

PROPOSED IMPROVEMENT BRIDGE PROJECT

TOWNS OF WEYBRIDGE AND NEW HAVEN
COUNTY OF ADDISON

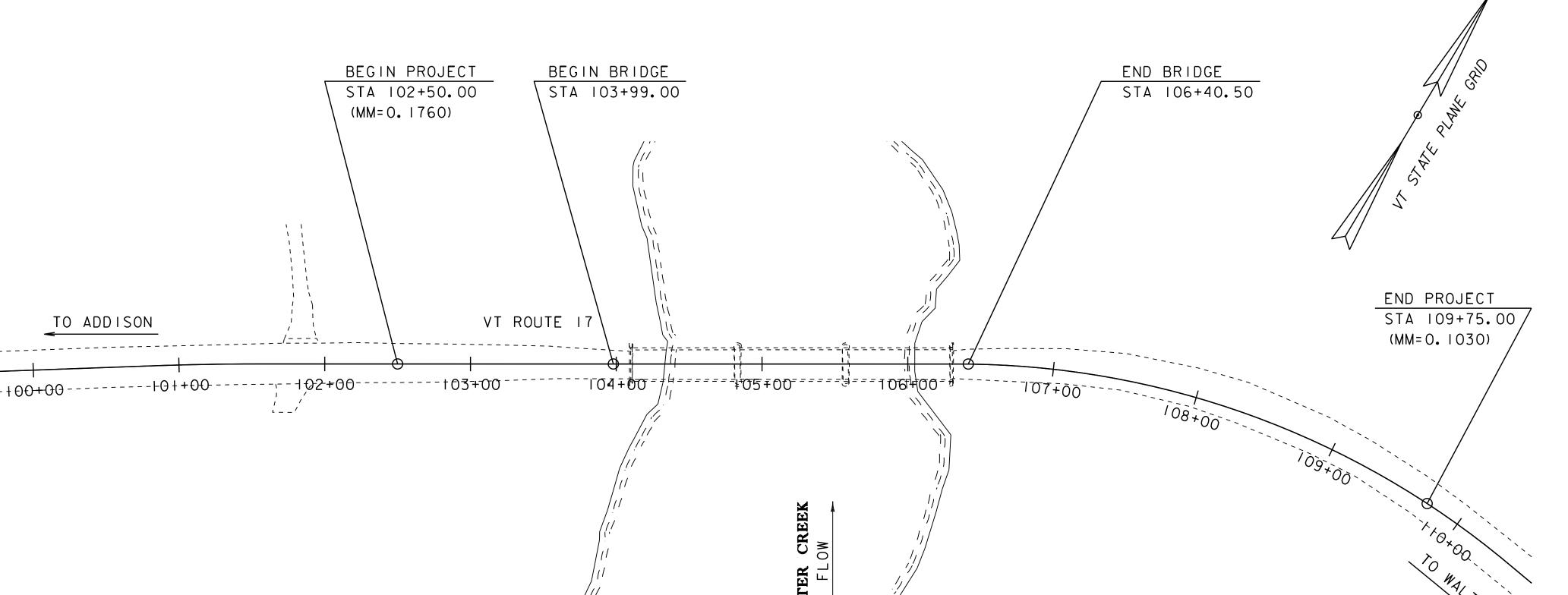
ROUTE NO : VT ROUTE 17 MINOR ARTERIAL BRIDGE NO : 8

PROJECT LOCATION: APPROXIMATELY 3.0 MILES EAST OF THE JUNCTION WITH VT ROUTE 22A

PROJECT DESCRIPTION: FULL REPLACEMENT OF BRIDGE NO. 8 ON VT ROUTE 17 BETWEEN WEYBRIDGE AND NEW HAVEN, OVER OTTER CREEK.

LENGTH OF BRIDGE: LENGTH OF ROADWAY: PROJECT LENGTH:

241.50 FEET 483.50 FEET 725.00 FEET



FINAL PLANS 20-APR-2017

CANADA

commonwealth of

MASSACHUSETTS

'State of NEW HAMPSHIRE

NEW YORK

\WEYBRIDGE-NEW HAVEN

BF 032-1(19)

DIRECTOR OF PROJECT DELIVERY
APPROVED DATE
PROJECT MANAGER : C.W. CARLSON P.E.
PROJECT NAME: WEYBRIDGE-NEW HAVEN PROJECT NUMBER: BF 032-1(19)

SHEET I OF 85 SHEETS

CONSTRUCTION IS TO BE CARRIED ON IN ACCORDANCE WITH THESE PLANS AND THE STANDARD SPECIFICATIONS FOR CONSTRUCTION DATED 2011, AS APPROVED BY THE FEDERAL HIGHWAY ADMINISTRATION ON JULY 20, 2011 FOR USE ON THIS PROJECT, INCLUDING ALL SUBSEQUENT REVISIONS AND SUCH REVISED SPECIFICATIONS AND SPECIAL PROVISIONS AS ARE INCORPORATED IN THESE PLANS.

QUALITY ASSURANCE PROGRAM: LEVEL 2

SURVEYED BY: L. ORVIS
SURVEYED DATE: I-10-2014

DATUM
VERTICAL NAVD88

HORIZONTAL

NAD83 (96)

PRELIMINARY INFORMATION SHEET (BRIDGE)

LRFD

INDEX OF SHEETS PLAN SHEETS TITLE SHEET PRELIMINARY INFORMATION SHEET 1 TYPICAL SECTIONS TYPICALS & DETAILS GENERAL NOTES QUANTITY SHEET 6 - 8 BRIDGE QUANTITY SHEET CONVENTIONAL SYMBOLOGY 12 TIE SHEET 13 - 15 LAYOUT SHEET 16 - 18 PROFILE & BANKING 19 - 21 UTILITY LAYOUT SHEET SIGN LAYOUT SHEET **ACCESS & PARKING LAYOUT** ACCESS & PARKING DETAILS 27 ACCESS & PARKING LANDSCAPING 28 LANDSCAPE PLANTING DETAILS 29 SITE TRAFFIC CONTROL & PCMB DETAILS 30 DETOUR PLAN 31 BORING INFORMATION SHEET 32 - 38 **BORING LOGS** PLAN & ELEVATION DECK TYPICAL 41 DECK REINFORCING PLAN BEGIN/END BRIDGE DETAILS 43 APPROACH SLAB DETAILS 44 DECK FRAMING PLAN 45 GIRDER DETAILS PIER BEARING DETAIL SHEET 47 ABUTMENT 1 PLAN & ELEVATION 48 ABUTMENT 2 PLAN & ELEVATION 49 ABUTMENT REINFORCING DETAILS 50 - 51 WINGWALL DETAILS 52 - 53 PIER DETAILS 54 REINFORCING STEEL SCHEDULE 55 - 56 RAIL DETAIL SHEET VT 17 CROSS SECTIONS MATERIAL TRANSITION & DETAILS 64 - 70 CHANNEL CROSS SECTIONS ACCESS DRIVE CROSS SECTIONS 71 - 72 73 **EPSC NARRATIVE** 74 - 76 EPSC EXISTING LAYOUT 77 - 79 EPSC CONSTRUCTION LAYOUT EPSC FINAL LAYOUT 83 - 85 **EPSC DETAILS**

STRUCTURES & HSD DETAIL SHEETS

SD-501.00	CONCRETE DETAILS AND NOTES	02-09-2012
SD-502.00	CONCRETE DETAILS AND NOTES	10-10-2012
SD-516.10	BRIDGE JOINT ASHPALTIC PLUG	08-29-2011
SD-601.00	STRUCTURAL STEEL DETAILS AND NOTES	06-04-2010
SD-602.00	STRUCTURAL STEEL PLATE GIRDER DETAILS AND NOTES	05-02-2011
HSD-400.01	SAFETY EDGE DETAILS	03-29-2016
HSD-621.06	GUARDRAIL TERMINAL LABEL DETAIL	02-27-2017

	STANDARDS LIST	
G-1	STEEL BEAM GUARDRAIL DETAILS (POST, DELINEATOR, TYPICALS)	03-10-2017
G-1D	STEEL BEAM GUARDRAIL DETAILS (END TERMINAL, ANCHOR, MEDIAN)	03-10-2017
S-360B	GUARDRAIL APPROACH SECTION, GALVANIZED 2 RAIL BOX BEAM	03-10-2017
S-363	THRIE BEAM TO STANDARD STEEL BEAM TRANSITION SECTION	02-02-2017
S-364A	BRIDGE RAILING, GALVANIZED 3 RAIL BOX BEAM	02-02-2017
T-1	TRAFFIC CONTROL GENERAL NOTES	04-25-2016
T-2	TRAFFIC SIGN GENERAL NOTES	04-25-2016
T-10	CONVENTIONAL ROADS CONSTRUCTION APPROACH SIGNING	08-06-2012
T-36	CONSTRUCTION ZONE LONGITUDINAL DROP-OFFS FOR PAVING	08-06-2012
T-40	DELINEATORS AND MILEPOSTS	01-02-2013
T-42	BRIDGE NUMBER PLAQUE	04-09-2014
T-44	MILEMARKER DETAILS STATE AND TOWN HIGHWAYS	04-09-2014
T-45	SQUARE TUBE SIGN POST AND ANCHOR	01-02-2013
T-56	STANDARD SIGN PLACEMENT	10-26-2015
T-94	TOWN & COUNTY LINE SIGNS	02-12-2010

HYDROLOG	IC DATA	Date: November 20	016	PROF	POSED STRUCT	JRE
DRAINAGE AREA: 858 sc				STRUCTURE TYPE:	2-span steel beam	bridge
CHARACTER OF TERRAIN:	Mixture fields and fo					
STREAM CHARACTERISTICS		id alluvial		CLEAR SPAN(NORM/	•	
NATURE OF STREAMBED :	Sand and gravel			VERTICAL CLEARAN		1BED:
				WATERWAY OF FULL	_ OPENING:	
PEAK FLOW DATA - ANNUAL	EXCEEDANCE PRO	BABILITY (AEP)				
				WATER SURFACE EL	_EVATIONS AT:	
43% = <u>5,500 cfs</u>	2% =	12,100 cfs				
10% = 8,900 cfs	1% =	13,600 cfs		43% AEP = 138.4'		VELC
4% = <u>10,500 cfs</u>	0.2% =	17,300 cfs		10% AEP = 145.2'		
				4% AEP = <u>146.2'</u>		
DATE OF FLOOD OF RECORD	D : Unknown			2% AEP = 147.2'		
ESTIMATED DISCHARGE:	Unknown			1% AEP = <u>148.0'</u>		
WATER SURFACE ELEV.:	Unknown					
NATURAL STREAM VELOCITY	/ : <u>@</u> 2% AEP = 4.2'			IS THE ROADWAY OV	/ERTOPPED BELOV	√ 1% AE
ICE CONDITIONS :	Moderate			FREQUENCY:	N/A	
DEBRIS:	Light			RELIEF ELEVATION:	151.3'	
DOES THE STREAM REACH N	MAXIMUM HIGHWATE	ER ELEV. RAPIDLY?	No	DISCHARGE OVER R	OAD @ 1% AEP:	None
IS ORDINARY RISE RAPID?	No					
IS STAGE AFFECTED BY UPS	STREAM OR DOWNS	TREAM CONDITIONS	S? No	BRIDGE LOW CHORD	ELEVATION:	
IF YES, DESCRIBE:				FREEBOARD:	<u>@</u> 2% AE	P = 1.2'
				SCOUR: Abutme	ent and pier to be plac	ed on lea
WATERSHED STORAGE:	2% HEADV	VATERS:		hadn't been done, so s		
	UNIFOR	RM:	X	REQUIRED CHANNEL	PROTECTION:	Stone
	IMMED	IATELY ABOVE SITE	:			
				PERM	IIT INFORMATIO	N
FXISTING ST	TRUCTURE INFO	RMATION				

EXISTING STRUCTURE INFORMATION

STRUCTURE TYPE:	3-span steel beam	bridge		
YEAR BUILT:	1934	•		
CLEAR SPAN(NORMA	L TO STREAM):	217'		
VERTICAL CLEARANC	E ABOVE STREAM	IBED:	22'	
WATERWAY OF FULL	OPENING:	3690 sq. ft.		
DISPOSITION OF STRU	JCTURE:	Remove and re	eplace	
TYPE OF MATERIAL UI	NDER SUBSTRUCT	URE:	See borings	
WATER SURFACE ELEVATIONS AT:				

43% AEP =	138.5'	VELOCITY =	4.8 fps
10% AEP =	145.2'	"	3.8 fps
4% AEP =	146.3'	"	4.1 fps
2% AEP =	147.3'	11	4.4 fps
1% AEP =	148.0'		4.8 fps

LONG TERM STREAMBED CHANGES: None noted

IS THE ROADWAY OVE	ERTOPPED BELOW 1% AEP:	No
FREQUENCY:	N/A	
RELIEF ELEVATION:	151.3'	

UPSTREAM STRUCTURE

DISCHARGE OVER ROAD @ 1% AEP: None

TOWN: Weybridg	ge	_ DISTANCE:	30,750'
HIGHWAY#:	TH 2	STRUCTURE #:	7
CLEAR SPAN:	162'	CLEAR HEIGHT:	
YEAR BUILT:	1970	FULL WATERWAY:	
STRUCTURE TYPE:	3-span steel beam bridge	_	

DOWNSTREAM STRUCTURE

TOWN: Vergenn	es	DISTANCE:	35,100'
HIGHWAY#:	VT 22A	STRUCTURE #:	27
CLEAR SPAN:	338'	CLEAR HEIGHT:	
YEAR BUILT:	1934, recontructed 1969	FULL WATERWAY:	
STRUCTURE TYPE:	5-span rooled beam bridge		

	LRFR LOAD RATING FACTORS						
OADING LEVELS				TRUCK			
OADING LLVLLS	H-20	HL-93	3S2	6 AXLE	3A. STR.	4A. STR.	5A SEMI
TONNAGE	20	36	36	66	30	34.5	38
NVENTORY	3.57	1.08	************			*******************************	
POSTING							
OPERATING	4.64	1.4	3.19	1.93	3.22	2.84	2.86
COMMENTS:							

	OSED SI	TRUCTUR	F
FINOR	USED S		

CLEAR SPAN(NORMAL TO STREAM):	237'
VERTICAL CLEARANCE ABOVE STREAMBED:	20'
WATERWAY OF FULL OPENING:	3810 sq. ft.

