 46 E 45	E 40		463	E 46 E 45	E 40 E 45	110	463	E 45 E 44	E 40 E 45
20	200	1224	123	673	E 43	E 45	112 123	674	E 43	E 45 E 44	122	674	E 44 E 43	E 45 E 44
30	252	1642	131	885	E 43	E 44	137	886	E 43	E 44	133	886	E 43	E 44
35	277	2088	131	1096	E 39	E 43	149	1097	E 43	E 43	146	1098	E 42	E 43
40	302	2623	146	1308	E 39	E 40	165	1336	E 44	E 41	163	1309	E 43	E 40
45	302	32.02	155	1519	E 38	E 38	178	1611	E 43	E 40	180	1592	E 44	E 40
50	349	3804	167	1731	E 38	E 36	188	1942	E 43	E 41	196	1946	E 45	E 41
60	392	5196	186	2212	E 38	E 34	205	2647	E 42	E 41	220	2768	E 45	E 43
70	442	6830	200	2848	E 36	E 33	228	3352	E 41	E 39	241	3588	E 44	E 42
80	497	8638	210	3534	E 34	E 33	254	4204	E 41	E 39	265	4503	E 43	E 42
90	549	10678	218	4233	E 32	E 32	276	5191	E 40	E 39	286	5490	E 42	E41
100	600	12893	224	4938	E 30	E 31	293	6178	E 39	E 38	302	6610	E 40	E41
	C00	PER E80		FL9AC Single	Engine			Two FL9AC	in line			Two FL9AC Bad	k to Back	
span	Max. Shear	Max. Moment	Max. Shear	Max. Moment	"E" R/	TING	Max. Shear	Max. Moment	"E" RA	TING	Max. Shear	Max. Moment	"E" RA	TING
(Feet)	(KIP)	(FT KIP)	(NUP)	(FT KIP)	SHEAR	MOMENT	(KIP)	(FT KIP)	SHEAR	MOMENT	(KIP)	(FT KIP)	SHEAR	MOMENT
5	80	100	57	71	E 57	E 57	57	71	E 57	E 57	57	71	E 57	E 57
10	120	22.5	75	142	E 50	E 50	75	142	E 50	E 50	75	142	E 50	E 50
15	160	500	93	2.53	E 47	E 41	93	253	E 47	E 41	91	2.53	E 45	E41
20	200	82.5	113	465	E45	E 45	113	466	E 45	E 45	111	466	E 44	E45
25	227	1224	124	678	E 44	E 44	124	679	E 44	E 44	123	679	E 43	E 44
30	252	1642	132	891	E 42	E 43	138	892	E 44	E 43	134	892	E 43	E 43
35	277	2088	137	1104	E 40	E 42	150	1105	E 43	E 42	147	1105	E 43	E 42
40	302	2623	147	1317	E 39	E 40	166	1346	E 44	E 41	164	1318	E 44	E40
45	327	3202	156	1530	E 38	E 38	179	1622	E 44	E 41	182	1603	E 44	E 40
50	349	3804	168	1743	E 39	E 37	189	1956	E 43	E 41	198	1960	E 45	E41
60	392	5196	187	2228	E 38	E 34	206	2666	E 42	E 41	222	2788	E 45	E 43
70	442	6830	201	2868	E 36	E 34	229	3376	E 42	E 40	243	3614	E 44	E 42
80	497	8638	212	3559	E 34	E 33	256	4234	E 41	E 39	267	4534	E 43	E 42
90	549	10678	220	4264	E 32	E 32	278	5228	E 40	E 39	288	5528	E 42	E41
400														
100	600	12893	226	4974	E 30	E 31	296	6222	E 39	E 39	304	6657	E 41	E41
100	600	12893	226	4974	E 30	E 31	296	6222	E 39	E 39	304	6657	E 41	E 41
100		12893 PER E80	226	4974 P32AC single		E 31	296	6222 Two P32AC		E 39	304	6657 Two P32AC Bad		E 41
span	COO		226 Max. Shear	P32AC single							304 Max. Shear	Two P32AC Bad	k to Back	E 41
	COO	PER E80		P32AC single	Engine			Two P32AC	in line			Two P32AC Bad	k to Back	
span	COO Max. Shear	PER E80 Max. Moment	Max. Shear	P32AC single Max. Moment (FT KIP) 87	Engine "E" R/	TING	Max. Shear	Two P32AC Max. Moment	in line "E" R4	TING	Max. Shear	Two P32AC Bao Max. Moment	:k to Back "E" R⁄	TING
span (Feet)	COO Max. Shear (XIP)	Max. Moment	Max. Shear	P32AC single Max. Moment (FT KIP)	Engine "E" R/ SHEAR	ATING	Max. Shear	Two P32AC Max. Moment	in line "E" RA SHEAR	ATING MOMENT	Max. Shear	Two P32AC Bao Max. Moment (FT KIP)	K to Back "E" RA SHEAR	ATING MOMENT
span (Feet) 5	COO Max. Shear (KIP) 80	PER E80 Max. Moment (FT KIP) 100	Max. Shear (KIP) 69	P32AC single Max. Moment (FT KIP) 87	Engine "E" R/ SHEAR E 69	ATING MOMENT E 70	Max. Shear (KIP) 69	Two P32AC Max. Moment (FT KP) 87	in line "E" RA SHEAR E 69	TING MOMENT E 70	Max. Shear (KD) 69 76 97	Two P32AC Bac Max. Moment (FT KIP) 87	K to Back "E" RA SHEAR E 69	ATING MOMENT E 70
span (Feet) 5 10	СОО Max. Shear (жр) 80 120	PER E80 Max. Moment (FT KIP) 100 22 5	Max. Shear (кир) 69 76	P32AC single Max. Moment (FT KIF) 87 174	Engine "E" R/ shear E 69 E 51	ATING MOMENT E 70 E 62	Max. Shear (KIP) 69 76	Two P32AC Max. Moment (F7 KP) 87 174	in line "E" RA SHEAR E 69 E 51	ATING MOMENT E 70 E 62	Max. Shear (KSP) 69 76	Two P32AC Bao Max. Moment (FT KIP) 87 174	K to Back "E" RA SHEAR E 69 E 51	атіng <i>момент</i> Е 70 Е 62
span (Feet) 5 10 15	COO Max. Shear (೫೫) 80 120 160	PER E80 Max. Moment (FT KIP) 100 22.5 500	Мах. Shear ( <sup>кир</sup> ) 69 76 97	P32AC single Max. Moment (FT KIP) 87 174 261	Engine "E" R/ <i>SHEAR</i> E 69 E 51 E 49	АТING <i>момелт</i> Е 70 Е 62 Е 42	Max. Shear (KIP) 69 76 97	Two P32AC Max. Moment (F7 KΦ) 87 174 261	in line "E" RA SHEAR E 69 E 51 E 51 E 49	атіng <i>момент</i> Е 70 Е 62 Е 42	Max. Shear (KD) 69 76 97	Two P32AC Bao Max. Moment (FT KIP) 87 174 261	K to Back "E" RA SHEAR E 69 E 51 E 51 E 49	АТING <i>момент</i> Е 70 Е 62 Е 42
span (Feet) 5 10 15 20	COO Max. Shear (XIP) 80 120 160 200	PER E80 Max. Moment (FT KIP) 100 22 5 500 82 5	Max. Shear (KUP) 69 76 97 108	Р32AC single Max. Moment (FT к/P) 87 174 261 415	Engine "E" R/ <i>SHEAR</i> E 69 E 51 E 49 E 43	TING MOMENT E 70 E 62 E 42 E 40	Max. Shear (XiP) 69 76 97 108	Two P32AC Max. Moment (FT KΦ) 87 174 261 415	in line "E" RA SHEAR E 69 E 51 E 49 E 43	TING MOMENT E 70 E 62 E 42 E 40	Max. Shear (KD) 69 76 97 108	Тwo P32AC Bao Max. Moment (FT КIР) 87 174 261 415	:k to Back "E" RA <i>SHEAR</i> E 69 E 51 E 49 E 43	АТІNG <i>момент</i> Е 70 Е 62 Е 42 Е 40
span (Feet) 5 10 15 20 25 30 35	COO Max. Shear ( <i>KP</i> ) 80 120 160 200 227 252 277	PER E80 Max. Moment (#7 ##) 100 225 500 825 1224 1642 2088	(KUP) 69 76 97 108 114 118 121	P32AC single Max, Moment (F7 KP) 87 174 261 415 584 753 922	Engine "E" R/ SHEAR E 69 E 51 E 49 E 43 E 40 E 38 E 35	ATING MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 35	Max. She ar (KIP) 69 76 97 108 114 129 142	Two P32AC Max. Moment (FT κΦ) 87 174 261 415 584 753 944	in line "E" R/ <i>SHБАR</i> E 69 E 51 E 49 E 43 E 40 E 41 E 41	TING MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 36	Max. Shear (XP) 69 76 97 108 115 131 145	Two P32AC Bao Max. Moment ( <i>FT KBP</i> ) 87 174 261 415 584 753 982	ск to Back "E" RA Б 69 Е 51 Е 49 Е 43 Е 40 Е 41 Е 41 Е 42	ATING MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 38
span (Feet) 5 10 15 20 25 30	COO Max. Shear (XEP) 80 120 160 200 227 252 277 252 277 302	PER E80 Max. Moment (FT xiP) 100 22 5 500 82 5 1224 1642	(KUP) 69 76 97 108 114 118	P32AC single Max. Moment (FT KF) 87 174 261 415 584 753	Engine "E" RV SHEAR E 69 E 51 E 49 E 43 E 40 E 38	TING MOMENT E 70 E 62 E 42 E 40 E 38 E 37	Мах. She ar (КИР) 69 76 97 108 114 129	Two P32AC Max. Moment (FT κΦ) 87 174 261 415 584 753	in line "E" R/ SHБАR E 69 E 51 E 49 E 43 E 40 E 41	TING MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 36 E 37	Max. Shear (XIP) 69 76 97 108 115 131 145 162	Two P32AC Bac Max. Moment (FT KIP) 87 174 261 415 584 753	K to Back "E" RA SHEAR E 69 E 51 E 49 E 43 E 40 E 41	ATING MOMENT E 70 E 62 E 42 E 40 E 38 E 37
span (Feet) 5 10 15 20 25 30 35 40 45	СОО Мах. Shear (кё) 80 120 160 200 227 252 252 252 277 302 327	PER E80 Max. Moment (FF xii) 100 225 500 825 1224 1642 2088 2623 3202	Max. Shear (KUP) 69 76 97 108 114 118 121 123 127	P32AC single Max. Moment (#7 #9) 87 174 261 415 584 753 922 1091 1260	Engine "E" R/ SHEAR E 69 E 51 E 49 E 43 E 43 E 40 E 38 E 35 E 33 E 31	ATING MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 35	Max. Shear (KIP) 69 76 97 108 114 129 142 159 172	Two P32AC Max. Moment (F7 x8) 87 174 261 415 584 753 944 1205 1469	in line "E" R/ <i>SHБАR</i> E 69 E 51 E 49 E 43 E 40 E 41 E 41	TING MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 36 E 37 E 37	Max. Shear (KP) 69 76 97 108 115 131 145 162 175	Two P32AC Bat Max. Moment (F7 kk) 87 174 261 415 584 753 982 1242 1535	Ck to Back "E" RA SHEAR E 69 E 51 E 49 E 43 E 40 E 41 E 42 E 43 E 43 E 43	TING MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 38 E 38 E 38 E 38 E 38
span (Feet) 5 10 15 20 25 30 35 40 45 50	COO Max. Shear (KF) 80 120 160 200 227 252 277 252 277 302 327 349	PER E80 Max. Moment (#7 K#) 100 225 500 825 1224 1642 2088 2623	Max. Shear (XIP) 69 76 97 108 114 118 114 118 121 123	P32AC single Max. Moment (FF KI) 87 174 261 415 584 753 922 1091 1260 1429	Engine "E" R/ SHEAR E 69 E 51 E 49 E 43 E 43 E 40 E 38 E 35 E 33 E 31 E 31	TING MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 35 E 33 E 31 E 30	Max. She ar (KIP) 69 76 97 108 114 129 142 159 142 159 172 183	Two P32AC Max. Moment (FT K8) 87 174 261 415 584 753 944 1205	in line "E" R/ SHEAR E 69 E 51 E 49 E 43 E 40 E 41 E 41 E 41 E 41 E 42 E 42 E 42 E 42	TING MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 36 E 37 E 37 E 38	Max. Shear (KD) 69 76 97 108 115 131 145 131 145 162 175 185	Two P32AC Bac Max. Moment (FT kib) 87 174 261 415 584 753 982 1242 1535 1871	Ekto Back "E" RA SHEAR E 69 E 49 E 43 E 49 E 43 E 40 E 41 E 42 E 43 E 43 E 43 E 43 E 43 E 43	TING MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 38 E 38 E 38
span (Feet) 5 10 15 20 25 30 35 40 45 50 60	COO Max. Shear (%P) 80 120 160 200 227 252 277 302 327 349 392	PER E80 Max. Moment (FT Kill) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196	Max. Shear (NUP) 69 76 97 108 114 118 121 123 127 135 156	P32AC single Max. Moment (FT KIP) 87 174 261 415 584 753 922 1091 1260 1429 1772	Engine "E" RV SHEAR E 69 E 51 E 49 E 43 E 40 E 38 E 35 E 33 E 31 E 31 E 32	TING MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 35 E 33 E 31 E 30 E 27	Max. She ar (KIP) 69 76 97 108 114 129 142 159 142 159 172 183 198	Two P32AC Max. Moment (F7 x8) 87 174 261 415 584 753 944 1205 1469	in line "E" RA SHEAR E 69 E 51 E 49 E 43 E 40 E 41 E 41 E 42 E 42 E 42 E 41	TING MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 36 E 37 E 36 E 37 E 38 E 37 E 38 E 37 E 38 E 38	Max. Shear (KP) 69 76 97 108 115 131 145 162 175 185 200	Two P32AC Bac Max. Moment (FT k0) 87 174 261 415 584 753 982 1242 1535 1871 2548	k to Back "E" RA SHEAR E 69 E 51 E 49 E 43 E 40 E 41 E 42 E 43 E 43 E 42 E 43 E 42 E 41	TING MOMENT E 70 E 62 E 42 E 40 E 38 E 38 E 38 E 38 E 38 E 39 E 39 E 39
span (/Feet) 5 10 15 20 25 30 35 40 45 50 60 70	COO Max. Shear (%) 80 120 160 200 227 252 277 302 327 302 327 349 392 442	PER E80 Max. Moment (FF xir) 100 225 500 825 1224 1642 2088 2623 3202 3804	Max. Shear (KDP) 69 76 97 108 114 118 121 123 127 135	Р32AC single Max. Moment (FT KH) 87 174 261 415 584 753 922 1091 1260 1429 17772 2177	Engine Engine SHEAR E 69 E 51 E 49 E 43 E 40 E 38 E 33 E 31 E 31 E 31	TING MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 35 E 33 E 31 E 30	Мах. Shear (XiP) 69) 76 97 108 114 129 142 159 172 183 198 211	Two P32AC Max. Moment (FT K8) 87 174 261 415 584 753 584 753 944 1205 1469 1805	in line "E" RA SHEAR E G9 E 51 E 49 E 43 E 40 E 41 E 41 E 42 E 42 E 42 E 42 E 42 E 41 E 38	TING MDMENT E 70 E 62 E 42 E 40 E 38 E 37 E 36 E 37 E 37 E 38 E 38 E 37 E 38 E 37 E 38 E 37	Max. Shear (KØ) 69 76 97 108 115 131 145 162 175 185 200 213	Two P32AC Bac Max. Moment (FT ki/) 87 174 261 415 584 753 982 1242 1535 1871 2548 3226	к to Back "E" R/ 5 нЕАР E 69 E 51 E 49 E 43 E 40 E 41 E 42 E 43 E 43 E 43 E 43 E 43 E 43 E 43 E 43	TING MOMENT E 70 E 62 E 42 E 40 E 38 E 38 E 38 E 38 E 38 E 39 E 38
span (Feet) 5 10 15 20 25 30 35 40 45 50 60 70 80	COO Max. Shear (KP) 80 120 160 200 227 252 277 302 327 302 327 349 392 442 497	PER E80 Max. Moment (FT Kill) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196	Max. Shear (NP) 69 76 97 108 114 118 121 123 127 135 156 173 186	P32AC single Max. Moment (FF ki) 87 174 261 415 584 753 922 1091 1260 1429 1772 2177 2777	Engine "E" RV SHEAR E 69 E 51 E 49 E 43 E 40 E 38 E 35 E 33 E 31 E 31 E 32	TING MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 35 E 33 E 31 E 30 E 27	Max. Shear (KIP) 69 76 97 108 114 129 142 142 159 172 183 198 211 230	Two P32AC Max. Moment (F7 x8) 87 174 261 415 584 753 944 1205 1469 1805 2483 3158 3832	in line "E" RA SHEAR E 69 E 51 E 49 E 43 E 40 E 41 E 41 E 42 E 42 E 42 E 41	TING MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 36 E 37 E 36 E 37 E 38 E 37 E 38 E 37 E 38 E 38	Max. Shear (KB) 69 76 97 108 115 131 145 162 175 185 200 213 233	Two P32AC Bat Max. Moment (F7 kk) 87 174 261 415 584 753 982 1242 1535 1871 2548 3226 3898	k to Back "E" R/ SHEAR E 69 E 51 E 49 E 43 E 40 E 41 E 42 E 43 E 43 E 43 E 42 E 43 E 42 E 41 E 39 E 37	TING MOMENT E 70 E 62 E 42 E 40 E 38 E 38 E 38 E 38 E 38 E 39 E 39 E 39
span (Feet) 5 10 15 20 25 30 35 30 35 30 35 40 45 50 60 70 80 90	COO Max. Shear ( <i>KP</i> ) 80 120 160 200 227 252 277 302 327 349 392 442 497 549	PER E80 Max. Moment (FF xir) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830 8638 10678	Max. Shear (##) 69 76 97 108 114 118 121 123 127 135 156 173 186 197	P32AC single Max. Moment (FT ki) 87 174 261 415 584 753 922 1091 1260 1429 1772 2177 2177 2772 3448	Engine Engine E E 69 E 51 E 49 E 43 E 40 E 38 E 35 E 33 E 31 E 31 E 31 E 32 E 33 E 33 E 31 E 32 E 33 E 32 E 33 E 32 E 33 E 33	TING MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 35 E 33 E 31 E 30 E 27 E 26 E 26 E 26	Max. Shear (xiii) 69 76 97 108 114 129 142 159 142 159 172 183 198 211 230 250	Two P32AC Max. Moment (FT K8) 87 174 261 415 584 753 944 1205 1469 1805 2483 3158 3832 4541	in line: "E" R/ SHEAR E 69 E 51 E 49 E 43 E 40 E 41 E 41 E 42 E 42 E 42 E 42 E 42 E 42 E 37 E 36	TING MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 36 E 37 E 36 E 37 E 38 E 37 E 38 E 38 E 37 E 38 E 37 E 38 E 33 E 33 E 33 E 33 E 33 E 33 E 33	Max. Shear (KB) 69 76 97 108 115 131 145 162 175 185 200 213 233 253	Two P32AC Bac Max. Moment (FT kb) 87 174 261 415 584 753 982 1242 1535 1871 2548 3226 3898 4619	k to Back "E" R/ SHEAR E 69 E 51 E 49 E 43 E 40 E 41 E 42 E 43 E 42 E 43 E 42 E 43 E 42 E 41 E 37 E 37	XTING MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 38 E 38 E 39 E 39 E 39 E 39 E 39 E 39 E 39 E 35
span (Feet) 5 10 15 20 25 30 35 40 45 50 60 70 80	COO Max. Shear (KP) 80 120 160 200 227 252 277 302 327 302 327 349 392 442 497	PER E80 Max. Moment (#7 K8) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830 8638	Max. Shear (NP) 69 76 97 108 114 118 121 123 127 135 156 173 186	P32AC single Max. Moment (FF ki) 87 174 261 415 584 753 922 1091 1260 1429 1772 2177 2777	Engine "E" R/ SHEAR E 51 E 49 E 43 E 40 E 38 E 33 E 31 E 33 E 31 E 32 E 31 E 30	MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 35 E 33 E 31 E 30 E 27 E 26 E 26	Max. Shear (KIP) 69 76 97 108 114 129 142 142 159 172 183 198 211 230	Two P32AC Max. Moment (F7 x8) 87 174 261 415 584 753 944 1205 1469 1805 2483 3158 3832	in line "E" Rf SHEAR E 51 E 49 E 43 E 40 E 41 E 41 E 41 E 42 E 42 E 42 E 42 E 42 E 42 E 43 E 38 E 37	MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 36 E 37 E 37 E 37 E 37 E 38 E 37 E 38 E 37 E 35	Max. Shear (KB) 69 76 97 108 115 131 145 162 175 185 200 213 233	Two P32AC Bat Max. Moment (F7 kk) 87 174 261 415 584 753 982 1242 1535 1871 2548 3226 3898	k to Back "E" R/ SHEAR E 69 E 51 E 49 E 43 E 40 E 41 E 42 E 43 E 43 E 43 E 42 E 43 E 42 E 41 E 39 E 37	TING MOMENT E 70 E 62 E 42 E 40 E 38 E 36 E 36
span (Feet) 5 10 15 20 25 30 35 30 35 30 35 40 45 50 60 70 80 90	COO Max. Shear (%) 80 120 160 200 227 252 277 302 327 349 392 442 497 549 600	PER E80 Max. Moment (FT Kill) 100 225 500 825 1224 1642 2088 2623 3202 3202 3202 3804 5196 6830 8638 10678 12893	Мах. Shear (жР) 69 76 97 108 114 118 121 123 127 135 156 173 186 197 205	P32AC single Max. Moment (FT KIP) 87 174 261 415 584 753 922 1091 1260 1260 1260 1260 12772 2177 27772 3448 4122	Engine Engine SHEAR E 69 E 51 E 49 E 43 E 40 E 33 E 33 E 31 E 31 E 32 E 31 E 30 E 29 E 27	TING MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 35 E 33 E 31 E 30 E 27 E 26 E 26 E 26	Max. Shear (xiii) 69 76 97 108 114 129 142 159 142 159 172 183 198 211 230 250	Two P32AC Max. Moment (FT KØ) 87 174 261 415 584 753 944 1205 1469 1805 2483 3158 3832 4541 5410	in line "E" RA SHEAR E 69 E 51 E 49 E 43 E 40 E 41 E 41 E 42 E 42 E 42 E 41 E 38 E 37 E 36 E 36	TING MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 36 E 37 E 36 E 37 E 38 E 37 E 38 E 38 E 37 E 38 E 37 E 38 E 33 E 33 E 33 E 33 E 33 E 33 E 33	Max. Shear (KB) 69 76 97 108 115 131 145 162 175 185 200 213 233 253 253 270	Two P32AC Bac Max. Moment (FF kb) 87 174 261 415 584 753 982 1242 1535 1871 2548 3226 3898 4619 5488	k to Back "E" R/ SHEAR E 69 E 51 E 49 E 43 E 40 E 41 E 42 E 43 E 53 E 63 E 76 E 77 E 77	MOMENT           E70           E 62           E 42           E 40           E 38           E 37           E 38           E 38           E 39           E 39           E 38           E 35           E 35
span (Feet) 5 10 15 20 25 30 35 40 45 50 60 70 80 90 100	COO Max. Shear ( <i>NP</i> ) 80 120 200 227 252 277 302 327 349 392 442 497 549 600	PER E80 Max. Moment (FT KB) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830 8638 10678 12893 PER E80	Мах. Shear (квр) 76 97 108 114 118 121 123 127 135 156 173 186 197 205	Р32AC single Max. Moment ( <i>FT KP</i> ) 87 174 261 415 584 753 922 1091 1260 1429 1777 2177 2177 2177 2177 2448 4122	Engine Engine SHEAR E 69 E 51 E 49 E 43 E 40 E 33 E 31 E 31 E 31 E 31 E 32 E 31 E 30 E 29 E 27 ngle Engine	TING MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 35 E 33 E 31 E 30 E 27 E 26 E 26 E 26 E 26	Мах. Shear (XUP) 69) 76 97 108 114 129 142 159 172 183 198 211 230 250 267	Two P32AC Max. Moment (FT KØ) 87 174 261 415 584 753 944 1205 1469 1805 2483 3158 3832 4541 5410 Two P40DC/P43	in line "E" R/ SHEAR E 69 E 51 E 49 E 43 E 40 E 41 E 41 E 42 E 42 E 42 E 42 E 42 E 42 E 42 E 42 E 42 E 43 E 37 E 36 E 36 E 36 E 36 E 36 E 20 E 36 E 37 E 36 E 36 E 37 E 36 E 37 E 36 E 37 E 36 E 37 E 38 E 37 E 36 E 37 E 38 E 37 E 36 E 36 E 37 E 36 E 36 E 37 E 36 E 37 E 36 E 36 E 37 E 36 E 36 E 37 E 36 E 36 E 36 E 37 E 36 E 36 E 36 E 37 E 36 E 36	TING MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 36 E 37 E 37 E 38 E 37 E 34 E 37 E 34 E 34	Max. Shear (KØ) 69 76 97 108 115 131 145 162 175 185 200 213 233 253 270	Two P32AC Bac Max. Moment (FT ki/) 87 174 261 415 584 753 982 1242 1535 1871 2548 3226 3898 4619 5488	к to Back "E" R	TING MOMENT E 70 E 62 E 42 E 40 E 38 E 36 E
span (řeet) 5 10 25 20 25 30 35 40 45 50 60 70 80 90 100 span	COO Max. Shear (%P) 80 120 160 200 227 252 277 302 327 349 392 442 497 549 600 Max. Shear	PER E80 Max. Moment (FF xir) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830 8638 10678 12893 PER E80 Max. Moment	Max. Shear (##) 69 76 97 108 114 118 121 123 127 135 156 173 186 197 205	P32AC single Max. Moment (FT ki) 87 174 261 415 584 753 922 1091 1260 1429 1772 2177 2177 2177 2177 2177 2177 21	Engine Engine SMEAR E 69 E 51 E 49 E 43 E 40 E 38 E 33 E 31 E 31 E 31 E 31 E 31 E 31 E 31	TING MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 35 E 33 E 31 E 30 E 27 E 26 E 26 E 26 E 26 ATING	Max. Shear (KIP) 69 76 97 108 114 129 142 159 172 183 198 211 230 250 267 Max. Shear	Two P32AC           Max. Moment           (FT K8)           87           174           261           415           584           753           944           1205           1469           1805           2483           3158           3832           4541           5410           Two P40DC/P4.           Max. Moment	in line: "E" RA SHEAR E 69 E 51 E 49 E 43 E 40 E 41 E 41 E 42 E 43 E 37 E 36 E 37 E 36 E 36 E 36 E 36 E 36 E 37 E 36 E 36 E 37 E 36 E 37 E 37 E 36 E 37 E 37 E 36 E 36 E 36 E 37 E 36 E 36	TING MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 34 E 34	Max. Shear (XIP) 69 76 97 108 115 131 145 162 175 185 200 213 233 253 270 Tw Max. Shear	Two P32AC Bac Max. Moment (FF kb) 87 174 261 415 584 753 982 1242 1535 1871 2548 3226 3898 4619 5488 4619 5488	k to Back "E" R/ SHEAR E 69 E 51 E 49 E 43 E 40 E 41 E 42 E 43 E 42 E 43 E 42 E 41 E 37 E 36 Back to Bac	VTING MOMENT E 70 E 62 E 42 E 42 E 38 E 38 E 38 E 38 E 38 E 39 E 38 E 37 E 38 E 37 E 38 E 38 E 38 E 38 E 39 E 39 E 39 E 39 E 39 E 38 E 39 E 39 E 38 E 38 E 39 E 38 E 38 E 39 E 38 E 39 E 38 E 36 E 36
span (Feet) 5 10 15 20 25 30 35 40 45 50 60 70 60 70 80 80 90 100 \$pan (Feet)	COO Max. Shear (RP) 80 120 160 200 227 252 277 302 327 302 327 349 392 442 497 549 600 Kax. Shear (RP)	PER E80 Max. Moment (FF κF) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830 8638 10678 12893 PER E80 Max. Moment (FF κF)	Мах. Shear (##) 69 76 97 108 114 118 121 123 127 135 156 173 186 197 205	P32AC single Max. Moment (FF KB) 87 174 261 415 584 753 922 1091 1260 1429 1772 2177 2177 2177 2177 3448 4122 P40DC/P42DC si Max. Moment (FF KB)	Engine Engine Engine E 69 E 51 E 49 E 43 E 40 E 38 E 35 E 33 E 31 E 31 E 32 E 32 E 31 E 32 E 32 E 32 E 31 E 32 E 32 E 32 E 32 E 31 E 32 E 33 E 33	ATING ADMANT E 70 E 62 E 42 E 40 E 38 E 37 E 35 E 33 E 37 E 35 E 33 E 31 E 30 E 27 E 26 E 26 E 26 E 26 E 26 ADMENT	Мах. Shear (XIP) 69 76 97 108 114 129 142 159 142 159 142 172 183 198 211 230 250 250 267	Тwo P32AC Max. Moment (FT кв) 87 174 261 415 584 753 944 1205 1469 1805 2483 3158 3832 4541 5410 Two P40DC/P43 Max. Moment (FT кв)	in line "E" R/ SHEAR E 69 E 51 E 49 E 43 E 40 E 41 E 41 E 41 E 42 E 42 E 42 E 42 E 42 E 42 E 42 E 43 E 37 E 36 E 36 E 36 E 36 E 36	TING MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 36 E 37 E 36 E 37 E 38 E 37 E 38 E 37 E 38 E 37 E 38 E 37 E 36 A 37 E 36 B 37 E 36 B 37 E 36 B 37 E 37 E 36 B 37 E 37 E 38 E 37 E 38 E 37 E 38 E 37 E 36 E 37 E 38 E 37 E 38 E 37 E 36 E 37 E 38 E 37 E 36 E 37 E 38 E 37 E 35 E 34 E 34 E 34 E 34 E 34 E 34 E 34 E 34 E 37 E 36 E 37 E 35 E 34 E 34	Max. Shear (KB) 69 76 97 108 115 131 145 162 175 185 200 213 233 253 270 Tw Max. Shear (KD)	Тwo P32AC Bac Max. Moment (FT кir) 87 174 261 415 584 753 982 1242 1535 1871 2548 3226 3298 4619 5488 4619 5488 70 Р40DC/P42DC Max. Moment (FT кir)	k to Back "E" R/ SHEAR E 69 E 51 E 49 E 43 E 40 E 41 E 42 E 43 E 42 E 43 E 42 E 43 E 42 E 41 E 37 E 37 E 36 Back to Back SHEAR	XTING MOMENT E 70 E 62 E 42 E 42 E 38 E 38 E 38 E 38 E 38 E 39 E 39 E 39 E 39 E 39 E 39 E 39 E 39 E 33 E 33 E 33 X X X X X X X X X X X X X
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span (Feet) 5 10 15 20 25 30 35 40 45 50 60 70 80 90 100 \$ \$ 90 100 \$ \$ 90 100 \$ \$ \$	COO Max. Shear (%P) 80 120 160 200 227 252 277 302 327 349 392 442 497 549 600 Max. Shear (%P) 80 120 160	PER E80 Max. Moment (FF xki) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830 8638 10678 12893 10678 12893 PER E80 Max. Moment (FF xki) 100 225 500	Max. Shear (##) 69 76 97 108 114 118 121 123 127 135 156 173 186 197 205 Max. Shear (##) 66 67 72 92	P32AC single Max. Moment (FF ki) 87 174 261 415 584 753 922 1091 1260 1429 1772 2177 2177 2177 2177 2177 2177 21	Engine Engine E egy E 69 E 51 E 49 E 43 E 40 E 38 E 33 E 31 E 31 E 31 E 31 E 31 E 31 E 32 E 31 E 30 E 29 E 27 ngle Engine "E" R/ SHEAR E 66 E 48 E 48 E 48 E 46	TING MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 35 E 33 E 31 E 30 E 27 E 26 E 26 E 26 E 26 MOMENT E 66 E 59 E 40	Max. Shear (KIP) 69 76 97 108 114 129 142 159 142 159 172 183 198 211 230 250 267 Max. Shear (KIP) 66 72 92	Two P32AC           Max. Moment           (FT K8)           87           174           261           415           584           753           944           1205           1469           1805           2483           3158           3832           4541           5410           Two P40DC/P4.           Max. Moment           (FT K8)           82           165           247	in line: "E" R/ SHEAR E 69 E 51 E 49 E 43 E 40 E 41 E 41 E 42 E 43 E 37 E 36 E 37 E 36 E 36 E 37 E 36 E 36 E 37 E 36 E 36 E 37 E 36 E 36 E 36 E 36 E 37 E 36 E 36 E 36 E 37 E 36 E 36	TING MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 36 E 37 E 36 E 37 E 38 E 37 E 36 E 37 E 38 E 37 E 36 E 37 E 40 E 56 E 59 E 40	Max. Shear (KP) 69 76 97 108 115 131 145 162 175 185 200 213 233 253 270 Tw Max. Shear (KP) 66 72 92	Two P32AC Bac           Max. Moment           (FT kb)           87           174           261           415           584           753           982           1242           1535           1871           2548           3226           3898           4619           5488           ro P40DC/P42DC           Max. Moment           (FF kb)           82           165           247	k to Back "E" R/ SHEAR E 69 E 51 E 49 E 43 E 40 E 41 E 42 E 43 E 42 E 43 E 42 E 41 E 37 E 36 Back to Bac "E" R/ SHEAR E 66 E 48 E 48 E 46	VTING MOMENT E 70 E 62 E 42 E 42 E 38 E 38 E 38 E 38 E 38 E 39 E 34 VING MOMENT E 66 E 59 E 40 E 40
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span (/keet) 5 10 15 20 25 30 35 40 35 40 45 50 60 70 80 60 70 80 90 100 100	COO           Max. Shear           (₩P)           80           120           160           200           227           252           277           302           327           349           392           442           497           549           600           COO           Max. Shear           (𝑘P)           80           120           160           200           227	PER E80 Max. Moment (FF xii) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830 8638 10678 12893 Max. Moment (FF xii) 100 225 500 825 1224	Мах. Shear (жр) 69 76 97 108 114 118 121 123 127 135 156 173 186 173 186 197 205 Мах. Shear (кр) 66 72 92 102	P32AC single Max. Moment (FT K8) 87 174 261 415 584 753 922 1091 1260 1429 1772 2177 2177 2177 2177 2177 2177 3448 4122 P40DC/P42DC si Max. Moment (FT K8) 82 165 247 393 553	Engine Engine Engine E 69 E 51 E 49 E 43 E 40 E 38 E 31 E 31 E 31 E 31 E 31 E 31 E 31 E 32 E 31 E 31 E 32 E 31 E 31 E 32 E 31 E 32 E 31 E 49 E 49 E 49 E 40 E 40 E 40 E 40 E 40 E 40 E 40 E 40	ATING MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 35 E 33 E 31 E 30 E 27 E 26 E 26 E 26 E 26 E 26 E 26 E 26 E 59 E 40 E 38 E 37 E 35 E 33 E 31 E 30 E 27 E 26 E 26 E 26 E 27 E 26 E 26 E 26 E 26 E 26 E 26 E 26 E 27 E 26 E 2	Мах. Shear (KIP) 69 76 97 108 114 129 142 159 142 159 142 172 183 198 211 172 172 183 198 211 230 250 267 267 Как. Shear (KIP) 66 72 92 72 92 102 108	Two P32AC           Max. Moment           (FT κØ)           87           174           261           415           584           753           944           1205           1805           2483           3158           3832           4541           5410           Two P400C/P4           (FT κØ)           82           165           247           393           553	in line "E" R/ SHEAR E 69 E 51 E 49 E 43 E 40 E 41 E 41 E 41 E 42 E 42 E 42 E 42 E 42 E 42 E 41 E 38 E 37 E 36 E 36 E 36 E 36 E 36 E 36 E 49 SHEAR E 69 E 43 E 49 E 43 E 41 E 42 E 42 E 42 E 42 E 42 E 42 E 42 E 42 E 42 E 43 E 38 E 36 E 49 E 49 E 49 E 41 E 38 E 36 E 36 E 36 E 36 E 36 E 36 E 36 E 49 E 41 E 42 E 44 E 38 E 36 E 36 E 36 E 36 E 36 E 40 E 41 E 44 E 44	TING MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 36 E 37 E 37 E 38 E 37 E 38 E 37 E 38 E 37 E 38 E 37 E 38 E 37 E 36 S 7 E 36 S 7 E 36 S 7 E 37 E 36 S 7 E 37 E 38 E 37 E 36 E 37 E 38 E 37 E 36 E 37 E 38 E 37 E 38 E 37 E 38 E 37 E 36 E 37 E 38 E 37 E 36 E 37 E 36 E 37 E 38 E 37 E 36 E 34 E 34 E 34 E 34 E 38 E 36 E 38 E 36 E 38 E 36 E 38 E 36 E 38 E 36 E 36 E 38 E 36 E 36 E 38 E 36 E 36 E 36 E 36 E 36 E 36 E 36 E 36 E 36 E 38 E 36 E 36	Max. Shear (KB) (KB) (AB) (AB) (AB) (AB) (AB) (AB) (AB) (A	Two P32AC Bac Max. Moment (FT kir) 87 174 261 415 584 753 982 1242 1535 1871 2548 3226 3898 4619 5488 26 3898 4619 5488 70 P40DC/P42DC Max. Moment (FT Kir) 82 165 247 393 553	k to Back "E" R/ shear E 69 E 51 E 49 E 43 E 40 E 41 E 42 E 43 E 42 E 41 E 39 E 37 E 36 Back to Bac "E" R/ shear E 66 E 48 E 66 E 48 E 41 E 38	AMOMENT           E70           E62           E42           E40           E38           E37           E38           E39           E38           E39           E38           E35           E34           XING           MOMENT           E66           E59           E40           E38           E34
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span (řeet) 5 10 25 30 35 40 45 50 60 70 80 90 100 \$ \$ 100 \$ \$ 100 15 \$ 20 25 30 35 35 40 0 35 50 60 70 70 80 90 100 100 15 5 30 35 35 30 35 35 30 35 35 35 35 30 35 5 35 30 35 5 30 35 5 30 35 5 30 35 5 30 35 5 30 35 5 35 30 35 5 30 30 35 5 30 35 35 30 35 5 30 35 30 35 30 35 5 30 35 30 35 35 30 35 35 30 35 30 35 30 35 30 35 30 35 30 35 30 30 35 30 30 30 30 30 30 30 30 30 30 30 30 30	COO Max. Shear (%P) 80 120 160 200 227 252 277 302 327 349 392 442 497 549 600 Max. Shear (%P) 80 120 160 200 227 252 277	PER E80 Max. Moment (FF xi/) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830 8638 10678 12893 10678 12893 10678 12893 PER E80 Max. Moment (FF xi/) 100 225 500 825 1224	Max. Shear (##) 69 76 97 108 114 118 121 123 127 135 156 173 186 197 205 Max. Shear (##) 66 67 2 205	P32AC single Max. Moment (FF ki) 87 174 261 415 584 753 922 1091 1260 1429 1772 2177 2177 2177 2177 2177 2177 21	Engine Engine Engine E 69 E 51 E 49 E 43 E 40 E 38 E 33 E 31 E 31 E 31 E 31 E 31 E 31 E 31	TING MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 35 E 33 E 31 E 30 E 27 E 26 E 38 E 35 E 33 E 35 E 35	Max. Shear (KIP) 69 76 97 108 114 129 142 159 142 159 172 183 198 211 230 250 267 Max. Shear (KIP) 66 67 2 92 102 102 102 102 102 103 105 105 105 105 105 105 105 105	Two P32AC Max. Moment (FT K8) 87 174 261 415 584 753 944 1205 1469 1805 2483 3158 3832 4541 5410 Two P40DC/P4. Max. Moment (FT K8) 82 165 247 393 553 713 894	in line: "E" R/ SHEAR E 69 E 51 E 49 E 43 E 40 E 41 E 41 E 42 E 44 E 66 E 48 E 48 E 49 E 48 E 49 E 48 E 48	TING MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 36 E 37 E 37 E 37 E 37 E 37 E 37 E 37 E 37	Max. Shear (KB) 69 76 97 108 115 131 145 162 175 185 200 213 233 253 270 Tw Max. Shear (KD) 66 72 92 102 109 124 137	Two P32AC Bac Max. Moment (FF kb) 87 174 261 415 584 753 982 1242 1535 1871 2548 3226 3898 4619 5488 4619 5488 4619 5488 4619 5488 4619 558 465 247 165 247 393 553 713 930	kto Back           "E" R/           SHEAR           E 69           E 51           E 49           E 44           E 42           E 43           E 42           E 43           E 42           E 43           E 42           E 43           E 42           E 41           E 37           E 37           E 36           Back to Back           SHEAR           E 66           E 48           E 46           E 41           E 39           E 40	VTING MOMENT E 70 E 62 E 42 E 42 E 38 E 38 E 38 E 38 E 38 E 39 E 33 E 34 VING MOMENT E 66 E 59 E 40 E 35 E 35 E 35 E 36 E 35 E 35 E 35 E 36 E 35 E 35 E 38 E 35 E 36 E 35 E 36 E 35 E 38 E 35 E 36 E 37 E 38 E 37 E 38 E 39 E 39 E 39 E 39 E 39 E 39 E 36 E 35 E 34 VING E 38 E 38 E 35 E 35 E 34 VING E 38 E 38 E 35 E 36 E 36 E 36 E 36 E 36 E 36 E 36 E 36
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span (feet) 5 10 15 20 25 30 35 40 45 50 60 70 80 60 70 80 90 100 5 100 15 5 10 10 5 10 15 5 10 40 45	COO Max. Shear (#P) 80 120 200 227 252 277 302 327 349 392 442 497 549 600 Max. Shear (#P) 80 120 160 200 227 252 277 302 327 302 327	PER E80 Max. Moment (Ff κir) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830 8638 10678 12893 06838 10678 12893 Max. Moment (Ff κir) 100 225 500 825 1224 1642 2088 2623 3202	Мах. Shear (##?) 69 76 97 108 114 118 121 123 127 135 156 173 186 173 186 197 205 Мах. Shear (#??) 66 72 92 102 102 108 112 117 120	P32AC single           Max. Moment           (FT KB)           87           174           261           415           584           753           922           1091           1260           1429           1772           2177           3448           4122           P40DC/P42DC si           Max. Moment           (FT KB)           82           165           247           333           713           873           1033           1193	Engine Engine Engine E 69 E 51 E 49 E 43 E 40 E 38 E 35 E 33 E 31 E 31 E 32 E 31 E 32 E 31 E 32 E 31 E 32 E 31 E 49 E 48 E 46 E 48 E 46 E 48 E 46 E 41 E 38 E 36 E 31 E 31 E 32 E 49 E 49 E 49 E 49 E 49 E 49 E 49 E 49	MOMENT           E 70           E 62           E 42           E 40           E 38           E 37           E 35           E 31           E 30           E 27           E 26           E 26           E 26           E 26           E 26           E 26           E 38           E 38           E 39           E 40           E 38           E 36           E 35           E 33           E 32           E 30	Мах. Shear (XLP) 69 76 97 108 1114 129 142 159 142 159 142 150 250 267 267 72 92 267 72 92 102 108 122 108 122 108	Two P32AC           Max. Moment           (FT κθ)           87           174           261           415           584           753           944           1205           1469           1805           2483           3158           3832           4541           5410           Two P40DC/P43           Max. Moment           (FT κθ)           82           165           247           333           553           713           894           1141           1392	in line "E" R/ SHEAR E 69 E 51 E 49 E 43 E 40 E 41 E 41 E 41 E 42 E 42 E 42 E 42 E 42 E 41 E 38 E 37 E 36 E 36 E 36 E 36 E 66 E 48 E 46 E 48 E 46 E 41 E 38 E 37 SHEAR E 66 E 48 E 49 E 49 E 49 SHEAR E 49 E 49 SHEAR E 49 E 49 SHEAR E 49 E 49 E 49 SHEAR E 49 E 49 E 49 SHEAR E 49 E 40 E 40	TING MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 36 E 37 E 36 E 37 E 38 E 37 E 38 E 37 E 35 E 34 E 34 MOMENT E 66 E 59 E 40 E 35 E 35 E 35	Max. Shear (KB) 69 76 97 108 115 131 145 162 175 185 200 213 233 253 270 Tw Max. Shear (KD) 66 72 92 102 109 124 137 153 165	Two P32AC Bac           Max. Moment           (FT κir)           87           174           261           415           584           753           982           1242           1535           1871           2548           3226           3898           4619           5488           00 MODC/P42DC           Max. Moment           (FT κir)           82           165           247           393           553           713           930           1177           1454	kto Back           "E" R/           SHEAR           E 69           E 51           E 49           E 41           E 42           E 43           E 42           E 43           E 42           E 43           E 42           E 43           E 42           E 37           E 36           Back to Bac           "E" RA           SHEAR           E 66           E 48           E 46           E 41           E 38           E 39           E 44           E 45           E 46           E 41           E 38           E 39           E 40           E 41           E 38           E 39           E 40           E 41           E 42	XTING AMOMENT E 70 E 62 E 42 E 42 E 33 E 34 XIING MOMENT E 66 E 59 E 40 E 30 E 35 E 34 XIING MOMENT E 66 E 59 E 40 E 35 E 36 E 36 E 36 E 35 E 36 E 36
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span (/eet) 5 10 25 30 35 40 45 50 60 70 80 90 100 \$ \$ 5 100 100 \$ \$ \$ 100 15 20 25 30 35 40 45 50 60 25 50 60 35 50 60 60 80 50 25 50 20 25 50 20 25 50 20 25 50 20 25 50 20 25 50 20 25 50 20 25 50 20 25 50 20 20 25 50 20 20 20 20 20 20 20 20 20 20 20 20 20	COO Max. Shear (%P) 80 120 120 227 252 277 302 327 349 392 442 497 549 600 <b>COO</b> Max. Shear (%P) 80 120 160 200 227 252 277 302 252 277 302 327 349 392	PER E80 Max. Moment (FF xir) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830 8638 10678 12893 10678 12893 10678 12893 PER E80 Max. Moment (FF xir) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196	Max. Shear (##) 69 76 97 108 114 118 121 123 127 135 156 173 186 173 186 197 205 Max. Shear (##) 66 72 92 102 108 72 92 102 102 102 102 102 102 102 102 102 10	P32AC single Max. Moment (FF ki) 87 174 261 415 584 753 922 1091 1260 1429 1772 2177 2177 2177 2177 2177 2177 21	Engine Engine Engine E 69 E 51 E 49 E 43 E 40 E 38 E 33 E 31 E 31 E 31 E 31 E 31 E 31 E 31	TING MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 35 E 33 E 31 E 30 E 27 E 26 E 26 E 26 E 26 E 26 E 26 E 26 E 59 E 40 E 38 E 35 E 33 E 32 E 33 E 32 E 35 E 33 E 32 E 35 E 33 E 32 E 30 E 26 E 38 E 37 E 35 E 33 E 31 E 30 E 27 E 26 E 38 E 37 E 38 E 37 E 35 E 33 E 31 E 30 E 27 E 26 E 26 E 26 E 26 E 38 E 37 E 36 E 35 E 33 E 32 E 30 E 26 E 26 E 26 E 26 E 38 E 35 E 40 E 38 E 35 E 33 E 32 E 30 E 38 E 36 E 38 E 36 E 38 E 36 E 38 E 35 E 33 E 32 E 35 E 35	Max. Shear (%09) 69 76 97 108 114 129 142 159 142 159 172 183 198 211 230 250 267 Max. Shear (%09) 66 72 92 102 102 102 102 102 103 122 134 150 163 173 188	Two P32AC Max. Moment (FT K8) 87 174 261 415 584 753 944 1205 1469 1805 2483 3158 3832 4541 5410 Two P40DC/P4. Max. Moment (FT K8) 82 165 247 333 553 713 894 1141 1392 1710 2352	in line: "E" R/ SHEAR E 69 E 51 E 49 E 43 E 40 E 41 E 41 E 42 E 44 E 42 E 44 E 42 E 44 E 44	TING MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 36 E 37 E 37 E 38 E 37 E 37 E 38 E 37 E 38 E 37 E 37 E 36 E 37 E 37 E 38 E 37 E 36 E 37 E 36 E 37 E 35 E 34 E 34 E 34 E 34 E 35 E 59 E 40 E 35 E 38 E 38 E 37 E 35 E 34 E 35 E 34 E 35 E 59 E 40 E 35 E 38 E 38 E 37 E 35 E 34 E 35 E 34 E 35 E 59 E 40 E 35 E 38 E 38 E 38 E 37 E 35 E 34 E 35 E 34 E 35 E 36 E 37 E 36 E 59 E 40 E 59 E 34 E 35 E 35 E 34 E 35 E 36 E 35 E 34 E 35 E 36 E 36 E 35 E 36 E 36	Max. Shear (XIP) 69 76 97 108 115 131 145 162 175 185 200 213 233 253 270 Tw Max. Shear (XIP) 66 72 92 102 109 124 137 153 165 175 190	Two P32AC Bac           Max. Moment           (F7 к/r)           87           174           261           415           584           753           982           1242           1535           1871           2548           3226           3898           4619           5488           vo P40DC/P42DC           Max. Moment           (FF к/r)           82           165           247           393           553           713           9300           1177           1454           1772           2414	kto Back           "E" R2           SHEAR           E 69           E 51           E 49           E 43           E 42           Back to Back           Back to Back           Back to Back           E 46           E 41           E 39           E 40           E 41           E 40	AMOMENT           E 70           E 62           E 42           E 42           E 38           E 37           E 38           E 38           E 39           E 38           E 39           E 38           E 38           E 38           E 38           E 39           E 36           E 35           E 34           VING           MOMENT           E 66           E 35           E 36           E 35           E 36           E 37           E 36           E 37           E 36           E 36           E 37           E 37           E 36           E 37
span (Feet) 5 10 15 20 25 30 35 40 45 50 60 70 80 90 100 \$ \$ 90 100 \$ \$ 90 100 \$ \$ \$ 5 10 60 25 5 10 60 25 5 10 60 25 70 70 \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$	COO Max. Shear (%P) 80 120 160 200 227 252 277 302 327 349 392 442 497 549 600 Max. Shear (%P) 80 120 160 200 227 252 277 252 277 302 327 349 392 442	PER E80 Max. Moment (FF xir) 100 222 5 500 82 5 1224 1642 2088 2623 3202 3804 5196 6830 8638 10678 12893 10678 12893 10678 12893 Max. Moment (FF xir) 100 22 5 500 82 5 1224 1642 2088 2623 3202 3804 5196 6830	Max. Shear (##) 69 76 97 108 114 118 121 123 127 135 156 173 186 197 205 Max. Shear (##) 66 67 2 205 Max. Shear (##) 66 63 72 102 108 115 115 117 120 128 115 117	P32AC single Max. Moment (FT KP) 87 174 261 415 584 753 922 1091 1260 1429 1772 2177 2177 2177 2177 2177 2177 21	Engine Engine Engine E 69 E 51 E 49 E 43 E 40 E 38 E 35 E 33 E 31 E 31 E 31 E 32 E 31 E 32 E 31 E 30 E 29 E 27 mgle Engine E 27 mgle Engine E 46 E 48 E 46 E 41 E 38 E 33 E 31 E 29 E 27 SHEAR E 46 E 41 E 38 E 34 E 45 E 45 E 45 E 45 E 45 E 45 E 45 E 4	VTING ACAMENT E 70 E 62 E 42 E 40 E 38 E 37 E 35 E 33 E 31 E 30 E 27 E 26 E 26 E 26 E 26 E 26 E 26 E 26 E 40 E 38 E 36 E 37 E 35 E 33 E 32 E 30 E 27 E 26 E 38 E 37 E 35 E 33 E 32 E 33 E 32 E 33 E 32 E 33 E 32 E 33 E 32 E 33 E 32 E 32 E 33 E 32 E 32 E 33 E 32 E 32 E 32 E 32 E 32 E 35 E 36 E 36 E 36 E 36 E 37 E 38 E 36 E 33 E 32 E 30 E 26 E 30 E 32 E 30 E 26 E 33 E 32 E 30 E 26 E 30 E 32 E 30 E 26 E 33 E 32 E 26 E 30 E 26 E 33 E 32 E 26 E 26 E 33 E 32 E 26 E 26 E 33 E 32 E 30 E 28 E 26 E 26 E 26 E 33 E 32 E 30 E 28 E 26 E 24	Мах. Shear (Xu9) 69 76 97 108 114 129 142 159 142 159 217 230 250 250 267 267 267 267 267 267 267 267 267 267	Two P32AC           Max. Moment           (FT κ#)           87           174           261           415           584           753           944           1205           1469           1805           2483           3158           3832           4541           5410           Two P40DC/P4           Max. Moment           (FT κ#)           82           165           247           393           553           713           894           1141           1392           1710           2352           2992	in line: "E" R/ SHEAR E 69 E 51 E 49 E 43 E 40 E 41 E 41 E 42 E 41 E 42 E 42 E 42 E 42 E 42 E 42 E 42 E 42 E 37 E 36 E 36 E 36 E 46 E 48 E 46 E 41 E 38 E 39 E 40 E 40 E 40 E 40 E 40 E 41 E 38 E 39 E 40 E 40	TING MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 36 E 37 E 38 E 37 E 38 E 37 E 38 E 37 E 38 E 37 E 36 E 37 E 38 E 37 E 36 E 37 E 38 E 37 E 36 E 37 E 36 E 37 E 38 E 37 E 36 E 37 E 38 E 37 E 38 E 37 E 36 E 37 E 38 E 37 E 36 E 36 E 36 E 38 E 36 E 35 E 34 E 35 E 35 E 35 E 36 E 35 E 35	Max. Shear (KB) 69 76 97 108 115 131 145 162 175 185 200 213 233 253 270 XW Max. Shear (KB) 66 72 92 102 109 124 137 153 165 175 190 202	Two P32AC Bac Max. Moment (FT kb) 87 174 261 415 584 753 982 1242 1535 1871 2548 3226 3898 4619 5488 4619 5488 4619 5488 4619 5488 465 247 393 553 713 933 553 713 930 11177 1454 1772 2414 3056	kto Back           "E" R/           SHEAR           E 69           E 51           E 49           E 41           E 42           E 43           E 42           E 43           E 42           E 43           E 42           E 43           E 42           Back to Back           Back to Back           Back to Back           E 46           E 41           E 38           E 42           SHEAR           E 46           E 41           E 38           E 39           E 40           E 41           E 39           E 37	XTING MOMENT E 70 E 62 E 42 E 42 E 42 E 38 E 38 E 38 E 38 E 39 E 33 E 34 XING MOMENT E 66 E 55 E 34 XING MOMENT E 66 E 53 E 36 E 36 E 36 E 36 E 36 E 37 E 38 E 36 E 37 E 38 E 38 E 37 E 38 E 37 E 38 E 38 E 37 E 38 E 38 E 37 E 38 E 38 E 39 E 30 E 36 E 35 E 34 XING MOMENT E 66 E 55 E 34 XING MOMENT E 66 E 55 E 36 E 36 E 56 E 36 E 37 E 38 E 36 E 35 E 34 XING MOMENT E 66 E 55 E 36 E 37 E 38 E 38 E 36 E 35 E 34 XING MOMENT E 56 E 38 E 38 E 38 E 36 E 35 E 34 XING E 38 E 38 E 36 E 35 E 34 XING E 38 E 36 E 36 E 35 E 36 E 37 E 38 E 36 E 35 E 34 XING E 38 E 36 E 35 E 36 E 37 E 36 E 37 E 36 E 37 E 36 E 37 E 36 E 37 E 36 E 37 E 36 E 37 E 36 E 36 E 36 E 37 E 37 E 36 E 37 E 37 E 36 E 37 E 37 E 37 E 37 E 37 E 36 E 37 E 37 E 36 E 37 E 37 E 36 E 37 E 36 E 36 E 37 E 36 E 3
span (/eet) 5 10 25 30 35 40 45 50 60 70 80 90 100 \$ \$ 5 100 100 \$ \$ \$ 100 15 20 25 30 35 40 45 50 60 25 50 60 35 50 60 60 80 50 25 50 20 25 50 20 25 50 20 25 50 20 25 50 20 25 50 20 25 50 20 25 50 20 25 50 20 20 25 50 20 20 20 20 20 20 20 20 20 20 20 20 20	COO Max. Shear (%P) 80 120 120 227 252 277 302 327 349 392 442 497 549 600 <b>COO</b> Max. Shear (%P) 80 120 160 200 227 252 277 302 252 277 302 327 349 392	PER E80 Max. Moment (FF xir) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830 8638 10678 12893 10678 12893 10678 12893 PER E80 Max. Moment (FF xir) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196	Max. Shear (##) 69 76 97 108 114 118 121 123 127 135 156 173 186 173 186 197 205 Max. Shear (##) 66 72 92 102 108 72 92 102 102 102 102 102 102 102 102 102 10	P32AC single Max. Moment (FF ki) 87 174 261 415 584 753 922 1091 1260 1429 1772 2177 2177 2177 2177 2177 2177 21	Engine Engine Engine E 69 E 51 E 49 E 43 E 40 E 38 E 33 E 31 E 31 E 31 E 31 E 31 E 31 E 31	TING MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 35 E 33 E 31 E 30 E 27 E 26 E 26 E 26 E 26 E 26 E 26 E 26 E 59 E 40 E 38 E 35 E 33 E 32 E 33 E 32 E 35 E 33 E 32 E 35 E 33 E 32 E 30 E 26 E 38 E 37 E 35 E 33 E 31 E 30 E 27 E 26 E 38 E 37 E 38 E 37 E 35 E 33 E 31 E 30 E 27 E 26 E 26 E 26 E 26 E 38 E 37 E 36 E 35 E 33 E 32 E 30 E 26 E 26 E 26 E 26 E 38 E 35 E 40 E 38 E 35 E 33 E 32 E 30 E 38 E 36 E 38 E 36 E 38 E 36 E 38 E 35 E 33 E 32 E 35 E 35	Max. Shear (%09) 69 76 97 108 114 129 142 159 142 159 172 183 198 211 230 250 267 Max. Shear (%09) 66 72 92 102 102 102 102 102 103 122 134 150 163 173 188	Two P32AC Max. Moment (FT K8) 87 174 261 415 584 753 944 1205 1469 1805 2483 3158 3832 4541 5410 Two P40DC/P4. Max. Moment (FT K8) 82 165 247 333 553 713 894 1141 1392 1710 2352	in line: "E" R/ SHEAR E 69 E 51 E 49 E 43 E 40 E 41 E 41 E 42 E 44 E 42 E 44 E 42 E 44 E 44	TING MOMENT E 70 E 62 E 42 E 40 E 38 E 37 E 36 E 37 E 37 E 38 E 37 E 37 E 38 E 37 E 38 E 37 E 37 E 36 E 37 E 37 E 38 E 37 E 36 E 37 E 36 E 37 E 35 E 34 E 34 E 34 E 34 E 35 E 59 E 40 E 35 E 38 E 38 E 37 E 35 E 34 E 35 E 34 E 35 E 59 E 40 E 35 E 38 E 38 E 37 E 35 E 34 E 35 E 34 E 35 E 59 E 40 E 35 E 38 E 38 E 38 E 37 E 35 E 34 E 35 E 34 E 35 E 36 E 37 E 36 E 59 E 40 E 59 E 34 E 35 E 35 E 34 E 35 E 36 E 35 E 34 E 35 E 36 E 36 E 35 E 36 E 36	Max. Shear (XIP) 69 76 97 108 115 131 145 162 175 185 200 213 233 253 270 Tw Max. Shear (XIP) 66 72 92 102 109 124 137 153 165 175 190	Two P32AC Bac           Max. Moment           (F7 к/r)           87           174           261           415           584           753           982           1242           1535           1871           2548           3226           3898           4619           5488           vo P40DC/P42DC           Max. Moment           (FF к/r)           82           165           247           393           553           713           9300           1177           1454           1772           2414	kto Back           "E" R2           SHEAR           E 69           E 51           E 49           E 43           E 42           Back to Back           Back to Back           Back to Back           E 46           E 41           E 39           E 40           E 41           E 40	AMOMENT           E 70           E 62           E 42           E 42           E 38           E 37           E 38           E 38           E 39           E 38           E 39           E 38           E 38           E 38           E 38           E 39           E 36           E 35           E 34           VING           MOMENT           E 66           E 35           E 36           E 35           E 36           E 37           E 36           E 37           E 36           E 36           E 37           E 37           E 36           E 37

Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

253

5124

E 34

E 32

255

5198



100

600

12893

194

3904

E 26

E 24

E 34

E 32

#### CONNECTICUT RAILROAD EQUIPMENT TABLE A<sup>(CONT)</sup> EQUIVALENT COOPER LOADING SUMMARY

				M. D. Classification				14.0 T	0		1	b4 4 6	. C	
cnan		PER E80 Max. Moment	May Shoar	M-2 Singl		ATING	May Shoar	M-2 Two Max. Moment	Cars "E" RA	TING	May Shoar	M-4 Single Max. Moment	e Car "E" RA	TING
span (Feet)	(KIP)	(FT KIP)	(KIP)	(FT KIP)	SHEAR	MOMENT	(KIP)	(FT KIP)	SHEAR	MOMENT	(KIP)	(FT KIP)	SHEAR	MOMENT
5	80	100	34	42	E 34	E 34	34	42	E 34	E 34	33	42	E 33	E 33
10	120	225	39	85	E 26	E 30	39	85	E 26	E 30	39	84	E 26	E 30
15	160	500	49	130	E 24	E 21	49	130	E 24	E 21	48	128	E 24	E 21
20	200	82.5	54	210	E 21	E 20	54	210	E 21	E 20	53	207	E 21	E 20
25	227	1224	56	2.92	E 20	E 19	56	293	E 20	E 19	56	288	E 20	E 19
30	252	1642	58	375	E 19	E 18	63	375	E 20	E 18	58	370	E 18	E 18
35	277	2088	60	458	E 17	E 18	70	458	E 20	E 18	59	451	E 17	E 17
40	302	2623	61	540	E 16	E 16	78	584	E 21	E 18	60	532	E 16	E 16
45	327	3202	62	623	E 15	E 16	84	713	E 21	E 18	61	614	E 15	E 15
50	349	3804	62	705	E 14	E 15	90	878	E 21	E 18	61	695	E 14	E15
60	392	5196	63	875	E 13	E 13	97	1209	E 20	E 19	63	863	E 13	E 13
70	442	6830	70	1045	E 13	E 12	103	1539	E 19	E 18	69	1030	E 12	E 12
80	497	8638	78	1215	E 13	E 11	107	1869	E 17	E 17	77	1198	E 12	E 11
90	549	10678	85	1385	E 12	E 10	112	2201	E 16	E 16	83	1365	E 12	E 10
100	600	12893	90	1587	E 12	E 10	120	2531	E 16	E 16	88	1564	E 12	E 10
ſ	C00	PER E80		M-4 Two	Cars			M-6 Single	e Car			M-6 Two	Cars	
span		Max. Moment	Max. Shear	Max. Moment	"E" R/	ATING	Max. Shear	Max. Moment	"E" RA	TING	Max. Shear	Max. Moment	"E" RA	TING
(Feet)	(KIP)	(FT KIP)	(KIP)	(FT KJP)	SHEAR	MOMENT	(KIP)	(FT KIP)	SHEAR	MOMENT	(KIP)	(FT KIP)	SHEAR	MOMENT
5	80	100	33	42	E 33	E 33	33	42	E 33	E 33	33	42	E 33	E 33
10	120	225	39	84	E 26	E 30	39	84	E 26	E 30	39	84	E 26	E 30
15	160	500	48	128	E 24	E 21	48	128	E 24	E 21	48	128	E 24	E21
20	200	82.5	53	207	E 21	E 20	53	207	E 21	E 20	53	207	E 21	E 20
25	227	1224	56	288	E 20	E 19	56	288	E 20	E 19	56	288	E 20	E 19
30	252	1642	62	370	E 20	E 18	58	370	E 18	E 18	62	370	E 20	E 18
35	277	2088	69	451	E 20	E 17	59	451	E 17	E 17	69	451	E 20	E 17
40	302	2623	77	576	E 20	E 18	60	532	E 16	E 16	77	576	E 20	E 18
45	327	32.02	83	703	E 20	E 18	61	614	E 15	E 15	83	703	E 20	E 18
50	349	3804	88	865	E 20	E 18	61	695	E 14	E 15	88	865	E 20	E 18
60	392	5196	96	1191	E 20	E 18	63	863	E 13	E 13	96	1191	E 20	E 18
70	442 497	6830	101	1517 1842	E 18	E 18	69 77	1030	E 12	E 12	101	1517	E 18	E 18
80 90	549	8638 10678	105 110	2168	E 17 E 16	E 17 E 16	83	1198 1365	E 12 E 12	E 11 E 10	105 110	1842 2168	E 17 E 16	E 17 E 16
100	600	12893	110	2494	E 16	E 15	88	1564	E 12	E 10	110	2494	E 16	E 15
100	000	12033	110	2121	110	15	00	1304	F 12	- 10	110	2151	L 10	L 1.5
	C00	PER E80		M-8 Singl	e Car			M-8 Two	Cars			BL20-GH Singl	e Engine	
span	Max. Shear	Max. Moment	Max. Shear	Max. Moment	"E" R/	ATING	Max. Shear	Max. Moment	"E" R/	TING	Max. Shear	Max. Moment	"E" RA	TING
(Feet)	10													
	(NIP)	(FT KIP)	(KIP)	(FT KIP)	SHEAR	MOMENT	(KIP)	(FT KIP)	SHEAR	MOMENT	(KIP)	(FT KIP)	SHEAR	MOMENT
5	80	100	36	45	E 36	E 36	36	45	E 36	E 36	66	82	E 66	E 66
10	80 120	100 225	36 41	45 90	E 36 E 28	E 36 E 32	36 41	45 90	E 36 E 28	E 36 E 32	66 73	82 165	E 66 E 48	E 66 E 59
10 15	80 120 160	100 225 500	36 41 52	45 90 138	E 36 E 28 E 26	E 36 E 32 E 22	36 41 52	45 90 138	E 36 E 28 E 26	E 36 E 32 E 22	66 73 92	82 165 247	E 66 E 48 E 46	E 66 E 59 E 40
10 15 20	80 120 160 200	100 225 500 825	36 41 52 57	45 90 138 222	E 36 E 28 E 26 E 23	E 36 E 32 E 22 E 22	36 41 52 57	45 90 138 222	E 36 E 28 E 26 E 23	E 36 E 32 E 22 E 22	66 73 92 102	82 165 247 393	E 66 E 48 E 46 E 41	E 66 E 59 E 40 E 38
10 15 20 25	80 120 160 200 227	100 225 500 825 1224	36 41 52 57 60	45 90 138 222 309	E 36 E 28 E 26 E 23 E 21	E 36 E 32 E 22 E 22 E 22 E 20	36 41 52 57 60	45 90 138 222 309	E 36 E 28 E 26 E 23 E 21	E 36 E 32 E 22 E 22 E 22 E 20	66 73 92 102 108	82 165 247 393 553	E 66 E 48 E 46 E 41 E 38	E 66 E 59 E 40 E 38 E 36
10 15 20 25 30	80 120 160 200 227 252	100 225 500 825 1224 1642	36 41 52 57 60 62	45 90 138 222 309 398	E 36 E 28 E 26 E 23 E 21 E 20	E 36 E 32 E 22 E 22 E 20 E 20 E 19	36 41 52 57 60 67	45 90 138 222 309 398	E 36 E 28 E 26 E 23 E 21 E 21	E 36 E 32 E 22 E 22 E 20 E 20 E 19	66 73 92 102 108 112	82 165 247 393 553 715	E 66 E 48 E 46 E 41 E 38 E 38 E 36	E 66 E 59 E 40 E 38 E 36 E 35
10 15 20 25 30 35	80 120 160 200 227 252 277	100 225 500 825 1224 1642 2088	36 41 52 57 60 62 63	45 90 138 222 309 398 485	E 36 E 28 E 26 E 23 E 21 E 20 E 18	E 36 E 32 E 22 E 22 E 20 E 19 E 19 E 19	36 41 52 57 60 67 74	45 90 138 222 309 398 487	E 36 E 28 E 26 E 23 E 21 E 21 E 21 E 21	E 36 E 32 E 22 E 22 E 20 E 19 E 19 E 19	66 73 92 102 108 112 117	82 165 247 393 553 715 874	E 66 E 48 E 46 E 41 E 38 E 36 E 36 E 34	E 66 E 59 E 40 E 38 E 36 E 35 E 34
10 15 20 25 30 35 40	80 120 160 200 227 252 277 302	100 225 500 825 1224 1642 2088 2623	36 41 52 57 60 62 63 63 64	45 90 138 222 309 398 485 572	E 36 E 28 E 26 E 23 E 21 E 20 E 18 E 17	E 36 E 32 E 22 E 22 E 20 E 19 E 19 E 17	36 41 52 57 60 67 74 83	45 90 138 222 309 398 487 622	E 36 E 28 E 26 E 23 E 21 E 21 E 21 E 21 E 22	E 36 E 32 E 22 E 22 E 20 E 19 E 19 E 19 E 19	66 73 92 102 108 112 117 127	82 165 247 393 553 715 874 1034	E 66 E 48 E 46 E 41 E 38 E 36 E 34 E 34	E 66 E 59 E 40 E 38 E 36 E 35 E 34 E 32
10 15 20 25 30 35 40 45	80 120 160 227 252 277 302 327	100 225 500 825 1224 1642 2088 2623 3202	36 41 52 57 60 62 63 64 65	45 90 138 222 309 398 485 572 661	E 36 E 28 E 26 E 23 E 21 E 20 E 18 E 17 E 16	E 36 E 32 E 22 E 22 E 20 E 19 E 19 E 17 E 17	36 41 52 57 60 67 74 83 90	45 90 138 222 309 398 487 622 760	E 36 E 28 E 26 E 23 E 21 E 21 E 21 E 21 E 22 E 22 E 22	E 36 E 32 E 22 E 22 E 20 E 19 E 19 E 19 E 19 E 19	66 73 92 102 108 112 117 127 138	82 165 247 393 553 715 874 1034 1196	E 66 E 48 E 46 E 41 E 38 E 36 E 34 E 34 E 34	E 66 E 59 E 40 E 38 E 36 E 35 E 34 E 32 E 30
10 15 20 25 30 35 40 45 50	80 120 160 200 227 252 277 302 327 349	100 225 500 825 1224 1642 2088 2623 3202 3804	36 41 52 57 60 62 63 64 65 66	45 90 138 222 309 398 485 572 661 748	E 36 E 28 E 26 E 23 E 21 E 20 E 18 E 17 E 16 E 15	E 36 E 32 E 22 E 22 E 20 E 19 E 19 E 17 E 17 E 16	36 41 52 57 60 67 74 83 90 95	45 90 138 222 309 398 487 622 760 934	E 36 E 28 E 26 E 23 E 21 E 21 E 21 E 21 E 22 E 22 E 22 E 22	E 36 E 32 E 22 E 22 E 20 E 19 E 19 E 19 E 19 E 19 E 19 E 20	66 73 92 102 108 112 117 127 138 150	82 165 247 393 553 715 874 1034 1196 1369	E 66 E 48 E 46 E 41 E 38 E 36 E 34 E 34 E 34 E 34 E 34	E 66 E 59 E 40 E 38 E 36 E 35 E 34 E 32 E 30 E 29
10 15 20 25 30 35 40 45	80 120 160 227 252 277 302 327	100 225 500 825 1224 1642 2088 2623 3202	36 41 52 57 60 62 63 64 65	45 90 138 222 309 398 485 572 661	E 36 E 28 E 26 E 23 E 21 E 20 E 18 E 17 E 16	E 36 E 32 E 22 E 22 E 20 E 19 E 19 E 17 E 17	36 41 52 57 60 67 74 83 90	45 90 138 222 309 398 487 622 760	E 36 E 28 E 26 E 23 E 21 E 21 E 21 E 21 E 22 E 22 E 22	E 36 E 32 E 22 E 22 E 20 E 19 E 19 E 19 E 19 E 19	66 73 92 102 108 112 117 127 138	82 165 247 393 553 715 874 1034 1196	E 66 E 48 E 46 E 41 E 38 E 36 E 34 E 34 E 34	E 66 E 59 E 40 E 38 E 36 E 35 E 34 E 32 E 30
10 15 20 25 30 35 40 45 50 60	80 120 160 227 252 277 302 327 349 392	100 225 500 825 1224 1642 2088 2623 3202 3804 5196	36 41 52 57 60 62 63 64 65 66 66 67	45 90 138 222 309 398 485 572 661 748 928	E 36 E 28 E 26 E 23 E 21 E 20 E 18 E 17 E 16 E 15 E 14	E 36 E 32 E 22 E 22 E 20 E 19 E 19 E 17 E 17 E 17 E 16 E 14	36 41 52 57 60 67 74 83 90 95 103	45 90 138 222 309 398 487 622 760 934 1286	E 36 E 28 E 26 E 23 E 21 E 21 E 21 E 21 E 22 E 22 E 22 E 22	E 36 E 32 E 22 E 22 E 20 E 19 E 19 E 19 E 19 E 19 E 19 E 20 E 20 E 20	66 73 92 102 108 112 117 127 138 150 169	82 165 247 393 553 715 874 1034 1196 1369 1881	E 66 E 48 E 46 E 41 E 38 E 36 E 34 E 34 E 34 E 34 E 34 E 35	E 66 E 59 E 40 E 38 E 36 E 35 E 34 E 32 E 30 E 29 E 29 E 29
10 15 20 25 30 35 40 45 50 60 70	80 120 160 200 227 252 277 302 327 349 392 442	100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830	36 41 52 57 60 62 63 64 65 66 67 74	45 90 138 222 309 398 485 577 661 748 928 1109	E 36 E 28 E 26 E 23 E 21 E 20 E 18 E 17 E 16 E 15 E 14 E 13	E 36 E 32 E 22 E 20 E 19 E 19 E 17 E 17 E 17 E 16 E 14 E 13	36 41 52 57 60 67 74 83 90 95 103 109	45 90 138 222 309 398 487 622 760 934 1286 1634	E 36 E 28 E 26 E 23 E 21 E 21 E 21 E 21 E 22 E 22 E 22 E 22	E 36 E 32 E 22 E 20 E 19 E 19 E 19 E 19 E 19 E 20 E 20 E 20 E 19	66 73 92 102 108 112 117 127 138 150 169 183	82 165 247 393 553 715 874 1034 1196 1369 1881 2519	E 66 E 48 E 46 E 41 E 38 E 36 E 34 E 34 E 34 E 34 E 34 E 35 E 33	E 66 E 59 E 40 E 38 E 36 E 35 E 34 E 32 E 30 E 29 E 29 E 29 E 30
10 15 20 25 30 35 40 45 50 60 70 80	80 120 160 227 252 277 302 327 349 392 442 497	100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830 8638	36 41 52 57 60 62 63 64 65 66 66 67 74 83	45 90 138 222 309 398 485 572 661 748 928 1109 1289	E 36 E 28 E 26 E 23 E 21 E 20 E 18 E 17 E 16 E 15 E 14 E 13 E 13	E 36 E 32 E 22 E 22 E 20 E 19 E 19 E 17 E 17 E 17 E 16 E 14 E 13 E 12	36 41 52 57 60 67 74 83 90 95 103 109 114	45 90 138 222 309 338 487 622 760 934 1286 1634 1986	E 36 E 28 E 26 E 23 E 21 E 21 E 21 E 22 E 22 E 22 E 22 E 21 E 20 E 18	E 36 E 32 E 22 E 22 E 20 E 19 E 19 E 19 E 19 E 20 E 20 E 20 E 19 E 18	66 73 92 102 108 112 117 127 138 150 169 183 193	82 165 247 393 553 715 874 1034 1196 1369 1881 2519 3163	E 66 E 48 E 46 E 41 E 38 E 36 E 34 E 34 E 34 E 34 E 34 E 35 E 33 E 31	E 66 E 59 E 40 E 38 E 36 E 35 E 34 E 32 E 30 E 29 E 29 E 29 E 30 E 29
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10 15 20 25 30 35 40 45 50 60 70 80 90 100 100 100 15 20 25 30 35 40 35 10 15 20 25 30 35 40 35 50 50 50 50 50 50 50 50 50 5	80 120 160 200 227 252 277 302 327 349 392 442 497 549 600 Max. Shear ( <i>kp</i> ) 80 120 160 200 227 252 277	100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830 8638 10678 12893 PER ESO Max. Moment (FF KIP) 100 225 500 825 1224 1642 2088	36 41 52 57 60 62 63 64 65 66 67 74 83 90 95 95 Max. Shear ( <i>kp</i> ) 66 73 92 102 108 123 135	45 90 138 222 309 398 485 572 661 748 928 109 1289 1469 1683 Two B120-G1 Max. Moment (FF xt/) 83 165 248 394 555 715 902	E 36 E 28 E 26 E 23 E 21 E 20 E 18 E 17 E 16 E 15 E 14 E 13 E 13 E 13 E 13 E 13 E 13 E 13 E 13	E 36 E 32 E 22 E 20 E 19 E 19 E 17 E 17 E 17 E 17 E 16 E 14 E 13 E 12 E 11 E 10 AVING MOMENT E 66 E 59 E 40 E 38 E 35 E 35 E 35	36 41 52 57 60 67 74 83 90 95 103 109 114 119 127 Max. Shear ( <i>kiP</i> ) 65 72 92 102 107 111 111	45 90 138 222 309 398 487 622 760 934 1286 1634 1986 2337 2686 GP35 Single Max. Moment (FT K <sup>B</sup> ) 82 164 246 390 549 710 888	E 36 E 28 E 26 E 23 E 21 E 21 E 21 E 22 E 22 E 22 E 22 E 21 E 20 E 18 E 17 E 17 E 17 E 17 E 17 E 17 E 18 E 48 E 46 E 48 E 46 E 41 E 48 E 23 E 48 E 23 E 48 E 23 E 21 E 21 E 21 E 21 E 21 E 21 E 21 E 21	E 36 E 32 E 22 E 22 E 20 E 19 E 19 E 19 E 19 E 19 E 20 E 20 E 20 E 19 E 18 E 18 E 18 E 18 E 17 VIING MOMENT E 65 E 55 E 39 E 38 E 35 E 33	66 73 92 102 108 112 117 127 138 150 169 183 193 201 207 207 8 8 8 8 8 9 201 207 8 72 92 102 107 122 134	82 165 247 393 553 715 874 1034 1196 1369 1881 2519 3163 3806 4444 Two GP35 Max. Moment (FT kP) 82 164 246 391 550 710 895	E 66 E 48 E 46 E 41 E 38 E 36 E 34 E 34 E 34 E 34 E 35 E 33 E 31 E 29 E 28 N line *E * RA SHEAR E 65 E 48 E 46 E 41 E 48 E 49 E 49 E 49 E 48 E 46 E 41 E 48 E 48 E 48 E 48 E 48 E 48 E 48 E 48	E 66 E 59 E 40 E 38 E 36 E 35 E 34 E 33 E 34 E 29 E 29 E 29 E 29 E 29 E 29 E 29 E 28 E 30 E 29 E 29 E 28 E 30 E 29 E 28 E 30 E 29 E 28 E 36 E 35 E 34 E 35 E 34 E 35 E 34 E 35 E 34 E 35 E 35 E 34 E 35 E 35 E 34 E 35 E 35 E 35 E 34 E 35 E 35 E 35 E 35 E 35 E 35 E 35 E 35
10 15 20 25 30 35 40 45 50 60 70 80 90 100 100 (Feet) 5 10 15 20 25 30 35 40 45 5 100 15 20 25 30 35 40 45 50 100 100 100 100 100 100 100	80 120 160 200 227 325 277 302 327 349 392 442 497 549 600 Max. Shear (KP) 80 120 160 200 227 252 277 302 327 327 325 277 302 327 327 327 327 327 327 327 32	100 225 500 825 1224 2088 2623 3202 3804 5196 6830 8638 10678 12893 PER E80 Max. Moment (FF KIP) 100 225 500 825 1224 1642 2088 2623	36 41 52 57 60 62 63 64 65 66 67 74 83 90 95 95 Max. Shear ( <i>KP</i> ) 66 73 92 102 108 123 135 151	45 90 138 222 309 398 485 572 661 748 928 928 109 1289 1469 1683 1469 1683 Two B120-GI Max. Moment (FF KB) 83 165 248 394 555 715 900 1149	E 36 E 28 E 26 E 23 E 21 E 20 E 18 E 17 E 16 E 15 E 14 E 13 E 13 E 13 E 13 E 13 E 13 E 13 E 13	E 36 E 32 E 22 E 22 E 20 E 19 E 19 E 17 E 17 E 16 E 14 E 17 E 16 E 14 E 12 E 11 E 10 ATING MOMENT E 66 E 59 E 40 E 38 E 36 E 35 E 35 E 35 E 32	36 41 52 57 60 67 74 83 90 95 103 109 114 119 127 Max. She ar ( <i>KIP</i> ) 65 72 92 102 107 111 116 126	45 90 138 222 309 398 487 622 760 934 1286 1634 1286 1634 1986 2337 2686 Max. Moment (FF KP) 82 164 246 390 549 710 888 1026	Е 36 Е 28 Е 26 Е 23 Е 21 Е 21 Е 21 Е 22 Е 22 Е 22 Е 22 Е 22 Е 22 Е 22 Е 21 Е 20 Е 18 Е 17 Е 17	E 36 E 32 E 22 E 22 E 20 E 19 E 19 E 19 E 19 E 20 E 20 E 20 E 19 E 19 E 19 E 19 E 19 E 19 E 19 E 19	66 73 92 102 108 112 117 127 138 150 169 183 193 201 207 207 8 8 8 8 93 201 207 8 7 207 207 207 207 207 207 207 207 207 2	82 165 247 393 553 715 874 1034 1196 1369 1881 2519 3163 3806 4444 4444 Two GP35 Max. Moment (Fr kill) 82 164 246 391 550 710 855 1141	E 66 E 48 E 46 E 41 E 38 E 36 E 34 E 34 E 34 E 34 E 34 E 35 E 33 E 31 E 29 E 28 m line "E" RA SHEAR E 65 E 48 E 46 E 41 E 38 E 39 E 39 E 40	E 66 E 59 E 40 E 35 E 35 E 34 E 35 E 34 E 35 E 29 E 29 E 29 E 29 E 29 E 29 E 29 E 28 MOMENT E 66 E 58 E 39 E 38 E 36 E 35 E 34 E 35 E 35 E 34 E 35 E 34 E 35 E 35 E 34 E 35 E 34 E 35 E 34 E 35 E 35 E 34 E 35 E 35 E 34 E 35 E 35 E 34 E 35 E 35 E 35 E 35 E 35 E 35 E 35 E 35
10 15 20 25 30 35 40 45 50 60 70 80 90 100 100 100 100 100 100 15 10 15 20 25 30 35 40 45 50 60 100 100 100 15 20 25 30 35 40 45 50 60 100 100 100 100 100 100 100	80 120 160 200 227 252 277 302 327 349 392 442 497 549 600 Max. Shear ( <i>(wP)</i> ) 80 120 160 200 227 252 277 302 327 349 392 327 327 349 392 327 349 392 327 349 392 327 349 392 327 327 327 327 327 327 327 32	100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830 8638 10678 12893 PER ESO Max. Moment (FF KIP) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196	36 41 52 57 60 62 63 64 65 66 67 74 83 90 95 95 Max. Shear ( <i>KP</i> ) 66 73 92 102 108 123 135 151 164 174 190	45 90 138 222 309 398 485 572 661 748 928 109 1289 1469 1683 165 1469 1683 165 248 394 555 715 902 1149 1404 1725 2367	E 36 E 28 E 26 E 22 E 22 E 22 E 22 E 18 E 17 E 16 E 15 E 14 E 13 E 13 E 13 E 13 E 13 E 13 E 13 E 13	E 36 E 32 E 22 E 22 E 20 E 19 E 19 E 19 E 17 E 17 E 16 E 14 E 17 E 16 E 14 E 12 E 11 E 10 ANDMENT E 66 E 59 E 40 E 38 E 35 E 35 E 35 E 35 E 36 E 36 E 36 E 32	36 41 52 57 60 67 74 83 90 95 103 109 114 119 127 Max. Shear ( <i>KIP</i> ) 65 72 92 102 107 111 116 126 137 149 168	45 90 138 222 309 398 487 622 760 934 1286 1634 1986 2337 2686 Wax. Moment (FT K#) 82 164 246 390 549 549 710 868 81026 1187 1359 1867	E 36 E 28 E 26 E 23 E 21 E 21 E 21 E 22 E 22 E 22 E 22 E 22	E 36 E 32 E 22 E 22 E 20 E 19 E 19 E 19 E 19 E 19 E 19 E 19 E 19	66 73 92 102 108 112 117 127 138 150 169 183 193 201 207 207 207 Max. Shear ( <i>k</i> <sup>(x)</sup> ) 65 72 92 102 107 122 107 122 134 150 163 1772 188	82 165 247 393 553 715 874 1034 1196 1369 1881 2519 3163 3806 4444 Two GP351 Max. Moment (FT KP) 82 164 246 391 550 710 895 1141 1394 1712 2349	E 66 E 48 E 46 E 41 E 38 E 36 E 34 E 34 E 34 E 34 E 33 E 31 E 29 E 29 E 29 E 29 E 29 E 29 E 29 E 29	E 66 E 59 E 40 E 38 E 36 E 35 E 34 E 33 E 34 E 29 E 29 E 29 E 29 E 29 E 29 E 28 E 29 E 28 E 30 E 29 E 28 E 36 E 35 E 34 E 35 E 34 E 35 E 34 E 35 E 35 E 34 E 35 E 34 E 35 E 35 E 34 E 35 E 34 E 35 E 34 E 35 E 34 E 35 E 35 E 34 E 35 E 34 E 35 E 35 E 34 E 30 E 29 E 29 E 29 E 29 E 29 E 29 E 29 E 29
10 15 20 25 30 35 40 45 50 60 70 80 90 100 100 5 10 15 20 25 30 35 40 45 5 10 15 5 10 15 20 25 30 50 50 50 100 50 100 50 100 10	80 120 160 200 227 325 277 302 327 349 392 442 497 549 600 Max. Shear ( <i>KP</i> ) 80 120 160 200 227 252 277 302 327 349 392 442	100 225 500 825 1224 2088 2623 3202 3804 5196 6830 8638 10678 12893 PER E80 Max. Moment (FF ki9) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830	36 41 52 57 60 62 63 64 65 66 67 74 83 90 95 95 Max. Shear ( <i>XIP</i> ) 66 73 92 102 108 123 135 151 164 174 190 212	45 90 138 222 309 398 485 572 661 748 928 109 1289 1469 1683 1469 1683 1469 1683 1469 1683 1469 1683 165 248 394 555 715 902 1149 1404 1725 2367 3007	E 36 E 28 E 26 E 23 E 21 E 20 E 18 E 17 E 16 E 15 E 14 E 13 E 13 E 13 E 13 E 13 E 13 E 13 E 13	E 36 E 32 E 22 E 20 E 19 E 19 E 17 E 17 E 16 E 14 E 13 E 12 E 11 E 10 ATING MOMENT E 66 E 59 E 40 E 38 E 36 E 35 E 35 E 35 E 35 E 35 E 35 E 35 E 35	36 41 52 57 60 67 74 83 90 95 103 109 114 119 127 Max. Shear (XIP) 65 72 92 102 107 111 116 126 137 149 168 182	45 90 138 222 309 398 487 622 760 934 1286 1634 1986 2337 2686 2337 2686 Max. Moment (FT KB) 82 164 246 390 549 710 888 1026 1187 710 888 1026 1187 1359 13667 2500	E 36 E 28 E 26 E 23 E 21 E 21 E 21 E 21 E 22 E 22 E 22 E 22	E 36 E 32 E 22 E 22 E 20 E 19 E 19 E 19 E 19 E 20 E 20 E 20 E 20 E 19 E 18 E 18 E 18 E 18 E 17 WING MOMENT E 65 E 58 E 39 E 38 E 36 E 33 E 31 E 30 E 31 E 30 E 31 E 30 E 32 E 32 E 32 E 33 E 31 E 30 E 32 E 32 E 32 E 33 E 31 E 30 E 32 E 32 E 33 E 31 E 30 E 32 E 32 E 33 E 31 E 30 E 32 E 33 E 31 E 30 E 33 E 33 E 33 E 33 E 33 E 33 E 33	66 73 92 102 108 112 117 138 150 169 183 193 201 207 207 207 207 8 8 8 92 102 107 107 122 134 150 163 172 188 210	82 165 247 393 553 715 874 1034 1196 1369 1881 2519 3163 3806 4444 Two GP35 Max. Moment (FT kP) 82 164 246 391 550 710 855 1141 1394 1712 2349 2984	E 66 E 48 E 46 E 41 E 38 E 36 E 34 E 34 E 34 E 34 E 35 E 33 E 31 E 29 E 28 N line "E" R2 SHEAR E 65 E 48 E 46 E 41 E 38 E 39 E 39 E 39 E 39 E 40 E 40 E 40 E 40 E 40 E 40 E 40 E 40	E 66 E 59 E 40 E 35 E 35 E 34 E 35 E 34 E 35 E 29 E 29 E 29 E 29 E 29 E 29 E 29 E 29
10 15 20 25 30 35 40 45 50 60 70 80 90 100 100 5 100 15 20 25 30 35 40 45 5 100 15 20 25 30 35 40 45 50 60 70 80 90 100 100 15 100 100 100 15 100 100	80 120 160 200 227 352 277 302 327 349 392 442 497 549 600 Max. Shear ( <i>KP</i> ) 80 120 160 200 227 252 277 302 327 327 327 327 327 327 327 32	100 225 500 825 1224 2088 2623 3202 3804 5196 6830 8638 10678 12893 Max. Moment (FF KU) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830	36 41 52 57 60 62 63 64 65 66 67 74 83 90 95 95 Max. Shear ( <i>KP</i> ) 66 73 92 102 108 123 135 151 164 174 190 212 235	45 90 138 222 309 398 485 572 661 748 928 928 928 1109 1289 1469 1683 Two B120-GI Max. Moment ( <i>FT KD</i> ) 83 165 248 394 555 715 902 1149 1404 1725 2367 3007 3817	E 36 E 28 E 26 E 23 E 21 E 20 E 18 E 17 E 16 E 15 E 14 E 13 E 13 E 13 E 13 E 13 E 13 E 13 E 13	E 36 E 32 E 22 E 22 E 20 E 19 E 19 E 17 E 17 E 16 E 14 E 17 E 16 E 14 E 12 E 11 E 10 ATING MOMENT E 66 E 59 E 40 E 35 E 35 E 35 E 35 E 35 E 35 E 35 E 35	36 41 52 57 60 67 74 83 90 95 103 109 114 119 127 Max. Shear ( <i>KIP</i> ) 65 72 92 102 107 111 116 126 137 149 168 182 192	45 90 138 222 309 398 487 622 760 934 1286 1634 1286 1634 1986 2337 2686 6P35 Single Max. Moment (FT κ#) 82 164 246 390 549 710 82 164 246 390 549 710 88 1026 1187 1359 1867 2500 3139	Е 36 Е 28 Е 26 Е 23 Е 21 Е 21 Е 21 Е 22 Е 22 Е 22 Е 22 Е 22 Е 22 Е 22 Е 21 Ε 20 Ε 18 Ε 17 Ε 17	E 36 E 32 E 22 E 22 E 20 E 19 E 19 E 19 E 20 E 20 E 20 E 19 E 19 E 19 E 19 E 19 E 19 E 19 E 19	66 73 92 102 108 112 117 127 138 150 169 183 193 201 207 8 8 70 65 72 92 102 107 122 107 107 122 134 150 163 172 134 150 163 172 188 210 233	82 165 247 393 553 715 874 1034 1196 1369 1881 2519 3163 3806 4444 Two GP35 Max. Moment (FT KUP) 82 164 246 391 550 710 895 1141 1394 1712 2349 2984 3788	E 66 E 48 E 46 E 41 E 38 E 36 E 34 E 34 E 34 E 34 E 35 E 33 E 31 E 29 E 28 n line "E" RA SHEAR E 65 E 48 E 46 E 41 E 38 E 39 E 39 E 39 E 40 E 40 E 40 E 40 E 40 E 40 E 40 E 40	E 66 E 59 E 40 E 35 E 35 E 34 E 35 E 34 E 30 E 29 E 29 E 29 E 29 E 29 E 29 E 29 E 29
10 15 20 25 30 35 40 45 50 60 70 80 90 100 100 5 10 15 20 25 30 35 40 45 5 10 15 5 10 15 20 25 30 50 50 50 100 50 100 50 100 10	80 120 160 200 227 325 277 302 327 349 392 442 497 549 600 Max. Shear ( <i>KP</i> ) 80 120 160 200 227 252 277 302 327 349 392 442	100 225 500 825 1224 2088 2623 3202 3804 5196 6830 8638 10678 12893 PER E80 Max. Moment (FF ki9) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830	36 41 52 57 60 62 63 64 65 66 67 74 83 90 95 95 Max. Shear ( <i>XIP</i> ) 66 73 92 102 108 123 135 151 164 174 190 212	45 90 138 222 309 398 485 572 661 748 928 109 1289 1469 1683 1469 1683 1469 1683 1469 1683 1469 1683 165 248 394 555 715 902 1149 1404 1725 2367 3007	E 36 E 28 E 26 E 23 E 21 E 20 E 18 E 17 E 16 E 15 E 14 E 13 E 13 E 13 E 13 E 13 E 13 E 13 E 13	E 36 E 32 E 22 E 20 E 19 E 19 E 17 E 17 E 16 E 14 E 13 E 12 E 11 E 10 ATING MOMENT E 66 E 59 E 40 E 38 E 36 E 35 E 35 E 35 E 35 E 35 E 35 E 35 E 35	36 41 52 57 60 67 74 83 90 95 103 109 114 119 127 Max. Shear (XIP) 65 72 92 102 107 111 116 126 137 149 168 182	45 90 138 222 309 398 487 622 760 934 1286 1634 1986 2337 2686 2337 2686 Max. Moment (FT KB) 82 164 246 390 549 710 888 1026 1187 710 888 1026 1187 1359 13667 2500	E 36 E 28 E 26 E 23 E 21 E 21 E 21 E 21 E 22 E 22 E 22 E 22	E 36 E 32 E 22 E 22 E 20 E 19 E 19 E 19 E 19 E 20 E 20 E 20 E 20 E 19 E 18 E 18 E 18 E 18 E 17 WING MOMENT E 65 E 58 E 39 E 38 E 36 E 33 E 31 E 30 E 31 E 30 E 31 E 30 E 32 E 32 E 32 E 33 E 31 E 30 E 32 E 32 E 32 E 33 E 31 E 30 E 32 E 32 E 33 E 31 E 30 E 32 E 32 E 33 E 31 E 30 E 32 E 33 E 31 E 30 E 33 E 33 E 33 E 33 E 33 E 33 E 33	66 73 92 102 108 112 117 138 150 169 183 193 201 207 207 207 207 8 8 8 92 102 107 107 122 134 150 163 172 188 210	82 165 247 393 553 715 874 1034 1196 1369 1881 2519 3163 3806 4444 Two GP35 Max. Moment (FT kP) 82 164 246 391 550 710 855 1141 1394 1712 2349 2984	E 66 E 48 E 46 E 41 E 38 E 36 E 34 E 34 E 34 E 34 E 35 E 33 E 31 E 29 E 28 N line "E" R2 SHEAR E 65 E 48 E 46 E 41 E 38 E 39 E 39 E 39 E 39 E 40 E 40 E 40 E 40 E 40 E 40 E 40 E 40	E 66 E 59 E 40 E 35 E 35 E 34 E 35 E 34 E 35 E 29 E 29 E 29 E 29 E 29 E 29 E 29 E 29

Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

#### CONNECTICUT RAILROAD EQUIPMENT TABLE A<sup>(CONT)</sup>

EQUIVALENT COOPER LOADING SUMMARY

	C00	PER E80		GP9 Single	Engine			Two GP9 i	n line			GP8 Single	Engine	
span	Max. Shear	Max. Moment	Max. Shear	Max. Moment	"E" RA	ATING	Max. Shear	Max. Moment	"E" RA	TING	Max. Shear	Max. Moment	"E" R	ATING
(Feet)	(KIP)	(FT KIP)	(KIP)	(FT KIP)	SHEAR	MOMENT	(KIP)	(FT KIP)	SHEAR	MOMENT	(KIP)	(FT KIP)	SHEAR	MOMENT
5	80	100	63	78	E 63	E 63	63	78	E 63	E 63	62	78	E 62	E 62
10	120	225	69	157	E 46	E 56	69	157	E 46	E 56	68	156	E 46	E 55
15	160	500	88	235	E 44	E 38	88	235	E 44	E 38	87	233	E 44	E 37
20	200	825	97	374	E 39	E 36	97	375	E 39	E 36	96	371	E 39	E 36
25	200	1224	103	526	E 36	E 34	103	527	E 36	E 34	102	521	E 36	E 34
30	252	1642	103	680	E 34	E 34	103	680	E 37	E 34	102	674	E 34	E 34
35	277	2088	117	831	E 34	E 32	129	865	E 37	E 33	116	825	E 33	E 32
40	302	2623	126	983	E 33	E 30	145	1101	E 38	E 34	125	975	E 33	E 30
45	327	3202	139	1154	E 34	E 29	156	1348	E 38	E 34	138	1145	E 34	E 29
50	349	3804	151	1381	E 35	E 29	166	1653	E 38	E 35	149	1370	E 34	E 29
60	392	5196	167	1945	E 34	E 30	184	2263	E 38	E 35	166	1930	E 34	E 30
70	442	6830	179	2552	E 32	E 30	207	2954	E 38	E 35	178	2531	E 32	E 30
80	497	8638	188	3164	E 30	E 29	229	3739	E 37	E 35	187	3138	E 30	E 29
90	549	10678	195	3775	E 28	E 28	245	4680	E 36	E 35	194	3745	E 28	E 28
100	600	12893	201	4382	E 27	E 27	261	5718	E 35	E 35	199	4347	E 27	E 27
		PER E80		Two GP8 i				220k Single Ho				220k Hopper C		
span	Max. Shear	Max. Moment	Max. Shear	Max. Moment	"E" RA	ATING	Max. Shear	Max. Moment	"E" RA	TING	Max. Shear	Max. Moment	"E" R/	ATING
(Feet)	(KIP)	(FT KIP)	(KIP)	(FT KIP)	SHEAR	MOMENT	(KIP)	(FT KIP)	SHEAR	MOMENT	(KIP)	(FT KIP)	SHEAR	MOMENT
5	80	100	62	78	E 62	E 62	55	69	E 55	E 55	55	69	E 55	E 55
10	120	225	68	156	E 46	E 55	78	138	E 52	E 49	78	138	E 52	E 49
15	160	500	87	233	E 44	E 37	89	267	E 44	E 43	94	267	E 47	E 43
20	200	825	96	372	E 39	E 36	94	401	E 38	E 39	113	453	E 45	E 44
25	227	1224	102	523	E 36	E 34	97	535	E 34	E 35	135	663	E 47	E 43
30	252	1642	116	674	E 37	E 33	99	668	E 32	E 33	149	930	E 47	E 45
35	277	2088	128	859	E 37	E 33	105	802	E 30	E 31	159	1197	E 46	E 46
40	302	2623	143	1092	E 38	E 33	115	940	E 30	E 29	167	1465	E 44	E 45
45	327	3202	155	1337	E 38	E 33	127	1077	E 31	E 27	173	1732	E 42	E 43
50	349	3804	165	1640	E 38	E 34	136	1215	E 31	E 26	182	2005	E 42	E 42
60	392	5196	183	2245	E 37	E 35	150	1673	E 31	E 26	205	2555	E 42	E 39
70	442	6830	206	2931	E 37	E 34	160	2208	E 29	E 26	223	3184	E 40	E 37
80	497	8638	227	3709	E 36	E 34	168	2742	E 27	E 25	236	3986	E 38	E 37
90	549	10678	243	4643	E 35	E 35	173	3277	E 25	E 25	247	4878	E 36	E 37
100	600	12893	259	5672	E 35	E 35	175	3812	E 24	E 24	255	5954	E 34	E 37
100	000	12055	235	5072	2.35	2.35	1/0	5012	1.24	L 24	255	5554	1.34	2.57
		PER E80		ACELA Single	Engine			Two ACELA	in line			AEM-7 Single	Engine	
span			Max. Shear	Max. Moment		ATING	May Shear	Max. Moment	"E" RA	TING	Max. Shear			ATING
(Feet)	(KIP)	(FT KIP)	(KIP)	(FT KIP)	SHEAR	MOMENT	(KIP)	(FT KIP)	SHEAR	MOMENT	(KIP)	(FT KIP)	SHEAR	MOMENT
(reet) 5	(K)P) 80	100	50	63	E 50	E 50	50	(FT KIP) 63	E 50	E 50	50	63	E 50	E 51
10	120	225	53	125	E 36	E 44	53	125	E 30	E 44	56	126	E 30	E 45
10	120	500	69	125	E 30	E 44 E 30	69	125	E 36	E 44 E 30	71	120	E 37	E 45 E 30
20	200	825	77	291	E 34 E 31		77	292	E 34 E 31	E 30	71	302	E 35	E 30
	200					E 28								
25		1224	82	413	E 29	E 27	82	413	E 29	E 27	83	424	E 29	E 28
30	252	1642	93	535	E 30	E 26	93	535	E 30	E 26	93	547	E 30	E 27
35	277	2088	103	694	E 30	E 27	103	696	E 30	E 27	102	680	E 30	E 26
40	302	2623	115	881	E 30	E 27	115	883	E 30	E 27	115	869	E 30	E 27
45	327	3202	124	1079	E 30	E 27	124	1082	E 30	E 27	124	1058	E 30	E 26
50	349	3804	132	1325	E 30	E 28	132	1325	E 30	E 28	132	1301	E 30	E 27
60	392	5196	143	1808	E 29	E 28	144	1811	E 29	E 28	144	1793	E 29	E 28
70	442	6830	151	2296	E 27	E 27	161	2297	E 29	E 27	152	2284	E 28	E 27
80	497	8638	157	2783	E 25	E 26	178	2917	E 29	E 27	158	2774	E 25	E 26
90	549	10678	162	3267	E 24	E 24	196	3590	E 28	E 27	163	3266	E 24	E 24
100	600	12893	166	3767	E 22	E 23	215	4350	E 29	E 27	167	3758	E 22	E 23
	CO0	PER E80		Two AEM-7	in line			AMD-103 sing	le Engine			Two AMD-10	3 in line	
span	Max. Shear	Max. Moment	Max. Shear	Max. Moment	"E" RA	ATING	Max. Shear	Max. Moment	"E" RA	TING	Max. Shear	Max. Moment	"E" R/	ATING
(Feet)	(KIP)	(FT KIP)	(KIP)	(FT KIP)	SHEAR	MOMENT	(KIP)	(FT KIP)	SHEAR	MOMENT	(KIP)	(FT KIP)	SHFAR	MOMENT

				TWO ALIVI-7	mme			AND-103 SING	ie Lingilie			TWO AND-10	Jinime	
span	Max. Shear	Max. Moment	Max. Shear	Max. Moment	"E" RA	"E" RATING		Max. Moment	"E" RA	TING	Max. Shear	Max. Moment	"E" RA	TING
(Feet)	(KIP)	(FT KIP)	(KIP)	(FT KIP)	SHEAR	MOMENT	(KIP)	(FT KIP)	SHEAR	MOMENT	(KIP)	(FT KIP)	SHEAR	MOMENT
5	80	100	50	63	E 50	E 51	68	85	E 68	E 68	68	85	E 68	E 68
10	120	225	56	126	E 37	E 45	75	170	E 50	E 60	75	170	E 50	E 60
15	160	500	71	189	E 35	E 30	95	255	E 48	E 41	95	255	E 48	E 41
20	200	825	78	302	E 31	E 29	105	406	E 42	E 39	105	406	E 42	E 39
25	227	1224	83	424	E 29	E 28	112	571	E 39	E 37	112	571	E 39	E 37
30	252	1642	95	547	E 30	E 27	116	737	E 37	E 36	125	737	E 40	E 36
35	277	2088	105	705	E 30	E 27	119	902	E 34	E 35	137	924	E 40	E 35
40	302	2623	117	894	E 31	E 27	121	1067	E 32	E 33	154	1179	E 41	E 36
45	327	3202	126	1101	E 31	E 28	124	1232	E 30	E 31	167	1437	E 41	E 36
50	349	3804	134	1348	E 31	E 28	132	1398	E 30	E 29	178	1766	E 41	E 37
60	392	5196	154	1902	E 31	E 29	153	1734	E 31	E 27	193	2429	E 39	E 37
70	442	6830	176	2546	E 32	E 30	170	2130	E 31	E 25	205	3090	E 37	E 36
80	497	8638	193	3304	E 31	E 31	182	2712	E 29	E 25	224	3749	E 36	E 35
90	549	10678	214	4158	E 31	E 31	192	3374	E 28	E 25	244	4443	E 36	E 33
100	600	12893	233	5042	E 31	E 31	200	4033	E 27	E 25	260	5293	E 35	E 33

Load chart values are applicable to specific structural members. For limitations, see Executive Summary.