VATER SURFACE ELEVATIONS AT:

FINAL HYDRAULIC REPORT

43% AEP = 1	38.4'	VELOCITY=	4.0 fps
10% AEP = 1	45.2'	II.	3.3 fps
4% AEP = 1	46.2'	u u	3.6 fps
2% AEP = 1	47.2'	u u	3.9 fps
1% AEP = 1	48.0'	u u	4.2 fps

THE ROADWAY OVERTOPPED BELOW 1% AEP: REQUENCY:

DISCHARGE OVER ROAD @ 1% AEP:	None	
BRIDGE LOW CHORD ELEVATION:	148 4	1

	<u> </u>	
SCOUR:	Abutment and pier to be placed on ledge. At time of final hydraulics, boring	ngs
hadn't been do	ne, so scour wasn't calculated.	

Stone Fill, Type II

PERMIT INFORMATION

AVERAGE DAILY FLOW:	-	DEPTH OR ELEVATION:
ORDINARY LOW WATER:	-	-

TEMPORARY BRIDGE REQUIREMENTS

STRUCTURE TYPE:	
CLEAR SPAN (NORMAL TO STREAM):	
VERTICAL CLEARANCE ABOVE STREAMBED:	-
WATERWAY AREA OF FULL OPENING: -	

ADDITIONAL INFORMATION

TRAI	FFIC MA	AINTENA	NCE N	IOTES

- 1. MAINTAIN TRAFFIC ON AN OFF SITE DETOUR.
- 2. TRAFFIC SIGNALS ARE NOT NECESSARY. 3. SIDEWALKS ARE NOT NECESSARY

ORDINARY HIGH WATER:

	DESIGN VALUES		
1.	DESIGN LIVE LOAD		HL-93
2.	FUTURE PAVEMENT	d p:	0.0 INCH
3.	ABUTMENT BEARING TO BEARING LENGTH (TWO SPANS)	L:	240.50 F
	(115.50 - 125.00) FT	_	
4.	MIN. MID-SPAN POS. CAMBER @ RELEASE (PRESTRESSED UNITS)	Δ:	
5.	PRESTRESSING STRAND	f y:	
6.	PRESTRESSED CONCRETE STRENGTH	f 'c:	
7.	PRESTRESSED CONCRETE RELEASE STRENGTH	f' ci:	
8.	CONCRETE, HIGH PERFORMANCE CLASS AA	f ′c∶_	4.0 KSI
9.	CONCRETE, HIGH PERFORMANCE CLASS A	f 'c:	4.0 KSI
10 .	CONCRETE, HIGH PERFORMANCE CLASS B	f 'c:	3.5 KSI
11 .	CONCRETE, CLASS C	f 'c:	3.0 KSI
<mark>12</mark> .	REINFORCING STEEL	f y:	60 KSI
13 .	STRUCTURAL STEEL AASHTO M270 (WEATHERING)	f y:	50 KSI
14 .	NOMINAL BEARING RESISTANCE OF SOIL	q n:_	4.0 KSF
15.	SOIL BEARING RESISTANCE FACTOR (REFER TO AASHTO LRFD)	φ:	
<mark>16</mark> .	NOMINAL BEARING RESISTANCE OF ROCK	q n:	70.0 KSF
17	POCK BEADING DESISTANCE EACTOR (DEEED TO AASHTO LDED)	Ψ.	0.45

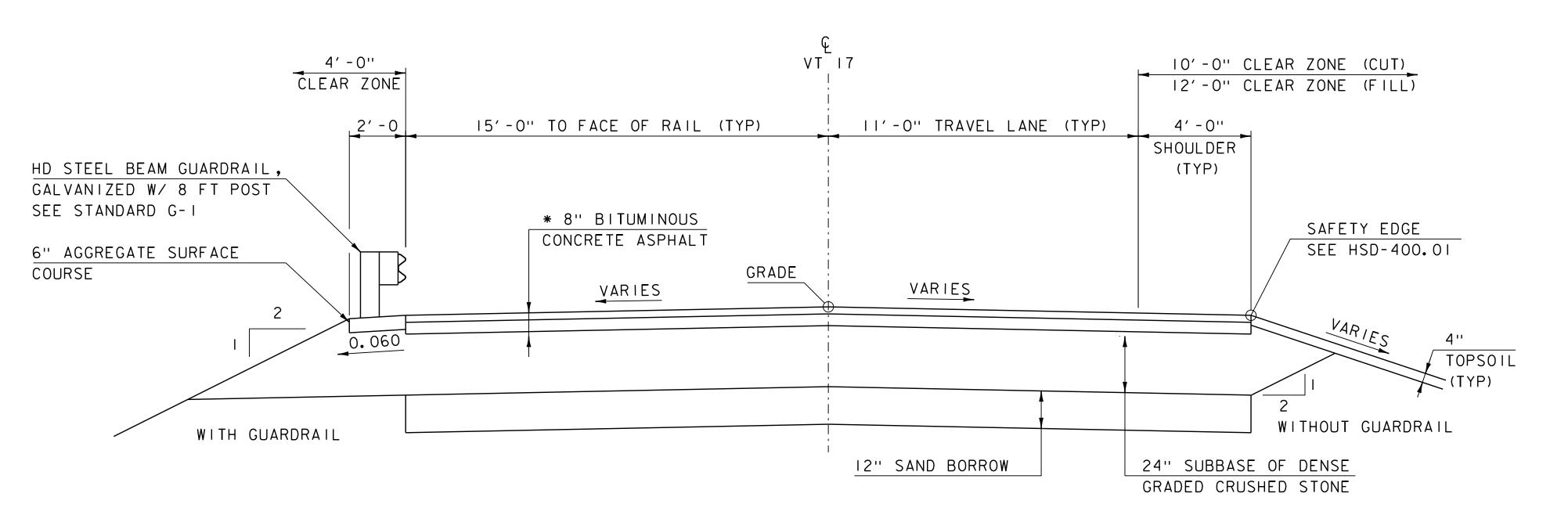
14.	NOMINAL BEARING RESISTANCE OF SOIL	q n:_	4.0 KSF
15.	SOIL BEARING RESISTANCE FACTOR (REFER TO AASHTO LRFD)	φ:	
<mark>16</mark> .	NOMINAL BEARING RESISTANCE OF ROCK	q n:	70.0 KSF
17.	ROCK BEARING RESISTANCE FACTOR (REFER TO AASHTO LRFD)	φ:	0.45
18.	PILE RESISTANCE FACTOR	φ:_	0.65
19.	LATERAL PILE DEFLECTION	Δ:	1.02" INCH
20.	BASIC WIND SPEED	V 3s:	

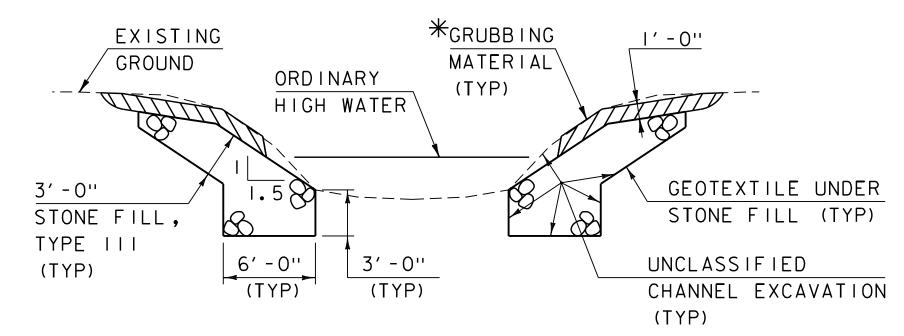
MINIMUM GROUND SNOW L	.OAD		p g:	
SEISMIC DATA	PGA:	0.65	S s:	
		•	S 1:	

PROJECT NAME:	WEYBRIDGE-NEW HAVEN
PROJECT NUMBER:	BF 032-1(19)

FILE NAME:	s12b552pi.dgn	PLOT DAT	E: 4	/20/2017	7	
PROJECT LEADER:	C.W. CARLSON	DRAWN B	Y:	M. LON	IGSTREE	:
DESIGNED BY:	D. PETERSON	CHECKED	BY:	D. PET	ERSON	
PRELIMINARY INFO	RMATION SHEET 1	SHEET	2	OF	85	

TRAFFIC DATA									
					- AS F	BUILT "REBAR" D	ETAIL		
YEAR	ADT	DHV	% D	% T	ADTT	20 year ESAL for flexible pavement from 2017 to 2037 : 1709000	LEVEL I	LEVEL II	LEVEL III
2017	1100	120	54	21.3	190	40 year ESAL for flexible pavement from 2017 to 2057 : 4076700	TYPE:	TYPE:	TYPE:
2037	1200	140	54	30.3	290	Design Speed: 45 mph	GRADE:	GRADE:	GRADE:
2037	1200	140	54	30.3	290	Design Speed: 45 mph	GRADE:	GRADE:	GRADE





TYPICAL CHANNEL SECTION (NOT TO SCALE)

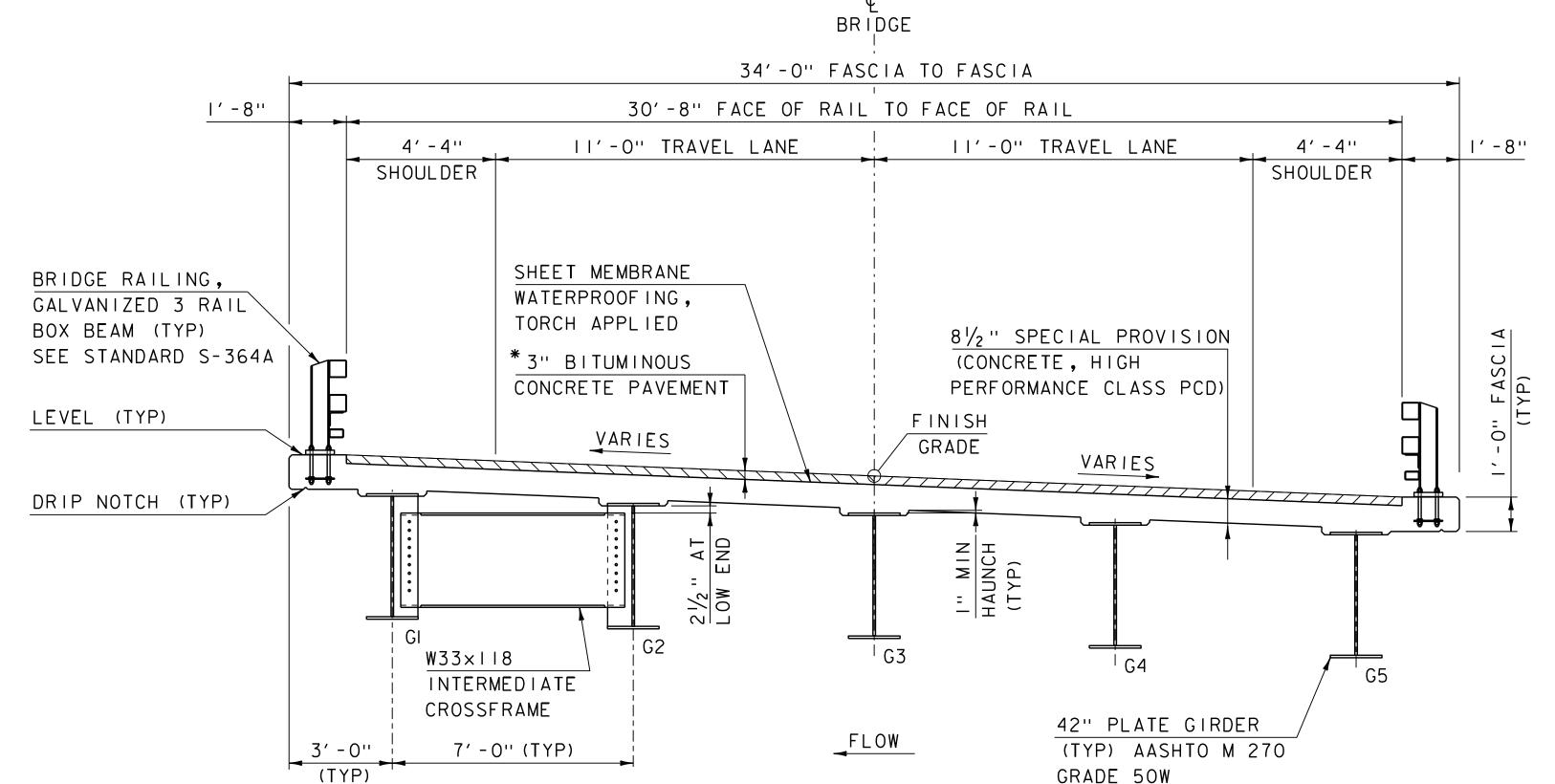
*GRUBBING MATERIAL SHALL NOT BE PLACED WITHIN 3 FEET
OF THE ABUTMENTS UNDER THE BRIDGE. WHENEVER CHANNEL SLOPE
INTERSECTS ROADWAY SUBBASE, GRUBBING MATERIAL SHALL BEGIN
AT THE BOTTOM OF SUBBASE.

ROADWAY TYPICAL SECTION

SCALE: $\frac{3}{8}$ " = 1'-0"

*2 LIFTS OF $1\frac{1}{2}$ " BITUM. CONC. PAVEMENT TYPE IVB OVER 2 LIFTS OF $2\frac{1}{2}$ " BITUM. CONC. PAVEMENT TYPE IIS

TYPE IIS SHALL BE PAID UNDER SPECIAL PROVISION (BITUMINOUS CONCRETE PAVEMENT, SMALL QUANTITY) AND TYPE IVB SHALL BE PAID FOR UNDER SPECIAL PROVISION (BITUMINOUS CONCRETE PAVEMENT, TYPE IVB)



BRIDGE TYPICAL SECTION

SCALE: $\frac{3}{8}$ " = 1'-0"

*2 LIFTS OF $1\frac{1}{2}$ " BITUM. CONC.

PAVEMENT TYPE IVB

TACK COAT: EMULSIFIED ASPHALT IS TO BE APPLIED AT A RATE OF 0.025 GAL/SY BETWEEN SUCCESSIVE COURSES OF PAVEMENT AND 0.080 GAL/SY ON COLD PLANED SURFACES AS DIRECTED BY THE ENGINEER.

JOINT SEALER.

<u>¾"</u> SAW CUT*Ж* JOINT SEALER, HOT OR COLD POURED. SHALL BE SLIGHTLY OVER FILLED I/4'' MIN. WIPE THEN WIPED FLUSH WITH A "V" OR ZONE (TYP) "U" SHAPED SQUEEGEE TO PROVIDE A I/4" WIPE ZONE EACH SIDE OF ROADWAY SURFACE JOINT. ASPHALTIC PLUG JOINT BINDER MAY BE USED AS A SUBSTITUTE JOINT SEALER PAVEMENT SURFACES TOP COURSE OF PAVEMENT TO BE SANDBLASTED ON BOTH SIDES OF JOINT $\frac{7}{8}$ "Ø HEAT RESISTANT FOAM BACKER ROD. $\frac{1}{4}$ WIDE × $\frac{1}{2}$ DEEP SAW CUT INTO COMPRESSION FIT REQUIRED TO ENSURE BOTTOM COURSE OF PAVEMENT TO THAT THE ROD POSITION IS MAINTAINED DURING FILLING OPERATION. COST TO BE BE MADE DURING THE SAME WORKDAY AS PLACEMENT. INCLUDED WITH UNIT PRICE BID FOR

SAWED PAVEMENT JOINT DETAIL

(NOT TO SCALE)

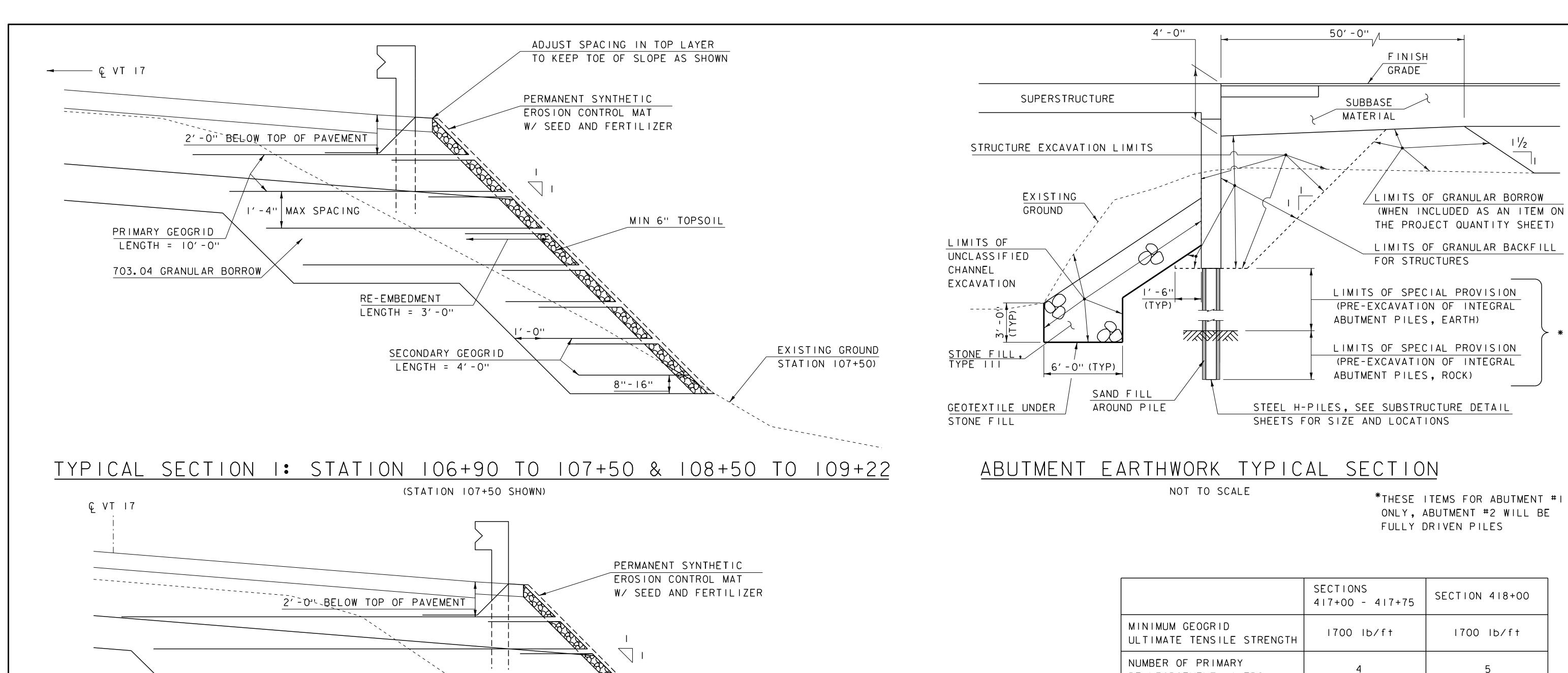
JOINT IS TO BE LOCATED ACCURATELY BY STRING LINING, OR OTHER MEANS, PRIOR TO PAVING, SO THAT THE SAW CUTS WILL BE MADE DIRECTLY OVER THE END OF CONCRETE DECK. JOINT SHALL BE CUT DRY IN A SINGLE PASS AND BE SEALED WITHIN 24 HOURS OR PRIOR TO EXPOSURE TO TRAFFIC. JOINT SHALL BE CLEANED PRIOR TO APPLYING THE JOINT SEALER.

MATERIAL TOLERANO	CES
(IF USED ON PROJECT)	
SURFACE	
- PAVEMENT (TOTAL THICKNESS)	+/- 1/4"
- AGGREGATE SURFACE COURSE	+/- 1/2"
SUBBASE	+/- "
SAND BORROW	+/- "

PROJECT NAME: WEYBRIDGE-NEW HAVEN PROJECT NUMBER: BF 032-1(19)

FILE NAME: sl2b552typ.dgn
PROJECT LEADER: C.W. CARLSON
DESIGNED BY: D. PETERSON
TYPICAL SECTIONS

PLOT DATE: 20-APR-2017
DRAWN BY: M. LONGSTREET
CHECKED BY: D. PETERSON
SHEET 3 OF 85



MIN 6" TOPSOIL

	417+00 - 417+75	SECTION 418+00
MINIMUM GEOGRID ULTIMATE TENSILE STRENGTH	1700 lb/ft	1700 lb/f†
NUMBER OF PRIMARY REINFORCEMENT LAYERS	4	5
LENGTH OF PRIMARY REINFORCEMENT	IO FT	I8 FT
MINIMUM NUMBER OF SECONDARY REINFORCEMENT LAYERS	3	3
LENGTH OF SECONDARY REINFORCEMENT	4 FT	4 FT
VERTICAL SPACING OF SECONDARY REINFORCEMENT	l6 in	16 in
TOP REINFORCEMENT LAYER LOCATION	2 FT BELOW PAVMNT SURFACE	2 FT BELOW PAVMNT SURFACE

TYPICAL SECTION 2: STATION 107+50 TO 108+50

I'-4" MAX SPACING

703.04 GRANULAR BORROW

PRIMARY GEOGRID LENGTH = 18'-0"

(STATION 108+00 SHOWN)

RE-EMBEDMENT

LENGTH = 3'-0"

SECONDARY GEOGRID

LENGTH = 4'-0"

PROJECT NAME: WEYBRIDGE-NEW HAVEN
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FILE NAME: sl2b552typ.dgn
PROJECT LEADER: C.W. CARLSON
DESIGNED BY: D. PETERSON
TYPICALS & DETAILS

EXISTING GROUND

STATION 108+00

PLOT DATE: 20-APR-2017
DRAWN BY: M. LONGSTREET
CHECKED BY: D. PETERSON
SHEET 4 OF 85

GENERAL

- 1. ALL MATERIALS AND CONSTRUCTION SHALL CONFORM TO THE VERMONT AGENCY OF TRANSPORTATION 2011 STANDARD SPECIFICATIONS FOR CONSTRUCTION, THE 2014 AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, AND THEIR LATEST REVISIONS.
- 2. ALL DIMENSIONS ARE HORIZONTAL OR VERTICAL, AND ARE GIVEN AT 68 DEGREES FAHRENHEIT, UNLESS NOTED OTHERWISE.

TRAFFIC CONTROL

- 3. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND IMPLEMENTATION OF A SITE SPECIFIC TRAFFIC CONTROL PLAN FOR ALL STAGES OF CONSTRUCTION. THE PLAN SHALL CLEARLY DETAIL HOW TRAFFIC WILL BE MAINTAINED. THE PLAN SHALL SPECIFY ALL CONSTRUCTION ACTIVITIES REQUIRING ALTERNATING ONE WAY TRAFFIC, RELATE THOSE ACTIVITIES TO THE CONSTRUCTION SCHEDULE, AND SHOW APPROPRIATE TEMPORARY TRAFFIC CONTROL. ALL COSTS WILL BE INCLUDED IN ITEM 900.645 "SPECIAL PROVISION (TRAFFIC CONTROL, ALL-INCLUSIVE)".
- 4. THE CONTRACTOR MAY SUBMIT A SITE SPECIFIC TRAFFIC CONTROL PLAN FOR THE WORK REQUIRED TO INSTALL THE PIER, TO BE APPROVED AS A SEPARATE SUBMITTAL FROM THE OVERALL TCP PLAN.
- 5. ALL ITEMS REQUIRED TO IMPLEMENT THE CONTRACTOR'S TRAFFIC CONTROL PLAN WILL NOT BE PAID FOR DIRECTLY BUT WILL BE INCLUDED IN THE BID PRICE FOR ITEM 900.645, "SPECIAL PROVISION (TRAFFIC CONTROL, ALL-INCLUSIVE)".
- 6. VT ROUTE 17 WILL BE CLOSED AT THE BRIDGE FOR THE ENTIRE BRIDGE CLOSURE PERIOD (BCP). A SIGNED DETOUR SHALL BE PROVIDED AS SHOWN IN THE PLANS. PAYMENT FOR THE DETOUR SIGNS SHALL BE INCLUDED IN ITEM 900.645, "SPECIAL PROVISION (TRAFFIC CONTROL, ALL-INCLUSIVE)". PORTABLE CHANGEABLE MESSAGE SIGNS SHALL BE PAID FOR SEPERATELY UNDER ITEM 641.15.
- 7. ALL SIGNING SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE MUTCD. FOR ADDITIONAL SIGNING INSTRUCTIONS SEE THE T SERIES OF THE STANDARD DRAWINGS. WHERE CONFLICTS EXIST, THE MUTCD SHALL GOVERN.

EARTHWORK

- 8. REMOVAL OF THE EXISTING STRUCTURE WILL BE PAID FOR UNDER ITEM 529.15, "REMOVAL OF STRUCTURE". THIS WORK SHALL INCLUDE REMOVAL OF THE EXISTING SUPERSTRUCTURE AS WELL AS ANY PORTIONS OF THE EXISTING ABUTMENTS THAT FALL OUTSIDE THE LIMITS OF STRUCTURE EXCAVATION OR UNCLASSIFIED CHANNEL EXCAVATION AND THE REMOVAL OF THE EXISTING PIERS TO STREAMBED ELEVATION. ANY EXISTING STEEL PLATES FOUND ON THE BRIDGE DECK SHALL REMAIN THE PROPERTY OF STATE OF VERMONT AND SHALL BE DELIVERED TO THE VTRANS NEW HAVEN DISTRICT GARAGE.
- 9. THE "STONE FILL, TYPE III" UNDER THE BRIDGE AS SHOWN IN THE PLANS SHALL BE PLACED BEFORE THE NEW SUPERSTRUCTURE IS SET.
- 10. THE CONTRACTOR MAY SUBSTITUTE SUBBASE MATERIAL FOR THE SAND BORROW SHOWN IN THE MATERIALS TRANSITION. THE SUBBASE MATERIAL SHALL BE THE TYPE SPECIFIED IN THE CONTRACT AND SHALL BE PLACED TO MEET THE SUBBASE SPECIFICATIONS. IF SUBBASE IS PLACED IN LIEU OF SAND BORROW, A GEOTEXTILE MEETING THE REQUIREMENTS OF ITEM 649.11 "GEOTEXTILE FOR ROAD BED SEPARATOR" SHALL BE PLACED BETWEEN THE SUBGRADE AND SUBBASE MATERIAL. ALL COSTS ASSOCIATED WITH THE SUBSTITUTION INCLUDING THE GEOTEXTILE WILL BE CONSIDERED INCIDENTAL TO 203.31 "SAND BORROW".
- 11. TEMPORARY CONSTRUCTION FILLS WITHIN THE WATERCOURSE, IN ACCORDANCE WITH THE PROJECT PLANS, SHALL CONSIST OF CLEAN STONE FILL ONLY. NO OTHER FILLING IN THE STREAM SHALL OCCUR WITHOUT PRIOR WRITTEN APPROVAL OF THE STREAM ALTERATION ENGINEER AND THE ARMY CORPS OF ENGINEERS, VERMONT PROJECT OFFICE.
- 12. THE TEMPORARY CAUSEWAY, TO INSTALL THE NEW PIER, SHALL BE REMOVED TO ORIGINAL GROUND PRIOR TO THE MARCH 15TH IN-STREAM WORK RESTRICTION PER PERMIT #. A CAUSEWAY CAN THEN BE INSTALLED AFTER THE END OF THE IN-STREAM RESTRICTION DATE OF JUNE 15TH. PAYMENT FOR EACH CAUSEWAY WILL BE MADE UNDER ITEM 900.620 SPECIAL PROVISION (TEMPORARY CAUSEWAY).