#### CONNECTICUT RAILROAD EQUIPMENT TABLE A<sup>(CONT)</sup> EQUIVALENT COOPER LOADING SUMMARY

	600			Two AMD 102 P	ack to Back			UUD 9 cingle	Engino			Two HUD 0	in line	
span		PER E80 Max. Moment		Two AMD-103 B Max. Moment	"E" R	TING	Max Shear	HHP-8 single Max. Moment	Engine "E" RA	TING	Max Shear	Two HHP-8 Max. Moment	"E" R4	TING
(Feet)	(KIP)	(FT KIP)	(KIP)	(FT KIP)	SHEAR	MOMENT	(KIP)	(FT KIP)	SHEAR	MOMENT	(KIP)	(FT KIP)	SHEAR	MOMENT
5	80	100	68	85	E 68	E 68	55	69	E 55	E 55	55	69	E 55	E 56
10	120	22.5	75	170	E 50	E 60	59	139	E 39	E 49	59	139	E 39	E 49
15	160	500	95	255	E 48	E 41	76	208	E 38	E 33	76	208	E 38	E 33
20	200	82.5	105	406	E 42	E 39	85	323	E 34	E 31	85	324	E 34	E 31
25	227	1224	113	571	E 40	E 37	90	459	E 32	E 30	90	459	E 32	E 30
30	252	1642	128	737	E 41	E 36	94	594	E 30	E 29	94	594	E 30	E 2 9
35	277	2088	143	975	E41	E 37	96	728	E 28	E 28	101	728	E 29	E 28
40	302	2623	159	1230	E 42	E 38	104	863	E 28	E 26	109	863	E 29	E26
45	327	3202	172	1522	E 42	E 38	111	998	E 27	E 25	120	998	E 29	E 2 5
50	349	3804	182	1851	E 42	E 39	122	1132	E 28	E 24	131	1201	E 30	E25
60	392	5196	197 210	2514	E 40	E 39	139	1527	E 28	E 24	146 159	1677	E 30 E 29	E 26
70	442 497	6830 8638	229	3175 3834	E 38 E 37	E 37 E 36	151 160	2059	E 27 E 26	E 24 E 24	176	2218 2803	E 29	E 26 E 26
90	549	10678	249	4545	E 36	E 34	167	3140	E 24	E 24	194	3510	E 28	E 26
100	600	12893	264	5396	E 35	E 33	172	3678	E 23	E 23	208	4343	E 28	E 27
	C00	PER E80		GP40 single	Engine			Two GP40	in line			GP40-2 single	e Engine	
span	Max. Shear	Max. Moment	Max. Shear	Max. Moment	"E" R4	ATING	Max. Shear	Max. Moment	"E" RA	TING	Max. Shear	Max. Moment	"E" R4	TING
(Feet)	(KIP)	(FT KIP)	(KIP)	(FT KIP)	SHEAR	MOMENT	(KIP)	(FT KIP)	SHEAR	MOMENT	(KDP)	(FT KIP)	SHEAR	MOMENT
5	80	100	70	87	E 70	E 70	70	88	E 70	E 70	64	80	E 64	E 64
10	120	22.5	77	175	E 51	E 62	77	175	E 51	E 62	70	160	E 47	E 57
15	160	500	98	262	E 49	E 42	98	263	E 49	E 42	90	240	E 45	E 38
20	200	82.5	108	417	E 43	E 40	108	418	E 43	E 41	99	381	E 40	E 37
25	227	1224	115	586	E 40	E 38	115	588	E 40	E 38	105	536	E 37	E 35
30	252	1642	119	758 927	E 38	E 37	130	758	E 41	E 37	109	693	E 35	E 34
35	277 302	2088 2623	122 129	927	E 35 E 34	E 36 E 33	143 160	957 1219	E 41 E 42	E 37 E 37	113 123	848 1003	E 33 E 33	E 32 E 31
40	302	3202	129	1269	E 34	E 33	174	1219	E 42 E 42	E 37	123	1160	E 33	E 29
50	349	3804	150	1438	E 34	E 30	184	1830	E 42	E 38	146	1328	E 33	E 28
60	392	5196	172	1871	E 35	E 29	200	2510	E 41	E 39	164	1824	E 34	E 28
70	442	6830	187	2482	E 34	E 29	219	3189	E 40	E 37	177	2443	E 32	E 29
80	497	8638	199	3165	E 32	E 29	243	3935	E 39	E 36	187	3067	E 30	E 28
90	549	10678	208	3847	E 30	E 29	263	4810	E 38	E 36	195	3691	E 28	E 28
100	600	12893	215	4524	E 29	E 28	278	5823	E 37	E 36	201	4309	E 27	E 27
						·								
		PER E80		Two GP40-2				RS-3 Single				RS-3 in I		
span	Max. Shear	Max. Moment		Max. Moment	"E" R4	ATING		Max. Moment	"E" RA			Max. Moment	"E" RA	TING
(Feet)	Max. Shear (NIP)	Max. Moment	(KIP)	Max. Moment (FT KIP)	"E" RA SHEAR	MOMENT	(КІР)	Max. Moment	"E" RA SHEAR	MOMENT	(KD <sup>2</sup> )	Max. Moment (FT KIP)	"E" RA SHEAR	MOMENT
(Feet) 5	Max. Shear (NIP) 80	Max. Moment (FT KIP) 100	(кир) 64	Max. Moment (FT KIP) 80	"E" RA SHEAR E 64	MOMENT E 64	(кір) 57	Max. Moment (FT KIP) 72	"E" RA SHEAR E 57	MOMENT E 57	(кв) 57	Max. Moment (FT KIP) 72	"E" RA SHEAR E 57	MOMENT E 57
(Feet) 5 10	Max. Shear (NIP) 80 120	Max. Moment (FT кир) 100 225	(кир) 64 70	Мах. Moment (FT КIР) 80 160	"E" RA SHEAR E 64 E 47	момент Е 64 Е 57	(кир) 57 61	Max. Moment (FT кФ) 72 143	"E" RA SHEAR E 57 E 41	MOMENT E 57 E 51	(ки) 57 61	Мах. Moment (FT КIР) 72 143	"E" RA SHEAR E 57 E 41	MOMENT E 57 E 51
(Feet) 5 10 15	Max. Shear (NIP) 80 120 160	Max. Moment (FT KIP) 100 225 500	(кар) 64 70 90	Мах. Moment (FT KIP) 80 160 240	"E" RA SHEAR E 64 E 47 E 45	MOMENT E 64 E 57 E 38	(кір) 57 61 79	Max. Moment (FT КФ) 72 143 215	"E" RA SHEAR E 57 E 41 E 39	MOMENT E 57 E 51 E 34	(кв) 57 61 79	Max. Moment (FT KIP) 72 143 215	"E" R# SHEAR E 57 E 41 E 39	момент Е 57 Е 51 Е 34
(Feet) 5 10	Max. Shear (NP) 80 120 160 200	Max. Moment (FT KIP) 100 22.5 500 82.5	(кир) 64 70	Мах. Moment (FT КIР) 80 160 240 382	"E" RA SHEAR E 64 E 47	момент Е 64 Е 57	(кир) 57 61	Max. Moment (FT кФ) 72 143	"E" RA SHEAR E 57 E 41	MOMENT E 57 E 51	(ки) 57 61	Мах. Moment (FT КIР) 72 143	"E" RA SHEAR E 57 E 41	MOMENT E 57 E 51
(Feet) 5 10 15 20	Max. Shear (NIP) 80 120 160	Max. Moment (FT KIP) 100 225 500	(кир) 64 70 90 99	Мах. Moment (FT KIP) 80 160 240	"E" RA SHEAR E 64 E 47 E 45 E 40	MOMENT E 64 E 57 E 38 E 37	(кир) 57 61 79 88	Max. Moment (FT КФ) 72 143 215 334	"E" RA SHEAR E 57 E 41 E 39 E 35	MOMENT E 57 E 51 E 34 E 32	(кр) 57 61 79 88	Max. Moment (FT κιρ) 72 143 215 334	"E" RA SHEAR E 57 E 41 E 39 E 35	момент Е 57 Е 51 Е 34 Е 32
(Feet) 5 10 15 20 25	Max. Shear (NP) 80 120 160 200 227	Max. Moment (FT καν) 100 225 500 825 1224	(кир) 64 70 90 99 105	Мах. Moment (FT КIР) 80 160 240 382 538	"E" RA SHEAR E 64 E 47 E 45 E 40 E 37	MOMENT E 64 E 57 E 38 E 37 E 35	(КІР) 57 61 79 88 93	Max. Moment (FT κω) 72 143 215 334 473	"E" RA SHEAR E 57 E 41 E 39 E 35 E 33	MOMENT E 57 E 51 E 34 E 32 E 31	(кю) 57 61 79 88 93	Мах. Moment (FT кир) 72 143 215 334 473	"E" RA SHEAR E 57 E 41 E 39 E 35 E 35 E 33	MOMENT E 57 E 51 E 34 E 32 E 31
(Feet) 5 10 15 20 25 30	Max. Shear (NP) 80 120 160 200 227 252	Max. Moment (FT κυ?) 100 225 500 825 1224 1642	(KIP) 64 70 90 99 105 119	Мах. Moment (FF кtP) 80 160 240 382 538 693	"E" R/ SHEAR E 64 E 47 E 45 E 40 E 37 E 38	MOMENT E 64 E 57 E 38 E 37 E 35 E 34	(кир) 57 61 79 88 93 93 97	Max. Moment           (FT KΦ)           72           143           215           334           473           612	"E" RA SHEAR E 57 E 41 E 39 E 35 E 33 E 31	MOMENT E 57 E 51 E 34 E 32 E 31 E 30	(квр) 57 61 79 88 93 104	Мах. Moment ( <i>FT кiP</i> ) 72 143 215 334 473 612	"E" R/ SHEAR E 57 E 41 E 39 E 35 E 33 E 33	MOMENT E 57 E 51 E 34 E 32 E 31 E 30
(Feet) 5 10 15 20 25 30 35 40 45	Max. Shear (NP) 80 120 160 200 227 227 252 277 302 327	Max. Moment (FT κt/) 100 225 500 825 1224 1642 2088 2623 3202	(KIP) 64 70 90 99 105 119 131 147 159	Мах. Moment (FT КШ) 80 160 240 382 538 693 875 11115 1362	"E" R/ SHEAR E 64 E 47 E 45 E 40 E 37 E 38 E 38 E 39 E 39 E 39	MOMENT E 64 E 57 E 38 E 37 E 35 E 34 E 34 E 34 E 34	(KIP) 57 61 79 88 93 97 107 116 129	Max. Moment           (FT KΦ)           72           143           215           334           473           473           751           891           1074	"E" RA SHEAR E 57 E 41 E 39 E 35 E 33 E 31 E 31 E 31 E 31 E 32	MOMENT E 57 E 51 E 34 E 32 E 31 E 30 E 29 E 27 E 27 E 27	(K\$P) 57 61 79 88 93 104 114 128 139	Мах. Moment (FT к/P) 72 143 215 334 473 612 761 976 1190	"E" R/ SHEAR E 57 E 41 E 39 E 35 E 33 E 33 E 33 E 33 E 34 E 34	MOMENT E 57 E 51 E 34 E 32 E 31 E 30 E 29 E 30 E 30 E 30
(Feet) 5 10 15 20 25 30 35 40 45 50	Max. Shear (RIP) 80 120 160 200 227 252 277 302 327 349	Max. Moment (FF KIP) 100 225 500 825 1224 1642 2088 2623 3202 3804	(KIP) 64 70 90 99 105 119 131 147 159 169	Мах. Moment (FF КИ) 80 160 240 382 538 693 875 1115 1362 1673	"E" RA SHEAR E 64 E 47 E 45 E 40 E 37 E 38 E 38 E 38 E 39 E 39 E 39 E 39	MOMENT E 64 E 57 E 38 E 37 E 35 E 34 E 34 E 34 E 34 E 34 E 35	(KIP) 57 61 79 88 93 97 107 116 129 139	Max. Moment (FT K#) 72 143 215 334 473 612 751 891 1074 1288	"E" RA SHEAR E 57 E 41 E 39 E 35 E 33 E 31 E 31 E 31 E 31 E 31 E 32 E 32	MOMENT E 57 E 51 E 34 E 32 E 31 E 30 E 29 E 27 E 27 E 27 E 27	(KDP) 57 61 79 88 93 104 104 114 128 139 148	Мах. Moment (FT ки) 72 143 215 334 473 612 761 976 1190 1457	"E" RA SHEAR E 57 E 41 E 39 E 35 E 33 E 33 E 33 E 33 E 34 E 34 E 34	MOMENT E 57 E 51 E 34 E 32 E 31 E 30 E 29 E 30 E 30 E 31
(Feet) 5 10 15 20 25 30 35 40 45 50 60	Max. Shear (NP) 80 120 160 200 227 252 277 302 327 349 392	Max. Moment (FF KH) 100 225 500 825 1224 1642 2088 2623 3202 3202 3202 5196	(KUP) 64 70 90 99 105 119 131 147 159 169 184	Max. Moment (FF KH) 80 160 240 382 538 693 875 1115 1362 1673 2295	"E" RA SHEAR E 64 E 47 E 45 E 40 E 37 E 38 E 39 E 39 E 39 E 39 E 39 E 38	MOMENT E 64 E 57 E 38 E 37 E 35 E 34 E 34 E 34 E 34 E 34 E 35 E 35	(KUP) 57 61 79 88 93 97 107 116 129 139 154	Max. Moment (FF KP) 72 143 215 334 473 612 751 891 1074 1288 1819	"E" RA SHEAR E 57 E 41 E 39 E 35 E 33 E 31 E 31 E 31 E 31 E 31 E 32 E 32 E 32 E 31	MOMENT E 57 E 51 E 34 E 32 E 31 E 30 E 29 E 27 E 27 E 27 E 28	(KP) 57 61 79 88 93 104 114 128 139 148 166	Мах. Moment (FT КИР) 72 143 215 334 473 612 761 976 1190 1457 2014	"E" RA SHEAR E 57 E 41 E 39 E 35 E 33 E 33 E 33 E 34 E 34 E 34 E 34	MOMENT E 57 E 51 E 34 E 32 E 31 E 30 E 29 E 30 E 30 E 30 E 31 E 31
(Feet) 5 10 15 20 25 30 35 40 45 50 60 70	Max. Shear (NP) 80 120 160 227 252 277 302 327 349 392 442	Max. Moment (FF KH) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830	(KUP) 64 70 90 99 105 119 131 147 159 169 184 205	Мах. Moment (Ff ки) 80 160 240 382 538 693 875 1115 1362 1673 2295 2916	"E" RA SHEAR E 64 E 47 E 45 E 40 E 37 E 38 E 38 E 39 E 39 E 39 E 39 E 38 E 37	молент E 64 E 57 E 38 E 37 E 35 E 34 E 34 E 34 E 34 E 34 E 35 E 35 E 35 E 34	(KIP) 57 61 79 88 93 97 107 116 129 139 154 165	Max. Moment (FT KP) 72 73 215 334 473 612 751 891 1074 1288 1819 2376	"E" RA SHEAR E 57 E 41 E 39 E 35 E 33 E 31 E 31 E 31 E 32 E 32 E 31 E 31 E 31 E 32 E 32 E 31 E 31	MOMENT E 57 E 51 E 34 E 32 E 31 E 30 E 29 E 27 E 27 E 27 E 27 E 28 E 28	(KP) 57 61 79 88 93 104 114 128 139 139 148 166 187	Max. Moment (FF KP) 72 143 215 334 473 612 761 976 1190 1457 2014 2667	"E" RA SHEAR E 57 E 41 E 39 E 35 E 33 E 33 E 33 E 33 E 34 E 34 E 34 E 34	MOMENT E 57 E 51 E 34 E 32 E 31 E 30 E 29 E 30 E 30 E 30 E 31 E 31 E 31
(Feet) 5 10 15 20 25 30 35 40 45 50 60 70 80	Max. Shear (NP) 80 120 160 200 227 252 277 302 327 302 327 349 392 442 497	Max. Moment (FF κtr) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830 8638	(KUP) 64 70 90 99 105 119 131 147 159 169 169 184 205 227	Мах. Moment (FF кв) 80 160 240 382 538 693 875 1115 1362 1673 2295 2916 3701	"E" RA SHEAR E 64 E 47 E 45 E 40 E 37 E 38 E 38 E 39 E 39 E 39 E 39 E 39 E 39 E 39 E 39	MOM/MVT E 64 E 57 E 38 E 37 E 35 E 34 E 34 E 34 E 34 E 34 E 35 E 35 E 35 E 34 E 34 E 34 E 34	(KIP) 57 61 79 88 93 97 107 116 129 139 154 165 173	Max. Moment (FT K#) 72 143 215 334 473 612 751 891 1074 1288 1819 2376 2932	"E" RA SHEAR E 57 E 41 E 35 E 33 E 31 E 31 E 31 E 32 E 32 E 30 E 28	момент E 57 E 51 E 34 E 32 E 31 E 30 E 29 E 27 E 27 E 27 E 27 E 28 E 28 E 28 E 27	(KD) 57 61 79 88 93 104 114 128 139 148 139 148 166 187 206	Мах. Moment (FF KIP) 72 143 215 334 473 612 761 976 1190 1457 2014 2667 3402	"E" RA SHEAR E 57 E 41 E 35 E 33 E 33 E 33 E 33 E 34 E 34 E 34 E 34	MOMENT E 57 E 51 E 34 E 32 E 31 E 30 E 30 E 30 E 31 E 31 E 31 E 31 E 32
(Feet) 5 10 15 20 25 30 35 40 45 50 60 70 80 90	Max. Shear (NP) 80 120 160 227 252 277 302 327 349 392 442 497 549	Max. Moment (FF KIP) 100 225 500 825 1224 1642 2028 2623 3202 3804 5196 6830 8638 10678	( <i>KUP</i> ) 64 70 90 99 105 119 131 147 159 169 184 205 227 245	Мах. Moment (FF KU) 80 160 240 382 538 693 875 1115 1362 1673 2295 2916 3701 4565	"E" RA SHEAR E 64 E 45 E 40 E 37 E 38 E 38 E 39 E 39 E 39 E 39 E 39 E 39 E 39 E 39 E 37 E 37 E 37 E 37 E 37 E 37 E 37 E 39 E 39 E 39 E 39 E 39 E 39 E 37 E 37 E 37 E 37 E 37 E 37 E 38 E 39 E 39 E 39 E 39 E 39 E 39 E 37 E 37 E 37 E 38 E 39 E 39 E 39 E 39 E 39 E 37 E 37 E 37 E 38 E 39 E 39 E 39 E 37 E 37 E 38 E 39 E 39 E 39 E 37 E 37 E 38 E 39 E 39 E 37 E 37 E 37 E 38 E 39 E 39 E 37 E 37 E 38 E 39 E 39 E 37 E 37 E 37 E 37 E 37 E 38 E 39 E 39 E 37 E 37 E 37 E 37 E 37 E 37 E 37 E 37 E 39 E 37 E 36 E 37 E	MOM/MV E 64 E 57 E 38 E 37 E 35 E 34 E 34 E 34 E 34 E 34 E 35 E 35 E 34 E 34 E 34 E 34 E 34 E 34 E 34	(XIP) 57 61 79 88 93 97 107 116 129 139 154 165 173 179	Max. Moment (FT K#) 72 143 215 334 473 612 612 751 891 1074 1288 1819 2376 2932 3489	"E" RA SHEAR E 57 E 41 E 39 E 35 E 33 E 31 E 31 E 31 E 32 E 32 E 31 E 32 E 31 E 32 E 32 E 31 E 32 E 32 E 31 E 32 E 33 E 3	MOMENT E 57 E 51 E 34 E 32 E 31 E 30 E 29 E 27 E 27 E 27 E 27 E 27 E 27 E 27 E 27	(KD) 57 61 79 88 93 104 114 128 139 148 166 187 206 222	Мах. Moment (FF KIP) 72 143 215 334 473 612 761 976 1190 1457 2014 2667 3402 4260	"E" RA SHEAR E 57 E 41 E 39 E 35 E 33 E 33 E 33 E 33 E 34 E 34 E 34 E 34	MOMENT E 57 E 51 E 34 E 32 E 31 E 30 E 29 E 30 E 30 E 30 E 31 E 31 E 31 E 31 E 32 E 32 E 32
(Feet) 5 10 15 20 25 30 35 40 45 50 60 70 80	Max. Shear (NP) 80 120 160 200 227 252 277 302 327 302 327 349 392 442 497	Max. Moment (FF κtr) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830 8638	(KUP) 64 70 90 99 105 119 131 147 159 169 169 184 205 227	Мах. Moment (FF кв) 80 160 240 382 538 693 875 1115 1362 1673 2295 2916 3701	"E" RA SHEAR E 64 E 47 E 45 E 40 E 37 E 38 E 38 E 39 E 39 E 39 E 39 E 39 E 39 E 39 E 33 E 37 E 37	MOM/MVT E 64 E 57 E 38 E 37 E 35 E 34 E 34 E 34 E 34 E 34 E 35 E 35 E 35 E 34 E 34 E 34 E 34	(KIP) 57 61 79 88 93 97 107 116 129 139 154 165 173	Max. Moment (FT K#) 72 143 215 334 473 612 751 891 1074 1288 1819 2376 2932	"E" RA SHEAR E 57 E 41 E 35 E 33 E 31 E 31 E 31 E 32 E 32 E 30 E 28	момент E 57 E 51 E 34 E 32 E 31 E 30 E 29 E 27 E 27 E 27 E 27 E 28 E 28 E 28 E 27	(KD) 57 61 79 88 93 104 114 128 139 148 139 148 166 187 206	Мах. Moment (FF KIP) 72 143 215 334 473 612 761 976 1190 1457 2014 2667 3402	"E" RA SHEAR E 57 E 41 E 35 E 33 E 33 E 33 E 33 E 34 E 34 E 34 E 34	MOMENT E 57 E 51 E 34 E 32 E 31 E 30 E 30 E 30 E 31 E 31 E 31 E 31 E 32
(Feet) 5 10 15 20 25 30 35 40 45 50 60 70 80 90	Max. Shear (NP) 80 120 160 200 227 252 277 302 327 349 392 442 497 549 600	Max. Moment (FF KH) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830 8638 10678 12893	( <i>KUP</i> ) 64 70 90 99 105 119 131 147 159 169 184 205 227 245	Мах. Moment (FF ки) 80 160 240 382 538 693 875 1115 1362 1673 2295 2916 3701 4565 5525	"E" RA SHEAR E 64 E 47 E 45 E 40 E 37 E 38 E 39 E 39 E 39 E 39 E 39 E 39 E 39 E 39	MOM/MV E 64 E 57 E 38 E 37 E 35 E 34 E 34 E 34 E 34 E 34 E 35 E 35 E 34 E 34 E 34 E 34 E 34 E 34 E 34	(XIP) 57 61 79 88 93 97 107 116 129 139 154 165 173 179	Max. Moment (FT KP) 72 73 73 72 73 73 75 75 75 75 891 1074 1288 1819 2376 2932 3489 4046	"E" RA SHEAR E 57 E 41 E 39 E 35 E 33 E 31 E 31 E 31 E 31 E 32 E 32 E 31 E 30 E 28 E 26 E 25	MOMENT E 57 E 51 E 34 E 32 E 31 E 30 E 29 E 27 E 27 E 27 E 27 E 27 E 27 E 27 E 27	(KDP) 57 61 79 88 93 104 114 128 139 148 166 187 206 222	Max. Moment (FF KP) 72 143 215 334 473 612 761 976 1190 1457 2014 2667 3402 4260 5217	"E" RA SHEAR E 57 E 41 E 39 E 35 E 33 E 34 E 34 E 34 E 34 E 34 E 34 E 34	MOMENT E 57 E 51 E 34 E 32 E 31 E 30 E 29 E 30 E 30 E 30 E 31 E 31 E 31 E 31 E 32 E 32 E 32
(Feet) 5 10 15 20 25 30 35 40 45 50 60 70 80 90	Max. Shear (NP) 80 120 160 200 227 252 277 302 327 302 327 349 392 442 497 549 600	Max. Moment (FF KIP) 100 225 500 825 1224 1642 2028 2623 3202 3804 5196 6830 8638 10678	( <i>KUP</i> ) 64 70 90 99 105 119 131 147 159 169 184 205 227 245 259	Мах. Moment (Ff ки) 80 160 240 382 538 693 875 1115 1362 1673 2295 2916 3701 4565 5525 U23-B Single	"E" RA SHEAR E 64 E 47 E 45 E 40 E 37 E 38 E 39 E 39 E 39 E 39 E 39 E 39 E 39 E 39	MOMENT E 64 E 57 E 38 E 37 E 35 E 34 E 34 E 34 E 34 E 34 E 34 E 34 E 34	(KUP) 57 61 79 88 93 97 107 116 129 139 154 165 173 179 184	Max. Moment (FT K#) 72 143 215 334 473 612 612 751 891 1074 1288 1819 2376 2932 3489	"E" RA SHEAR E 57 E 41 E 39 E 35 E 33 E 31 E 31 E 31 E 31 E 32 E 32 E 31 E 30 E 28 E 26 E 25	момент E 57 E 51 E 34 E 32 E 31 E 30 E 29 E 27 E 27 E 27 E 27 E 28 E 28 E 28 E 27 E 26 E 25	(KP) 57 61 79 88 93 104 114 128 139 148 166 187 206 222 240	Мах. Moment (FF KIP) 72 143 215 334 473 612 761 976 1190 1457 2014 2667 3402 4260	"E" R/ SHEAR E 57 E 41 E 39 E 35 E 33 E 34 E 34 E 34 E 34 E 34 E 34 E 34 E 32 E 32 E 32 E 32 E 32 E 32	MOMENT E 57 E 51 E 34 E 32 E 31 E 30 E 29 E 30 E 30 E 30 E 31 E 31 E 31 E 31 E 32 E 32 E 32
(Feet) 5 10 25 30 40 45 50 60 70 80 90 100	Max. Shear (NP) 80 120 160 200 227 252 277 302 327 302 327 349 392 442 497 549 600	Max. Moment (FF KH) 100 225 500 825 1224 1642 2088 2623 3202 3202 3804 5196 6830 8638 10678 12893 PER E80	( <i>KUP</i> ) 64 70 90 99 105 119 131 147 159 169 184 205 227 245 259	Мах. Moment (Ff ки) 80 160 240 382 538 693 875 1115 1362 1673 2295 2916 3701 4565 5525 U23-B Single	"E" R/A SHEAR E 64 E 45 E 40 E 37 E 38 E 39 E 39 E 39 E 39 E 39 E 39 E 39 E 39	MOMENT E 64 E 57 E 38 E 37 E 35 E 34 E 34 E 34 E 34 E 34 E 34 E 34 E 34	(KUP) 57 61 79 88 93 97 107 116 129 139 154 165 173 179 184	Max. Moment (FT KP) 72 73 143 215 334 473 612 751 891 1074 1288 1819 2376 2932 3489 2493 4046 U23-B in	"E" RA SHEAR E 57 E 41 E 39 E 35 E 33 E 31 E 31 E 31 E 31 E 32 E 33 E 31 E 32 E 33 E 31 E 32 E 32 E 33 E 31 E 30 E 28 E 26 E 25 E 25	момент E 57 E 51 E 34 E 32 E 31 E 30 E 29 E 27 E 27 E 27 E 27 E 28 E 28 E 28 E 27 E 26 E 25	(KP) 57 61 79 88 93 104 114 128 139 148 166 187 206 222 240	Мах. Moment (FF кк) 72 143 215 334 473 612 761 976 1190 1457 2014 2667 3402 4260 5217 B40-8 Single	"E" R/ SHEAR E 57 E 41 E 39 E 35 E 33 E 34 E 34 E 34 E 34 E 34 E 34 E 34 E 32 E 32 E 32 E 32 E 32 E 32	MOMENT E 57 E 51 E 34 E 32 E 31 E 30 E 29 E 30 E 30 E 30 E 31 E 31 E 31 E 32 E 32 E 32 E 32 E 32
(Feet) 5 10 25 30 40 45 50 60 70 80 90 100 span	Max. Shear (NP) 80 120 160 200 227 252 277 302 327 349 392 442 497 549 600 <b>COO</b> Max. Shear	Max. Moment (FF KB) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830 8638 10678 12893 PER E80 Max. Moment	( <i>KUP</i> ) 64 70 90 99 105 119 131 147 159 169 184 205 227 245 259 Max. Shear	Мах. Moment (FF кв) 80 160 240 382 538 693 875 1115 1362 1673 2295 22916 3701 4565 5525 U23-B Single	"E" R/4 SHEAR E 64 E 45 E 40 E 37 E 38 E 39 E 37 E 37 E 37 E 38 E 39 E 39 E 39 E 39 E 39 E 39 E 39 E 39 E 37 E 37 E 38 E 39 E 39 E 39 E 37 E 37 E 38 E 37 E 37 E 38 E 39 E 39 E 39 E 37 E 37 E 36 E 37 E 37 E 36 E 37 E 37 E 36 E 35 E 37 E 36 E 37 E 37 E 36 E 37 E 37 E 36 E 37 E 37 E 36 E 37 E 37 E 36 E 35 E 37 E 38 E 37 E 38 E 37 E 38 E 37 E 36 E 37 E 36 E 35 E 35 E 35 E 8 E 8 E 8 E 8 E 8 E 8 E 8 E 8	MOMENT E 64 E 57 E 38 E 37 E 35 E 34 E 34 E 34 E 34 E 34 E 34 E 34 E 34	(XUP) 57 61 79 88 93 97 107 116 129 139 154 165 173 179 184	Max. Moment (FT KP) 72 143 215 334 473 612 751 891 1074 1288 1819 2376 2932 3489 4046 U23-B in Max. Moment	"E" RA SHEAR E 57 E 41 E 39 E 35 E 33 E 31 E 31 E 31 E 32 E 35 E 33 E 31 E 32 E 32 E 32 E 35 E 33 E 31 E 32 E 33 E 3	момент E 57 E 51 E 34 E 32 E 31 E 30 E 29 E 27 E 27 E 27 E 27 E 28 E 28 E 27 E 26 E 25	(KP) 57 61 79 88 93 104 114 128 139 148 166 187 206 222 240 Max. Shear	Мах. Moment (FF KIP) 72 143 215 334 473 612 761 976 1190 1457 2014 2667 3402 4260 5217 B40-8 Single Max. Moment	"E" R/ SHEAR E 57 E 41 E 39 E 35 E 33 E 33 E 34 E 34 E 34 E 34 E 34 E 34 E 34 E 33 E 32 E 32 E 32 E 82 E 82 E 82 E 82 E 82 E 85 E 8	MOMENT E 57 E 51 E 34 E 32 E 31 E 30 E 30 E 30 E 30 E 31 E 31 E 31 E 31 E 31 E 32 E 32 E 32 E 32 E 32 E 32
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(Feet) 5 10 20 25 30 25 30 40 45 50 60 70 80 90 100 \$pan (Feet) 5 10 15 20	Max. Shear (NP) 80 120 160 227 227 252 277 302 327 302 327 349 392 442 497 549 600 Max. Shear (NP) 80 120 160 200	Max. Moment (FF KB) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830 8638 10678 12893 PER E80 Max. Moment (FF KB) 100 225 500 825	(KUP) 64 70 90 99 105 119 131 131 147 159 169 184 205 227 245 259 Max. Shear (KUP) 60 65 83 93	Мах. Moment (FF KH) 80 160 240 382 538 693 875 1115 1362 1673 2295 2295 2295 2916 3701 4565 5525 U23-B Single Max. Moment (FF KH) 76 151 227 353	"E" R/A SHEAR E 64 E 47 E 45 E 40 E 37 E 38 E 39 E 37 E 37 E 36 E 35 E 36 E 35 E 35 E 36 E 35 E 35 E 36 E 40 E 37 E 36 E 35 E 35 E 60 E 42 E 60 E 42 E 37 E 37 E 46 E 46 E 47 E 46 E 46 E 47 E 46 E 47 E 46 E 47 E 46 E 42 E 42 E 37 E 37 E 37 E 42 E 42 E 37 E 37 E 37 E 37 E 42 E 42 E 37 E 37 E 37 E 37 E 42 E 42 E 37 E 37 E 37 E 42 E 42 E 37 E 37 E 37 E 37 E 42 E 42 E 37 E 37	молиемт E 64 E 57 E 38 E 37 E 35 E 34 E 34 E 34 E 34 E 34 E 34 E 34 E 34	(КиР) 57 61 79 88 93 97 107 116 129 139 154 165 173 179 184 Мах. Shear (КИР) 60 67 85 99	Max. Moment (FT K#) 72 143 215 334 473 612 751 891 1074 1288 1074 1288 1074 2376 2336 2932 3489 4046 U23-B in Max. Moment (FT K#) 76 151 227 363	"E" RA SHEAR E 57 E 41 E 39 E 35 E 33 E 31 E 31 E 31 E 32 E 4 E 4 E 4 E 4 E 4 E 4 E 4 E 4	момент E 57 E 51 E 34 E 32 E 31 E 30 E 29 E 27 E 26 E 25 MOMENT E 61 E 54 E 36 E 35 E 35	(KDP) 57 61 79 88 93 104 114 128 139 148 166 187 206 222 240 222 240 <b>Max. Shear</b> (KDP) 68 75 95 105	Max. Moment           (FT KIP)           72           143           215           334           473           612           761           976           1190           1457           2014           2667           3402           4260           5217           B40-8 Single           Max. Moment           (FF KIP)           85           170           255           406	"E" R/ SHEAR E 57 E 41 E 39 E 35 E 33 E 33 E 33 E 33 E 34 E 34 E 34 E 34 E 34 E 34 E 34 E 34 E 33 E 32 E 32 Engine "E" R/ SHEAR E 68 E 68 E 68 E 42	MOMENT E 57 E 51 E 34 E 32 E 31 E 30 E 30 E 30 E 30 E 30 E 31 E 31 E 31 E 31 E 31 E 32 E 32 E 32 E 32 E 32 E 32 E 32 E 32
(Feet) 5 10 20 25 30 25 30 40 45 50 60 70 80 90 100 \$ \$ 100 \$ \$ 100 \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$	Max. Shear (NP) 80 120 160 227 252 277 302 327 349 392 442 497 549 600 Max. Shear (NP) 80 120 160 200 227	Max. Moment (FF KH) 100 225 500 825 1224 1642 2088 2623 3202 3202 3202 3804 5196 6830 8638 10678 10678 10678 12893 PER E80 Max. Moment (FF KH) 100 225 500 825 1224	(KUP) 64 70 90 99 105 119 131 147 159 169 184 205 227 245 259 Max. Shear (KUP) 60 65 83 93 98	Max. Moment           (FF кй)           80           160           240           382           538           693           875           1115           1362           1673           2295           2916           3701           4565           5525           U23-8 Single           Max. Moment           (FF кй)           76           151           227           353           500	"E" R/ SHEAR E 64 E 45 E 40 E 37 E 38 E 39 E 37 E 37 E 35 E 60 E 43 E 42 E 42 E 42 E 7 E 35 E 35	молиемт E 64 E 57 E 38 E 37 E 35 E 34 E 35 E 35 E 34 E 35 E 35 E 34 E 35 E 34 E 35 E 35 E 34 E 34 E 35 E 35 E 34 E 34 E 34 E 35 E 35 E 34 E 33 E 35 E 34 E 33 E 34 E 33 E 34 E 34 E 33 E 34 E 34 E 33 E 34 E 34 E 33 E 34 E 34 E 34 E 34 E 33 E 34 E 34 E 33 E 34 E 33 E 34 E 33 E 34 E 33 E 34 E 33 E 35 E 35	( <i>кир</i> ) 57 61 79 88 93 97 107 116 129 139 154 165 173 179 184 Мах. Shear ( <i>кир</i> ) 60 67 85 99 9116	Max. Moment (FF KP) 72 143 215 334 473 612 751 891 1074 1288 1819 2376 2932 3489 4046 U23-B in Max. Moment (FF KP) 76 151 227 363 582	"E" RA SHEAR E 57 E 41 E 39 E 35 E 33 E 31 E 31 E 31 E 31 E 31 E 31 E 32 E 32 E 33 E 28 E 26 E 25 Ime "E" RA SHEAR E 60 E 45 E 45 E 40 E 41	момент E 57 E 51 E 34 E 32 E 31 E 30 E 29 E 27 E 27 E 27 E 27 E 28 E 27 E 27 E 28 E 27 E 27 E 26 E 25 MOMENT E 61 E 54 E 53 E 35 E 38	(KD) 57 61 79 88 93 104 114 128 139 148 166 187 206 222 240 X206 222 240 Max. Shear (KD) 68 75 95 105 112	Max. Moment           (FF KP)           72           143           215           334           473           612           761           976           1190           1457           2014           2667           3402           4260           5217           B40-8 Single           Max. Moment           (FF KP)           85           170           255           406           571	"E" R/ SHEAR E 57 E 31 E 39 E 35 E 33 E 34 E 32 Engine "E" R/ SHEAR E 68 E 50 E 42 E 39	MOMENT E 57 E 51 E 34 E 32 E 31 E 30 E 30 E 30 E 30 E 31 E 31 E 31 E 31 E 31 E 32 E 32 E 32 E 32 E 32 E 32 E 32 E 32
(Feet) 5 10 25 30 40 45 50 60 70 80 90 100 5 100 15 20 25 30 100 15 20 25 30 15 20 25 30 15 20 25 30 25 30 25 30 25 30 25 30 20 25 30 25 30 20 25 30 25 30 25 30 25 30 20 25 30 25 30 20 25 30 20 25 30 20 25 30 20 25 30 20 25 30 20 25 30 20 25 30 20 25 30 20 25 30 20 25 35 20 20 25 20 25 20 20 25 20 20 25 20 20 20 25 20 20 20 20 20 20 20 20 20 20	Max. Shear (NP) 80 120 200 227 252 277 302 327 349 392 442 497 549 600 Max. Shear (NP) 80 120 160 200 227 252	Max. Moment (FF KH) 100 225 500 825 1224 1642 2088 2623 3202 3202 3804 5196 6830 8638 10678 12893 PER E80 Max. Moment (FF KH) 100 225 500 825 1224 1642	( <i>KVP</i> ) 64 70 99 105 119 131 147 159 169 184 205 227 245 259 Max. Shear ( <i>KVP</i> ) 60 65 83 93 98 102	Мах. Moment (FF ки) 80 160 240 382 538 693 875 1115 1362 1673 2295 2916 3701 4565 5525 U23-B Single Max. Moment (FF ки) 76 151 227 353 500 647	"E" R/ SHEAR E 64 E 45 E 40 E 37 E 38 E 39 E 39 E 39 E 39 E 39 E 39 E 39 E 39 E 37 E 36 E 35 E 35 E 42 E 42 E 45 E 45 E 40 E 45 E 40 E 37 E 38 E 39 E 38 E 37 E 36 E 85 E 86 E 86 E 86 E 87 E 87	MOMENT E 64 E 57 E 38 E 37 E 35 E 34 E 34 E 34 E 34 E 34 E 34 E 34 E 34	(XUP) 57 61 79 88 93 97 107 116 129 139 154 165 173 179 184 Max. Shear (XUP) 60 60 67 85 99 116 131	Max. Moment (FT KP) 72 73 73 73 73 73 73 75 75 75 75 75 75 75 75 75 75	"E" RA SHEAR E 57 E 39 E 35 E 31 E 31 E 31 E 31 E 32 E 32 E 31 E 30 E 28 E 26 E 25 Ime "E" RA SHEAR E 43 SHEAR E 43 E 43	момент E 57 E 51 E 34 E 32 E 31 E 30 E 29 E 27 E 27 E 27 E 27 E 27 E 28 E 28 E 27 E 26 E 25 MOMENT E 61 E 54 E 36 E 35 E 38 E 39	(KDP) 57 61 79 88 93 104 114 128 139 148 166 187 206 222 240 240 Max. Shear (KDP) 68 75 95 105 112 116	Мах. Moment (FF KIP) 72 143 215 334 473 612 761 976 1190 1457 2014 2667 3402 4260 5217 840-8 Single Max. Moment (FF KIP) 85 170 255 406 571 737	"E" R/ SHEAR E 57 E 41 E 39 E 35 E 33 E 34 E 34 E 34 E 34 E 34 E 34 E 34 E 34 E 32 E 33 E 34 E 32 E 33 E 32 E 33 E 33	MOMENT E 57 E 51 E 34 E 32 E 31 E 30 E 30 E 30 E 30 E 30 E 31 E 31 E 31 E 31 E 32 E 32 E 32 E 32 E 32 E 32 E 32 E 32
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(Feet) 5 10 20 25 30 40 45 50 60 70 70 80 90 100 \$pan (Feet) 5 10 5 20 25 30 35 40 40 45 5 10 40 45 5 10 10 10 10 10 10 10 10 10 10	Max. Shear (NP) 80 120 200 227 252 277 302 327 349 392 442 497 549 600 Kax. Shear (NP) 80 120 160 200 227 252 277 302	Max. Moment (FF KH) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830 8638 10678 10678 10678 12893 202 8638 10678 10678 10678 10678 10678 10678 1075 100 225 500 825 1224 1642 2088 2623	( <i>KUP</i> ) 64 70 90 99 105 119 131 147 159 169 184 205 227 245 259 Max. Shear ( <i>KIP</i> ) 60 65 83 93 98 102 105 107	Max. Moment           (Ff KB)           80           160           240           382           538           693           875           1115           1362           1673           2295           2916           3701           4565           5525           U23-8 Single           Max. Moment           (FF KB)           227           353           500           647           794           941	"E" R/ SHEAR E 64 E 47 E 45 E 40 E 37 E 38 E 39 E 37 E 36 E 35 E 42 E 42 E 37 E 35 E 35	молиемт E 64 E 57 E 38 E 37 E 35 E 34 E 36 E 30 E	(КиР) 57 61 79 88 93 97 107 116 129 139 154 165 173 179 184 Мах. Shear (КиР) 60 67 85 99 116 131 147 159	Max. Moment (FT K#) 72 143 215 334 473 612 751 891 1074 1288 1819 2376 2932 3489 4046 2932 3489 4046 V23-B in Max. Moment (FT K#) 76 151 227 363 582 809 1044 1339	"E" RA SHEAR E 57 E 41 E 39 E 35 E 33 E 31 E 31 E 31 E 32 E 31 E 32 E 31 E 32 E 31 E 32 E 31 E 30 E 28 E 26 E 25 SHEAR E 40 E 43 E 40 E 41 E 42 E 42 E 42 E 42 E 42 E 42	MOMENT E 57 E 51 E 34 E 32 E 31 E 30 E 29 E 27 E 27 E 27 E 27 E 27 E 28 E 27 E 28 E 27 E 28 E 27 E 26 E 25 CTING MOMENT E 61 E 54 E 35 E 38 E 35 E 38 E 35 E 40 E 41	(KP) 57 61 79 88 93 104 114 128 139 148 166 187 206 222 240 Max. Shear (KP) 68 75 95 105 112 116 119 122	Max. Moment           (FT KIP)           72           143           215           334           473           612           761           976           1190           1457           2014           2667           3402           4260           5217           B40-8 Single           Max. Moment           (FF KIP)           85           170           255           406           571           737           902           1067	"E" R/ SHEAR E 57 E 41 E 39 E 35 E 33 E 33 E 34 E 34 E 34 E 34 E 34 E 34 E 34 E 34 E 33 E 32 Engine "E" R/ SHEAR E 68 E 59 E 48 E 42 E 39 E 37 E 32 E 32 E 32 E 32 E 32 E 32 E 32 E 32 E 32 E 33 E 34 E 32 E 32 E 87 E 87	момент E 57 E 51 E 34 E 32 E 31 E 30 E 30 E 30 E 30 E 30 E 31 E 31 E 31 E 31 E 31 E 32 E 31 E 30 E 31 E 31 E 31 E 31 E 31 E 32 E 33 E 35 E 35
(Feet) 5 10 25 30 25 30 40 45 50 60 70 80 90 100 5 10 5 10 25 30 35 50 60 70 0 80 90 100 15 20 25 30 35 50 60 70 25 30 35 50 60 70 80 90 100 100 100 100 100 100 100	Max. Shear (NP) 80 120 160 200 227 252 277 302 327 349 392 442 497 549 600 Max. Shear (NP) 80 120 160 200 227 252 277 302 327	Max. Moment (FF KH) 100 225 500 825 1224 1642 2088 2623 3202 3202 3204 5196 6830 8638 10678 10678 10678 12893 PER E80 Max. Moment (FF KH) 100 225 500 825 1224 1642 2088 2623 3202	( <i>KDP</i> ) 64 70 99 105 119 131 131 147 159 169 184 205 227 245 259 Max. Shear ( <i>KDP</i> ) 60 65 83 93 98 102 105 107 113	Max. Moment           (FF кй)           80           160           240           382           538           693           875           1115           1362           1673           2295           2916           3701           4565           5525           U23-8 Single           Max. Moment           (FF кй)           76           151           227           353           500           647           941           1088	"E" R/ SHEAR E 64 E 45 E 40 E 37 E 38 E 39 E 42 E 37 E 35 E 32 E 35 E 32 E 3	ACAMENT E 64 E 57 E 38 E 37 E 35 E 34 E 34 E 34 E 34 E 34 E 34 E 34 E 34	(КиР) 57 61 79 88 93 97 107 116 129 139 154 165 173 179 184 Мах. Shear (КИР) 60 67 85 99 116 131 147 159 168	Max. Moment (FF KP) 72 143 215 334 473 612 751 891 1074 1288 1819 2376 2932 3489 4046 U23-8 in Max. Moment (FF KP) 76 151 227 363 582 809 1044 1339 1633	"E" RA SHEAR E 57 E 39 E 35 E 33 E 31 E 31 E 31 E 31 E 31 E 31 E 32 E 32 E 32 E 33 E 28 E 28 E 28 E 28 E 25 SHEAR E 60 E 45 E 45 E 41 E 42 E 41	момент E 57 E 51 E 34 E 32 E 31 E 30 E 29 E 27 E 28 E 29 E 29 E 30 E 29 E 27 E 27 E 27 E 27 E 28 E 27 E 27 E 27 E 27 E 27 E 28 E 29 E 29 E 29 E 30 E 29 E 30 E 29 E 29 E 27 E 27 E 27 E 28 E 29 E 29 E 20 E 29 E 29 E 20 E 20 MOMENT E 61 E 35 E 38 E 39 E 41 E 41	(KDP) 57 61 79 88 93 104 114 128 139 148 166 187 206 222 240 <b>Max. Shear</b> (KDP) 68 75 95 105 112 116 119 1122 131	Max. Moment           (FF KP)           72           143           215           334           473           612           761           976           1190           1457           2014           2667           3402           4260           5217           B40-8 Single           Max. Moment           (FF KP)           85           170           255           406           571           737           902           1067           1232	"E" R/ SHEAR E 57 E 39 E 35 E 33 E 33 E 34 E 32 Engine "E" R/ SHEAR E 68 E 50 E 48 E 50 E 48 E 50 E 48 E 57 E 32 E 33 E 34 E 32 E 32 E 32 E 32 E 33 E 34 E 34 E 34 E 34 E 34 E 34 E 34 E 32 E 34 E 34 E 34 E 32 E 37 E 34 E 32 E 32	MOMENT E 57 E 51 E 34 E 32 E 31 E 30 E 30 E 30 E 31 E 31 E 31 E 31 E 31 E 32 E 32 E 32 E 32 E 32 E 32 E 32 E 32
(Feet) 5 10 25 30 40 45 50 40 45 50 10 5 10 15 20 25 30 35 40 45 5 10 15 20 25 30 35 40 45 5 10 45 5 5 5 5 5 5 5 5 5 5 5 5 5	Max. Shear (NP) 80 120 200 227 252 277 302 327 349 392 442 497 549 600 Max. Shear (NP) 80 120 160 200 227 252 277 302 327 349	Max. Moment (FF ки) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830 8638 10678 12893 Max. Moment (FF ки) 12893 Max. Moment (FF ки) 225 500 825 1224 1642 2088 2623 3202 320	( <i>KVP</i> ) 64 70 99 105 119 131 147 159 169 184 205 227 245 259 Max. Shear ( <i>KVP</i> ) 60 65 83 93 93 98 102 107 113 119	Мах. Moment (FF ки) 80 160 240 382 538 693 875 1115 1362 1673 2295 2916 3701 4565 525 U23-B Single Мах. Moment (FF ки) 76 151 227 353 500 647 794 941 1088 1235	"E" R/ SHEAR E 64 E 45 E 40 E 37 E 38 E 39 E 39 E 39 E 39 E 39 E 39 E 39 E 37 E 36 E 35 E 35 E 60 E 43 E 42 E 43 E 42 E 35 E 32 E 30 E 28 E 27	MOMENT E 64 E 57 E 38 E 37 E 35 E 34 E 34 E 34 E 34 E 34 E 34 E 34 E 34	(КиР) 57 61 79 88 93 97 107 116 129 139 154 165 173 179 184 Мах. Shear (КИР) 60 67 85 99 116 131 147 159 168 175	Max. Moment (FT KP) 72 73 72 73 73 73 73 75 75 75 75 75 75 75 75 75 75	"E" RA SHEAR E 57 E 39 E 35 E 31 E 31 E 31 E 31 E 31 E 32 E 32 E 33 E 31 E 32 E 32 E 32 E 32 E 32 E 32 E 32 E 33 E 31 E 30 E 28 E 28 E 26 E 25 E 35 E 31 E 30 E 28 E 26 E 25 E 31 E 31 E 30 E 28 E 26 E 26 E 25 E 31 E 31 E 30 E 28 E 26 E 2	MOMENT E 57 E 51 E 34 E 32 E 31 E 30 E 29 E 27 E 27 E 27 E 27 E 27 E 27 E 27 E 27	(KDP) 57 61 79 88 93 104 114 128 139 148 166 187 206 222 240 240 Max. Shear (KDP) 68 75 95 105 112 116 119 122 131 141	Max. Moment           (FF KP)           72           143           215           334           473           612           761           976           1190           1457           2014           2667           3402           4260           5217           B40-8 Single           Max. Moment           (FF KP)           85           170           255           406           571           737           902           1067           1232           1398	"E" R/ SHEAR E 57 E 41 E 39 E 35 E 33 E 33 E 34 E 34 E 34 E 34 E 34 E 34 E 34 E 34 E 32 Engine "E" R/ SHEAR E 68 E 50 E 48 E 50 E 48 E 43 E 37 E 37 E 34 E 32 E 33 E 3	MOMENT E 57 E 51 E 34 E 32 E 31 E 30 E 30 E 30 E 30 E 30 E 31 E 31 E 31 E 31 E 32 E 32 E 32 E 32 E 32 E 32 E 32 E 32
(Feet) 5 10 25 30 25 30 40 45 50 60 70 80 90 100 100 100 15 20 5 100 155 20 25 30 35 40 45 50 60 60	Max. Shear (NP) 80 120 160 200 227 252 277 302 327 349 392 442 497 549 600 Max. Shear (NP) 80 120 160 200 227 252 227 252 227 252 227 302 327 327 327	Max. Moment (FF KH) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830 8638 10678 12893 0678 12893 0678 12893 078 500 825 1224 1642 2088 1642 2088 2623 3202 320	( <i>KUP</i> ) 64 70 90 99 105 119 131 147 159 169 184 227 245 259 Max. Shear ( <i>KUP</i> ) 60 65 83 93 98 102 105 107 113 119 139	Max. Moment           (Ff KH)           80           160           240           382           538           693           875           1115           1362           1673           2295           2916           3701           4565           5525           U23-B Single           Max. Moment           (Ff KH)           227           353           500           647           794           941           1088           1235           1533	"E" R/ SHEAR E 64 E 45 E 40 E 37 E 38 E 39 E 37 E 36 E 35 E 35 E 64 E 37 E 37 E 36 E 35 E 35 E 87 E 87	MOMENT E 64 E 57 E 38 E 37 E 35 E 34 E 34 E 34 E 34 E 34 E 34 E 34 E 34	(КиР) 57 61 79 88 93 97 107 116 129 139 154 165 173 179 184 Мах. Shear (КиР) 60 67 85 99 116 131 147 159 168 175 186	Max. Moment           (FT K#)           72           334           215           334           473           612           751           891           1074           2376           2336           1819           2376           932           3489           U23-8 in           Max. Moment           (FT K#)           751           227           363           582           809           1044           1339           1633           1927           2526	"E" RA SHEAR E 57 E 39 E 35 E 31 E 31 E 31 E 31 E 32 E 32 E 33 E 31 E 32 E 4 E 4 E 4 E 4 E 4 E 4 E 4 E 4	момент E 57 E 51 E 34 E 32 E 31 E 30 E 29 E 27 E 27 E 27 E 27 E 27 E 27 E 27 E 27 E 28 E 27 E 26 E 25 MOMENT E 64 E 36 E 35 E 38 E 39 E 40 E 41 E 41 E 41 E 39	(K\$P) 57 61 79 88 93 104 114 128 139 148 166 187 206 222 240 Max. Shear (K\$P) 68 75 95 105 112 116 119 122 116 119 122 131 141 162	Max. Moment           (FF KF)           72           143           215           334           473           612           761           976           1190           1457           2014           2667           3402           4260           5217           B40-8 Single           Max. Moment           (FF KF)           406           571           737           902           1067           1232           1398           1759	"E" R/ SHEAR E 57 E 41 E 39 E 35 E 33 E 34 E 34 E 34 E 34 E 34 E 34 E 34 E 34 E 32 E 32 Engine "E" R/ SHEAR E 68 E 68 E 68 E 69 E 48 E 42 E 37 E 34 E 32 E 37 E 34 E 32 E 32 E 32 E 32 E 33 E 32 E 32 E 33 E 32 E 32 E 32 E 33 E 32 E 32 E 33 E 34 E 32 E 33 E 33	MOMENT E 57 E 51 E 34 E 32 E 31 E 30 E 30 E 30 E 30 E 30 E 31 E 31 E 32 E 32 E 32 E 32 E 32 E 32 E 32 E 32
(Feet) 5 10 25 30 25 30 40 45 50 60 90 100 5 20 25 30 90 100 5 20 25 30 90 100 5 20 25 30 35 35 40 40 45 50 60 70 70 70 70 70 70 70 70 70 7	Max. Shear (NP) 80 120 160 200 227 252 277 302 327 349 392 442 497 549 600 <b>Max. Shear</b> (NP) 80 120 160 120 160 227 227 252 277 302 327 349 392 442	Max. Moment (FF KH) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830 8638 10678 12893 10678 12893 PER E80 Max. Moment (FF KH) 100 225 500 825 1224 1642 2088 2623 3202 3202 3202 3204 1645 1295	( <i>KUP</i> ) 64 70 90 99 105 119 131 147 159 169 184 227 245 259 Max. Shear ( <i>KUP</i> ) 60 65 83 93 98 102 105 107 113 119 139 153	Max. Moment           (Ff KB)           80           160           240           382           538           693           875           1115           1362           295           2916           3701           4565           5525           U23-B Single           Max. Moment           (FF KB)           76           151           227           353           500           647           794           941           1088           1235           1533           1937	"E" R/ SHEAR E 64 E 45 E 40 E 37 E 38 E 39 E 37 E 36 E 35 E 35	молиемт E 64 E 57 E 38 E 37 E 35 E 34 E 36 E 34 E 36 E 32 E 32 E 30 E 29 E 27 E 26 E 24 E 24 E 30 E 29 E 27 E 24 E 24 E 24 E 24 E 30 E 29 E 27 E 24 E 22 E 24 E 23 E 24 E 24 E 23 E 24 E 24 E 23 E 24 E	(КиР) 57 61 79 88 93 97 107 116 129 139 154 165 173 179 184 Мах. Shear (КиР) 60 67 85 99 116 131 147 159 168 175 186 203	Max. Moment (FT KP) 72 133 215 334 473 612 751 891 1074 1288 189 2376 2932 3489 2376 2932 3489 2376 2932 3489 2376 2932 3489 2376 2932 3489 2376 2932 3489 2376 2932 3489 2376 2932 3489 2376 2932 3489 2376 2932 3489 2037 363 582 809 1044 1339 1633 1927 2526 3131	"E" RA SHEAR E 57 E 41 E 39 E 35 E 33 E 31 E 31 E 31 E 32 E 31 E 32 E 31 E 32 E 31 E 32 E 31 E 32 E 31 E 32 E 33 E 40 E 45 E 43 E 40 E 41 E 42 E 42 E 42 E 42 E 41 E 42 E 38 E 37	момент E 57 E 51 E 34 E 32 E 31 E 30 E 29 E 27 E 27 E 27 E 27 E 27 E 27 E 28 E 28 E 27 E 28 E 28 E 27 E 26 E 25 MOMENT E 61 E 54 E 35 E 38 E 39 E 40 E 41 E 41 E 41 E 39 E 37	(KP) 57 61 79 88 93 104 114 128 139 148 166 187 206 222 240 Max. Shear (KP) 68 75 95 105 112 116 119 122 131 162 178	Max. Moment           (FT KIP)           72           143           215           334           473           612           761           976           1190           1457           2014           2667           3402           4260           5217           B40-8 Single           Max. Moment           (FF KIP)           85           170           255           406           571           733           902           1067           1232           1398           1759           2297	"E" R/ SHEAR E 57 E 41 E 39 E 35 E 33 E 33 E 34 E 34 E 34 E 34 E 34 E 34 E 34 E 34 E 34 E 32 E 32 E 32 E 82 E 83 E 82 E 83 E 82 E 83 E 82 E 83 E 8	MOMENY E 57 E 51 E 34 E 32 E 31 E 30 E 30 E 30 E 30 E 30 E 31 E 31 E 31 E 31 E 31 E 31 E 31 E 32 E 32 E 32 E 32 E 32 E 32 E 32 E 32
(Feet) 5 10 25 30 25 30 40 45 50 60 70 80 90 100 100 100 15 20 5 100 155 20 25 30 35 40 45 50 60 60	Max. Shear (NP) 80 120 160 200 227 252 277 302 327 349 392 442 497 549 600 Max. Shear (NP) 80 120 160 200 227 252 227 252 227 252 227 302 327 327 327	Max. Moment (FF KH) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830 8638 10678 12893 0678 12893 0678 12893 078 500 825 1224 1642 2088 1642 2088 2623 3202 320	(KUP) 64 70 99 105 119 131 131 147 159 169 184 205 227 245 259 Max. Shear (KUP) 60 65 83 93 98 102 105 107 113 119 139 139 133 164	Max. Moment           (Ff KH)           80           160           240           382           538           693           875           1115           1362           1673           2295           2916           3701           4565           5525           U23-B Single           Max. Moment           (Ff KH)           227           353           500           647           794           941           1088           1235           1533	"E" R/ SHEAR E 64 E 45 E 40 E 37 E 38 E 39 E 39 E 39 E 39 E 39 E 39 E 39 E 37 E 36 E 37 E 36 E 35 E 60 E 43 E 42 E 42 E 42 E 42 E 42 E 42 E 42 E 45 E 40 E 37 E 37 E 36 E 37 E 36 E 37 E 35 E 60 E 43 E 42 E 4	молиемт E 64 E 57 E 38 E 37 E 35 E 34 E 34 E 34 E 34 E 34 E 34 E 34 E 34	(xuP) 57 61 79 88 93 97 107 116 129 139 154 165 173 179 184 Мах. Shear (xuP) 60 67 85 99 116 67 85 99 116 131 147 159 168 175 186 203 223	Max. Moment           (FT K#)           72           334           215           334           473           612           751           891           1074           2376           2336           1819           2376           932           3489           U23-8 in           Max. Moment           (FT K#)           751           227           363           582           809           1044           1339           1633           1927           2526	"E" RA SHEAR E 57 E 37 E 37 E 31 E 31 E 31 E 31 E 31 E 31 E 31 E 31 E 32 E 32 E 31 E 30 E 26 E 25 SHEAR E 40 E 41 E 42 E 42 E 41 E 40 E 37 E 36	момент E 57 E 51 E 34 E 32 E 31 E 30 E 29 E 27 E 26 E 25 MOMENT E 61 E 54 E 36 E 35 E 38 E 39 E 40 E 41 E 41 E 41 E 37 E 37 E 35	(KDP) 57 61 79 88 93 104 114 114 128 139 148 166 187 206 222 240 X00 222 240 X00 X00 222 240 X00 X00 X00 X00 X00 X00 X00 X00 X00 X	Max. Moment           (FF KP)           72           143           215           334           473           612           761           976           1190           1457           2014           2667           3402           4260           5217           B40-8 Single           Max. Moment           (FF KP)           85           170           255           406           571           737           902           1067           1232           1398           1759           2297           2958	"E" R/ SHEAR E 57 E 35 E 33 E 34 E 33 E 34 E 34 E 34 E 34 E 34	MOMENT E 57 E 51 E 34 E 32 E 31 E 30 E 30 E 30 E 30 E 30 E 30 E 31 E 31 E 31 E 31 E 31 E 32 E 32 E 32 E 32 E 32 E 32 E 32 E 32
(Feet) 5 10 20 25 30 35 40 45 50 60 70 80 90 100 \$ \$ 100 \$ \$ 100 \$ \$ \$ 10 45 \$ 50 60 90 100 \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$	Max. Shear (%P) 80 120 120 227 252 277 302 327 349 392 442 497 549 600 Max. Shear (%P) 80 120 160 200 227 252 277 302 327 252 277 302 327 349 392 442 497	Max. Moment (FF KH) 100 225 500 825 1224 2088 2623 3202 3204 5196 6830 8638 10678 12893 PER E80 Max. Moment (FF KH) 100 225 500 825 1224 1642 2088 2623 3202 3202 3202 3202 3202 3202 3202 3203 3202 3203 3202 3203 3202 3203 3202 3203 3204 3205 3205 3205 3205 3204 3205 3	( <i>KUP</i> ) 64 70 90 99 105 119 131 147 159 169 184 227 245 259 Max. Shear ( <i>KUP</i> ) 60 65 83 93 98 102 105 107 113 119 139 153	Max. Moment           (FF ки)           80           160           240           382           538           693           875           1115           1362           1673           2295           2916           3701           4565           5525           U23-8 Single           Max. Moment           (FF ки)           76           151           227           353           500           647           794           941           1088           1235           1533           1937           2498	"E" R/ SHEAR E 64 E 45 E 40 E 37 E 38 E 39 E 37 E 36 E 35 E 35	молиемт E 64 E 57 E 38 E 37 E 35 E 34 E 36 E 34 E 36 E 32 E 32 E 30 E 29 E 27 E 26 E 24 E 24 E 30 E 29 E 27 E 24 E 24 E 24 E 24 E 30 E 29 E 27 E 24 E 22 E 24 E 23 E 24 E 24 E 23 E 24 E 24 E 23 E 24 E	(КиР) 57 61 79 88 93 97 107 116 129 139 154 165 173 179 184 Мах. Shear (КиР) 60 67 85 99 116 131 147 159 168 175 186 203	Max. Moment (FF K#) 72 143 215 334 473 612 751 891 1074 1288 1819 2376 2932 3489 4046 U23-B in Max. Moment (FF K#) 76 151 227 363 582 809 1044 1339 1633 1927 2526 3131 3736	"E" RA SHEAR E 57 E 41 E 39 E 35 E 33 E 31 E 31 E 31 E 32 E 31 E 32 E 31 E 32 E 31 E 32 E 31 E 32 E 31 E 32 E 33 E 40 E 45 E 43 E 40 E 41 E 42 E 42 E 42 E 42 E 41 E 42 E 38 E 37	момент E 57 E 51 E 34 E 32 E 31 E 30 E 29 E 27 E 27 E 27 E 27 E 27 E 27 E 28 E 28 E 27 E 28 E 28 E 27 E 26 E 25 MOMENT E 61 E 54 E 35 E 38 E 39 E 40 E 41 E 41 E 41 E 39 E 37	(KP) 57 61 79 88 93 104 114 128 139 148 166 187 206 222 240 Max. Shear (KP) 68 75 95 105 112 116 119 122 131 162 178	Max. Moment           (FT KIP)           72           143           215           334           473           612           761           976           1190           1457           2014           2667           3402           4260           5217           B40-8 Single           Max. Moment           (FF KIP)           85           170           255           406           571           733           902           1067           1232           1398           1759           2297	"E" R/ SHEAR E 57 E 41 E 39 E 35 E 33 E 33 E 34 E 34 E 34 E 34 E 34 E 34 E 34 E 34 E 34 E 32 E 32 E 32 E 82 E 83 E 82 E 83 E 82 E 83 E 82 E 83 E 8	MOMENT E 57 E 51 E 34 E 32 E 31 E 30 E 30 E 30 E 30 E 30 E 31 E 31 E 31 E 31 E 31 E 31 E 31 E 32 E 32 E 32 E 32 E 32 E 32 E 32 E 32

Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts

#### CONNECTICUT RAILROAD EQUIPMENT TABLE A<sup>(CONT)</sup> EQUIVALENT COOPER LOADING SUMMARY

		050 500		P 40 0	Date			Clast 22.C	140.00			2201.112	a far the	
		PER E80	May Chara	B40-8 in		ATING	May Char	Single 220k/			May Char	220k AAR car		
span				Max. Moment		ATING		Max. Moment			Max. Shear	Max. Moment		1
(Feet) 5	(KIP) 80	(FT KIP) 100	(KIP) 68	(FT KIP) 85	SHEAR E 68	MOMENT E 68	(KIP) 55	(FT KIP) 69	SHEAR E 55	MOMENT E 55	(KD) 55	(FT KIP) 69	SHEAR E 55	MOMENT E 55
10	120	22.5	75	170	E 50	E 60	78	137	E 52	E 49	78	138	E 52	E 49
15	160	500	95	255	E 48	E 41	89	267	E 44	E 43	98	274	E 49	E 44
20	200	82.5	105	406	E 42	E 39	94	401	E 38	E 39	119	480	E 48	E 47
25	227	1224	112	571	E 39	E 37	103	53.5	E 36	E 35	139	709	E 49	E 46
30	252	1642	122	737	E 39	E 36	116	668	E 37	E 33	153	976	E 48	E 48
35	277	2088	134	902	E 39	E 35	131	83.5	E 38	E 32	162	1243	E 47	E 48
40	302	2623	150	1122	E40	E 34	142	1054	E 38	E 32	176	1511	E 47	E 46
45	327	3202	163	1377	E 40	E 34	151	1322	E 37	E 33	192	1785	E 47	E 45
50	349	3804	174	1674	E 40	E 35	158	1589	E 36	E 33	209	2129	E 48	E45
60	392	5196	190	2335	E 39	E 36	168	2124	E 34	E 33	244	2896	E 50	E45
70	442	6830	206	2996	E 37	E 35	175	2658	E 32	E 31	272	3937	E 49	E 46
80	497	8638	226	3657	E 36	E 34	181	3193	E 29	E 30	303	5082	E 49	E 47
90	549	10678	247	4477 5393	E 36	E 34	185	3728	E 27	E 28	330	6433	E 48	E 48
100	600	12893	263	5393	E 35	E 33	189	4262	E 2.5	E 26	3.52	7808	E 47	E 48
	C00	PER E80		Single 263k/	AAR car			263K AAR Car	rs in line			Single 286k	AR car	
span		Max. Moment	Max. Shear			ATING	Max. Shear	Max. Moment	"E" RA	TING	Max. Shear			ATING
(Feet)	(KIP)	(FT KIP)	(KUP)	(FT KIP)	SHEAR	MOMENT	(KIP)	(FT KP)	SHEAR	MOMENT	(KIP)	(FT KIP)	SHEAR	MOMENT
5	80	100	66	82	E 66	E 66	66	82	E 66	E 66	71	89	E 71	E 72
10	120	22.5	93	164	E 62	E 58	93	164	E 62	E 58	101	179	E 68	E 64
15	160	500	106	320	E 53	E 51	117	327	E 58	E 52	115	348	E 58	E 56
20	200	82.5	112	479	E45	E 46	142	574	E 57	E 56	122	521	E 49	E 51
25	227	1224	116	639	E41	E 42	166	847	E 59	E 55	126	695	E 45	E 4 5
30	252	1642	120	799	E 38	E 39	182	1167	E 58	E 57	130	869	E 41	E 42
35	277	2088	131	959	E 38	E 37	194	1486	E 56	E 57	142	1043	E 41	E 40
40	302	2623	147	1123	E 39	E 34	203	1806	E 54	E 55	160	1221	E 42	E 37
45	327	3202	160	1288	E 39	E 32	214	2134	E 52	E 53	174	1400	E 43	E 35
50	349	3804	170	1527	E 39	E 32	228	2463	E 52	E 52	185	1661	E 42	E35
60	392	5196	186	2160	E 38	E 33	262	3140	E 53	E 48	202	2349	E 41	E 36
70	442 497	6830	197 205	2799 3438	E 36 E 33	E 33 E 32	299 328	4042	E 54 E 53	E 47 E 48	214 223	3044 3739	E 39 E 36	E 36
90	549	8638 10678	203	4077	E 33	E 32	354	6506	E 55	E 48	230	4434	E 34	E 3 5 E 3 3
100	600	12893	211	4077	E 29	E 29	385	7969		E 49				E 32
100	000													
					622	125	200	7909	E 51	E 49	236	5129	E 31	1.72
	COO	PER E80				125								
span		PER E80 Max. Moment		286K AAR Ca Max. Moment	rs in line	ATING	2	63k Single Triple Max. Moment				53k Triple Hoppe	r Cars in lin	
span (Feet)			Max. Shear	286K AAR Ca	rs in line		2	63k Single Triple	Hopper car		20	53k Triple Hoppe	r Cars in lin	e
	Max. Shear	Max. Moment	Max. Shear	286K AAR Ca Max. Moment	rs in line "E" R	ATING	2 Max. Shear	63k Single Triple Max. Moment	Hopper car "E" R4	TING	20 Max. Shear	53k Triple Hoppe Max. Moment	r Cars in lin "E" R	e ATING
(Feet)	Max. Shear	Max. Moment	Max. Shear	286K AAR Car Max. Moment (FT KIP)	rs in line "E" R SHEAR	ATING	2 Max. Shear (KIP)	63k Single Triple Max. Moment (हर ४७)	Hopper car "E" RA SHEAR	ATING	20 Max. Shear (KIP)	53k Triple Hoppe Max. Moment (FT KIP)	r Cars in lin "E" R SHEAR	e ATING MOMENT
(Feet) 5	Max. Shear (KIP) 80	Max. Moment (FT KIP) 100	Max. Shear (KIP) 71	286K AAR Car Max. Moment (FT KIP) 89	rs in line "E" Rø SHEAR E 71	ATING MOMENT E 71	2 Max. Shear (KIP) 66	63k Single Triple Max. Moment (FT KB) 82	Hopper car "E" RA SHEAR E 66	ATING MOMENT E 66	20 Max. Shear (۲۵۶) 66	53k Triple Hoppe Max. Moment (FT KIP) 82	r Cars in lin "E" Ri SHEAR E 66	e ATING MOMENT E 66
(Feet) 5 10 15 20	Max. Shear (KIP) 80 120 160 200	Max. Moment (FT KIP) 100 225 500 825	Max. Shear (KUP) 71 101 127 155	286K AAR Cai Max. Moment (FT KIP) 89 179 356 624	rs in line "E" R/ <i>SHEAR</i> E 71 E 68 E 63 E 63 E 62	ATING MOMENT E 71 E 64 E 57 E 61	2 Max. Shear (KiP) 66 93 106 112	63k Single Triple Max. Moment (FT κΦ) 82 164 320 479	Hopper car "E" RA SHEAR E 66 E 62 E 53 E 45	ATING MOMENT E 66 E 58 E 51 E 46	20 Max. Shear (KD) 66 93 115 139	53k Triple Hoppe Max. Moment (FT КIР) 82 164 320 559	r Cars in lin "E" R <i>SHEAR</i> E 66 E 62 E 57 E 56	e ATING E 66 E 58 E 51 E 54
(Feet) 5 10 15 20 25	Max. Shear (KIP) 80 120 160 200 227	Max. Moment (FT KUP) 100 225 500 825 1224	Мах. Shear ( <sup>KIP</sup> ) 71 101 127 155 181	286K AAR Car Max. Moment (FT KIP) 89 179 356 624 921	rs in line "E" RV SHEAR E 71 E 68 E 63 E 63 E 62 E 64	ATING MOMENT E 71 E 64 E 57 E 61 E 60	2 Max. Shear (KIP) 66 93 106 112 116	63k Single Triple Max. Moment (FT κΦ) 82 164 320 479 639	Hopper car "E" R <i>A</i> SHEAR E 66 E 62 E 53 E 45 E 41	TING MOMENT E 66 E 58 E 51 E 46 E 42	20 Max. Shear ( <i>KP</i> ) 66 93 115 139 164	53k Triple Hoppe Max. Moment (FT КIР) 82 164 320 559 822	r Cars in lin "E" R SHEAR E 66 E 62 E 57 E 56 E 58	e ATING <i>MOMENT</i> E 66 E 58 E 51 E 54 E 54
(Feet) 5 10 15 20 25 30	Max. Shear (KIP) 80 120 160 200 227 252	Мах. Moment (F7 к/P) 100 225 500 825 1224 1642	(KIP) 71 101 127 155 181 198	286K AAR Car Max. Moment (FT xir) 89 179 356 624 921 1269	rs in line "E" RV SHEAR E 71 E 68 E 63 E 63 E 62 E 64 E 63	ATING MOMENT E 71 E 64 E 57 E 61 E 60 E 62	2 Max. Shear (KIP) 66 93 106 112 116 119	63k Single Triple Max. Moment (FT κΦ) 82 164 320 479 639 799	Hopper car "E" R/ SHEAR E 66 E 62 E 53 E 45 E 41 E 38	TING MOMENT E 66 E 58 E 51 E 46 E 42 E 39	20 Max. Shear (KB) 66 93 115 139 164 180	63k Triple Hoppe Max. Moment (FT кіг) 82 164 320 559 822 1141	r Cars in lin "E" R <i>SHEAR</i> E 66 E 62 E 57 E 56 E 58 E 57	е ATING момент E 66 E 58 E 51 E 54 E 54 E 54 E 56
(Feet) 5 10 15 20 25 30 35	Max. Shear (%P) 80 120 160 200 227 252 277	Max. Moment (F7 к/P) 100 225 500 825 1224 1642 2088	(KUP) 71 101 127 155 181 198 211	286K AAR Cai Max. Moment (FT KIP) 89 179 356 624 921 1269 1616	rs in line "E" R SHEAR E 71 E 68 E 63 E 63 E 62 E 64 E 63 E 61	ATING MOMENT E 71 E 64 E 57 E 61 E 60 E 62 E 62	2 Max. Shear (KIP) 66 93 106 112 116 119 126	63k Single Triple Max. Moment (FT K <sup>Ø</sup> ) 82 164 320 479 639 799 959	Hopper car "E" R/ SHEAR E 66 E 62 E 53 E 45 E 41 E 38 E 36	ATING MOMENT E 66 E 58 E 51 E 46 E 42 E 39 E 37	20 Max. Shear ( <i>KB</i> ) 66 93 115 139 164 180 192	53k Triple Hoppe Max. Moment ( <i>FT КIP</i> ) 82 164 320 559 822 1141 1461	r Cars in lin "E" R E 66 E 62 E 57 E 56 E 58 E 58 E 57 E 56	e ATING MOMENT E 66 E 58 E 51 E 54 E 54 E 54 E 56 E 56
(Feet) 5 10 15 20 25 30 35 40	Max. Shear (%) 80 120 160 200 227 252 277 302	Max. Moment           (FT KIP)           100           225           500           825           1224           1642           2088           2623	Max. Shear (KIP) 71 101 127 155 181 198 211 220	286K AAR Cat Max, Moment (FT KH) 89 179 356 624 921 1269 1616 1964	rs in line "E" R SHEAR E 71 E 68 E 63 E 63 E 62 E 64 E 63 E 61 E 58	ATING MOMENT E 71 E 64 E 57 E 61 E 60 E 62 E 62 E 60	2 Max. Shear (KIP) 66 93 106 112 116 119 126 139	63k Single Triple Max. Moment (FT K#) 82 164 320 479 639 799 959 1123	Hopper car "E" R/ SHEAR E 66 E 62 E 53 E 45 E 45 E 45 E 41 E 38 E 36 E 37	ATING MOMENT E 66 E 58 E 51 E 46 E 42 E 39 E 37 E 34	20 Max. Shear ( <i>KB</i> ) 66 93 115 139 164 139 164 180 192 201	53k Triple Hoppe Max. Moment (FT кi#) 82 164 320 559 822 1141 1461 1461 1781	r Cars in lin "E" R E 66 E 62 E 57 E 56 E 58 E 57 E 56 E 57 E 56 E 53	e ATING MOMENT E 66 E 58 E 51 E 54 E 54 E 56 E 56 E 54
(Feet) 5 10 15 20 25 30 35 40 45	Max. Shear (%) 80 120 160 200 227 252 277 302 327	Мах. Moment (F7 кв) 100 225 500 825 1224 1642 2088 2623 3202	Max. Shear (KIP) 71 101 127 155 181 198 211 220 232	286K AAR Ca Max. Moment (F7 xii) 89 179 356 624 921 1269 1616 1964 2321	rs in line "E" R/ SHEAR E 71 E 68 E 63 E 63 E 64 E 63 E 61 E 58 E 57	ATING MOMENT E 71 E 64 E 57 E 61 E 60 E 62 E 62 E 60 E 58	2 Max. Shear (KIP) 66 93 106 112 116 119 126 139 153	63k Single Triple Max. Moment (77 K <sup>g</sup> ) 82 164 320 479 639 799 959 1123 1288	Hopper car "E" R/ SHEAR E 66 E 62 E 53 E 45 E 41 E 38 E 36 E 37 E 37	ATING MOMENT E 66 E 58 E 51 E 46 E 42 E 39 E 37 E 34 E 32	20 Max. Shear ( <i>KS</i> ) 66 93 115 139 164 180 192 201 208	63k Triple Hoppe Max. Moment (FT NP) 82 164 320 559 822 1141 1461 1781 2104	r Cars in lin "E" R SHEAR E 66 E 62 E 57 E 56 E 57 E 56 E 57 E 56 E 53 E 51	e ATING MOMENT E 66 E 58 E 51 E 54 E 54 E 54 E 56 E 54 E 54 E 53
(Feet) 5 10 15 20 25 30 35 40 45 50	Max. Shear (KIP) 80 120 160 200 227 252 277 302 327 349	Max. Moment (FT KIP) 100 225 500 825 1224 1642 2088 2623 3202 3804	Max. Shear (KEP) 71 101 127 155 181 198 211 220 232 248	286K AAR Ca Max. Moment (FF KI) 89 179 356 624 921 1269 1616 1964 2321 2678	rs in line "E" R SHEAR E 71 E 68 E 63 E 62 E 64 E 63 E 61 E 58 E 57 E 57 E 57	ATING MOMENT E 71 E 64 E 57 E 61 E 60 E 62 E 62 E 60 E 58 E 56	2 Max. Shear (KIP) 66 93 106 112 116 119 126 139 153 164	63k Single Triple Max. Moment (FT κθ) 82 164 320 479 639 799 959 1123 1288 1458	Hopper car "E" R/ SHEAR E 66 E 62 E 53 E 45 E 45 E 41 E 38 E 36 E 37 E 37 E 37 E 38	TING MOMENT E 66 E 58 E 51 E 46 E 42 E 39 E 37 E 34 E 32 E 31	20 Max. Shear (x\$) 66 93 1115 139 164 180 192 201 208 220	53k Triple Hoppe Max. Moment (FT ки) 82 164 320 559 822 1141 1461 1781 2104 2433	r Cars in lin "E" RU SHEAR E 66 E 62 E 57 E 56 E 57 E 56 E 57 E 56 E 53 E 51 E 50	e ATING E 66 E 58 E 51 E 54 E 54 E 56 E 56 E 56 E 54 E 53 E 51
(Feet) 5 10 15 20 25 30 35 40 45 50 60	Max. Shear (%) 80 120 160 200 227 252 277 302 327 349 392	Max. Moment (FT KIP) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196	Max. Shear (KBP) 71 101 127 155 181 198 211 220 232 248 285	286K AAR Ca Max. Moment (FT KH) 89 179 356 624 921 1269 1616 1964 2321 2678 3414	rs in line "E" Rv SHEAR E 71 E 68 E 63 E 62 E 64 E 63 E 61 E 58 E 57 E 58	ATING MOMENT E 71 E 64 E 57 E 61 E 60 E 62 E 62 E 62 E 62 E 58 E 56 E 53	2 Max. She ar (KP) 66 93 106 112 116 119 126 139 153 164 180	63k Single Triple Max. Moment (FF KØ) 82 164 320 479 639 799 959 1123 1288 1458 2020	E Hopper car "E" RA SHAAR E 66 E 62 E 53 E 45 E 41 E 38 E 36 E 37 E 38 E 37	TING MOMENT E 66 E 58 E 51 E 46 E 42 E 39 E 37 E 34 E 37 E 34 E 32 E 31 E 31	20 Max. Shear (KP) 66 93 115 139 164 180 192 201 201 208 220 251	63k Triple Hoppe Max. Moment (FF 80) 82 164 320 559 822 1141 1461 1781 2104 2433 3090	r Cars in lin "E" R SHEAR E 66 E 62 E 57 E 56 E 58 E 57 E 56 E 53 E 51 E 50 E 51	e ATING MOMENT E 66 E 58 E 51 E 54 E 54 E 54 E 56 E 56 E 56 E 53 E 51 E 53 E 51 E 48
(Feet) 5 10 15 20 25 30 35 40 45 50	Max. Shear (KIP) 80 120 160 200 227 252 277 302 327 349	Max. Moment (FT KIP) 100 225 500 825 1224 1642 2088 2623 3202 3804	Max. Shear (KEP) 71 101 127 155 181 198 211 220 232 248	286K AAR Ca Max. Moment (FF KI) 89 179 356 624 921 1269 1616 1964 2321 2678	rs in line "E" R SHEAR E 71 E 68 E 63 E 62 E 64 E 63 E 61 E 58 E 57 E 57 E 57	ATING MOMENT E 71 E 64 E 57 E 61 E 60 E 62 E 62 E 60 E 58 E 56	2 Max. Shear (KIP) 66 93 106 112 116 119 126 139 153 164	63k Single Triple Max. Moment (FT κθ) 82 164 320 479 639 799 959 1123 1288 1458	Hopper car "E" R/ SHEAR E 66 E 62 E 53 E 45 E 45 E 41 E 38 E 36 E 37 E 37 E 37 E 38	TING MOMENT E 66 E 58 E 51 E 46 E 42 E 39 E 37 E 34 E 32 E 31	20 Max. Shear (x\$) 66 93 1115 139 164 180 192 201 208 220	53k Triple Hoppe Max. Moment (FT ки) 82 164 320 559 822 1141 1461 1781 2104 2433	r Cars in lin "E" RU SHEAR E 66 E 62 E 57 E 56 E 57 E 56 E 57 E 56 E 53 E 51 E 50	e ATING E 66 E 58 E 51 E 54 E 54 E 56 E 56 E 56 E 54 E 53 E 51
(Feet) 5 10 15 20 25 30 35 40 45 50 60 70	Max. Shear (%) 80 120 160 227 252 277 302 327 349 392 442	Max. Moment (FF KF) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830	Max. Shear (NP) 71 101 127 155 181 198 211 220 232 248 285 325	286K AAR Ca Max. Moment (FT kii) 89 179 356 624 921 1269 1616 1964 2321 2678 3414 4396	rs in line "E" R SHEAR E 68 E 63 E 62 E 64 E 63 E 64 E 63 E 61 E 58 E 57 E 57 E 58 E 59	ATING MOMENT E 71 E 64 E 57 E 61 E 60 E 62 E 62 E 62 E 62 E 58 E 56 E 53 E 51	2 Max. Shear ( <i>Ki</i> ) 66 93 106 112 116 119 126 139 153 164 180 192	63k Single Triple Max. Moment (FT κØ) 82 164 320 479 639 799 959 1123 1288 1458 2020 2660	Hopper car "E" R/ SHEAR E 66 E 62 E 53 E 45 E 45 E 45 E 37 E 37 E 37 E 37 E 35	TING MOMENT E 66 E 58 E 51 E 46 E 42 E 39 E 37 E 34 E 32 E 31 E 31	20 Max. Shear ( <i>KP</i> ) 66 93 115 139 164 180 192 201 208 220 220 251 286	53k Triple Hoppe Max. Moment (FT ki) 82 164 320 559 822 1141 1461 1781 2104 2433 3090 3868	r Cars in lin "E" R SHEAR E 66 E 62 E 57 E 56 E 58 E 57 E 56 E 53 E 51 E 52	e ATING E 66 E 58 E 51 E 54 E 54 E 56 E 56 E 56 E 54 E 53 E 51 E 54 E 53 E 51 E 54 E 54 E 53 E 51 E 54 E 54 E 54 E 54 E 54 E 54 E 54 E 55 E 55
(Feet) 5 10 15 20 25 30 35 40 45 50 60 70 80	Max. Shear (XEP) 80 120 160 200 227 252 277 302 327 302 327 349 392 442 497	Max. Moment (FF KiP) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830 8638	Max. Shear (wp) 71 101 127 155 181 198 211 220 232 248 248 285 325 356	286K AAR Ca Max. Moment (F7 KH) 89 179 356 624 921 1269 1616 1964 2321 2678 3414 4396 5684	rs in line "E" R SHEAR E 7 E 63 E 63 E 63 E 63 E 63 E 64 E 63 E 61 E 58 E 57 E 57 E 58 E 59 E 57	ATING MOMENT E 71 E 64 E 57 E 61 E 60 E 62 E 62 E 62 E 62 E 58 E 56 E 53 E 51 E 53	2 Max. Shear (%) 66 93 106 112 116 119 126 139 153 164 180 192 201	63k Single Triple Max. Moment (FT K#) 82 164 320 479 639 799 959 1123 1288 1458 2020 2660 3299	Hopper car "E" R/ SHEAR E 66 E 62 E 41 E 38 E 45 E 41 E 38 E 37 E 37 E 37 E 38 E 37 E 37 E 37 E 38 E 37 E 3	TTING MOMENT E 66 E 58 E 51 E 46 E 42 E 37 E 34 E 32 E 31 E 31 E 31	2( Max.Shear (%) 66 93 115 139 164 180 192 201 208 220 201 208 220 251 286 316	53k Triple Hoppe Max. Moment (FT кi/) 82 164 320 559 822 1141 1461 1461 1781 2104 2433 3090 3868 4888	r Cars in lin "E" R SHEAR E 66 E 62 E 57 E 56 E 58 E 57 E 56 E 53 E 51 E 50 E 51 E 52 E 51	e ATING MAMENT E 66 E 58 E 51 E 54 E 54 E 54 E 54 E 54 E 54 E 54 E 53 E 51 E 54 E 55 E 55 E 55 E 55 E 55 E 55 E 55
(Feet) 5 10 20 25 30 35 40 45 50 60 70 80 90	Max. Shear (KP) 80 120 160 200 227 252 277 302 327 349 392 442 497 549	Max. Moment (FT KiP) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830 8638 10678	Max. Shear (NP) 71 101 127 155 181 198 211 220 232 248 285 325 325 356 385	286K AAR Ca Max. Moment (FF Kit) 89 179 356 624 921 1269 1616 1964 2321 2678 3414 4396 55684 7075	rs in line "E" R <i>SHEAR</i> E 71 E 68 E 63 E 62 E 64 E 63 E 61 E 58 E 57 E	ATING MOMENT E 71 E 64 E 57 E 61 E 60 E 62 E 60 E 52 E 56 E 53 E 51 E 53 E 53	2 Max. Shear (%) 66 93 106 112 116 119 126 139 153 164 180 192 201 208	63k Single Triple Max. Moment (FT κθ) 82 164 320 479 639 799 959 1123 1288 1458 2020 2660 3299 3938	Hopper car "E" RA SHEAR E 66 E 62 E 53 E 45 E 41 E 38 E 36 E 37 E 38 E 37 E 32 E 30 E 37 E 38 E 32 E 30 E 3	ATING MOMENT E 66 E 58 E 51 E 46 E 42 E 39 E 37 E 34 E 32 E 31 E 31 E 31 E 31 E 31 E 31 E 31	2( Max. Shear ( <i>KB</i> ) 66 93 115 139 164 180 192 201 208 220 251 286 316 340	63k Triple Hoppe Max. Moment (FT xk) 82 164 320 559 822 1141 1461 1781 2104 2433 3090 3868 4888 6167	r Cars in lin "E" R SHEAR E 66 E 62 E 57 E 56 E 58 E 57 E 56 E 53 E 51 E 50 E 51 E 51 E 51 E 49	e ATING E 66 E 58 E 51 E 54 E 54 E 56 E 56 E 56 E 56 E 55 E 55 E 55 E 55
(Feet) 5 10 20 25 30 35 40 45 50 60 70 80 90	Max. Shear (XEP) 80 120 200 227 252 277 302 327 302 327 349 392 442 497 549 600	Max. Moment (FF KB) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830 8638 10678 12893 PER E80	Max. Shear (wp) 71 101 127 155 181 198 211 220 232 248 248 285 325 356 385 418	286K AAR Ca Max. Moment (F7 KH) 89 179 356 624 921 1269 1616 1964 2321 2678 3414 4396 5684 7075 8666 88666	rs in line "E" R SHEAR E 71 E 68 E 63 E 64 E 63 E 64 E 63 E 64 E 57 E 58 E 57 E 57 E 58 E 59 E 57 E 58 E 56 E 56	ATING MOMENT E 71 E 64 E 57 E 61 E 60 E 62 E 60 E 58 E 56 E 53 E 53 E 53 E 54	2 Max. Shear (kP) 66 93 106 112 116 119 126 139 153 164 180 192 201 208 201 208 213	63k Single Triple Max. Moment (FT KØ) 82 164 320 479 639 799 959 1123 1288 1458 2020 2660 3299 3938 4577 86k Triple Hoppe	Hopper car "E" R/ SHEAR E 66 E 62 E 41 E 38 E 45 E 41 E 38 E 37 E 37 E 37 E 37 E 37 E 37 E 37 E 37 E 38 E 37 E 32 E 30 E 28 er car in line	ATING MOMENT E 66 E 58 E 51 E 46 E 42 E 39 E 37 E 34 E 31 E 31 E 31 E 30 E 28	2( Max. Shear ( <i>KB</i> ) 66 93 115 139 164 180 192 201 208 220 251 286 316 340	63k Triple Hoppe Max. Moment (FT xk) 82 164 320 559 822 1141 1461 1781 2104 2433 3090 3868 4888 6167	r Cars in lin "E" R SHEAR E 66 E 62 E 57 E 56 E 58 E 57 E 56 E 53 E 51 E 50 E 51 E 51 E 51 E 49	e ATING E 66 E 58 E 51 E 54 E 54 E 56 E 56 E 56 E 56 E 55 E 55 E 55 E 55
(Feet) 5 10 20 25 30 35 40 45 50 60 70 80 90	Max. Shear (XEP) 80 120 200 227 252 277 302 327 302 327 349 392 442 497 549 600	Max. Moment (FT KIP) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830 8638 10678 12893 PER E80 Max. Moment	Max. Shear (wp) 71 101 127 155 181 198 211 220 232 248 248 285 325 356 385 418	286K AAR Ca Max. Moment (FF kit) 89 179 356 624 921 1269 1616 1964 2321 2678 3414 4396 5584 7075 8666 86K Single Triple Max. Moment	rs in line "E" R SHEAR E 71 E 68 E 63 E 64 E 63 E 64 E 63 E 64 E 57 E 58 E 57 E 57 E 58 E 59 E 57 E 58 E 56 E 56	ATING MOMENT E 71 E 64 E 57 E 61 E 60 E 62 E 60 E 52 E 60 E 58 E 56 E 53 E 53 E 53 E 53 E 53 E 54 ATING	2 Max. Shear (kP) 66 93 106 112 116 119 126 139 153 164 180 192 201 208 201 208 213	63k Single Triple Max. Moment ( <i>FT κθ</i> ) 82 1164 320 479 639 799 959 1123 1288 1458 2020 2660 3299 3938 4577 3938 4577	Hopper car "E" RA SHEAR E 66 E 62 E 45 E 41 E 38 E 35 E 41 E 38 E 37 E 37 E 38 E 37 E 37 E 38 E 37 E 32 E 30 E 28 er cars in line "E" RA	ATING MOMENT E 66 E 58 E 51 E 46 E 42 E 39 E 37 E 34 E 32 E 31 E 31 E 31 E 31 E 30 E 28 ATING	2( Max. Shear ( <i>KB</i> ) 66 93 115 139 164 180 192 201 208 220 251 286 316 340	63k Triple Hoppe Max. Moment (FT xk) 82 164 320 559 822 1141 1461 1781 2104 2433 3090 3868 4888 6167	r Cars in lin "E" R SHEAR E 66 E 62 E 57 E 56 E 58 E 57 E 56 E 53 E 51 E 50 E 51 E 51 E 51 E 49	e ATING E 66 E 58 E 51 E 54 E 54 E 56 E 56 E 56 E 56 E 55 E 55 E 55 E 55
(Feet) 5 10 20 25 30 35 40 45 50 60 70 80 90 100 \$pan (Feet)	Max. Shear (κε) 80 120 160 227 252 277 302 327 349 392 442 497 549 600 <b>COO</b> Max. Shear (κε)	Мах. Moment (FT KiP) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830 8638 10678 12893 РЕК E80 Мах. Moment (FT KiP)	Max. Shear (KIP) 71 101 127 155 181 198 211 220 232 248 285 325 325 325 356 385 418 22 Max. Shear (KIP)	286K AAR Ca Max. Moment (FF KØ) 89 179 356 624 921 1269 1616 1964 2321 2678 3414 4396 5684 7075 8666 8666 8666 Single Triple Max. Moment (FF KØ)	rs in line "E" R SHEAR E 71 E 68 E 63 E 63 E 64 E 63 E 64 E 63 E 64 E 57 E 57 E 57 E 57 E 58 E 59 E 57 E 56 E 57 E 56 E 57 E 57 E 58 E 59 E 57 E 56 E 56 E 57 E 58 E 59 E 57 E 56 E 56 E 57 E 58 E 59 E 57 E 56 E 56 E 57 E 56 E 56 E 56 E 57 E 56 E 56 E 57 E 56 E 57 E 56 E 56 E 56 E 56 E 56 E 57 E 56 E 56	ATING MOMENT E 71 E 64 E 57 E 61 E 60 E 62 E 60 E 58 E 56 E 53 E 51 E 53 E 53 E 54 ATING MOMENT	2 Max. Shear (KP) 66 93 106 112 116 119 126 139 153 164 180 192 201 208 213 201 208 213 213	63k Single Triple Max. Moment (FT к#) 82 164 320 479 639 799 959 1123 1288 1458 2020 2660 3299 3938 4577 36k Triple Hopper Max. Moment (FT к#)	Hopper car "E" RA SHEAR E 66 E 62 E 53 E 45 E 41 E 38 E 36 E 37 E 38 E 37 E 38 E 37 E 32 E 30 E 28 er Carsin line "E" RA SHEAR	ATING MOMENT E 66 E 58 E 51 E 46 E 42 E 39 E 37 E 34 E 31 E 31 E 31 E 31 E 31 E 30 E 28 MOMENT	2( Max. Shear ( <i>KB</i> ) 66 93 115 139 164 180 192 201 208 220 251 286 316 340	63k Triple Hoppe Max. Moment (FT xk) 82 164 320 559 822 1141 1461 1781 2104 2433 3090 3868 4888 6167	r Cars in lin "E" R SHEAR E 66 E 62 E 57 E 56 E 58 E 57 E 56 E 53 E 51 E 50 E 51 E 51 E 51 E 49	e ATING E 66 E 58 E 51 E 54 E 54 E 54 E 56 E 56 E 56 E 56 E 53 E 51 E 48 E 45 E 45 E 45 E 45 E 45 E 45 E 46
(Feet) 5 10 25 20 25 30 40 45 5 60 70 80 90 100 \$pan (Feet) 5	Max. Shear (XFP) 80 120 200 227 252 277 302 327 349 392 442 497 549 600 Max. Shear (XFP) 80	Max. Moment (FF KF) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830 8638 10678 12893 PER E80 Max. Moment (FF KF) 100	Max. Shear (REP) 71 101 127 155 181 198 211 220 232 248 285 325 325 325 325 356 385 418 Max. Shear (REP) 71	286K AAR Ca Max. Moment (FT KH) 89 179 356 624 921 1269 1616 1964 2321 2678 3414 4396 5684 7075 8666 86K Single Triple Max. Moment (FT KH) 89	rs in line "E" R SHEAR E 71 E 68 E 63 E 62 E 64 E 63 E 61 E 58 E 57 E 58 E 57 E 58 E 57 E 58 E 59 E 57 E 56 E 56 E 40 P E 7 E 58 E 57 E 56 E 56 E 56 E 57 E 57 E 7 E 7 E 56 E 57 E 7 E 56 E 57 E 7 E 7 E 7 E 7 E 7 E 7 E 7 E	ATING MOMENT E 71 E 64 E 57 E 61 E 60 E 58 E 56 E 53 E 51 E 53 E 54 ATING MOMENT E 72	2 Max. Shear (KP) 66 93 106 112 116 119 126 139 153 164 180 192 201 201 201 208 213 213 24 Max. Shear	63k Single Triple Мах. Moment (FT кФ) 82 164 320 479 639 799 959 1123 1288 1458 2020 2660 3299 3938 4577 86k Triple Hoppe Мах. Moment (FT к₱) 89	Hopper car "E" R/ SHEAR E 66 E 62 E 53 E 45 E 41 E 38 E 36 E 37 E 38 E 37 E 35 E 32 E 30 E 28 er Cars in lint "E" R/ SHEAR E 71	TTING MOMENT E 66 E 58 E 51 E 46 E 42 E 39 E 37 E 34 E 32 E 31 E 32 E 32 E 31 E 31 E 31 E 31 E 32 E 32 E 31 E 31 E 31 E 31 E 32 E 32 E 31 E 31 E 31 E 31 E 31 E 32 E 32 E 31 E 32 E 32 E 32 E 32 E 32 E 31 E 31 E 31 E 31 E 31 E 31 E 32 E 33 E 3	2( Max. Shear ( <i>KB</i> ) 66 93 115 139 164 180 192 201 208 220 251 286 316 340	63k Triple Hoppe Max. Moment (FT xk) 82 164 320 559 822 1141 1461 1781 2104 2433 3090 3868 4888 6167	r Cars in lin "E" R SHEAR E 66 E 62 E 57 E 56 E 58 E 57 E 56 E 53 E 51 E 50 E 51 E 51 E 51 E 49	e ATING E 66 E 58 E 51 E 54 E 54 E 54 E 56 E 56 E 56 E 56 E 53 E 51 E 48 E 45 E 45 E 45 E 45 E 45 E 45 E 46
(Feet) 5 10 25 30 40 45 50 60 70 80 90 100 span (Feet) 5 10	Max. Shear (XEP) 80 120 160 200 227 252 277 302 327 349 392 442 497 549 600 Max. Shear (XEP) 80 120	Мах. Moment (FF кв) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830 8638 10678 12893 <b>PER E80</b> Мах. Moment (FF кв) 200 225	Мах. Shear (КВР) 71 101 127 155 181 198 211 220 232 248 285 325 325 325 325 325 3356 385 418 Мах. Shear (КВР) 71 101	286K AAR Ca Max. Moment (FT KiP) 89 179 356 624 921 1269 1616 1964 2321 2678 3414 4396 5684 7075 86466 8646 8666 864 Single Triple Max. Moment (FT KiP) 89 179	rs in line "E" R SHEAR E 71 E 68 E 63 E 62 E 64 E 63 E 61 E 58 E 57 E 57 E 57 E 57 E 57 E 57 E 57 E 56 E 57 E 56 E 57 E 56 E 57 E 56 E 57 E 56 E 57 E 57 E 56 E 57 E 56 E 57 E 57 E 56 E 57 E 56 E 57 E 57 E 56 E 57 E 57 E 58 E 57 E 57 E 58 E 57 E 57 E 58 E 57 E 58 E 57 E 57 E 58 E 57 E 58 E 57 E 57 E 58 E 57 E 58 E 57 E 57 E 56 E 56 E 57 E 56 E 56 E 57 E 57 E 56 E 56 E 56 E 57 E 57 E 56 E 56 E 57 E 56 E 56 E 56 E 57 E 57 E 56 E 56 E 56 E 56 E 57 E 57 E 56 E 56 E 57 E 57 E 56 E 56 E 56 E 57 E 56 E 56 E 56 E 56 E 57 E 56 E 56 E 56 E 56 E 57 E 56 E 57 E 56 E 56 E 56 E 56 E 57 E 56 E 56 E 56 E 56 E 56 E 57 E 56 E 57 E 56 E 56 E 56 E 57 E 56 E 56 E 56 E 57 E 56 E 56 E 56 E 56 E 57 E 56 E 57 E 56 E 57 E 56 E 56 E 57 E 56 E 56 E 57 E 56 E 56 E 57 E 56 E 57 E 57 E 56 E 57 E 57 E 56 E 57 E 57 E 57 E 57 E 57 E 57 E 57 E 57 E 56 E 57 E 56 E 57 E 57 E 57 E 57 E 57 E 56 E 57 E 56 E 57 E 56 E 57 E 56 E 57 E 56 E 57 E 57 E 56 E 57 E 57 E 56 E 57 E 57 E 56 E 57 E 57	ATING MOMENT E 71 E 64 E 57 E 61 E 60 E 62 E 62 E 60 E 58 E 56 E 53 E 51 E 53 E 51 E 53 E 54 ATING MOMENT E 72 E 64	2 Max. Shear (%) 66 93 106 112 116 119 126 139 153 164 180 192 201 208 213 208 213 213 208 213	63k Single Triple Max. Moment (FT κØ) 82 164 320 479 639 799 959 1123 1288 1458 2020 2660 3299 3938 4577 3938 4577 36k Triple Hoppe Max. Moment (FT κØ) 89 179	Hopper car "E" R/ SHEAR E 66 E 62 E 53 E 45 E 41 E 38 E 36 E 37 E 37 E 35 E 32 E 30 E 28 F 20 E 28 R 37 E 35 E 32 E 30 E 28 E 37 E 35 E 32 E 35 E 37 E 35 E 37 E 35 E 37 E 35 E 37 E 37 E 35 E 32 E 30 E 28 E 37 E 37 E 35 E 32 E 30 E 28 E 37 E 35 E 38 E 37 E 38 E 3	ATTING MOMENT E 66 E 58 E 51 E 46 E 42 E 39 E 37 E 34 E 32 E 31 E 31 E 31 E 31 E 31 E 30 E 28 ATTING MOMENT E 71 E 64	2( Max. Shear ( <i>KB</i> ) 66 93 115 139 164 180 192 201 208 220 251 286 316 340	63k Triple Hoppe Max. Moment (FT xk) 82 164 320 559 822 1141 1461 1781 2104 2433 3090 3868 4888 6167	r Cars in lin "E" R SHEAR E 66 E 62 E 57 E 56 E 58 E 57 E 56 E 53 E 51 E 50 E 51 E 51 E 51 E 49	e ATING E 66 E 58 E 51 E 54 E 54 E 54 E 56 E 56 E 56 E 56 E 53 E 51 E 48 E 45 E 45 E 45 E 45 E 45 E 45 E 46
(Feet) 5 10 25 30 25 30 35 30 40 45 50 60 70 80 90 100 \$ \$ 90 100 \$ \$ 10 90 100	Мах. Shear (ЖР) 80 120 200 227 252 277 302 327 302 327 349 392 442 497 549 600 Ках. Shear (КР) 80 120 160	Max. Moment (FT κ/θ) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830 8638 10678 12893 PER E80 Max. Moment (FT κ/θ) 100 225 500	Max. Shear (%P) 71 101 127 155 181 198 211 220 232 248 285 325 325 325 325 3356 385 418 22 Max. Shear (%P) 71 101 115	286K AAR Ca Max. Moment (FF Kit) 89 179 356 624 921 1269 1616 1964 2321 2678 3414 4396 5584 7075 8666 86K Single Triple Max. Moment (FF Kit) 89 179 348	rs in line "E" R SHEAR E 71 E 68 E 63 E 63 E 64 E 63 E 64 E 53 E 57 E 57 E 57 E 57 E 57 E 57 E 57 E 58 E 59 E 57 E 56 E 57 E 58 E 56 E 57 E 58 E 56 E 58 E 58 E 58 E 58 E 58 E 58 E 58 E 58 E 56 E 56 E 56 E 56 E 56 E 56 E 58 E 58	ATING MOMENT E 71 E 64 E 57 E 61 E 60 E 62 E 60 E 52 E 60 E 58 E 56 E 53 E 53 E 53 E 53 E 53 E 54 ATING MOMENT E 72 E 64 E 56	2 Max. Shear (KP) 66 93 106 112 116 119 126 139 153 164 180 192 201 208 213 201 208 213 201 208 213 201 208 213	63k Single Triple Max. Moment (FT κθ) 82 1164 320 479 639 959 959 1123 1288 1458 2020 2660 3299 3938 4577 3938 4577 3938 4577 3938 4577 3938 4577 3938	Hopper car "E" RA SHEAR E 66 E 62 E 45 E 41 E 38 E 35 E 41 E 38 E 37 E 37 E 38 E 37 E 37 E 38 E 37 E 32 E 30 E 28 er Cars in line "E" RA SHEAR E 71 E 68 E 62	ATING MOMENT E 66 E 58 E 51 E 46 E 42 E 39 E 37 E 34 E 32 E 31 E 31 E 31 E 31 E 31 E 30 E 28 ATING MOMENT E 71 E 66 E 58 E 51 E 58 MOMENT E 75 E 75	2( Max. Shear ( <i>KB</i> ) 66 93 115 139 164 180 192 201 208 220 251 286 316 340	63k Triple Hoppe Max. Moment (FT xk) 82 164 320 559 822 1141 1461 1781 2104 2433 3090 3868 4888 6167	r Cars in lin "E" R SHEAR E 66 E 62 E 57 E 56 E 58 E 57 E 56 E 53 E 51 E 50 E 51 E 51 E 51 E 49	e ATING E 66 E 58 E 51 E 54 E 54 E 54 E 56 E 56 E 56 E 56 E 53 E 51 E 48 E 45 E 45 E 45 E 45 E 45 E 45 E 46
(Feet) 5 10 25 30 25 30 40 45 50 60 70 80 90 100 \$pan (Feet) 5 10 15 20	Max. Shear           (KP)           80           120           160           200           227           252           277           302           327           349           392           442           497           549           600           Max. Shear           (KP)           80           120           160           200	Max. Moment (FF KiP) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6630 8638 10678 12893 PER E80 Max. Moment (FF KiP) 100 225 500 825	Max. Shear (NP) 71 101 127 155 181 198 211 220 232 248 285 325 325 325 325 325 325 325 325 325 32	286К ААR Ca Мах. Moment (FF кir) 89 179 356 624 921 1269 1616 1964 2321 2678 3414 4396 5584 7075 8666 86K Single Triple Мах. Moment (FF кir) 89 179 348 521	rs in line "E" R SHEAR E 71 E 68 E 63 E 63 E 64 E 63 E 64 E 57 E 57 E 57 E 58 E 59 E 57 E 56 E 56 E 56 E 71 E 68 E 71 E 68 E 58 E 49	ATING MOMENT E 71 E 64 E 57 E 61 E 60 E 62 E 60 E 52 E 53 E 51 E 53 E 53 E 54 ATING MOMENT E 72 E 64 E 55 E 51	2 Max. Shear (%) 66 93 106 112 116 119 126 139 153 164 180 192 201 208 213 201 208 213 201 208 213 201 208 213	63k Single Triple Max. Moment (FT #9) 82 164 320 4779 639 799 959 1123 1288 1458 2020 2660 3299 3938 4577 3938 4577 36k Triple Hopper Max. Moment (FT #9) 89 179 348 608	Hopper car "E" RA SHEAR E 66 E 62 E 53 E 45 E 41 E 38 E 36 E 37 E 38 E 37 E 38 E 37 E 38 E 37 E 38 E 37 E 32 E 30 E 28 er Carsin line E 71 E 68 E 62 E 61	ATTING           MOMENT           E 66           E 58           E 51           E 46           E 42           E 39           E 37           E 34           E 32           E 31           E 31           E 31           E 31           E 31           E 30           E 28           STING           MOMENT           E 71           E 64           E 59	2( Max. Shear ( <i>KB</i> ) 66 93 115 139 164 180 192 201 208 220 251 286 316 340	63k Triple Hoppe Max. Moment (FT xk) 82 164 320 559 822 1141 1461 1781 2104 2433 3090 3868 4888 6167	r Cars in lin "E" R SHEAR E 66 E 62 E 57 E 56 E 58 E 57 E 56 E 53 E 51 E 50 E 51 E 51 E 51 E 49	e ATING E 66 E 58 E 51 E 54 E 54 E 54 E 56 E 56 E 56 E 56 E 53 E 51 E 48 E 45 E 45 E 45 E 45 E 45 E 45 E 46
(Feet) 5 10 25 20 25 30 40 45 50 60 70 80 90 100 100 \$pan (Feet) 5 10 15 20 20 25 20 20 20 25 20 20 20 20 20 20 20 20 20 20	Max. Shear (XFP) 80 120 200 227 252 277 302 327 349 392 442 497 549 600 Max. Shear (KFP) 80 120 160 200 227	Max. Moment (FF KF) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830 8638 10678 10678 10678 12893 PER E80 Max. Moment (FF KF) 100 225 500 825 1224	Max. Shear (RP) 71 101 127 155 181 198 211 220 232 248 285 325 325 325 325 325 325 325 325 325 32	286К ААК Са Мах. Moment (FF K8) 89 179 356 624 921 1269 1616 1964 2321 2678 3414 4396 5684 7075 8666 86K Single Triple Мах. Moment (FF K8) 89 179 348 521 695	rs in line WE R SHEAR E 71 E 68 E 63 E 63 E 64 E 63 E 64 E 63 E 64 E 58 E 57 E 57 E 57 E 58 E 59 E 57 E 56 E 56 E 71 E 68 E 71 E 68 E 71 E 68 E 71 E 68 E 49 E 45	ATING MOMENT E 71 E 64 E 57 E 61 E 60 E 62 E 60 E 52 E 60 E 53 E 51 E 53 E 54 ATING MOMENT E 72 E 64 E 57 E 61 E 7 E 61 E 62 E 62 E 60 E 53 E 51 E 53 E 54 ATING MOMENT E 7 E 64 E 57 E 61 E 62 E 60 E 53 E 51 E 53 E 54 ATING MOMENT E 7 E 64 E 57 E 61 E 62 E 60 E 53 E 51 E 53 E 54 ATING MOMENT E 7 E 64 E 57 E 61 E 60 E 53 E 51 E 53 E 54 ATING MOMENT E 7 E 64 E 57 E 61 E 53 E 54 ATING MOMENT E 7 E 64 E 55 E 53 E 54 ATING MOMENT E 72 E 64 E 55 E 51 E 72 E 64 E 55 E 51 E 72 E 64 E 55 E 51 E 72 E 64 E 55 E 51 E 51 E 72 E 64 E 55 E 51 E 72 E 75 E 72 E 75 E 75	2 Max. Shear (KP) 66 93 106 112 116 119 126 139 153 164 180 192 201 208 213 201 208 213 201 208 213 201 208 213 213 213 213 214 215 151 178	63k Single Triple Max. Moment (FT κθ) 82 164 320 479 639 799 959 1123 1288 1458 2020 2660 3299 3938 4577 Max. Moment (FT κθ) 89 179 348 608 894	Hopper car "E" R/ SHEAR E 66 E 62 E 53 E 45 E 41 E 38 E 36 E 37 E 38 E 37 E 38 E 37 E 35 E 32 E 30 E 28 er Cars in line "E" R/ SHEAR E 71 E 68 E 62 E 61 E 63	TTING MOMENT E 66 E 58 E 51 E 46 E 42 E 39 E 37 E 34 E 31 E 31 E 31 E 31 E 31 E 31 E 31 E 31 E 31 E 30 E 28 TTING MOMENT E 71 E 64 E 59 E 59 E 58	2( Max. Shear ( <i>ksb</i> ) 66 93 115 139 164 180 192 201 208 220 251 286 316 340	63k Triple Hoppe Max. Moment (FT xk) 82 164 320 559 822 1141 1461 1781 2104 2433 3090 3868 4888 6167	r Cars in lin "E" R SHEAR E 66 E 62 E 57 E 56 E 58 E 57 E 56 E 53 E 51 E 50 E 51 E 51 E 51 E 49	e ATING E 66 E 58 E 51 E 54 E 54 E 54 E 56 E 56 E 56 E 56 E 53 E 51 E 48 E 45 E 45 E 45 E 45 E 45 E 45 E 46
(Feet) 5 10 25 30 40 45 50 60 70 80 90 100 100 \$ 5 10 15 20 25 30 10 25 30 35 40 40 45 50 10 50 15 15 15 15 15 15 15 15 15 15	Max. Shear (XFP) 80 120 160 200 227 252 277 302 327 349 392 442 497 549 600 Max. Shear (XFP) 80 120 160 200 227 252	Max. Moment (FF KF) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830 8638 10678 12893 PER E80 Max. Moment (FF KF) 225 500 825 1200 825 1224 1642	Max. Shear (NUP) 71 101 127 155 181 198 211 220 232 248 285 325 356 385 418 285 325 356 385 418 285 325 356 385 418 20 20 217 20 22 248 285 325 356 385 418	286K AAR Ca Max. Moment (FT KH) 89 179 356 624 921 1269 1616 1964 2321 2678 3414 4396 5684 7075 8866 86K Single Triple Max. Moment (FT KH) 89 179 348 521 695 8869	rs in line "E" R SHEAR E 71 E 68 E 63 E 62 E 64 E 63 E 61 E 58 E 57 E 58 E 57 E 58 E 57 E 56 E 56 Hopper cal SHEAR E 71 E 68 E 58 E 71 E 68 E 58 E 45 E 41	ATING MOMENT E 71 E 64 E 57 E 61 E 60 E 62 E 62 E 62 E 62 E 62 E 53 E 51 E 53 E 51 E 53 E 54 ATING MOMENT E 72 E 64 E 56 E 54 ATING MOMENT	2 Max. Shear (KP) 66 93 106 112 116 119 126 139 153 164 180 192 201 201 201 201 201 203 213 213 213 213 213 213 213	63k Single Triple Max. Moment (FT κΦ) 82 164 320 479 639 799 959 1123 1288 1458 2020 2660 3299 3938 4577 2660 3299 3938 4577 86k Triple Hoppe Max. Moment (FT κΦ) 89 179 348 608 834 1241	Hopper car "E" R/ SHEAR E 66 E 62 E 53 E 45 E 41 E 38 E 36 E 37 E 37 E 35 E 32 E 30 E 28 er Cars in line "E" R/ SHEAR E 71 E 68 E 62 E 63 E 62	ATTING MOMENT E 66 E 58 E 51 E 46 E 42 E 39 E 37 E 34 E 32 E 31 E 32 E 35 E 35	2( Max. Shear ( <i>ksb</i> ) 66 93 115 139 164 180 192 201 208 220 251 286 316 340	63k Triple Hoppe Max. Moment (FT xk) 82 164 320 559 822 1141 1461 1781 2104 2433 3090 3868 4888 6167	r Cars in lin "E" R SHEAR E 66 E 62 E 57 E 56 E 58 E 57 E 56 E 53 E 51 E 50 E 51 E 51 E 51 E 49	e ATING E 66 E 58 E 51 E 54 E 54 E 54 E 56 E 56 E 56 E 56 E 53 E 51 E 48 E 45 E 45 E 45 E 45 E 45 E 45 E 46
(Feet) 5 10 20 25 30 40 45 50 60 70 80 90 100 \$ \$ 5 10 10 15 20 25 30 40 45 50 60 70 80 90 100 100 100 100 100 100 100	Max. Shear           (𝔅𝒫)           80           120           160           200           227           252           277           302           327           392           442           497           549           600           Max. Shear           (𝔅𝒫)           120           160           200           227           252           277	Max. Moment (FT kiP) 100 225 500 825 1224 1642 2088 2623 3202 3202 3202 3804 5196 6830 8638 10678 12893 <b>PER E80</b> Max. Moment (FT KiP) 100 225 500 825 1224 1642 2088	Max. Shear (%P) 71 101 127 155 181 198 211 220 232 248 285 325 325 325 325 3356 385 418 22 Max. Shear (%P) 71 101 115 122 126 129 137	286K AAR Ca Max. Moment (FF xi/) 89 179 356 624 921 1269 1616 1964 2321 2678 3414 4396 5584 7075 8666 8665 8665 889 179 348 521 695 869 1043	rs in line "E" R SHEAR E 71 E 68 E 63 E 63 E 64 E 63 E 64 E 53 E 57 E 57 E 57 E 57 E 58 E 57 E 56 E 57 E 58 E 59 E 57 E 56 E 56 E 56 E 56 E 56 E 56 E 56 E 56 E 57 E 58 E 59 E 57 E 58 E 59 E 57 E 58 E 59 E 57 E 56 E 57 E 58 E 59 E 57 E 56 E 56 E 56 E 56 E 56 E 56 E 57 E 58 E 59 E 57 E 56 E 56 E 56 E 57 E 58 E 59 E 57 E 56 E 56 E 57 E 56 E 56 E 57 E 56 E 56 E 57 E 58 E 59 E 57 E 56 E 56 E 57 E 58 E 59 E 57 E 56 E 56 E 57 E 58 E 59 E 57 E 56 E 56 E 58 E 59 E 57 E 56 E 56 E 58 E 58 E 59 E 58 E 59 E 56 E 58 E 58	ATING MOMENT E 71 E 64 E 57 E 61 E 60 E 62 E 62 E 60 E 58 E 56 E 53 E 53 E 53 E 53 E 53 E 53 E 53 E 54 ATING MOMENT E 72 E 64 E 56 E 51 E 42 E 64 E 56 E 51 E 64 E 56 E 51 E 54 ATING MOMENT E 7 E 64 E 53 E 54 ATING MOMENT E 7 E 64 E 53 E 54 ATING MOMENT E 7 E 64 E 53 E 54 ATING MOMENT E 7 E 64 E 53 E 54 ATING MOMENT E 7 E 64 E 54 ATING ATING MOMENT E 7 E 64 ATING ATINA ATING ATING ATING ATING ATING ATING ATING AT	2 Max. Shear (KP) 66 93 106 112 116 119 126 139 153 164 180 192 201 208 213 201 208 213 201 208 213 201 208 213 213 201 208 213 213 213 213 213 213 213 213 213 213	63k Single Triple Max. Moment (FT κθ) 82 1164 320 479 639 799 959 1123 1288 1458 2020 2660 3299 3938 4577 3938 4577 3938 4577 86k Triple Hoppe Max. Moment (FT κθ) 89 179 348 608 894 1241 1589	Hopper car "E" R7 SHFAR E 66 E 62 E 45 E 41 E 38 E 35 E 41 E 38 E 37 E 38 E 37 E 37 E 36 E 4 E 6 E 6 E 6 E 6 E 6 E 6 E 6 E 6	ATING MOMENT E 66 E 58 E 51 E 46 E 42 E 39 E 37 E 34 E 32 E 31 E 31 E 31 E 31 E 31 E 31 E 31 E 30 E 28 ADMENT E 71 E 66 E 59 E 58 E 59 E 58 E 60 E 66 E 69 E 58 E 60 E 66 E 78 E 78 E 78 E 78 E 77 E 34 E 32 E 31 E 30 E 28 ADMENT E 78 E 78 E 78 E 78 E 78 E 78 E 78 E 78 E 78 E 77 E 77 E 78 E 77 E 77 E 77 E 78 E 77 E 77	2( Max. Shear ( <i>ksb</i> ) 66 93 115 139 164 180 192 201 208 220 251 286 316 340	63k Triple Hoppe Max. Moment (FT xk) 82 164 320 559 822 1141 1461 1781 2104 2433 3090 3868 4888 6167	r Cars in lin "E" R SHEAR E 66 E 62 E 57 E 56 E 58 E 57 E 56 E 53 E 51 E 50 E 51 E 51 E 51 E 49	e ATING E 66 E 58 E 51 E 54 E 54 E 54 E 56 E 56 E 56 E 56 E 53 E 51 E 48 E 45 E 45 E 45 E 45 E 45 E 45 E 46
(Feet) 5 10 25 30 25 30 40 45 50 60 70 80 90 100 \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$	Max. Shear           (№Р)           80           120           160           200           227           252           277           302           327           349           392           442           497           549           600           Max. Shear           (№Р)           80           120           160           200           227           252           277           302	Max. Moment (FT KIP) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 66830 8638 10678 12893 PER E80 Max. Moment (FF KIP) 100 225 500 825 1224 1642 2088 2623	Max. Shear (NP) 71 101 127 155 181 198 211 220 232 248 285 325 325 325 325 325 325 325 325 325 32	286K AAR Ca Max. Moment (FF Kit) 89 179 356 624 921 1269 1616 1964 2321 2678 3414 4396 5584 7075 8666 85K Single Triple Max. Moment (FF Kit) 89 179 348 521 695 869 1043 1221	rs in line "E" R SHEAR E 71 E 68 E 63 E 63 E 64 E 63 E 64 E 63 E 64 E 57 E 57 E 57 E 57 E 58 E 57 E 57 E 56 E 56 E 56 E 71 E 68 E 71 E 71 E 68 E 71 E 71 E 68 E 71 E 71 E 68 E 71 E 75 E 77 E 75 E 77 E 77	ATING MOMENT E 71 E 64 E 57 E 61 E 60 E 62 E 60 E 52 E 53 E 51 E 53 E 54 ATING MOMENT E 72 E 64 E 55 E 51 E 45 E 45 E 45 E 42 E 42 E 40 E 37	2 Max. Shear (%) 66 93 106 112 116 119 126 139 153 164 180 192 201 208 213 201 208 213 201 208 213 201 208 213 201 208 213	63k Single Triple Max. Moment (FT κ9) 82 164 320 479 639 799 959 1123 1288 1458 2020 2660 3299 3938 4577 3938 4577 3938 4577 3938 4577 386k Triple Hopper Max. Moment (FT κ9) 348 608 894 1259 348 608 894 1559 1936	Hopper car "E" RA SHFAR E 66 E 62 E 53 E 45 E 41 E 38 E 36 E 37 E 38 E 4 E 6 B 6 B 7 E 8 SHFAR E 6 E 6 E 6 E 6 E 6 E 6 E 6 E 6	ACTING           MOMENT           E 66           E 58           E 51           E 46           E 42           E 39           E 37           E 34           E 32           E 31           E 31           E 31           E 31           E 31           E 31           E 30           E 28           XTING           MOMENT           E 71           E 64           E 59           E 58           E 600           E 61           E 59	2( Max. Shear ( <i>ksb</i> ) 66 93 115 139 164 180 192 201 208 220 251 286 316 340	63k Triple Hoppe Max. Moment (FT xk) 82 164 320 559 822 1141 1461 1781 2104 2433 3090 3868 4888 6167	r Cars in lin "E" R SHEAR E 66 E 62 E 57 E 56 E 58 E 57 E 56 E 53 E 51 E 50 E 51 E 51 E 51 E 49	e ATING E 66 E 58 E 51 E 54 E 54 E 54 E 56 E 56 E 56 E 56 E 53 E 51 E 48 E 45 E 45 E 45 E 45 E 45 E 45 E 46
(Feet) 5 10 25 30 40 45 5 40 45 5 10 15 20 80 90 100 100 15 20 25 30 100 15 20 25 30 35 40 40 45 5 10 15 20 25 35 40 40 45 5 10 15 20 25 35 40 40 45 5 10 15 15 15 15 15 15 15 15 15 15	Max. Shear (XFP) 80 120 200 227 252 277 302 327 349 392 442 497 549 600 Max. Shear (KFP) 80 120 160 200 227 252 277 302 277 302	Max. Moment (FF KP) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830 8638 10678 10678 10678 12893 PER E80 Max. Moment (FF KP) 100 225 500 825 1224 1642 2088 2623 3202	Мах. Shear (КВР) 71 101 127 155 181 198 211 220 232 248 285 325 325 325 325 325 325 325 325 325 32	286К ААК Са Мах. Moment (Ff Ki) 89 179 356 624 921 1269 1616 1964 2321 2678 3414 4396 5684 7075 8666 8665 8666 8665 879 179 348 521 695 869 1043 1221 1400	rs in line WE WR SHEAR E 71 E 68 E 63 E 63 E 64 E 63 E 64 E 63 E 64 E 63 E 64 E 57 E 57 E 57 E 57 E 58 E 59 E 57 E 56 E 56 E 71 E 68 E 71 E 68 E 49 E 45 E 41 E 40 E 41 E 40 E 41 E 41	ATING MOMENT E 71 E 64 E 57 E 61 E 60 E 62 E 60 E 58 E 56 E 53 E 51 E 53 E 53 E 53 E 54 ATING MOMENT E 72 E 64 E 56 E 51 E 45 E 42 E 45 E 42 E 45 E 45 E 45 E 45 E 51 E 7 E 7 E 7 E 61 E 62 E 60 E 62 E 60 E 58 E 56 E 53 E 51 E 53 E 53 E 54 ATING MOMENT E 7 E 64 E 7 E 60 E 62 E 60 E 58 E 56 E 53 E 51 E 53 E 54 ATING MOMENT E 7 E 64 E 7 E 7 E 60 E 58 E 56 E 53 E 51 E 53 E 54 ATING MOMENT E 7 E 64 E 56 E 57 E 53 E 54 ATING MOMENT E 72 E 64 E 55 E 55 E 53 E 54 ATING MOMENT E 72 E 64 E 55 E 53 E 54 ATING MOMENT E 72 E 64 E 55 E 51 E 72 E 64 E 55 E 51 E 72 E 64 E 55 E 53 E 51 E 72 E 64 E 55 E 51 E 72 E 64 E 55 E 53 E 51 E 72 E 64 E 55 E 51 E 72 E 64 E 55 E 51 E 72 E 64 E 55 E 73 E 72 E 64 E 75 E 77 E 72 E 64 E 75 E 77 E 7	2 Max. Shear (KP) 66 93 106 112 116 119 126 139 153 164 180 192 201 208 213 201 208 213 201 208 213 213 71 101 125 151 178 196 209 219 226	63k Single Triple Max. Moment (FF & 2) 82 164 320 479 639 799 959 1123 1288 1458 2020 2660 3299 3938 4577 3938 3938 4577 3938 4577 3938 3938 4577 3938 4577 3938 4577 3938 4577 3938 4577 3938 4577 3938 4577 3938 4577 3938 4577 3938 4577 3938 4577 4577 4578 4577 4579 4579 4579 4579 4579 4579 4579	Hopper car "E" R/ SHEAR E 66 E 62 E 53 E 45 E 41 E 38 E 36 E 37 E 38 E 37 E 38 E 37 E 32 E 30 E 28 er cars in line "E" R/ SHEAR E 71 E 68 E 62 E 61 E 63 E 62 E 60 E 55	TING MOMENT E 66 E 58 E 51 E 46 E 42 E 39 E 37 E 34 E 31 E 31 E 31 E 31 E 31 E 31 E 31 E 30 E 28 CTING MOMENT E 71 E 64 E 59 E 58 E 60 E 61 E 58 E 60 E 61 E 58 E 51 E 66 E 58 E 51 E 42 E 37 E 34 E 31 E 31 E 31 E 31 E 31 E 31 E 31 E 30 E 28 E 50 E 58 E 60 E 65 E 59 E 58 E 60 E 65 E 59 E 58 E 66 E 59 E 58 E 66 E 59 E 58 E 59 E 57 E 59 E 57 E 57 E 57 E 57 E 58 E 59 E 57 E 57 E 57 E 57 E 58 E 59 E 57 E 57	2( Max. Shear ( <i>ksb</i> ) 66 93 115 139 164 180 192 201 208 220 251 286 316 340	63k Triple Hoppe Max. Moment (FT xk) 82 164 320 559 822 1141 1461 1781 2104 2433 3090 3868 4888 6167	r Cars in lin "E" R SHEAR E 66 E 62 E 57 E 56 E 58 E 57 E 56 E 53 E 51 E 50 E 51 E 51 E 51 E 49	e ATING E 66 E 58 E 51 E 54 E 54 E 54 E 56 E 56 E 56 E 56 E 53 E 51 E 48 E 45 E 45 E 45 E 45 E 45 E 45 E 46
(Feet) 5 10 25 30 40 45 50 60 70 80 90 100 5 10 15 20 25 30 35 40 45 5 10 15 20 25 30 35 40 45 5 5 5 5 5 5 5 5 5 5 5 5 5	Max. Shear ( <i>WP</i> ) 80 120 227 252 277 302 327 349 392 442 497 549 600 Max. Shear ( <i>WP</i> ) 80 120 160 200 227 252 277 252 277 302 327 349	Max. Moment (FF к//) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830 8638 10678 12893 Мах. Moment (FF к//) 225 500 825 100 225 500 825 1224 1642 2088 2623 3202 3804	Max. Shear (RBP) 71 101 127 155 181 198 211 220 232 248 285 325 325 325 325 325 325 325 32	286K AAR Ca Max. Moment (FT KB) 89 179 356 624 921 1269 1616 1964 2321 2678 3414 4396 5684 7075 8666 86k Single Triple Max. Moment (FT KB) 89 179 348 521 695 869 1043 1221 1400 1585	rs in line "F" R SHEAR E 71 E 68 E 63 E 62 E 64 E 63 E 64 E 63 E 61 E 58 E 57 E 58 E 57 E 58 E 59 E 57 E 58 E 59 E 57 E 56 E 56 F 56 E 57 E 56 E 56 E 56 E 57 E 56 E 56 E 56 E 56 E 56 E 58 E 49 E 45 E 41 E 40 E 41 E 41 E 41	ATING MOMENT E 71 E 64 E 57 E 61 E 60 E 62 E 62 E 60 E 58 E 56 E 53 E 51 E 53 E 53 E 54 ATING MOMENT E 72 E 64 E 56 E 57 E 64 E 57 E 42 E 64 E 57 E 53 E 54 ATING MOMENT E 72 E 64 E 55 E 53 E 54 ATING MOMENT E 72 E 64 E 56 E 53 E 54 ATING MOMENT E 72 E 64 E 56 E 55 E 53 E 54 ATING MOMENT E 72 E 64 E 56 E 55 E 53 E 54 ATING MOMENT E 72 E 64 E 56 E 55 E 53 E 54 ATING E 56 E 55 E 55 E 55 E 53 E 54 ATING E 56 E 56 E 56 E 56 E 56 E 57 E 53 E 54 ATING MOMENT E 72 E 64 E 56 E 55 E 55	2 Max. Shear (KP) 66 93 106 112 116 119 126 139 153 164 180 192 201 208 213 Max. Shear (KP) 71 101 125 151 178 196 209 219 226 239	63k Single Triple Max. Moment (FT KØ) 82 164 320 479 639 799 959 1123 1238 1458 2020 2660 3299 3293 4577 88 66 Triple Hoppe Max. Moment (FT KØ) 89 179 348 608 894 1241 1589 1936 22288 2646	Hopper car "E" R/ SHEAR E 66 E 62 E 53 E 45 E 41 E 38 E 36 E 37 E 38 E 37 E 38 E 37 E 35 E 32 E 30 E 28 er Cars in lind "E" R/ SHEAR E 71 E 68 E 62 E 61 E 63 E 62 E 63 E 62 E 63 E 62 E 63 E 62 E 55 E 55 E 55	MOMENT           E 66           E 58           E 51           E 46           E 39           E 37           E 34           E 31           E 30           E 28           WING           MOMENT           E 71           E 64           E 56           E 57           E 56	2( Max. Shear ( <i>ksb</i> ) 66 93 115 139 164 180 192 201 208 220 251 286 316 340	63k Triple Hoppe Max. Moment (FT xk) 82 164 320 559 822 1141 1461 1781 2104 2433 3090 3868 4888 6167	r Cars in lin "E" R SHEAR E 66 E 62 E 57 E 56 E 58 E 57 E 56 E 53 E 51 E 50 E 51 E 51 E 51 E 49	e ATING E 66 E 58 E 51 E 54 E 54 E 56 E 56 E 56 E 56 E 55 E 55 E 55 E 55
(Feet) 5 10 25 30 40 45 5 40 45 5 10 15 20 80 90 100 100 15 20 25 30 100 15 20 25 30 35 40 40 45 5 10 15 20 25 35 40 40 45 5 10 15 20 25 35 40 40 45 5 10 15 15 15 15 15 15 15 15 15 15	Max. Shear (XFP) 80 120 200 227 252 277 302 327 349 392 442 497 549 600 Max. Shear (KFP) 80 120 160 200 227 252 277 302 277 302	Max. Moment (FF KP) 100 225 500 825 1224 1642 2088 2623 3202 3804 5196 6830 8638 10678 10678 10678 12893 PER E80 Max. Moment (FF KP) 100 225 500 825 1224 1642 2088 2623 3202	Мах. Shear (КВР) 71 101 127 155 181 198 211 220 232 248 285 325 325 325 325 325 325 325 325 325 32	286К ААК Са Мах. Moment (Ff Ki) 89 179 356 624 921 1269 1616 1964 2321 2678 3414 4396 5684 7075 8666 8665 8666 8665 879 179 348 521 695 869 1043 1221 1400	rs in line WE WR SHEAR E 71 E 68 E 63 E 63 E 64 E 63 E 64 E 63 E 64 E 63 E 64 E 57 E 57 E 57 E 57 E 58 E 59 E 57 E 56 E 56 E 71 E 68 E 71 E 68 E 49 E 45 E 41 E 40 E 41 E 40 E 41 E 41	ATING MOMENT E 71 E 64 E 57 E 61 E 60 E 62 E 60 E 58 E 56 E 53 E 51 E 53 E 53 E 53 E 54 ATING MOMENT E 72 E 64 E 56 E 51 E 45 E 42 E 45 E 42 E 45 E 45 E 45 E 45 E 51 E 7 E 7 E 7 E 61 E 62 E 60 E 62 E 60 E 58 E 56 E 53 E 51 E 53 E 53 E 54 ATING MOMENT E 7 E 64 E 7 E 60 E 62 E 60 E 58 E 56 E 53 E 51 E 53 E 54 ATING MOMENT E 7 E 64 E 7 E 7 E 60 E 58 E 56 E 53 E 51 E 53 E 54 ATING MOMENT E 7 E 64 E 56 E 57 E 53 E 54 ATING MOMENT E 72 E 64 E 55 E 55 E 53 E 54 ATING MOMENT E 72 E 64 E 55 E 53 E 54 ATING MOMENT E 72 E 64 E 55 E 51 E 72 E 64 E 55 E 51 E 72 E 64 E 55 E 53 E 51 E 72 E 64 E 55 E 51 E 72 E 64 E 55 E 53 E 51 E 72 E 64 E 55 E 51 E 72 E 64 E 55 E 51 E 72 E 64 E 55 E 73 E 72 E 64 E 75 E 77 E 72 E 64 E 75 E 77 E 7	2 Max. Shear (KP) 66 93 106 112 116 119 126 139 153 164 180 192 201 208 213 201 208 213 201 208 213 213 71 101 125 151 178 196 209 219 226	63k Single Triple Max. Moment (FF & 2) 82 164 320 479 639 799 959 1123 1288 1458 2020 2660 3299 3938 4577 3938 3938 4577 3938 4577 3938 3938 4577 3938 4577 3938 4577 3938 4577 3938 4577 3938 4577 3938 4577 3938 4577 3938 4577 3938 4577 3938 4577 4577 4578 4577 4579 4579 4579 4579 4579 4579 4579	Hopper car "E" R/ SHEAR E 66 E 62 E 53 E 45 E 41 E 38 E 36 E 37 E 38 E 37 E 38 E 37 E 32 E 30 E 28 er cars in line "E" R/ SHEAR E 71 E 68 E 62 E 61 E 63 E 62 E 60 E 55	TING MOMENT E 66 E 58 E 51 E 46 E 42 E 39 E 37 E 34 E 31 E 31 E 31 E 31 E 31 E 31 E 31 E 30 E 28 CTING MOMENT E 71 E 64 E 59 E 58 E 60 E 61 E 58 E 60 E 61 E 58 E 51 E 66 E 58 E 51 E 42 E 37 E 34 E 31 E 31 E 31 E 31 E 31 E 31 E 31 E 30 E 28 E 50 E 58 E 60 E 65 E 59 E 58 E 60 E 65 E 59 E 58 E 66 E 59 E 58 E 66 E 59 E 58 E 59 E 57 E 59 E 57 E 57 E 57 E 57 E 58 E 59 E 57 E 57 E 57 E 57 E 58 E 59 E 57 E 57	2( Max. Shear ( <i>ksb</i> ) 66 93 115 139 164 180 192 201 208 220 251 286 316 340	63k Triple Hoppe Max. Moment (FT xk) 82 164 320 559 822 1141 1461 1781 2104 2433 3090 3868 4888 6167	r Cars in lin "E" R SHEAR E 66 E 62 E 57 E 56 E 58 E 57 E 56 E 53 E 51 E 50 E 51 E 51 E 51 E 49	e ATING E 66 E 58 E 51 E 54 E 54 E 56 E 56 E 56 E 56 E 55 E 55 E 55 E 55

Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

344

369 400



80

90 100 497

549 600 8638

10678 12893 218

226 232 3587

4283 4978 E35

E 33 E 31 E 33

E 32 E 31 5316

6706 8096 E 55

E 54

E 53

E 49

E 50 E 50

#### CONNECTICUT RAILROAD LINE TABLES

The Following Tables are a list the rail lines in Connecticut and their load restrictions. The Load restriction is the restriction of the maximum weight allowed on the line.

CONNECTICUT RAILROAD LINES LOAD RESTRICTIONS						
Owner	Connecticut Rail	Weight Limits				
Owner	Lines	(pounds)				
CTDOT	New Haven Line	263,000				
CTDOT	New Canaan Branch	263,000				
CTDOT	Danbury Branch	263,000				
CTDOT	Waterbury Branch – Lower	263,000				
CTDOT	Waterbury Branch – Upper	263,000				
Amtrak	Springfield Line (Hartford – Enfield)	263,000				
Amtrak	Springfield Line (New Haven – Hartford)	263,000				
Amtrak	Shore Line (Groton – Stonington)	286,000				
Amtrak	Shore Line (New Haven – Groton)	286,000				
CTDOT	Berkshire Line – North Section	286,000				
HRRC	Berkshire Line – South Section	286,000				
HRRC	Maybrook Line	286,000				
CTDOT	Torrington Branch	263,000				
CTDOT	Terryville Secondary – South Section	263,000				
STRR	Terryville Secondary – North Section	263,000				
CSX	Lower Middletown Secondary	263,000				
CSX	Stratford Industrial Track	263,000				
CSOR	Armory (East Windsor Sec.) – South Section	286,000*				
CSOR	Manchester Secondary	$286,000^{*}$				
CSOR	Suffield Branch	$286,000^*$				

#### Table B: Load Restrictions as of June 2011

Weight Limits apply to maximum gross weight of a freight car with a configuration conforming to AAR standards for unrestricted interchange. Weight limits are not directly applicable for locomotives which require individual analysis.

Requires bridge upgrades on adjacent rail lines



OwnerConnecticut Rail LinesWeight Limits (pounds)CTDOTBradley Spur286,000*PWRRMiddletown Secondary – Middle263,000CTDOTMiddletown Secondary – Upper263,000PWRRPlainfield Secondary (Norwich)286,000 ModifiedCTDOTWillimantic Secondary – West Section286,000 ModifiedPWRRWillimantic Secondary – East Section263,000PWRRWillimantic Secondary – East Section263,000PWRRWillimantic Secondary – East Section263,000PWRRBranford Line263,000CTDEPValley Line263,000NECRPalmer Line263,000CTDOTArmory (East Windsor Sec.) – North Section286,000*CTDOTGriffins Industrial Track286,000*	CONNECTICUT RAILROAD LINES LOAD RESTRICTIONS							
CTDOTBradley Spur286,000*PWRRMiddletown Secondary – Middle263,000CTDOTMiddletown Secondary – Upper263,000PWRRPlainfield Secondary (Norwich)286,000 ModifiedCTDOTWillimantic Secondary – West Section286,000 ModifiedPWRRWillimantic Secondary – East Section263,000PWRRWillimantic Secondary – Upper263,000PWRRWillimantic Secondary – Upper263,000PWRRBranford Line263,000PWRRBranford Line263,000CTDEPValley Line263,000NECRPalmer Line263,000CTDOTArmory (East Windsor Sec.) – North Section286,000*	Owner	Connecticut Rail	Weight Limits					
PWRRMiddletown Secondary – Middle263,000CTDOTMiddletown Secondary – Upper263,000PWRRPlainfield Secondary (Norwich)286,000 ModifiedCTDOTWillimantic Secondary – West Section263,000PWRRWillimantic Secondary – East Section263,000PWRRBSRRBranford Line263,000BSRRBranford Line263,000NECRPalmer Line263,000CTDOTArmory (East Windsor Sec.) – North Section286,000*	Owner	Lines	(pounds)					
PWRRMiddle263,000CTDOTMiddletown Secondary – Upper263,000PWRRPlainfield Secondary (Norwich)286,000 ModifiedCTDOTWillimantic Secondary – West Section263,000PWRRWillimantic Secondary – East Section263,000PWRRWillimantic Secondary – East Section263,000BSRRBranford Line263,000CTDEPValley Line263,000NECRPalmer Line263,000CTDOTArmory (East Windsor Sec.) – North Section286,000*	CTDOT	Bradley Spur	286,000*					
CTDOTUpper263,000PWRRPlainfield Secondary (Norwich)286,000 ModifiedCTDOTWillimantic Secondary – West Section263,000PWRRWillimantic Secondary – East Section263,000BSRRBranford Line263,000CTDEPValley Line263,000NECRPalmer Line263,000CTDOTArmory (East Windsor Sec.) – North Section286,000*	PWRR	5	263,000					
PWRR(Norwich)286,000 MiddlifedCTDOTWillimantic Secondary – West Section263,000PWRRWillimantic Secondary – East Section263,000BSRRBranford Line263,000CTDEPValley Line263,000NECRPalmer Line263,000CTDOTArmory (East Windsor Sec.) – North Section286,000*	CTDOT	5	263,000					
CTDO1West Section263,000PWRRWillimantic Secondary – East Section263,000BSRRBranford Line263,000CTDEPValley Line263,000NECRPalmer Line263,000CTDOTArmory (East Windsor Sec.) – North Section286,000*	PWRR	5	286,000 Modified					
PWRREast Section263,000BSRRBranford Line263,000CTDEPValley Line263,000NECRPalmer Line263,000CTDOTArmory (East Windsor Sec.) – North Section286,000*	CTDOT	5	263,000					
CTDEPValley Line263,000NECRPalmer Line263,000CTDOTArmory (East Windsor Sec.) - North Section286,000*	PWRR	5	263,000					
NECRPalmer Line263,000CTDOTArmory (East Windsor Sec.) - North Section286,000*	BSRR	Branford Line	263,000					
CTDOT Armory (East Windsor Sec.) – North Section 286,000*	CTDEP	Valley Line	263,000					
Sec.) – North Section 286,000	NECR	Palmer Line	263,000					
CTDOT Griffins Industrial Track 286,000 <sup>*</sup>	CTDOT	5	286,000*					
	CTDOT	Griffins Industrial Track	286,000*					

Table B cont.: Load Restrictions as of June 2011

Weight limits apply to maximum gross weight of a freight car with a configuration conforming to AAR standards for unrestricted interchange. Weight limits are not directly applicable for locomotives which require individual analysis.

Requires bridge upgrades on adjacent rail lines

Railroad Owner - Abbreviation List

- CTDOT State of Connecticut Department of Transportation
- AMTRAK National Railroad Passenger Corporation
- CSX CSX Transportation
- PWRR Providence and Worcester Railroad Company
- NECR New England Central Railroad<sup>1</sup>
- CSOR Connecticut Southern Railroad <sup>1</sup>
- HRRC Housatonic Railroad Company
- CNZR Central New England Railroad
- NRR Naugatuck Railroad Company
- STRR Springfield Terminal Railway
- BSRR Branford Steam Railroad
- CTDEP State of Connecticut Department of Environmental Protection

1 RailAmerica Incorporated is a holding company that owns and operates freight railroads. They have two (2) subsidiaries that operate in Connecticut: CSOR and NECR.

LOCHNER

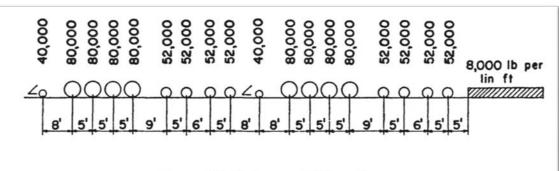


Figure 15-1-2. Cooper E 80 Load

## Table 15-1-15. Maximum Moments, Shears and Pier (or Floorbeam) Reactions for Cooper E 80 Live Load or Alternate Live Load All Values are for one rail (one-half track load)

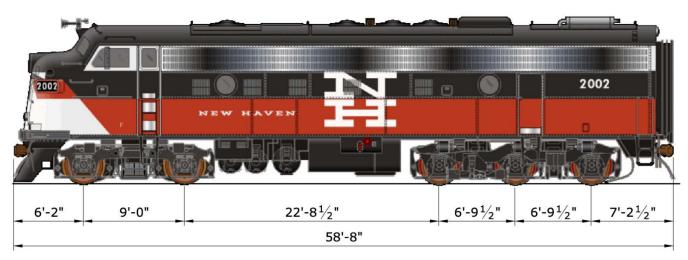
Span Length	Maxim Mome		Maxir Morr	nent	Kips Pier R							num action
Ft	Ft–Kips		Quarter Point Ft–Kips		At End		At Quarter Point		At Center		Kips (2)	
	E-80	Alt.	E-80	Alt.	E-80	Alt.	E-80	Alt.	E-80	Alt.	E-80	Alt.
5	50.00	62.50	37.50	46.88	40.00	50.00			20.00	25.00	40.00	50.00
6	60.00	75.00	45.00	56.25	46.67	58.33	30.00	37.50	20.00	25.00	53.33	58.33
7	70.00	87.50	55.00	68.75	51.43	64.29	31.43	39.29	20.00	25.00	62.86	71.43
8	80.00	100.00	70.00	87.50	55.00	68.75	35.00	43.75	20.00	25.00	70.00	81.25
9	93.89	117.36	85.00	106.25	57.58	72.22	37.78	47.23	20.00	25.00	75.76	88.89
10	112.50	140.63	100.00	125.00	60.00	75.00	40.00	50.00	20.00	25.00	80.00	95.00
11	131.36	164.20	115.00	143.75	65.45	77.27	41.82	52.28	21.82	27.28	87.28	100.00
12	160.00	188.02	130.00	162.50	70.00	83.33	43.33	54.17	23.33	29.17	93.33	108.33
13	190.00	212.83	145.00	181.25	73.84	88.46	44.61	55.76	24.61	30.76	98.46	115.39
14	220.00	250.30	165.00	200.00	77.14	92.86	47.14	57.14	25.71	32.14	104.29	121.43
16	280.00	325.27	210.00	250.00	85.00	100.00	52.50	62.50	27.50	34.38	113.74	131.25
18	340.00	400.24	255.00	318.79	93.33	111.11	56.67	68.05	28.89	36.11	121.33	138.89
20	412.50	475.00	300.00	362.50	100.00	120.00	60.00	72.50	28.70	37.50	131.10	145.00
24	570.42	668.75	420.00	500.00	110.83	133.33	70.00	83.33	31.75	41.67	147.92	154.17
28	730.98	866.07	555.00	650.00	120.86	142.86	77.14	92.86	34.29	46.43	164.58	
32	910.85	1064.06	692.50	800.00	131.44	150.00	83.12	100.00	37.50	50.00	181.94	
36	1097.30	1262.50	851.50	950.00	141.12	155.56	88.90	105.56	41.10	55.56	199.06	
40	1311.3	1461.25	1010.50	1100.00	150.80	160.00	93.55	110.00	44.00	60.00	215.90	
45	1601.2	1710.00	1233.60	1287.48	163.38	164.44	100.27	114.45	45.90	64.45	237.25	
50	1901.80	1959.00	1473.00	1481.05	174.40		106.94	118.42	49.73	68.00	257.52	
55	2233.10		1732.30		185.31		113.58	120.91	52.74	70.91	280.67	
60	2597.80		2010.00		196.00		120.21	123.33	55.69	73.33	306.42	
70	3415.00		2608.20		221.04		131.89		61.45	77.14	354.08	
80	4318.90		3298.00		248.40		143.41		67.41	80.00	397.70	
90	5339.10		4158.00		274.46		157.47		73.48	82.22	437.15	
100	6446.30		5060.50		300.00		173.12		78.72	84.00	474.24	
120	9225.40		7098.00		347.35		202.19		88.92	01.00	544.14	
120	12406.00		9400.00		392.59		230.23		101.64		614.91	

#### Page 12

Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts

## FL9m LOCOMOTIVE

WEIGHT: 282,000 LBS GROSS



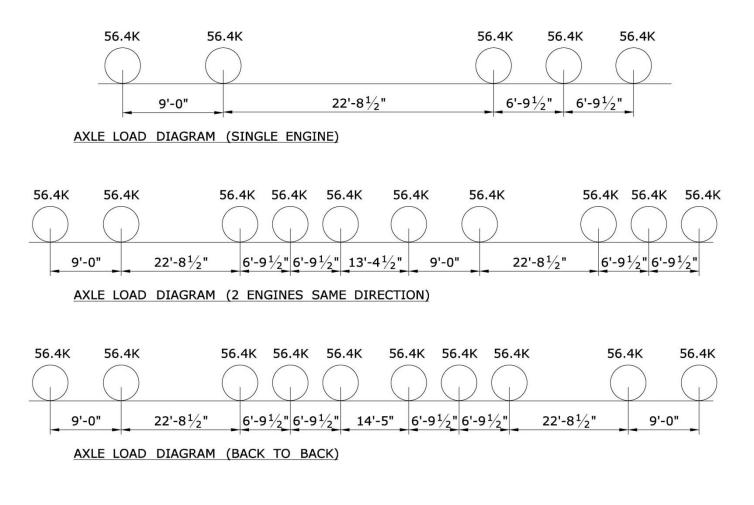


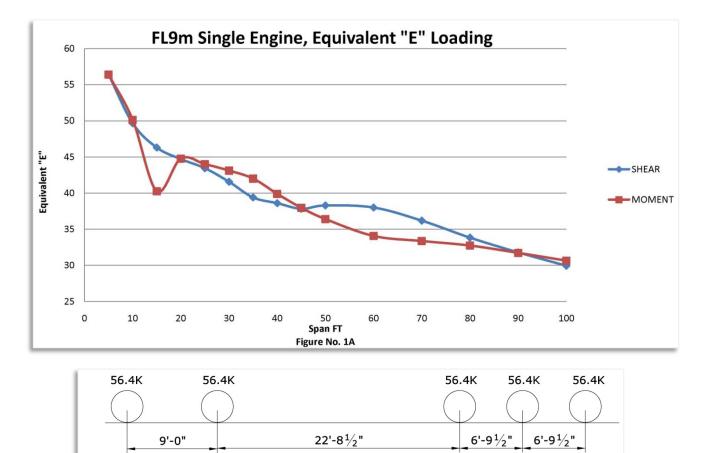
FIGURE NO. 1



FL9m Single Engine Equivalent Cooper Load						
SPAN FT	SHEAR	MOMENT				
5	E 56.39	E 56.40				
10	E 49.66	E 50.13				
15	E 46.29	E 40.26				
20	E 44.69	E 44.78				
25	E 43.43	E 44.00				
30	E 41.55	E 43.11				
35	E 39.38	E 42.00				
40	E 38.59	E 39.89				
45	E 37.82	E 37.96				
50	E 38.28	E 36.40				
60	E 37.99	E 34.06				
70	E 36.17	E 33.36				
80	E 33.82	E 32.73				
90	E 31.78	E 31.72				
100	E 29.93	E 30.64				
	Table No. 4A					

FL9m Single	FL9m Single Engine Equivalent Cooper Load								
SPAN FT	SHEAR	MOMENT							

Table No. 1A



Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

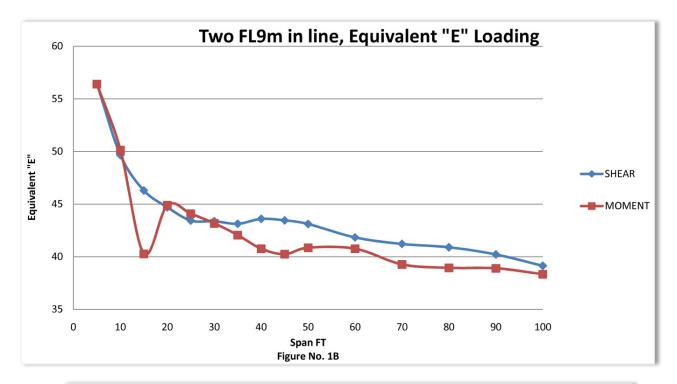
AXLE LOAD DIAGRAM (SINGLE ENGINE)

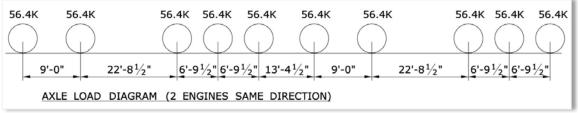
Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts

I wo FL9m in line Equivalent Cooper Load					
SPAN FT	SHEAR	MOMENT			
5	E 56.39	E 56.40			
10	E 49.66	E 50.13			
15	E 46.29	E 40.26			
20	E 44.69	E 44.78			
25	E 43.43	E 44.00			
30	E 43.37	E 43.11			
35	E 43.12	E 42.00			
40	E 43.59	E 40.69			
45	E 43.45	E 40.20			
50	E 43.10	E 40.80			
60	E 41.83	E 40.72			
70	E 41.21	E 39.24			
80	E 40.89	E 38.93			
90	E 40.21	E 38.88			
100	E 39.13	E 38.33			



Table No. 1B





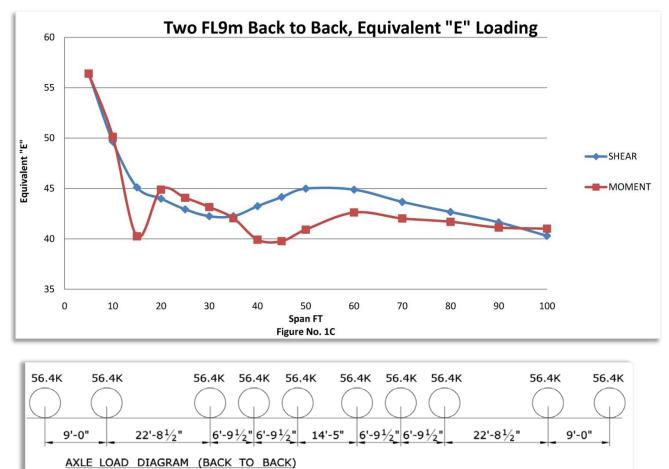
Load chart values are applicable to specific structural members. For limitations, see Executive Summary.



	Two TESHT Back to Back Equivalent Cooper Load					
SPAN FT	SHEAR	MOMENT				
5	E 56.39	E 56.40				
10	E 49.66	E 50.13				
15	E 45.12	E 40.26				
20	E 43.99	E 44.89				
25	E 42.94	E 44.08				
30	E 42.25	E 43.16				
35	E 42.26	E 42.05				
40	E 43.25	E 39.92				
45	E 44.15	E 39.78				
50	E 44.99	E 40.92				
60	E 44.89	E 42.62				
70	E 43.68	E 42.03				
80	E 42.67	E 41.70				
90	E 41.64	E 41.13				
100	E 40.31	E 41.02				
	Table No. 10					



Table No. 1C

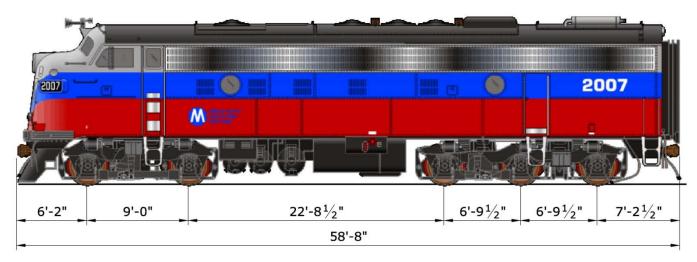


Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts

## **FL9 AC LOCOMOTIVE**

WEIGHT: 284,000 LBS GROSS



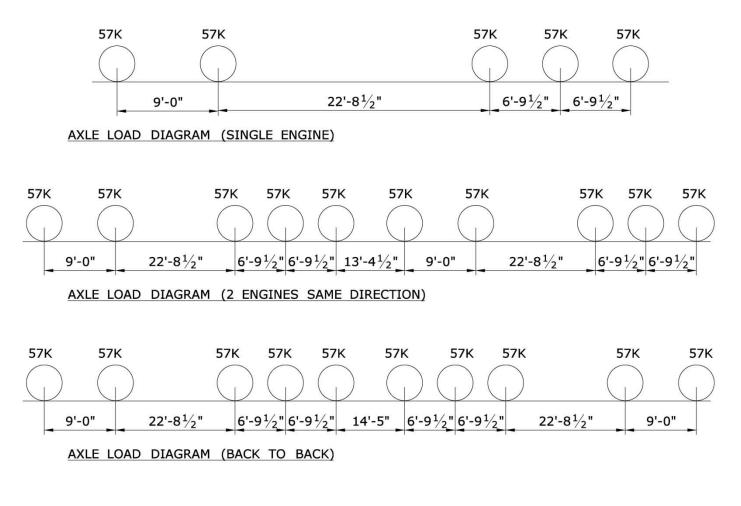


FIGURE NO. 2

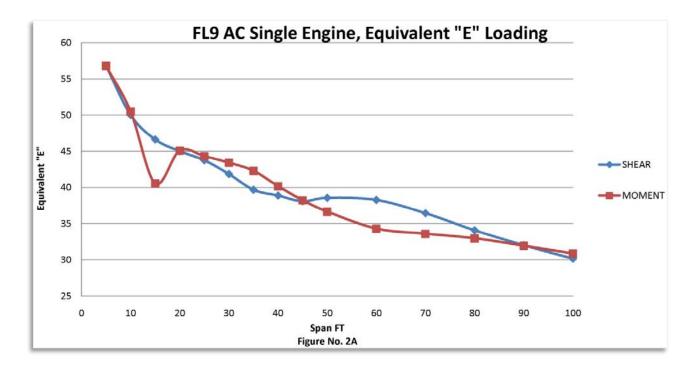


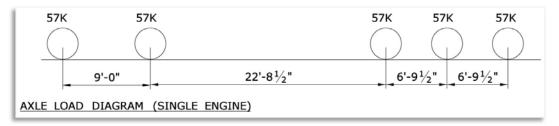
Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts

FL9AC Single Engine Equivalent Cooper Load						
SPAN FT	SHEAR	MOMENT				
5	E 56.79	E 56.80				
10	E 50.01	E 50.49				
15	E 46.62	E 40.55				
20	E 45.01	E 45.09				
25	E 43.74	E 44.32				
30	E 41.85	E 43.41				
35	E 39.66	E 42.30				
40	E 38.87	E 40.17				
45	E 38.08	E 38.23				
50	E 38.55	E 36.66				
60	E 38.26	E 34.30				
70	E 36.43	E 33.60				
80	E 34.06	E 32.96				
90	E 32.01	E 31.94				
100	E 30.14	E 30.86				
	Table No. 24					

FL9AC Single Engine Equivalent Cooper Load

Table No. 2A





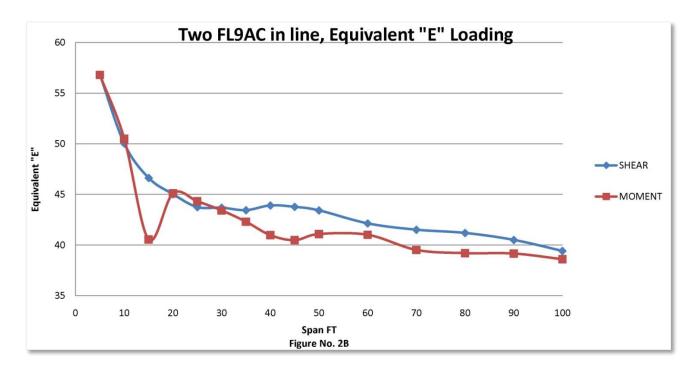
Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

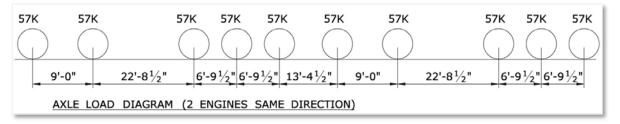
Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts

Two FL9AC III line Equivalent Cooper Load						
SPAN FT	SHEAR	MOMENT				
5	E 56.79	E 56.80				
10	E 50.01	E 50.49				
15	E 46.62	E 40.55				
20	E 45.01	E 45.10				
25	E 43.74	E 44.32				
30	E 43.67	E 43.41				
35	E 43.43	E 42.30				
40	E 43.90	E 40.97				
45	E 43.76	E 40.48				
50	E 43.41	E 41.09				
60	E 42.12	E 41.01				
70	E 41.50	E 39.52				
80	E 41.18	E 39.20				
90	E 40.49	E 39.16				
100	E 39.41	E 38.60				

#### Two FL9AC in line Equivalent Cooper Load

Table No. 2B





Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

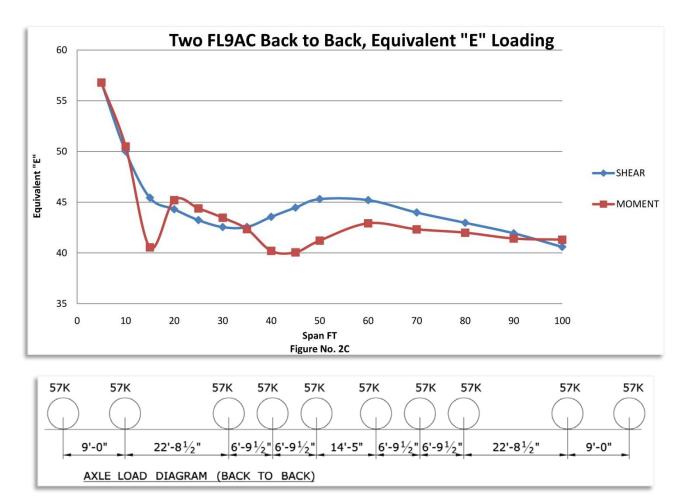


Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts

I WO FL9AC Back to Back Equivalent Cooper Load						
SPAN FT	SHEAR	MOMENT				
5	E 56.79	E 56.80				
10	E 50.01	E 50.49				
15	E 45.44	E 40.55				
20	E 44.31	E 45.21				
25	E 43.24	E 44.39				
30	E 42.55	E 43.47				
35	E 42.56	E 42.35				
40	E 43.56	E 40.20				
45	E 44.47	E 40.06				
50	E 45.31	E 41.21				
60	E 45.21	E 42.92				
70	E 43.99	E 42.33				
80	E 42.97	E 42.00				
90	E 41.94	E 41.42				
100	E 40.59	E 41.31				
	Table No. 20					

Two FL9AC Back to Back Equivalent Cooper Load

Table No. 2C

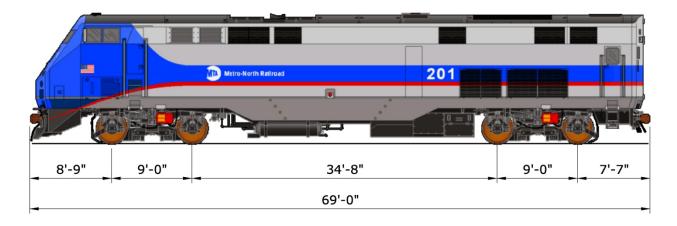


Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts

## **P32AC (GENESIS) LOCOMOTIVE**

WEIGHT: 278,000 LBS GROSS



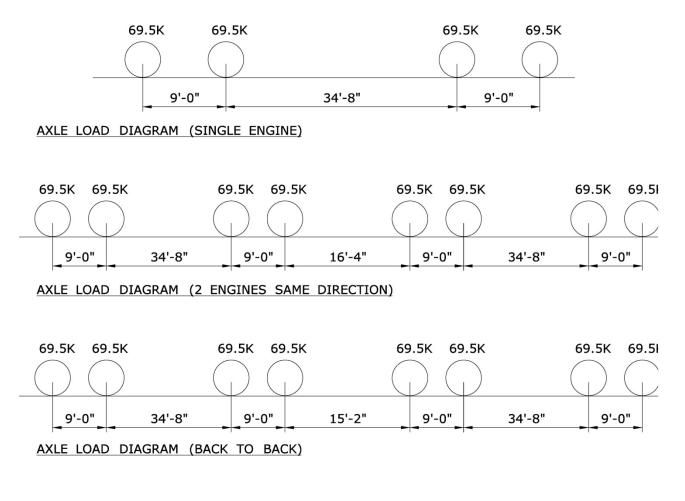


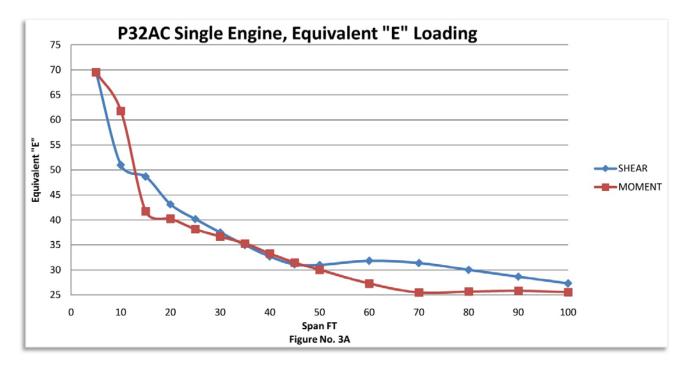
FIGURE NO. 3

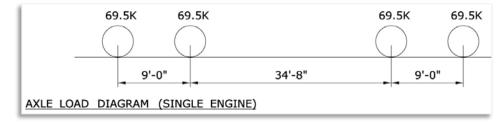


PSZAC single Engine Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 69.49	E 69.50
10	E 50.96	E 61.78
15	E 48.65	E 41.70
20	E 43.09	E 40.25
25	E 40.17	E 38.17
30	E 37.51	E 36.68
35	E 34.98	E 35.32
40	E 32.68	E 33.27
45	E 31.11	E 31.47
50	E 31.01	E 30.04
60	E 31.83	E 27.29
70	E 31.39	E 25.50
80	E 30.02	E 25.68
90	E 28.66	E 25.83
100	E 27.30	E 25.57
Table No. 24		



Table No. 3A





Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

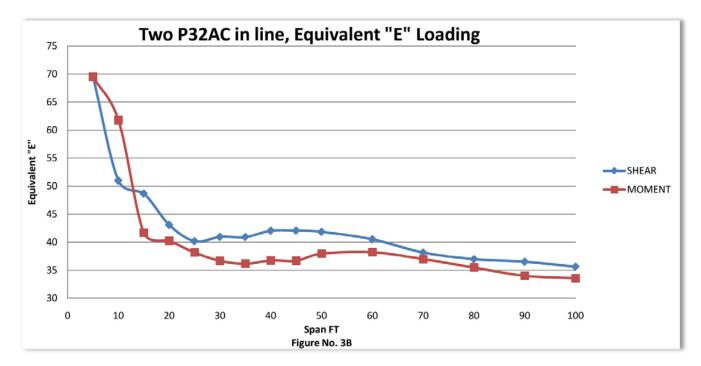
Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts

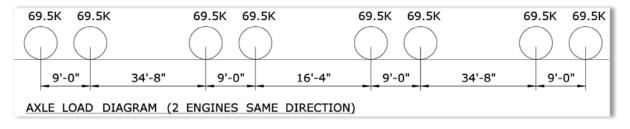


Two PSZAC III III e Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 69.49	E 69.50
10	E 50.96	E 61.78
15	E 48.65	E 41.70
20	E 43.09	E 40.25
25	E 40.17	E 38.17
30	E 40.94	E 36.68
35	E 40.91	E 36.17
40	E 42.04	E 36.75
45	E 42.07	E 36.71
50	E 41.85	E 37.97
60	E 40.50	E 38.23
70	E 38.16	E 36.99
80	E 36.96	E 35.49
90	E 36.50	E 34.02
100	E 35.62	E 33.57



Table No. 3B





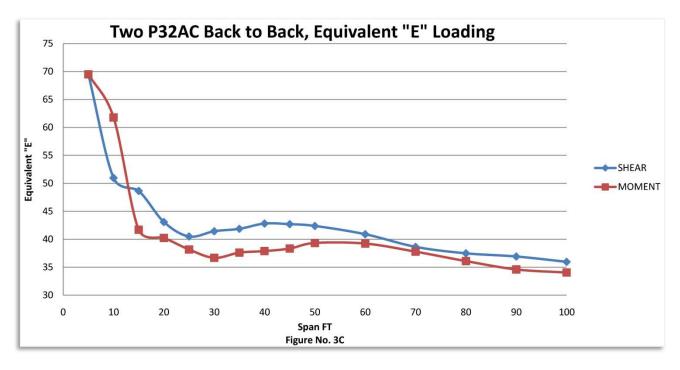
Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

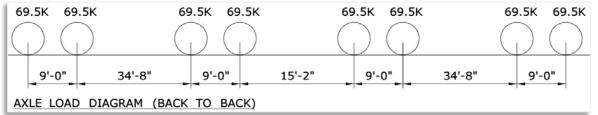


Two P32AC Back to Back Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 69.49	E 69.50
10	E 50.96	E 61.78
15	E 48.65	E 41.70
20	E 43.09	E 40.25
25	E 40.50	E 38.17
30	E 41.43	E 36.68
35	E 41.87	E 37.61
40	E 42.81	E 37.89
45	E 42.70	E 38.35
50	E 42.38	E 39.34
60	E 40.90	E 39.24
70	E 38.64	E 37.79
80	E 37.48	E 36.10
90	E 36.91	E 34.60
100	E 35.95	E 34.05
Table No. 20		

Two P32AC Back to Back Equivalent Cooper Load

Table No. 3C





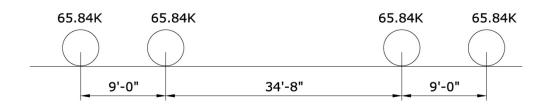
Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts

# P40DC/P42DC (GENESIS) LOCOMOTIVE

WEIGHT: 263,340 LBS GROSS





#### AXLE LOAD DIAGRAM (SINGLE ENGINE)



AXLE LOAD DIAGRAM (2 ENGINES SAME DIRECTION)

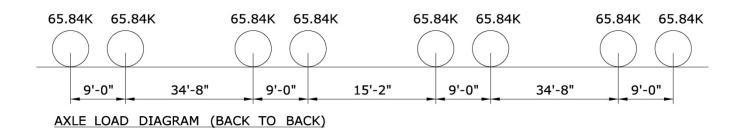


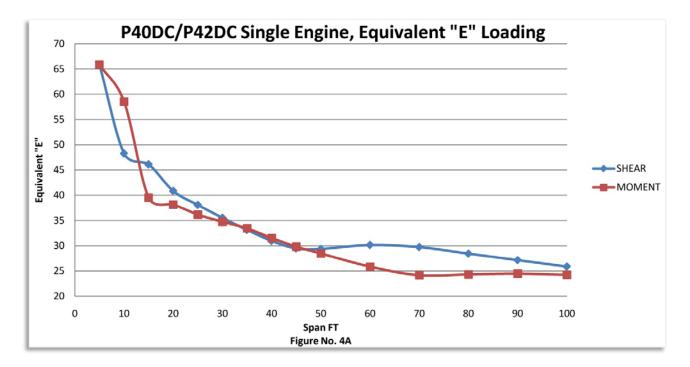
FIGURE NO. 4

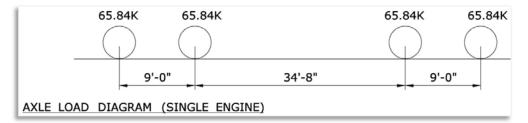


P40DC/P42DC Single Engine Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 65.83	E 65.84
10	E 48.27	E 58.52
15	E 46.08	E 39.50
20	E 40.82	E 38.13
25	E 38.05	E 36.16
30	E 35.53	E 34.75
35	E 33.14	E 33.46
40	E 30.95	E 31.51
45	E 29.47	E 29.81
50	E 29.38	E 28.46
60	E 30.15	E 25.85
70	E 29.73	E 24.16
80	E 28.43	E 24.32
90	E 27.14	E 24.47
100	E 25.86	E 24.23
Table No. 44		

P40DC/P42DC Single Engine Equivalent Cooper Load

Table No. 4A





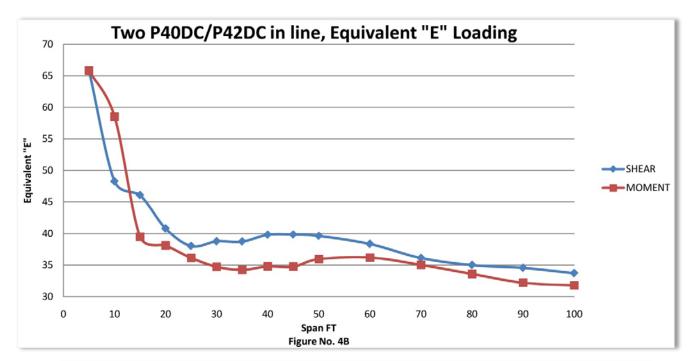
Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

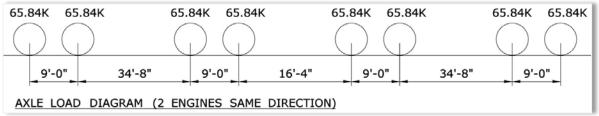
Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts

Two P40DC/P42DC III III e Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 65.83	E 65.84
10	E 48.27	E 58.52
15	E 46.08	E 39.50
20	E 40.82	E 38.13
25	E 38.05	E 36.16
30	E 38.78	E 34.75
35	E 38.75	E 34.27
40	E 39.82	E 34.81
45	E 39.85	E 34.77
50	E 39.64	E 35.96
60	E 38.37	E 36.21
70	E 36.15	E 35.04
80	E 35.02	E 33.62
90	E 34.57	E 32.23
100	E 33.74	E 31.80

Two P40DC/P42DC in line Equivalent Cooper Load

Table No. 4B





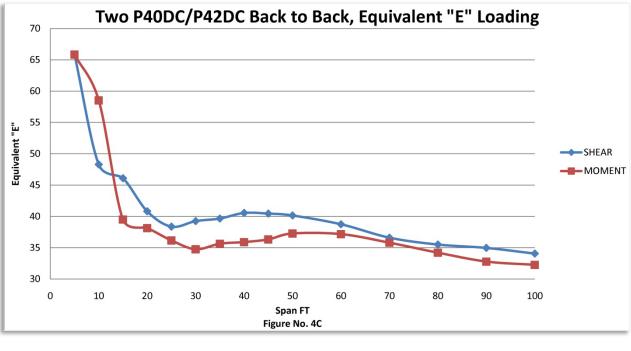
Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

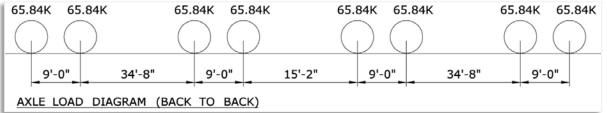


	Two F40DC/F42DC back to back Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT	
5	E 65.83	E 65.84	
10	E 48.27	E 58.52	
15	E 46.08	E 39.50	
20	E 40.82	E 38.13	
25	E 38.36	E 36.16	
30	E 39.25	E 34.75	
35	E 39.66	E 35.63	
40	E 40.55	E 35.89	
45	E 40.45	E 36.32	
50	E 40.14	E 37.27	
60	E 38.74	E 37.17	
70	E 36.60	E 35.79	
80	E 35.50	E 34.20	
90	E 34.96	E 32.78	
100	E 34.06	E 32.25	
	Table No. 40		

Two P40DC/P42DC Back to Back Equivalent Cooper Load

Table No. 4C



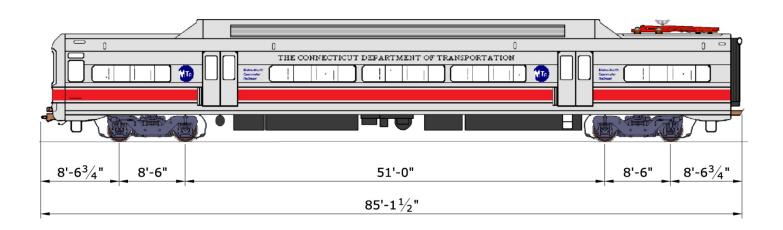


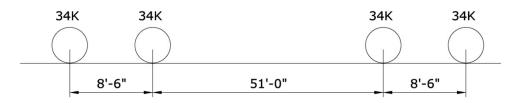
Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts

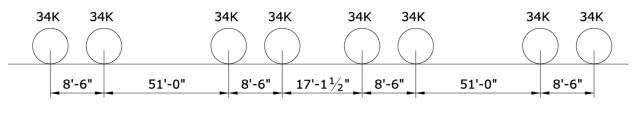
### **METRO-NORTH M-2 COACH**

WEIGHT: 136,000 LBS GROSS (MAXIMUM)





AXLE LOAD DIAGRAM (SINGLE ENGINE)



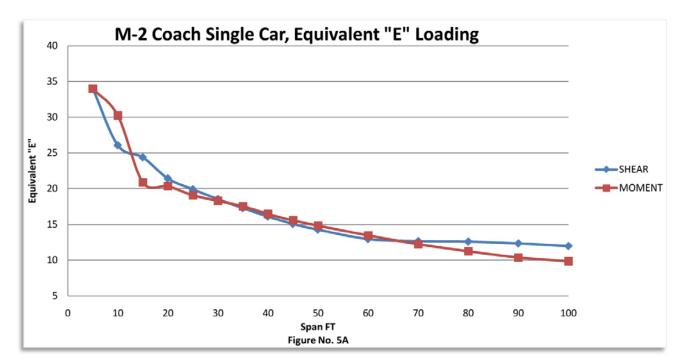
AXLE LOAD DIAGRAM (2 CARS)

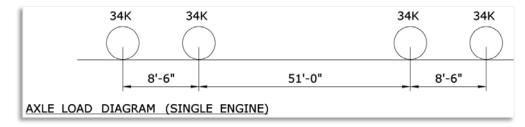
#### FIGURE NO. 5

M-2 Coach Single Car Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 34.00	E 34.00
10	E 26.06	E 30.22
15	E 24.36	E 20.85
20	E 21.42	E 20.33
25	E 19.89	E 19.07
30	E 18.53	E 18.29
35	E 17.25	E 17.53
40	E 16.10	E 16.46
45	E 15.06	E 15.57
50	E 14.26	E 14.84
60	E 12.95	E 13.48
70	E 12.66	E 12.25
80	E 12.59	E 11.26
90	E 12.33	E 10.38
100	E 11.97	E 9.85
Table No. 54		

M-2 Coach Single Car Equivalent Cooper Load

Table No. 5A





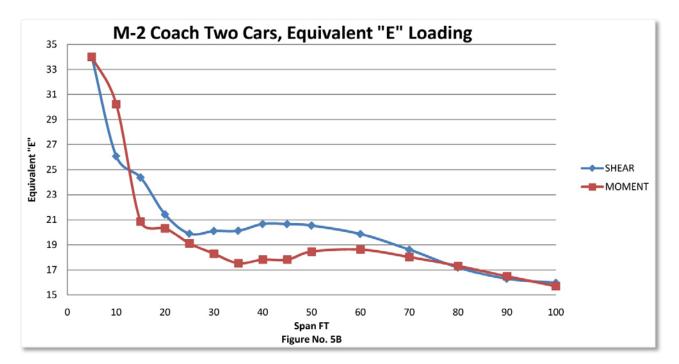
Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

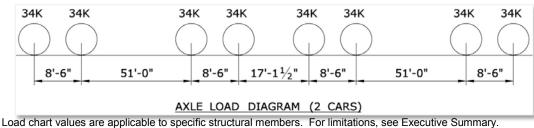
Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts

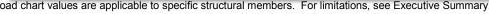
wi-2 Coach Two Cars Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 34.00	E 34.00
10	E 26.06	E 30.22
15	E 24.36	E 20.85
20	E 21.42	E 20.33
25	E 19.89	E 19.12
30	E 20.10	E 18.29
35	E 20.13	E 17.53
40	E 20.66	E 17.82
45	E 20.66	E 17.82
50	E 20.54	E 18.46
60	E 19.86	E 18.62
70	E 18.61	E 18.03
80	E 17.19	E 17.31
90	E 16.29	E 16.49
100	E 15.97	E 15.70
Table Na ED		



Table No. 5B



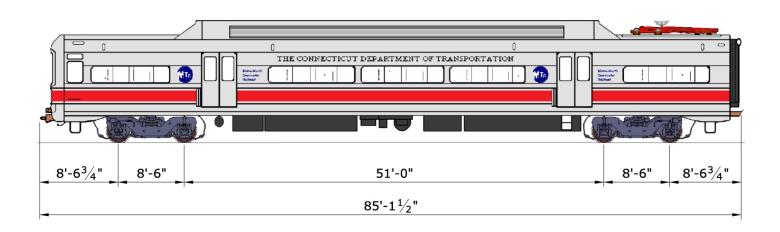


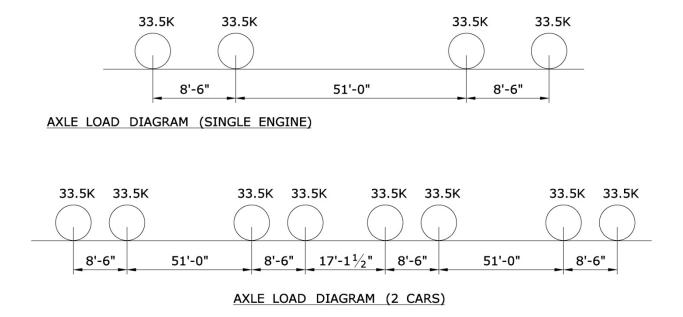




### **METRO-NORTH M-4 COACH**

WEIGHT: 134,000 LBS GROSS (MAXIMUM)





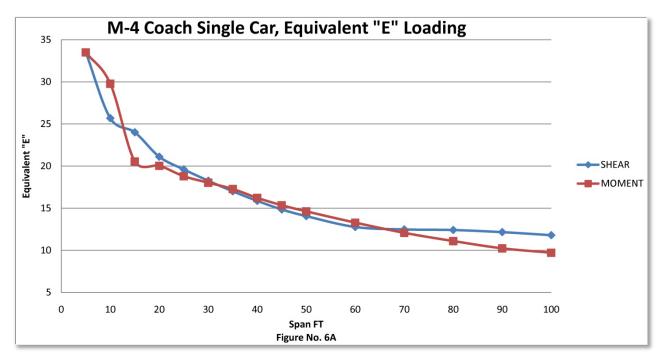
#### FIGURE NO. 6

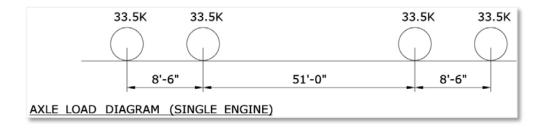
Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts

IVI-4 Coach Single Car Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 33.50	E 33.50
10	E 25.68	E 29.78
15	E 24.01	E 20.55
20	E 21.10	E 20.03
25	E 19.60	E 18.79
30	E 18.26	E 18.02
35	E 17.00	E 17.27
40	E 15.86	E 16.22
45	E 14.84	E 15.34
50	E 14.05	E 14.62
60	E 12.76	E 13.28
70	E 12.47	E 12.07
80	E 12.40	E 11.09
90	E 12.15	E 10.23
100	E 11.79	E 9.70

M-4 Coach Single Car Equivalent Cooper Load

Table No. 6A





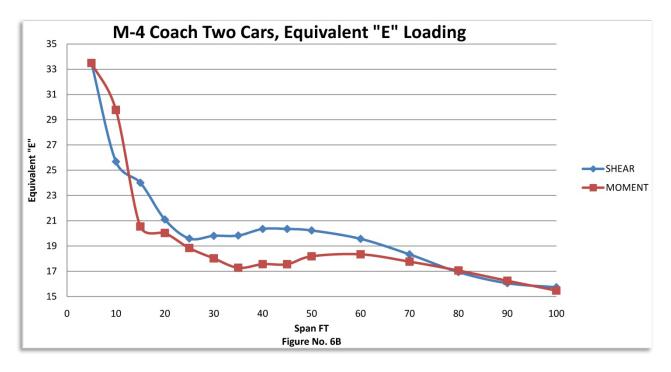
Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

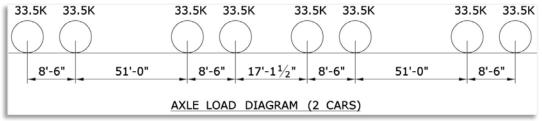


IVI-4 Coach Two Cars Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 33.50	E 33.50
10	E 25.68	E 29.78
15	E 24.01	E 20.55
20	E 21.10	E 20.03
25	E 19.60	E 18.84
30	E 19.81	E 18.02
35	E 19.83	E 17.27
40	E 20.35	E 17.56
45	E 20.35	E 17.56
50	E 20.23	E 18.19
60	E 19.57	E 18.34
70	E 18.34	E 17.76
80	E 16.94	E 17.06
90	E 16.06	E 16.25
100	E 15.73	E 15.47
Table No. CD		

M-4 Coach Two Cars Equivalent Cooper Load

Table No. 6B



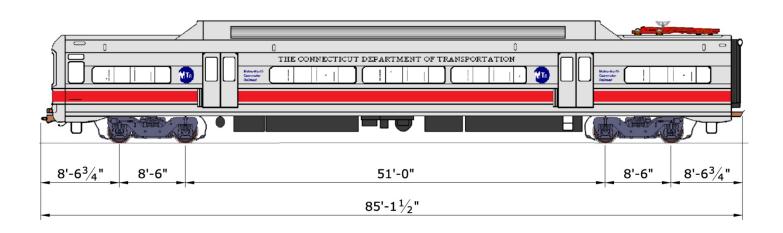


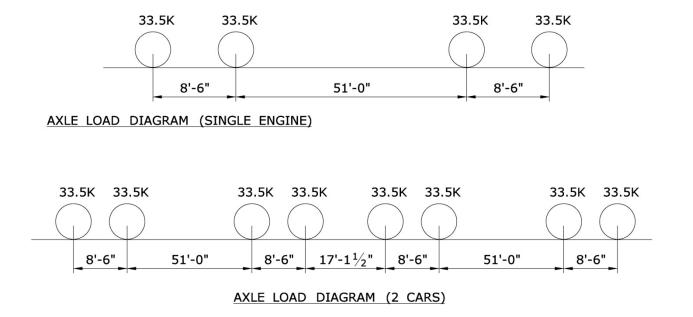
Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts

### **METRO-NORTH M-6 COACH**

WEIGHT: 134,000 LBS GROSS (MAXIMUM)



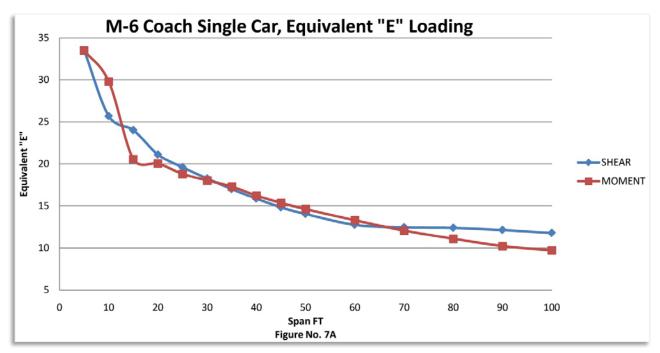


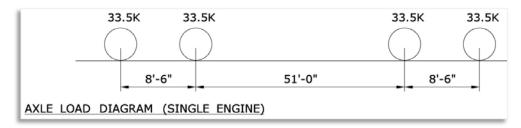
#### FIGURE NO. 7

SPAN FT	SHEAR	MOMENT
5	E 33.50	E 33.50
10	E 25.68	E 29.78
15	E 24.01	E 20.55
20	E 21.10	E 20.03
25	E 19.60	E 18.79
30	E 18.26	E 18.02
35	E 17.00	E 17.27
40	E 15.86	E 16.22
45	E 14.84	E 15.34
50	E 14.05	E 14.62
60	E 12.76	E 13.28
70	E 12.47	E 12.07
80	E 12.40	E 11.09
90	E 12.15	E 10.23
100	E 11.79	E 9.70
Table No. 7A		



Table No. 7A





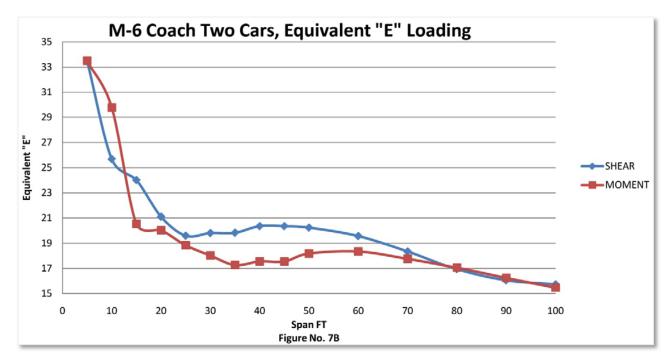
Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

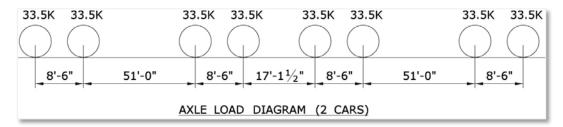
Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts

IVI-6 Coach	I wo Cars Equivalent Cooper Load	
SPAN FT	SHEAR	MOMENT
5	E 33.50	E 33.50
10	E 25.68	E 29.78
15	E 24.01	E 20.55
20	E 21.10	E 20.03
25	E 19.60	E 18.84
30	E 19.81	E 18.02
35	E 19.83	E 17.27
40	E 20.35	E 17.56
45	E 20.35	E 17.56
50	E 20.23	E 18.19
60	E 19.57	E 18.34
70	E 18.34	E 17.76
80	E 16.94	E 17.06
90	E 16.06	E 16.25
100	E 15.73	E 15.47



Table No. 7B



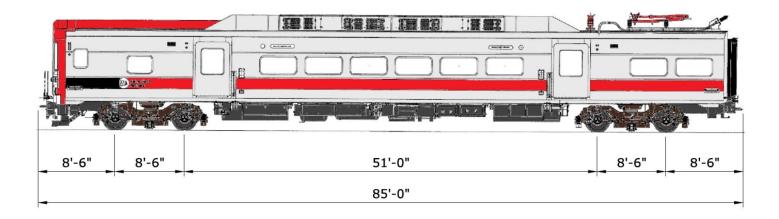


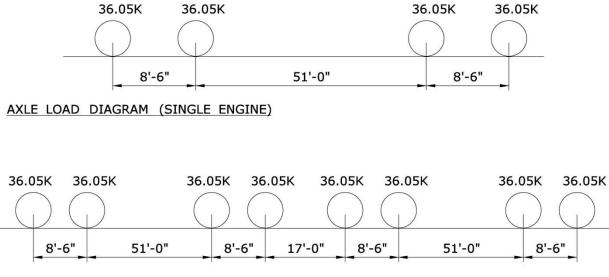
Load chart values are applicable to specific structural members. For limitations, see Executive Summary.



### **METRO-NORTH M-8 COACH**

WEIGHT: 144,200 LBS GROSS (MAXIMUM)





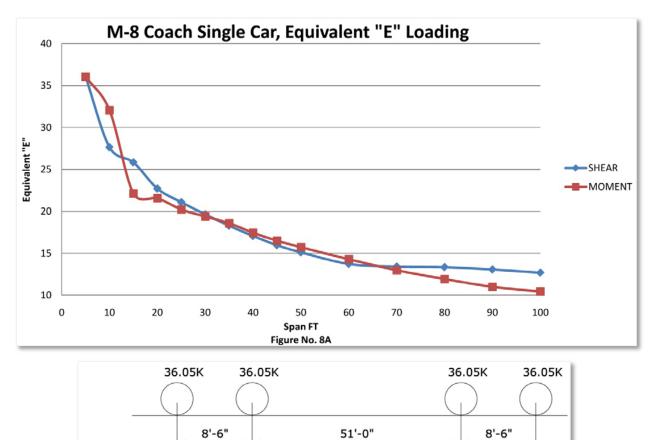
AXLE LOAD DIAGRAM (2 CARS)



IVI-8 Coach Single Car Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 36.05	E 36.05
10	E 27.64	E 32.04
15	E 25.83	E 22.11
20	E 22.71	E 21.56
25	E 21.09	E 20.22
30	E 19.65	E 19.39
35	E 18.29	E 18.59
40	E 17.07	E 17.45
45	E 15.97	E 16.51
50	E 15.12	E 15.73
60	E 13.73	E 14.29
70	E 13.42	E 12.98
80	E 13.35	E 11.94
90	E 13.07	E 11.01
100	E 12.69	E 10.44

M-8 Coach Single Car Equivalent Cooper Load

Table No. 8A



Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

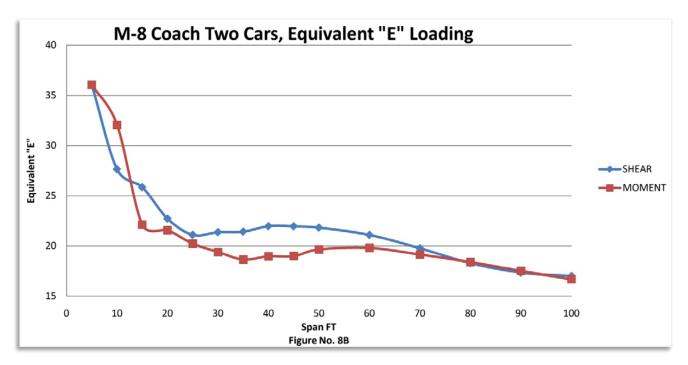
AXLE LOAD DIAGRAM (SINGLE ENGINE)

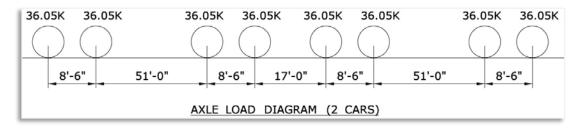


IVI-8 Coach	I wo Cars Equivalent Cooper Load	
SPAN FT	SHEAR	MOMENT
5	E 36.05	E 36.05
10	E 27.64	E 32.04
15	E 25.83	E 22.11
20	E 22.71	E 21.56
25	E 21.10	E 20.22
30	E 21.36	E 19.39
35	E 21.42	E 18.65
40	E 21.96	E 18.97
45	E 21.95	E 18.99
50	E 21.82	E 19.65
60	E 21.09	E 19.80
70	E 19.76	E 19.14
80	E 18.28	E 18.39
90	E 17.33	E 17.51
100	E 16.99	E 16.66



Table No. 8B



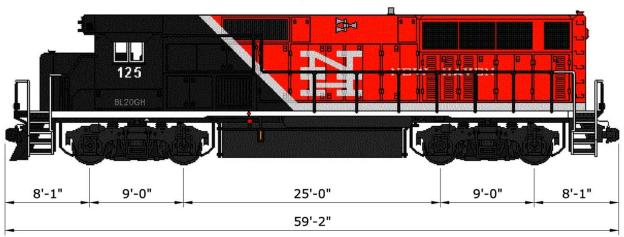


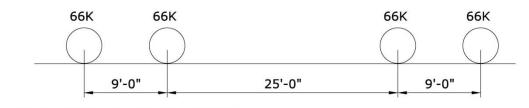
Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts

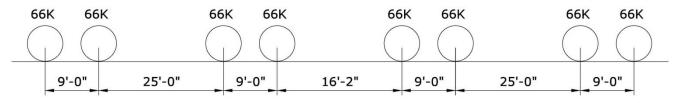
### **BL20-GH LOCOMOTIVE**

WEIGHT: 264,000 LBS GROSS

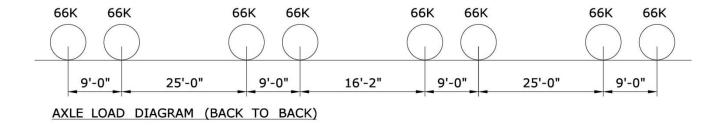




#### AXLE LOAD DIAGRAM (SINGLE ENGINE)



AXLE LOAD DIAGRAM (2 ENGINES SAME DIRECTION)

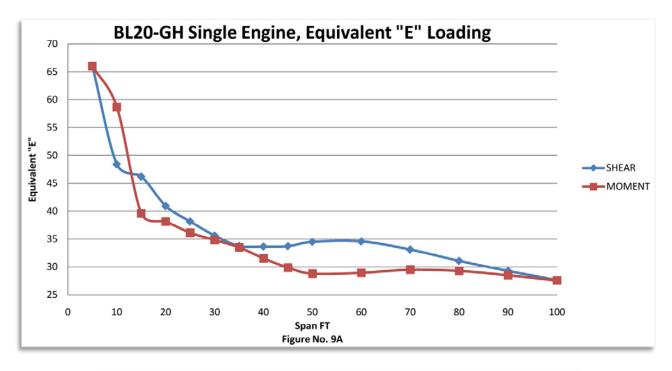


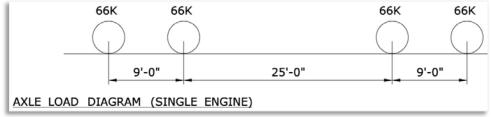
#### FIGURE NO. 9

BLZU-GH Single Engine Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 65.99	E 66.00
10	E 48.40	E 58.67
15	E 46.20	E 39.60
20	E 40.92	E 38.13
25	E 38.14	E 36.13
30	E 35.62	E 34.84
35	E 33.76	E 33.51
40	E 33.65	E 31.54
45	E 33.73	E 29.89
50	E 34.49	E 28.80
60	E 34.57	E 28.96
70	E 33.11	E 29.51
80	E 31.07	E 29.29
90	E 29.28	E 28.51
100	E 27.63	E 27.57



Table No. 9A





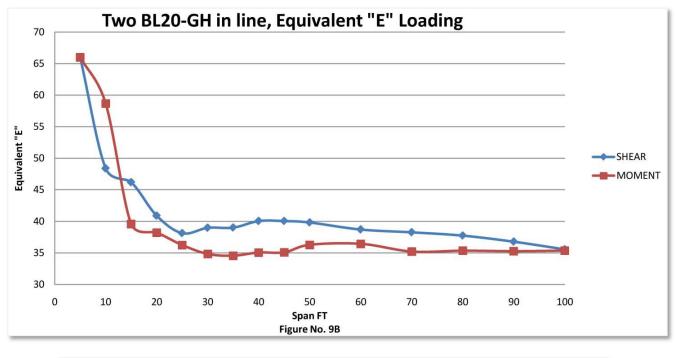
Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

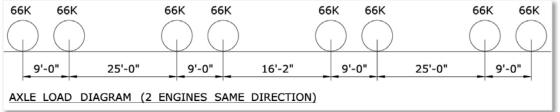
Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts

I wo BL20-GH in line Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 65.99	E 66.00
10	E 48.40	E 58.67
15	E 46.20	E 39.60
20	E 40.92	E 38.22
25	E 38.14	E 36.25
30	E 38.99	E 34.84
35	E 39.03	E 34.56
40	E 40.06	E 35.06
45	E 40.07	E 35.09
50	E 39.84	E 36.28
60	E 38.72	E 36.44
70	E 38.28	E 35.22
80	E 37.76	E 35.35
90	E 36.79	E 35.27
100	E 35.58	E 35.36

Two BL20-GH in line Equivalent Cooper Load

Table No. 9B





Load chart values are applicable to specific structural members. For limitations, see Executive Summary.





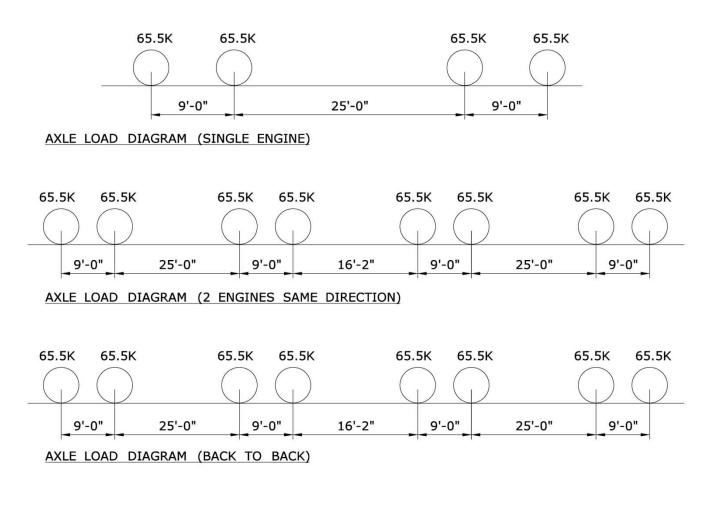


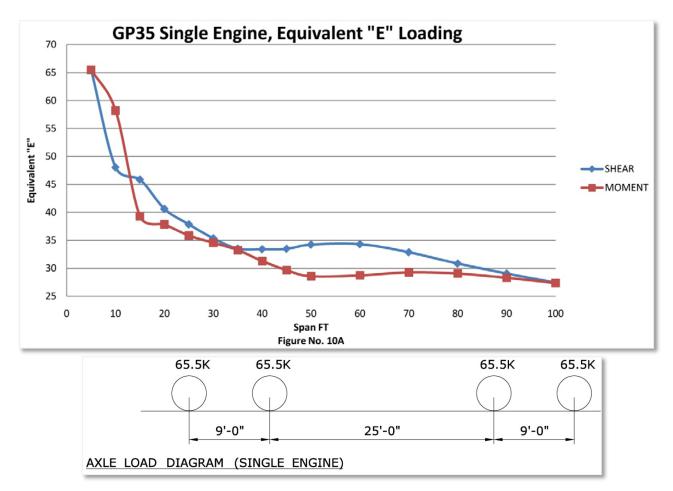
FIGURE NO. 10

LOCHNER

GP35 Single Engine Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 65.49	E 65.50
10	E 48.03	E 58.22
15	E 45.85	E 39.30
20	E 40.61	E 37.84
25	E 37.85	E 35.85
30	E 35.35	E 34.57
35	E 33.51	E 33.25
40	E 33.40	E 31.30
45	E 33.47	E 29.66
50	E 34.23	E 28.58
60	E 34.31	E 28.74
70	E 32.85	E 29.28
80	E 30.84	E 29.07
90	E 29.06	E 28.30
100	E 27.42	E 27.37

GP35 Single Engine Equivalent Cooper Load





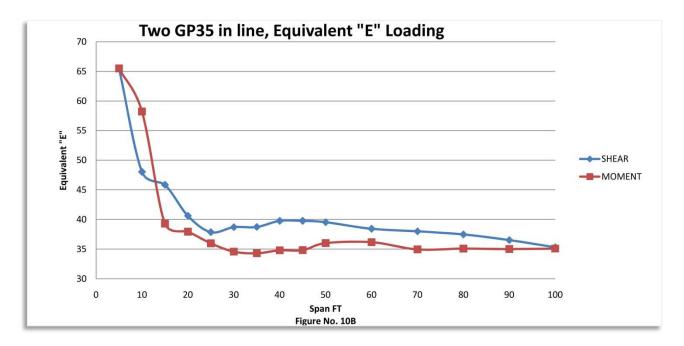
Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

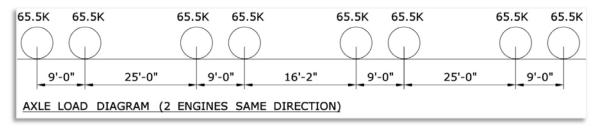


Two GP35 In line Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 65.49	E 65.50
10	E 48.03	E 58.22
15	E 45.85	E 39.30
20	E 40.61	E 37.93
25	E 37.85	E 35.97
30	E 38.70	E 34.57
35	E 38.73	E 34.30
40	E 39.76	E 34.79
45	E 39.76	E 34.82
50	E 39.54	E 36.01
60	E 38.43	E 36.16
70	E 37.99	E 34.95
80	E 37.47	E 35.08
90	E 36.52	E 35.01
100	E 35.31	E 35.09



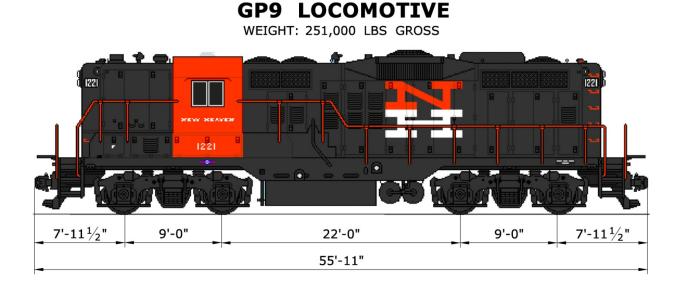
Table No. 10B





Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts



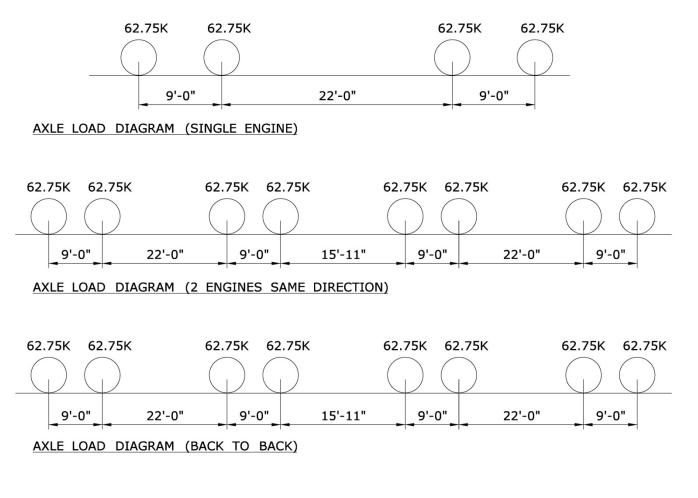


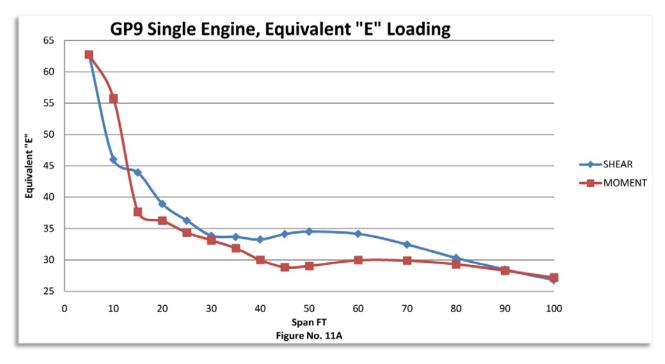
FIGURE NO. 11

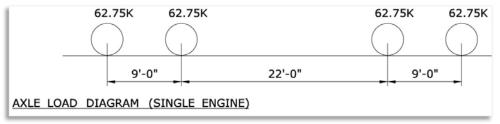


GP9 Single Engine Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 62.74	E 62.75
10	E 46.01	E 55.78
15	E 43.92	E 37.65
20	E 38.90	E 36.26
25	E 36.27	E 34.35
30	E 33.86	E 33.12
35	E 33.65	E 31.86
40	E 33.25	E 29.98
45	E 34.11	E 28.84
50	E 34.52	E 29.03
60	E 34.15	E 29.95
70	E 32.45	E 29.89
80	E 30.30	E 29.30
90	E 28.45	E 28.29
100	E 26.77	E 27.19



Table No. 11A





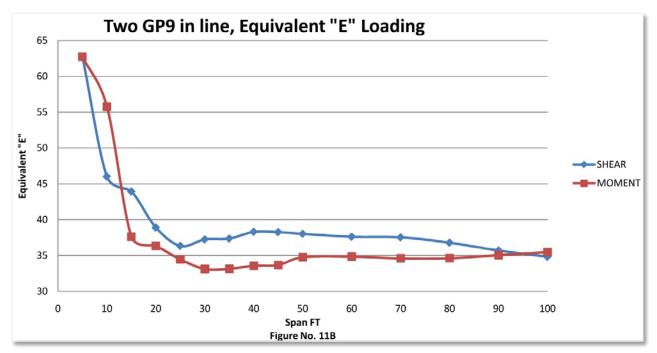
Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

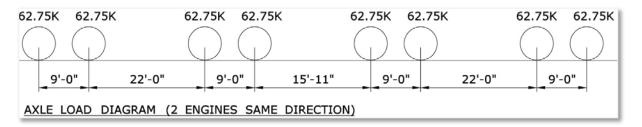
Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts

Two GP9 III III e Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 62.74	E 62.75
10	E 46.01	E 55.78
15	E 43.92	E 37.65
20	E 38.90	E 36.34
25	E 36.34	E 34.46
30	E 37.24	E 33.12
35	E 37.37	E 33.16
40	E 38.30	E 33.57
45	E 38.26	E 33.68
50	E 38.02	E 34.77
60	E 37.62	E 34.85
70	E 37.53	E 34.61
80	E 36.78	E 34.63
90	E 35.69	E 35.06
100	E 34.83	E 35.48

Two GP9 in line Equivalent Cooper Load

Table No. 11B



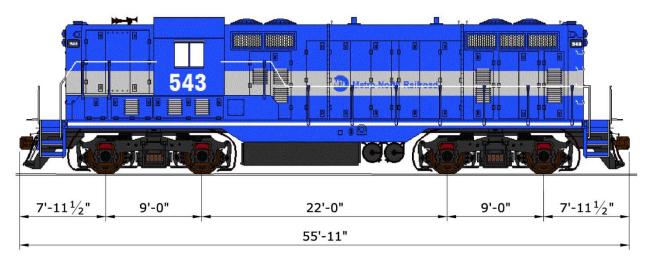


Load chart values are applicable to specific structural members. For limitations, see Executive Summary.



**GP8 LOCOMOTIVE** 

WEIGHT: 249,000 LBS GROSS



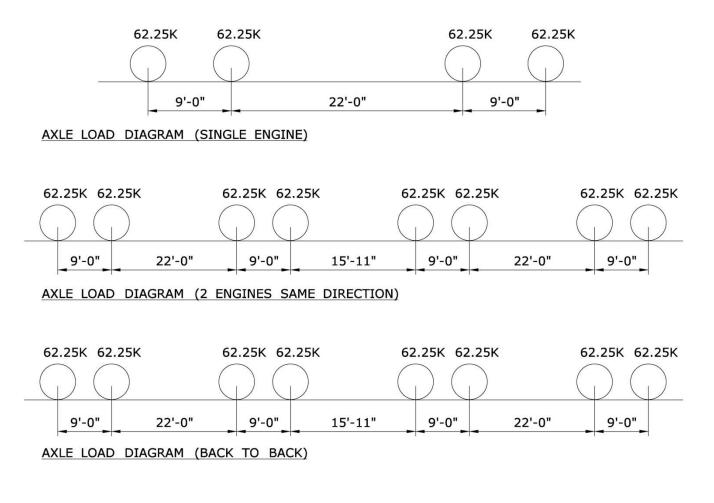
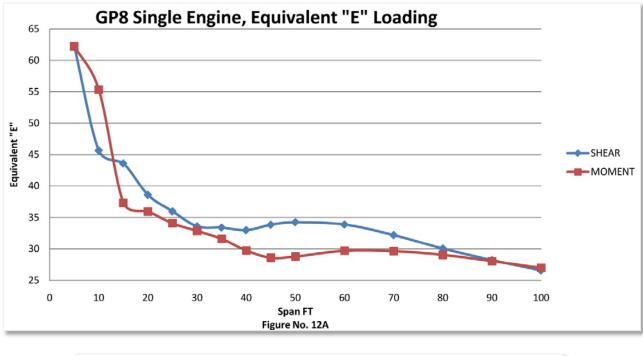


FIGURE NO. 12

GP8 Single Engine Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 62.24	E 62.25
10	E 45.65	E 55.33
15	E 43.57	E 37.35
20	E 38.59	E 35.97
25	E 35.98	E 34.07
30	E 33.59	E 32.86
35	E 33.39	E 31.60
40	E 32.98	E 29.74
45	E 33.84	E 28.61
50	E 34.24	E 28.80
60	E 33.88	E 29.71
70	E 32.19	E 29.65
80	E 30.06	E 29.07
90	E 28.22	E 28.06
100	E 26.56	E 26.97



Table No. 12A





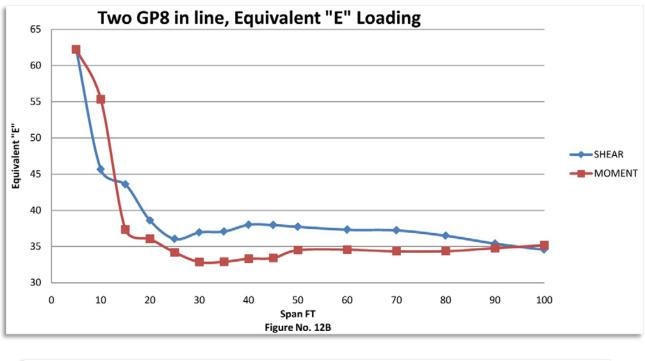
Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

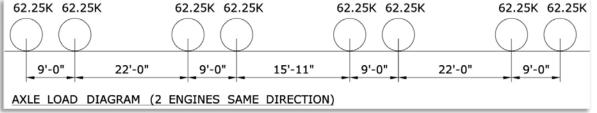


Two GP8 in line Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 62.24	E 62.25
10	E 45.65	E 55.33
15	E 43.57	E 37.35
20	E 38.59	E 36.05
25	E 36.05	E 34.19
30	E 36.94	E 32.86
35	E 37.07	E 32.89
40	E 37.99	E 33.30
45	E 37.96	E 33.42
50	E 37.72	E 34.49
60	E 37.32	E 34.57
70	E 37.23	E 34.33
80	E 36.49	E 34.35
90	E 35.41	E 34.78
100	E 34.55	E 35.20



Table No. 12B



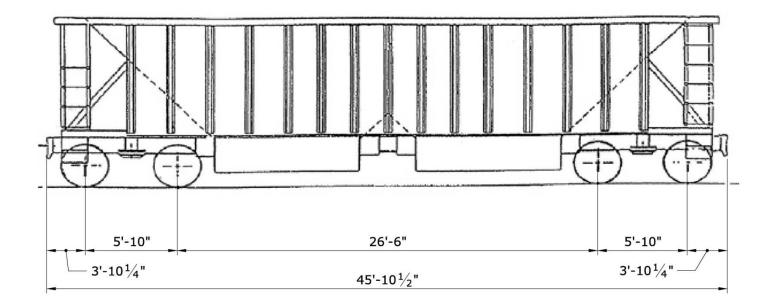


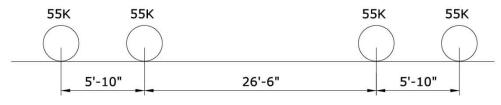
Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts

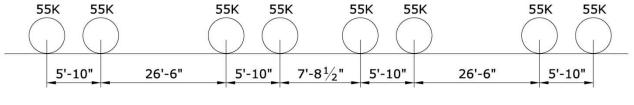
## 220,000 LBS HOPPER CAR

WEIGHT: 220,000 LBS GROSS (MAXIMUM)





AXLE LOAD DIAGRAM (SINGLE CAR)



AXLE LOAD DIAGRAM (2 CARS)

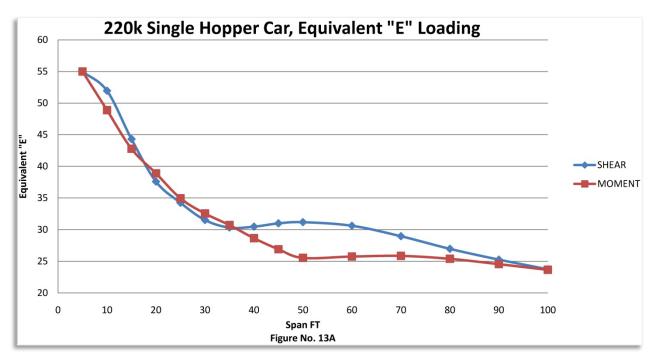
FIGURE NO. 13



SPAN FT	SHEAR	MOMENT
5	E 55.00	E 55.00
10	E 51.94	E 48.89
15	E 44.31	E 42.78
20	E 37.58	E 38.89
25	E 34.24	E 34.95
30	E 31.53	E 32.57
35	E 30.33	E 30.73
40	E 30.47	E 28.66
45	E 31.00	E 26.91
50	E 31.18	E 25.54
60	E 30.62	E 25.76
70	E 28.96	E 25.86
80	E 26.97	E 25.40
90	E 25.26	E 24.55
100	E 23.74	E 23.65
Table No. 424		



Table No. 13A





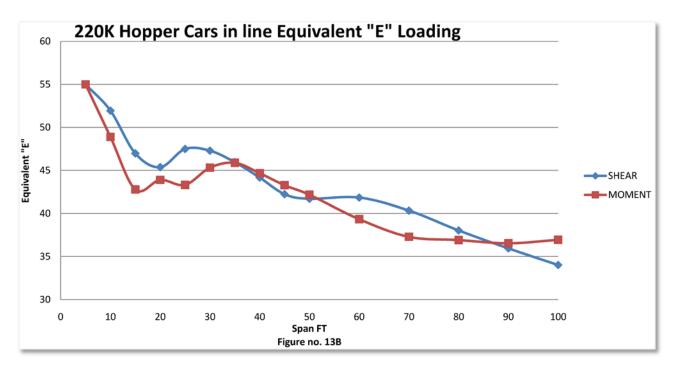
Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

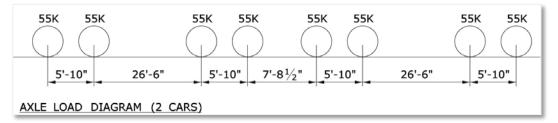
Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts

220k Hopper Cars in line Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 54.99	E 55.00
10	E 51.94	E 48.89
15	E 46.98	E 42.78
20	E 45.37	E 43.89
25	E 47.49	E 43.31
30	E 47.29	E 45.31
35	E 45.95	E 45.88
40	E 44.16	E 44.67
45	E 42.23	E 43.28
50	E 41.70	E 42.17
60	E 41.84	E 39.34
70	E 40.34	E 37.29
80	E 38.03	E 36.91
90	E 35.95	E 36.54
100	E 34.01	E 36.94

220k Hopper Cars in line Equivalent Cooper Load

Table No. 13B





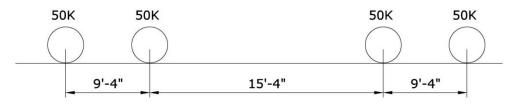
Load chart values are applicable to specific structural members. For limitations, see Executive Summary.



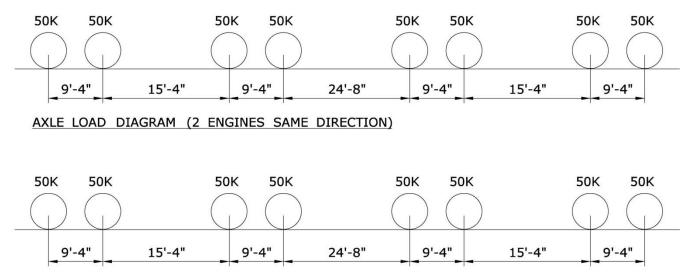
ACELA LOCOMOTIVE

WEIGHT: 200,000 LBS GROSS





### AXLE LOAD DIAGRAM (SINGLE ENGINE)



Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts

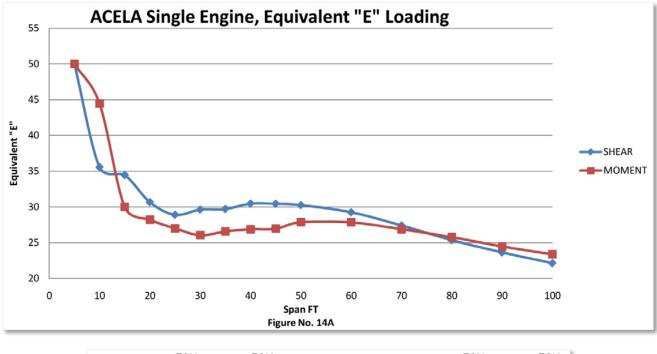
AXLE LOAD DIAGRAM (BACK TO BACK)

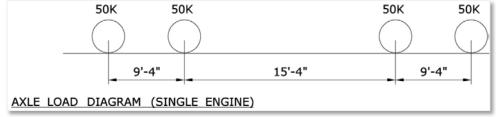
FIGURE NO. 14

ACELA Single Engine Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 50.00	E 50.00
10	E 35.56	E 44.44
15	E 34.44	E 30.00
20	E 30.67	E 28.22
25	E 28.90	E 27.01
30	E 29.63	E 26.05
35	E 29.71	E 26.58
40	E 30.46	E 26.88
45	E 30.45	E 26.96
50	E 30.26	E 27.87
60	E 29.25	E 27.84
70	E 27.41	E 26.89
80	E 25.35	E 25.78
90	E 23.64	E 24.47
100	E 22.13	E 23.37



Table No. 14A





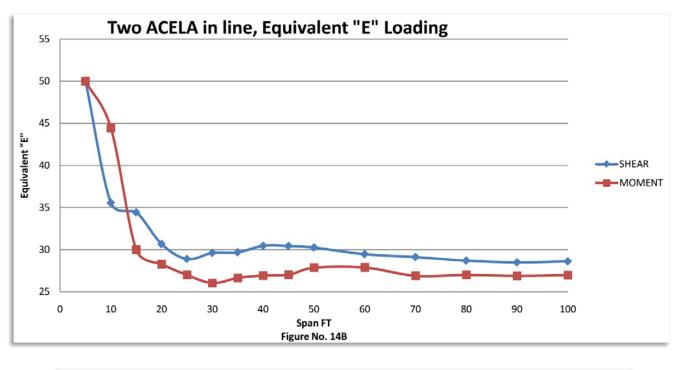
Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

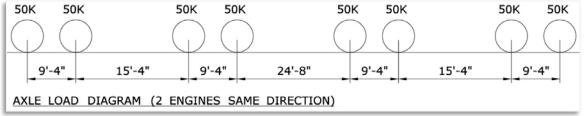


I wo ACELA in line Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 50.00	E 50.00
10	E 35.56	E 44.45
15	E 34.44	E 30.00
20	E 30.67	E 28.28
25	E 28.90	E 27.01
30	E 29.63	E 26.05
35	E 29.71	E 26.66
40	E 30.46	E 26.94
45	E 30.45	E 27.03
50	E 30.26	E 27.87
60	E 29.48	E 27.88
70	E 29.13	E 26.91
80	E 28.71	E 27.01
90	E 28.50	E 26.90
100	E 28.62	E 26.99



Table No. 14B





Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts

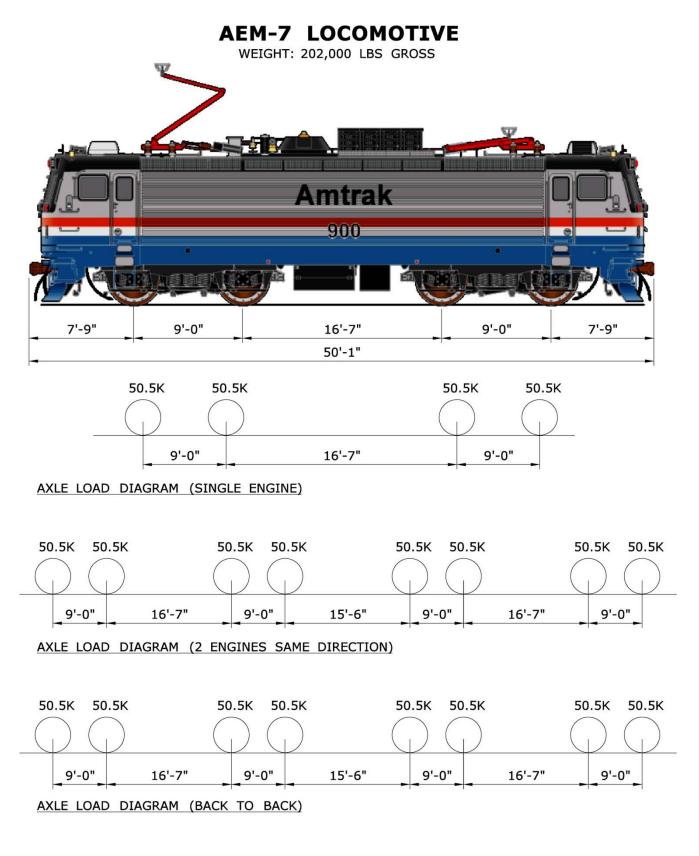


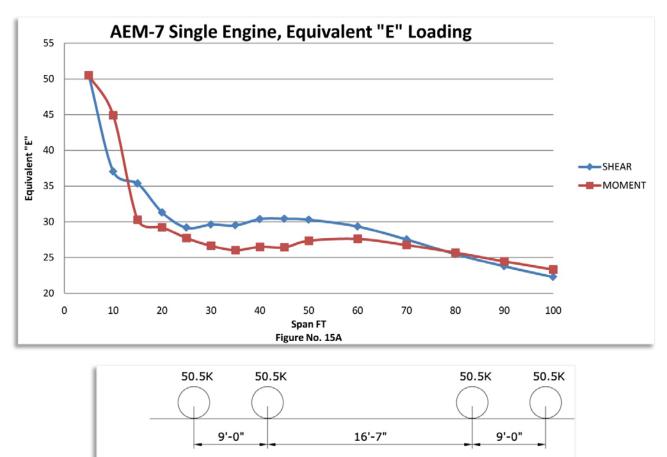
FIGURE NO. 15



AEIVI-7 Single Engine Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 50.49	E 50.50
10	E 37.03	E 44.89
15	E 35.35	E 30.30
20	E 31.31	E 29.25
25	E 29.19	E 27.73
30	E 29.61	E 26.65
35	E 29.52	E 26.04
40	E 30.38	E 26.50
45	E 30.43	E 26.44
50	E 30.29	E 27.36
60	E 29.34	E 27.61
70	E 27.53	E 26.76
80	E 25.49	E 25.69
90	E 23.78	E 24.47
100	E 22.28	E 23.32



Table No. 15A



Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

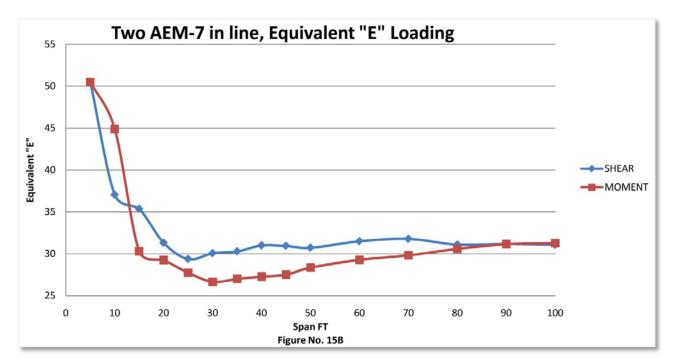
AXLE LOAD DIAGRAM (SINGLE ENGINE)

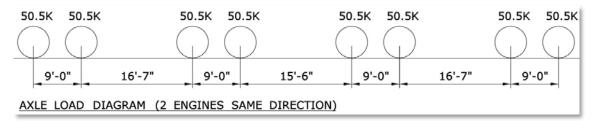
Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts

Two AEIVI-7 In line Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 50.49	E 50.51
10	E 37.03	E 44.90
15	E 35.35	E 30.30
20	E 31.31	E 29.25
25	E 29.36	E 27.74
30	E 30.06	E 26.66
35	E 30.28	E 27.01
40	E 30.99	E 27.27
45	E 30.93	E 27.51
50	E 30.71	E 28.35
60	E 31.49	E 29.29
70	E 31.77	E 29.82
80	E 31.09	E 30.60
90	E 31.18	E 31.15
100	E 31.06	E 31.28



Table No. 15B

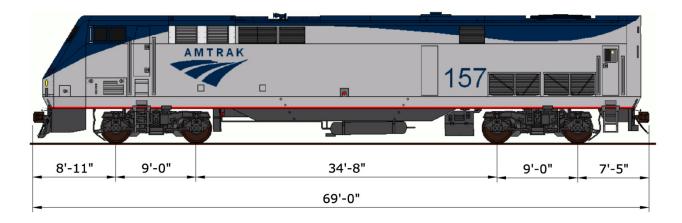






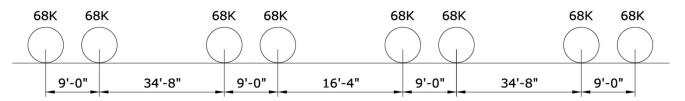
# AMD-103 (GENESIS) LOCOMOTIVE

WEIGHT: 272,000 LBS GROSS





AXLE LOAD DIAGRAM (SINGLE ENGINE)



AXLE LOAD DIAGRAM (2 ENGINES SAME DIRECTION)

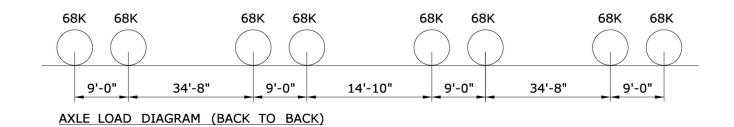
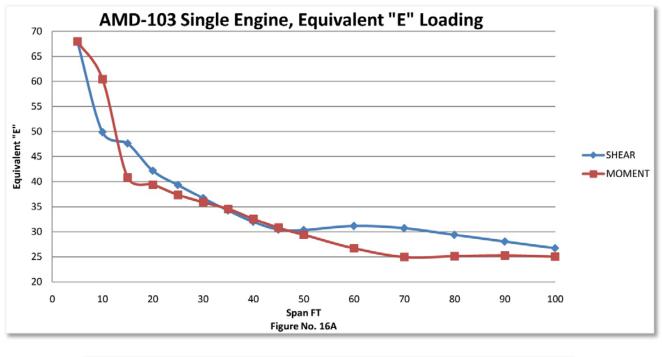


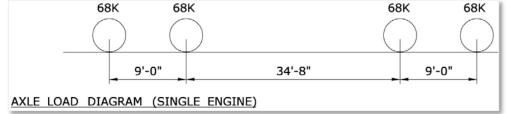
FIGURE NO. 16

AIVID-105 Single Lingine Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 67.99	E 68.00
10	E 49.86	E 60.44
15	E 47.60	E 40.80
20	E 42.16	E 39.38
25	E 39.30	E 37.35
30	E 36.70	E 35.89
35	E 34.23	E 34.56
40	E 31.97	E 32.55
45	E 30.44	E 30.79
50	E 30.34	E 29.40
60	E 31.15	E 26.70
70	E 30.71	E 24.95
80	E 29.37	E 25.12
90	E 28.04	E 25.27
100	E 26.72	E 25.02



Table No. 16A



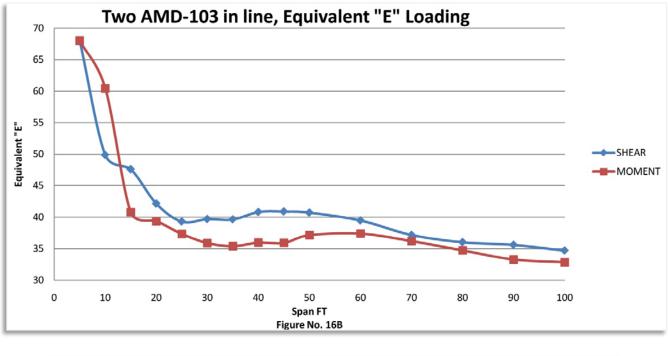


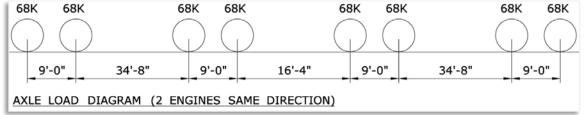


Two AMD-103 in line Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 68.00	E 68.01
10	E 49.86	E 60.45
15	E 47.60	E 40.80
20	E 42.16	E 39.38
25	E 39.30	E 37.35
30	E 39.70	E 35.89
35	E 39.65	E 35.39
40	E 40.83	E 35.95
45	E 40.91	E 35.91
50	E 40.73	E 37.14
60	E 39.47	E 37.40
70	E 37.19	E 36.20
80	E 36.03	E 34.73
90	E 35.60	E 33.28
100	E 34.70	E 32.84

Two AMD-103 in line Equivalent Cooper Load

Table No. 16B





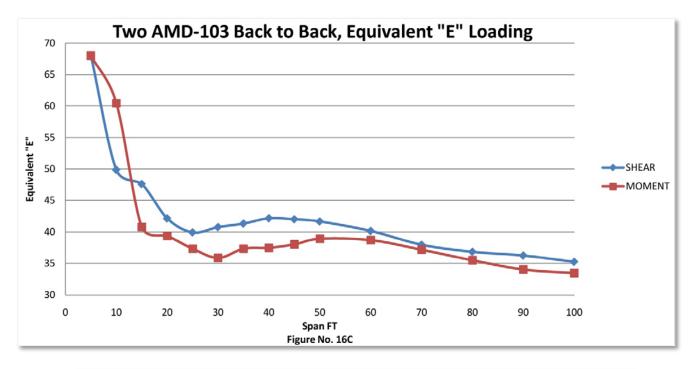
Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

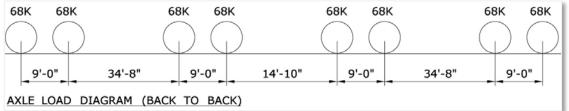
Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts

Two AMD-105 back to back Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 68.00	E 68.01
10	E 49.86	E 60.45
15	E 47.60	E 40.80
20	E 42.16	E 39.38
25	E 39.94	E 37.35
30	E 40.78	E 35.89
35	E 41.33	E 37.34
40	E 42.18	E 37.50
45	E 42.02	E 38.04
50	E 41.67	E 38.93
60	E 40.17	E 38.71
70	E 37.98	E 37.19
80	E 36.85	E 35.51
90	E 36.26	E 34.05
100	E 35.25	E 33.48

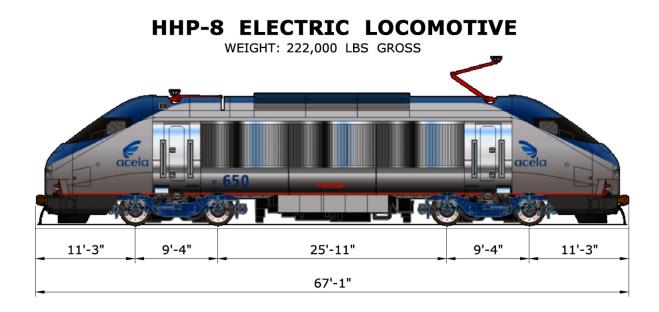


Table No. 16C









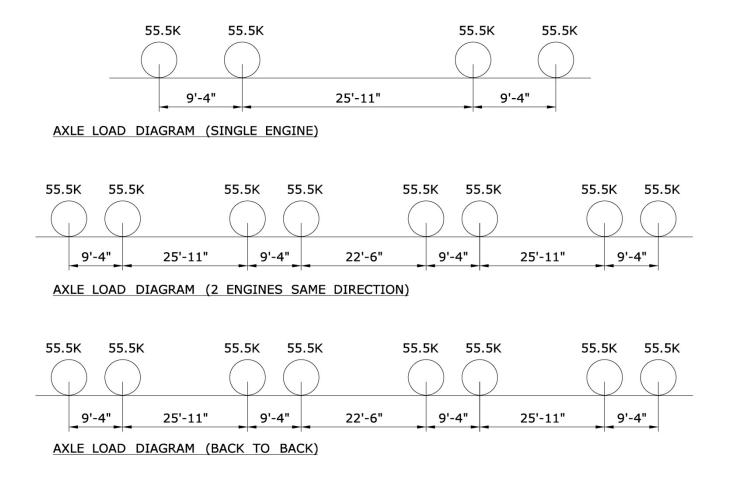
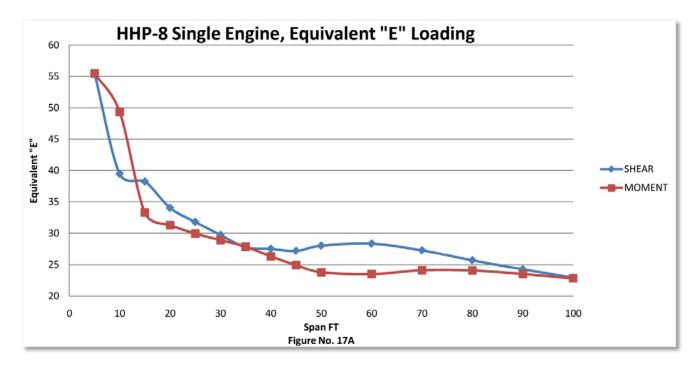


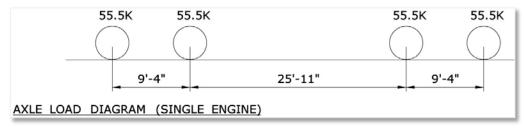
FIGURE NO. 17

HHP-8 Single Engine Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 55.50	E 55.50
10	E 39.47	E 49.33
15	E 38.23	E 33.30
20	E 34.04	E 31.32
25	E 31.82	E 29.98
30	E 29.76	E 28.92
35	E 27.78	E 27.88
40	E 27.54	E 26.33
45	E 27.21	E 24.94
50	E 28.03	E 23.81
60	E 28.35	E 23.50
70	E 27.29	E 24.12
80	E 25.70	E 24.08
90	E 24.28	E 23.52
100	E 22.95	E 22.82



Table No. 17A



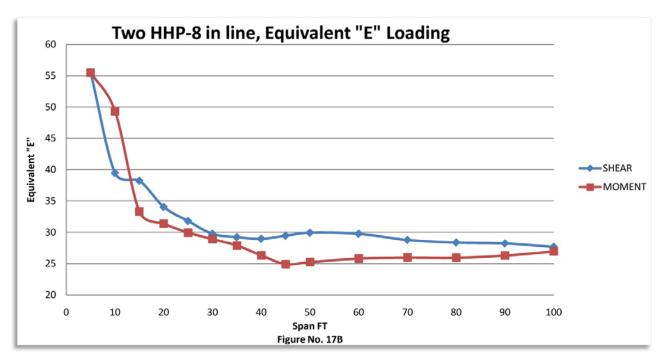


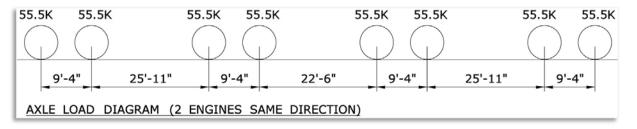


Two HHP-8 in line Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 55.50	E 55.50
10	E 39.47	E 49.33
15	E 38.23	E 33.30
20	E 34.04	E 31.40
25	E 31.82	E 29.98
30	E 29.76	E 28.92
35	E 29.23	E 27.91
40	E 28.97	E 26.33
45	E 29.47	E 24.94
50	E 29.94	E 25.26
60	E 29.76	E 25.82
70	E 28.78	E 25.98
80	E 28.38	E 25.96
90	E 28.23	E 26.30
100	E 27.69	E 26.95



Table No. 17B





Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts

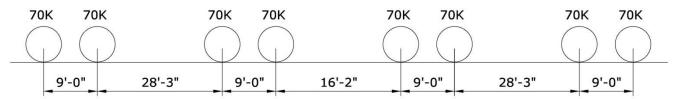
**GP40/GP40-2H LOCOMOTIVE** 

WEIGHT: 280,000 LBS GROSS





AXLE LOAD DIAGRAM (SINGLE ENGINE)



AXLE LOAD DIAGRAM (2 ENGINES SAME DIRECTION)

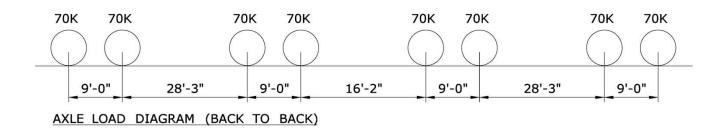


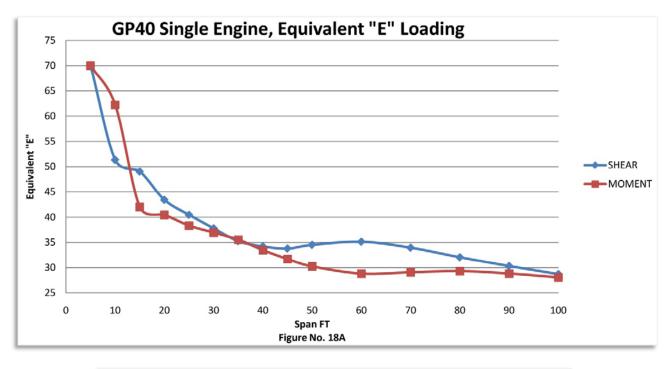
FIGURE NO. 18

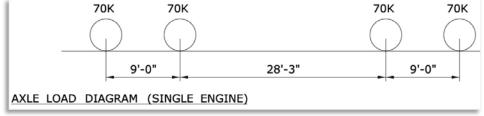


GP40 Single Engine Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 69.99	E 70.00
10	E 51.33	E 62.22
15	E 49.00	E 42.00
20	E 43.40	E 40.44
25	E 40.46	E 38.32
30	E 37.78	E 36.95
35	E 35.23	E 35.54
40	E 34.19	E 33.45
45	E 33.77	E 31.70
50	E 34.50	E 30.24
60	E 35.12	E 28.81
70	E 33.94	E 29.07
80	E 32.04	E 29.31
90	E 30.32	E 28.82
100	E 28.70	E 28.07

**GP40 Single Engine Equivalent Cooper Load** 

Table No. 18A





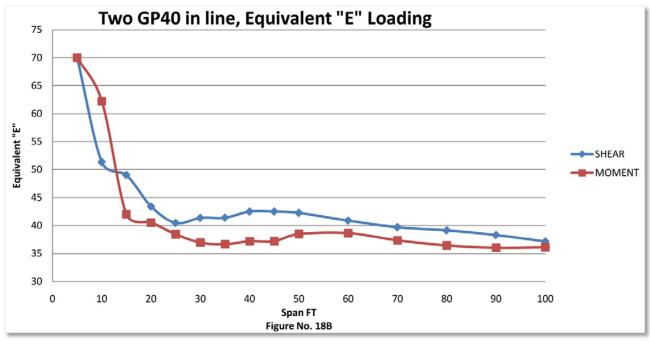
Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

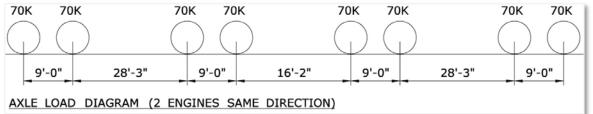
Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts

Two GP40 In line Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 70.00	E 70.01
10	E 51.33	E 62.23
15	E 49.00	E 42.00
20	E 43.40	E 40.54
25	E 40.46	E 38.44
30	E 41.36	E 36.95
35	E 41.40	E 36.65
40	E 42.49	E 37.18
45	E 42.50	E 37.21
50	E 42.25	E 38.48
60	E 40.87	E 38.65
70	E 39.68	E 37.35
80	E 39.13	E 36.44
90	E 38.29	E 36.03
100	E 37.13	E 36.13

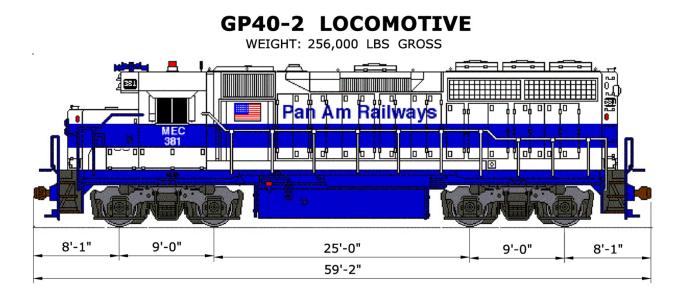
Two GP40 in line Equivalent Cooper Load

Table No. 18B



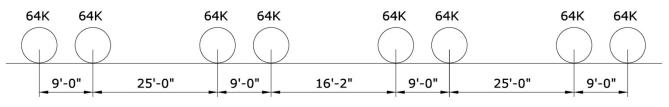




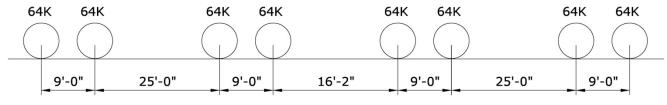




#### AXLE LOAD DIAGRAM (SINGLE ENGINE)



AXLE LOAD DIAGRAM (2 ENGINES SAME DIRECTION)



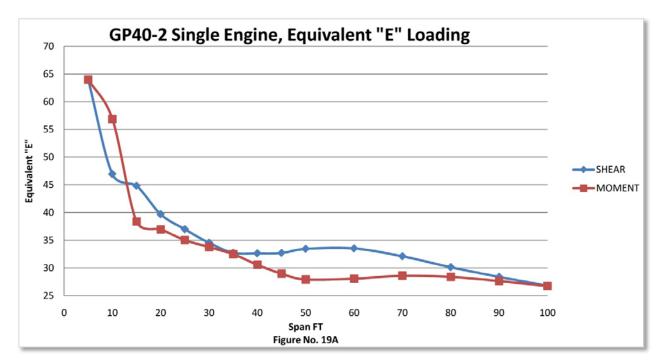
AXLE LOAD DIAGRAM (BACK TO BACK)

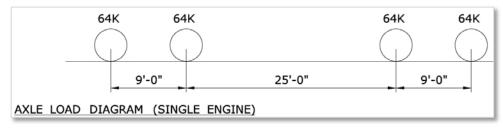
FIGURE NO. 19

GP40-2 Single Engine Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 63.99	E 64.00
10	E 46.93	E 56.89
15	E 44.80	E 38.40
20	E 39.68	E 36.98
25	E 36.99	E 35.03
30	E 34.54	E 33.78
35	E 32.74	E 32.49
40	E 32.63	E 30.58
45	E 32.71	E 28.98
50	E 33.45	E 27.93
60	E 33.52	E 28.08
70	E 32.10	E 28.61
80	E 30.13	E 28.40
90	E 28.39	E 27.65
100	E 26.79	E 26.74

GP40-2 Single Engine Equivalent Cooper Load

Table No. 19A



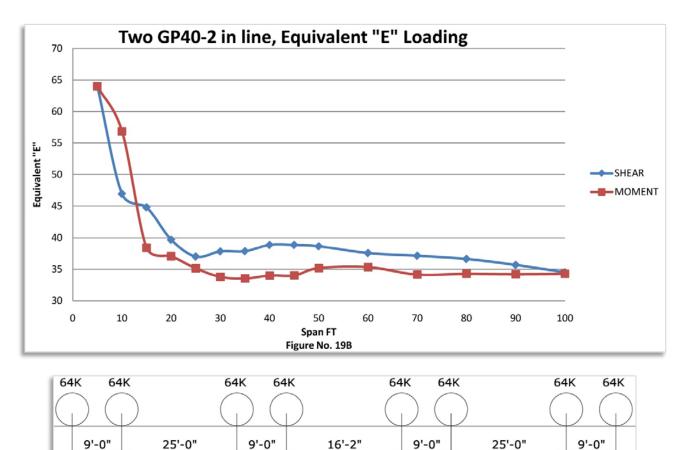




Two GP40-2 in line Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 63.99	E 64.00
10	E 46.93	E 56.89
15	E 44.80	E 38.40
20	E 39.68	E 37.07
25	E 36.99	E 35.15
30	E 37.81	E 33.78
35	E 37.85	E 33.51
40	E 38.85	E 34.00
45	E 38.85	E 34.02
50	E 38.63	E 35.18
60	E 37.55	E 35.34
70	E 37.12	E 34.15
80	E 36.61	E 34.28
90	E 35.68	E 34.20
100	E 34.50	E 34.28
	Table No. 40D	



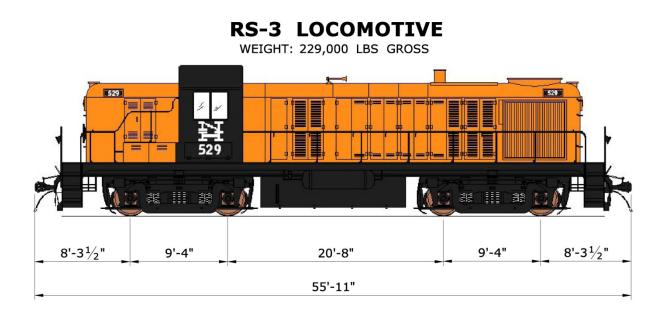
Table No. 19B

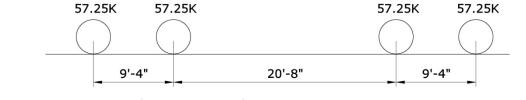


AXLE LOAD DIAGRAM (2 ENGINES SAME DIRECTION)

Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts





AXLE LOAD DIAGRAM (SINGLE ENGINE)

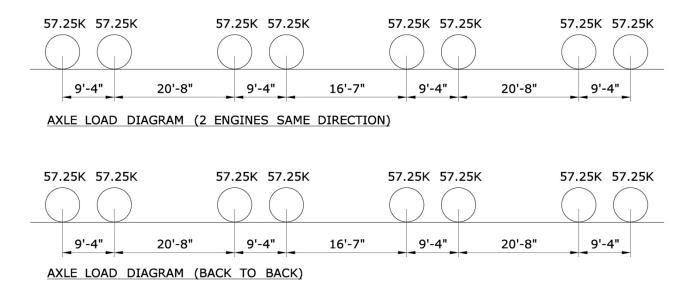


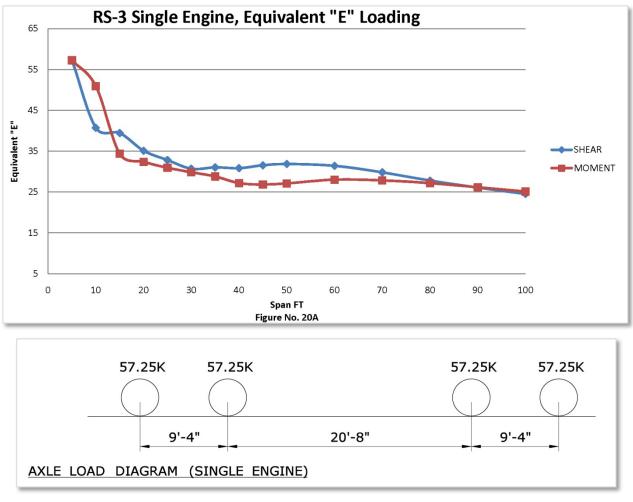
FIGURE NO. 20



RS-3 Single Engine Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 57.24	E 57.25
10	E 40.71	E 50.89
15	E 39.44	E 34.35
20	E 35.11	E 32.38
25	E 32.82	E 30.92
30	E 30.69	E 29.83
35	E 31.02	E 28.79
40	E 30.84	E 27.16
45	E 31.54	E 26.83
50	E 31.84	E 27.09
60	E 31.41	E 28.01
70	E 29.80	E 27.83
80	E 27.80	E 27.16
90	E 26.08	E 26.14
100	E 24.53	E 25.10
Table No. 204		



Table No. 20A



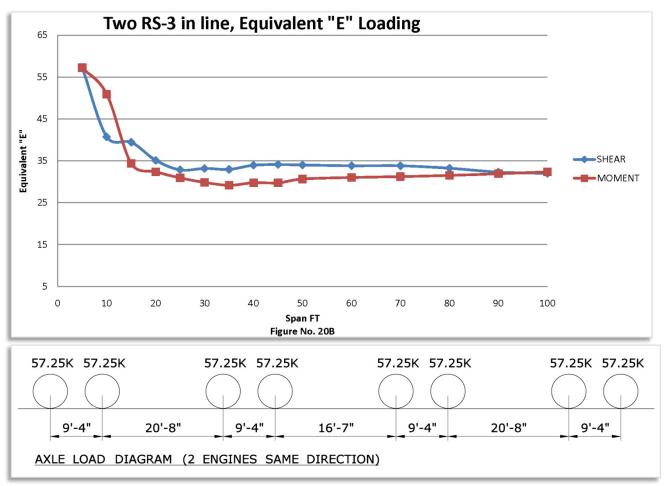
Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts

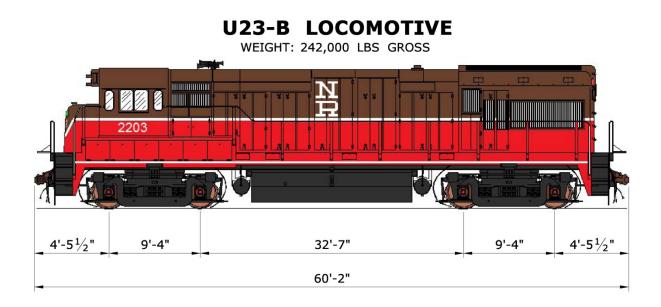
Two K3-5 III line Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 57.24	E 57.25
10	E 40.71	E 50.89
15	E 39.44	E 34.35
20	E 35.11	E 32.38
25	E 32.82	E 30.92
30	E 33.17	E 29.83
35	E 32.95	E 29.16
40	E 33.93	E 29.76
45	E 34.08	E 29.74
50	E 33.99	E 30.65
60	E 33.80	E 31.01
70	E 33.80	E 31.24
80	E 33.21	E 31.50
90	E 32.29	E 31.92
100	E 31.98	E 32.37

Two RS-3 in line Equivalent Cooper Load

Table No. 20B







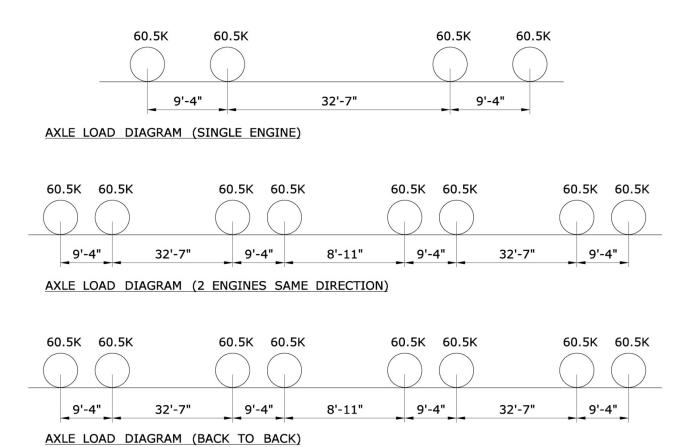


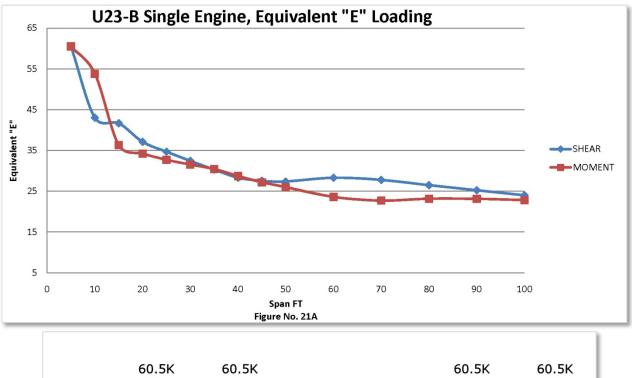
FIGURE NO. 21

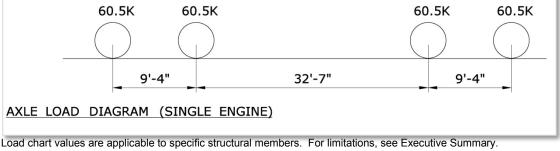
Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts

U23-B Single Engine Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 60.49	E 60.50
10	E 43.02	E 53.78
15	E 41.67	E 36.30
20	E 37.10	E 34.22
25	E 34.68	E 32.68
30	E 32.44	E 31.52
35	E 30.28	E 30.42
40	E 28.31	E 28.70
45	E 27.55	E 27.19
50	E 27.39	E 25.98
60	E 28.29	E 23.60
70	E 27.77	E 22.69
80	E 26.48	E 23.14
90	E 25.22	E 23.12
100	E 24.00	E 22.80

U23-B Single Engine Equivalent Cooper Load

Table No. 21A



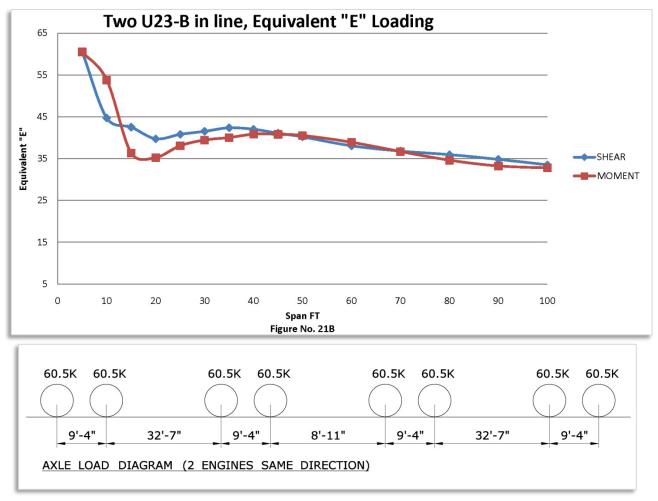




Two 025-b in line Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 60.49	E 60.50
10	E 44.70	E 53.78
15	E 42.52	E 36.30
20	E 39.73	E 35.24
25	E 40.79	E 38.06
30	E 41.51	E 39.42
35	E 42.35	E 40.02
40	E 42.00	E 40.83
45	E 41.06	E 40.79
50	E 40.17	E 40.52
60	E 38.03	E 38.89
70	E 36.79	E 36.67
80	E 35.93	E 34.60
90	E 34.79	E 33.22
100	E 33.49	E 32.76

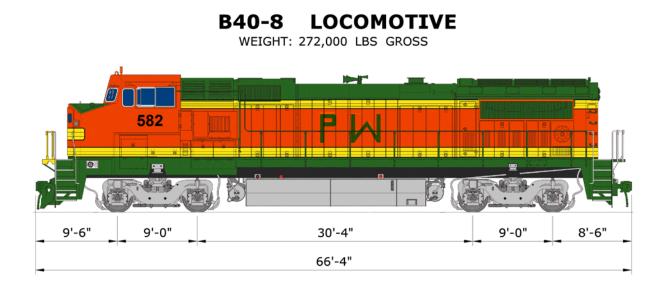


Table No. 21B



Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts





AXLE LOAD DIAGRAM (SINGLE ENGINE)

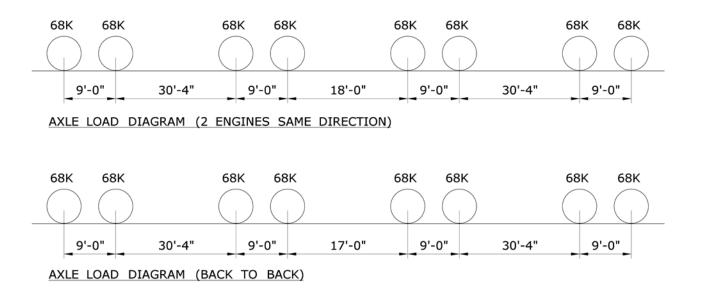


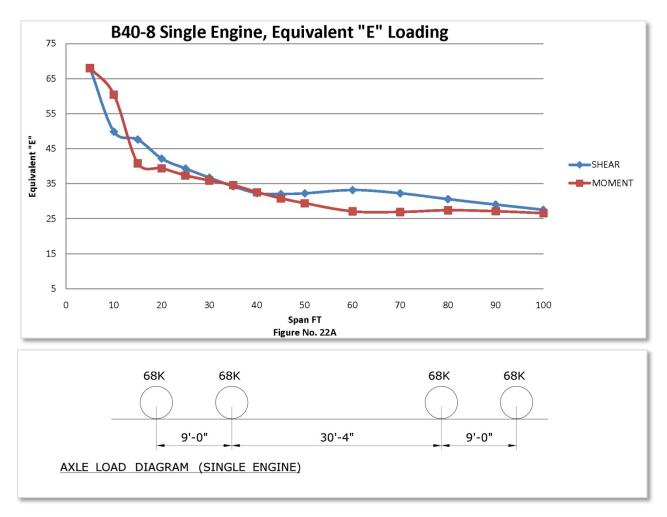
FIGURE NO. 22



B40-8 Single Engine Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 67.99	E 68.00
10	E 49.86	E 60.44
15	E 47.60	E 40.80
20	E 42.16	E 39.38
25	E 39.30	E 37.35
30	E 36.70	E 35.89
35	E 34.23	E 34.56
40	E 32.27	E 32.55
45	E 32.04	E 30.79
50	E 32.21	E 29.40
60	E 33.15	E 27.08
70	E 32.23	E 26.90
80	E 30.56	E 27.40
90	E 28.99	E 27.11
100	E 27.50	E 26.56
Table No. 22A		

**B40-8 Single Engine Equivalent Cooper Load** 

Table No. 22A



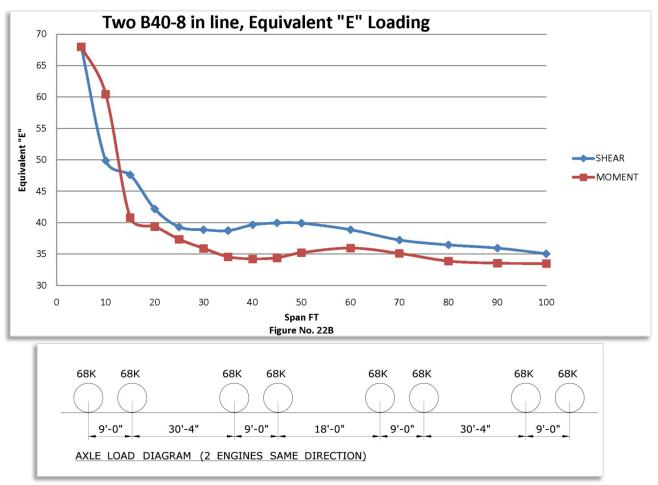
Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts

Two B40-6 III III e Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 67.99	E 68.00
10	E 49.86	E 60.44
15	E 47.60	E 40.80
20	E 42.16	E 39.38
25	E 39.30	E 37.35
30	E 38.85	E 35.89
35	E 38.71	E 34.56
40	E 39.63	E 34.22
45	E 39.92	E 34.40
50	E 39.90	E 35.20
60	E 38.86	E 35.95
70	E 37.21	E 35.09
80	E 36.44	E 33.87
90	E 35.93	E 33.54
100	E 35.03	E 33.46

Two B40-8 in line Equivalent Cooper Load

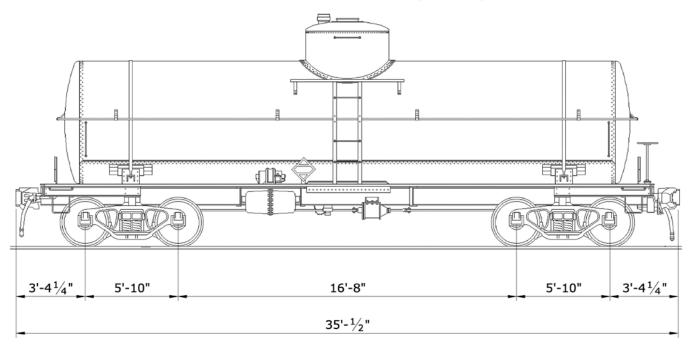
Table No. 22B

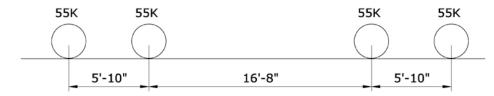




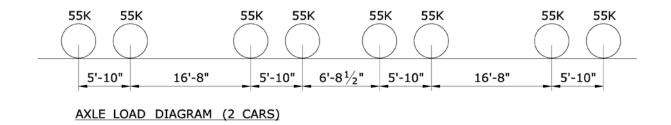
## 220,000 LBS AAR FREIGHT CAR

WEIGHT: 220,000 LBS GROSS (MAXIMUM)