CONCRETE

- 13. WATER REPELLENT, SILANE SHALL BE APPLIED TO ALL EXPOSED CONCRETE ON THE BRIDGE SUPERSTRUCTURE AND SUBSTRUCTURE, EXCEPT FOR THE BOTTOM OF THE DECK BETWEEN THE DRIP NOTCHES.
- 14. ITEM 900.608 SPECIAL PROVISION (CONCRETE HIGH PERFORMANCE, CLASS PCD)(FPQ): USE FOR THE DECK, INTEGRAL ABUTMENT CURTAIN WALL AND WINGWALLS ABOVE THE PILE CAP CONSTRUCTION JOINT.
- 15. CONCRETE FOR THE APPROACH SLAB CLOSURE POURS AND ABUTMENT PILE CAVITIES SHALL MEET THE REQUIREMENTS OF ITEM 900.608, "SPECIAL PROVISION (HIGH PERFORMANCE CONCRETE, RAPID SET)(FPQ)".
- 16. THE CONCRETE EDGES ALONG THE LONGITUDINAL CLOSURE POURS OF THE APPROACH SLABS SHALL BE TREATED TO PROVIDE A ROUGHENED/ EXPOSED AGGREGATE SURFACE. THE AMPLITUDE OF THE EXPOSED AGGREGATE SHALL BE A MINIMUM OF 1/8". THE FABRICATOR SHALL INDICATE THE METHOD USED TO ACHIEVE THIS SURFACE ON THE FABRICATION DRAWINGS AND METHOD USED TO PROTECT THE REINFORCING STEEL.
- 17.ALL LIFTING POINTS IN THE SUPERSTRUCTURE SHALL BE REMOVABLE TO THE MINIMUM CLEAR COVER FOR REINFORCING STEEL SPECIFIED IN THE PLANS. THE LIFTING POINTS SHALL BE DETAILIED IN THE APPROPRIATE FABRICATION DRAWING. PAYMENT FOR THIS WORK WILL BE CONSIDERED INCIDENTAL TO THE APPROPRIATE PRECAST ITEM.
- 18.ALL RECESSED LIFTING POINTS, DOWEL DUCTS, AND GROUT PAD BENEATH THE PIER CAP SHALL BE FILLED WITH A TYPE IV MORTAR PER SUBSECTION 707.03 AND WILL BE CONSIDERED INCIDENTAL TO THE APPROPRIATE PRECAST ITEM. A MINIMUM COMPRESSIVE STRENGTH OF 4000 PSI, AS DETERMINED BY FIELD-CURED TEST CYLINDERS, SHALL BE ACHIEVED IN THE MORTAR PRIOR TO LOADING.
- 19. ALL FORM SUPPORTS AND FORM TIES THAT ARE TO REMAIN PERMANENTLY IN THE CONCRETE ABOVE THE BRIDGE SEAT SHALL BE GALVANIZED AND CONFORM TO SECTION 726 OF THE STANDARD SPECIFICATIONS.

REINFORCING STEEL

- 20. ALL REINFORCING STEEL IN THE PRECAST APPROACH SLABS, ABUTMENTS, AND WINGWALLS SHALL MEET THE REQUIREMENTS FOR LEVEL III CORROSION RESISTANCE IN ACCORDANCE WITH SECTION 507 AND ARE MARKED WITH A ".3" IN THEIR SUFFIX. PAYMENT FOR ALL APPROACH SLAB AND PRECAST ABUTMENT REINFORCING WILL BE MADE UNDER THE APPROPRIATE SECTION 540 CONTRACT ITEM. THE ADDITIONAL LONGITUDINAL STEEL IN THE APPROACH SLAB CLOSURE POURS SHALL BE LEVEL III AND BE INCIDENTAL TO THE PRECAST APPROACH SLAB CONTRACT ITEMS.
- 21. TEST BARS FOR THE PRECAST UNITS SHALL BE PROVIDED IN ACCORDANCE WITH THE "VERMONT AGENCY OF TRANSPORTATION MATERIAL SAMPLING MANUAL" AVAILABLE ON THE AGENCY WEBSITE.
- 22. ALL GROUTED COUPLERS FOR BAR REINFORCEMENT SHALL MEET THE REQUIREMENTS OF SECTION 507. EPOXY COATED MECHANICAL SPLICES SHALL BE ALLOWED FOR LEVEL III REINFORCEMENT CONNECTIONS.

- 23. GROUT FOR GROUTED COUPLERS FOR BAR REINFORCEMENT SHALL BE APPROVED BY THE SPLICE MANUFACTURER. THE CONTRACTOR SHALL SUBMIT A GROUTING PROCEDURE PROPOSAL TO THE ENGINEER, INCLUDING A PREMIX NAME BRAND FOR APPROVAL.
- 24. A TEMPLATE SHALL BE USED FOR THE LAYOUT OF GROUTED COUPLERS FOR BAR REINFORCEMENT. THE SAME TEMPLATE SHALL BE USED FOR MATCHING FACES OF EACH CONNECTION.
- 25. ALL REINFORCING STEEL IN THE CAST-IN-PLACE PORTION OF THE PIER SHALL MEET THE REQUIREMENTS FOR LEVEL I CORROSION RESISTANCE. REINFORCING STEEL EXTENDING INTO AND IN THE PRECAST PIER CAP SHALL MEET THE REQUIREMENT FOR LEVEL III CORROSION RESISTANCE.
- 26. REINFORCING STEEL IN THE DECK WILL INCLUDE BOTH LEVEL III AND GLASS FIBER REINFORCED POLYMER (GFRP) REINFORCING BARS. BARS MARKED ".G" IN THEIR SUFFIX INDICATE GFRP BARS.
- 27. THE TOP MAT OF GFRP REINFORCING BARS IN THE DECK SHALL BE TIED DOWN TO AVOID FLOATING DURING DECK CASTING. PAYMENT SHALL BE INCIDENTAL TO ITEM 900.640, "SPECIAL PROVISION (REINFORCING BAR, GFRP) (#5)" OR ITEM 900.640, "SPECIAL PROVISION (REINFORCING BAR, GFRP) (#6)", AS APPROPRIATE.
- 28. MINIMUM CLEAR COVER FOR REINFORCING STEEL SHALL BE AS FOLLOWS:

ALONG BACK FACES OF WALLS AGAINST EARTH: 2 INCH ALONG TOP SURFACE OF DECK SLAB: 1.5 INCH ALONG BOTTOM SURFACE OF DECK SLAB: 1.5 INCH ELSEWHERE UNLESS OTHERWISE INDICATED: 3 INCH

29. GFRP REINFORCING BAR DESIGN VALUES:

a) MINIMUM TENSILE STRENGTH

MINIMUM TENSILE STRENGTH 100,000 PSI MINIMUM ELASTIC MODULUS 6,150,000 PSI

PRECAST ABUTMENTS AND PIER

- 30. THE UNIT PRICE FOR EACH PRECAST ABUTMENT SHALL INCLUDE THE ASSOCIATED WINGWALLS, AND ALL LABOR AND MATERIALS TO CONNECT WINGWALLS TO THE PILE CAPS. THIS WORK WILL BE PAID FOR UNDER THE APPROPRIATE PRECAST CONCRETE STRUCTURE ABUTMENT PAY ITEM.
- 31. CORRUGATED STEEL PIPES IN THE PRECAST ABUTMENTS FOR PILE CAVITIES AND ANCHOR BOLT CAVITIES SHALL MEET THE REQUIREMENTS OF SUBSECTION 711.01, BE COATED IN ACCORDANCE WITH AASHTO M 218, AND BE TYPE 1. ALL COSTS ASSOCIATED WITH PLACING THE CORRUGATED STEEL PIPES SHALL BE INCLUDED IN THE BID PRICE FOR EACH 540.10 AND 900.645, "SPECIAL PROVISION, (CONTRACTOR-FABRICATED PRECAST CONCRETE STRUCTURE)" CONTRACT ITEM AS APPROPRIATE.
- 32. CORRUGATED POST-TENSIONING DUCTS IN THE PRECAST APPROACH SLABS AND PRECAST PIER CAP FOR ANCHOR BOLT AND DOWEL CONNECTIONS SHALL BE CONSTRUCTED FROM EITHER POLYETHYLENE OR POLYPROPYLENE. THE DUCT SHALL HAVE A MINIMUM MATERIAL THICKNESS OF 0.080 IN. +/- 0.010 IN. AND SHALL HAVE A WHITE COATING ON THE OUTSIDE OR SHALL BE OF WHITE MATERIAL WITH ULTRAVIOLET STABILIZERS ADDED. POLYETHYLENE DUCT SHALL BE FABRICATED FROM RESINS MEETING OR EXCEEDING THE REQUIREMENTS OF ASTM D 3350 WITH A CELL CLASSIFICATION OF 345464A. POLY PROPYLENE DUCT SHALL BE FABRICATED FROM RESINS MEETING OR EXCEEDING THE REQUIREMENTS OF ASTM D 4101 WITH A CELL CLASSIFICATION RANGE OF PP0340B44544 TO PP0340B65884. ALL COSTS ASSOCIATED WITH PLACING THE DUCTS SHALL BE INCLUDED IN THE BID PRICE FOR EACH 540.10 AND 900.640, "SPECIAL PROVISION, (CONTRACTOR-FABRICATED PRECAST CONCRETE STRUCTURE)" CONTRACT ITEM AS APPROPRIATE.
- 33. AN OPTIONAL VERTICAL CONSTRUCTION JOINT IS SHOWN FOR ABUTMENTS #1 AND #2. THIS IS ALLOWED TO REDUCE THE WEIGHT OF THE ABUTMENTS FOR HANDLING, IF NECESSARY. ALL COSTS ASSOCIATED WITH CONSTRUCTING THE VERTICAL CONSTRUCTION JOINT WILL BE INCLUDED IN THE COST OF THE ASSOCIATED CONTRACT ITEM. REINFORCING DOES NOT NEED TO BE CONTINUOUS BETWEEN ADJACENT PILE CAPS PIECES. A GROUTED SHEAR KEY IS REQUIRED BETWEEN THE PIECES AND THE DETAILS ARE INCLUDED IN THE PLANS.
- 34. BACKFILL SHALL NOT BE COMPLETED UNTIL SPLICE CONNECTOR GROUT HAS REACHED 85% OF THE MANUFACTURER SPECIFIED STRENGTH.

STRUCTURAL STEEL

- 35. SOME OF THE EXISTING STRUCTURAL STEEL IS PAINTED WITH A MATERIAL THAT MAY CONTAIN LEAD. THE CONTRACTOR SHALL FOLLOW ALL APPLICABLE REGULATIONS WHEN HANDLING AND WORKING WITH THIS STEEL. THE REMOVED STRUCTURAL STEEL IS THE PROPERTY OF THE CONTRACTOR. THE CONTRACTOR SHALL INDEMNIFY AND HOLD THE STATE, ITS OFFICERS, AND EMPLOYEES HARMLESS CONCERNING THE CONTRACTOR'S USE OR DISPOSITION OF THE REMOVED EXISTING STRUCTURAL STEEL.
- 36. UNLESS OTHERWISE NOTED, ALL NEW STRUCTURAL STEEL SHALL CONFORM TO AASHTO M270 GRADE 50W AND SHALL BE PAID FOR UNDER ITEM 506.55, "STRUCTURAL STEEL (PLATE GIRDER)".
- 37. FLEMING BRACKETS OR SIMILAR FALSEWORK SHALL BE SPACED AS REQUIRED BY DESIGN, BUT SHALL BE LIMITED TO A MAXIMUM SPACING OF 4 FEET. THE DESIGN OF FALSEWORK SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.
- 38. ANY BOLT HOLES IN THE WEBS OF FASCIA GIRDERS NOT OTHERWISE FILLED SHALL BE FILLED WITH BUTTON HEAD OR HEX HEAD BOLTS. THE BOLTS SHALL BE TIGHTENED IN ACCORDANCE WITH SUBSECTION 506.19 OF THE STANDARD SPECIFICATIONS.
- 39. STRUCTURAL STEEL MEMBERS DESIGNATED "CVN" IN THE PLANS SHALL BE CHARPY V-NOTCH TESTED IN ACCORDANCE WITH SUBSECTION 714.01 OF THE STANDARD SPECIFICATIONS.
- 40. AFTER SUPERSTRUCTURE STEEL HAS BEEN ERECTED, ELEVATIONS ALONG THE TOP OF THE GIRDERS SHALL BE TAKEN AS DIRECTED BY THE ENGINEER FOR USE IN DETERMINING FINISHED GRADES.
- 41. ALL WELDING SHALL CONFORM TO THE PROVISIONS OF SUBSECTION 506.10.
- 42. GIRDER WEBS AND CROSS FRAMES SHALL BE PLUMB IN FINAL POSITION.

H-PILES

- 43. FOR ESTIMATING PURPOSES, THE PILE TIP ELEVATIONS WERE ASSUMED AS SHOWN ON THE BORING LOGS. THE ACTUAL IN PLACE LENGTHS MAY VARY.
- 44. THE PILES SHALL BE HP 12 X 84.
- 45. TO PREVENT DAMAGE TO THE PILES, PILE SHOES ARE REQUIRED FOR DRIVEN PILES AND SHALL CONFORM TO SUBSECTION 505.04 (f).