AXLE LOAD DIAGRAM (SINGLE CAR)



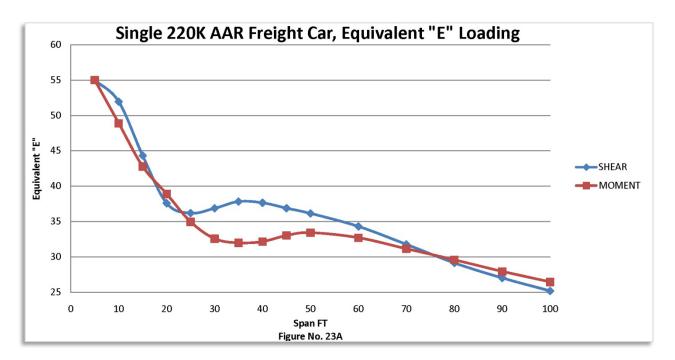
Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts

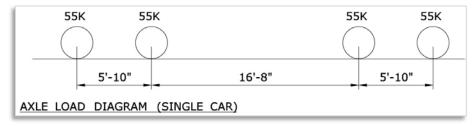
FIGURE NO. 23

Single 220K AAN Car Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 55.00	E 55.00
10	E 51.94	E 48.89
15	E 44.31	E 42.78
20	E 37.58	E 38.89
25	E 36.18	E 34.95
30	E 36.86	E 32.57
35	E 37.82	E 31.98
40	E 37.64	E 32.15
45	E 36.88	E 33.02
50	E 36.14	E 33.42
60	E 34.30	E 32.70
70	E 31.76	E 31.14
80	E 29.14	E 29.57
90	E 27.01	E 27.93
100	E 25.18	E 26.45

Single 220k AAR Car Equivalent Cooper Load

Table No. 23A





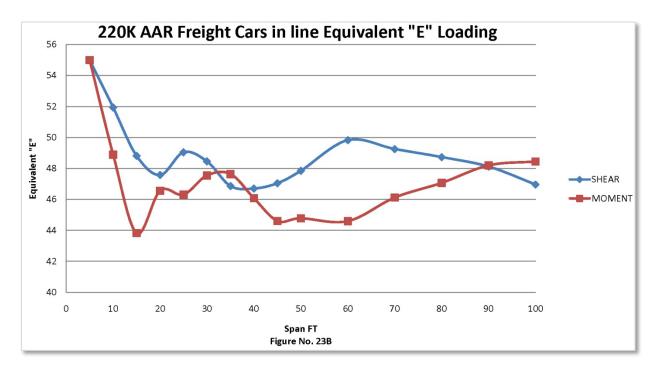
Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

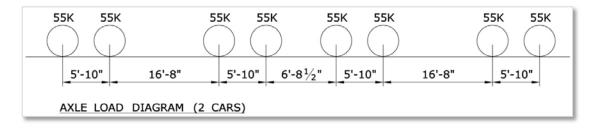


220k AAR Cars in line Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 54.99	E 55.00
10	E 51.94	E 48.89
15	E 48.81	E 43.82
20	E 47.57	E 46.56
25	E 49.04	E 46.31
30	E 48.45	E 47.55
35	E 46.86	E 47.63
40	E 46.70	E 46.07
45	E 47.04	E 44.61
50	E 47.84	E 44.77
60	E 49.82	E 44.59
70	E 49.25	E 46.12
80	E 48.73	E 47.06
90	E 48.12	E 48.19
100	E 46.96	E 48.45



Table No. 23B



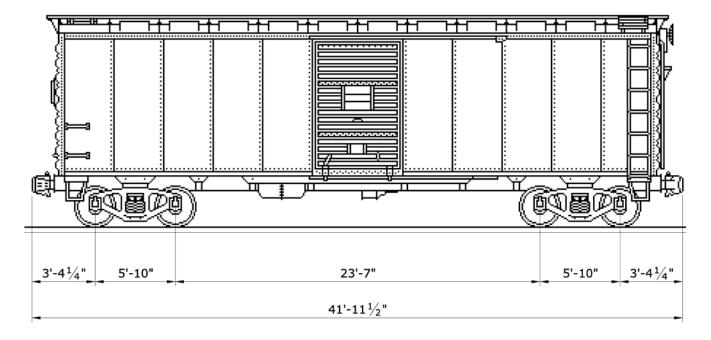


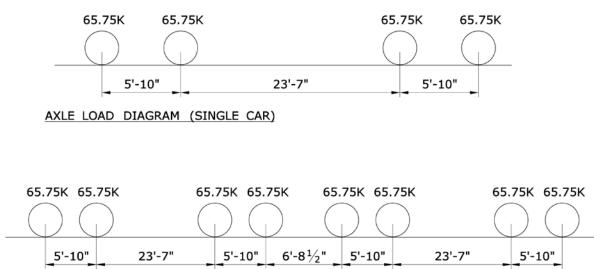
Load chart values are applicable to specific structural members. For limitations, see Executive Summary,

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#### 263,00 LBS AAR FREIGHT CAR

WEIGHT: 263,000 LBS GROSS (MAXIMUM)





AXLE LOAD DIAGRAM (2 CARS)

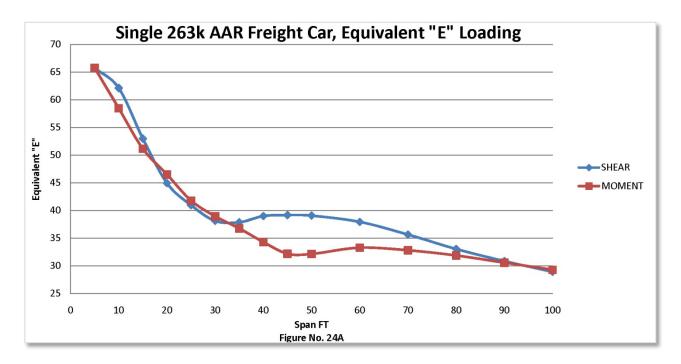
FIGURE NO. 24

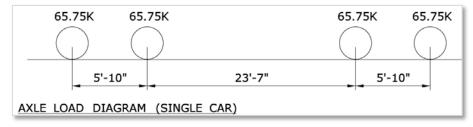


Single 263K AAR Car Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 65.75	E 65.75
10	E 62.10	E 58.45
15	E 52.97	E 51.14
20	E 44.93	E 46.49
25	E 40.94	E 41.78
30	E 38.09	E 38.93
35	E 37.84	E 36.74
40	E 38.97	E 34.26
45	E 39.14	E 32.17
50	E 39.04	E 32.12
60	E 37.91	E 33.25
70	E 35.62	E 32.78
80	E 33.01	E 31.84
90	E 30.82	E 30.55
100	E 28.89	E 29.27



Table No. 24A





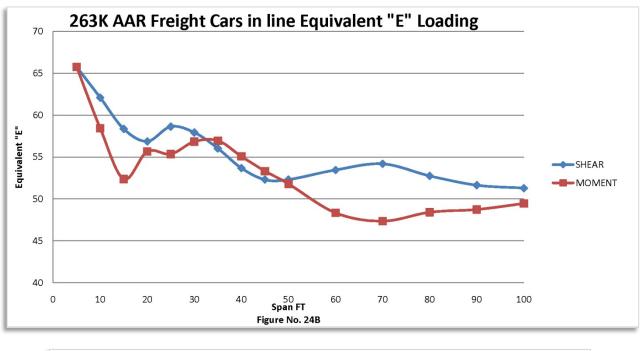
Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

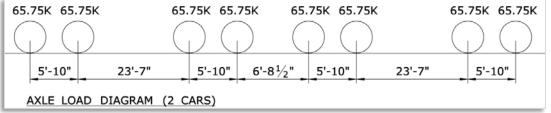
Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts

263K AAR Cars in line Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 65.74	E 65.75
10	E 62.09	E 58.44
15	E 58.35	E 52.38
20	E 56.87	E 55.66
25	E 58.62	E 55.36
30	E 57.92	E 56.84
35	E 56.02	E 56.94
40	E 53.67	E 55.08
45	E 52.29	E 53.32
50	E 52.30	E 51.80
60	E 53.45	E 48.34
70	E 54.17	E 47.35
80	E 52.74	E 48.41
90	E 51.63	E 48.74
100	E 51.28	E 49.45



Table No. 24B

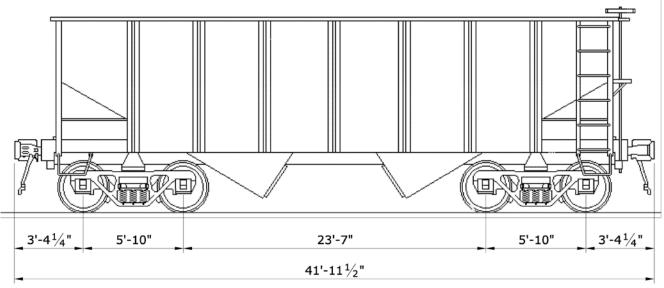






## 286,000 LBS AAR FREIGHT CAR

WEIGHT: 286,000 LBS GROSS (MAXIMUM)





AXLE LOAD DIAGRAM (SINGLE CAR)

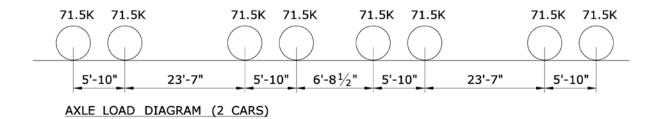
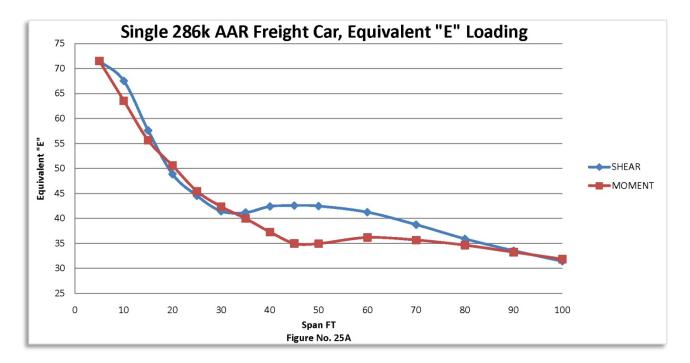


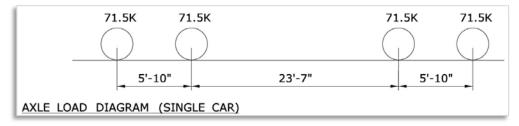
FIGURE NO. 25

Single 200k AAN Car Equivalent Cooper Load		
SPAN FT	SHEAR	MOMENT
5	E 71.50	E 71.50
10	E 67.53	E 63.56
15	E 57.60	E 55.61
20	E 48.86	E 50.56
25	E 44.52	E 45.43
30	E 41.42	E 42.34
35	E 41.15	E 39.95
40	E 42.38	E 37.25
45	E 42.56	E 34.98
50	E 42.45	E 34.93
60	E 41.22	E 36.16
70	E 38.73	E 35.65
80	E 35.89	E 34.63
90	E 33.51	E 33.22
100	E 31.41	E 31.83

Single 286k AAR Car Equivalent Cooper Load

Table No. 25A



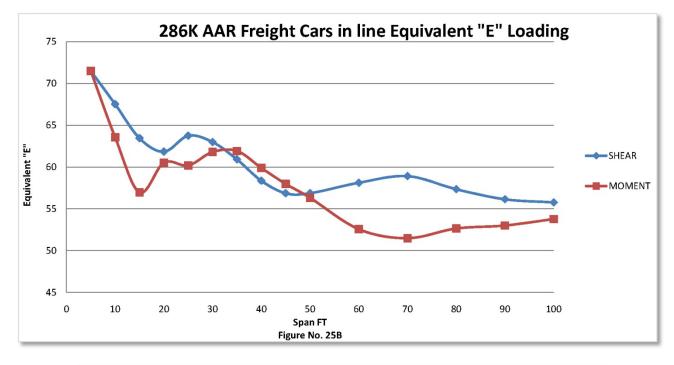


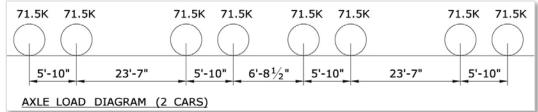


200K AAN TIEIg	200K AAK Freight Cars in Line Equivalent Cooper Load	
SPAN FT	SHEAR	MOMENT
5	E 71.49	E 71.50
10	E 67.52	E 63.56
15	E 63.45	E 56.96
20	E 61.84	E 60.52
25	E 63.75	E 60.20
30	E 62.99	E 61.81
35	E 60.91	E 61.92
40	E 58.36	E 59.89
45	E 56.86	E 57.98
50	E 56.87	E 56.33
60	E 58.12	E 52.57
70	E 58.91	E 51.49
80	E 57.35	E 52.64
90	E 56.15	E 53.00
100	E 55.77	E 53.77

286k AAR Freight Cars In Line Equivalent Cooper Load

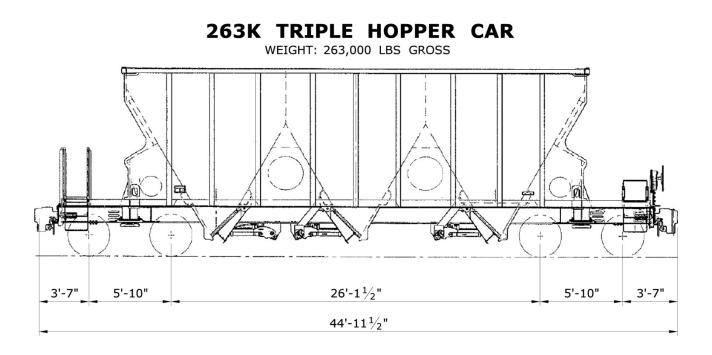
Table No. 25B





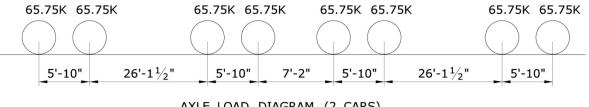
Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts





AXLE LOAD DIAGRAM (SINGLE CAR)



AXLE LOAD DIAGRAM (2 CARS)

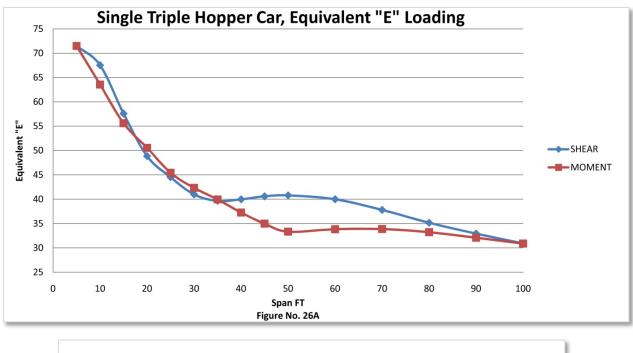
FIGURE NO. 26

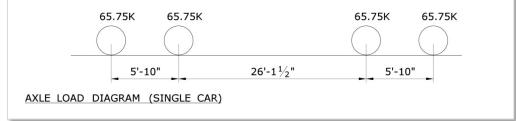


SPAN FT	SHEAR	MOMENT
5	E 65.75	E 65.75
10	E 62.10	E 58.45
15	E 52.97	E 51.14
20	E 44.93	E 46.49
25	E 40.94	E 41.78
30	E 37.69	E 38.93
35	E 36.46	E 36.74
40	E 36.76	E 34.26
45	E 37.32	E 32.17
50	E 37.50	E 30.66
60	E 36.77	E 31.11
70	E 34.75	E 31.15
80	E 32.33	E 30.55
90	E 30.28	E 29.50
100	E 28.44	E 28.40
	Table Nie 200	

Single 263k Triple Hopper Car Equivalent Cooper Load

Table No. 26A





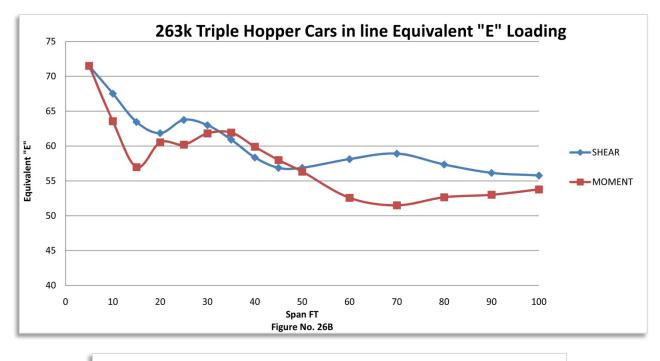
Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

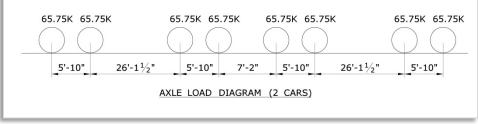
Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts

263K Triple Hopper Cars in Line Equivalent Cooper Loa		
SPAN FT	SHEAR	MOMENT
5	E 65.74	E 65.75
10	E 62.09	E 58.44
15	E 57.35	E 51.14
20	E 55.67	E 54.19
25	E 57.77	E 53.72
30	E 57.28	E 55.61
35	E 55.52	E 55.98
40	E 53.27	E 54.31
45	E 50.89	E 52.57
50	E 50.45	E 51.16
60	E 51.13	E 47.58
70	E 51.82	E 45.30
80	E 50.91	E 45.27
90	E 49.49	E 46.20
100	E 49.02	E 46.20

263k Triple Hopper Cars In Line Equivalent Cooper Load

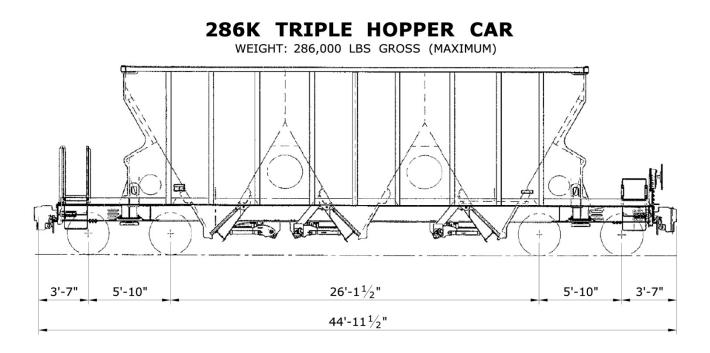
Table No. 26B





Load chart values are applicable to specific structural members. For limitations, see Executive Summary.







AXLE LOAD DIAGRAM (SINGLE CAR)

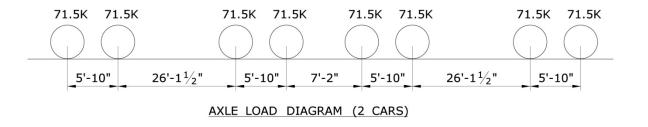


FIGURE NO. 27

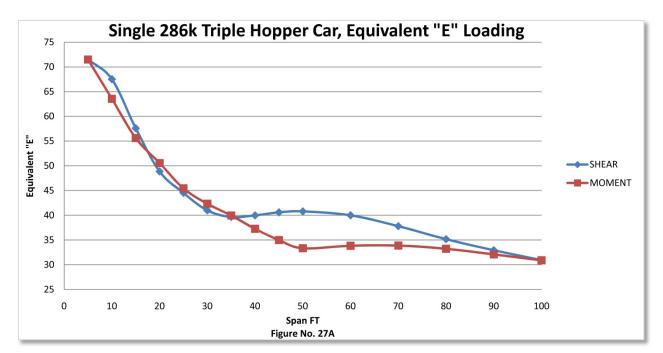
Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts

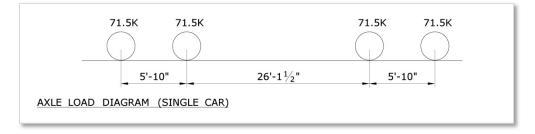
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•9.e =••••	le nopper car Equiva	
SPAN FT	SHEAR	MOMENT
5	E 71.50	E 71.50
10	E 67.53	E 63.56
15	E 57.60	E 55.61
20	E 48.86	E 50.56
25	E 44.52	E 45.43
30	E 40.98	E 42.34
35	E 39.65	E 39.95
40	E 39.97	E 37.25
45	E 40.59	E 34.98
50	E 40.78	E 33.34
60	E 39.99	E 33.83
70	E 37.79	E 33.88
80	E 35.16	E 33.22
90	E 32.93	E 32.09
100	E 30.93	E 30.89

Single 286k Triple Hopper Car Equivalent Cooper Load

Table No. 27A





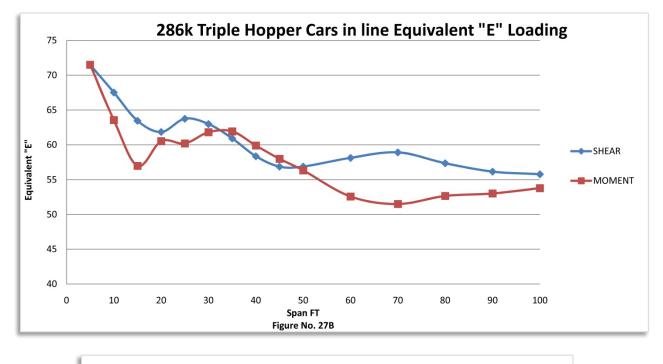
Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

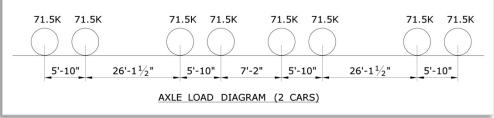


286к тгіріе норр	er Cars In Line Equiva	alent Cooper Loa
SPAN FT	SHEAR	MOMENT
5	E 71.49	E 71.50
10	E 67.52	E 63.56
15	E 62.36	E 55.61
20	E 60.53	E 58.93
25	E 62.83	E 58.42
30	E 62.29	E 60.48
35	E 60.37	E 60.88
40	E 57.92	E 59.06
45	E 55.34	E 57.16
50	E 54.86	E 55.64
60	E 55.60	E 51.74
70	E 56.35	E 49.27
80	E 55.36	E 49.23
90	E 53.82	E 50.24
100	E 53.31	E 50.24
	Table No. 270	

286k Triple Hopper Cars In Line Equivalent Cooper Load

Table No. 27B

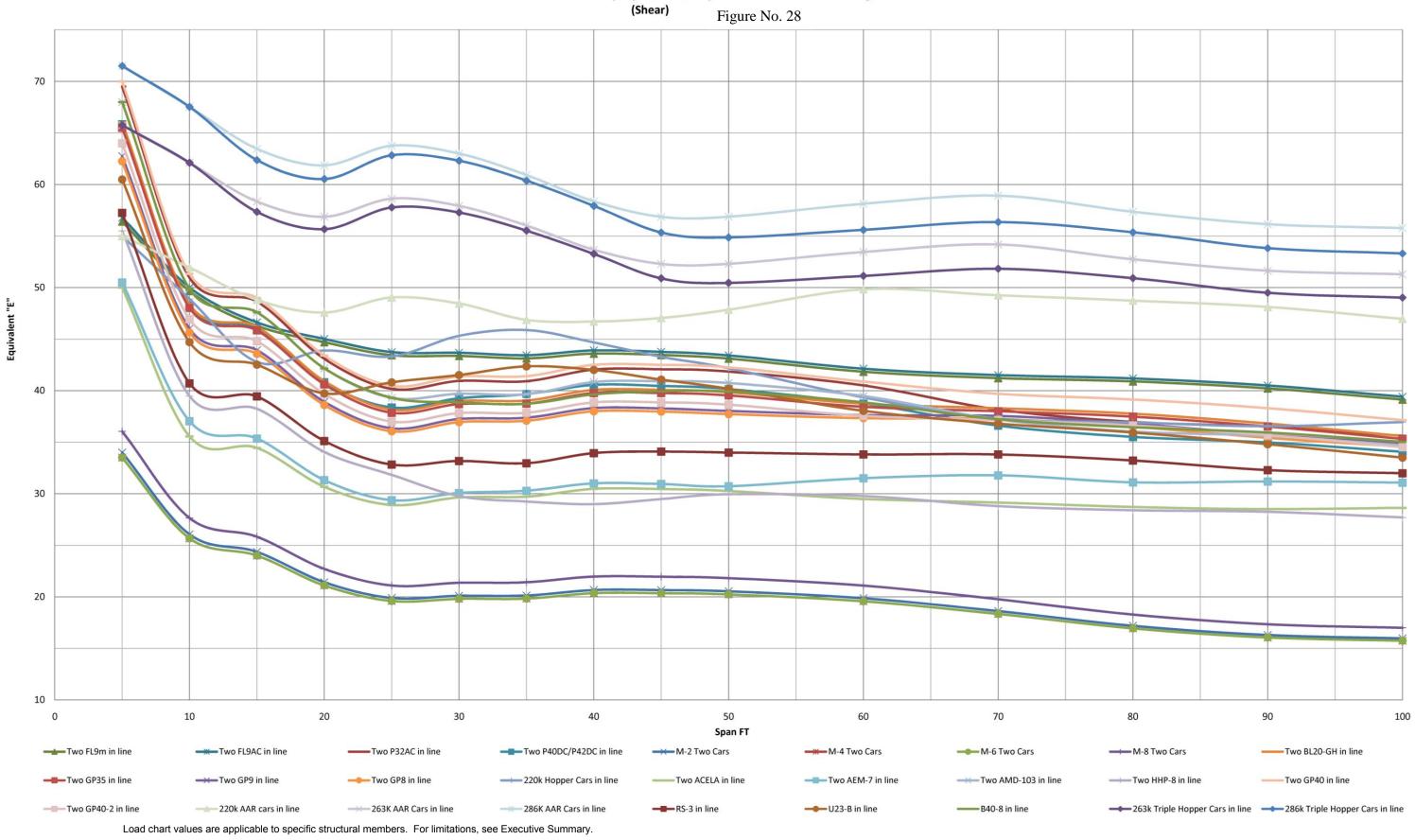




Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

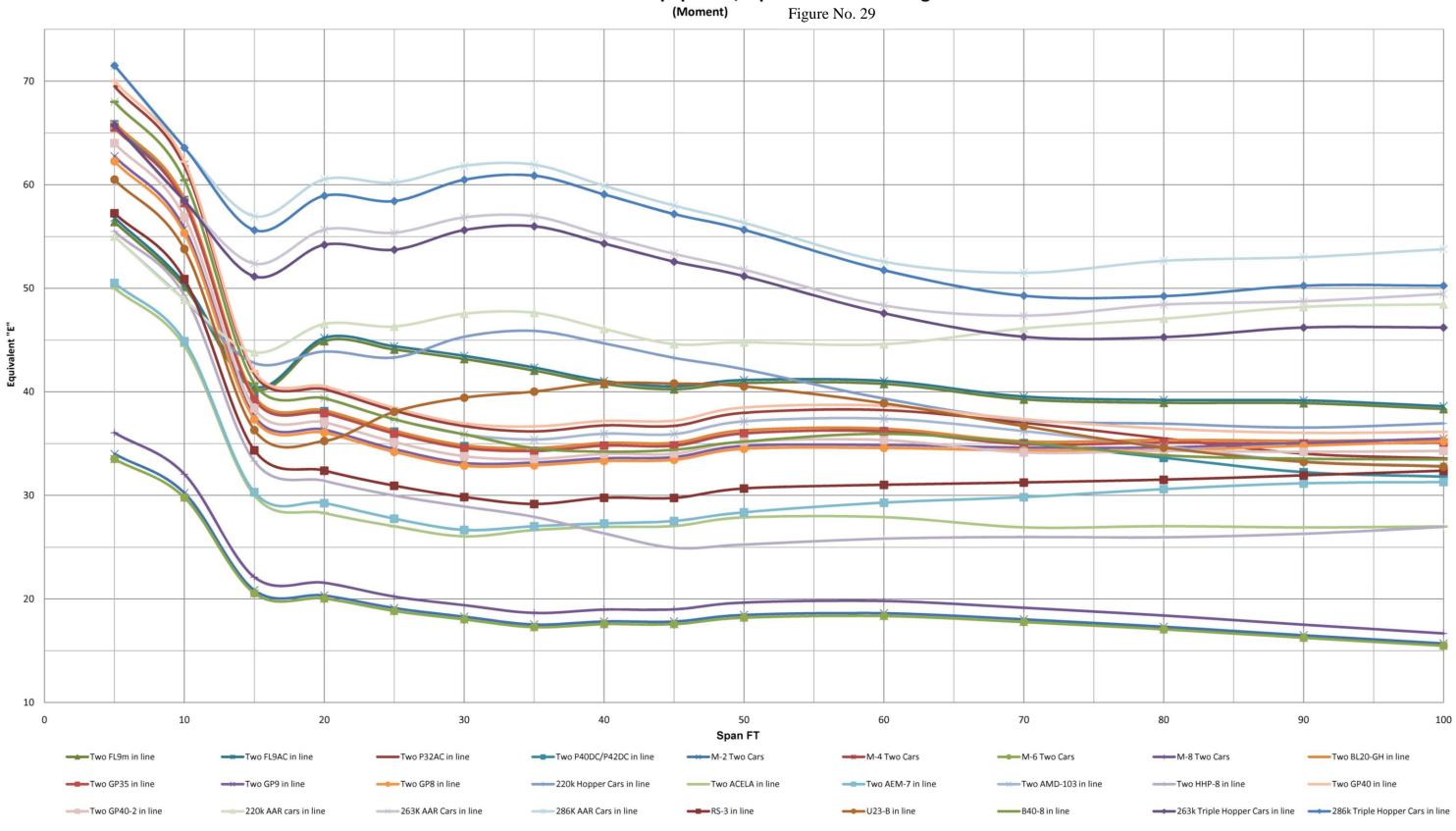
Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts LOCHNER

## Connecticut Railroad Equipment, Equivalent "E" Loading



LOCHNER

## Connecticut Railroad Equipment, Equivalent "E" Loading





## **ILLUSTRATIVE EXAMPLE NO 1:**

The following example will show how to make use of this manual to determine which equipment is safe to run across a specific bridge. Bridge No. 01318R Metro-North (Mainline) over Route 113 (Main Street), mile point 59.01 in Stratford, will be used in the following example. Information is taken from the Railroad Bridge Inspection, Load Rating report for ConnDot Bridge No. 01318R (Metro-North Bridge M.P. 59.01), dated November 12, 2009 and prepared by WSP-SELLS. Copies of the report pages containing information used in this example are attached as Figure Nos. 33, 34 and 35.

- STEP 1 To assess the adequacy of the structure to safely support Metro-North equipment, obtain the controlling member E ratings for the Structure from the Rating Summary. The Rating Summary for Bridge 01318R, tells us that the controlling member is Girder 4 (Figure No. 33) with a controlling E rating of E56 for Flexure and in the Girder 4 analysis from the load rating report gives us a controlling E rating of E59.8 for Shear.
- **STEP 2** Obtain the member span length from bridge structure data in the load rating for the controlling members, which in our case is girder 4. Page 93 of the load rating (Figure No. 35) shows an effective span length of the girder of 40.0'.
- STEP 3 Plot the E- rating and the span length on the Railroad equipment Equivalent "E" loading Diagrams for moment and shear. For the Shear diagram, locate the 40.0' span length of the member on the horizontal axis and locate the E59.8 rating on the vertical axis. The intersection of these two lines represents the capacity of the member. Load curves that lie above the intersection point represent equipment that will overstress the member. Load curves that lie below the intersection point represent equipment that can traverse the structure without overstressing the member. See Figure Nos. 30, and 31 for plotted results. From Figure 30, it is evident that two 286K AAR Cars in line produce the maximum Shear within a 40.0' span. This maximum Shear is an Equivalent Cooper Load of E57.97 (Table No. 22B). The E57.97 maximum Equivalent Cooper Load is less than the E59.8 shear capacity of the member. Therefore, the member is not overstressed in shear. From Figure 31, it is evident that the 286K AAR Cars and the Triple Hopper Cars load curves lie above the member capacity point, with their loads generating a moment that relates to Equivalent Cooper loads of E59.88 (Table No. 22B) and E58.97 (Table No. 23B) respectively. Therefore, those two pieces of equipment when operated in line will overstress the girder with a Moment capacity of E56 (Figure No. 34). From the diagram, the largest piece of equipment that traverses the structure without overstressing the girder is the 263K AAR car which produces a Cooper load of E55 for the 40.0' span (Table No.21B) which is less than the girder moment capacity of E56.

The equivalent Cooper loading summary (Table A) can also be used to determine a member's adequacy.

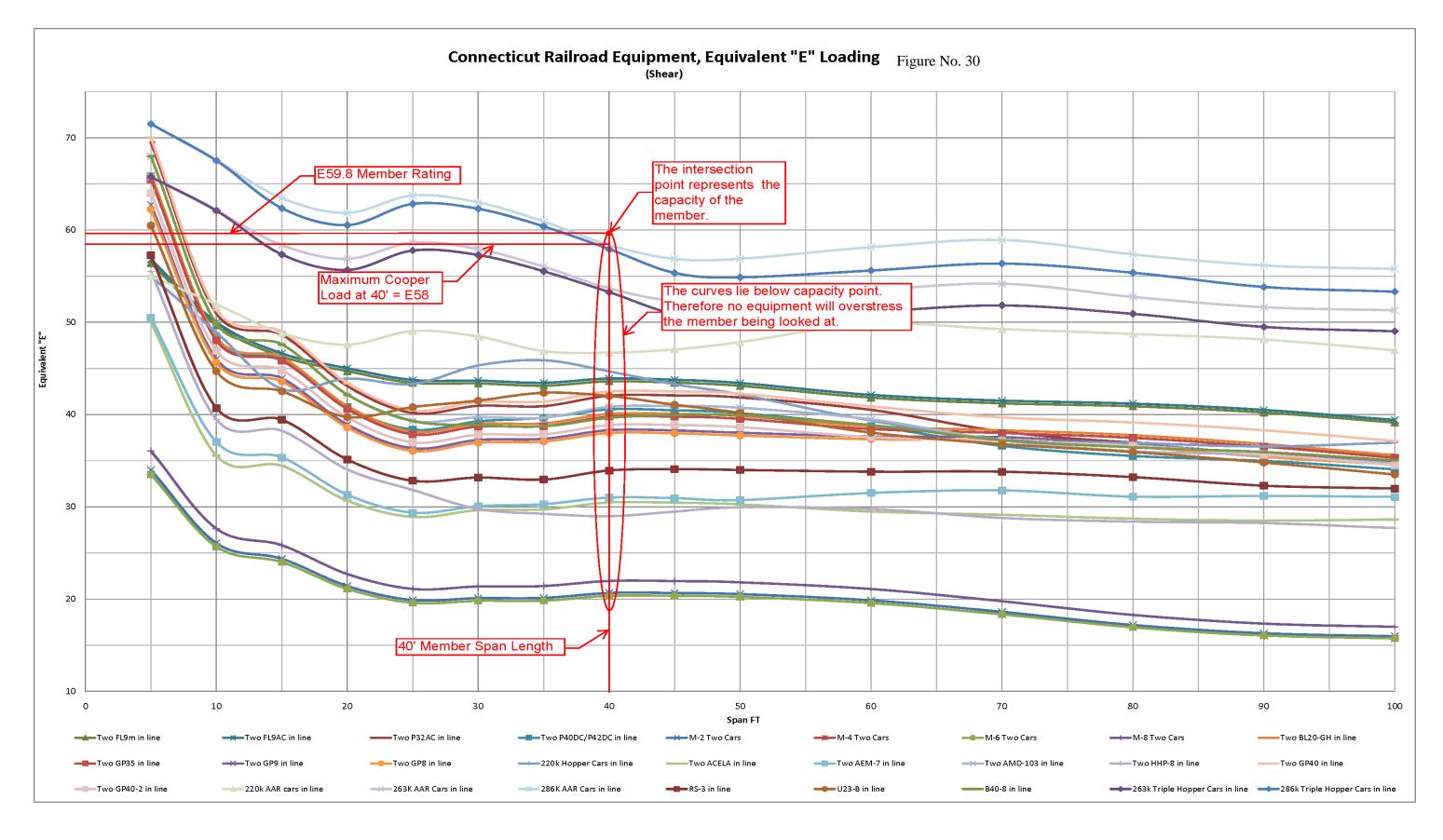
Locate the table for the specific equipment being checked. The P32AC locomotive (two engines in line) will be used for this example, see Figure No. 32.

**STEP 1** Locate the member span in the far left column. Follow that row horizontally to the appropriate E rating column for the specific equipment. The number in that box represents the Equivalent Cooper Load of the specific equipment for the given span. The E rating from the inspection report represents the capacity of the member. If the E rating from the table (load) is higher than the E rating from the Load Rating (capacity), the equipment will overstress the member. See Figure No. 31 for plotted results. The same method should be used for other types of equipment as required.



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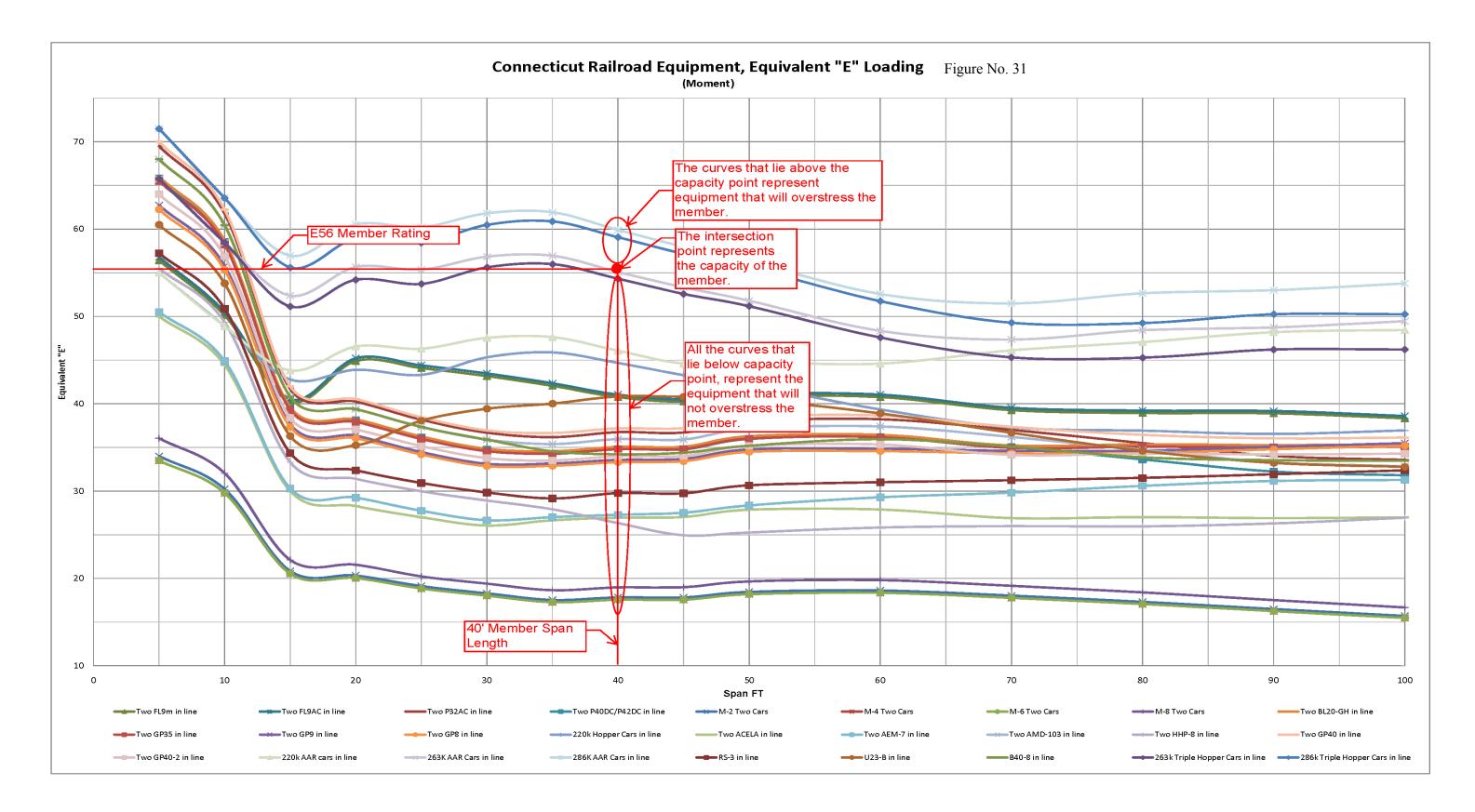




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Connecticut Department of Transportation Office of Rail Equivalent Cooper Load Charts

Load chart values are applicable to specific structural members. For limitations, see Executive Summary.



Load chart values are applicable to specific structural members. For limitations, see Executive Summary.



### CONNECTICUT RAILROAD BRIDGES TABLE A EQUIVALENT COOPER LOADING SUMMARY

		PER E80		FL9m Single				Two FL9m i				Two FL9m Bac		
span		Max. Moment		Max. Moment		ATING	Max. Shear	Max. Moment		ATING	Max. Shear	Max. Moment		ATING
(Feet)	(KIP)	(FT KIP)	(KIP)	(FT KIP)	SHEAR	MOMENT	(KI <b>P</b> )	(FT KIP)	SHEAR	MOMENT	(KIP)	(FT KIP)	SHEAR	MOMENT
5	80	100	56	71	E 56	E 56	56	71	E 56	E 56	56	70	E 56	E 56
10	120	225	74	141	E 50	E 50	74	141	E 50	E 50	74	141	E 50	E 50
15	160	500	93	252	E 46	E 40	93	252	E 46	E 40	90	252	E 45	E 40
20	200	825	112	462	E 45	E 45	112	463	E 45	E 45	110	463	E 44	E 45
25	227	1224	123	673	E 43	E 44	123	674	E 43	E 44	122	674	E 43	E 44
30	252	1642	131	885	E 42	E 43	137	886	E 43	E 43	133	886	E 42	E 43
35	277	2088	136	1096	E 39	E 42	149	1097	E 43	E 42	145	1098	E 42	E 42
40	302	2623	146	1308	E 39	E 40	165	1336	E 44	E 41	163	1309	E 43	E 40
45	327	3202	155	1519	E 38	E 38	178	1511	E 43	E 40	180	1592	E 44	E 40
50	349	3804	167	1731	E 38	E 36	188	1942	E 43	E 41	196	1946	E 45	E 41
60	392	5196	186	2212	E 38	E 34	205	2647	E 42	E 41	220	2768	E 45	E 43
70	442	6830	200	2848	E 36	E 33	228	3352	E 41	E 39	241	3588	E 44	E 42
80	497	8638	210	3534	E 34	E 33	254	4204	E 41	E 39	265	4503	E 43	E 42
90	549	10678	218	4233	E 32	E 32	276	5191	E 40	E 39	285	5490	E 42	E 41
100	600	12893	224	4938	E 30	E 31	293	6178	E 39	E 38	302	6610	E 40	E 41
100	000	12055	224	4950	2.50	LJI	233	01/0	L 33	1.30	302	0010	L 40	L 41
	600	PER E80		FL9AC Single	Engine			Two FL9AC i	in line			Two FL9AC Bac	kto Back	
			Mary Change			ATING	Mary Change			TINC	Mary Change			TINC
span		Max. Moment	Max. Shear	Max. Moment		ATING	Max. Shear	Max. Moment		ATING	Max. Shear	Max. Moment		ATING
(Feet)	(KIP)	(FT KIP)	(K1P)	(FT KIP)	SHEAR	MOMENT	(KIP)	(FT KIP)	SHEAR	MOMENT	(KIP)	(FT KIP)	SHEAR	MOMENT
5	80	100	57	71	E 57	E 57	57	71	E 57	E 57	57	71	E 57	E 57
10	120	225	75	142	E 50	E 50	75	142	E 50	E 50	75	142	E 50	E 50
15	160	500	93	253	E 47	E 41	93	253	E 47	E 41	91	253	E 45	E 41
20	200	825	113	465	E 45	E 45	113	466	E 45	E 45	111	466	E 44	E 45
25	227	1224	124	678	E 44	E 44	124	679	E 44	E 44	123	679	E 43	E 44
30	252	1642	132	891	E 42	E 43	138	892	E 44	E 43	134	892	E 43	E 43
35	277	2088	137	1104	E 40	E 42	150	1105	E 43	E 42	147	1105	E 43	E 42
40	302	2623	147	1317	E 39	E 40	166	1346	E 44	E 41	164	1318	E 44	E 40
45	327	3202	156	1530	E 38	E 38	179	1522	E 44	E 41	182	1603	E 44	E 40
50	349	3804	168	1743	E 39	E 37	189	1956	E 43	E 41	198	1960	E 45	E 41
60	392	5196	187	2228	E 38	E 34	206	2666	E 42	E 41	222	2788	E 45	E 43
70	442	6830	201	2868	E 36	E 34	229	3376	E 42	E 40	243	3614	E 44	E 42
80	497	8638	212	3559	E 34	E 33	256	4234	E 41	E 39	267	4534	E 43	E 42
90	549	10678	220	4264	E 32	E 32	278	5228	E 40	E 39	288	5528	E 42	E 41
100	600	12893	226	4974	E 30	E 31	296	6222	E 39	E 39	304	6657	E 41	E 41
	C00	PER E80		P32AC single	Engine			Two P32AC	in line			Two P32AC Bac	k to Back	
span		Max. Moment	Max Shear	Max. Moment		ATING	Max Shear	Max. Moment		ATING	Max. Shear			ATING
(Feet)	(KIP)	(FT KIP)	(KIP)	(FT KIP)	SHEAR	MOMENT	(KiP)	(FT KIP)	SHEAR	MOMENT	(KIP)	(FT KIP)	SHEAR	MOMENT
5	80	100	69	87	E 69	E 70	69	87	E 69	E 70	69	87	E 69	E 70
10	120	225	76	174	E 51	E 62	76	174	E 51	E 62	76	174	E 51	E 62
15	120	500	97	261		E 62	97	261		E 62	97	261	E 49	
					E 49				E 49					E 42
20	200	825	108	415	E 43	E 40	108	415	E 43	E 40		lent Cooper		
25	227	1224	114	584	E 40	E 38	114	584	E 40	E 38	Capaci	ty from load	rating =	E59.8 📙
30	252	1642	118	753	E 38	E 37	129	753	E 41	z 37	131	/55		
35	277	2088	121	922	E 35	E 35	142	944	E 41	E 36	145	982	E 42	E 38
40	302	2623	123	1091	E 33	E 33	159	1205	E 42	E 37	162	1242	E 43	E 38
45	327	3202	127	1260	E 31	E 31	172	1469	E 42	Load (	E42) is les	s than capa	city 📙	E 38
50	349	3804	135	1429	E 31	E 30	183	1805	E 42	(E59.8	), therefor	e the equipm	nent 📙	E 39
60	392	5196	156	1772	E 32	E 27	198	2483	E 41			ess the mem		E 39
70	442	6830	173	2177	E 31	E 26	211	3158	E 38					E 38
80	497	8638	186	2772	E 30	E 26	230	3832	E 37	E 35	233	3898	E 37	E 36
90	549	10678	197	3448	E 29	E 26	250	4541	E 36	E 34	253	4619	E 37	E 35
100	600	12893	205	4122	E 27	E 26	267	5410	E 36	E 34	270	5488	E 36	E 34
	C00	PER E80	F	40DC/P42DC sin	gle Engine			Two P40DC/P42	DC in line		Tw	o P40DC/P42DC	Back to Ba	:k
span				Max. Moment		ATING	Max. Shear	Max. Moment		ATING		Max. Moment		TING
(Feet)	(KIP)	(FT KIP)	(KIP)	(FT KIP)	SHEAR	MOMENT	(KIP)	(FT KIP)	SHEAR	MOMENT	(KIP)	(FT KIP)	SHEAR	MOMENT
5	80	100	66	82	E 66	E 66	66	82	E 66	E 65	66	82	E 66	E 66
10	120	225	72	165	E 48	E 59	72	165	E 48	E 59	72	165	E 48	E 59
15	120	500	92	247	E 46	E 59 E 40	92	247	E 46	E 39	92	247	E 46	E 59 E 40
20	200	825	102	393	E 40	E 40 E 38	102	393	E 40	E 38	102	393	E 40	E 40 E 38
25	200		102		E 41 E 38			553				553		
		1224		553		E 36	108		E 38	E 36	109		E 38	E 36
30	252	1642	112	713	E 36	E 35	122	713	E 39	E 35	124	713	E 39	E 35
35	277	2088	115	873	E 33	E 33	134	894	E 39	E 34	137	930	E 40	E 36
40	302	2623	117	1033	E 31	E 32	150	1141	E 40	E 35	153	1177	E 41	E 36
45	327	3202	120	1193	E 29	E 30	163	1392	E 40	E 35	165	1454	E 40	E 36
50	349	3804	128	1353	E 29	E 28	173	1710	E 40	E 36	175	1772	E 40	E 37
60	392	5196	148	1679	E 30	E 26	188	2352	E 38	E 36	190	2414	E 39	E 37
70	442	6830	164	2062	E 30	E 24	200	2992	E 36	E 35	202	3056	E 37	E 36
80	497	8638	177	2626	E 28	E 24	218	3530	E 35	E 34	221	3692	E 36	E 34
90	549	10578	186	3266	E 27	E 24	237	4301	E 35	E 32	240	4375	E 35	E 33
	600	12002	104	2004	E 26	5.24	252	5124	E 24	5.22	255	E100	5.24	E 22

Figure No. 32 Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

E 24



100

600

12893

194

3904

E 26

Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts

253

5124

E 34 E 32 255

E 34 E 32

5198

## BRIDGE 01318R OVER MAIN STREET (RTE 113)

### **Controlling Member:**

### Girder 4; Tracks 2 & 4

#### AS-BUILT RATING

Normal	Maximum	Fatigue
E	E	E
57	73	33

Flexure at Midspan controls for normal rating Flexure at Midspan controls for maximum rating Fatigue at Midspan controls

### **GIRDER RATINGS**

Tracks 2&4

#### AS-BUILT RATINGS PER TRACK

Girders 2 & 3 Control

indoio L o	0 00111101		
Normal	Maximum	Fatigue	
E	E	E	Track #1
57	73	33	

Flexure at Midspan controls for normal rating Flexure at Midspan controls for maximum rating Fatigue at Midspan controls

#### Girder 3 Controls

	Fatigue	Maximum	Normal
Track	E	E	E
	33	73	57

#2

Flexure at Midspan controls for normal rating Flexure at Midspan controls for maximum rating Fatigue at Midspan controls

#### Girder 2 Controls

al	Maximum	Fatigue	
	E	E	Track #3
	73	33	

Flexure at Midspan controls for normal rating Flexure at Midspan controls for maximum rating

Fatigue at Midspan controls

### Girder 4 Controls

Normal	Maximum	Fatigue	
E	E	E	Track #4
57	74	33	

Flexure at Midspan controls for normal rating Flexure at Midspan controls for maximum rating Fatigue at Midspan controls

#### AS-INSPECTED RATING

Girder 4 C	Irder 4 Control		
Normal	Maximum	Fatigue	
E	E	E	
56	72	32	

Flexure at Midspan controls for normal rating Flexure at Midspan controls for maximum rating Fatigue at Midspan controls

### AS-INSPECTED RATINGS PER TRACK

Girders 2	& 3 Control	
Normal	Maximum	Fatigue
E	E	E
57	73	33

Flexure at Midspan controls for normal rating Flexure at Midspan controls for maximum rating Fatigue at Midspan controls

Girder 4 C	Girder 4 Controls		
Normal	Maximum	Fatigue	
E	E	E	
56	72	32	

Flexure at Midspan controls for normal rating Flexure at Midspan controls for maximum rating Fatigue at Midspan controls

### Girder 2 Controls

Normal	Maximum	Fatigue
E	E	E
57	73	33

Flexure at Midspan controls for normal rating Flexure at Midspan controls for maximum rating Fatigue at Midspan controls

#### Girder 4 Controls

Normal	Maximum	Fatigue
E	E	E
56	72	32

Flexure at Midspan controls for normal rating Flexure at Midspan controls for maximum rating Fatigue at Midspan controls

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Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts

## LOCHNER

Calculated by: TA Date: 12-22-09 Checked by: MF Date: 1-6-10 WSP • SELLS Bridge 01318R MP 59.01 Girder 4 Rating ConnDOT Metro North Load Rating Job: 080080

Summary of Shear Ratings at Ends

As-Built Load Rating of:

Rating of: Shear at Ends of Girder 4, Span 2

Normal	Maximum
E	E
59.8	91.1

As-Inspected Load Rating of: Shear at Ends of Girder 4, Span 2

Normal	Maximum
E	E
59.8	91.1

\*Due to the end stringer bays being partly supported by the end floorbeams which directly transfer the load to the bearings via the girder bearing stiffeners, the live load span length for girder shear is shortened by the average end bay length.

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Figure No. 34



Calculated by: Checked by:		Date: 12-22-0 Date: 1-6-10	Bridge	VSP • SELLS 01318R MF Girder 4 Ratin	9 59.01	ConnDOT Metro North Load Rating Job: 080080
			ds (15-7.3.4.3, 15-1.3			
Fy	30	ksi	Yield strength (assur	ned)		
	0.490	kips/ft <sup>3</sup>	Steel unit weight		AREMA Table 15-1-5	5
	0.060	kips/ft <sup>3</sup>	Timber unit weight		AREMA Table 15-1-	5
	0.120	kips/ft <sup>3</sup>	Ballast unit weight		AREMA Table 15-1-5	5
к	0.200 0.7	kips/ft	Rail and details weig Strength Parameter	Contraction of the second s	AREMA 15-1.3.2 (b)	
BRIDGE STR	UCTURE	DATA				
Girder/FB/S	Str Syster	n	Bridge type			
	Ope		Deck type (Ballasted	or Open)		
	189		Year built			
	193		Rehabilitation year			
Rte 113	(Main S		Feature crossed			
		3 5	Number of spans	Spop 2		
		4	Number of girders in Number of tracks	Span 2		
		0 °	Skew angle in degre	85		
L	40.0				CL bearing to CL beari	ing)
Sg1		0 ft	Girder spacing in Tra	CONTRACTOR OF THE OWNER	3	
S <sub>g2</sub>		0 ft	Girder spacing in Tra			
Og2		0 ft	Max unbraced length			
S <sub>str</sub>		0 ft	Stringer spacing			
	12.0			on longth		
L <sub>FB1</sub>			Track 2 floorbeam sp			
L <sub>FB2</sub>		0 ft	Track 4 floorbeam sp			
Sr		5 ft	Spacing between rai			
		2	Number of rails supp	orted by Gird	ler	
As-Built Sect	ion Prop	erties of:	Girder 4	(from plan	s)	
	14" :	3/4"	Top flange plate	4		
	14" :	× 3/4"	Top flange plate	3		
	14" )	< 3/4"	Top flange plate	2		
		< 13/16"	Top flange plate	1		
6 ">	6" x	5/8"	Top Angle			
		( 11/16"	Web plate from bear	-	off	
	47 1/2" >		Web plate at Midspa	n		
6 "x	6" x	5/8"	Bottom Angle			
		< 11/16" < 11/16"	Bottom flange plate	1		
		< 11/16" < 5/8"	Bottom flange plate Bottom flange plate	2 3		
		< 5/8"	Bottom flange plate	4		
	47.	5 in	Web depth			
		8 in	Depth of girder betwe	een back of a	ingles	
	0.2	5 in	Vertical gap betweer	web edge a	nd angle	
_		2 in	Centroid of angle fro			
		3 in <sup>2</sup>	Area of angle		Angles	
	24.	1 in⁴	Moment of inertia of	angle, I <sub>x</sub>	Angles	
	0.62	5 in	Thickness of angles			_
	0.87		Rivet diameter			
	0.93		Punch hole diameter			
	10	0 in	Assumed rivet hole of	iameter		*

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Figure No. 35

Connecticut Department of Transportation Office of Rail Connecticut Railroad Equipment Equivalent Cooper Load Charts

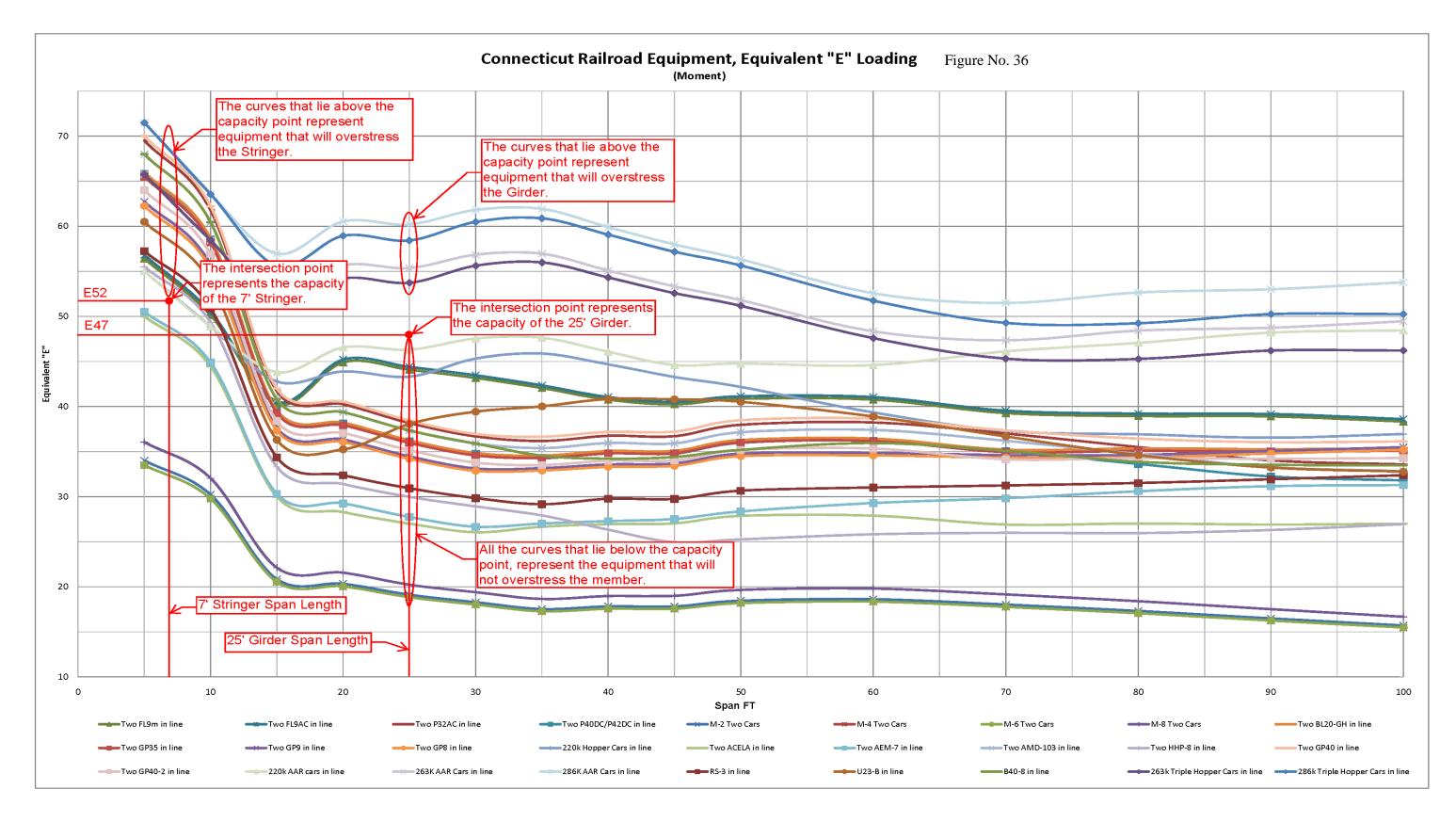
# LOCHNER

## **GENERAL EXAMPLE NO 2:**

The lowest E rating does not always represent the weakest member in the bridge. The capacity of a member depends upon its span length. The following general example will illustrate this. Assume a bridge has a 25'-0" span girder with an E47 rating for moment capacity and a 7'-0" span stringer with an E52 rating for moment capacity. From Figure 36, it can be seen that the Girder is adequate for all the equipment except the three heaviest freight cars. However, Figure 36 also shows that the stringer is overstressed by a majority of equipment except the M series Coaches and the lightest Amtrak equipment. In this case, the member with the lowest E rating (E47) does not control. Not only is it important to check the member with the lowest E-rating, but members with shorter span lengths must be checked as well.

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Connecticut Department of Transportation Office of Rail Equivalent Cooper Load Charts

Load chart values are applicable to specific structural members. For limitations, see Executive Summary.

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### **APPENDIX A**

### AAR Manual of Standards and Recommended Practices Design, Fabrication, and Construction of Freight Cars

M-1001

CHAPTER 2.

### CHAPTER 2. GENERAL DATA

### 2.1 Design Data

### 2.1.1 Scope

The basic design data in this section apply to all cars intended for interchange service.

### 2.1.2 Dimensions and Weights

### 2.1.2.1 Limiting Outline

Where cars are intended for unrestricted interchange service, they must comply with the requirements of Plates B and B-1 shown in *MSRP* Section C. Cars having outside dimensions that exceed those of Plate B to a limited extent may be built for limited interchange service provided none of these outside dimensions go beyond those as outlined in the appropriate plates.

### 2.1.2.2 Weight and Axle Spacing Criteria

Type	Maximum Tra	aximum Track Load (lb)		
Truck	Per Car	Per Axle	(in.)	
4 wheel	286,000	71,500	36	
6 wheel	315,000	52,500	30	
6 wheel	360,000	60,000	33	
6 wheel	394,500	65,750	36	

The following table was developed to show the minimum truck centers and minimum length over pulling faces for 70-ton and 100-ton freight cars having standard overhang dimensions.

Gross Rail Load (Ib)	Bolster Centerline to Striker Face (Overhang) (in.)	Truck Centers (Minimum)	Length Over Pulling Faces (Minimum)
286,000 and 263,000	4 ft 11 1/2 in.	29 ft 5 in.	41 ft 11 1/2 in.
	5 ft 5 ½ in.	28 ft 4 1/2 in.	41 ft 11 in.
	5 ft 11 1/2 in.	27 ft 4 in.	41 ft 10 1/2 in.
220,000	4 ft 11 1⁄2 in.	22 ft 6 in.	35 ft 0 1/2 in.
	5 ft 5 1⁄2 in.	21 ft 6 in.	35 ft 0 1/2 in.
	5 ft 11 1⁄2 in.	20 ft 6 in.	35 ft 0 1⁄2 in.

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# APPENDIX 10 LIST OF RAILROAD CONTACT PERSONS

### Off-System Railroad Contact Numbers

Railroad	Contact Person	Phone Number	Cell Number
Branford Steam Railroad	1 Forest Road, North Bran	ford, Connecticut 06471	
Vice President of Operations	Gary E. Wall	(203) 484-1412	
Train Dispatcher	Various	(203) 484-1409	
C & S	Jim Laske	(203) 484-1407	
Track & Bridge	Jim Laske	(203) 484-1407	
Email Address			

### Central New England Railroad

#### 44 Cedar Ridge Road, Newington, Connecticut 06111

Railroad Owner	Amedde Belliveau	(860) 666-1636	(860) 841-2802
Train Dispatcher	Amedde Belliveau	(860) 666-1636	(860) 841-2802
C & S	Amedde Belliveau	(860) 666-1636	(860) 841-2802
Track & Bridge	Amedde Belliveau	(860) 666-1636	(860) 841-2802
Email Address	support@cnzrr.com		

#### **Connecticut Southern Railroad** 1 Depot Street, Palmer, Massachusetts 01069 General Manager (802) 527-3401 (802) 393-0349 Steve Coomes Train Dispatcher (860) 291-1700 Various C & S Ron Bocash (802) 527-3461 (802) 373-8397 Track & Bridge Jim Bahlke (802) 527-3585 (802) 373-2781 Safety Manager (972) 743-0403 Tres C. Meyer Email Address steve.coomes@railamerica.com

#### CSX 500 Water Street, Jacksonville, Florida 32202 Railroad President Michael Ward Train Dispatcher/Railroad Emergency (800) 232-0144 Various Principal Engineer Carl Roe (904) 359-1036 Track & Bridge Supervisor (413) 427-9859 Roy Squires Yard Master (North Haven) (203) 786-6505 Tom Hollingworth Email Address Carl\_Roe@csx.com

#### **Housatonic Railroad**

#### P.O. Box 1146, Canaan, Connecticut 06018-1146

Railroad President	John Hanlon	(860) 824-0850, ext. 16	
Train Dispatcher	Various	(860) 824-0850, ext. 12	
C & S	Mat Boardman	(860) 824-0850, ext. 17	(860) 307-7021
Track & Bridge	Mat Boardman	(860) 824-0850, ext. 17	(860) 307-7021
Email Address	mat.boardman@hrr.com; r.abramson@hrrc.com		

### Naugatuck Railroad

#### 242 East Main Street, Thomaston, Connecticut 06787

j			
Railroad Owner	Ralph Harris	(860) 283-7245	
Train Dispatcher	Ralph Harris	(860) 283-7245	
C & S	Ralph Harris	(860) 283-7245	
Track & Bridge	Ralph Harris	(860) 283-7245	
Email Address	president@rmne.org		

#### New England Central Railroad 1 Depot Street, Palmer, Massachusetts, 01069 General Manager (802) 393-0349 (802) 527-3401 Steve Coomes Train Dispatcher Various (802) 527-3500 C & S Ron Bocash (802) 527-3461 (802) 373-8397 Track & Bridge Jim Bahlke (802) 527-3585 (802) 373-2781 Safety Manager (972) 743-0403 Tres C. Meyers Email Address steve.coomes@railamerica.com

### Pan Am Railways (Boston & Maine) 1700 Iron Horse Park, North Billerica, Massachusetts 01862

Railroad President	David Fink	(978) 663-1131	
Operations Center	Various	(800) 955-9208	
C & S	Tim Kunzuer	(978) 663-6961	
Track & Bridge	George Thayer	(978) 663-6973	
Email Address	customerservice@panamrailways.com		

### Providence & Worcester

### 75 Hammond Street, Worcester, Massachusetts 01610

Railroad President	P. Scott Conti	(508) 755-4000	
Train Dispatcher	Various	(508) 755-4000 ext. 400	
Director of Engineering	Bernie Cartier	(508) 755-4000 ext. 380	(508) 726-9644
C & S	Denis Glaude	(508) 755-4000 ext. 280	(508) 726-8935
Track & Bridge	Dick Ross	(508) 755-4000 ext. 252	(508) 294-9001
Email Address	CustomerServiceResponse	@pwrr.com	

### Valley Railroad 1 Railroad Avenue, P.O. Box 452, Essex, Connecticut 06426

Railroad President	Robert Bell	(860) 767-0103	
Train Dispatcher	Various	(860) 767-2021	
C & S	Robert Bradway	(860) 767-0103	
Track & Bridge	Robert Bradway	(860) 767-0103	
Email Address	valley.railroad@snet.net	•	•

### Metro North Railroad contact numbers

Metro North Railroad (MNR)	347 Madison Av	venue, New York, N	Y 10017-3739	
	Contact Person	Phone Number	Cell Phone Number	E-mail address
Railroad Traffic Control (RTC)		(800) 724-3005		
Track & Structure Department	Leon Kagan	(203) 975-4131		kagan@mnr.org
Track & Structure Department	Alan Morton	(845) 905-3834	(646) 281-4103	morton@mnr.org
	Thomas McLoughlin	(646) 765-1802	(203) 710-4283	mcloughlin@mnr.org
Bridge Strikes - Contact	Donald Migliozzi	(917) 295-7091	(203) 988-5822	Migliozzi@mnr.org
	Alan Morton	(845) 905-3834	(646) 281-4103	morton@mnr.org

## 247 Madinary America Name Varila NIV 10017 2720

### **Office of Rail contact numbers**

	Beer	,	
	<b>Contact Person</b>	Phone Number	E-mail address
Office of Rail Design	Haresh Dholakia	(860) 594-3173	hareshkumar.dholakia@ct.gov
	Gustavo Melo	(860) 594-2888	gustavo.melo@ct.gov
	Rosmery Rodriguez	(860) 594-2127	rosmery.rodriguez@ct.gov
	Jacob Booth	(860) 594-2862	jacob.booth@ct.gov

### Office of Rail 2800 Berlin Turnpike, Newington, CT 06131-7546

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## APPENDIX 11 INSPECTION SCHEDULE FOR METRO-NORTH RAILROAD BRIDGES

Track Chart	Town	Bridge	Location	Mile	Deck Area	Last Day	propose	cycle ed Date of utine	Date of \	e proposed /erification ection	Roι	duled Itine Ition by	Verifi	eduled ication ction by	Last I Rou Inspe	tine	Verifi	Day of ication ection
Name		No.		Point		of Insp.	2011	2012	2011	2012	2011	2012	2011	2012	2011	2012	2011	2012
DANBURY	NORWALK	04134R	MARSHALL STREET	0.11	530	04/03/09	04/03/11			04/02/12	10/18/11			10/17/12	11/14/11			
DANBURY	NORWALK	08200R	ANN STREET	0.19	2052	05/15/09	05/15/11			05/14/12	10/26/11			10/25/12	11/15/11			
DANBURY	NORWALK	08225R	REED STREET	0.36	2816			01/01/12	01/01/11			01/03/12	09/15/11		10/05/11			
DANBURY	NORWALK	08201R	NORWALK RIVER - STEEL GIRDER	1.56	2614	04/01/09	04/01/11			03/31/12	10/13/11			10/12/12	10/21/11			
DANBURY	NORWALK	08202R	NORWALK RIVER - STEEL GIRDER	3.20	2117	03/19/10		03/18/12	03/19/11			05/27/12	10/11/11				10/05/11	1
DANBURY	WILTON	08203R	BROOK - MULTIBEAM DECK	5.12	144	06/01/09	06/01/11			05/31/12	11/02/11			11/01/12	10/28/11			1
DANBURY	WILTON	08204R	STREAM - MULTIBEAM DECK	6.43	360	06/01/09	06/01/11			05/31/12	11/02/11			11/01/12	10/28/11			
DANBURY	WILTON	08205R	NORWALK RIVER - STEEL GIRDER	6.64	495	06/03/09	06/03/11			06/02/12	11/04/11			11/03/12	10/31/11			
DANBURY	WILTON	08206R	NORWALK RIVER - STEEL GIRDER	8.70	510	06/03/09	06/03/11			06/02/12	11/04/11			11/03/12	10/31/11			
DANBURY	WILTON	08207R	NORWALK RIVER - STEEL GIRDER	9.42	672	03/16/10		03/15/12	03/16/11			05/14/12	10/05/11				09/26/11	
DANBURY	WILTON	08208R	BROOK - CULVERT MASONRY	9.91	252	03/18/10		03/17/12	03/18/11			05/16/12	10/07/11				09/26/11	
DANBURY	WILTON	08209R	OLD MILL ROAD	11.01	320	08/21/09	08/21/11			08/20/12	11/11/11			11/10/12	10/26/11			1
DANBURY	WILTON	08210R	NORWALK RIVER - STEEL GIRDER	11.55	1570	04/26/10		04/25/12	04/26/11			07/04/12	11/22/11				10/06/11	1
DANBURY	WILTON	08211R	FACTORY POND	12.17	588	04/01/09	04/01/11			03/31/12	10/13/11			10/12/12	10/26/11			1
DANBURY	REDDING	08213R	OLD REDDING ROAD	14.16	208	08/21/09	08/21/11			08/20/12	11/11/11			11/10/12	10/26/11			
DANBURY	REDDING	08214R	SIMPAUG TURNPIKE	14.80	199	03/24/10		03/23/12	03/24/11			06/01/12	10/24/11				10/04/11	
DANBURY	REDDING	08215R	UMPAWAUG POND BROOK - STEEL GIRDER	16.41	248	06/02/09	06/02/11			06/01/12	11/03/11			11/02/12	10/17/11			1
DANBURY	REDDING	08216R	SAUGATUCK RIVER - STEEL GIRDER	17.09	392	06/02/09	06/02/11			06/01/12	11/03/11			11/02/12	10/17/11			
DANBURY	BETHEL	01020R	GRASSY PLAIN ROAD (ROUTE 53)	19.64	468	03/29/10		03/28/12	03/29/11			06/06/12	10/26/11					
DANBURY	BETHEL	08217R	BROOK - CONCRETE	19.99	390	02/22/10		02/22/12	02/22/11			04/22/12	09/16/11				10/04/11	
DANBURY	BETHEL	08218R	SYMPAUG BROOK - MULTIBEAM DECK	21.41	216	11/20/09	11/20/11			11/19/12	12/20/11			12/19/12	12/07/11			
DANBURY	DANBURY	08219R	SYMPAUG BROOK - CONCRETE SLAB	21.52	216	11/20/09	11/20/11			11/19/12	12/20/11			12/19/12	12/07/11			
DANBURY	DANBURY	08220R	STILL RIVER - STEEL GIRDER	22.39	2520	03/17/10		03/16/12	03/17/11			05/15/12	10/06/11				09/27/11	
DANBURY	DANBURY	05100R	STILL RIVER - CONCRETE CULVERT	22.94	1680	03/04/10		03/03/12	03/04/11			05/02/12	09/22/11				09/27/11	
DANBURY	DANBURY	08223R	STILL RIVER - CONCRETE CULVERT	23.18	8820	03/15/10		03/14/12	03/15/11			05/13/12	10/04/11					
DANBURY	DANBURY	04290R	STILL RIVER - CONCRETE CULVERT	23.42	2537	03/18/10		03/17/12	03/18/11			05/26/12	10/10/11					
MAINLINE	GREENWICH	03943R	NORTH WATER STREET	26.10	2068	03/10/10		03/09/12	03/10/11		03/16/11			03/15/12	05/02/11			
MAINLINE	GREENWICH	08000R	PEDESTRIAN UNDERPASS AT HAMILTON AVENUE	26.79	684	03/10/10		03/09/12	03/10/11		03/17/11			03/16/12	05/02/11			
MAINLINE	GREENWICH	03945R	HAMILTON AVENUE	26.79	1296	03/10/10		03/09/12	03/10/11		03/17/11			03/16/12	05/02/11			
MAINLINE	GREENWICH	03947R	FIELD POINT ROAD	27.77	2160	03/13/09	03/13/11			03/12/12	03/10/11			03/09/12	05/02/11			
MAINLINE	GREENWICH	03673R	ARCH STREET (SR 742)	28.06	5304	11/04/09	11/04/11			11/03/12	03/25/11			03/24/12	05/02/11			
MAINLINE	GREENWICH	08002R	PEDESTRIAN OVERPASS FOOTBRIDGE	28.16	385	04/06/09	04/06/11			04/05/12	04/25/11			04/24/12	04/25/11			
MAINLINE	GREENWICH	03946R	STEAMBOAT ROAD	28.22	3068	03/10/10		03/09/12	03/10/11		03/18/11			03/17/12	04/25/11			
MAINLINE	GREENWICH	08003R	DAVIS AVENUE #2	28.48	1824	03/10/10		03/09/12			03/21/11			03/20/12	05/04/11			
MAINLINE	GREENWICH	08004R	INDIAN HARBOR (DAVIS MILL POND)	28.68	3400	03/11/10		03/10/12	03/11/11			05/09/12	10/03/11				10/11/11	
MAINLINE	GREENWICH	08005R	SACHEM ROAD	29.50	2240	03/13/09	03/13/11			03/12/12	03/15/11			03/14/12	05/04/11			
MAINLINE	GREENWICH	08006R	LUKES CROSSING (SOUND SHORE DRIVE)	29.68	1672	03/13/09	03/13/11			03/12/12	03/16/11			03/15/12	05/04/11			
MAINLINE	GREENWICH	08008R	MIANUS RIVER (COS COB BRIDGE) - MOVABLE	29.90	58262		11/02/11			11/01/12					11/02/01			
MAINLINE	GREENWICH	08009R	ARCH STREET #2 - MASONRY ARCH	31.03	1330	11/05/09	11/05/11			11/04/12	03/11/11				05/04/11			
MAINLINE	GREENWICH	08010R	STREAM - MASONRY CULVERT	31.12	1120	11/10/09	11/10/11			11/09/12	04/26/11			04/25/12	04/26/11			
MAINLINE	GREENWICH	03948R	SOUND BEACH AVENUE	31.29	3416	03/26/10		03/25/12	03/26/11		03/17/11			03/16/12	05/03/11			
MAINLINE	GREENWICH	03955R	TOMAC AVENUE	31.62	1800	03/09/10		03/08/12	03/09/11		03/18/11			03/17/12	05/03/11			
MAINLINE	STAMFORD	03680R	GREENWICH AVENUE	32.81	3465	10/20/08	10/20/10			10/20/11	03/15/11			03/14/12	05/03/11			
MAINLINE	STAMFORD	08011R	RIPPOWAM RIVER - TRUSS	32.85	7280	04/30/09	04/30/11			04/29/12	04/27/11			04/26/12	04/28/11			

Track Chart	Town	Bridge	Location	Mile	Deck Area	Last Day	propose	cycle ed Date of utine	Date of V	e proposed /erification ection	Sche Rou Inspec		Verifi	eduled cation ction by		Day of Itine ection	Last D Verific Inspe	
Name	10will	No.	Location	Point	Deon Areu	of Insp.	2011	2012	2011	2012	2011	2012	2011	2012	2011	2012	2011	2012
MAINLINE	STAMFORD	03683R	WASHINGTON BOULEVARD (STATE ROUTE 493)	32.97	7700	03/13/09	03/13/11			03/12/12	03/01/11			02/29/12	05/03/11			
MAINLINE	STAMFORD	08012R	ATLANTIC STREET	33.19	4580	10/21/08	10/21/10			10/21/11	03/03/11			03/02/12	05/05/11			
MAINLINE	STAMFORD	03678R	CANAL STREET	33.41	5850	10/20/08	10/20/10			10/20/11	03/07/11			03/06/12	05/05/11			
MAINLINE	STAMFORD	03686R	ELM STREET	33.75	6144	10/20/08	10/20/10			10/20/11	03/08/11			03/07/12	05/06/11			
MAINLINE	STAMFORD	02237R	EAST MAIN STREET (U.S. ROUTE 1)	34.17	6825	10/21/08	10/21/10			10/21/11	04/12/11			04/11/12	05/05/11			
MAINLINE	STAMFORD	08013R	HAMILTON AVENUE	34.72	3060	03/09/10		03/08/12	03/09/11		03/14/11			03/13/12	05/05/11			
MAINLINE	STAMFORD	08014R	NOROTON RIVER - MASONRY ARCH	35.58	2646	04/05/10		04/04/12	04/05/11			06/13/12	10/28/11				10/12/11	
MAINLINE	DARIEN	08015R	NOROTON STATION PEDESTRIAN OVERPASS	36.24	525	10/22/09	10/22/11			10/21/12	11/21/11			11/20/12	11/08/11			
MAINLINE	DARIEN	08016R	STONY BROOK - MASONRY CULVERT	37.16	812	04/05/10		04/04/12	04/05/11			06/13/12	10/27/11				12/15/11	
MAINLINE	DARIEN	08017R	CUMMINGS BROOK - MASONRY CULVERT	37.36	360	08/13/09	08/13/11			08/12/12	10/12/11			10/11/12	05/18/11			
MAINLINE	DARIEN	04142R	LEROY AVENUE #1	37.59	1616	03/08/10		03/07/12	03/08/11			05/06/12	09/26/11				12/19/11	
MAINLINE	DARIEN	00316R	BOSTON POST ROAD (U.S. ROUTE 1)	37.82	3840	04/26/10		04/25/12	04/26/11			07/04/12	11/28/11				12/15/11	
MAINLINE	DARIEN	08018R	GOODWIVES RIVER - CONCRETE DECK	38.00	1144	08/13/09	08/13/11			08/12/12	10/12/11			10/11/12	05/18/11			
MAINLINE	DARIEN	08020R	TOKENEKE BROOK - 2 STONE CULVERTS	38.53	720	12/07/09	12/07/11			12/06/12	12/27/11			12/26/12	05/19/11			
MAINLINE	DARIEN	08021R	TOKENEKE BROOK #2 - MASONRY CULVERT	38.68	600	08/13/09	08/13/11			08/12/12	10/12/11			10/11/12	05/19/11			 I
MAINLINE	DARIEN	04271R	RAYMOND STREET	38.93	1392	12/08/09	12/08/11			12/07/12	12/28/11			12/27/12				 I
MAINLINE	DARIEN	08022R	FIVE MILE RIVER - BRICK ARCH	39.07	2795	02/11/09	02/11/11			02/11/12	06/11/11			06/10/12				 I
MAINLINE	NORWALK	08023R	DRY STREAM - MASONRY CULVERT	39.08	384	03/08/10		03/07/12	03/08/11			05/06/12	09/23/11				10/10/11	 I
MAINLINE	NORWALK	04132R	ROWAYTON AVENUE	39.11	1778	02/17/09	02/17/11			02/17/12	09/22/11			09/21/12	12/19/11			 I
MAINLINE	NORWALK	08025R	FRANKLIN STREET (SPRING STREET)	40.89	3000	05/14/09				05/13/12	10/25/11			10/24/12				 I
MAINLINE	NORWALK	08026R	STATION UNDERPASS	41.02	500	11/06/09	11/06/11			11/05/12	12/06/11			12/05/12	05/17/11			 I
MAINLINE	NORWALK	08027R	MONROE STREET	41.12	7040	03/08/10		03/07/12	03/08/11			05/06/12	09/27/11					
MAINLINE	NORWALK	03693R	WASHINGTON & MAIN STREETS	41.28	7008	04/15/10		04/14/12	04/15/11			06/23/12	11/18/11					 I
MAINLINE	NORWALK	04288R	NORTH WATER STREET & NORWALK RIVER - MOVABLE	41.51	33840	03/22/10			03/22/11			05/30/12	10/13/11					
MAINLINE	NORWALK	04131R	FORT POINT STREET	41.79	2000	03/11/09	03/11/11			03/10/12	10/07/11				11/09/11			 I
MAINLINE	NORWALK	08028R	OSBORNE AVENUE	41.96	2064	02/17/09				02/17/12	09/23/11			09/22/12	10/27/11			
MAINLINE	NORWALK		EAST AVENUE	42.14	1968		03/13/11			03/12/12					11/10/11			 I
MAINLINE	NORWALK		BROOK - MASONRY CULVERT	42.26	480		01/13/11			01/13/12				04/14/12				
MAINLINE	NORWALK		STRAWBERRY HILL AVENUE	42.37	1767	02/17/09				02/17/12				07/25/12				 I
MAINLINE	WESTPORT	08290R	INDIAN RIVER - TWO CONCRETE PIPES 7' DIAMETER	43.80	3233		01/13/11			01/13/12				09/18/12				
MAINLINE	WESTPORT		SAUGATUCK AVENUE (ROUTE 136)	43.97	3024		02/18/11			02/18/12				08/01/12				 I
MAINLINE	WESTPORT		NEW STATION UNDERPASS	44.12	1021	01/11/10			01/11/11			03/11/12	09/15/11				10/10/11	 I
MAINLINE	WESTPORT		STATION UNDERPASS - CONCRETE CULVERT	44.20	785	04/02/09		-		04/01/12	10/14/11				09/06/11			 I
MAINLINE	WESTPORT		FERRY LANE & SAUGATUCK RIVER - MOVABLE	44.32	25190		02/08/11			02/08/12				07/24/12				
MAINLINE	WESTPORT	-		44.70	2200	03/04/09								-	10/19/11			
MAINLINE	WESTPORT		SHERWOOD MILL POND - CONCRETE CULVERT	45.75	336		10/13/11			10/12/12	11/21/11				12/19/11			
MAINLINE	WESTPORT		MUDDY BROOK - CONCRETE CULVERT	46.11	436		10/13/11			10/12/12					12/19/11			
MAINLINE	WESTPORT		NEW CREEK ROAD	47.15	1954		02/18/11			02/18/12				07/27/12				
MAINLINE	WESTPORT		GREENS FARMS BROOK - METAL PIPE CULVERT	47.29	600	10/13/09				10/12/12					12/19/11			
MAINLINE	WESTPORT		MAPLE LANE	47.44	2100	02/18/09				02/18/12					11/11/11			
MAINLINE	WESTPORT		SASCO RIVER - MULTIBEAM DECK	48.29	2000	08/10/09		1		08/09/12					12/19/11			
MAINLINE	FAIRFIELD		WESTWAY ROAD	48.65	2697		02/23/11	1			07/06/11			07/05/12				
MAINLINE	FAIRFIELD	_	CENTER STREET	48.81	1925	02/24/09					10/03/11				10/19/11			
MAINLINE	FAIRFIELD		SPRUCE STREET	48.91	1584	02/25/09		-		02/24/12	10/03/11				10/19/11			
MAINLINE	FAIRFIELD		OLD POST ROAD	49.01	1700		02/26/11				10/04/11				10/19/11			
		00042R		49.01	1700	02/20/09	02/20/11			02/20/12	10/03/11			10/04/12	10/19/11			ı

Track Chart	Town	Bridge	Location	Mile	Deck Area	Last Day	propose	cycle d Date of utine	Date of V	e proposed /erification ection	Rou	duled Itine Ition by	Verifi	eduled cation ction by	Last Da Routi Inspec	ne	Verific	Day of cation ection
Name		No.		Point		of Insp.	2011	2012	2011	2012	2011	2012	2011	2012	2011	2012	2011	2012
MAINLINE	FAIRFIELD	08043R	MILL RIVER - MULTIBEAM DECK	49.66	3744	08/11/09	08/11/11			08/10/12	10/10/11			10/09/12				
MAINLINE	FAIRFIELD	08044R	NORTH PINE CREEK ROAD	50.02	1680	03/30/09	03/30/11			03/29/12	10/11/11			10/10/12	10/24/11			
MAINLINE	FAIRFIELD	04197R	MILL PLAIN ROAD	50.29	1968	03/30/09	03/30/11			03/29/12	10/11/11			10/10/12	10/24/11			
MAINLINE	FAIRFIELD	04198R	ROUND HILL ROAD	50.90	1650	03/30/09	03/30/11			03/29/12	10/11/11			10/10/12	10/24/11			
MAINLINE	FAIRFIELD	01344R	NORTH BENSON ROAD (ROUTE 135)	51.12	1724	04/07/10		04/06/12	04/07/11			06/15/12	11/01/11					
MAINLINE	FAIRFIELD		FAIRFIELD CREEK - MULTIBEAM DECK	51.68	960	04/07/09	04/07/11			04/06/12	10/19/11			10/18/12				
MAINLINE	FAIRFIELD	08047R	ASH CREEK - MULTIBEAM DECK	53.00	1900	05/13/09	05/13/11			05/12/12	10/24/11			10/23/12				
MAINLINE	BRIDGEPORT	03635R	FAIRFIELD AVENUE (ROUTE 130)	53.42	5724	04/07/10		04/06/12	04/07/11			06/15/12	11/07/11					
MAINLINE	BRIDGEPORT		BOSTWICK AVENUE	53.60	3200	11/18/09	11/18/11			11/17/12	12/18/11			12/17/12				
MAINLINE	BRIDGEPORT		HANCOCK AVENUE	53.72	3400	04/07/10		04/06/12	04/07/11			06/15/12	11/02/11					
MAINLINE	BRIDGEPORT	_	HOWARD AVENUE	53.84	3264	09/28/09				09/27/12	11/02/11			11/01/12				
MAINLINE	BRIDGEPORT		WORDIN AVENUE	54.07	3564	04/07/10		04/06/12	04/07/11			06/15/12	11/04/11				10/24/11	
MAINLINE	BRIDGEPORT	_	IRANISTAN AVENUE	54.22	3496	04/07/10		04/06/12					11/03/11					
MAINLINE	BRIDGEPORT		SOUTH AVENUE	54.44	4224	04/08/10		04/07/12					11/11/11				10/26/11	
MAINLINE	BRIDGEPORT		PARK AVENUE	54.58	4752	04/08/10			04/08/11				11/14/11				10/25/11	
MAINLINE	BRIDGEPORT		MYRTLE AVENUE	54.70	2928	09/29/09		04/07/12	04/00/11	09/28/12	11/03/11	00/10/12	10/1-0/11	11/02/12			10/20/11	<u> </u>
MAINLINE	BRIDGEPORT		WARREN STREET	54.77	2436	02/22/10	00/20/11	02/22/12	02/22/11	00/20/12	11/00/11	04/22/12	09/16/11	11/02/12			10/14/11	<u> </u>
MAINLINE	BRIDGEPORT		LAFAYETTE STREET	54.84	2436	02/22/10			02/22/11			04/22/12					10/14/11	<u> </u>
MAINLINE	BRIDGEPORT		BROAD STREET	54.93	2990	03/24/10			03/24/11				10/25/11				12/15/11	
MAINLINE	BRIDGEPORT		MAIN STREET #1	54.98	3400	02/22/10		02/22/12				04/22/12					10/24/11	
MAINLINE	BRIDGEPORT	-	HOUSATONIC CROSSING	55.13	6090	11/19/09	11/10/11	02/22/12	02/22/11	11/18/12	12/19/11	04/22/12	03/13/11	12/18/12			10/24/11	
MAINLINE	BRIDGEPORT	-	SOUTH PEDESTRIAN OVERPASS	55.35	5653	04/29/10		04/28/12	04/29/11	11/10/12	12/13/11	07/07/12	12/13/11	12/10/12			12/05/11	
MAINLINE	BRIDGEPORT		UNION STREET	55.40	2552	03/30/09		04/20/12	04/23/11	03/29/12	10/12/11	01/01/12	12/13/11	10/11/12	11/21/11		12/03/11	
MAINLINE	BRIDGEPORT		BRIDGEPORT HARBOR - REINFORCED CONCRETE SLAE		51308	03/05/10		03/04/12	02/05/11	03/29/12	10/12/11	05/03/12	00/22/11	10/11/12	11/21/11			<u> </u>
MAINLINE	BRIDGEPORT		NORTH PEDESTRIAN OVERPASS	55.41	5211	03/03/10		03/04/12				07/07/12					12/05/11	l
MAINLINE	BRIDGEPORT	-	BRIDGEPORT HARBOR - PEDESTRIAN TUNNEL	55.50	1078		09/08/10		04/23/11	09/08/11	06/07/11	07/07/12	12/00/11	06/06/12			12/03/11	l
	BRIDGEPORT		STATION VIADUCT	55.50	22500		10/23/10			10/23/11				06/08/12	07/14/11			l
														06/07/12				<u> </u>
	BRIDGEPORT			55.61	9110		10/23/10 10/23/10				06/08/11 06/10/11			06/09/12				<b> </b>
	BRIDGEPORT BRIDGEPORT			55.69	70860													<b> </b>
		-	PEQUONNOCK RIVER - MOVABLE	55.90	32941	08/16/09				08/15/12				07/26/12				<b> </b>
	BRIDGEPORT			55.91	12875	01/09/09					06/21/11							<b> </b>
	BRIDGEPORT			55.95	4410		12/05/10				06/17/11 06/13/11			06/16/12				<b> </b>
	BRIDGEPORT			55.97	5735 5717		11/18/10 12/05/10							06/12/12				<b> </b>
	BRIDGEPORT			55.98							06/20/11							<b> </b>
	BRIDGEPORT	-		55.99	7788		11/20/10				06/14/11			06/13/12				<b> </b>
	BRIDGEPORT			56.00	4173		12/05/10				06/16/11			06/15/12				<u> </u>
	BRIDGEPORT		KOSSUTH STREET	56.10	2402		12/05/10		04/00/44	12/05/11	06/15/11	06/16/10	11/10/14	06/14/12	07/13/11		10/00/44	<u> </u>
	BRIDGEPORT		EAST MAIN STREET (ROUTE 127)	56.20	3400	04/08/10		04/07/12				06/16/12					12/08/11	<u> </u>
	BRIDGEPORT			56.35	3100	04/08/10			04/08/11			06/16/12					12/15/11	<u> </u>
	BRIDGEPORT	-		56.46	2950	04/08/10		04/07/12	04/08/11	00/11/10	40/44/4	06/16/12	11/08/11				12/15/11	<b> </b>
	BRIDGEPORT		YELLOW MILL POND - BRICK ARCH	56.68	1056	08/12/09				08/11/12				10/10/12				<b> </b>
MAINLINE	BRIDGEPORT			56.77	3534	09/30/09					11/16/11			11/15/12				<b> </b>
MAINLINE	BRIDGEPORT		BISHOP AVENUE	57.46	4320		10/01/11			09/30/12				11/04/12				<b> </b>
MAINLINE	BRIDGEPORT		BRUCE BROOK - CONCRETE PIPE 9' DIAMETER	57.54	3380		10/09/11			10/08/12				11/07/12				<b> </b>
MAINLINE	STRATFORD			57.62	2573	08/20/09				08/19/12	10/19/11			10/18/12				<b> </b>
MAINLINE	STRATFORD	08078R	WEST BROAD STREET	58.72	1919	08/20/09	08/20/11			08/19/12	10/19/11			10/18/12				1