- 46. THE PILE LOCATIONS AT ABUTMENT 1 SHALL BE PRE-EXCAVATED A MINIMUM OF THREE (3) FEET INTO COMPETENT BEDROCK. THE MINIMUM REQUIRED PILE LENGTH AT ABUTMENT 1 IS 14 FEET BELOW THE BOTTOM OF THE PILE CAP. PRE-EXCAVATED HOLES SHALL BE A MINIMUM OF 24 INCHES IN DIAMETER. THE PILES AT ABUTMENT 1 SHALL BE SEATED ON THE BEDROCK WITH A PILE DRIVING IMPACT HAMMER TO A NOMINAL AXIAL PILE DRIVING RESISTANCE OF 284 KIPS.
- 47. THE PILES AT ABUTMENT 2 SHALL BE DRIVEN TO A NOMINAL AXIAL PILE DRIVING RESISTANCE (RNDR) OF 360KIPS, PROVIDED A MINIMUM PENETRATION OF 39 FEET BELOW THE BOTTOM OF PILE CAP HAS BEEN ACHIEVED AND THE PILE IS SEATED ON BEDROCK.
- 48.A MINIMUM OF TWO DYNAMIC PILE TESTS SHALL BE CONDUCTED ON PILES AT ABUTMENT 2. NO LOAD TESTING IS REQUIRED AT ABUTMENT 1.
- 49. THE TOPS OF THE PILES AFTER DRIVING OR PLACEMENT SHALL NOT VARY FROM THE POSITION SHOWN ON THE PLANS BY MORE THAN 3 INCHES. THE PILE ORIENTATION SHALL NOT VARY BY MORE THAN 5 DEGREES. THE CONTRACTOR SHALL DEMONSTRATE HOW THE TOLERANCES WILL BE MET TO THE SATISFACTION OF THE ENGINEER. THESE MEASURES SHALL BE DEMONSTRATED IN A SUBMITTAL TO BE ACCEPTED BEFORE PILE DRIVING COMMENCES.
- 50. PAYMENT FOR PRE-EXCAVATION WILL BE CONSIDERED INCIDENTAL TO ITEM 900.640, "SPECIAL PROVISION (PRE-EXCAVATION OF INTEGRAL ABUTMENTS PILES, EARTH)" OR ITEM 900.640, "SPECIAL PROVISION (PRE-EXCAVATION OF INTEGRAL ABUTMENTS PILES, ROCK)". THE ENTIRE PRE-EXCAVATED HOLE SHALL BE BACKFILLED WITH SAND AND THE PILES DRIVEN THROUGH THE SAND. SAND SHALL CONFORM TO THE REQUIREMENTS OF SUBSECTION 703.03. REFER TO THE SPECIAL PROVISIONS FOR ADDITIONAL INFORMATION.

<u>PIER</u>

- 51. FOOTINGS FOR SUBSTRUCTURES ON BEDROCK SHALL BE PLACED ON CLEAN COMPETENT ROCK. ALL LOOSE ROCK AND DEBRIS SHALL BE REMOVED
- 52. UPON COMPLETION OF THE EXCAVATION FOR SUBSTRUCTURES FOUNDED ON BEDROCK AND PRIOR TO PLACING FORMWORK, THE ENGINEER SHALL NOTIFY THE PROJECT MANAGER AND GEOTECHNICAL ENGINEERING MANAGER. THE AGENCY GEOLOGIST AND/OR GEOTECHNICAL ENGINEER WILL DETERMINE IF THE BEDROCK IS COMPETENT TO OBTAIN THE NOMINAL BEARING RESISTANCE AS SHOWN ON THE PLANS. FIVE (5) WORKING DAYS FROM NOTIFICATION SHALL BE ALLOWED TO MAKE THE INSPECTION AND THE DETERMINATION FOR THE COMPETENCY OF THE BEDROCK.
- 53. IF NECESSARY, THE LIMITS OF SUBFOOTINGS SHALL BE 1 FT OUTSIDE THE HORIZONTAL LIMITS OF THE FOOTING. ANY CONCRETE REQUIRED FOR SUBFOOTINGS WILL BE PAID UNDER ITEM 541.30, "CONCRETE, CLASS C." AN ESTIMATED QUANTITY OF 17 CY OF ITEM 541.30 HAS BEEN INCLUDED IN THE CONTRACT. THE TOP SURFACE OF THE SUBFOOTING SHALL BE INTENTIONALLY ROUGHENED TO A 1/4 INCH AMPLITUDE.
- 54. THE CAST-IN-PLACE PORTION OF THE PIER MAY BE CONSTRUCTED PRIOR THE BRIDGE CLOSURE. IF THE PIER IS TO BE LEFT INSTALLED THROUGH WINTER AND SPRING, THE STEM SHALL BE CONSTRUCTED TO A MINIMUM ELEVATION OF 144.92.
- 55. A TEMPLATE OF THE DUCT LOCATIONS SHALL BE MADE TO CORRIDINATE INSTALLATION AND FITUP BETWEEN THE CAST-IN-PLACE PORTION OF THE PIER AND THE PRECAST PIER CAP.

REINFORCED SOIL SLOPE

56. FOR INSTALLATION OF THE GUARDRAIL, A MINIMUM OF THE TOP TWO LAYERS OF GEOGRID WILL BE PUNCTURED TO EMBED THE RAIL. IF WOODEN POSTS ARE USED, SECTIONS SHALL BE BOXED OUT IN EACH GEOGRID LAYER DURING INSTALLATION TO ENSURE THE GEOGRID ENDURES NO ADDITIONAL DAMAGE DURING GUARDRAIL INSTALLATION. ALTERNATIVELY, A WEDGE-SHAPED SHOE ATTACHMENT TO FACILITATE INSTALLATION DURING DRIVING IN RAILS COULD ALSO BE SPECIFIED. STEEL POSTS CAN BE DRIVEN THROUGH THE GEOGRID WITHOUT ADVERSELY AFFECTING THE PERFORMANCE OF THE REINFORCEMENT.

MISCELLANEOUS

- 57. THE EXISTING FENCE BETWEEN STATION 102+50 LT AND 104+20 LT HAS BEEN PARTIALLY REMOVED OR FALLEN OVER. THE REMAINING PORTIONS OF THIS FENCE SHALL BE REMOVED AND PAID FOR UNDER ITEM 620.55 "REMOVAL OF EXISTING FENCE".
- 58. AN ADDITIONAL 50 LF OF ITEM 900.640 SPECIAL PROVISION (BARBED WIRE FENCE) HAS BEEN ADDED TO THE PROJECT IF ANY OF THE EXISTING FENCING ON THE EAST SIDE OF THE PROJECT NEEDS TO BE REPLACED.
- 59. THE CONTRACTOR SHALL CONTACT THE AGENCY OF NATURAL RESOURCES FISH & WILDLIFE FACILITIES AND LANDS ADMINISTRATOR PRIOR TO CONSTRUCITON OF THE PARKING ACCESS AREA DN RIVER ACCESS STAIRS.

PROJECT NAME: WEYBRIDGE-NEW HAVEN PROJECT NUMBER: BF 032-1(19)

FILE NAME: sI2b552forms.dgn PLOT DATE: 20-APR-2017
PROJECT LEADER: C.W. CARLSON DRAWN BY: M. LONGSTREET
DESIGNED BY: D. PETERSON CHECKED BY: D. PETERSON
GENERAL NOTES SHEET 5 OF 85

QUANTITY SHEET 1

March Marc		SUN	MARY OF ES	STIMATED QU	ANTITIES				тот	ALS	DESCRIPTIONS			DETAILED SUMMARY OF QUANTITIES
				ROADWAY	LANDSCAPING	EROSION CONTROL	BRIDGE	FULL C.E. ITEMS	GRAND TOTAL	FINAL UNIT	ITEMS	ITEM NUMBER	ROUND	QUANTITIES UNIT ITEMS
				1					1	LS	CLEARING AND GRUBBING, INCLUDING INDIVIDUAL TREES AND STUMPS	201.10		EARTHWORKS SUMMARY
1				2610					2610	CY	COMMON EXCAVATION	203.15		
1				20					20	CY	SOLID ROCK EXCAVATION	203.16		1827 CY COMMON EXCAVATION (2610 x .7) 342 CY UNCLASSIFIED CHANNEL EXCAVATION (1140 x 0.3)
				1140					1140	CY	UNCLASSIFIED CHANNEL EXCAVATION	203.27		78 CY STRUCTURE EXCAVATION (260 x 0.3)
				930					930	CY	SAND BORROW	203.31		
				1					1	CY	TRENCH EXCAVATION OF EARTH, EXPLORATORY (N.A.B.I.)	204.22		
Column C							260		260	CY	STRUCTURE EXCAVATION	204.25		
							80		80	CY	GRANULAR BACKFILL FOR STRUCTURES	204.30		415 CY TOTAL FILL REQUIRED
1							1		1	LS	COFFERDAM	208.40		1835 CY TOTAL WASTE
1				467					467	SY	COLD PLANING, BITUMINOUS PAVEMENT	210.10		
27				1790					1790	CY	SUBBASE OF DENSE GRADED CRUSHED STONE	301.35		N.A.B.I. = NOT A BID ITEM
1				40					40	CY	AGGREGATE SURFACE COURSE	401.10		
				23					23	CWT	EMULSIFIED ASPHALT	404.65		
1				1					1	LU	PRICE ADJUSTMENT, ASPHALT CEMENT (N.A.B.I.)	406.50		
							1		1	LS	FURNISHING EQUIPMENT FOR DRIVING PILING	504.10		
1							279		279	LF	STEEL PILING, HP 12 X 84	505.165		
1000 1000 14 PRE-SCRIBSTELLEREL 20/11 1 1 1 1 1 1 1 1 1							2		2	EACH	DYNAMIC PILE LOADING TEST	505.45		
11:54							335819		335819	LB	STRUCTURAL STEEL, PLATE GIRDER (FPQ)	506.55		
							13030		13030	LB	REINFORCING STEEL, LEVEL I	507.11		
							19134		19134	LB	REINFORCING STEEL, LEVEL III (FPQ)	507.13		
1							80		80	LF	DRILLING AND GROUTING DOWELS	507.16		
1 1 1 1 1 1 1 1 1 1							1		1	LS	SHEAR CONNECTORS (2400 - 7/8" X 7")	508.15		
							20		20	GAL	WATER REPELLENT, SILANE	514.10		
62 62 LF JONT SCALER HOT POLITICED 524.11							62		62	LF	BRIDGE EXPANSION JOINT, ASPHALTIC PLUG	516.10		
485							805		805	SY	SHEET MEMBRANE WATERPROOFING, TORCH APPLIED	519.20		
1 1 2 1 EACH REMOVAL OF STRUCTURE (5000 SF - EST.) 520 15 5 5 EACH BEARNG DEVICE ASSEMBLY, ELASTOWERK: PAD WIEXT, LOAD PLATES 531 18 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1							62		62	LF	JOINT SEALER, HOT POURED	524.11		
							485		485	LF	BRIDGE RAILING, GALVANIZED 3 RAIL BOX BEAM	525.335		
BEGIN OPTION CC							1		1	EACH	REMOVAL OF STRUCTURE (5090 SF - EST.)	529.15		
1 1 LS PRECAST CONCRETE STRUCTURE (ABUTMENT#1) 540.10 1 1 LS SPECIAL PROVISION (ABUIMENT#1) 540.10 END OPTION ED 1 1 LS PRECAST CONCRETE STRUCTURE (ABUTMENT#1) 540.10 END OPTION ED 1 1 LS PRECAST CONCRETE STRUCTURE (ABUTMENT#2) 540.10 1 1 LS SPECIAL PROVISION (ABUTMENT#2) 900.645 END OPTION ED 5 40.10 5 40.10 END OPTION ED 6 5 5 6 10 END OPTION ED 6 5 6 10 6 5 6 10 6 5 6 10 7 5 6 10 8 6 6 10 8 6 6 10 9 7 7 8 6 10 9 8 7 8 7 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8							5		5	EACH	BEARING DEVICE ASSEMBLY, ELASTOMERIC PAD W/ EXT. LOAD PLATES	531.18		
1 1 1 LS SPECIAL PROVISION (ABUTIMENT #1) 900.645 END OPTION CC BFGIN OPTION DD 1 1 1 LS PRECAST CONCRETE STRUCTURE (ABUTIMENT #2) 540.10 END OPTION DD 1 1 LS SPECIAL PROVISION (ABUTIMENT #2) 900.645 END OPTION DD END OPTION DD END OPTION DD SEGIN OPTION AA BEGIN OPTION AA 1 1 LS PRECAST CONCRETE STRUCTURE (APPROACH SLAB #1) 540.10											BEGIN OPTION CC			
END OPTION CC SEGIN OPTION DD SEGIN OPTION AA SEGIN OPTION							1		1	LS	PRECAST CONCRETE STRUCTURE (ABUTMENT #1)	540.10		
							1		1	LS	SPECIAL PROVISION (ABUTMENT #1)	900.645		
1 1 1 LS PRECAST CONCRETE STRUCTURE (ABUTMENT #2) 540.10 1 1 1 LS SPECIAL PROVISION (ABUTMENT #2) 900.645 END OPTION DD BEGIN OPTION AA 1 1 LS PRECAST CONCRETE STRUCTURE (APPROACH SLAB #1) 540.10											END OPTION CC			
											BEGIN OPTION DD			
END OPTION DD BEGIN OPTION AA 1							1		1	LS	PRECAST CONCRETE STRUCTURE (ABUTMENT #2)	540.10		
BEGIN OPTION AA 1 1 1 LS PRECAST CONCRETE STRUCTURE (APPROACH SLAB #1) 540.10							1		1	LS	SPECIAL PROVISION (ABUTMENT #2)	900.645		
1 LS PRECAST CONCRETE STRUCTURE (APPROACH SLAB #1) 540.10											END OPTION DD			
											BEGIN OPTION AA			
1 1 LS SPECIAL PROVISION (APPROACH SLAB #1) 900.645							1		1	LS	PRECAST CONCRETE STRUCTURE (APPROACH SLAB #1)	540.10		
							1		1	LS	SPECIAL PROVISION (APPROACH SLAB #1)	900.645		

PROJECT NAME: WEYBRIDGE-NEW HAVEN PROJECT NUMBER: BF 032-1(19)

FILE NAME: sl2b552forms.dgn
PROJECT LEADER: C.W. CARLSON
DESIGNED BY: D. PETERSON
QUANTITY SHEET I

PLOT DATE: 20-APR-2017
DRAWN BY: M. LONGSTREET
CHECKED BY: D. PETERSON
SHEET 6 OF 85

QUANTITY SHEET 2

	SUM	IMARY OF ES	TIMATED QU	ANTITIES				тот	ALS	DESCRIPTIONS			DETAILED SUMMARY OF QUANTITIES	
			ROADWAY	LANDSCAPING	EROSION CONTROL	BRIDGE	FULL C.E. ITEMS	GRAND TOTAL	FINAL UNIT	ITEMS ITEM NU	MBER	ROUND QL	JANTITIES UNIT	ITEMS
										END OPTION AA			N.A.B.I. = NOT A BID ITEM	
										BEGIN OPTION BB				
						1		1	LS	PRECAST CONCRETE STRUCTURE (APPROACH SLAB #2) 540.1	0			
						1		1	LS	SPECIAL PROVISION (APPROACH SLAB #2) 900.6	45			
										END OPTION BB				
						1		1	LS	PRECAST CONCRETE STRUCTURE (PIER CAP) 540.1	0			
						106		106	CY	CONCRETE, CLASS B 541.2	5			
						17		17	CY	CONCRETE, CLASS C 541.3	0			
			330			330		660	СҮ	STONE FILL, TYPE III 613.1	2			
			4					4	EACH	YIELDING MARKER POSTS 619.1	7			
			399					399	LF	REMOVING AND RESETTING FENCE 620.5	0			
			190					190	LF	REMOVAL OF EXISTING FENCE 620.5	5			
			732					732	LF	HD STEEL BEAM GUARDRAIL, GALVANIZED W/8 FEET POSTS 621.2	15			
			4					4	EACH	ANCHOR FOR STEEL BEAM RAIL 621.6	0			
			870					870	LF	REMOVAL AND DISPOSAL OF GUARDRAIL 621.8	0			
			100					100	HR	UNIFORMED TRAFFIC OFFICERS 630.1	0			
			1000					1000	HR	FLAGGERS 630.1	5			
							1	1	LS	FIELD OFFICE, ENGINEERS 631.1	0			
							1	1	LS	TESTING EQUIPMENT, CONCRETE 631.1	6			
							1	1	LS	TESTING EQUIPMENT, BITUMINOUS 631.1	7			
							3000	3000	DL	FIELD OFFICE TELEPHONE (N.A.B.I.) 631.2	6			
			1					1	LS	MOBILIZATION/DEMOBILIZATION 635.1	1			
			7					7	EACH	PORTABLE CHANGEABLE MESSAGE SIGN 641.1	5			
			1850					1850	LF	4 INCH WHITE LINE 646.2	0			
			1850					1850	LF	4 INCH YELLOW LINE 646.2	1			
			440					440	SY	GEOTEXTILE FOR ROADBED SEPARATOR 649.1	1			
						810		810	SY	GEOTEXTILE UNDER STONE FILL 649.3	1			
					346			346	SY	GEOTEXTILE FOR SILT FENCE 649.5	1			
					112			112	SY	GEOTEXTILE FOR SILT FENCE, WOVEN WIRE REINFORCED 649.5	15			
					220			220	SY	GEOTEXTILE FOR FILTER CURTAIN 649.6	1			
					30			30	LB	SEED 651.1	5			
					20			20	LB	SEED, WINTER RYE 651.1	7			
					200			200	LB		8			
					1			1	TON	AGRICULTURAL LIMESTONE 651.2				
					1			1	TON	HAYMULCH 651.2	5			
					190			190	CY	TOPSOIL 651.3				
					560			560	SY	GRUBBING MATERIAL 651.4	0			
					1			1	LS	EPSC PLAN 652.1	0			
					200			200	HR	MONITORING EPSC PLAN 652.2				
					1			1	LU					
												BBO IEC	T NAME: WEYBRIDGE - NE	

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QUANTITY SHEET 2

PLOT DATE: 20-APR-2017
DRAWN BY: M. LONGSTREET
CHECKED BY: D. PETERSON
SHEET 7 OF 85

QUANTITY SHEET 3

SUMMARY OF ESTIMATED QUA	ANTITIES	то	TALS	DESCRIPTIONS			DETAILED SUMMARY OF QUANTITIES
ROADWAY	LANDSCAPING EROSION CONTROL BRIDGE	FULL C.E. GRAND TOTA	L FINAL UNIT	ITEMS	ITEM NUMBER	ROUND QUANTIT	IES UNIT ITEMS
	500	500	SY	TEMPORARY EROSION MATTING	653.20		N.A.B.I. = NOT A BID ITEM
	580	580	SY	PERMANENT EROSION MATTING	653.21		
	30	30	CY	VEHICLE TRACKING PAD	653.35		
	1	1	EACH	FILTER BAG	653.45		
	600	600	LF	BARRIER FENCE	653.50		
	1160	1160	LF	PROJECT DEMARCATION FENCE	653.55		
	3	3	EACH	DECIDUOUS TREES (ACER SACCHARINUM) (CONT.) (30")	656.30		
	1	1	EACH	DECIDUOUS TREES (QUERCUS BICOLOR) (B&B) (2-2.5")	656.30		
	24	24	EACH	DECIDUOUS SHRUBS (CEPHALANTHUS OCCIDEBTALIS)(CONT.)(30")	656.35		
	15	15	EACH	DECIDUOUS SHRUBS (CORNUS AMOMUM)(CONT.)(30")	656.35		
	5	5	MGAL	LANDSCAPE WATERING	656.65		
	30	30	CY	LANDSCAPE BACKFILL, TRUCK MEASUREMENT	656.80		
7		7	SF	TRAFFIC SIGNS, TYPE A	675.20	0.75	
113		113	LF	SQUARE TUBE SIGN POST AND ANCHOR	675.341		
10		10	EACH	REMOVING SIGNS	675.50		
4		4	EACH	ERECTING SALVAGED SIGNS	675.60		
4		4	EACH	DELINEATOR WITH STEEL POST	676.10		
1		1	LU	PRICE ADJUSTMENT, FUEL (N.A.B.I.)	690.50		
	281	281	CY	SPECIAL PROVISION (CONCRETE HIGH PERFORMANCE, CLASS PCD)(FPQ)	900.608		
	11	11	CY	SPECIAL PROVISION (HIGH PERFORMANCE CONCRETE, RAPID SET)(FPQ)	900.608		
192000		192000	DL	SPECIAL PROVISION (INCENTIVE/DISINCENTIVE) (N.A.B.I.)	900.615		
13		13		SPECIAL PROVISION (CPM SCHEDULE)	900.620		
4		4		SPECIAL PROVISION (GUARDRAIL THRIE BEAM APPROACH SECTION, GALVANIZED	900.620		
T		7	Entori	3 RAIL BOX BEAM)	300.020		
22		22	EACH	SPECIAL PROVISION (RIVER BOULDER)	900.620		
	2	2	EACH	SPECIAL PROVISION (TEMPORARY CAUSEWAY)	900.620		
50		50	LF	SPECIAL PROVISION (BARBED WIRE FENCE)	900.640		
	56	56	LF	SPECIAL PROVISION (PRE-EXCAVATION OF ABUTMENT PILES, EARTH)	900.640		
	28	28	LF	SPECIAL PROVISION (PRE-EXCAVATION OF ABUTMENT PILES, ROCK)	900.640		
	15867	15867	LF	SPECIAL PROVISION (REINFORCING BAR, GFRP) (#5)(FPQ)	900.640		
	39019	39019	LF	SPECIAL PROVISION (REINFORCING BAR, GFRP) (#6)(FPQ)	900.640		
1		1	LS	SPECIAL PROVISION (RIVER ACCESS STAIRS)	900.645		
1		1	LS	SPECIAL PROVISION (TRAFFIC CONTROL, ALL-INCLUSIVE)	900.645		
1		1	LU	SPECIAL PROVISION (MAT DENSITY PAY ADJUSTMENT, SMALL QUANTITY)	900.650		
				(N.A.B.I)	000.050		
			LU	SPECIAL PROVISION (MAT DENSITY PAY ADJUSTMENT, TYPE IVB) (N.A.B.I)	900.650		
1			LU	SPECIAL PROVISION (MIXTURE PAYADJUSTMENT) (N.A.B.I.)	900.650		
490		490	SY	SPECIAL PROVISION (REINFORCED SOIL SLOPE)	900.675		
540		540	TON	SPECIAL PROVISION (BITUMINOUS CONCRETE PAVEMENT, SMALL QUANTITY)	900.680		
365	138	503	TON	SPECIAL PROVISION (BITUMINOUS CONCRETE PAVEMENT, TYPE IVB)	900.680		
							ME: WEYRRIDGE-NEW HAVEN

PROJECT NAME: WEYBRIDGE-NEW HAVEN PROJECT NUMBER: BF 032-1(19)

FILE NAME: sl2b552forms.dgn
PROJECT LEADER: C.W. CARLSON
DESIGNED BY: D. PETERSON
QUANTITY SHEET 3

PLOT DATE: 20-APR-2017
DRAWN BY: M. LONGSTREET
CHECKED BY: D. PETERSON
SHEET 8 OF 85

BRIDGE QUANTITY SHEET 1

SUMMARY OF BRIDGE QUANTITIES				TOTALS			DESCRIPTIONS		DETAILED SUMMARY OF QUANTITIES			
		SUPER STRUCTURE	APPROACH SLAB NO. 1	APPROACH SLAB NO. 2	ABUTMENT NO. 1	PIER	ABUTMENT NO. 2	BRIDGE TOTAL	UNIT	ITEMS	ITEM NUMBER	QUANTITIES UNIT ITEMS
					115		145	260	CY	STRUCTURE EXCAVATION	204.25	
					30		50	80	CY	GRANULAR BACKFILL FOR STRUCTURES	204.30	
						1		1	LS	COFFERDAM	208.40	
							1	1	LS	FURNISHING EQUIPMENT FOR DRIVING PILING	504.10	
					84		195	279	LF	STEEL PILING, HP 12 X 84	505.165	
							2	2	EACH	DYNAMIC PILE LOADING TEST	505.45	
		335819						335819	LB	STRUCTURAL STEEL, PLATE GIRDER (FPQ)	506.55	
						13030		13030	LB	REINFORCING STEEL, LEVEL I	507.11	
		14257			2367		2510	19134	LB	REINFORCING STEEL, LEVEL III (FPQ)	507.13	
						80		80	LF	DRILLING AND GROUTING DOWELS	507.16	
		1						1	LS	SHEAR CONNECTORS (2400 - 7/8" X 7")	508.15	
		8			3	6	3	20	GAL	WATER REPELLENT, SILANE	514.10	
		62						62	LF	BRIDGE EXPANSION JOINT, ASPHALTIC PLUG	516.10	
		805						805	SY	SHEET MEMBRANE WATERPROOFING, TORCH APPLIED	519.20	
		62						62	LF	JOINT SEALER, HOT POURED	524.11	
		485						485	LF	BRIDGE RAILING, GALVANIZED 3 RAIL BOX BEAM	525.335	
		1						1	EACH	REMOVAL OF STRUCTURE (5090 SF - EST.)	529.15	
						5		5	EACH	BEARING DEVICE ASSEMBLY, ELASTOMERIC PAD W/ EXT. LOAD PLATES	531.18	
										BEGIN OPTION CC		
					1			1	LS	PRECAST CONCRETE STRUCTURE (ABUTMENT #1)	540.10	
					1			1	LS	SPECIAL PROVISION (ABUTMENT #1)	900.645	
										END OPTION CC		
										BEGIN OPTION DD		
							1	1	LS		540.10	
							1	1		SPECIAL PROVISION (ABUTMENT #2)	900.645	
										END OPTION DD		
										BEGIN OPTION AA		
			1					1	LS	PRECAST CONCRETE STRUCTURE (APPROACH SLAB #1)	540.10	
			1					1	LS	SPECIAL PROVISION (APPROACH SLAB #1)	900.645	
			· ·					·		END OPTION AA	223.000	
										BEGIN OPTION BB		
				1				1	LS	PRECAST CONCRETE STRUCTURE (APPROACH SLAB #2)	540.10	
				1				1	LS		900.645	
				1				'	Lo	END OPTION BB	300.040	
						1		1	1.0		540.10	
						106		106	LS	PRECAST CONCRETE STRUCTURE (PIER CAP) CONCRETE, CLASS B	540.10	
						17		17	CY	CONCRETE, CLASS B CONCRETE, CLASS C	541.25	
					475	11	AEF					
					175		155	330	CY	STONE FILL, TYPE III	613.12	
		007			470		340	810	SY	GEOTEXTILE UNDER STONE FILL SPECIAL PROVISION (CONCRETE HIGH PERFORMANCE, CLASS PORVERO)	649.31	
		227			27		27	281	CY	SPECIAL PROVISION (CONCRETE HIGH PERFORMANCE, CLASS PCD)(FPQ)	900.608	
												PROJECT NAME: WEYBRIDGE-NEW HAVEN

PROJECT NAME: WEYBRIDGE-NEW HAVEN PROJECT NUMBER: BF 032-1(19)

FILE NAME: sl2b552forms.dgn
PROJECT LEADER: C.W. CARLSON
DESIGNED BY: D. PETERSON
BRIDGE QUANTITY SHEET I

PLOT DATE: 20-APR-2017
DRAWN BY: M. LONGSTREET
CHECKED BY: D. PETERSON
SHEET 9 OF 85

BRIDGE QUANTITY SHEET 2

	SUMMARY OF BRIDGE QUANTITIES									TOTALS DESCRIPTIONS			DETAILED SUMMARY OF QUANTITIES	
				SUPER STRUCTURE	APPROACH SLAB NO. 1	APPROACH SLAB NO. 2	ABUTMENT NO. 1	PIER	ABUTMENT NO. 2	BRIDGE TOTAL	UNIT	ITEMS	ITEM NUMBER	QUANTITIES UNIT ITEMS
					2.5	2.5	3		3	11	CY	SPECIAL PROVISION (HIGH PERFORMANCE CONCRETE, RAPID SET)(FPQ)	900.608	
								2		2	EACH	SPECIAL PROVISION (TEMPORARY CAUSEWAY)	900.620	
								56		56	LF	SPECIAL PROVISION (PRE-EXCAVATION OF ABUTMENT PILES, EARTH)	900.640	
								28		28	LF	SPECIAL PROVISION (PRE-EXCAVATION OF ABUTMENT PILES, ROCK)	900.640	
				15867						15867	LF	SPECIAL PROVISION (REINFORCING BAR, GFRP) (#5)(FPQ)	900.640	
				39019						39019	LF	SPECIAL PROVISION (REINFORCING BAR, GFRP) (#6)(FPQ)	900.640	
				138						138	TON	SPECIAL PROVISION (BITUMINOUS CONCRETE PAVEMENT, TYPE IVB)	900.680	
										<u> </u>				<u> </u>
1														PROJECT NAME. WEVDDIDCE_NEW HAVEN

PROJECT NAME: WEYBRIDGE-NEW HAVEN PROJECT NUMBER: BF 032-1(19)

FILE NAME: sl2b552forms.dgn
PROJECT LEADER: C.W. CARLSON
DESIGNED BY: D. PETERSON
BRIDGE QUANTITY SHEET 2

PLOT DATE: 20-APR-2017
DRAWN BY: M. LONGSTREET
CHECKED BY: D. PETERSON
SHEET 10 OF 85

GENERAL INFORMATION

SYMBOLOGY LEGEND NOTE

THE SYMBOLOGY ON THIS SHEET IS INTENDED TO COVER STANDARD CONVENTIONAL SYMBOLOGY. THE SYMBOLOGY IS USED FOR EXISTING & PROPOSED FEATURES WITH HEAVIER LINEWEIGHT, IN COMBINATION WITH PROJECT ANNOTATION, AS NOTED ON PROJECT PLAN SHEETS. THIS LEGEND SHEET COVERS THE BASICS. SYMBOLOGY ON PLANS MAY VARY. PLAN ANNOTATIONS AND NOTES SHOULD BE USED TO CLARIFY AS NEEDED.