Track Chart	Town	Bridge	Location	Mile	Deck Area	Last Day	propose		Date of V	e proposed /erification ection	Rou	eduled utine ction by	Sche Verifie Inspec	catio
Name		No.		Point		of Insp.	2011	2012	2011	2012	2011	2012	2011	20
MAINLINE	STRATFORD	08079R	KING STREET	58.88	1850	08/20/09	08/20/11			08/19/12	10/19/11			10/
MAINLINE	STRATFORD	01318R	MAIN STREET (ROUTE 113)	59.01	3692	11/12/09	11/12/11			11/11/12	12/12/11			12/
MAINLINE	STRATFORD	01312R	EAST MAIN STREET (ROUTE 110)	59.96	3308	08/20/09	08/20/11			08/19/12	10/19/11			10/
MAINLINE	STRATFORD	08080R	HOUSATONIC RIVER (DEVON BRIDGE) - MOVABLE	60.42	76433	09/23/09	09/23/11			09/22/12	08/08/11			08/
MAINLINE	MILFORD	08081R	BEAVER CREEK - MASONRY ARCH	61.62	384	02/12/09	02/12/11			02/12/12	09/20/11			09/
MAINLINE	MILFORD	08082R	BEARDSLEY AVENUE	62.94	1850	02/16/09	02/16/11			02/16/12	09/21/11			09/
MAINLINE	MILFORD	03640R	HIGH STREET	63.27	3135	02/19/09	02/19/11			02/19/12	09/28/11			09/
MAINLINE	MILFORD	03644R	RIVER STREET	63.44	3840	02/18/09	02/18/11			02/18/12	09/27/11			09/
MAINLINE	MILFORD	08083R	WEPAWAUG RIVER & PROSPECT STREET	63.53	6500	02/25/10		02/25/12	02/25/11			04/25/12	09/21/11	
MAINLINE	MILFORD	08084R	GULF STREET	63.83	900	05/18/09	05/18/11			05/17/12	10/27/11			10/
MAINLINE	MILFORD	08085R	PEDESTRIAN UNDERPASS AT GULF STREET	63.84	549	05/18/09	05/18/11			05/17/12	10/27/11			10/2
MAINLINE	MILFORD	08086R	INDIAN RIVER - MULTIBEAM DECK	64.59	1748	11/10/09	11/10/11			11/09/12	12/10/11			12/
MAINLINE	MILFORD	08087R	OLD GATE LANE	64.74	1600	05/18/09	05/18/11			05/17/12	11/01/11			10/
MAINLINE	MILFORD	08088R	QUIRK POND BROOK - MASONRY CULVERT	66.29	600	10/29/09	10/29/11			10/28/12	11/28/11			11/
MAINLINE	MILFORD	08090R	DEPOT ROAD	66.66	971	10/29/09	10/29/11			10/28/12	11/28/11			11/
MAINLINE	ORANGE	08091R	OYSTER RIVER - MASONRY CULVERT	67.50	1360	03/10/10		03/09/12	03/10/11			05/08/12	09/29/11	
MAINLINE	WEST HAVEN	08092R	STREAM - STONE/CONCRETE CULVERT	67.98	540	02/25/10		02/25/12	02/25/11			04/25/12	09/20/11	
MAINLINE	WEST HAVEN	08093R	MORGAN LANE	68.11	1550	05/18/09	05/18/11			05/17/12	10/31/11			10/
MAINLINE	WEST HAVEN	08096R	COVE RIVER - MASONRY CULVERT	69.19	1160	03/10/10		03/09/12	03/10/11			05/08/12	09/28/11	
MAINLINE	WEST HAVEN	01403R	SAW MILL ROAD (ROUTE 162)	69.66	3185	04/09/10		04/08/12	04/09/11			06/17/12	11/17/11	
MAINLINE	WEST HAVEN		CAMPBELL AVENUE	70.19	3050	04/09/10		04/08/12	04/09/11			06/17/12	11/16/11	
MAINLINE	WEST HAVEN	08098R	WASHINGTON AVENUE	70.36	2900	04/09/10		04/08/12	04/09/11			06/17/12	11/15/11	
MAINLINE	WEST HAVEN		WEST RIVER - CONCRETE BOX	71.26	7192	03/22/10			03/22/11			05/30/12		
MAINLINE	NEW HAVEN		STATION UNDERPASS	72.28	2880	06/02/10			06/02/11				12/15/11	
MAINLINE	NEW HAVEN		STATE STREET STATION OVERPASS	72.80	1413	04/30/10			04/30/11				12/14/11	_
NEW CANAAN			VIADUCT ROAD	2.81	1885	04/06/10			04/06/11		03/22/11			03/2
NEW CANAAN			NOROTON RIVER - MULTIBEAM DECK	3.77	296	03/31/09				03/30/12	07/29/11			07/
	NEW CANAAN	-	MERRITT PARKWAY (ROUTE 15)	5.76	1008	04/01/10		03/31/12	04/01/11		04/14/11			04/
	NEW CANAAN		OLD STAMFORD ROAD (ROUTE 106)	6.16	795	04/21/10			04/21/11		03/25/11			03/
	NEW CANAAN		WATERWAY - MASONRY CULVERT	7.00	787	04/21/10			04/21/11			06/29/12	11/21/11	
WATERBURY			UNNAMED STREAM - CONCRETE & RAIL DECK	3.80	160		10/12/11			10/11/12	11/18/11			11/
WATERBURY			BROOK - MASONRY CULVERT	4.14	552		10/12/11				11/18/11			11/
WATERBURY			BEARDS FARMWAY	4.55	528	06/04/09				06/03/12				11/
WATERBURY		-	FARMWAY CROSSING	4.88	572	06/04/09				06/03/12				11/
WATERBURY			GOLF CROSSING - REINFORCED CONCRETE SLAB	5.01	442		06/04/11			06/03/12				11/
WATERBURY				7.02	1696		10/12/11				11/18/11			11/
WATERBURY				7.36	560	04/28/10		04/27/12	04/28/11			07/06/12	12/06/11	_
WATERBURY		08258R DAVIS BROOK - 2 CONCRETE CULVERTS 08260R TWO MILE BROOK (TURKEY BROOK) - CONCRETE CUL 08261R STREAM - UNNAMED STREAM		7.92	660	04/28/10			04/28/11			-	12/07/11	-
WATERBURY			NAUGATUCK RIVER - STEEL TRUSS	8.62	3948		12/11/11		00	12/10/12	08/08/11			08/
WATERBURY			NAUGATUCK RIVER FLOOD PLAIN	8.68	4680	07/06/09				07/05/12			<u> </u>	11/
WATERBURY			NAUGATUCK RIVER - STEEL TRUSS	10.30	9827	09/16/09				09/15/12		1	<u> </u>	08/
WATERBURY			RACEWAY FARREL COMPANY - MASONRY ARCH	10.85	768		10/26/11				08/25/11			08/
WATERBURY			SPILLWAY	11.36	2366		10/26/11				08/26/11		<u> </u>	08/
WATERBURY			CANAL - STEEL GIRDER	12.57	1170	05/18/09				05/17/12				10/2
WATERBURY			MUD BROOK - MASONRY CULVERT	13.20	396	04/27/10		04/26/12	04/27/11	00/17/12	10/20/11	07/05/12	11/29/11	_
	SEYMOUR		MAIN STREET (ROUTES 115 & 313)	14.29	1664		04/08/11	07/20/12	J-1/21/11	04/07/12	10/00/44	01/03/12	11/23/11	10/

;	tions				
i	duled cation tion by	Last D Rout Inspec	ine	Last D Verific Inspe	ation
	2012	2011	2012	2011	2012
	10/18/12				
	12/11/12				
	10/18/12				
	08/07/12				
	09/19/12	10/25/11			
	09/20/12				
	09/27/12				
	09/26/12				
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				11/17/11	
	10/30/12				
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				12/14/11	
				12/14/11	
				12/09/11	
				11/23/11	
	03/21/12	05/06/11			
	07/28/12	04/29/11			
	04/13/12	05/06/11			
	03/24/12	05/06/11			
				10/07/11	
	11/17/12	12/06/11			
	11/17/12	12/06/11			
	11/06/12	11/04/11			
	11/06/12	11/04/11			
	11/06/12	11/04/11			
	11/17/12	12/06/11			
1					

12/13/11 12/13/11

12/13/11

08/07/12

08/10/12

08/25/12

11/08/12 12/05/11

08/24/12 12/01/11

10/27/12 11/01/11

10/19/12 11/22/11

Track Chart Name	Town	Bridge No.	Location	Mile Point	Deck Area	Last Day of Insp.	propose	cycle d Date of utine	Date of V			duled Itine Ition by	tine Verification tion by Inspection by		ication Routine		Last D Verific Inspe	cation
Name		NO.		1 Onic		or msp.	2011	2012	2011	2012	2011	2012	2011	2012	2011	2012	2011	2012
WATERBURY	SEYMOUR	08270R	JAMES STREET FOOTBRIDGE	14.45	608	04/08/09	04/08/11			04/07/12	10/20/11			10/19/12	11/03/11			
WATERBURY	SEYMOUR	01063R	DERBY AVENUE (ROUTE 67)	14.61	754	04/27/10		04/26/12	04/27/11			07/05/12	11/30/11					
WATERBURY	SEYMOUR+A1	08272R	NAUGATUCK RIVER - MULTIBEAM DECK	14.64	2763	04/02/09	04/02/11			04/01/12	10/17/11			10/16/12	11/15/11			
WATERBURY	BEACON FALLS	08274R	OLD PINE'S BRIDGE ROAD	16.79	150	06/05/09	06/05/11			06/04/12	11/08/11			11/07/12	11/02/11			
WATERBURY	BEACON FALLS	08275R	HEMP SWAMP BROOK - MASONRY CULVERT	16.81	1140	06/05/09	06/05/11			06/04/12	11/08/11			11/07/12	11/02/11			
WATERBURY	BEACON FALLS	08276R	SPRUCE BROOK - MULTIBEAM DECK	19.21	180	07/07/09	07/07/11			07/06/12	11/08/11			11/07/12	12/01/11			
WATERBURY	NAUGATUCK	08277R	SUGAR BUSH BROOK - MULTIBEAM DECK	20.11	180	10/27/09	10/27/11			10/26/12	11/26/11			11/25/12	12/02/11			
WATERBURY	NAUGATUCK	08278R	US RUBBER COMPANY - PEDESTRIAN TUNNEL	21.22	336	09/14/09	09/14/11			09/13/12	11/14/11			11/13/12	11/30/11			
WATERBURY	NAUGATUCK	08279R	PRIVATE ROAD AT UNIROYAL	21.36	504	09/14/09	09/14/11			09/13/12	11/14/11			11/13/12	11/30/11			
WATERBURY	NAUGATUCK	08280R	LONGMEADOW POND BROOK - CONCRETE ARCH	21.48	6240	10/02/09	10/02/11			10/01/12	11/17/11			11/16/12	12/01/11			
WATERBURY	NAUGATUCK	08281R	OVERFLOW - CONCRETE SLAB	21.65	5440	02/22/10		02/22/12	02/22/11			04/22/12	09/20/11				11/22/11	
WATERBURY	NAUGATUCK	04224R	MAPLE STREET	21.74	972	11/02/09	11/02/11			11/01/12	12/02/11			12/01/12	12/12/11			
WATERBURY	NAUGATUCK	08283R	HOP BROOK - MULTIBEAM DECK	22.42	359	10/27/09	10/27/11			10/26/12	11/26/11			11/25/12	11/30/11			
WATERBURY	NAUGATUCK	08284R	BRIDGE STREET (GENERAL PULASKI WALK)	22.74	754	09/14/09	09/14/11			09/13/12	11/15/11			11/14/12	12/12/11			
WATERBURY	WATERBURY	03723R	BRISTOL STREET #1	24.10	611	09/14/09	09/14/11			09/13/12	11/15/11			11/14/12	12/14/11			
WATERBURY	WATERBURY	08285R	STREAM - STONE/CONCRETE CULVERT	24.34	1320	04/27/10		04/26/12	04/27/11			07/05/12	12/05/11				11/22/11	
WATERBURY	WATERBURY	04232R	WASHINGTON AVENUE #1	26.18	2144	04/09/09	04/09/11			04/08/12	1021/11			12/30/00	12/14/11			
WATERBURY	WATERBURY	04235R	BANK STREET #1	26.35	2054	04/09/09	04/09/11			04/08/12	1021/11			12/30/00	12/13/11			
WATERBURY	WATERBURY	08286R	NAUGATUCK RIVER - STEEL GIRDER	26.42	3435	11/17/09	11/17/11			11/16/12	09/08/11			09/07/12	12/02/11			
WATERBURY	WATERBURY	08287R	JACKSON STREET	26.50	2074	04/09/09	04/09/11			04/08/12	1021/11			12/30/00	12/13/11			

Version: 1.0 - February, 2012

## APPENDIX 12 INSPECTION SCHEDULE FOR OFF-SYSTEM RAILROAD BRIDGES

## State of Connecticut-Department of Transportation-Office of Rails Inspection Schedule for Offsystem Railroad Bridges-Routine and Verification Inspection

			If	ispection a	Schedule for Offs	ystem	Rainoa		iges-ro	utine and v		on inspe	clion	•				
BRIDGE NO.	M.P.	RAILROAD	TRACK CHART NAME	TOWN	LOCATION	Deck Area	Last Day of Inspection	Date o	le proposed of Routine pection	Proposed Date of Verification inspection	Scheduled Date of Insp.by Purcell	Last for Verification Insp.by PB	Last Day of Inspection	2 yr cycle pro of Verificatior		2 yr cycle proposed Date of Routine inspection	Last Day of Insp.by PB	Last Day of Insp.by Purcell
								2011	2012	from 7/1/2011	Routine	Verification	Routine	2012	2013	2013	Routine	Verification
00430R	32.25	RAILS TO TRAILS*	AVON SECONDARY	FARMINGTON	FARMINGTON AVENUE	1098												
00479R	3.20	PROV & WORCESTER	WETHERSFIELD SEC.	HARTFORD	AIRPORT ROAD (SR 530)	1037	02/26/09	02/26/11			03/17/11		03/17/11	03/17/12		03/17/13		
00522R		PROV & WORCESTER	MIDDLETOWN SEC.	MIDDLETOWN	ROUTE 66	4400	06/18/10		06/17/12	09/01/11	06/17/12							
00600R		BOSTON & MAINE	TORRINGTON SEC.	WATERBURY	WEST MAIN STREET (SR 847)	4929	06/03/10		06/02/12	09/07/11	06/02/12	10/31/11			10/31/13			
00639R		PROV & WORCESTER	PORTLAND IND. TRK. CROMWELL IND. TRACK	MIDDLETOWN MIDDLETOWN		19520 1093	11/03/10	00/40/44	11/02/12	11/03/11	11/02/12		00/40/44	09/19/12		00/40/42		
00863R 01373R		PROV & WORCESTER PROV & WORCESTER	MIDDLETOWN SEC.	MIDDLETOWN	HARTFORD AVENUE RTE 147 (BAILEYVILLE ROAD)	880	08/12/09 03/04/10	08/12/11	03/03/12	07/20/11	08/12/11 03/03/12	11/14/11	09/19/11	09/19/12	11/14/13	09/19/13		
01373R		RAILS TO TRAILS*	AVON SECONDARY	AVON	ARCH ROAD	1299	03/04/10		03/03/12	07/20/11	03/03/12	11/14/11			11/14/13			
01849R		PROV & WORCESTER	WILLIMANTIC BRANCH	WINDHAM	ABANDONED RD/PEDESTRIAN	1571	12/01/09	12/01/11			12/01/11		12/01/11	12/01/12		12/01/13		
04234R	24.34	BOSTON & MAINE	TORRINGTON SEC.	WATERBURY	FREIGHT STREET	6717	09/10/09	09/10/11			09/10/11		09/21/11	09/21/12		09/21/13		
04242R	4.68	INACTIVE	NEW BRITAIN SEC.	NEW BRITAIN	ROUTE 9 (BUSWAY)	13754	07/10/09	07/10/11			07/10/11		07/29/11	07/29/12		07/28/13		
09000R		PROV & WORCESTER	CROMWELL IND. TRACK	MIDDLETOWN	SEBETHE RIVER	2483	06/23/09	06/23/11			06/23/11		07/10/11	07/10/12		07/09/13		
09001R		PROV & WORCESTER	EAST BERLIN IND. TRACK	MIDDLETOWN		1030	06/24/09	06/24/11			06/24/11		10/11/11	10/11/12		10/10/13		
09002R		PROV & WORCESTER	LAUREL IND. TRACK	MIDDLETOWN	GREEN STREET	500	01/27/09	01/27/11			01/26/11		01/26/11	01/26/12		01/25/13		
09003R 09004R		PROV & WORCESTER	LAUREL IND. TRACK		SUMNER BROOK	1518 230	08/17/09 01/27/09	08/17/11 01/27/11			08/17/11 02/18/11		10/10/11 02/18/11	10/10/12 02/18/12		10/09/13 02/17/13		
09004R 09005R		PROV & WORCESTER PROV & WORCESTER	LAUREL IND. TRACK LAUREL IND. TRACK	MIDDLETOWN MIDDLETOWN	EASTERN DRIVE RESERVOIR BROOK	306	07/23/09	07/23/11			07/23/11		06/28/11	06/28/12		06/27/13		
09006R		PROV & WORCESTER	LAUREL IND. TRACK	MIDDLETOWN	INDIAN HILL BROOK	320	07/29/09	07/29/11			07/29/11		06/29/11	06/29/12		06/28/13		
09007R		PROV & WORCESTER	LAUREL IND. TRACK	MIDDLETOWN	FOREST CREEK	246	02/12/09	02/12/11			01/20/11		01/20/11	01/20/12		01/19/13		
09008R		PROV & WORCESTER	LAUREL IND. TRACK	MIDDLETOWN	ASYLUM CREEK	280	01/26/09	01/26/11			01/20/11		01/20/11	01/20/12		01/19/13		
09009R	3.70	PROV & WORCESTER	LAUREL IND. TRACK	MIDDLETOWN	MAROMAS CREEK	240	03/19/09	03/19/11			02/24/11		02/24/11	02/24/12		02/23/13		
09010R	16.75	PROV & WORCESTER	MIDDLETOWN SEC.	MIDDLEFIELD	CATTLE PASS	150	02/01/10		02/01/12	07/19/11	02/01/12	11/18/11			11/18/13			
09011R		PROV & WORCESTER	MIDDLETOWN SEC.	MIDDLEFIELD	COGINCHAUG RIVER	682	08/18/09	08/18/11			08/18/11		10/12/11	10/12/12		10/11/13		
09012R		PROV & WORCESTER	MIDDLETOWN SEC.	MIDDLEFIELD	FARMWAY	390	04/15/10		04/14/12	07/15/11	04/14/12	11/11/11			11/11/13			
09013R		PROV & WORCESTER	MIDDLETOWN SEC.			1243	04/15/10	40/00/44	04/14/12	07/12/11	04/14/12	10/27/11	40/00/44	40/00/40	10/27/13	10/01/10		
09014R 09015R		PROV & WORCESTER PROV & WORCESTER	MIDDLETOWN SEC. MIDDLETOWN SEC.	MIDDLETOWN MIDDLETOWN	COGINCHAUG RIVER UNKNOWN STREAM	1419 108	10/29/09 10/01/09	10/29/11 10/01/11			10/29/11 10/01/11		12/02/11	12/02/12		12/01/13		
09010R		BOSTON & MAINE	TORRINGTON SEC.	WATERBURY	CRANE STREET	2883	09/21/09	09/21/11			09/21/11		09/22/11	09/22/12		09/21/13		
09100R		BOSTON & MAINE	TORRINGTON SEC.	WATERBURY	CONC. BOX/PIPE CULVERT	840	09/14/10	03/21/11	09/13/12	09/14/11	09/13/12	10/25/11	03/22/11	03/22/12	10/25/13	03/21/10		
09104R		NAUGATUCK	TORRINGTON SEC.	WATERBURY	MAHAN CANAL	260	05/27/09	05/27/11			05/27/11		05/24/11	05/24/12		05/23/13		
09105R	1.31	NAUGATUCK	TORRINGTON SEC.	WATERBURY	BROOK	340												
09106R	1.45	NAUGATUCK	TORRINGTON SEC.	WATERBURY	HANCOCK BROOK	1714	08/14/09	08/14/11			08/14/11							
09107R		NAUGATUCK	TORRINGTON SEC.	WATERBURY	AMERICAN PIN FOOTBRIDGE	1166	01/21/09	01/21/11			02/23/11		02/23/11	02/23/12		02/22/13		
09108R		NAUGATUCK	TORRINGTON SEC.	WATERBURY		2924	08/31/09	08/31/11			08/31/11		11/16/11	11/16/12		11/15/13		
09109R		NAUGATUCK NAUGATUCK	TORRINGTON SEC.	WATERTOWN		582	01/22/09	01/22/11			02/17/11		02/17/11	02/17/12		02/16/13		
09110R 09111R	-	NAUGATUCK	TORRINGTON SEC. TORRINGTON SEC.	WATERTOWN THOMASTON	NAUGATUCK RIVER BROOK	3624 534	08/31/09 01/22/09	08/31/11 01/22/11			08/31/11 02/17/11		11/17/11 02/17/11	11/17/12 02/17/12		11/16/13 02/16/13		
09111R		NAUGATUCK	TORRINGTON SEC.	THOMASTON	RAILROAD STREET ANNEX	322	02/04/10	01/22/11	02/04/12	07/11/11	02/04/12	10/27/11	02/11/11	02/11/12	10/27/13	02/10/13		
09113R		NAUGATUCK	TORRINGTON SEC.	THOMASTON	BROOK - CULVERT	393	09/27/10		09/26/12	09/27/11	09/26/12	10/27/11			10/27/13			
09114R	10.01	NAUGATUCK	TORRINGTON SEC.	THOMASTON	NAUGATUCK RIVER DAM SP.	2718	08/20/09	08/20/11			08/20/11		11/15/11	11/15/12		11/14/13		
09115R	10.16	NAUGATUCK	TORRINGTON SEC.	THOMASTON	DAM CONTROL	3880	07/02/09	07/02/11			07/02/11		07/20/11	07/20/12		07/19/13		
09116R		NAUGATUCK	TORRINGTON SEC.	LITCHFIELD	BROOK - CULVERT-CONC. PIPE	460	05/07/09	05/07/11			04/12/11		04/12/11	04/12/12		04/11/13		
09117R		NAUGATUCK	TORRINGTON SEC.	LITCHFIELD	CULVERT	1130	03/03/10		03/02/12	07/06/11	03/02/12	10/26/11			10/26/13			
09118R		NAUGATUCK	TORRINGTON SEC.		BROOK	855	05/07/09	05/07/11			04/12/11		04/12/11	04/12/12		04/11/13		
09119R 09120R		NAUGATUCK NAUGATUCK	TORRINGTON SEC. TORRINGTON SEC.	LITCHFIELD	CULVERT SPRUCE BROOK	765	09/28/09	09/28/11			09/28/11		11/21/11	11/21/12		11/20/13		
09120R		NAUGATUCK	TORRINGTON SEC.	TORRINGTON	BROOK - CULVERT	290	09/28/09	09/28/11			09/28/11	+	04/15/11	04/15/12		04/14/13		
09121R		NAUGATUCK	TORRINGTON SEC.	TORRINGTON	GULF STREAM	478	04/09/09	04/09/11			04/15/11	1	04/15/11	04/15/12		04/14/13		
09123R		NAUGATUCK	TORRINGTON SEC.	TORRINGTON	SUMMER STREET	1971	10/30/09	10/30/11			10/30/11		09/28/11	09/28/12		09/27/13		
09124R		NAUGATUCK	TORRINGTON SEC.	TORRINGTON	PASSWAY	623	10/30/09	10/30/11			10/30/11		09/28/11	09/28/12		09/27/13		
09125R		NAUGATUCK	TORRINGTON SEC.	TORRINGTON	NAUGATUCK RIVER	2128	09/15/09	09/15/11			09/15/11		11/18/11	11/18/12		11/17/13		
09200R		HOUSATONIC	NEW MILFORD SEC.	NEW MILFORD	STREAM - STONE CULVERT	516	05/20/09	05/20/11			05/20/11		05/26/11	05/26/12		05/25/13		
09201R		HOUSATONIC	NEW MILFORD SEC.	NEW MILFORD	BROOK - STONE CULVERT	252	05/20/09	05/20/11			05/20/11		05/26/11	05/26/12		05/25/13		
09202R		HOUSATONIC		NEW MILFORD	BROOK - CULVERT	140	11/09/09	11/09/11			11/09/11							
09203R 09204R		HOUSATONIC HOUSATONIC	NEW MILFORD SEC. NEW MILFORD SEC.	NEW MILFORD	BROOK - CULVERT SWAMP (CULVERT)	287 254	11/09/09 03/16/09	11/09/11 03/16/11			11/09/11 05/18/11		05/17/11	05/17/12		05/16/13		
09204R 09206R		HOUSATONIC	NEW MILFORD SEC.	KENT	COBBLE BROOK	254 634	03/16/09	03/10/11	08/04/12	08/05/11	05/18/11	10/17/11	00/17/11	00/17/12	10//17/13	00/10/13		
09206R		HOUSATONIC	NEW MILFORD SEC.	KENT	MAUWEE BROOK	159	11/10/09	11/10/11	00/04/12	00/00/11	11/10/11	10/17/11			10//17/13			
09208R		HOUSATONIC	NEW MILFORD SEC.	KENT	KENT FALLS BROOK-CULVERT	776	11/10/09	11/10/11			11/10/11	1	1					
09209R		HOUSATONIC	NEW MILFORD SEC.	KENT	CULVERT-DBL 4' CULVERTS													
09210R	29.50	HOUSATONIC	NEW MILFORD SEC.	CORNWALL	DEEP BROOK	221	08/03/10		08/02/12	08/03/11	08/02/12	10/17/11			10/17/13			
09211R		HOUSATONIC	NEW MILFORD SEC.	CORNWALL	MILLARD BROOK	221	07/22/09	07/22/11			07/22/11		07/12/11	07/11/12		07/11/13		
09212R		HOUSATONIC	NEW MILFORD SEC.	CORNWALL	GUNN BROOK	175	08/03/10		08/02/12	08/03/11	08/02/12	10/18/11	ļ		10/18/13			
09213R		HOUSATONIC	NEW MILFORD SEC.	CORNWALL		1147	08/04/10		08/03/12	08/04/11	08/03/12	10/18/11	-		10/18/13			
09214R 09215R		HOUSATONIC HOUSATONIC	NEW MILFORD SEC. NEW MILFORD SEC.	CORNWALL CORNWALL	FURNACE BROOK BROOK	910 225	09/21/10 08/04/10		09/20/12 08/03/12	09/21/11 08/04/11	09/20/12 08/03/12	10/18/11 10/18/11			10/18/13 10/18/13			
09215R 09217R		HOUSATONIC	NEW MILFORD SEC.	CORNWALL	MILL BROOK	310	08/04/10		08/03/12	08/04/11	08/03/12	10/18/11	+		10/18/13			
0921/K	30.00		NEW WILL OND SEC.	CONNWALL		310		A12-1	UJ/Z1/1Z	03/22/11	03/21/12	10/13/11			10/13/13			

# State of Connecticut-Department of Transportation-Office of Rails Inspection Schedule for Offsystem Railroad Bridges-Routine and Verification Inspection

BRIDGE NO.	M.P.	RAILROAD	TRACK CHART NAME	TOWN		Deck Area	Last Day of Inspection	2 yr cyc Date c	le proposed of Routine pection	Proposed Date of Verification inspection	Scheduled Date of Insp.by Purcell	Last for Verification Insp.by PB	Last Day of Inspection Routine	2 yr cycle pro of Verification		2 yr cycle proposed Date of Routine inspection	Last Day of Insp.by PB	Last Day of Insp.by Purcell
								2011	2012	from 7/1/2011	Routine	Verification	Routine	2012	2013	2013	Routine	Verification
09218R	40.10	HOUSATONIC	NEW MILFORD SEC.	CORNWALL	STREAM	210	10/14/09	10/14/11			10/14/11		11/11/11	11/11/12		11/10/13		
09219R		HOUSATONIC	NEW MILFORD SEC.	CANAAN	CHILD POND BROOK	332	07/22/09	07/22/11			07/22/11		07/12/11	07/12/12		07/11/13		
09220R		HOUSATONIC	NEW MILFORD SEC.	CANAAN	BROOK - STONE SLAB	414	05/21/09	05/21/11			05/21/11	10/19/11	05/27/11	05/27/12		05/26/13		
09221R		HOUSATONIC HOUSATONIC	NEW MILFORD SEC. NEW MILFORD SEC.	CANAAN CANAAN	WATER STREET BRANCH HOUSATONIC RIVER	320 580	10/13/09 08/10/10	10/13/11	08/09/12	08/10/11	10/13/11 08/09/12	10/19/11	11/10/11	11/10/12	10/19/13	11/09/13		<u> </u>
09222R 09223R			NEW MILFORD SEC.	CANAAN	HOLLENBECK RIVER	1073	08/10/10		08/10/12	08/10/11	08/09/12	10/20/11			10/20/13			<u> </u>
09223R		HOUSATONIC	NEW MILFORD SEC.	NORTH CANAAN	BLACKBERRY RIVER	1264	09/23/10		09/22/12	09/23/11	09/22/12	10/24/11			10/24/13			
09225R		HOUSATONIC	NEW MILFORD SEC.	NORTH CANAAN	NORTH BLACKBERRY RIVER	315	10/14/09	10/14/11	00,22,12	00,20,11	10/14/11		11/11/11	11/11/12	10/2 1/10	11/10/13		<u> </u>
09300R		RAILS TO TRAILS*	KENDALL'S IND. TRACK	WINDHAM	NATCHAUG RIVER	2471												
09301R		PROV & WORCESTER	WILLIMANTIC BRANCH	SPRAGUE	WALDO BROOK	680	11/01/10		10/31/12	11/01/11	10/31/12	11/15/11			11/15/13			
09302R		PROV & WORCESTER	WILLIMANTIC BRANCH	SCOTLAND	MERRICK BROOK	600	02/02/10		02/02/12	08/10/11	02/02/12	11/03/11			11/03/13			
09303R		PROV & WORCESTER	WILLIMANTIC BRANCH	WINDHAM	FROG BROOK	120	03/17/09	03/17/11			05/18/11	11/15/11	05/18/11	05/18/12	11/15/13	05/17/13		<b></b>
09304R		PROV & WORCESTER		WINDHAM	SHETUCKET RIVER	2450	05/26/10		05/25/12	08/08/11	05/25/12	11/21/11			11/21/13			<b></b>
09305R		PROV & WORCESTER		WINDHAM		836	02/19/10		02/19/12	08/04/11	02/19/12	11/02/11			11/02/13			<u> </u>
09306R 09307R		PROV & WORCESTER PROV & WORCESTER	WILLIMANTIC BRANCH WILLIMANTIC BRANCH	WINDHAM WINDHAM	WILLIMANTIC RIVER STATION FOOTBRIDGE	5427 2388	05/27/10 02/25/09	02/25/11	05/26/12	08/01/11	05/26/12 03/25/11	11/16/11	03/25/11	03/25/12	11/16/13	03/24/13		<u> </u>
09307R 09800R		RAILS TO TRAILS*	AVON SECONDARY	FARMINGTON	SCOTT SWAMP BROOK	750	02/23/09	02/23/11			03/23/11		03/23/11	03/23/12		03/24/13		
09802R		RAILS TO TRAILS*	AVON SECONDARY	FARMINGTON	FARMINGTON RIVER	3960												
09803R		RAILS TO TRAILS*	AVON SECONDARY	AVON	THOMPSON BROOK	465												
09804R		RAILS TO TRAILS*	AVON SECONDARY	AVON	NOD BROOK	845												
09805R	37.81	RAILS TO TRAILS*	AVON SECONDARY	AVON	ROSEWOOD ROAD	516												
09806R		RAILS TO TRAILS*	AVON SECONDARY	SIMSBURY	HOP BROOK	336												
09807R		RAILS TO TRAILS*	AVON SECONDARY	SIMSBURY	ROBERT'S CATTLE PASS	150												
09808R		RAILS TO TRAILS*		EAST GRANBY	FARMINGTON CANAL													
09809R		RAILS TO TRAILS*		EAST GRANBY	SALMON RIVER	3270												
09810R		RAILS TO TRAILS*		EAST GRANBY		280												L
09811R 09812R		RAILS TO TRAILS* RAILS TO TRAILS*	AVON SECONDARY AVON SECONDARY	SUFFIELD SUFFIELD	MUDDY RIVER-CONC. ARCH FARMINGTON CANAL	1464 340												
09812R		RAILS TO TRAILS		SUFFIELD	STREAM	211					-							
09814R		CENTRAL NEW ENGLAND	E. LONGMEADOW SEC.	SOUTH WINDSOR	DRY BROOK	211	04/20/10		04/19/12	08/12/11	04/19/12	11/07/11			11/07/13			
09815R		CENTRAL NEW ENGLAND	E. LONGMEADOW SEC.	EAST WINDSOR	KETCH BROOK	281	03/02/10		03/01/12	08/15/11	03/01/12	11/07/11			11/07/13			<u> </u>
09816R	15.81	CENTRAL NEW ENGLAND	E. LONGMEADOW SEC.	EAST WINDSOR	BROAD BROOK AND PRIV. RD.	880	03/01/10		02/29/12	08/16/11	02/29/12	11/08/11			11/08/13			
09817R		CENTRAL NEW ENGLAND	E. LONGMEADOW SEC.	EAST WINDSOR	CULVERT	605	03/02/10		03/01/12	08/18/11	03/01/12	11/08/11			11/08/13			
09818R		CENTRAL NEW ENGLAND	E. LONGMEADOW SEC.	ENFIELD	SCANTIC RIVER	2340	12/21/09	12/21/11			12/21/11							
09819R		CENTRAL NEW ENGLAND	E. LONGMEADOW SEC.	ENFIELD	WATER STREET	1020	11/24/09	11/24/11			11/24/11							<b></b>
09820R		CENTRAL NEW ENGLAND	E. LONGMEADOW SEC.	ENFIELD	TERRY BRK-2 CULVERTS	1615	03/17/09	03/17/11			03/09/11		03/09/11	03/09/12		03/08/13		
09821R 09823R		CENTRAL NEW ENGLAND	E. LONGMEADOW SEC.	ENFIELD HARTFORD	STREAM CULVERT	208	11/23/09	11/23/11			11/23/11							<b></b>
09823R			GRIFFIN'S INDUST. TRK.	HARTFORD	STREAM	330	04/19/10		04/18/12	08/22/11	04/18/12	11/09/11			11/09/13			
09825R			GRIFFIN'S INDUST. TRK.	BLOOMFIELD	STREAM (WASH BROOK)	280	04/19/10		04/18/12	08/23/11	04/18/12	11/09/11			11/09/13			
09826R		INACTIVE	NEW BRITAIN SEC.	NEW BRITAIN	CEMETERY UNDERPASS(BW)	645	10/15/09	10/15/11	0 1, 10, 12	00,20,11	10/15/11		11/09/11	11/09/12	1,00,10	11/08/13		<u> </u>
09827R		INACTIVE	NEW BRITAIN SEC.	NEW BRITAIN	PIPER BROOK (BUSWAY)	1782	01/19/08	01/18/10			01/11/11		01/11/11	01/11/12		01/10/13		
09828R	14.30	PROV & WORCESTER	WETHERSFIELD SEC.	CROMWELL	SOUTH STREET	308	03/04/10		03/03/12	07/05/11	03/03/12	11/01/11			11/01/13			
09829R		PROV & WORCESTER	WETHERSFIELD SEC.	CROMWELL	NOOKS HILL ROAD	319	03/20/09	03/20/11			03/23/11							
09830R		PROV & WORCESTER		ROCKY HILL	EVAN'S ROAD	139	02/13/09	02/13/11	ļ		01/25/11	1	01/25/11	01/25/12	-	01/24/13		<b> </b>
09831R		PROV & WORCESTER		ROCKY HILL		600	03/19/09	03/19/11			04/18/11		04/18/11	04/18/12		04/17/13		L
09832R		PROV & WORCESTER	WETHERSFIELD SEC. WETHERSFIELD SEC.	WETHERSFIELD HARTFORD		4000	02/20/00	02/20/44			02/22/11		05/10/14	05/10/10		05/11/10		
09833R 09834R		PROV & WORCESTER PROV & WORCESTER	WETHERSFIELD SEC.	HARTFORD	FOLLY BROOK-CULVERT THRASHER'S CROSSING	1980 594	03/20/09 04/08/09	03/20/11 04/08/11			02/22/11 02/22/11	+	05/12/11 02/22/11	05/12/12 02/22/12		05/11/13 02/21/13		ł
09835R		PROV & WORCESTER	WETHERSFIELD SEC.	CROMWELL	FIRE ACCESS ROAD	190	04/08/09	04/08/11			03/22/11	1	<i>VLILLI</i>	54124112		52121115		<u> </u>
09836R		PROV & WORCESTER	WETHERSFIELD SEC.	ROCKY HILL	DIVIDEND BROOK	864	02/24/09	02/24/11			01/25/11	1	01/25/11	01/25/12		01/24/13		t
09901R		ABANDONED		EAST HAMPTON	MINE BROOK-STONE CULV.	3667	01/10/09	01/10/11			01/07/11	1	01/07/11	01/07/12	1	01/10/13		
09902R	31.53	ABANDONED	AIRLINE VL MAP 54-64/30	EAST HAMPTON	PINE BROOK	76	01/12/09											
09903R		ABANDONED		EAST HAMPTON	MUDDY GUTTER BROOK	1919	01/10/09	01/10/11			01/07/11		01/07/11	01/07/12		01/10/13		L
09904R		ABANDONED		EAST HAMPTON	MAIN STREET	86	01/13/09	01/13/11			01/07/11		01/07/11	01/07/12		01/06/13		<u> </u>
09905R		ABANDONED		EAST HAMPTON	BROOK	760	01/13/09	01/13/11			01/11/11		01/11/11	01/11/12		01/10/13		<b> </b>
09908R		ABANDONED			MOOSUP RIVER	1447	03/18/09	03/18/11			04/07/11							<u> </u>
09909R		ABANDONED RAILS TO TRAILS*																
09911R 09912R		RAILS TO TRAILS*	FARMINGTON VALLEY GREENWAY VAL MAP 56-60/4	FARMINGTON FARMINGTON	HYDE BROOK-STONE ARCH OLD BROOK-ARCH CULVERT	704												
09912R 09913R		RAILS TO TRAILS*		CANTON	FARMINGTON RIVER													
09914R		RAILS TO TRAILS*	VAL MAP 56-60/8	CANTON	FARMINGTON CANAL	7716												
09916R		INACTIVE	VAL. MAP 57-72/54	WINCHESTER	STILL RIVER	281	01/20/09	01/20/11			01/28/11		01/28/11	01/28/12		01/27/13		
09921R				BURLINGTON	BARNES BROOK - DEP													
09922R	6.14	RAILS TO TRAILS*	VAL MAP 56-60/7	BURLINGTON	CEDAR BROOK - DEP													
09923R	6.37	RAILS TO TRAILS*	VAL MAP 56-60/7	BURLINGTON	BROOK - DEP													

Version: 1.0 - February, 2012

# APPENDIX 13 OFFICE OF RAIL INSPECTION FORMS

BRI- 9	Construction Punch List
BRI-10	Concrete Deterioration Worksheet
BRI-11	Seismic Data Sheet
BRI-12	Fracture Critical Data Sheet
BRI-13	Photo Log
BRI-14	Sliding Bearing Measurement Sheet
BRI-15	Rocker Bearing Measurement Sheet
BRI-16	Pot Bearing Measurement Sheet
BRI-17	Joint Measurement Sheet
BRI-18	Bridge Inspection Report Form
BRI-24	Incident Report Form
BRI-25	Under Entry Structure Inventory & Appraisal Form
BRI-27	Inspection Report Transmittal Form
BRI-29	Pin & Hanger Data/Analysis Form
BRI-30	Hinge Data/Analysis Form
BRI-39	Structure Inventory & Appraisal Form
BRI-58	Underwater Inspection Report Form
BRI-59	Underwater Inspection Structure Data Form

BUREAU OF PUBLIC TRANSPORTATION OFFICE OF RAIL		DATE:
CONSTRUCTION PUNCH LIST	FormBRI-9, Rev. 01/12	PAGE 1 of 2
Construction Project Number:		
Bridge No Town:	Route:	
Feature Intersected:		
Bridge Type:		······
Mile Point:	Construction District:	
Construction Company Name:		
D.O.T. Construction Inspectors Name:		
List of Items to be Corrected or Completed	d:	
1		
2		
3		
4		
5.		
6		
7		
8		
9		
10		

PUNCH LIST

BUREAU OF PUBLIC TRANSPORTATION OFFICE OF RAIL		DATE:
CONSTRUCTION PUNCH LIST	FormBRI-9, Rev. 01/12	PAGE 2 of 2
Bridge No		
Photo Log:		
1		
2		
3		
4		
5		
Additional Comments (If any):		
Date copy sent to Construction:		
Submitted By:		Date:
Checked By:		Date:
PUNCH LIST		

### **CONCRETE DECK DETERIORATION WORKSHEET**

**Purpose:** This form is used to tally the deteriorated areas on both the topside (of bare concrete decks) and underside of the deck. The areas tallied are then used to calculate what percentage of the deck is deteriorated. The maximum % deterioration in any one span and the total % deterioration for the structure are then reviewed to determine the concrete deck condition rating in accordance with Section 10.1.

#### **Instructions for completing Form BRI-10:**

- 1. Estimate the area (in square feet) of each type of deterioration in each span. This can be done by totaling all the individual areas drawn on a deck sketch or by some rational method of estimating the area.
  - Example: 25% of each 6' x 20' bay has map cracking with efflorescence. Assume there are 8 bays. The number to be recorded under "Map Cracking: w/Efflorescence" = 25% x 6' x 20' x 8 bays = 240 sq. ft.

For multi-span structures, total the deteriorations for the whole bridge across each row to the column on the right side of the form.

- 2. Calculate the deck area of each span using the plans. This should be the outto-out of deck dimension multiplied by the center to center of deck joints dimension. For multi-span structures, total the deck area for the whole bridge across to the column on the right side of the form.
- 3. Calculate the % spalled and delaminated on the topside of bare decks in each span by dividing the spalled and delaminated area by the total span area. Do the same in the total column (total % deterioration is calculated by summing the areas of deterioration and dividing by the total deck area, not by summing the individual span %'s).
- 4. Calculate the % deterioration on the bottom by adding the spalled, delaminated, scaled, cracked and honeycomb areas, and dividing by the total span area in each span. Do the same in the total column (total % deterioration is calculated by summing the areas of deterioration and dividing by the total deck area, not by summing the individual span %'s).

### **CONCRETE DETERIORATION WORKSHEET**

Form BRI-10, Rev. 9/01

				Deter	rioration E	By Span -	In Squai	re Feet			
					S	pan Num	ber				
Deterioration Type	1	2	3	4	5	6	7	8	9	10	Total
	Тор:	Top:	Тор:	Тор:	Тор:	Тор:	Тор:	Тор:	Тор:	Тор:	Тор:
Spalled and Delaminated Areas	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:
	Тор:	Тор:	Тор:	Тор:	Тор:	Тор:	Тор:	Тор:	Тор:	Тор:	Тор:
Scale (Moderate to Severe Only)	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:
Cracks: w/Efflorescence (use 6 in. width x length)	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:
w/o Efflo. (use 3 in. width x length)	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:
Map Cracking: w/Efflorescence (use full area)	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:
w/o Efflo. (use 50% of area)	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:
Honeycombed Areas (only areas more than 1 1/2 in. deep)	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:
Total Span Area (Square Feet)											
% Spalled and Delaminated on Top											
% Deterioration on Bottom											

Prepared By:Date:	Bridge # Mile Point:
	Town:
Entered By:Date:	Feature Crossed:
	Line Name:
	Operator:
Feature Crossed Information: ADTyear of ADT	
Community Lifeline(y/n)	
Major River Crossing(y/n)	
Feature Carried Information:	
Utilization	
Freight Less than 10	More than 10 ns/day passenger trains/day
Service only passenger train	is/day passenger trains/day
Detour availableYes	Inconvenient None
Alternate Service (Bus) available(y/	
	<b>11</b> /
Structure Information:	
Bridge Construction	
	le span or Multiple
	inuous multiple spans simple spans
Number of Spans	Number of longitudinal girders Moveable Bridge(y/n)
Ballasted Deck (y/n)	Moveable Bridge (y/n)
I otal Bridge length $\Pi$ M	lax single spanft
Bridge width ft	t Skewdegrees
Abutment heightftIs abut ma	sonry or unreinforced conc(y/n)
Desving and Dridge Seat Construction	
Bearing and Bridge Seat Construction Expansion Bearing Type	If rocker, HeightInches
Are Bearings on Pedestals (pad > 6"	high) (y/n)
Expansion Bearings Transversely Re	
Expansion Dearings Transversery Re	Shamed(ym)
Vulnerable Bearing Information	
Expansion Bearing Longitudinal Sup	port Length Inches
Expansion Bearing Transverse Suppo	• • •
Effective Lengthft, Effect	tive Heightft
Pier Construction No pier (single sp	van bridge)
Steel or ductile conc.	Steel or ductile conc. Wall type
Steel or ductile conc sing	le column Pier
Is pier founded on battered pile	<u>(y/n)</u>
Is material masonry or unreinforced co	oncrete(y/n)

## Revised March 27, 2000 BRI-11 SEISMIC DATA SHEET FOR RAILROAD BRIDGES

CDOT Office of Rail

### CONNECTICUT DEPARTMENT OF TRANSPORTATION FRACTURE CRITICAL MEMBERS/FATIGUE PRONE DETAILS INSPECTION DATA SHEET

Form BRI12, Rev 9/97

Bridge No:	Fracture Critical Inspe FC Insp Freq: Mo	ction Date:	
Year Built:		This FC Type Cou	
Town:		ADT:	Year of ADT:
Facility Carried:		Structure Type:	% Truck:
Feature Intersected:			
Access Equipment Needed:		<u></u>	
Traffic Control Required:			
Reference to Plans:			· · · ·
MEMBER/DETAIL TYPE #			Fracture Critical
Fatigue Category:   Steel     Description:	Type:		Fatigue Prone
Inspection Procedure:			
MEMBER/DETAIL TYPE #			· · · · · · · · · · · · · · · · · · ·
Member/Detail Type:	 Type:		Fracture Critical
Description:			
Inspection Procedure:			

### CONNECTICUT DEPARTMENT OF TRANSPORTATION FRACTURE CRITICAL MEMBERS/FATIGUE PRONE DETAILS INSPECTION DATA SHEET

Form BRI12, Rev 9/97

Member/Detail Type:   Fatigue Category:   Steel Type:   Patigue Prone   Description:     Inspection Procedure:   MEMBER/DETAIL TYPE #   Member/Detail Type:   Fracture Critical   Fatigue Category:   Steel Type:   Patigue Prone   Description:     Inspection Procedure:     MEMBER/DETAIL TYPE #     Member/Detail Type:   Fatigue Category:   Steel Type:   Practure Critical   Fatigue Prone   Description:   Inspection Procedure:   Member/Detail Type:   Steel Type:   Steel Type:   Inspection Procedure:     Inspection Procedure:	MEMBER/DETAIL TYPE #	
Fatigue Category:       Steel Type:       Image: Prone         Description:       Image: Prone       Image: Prone         MEMBER/DETAIL TYPE #       Image: Prone       Image: Prone         Member/Detail Type:       Image: Prone       Image: Prone         Description:       Image: Prone       Image: Prone         Description:       Image: Prone       Image: Prone         Inspection Procedure:       Image: Prone       Image: Prone         Member/Detail Type:       Image: Prone       Image: Prone         Description:       Image: Prone       Image: Prone         Inspection Procedure:       Image: Prone       Image: Prone         Description:       Image: Prone       Image: Prone         Image: Prone       Image: Prone       Image: Prone         Image: Prone </th <th></th> <th>Eracture Critical</th>		Eracture Critical
Description:		[manual promotion
Inspection Procedure:   MEMBER/DETAIL TYPE #   Member/Detail Type:   Fatigue Category:   Steel Type:   Description:   Inspection Procedure:   MEMBER/DETAIL TYPE #   Member/Detail Type:   Fatigue Category:   Steel Type:   Fracture Critical   Fatigue Category:   Steel Type:   Fracture Critical   Fatigue Category:   Steel Type:   Inspection Procedure:		
MEMBER/DETAIL TYPE #   Member/Detail Type:   Fatigue Category:   Steel Type:   Inspection Procedure:   MEMBER/DETAIL TYPE #   Member/Detail Type:   Fracture Critical   Fracture Critical   Fracture Critical   Inspection Procedure:   Inspection Procedure:	Description:	· · · · · · · · · · · · · · · · · · ·
Member/Detail Type:	Inspection Procedure:	
Fatigue Category: Steel Type:   Description:	MEMBER/DETAIL TYPE #	
Description: Inspection Procedure: MEMBER/DETAIL TYPE # Member/Detail Type: Fracture Critical Fatigue Category: Steel Type: Fatigue Prone Description: Inspection Procedure:	Member/Detail Type:	Fracture Critical
Inspection Procedure:     MEMBER/DETAIL TYPE #     Member/Detail Type:     Fatigue Category:     Steel Type:     Description:     Inspection Procedure:	Fatigue Category:   Steel Type:	Fatigue Prone
MEMBER/DETAIL TYPE #         Member/Detail Type:         Fatigue Category:       Steel Type:         Description:         Inspection Procedure:	Description:	
Member/Detail Type:	Inspection Procedure:	
Fatigue Category:       Steel Type:       Fatigue Prone         Description:       Inspection Procedure:       Inspection Procedure:	MEMBER/DETAIL TYPE #	
Description:	Member/Detail Type:	Fracture Critical
Inspection Procedure:	Fatigue Category: Steel Type:	Fatigue Prone
	Description:	
MEMBED/DETAIL TVDE #	Inspection Procedure:	
	MEMBER/DETAIL TYPE #	
Member/Detail Type:	Member/Detail Type:	Fracture Critical
Fatigue Category:	Fatigue Category: Steel Type:	Fatigue Prone
Description:	Description:	
Inspection Procedure:	Inspection Procedure:	

# PHOTO LOG Form BRI-13, Rev. 9/97

Bridge Informatio	n System
Image Inventory	
Bridge No.	Date
Town:	Photographer:
Carried / Crossed	٠ <u>ــــــــــــــــــــــــــــــــــــ</u>
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· · –	-,,				Ē	BRIDGE N	10.		DATE:
F.	IEL	D .	NO	ES	-	REW:		<u></u>	SHEET
	<u></u>		<u>S</u>	LIDIN	BE/	ARING	MEAS	SUREMEN	TS
FRC	Later Misoli	gnment		Seat	Beam	sure and "R"	—Sole F —Sliding /-Mason	Plate Plate ry Plate omment On: ~ –	Span No. = Substructure Unit = Temperature =F Presence of keepers or we done on bearings. Undermining of bearing. Attach sketch with dimens Cracking of plates or weld Condition of anchor bolts.
	·.	Mov	ement			Condition		· · · · · · · · · ·	
Beam	"["	"R"	Mode Exp. or Contr.			Normal Mov't?			Comments
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		140		<u> </u>	CREW:	SHEET
		R		(ER E	SEARING MEASUREM	ENTS
Snon	No. =				roma dau≕io, rev. 3/3/	, <b>1</b>
	ucture				· · · · · · · · · · · · · · · · · · ·	Beam
	=	·		-		
Tempe	erature =	<u>•</u> F				R =
				lhe F bearin	ront" of the "F" is the side the fixed bearing,	✓ "B"
	= Sin <sup>-1</sup> (F-E	3)/W		facing	the fixed bearing.	
Y =	= R TAN O	- 		· .	Ţ	
NOTE;						W Plate
"F" &	"B" should	be measure	d at	the left	side	1
to the	s of the ro front face	of the sub	structu	ine ciusi Ire on 1	kewed bridges.	₩ =
Beam	<b>"</b> F"	"B"	Y	Cont. or Exp.	Со	mments
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· · · · · · · · · · · · · · · · · · ·			BRI	DGE NO.	<u> </u>	DATE:	
FIE	LD NO	JIES	CRE	W:		SHEET	· · · · · · · · · · · · · · · · · · ·
		POT B	EARIN	G MEA	SUREMENTS		
Notes Cuided	overeiter hoor						
expansion be	expansion bear arings do not h	iove keeper bo	n-guided ors.		Beam		
(see notes b	elow)					· · · ·	
		· ·	}========	Leveling Plate Sole Plate			Front of Bearing
		Piston		Keeper Bar Base Pot			is the Side Towards Fixity.
ſ	Left	Lateral	Right	2	Front of keeper to	front of base	Expansion
	Side Ma	asurements	Side		Front of keeper to Negative if keeper p base.	asses front of	measurement O left side &
	FRO	ONT VIEW				E VIEW	right side Take at Front
1 -Left e	: Right are determi	ned when facing	the Front (	of the Bearin	g		of Bearing.
-For B	n-guided bearings,	, measure from s	ide of sole	e plate to sid	e of piston O center	line of piston.	
-							
2 -For m			ion from fr	ront of sole	plate to front of pis		
2 -For m	& Substructure	Unit =			plate to front of pis		bearing. re = <u>F</u>
2 -For n Span No.	& Substructure Expansio	Unit =	La	teral	plate to front of pis	Temperatu	
2 -For n Span No.	& Substructure Expansio Measurement	Unit =			plate to front of pis		
<ol> <li>2 -For n</li> <li>Span No.</li> <li>Beam Exp</li> </ol>	& Substructure Expansio Measurement	Unit = n Side of Brg.	La	teral	plate to front of pis	Temperatu	
<ol> <li>For n</li> <li>Span No.</li> <li>Beam Exp</li> </ol>	& Substructure Expansio Measurement	Unit = n Side of Brg.	La	teral	plate to front of pis	Temperatu	
<ol> <li>For n</li> <li>Span No.</li> <li>Beam Exp</li> </ol>	& Substructure Expansio Measurement	Unit = n Side of Brg.	La	teral	plate to front of pis	Temperatu	
<ol> <li>For n</li> <li>Span No.</li> <li>Beam Exp</li> </ol>	& Substructure Expansio Measurement	Unit = n Side of Brg.	La	teral	plate to front of pis	Temperatu	
2 -For n Span No. Beam Exp	& Substructure Expansio Measurement	Unit = n Side of Brg.	La	teral Rîght		Temperatu	
2 -For n Span No. Beam Exp	& Substructure Expansio Measurement	Unit = n Side of Brg.	La	teral Rîght		Temperatu	
<ol> <li>For n</li> <li>Span No.</li> <li>Beam Exp</li> </ol>	& Substructure Expansio Measurement	Unit = n Side of Brg.	La	teral Rîght		Temperatu	
<ol> <li>2 -For n</li> <li>Span No.</li> <li>Beam Exp</li> </ol>	& Substructure Expansio Measurement	Unit = n Side of Brg.	La	teral Rîght		Temperatu	
2 -For n Span No. Beam Exp	& Substructure Expansio Measurement	Unit = n Side of Brg.	La	teral Rîght		Temperatu	
2 -For n Span No. Beam Exp	& Substructure Expansio Measurement	Unit = n Side of Brg.	La	teral Rîght		Temperatu	
2 -For n Span No. Beam Exp	& Substructure Expansio Measurement	Unit = n Side of Brg.	La	teral Rîght		Temperatu	
<ol> <li>For n</li> <li>Span No.</li> <li>Beam Exp</li> </ol>	& Substructure Expansio Measurement	Unit = n Side of Brg.	La	teral Rîght		Temperatu	
2 -For n Span No. Beam Exp	& Substructure Expansio Measurement	Unit = n Side of Brg.	La	teral Rîght		Temperatu	
2 -For n Span No. Beam Exp	& Substructure Expansio Measurement	Unit = n Side of Brg.	La	teral Rîght		Temperatu	
<ol> <li>For n</li> <li>Span No.</li> <li>Beam Exp</li> </ol>	& Substructure Expansio Measurement	Unit = n Side of Brg.	La	teral Rîght		Temperatu	

			J	DINT N	IEASU	7	Measurements Taken By:							
Brid	gə No.:	- 							Town:			Measurements Reviewed By:		· .
Deck		Effect		inter Measu	irements		nmer Meas	urements		erential Mo	·····			e Between
Joint Location	Joint Type <sup>1</sup>	Span <sup>2</sup> (ft.)	Temp "F		Right (in.)	Temp °F		Right (in.)	Ac Left (in.)	tual Right (in.)	by Formula	Comments		Calc'd Mov't Right (in.)
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	<u>]                                    </u>	<u>}</u>	<u> </u>	<u> </u>	<u> </u>		ļ	<u> </u>						<u> </u>
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1. The "Deck Joint Type" should be "plug" for Asphaltic Plug joints, "strip" for Strip Seals, "seg." for Segmental Joints, etc.

2. The "Effective Span" is the length contributing to expansion at the joint. This should be 0 at fixed joints of single span bridges.

BRIDGE #:		INSPECTION DATE	:
INSPECTION TYPE: INSPECTION PERFORME	D BY:		
TOWN:		FEATURE CARRIED:         FEATURE INTERSECTED         MAIN DESIGN:	YEAR BUILT:
INSPECTION VISITS:		INSPECTORS:	· · ·
58. DECK			
OVERLAY		<u>}</u>	
DECK STR. CONDITION			
CURBS			
MEDIAN			
SIDEWALKS			
PARAPET			
RAILING			
PAINT			
FENCE			
DRAINS			
LIGHTING STANDARD			
UTILITIES TYPE/SIZE			
CONSTRUCTION JOINTS			
EXPANSION JOINTS			
59. SUPERSTRUCTURE			OVERALL RATING
BEARING DEVICES			
STRINGERS			
GIRDERS			
FLOOR BEAMS			
TRUSSES-GENERAL			
TRUSSES-PORTALS			
TRUSSES-BRACING			
PAINT			
RUST			
MACHINERY MOV SPAN			

BRIDGE #:	INSPECTION DATE:
RIVETS & BOLTS	
WELDS & CRACKS	
TIMBER DECAY	
COLLISION DAMAGE	
DEFLECT. UNDER LOAD	
VIBR. UNDER LOAD	
STAND PIPES	
BARREL LADDERS	
	ARE BARREL LADDERS OSHA COMPLIANT?
60. SUBSTRUCTURE	
ABUTMENTS-STEM	<u>a</u>
ABUTMENTS-BACKWALL	
ABUTMENTS-FOOTINGS	
ABUTSETTLEMENT	
ABUTMENTS-WINGWALLS	
PIERS/BENTS-CAPS	
PIERS/BENTS-PILE BENT	
PIERS/BENTS-COLUMN	
PIERS/BENTS-FOOTINGS	
PIERS/BENTS-SETTLEMent	
EROSION-SCOUR	
CONCRETE CRACK-SPALL	
STEEL CORROSION	
PAINT	
COLLISION DAMAGE	
DEBRIS	
61. CHANNEL PROTECTION	
CHANNEL SCOUR	
EMBANKMENT EROSION	
DEBRIS	
Printed on 9/24/01 9:05:06 AM	Page 2 of 4

BRIDGE #:		
CHANNEL CHANGE		
FENDER SYSTEM	זר	
SPUR DIKES & JETTIES	5	
RIP RAP		
62. CULVERTS & RETAINI	NG V	
	<u>ting</u>	
	╧┥┟	
STEEL	╼╼┙ [ ┶╾┑┍	
	╧┤╏	
HEADWALL		
CUTOFF WALL	╧╏	
RETAINING WALL STEM	╡┝	
FOOTING	╧╎┝	
APPROACH CONDITION	[	OVERALL RATING
APPROACH SLAB	A <u>ting</u>	
	╧┙┟	
APPROACH GUIDE RAIL	╡╏	
APPROACH PAVEMENT		
TRAFFIC SAFETY FEATURES	'∟ S:	
BRIDGE RAILINGS		
TRANSITIONS	$\Box \overline{[}$	
APPROACH GUARDRAILS	$\Box \bar{[}$	
APPR. GUARDRAIL ENDS		
LOAD POSTING		
SINGLE UNIT (TONS)		
HS (TONS)	ЭĪ	
4 AXLE (TONS)	ΠĪ	
2S3 (TONS)	$\Box \bar{[}$	
ADVANCE WARNING Y/N		

BRIDGE #:		INSP	ECTION DATE:	
MISC.				
MIN VERT. UNDERCLR. POSTED CLR. UNDER BRID	GE' ["			
POSTED CLR. ON BRIDGE ADVANCE WARNING (Y/N) SPEED LIMIT (IF ANY)				
CHARACTER OF TRAFFIC			······································	
ADDITIONAL NOTES		<u></u>		
ADDITIONAL COMMENTS:			······································	
	· · · · · · · · · · · · · · · · · · ·			
Inspectors' Signatures:	1)		Date:/ _ /	
	2)		Date://	
	3)		Date://	
	4)		Date://	
P.E. Signature: P.E.#:			Date:///	• •
Reviewed by:		Срот	Date://	

State of Connecticut Department of Transportation Bureau of Public Transportation	SheetOf Form BRI-24, Rev. 01/12
	NT REPORT
Town:	Bridge No.
Route/Street:	
Over:	Туре:
Inspected by:	Date: <u>/ /</u>
Date of Incident: / / / Time o	f Incident: "D" no.:
Details of Incident:	
	· · · · · · · · · · · · · · · · · · ·
Immediate Actions Taken:	
Additional Remarks:	······································
	·····

INCIDENT.RPT

BRUDGE     TOWN NAME     NBIS BRG     STAT       NUMBER     TOWN NAME     NBIS BRG     STAT       NUMBER     LGTH     DEPARTM       FACILITY CARRIED     FEATURE CROSSED     DIVISION OF BI	STATE OF CUNNECTICUT DEPARTMENT OF TRANSPORTATION DIVISION OF BRIDGE SAFETY EVALUATION	
	INVENTORY ROUTE UNDER STRUCTURE EVALUATION	
	FORM BRI-25 REV 10/00	
REVIEWED BY: DATE: SHEET	OF (INSP. REPORT)	
DESCRIPTION:	26) INV. RTE. FUNCT CLASSIFICATION	
5) INVENTORY ROUTE:	100) DEFENSE HIGHWAY DESIGNATION	
A) RECORD TYPE B) POLITIE SIGNING DEPERTY	104) HIGHWAY SYSTEM OF INV. ROUTE	
C) DESIGNATED LEVEL OF SERVICE		
D) ROUTE NO.		
11) MILE POINT (INV.RTE)		
AGE & SERVICE	POSTED SIGNS	
+ 28B) NUMBER OF INV.ROUTE LANES	+ POSTED VERT. CLR UNDER BRUDGE	Ŀ.
109)	COMMENTS:	
	· · · · · · · · · · · · · · · · · · ·	·
* 41) INV ROUTE OPERATIONAL STATUS		
19) BYPASS DETOUR LENGTH		
TRIC DATA		
+ 10) INV. KIE. MIN. VEKI. CLEAKANCE <b>11 III III III</b>		
لي		
and the second s		
+ 53) RLOG MIN VERT CLR OVER BRG RDWAY		
<ul> <li>VERIFY EVERY INSPECTION 28B, 10, 47, 53, 55, 56 &amp; POSTED VERT CLEARANCE UNDER THE BRIDGE</li> <li>** MUST BE FILLED OUT OR VERIFIED ON THE FIRST INSPECTION MADE BASED ON THE NEW FHWA GUIDE 102</li> </ul>	DGE VA GUIDE 102	

#### STATE OF CONNECTICUT DEPARTMENT OF TRANSPORTATION BUREAU OF PUBLIC TRANSPORTATION OFFICE OF RAIL

#### INSPECTION REPORT TRANSMITTAL FORM FORM BRI-27, REV. 01/12

OFFICE OF KAIL	Structure No	Tow <u>n</u> :	
Inspectors:		Date:	
FORMS (bound in report)	<u>TABLE OF</u>	<u>CONTENTS</u> No. of She E	eets Enclosed
Railroad Maintenance Memo	orandum (RMM)		
Title Cover Sheet			···· -
BRI-27, Inspection Report T	ransmittal Form (Table of Co	ontents)	···· <sup></sup>
Bridge Location Map			
Executive Summary			
Time at Site Including Flagg	ing		
BRI-9, Construction Punch I	.ist		
BRI-11, Seismic Screening I	Data Sheet		
BRI-12, Fracture Critical Ins	pection Data Sheet	••••••	···
BRI-18, Bridge Inspection Fo	orm		
BRI-24, Incident Report For	n		
BRI-25, Under Entry SI&A I	Form		
BRI-39, RR Bridge SI&A Fo	rm		···· -
Field Notes (Include Forms E BRI-16, BRI-17, BRI		I-15,	<i>-</i>
Calculations: Load Rating Evaluation	on		••-
			·
BRI-58, Underwater Inspectio	n Report Form		
BRI-59, Underwater Inspectio	on Structure Data Form		

PIN & HANGER DATA SHEET Form BRI-29, Rev. 6/99 Measurements Taken By												Measurements Taken By: D	ate:
Bridg	je No.:						-	COLUMN TWO IS NOT THE OWNER.	Town:				·
	Hanger	Location:						Effective	span for	Movement:		(ft) Page:	of
Веат No.	V (in)	J (in)	Τ <i>r</i> (in)	B <i>r</i> (in)	T/ (in)	B/ (in)	Out to Out (in)	Secondary System Type	Gap <sup>1</sup> (Y/N)	Nut Restraint System	Temp	Comments	
1													
2													
3													
4			·										
5									•				
6													
7									1				
8			-			· · ·		-,					
9							-						
10													

#### Notes:

1) For Pin & Hanger assemblies with a redundant support system, indicate if there is a gap between the redundant system (bearing) and the bottom flange of the suspended girder.

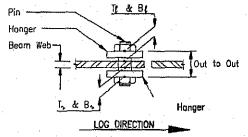
2) All measurements are taken in reference to log direction.

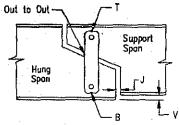
V : Vertical misalignment of girders @ left edge of girder's bottom flange.

J : Joint opening between webs, measured just above the

bottom flange fillet, on the left face of the girder's web. Out to Out : The out-to-out of hangers taken at the leading edge, based on log direction.

3) Use a permanent marker to indicate locations of field measurements.





	PIN &	HAN	GER I	DATA	SHEE	ET		Form BRI-29	, Rev. 6/9	9		Measurements Taken By: Date:
Bridge No.: 0 Tov									Town:	0		
	Hanger	Location:			0			Effective	span for	Movement:	0	Page: of
Beam No.	V (in)	J (in)	T <i>/</i> * (in)	B/* (in)	T/ (in)	B/ (in)	Out to Out (in)	Secondary System Type	Gap <sup>1</sup> (Y/N)	Nut Restraint System	Temp	Comments
1												
2												
3												
4												
5												
6										· .		
7								· ·				
8												
9								·				
10	· ·			[			1					

#### Notes:

1) For Pin & Hanger assemblies with a redundant support system, indicate if there is a gap between the redundant system (bearing) and the bottom flange of the suspended girder.

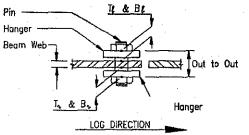
2) All measurements are taken in reference to log direction.

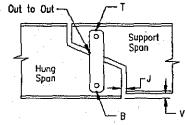
- V : Vertical misalignment of girders @ left edge of girder's bottom flange.
- J : Joint opening between webs, measured just above the
  - bottom flange fillet, on the left face of the girder's web.

Out to Out : The out-to-out of hangers taken at the leading edge,

based on log direction.

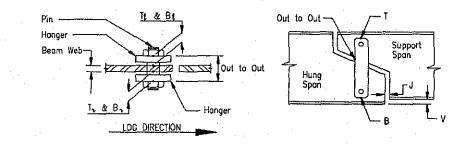
3) Use a permanent marker to indicate locations of field measurements.





	PIN &	HANG		IALYS	IS SHE	ET	Form BRI-	29, Røv. 6/9	9		Measu	rements Reviewed By: Date:
Brid		0 r Location:				•	Town:	0 Effective	span for M	ovement: 0	(ft)	page: of
Веат No.	CALCUL	ATED DIFFEI	RENCES BET	IWEEN MEAS	SUREMENTS		& DATAZ	Differential Movement by Formula "J " (in)	Between			Keviewer's comments
1 2	0	0 0	0 0	0 0	0 0	0	0 0	0	0			
3	0	0	0	0	0	0	0 0	0	0		· · · · ·	
5 •6	0	0	0 0	0	0	0 0	0 0	0	0 0			
7 8	0 0	0 0	0 0	0	0 0	0 0	0	0	0			
9 10	0	0	0	0	0	0	0	0	0 0			

Additional Review Comments:



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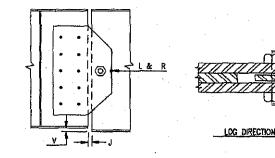
	HINGE	DATA	SHEET		Form BRI-30, R	lev. 9/97			Measurements Taken By	/: Date:		
	Bridge No.:							Town:		Date:		
	Hing	e Located:	•					Effective span for Movement:	(ft)	Page: of		
Beam No.	V (in)	J (in)	R (in)	L (in)	Secondary System Type	Gap <sup>1</sup> (Y/N)	Nut Restraint System		Comments			
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3						· .						
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9												
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#### Notes:

 For Hinge assemblies with a redundant support system, indicate if there is a gap between the redundant system (bearing) and the bottom flange of the suspended girder.

- 2) All measurements are taken in reference to log direction.
   V : Vertical misalignment of girders @ left edge of girder's bottom flange.
  - J : Joint opening between webs, measured just above the bottom flange fillet, on the left face of the girder's web.

3) Use a permanent marker to indicate locations of field measurements.



HINGE DATA SHEET Form BRI-30, Rev. 6/99					Form BRI-30,	Rev. 6/99			Measurements Taken B	/: Date:
Bridge No.:		• • •	<u> </u>	<u></u>	: : :		Town:	(ft)	Page: of	
	Hing	e Locateo;					, E	ffective span for Movement:		0
Beam No	V (in)	J (in)	R (in)	L. (in)	Secondary System Type	Gap <sup>1</sup> (Y/N)	Nut Restraint System		Comments	
1										
2										
3									· · ·	
4								· · · ·		
5										
6										
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8				~ -					· · · · · · · · · · · · · · · · · · ·	
9										
10										

#### Notes:

1) For Hinge assemblies with a redundant support system,

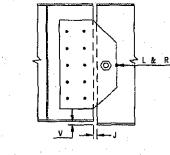
indicate if there is a gap between the redundant system (bearing) and the bottom flange of the suspended girder.

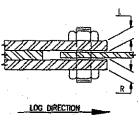
2) All measurements are taken in reference to log direction.

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3) Use a permanent marker to indicate locations of field measurements.





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	HINGE	ANALY	SIS SH	IEET	Form BRI-30, Rev. 9/97 Measurements Reviewed By: D	wiewed By: Date:	
Brid	1	0 er Location:		· · · · · · · · · · · · ·	0     Town:     0       0     Effective span for Movement:     0	of	
eam No.			RENCES B				
Bean	V (mm)	J _(mm)	R (mm)	L (mm)	Reviewer's Comments		
1	0	0	0	0			
2	0	0	0	0			
3	0	0	0	0			
4	0	0	0	Ó			
5	0	0	0.	0			
6	0	0	0	0			
7	0	0	0	0			
8	0	0	0	0			
9	0	0	0	0			
10	0	0	0	0	<u> </u>		

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• i⊚

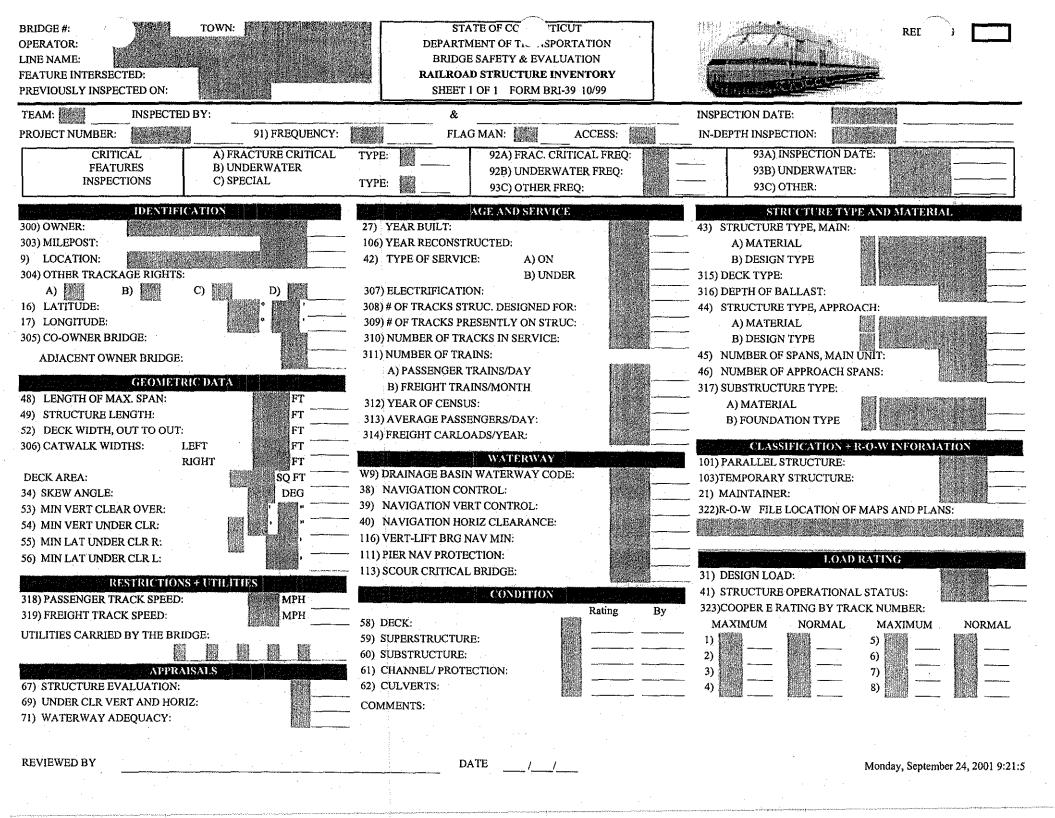
ر معا م

L&R

# Additional Review Comments:

File: Hng\_form.xis(Analysis), Printed: 9/24/01

LOG DIRECTION



### Connecticut Department of Transportation UNDERWATER INSPECTION

			BRI-58 Form	·	
Bridge No:	00001			Town:	GREENWICH
Inspection Date:	06/22/02			Route Carried:	00095
	00/22/02			Feature Crossed:	RAM RIVER,S WATER ST
ITEM	RATING	REMARKS			
60. SUBSTRUCTUR	<b>E</b>				
ABUTMENT 1:					
STEM					
FOOTING					
EROSION					
SETTLEMENT			······································	······································	
SCOUR					
WINGWALLS					
General remarks:					
ABUTMENT 2:					
STEM					
OOTING					
EROSION					
SETTLEMENT					
SCOUR					
WINGWALLS					
General remarks:			· · · · · · · · · · · · · · · · · · ·	· · · · · · · · · · · · · · · · · · ·	
PIER NO. 1					
PILES					
STEM				· · · · · · · · · · · · · · · · · · ·	
FOOTING					
SCOUR					
SETTLEMENT			Ang kang bertapat dan sebesah dan sebesa		
General remarks:					

#### 61. CHANNEL& CHANNEL PROTECTION:

CHANNEL SCOUR	
FMBANKMENT EROSION	
BRIS	

05/16/01

Page 1 of 2

# Connecticut Department of Transportation UNDERWATER INSPECTION

· · · · ·			BRI-58 Form	<b>-</b>	GREENWICH
Bridge No:	00001			Town: Route Carried;	
Inspection Date:	06/22/02				RAM RIVER,S WATER ST
ITEM	RATING	REMARKS			
VEGETATION					
CHANNEL CHANGE		[			
FENDER SYSTEM					
SPUR DIKES & JETTIE					
RIP RAP					
General remarks:					
inspected by:		-	Date:		·····
Inspected by:	<u></u>		Date:	and the second	a and a straight of the straig
D.O.T. reviewed by:			Date:		

## Connecticut Department of Transportation UNDERWATER INSPECTION

BRI-59 Form

	Bridge No:	00001	Date Inspected:	06/2	2/02
Job Number:		· · · · · · · · · · · · · · · · · · ·		Client:	Connecticut D.O.T.
Route: 0	0095	Mile point: 0.0	00	City:	GREENWICH
FeatureCrossed: B	YRAM RIVER,	S WATER ST		State:	CT
Inspector:			A	ssistants:	:
Time Arrived:			Time	Departed:	
Time In Water:			Time Out	of Water:	:
Type of Inspection:					
Year built: 0		Total Length: 1	262 N	o. Spans:	3
Bridge Type:					
tal Number of Piers: 0	)		Piers in t	he Water:	
Type of Piers:					· .
Abutments:			· · · · · · · · · · · · · · · · · · ·		an ann an tha an tha an tao
lottom Composition:					
Previous U/W Insp:		• • • • • • • • • • • • • • • • • • • •			· · · · · · · · · · · · · · · · · · ·
Marine Growth:					
Max. Water Depth:					
Max. Depth at Pier/Abut.:		· ·			
Current Strength:					
U/W Visibility:					
Type of Water:		······			
Access to Bridge:					<b></b>
Remarks:					· · · · · ·
spection Equipment					
Number of Boats: 0	)	RR Prot	tection: No		
Boat Size: 0	)	Equipment Com	ments:		
Dive Station:					
pected by:			Date:		
pected by:	<u> </u>	· · · · · · · · · · · · · · · · · · ·	Date:		
			Date:		

Version: 1.0 - February, 2012

# APPENDIX 14 TYPICAL BRIDGE INSPECTION REPORT

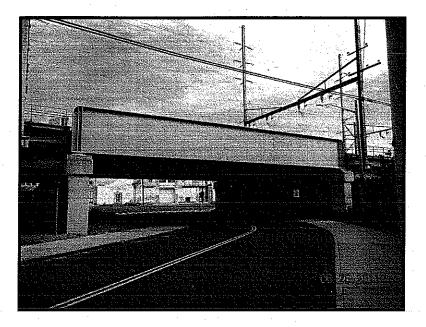
	TATE OF CONNECTICUT TMENT OF TRANSPORTATION	Subject:	Metro-North Mainline Bridge No. 08068R over
	Sample		Noble Street Mile Point 55.98, Bridgeport Routine Inspection Report
	Memorandum	Date:	
Tran	ames E. Fox sportation Principal Engineer au of Public Transportation	From:	Jayantha Mather 594-2885 Transportation Principal Engineer Bureau of Public Transportation
crack expa	Attached are four copies of the most r ture. The deficiencies reported for this s oadway under the bridge were found is in the welds connecting the ballast nsion bearing at pier 0 was found to be enheit (2008) and 75 degrees Fahrenheit	structure ar with broker t retainer p frozen (No	e as follows. Four light fixtures fo n diffusers and bulbs. There are plates to knee braces. Girder 2
	The Office of Rails' Design Unit has the	e following r	ecommendations:
	<ol> <li>Replace the light fixtures and cov further damage (4 EA) (Priority D) 08-08068R).</li> </ol>	er each lig . See Phot	ht with a steel mesh to prevent to No. 11. <b>(Previous RMM No.</b>
	2. Repair the cracked welds (LS) (In note sheet 6. (Previous RMM Note)	Priority D). . 08-08068	See Photos Nos. 7-10 and field <b>R)</b> .
· .	3. Reset the expansion bearing (1 EA	(Priority [	D). See Photo No. 13.
Laber	If you have any further questions conce at (860) 594-2890.	erning this	matter, please contact Mr. Lev
Attack	iments		
CC:	Mr. David B. Willard (1 Set)		
Micha	el Lee/ml		
CC:	Jayantha Mather Lev Laber – Jay Young – Haresh Dhola Rail File	kia – Jen S	cott – Abraham Monger (1 Set)

## RAILROAD BRIDGE INSPECTION (SAMPLE) STATE PROJECT NO. 300-097

CONNDOT BRIDGE NO. 08068R (METRO-NORTH BRIDGE M.P. 55.98)

#### METRO-NORTH RAILROAD (MAINLINE) OVER NOBLE AVENUE BRIDGEPORT, CONNECTICUT

### **THROUGH GIRDER**



#### **ROUTINE INSPECTION**

July 12, 2011

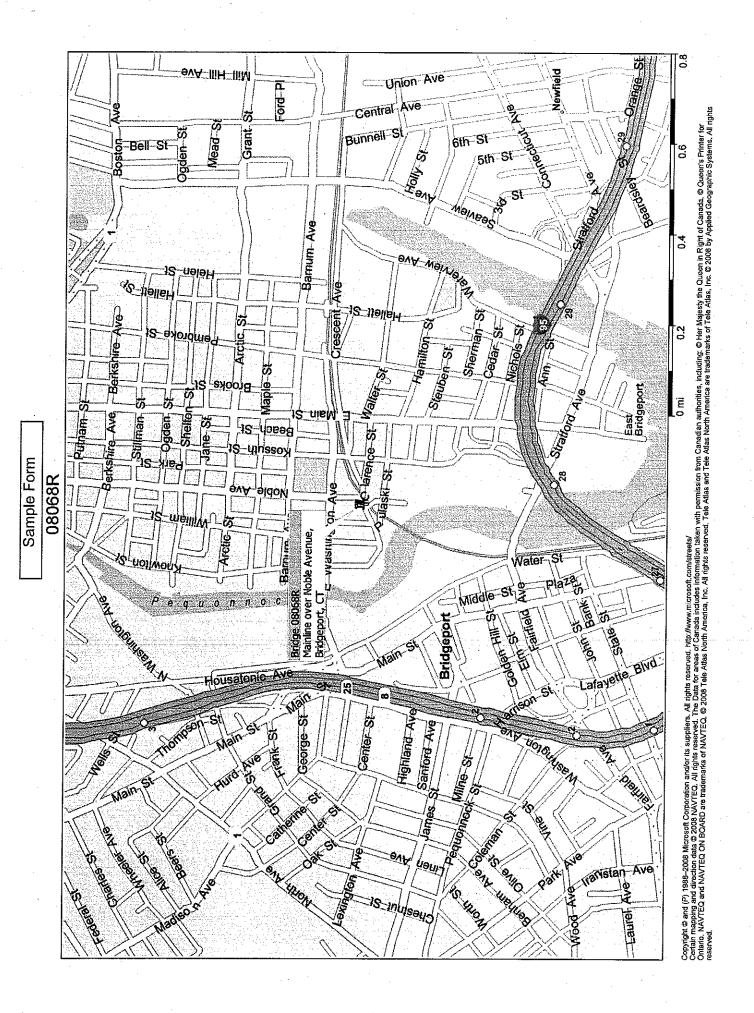
Prepared By: WSP - SELLS Transportation & Infrastructure Danbury, Connecticut

# WSP · SELLS

### STATE OF CONNECTICUT DEPARTMENT OF TRANSPORTATION BUREAU OF PUBLIC TRANSPORTATION OFFICE OF RAIL

### INSPECTION REPORT TRANSMITTAL FORM FORM BRI-27, REV. 01/12

	Structure No	Tow <u>n :</u>	
Inspectors:		Dat	te:
FORMS (bound in report)	TABLE OF CO		. of Sheets
Railroad Maintenance Memora	andum (RMM)		Enclosed
Title Cover Sheet			
BRI-27, Inspection Report Tra	nsmittal Form (Table of Conten	ts)	
Bridge Location Map	· · · · · · · · · · · · · · · · · · ·		
Executive Summary	·····		<del>-</del>
Time at Site Including Flaggin	g		
BRI-9, Construction Punch Li	st		
BRI-11, Seismic Screening Da	ita Sheet		
BRI-12, Fracture Critical Insp	ection Data Sheet		
BRI-18, Bridge Inspection For	m		
BRI-24, Incident Report Form			
BRI-25, Under Entry SI&A Fo	9 <b>rm</b>	·····	
BRI-39, RR Bridge SI&A For	m		
	RI-10, BRI-13, BRI-14, BRI-15, 29, BRI-30)		
Calculations: Load Rating Evaluation	n		
Photo Sheets			
Back-up Material			
BRI-58, Underwater Inspectior	a Report Form	•••••••••••••••••••••••••••••••••••••••	
BRI-59, Underwater Inspectio	n Structure Data Form		······



#### EXECUTIVE SUMMARY (SAMPLE)

### 07/12/2011

Bridge No. 08068R (Mile Point 55.98) carries four Metro-North Railroad Mainline tracks over Noble Street in the City of Bridgeport. This one-span bridge was built in 1998 and has an overall length of 78 feet and a deck width of 73.3 feet. The superstructure consists of a steel deck on girder-floorbeam system, which is supported by reinforced concrete piers. This structure abuts Bridge No. 08067R to the south and Bridge No. 08101R to the north. The minimum field vertical clearance under the bridge was measured to be 15 feet – 8 inches.

WSP • SELLS, under CONNDOT contract 300-097, conducted a routine inspection of this structure on June 20 and July 12, 2011.

The bridge was found to be in good condition (Overall Rating = 7). The deck was found to be in good condition (Overall Rating = 7). The underside of deck contains scattered areas of missing paint and light rust on the welds. The superstructure was found to be in good condition (Overall Rating = 7). The superstructure contains minor scrapes in the steel members, two loose bolts, light rust on bolts and one expansion bearing frozen (No change in position at 40 degrees Fahrenheit (2008) and 75 degrees Fahrenheit (2011)). The substructure was found to be in very good condition (Overall Rating = 8).

The recommendations for this bridge are as follows:

### **Repair Recommendations**

- 1. Four light fixtures for the roadway under the bridge were found with broken diffusers and bulbs. Repair the light fixtures and cover each light with a steel mesh to prevent further damage (4 EA).
- 2. There are cracks in the welds connecting the retainer plates to knee braces. A few cracks have extended since the last inspection but none have propagated into the base metal. Repair the cracks (LS).
- 3. Girder 2 expansion bearing at pier 0 was found to be frozen. Reset the bearing (1 EA).

#### **Rehabilitation Measures**

No rehabilitation measures are recommended.