D O W ADDDEVIATIONS (CODES) A SYMBOLS

R. O. W.	ABBREV	IATIONS (CODES) & SYMBOLS
POINT	CODE	DESCRIPTION
	СН	CHANNEL EASEMENT
	CONST	CONSTRUCTION EASEMENT
	CUL	CULVERT EASEMENT
	D&C	DISCONNECT & CONNECT
	DIT	DITCH EASEMENT
	DR	DRAINAGE EASEMENT
	DRIVE	DRIVEWAY EASEMENT
	EC	EROSION CONTROL
	HWY	HIGHWAY EASEMENT
	I&M	INSTALL & MAINTAIN EASEMENT
	LAND	LANDSCAPE EASEMENT
	R&RES	REMOVE & RESET
	R&REP	REMOVE & REPLACE
	SR	SLOPE RIGHT
	UE	UTILITY EASEMENT
	(P)	PERMANENT EASEMENT
	(T)	TEMPORARY EASEMENT
	BNDNS	BOUND SET
	BNDNS	BOUND TO BE SET
\odot	IPNF	IRON PIN FOUND
	IPNS	IRON PIN TO BE SET
\boxtimes	CALC	EXISTING ROW POINT
\circ	PROW	PROPOSED ROW POINT
[LENG	TH]	LENGTH CARRIED ON NEXT SHEET

COMMON TOPOCRAPHIC POINT SYMBOLS

COMMON	I TOPOGE	RAPHIC POINT SYMBOLS
POINT	CODE	DESCRIPTION
(1)	APL	BOUND APPARENT LOCATION
0	BM	BENCHMARK
•	BND	BOUND
	CB	CATCH BASIN
þ	COMB	COMBINATION POLE
	DITHR	DROP INLET THROATED DNC
¢	EL	ELECTRIC POWER POLE
0	FPOLE	FLAGPOLE
\odot	GASFIL	GAS FILLER
\odot	GP	GUIDE POST
M	GS0	GAS SHUT OFF
0	GUY	GUY POLE
0	GUYW	GUY WIRE
M	GV	GATE VALVE
	Н	TREE HARDWOOD
Δ	HCTRL	CONTROL HORIZONTAL
△	HVCTRL	CONTROL HORIZ. & VERTICAL
\Diamond	HYD	HYDRANT
@	IP	IRON PIN
©	IPIPE	IRON PIPE
¢	LI	LIGHT - STREET OR YARD
8	MB	MAILBOX
0	MH	MANHOLE (MH)
•	MM	MILE MARKER
Θ	PM	PARKING METER
•	PMK	PROJECT MARKER
0	POST	POST STONE/WOOD
* **	RRSIG	RAILROAD SIGNAL
•	RRSL	RAILROAD SWITCH LEVER
(E)	S	TREE SOFTWOOD
	SAT	SATELLITE DISH
(B)	SHRUB	SHRUB
\overline{o}	SIGN	SIGN
A	STUMP	STUMP
-0-	TEL	TELEPHONE POLE
⊙	TIE	TIE
0 0	TSIGN	SIGN W/DOUBLE POST
人	VCTRL	CONTROL VERTICAL
0	WELL	WELL
M	WSO	WATER SHUT OFF

THESE ARE COMMON VAOT SURVEY POINT SYMBOLS FOR EXISTING FEATURES, ALSO USED FOR PROPOSED FEATURES WITH HEAVIER LINEWEIGHT, IN COMBINATION WITH PROPOSED ANNOTATION.

PROPOSED GEOMETRY CODES

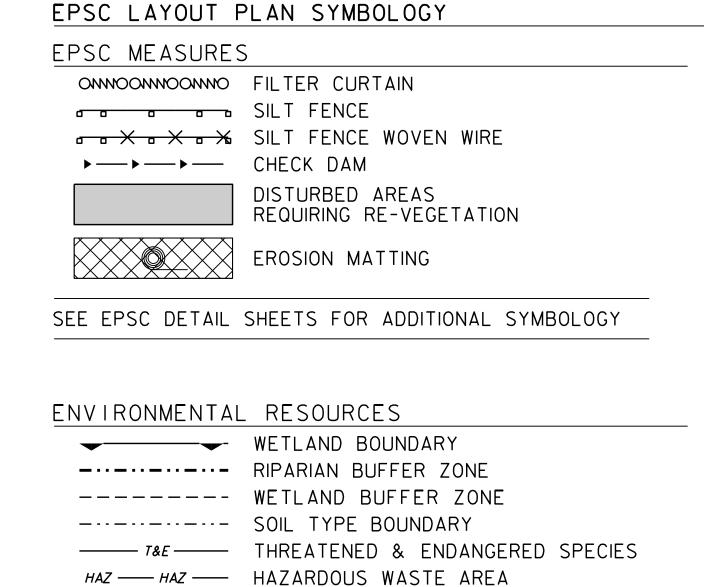
<u> </u>	ED GEOMETRI CODES
CODE	DESCRIPTION
PC	POINT OF CURVATURE
PI	POINT OF INTERSECTION
CC	CENTER OF CURVE
PT	POINT OF TANGENCY
PCC	POINT OF COMPOUND CURVE
PRC	POINT OF REVERSE CURVE
POB	POINT OF BEGINNING
POE	POINT OF ENDING
STA	STATION PREFIX
АН	AHEAD STATION SUFFIX
BK	BACK STATION SUFFIX
D	CURVE DEGREE OF (100FT)
R	CURVE RADUIS OF
T	CURVE TANGENT LENGTH
L	CURVE LENGTH OF
E	CURVE EXTERNAL DISTANCE

UNDERGROUND UT	ILITIES
— UGU — · · — ·	UTILITY (GENERIC-UNKNOWN)
— UT — · · — ·	TELEPHONE
— UE — · · · — ·	ELECTRIC
— UC — · · · — ·	CABLE (TV)
— UEC — · · · — ·	ELECTRIC+CABLE
— UET — · · — ·	ELECTRIC+TELEPHONE
— UCT — · · - ·	CABLE+TELEPHONE
— UECT — · · — ·	ELECTRIC+CABLE+TELEP.
— G — · · · — ·	GAS LINE
	WATER LINE
— s — · · - ·	SANITARY SEWER (SEPTIC)
ABOVE GROUND U	TILITIES (AERIAL)
	UTILITY (GENERIC-UNKNOWN)
— T — · · - ·	- TELEPHONE
— E — · · – ·	· - ELECTRIC
— c — · · - ·	CABLE (TV)
— EC — · · - ·	ELECTRIC+CABLE
— ET — · · · - ·	ELECTRIC+TELEPHONE
— AER E&T — · · -	- · ELECTRIC+TELEPHONE
— CT — · · · - ·	CABLE+TELEPHONE
— ECT — · · · - ·	ELECTRIC+CABLE+TELEP.
	UTILITY POLE GUY WIRE
PROJECT CONSTRU	JCTION SYMBOLOGY
	& LAYOUT SYMBOLOGY
	- CLEAR ZONE
	- PLAN LAYOUT MATCHLINE
	ICTION FEATURES
	JCTION FEATURES
	→ TOP OF CUT SLOPE
	→ TOE OF FILL SLOPE
8 8 8 8 8	
	BOTTOM OF DITCH € =: CULVERT PROPOSED
	CULVERI PRUPUSED