	•		• •		
			· · · ·		
тім	E AT SITE	Project No.			7
		Bridge No.	Sheet		1
Date:	Crew:				7
	Equipment Used:				
Time:	Police:		Flagman:		
	(Name/Time)		(Name/Time)		
Work Done:					
					1
Date:	Crew:				<b>]</b>
	Equipment Used:			-	7
Time:	Police:		Flagman:		7
	(Name/Time)		(Name/Time)		
Work Done:					ľ
	· .				
Date:	Crew:		·····		7
	Equipment Used:				1
Time:	Police:		Flagman:		1
-	(Name/Time)		(Name/Time)		
Work Done:	an ann ann an t-			· · · · ·	1
Date:	Crew:				
	Equipment Used:			·	]
Time:	Police:	· · ·	Flagman:		1
<u> </u>	(Name/Time)		(Name/Time)		
Nork Done:	· · · · · · · · · · · · · · · · · · ·				1
					]
Date:	Crew:				
	Equipment Used:	· · · · · · · · · · · · · · · · · · ·			
Time:	Police:		Flagman:	· · ·	
Nork Done:	(Name/Time)		(Name/Time)		
tone.					1
Date:	Crew:		······································		
Time:	Equipment Used: Police:		Flagman:		ĺ
	(Name/Time)		(Name/Time)		
Vork Done:					
······································	· · · · · · · · · · · · · · · · · · ·				1
				· .	· .
		•			

BUREAU OF PUBLIC TRANSPORTATION OFFICE OF RAIL	DATE:		
CONSTRUCTION PUNCH LIST	FormBRI-9, Rev. 01/12	PAGE 1 of 2	
Construction Project Number:			
Bridge No Town:	Route:		
Feature Intersected:			
Bridge Type:		······	
Mile Point:	Construction District:		
Construction Company Name:			
D.O.T. Construction Inspectors Name:			
List of Items to be Corrected or Completed	d:		
1			
2			
3			
4			
5.			
6			
7			
8			
9			
10			

PUNCH LIST

BUREAU OF PUBLIC TRANSPORTATION OFFICE OF RAIL	DATE:		
CONSTRUCTION PUNCH LIST	FormBRI-9, Rev. 01/12	PAGE 2 of 2	
Bridge No			
Photo Log:			
1			
2			
3			
4			
5			
Additional Comments (If any):			
Date copy sent to Construction:			
Submitted By:		Date:	
Checked By:		Date:	
PUNCH LIST			

		BRI-11
<b>SEISMIC DATA SHEET FOR RA</b>	AILROAD BRIDGES	

Prepared By:Date:		ridge #	Mile Point:
	Т	own:	
Entered By:Date:	F	eature Crossed:	· · · · · · · · · · · · · · · · · · ·
	L	ine Name:	
	0	perator:	·
Feature Crossed Informatio			
ADT year o			
Community Lifeline			
Major River Crossing	_(y/n)		
Feature Carried Information	n:		
Utilization			·
Freight	Less than 10	More than	10
Service only	passenger trains/day	passenger	trains/day
Detour available	YesIncon	venientNone	
Alternate Service (Bu	s) available(y/n)		
			·
Structure Information:			
Bridge Construction			
Continuous	Single span o	or N	fultiple
W/Integral abu	ts continuous m	ultiple spans sin	ple spans
Marshar 60		· · · · · · · · ·	
Number of Spa	ns Numl (y/n) Move ngthft Max single ft Skew	per of longitudinal gire	ders
Total Dridge la	(y/n) Move	able Bridge(y	//n)
Total Bridge tel Dridge width	ngtn ft Max single	spanf	
A hutmont hoir!	It Skew	· · · · ·	degrees
Abument neigi	ntftIs abut masonry or t	unreinforced conc.	(y/n)
· · · ·			
Bearing and Bridge Se	at Construction		
	ring Type	If rocker Height	Inches
Are Bearings of	n Pedestals (pad > 6" high)	(v/n)	Inches
Expansion Bea	rings Transversely Restrained_	(y/n)	and the second
P	go manovorbory restrained_	(y/ii)	, ,
Vulnerable Bearing Inf	ormation		
	ing Longitudinal Support Leng	th Inc	her
Expansion Beau	ing Transverse Support Length	Inche	
<b>F</b>	ing manorense support isengu		<b>3</b>
Effective Lengt	hft, Effective Heigh	nt ft	
	, 2110161	It	
Pier Construction	No pier (single span bridge		
Steel or ductile	conc. Steel or d	uctile conc.	Wall type
Multi-column be	conc. Steel or d ent single column	1	Pier
			* 191
Is pier founded of	on battered pile $(y/n)$		
	nry or unreinforced concrete	(v/n)	
Is transverse mo	vement between pier and beam	s accommodated	(y/n)
IIIO	Pier and better pier and beam		(y/ii)

CDOT Office of Rail

## CONNECTICUT DEPARTMENT OF TRANSPORTATION FRACTURE CRITICAL MEMBERS/FATIGUE PRONE DETAILS

	Form BRI12, Rev 9/97		
Bridge No:	Fracture Critical Inspe FC Insp Freq:Mo	ction Date:	e:
Town:		ADT:	Year of ADT:
Facility Carried:		Structure Type:	% Truck:
Feature Intersected:			
Access Equipment Needed:			
Traffic Control Required:		· · · · · · · · · · · · · · · · · · ·	
Reference to Plans:			
MEMBER/DETAIL TYPE #			· .
Member/Detail Type:	· · · · · · · · · · · · · · · · · · ·	·····	Fracture Critical
Fatigue Category: Stee	el Type:		Fatigue Prone
Description:		· · · · · · · · · · · · · · · · · · ·	
Inspection Procedure:			
MEMBER/DETAIL TYPE #			· · · · · · · · · · · · · · · · · · ·
Member/Detail Type:			Fracture Critical
Fatigue Category: Stee	l Type:		Fatigue Prone
Description:			
Inspection Procedure:			

## CONNECTICUT DEPARTMENT OF TRANSPORTATION FRACTURE CRITICAL MEMBERS/FATIGUE PRONE DETAILS INSPECTION DATA SHEET

Form BRI12, Rev 9/97

MEMBER/DETAIL	<b>TYPE #</b>	
Member/Detail Type:		Fracture Critical
Fatigue Category:	Steel Type:	Fatigue Prone
Description:		
Inspection Procedure:		
MEMBER/DETAIL	<b>TYPE #</b>	
Member/Detail Type:		Fracture Critical
Fatigue Category:	Steel Type:	Fatigue Prone
Description:		1999
Inspection Procedure:		
MEMBER/DETAIL	TYPE #	
Member/Detail Type:		Fracture Critical
Fatigue Category:	Steel Type:	Fatigue Prone
Description:		
Inspection Procedure:		
MEMBER/DETAIL	ТҮРЕ #	
Member/Detail Type:		Fracture Critical
Fatigue Category:	Steel Type:	Fatigue Prone
Description:		
Inspection Procedure:		

	AMPLE			· · · · · · · · · · · · · · · · · · ·	Bridge	• •	
	oquired: N	Snooper Re	7/12/2011	l: 1	INSPECTION DATE	68R	
	sed: N	Snooper Us	12/5/2008	TION DATE:	PREVIOUS INSPEC	ROUTINE	nspection Type:
ER/FLR BE	STEEL GIRI	: 0000	998 YEAR REBUILT	YEAR BUILT: 19		DGEPORT	IOWN: BRI
		INUE	RSECTED: NOBLE AVE	FEATURE INTER		NLINE	INE: MAI
		RIV	MIE OF PEQUONNOCK	LOCATION: .1 M	RAILROAD	RO-NORTH I	PERATOR: ME
			INSPECTION VISITS:	· · ·	-	VI:	NSPECTION TEA
9:00 AM	Start Time:	6/20/2011	Inspection Date:		3):	EE	1): MICHAEL I
2:00 PM	End Time:	75 °F	Temperature:		4):	IUZZA	2): ADAM UGI
8:15 AM	Start Time: End Time:		Inspection Date: Temperature:				
1:30 PM	2110 mma;	00 F			· · · · · · · · · · · · · · · · · · ·		
RATING	OVERALL			· . ·			58. DECK
VISIBLE DUE	ND WHICH IS NOT		G OVERLAIN WITH 1-1/2" OF D STEEL RAILS REST ON B	E WATERPROOFING	THERE IS MEMBRAN	RATIN	. OVERLAY
CONNECTING	ATTERED WELDS	ORBEAMS, SC AVE BACKER	THE TOP FLANGE OF FLOC EXHIBIT LIGHT RUST AND H V AREAS OF MISSING PAINT	ECK IS WELDED TO T	THE STEEL PLATE D THE DECK PLATE TO	TION 7	. DECK-STR. COND
S EXHIBIT	P RETAINER PLAT	TEM. THE TO	ARE RATED UNDER THIS I	ALLAST RETAINERS	THE STEEL PLATE B	5	CURBS
THE LAST	ROPAGATED SINC	CKS HAVE PR	ACKS WERE NOTED IN TH OTOS 7 AND 9). SOME CRA IO CRACKS WERE FOUND 1 AND 6.	EE BRACES (SEE PHO HOTOS 8 AND 10). NO	PLATES TO THE KNE		
			······································	·····		. <b>N</b>	MEDIAN
	· · ·					N	SIDEWALKS
· · · · · · · · · · · · · · · · · · ·			-			N	PARAPET
· · · · · · · · · · · · · · · · · · ·						N	RAILING
THE	HE DECK PLATE C	BEAMS TO TH SCATTERED	CONNECTING THE FLOOR	AINT AT THE WELDS	THERE IS MISSING PAUNDERSIDE, AS WEL	6	PAINT
DNING			RE IS NO INDICATION THAT			N	DRAINS
ACENT			VAY UNDER THIS STRUCTU BROKEN DIFFUSERS AND E			RD 3	LIGHTING STANDA
BE PART OF	AS NOTED TO CO	VER PIER 1 W PHOTO 12).	WEST SIDE OF GIRDER 1 O SECTION OF CONDUIT (SEE RE, IS NOT ADDRESSED IN	ATTACHED TO THE V IES AND A MISSING S	THE JUNCTION BOX A DISCONNECTED WIR	ZE. 7	I. UTILITIES TYPE/SI
	· · · · · · · · · · · · · · · · · · ·		······································	· · · · ·		DINTS N	CONSTRUCTION J
			OF THE STRUCTURE ARE L ERE NO NOTABLE DEFECT			8	B. EXPANSION JOINT
ATING 7	OVERALL					TURE	). SUPERSTRUC
	NGE IN POSITION	ZEN (NO CHA	0 WAS FOUND TO BE FROM DEGREES FAHRENHEIT (2	N BEARING AT PIER	GIRDER 2 EXPANSIO	5	BEARING DEVICES
HE MASONR	(SEE PHOTO 14). STAL DUE TO THE	Y ENGAGED	D THE NUTS ARE NOT FULL " EDGE GAPS TO THE CONO SIDE. SEE FIELD NOTE SHE	ER 1 ARE TILTED AND RDER 2 EXHIBITS 1/4"	FOR GIRDER 1 AT PIE PLATE AT PIER 0, GIR		•
						N	STRINGERS
S	R BOTTOM FLANG	THE GIRDER	SCATTERED LOCATIONS OF ELD NOTE SHEET 6.	S AND GOUGES AT S TRUCTURE, SEE FIE	THERE ARE SCRAPES	7	GIRDERS
O NOTE	OTO 18). SEE FIEL	PES (SEE PH	3 13 AND 15 CONTAIN SCRA			7	FLOOR BEAMS

## Connecticut Department of Transportation Bridge Inspection Report BRI-18

BRIDGE #: 08068R		INSPECTION DATE: 7/12/2011 Snooper Required: N PREVIOUS INSPECTION DATE: 12/5/2008 Snooper Used: N
TRUSSES-PORTALS	N	
TRUSSES-BRACING	N	
6. PAINT	7	THERE IS MISSING PAINT AT SCRAPES, BOLTS AND WELDS.
7. RUST	7	THERE IS LIGHT RUST AT AREAS OF MISSING PAINT (SCRAPES, BOLTS, WELDS).
8. MACHINERY MOV. SPAN	N	
9. RIVETS AND BOLTS	7	UNDER TRACK 1, ONE BOLT IS LOOSE AT THE LONGITUDINAL DIAPHRAGM TO FLOORBEAM CONNECTION FOR FLOORBEAMS 24 AND 25 (SEE PHOTOS 15 AND 16). SCATTERED NUTS AND BOLTS HAVE LIGHT RUST (SEE PHOTO 17).
10. WELDS-CRACKS	8	
11. TIMBER DECAY	N	
12. CONCRETE CRACKING	N	
13. COLLISION DAMAGE	7	THERE ARE MINOR SCRAPES NOTED ON THE GIRDER AND FLOORBEAM BOTTOM FLANGES THROUGHOUT THE STRUCTURE.
14. ALIGNMENT OF MEMBERS	8	
15. DEFLECTION UNDER LOAD	N	(N) NORMAL (E) EXCESSIVE
16. VIBRATION UNDER LOAD	N	(N) NORMAL (E) EXCESSIVE
60. SUBSTRUCTURE:		OVERALL RATING B
•	ATING	
1. ABUTMENTŞ STEM	N	THE STRUCTURE ABUTS BRIDGE 08067R TO THE SOUTH AND BRIDGE 08101R TO THE NORTH AND DOES NOT HAVE ABUTMENTS (SEE PHOTO 2).
BACKWALL	N	
FOOTING	N	
SETTLEMENT	N	
WINGWALLS	N	
PIERS OR BENTS-CAPS	8 -	THE PIER CAPS EXHIBIT RUST STAINS. THE VERTICAL EDGES OF THE PEDESTALS AT BOTH PIERS ARE NOT CHAMFERED, BUT TO NO DETRIMENTAL EFFECT.
PILE BENT	N .	
COLUMN	8	THE COLUMNS WERE FOUND TO HAVE NO NOTABLE DEFICIENCIES.
FOOTING		
	N	NOT VISIBLE.
SETTLEMENT	N N	
SETTLEMENT 2. EROSION-SCOUR		
	N	
2. EROSION-SCOUR	N 8	
2. EROSION-SCOUR 3. CONGRETE CRACK-SPALL	N 8 8	
2. EROSION-SCOUR 3. CONCRETE CRACK-SPALL 4. STEEL CORROSION	N 8 8 N	
2. EROSION-SCOUR 3. CONCRETE CRACK-SPALL 4. STEEL CORROSION 5. PAINT	N 8 8 N	

61. CHANNEL & PROTECTION

OVERALL RATING N

8/2/2011 12:17:36 PM

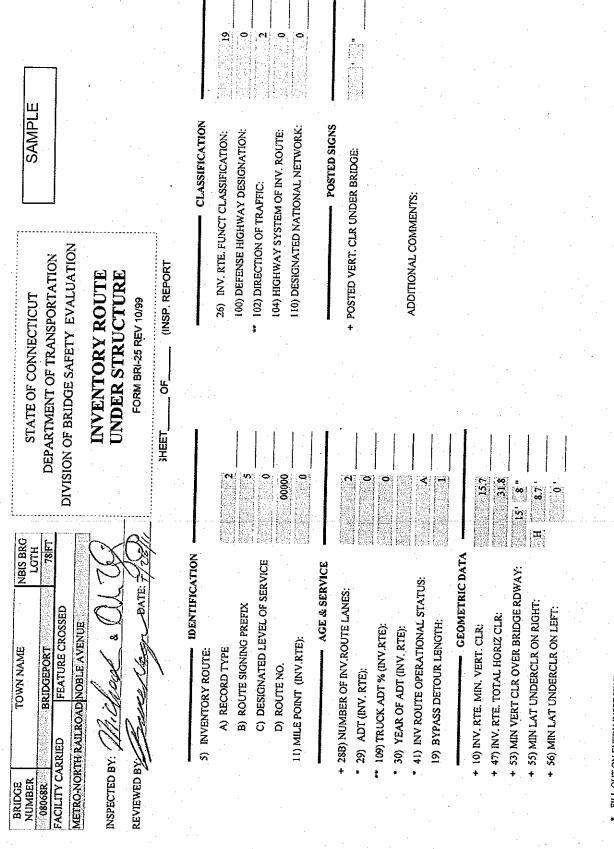
BRIDGE #: 08068R Inspection Type: ROUTINE		TION DATE: US INSPECTION DATE:	7/12/2011 12/5/2008	Snooper Required Snooper Used:	я: N N	
62. CULVERTS & RETAIN	NG WALL	S		(	OVERALL RATING	N
63. ESTIMATED REMAINI		YEARS				
63. ES HIMATED REMAINI				· · · · · · · · · · · · · · · · · · ·		
64. PERMIT CAPACITY (O	FFICE ITE	M)				
65. APPROACH CONDITIC			999/24,4,4,8848/2492224,4,4,4,4,4,4,4,4,4,4,4,4,4,4,4,4,	(	OVERALL RATING	P
R. APPROACH SLAB	ATING	RUCTURE ABUTS BRIDGE 08	067R TO THE SOUTH	AND BRIDGE 08101R TO THE	NORTH.	
2. RELIEF JOINTS	1					
3. APPROACH GUIDE RAIL	ı.					
PAVEMENT	I		· · · · · · · · · · · · · · · · · · ·		· · · · · · · · · · · · · · · · · · ·	
EMBANKMENT P	•					
4. TRAFFIC SAFETY FEATURE				·······		
66. RATED LOADING			· · · · · · · · · · · · · · · · · · ·			
1. POSTED LOADING: RA	11NG					
SINGLE UNIT (TONS)	·					
SEMI TRAILER (TONS)				······		
3. ADVANCE WARNING Y/N N	·					
4. LEGIBILITY N						
5. VISIBILITY/LOCATION N						
		INSPECTORS APPR	AISAL OF OVER	ALL STRUCTURE CON	DITION:	7
MIN VERT. UNDERCLR.	15'-8"					
	N					
	N 30 MPH	FOR PASSENGER TRAIL				
CHARACTER OF TRAFFIC:		HEAVY, COMMUTER TR				]
ADDITIONAL NOTES:						
INVENTORY OF SHEETS IN CO INV.: 8 PHOTOS: ADDITIONAL COMMENTS:			SMNTS: 1	CLEAR DIAG. 1 AT	TACH: 7	

## Connecticut Department of Transportation Bridge Inspection Report BRI-18

BRIDGE #: 08068R Inspection Type: ROUTINE	INSPECTION DATE: PREVIOUS INSPECTION DATE:	7/12/2011 12/5/2008	Snooper Required: N Snooper Used: N
Inspectors' Signatures:	1) Michage		Date: 7128111
	a) Oil 17		Date: 7,28,11
:		· · · · · · · · · · · · · · · · · · ·	Date://
4	l)		Date://
P.E. Signature: P.E.#:	<u> 17786</u>	Jean	Date: <u>-7</u> .1 <u>2</u> <u>8</u> 1 <u>//</u>
Reviewed by:	۲۰۰۰ - ۲۰۰۰ - ۲۰۰۰ - ۲۰۰۰ - ۲۰۰۰ - ۲۰۰۰ - ۲۰۰۰ - ۲۰۰۰ - ۲۰۰۰ - ۲۰۰۰ - ۲۰۰۰ - ۲۰۰۰ - ۲۰۰۰ - ۲۰۰۰ - ۲۰۰۰ - ۲۰۰۰	СДОТ	Date://

State of Connecticut Department of Transportation Bureau of Public Transportation	SheetOf Form BRI-24, Rev. 01/12
	CIDENT REPORT
Town:	Bridge No.
Route/Street:	
Over:	Туре:
Inspected by:	Date: / _/
Date of Incident: / / / Tin	ne of Incident: "D" no.:
Details of Incident:	
	· · · · · · · · · · · · · · · · · · ·
Immediate Actions Taken:	
Additional Remarks:	

INCIDENT.RPT

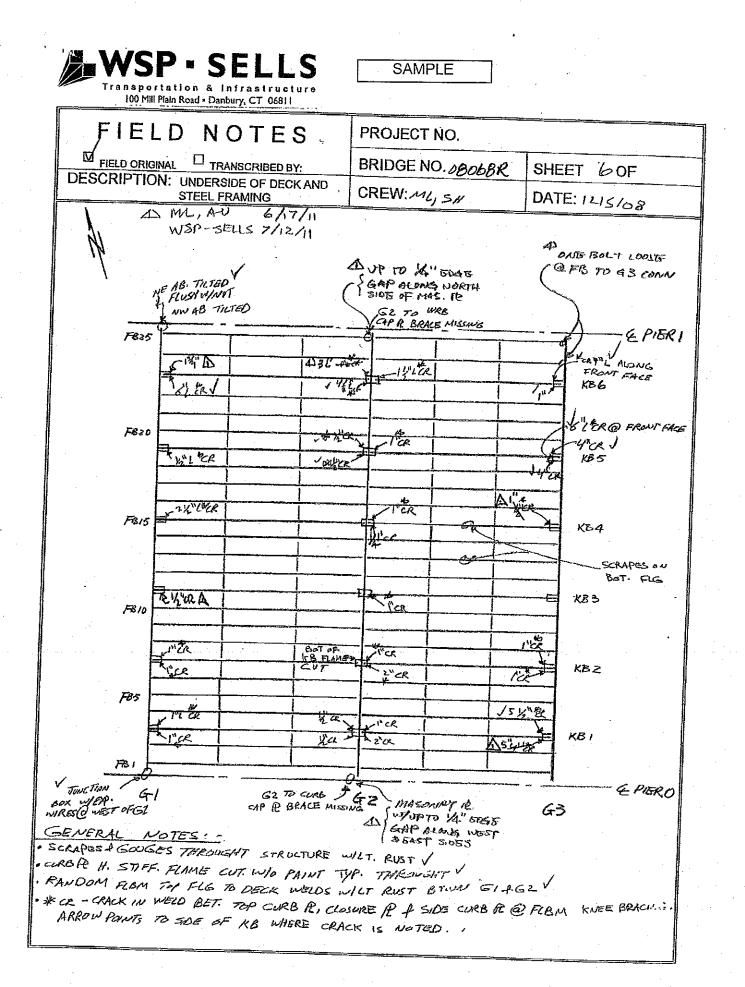


FILL OUT ON EVERY INSPECTION 28, 109, 30, 41

+ VERIFY EVERY INSPECTION 288, 10, 47, 53, 55, 56 & POSTED VERT CLEARANCE UNDER THE BRIDGE

\*\* MUST BE FILLED OUT OR VERIFIED ON THE FIRST INSPECTION MADE BASED ON THE NEW FHWA GUIDE 109, 102

SAMPLE RED FLAG	IN-DEPTH INSPECTION: 12/5/2008 93A) INSPECTION DATE: 7/12/2011 93B) UNDERWATER: 93C) OTHER:	43) STRUCTURE TYPE, MAIN:         43) STRUCTURE TYPE, MAIN:         A) MATERIAL         B) DESIGN TYPE         315) DECK TYPE:         315) DECK TYPE:         316) DEPTH OF BALLAST:         317) SUBSTRUCTURE TYPE         A) MATERIAL         B) DESIGN TYPE         0         7) MATERIAL         10) NABER OF APPROACH SPANS:         117) SUBSTRUCTURE TYPE:         A) MATERIAL         2) DESTRUCTURE TYPE:         A) MATERIAL         2) DESTRUCTURE TYPE:         317) SUBSTRUCTURE TYPE:         317) SUBSTRUCTURE:	Thursday, July 21, 2011 12:45:35 P
VIECTICUT ANSPORTATION EVALUATION RE INVENTORY M BRU-39_10/99	92A) FRAC: CRITICAL FREQ: 24 92B) UNDERWATER FREQ: 24 93C) OTHER FREQ: 93C) OTHER FREQ:	D:       1998         D:       A) ON       1998         B) UNDER       1         ESIGNED FOR:       2         LY ON STRUC:       4         NDHH       1         Chay       1         MANH       6610         SDAY:       6610         MANY CODE:       99         SYDAY:       6610         MANY CODE:       0000	DATE 7 221//
RIDGEPORT ENUE	RACTURE CRITICAL TYPE: C UNDERWATER SPECIAL TYPE:	CONNDOT S5.98 NOCK RIV 0 11,1,1 0 0 0 0 0 0 0 0 0 0 0 0 0	leen -
BRIDGE #: 08068R TOWN: B OPERATOR: METRO-NORTH RALROAD LINE NAME: MAINLINE FEATURE INTERSECTED: NOBLE AV PREVIOUSLY INSPECTED ON: 12/5/2008 TEAM: 10 NSPECTED BY: PROJECT NUMBER: 300-097	CRITICAL FEATURES INSPECTIONS	RACKAK RACKAK B B DE: DE: T OWNE T OWNE OWNE T OWNE OWNE T OWNE T OWNE T OWNE T OWNE T OWNE O	REVIEWED BY



FIELD NOTE	S	PROJECT	NO.		· · · · · · · · · · · · · · · · · · ·	
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Bridge No.: \_\_\_\_\_ Date: \_\_\_\_

# CONCRETE DETERIORATION WORKSHEET Form BRI-10, Rev. 9/01

				Det	erioration	By Span	- In Squar	e Feet			
					S	Span Num	ber				
Deterioration Type	1	2	3	• . 4 .	5	6	7	8	9	10	Total
	Тор:	Тор:	Тор:	Тор:	Тор:	Тор: •.	Тор.	Тор:	Тор:	Top:	Top:
Spalled and Delaminated Areas	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:
	Тор	Тор:	Тор:	Тор:	Тор:	Тор:	<b>Тор</b> :	Тор:	Тор:	Тор:	Top:
Scale (Moderate to Severe Only)	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:
Cracks: w/Efflorescence (use 6 in. width x length)	Bot:	Bot:	Bot:	Bot.	Bot:	Bot.	Bot:	Bot:	Bot:	Bot:	Bot:
w/o Efflo. (use 3 in. width x length)	Bot:	Bot:	Bot	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:
Map Cracking: w/Efflorescence (use full area)	Bot:	Bot:	Bot:	Bot:	Bot:	Bot	Bot;	Bot:	Bot:	Bot:	Bot:
w/o Efflo. (use 50% of area)	Bot:	Bot:	Bot:	Bot:	Bot;	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:
Honeycombed Areas (only areas more than 1 1/2 in. deep)	Bot:	Bot:	Bot;	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot:	Bot;
· ·			· ·								
Total Span Area (Square Feet)										-	
% Spalled and Delaminated on Top											
% Deterioration on Bottom			. •								

Figure 6.1.2a - Concrete Deterioration Worksheet

# PHOTO LOG Form BRI-13, Rev. 9/97

Bridge Informatio	
Image Inventory	
Bridge No.	
1	Photographer:
Film Frame #	Image Description
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· ·	
415m2(co.upg	

		R R J nment		LIDING	Beam	sure and "R"	—Sole P —Sliding ~Masoni	Plate y Plate mment On: - -	SHEET Span No. = Substructure Unit = Temperature =*F Presence of keepers or work done on bearings. Undermining of bearing. Attach sketch with dimensior Cracking of plates or welds. Condition of anchor bolts.	ns.
		Move	ement	· · · · · · · · · · · · · · · · · · ·	1	Conditior				
Beam	29 J 29	"R"	Mode Exp. or Contr.	Lateral Misalign.	Bearing Frozen?	Normal Mov't?	Rust ? H/M/L		Comments	
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		· ·			SEARING MEASUREMENTS
Span N	0. =	_			
Substru Unit =	cture				Beam
Temper	ature = _ Sin <sup>-1</sup> (F-1	<u>•</u> F		The "F bearing facing	Front" of the $F^*$ $F^*$ $B^*$ $B^*$
Y =	r tan 0				
NOTE:					Masc
"F" & ' corners	of the ro	be measure cker or on of the sub	the s	ide close	side est to skewed bridges.
Beam	*F*	"B"	Y	Cont. or Exp.	Comments
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expansion	on bearings ites below)	do not h	lave keeper b	ars.	-	Bear	n		
(000 110					Leveling Plat	e			
	. L- 	·		╤┟╸	Sole Plate Keeper Bar		· ··· · · ···		Front of is the Sid
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					$\overline{\mathbf{D}}$				+
	LeftSide		Lateral asurements	Right Side	<u>t</u> '(2)	Front of keeper Negative if keep	to front oper passes	of base. —— front of	Expansion measurem
1.1		FRC	DNT_VIEW			base.	SIDE V		O left sid right side
$\bigcirc$	-Left & Richt		ned when facing	the Front	of the Rearin				Take at F of Bearing
	-For non-gui	led bearings,	measure from s	lide of sol	e plate to sid	e of piston O c		-	
			measure expans	ion from t	front of sole (	plate to front of	piston O	center line of	f bearing.
Coan	i No. & Su	ibstructure	Unit =			·		Temperatu	ire = <u>F</u>
	r								
	Exp. Mea	Expansion surement			nteral			mments	<u> </u>
Beam	Exp. Mea	. ,	n Side of Brg. (N,S,E,W)	Lo	nteral Right		Co	mments	
		surement	Side of Brg.				Co	mments	· · · · · · · · · · · · · · · · · · ·
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			3	<b>NINT N</b>	JOINT MEASUREN	RE	MENTS	S	Form BRI-	Form BRI-17, Rev. 9/97		Measurements Taken Bv.		
BMG	Bridge No.:	, II	• 1				-		Town:			Measurements Reviewed By:	~	
Deck	Deck	Effect		Winter Measurements	Jrements	Sur	nmer Meas	umer Measurements	Difi	Differentiat Movements	Vemente		22,52	
Joint		Span <sup>2</sup>	Ë,	Date:		Temp			Ac	Actual	pv.	Commente		Lutterence Between
Location	Type'	Ĵ.	*	Left (in.)	Right (in.)	. u.	L1	Right (in.)	Left (ii	Right (in.)	Formula		Left (in.)	Left (in.)   Rinht (in.)
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1. The "De 2. The "Eff	ck Joint ective Si	Type's pan'is t	the len	be "plug" igth contri	<ol> <li>The "Deck Joint Type" should be "plug" for Asphaltic Plug</li> <li>The "Effective Span" is the length contributing to expansio</li> </ol>	ic Plu( :pansi	g joints, "s on at the j(	trip" for Str oint. This s	ip Seals, "s hould be 0	seg." for Se <sub>t</sub> at fixed ioin	joints, "strip" for Strip Seals, "seg." for Segmental Joints, etc. In at the joint. This should be 0 at fixed inints of single shan helder	etc. an hridaec		•
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	PIN &	PIN & HANGER DATA SHEET	GER	DATA	SHE			Form BRI-29, Rev. 6/99	1, Rev. 6/9(	D		Measurements Taken By: Dat	Date:
Bridg	Bridge No.:							· •	Town:				
	Hanger	Hanger Location:						Effective	e span for	Effective span for Movement:		(ft) Page: of	oť
eam No.	>	ر ب	بر ۲	B,	1/	B/	ă e ă Õ <sup>e</sup> Õ	Secondary System	Gap_	Nut Restraint	qməT	Comments	
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		I) FOLFTILLO HARIGELASSERIDIES WITH A FOUNDART SUPPORT SYSTEM,	FINDINES W.	nin a requi	noam sup	port syste	Ĕ,			ĉ			

indicate if there is a gap between the redundant system (bearing) and the bottom flange of the suspended girder.

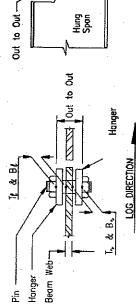
V : Vertical misalignment of girders @ left edge of girder's 2) All measurements are taken in reference to log direction.

bottom flange.

J : Joint opening between webs, measured just above the

Out to Out : The out-to-out of hangers taken at the leading edge, bottom flange fillet, on the left face of the girder's web. based on log direction.

3) Use a permanent marker to indicate locations of field measurements.



Support Span

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	PIN 8	HAN	GER	PIN & HANGER DATA SHEET	SHEI			Form BRI-29, Rev. 6/99	, Rev. 6/9	0		Measurements Taken By: Date:	
Brid	Bridge No.:	.0			· ·	0		•	Town:	0			
	Hanger	Hanger Location:			0			Effectiv	e span for	Effective span for Movement:	0	Page: of	
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Notes:

 For Pin & Hanger assemblies with a redundant support system, indicate if there is a gap between the redundant system (bearing)

inducate in under is a gap between the required to system (t and the bottom flange of the suspended girder.

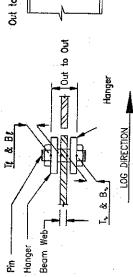
All measurements are taken in reference to log direction.
 Vertical misalignment of girders @ left edge of girder's

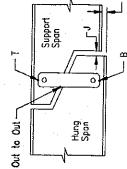
bottom flange.

J : Joint opening between webs, measured just above the bottom flange fillet, on the left face of the girder's web.

Out to Out : The out-to-out of hangers taken at the leading edge, based on log direction.

3) Use a permanent marker to indicate locations of field measurements.





Date: 5 Keviewer's comments page: Support Measurements Reviewed By: 2 œ Ţ ٥ C Out to Out ---Hung Spon € AZZZ CON to Out to Out 0 – Hanger Effective span for Movement: LOC DIRECTION 11 & Br Between Actual & Difference Calc. J h (in) 0 0 0 Φ 0 0 0 0 ò 0 11/14 I, & B. Form BRI-29, Rev. 6/99 Differential Movement Out to Out by Formula (in) **"** (in) 0 Beam Web-0 0 0 0 o 0 Ó 0 0 0 Honger Pin L Town: CALCULATED DIFFERENCES BETWEEN MEASUREMENTS ON DATA1 & DATA2 (i 0 0 Ó 0 o 0 0 o 0 0 (<u></u> 6 **PIN & HANGER ANALYSIS SHEET** 0 0 0 0 0 0 0 0 o 0 (in) o 0 0 0 0 o 0 0 0 ο 0 Ň (j.) **Additional Review Comments:** 0 0 0 0 0 0 0 o  $\circ$ 0 Φ (in) ۲\_ o 0 0 Φ φ 0 0 0 0 0 Hanger Location: (j Bridge No.: 0 7 0 0 0 0 0 Ò Φ 0 0 C Ē > ¢ 0 0 0 ¢ ¢ o o o Ģ 10 .oN msea 2 ŝ က 4 ¢ ŋ 2

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0	Bridge No.:			•	 . '			Town		
	Hìng	Hinge Located:	-					vement:	(ft) Page: of	uate: ge: of
Beam No.	> (ij	ۍ <u>(</u>	٤ ۲	<u>و</u> ب	Secondary System Type	Gap <sup>1</sup> V/N/	Nut Restraint Svetern		Comments	
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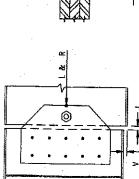
1) For Hinge assemblies with a redundant support system, indicate if there is a gap between the redundant system (bearing)

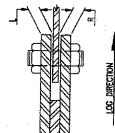
and the bottom flange of the suspended girder. 2) All measurements are taken in reference to log direction. V : Vertical misalignment of girders @ left edge of girder's

bottom flange.

 ${f J}$  : Joint opening between webs, measured just above the

bottom flange fillet, on the left face of the girder's web. 3) Use a permanent marker to indicate locations of field measurements.





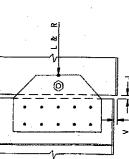
	HINGE	<b>HINGE DATA SHEET</b>	SHEET		Form BRI-30	Form BRI-30, Rev. 6/99			Measurements Taken By:	Date:
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oNu					Secondary		Nut			
Bear	> (£	ل (r)	R (j	J (j	System Type	Gap <sup>1</sup> (Y/N)	Restraint System		Comments	
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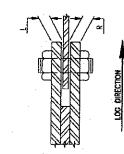
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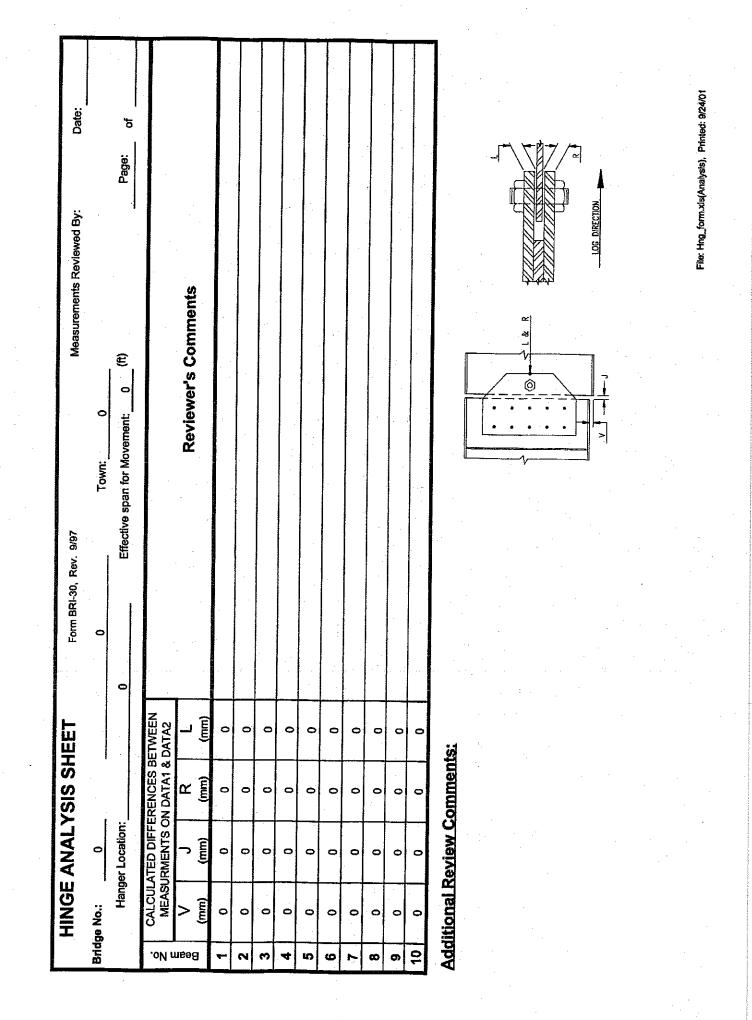
For Hinge assemblies with a redundant support system, indicate if there is a gap between the redundant system (bearing)

and the bottom flange of the suspended girder. 2) All measurements are taken in reference to log direction. V : Vertical misalignment of girders @ left edge of girder's

bottom flange fillet, on the left face of the girder's web. 3) Use a permanent marker to indicate locations of field measurements. J : Joint opening between webs, measured just above the bottom flange.







Bridge No.		Project No.		
Town		Inspected By		
Feature Carried		Inspected By		
Feature Crossed		Date Inspected		
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Photo No. 1: Bridge identification number	n number	Photo No. 2:		
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## Back-up Material Sheets Bridge No. Date:

**Connecticut Department of Transportation** SAMPLE **UNDERWATER INSPECTION BRI-58 Form** Bridgeport Town: Bridge No: 08064R Mainline **Route Carried:** Inspection Date: 11/4/2008 Feature Crossed: Pequonnock River REMARKS RATING **ITEM** Localized scour has exposed the footing inside the sheetpile cofferdam up to 6.5' in vertical height. 0. SUBSTRUCTURE 7 However an average of 1.0 of aggradation has occurred since previous inspection. Not inspected - Out of the water. Ν BUTMENT 1: N STEM N FOOTING EROSION N SETTLEMENT N SCOUR Ν Ν WINGWALLS General remarks: Not inspected - Out of the water. N BUTMENT 2: Ν STEM Ν FOOTING Ν EROSION SETTLEMENT N SCOUR N N WINGWALLS General remarks: Generally minor scour and aggradation has occurred since the previous inspection, with the exception 6 PIER NO. 1 of the southeast comer that exhibits 2.6' of local scour. PILES N Not visible. The stem exhibits 5% or less missing mortar in the stone masonry joints. 7 STEM The exposed steel sheet piling exhibits moderate to heavy corrosion. 7 FOOTING Generally minor scour and aggradation has occurred since the previous inspection, with the exception SCOUR 6 of the southeast corner exhibits 2.6' of local scour. 8 SETTLEMENT The Fender system is in good condition. General remarks: The exposed sheet pilling exhibits moderate to severe corrosion. PIER NO. 2 7 N Not visible. PILES The stem exhibits 5% or less missing mortar in the stone masonry joints. STEM 7 7 The exposed sheet piling exhibits moderate to severe corrosion. FOOTING Primarily aggradation up to 2.5' has occurred since the previous inspection SCOUR 7 8 SETTLEMENT

2/30/2008

Page 1 of 2

Connecticut Department of Transportation SAMPLE UNDERWATER INSPECTION

			BRI-58 For	n	
Bridge No:	08064F	<b>č</b>		Town:	Bridgeport
Inspection Date:	11/4/2008	- - 		Route Carried:	Mainline
				Feature Crossed:	Pequonnock River
ITEM	RATING	REMARKS			
General remarks:	The fende	er system is in good conditi	on.		
PIER NO. 3	6	Localized scour has expo however an average of 1'	sed the footing ir of aggradation ha	side the sheetpile cofferdam up to 6.5" i s occurred since the previous inspectio	n vertical height; n.
PILES	N	Not visible.			
STEM	7	The stem exhibits 5% or le	ess missing mort	ar in the stone masonry joints.	
FOOTING	7	The exposed sheet piling	exhibits moderate	to heavy corrosion.	
SCOUR	6	Localized scour has exposi however an average of 1' of	sed the footing in of appradation ha	side the sheetpile cofferdam up to 6.5' ir s occurred since the previous inspection	vertical height;
SETTLEMENT	8				<b>.</b>
General remarks:	Pier 3 is fe	ounded on steel H-piles.			· · · · · · · · · · · · · · · · · · ·
1. CHANNEL& CHA	NNEL PRO				
	6	The channel is moderately penetrations of 0.5' to 3.0',	susceptible to s and typical flow	cour due to the sand and silt bottom cor of 1/2 F.P.S.	nposition, with
CHANNEL SCOUR	6	The channel is moderately penetrations of 0.5' to 3.0',	susceptible to sand typical flow	cour due to the sand and silt bottom con of 1/2 F.P.S.	nposition, with
EMBANKMENT EROSIC	8				
DEBRIS	8		· .		
VEGETATION	8		••••••••••••••••••••••••••••••••••••••		
CHANNEL CHANGE	7	Little or no channel change	has occurred si	ice the previous inspection.	
FENDER SYSTEM	7	Generally, the timber fende the tidal zone due to freeze	er system is in go thaw damage	od condition, with minor delamination u	p to 1/4" deep in
SPUR DIKES & JETTIE	N				
RIP RAP	7	An isolated pocket of riprar	was observed a	ong the south elevation of pier No.1	
General remarks:					
		1 11	······		
spected by:	R	TLan	Date:	Feb. 27, 2010	
	آرم	XIL		Feb. 27, 2010 Feb. 27, 2010	. ·
pected by:	Chais	mar (Bar)	_ Date: _	10. 61, 6010	- -
9.7. reviewed by: _			_ Date:		
			• • • • • • • • • • • • • • • • •		——————————————————————————————————————

0/2008

# Connecticut Department of Transportation

			Form	N SAMPLE	]
	Bridge No:		Date Inspected:	11/3/2008	· .
Job Number:	300-097	·		Client: Connecticu	t D.O.T.
Route:	Mainline	Mile point: 55.90		City: Bridgeport	·
FeatureCrossed:	Pequonnock Riv	ver		State: CT	
Inspector:	Clay Carlson	· ·	Ass	istants: Chris Drake	e/Ken Mischou
Time Arrived:	7:30 AM		Time De	parted: 3:30 PM	
Time In Water:	9:15 AM		Time Out of	Water: 12:15 PM	
Type of Inspection:	In-Depth				
Year built:	1998	Total Length: 462	No.	Spans: 5	
Bridge Type:	Steel Movable B	lascule			
Total Number of Piers:	6	· · ·	Piers in the	Water: 3	
Type of Piers:	Stone Masonry I	acing over Concrete		• •	
Abutments:	N/A	н. Н	· · · ·	·	
Bottom Composition:	Silt, sand and m solated pocket c No. 1.	iscellaneous man ma of riprap .08'-1.5' in di	de debris, with 0.5' ameter was observe	- 3.0' penetration (T) ed along the south el	rp). An evation of pier
Previous U/W Insp:	10/9/2006		· · ·		
Marine Growth:	Moderate barnad	cles and soft marine g	rowth.		
Max. Water Depth: 2	28.8'				
Max. Depth at Pier: 2	23.0'				
Current Strength: .	5' FPS				
U/W Visibility: 3	3.0 <sup>1</sup>				
Type of Water: S	Salt				. * . *
Access to Bridge:	Boat				
Remarks: S	Supported by ste	el H-piles.	· .		
spection Equipment					
Number of Boats: 1	•	RR Protectic	n: No		
Boat Size: 2		Equipment Commen			
Dive Station: Y		-1			
		0 1			
pected by:	<u>lly p</u>	tt	ate:	27. 1010	<u> </u>
pected by:	huis Deke	<u>2 14</u> D	ate: <u>frø.</u>	27, 2010	
.T. reviewed by:		D	ate:		
2010		·			Page 1

Version: 1.0 - February, 2012

## APPENDIX 15 S-PROGRAM SUMMARY

S-BRIDGE REPAIR PROGRAM SUMMARY

				• = = • .									
TRACK NAME BRIDGE # TOWN	LOCATION	M.P.	8 6	4		2		1		3	5	7	ACTIVE INACTIVE
DANBURY 04134R NORWALK	*MARSHALL STREET	0.11		N/A				S18			N/A		
DANBURY 08200R NORWALK	*ANN STREET	0.19	N/A			S18 & S20	N/A	S18 & S20			N/A		S9
DANBURY 08202R NORWALK	*NORWALK RIVER	3.20		N/A				S18 & S19			N/A		S14 N/A
DANBURY 08203R WILTON	*BROOK	5.12		N/A				S17			N/A		<b>S17</b> N/A
DANBURY 08204R WILTON	*STREAM	6.43		N/A				S20			N/A		
DANBURY 08205R WILTON	*NORWALK RIVER	6.64		N/A				S20			N/A		
DANBURY 08206R WILTON	*NORWALK RIVER	8.70		N/A				S20			N/A		
DANBURY 08207R WILTON	*NORWALK RIVER	9.42		N/A				S20			N/A		
DANBURY 08208R WILTON	*BROOK	9.91		N/A				S17			N/A		
DANBURY 08215R REDDING	*UMPAWAUG POND BROOK	16.41					N/A						S14 N/A
DANBURY 01020R BETHEL	GRASSY PLAIN ROAD (RTE. 53)	19.64					N/A						S6 N/A
DANBURY 08217R BETHEL	*NORWALK RIVER	19.99		N/A				S20			N/A		
DANBURY 08218R BETHEL	*SYMPAUG BROOK	21.41					N/A						<b>S11</b> N/A
MAIN LINE 03943R GREENWICH	NORTH WATER STREET	26.10	N/A	S7	N/A	S7	N/A	S6	N/A	S6	N/A		N/A
MAIN LINE 03946R GREENWICH	*STEAMBOAT ROAD	28.22	N/A	S10 & S13	N/A	S11 & S13	N/A	S12 & S13	N/A	S13	N/A		N/A
MAIN LINE 08003R GREENWICH	*DAVIS AVENUE #2	28.48	N/A	S10	N/A	S11 & S20	N/A	S12 & S20	N/A	S13 & S20	N/A		N/A
MAIN LINE 08004R GREENWICH	*INDIAN HARBOR	28.68	N/A	S10	N/A	S11	N/A	S12 0 520	N/A	S13	N/A		N/A
MAIN LINE 08005R GREENWICH	*SACHEM ROAD	29.48	N/A N/A	S10 & S18 & S19	N/A	S9 & S11 & S19	N/A	S12 & S19	N/A	S13 & S19	N/A N/A	N/A	N/A
MAIN LINE 08006R GREENWICH	*LUKES CROSSING (S. SHORE DR)	29.68	N/A N/A	S10 & S18	N/A	S11 & S19	N/A	S12 & S19	N/A	S12 & S13 & S19	N/A N/A	IN/A	N/A
MAIN LINE 03948R GREENWICH	*SOUND BEACH AVENUE	31.29	N/A N/A	S11 & S14	S11	S11 & 519 S11	S11	S6 & S11	S11 & S14	S6, S11 & S14	N/A N/A		N/A
MAIN LINE 03955R GREENWICH	*TOMAC AVENUE	31.62	N/A	S15	S15	S15	S12 & S15	S15	\$12	S12 & S15	N/A		N/A
MAIN LINE 03680R STAMFORD	*GREENWICH AVENUE	32.81	N/A	S9 & S10	\$11	S11 & S13	S8 & S12	S8, S12	S8 & S13	S8 & S13	N/A		N/A
MAIN LINE 08012R STAMFORD	*ATLANTIC STREET	33.19	N/A	S16 & S18 & S19	S16 & S19	S16 & S17 & S18 & S19		S-18		S-18		N/A	N/A
MAIN LINE 03678R STAMFORD	*CANAL STREET	33.41	N/A N/A	S9	N/A	S11	N/A	S12	N/A	S8	N/A <b>S15</b>	N/A <b>S15</b>	N/A
MAIN LINE 03686R STAMFORD	*ELM STREET	33.75	S8, S17 & S19 & S20 S20 S7,S8,S17 & S19 & S20 S20	S17 & S19 & S20	S20	S11, S17 & S19 & S20	S20	S14,S15,S17 & S19 & S20		S8, S17 & S19 & S20	S20 S17 & S19 & S20		N/A
MAIN LINE 02237R STAMFORD	*EAST MAIN STREET (US RTE. 1)	34.17	N/A	S9, S10, S12 & S19	\$12	S11 & S12	S12	S11 & S12	S12	S9, S11 & S12	S12 S12	N/A	N/A
MAIN LINE 08013R STAMFORD	*HAMILTON AVENUE	34.72	N/A	S10 & S13 & S20	S11 & S13	S11 & S13 & S20	S12 & S13 & S20	S12 & S13 & S20	S12 & S13	S9 & S13 & S20	S9 & S13 S13 & S20	N/A	N/A
MAIN LINE 08014R STAMFORD	NOROTON RIVER	35.58	N/A	S10							N/A		N/A
MAIN LINE 08015R DARIEN	*STATION PED. OVERPASS	36.24	N/A				S14				N/A		N/A
MAIN LINE 04271R DARIEN	RAYMOND STREET	38.93	N/A	S7 & S19	N/A	S7 & S18	N/A	S7, S8 & S17 & S18	N/A	S7, S8 & S16	N/A	İ	N/A
MAIN LINE 08022R DARIEN	FIVE MILE RIVER	39.07	N/A N/A		1973		19/73	0.,0000110010	19/14	S6	N/A N/A		N/A
MAIN LINE 00022R DARIEN MAIN LINE 04132R NORWALK	ROWAYTON AVENUE #1	39.07	N/A N/A	1	N/A		N/A	#3088	N/A		N/A N/A		N/A N/A
	*FRANKLIN STREET	40.89	N/A N/A		S15	C4E			S14 & S15			N/A	N/A N/A
				045	315	S15	S14 & S15	S14 & S15	314 & 313	S15	S15	IN/A	
MAIN LINE 03693R NORWALK	*WASHINGTON & MAIN STREET	41.28	N/A N/A	S15			S15			S15	N/A		N/A
MAIN LINE 08028R NORWALK	*OSBORNE AVENUE	41.96	N/A	S20		S20		S20		\$20			
MAIN LINE 03691R NORWALK	*EAST AVENUE	42.14	N/A	S9 & S13 & S20	S13 & S20	S9 & S13 & S20	S13 & S20	S9 & S13 & S20	S13 & S20	S9 & S13 & S20	N/A		N/A
MAIN LINE 08032R WESTPORT	*SAUGATUCK RIVER	44.32	N/A	S11	N/A	S11	N/A	S11	N/A	S11	N/A		N/A
MAIN LINE 08035R WESTPORT	*NEW CREEK ROAD	47.15	N/A	S7,S16 & S17	N/A	S6, S7 & S16	N/A	S7 & S16	N/A	S7,S16 & S17	N/A		N/A
MAIN LINE 04198R FAIRFIELD	*ROUND HILL ROAD	50.90	N/A	S12	N/A	S12	N/A	S12	N/A	S12	N/A		N/A
MAIN LINE 08045R FAIRFIELD	*FAIRFIELD CREEK	51.68	N/A	S14 & S20	N/A	S14 & S20	N/A	S14 & S20	N/A	S14 & S20	N/A		N/A
MAIN LINE 03635R BRIDGEPORT	FAIRFIELD AVENUE	53.42	N/A	S9	S8 & S9	S9		S9	S8 & S9	S9	N/A		N/A
MAIN LINE 08048R BRIDGEPORT		53.60	N/A	S9, S10 & S14	S9 & S14	S9, S10 & S14	S9 & S14	S10	S9 & S14	S9, S10 & S14	N/A		N/A
MAIN LINE 08050R BRIDGEPORT		53.72	N/A	S13 & S20	S20	S13 & S20	S20	S13 & S20	S20	S13 & S20	N/A		N/A
MAIN LINE 08050R BRIDGEPORT		53.84	N/A N/A	S12	N/A	S12	N/A	S12	N/A	S12	N/A		N/A
MAIN LINE 08050R BRIDGEPORT		54.07	N/A N/A	S12 S19	N/A N/A	S12 S19	N/A N/A		N/A N/A	S12 S19	N/A N/A		IN/A
								S19					
MAIN LINE 08052R BRIDGEPORT		54.22	N/A	S16 & S17	N/A	S17 & S18	N/A	\$17	N/A	\$17	N/A		N/A
MAIN LINE 08053R BRIDGEPORT		54.44	N/A	S9, S14 & S15	S15	S9 & S15	S15	S9, S14 & S15	S14	S9, S14 & S15	N/A		N/A
MAIN LINE 03638R BRIDGEPORT	*PARK AVENUE	54.58	N/A	S17		S17 & S18		S8, S9 & S16		S16 & S17	N/A		N/A
MAIN LINE 08054R BRIDGEPORT	*MYRTLE AVENUE	54.70	N/A	S10,S11 & S16 & S17	S9 & S16	S9 & S17 & S18	S9&S17	S9, S10 & S11 & S17	S9 & S17	S10 & S17	N/A		N/A
MAIN LINE 08055R BRIDGEPORT	*WARREN STREET	54.77	N/A	S16 & S17	S18	S17 & S18	S18	S17 & S19		S17 & S19	N/A		N/A
MAIN LINE 08056R BRIDGEPORT	LAFAYETTE STREET	54.84	N/A	S9 & S20	N/A	S9 & S20	N/A	S9 & S20	N/A	S8 & S20	N/A		N/A
MAIN LINE 08057R BRIDGEPORT	*BROAD STREET	54.93	N/A	S7 & S16 & S17	S16 & S17	S7 & S17 & S18	S18	S7,S13, S17,S18 & S19		S7 & S17	N/A		N/A
MAIN LINE 08058R BRIDGEPORT	*MAIN STREET #1	54.98	N/A							S13	N/A		N/A
MAIN LINE 08060R BRIDGEPORT	*UNION STREET	55.40	N/A	S16 & S19	S16	S16	S16	S17	S17	S17,S18 & S19	N/A	N/A	N/A
MAIN LINE 03639R BRIDGEPORT	EAST MAIN STREET (RTE. 127)	56.20	N/A	S19	S19						N/A		
MAIN LINE 08070R BRIDGEPORT		56.35		S15 & S20	S20	S15 & S20	S20	S14 & S15 & S20		S14 & S20	N/A	1	N/A
MAIN LINE 08071R BRIDGEPORT		56.46	N/A	S15 & S20	S15	S15 & S20	S20	S15 & S20	S15	S15	N/A N/A		N/A
	*SEAVIEW AVENUE	56.77	N/A S20	S12 & S15 & S20	S15 & S20	S12 & S15 & S20	\$20 \$20	S12 & S15 & S20	S15 & S20	S12 & S15 & S20	N/A N/A		N/A
MAIN LINE 08075R BRIDGEPORT		57.46	N/A S9, S10 & S19 & S20 S9 & S17 & S18			S9 & S19 & S20	S16	S9,S10,S16 & S19 & S20		S9,S10,S16 & S19 & S20		N/A	N/A N/A
MAIN LINE 08077R STRATFORD	*BRUCE AVENUE	57.62	N/A S18 & S20 S20 S20	S18 & S20	S17 & S18 & S20	S17 & S20	S17 & S20	S16 & S20	S16 & S20	S16 & S20	00 0 010 0 020 03	N/A N/A	N/A N/A
MAIN LINE 08077R STRATFORD	*WEST BROAD STREET	57.62	N/A 318 & 320 520 N/A	S7 & S13 & S20	S17 & S18 & S20 S13		S17 & S20 S13	S16 & 520 S13	S16 & S20 S13 & S20	S16 & S20 S7 & S13	N/A	IN/A	N/A N/A
						S13 & S20							
MAIN LINE 08079R STRATFORD	*KING STREET	58.88	N/A	S12	S12	\$12 \$7 \$42 \$40	S12	S12	S12	S12	N/A		N/A
MAIN LINE 01318R STRATFORD	*MAIN STREET (RTE. 113)	59.01	N/A	S8 & S13 & S18		S7,S13, S19	S7,S13 & S19 & S20	S6, S7 & S13 & S20	S7 & S13 & S20	S6 & S13 & S20	N/A		N/A
MAIN LINE 01312R STRATFORD	*EAST MAIN STREET (RT 110)	59.96	N/A S19 N/A	S19	S19						N/A		
MAIN LINE 08084R MILFORD	*GULF STREET	63.83	N/A	S18	N/A	S19	N/A	S19	N/A		N/A		
MAIN LINE 08090R MILFORD	DEPOT ROAD	66.66	N/A	S6	N/A	S6	N/A	S6	N/A	S6	N/A		N/A
MAIN LINE 08093R WEST HAVEN	MORGAN LANE	68.11	N/A		N/A		N/A			N/A	S10	N/A	N/A
MAIN LINE 08097R WEST HAVEN	*CAMPBELL AVENUE	70.19	N/A	S13 & S16	N/A	S13	N/A	S13		N/A	S13	N/A	N/A
MAIN LINE 08098R WEST HAVEN		70.36	N/A	S14 & S20	S14 & S20	S14 & S20	S14 & S20	S14 & S20	S14 & S20	N/A	S14 & S20	N/A	N/A
NEW CANAAN 01302R NEW CANAAN		6.16					N/A				0		<b>S6</b> N/A
NEW CANAAN 08154R NEW CANAAN		7.00		N/A				S17 & S18 &19			N/A		S17 N/A
WATERBURY 08254R MILFORD	BEARDS FARMWAY	4.55		19/73			N/A					I	S6 & S7 N/A
WATERBURY 08262R DERBY	*NAUGATUCK RIVER	4.55 8.62					N/A N/A						
			<u> </u>	K1/A			19/75	040			N/A	<b> </b>	S12 S12
WATERBURY 08264R ANSONIA	*NAUGATUCK RIVER	10.30		N/A			N1/A	<b>S18</b>			N/A	<b> </b>	
WATERBURY 08265R ANSONIA	*TAIL RACE	10.55					N/A						S12 N/A
WATERBURY 08267R ANSONIA	*SPILLWAY	11.36					N/A						S13
WATERBURY 08268R SEYMOUR	*CANAL	12.57					N/A						S14&S15 N/A
WATERBURY 08270R SEYMOUR	*JAMES ST. FOOTBRIDGE	14.45					N/A						S10
WATERBURY 08272R SEYMOUR	*NAUGATUCK RIVER	14.64					N/A						#5265 N/A
WATERBURY 04224R NAUGATUCK	*MAPLE STREET	21.74		N/A				S20			N/A	1	
WATERBURY 08284R NAUGATUCK	*BRIDGE STREET	22.74					N/A		•				S14 N/A
WATERBURY 03723R WATERBURY	*BRISTOL STREET	24.10	N/A					S20			N/A		1973
WATERBURY 04232R WATERBURY		26.18	1975				N/A	020			N/A		S6&S16
WATERBURY 04232R WATERBURY WATERBURY 08287R WATERBURY		26.18		N/A			1.11/71	S20			N/A		000010
WATENDONT 002078 WATERBURY	JAONOON STREET	20.00	1	IN/A				320			19/71		
	0.0		S40 041 040	-011	A15	040	0.47	-0.10	0.40	2.00	* 0455 51 50 51 5		
PROGRAM S6 S7	S8	S9	S10 S11 S12 S13	S14	S15	S16	S-17	S-18	S-19	S-20	* CADD FILES ON C	UNINDO L'S FILE	
FISCAL YEAR 96-97 97-98	98-99	99-00	00-01 01-02 02-03 03-04	04-05	05-06	06-07	06-07	08-09	09-10	11-12			

#### APPENDIX 16 LIST OF RAILROAD BRIDGES AND MICROFILM AVAILABILITY

# APPENDIX 16 LIST OF RAILROAD BRIDGES SORTED BY STATE BRIDGE NUMBER AND MICROFILM A

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	Å	2	RAILROAD	TRACK CHART NAME	TOWN	DT	ST. BRID. NO.	TYPE		TRK MI		OLD NO.	RTE	RTE. MI BR TY	P DECK	ТК	SP LGN		.т
	12		METRO-NORTH	MAINLINE WATERBURY BRANCH	BRIDGEPORT	3	00105A 03191A	H	CT TURNPIKE I-95 CONN ROUTE 84 EAST	54.4 26.74	54.4 26.74	54.39	95 84	28.5 IB 32.02 IB		4			1967
		E	METRO-NORTH	WATERBURY BRANCH	The second s							the second				1			1050
		L.			and the second design of the s			Н								4			
			METRO-NORTH		STAMFORD	3	32		CT TURNPIKE I-95 SO.	34.02	34.02	34.02		8.4 TG		4	13 10	65	
Sector         Sector<	BOX 1			and the second se		-	Cashir and Cashir									4			
			and the second	the second s		-		Н				53.28	95			4	1	90	1958
Second         Second<			and the second sec													4			
Image: Note of the second se		and the second se													-	4			
District         Non-State         Non-State <th< td=""><td>BOX 1</td><td>CONNDOT</td><td>METRO-NORTH</td><td>MAINLINE</td><td></td><td></td><td></td><td>-</td><td>the second /td><td></td><td></td><td>the second s</td><td>1</td><td></td><td>OD</td><td>4</td><td></td><td></td><td>1894 300-00</td></th<>	BOX 1	CONNDOT	METRO-NORTH	MAINLINE				-	the second			the second s	1		OD	4			1894 300-00
Intro         No. 10.000007         Observation         <			to the second se										1			4			
No.         No. <td>BOX 1</td> <td>the second s</td> <td>and the second se</td> <td>MAINLINE</td> <td>NEW HAVEN</td> <td>3</td> <td>334</td> <td>Н</td> <td>WATER STREET US 1</td> <td>72.49</td> <td>72.49</td> <td></td> <td>1</td> <td>48.05 TT</td> <td></td> <td>8</td> <td></td> <td></td> <td></td>	BOX 1	the second s	and the second se	MAINLINE	NEW HAVEN	3	334	Н	WATER STREET US 1	72.49	72.49		1	48.05 TT		8			
Profile         Profile <t< td=""><td></td><td></td><td>the second se</td><td></td><td></td><td>-</td><td></td><td></td><td></td><td></td><td></td><td>the second se</td><td>8</td><td></td><td></td><td>1</td><td></td><td></td><td>1961</td></t<>			the second se			-						the second se	8			1			1961
No. 2007         Submodel Market Science         3         Tab.         A         East Production         355         -A         65         57/10         I         1         100        100         100 <th< td=""><td></td><td></td><td>and the second se</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>68</td><td></td><td></td><td>1</td><td></td><td></td><td></td></th<>			and the second se										68			1			
Image         Description         Description <th< td=""><td>BOX 1</td><td></td><td></td><td>the state of the s</td><td></td><td></td><td></td><td>Н</td><td>the second se</td><td></td><td></td><td>3.35</td><td></td><td></td><td></td><td>1</td><td></td><td>and a second /td><td></td></th<>	BOX 1			the state of the s				Н	the second se			3.35				1		and a second	
Diverse         Particle Service         Paritinte Service         Particle Service					and the second se			H				7.29	and the second se			1			
PUTK         VERTICAL DEPENDED         EXAMPLE         F. 2         UNIT         UNIT <thunit< th=""> <thunit< th="">         UNIT<td>BOX 1</td><td>AMTRAK</td><td>METRO-NORTH</td><td>WATERBURY BRANCH</td><td>DERBY</td><td></td><td>the second se</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>OD</td><td>1</td><td></td><td></td><td></td></thunit<></thunit<>	BOX 1	AMTRAK	METRO-NORTH	WATERBURY BRANCH	DERBY		the second se								OD	1			
Control         Control <t< td=""><td></td><td></td><td>and the second se</td><td></td><td></td><td>-</td><td></td><td></td><td></td><td></td><td></td><td>and the second s</td><td></td><td></td><td>OD</td><td>2</td><td></td><td></td><td>1961</td></t<>			and the second se			-						and the second s			OD	2			1961
DEC         ONLY OF CONTROL         MERCE DIFFE TO A         OPE TO A        OPE TO A         OPE TO A <td></td> <td></td> <td></td> <td></td> <td>REDDING</td> <td>3</td> <td>1304</td> <td>H</td> <td>REDDING ROAD</td> <td>11.79</td> <td>11.79</td> <td>11.79</td> <td>107</td> <td>0.25 IB</td> <td></td> <td>1</td> <td>2 1</td> <td>19</td> <td>1954</td>					REDDING	3	1304	H	REDDING ROAD	11.79	11.79	11.79	107	0.25 IB		1	2 1	19	1954
EXATT         Extension         MARCHAR         Extension         A         State         I         Extension         State         C         S <td></td> <td>the second se</td> <td></td> <td></td> <td>the second s</td> <td></td> <td>the state of the s</td> <td>Н</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>4</td> <td></td> <td></td> <td></td>		the second se			the second s		the state of the s	Н								4			
DDC         DMUCRE         PERCENT         PATHERS (PRAVE)         CARE         PATHERS (PRAVE)         PATHERS (PRAVE)         PATHERS (PRAVE) </td <td>BUX 1</td> <td></td> <td>and the second se</td> <td></td> <td>and the second data and the second data</td> <td></td> <td>1613</td> <td>н</td> <td>and a second /td> <td>46.25</td> <td>46.25</td> <td>46.25</td> <td>476</td> <td>0.22 IB</td> <td></td> <td>4</td> <td>1</td> <td>71</td> <td>1957</td>	BUX 1		and the second se		and the second data		1613	н	and a second	46.25	46.25	46.25	476	0.22 IB		4	1	71	1957
Constant	BOX 1	AMTRAK	METRO-NORTH			+ +	A DESCRIPTION OF THE OWNER OWNER OF THE OWNER OWNER OF THE OWNER OWNE		and the second				727	the second se		1			
EVENCE OF         WALE BOY READED         BALE WALE ALL         4         ISS         H         BALE WEEK SACE         5         C         BALE													8			1	5 7	04	
DOT:         ONTRO         ONTRO         DAT         MALE         DAT         MALE         DAT         DAT <thdat< th="">        DAT         DAT         DA</thdat<>		CONNDOT	METRO-NORTH	WATERBURY BRANCH	BEACON FALLS	-							42			1			1966 1970 TURNII
BODY         DEVICE ONNOTION         MALESANDE PORCE         DATA         DEVICE ONNOTION         CP         ID         ID         SE         ID           DOT         PATA         MARINES         DATA         DATA <td< td=""><td>BOX 1</td><td></td><td></td><td></td><td></td><td></td><td></td><td>A</td><td></td><td></td><td></td><td></td><td>/</td><td></td><td></td><td>4</td><td></td><td></td><td></td></td<>	BOX 1							A					/			4			
BODY         ALTINGY         DEPTEX CONTIN         MARK NO         ME (199)         1         CONTINUE         CON			METRO-NORTH	WATERBURY BRANCH	MILFORD	3	3642	A	NAUGATUCK AVENUE		0.08					1			1911 EAST V
No.         CONARGE         METER ADDRETIN         MARINE         Difference         A         SPRAM CALL DIAL								A	and the second se							4			
BOY         OWNER         WIRE         DATE         DATE <thdate< th="">         DATE         DATE         <thd< td=""><td></td><td></td><td>and the second se</td><td></td><td></td><td></td><td>and the second se</td><td>A</td><td></td><td></td><td></td><td>26.35</td><td></td><td>Π</td><td></td><td>4</td><td>1</td><td>74</td><td>1892</td></thd<></thdate<>			and the second se				and the second se	A				26.35		Π		4	1	74	1892
Bit # W         With Grown         Bit # W				and the second se	and the state of the	-	the second s	A								4			
BOT         MATHREDOK         METHOD         3         BABO         A         CARRENT         G 21         G 21 <thg 21<="" th=""> <thg 21<="" th="">         G 21         <t< td=""><td></td><td></td><td>and a second /td><td></td><td>and the second se</td><td></td><td>the state of the s</td><td>A</td><td></td><td></td><td></td><td>and the second se</td><td></td><td></td><td>-</td><td>4</td><td></td><td></td><td></td></t<></thg></thg>			and a second		and the second se		the state of the s	A				and the second se			-	4			
BOY         MAYNEROK IN         MANUAR         DOKEN         S         Diego         A         FOLD         S         Diego         A         FOLD         S         Diego         A         FOLD         S         Diego         A         FOLD         S         Diego         FOLD         A	BOX 1	MAYBROOK	METRO-NORTH	the second se	the same distance and the same state of the same		the second s	A								4			
Box         CONNECT         LETENORITH         MARLINE         OPENPAL         3         3840         A         ONE STREET         468         C21         MTT         4         C1         73         198           Cont         MARLENCE         MERCE STREET         468         C26         C26         FT         4         5         6         198           Soct         MARLENCE         MERCE STREET         488         C26         C26         FT         4         5         6         198           Soct         MARLENCE         MERCE STREET         3         SSS2         A         SSS2         SSS2         A         SSS2         A         SSS2         A         SSS2         SSS2         A         SSS2         SSS2         SSS2<	the second se							A								4			
DONADO         METEONODY         MARLINE         NORVMUK         3         9851         A         PrevAGE STREET         4751         4743         4744				MAINLINE	NORWALK	3	3849	А	LOWE STREET	40.8	40.8					4			
BAYER DOCK METERODORTH         MARLINE         WESTPORT         3         3882         A         BALLS ROAD         451         452         450         TT         4         1         56         100           CONDECT         MARLANE         METDONOTH         MARLANE         M	BOX 1							-				the second se				4			
BAYERBOOK         MATRIEOK         METCO-SUCHT         MANNEL         MERCO-SUCHT         Solar         So	BOX 1										45.12	45.09		Π	2	4	1	56	1890
BOX1       CONNOCT       METRO-ADD(T)       MANIE, NE       MUECOADD(T)       MANIE, NE       NUELONADD(T)       METRO-ADD(T)       MANIE, NE       NUELONADD(T)       METRO-ADD(T)       MANIE, NE       NUELONADD(T)       TO       4       4       4       607       GOX       GOX       GOX       MANIE       NUELONADD(T)       MANIE, NE       NUELONADD(T)       TO       4       2       77       F	2.014							A								4			
STAT         CONNOT         METRO-NORTH         MARLINE         NYH MAKEN         3         3886         A         (FOAMACT         71.2 <th71.2< th=""> <th71.2< th=""> <th71.2< th=""></th71.2<></th71.2<></th71.2<>			and the second	and the second se	and the second se			A	and the second							4			
BOX 1         CONNOT         METRO-NORTH         MANUNE         NEWHAVEN         3         SB71         A         PARE STREET         72.58         72.68         CA         4         4         44           DGX1         ATTRAK         METRO-NORTH         MANUNE         NEWHAVEN         3         SB71         A         CORWA STREET         72.58         CA         4         4         45         49           DGX1         ATTRAK         METRO-NORTH         MANUNE         NEWHAVEN         3         SB71         A         CORWA STREET         72.51         72.11         C2.11	BOX 1	CONNDOT	METRO-NORTH			-	the second se									4			1002
BOXT         AMTRAX         METRO-NORTH         MANN.NE         NEWHAVEN         3         CROWN STREET         72.63         72.63         72.63         CA         4         5         194           BOXT         6 AM         METRO-NORTH         MANN.NE         NEWHAVEN         3         3873         A         COLVER STREET         72.1         CA         4         4         150         A           CONNOOT         METRO-NORTH         MANN.NE         NEWHAVEN         3         3873         A         COLVER STREET         72.1         CA         4         4         4         150         150         150         CA         4         4         150         150         150         150         CA         4         4         150			the second se					A								4			1963
BOX         CONNOM         METRO-MORTH         MANILINE         NEW HAVEN         3         OBMAN         CONNOT         METRO-MORTH         CA         4         4         4         150           CONNOM         METRO-MORTH         MANILINE         REW HAVEN         3         3874         A         GRAND AVENUE         72.94				and the second state of th	the supervise statement of the supervise statement of the supervise statement of the supervise statement of the	3		A		72.63	72.63			CA		4	5 1	94	
CONNOCT         METRO-MORTH         MANNE         NEW HAVEN         3         9874         A         GRAND AVENUE         72.94         72.94         72.94         CA         2         3         150         1507           CORMAUL         METRO-MORTH         MALLINE         BRIOGEPORT         3         3954         H         TRANSPORTATION CNTR.         5519         65.17         25.17								A							-	4			AMTRA
BOXT CONNAL         WATERBURY BRANCH         WATERBURY BRANCH         WATERBURY BRANCH         4002         A         SOUTH LEONARD STREET         25.17         25.77         TG         1         55         1922           BOXT CONNAL         METRO-NORTH         MAINLINE         NEW HAVEN         3         4000         A         ALINGS CROSSING         68.85         68.85         68.87         16         6         4         1         80         157.0           BOXT CONNOT         MAINLINE         NEW HAVEN         3         4002         A         DEWIT STREET         71.66         16         6         4         1         77         1914           BOXT CONNOT         MANILINE         NEW HAVEN         3         4027         A         LAMBERTON STREET         71.6         71.6         17.6         1         2.6         1922         1931         1922           CVR METRO-NORTH         MAINLY BRANCH         NORTH MAINLY BRANCH         NORTHON MAINLY BRANCH         3         4422         A         104.1         102.5         1.3         1.3         1.6         1.6         1.6         1.6         1.6         1.6         1.6         1.6         1.6         1.6         1.6         1.6         1.6	BUXT			and the second se	NEW HAVEN	3	3874	-	GRAND AVENUE	72.94	72.94	72.94				2			1907 AMTRA
AMTERAK         METRO-NORTH         MANILINE         WEST HAVEN         3         4006         A         ALLINGS CROSSING         68.85         68.74         IB         4         5         107           BOXT         AMTERAK         METRO-NORTH         MANILINE         NEW HAVEN         3         4008         H         ELGARSSO BLVO.         77.185         10         0.42 DE         4         1970           BOXT         MATTRAK         METRO-NORTH         MANILINE         NEW HAVEN         3         4002         A         LAMBERTON STREET         71.66         T1.66         T1.6         4         1         77         1914           BOXT         MATTRO-NORTH         DANULY BRANCH         NORVALK         3         4046         A         BURSTREET         1.53	POX 1													TG		1	1	55	1929
BOX         ICONNODT         METRO-NORTH         MAINLINE         NEW HAVEN         3         4025         A         DEWIT STREET         71.66         71.66         71.66         TG         4         1         77         1914           GOX         MAYROOK         METRO-NORTH         DANBURY BRANCH         NORWALK         3         4046         A         BURNELSTREET         1.53         1.53         PC         1         3         276         1982           B & M         METRO-NORTH         DANBURY BRANCH         NORWALK         3         4046         A         WALSTREET         1.47         1.47         1.47         1.47         1.47         1.48         METRO-NORTH         DANBURY BRANCH         WILTON         3         4261         A         WOLFPIT ROAD WESTBOD         6.25         6.25         DT         1         1         68         1979           B & M         METRO-NORTH         MANLINE         FAIRFILD         3         4354         A         MELGO-NORTH         ADAULY BRANCH         WILTON         3         4354         A         MELGO-NORTH         ADAULY BRANCH         WILTON         3         4354         A         MELGO-NORTH         MANLINE         FAIRFILD         3         4354 </td <td>BUXT</td> <td></td> <td>METRO-NORTH</td> <td>MAINLINE</td> <td>WEST HAVEN</td> <td>3</td> <td>4006</td> <td>A</td> <td>ALLINGS CROSSING</td> <td>68.85</td> <td>68.85</td> <td>68.74</td> <td></td> <td>IB</td> <td></td> <td>4</td> <td>3 1</td> <td>81</td> <td>1970</td>	BUXT		METRO-NORTH	MAINLINE	WEST HAVEN	3	4006	A	ALLINGS CROSSING	68.85	68.85	68.74		IB		4	3 1	81	1970
DOX1         IMVBROK         METRO-MORTH         MANUNE         NEW HAVEN         3         4027         A         LAMBERTON STREET         71.6 <t< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>and the second sec</td><td>10</td><td></td><td></td><td>4</td><td></td><td></td><td>1977 CONRA</td></t<>												and the second sec	10			4			1977 CONRA
CVRR         METRO-NORTH         DANBURY BRANCH         NORWALK         3         4046         A         BURNELL STREET         1.53         1.53         PC         1         3         276         1982           B & M         METRO-NORTH         DANBURY BRANCH         NORWALK         3         4461         A         WALL STREET         1.47         1.48         1.47         1.48         1.47         1.48         1.47         1.48         1.47         1.48         1.47         1.47         1.47         1.47         1.48         1.47         1.48         1.47         1.47         1.47         1.47         1.47         1.47         1.47         1.47         1.47         1.47         1.43         1.43         1.43         1.43         1.43         1.43         1.43         1.43         1.43         1.43         1.43         1.43         1.43         1.43         1.43         1.43										71.6	71.6	71.6		TG		4	2 1	61	1962
B & M         METRO-NORTH         DANBURY BRANCH         WILTON         3         4281         A         WOLFPIT ROAD EXETEND         6.25         6.25         DT         1         1         86         1979 W           CONNOT         METRO-NORTH         DANBURY BRANCH         WILTON         3         4342         A         WOLFPIT ROAD EXETEND         6.25         6.25         DT         1         1         86         1979 W           B & M         METRO-NORTH         MAININE         FARFIELD         3         4354         A         WOLFT ROAD EXETEND         6.25         6.25         DT         1         1         86         1979 W           P & W         METRO-NORTH         MAININE         FEROL         3         4357         A         B6ACHSIDE AVENUE         46.94 46.94 46.94         94.2         18.2         13.1 PC         1         1         18.3         1981           AMTRAK         METRO-NORTH         WATERBURY BRANCH         B6CON FALLS         4         4338         H         CONN ROUTE 818         18.17         18.17         18.17         18.17         18.17         18.17         18.17         18.17         18.17         18.17         18.17         18.17         18.17         18.17		CVRR	METRO-NORTH	DANBURY BRANCH	NORWALK			A								1			1965 1946 TUNNE
CONNOCT         METRO-NORTH         DANULY BRANCH         WILTON         3         4342         A         WOLPT ROAD EASTRND         6.25         6.25         D.T         1         1         86         1979E           B0X1         CONNOCT         METRO-NORTH         MAINLINE         VAIRPELD         3         4357         A         BEACHSIDE AVENUE         46.94         46.94         46.94         DG         4         1         108         1982           B & M         METRO-NORTH         WAITERBURY BRANCH         BEACON FALLS         4         4394         H         CONN ROUT E 8         17.29         6         21.31         PC         1         1         183         1981           AMTRAK         METRO-NORTH         WATERBURY BRANCH         BEACON FALLS         4         4396         H         CONN ROUT E 817         18.17         18.37         8         21.99         8         1         3         536         1981           BOX1         AMTRAK         METRO-NORTH         WAIRING BRANCH         BEACON FALLS         4         4392         H         COURTAND AVENUE         35.22         35.21         106         0.70         4         2.97         1982         18.4         18.4         18.4 </td <td>BOX 1</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>A</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>CONTRACTOR OF A DESCRIPTION OF A DESCRIP</td> <td></td> <td>1</td> <td></td> <td>86</td> <td>1979 WESTB</td>	BOX 1							A						CONTRACTOR OF A DESCRIPTION OF A DESCRIP		1		86	1979 WESTB
B & W         METRO-NORTH         MAINLINE         WESTPORT         3         4357         A         BEACHSIDE AVENUE         46.94 <td></td> <td>CONNDOT</td> <td>METRO-NORTH</td> <td>DANBURY BRANCH</td> <td>WILTON</td> <td>3</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>1</td> <td></td> <td></td> <td>1979 EASTB</td>		CONNDOT	METRO-NORTH	DANBURY BRANCH	WILTON	3										1			1979 EASTB
B & M         METRO-NORTH         WATERBURY BRANCH         BEACON FALLS         4         4384         H         CONN ROUTE B         17.29         17.28         17.28         17.29         1         1         1         133         1981           AMTRAK         METRO-NORTH         WATERBURY BRANCH         BEACON FALLS         4         4396         H         CONN ROUTE BSB         18.2         17.28         18.4         8         21.99         B         1         3         556         1981           BOX1         AMTRAK         METRO-NORTH         WATERBURY BRANCH         BEACON FALLS         4         4397         H         CONN ROUTE BNB         18.17         18.17         18.37         18         21.99         B         4         3         536         1982           BOX1         AMTRAK         METRO-NORTH         MAININE         STAMFORD         3         4398         H         COURTLAND AVE SPUR         35.22         35.22         35.21         106         0.84 PC         1         1         29         199/80         1931           BOX1         BAY MERO-NORTH         DANBURY BRANCH         REDDING         4         522         A         SIMPAUG TURNPIKE         16.21         16.21         <	BOX 1		and the second se					A					-			4			
BOX 1.         AMTRAK         METRO-NORTH         WATERBURY BRANCH         BEACON FALLS         4         4387         H         CONN ROUTE BNB         18.17		B & M	METRO-NORTH	WATERBURY BRANCH	BEACON FALLS	4	4384		CONN ROUTE 8	17.29	17.29		8	21.31 PC		1	1 1	83	1981
BOX 1         MAINLINE         STAMFORD         3         4398         H         COURTLAND AVENUE         35.22         35.21         106         0.76         DG         4         3         213         1982           B & M         METRO-NORTH         MAINLINE         STAMFORD         3         4399         H         COURTLAND AVE SPUR         35.22         35.21         106         0.76         DG         4         3         213         1982           B & M         METRO-NORTH         DANBURY BRANCH         REDDING         4         522         AS         35.21         106         0.76         0.64         3         213         1982           BOX 1         MAYBROK         METRO-NORTH         DANBURY BRANCH         WILTON         3         5280         H         US ROUTE 7         7.87         7.87         7.87         7.87         7.89         PC         1         1         34         1982           AMTRAK         METRO-NORTH         MAINLINE         NORWALK         3         5304         A         HIGHLAND AVENUE         39.72         38.72         IB         4         1         107         1972           BOX 1         BATRAK         METRO-NORTH         MAINLINE<	POV 4							н					8			1			
BOX1         MAYBROOK         METRO-NORTH         DANBURY BRANCH         REDDING         4         5225         A         SIMPAUG TURNPIKE         16.21         1			METRO-NORTH	MAINLINE	STAMFORD	3	4398	н	COURTLAND AVENUE	35.22	35.22	35.21		0.76 DG		4	3 2	13	1982
BX1         METRO-NORTH         DANBURY BRANCH         WILTON         3         5260         H         US ROUTE 7         7.8	POX 4							H					106		_	1			1975
BOX 1         PRIVATE         METRO-NORTH         MAINLINE         NORWALK         3         5304         A         HIGH-LAND AVENUE         39,72         39,72         38,72         IB         4         1         133         1982           AMTRAK         METRO-NORTH         MAINLINE         NORWALK         3         5333         A         ML. KING JR. DRIVE         40,73 <td></td> <td></td> <td></td> <td></td> <td>WILTON</td> <td></td> <td>5260</td> <td>Н</td> <td>US ROUTE 7</td> <td>7.87</td> <td>7.87</td> <td>7.87</td> <td>7</td> <td>7.89 PC</td> <td></td> <td>1</td> <td>1</td> <td>84</td> <td></td>					WILTON		5260	Н	US ROUTE 7	7.87	7.87	7.87	7	7.89 PC		1	1	84	
BOX1         METRO-NORTH         MAINLINE         FAIRFIELD         3         5432         A         BLACK ROCK TURNPIKE         52.52         52.56         PC         4         2         114         1987         B           BOX1         AMTRAK         METRO-NORTH         MAINLINE         WESTPORT         3         5573         A         HILLS POINT ROAD         45.51         45.51         45.48         DG         4         1         60         1988           BOX1         AMTRAK         METRO-NORTH         MAINLINE         FAIRFIELD         3         5573         A         HILLS POINT ROAD         45.51         45.51         45.48         DG         4         1         60         1988           BOX1         B& M         METRO-NORTH         MAINLINE         FAIRFIELD         3         5578         A         WONDWONT ROAD         65.99         66.02         PC         4         1         138         1987         N           BOX1         AMTRAK         METRO-NORTH         MAINLINE         FAIRFIELD         3         5786         A         WONDWONT ROAD         65.99         66.02         PC         4         3         152         1989         1880         IAMTRAK         MET		PRIVATE	METRO-NORTH													4			
BOX 1         AMTRAK         METRO-NORTH         MAINLINE         WESTPORT         3         5573         A         HILLS POINT ROAD         45.51         45.51         45.48         DG         4         1         60         1988           BOX 1         BAM         METRO-NORTH         MAINLINE         FAIRFIELD         3         5646         A         GRASMERE BAVENUE         51.86         51.83         PC         4         1         63         1988           BOX 1         CONRAIL         METRO-NORTH         MAINLINE         MILFORD         3         5746         A         WOODWONT ROAD         65.99         66.02         PC         4         1         63         1988           BOX 1         CONRAIL         METRO-NORTH         MAINLINE         FAIRFIELD         3         5746         A         WOODWONT ROAD         65.99         66.02         PC         4         1         138         1987         M           BOX 1         AMTRAK         METRO-NORTH         MAINLINE         FAIRFIELD         3         5796         A         UNQUOWA ROAD         50.61         50.61         126.14 PC         4         3         152         1989           BOX 1         AMTRAK <td< td=""><td>BOX 1</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>4</td><td></td><td></td><td>1972 1987 BLACK</td></td<>	BOX 1															4			1972 1987 BLACK
BOX1         CONRAIL         METRO-NORTH         MAINLINE         MILFORD         3         5746         A         WOODMONT ROAD         65.99         66.02         DG         4         1         138         1987         N           BOX1         CONRAIL         METRO-NORTH         MAINLINE         FAIRFIELD         3         5788         A         UNQUOWA ROAD         50.61         50.62         PC         4         3         231         1989           BOX1         AMTRAK         METRO-NORTH         MAINLINE         FAIRFIELD         3         5788         A         UNQUOWA ROAD         50.61         50.62         PC         4         3         231         1989           BOX1         AMTRAK         METRO-NORTH         MAINLINE         FAIRFIELD         3         5795         A         SELECK STREET         32.36         32.37         DG         4         1         101         1919/89           BOX1         B & M         METRO-NORTH         MAINLINE         GREENWICH         3         5808         A         RLVERSIDE AVENUE         30.25         30.25         30.25         30.26         DG         4         1         101         1919/89           BOX1         B & M	BOX 1	AMTRAK	METRO-NORTH	MAINLINE	WESTPORT	3	5573	A	HILLS POINT ROAD	45.51	45.51	45.48			_	4	1	50 ·	1988
BOX 1         AMTRAK         METRO-NORTH         MAINLINE         FAIRFIELD         3         5788         A         UNQUOWA ROAD         50.61         50.62         PC         4         3         231         1989           BOX 1         AMTRAK         METRO-NORTH         MAINLINE         FAIRFIELD         3         5794         H         KINGS HGHWAY US 1         51.63         51.6         1         26.14 PC         4         3         152         1989           BOX 1         MATRAK         METRO-NORTH         MAINLINE         FAIRFIELD         3         5794         H         KINGS HGHWAY US 1         51.63         51.6         1         26.14 PC         4         3         152         1989           BOX 1         B & M         METRO-NORTH         MAINLINE         STAMFORD         3         5795         A         SELLECK STREET         32.36         32.37         DG         4         1         110         1919/89           BOX 1         B & M         METRO-NORTH         MAINLINE         GREENWICH         3         5808         A         RIVERSIDE AVENUE         30.236         32.36         23.4         1         37.46         4         3         171         1989				and the second distance of the second distanc		_		A								4			1988 1987 WOOD
BOX1         MAINUNA         METRO-NORTH         MAINUNE         STAMFORD         3         5795         A         SELLECK STREET         32.36         32.37         DG         4         1         10         1919/89           BOX1         B & M         METRO-NORTH         MAINLINE         GREENWICH         3         5808         A         RIVERSIDE AVENUE         30.25         30.25         30.26         DG         4         1         10         1919/89           BOX1         B & M         METRO-NORTH         MAINLINE         GREENWICH         3         5808         A         RIVERSIDE AVENUE         30.25         30.26         DG         4         3         171         1989           BOX1         MAYBROOK         METRO-NORTH         MAINLINE         MILFORD         3         5809         H         BOSTON POST ROADUS 1         62.28         62.34         1         37.6         C         4         3         138         1989           BOX1         MAYBROOK         METRO-NORTH         WATERBURY BRANCH         BEACON FALLS         4         6018         A         LOPONS ROAD         18.02         18.02         18.02         18.02         18.02         18.03         12.03         20.63	BOX 1	AMTRAK	METRO-NORTH	MAINLINE	FAIRFIELD	3	5788	A	UNQUOWA ROAD	50.61	50.61	50.62		PC	_	4	3 2	31 *	1989
BOX1         B & M         METRO-NORTH         MAINLINE         GREENWICH         3         5808         A         RIVERSIDE AVENUE         30.25         30.26         DG         4         3         171         1989           BOX1         MAYBROOK         METRO-NORTH         MAINLINE         MILFORD         3         5809         H         BOSTON POST ROAD US 1         62.28         62.34         1         37.46         PC         4         3         183         1988           BOX1         P& W         METRO-NORTH         WATERBURY BRANCH         BEACON FALLS         4         6018         A         LOPUS ROAD         18.02         18.02         18.12         PC         1         1         63         1991           BOX1         CONRAIL         METRO-NORTH         MAINLINE         FAIRFIELD         3         6211         H         BOSTON POST ROAD US 1         49.35         49.34         1         23.63         DG         4         2         288         1992								H					1			4			1989
BOX 1         MAYBROOK         METRO-NORTH         MAINLINE         MILFORD         3         5809         H         BOSTON POST ROAD US 1         62.28         62.34         1         37.46         PC         4         3         183         1988           BOX 1         P&W         METRO-NORTH         WATERBURY BRANCH         BEACON FALLS         4         6018         A         LOPUS ROAD         18.02         18.12         PC         1         1         63         1991           BOX 1         CONRAIL         METRO-NORTH         MAINLINE         FAIRFIELD         3         6211         H         BOSTON POST ROAD US 1         49.35         49.34         1         23.63         DG         4         2         288         1992	BOX 1	B & M	METRO-NORTH	MAINLINE	GREENWICH	3	5808	A	RIVERSIDE AVENUE	30.25	30.25	30.26		DG		4	3 1	71 *	
BOX 1 CONRAIL METRO-NORTH MAINLINE FAIRFIELD 3 6211 H BOSTON POST ROAD US 1 49.35 49.35 49.34 1 23.63 DG 4 2 288 1992	BOX 1	MAYBROOK					the second s	H					1			4			
				the second se				Н		49.35	49.35	49.34	1	23.63 DG		4	2 2	38	1992
				MAINLINE	DARIEN	3	00316R	U	BOSTON POST ROAD US 1	37.82	37.82	37.8	1	11.57 TG	OD	4	3	30 1	1894 300-003

-VAILABI	611	rc g	Are Dire ni	1200
REMARKS	PLAN #	PLAN TYPE	COMMENTS	LOCATIO
0.0000				
00-003				<u> </u>
	1			
00-003		FLAT, ROLL		
	1			
	-			
JRNING ROADWAY	0.000			
AST WYE TRACK				
	11			
		ROLL		
	-			-
ATRAK (SPRG) 0.56				
ITRAK (SPRG) 0.68				
		-		
ONRAIL (42CC) 0.93		DOLL		
	<u> </u>	ROLL		
NNEL				
ESTBOUND				
STBOUND				
				1
ACK ROCK TPK.				
			5	
DODMONT RD.				
		1		
	17			

SHADING=BRIDGES IN MICROFILM 1064

#### LIST OF RAILROAD BRIDGES SORTED BY STATE BRIDGE NUMBER

1								18-04-11004-110-110-110-1		3 SONTED DI STATE							
BÓX # OWNER	RAILROAD	TRACK CHART NAME	and the second s	DT	ST. BRID. NO.	TYPE	LOCATION	TRK MI	RR. NO.	OLD NO. RTE	RTE. MI	BR TYP	DECK TK	SP	LGN	YR BLT REMARKS PLAN # P	PLAN TYPE COMMENTS
	METRO-NORTH	NEW CANAAN	NEW CANAAN	3	00710R	U	MERRITT PARKWAY	5.76	5.76	5.76	15 13.32		CS	1 2	64		LAT, ROLL
	METRO-NORTH	DANBURY BRANCH	BETHEL	4	01020R	U	GRASSY PLAIN ROAD	19.64	19.64		53 19.02		OD	1 1	39	1909 300-003	
	METRO-NORTH	WATERBURY BRANCH	and the second	4	01063R	U	MAIN STREET	14.61	14.61	14.54	67 26.74	TG 0	OD	1 2	60	1906 300-003 F	LAT, ROLL
	METRO-NORTH	NEW CANAAN	NEW CANAAN	3	01302R	U	OLD STAMFORD ROAD	6.16	6.16		06 4.91	TG 0	OD	1 1	52	1906 300-003	
	METRO-NORTH	MAINLINE	STRATFORD	3	01312R	U	EAST MAIN STREET	59,96	59.96		10 0.2	TG (	OD	5 1	44	1905 300-003 37 R	ROLL
and the second s	METRO-NORTH	MAINLINE	STRATFORD	3	01318R	U	MAIN STREET	59.01	59.01	59.04 1	13 6.13	TG 0	OD	4 3	69	1893 300-003	
	METRO-NORTH	WATERBURY BRANCH		4	01321R ·	U	MAIN STREET	14.29	14.29	14.23 1	15 5.34	TG (	OD	1 2	64	1906 300-003 43	
a warmen and a second se	METRO-NORTH	MAINLINE	FAIRFIELD	3	01344R	U	NORTH BENSON ROAD	51.12	51.12	51.09 1	35 0.05	TG 0	CS	4 1	33	1940 050-199	
BONL CONNDOT	METRO-NORTH	MAINLINE	WESTPORT	3	01348R	U	SAUGATUCK AVENUE	43.97	43.97	43.97 1	36 8.16	TG		4 1	42	913/40	
BOX 1 CONNDOT	METRO-NORTH	MAINLINE	WEST HAVEN	3	01403R	U	SAW MILL ROAD	69.66	69.66	69.66 1	62 8.46	TG 0	CS	4 1	61	1962	
BOX 1 CONNDOT	METRO-NORTH	MAINLINE	STAMFORD	3	02237R	U	MAIN STREET US 1	34.17	34.17	34.16	1 8.02	TG (	OD	5 3	105	1896	
BOX 1 CONNDOT	METRO-NORTH	MAINLINE	BRIDGEPORT	3	03635R	U	FAIRFIELD AVENUE	53.42	53.42	53.43		TG (	OD	4 3	102	1895 300-003 25	
BOX 1 CONNDOT	METRO-NORTH	MAINLINE	BRIDGEPORT	3	03636R	U	STRATFORD AVENUE	55.61	55.61	55.56		TG		4 4	165	1903 015-147	
BOX 1 CONNDOT	METRO-NORTH	MAINLINE	BRIDGEPORT	3	03638R	U	PARK AVENUE	54.58	54.58	54.55		TG (	OD	4 3	98	1900 28 F	LAT with 03680R, 03638R, and 08056R
CONNDOT	METRO-NORTH	MAINLINE	BRIDGEPORT	3	03639R	U	EAST MAIN STREET	56.2	56.2	56.19		TG (	OD	4 3	68	1902 54	
BOX 1 CONNDOT	METRO-NORTH	MAINLINE	MILFORD	3	03640R	U	HIGH STREET	63.27	63.27	63.31			OD	4 3	57	1894 300-003 170-1509 38 FI	ΙΔΤ
BOX 1 CONNDOT	METRO-NORTH	MAINLINE	MILFORD	3	03644R	U	RIVER STREET	63.44	63,44	63.45		TG (	OD	4 3	80	1894 170-1375 300-003 39 FI	
BOX 1 CONNDOT	METRO-NORTH	MAINLINE	GREENWICH	3	03673R	U	ARCH STREET	28.06	28.06	28.07		DG 0	OD	4 1	55		LAT. ROLL
BOX 1 CONNDOT	METRO-NORTH	MAINLINE	STAMFORD	3	03678R	U	CANAL STREET	33.41	33.41	33.4			OD	7 1	65		LAT with 03680R, 03638R, and 08056R
BOX 1 CONNDOT	METRO-NORTH	MAINLINE	STAMFORD	3	03680R	U	GREENWICH AVENUE	32.81	32.81	32.8		TG C	OD	4 3	70	1896 49 FI	
BOX 1 CONNDOT	METRO-NORTH	MAINLINE	STAMFORD	3	03683R	U	WASHINGTON BLVD.	32.97	32.97	32.97		TG C	OD	6 4	100		LAT No. 1 Flat, No. 2 Flat
BOX 1 CONNDOT	METRO-NORTH	MAINLINE	STAMFORD	3	03686R	U	ELM STREET	33.75	33.75	33.76	The state of the local division of the local		DD	8 3	61	1896	EAT NO. 1 Flat, NO. 2 Flat
BOX 1 CONNDOT	METRO-NORTH	MAINLINE	NORWALK	3	03691R	U	EAST AVENUE	42.14	42.14	42.14		TG C	DD	4 1	41	1905 170-1375 300-003 20 R	011
BOX 1 CONNDOT	METRO-NORTH	MAINLINE	NORWALK	3	03692R	U	STRAWBERRY HILL AVE.	42.35	42.35	42.37		TG I	P	4 1	33	1940	
BOX 1 CONNDOT	METRO-NORTH	MAINLINE	NORWALK	3	03693R	U	WASHINGTON/MAIN STS	41.28	41.28	41.28	1 1			4 1	146		LAT
the second se	METRO-NORTH	WATERBURY BRANCH		4	03723R	U	BRISTOL STREET # 1	24.1	24.1	24.3	1-1			1 1	44	1918 46	
	METRO-NORTH	MAINLINE	GREENWICH	3	03943R	U	NORTH WATER STREET	26.1	26.1	26.12				4 1	44		LAT, ROLL
	METRO-NORTH	MAINLINE	GREENWICH	3	03945R	U	HAMILTON AVENUE	26.79	26.79	26.8				4 1	27	1893 170-1375 11 FL	
and a second to a second secon	METRO-NORTH	MAINLINE	GREENWICH	3	03946R	U	STEAMBOAT ROAD	28.22	28.22	28.22				4 1	56	1895 GREENWICH AVENUE 300-003	
	METRO-NORTH	MAINLINE	GREENWICH	3	03947R	Ū	FIELD POINT ROAD	27.77	27.77	27.78		SA		4 4	40	1895 GREENWICH AVENUE 300-003	
	METRO-NORTH	MAINLINE	GREENWICH	3	03948R	U	SOUND BEACH AVENUE	31.29	31.29	31.29			CS	4 2	56	1929	
	METRO-NORTH	MAINLINE	GREENWICH	3	03955R	Ū	TOMAC AVENUE	31.61	31.61	31.62				4 4	20		
	METRO-NORTH	MAINLINE	WESTPORT	3	03963R	tu	COMPO ROAD	44.7	44.7	44.67	++	TG C		4	33	1895 300-003 14, 48 1941	
the second	METRO-NORTH	MAINLINE	NORWALK	3	04131Ŕ	tu -	FORT POINT STREET	-41.78	44.7	41.82	-		cs		40		
	METRO-NORTH	MAINLINE	NORWALK	3	04131R 04132R	ŭ	ROWAYTON AVENUE # 1	39.12	39.12	39.11			DD	4 1	3/	1941 300-003	
and the second se	METRO-NORTH	DANBURY BRANCH	NORWALK	3	04134R		MARSHALL STREET	0.11	0.11	0.11				4 1	30	1893 19	
BOX 1 CONNDOT		MAINLINE	DARIEN	3	04142R		LEROY AVENUE # 1	37.59	37.59	37.59					49	1895	
BOX 1 CONNDOT	+	MAINLINE	FAIRFIELD	3	04197R		MILL PLAIN ROAD							4 1	29	1894 16	
the second se	METRO-NORTH	MAINLINE	FAIRFIELD	3	04198R		ROUND HILL ROAD	50.29	50.29	50.3				4 1	39	1941	
BOX 1 CONNDOT		MAINLINE	FAIRFIELD	_	and the second se	0		50.9	50.9	50.88			S	4 1	33	1940 24	
				3	04200R	0	CENTER STREET	48.81	48.81	48.83	_		CS	4 1	33	1940	
	METRO-NORTH	WATERBURY BRANCH	NAUGATUCK	4	04224R	U	MAPLE STREET	21.74	21.74	21.73			DD	1 1	66	1913	
BOX 1 CONNDOT		WATERBURY BRANCH		4	04232R	U	WASHINGTON AVENUE # 1	26.18	26.18	26.38	_			1 1	63	1926	
	METRO-NORTH	WATERBURY BRANCH	WATERBURY	4	04235R	U	BANK'S STREET # 1	26.35	26.35	26.63		and the second se		2 1	78		
	METRO-NORTH	MAINLINE	DARIEN	3	04271R	U	RAYMOND STREET	38.93	38.93	38.93		DG		4 1	27	1894 300-003	
watching and an owner of the second	METRO-NORTH	MAINLINE	NORWALK	3	04288R	M	NORWALK RIVER	41.51	41.51	41.47		DT		4 4	462	1896 WALK MOV BR-SWING 58 FL	AT with 08080R, 08008R, and 08034R
	METRO-NORTH	MAINLINE	GREENWICH	3	08000R	U	STATION UNDERPASS	26.79	26.79	26.79		CP		4 1	8	1936	
	METRO-NORTH	MAINLINE	GREENWICH	3	08001R	U	CULVERT AT FIELD PT RD	27.77	27.77	27.78		SA		4 1	12	1895	
	METRO-NORTH	MAINLINE	GREENWICH	3	08002R	Z	PEDESTRIAN FOOTBRDG	28.16	28.16	28.16		TG		4 1	54	1904 12	
BOX 2 CONNDOT		MAINLINE	GREENWICH	3	08003R	U	DAVIS AVENUE # 2	28.48	28.48	28.49			DO	4 1	34	1894	
the second distance of	METRO-NORTH	MAINLINE	GREENWICH	3	08004R	U	INDIAN HARBOR	28.68	28.68	28.68		DG C	DO	4 1	64	1895 300-003	
	METRO-NORTH	MAINLINE	GREENWICH	3	08005R	U	SACHEM ROAD	29.48	29.48	29.5		DG O	DO	6 1	28	1893	
	METRO-NORTH	MAINLINE	GREENWICH	3	08006R	U	LUKES CROSSING	29.68	29.68	29.69		DG		4 1	36		
the second	METRO-NORTH	MAINLINE	GREENWICH	3	08007R	S	COAL SIDING TRESTLE	29.8	29.8	29.8		TI		1 30	360	ABANDONED	
BOX 2 CONNDOT		MAINLINE	GREENWICH	3	08008R	M	MIANUS RIVER	29.9	29.9	29.91	1	DG		4 15	1089	1904 COB MOV BR BASCULE 13, 47, 58 FL	AT. ROLL with 08080R 04288R and 08033R
	METRO-NORTH	MAINLINE	GREENWICH	3	08009R	U	ARCH STREET # 2	31.03	31.04	31.04	5	SA		4 1	14	1895 300-003 FL	AT
	METRO-NORTH	MAINLINE	GREENWICH	3	08010R	U	STREAM	31.12	31.12	31.13	5	SA		4 1	10		
	METRO-NORTH	MAINLINE	STAMFORD	3	08011R	U	RIPPAWAM RIVER	32.84	32.84	32.85	l l	DT		4 1	120	1896	
BOX 2 CONNDOT		MAINLINE	STAMFORD	3	08012R	U	ATLANTIC STREET	33.19	33.19	33.18	1	TG O	D	5 2	75	1896 15 FL	AT
BOX 2 CONNDOT		MAINLINE	STAMFORD	- 3	08013R	U	HAMILTON AVENUE	34.72	34.72	34.71	1	TG O	D	5 2	51	1896	
BOX 2 CONNDOT		MAINLINE	STAMFORD	3	08014R	U	NOROTON RIVER	35.58	35.58	35.58	E	BA		4 1	27	1894	AT 1000000000000000000000000000000000000
	METRO-NORTH	MAINLINE	DARIEN	3	08015	Z	PEDESTRIAN BRIDGE	36.24	36.24	36.24		TG		4 1	72	1972 NOROTON HEIGHTS	
BOX 2 CONNDOT		MAINLINE	DARIEN	3	08016R	U	STONY BROOK	37.16	37.16	37.16	1	SA		4 1	10	1893	
	METRO-NORTH	MAINLINE	DARIEN	3	08017R	U	CUMMINGS BROOK	37.36	37.36	37.36		SA		4 1	6	1893	
	METRO-NORTH	MAINLINE	DARIEN	3	08018R	U	GOOD WIVES RIVER	38	38	37.98		ВС	S	4 1	19	1946	
	METRO-NORTH	MAINLINE	DARIEN	3	08019R	U	STREAM	38.36	38.36			SB		4 1	3	2.5' X 3' BOX	
	METRO-NORTH	MAINLINE	DARIEN	3	08020R	U	STREAM	38.53	38.53		5	SB		4 2	5	TWIN 2.5' X 3' BOX	
	METRO-NORTH	MAINLINE	DARIEN	3	08021R	U	STREAM	38.68	38.68	38.68		SA		4 1	6	1893	
BOX 2 CONNDOT		MAINLINE	DARIEN	3	08022R	U	FIVE MILE RIVER	39.07	39.07	39.06	5	SA		4 1	26	1893 300-003 18	
	METRO-NORTH	MAINLINE	NORWALK	3	08023R	U	DRY STREAM	39.08	39.08	39.08		SA		4 1	8	1893	
	METRO-NORTH	MAINLINE	NORWALK	3	08024R	U	FARM CREEK	39.55	39.55	39.55	5	SB		4 1	1	1' X 3' BOX	
	METRO-NORTH	MAINLINE	NORWALK	3	08025R	U	SPRING STREET # 2	40.89	40.89	40.93	Т	rg o	D	4 1	47	1896 300-003	
	METRO-NORTH	MAINLINE	NORWALK	3	08026R	U	STATION UNDERPASS	41.02	41.02	41.02		BA		4 1	10	1895	
BOX 2 CONNDOT	METRO-NORTH	MAINLINE	NORWALK	3	08027R	U	MONROE STREET	41.12	41.12	41.1			D	6 3	85	1895 170-1509 SUMMER 96 300-003	
BOX 2 CONNDOT	METRO-NORTH	MAINLINE	NORWALK	3	08028R	U	OSBORN AVENUE	41.96	41.96	41.99		rG		4 1	39	1894 300-003	
BOX 2 CONNDOT	METRO-NORTH	MAINLINE	NORWALK	3	08029R	U	CULVERT	42.26	42.26	42.26	E	3A		4 1	6		
CONNDOT	METRO-NORTH	MAINLINE	WESTPORT	3	08030R	U	SAUGATUCK AVENUE SIDETI	43.97	43.97	43.97	1	rg lo	D	5 1	42	1913	
BOX 2 CONNDOT	METRO-NORTH	MAINLINE	WESTPORT	3	08031R	U	STATION UNDERPASS	44.2	44.2	44.2		BA		4 1	10	1906	
				5	a stand and a stand												No. 1 Flat, No. 2 Flat, with 08080R,
	METRO-NORTH	MAINLINE	WESTPORT	3	08032R	M	SAUGATUCK RIVER	44.32	44.32	44.3		G		4 6	458	1904 SAGA MOV BR- BASCULE 58 FLA	
BOX 2 CONNDOT		MAINLINE	WESTPORT	3	08033R	U	SHERWOOD MILLPOND	45.75	45.75	45.75		CP		4 1	6	1951	
BOX 2 CONNDOT			WESTPORT	3	08034R	U	MUDDY BROOK	46.11	46.11	46.11	1	CP		4 1	8	1951	
BOX 2 CONNDOT		MAINLINE			08035R	U	NEW CREEK ROAD	47.15	47.15	47.16		B OI	D	4 1	33	1934 300-003 21	
BOX 2 CONNDOT BOX 2 CONNDOT	METRO-NORTH	MAINLINE MAINLINE	WESTPORT	3	000358		and the second	47.29	47.29	47.29		SA		4 1		21 Z1	
BOX 2 CONNDOT BOX 2 CONNDOT BOX 2 CONNDOT	METRO-NORTH			3	08036R	U	GREENS FARMS BROOK						<ul> <li>An el recent de contracter de la contracter de la contracter de la contracter</li></ul>		61		T with 09014D 00000D
BOX 2 CONNDOT BOX 2 CONNDOT BOX 2 CONNDOT CONNDOT	METRO-NORTH METRO-NORTH METRO-NORTH	MAINLINE	WESTPORT		and the second se	U U	GREENS FARMS BROOK MAPLE LANE	47.44	47.44	47.44	11	B CS	S	4 1	6 		AT with 08014R, 08086R, and 08202R
BOX 2 CONNDOT BOX 2 CONNDOT BOX 2 CONNDOT CONNDOT BOX 2 CONNDOT	METRO-NORTH METRO-NORTH METRO-NORTH	MAINLINE MAINLINE	WESTPORT WESTPORT	3	08036R	บ บ บ				47.44	31			4 1	40 42	1941 22	AT with 08014R, 08086R, and 08202R
BOX 2 CONNDOT BOX 2 CONNDOT BOX 2 CONNDOT CONNDOT BOX 2 CONNDOT BOX 2 CONNDOT	METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH	MAINLINE MAINLINE MAINLINE	WESTPORT WESTPORT WESTPORT	3	08036R 08037R	U U U U	MAPLE LANE	47.44 48.29	47.44 48.29	48.36	E	B CS	S	4 1 4 1	42	1941 22 1940 2	AT with 08014R, 08086R, and 08202R
BOX 2 CONNDOT BOX 2 CONNDOT BOX 2 CONNDOT CONNDOT BOX 2 CONNDOT BOX 2 CONNDOT BOX 2 CONNDOT BOX 2 CONNDOT	METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH	MAINLINE MAINLINE MAINLINE MAINLINE	WESTPORT WESTPORT WESTPORT WESTPORT	3 3 3	08036R 08037R 08038R 08039R	U U U U U	MAPLE LANE SASCO RIVER WESTWAY ROAD	47.44 48.29 48.65	47.44 48.29 48.65	48.36 48.64		B CS	S			1941 22	AT with 08014R, 08086R, and 08202R
BOX 2 CONNDOT BOX 2 CONNDOT BOX 2 CONNDOT CONNDOT BOX 2 CONNDOT BOX 2 CONNDOT BOX 2 CONNDOT CONNDOT	METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH	MAINLINE MAINLINE MAINLINE MAINLINE MAINLINE	WESTPORT WESTPORT WESTPORT FAIRFIELD FAIRFIELD	3 3 3 3 3 3	08036R 08037R 08038R 08039R 08039R 08040R		MAPLE LANE SASCO RIVER WESTWAY ROAD CULVERT	47.44 48.29 48.65 48.8	47.44 48.29 48.65 48.8	48.36 48.64 48.8	S	B CS G CS SA	S S	4 1 4 1 4 1 4 1 4 1	42 49 5	1941         22           1940	AT with 08014R, 08086R, and 08202R
BOX 2 CONNDOT BOX 2 CONNDOT BOX 2 CONNDOT CONNDOT BOX 2 CONNDOT BOX 2 CONNDOT BOX 2 CONNDOT CONNDOT BOX 2 CONNDOT	METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH	MAINLINE MAINLINE MAINLINE MAINLINE MAINLINE MAINLINE MAINLINE	WESTPORT WESTPORT WESTPORT FAIRFIELD FAIRFIELD FAIRFIELD	3 3 3 3 3 3 3	08036R 08037R 08038R 08039R 08040R 08041R		MAPLE LANE SASCO RIVER WESTWAY ROAD CULVERT SPRUCE STREET	47.44 48.29 48.65 48.8 48.91	47.44 48.29 48.65 48.8 48.91	48.36 48.64 48.8 48.89	S T	B CS TG CS SA CS TG CS	S S	4 1 4 1 4 1 4 1 4 1 4 1 4 1	42 49 5 33	1941 22 1940 1941	AT with 08014R, 08086R, and 08202R
BOX 2 CONNDOT BOX 2 CONNDOT BOX 2 CONNDOT CONNDOT BOX 2 CONNDOT BOX 2 CONNDOT BOX 2 CONNDOT CONNDOT BOX 2 CONNDOT BOX 2 CONNDOT BOX 2 CONNDOT	METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH	MAINLINE MAINLINE MAINLINE MAINLINE MAINLINE MAINLINE MAINLINE	WESTPORT WESTPORT WESTPORT FAIRFIELD FAIRFIELD FAIRFIELD FAIRFIELD	3 3 3 3 3 3 3 3 3	08036R 08037R 08038R 08039R 08040R 08041R 08041R		MAPLE LANE SASCO RIVER WESTWAY ROAD CULVERT SPRUCE STREET OLD POST ROAD	47.44 48.29 48.65 48.8 48.91 49.01	47.44 48.29 48.65 48.8 48.91 49.01	48.36 48.64 48.8 48.89 49.08	S T T	B CS TG CS SA G TG CS TG CS	S S	4 1 4 1 4 1 4 1 4 1 4 1 4 1	42 49 5 33 33	1941         22           1940         1941           1941         1941           1940         1940	AT with 08014R, 08086R, and 08202R
BOX 2 CONNDOT BOX 2 CONNDOT BOX 2 CONNDOT CONNDOT BOX 2 CONNDOT BOX 2 CONNDOT BOX 2 CONNDOT CONNDOT BOX 2 CONNDOT	METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH	MAINLINE MAINLINE MAINLINE MAINLINE MAINLINE MAINLINE MAINLINE	WESTPORT WESTPORT WESTPORT FAIRFIELD FAIRFIELD FAIRFIELD	3 3 3 3 3 3 3	08036R 08037R 08038R 08039R 08040R 08041R		MAPLE LANE SASCO RIVER WESTWAY ROAD CULVERT SPRUCE STREET	47.44 48.29 48.65 48.8 48.91	47.44 48.29 48.65 48.8 48.91	48.36 48.64 48.8 48.89	S T T	B CS TG CS SA CS TG CS	S S S S	4         1           4         1           4         1           4         1           4         1           4         1           4         1           4         1           4         1           4         1           4         1           4         1           4         1           4         1           4         1           4         1           4         1	42 49 5 33	1941 22 1940 1941	AT with 08014R, 08086R, and 08202R

SHADING=BRIDGES IN MICROFILM 2044

# OWNER	RAILROAD	TRACK CHART NA	ME TOWN	DT	ST. BRID. NO.	TYPE	LOCATION	TRK MI	RR. NO	OLD NO.	RTE	RTE. MI BR TYP	DECK					AN # 101 AN TO 105	
	METRO-NORTH	MAINLINE	FAIRFIELD	3		U	CULVERT	52.29					DECK	TK SP	LGN 2		MARKS PL	AN # PLAN TYPE	COMMENTS
	METRO-NORTH	MAINLINE	FAIRFIELD	3	the second se	UI II	ASH CREEK	53		53 53.03		30		4	1 2	2' X 3' BOX			
	METRO-NORTH	MAINLINE	BRIDGEPORT			- U	the second se		-				CS	4	1 37	1940			
	and the second se			3	and the second se	U	BOSTWICK AVENUE	53.6					OD	4	3 66	1900		26	
	METRO-NORTH	MAINLINE	BRIDGEPORT	3	the second se	U	HANCOCK AVENUE	53.72					OD	4	3 66	1900 300-003			
	METRO-NORTH	MAINLINE	BRIDGEPORT	3		U	HOWARD AVENUE	53.84				DG	OD	4	3 64	1900 300-003		51	
	METRO-NORTH	MAINLINE	BRIDGEPORT	3	08051R	. U	WORDIN AVENUE	54.07	7. 54.0	54.06		DG	OD	4	3 77	1900 300-003			
CONNDOT	METRO-NORTH	MAINLINE	BRIDGEPORT	3	08052R	U	IRANISTAN AVENUE	54.23	3 54.2	23 54.22		DG	OD	4	3 74	1900 300-003	27	52	
CONNDOT	METRO-NORTH	MAINLINE	BRIDGEPORT	3	08053R	U	SOUTH AVENUE	54.44		the second se			OD	4	3 85	1900			
	METRO-NORTH	MAINLINE	BRIDGEPORT	3		10	MYRTLE STREET	54.7					OD	4	3 61			53	
	METRO-NORTH	MAINLINE	BRIDGEPORT	3		- IU	WARREN STREET	54.77						4	0 01	1900			
			BRIDGEFORT		0000011		WARNEN STREET	54.77	1 34.1	54.70		DG	OD	4	3 5/	1900		29	
CONNDOT	METRO-NORTH		PRIDOFRODT		000505						1	2.0							with 08077R and 08070R, with
	and the second se	MAINLINE	BRIDGEPORT	3	08056R	U	LAFAYETTE STREET	54.84					OD	4	3 58	1900 300-003		30 FLAT	03680R, 03678R, and 03638R
	METRO-NORTH	MAINLINE	BRIDGEPORT	3		U	BROAD STREET	54.93				DG	OD	4	3 61	1900 300-003			
	METRO-NORTH	MAINLINE	BRIDGEPORT	3		U	MAIN STREET # 1	54,98	3 54.9	54.98		TG	OD	4	3 68	300-003		ROLL	
CONNDOT	METRO-NORTH	MAINLINE	BRIDGEPORT	3	08059R	U	HOUSATONIC CROSSING	55.13	3 55.1	3 55.15		IB	CS	4	1 17	1903 ABANDONE	DRR	31	
CONNDOT	METRO-NORTH	MAINLINE	BRIDGEPORT	3	08060R	U	UNION STREET	55.4	4 55.	.4 55.36		TG	OD	4	1 36	1903 300-003			
CONNDOT	METRO-NORTH	MAINLINE	BRIDGEPORT	3	08061R	U	STATION VIADUCT	55.41	1 55.4				cs	4 2	4 690	1974 015-147			
CONNDOT	METRO-NORTH	MAINLINE	BRIDGEPORT	3		U	VIADUCT	55.51				DG		4	8 320	1904 015-147		00 51 17	
CONNDOT	METRO-NORTH	MAINLINE	BRIDGEPORT	3		e lu	STATION VIADUCT	55.69				DG						33 FLAT	
	METRO-NORTH	MAINLINE	BRIDGEPORT	and the second se	the second se		The second se							4	8 320	1904 015-147		FLAT	
the state of the s	and the second se			3	and a second	IVI	PEQUONOCK RIVER	55.9				TG		4	5 359	1902 015-147		FLAT	
	METRO-NORTH	MAINLINE	BRIDGEPORT	3	and the second se	U	VIADUCT	55.91				DG		4	5 225	1902 015-147			
	METRO-NORTH	MAINLINE	BRIDGEPORT	3		U	PULASKI STREET	55.95				TG		4	3 67	1902 015-147			
the second se	METRO-NORTH	MAINLINE	BRIDGEPORT	3	08067R	U	VIADUCT	55.97	55.9	55.94		DG		4	4 176	1902 015-147			
CONNDOT	METRO-NORTH	MAINLINE	BRIDGEPORT	3	08068R	U	NOBLE AVENUE	55.98				TG		4	9 203	1902 015-147			
CONNDOT	METRO-NORTH	MAINLINE	BRIDGEPORT	3	and the second sec	U	KOSSUTH STREET	56.1					OD	4	3 58	1902 015-147			
	METRO-NORTH	MAINLINE	BRIDGEPORT	3		U	PEMBROKE STREET	56.35					OD D		3 60			045117	10 000775 1115 115 1
	METRO-NORTH	MAINLINE	BRIDGEPORT	3		<u>l</u> u	HALLETT STREET	56.46						4		1902		34 FLAT, ROLL	with 08077R AND 08056R
	METRO-NORTH	the state of the s	BRIDGEPORT			11							OD	4	3 57	1902			
and the second sec		MAINLINE		3	and the second se	0	YELLOW MILL POND	56.68				SA		4	1 16	1902			
	METRO-NORTH	MAINLINE	BRIDGEPORT	3		U	YELLOW MILL POND SIDNG					TI		1 1	0 120	1902 SIDING TRE	STLE		
	METRO-NORTH	MAINLINE	BRIDGEPORT	3		U	SEAVIEW AVENUE	56.77				TG	OD	5	3 53	1902		55	
	METRO-NORTH	MAINLINE	BRIDGEPORT	3	* 08075R	U	BISHOP AVENUE	57.46	57.4	6 57.44		TG	OD	6	3 58	1903		35	
CONNDOT	METRO-NORTH	MAINLINE	BRIDGEPORT	3	08076R	U	BRUCE BROOK	57.54				SA		6	1 8	1905			
CONNDOT I	METRO-NORTH	MAINLINE	STRATFORD	3		U	BRUCE AVENUE	57.62					OD	6	1 34	1906		ELAT.	with 080700 + 000500
	METRO-NORTH	MAINLINE	STRATFORD	3		du -	WEST BROAD STREET	58.72			and the second second	the second se	OD		1 00	the second se		FLAT	with 08070R and 08056R
	METRO-NORTH	MAINLINE	STRATFORD	3	08079R	ŭ					-	the second se	-	4	36	1893	36,	56	
50111001		DOCTIVE INC.	GIVATEORD		UOUT SR /		KING STREET	58.88	58.8	8 58.92		TG	OD	4	1 35	1893 300-003			
															1 1				
										1 1					1 1				with 08008R, 04288R, and 08032R,
and the second se	METRO-NORTH	MAINLINE	STRATFORD	3	08080R	M	HOUSATONIC RIVER	60.42	60.4	2 60.44				4	7 1052	1905 DEVON MOV	BR-BASCULE	58 FLAT, ROLL	Devon Bridge Project 92-3641990
CONNDOT I	METRO-NORTH	MAINLINE	MILFORD	3	08081R	U	BEAVER CREEK	61.62	61.6	2 61.78		SA		4	1 8	1893			Seren Singer reject of thees
CONNDOT I	METRO-NORTH	MAINLINE	MILFORD	3	08082R	U	BEARDSLEY AVENUE	62.94	62.9	4 62.98		DG	OD D	4	1 34	1893			
	METRO-NORTH	MAINLINE	MILFORD	3	the state of the s	U	WEPAWAUG RIVER	63.53					OD		2 127		MMED 4005	FLAT DOLL	
	METRO-NORTH	MAINLINE	MILFORD	3	08084R	10	GULF STREET	63.83							and the second se	1893 300-T028 SU	INIMER 1995	FLAT, ROLL	
	METRO-NORTH	MAINLINE			and the second se	10	a second s			The second			OD	4	1 27	1894			
the second se	the state of the s		MILFORD	3	08085R	10	PED UNDERPASS GULE ST	63.84					OD	4	1 8	1959			
	METRO-NORTH	MAINLINE	MILFORD	3	08086R	U	INDIAN RIVER	64.59					OD	4	1 35	1917		FLAT	with 08014R, 08036R, and 08202R
	METRO-NORTH	MAINLINE	MILFORD	3	08087R	U	OLD GATE LANE	64.74				TG	OD	4	1 27	1893 170-1375	1	40 FLAT	
CONNDOT I	METRO-NORTH	MAINLINE	MILFORD	3	08088R	U	QUIRK POND BROOK	66.29	66.29	9 66.29		SA		4	1 6	1893			
CONNDOT I	METRO-NORTH	MAINLINE	MILFORD	3	08089R	U	ROCK LANE	66.35				DG	OD	A	1 47	1893 300-003 170-	1375		
	METRO-NORTH	MAINLINE	MILFORD	3	08090R	U	DEPOT ROAD	66.66		and the second se			OD		1 27		10/0	44	
	METRO-NORTH	MAINLINE	ORANGE	3	08091R	1u	OYSTER RIVER	67.5	67.5						1 21	1893 170-1375		41	
				-								SA		4	8	1893			
	METRO-NORTH	MAINLINE	WEST HAVEN	3	08092R	U	STREAM	67.98	67.98			SA		4	1 6	1893			
	METRO-NORTH	MAINLINE	WEST HAVEN	3	08093R	U	MORGAN LANE	68.11	68.11	1 68.1		DG	OD	4	1 29	1917			
CONNDOT	METRO-NORTH	MAINLINE	WEST HAVEN	3	08094R	U	CULVERT	68.14	68.14	4		SA		4	2 3				
CONNDOT I	METRO-NORTH	MAINLINE	WEST HAVEN	3	08095R	U	CULVERT	68.89	68.89	9 68.8		CP		4	2 7	1995 TWIN 42" PIP	ES 300-003		
CONNDOT N	METRO-NORTH	MAINLINE	WEST HAVEN	3	08096R	U	COVE RIVER	69.19	69.19	9 69.19		SA		4	1 8	1893	20 000 000		
CONNDOT M	METRO-NORTH	MAINLINE	WEST HAVEN	3		10	CAMPBELL AVENUE	70.19	70.19				OD						
	METRO-NORTH	MAINLINE	WEST HAVEN	3	08098R	10	WASHINGTON AVENUE	70.36						4	00	1894			
	METRO-NORTH	MAINLINE	WEST HAVEN	3										4	3 54	1894			
		and the second se	and the state of t			0	WEST RIVER	71.26					CS	4	3 90	1930 301-003 SUM	IMER 1995	FLAT	
	METRO-NORTH	MAINLINE	NEW HAVEN	3		U	STATION UNDERPASS	72.28		8 72.28		IB (	CS	8	2 30	1929		ROLL	
	METRO-NORTH	MAINLINE	BRIDGEPORT		08101R		VIADUCT	55.99										FLAT	
	METRO-NORTH	NEW CANAAN	STAMFORD	3		U	VIADUCT AVENUE	2.81	2.81	1 2.81		DG	OD	1	1 17	1903 170-1375 300	-003	FLAT, ROLL	
CONNDOT IN	METRO-NORTH	NEW CANAAN	STAMFORD	3	08151R	U	NOROTON RIVER	3.77	3.77				OD	1	1 33	1907		. Sti, NOLL	
CONNDOT IN	METRO-NORTH	NEW CANAAN	DARIEN	3		U	STEVENS CROSSING	5.13				the second se	OD D	1	1 15	1903			
	METRO-NORTH	NEW CANAAN	NEW CANAAN	3	08153R	lu	SMITHS CROSSING	5.91					OD		1 10	and the second se			
the state of the s	METRO-NORTH	NEW CANAAN	NEW CANAAN	3	08154R	tu -	WATERWAY	5.51		7 3.91		SA			1 12	1903 300-003			
	METRO-NORTH	DANBURY BRANCH	NORWALK	3	08104R		ANN STREET	0.40	0.40			the second se	00		6				
	METRO-NORTH					Hi	and the second	0.19					OD	3	1 52	1895		6	2
		DANBURY BRANCH	NORWALK	3		L'	NORWALK RIVER	1.56				TG		1	2 117	1905			
	METRO-NORTH	DANBURY BRANCH	NORWALK	3	08202R	10	NORWALK RIVER	3.2				and a subscription of the	DD	1 :	2 162	1905 300-003		7 FLAT	with 08014R, 08036R, and 08086R
	METRO-NORTH	DANBURY BRANCH	WILTON	3	08203R	U.	BROOK	5.12		and the second s		ID (	DD	1	1 12	1919		8	1
CONNDOT	METRO-NORTH	DANBURY BRANCH	WILTON	3	08204R	U	STREAM	6.43	6.43	3 6.43			DD	1 :	2 36	1904			
CONNDOT IN	METRO-NORTH	DANBURY BRANCH	WILTON	3	08205R	U	NORWALK RIVER	6.64				and the state of t	DD	1	48	1919			
CONNDOT IN	METRO-NORTH	DANBURY BRANCH	WILTON	3	08206R		NORWALK RIVER	8.7					DD	1	56	1896			+
	METRO-NORTH	DANBURY BRANCH	WILTON	3	08207R	U	NORWALK RIVER	9.42						1 ,	2 81	1050			+
CONNDOT IN	METRO-NORTH	DANBURY BRANCH	WILTON	3	08208R	tu l	CULVERT	9.91	9.91			SB SB							
	and the second sec	DANBURY BRANCH				ti									6		ONEY HILL RD		
CONNDOT			WILTON	3	08209R		OLD MILL ROAD	11.01	11.01				DD	1 .	29	1909			
ONNDOT N	METRO-NORTH	DANBURY BRANCH	WILTON	3	08210R		NORWALK RIVER	11.55				and the second sec	DD	1 3	3 156	1896			
CONNDOT N CONNDOT N CONNDOT N		DANBURY BRANCH	REDDING	3	08211R	U	FACTORY POND	12.17	12.17	7 12.17		DG C	DD	1 1	46	1904			
CONNDOT N CONNDOT N CONNDOT N CONNDOT N	METRO-NORTH	DANBURY BRANCH	REDDING	4	08212R	U	BRANCHVILLE BROOK	12.83	12.83	3 12.83			CS	1	6				
CONNDOT M CONNDOT M CONNDOT M CONNDOT M CONNDOT M	METRO-NORTH	DANBURY BRANCH	REDDING	4	08213R		OLD REDDING ROAD	14.16				and the second se	DD	1 .	23	1904			
CONNDOT M CONNDOT M CONNDOT M CONNDOT M CONNDOT M		LEANDON I DIVINGH	REDDING	4	08214R		SIMPAUG TURNPIKE	14.8				SA		1	13	300-003 116-1	122	QELAT	No 1 ELAT No O Fiel
CONNDOT M CONNDOT M CONNDOT M CONNDOT M CONNDOT M CONNDOT M	METRO-NORTH	DANBURY BRANCH	REDDING	4			SIMPAUG TURNPIKE	14.8		and the second se		SA						9 FLAT	No. 1 FLAT, No. 2 Flat
CONNDOT M CONNDOT M CONNDOT M CONNDOT M CONNDOT M CONNDOT M	METRO-NORTH METRO-NORTH METRO-NORTH	DANBÜRY BRANCH		4	08215R		UMPAWAUG POND BRK	-							13	300-003 116	D-FIUU4		
CONNDOT N CONNDOT N CONNDOT N CONNDOT N CONNDOT N CONNDOT N CONNDOT N	METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH	DANBURY BRANCH DANBURY BRANCH	and a sub-	1 4				16.41						1 1	25	1904			
CONNDOT N CONNDOT N CONNDOT N CONNDOT N CONNDOT N CONNDOT N CONNDOT N CONNDOT N	METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH	DANBURY BRANCH DANBURY BRANCH DANBURY BRANCH	REDDING		08216R		SAUGATUCK RIVER	17.09					DO	1 1	44	1904	-		
CONNDOT N CONNDOT N CONNDOT N CONNDOT N CONNDOT N CONNDOT N CONNDOT N CONNDOT N CONNDOT N	METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH	DANBÚRY BRANCH DANBURY BRANCH DANBURY BRANCH DANBURY BRANCH	REDDING REDDING	4	the second se	111	CULVERT	19.23	19.23	3 19.23	T	SB		1 1	5	4.4' X 3.6' BOX	x		
CONNDOT N CONNDOT N CONNDOT N CONNDOT N CONNDOT N CONNDOT N CONNDOT N CONNDOT N CONNDOT N CONNDOT N	METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH	DANBURY BRANCH DANBURY BRANCH DANBURY BRANCH DANBURY BRANCH DANBURY BRANCH	REDDING REDDING BETHEL	4	08217R	_	and the second se	1 01 11	21.41	1 21.41		IIB C	DD	1 1	18	1919 300-003			
CONNDOT N CONNDOT N CONNDOT N CONNDOT N CONNDOT N CONNDOT N CONNDOT N CONNDOT N CONNDOT N CONNDOT N	METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH	DANBÚRY BRANCH DANBURY BRANCH DANBURY BRANCH DANBURY BRANCH	REDDING REDDING		the second se	_	SYMPAUG BROOK	21.41	21.41									ROLL	
CONNDOT A CONNDOT A	METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH	DANBURY BRANCH DANBURY BRANCH DANBURY BRANCH DANBURY BRANCH DANBURY BRANCH	REDDING REDDING BETHEL	4	08217R	U	SYMPAUG BROOK							1 1				ROLL	
CONNDOT N CONNDOT N	METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH	DANBÜRY BRANCH DANBURY BRANCH DANBURY BRANCH DANBURY BRANCH DANBURY BRANCH DANBURY BRANCH DANBURY BRANCH	REDDING REDDING BETHEL BETHEL DANBURY	4 4 4	08217R 08218R 08219R	U U	SYMPAUG BROOK SYMPAUG BROOK	21.52	21.52	2 21.52		IB C	CS CS	1 1	18	1936			
CONNDOT N CONNDOT N	METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH METRO-NORTH	DANBÜRY BRANCH DANBURY BRANCH DANBURY BRANCH DANBURY BRANCH DANBURY BRANCH DANBURY BRANCH DANBURY BRANCH	REDDING REDDING BETHEL BETHEL DANBURY DANBURY	4 4 4 4	08217R 08218R 08219R 08220R	บ บ บ	SYMPAUG BROOK SYMPAUG BROOK STILL RIVER	21.52 22.39	21.52 22.39	2 21.52 2 22.39		IB C TG S		1 1	18 134	1936 1975		FLAT	
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SHADING=BRIDGES IN MICROFILM

BUX # RAILROAD TRACK CHART NAME OWNER TOWN DT ST. BRID. NO TYPE LOCATION TRK MI RR. NO. OLD NO. RTE RTE. MI BR TYP DECK TK SP LGN YR BLT ONNDOT METRO-NORTH WATERBURY BRANCH MILFORD 08253R **CULVERT** 4 1 4 4 14 4.1 CONNDOT METRO-NORTH WATERBURY BRANCH MILFORD BOX 3 3 08254R BEARDS FARMWAY 4 55 4.5 4.5 OD 1917 BOX 3 CONNDOT METRO-NORTH WATERBURY BRANCH MILFORD 3 08255R GOLE CROSSING 4 88 4.88 4 89 1983 BOX 3 CONNDOT METRO-NORTH WATERBURY BRANCH MILFORD 08256R 3 DRAINAGE PIPE 4.9 49 4 92 CM CONNDOT METRO-NORTH WATERBURY BRANCH MILFORD BOX 3 08257R GOLF CROSSING 5.01 5.01 4.97 15 1982 CONNDOT METRO-NORTH WATERBURY BRANCH ORANGE 7.02 7.02 BOX 3 3 08258R DAVIS BROOK 6.99 11 CONNDOT METRO-NORTH WATERBURY BRANCH ORANGE 3 08259R CULVERT 7.22 7.29 7.2 10 BOX 3 CONNDOT METRO-NORTH WATERBURY BRANCH ORANGE 3 08260R TWO MILE BROOK 7.36 7.36 1901 TI CONNDOT METRO-NORTH WATERBURY BRANCH DERBY 4 08261R BROOK 7.92 7.92 BOX 3 CONNDOT METRO-NORTH WATERBURY BRANCH DERBY 4 08262R NAUGATUCK RIVER 8.62 8.62 8.59 OD 329 1903 30 CONNDOT METRO-NORTH WATERBURY BRANCH DERBY 4 08263R VIADUCT 8.68 8.6 8.69 Dr 250 318 1965 BOX 3 CONNDOT METRO-NORTH WATERBURY BRANCH ANSONIA 4 08264R NAUGATUCK RIVER 10.3 10.25 TT 10. OD 1904 30 BOX 3 CONNDOT METRO-NORTH WATERBURY BRANCH ANSONIA 08265R 10 55 4 TAIL RACE 10.5 10 49 DC 1903 300 52 BOX 3 ONNDOT METRO-NORTH WATERBURY BRANCH ANSONIA RACEWAY FARREL CO 4 10.85 08266R 10.85 10.72 ONNDOT METRO-NORTH BOX 3 WATERBURY BRANCH ANSONIA 4 08267R SPILLWAY 11.36 11.36 11.29 TG 1904 90 BOX 3 ONNDOT METRO-NORTH WATERBURY BRANCH SEYMOUR 4 CANAL 12.57 08268R 12 57 12 51 TG OD 90 1904 PH ONNDOT METRO-NORTH WATERBURY BRANCH SEYMOUR 08269R BOX 3 4 MUD BROOK 13.2 13.2 13.14 SA 1904 BOX 3 C ONNDOT METRO-NORTH WATERBURY BRANCH SEYMOUR 4 JAMES ST. FOOTBRIDGE 08270R 14.45 14.45 14.39 123 1906 BOX 3 CONNDOT METRO-NORTH WATERBURY BRANCH SEYMOUR 4 08271R TRESTLE 14.61 14.61 14.54 DG 150 BOX 3 C ONNDOT METRO-NORTH WATERBURY BRANCH SEYMOUR 4 08272R NAUGATUCK RIVER 14.64 14.64 14.59 DG OD 307 1903 BOX 3 CONNDOT METRO-NORTH WATERBURY BRANCH SEYMOUR 4 08273R CULVERT 14.68 14.63 14.68 CM BOX 3 CONNDOT METRO-NORTH WATERBURY BRANCH BEACON FALLS 4 08274R OLD PINE'S BRIDGE ROAD 16.79 16.79 17.73 OD 1952 BOX 3 CONNDOT METRO-NORTH WATERBURY BRANCH BEACON FALLS 4 08275R HEMP SWAMP BROOK 16.81 16.81 16.75 CA ONNDOT METRO-NORTH WATERBURY BRANCH BEACON FALLS - 4 08276R SPRUCE BROOK 19.21 19.2 19.33 1947 300 DG ONNDOT METRO-NORTH WATERBURY BRANCH NAUGATUCK 4 08277R SUGAR BUSH BROOM 20.1 20.11 20.26 OD 1960 ONNDOT METRO-NORTH WATERBURY BRANCH NAUGATUCK 08278R 4 US RUBBER CO 21.22 21.22 21.22 CD חכ 1930 ONNDOT METRO-NORTH WATERBURY BRANCH NAUGATUCK BOX 3 4 08279R PRIVATE PASSWAY 21.36 21.36 21.36 OD 1960 ONNDOT METRO-NORTH WATERBURY BRANCH NAUGATUCK 4 08280R LONGMEADOW PND BRK 21.48 21.48 21.48 CA 1906 42 BOX 3 ONNDOT METRO-NORTH WATERBURY BRANCH NAUGATUCK 4 08281R OVERFLOW 21.65 21.65 21.65 CA 1906 BOX 3 CONNDOT METRO-NORTH WATERBURY BRANCH NAUGATUCK 4 08282R CANAL 22.11 22.42 22.11 SA 16 22.37 ONNDOT METRO-NORTH WATERBURY BRANCH NAUGATUCK 4 HOP BROOM 08283F 22.42 DG OD 45 1905 BOX 3 ONNDOT METRO-NORTH WATERBURY BRANCH NAUGATUCK 4 08284R BRIDGE STREET 22.74 22.74 22.68 TG OD 57 1906 BOX 3 CONNDOT METRO-NORTH WATERBURY BRANCH WATERBURY 4 08285R STREAM 24.34 24.34 24.5 SA 1905 BOX 3 CONNDOT METRO-NORTH WATERBURY BRANCH WATERBURY 4 08286R NAUGATUCK RIVER DG 26.42 26.42 26.7 OD 228 1907 CONNDOT METRO-NORTH WATERBURY BRANCH WATERBURY 4 08287R JACKSON STREET 26.5 26.78 26.5 OD ONNDOT METRO-NORTH WATERBURY BRANCH WATERBURY 4 08288R GAS CONDUIT 26.64 26.64 26.9 CA ONNDOT METRO-NORTH WATERBURY BRANCH WATERBURY STATION UNDERPAS 4 08289R 26.82 26.82 27.19 CB PENNCTRL INACTIVE R.RI WILSON PT. IND. TRK NORWALK 3 1346R MEADOW ST 40.28 40.28 4.12 TG 64 1909 WIL CONNDOT METRO-NORTH MAINLIN WESTPORT 8290 INDIAN RIVER 43.8 43 86 BOX 4 CONNDOT METRO-NORTH MAINLINE BRIDGEPORT 8064R PEQUONOCK RIVER 55,9 55.9 55.8 359 1902 015 TG BOX 4 CONNDOT METRO-NORTH MAINLINE WEST HAVEN 3 8099 WEST RIVER 71.26 71.26 71.26 90 1930 301 BOX 4 CONNDOT METRO-NORTH MAINLINE MILFORD 3 8083 WEPAWAUG RIVER/ PROSP 63.53 63.53 63.5 127 1893 300 PEQUONOCK RIVER BOX 5 CONNDOT METRO-NORTH MAIN! INF BRIDGEPORT 3 8064R 55.9 55.9 55.82 359 1902 015 ONNDOT METRO-NORTH MAINI INF BRIDGEPORT 8104R BRIDGEPORT STATION TUNN 55.5 ONNDOT PROV & WORCESTEF MIDDI FTOWN SF MIDDLETOWN 1 00522R ROUTE 66 21.18 23.08 23.08 14.37 DG OD 56 1898 UN ONNDOT PROV & WORCESTER WILLIMANTIC BRANCH WINDHAM 2 01849R S. MAIN (ABANDONED) ST. 22.55 57.97 60 22.55 TG 22.5 51 1892 CV 09802R ONNDOT IRAILS TO TRAILS AVON SECONDARY FARMINGTON 4 FARMINGTON RIVER 398 1956 CONNDOT PROV & WORCESTER PORTLAND TRK **RT 9. & CONNECTICUT RIVE** MIDDLETOWN 1 0639R 0.04 24.26 24.26 23.33 TG 57 1911 ONNDOT PROV & WORCESTER MIDDLETOWN 0863R HARTFORD AVENUE 16.01 CONNDOT NAUGATUCK TORRINGTON SEC. WATERTOWN 4 09110R NAUGATUCK RIVER 32.89 267 5.41 32.89 1907 PRIVATE HOUSATONIC RAILROAD DERBY 104 31 HOUSATONIC RIVER PROV & WORCESTER PRESTON & LEDYARD PRIVATE FARMINGTON RIVER CONNDOT RAILS TO TRAILS VAL MAP 56-60/8 CANTON 09913R 7 54 CONNDEP RAILS TO TRAILS VAL MAP 56-60/6 BURLINGTON 4 09921R BARNES BROOK-DEP 5.61 5.61 40 CEDAR BROOK-DEP RAILS TO TRAILS VAL MAP 56-60/7 BURLINGTON 09922R 6.14 CONNDEP **RAILS TO TRAILS** VAL MAP 56-60/7 BURLINGTON 4 09923R BROOK-DEP 6.37 6.37 SB CONNDOT | RAILS TO TRAILS VAL MAP 56-60/8 CANTON 4 09914R FARMINGTON CANA 7.63 AMTRAK AMTRAK SPRINGFIELD LINE WINDSOR 1 00451R 01074R PALISADO AVENUE 43.08 43.08 3.9 TC 191 TERRYVILLE SEC. B & M STR (GTI) BRISTOL 1 BURLINGTON AV. RT. 69 12 5.1 50.42 69 28.42 164 191 STR (GTI) BERLIN B&M BERLIN SECONDARY 1 01100R ROUTE 372 0.95 7.12 372 4.46 T 196 CSO CSO WETHERSFIELD SEC. HARTFORD 03277R 1 I-91 NORTH 11 45.5 45 4 38.03 01 1958 TUN ABANDONED RR. CONNDEP CANAL SECONDARY CHESIRE 03981R 1 WEST JOHNSON ROAD 18.91 18.91 18,91 DD 190 PRIVATE PRIVATE CANAL SECONDARY NEW HAVEN 3 03997R PROSPECT STREET 1.17 1.17 1 17 191 WILLIMANTIC BRANCH EAST HARTFORD CSO 06052R MAIN STREET 2.16 28.87 28.87 36.32 DC 133 199 CONNDOT PROV & WORCESTE LAUREL IND. TRACK MIDDLETOWN 1 9008 ASYLUM CREEK 3.23 27.14 27,14 DG 24 1904 CONNDOT RAILS TO TRAILS AVON SECONDARY FARMINGTON 4 09802R FARMINGTON RIVER 32 32 DG OD 398 1956

LIST OF RAILROAD BRIDGES SORTED BY STATE BRIDGE NUMBER

	REMARKS	PLAN #	PLAN TYPE	COMMENTS	LOCATIO
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17					
33					
32					
	TWIN 66" PIPES 300-003		FLAT		1
	9.3' x 3.5' BOX				1
1	TURKEY BROOK				
	3' x 7.5' BOX				
3	300-003		FLAT		
5					
4	300-003		1		
3	300-003				
4					
	PHELPS DAM	-			
4					
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	KERITE CO.	1			
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D.	301-003 SUMMER 1995				
3	300-T028 SUMMER 1995				
2	015-147 301-003 SUMMER 1995 300-T028 SUMMER 1995 015-147	1			
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t	UNDER CONSTRUCTION	JE	FLAT, ROLL	No. 1 Flat, No. 2 Flat (IFM-OF-R	1
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APPENDIX

#### OFFSYSTEM BRIDGES IN MICROFILMS

	N N	OFFSY				BRIDGES									5/26/99 5/9/9
OWNER RAILROAD	LINENAME	TOWN		ST BR NO											RIOR COND REMARKS
CONNDOT CONN CENTR				00863R		HARTFORD AVENUE	16.01					67	804	1925	1 3
CONNDOT ICONN CENTR		C. MIDDLEFIELD		01373R	U	BAILEYVILLE ROAD		18.82	18.82 DG			78	936	1898	1 3 AT LONG HILL ROAD
CONNOCT CONN CENTR				09000R		SEBETHE RIVER		30.91	30.91 TG			188	2256	1910	1 4
CONNDOTICONN CENTR				09002R	U	GREEN STREET		30.35	30.35 IB		1 1	23	276	1911	1 3 IOVER ABANDONED RC
CONNDOT CONN CENTR				09004R	U	RESERVIOR BROOK		29.24	29.24 IB 28.44 DG		1 1		360	1911	
CONNDOT CONN CENTR				09008R	U	ASYLUM CREEK		28.44	27.14 DG		1 1		288	1903	1 4
CONNDOT CONN CENTR				09009R	U	MAROMAS CREEK	3.7		26.7 DG		1 1		240	1904	1 4
CONNDOT ICONN CENTR				09010R	U	CATTLE PASS	16.75		18.65 IB		1 1		144	1904	1 3
ONNDOT CONN CENTR				09011R	U	COGINCHAUG RIVER	17.86		19.76 DG		1 1		648	1898	1 3
ONNDOT ICONN CENTR				09013R	U	COGINCHAUG RIVER	18.77		20.67 DT			113	1356	1898	1 4
ONNDOT CONN CENTR				09014R	ANT	COGINCHAUG RIVER	19.93		21.83 DT			118	1416	1898	1 4
ONNDOT CONN CENTR				-00016R-/3	M	CONNECTICUT RIVER		24.39	24.39 TT			1162	13944	1911	3 ALL EXCEPT MECH/EL
ONNDOT INACTIVE RA				09802R		FARMINGTON RIVER @		32	DG		1 4	and the second s	4776	1956	4
ONNDOT INACTIVE RA	ROAD WETHERSFIELD			09828R	U	SOUTH STREET	32.23		32.23		1	65	780		5 NO INS
ONNDOT HNACTIVE RA				09829R	U	NOOK'S ROAD		33.83	33.83		1	65	7801		5 NO INS
ONNDOT INACTIVE RA	ROAD WETHERSFIELD	SEC. CROMWELL	/ 1	09830R		FIRE ACCESS ROAD		35.71			1	10	120	1910	5INO INS
COLDOT NAVERTY	ER TOPPINGTO	ONGER WATERTON	UN	091102	-10	NAVE ATUEN RI	VER- 328	7				i			1
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5/26/99 5/9/96-

#### APPENDIX 17 PLANS AND MICROFILM AVAILABILITY FOR METRO-NORTH BRIDGES

#### State of Connecticut-Department of Transportation-Office of Rail Metro-North Railroad Plans and Microfilm Availablity APPENDIX

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1-

Track Chart Name	Town	Bridge No.	Location	Mile Point	Existing Plans Y/N	Existing Microfilms Y/N
DANBURY	NORWALK	04134R	MARSHALL STREET	0.11		Yes
DANBURY	NORWALK	08200R	ANN STREET	0.19		Yes
DANBURY	NORWALK	08225R	REED STREET	0.36		
DANBURY	NORWALK	08201R	NORWALK RIVER - STEEL GIRDER	1.56	Yes	Yes
DANBURY	NORWALK	08202R	NORWALK RIVER - STEEL GIRDER	3.20	Yes	Yes
DANBURY	WILTON	08203R	BROOK - MULTIBEAM DECK	5.12	Yes	Yes
DANBURY	WILTON	08204R	STREAM - MULTIBEAM DECK	6.43		Yes
DANBURY	WILTON	08205R	NORWALK RIVER - STEEL GIRDER	6.64		Yes
DANBURY	WILTON	08206R	NORWALK RIVER - STEEL GIRDER	8.70		Yes
DANBURY	WILTON		NORWALK RIVER - STEEL GIRDER	9.42		Yes
DANBURY	WILTON		BROOK - CULVERT MASONRY	9.91		
DANBURY	WILTON		OLD MILL ROAD	11.01		Yes
DANBURY	WILTON		NORWALK RIVER - STEEL GIRDER	11.55		Yes
DANBURY	WILTON		FACTORY POND	12.17		Yes
DANBURY	REDDING		OLD REDDING ROAD	14.16		
DANBURY	REDDING		SIMPAUG TURNPIKE	14.10		Yes
DANBURY	REDDING		UMPAWAUG POND BROOK - STEEL GIRDER	16.41	Yes	Yes
DANBURY	REDDING		SAUGATUCK RIVER - STEEL GIRDER			Yes
DANBURY	BETHEL		GRASSY PLAIN ROAD (ROUTE 53)	17.09		Yes
DANBURY	BETHEL	and the second second	BROOK - CONCRETE	19.64		Yes
DANBURY	BETHEL		SYMPAUG BROOK - MULTIBEAM DECK	19.99		
DANBURY	DANBURY		SYMPAUG BROOK - CONCRETE SLAB	21.41		Yes
DANBURY	DANBURY			21.52		Yes
DANBURY	DANBURY			22.39		
				22.94		
	DANBURY		STILL RIVER - CONCRETE CULVERT	23.18		
	DANBURY		STILL RIVER - CONCRETE CULVERT	23.42		
	GREENWICH		NORTH WATER STREET	26.10	Yes	Yes
	GREENWICH		HAMILTON AVENUE	26.79	Yes	Yes
	GREENWICH		PEDESTRIAN UNDERPASS AT HAMILTON AVENUE	26.79		Yes
MAINLINE	GREENWICH		FIELD POINT ROAD	27.77		Yes
	GREENWICH		ARCH STREET (SR 742)	28.06	Yes	Yes
MAINLINE	GREENWICH		PEDESTRIAN OVERPASS FOOTBRIDGE	28.16	Yes	Yes
AINLINE	GREENWICH		STEAMBOAT ROAD	28.22	Yes	Yes
	GREENWICH		DAVIS AVENUE #2	28.48		Yes
	GREENWICH	08004R	INDIAN HARBOR (DAVIS MILL POND)	28.68	Yes	
MAINLINE	GREENWICH		SACHEM ROAD	29.50	Yes	Yes
AINLINE	GREENWICH	08006R	LUKES CROSSING (SOUND SHORE DRIVE)	29.68		Yes
AINLINE	GREENWICH	08008R	MIANUS RIVER (COS COB BRIDGE) - MOVABLE	29.90	Yes	
AINLINE	GREENWICH	08009R	ARCH STREET #2 - MASONRY ARCH	31.03		Yes
AINLINE	GREENWICH	08010R	STREAM - MASONRY CULVERT	31.12		
IAINLINE	GREENWICH	03948R	SOUND BEACH AVENUE	31.29		Yes
IAINLINE	GREENWICH	03955R	TOMAC AVENUE	31.62	Yes	Yes
AINLINE	STAMFORD	03680R	GREENWICH AVENUE	32.81		Yes
IAINLINE	STAMFORD	08011R	RIPPOWAM RIVER - TRUSS	32.85	Yes	
IAINLINE	STAMFORD	03683R	WASHINGTON BOULEVARD (STATE ROUTE 493)	32.97	Yes	Yes
IAINLINE	STAMFORD	08012R	ATLANTIC STREET	33.19	Yes	Yes
AINLINE	STAMFORD	03678R	CANAL STREET	33.41		Yes
AINLINE	STAMFORD	03686R	ELM STREET	33.75	Yes	Yes
1AINLINE	STAMFORD	02237R	EAST MAIN STREET (U.S. ROUTE 1)	34.17	Yes	Yes
1AINLINE	STAMFORD		HAMILTON AVENUE	34.72	Yes	Yes
AINLINE	STAMFORD	Survey of the survey of the state	NOROTON RIVER - MASONRY ARCH	35.58	Yes	Yes

Copy of Master MNR Inspection List with Plans and Microfilm Listing.xls

#### State of Connecticut-Department of Transportation-Office of Rail Metro-North Railroad Plans and Microfilm Availablity

4

Track Chart Name	Town	Bridge No.	Location	Mile Point	Existing Plans Y/N	Existing Microfilms Y/N
MAINLINE	DARIEN	08015R	NOROTON STATION PEDESTRIAN OVERPASS	36.24		
MAINLINE	DARIEN	08016R	STONY BROOK - MASONRY CULVERT	37.16		Yes
MAINLINE	DARIEN	08017R	CUMMINGS BROOK - MASONRY CULVERT	37.36		Yes
MAINLINE	DARIEN	04142R	LEROY AVENUE #1	37.59	Yes	Yes
MAINLINE	DARIEN	00316R	BOSTON POST ROAD (U.S. ROUTE 1)	37.82	Yes	Yes
MAINLINE	DARIEN	08018R	GOODWIVES RIVER - CONCRETE DECK	38.00		Yes
MAINLINE	DARIEN	08020R	TOKENEKE BROOK - 2 STONE CULVERTS	38.53		
MAINLINE	DARIEN	08021R	TOKENEKE BROOK #2 - MASONRY CULVERT	38.68		
MAINLINE	DARIEN	04271R	RAYMOND STREET	38.93		
MAINLINE	DARIEN	08022R	FIVE MILE RIVER - BRICK ARCH	39.07	Yes	Yes
MAINLINE	NORWALK	08023R	DRY STREAM - MASONRY CULVERT	39.08		
MAINLINE	NORWALK	04132R	ROWAYTON AVENUE	39.11	Yes	Yes
MAINLINE	NORWALK	08025R	FRANKLIN STREET (SPRING STREET)	40.89		Yes
MAINLINE	NORWALK	08026R	STATION UNDERPASS	41.02		
MAINLINE	NORWALK	08027R	MONROE STREET	41.12		Yes
MAINLINE	NORWALK	03693R	WASHINGTON & MAIN STREETS	41.28	Yes	Yes
MAINLINE	NORWALK	04288R	NORTH WATER STREET & NORWALK RIVER - MOVABLE		Yes	Yes
MAINLINE	NORWALK		FORT POINT STREET	41.79		Yes
MAINLINE	NORWALK	08028R	OSBORNE AVENUE	41.96		Yes
MAINLINE	NORWALK	03691R	EAST AVENUE	42.14	Yes	Yes
MAINLINE	NORWALK	08029R	BROOK - MASONRY CULVERT	42.26		Yes
MAINLINE	NORWALK		STRAWBERRY HILL AVENUE	42.37		Yes
MAINLINE	WESTPORT		INDIAN RIVER - TWO CONCRETE PIPES 7' DIAMETER	43.80		
MAINLINE	WESTPORT		SAUGATUCK AVENUE (ROUTE 136)	43.97		Yes
	WESTPORT	NAME OF TAXABLE PARTY.	NEW STATION UNDERPASS	44.12		
	WESTPORT		STATION UNDERPASS - CONCRETE CULVERT	44.12		Yes
	WESTPORT		FERRY LANE & SAUGATUCK RIVER - MOVABLE	44.32	Yes	Yes
	WESTPORT		COMPO ROAD	44.70		Yes
	WESTPORT	Careford Contracts Income	SHERWOOD MILL POND - CONCRETE CULVERT	44.70		Yes
MAINLINE	WESTPORT		MUDDY BROOK - CONCRETE CULVERT	46.11		Yes
	WESTPORT	CONTRACTOR STATE	NEW CREEK ROAD	40.11		10.00
	WESTPORT		GREENS FARMS BROOK - METAL PIPE CULVERT	47.13	Yes	Yes
	WESTPORT		MAPLE LANE	47.44	 Vaa	
	WESTPORT	and the second se	SASCO RIVER - MULTIBEAM DECK	48.29	Yes	Yes
	FAIRFIELD		WESTWAY ROAD	48.65		Yes
	FAIRFIELD		CENTER STREET	48.81		Yes
	FAIRFIELD	And the second statement	SPRUCE STREET	_		Yes
	FAIRFIELD		OLD POST ROAD	48.91		Yes
	FAIRFIELD		MILL RIVER - MULTIBEAM DECK	49.01		Yes
	FAIRFIELD		NORTH PINE CREEK ROAD	49.66	Yes	Yes
	FAIRFIELD		MILL PLAIN ROAD	50.02		Yes
	FAIRFIELD			50.29		Yes
				50.90	Yes	Yes
				51.12		Yes
	FAIRFIELD		FAIRFIELD CREEK - MULTIBEAM DECK	51.68	Yes	Yes
	BRIDGEPORT			53.00	Yes	Yes
			FAIRFIELD AVENUE (ROUTE 130)	53.42		Yes
				53.60	Yes	Yes
				53.72		Yes
	BRIDGEPORT	USUSUR	HOWARD AVENUE	53.84		Yes
	BRIDGEPORT	000540	WORDIN AVENUE	54.07		Yes

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#### State of Connecticut-Department of Transportation-Office of Rail Metro-North Railroad Plans and Microfilm Availablity

4

Track Chart Name	Town	Bridge No.	Location	Mile Point	Existing Plans Y/N	Existing Microfilms Y/N
MAINLINE	BRIDGEPORT	08053R	SOUTH AVENUE	54.44	Yes	Yes
MAINLINE	BRIDGEPORT		PARK AVENUE	54.58	Yes	Yes
MAINLINE	BRIDGEPORT	and the second s	MYRTLE AVENUE	54.70	Yes	Yes
MAINLINE	BRIDGEPORT		WARREN STREET	54.77	Yes	Yes
MAINLINE	BRIDGEPORT		LAFAYETTE STREET	54.84		Yes
MAINLINE	BRIDGEPORT		BROAD STREET	54.93	Yes	Yes
MAINLINE	BRIDGEPORT	08058R	MAIN STREET #1	54.98		Yes
MAINLINE	BRIDGEPORT	08059R	HOUSATONIC CROSSING	55.13	Yes	Yes
MAINLINE	BRIDGEPORT		SOUTH PEDESTRIAN OVERPASS	55.35		
MAINLINE	BRIDGEPORT		UNION STREET	55.40	Yes	Yes
MAINLINE	BRIDGEPORT	08108R	NORTH PEDESTRIAN OVERPASS	55.41		Yes
MAINLINE	BRIDGEPORT		BRIDGEPORT HARBOR - REINFORCED CONCRETE SLAE	0.0000000000000		
MAINLINE	BRIDGEPORT		BRIDGEPORT HARBOR - PEDESTRIAN TUNNEL	55.50		
MAINLINE	BRIDGEPORT	08062R	STATION VIADUCT	55.51	Yes	
MAINLINE	BRIDGEPORT		STRATFORD AVENUE (ROUTE 130)	55.61		Yes
MAINLINE	BRIDGEPORT	08063R	VIADUCT - PARKING LOT NORTH	55.69		Yes
MAINLINE	BRIDGEPORT		PEQUONNOCK RIVER - MOVABLE	55.90	Yes	Yes
MAINLINE	BRIDGEPORT	08065R	VIADUCT	55.91		Yes
MAINLINE	BRIDGEPORT		PULASKI STREET	55.95		Yes
MAINLINE	BRIDGEPORT		VIADUCT	55.95		
MAINLINE	BRIDGEPORT		NOBLE AVENUE			Yes
MAINLINE	BRIDGEPORT		VIADUCT	55.98		Yes
MAINLINE	BRIDGEPORT		CLARENCE STREET	55.99		
MAINLINE	BRIDGEPORT		KOSSUTH STREET	56.00		
MAINLINE	BRIDGEPORT		EAST MAIN STREET (ROUTE 127)	56.10		Yes
MAINLINE	BRIDGEPORT		PEMBROKE STREET	56.20		Yes
MAINLINE	BRIDGEPORT		HALLETT STREET	56.35	Yes	Yes
MAINLINE	BRIDGEPORT		YELLOW MILL POND - BRICK ARCH	56.46	Yes	Yes
MAINLINE	BRIDGEPORT		SEAVIEW AVENUE	56.68		Yes
MAINLINE	BRIDGEPORT		BISHOP AVENUE	56.77		Yes
	BRIDGEPORT		BRUCE BROOK - CONCRETE PIPE 9' DIAMETER	57.46	Yes	Yes
MAINLINE	STRATFORD	State of the local division of the	BRUCE AVENUE	57.54		Yes
MAINLINE	STRATFORD		WEST BROAD STREET	57.62		Yes
MAINLINE	STRATFORD		KING STREET	58.72	Yes	Yes
MAINLINE	STRATFORD		MAIN STREET (ROUTE 113)	58.88		Yes
MAINLINE	STRATFORD		EAST MAIN STREET (ROUTE 110)	59.01		
MAINLINE	STRATFORD		HOUSATONIC RIVER (DEVON BRIDGE)	59.96		Yes
MAINLINE	MILFORD		BEAVER CREEK - MASONRY ARCH	60.42	Yes	Yes
	MILFORD		BEAVER CREEK - MASONRY ARCH BEARDSLEY AVENUE	61.62		Yes
	MILFORD		HIGH STREET	62.94		Yes
MAINLINE	MILFORD			63.27	Yes	Yes
MAINLINE				63.44	Yes	Yes
	MILFORD		WEPAWAUG RIVER & PROSPECT STREET	63.53	Yes	Yes
				63.83		Yes
MAINLINE	MILFORD		PEDESTRIAN UNDERPASS AT GULF STREET	63.84		Yes
			INDIAN RIVER - MULTIBEAM DECK	64.59	Yes	Yes
	MILFORD			64.74	Yes	Yes
	MILFORD	And a second second	QUIRK POND BROOK - MASONRY CULVERT	66.29		Yes
	MILFORD			66.66	Yes	Yes
			OYSTER RIVER - MASONRY CULVERT	67.50		Yes
	WEST HAVEN		STREAM - STONE/CONCRETE CULVERT	67.98		
MAINLINE	WEST HAVEN	08093R	MORGAN LANE	68.11		Yes

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#### APPENDIX 18 MAP FILE LEGEND

#### Map File Legend

<u>BOX 15</u>	<u>BOX 14</u>	BOX 13
<u>BOA 15</u>	BOA 14	BUA 15
BRIDGE	NEW HAVEN YARD	NEW HAVEN YARD
08080R	NEW HAVEN SIGNAL	"B" PARCEL
000000	LAYOUTS	DANBURY
	ELECTICAL POWER SITE	GEORETOWN STATION
	PLANS	Misc.
	BETHEL/DANBURY	wiise.
	STATION	
	Misc.	
	WIISC.	
<u>BOX 12</u>	<u>BOX 11</u>	<u>BOX 10</u>
BRIDGE	BRIDGE	BRIDGE
08037R-08078R	03723R-08036R	09110R-09008R
08057K-08078K	05725R-08050R	09110K-09008K
BOX 9	<u>BOX 8</u>	<u>BOX 7</u>
New Haven yard	BRIDGE	BRIDGE
MISC plans	00046R-03693R	08083R-0639R
-		
<u>BOX 12</u>	<u>BOX 5</u>	<u>BOX 4</u>
MISC. property	Misc.	345KV
plans	Station plans	
DOV 2	DOV 2	DOV 1
<u>BOX 3</u>	<u>BOX 2</u>	<u>BOX 1</u>
Misc.	MISC.	345KV
	New Haven Tunnel	
New Haven station	Versailles Willimantic	
	Mainline Control	
Station plans New Haven station plans Bridgeport station plans	Versailles Willimantic	

#### APPENDIX 19 BASEMENT STORED FILES BY PROJECT AND OTHER

	Box Information				Plan Information	
Project Number	Box Description	Label Number	Location of Box	Project Number	Plan Description	Plan Location
0058-0260	School St. Quad Gates	100	1st Row Right Side	0058-0277	School Street	2nd Row Right Side
0058-0260	School Street	100		0058-0277	School Street	2nd Row Right Side
09-800	Plans & Specs Greiner Environmental/ ADA	23	2nd Row Left Side	0058-0277	School Street and West Mystic Ave.	2nd Row Right Side
09-800	ADA/Environmental Plans and Specs Greiner	23	2nd Row Left Side	0058-0277	School Street	2nd Row Right Side
135-166	Metro North Invoices	24	2nd Row Left Side	0058-0277	School Street Improvements	2nd Row Right Side
135-208	Stamford	115		0058-0277	School Street Improvements Repair	2nd Row Right Side
135-216	CO/ Submittals	54	2nd Row Right Side	09-800	Bethel Station Relocation	2nd Row Right Side
136-200's		21	1st Row Right Side	135-175	Stamford	2nd Row Right Side
15-147	Peck (12 Boxes)	A	2nd Row Left Side	300-0017	Danbury Branch, New Canaan Rail Line Plan Maps	2nd Row Right Side
15-147	Peck (14 Boxes)	A	2nd Row Right Side	301-0001	New Haven Interlocking	2nd Row Right Side
15-157	East Bridgeport Daily Reports	49	2nd Row Right Side	301-0001	NH Interlocking	2nd Row Right Side
15-157	East Bridgeport Kaiser Engs, Francini Const.	49	2nd Row Right Side	301-0001		2nd Row Right Side
15-157	East Bridgeport	49	2nd Row Right Side	301-006	Stamford Station	2nd Row Right Side
	East Bridgeport, Stamford Tunnel Lobby, BROT Restoration of Rail System	10	Zha riow riight oldo	001 000		Zha riow right oldo
15-157	1994	40	2nd Dow Dight Side	34-234	Danbury Branch	2nd Dow Dight Side
15-157		49	2nd Row Right Side	50-175	NH Station	2nd Row Right Side
	East Bridgeport, NH Maint. Facility Change Orders	49	2nd Row Right Side			2nd Row Right Side
161-120, 301-001	Wilton	16	2nd Row Left Side	502-2200-166	Norwalk Submarine Cable Relocation	2nd Row Right Side
170-1375	Fairfield Ave, Bridgeport	78		884-497	Danbury Line	2nd Row Right Side
170-1375	East Ave Railroad Bridge, Norwalk	78	Ou d Davy Diskt Oide	92-350	NH Rehab	2nd Row Right Side
170-1506	Bridge Inspection MNR WO 2932, 2933, 3005	39	2nd Row Right Side	92-414	New Haven Diesel Shop	2nd Row Right Side
170-1616		34	1st Row Right Side	92-414	New Haven Line	2nd Row Right Side
170-850, 300-048,	Misc. Invoices 1997-1999	63	2nd Row Right Side	92-435	Tomliason Bridge	2nd Row Right Side
170-850, 301-003	West River	56	2nd Row Right Side	k3173/9534	Norwalk River	2nd Row Right Side
300-0018	New Haven and Bridgeport Station	73	2nd Row Right Side		New Haven Interlocking	2nd Row Right Side
300-0062	Network Infrastructure (2002) 2002 Site Books	3	2nd Row Left Side		Guilford Station	2nd Row Right Side
300-0062	Network Infrastructure Project 100% Design	3	2nd Row Left Side		New Haven Railroad Track Design	2nd Row Right Side
300-0062	Network Infrastructure Project Plans, Letters, Reports, and Test	3	2nd Row Left Side		New Haven Interlocking 1 of 2	2nd Row Right Side
300-0062	Network Infrastructure Project GE Transportation Systems Box 2	3	2nd Row Left Side		New Haven Station Phase 1	2nd Row Right Side
300-0062	Network Infrastructure (2002) Site Books	3	2nd Row Left Side		Ridgefield Route US 7, CT 102	2nd Row Right Side
300-0062	Network Infrastructure Project (Hudson/ Harlem Side NY) 100% Design Books	3	2nd Row Left Side		New Haven Railroad East Interlock	2nd Row Right Side
300-0062	Network Infrastructure Project (Hudson/ Harlem NY) 100% Design Site Books	3	2nd Row Left Side		Danbury Branch	2nd Row Right Side
300-0062	Network Infrastructure 2002	3	2nd Row Left Side		New Haven Station Map	2nd Row Right Side
300-0062	Network Infrastructure Project 100% Final Design Submittals 9/12/2002	3	2nd Row Left Side		Catenary Repairs by UI/NU	2nd Row Right Side
300-0062	Network Infrastructure (2002) Preliminary Submittals	3	2nd Row Left Side		Williams Fiber Optic Hartford to RI	2nd Row Right Side
300-0062	Engineering Furnishing and Intergration of a Fiber Optic Communication	3	2nd Row Left Side		Danbury Signalization Project Plans	2nd Row Right Side
300-0062	Network	3	2nd Row Left Side		New Canaan Fiber Optics	2nd Row Right Side
300-0062	Network Infrastructure 8/2002 100% Site Books	3	2nd Row Left Side		Williams Fiber Optic Greenwich to Hartford	2nd Row Right Side
300-0062	Network Infrastructure Contracts, Ect.	3	2nd Row Left Side		Communication Conduit Installation State Rd Xing District 3	2nd Row Right Side
300-0062	Network Infrastructure Factory Acceptance Test	3	2nd Row Left Side		Communication Conduit Installation State RT. 5 North Haven	2nd Row Right Side
300-0062	Network Infrastructure (2001)	3	1st Row Right Side		New Haven Station Phase 2	2nd Row Right Side
300-0062	Network Infrastructure Engineering and Furnishing and Negotiations 2001	3	1st Row Right Side		Stamford Transportation Center Interlocking	2nd Row Right Side
300-0062	Network Infrastructure 1998-1999	3	1st Row Right Side		New Haven Line CP-230	2nd Row Right Side
300-0062	Network Infrastructure 1998-1999	3	1st Row Right Side		New Haven Yard Report	2nd Row Right Side
	Network Infrastructure NY Section of NH Line 100% Design Submittal and Site					<u> </u>
300-0062	Books	3	1st Row Right Side		Amtrak High Speed Config. NH-7Boston 1995	2nd Row Right Side
300-0062	Network Infrastructure (2000)	3	1st Row Right Side		Amtrak High Speed Railline Improvements	2nd Row Right Side
300-0062	Network Infrastructure (2001)	3	2nd Row Right Side		Galleria at Long Wharf, New Haven	2nd Row Right Side
300-0062	Network Infrastructure (1999)	3	2nd Row Right Side		Stamford Station Center Platforms	2nd Row Right Side
300-0062	Network Infrastructure (1997-1998)	3	2nd Row Right Side		Communication Conduit Installation Redding	2nd Row Right Side
300-0074	New Haven Radio Project 60% Submittal	14	2nd Row Left Side		Stamford Station Project	2nd Row Right Side
300-0074	NH Radio Project 2001-2002	14	1st Row Right Side		Westbrook Factory Stores	2nd Row Right Side
300-0074	NH Radio Project MNR Study and 90% Submittal	14	1st Row Right Side		Stamford Station Center Platforms	2nd Row Right Side

Box Information				Plan Information		
Project Number	Box Description	Label Number	Location of Box	<b>Project Number</b>	Plan Description	Plan Location
300-0074	NH Radio Project	14			NHL Electrification Old Plans '49	2nd Row Right Side
300-0074	Radio Upgrade/ Enhancements	14			Loco Service Agreement New Haven	2nd Row Right Side
300-030	West Haven Culvert, Davis Brook Culvert, Seymour R/N	43	2nd Row Right Side		Stamford Station Center Platforms Signal Power Locations	2nd Row Right Side
300-031	Substructure Repairs on RR Bridges	31	1st Row Right Side		Stamford Platforms	2nd Row Right Side
300-033	CHA Invoices, and Agreement	32	1st Row Right Side		Stamford Center Island Platforms	2nd Row Right Side
300-033	CHA Assignments, Scope of Work, Correspondence	32	2nd Row Right Side		NHL Stamford Station Project	2nd Row Right Side
300-033	South Ave, Bridgeport	80			Hanger, New Canaan Bridge	2nd Row Right Side
300-033	W.O. 3044	80			New Haven Interlocking	2nd Row Right Side
300-033	W.O. 3045	80			Demolition Plan of Viaduct Road	2nd Row Right Side
300-033	W.O. 3046	80			36" Tideflex Valve '94	2nd Row Right Side
300-036, 170-1504	W.O. 3034 Aglas Bridge Inspection and Greiner Bridge Inspection	69	2nd Row Right Side		RT 113 Stratford, CT Lordship Bull Xing '90	2nd Row Right Side
300-048	Wepawaug Construction	6	2nd Row Left Side		Signal Control System '86	2nd Row Right Side
300-048	Wepawaug General Files	6	2nd Row Right Side		New Haven Interlocking '92	2nd Row Right Side
300-074	NH Radio Project	14	2nd Row Right Side			
					•	
300-077	Devon Bridge 2000-2005 Vol 2 of _ by Art DiCesare, Inc	97				
300-077	Devon Bridge 2000-2005 Vol 1 of _ by Art DiCesare, Inc	97				
300-087	Construction Files to 4 Bridges Noroton River, Greens Farm Bk, Norwalk River,	30	1st Row Right Side	1		
300-087	Repairs to 4 MNR Bridges Noroton River, Greens Farm Bk, Norwalk River,	30	1st Row Right Side	1		
300-101	Main Street, Bridgeport	77	1st Row Right Side	1		
300-102	Long Meadow Pond Brook Naugatuck, Ash Creek Fairfield, RT 106 New	76	Ŭ	1		
301-0001, 94-414,				1		
94-246, 34-225,						
300-035		26	2nd Row Left Side			
301-003	West River Construction	62	2nd Row Right Side			
301-003	West River Design/ Construction	62	2nd Row Right Side	1		
301-003	West River, Envr/Misc	62	2nd Row Right Side	1		
301-0040	Walk/Saga Drawing 2007	107	Ŭ	1		
301-0040	Walk/ Saga Bridges CP 241 Signal Mod.	107		1		
301-0040	Walk/ Saga	107		1		
301-0040	Walk/ Saga (5 Boxes)	107		1		
301-006, 020, 021,				1		
135-206, 208	Stamford Center Island Platform	В	2nd Row Right Side			
301-006, 020, 021,				1		
135-206, 208	Stamford Yard 1994	В	2nd Row Right Side			
301-006, 020, 021,				1		
135-206, 208	Stamford Station Center Island Platform Project	В	1st Row Right Side			
301-006, 020, 021,				]		
135-206, 208	Stamford Transportation Center Shop Drawings	В	1st Row Right Side			
301-006, 020, 021,				1		
135-206, 208	Stamford Center Island Platforms (21 Boxes)	В	2nd Row Left Side			
301-006, 020, 021,						
135-206, 208	5700 & Stamford Trans Center	В	2nd Row Left Side	]		
301-023, 301-028	East Cut Drainage, West Cut Retaining Walls, Northeast High Speed Rail Improvement Project Amtrak, NH Yard Car & Diesel Shop, Remedial Manag. Plan, Wepawaug River	25	2nd Row Left Side			
301-025	Catenary Replacement B/T 1045 & 73-16 (AM) / Contract for 92-529 State Street Station	46	2nd Row Right Side			
301-039	NH Railyard Switching Plan	65	2nd Row Right Side	1		
			Δ10_2	-		

Box Information				Plan Information		
Project Number	Box Description	Label Number	Location of Box	Project Number	Plan Description	Plan Location
301-041	Documents CP 248 Tie-In Design	117			•	
301-043	Depot Road and Rock Lane	27	1st Row Right Side	1		
301-045	Washington and Main, South Norwalk MNR WO NO. 3096, 3097	36	2nd Row Right Side	1		
301-053	Old Gate Lane, Construction Catenary "D"	33	2nd Row Right Side	1		
301-059	River St., High St., Old Gate Rd Milford Bridges	112	Ŭ Ŭ			
301-065/71	CP-248 Invoices and Misc.	106	1st Row Right Side	1		
301-065/71	CP-248 Files	106	1st Row Right Side	1		
301-070	C1b	79		1		
301-070	North Benson Road	79		1		
301-074	Emergency at Walk Bridge West and East Channel Submarine Cables	108		1		
301-074	Emergency at Walk Bridge East Channel	108				
301-074	Walk Claims 3 Boxes	108				
301-075	Westway Road	81				
	Dennis Murphy 2003 Go-No-Go Signals along NHL Danbury, Bethel, Redding,					
302-0007	Ridgefield, Wilton, Norwalk	5	2nd Row Left Side			
302-0007/ 0034-0234	Danbury CTC (16 Boxes)	75		1		
302-005	Simpaug Turnpike Danbury Branch Redding	58	2nd Row Right Side	1		
302-006	RTE 53, Bethel	45	2nd Row Right Side	1		
302-006	RTE 53, Bethel	45	2nd Row Right Side	1		
302-009, 301-035,	Emergency Projects- Marshall St. Norwalk, Westport Culvert, New Haven	10				
301-080, 153-107	Catenary, Watertown Pier Repairs	11	2nd Row Left Side			
302-010	Emergency Declaration for Norwalk Retaining Wall Box 1 of 2	38	2nd Row Right Side			
302-010	Emergency Declaration for Norwalk Retaining Wall Box 2 of 2	38	2nd Row Right Side			
303-004	Viaduct Road, Stamford	7	2nd Row Left Side	1		
303-004	Viaduct Road, Stamford	7	2nd Row Right Side	1		
304-002	Waterbury Station and New Haven Station	74	2nd Row Right Side	1		
304-007	Emergency Decl. Seymour Canal	96	1st Row Right Side	1		
			Ŭ	1		
34-225	Danbury Station Relocation	66	2nd Row Right Side			
01220			Zhà riow right olao			
50 100	North Benson Road Railroad Bridge	4	and Davy Laft Cide			
50-199	School St. Doc Ref and West Mystic Bills (2 Boxes)	100	2nd Row Left Side			
58-260		100				
82-280	Middletown Bridge (P & W Repairs) 1999	17	2nd Row Left Side	-		
82-280	Middletown Swing Bridge Information 1999	17	1st Row Right Side	4		
82-280	Middletown Swing Bridge Pier # 5 Fender Replacement Box 2 of 2	17	1st Row Right Side	4		
82-280	Middletown Swing Bridge and Cluster	17	1st Row Right Side			
82-280	Middletown Swing Bridge Pier # 5 Fender Replacement Box 1 of 2	17	1st Row Right Side	1		
82-280	Middletown Swing Bridge	17	2nd Row Right Side	1		
82-280	Middletown Swing Bridge Rehab	17	2nd Row Right Side	1		
888-446, 888-429,			•	1		
888-427, 884-450	D. Chase Misc. Project Records	59	2nd Row Right Side			
, , , , , , , , , , , , , , , , , , , ,	, , , , , , , , , , , , , , , , , , ,			1		
92-380	New Haven Platforms "B" and "C" agreement Box 1 of 3	9	2nd Row Left Side			
92-380	MNR and Gooding O' Dea Invoices Box 2 of 3	9	2nd Row Left Side	1		
92-380	Platforms "B" and "C" Design Box 3 of 3	9	2nd Row Left Side	1		
92-426	New Haven	101	1st Row Right Side	1		
<u> </u>				4		

	Box Information				Plan Information	
Project Number	Box Description	Label Number	Location of Box	Project Number	Plan Description	Plan Location
Design Pr. 170-1375						
Const. Pr. 170-1509,						
Design Pr. 170-1509						
	Boston Post Road, Darien and Monroe Street, Norwalk	13	2nd Row Left Side			
Design Pr. 170-1375						
Const. Pr. 300-098	Rowayton Ave. in Norwalk	12	2nd Row Left Side			
S-4	Laber	55	2nd Row Right Side			
	Proposed New Haven Yard Improvements	2	2nd Row Left Side			
	Scientech 2001-2000 (Doct. Submittals, E-mail, Letters)	4	2nd Row Left Side			
	New Haven Rail Yard	8	2nd Row Left Side			
	Bureau of Public Trans Picnic Records	10	2nd Row Left Side			
	Danbury Branch Catenaries 2002	15	2nd Row Left Side			
	Danbury 9/15/1993 & ADA	18	2nd Row Left Side			
	Inductive Grounding Study NHL and Northern Telecom Manuals	19	2nd Row Left Side			
	Rails Storage- Sasco Creek	20	2nd Row Left Side			
	Scientech Doct. Submittals 1999-2000	4	2nd Row Left Side			
	Catenary Report- NH Mainline	22	2nd Row Left Side			
	NHL Service Meeting Minutes	28	1st Row Right Side			
	Bridge Invoices D. Murphy	29	1st Row Right Side			
	Catenary Report- NH Mainline	22	1st Row Right Side			
	Catenary "A" Construction- Arch St., Hamilton St., N. Water St. (Greenwich)	35	1st Row Right Side			
	Signal and FRA ITS Info	37	2nd Row Right Side			
	Fiber Optic Communication System	40	2nd Row Right Side			
	Fiber Optic Communication Project	41	2nd Row Right Side	4		
	Clough-Harbor Assignments General Corresp/ Asylum Crk/Norwalk Rvr Study/					
	Noroton Rvr Study/ Seismic Screening Box 1 of 2	42	2nd Row Right Side			
	Clough-Harbor Assignments General Corresp/ Asylum Crk/Norwalk Rvr Study/					
	Noroton Rvr Study/ Seismic Screening Box 2 of 2	42	2nd Row Right Side			
	CL & P Cos Cob RR Supply Substation As- Builts	44	2nd Row Right Side	4		
	Coloner New Concer Freedon Discreme Descert Corr. Components for NUL	47	and Davy Dight Cide			
	Catenary New Canaan Erection Diagrams Dossert Corp. Components for NHL Catenary Standards Constant Tension Catenary 1998	47 48	2nd Row Right Side 2nd Row Right Side			
	Misc. Files D- Chase (Capital, PMOs, Peck Status Reports)	50	2nd Row Right Side			
		50		1		
	MNR Invoices W.O. 3007, 3025, 3027, 2832, 2799, 3026, 2608, 2798, 3008,					
	2607, 2841, 2421, 3022, 3006	51	2nd Row Right Side			
	NH Railyard Demolition Photos and Stamford Station	52	2nd Row Right Side			
	Misc. Bridge Invoices Murphy 1998-1999	53	2nd Row Right Side			
	Railroad Improvements NH Yard, Williams Comm ETC	57	2nd Row Right Side			
	Northeast Telecom Manuals, Inductive Grounding Study, Catenary Report NH	68	2nd Row Right Side	4		
	MNR Railroad Contract No. 9438, Design and Engineering Services/ S and C					
	A08, Task #004 Perform Radio Frequency Survey in CT	61	2nd Row Right Side	I		
	CP-248/ CP-261 2003	60	2nd Row Right Side	1		
	TCG Fiber Optic Proj. RR Improvements	64	2nd Row Right Side	4		
	F. Chojnicki	67	2nd Row Right Side	4		
	South Norwalk Station and Parking Garage	70	2nd Row Right Side	1		
	South Norwalk Station and Parking Garage/ Waterbury Station	71	2nd Row Right Side	1		
	Bridgeport Intermodal Facility and New Canaan Railroad	72	2nd Row Right Side	4		
	Various Projects 1997-2000	82	1st Row Right Side	4		
	Contracts 1993 and Quarterly Reports 1976-1980	83	1st Row Right Side	1		

	Box Information				Plan Information	
Project Number	Box Description	Label Number	Location of Box	Project Number	Plan Description	Plan Location
	Various Projects 1997-1999	84	1st Row Right Side			
	Various Projects Correspondants 1992, 1999-2000	85	1st Row Right Side	1		
	Various Project Correspondants 1992	86	1st Row Right Side	1		
	Various Project Correspondants 1992-1998	87	1st Row Right Side	1		
	Scientech 2000 Box 1 of	4	1st Row Right Side	1		
	Scientech 2000 Box 2 of	4	1st Row Right Side	1		
	Various Project Correspondants 1989-1993	89		1		
	Various Project Correspondants 1995, 1997, 2000	90	1st Row Right Side	1		
	Quarterly Reports 1981-1989, Contracts 1997	91	1st Row Right Side	1		
	Various Projects 1997 Provisions	92	1st Row Right Side	1		
	Contracts and Correspondants 1998	93	1st Row Right Side	1		
	Various Project Correspondants 1995-2000	94	1st Row Right Side	1		
	Short Circuit Test Data 1985 Bridgeport Section	95	1st Row Right Side	1		
	Correspondence, Contracts 1985-1987	98	1st Row Right Side	1		
	New Haven Change Orders	99	1st Row Right Side	1		
	Various Projects Correspondants 1987-1991	102	1st Row Right Side	1		
	Various Projects Correspondants 1985-1990	103	1st Row Right Side	1		
	Various Projects Correspondants 1985-1987	104	1st Row Right Side	1		
	Various Projects Change Orders 1988-1989	105	1st Row Right Side	1		
	M/N Invoices Sect. A 3 of 4	109	1st Row Right Side	1		
	M/N Invoices Sect. A 2 of 4	109	1st Row Right Side	1		
	M/N Invoices Sect. A 1 of 4	109	1st Row Right Side	1		
	New Railroad Bridge over Reed St. Norwalk	110		1		
	Photos	111	1st Row Right Side	1		
	Lochner- HRA (Rail Contracts)	113		1		
	Lochner On- Call 2000	114		1		
	345 kV Lines UI and NU (2 Boxes)	116		]		
	Steve Misc. (CP 241, NHL, Radio Project, Danbury Yard)	118				
	ATT Cable on Danbury, Williams on Danbury, New Canaan, ALS Plans, TCG, Mainline Cable Plans	119				
	Scientech	4		1		
		+ *		1		
				1		
				1		
				1		
				1		
				1		
				1		

#### APPENDIX 20 LOCATION OF BRIDGE PLANS (FROM BRIDGE SAFETY AND EVALUATION) IN BASEMENT STORAGE

Bridge No.	Plan Typ	e Comments
00451 R	Roll	
00522 R	Flat	No. 1 Flat
00522 R	Flat	No. 2 Flat
00522 R 00710 R	Roll	
00710 R	Flat Roll	
01063 R	Flat	
01063 R	Roll	
01074 R	Flat	
01100 R	Roll	
01312 R	Roll	
01849 R	Roll	
03277 R	Roll	
03638 R	Flat	with 03680R, 03678R, and 08056R
03640 R	Flat	
03644 R	Flat	
03673 R 03673 R	Flat	
03678 R	Roll Flat	with 02000D 0200D
03680 R	Flat	with 03680R, 03638R, and 08056R
03683 R	Flat	with 03678R, 03638R, and 08056R No. 1 Flat
03683 R	Flat	No. 2 Flat
03691 R	Roll	
03693 R	Flat	
03867 R	Roll	
03943 R	Flat	
03943 R	Roll	
03945 R	Flat	
03981 R	Flat	
03997 R	Roll	
04025 R	Roll	(II. 000000 00000
04288 R 06052 R	Flat	with 08080R, 08008R, and 08034R
08008 R	Roll Flat	with 02020D 04020D 1 00020D
08008 R	Roll	with 08080R, 04288R, and 08033R
08008 R	Flat	
08009 R	Flat	
08012 R	Flat	
08014 R	Flat	w/ 08036R, 08086R, and 08202R
08032 R	Flat	No. 1 Flat C CMCPA
08032 R	Flat	No. 1 Flat C CMCPA
08032 R	Flat	with 08080R, 08008R, and 04288R
08036 R	Flat	w/ 08014R, 08086R, and 08202R
08056 R 08056 R	Flat	with 08077R and 08070R
08058 R	Flat Roll	with 03680R, 03678R, and 03638R
08062 R	Flat	
08063 R	Flat	
08064 R	Flat	
08070 R	Flat	with 08077R and 08056R
08070 R	Roll	
08077 R	Flat	with 08070R and 08056R
08080 R	Flat	
08080 R	Flat	with 08008R, 04288R, and 08032R
08080 R	Roll	
08083 R	Flat	
08083 R 08083 R	Flat	
08083 R 08080 R	Roll	Dovon Pridze Dela 100 contra
08086 R	Flat	Devon Bridge Project 92-3641990
08087 R	Flat /	w/ 08014R, 08036R, and 08202R
08099 R	Flat	CAMPBBUL M.P. 70.19
08099 R	Flat	
08100 R	Roll	
08101 R	Flat	
08150 R	Flat	
08150 R	Roll	
08150 R	Roll	
08202 R	Flat	w/ 08014R, 08036R, and 08086R
08214 R 08214 R	Flat	No. 1 Flat
	Flat	No. 2 Flat
	Doll	
08218 R	Roll Flat	
	Flat	
08218 R 08220 R	the second s	

APPENDIX 20

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COMITION Bridge PLANS(FROM BRIDGE SAFETY HNDEVALVATION IN BASEMENT

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