	TOP OF CUT SLOPE
0 0 0	TOE OF FILL SLOPE
8 8 8 8 8	STONE FILL
	BOTTOM OF DITCH &
	CULVERT PROPOSED
	STRUCTURE SUBSURFACE
PDFPDF	PROJECT DEMARCATION FENCE
BF -× × - BF -× × -	BARRIER FENCE
××××××××××××××××××××××××××××××××××××××	TREE PROTECTION ZONE (TPZ)
///////////////////////////////////////	STRIPING LINE REMOVAL
~~~~	SHEET PILES

# CONVENTIONAL BOUNDARY SYMBOLOGY

CONVENTIONAL BOOM	DART STMBULUGT
BOUNDARY LINES	
TOWN LINE	TOWN BOUNDARY LINE
COUNTY LINE	COUNTY BOUNDARY LINE
STATE LINE	STATE BOUNDARY LINE
<del></del>	PROPOSED STATE R.O.W. (LIMITED ACCESS)
	PROPOSED STATE R.O.W.
	STATE ROW (LIMITED ACCESS)
	STATE ROW
	TOWN ROW
- · - · - · - · - ·	PERMANENT EASEMENT LINE (P)
	TEMPORARY EASEMENT LINE (T)
+ + + + + + + + + + + + + + + + + + + +	SURVEY LINE
$\frac{P}{L}$ $\frac{P}{L}$ $\frac{P}{L}$	PROPERTY LINE (P/L)
SR SR SR O	SLOPE RIGHTS
6f ———— 6f ———	6F PROPERTY BOUNDARY
4f ———— 4f ———	4F PROPERTY BOUNDARY
HAZ HAZ	HAZARDOUS WASTE



# ARCHEOLOGICAL & HISTORIC

— FLOOD PLAIN — FLOOD PLAIN

→ STORM WATER

——— AG ——— AGRICULTURAL LAND

---- HABITAT ---- FISH & WILDLIFE HABITAT

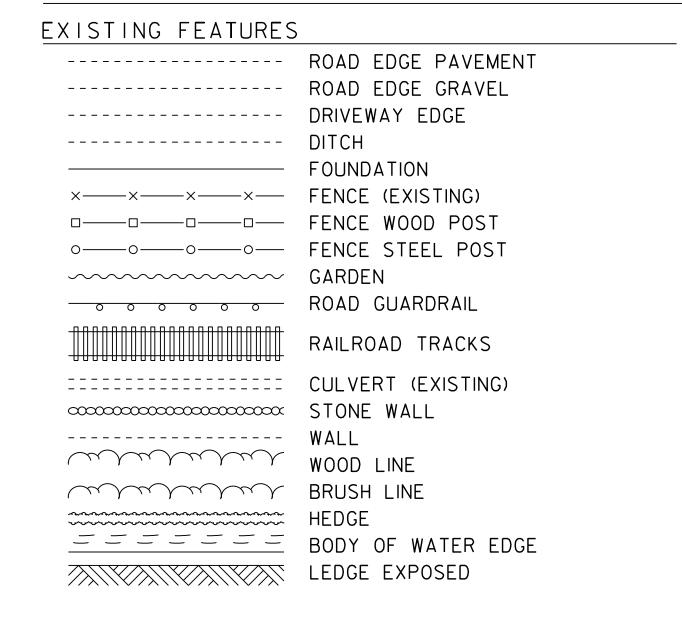
-√-OHW--√- ORDINARY HIGH WATER (OHW)

— - - USDA FOREST SERVICE LANDS

— · · · — WILDLIFE HABITAT SUIT/CONN

——— ARCH ———	ARCHEOLOGICAL BOUNDARY	
	HISTORIC DISTRICT BOUNDARY	
—— HISTORIC ——	HISTORIC AREA	
(H)	HISTORIC STRUCTURE	

# CONVENTIONAL TOPOGRAPHIC SYMBOLOGY

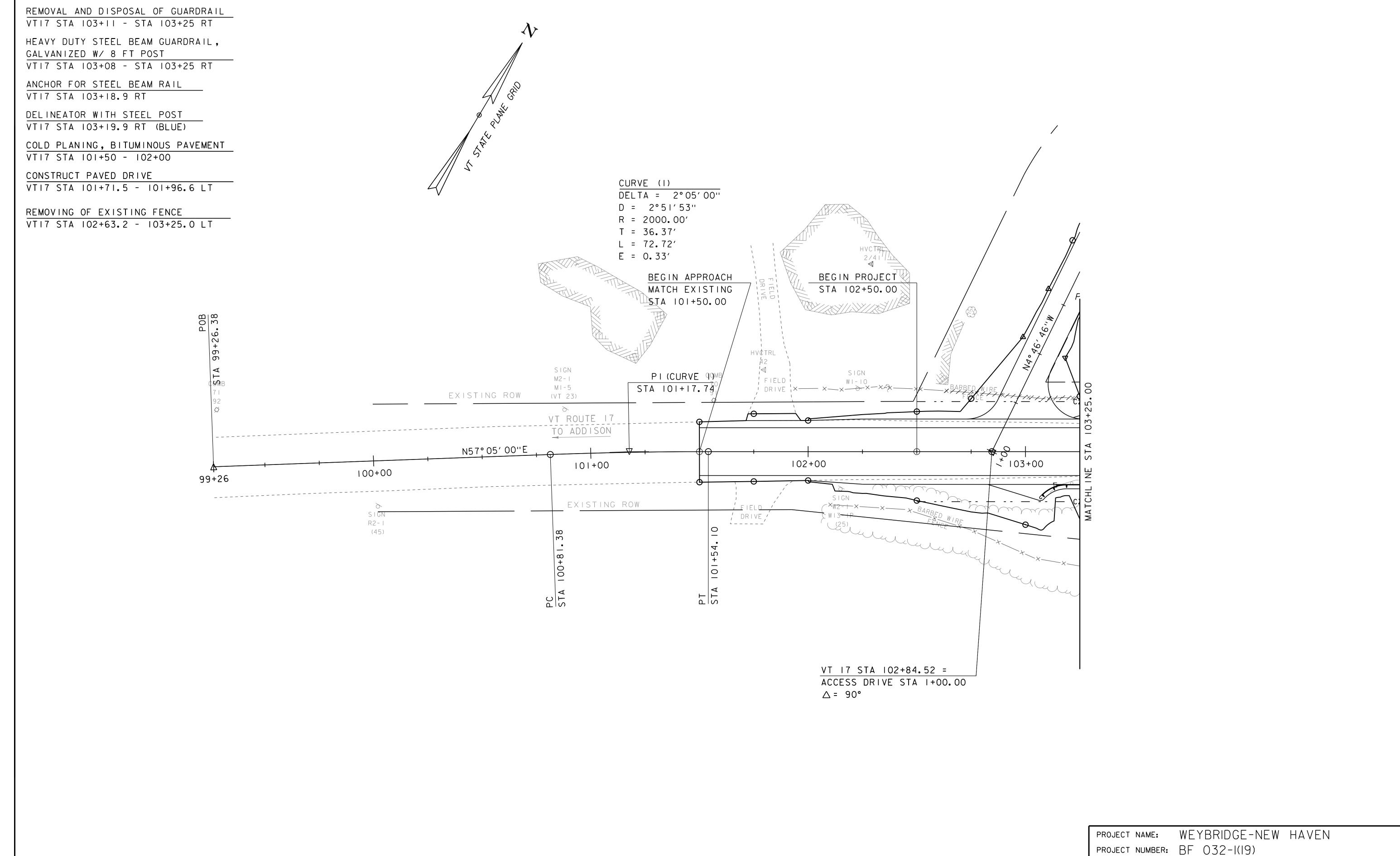


PROJECT NAME: WEYBRIDGE-NEW HAVEN PROJECT NUMBER: BF 032-1(19)

FILE NAME: sl2b552forms.dgn PROJECT LEADER: C.W. CARLSON DESIGNED BY: D. PETERSON CONVENTIONAL SYMBOLOGY

PLOT DATE: 20-APR-2017 DRAWN BY: M. LONGSTREET CHECKED BY: D. PETERSON SHEET II OF 85

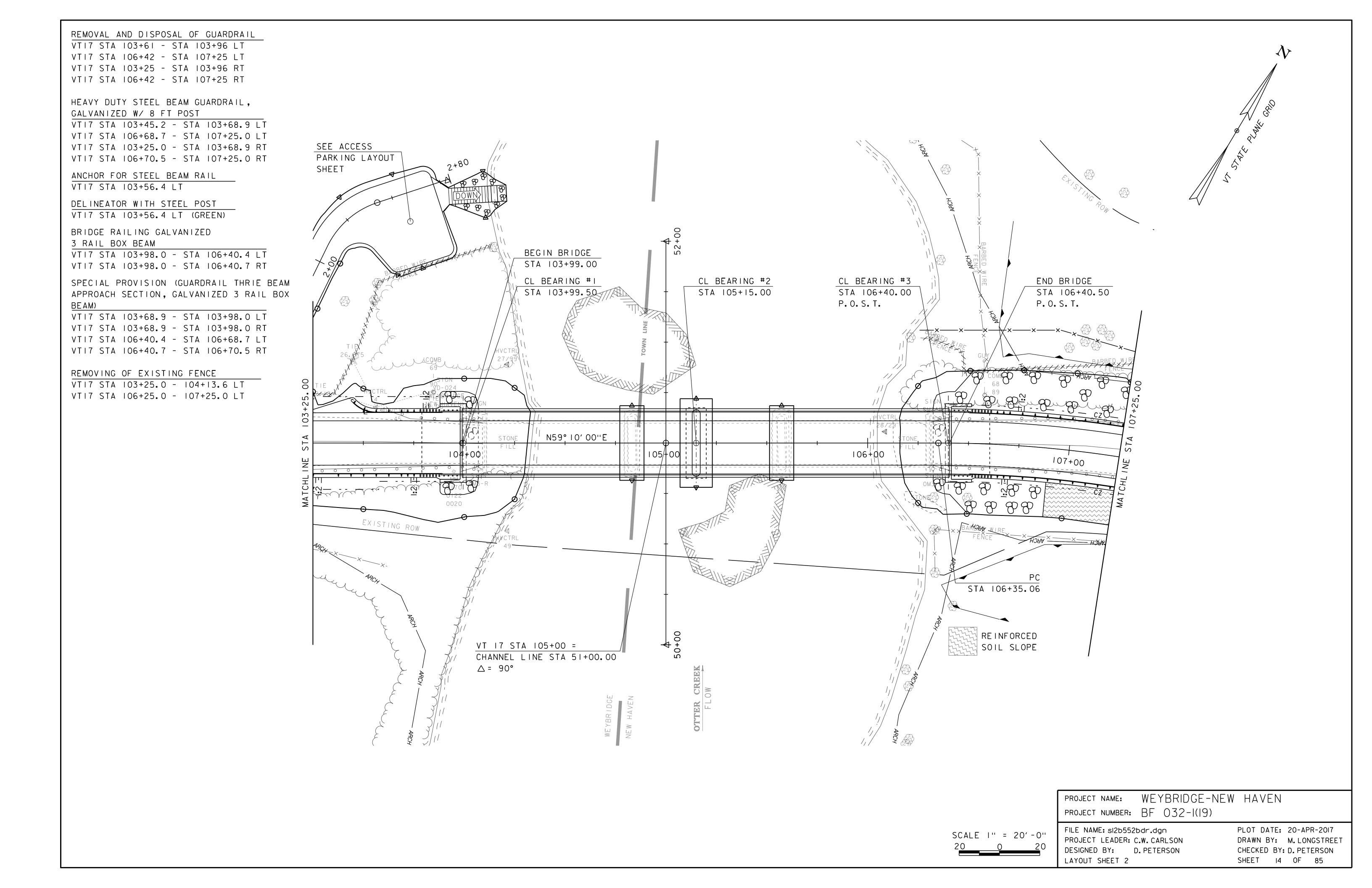
HVCTRL # HVCTRL # 2 STANDARD DISK STAMPED STANDARD DISK STAMPED Chalker Az Mk Chalker 580550.5810 578921.0470 1444341.9110 1444009.969 ELEV. = 201.246 ELEV. = 157.939 To reach from the intersection of VT routes I7 and 23 go east along route I7 for 0.15 mi to the site of the mark on the left, on a small rise, just east of a field drive. The mark is set flush with ground surface in the top of a massive rock outcrop. It is86.0 ft north of and about 6.6 ft higher than the centerline of route I7, 50 ft east of and about I0 ft higher than the centerline of the field drive, I31.2 ft southeast of the southeast corner of a equipment barn, 49 ft west northwest of a I8 in. oak, and 57 ft north of a fiberglass witness post in an east-west wire fenceline. To reach from the intersection of VT routes I7 and 23 go east along route I7 for 0.4 mi to the intersection of Hallock Road left and Quaker Village Road right. Turn left and go north along Hallock Road for 0.3 mi to an old concrete barn foundation on the left and the site of the mark on the left, set in the top of the most southernly of two concrete pads in the foundation structure. It is about opposite a 2 I/2 story barn on the right. It is 37.1 ft west of and about level with the centerline of Hallock Road, I34.5 ft south of a pole no. 3002/5-1, 4.9 ft northwest of the southeast corner of the concrete pad, 4.9 ft southwest of the northeast corner of the concrete pad, and 39.4 ft north northwest of a fiberglass witness post at a wire fence corner.  $\bigcirc$ corner. * DESCRIPTION PROVIDED BY VERMONT AGENCY OF TRANSPORTATION GEODETIC SURVEY UNIT  $\bigcirc$ * RECOVERED 8-26-14 LGO, HAM, GAH HVCRTL #5 HVCTRL #6 HVCTRL #3 HVCTRL #4 NORTH = 578564.3820 NORTH = 579075.5590 NORTH = 579239.6280 NORTH = 578927.4270 EAST = 1445170.5110 EAST = 1443636.410 EAST = 1444154.3370 EAST = 1445926.650 ELEV. = 152.691 ELEV. = 155.226 ELEV. = 203.264 ELEV. = 228.184 BM #6 Med Ash RRSIR @ 232.464 Med. Locust BM #5 RRSIR 201.975 Small Red\ Cedar ...  $\bigcirc$ >  $\triangleleft$ Small Ash * MAIN TRAVERSE COMPLETED 1/17/01by L. Orvis (P.C) & J. Hulett NORTH = NORTH = NORTH = NORTH =  $\bigcirc$ EAST = EAST = EAST = EAST =  $\geq$  $\bigcirc$  $\triangleleft$ PROJECT NAME: WEYBRIDGE-NEW HAVEN DATUM PROJECT NUMBER: BF 032-1(19) NAVD 88 VERTICAL FILE NAME: sl2b552forms.dgn PLOT DATE: 20-APR-2017 NAD 83 (96) HORIZONTAL PROJECT LEADER: C.W. CARLSON DRAWN BY: M. LONGSTREET DESIGNED BY: D. PETERSON CHECKED BY: D. PETERSON ADJUSTMENT _____none TIE SHEET SHEET I2 OF 85



SCALE I'' = 20'-0"
20 0 20

FILE NAME: sl2b552bdr.dgn
PROJECT LEADER: C.W. CARLSON
DESIGNED BY: D. PETERSON
LAYOUT SHEET I

PLOT DATE: 20-APR-2017
DRAWN BY: M. LONGSTREET
CHECKED BY: D. PETERSON
SHEET 13 OF 85



REMOVAL AND DISPOSAL OF GUARDRAIL

VT17 STA 107+25.0 - STA 109+66.8 LT

VT17 STA 107+25.0 - STA 110+08.9 RT

HEAVY DUTY STEEL BEAM GUARDRAIL,

GALVANIZED W/ 8 FT POST

VTI7 STA 107+25.0 - STA 109+73.1 LT

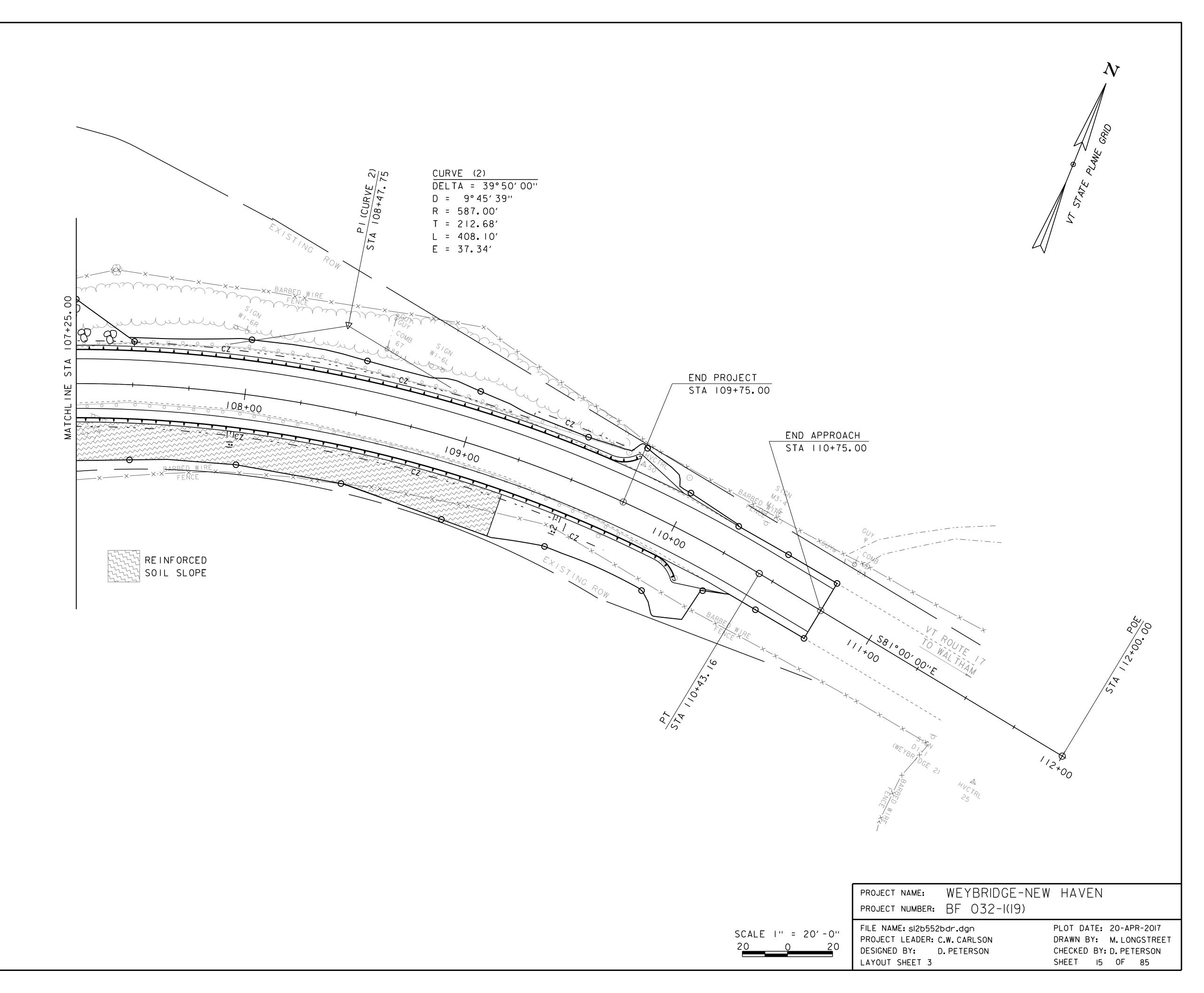
VTI7 STA 107+25.0 - STA 110+10.3 RT

ANCHOR FOR STEEL BEAM RAIL VTI7 STA 109+62.2 LT VTI7 STA 109+98.6 RT

DELINEATOR WITH STEEL POST VTI7 STA 109+61.3 LT (BLUE) VTI7 STA 109+97.6 RT (GREEN)

COLD PLANING, BITUMINOUS PAVEMENT VT17 STA 110+25 - 110+75

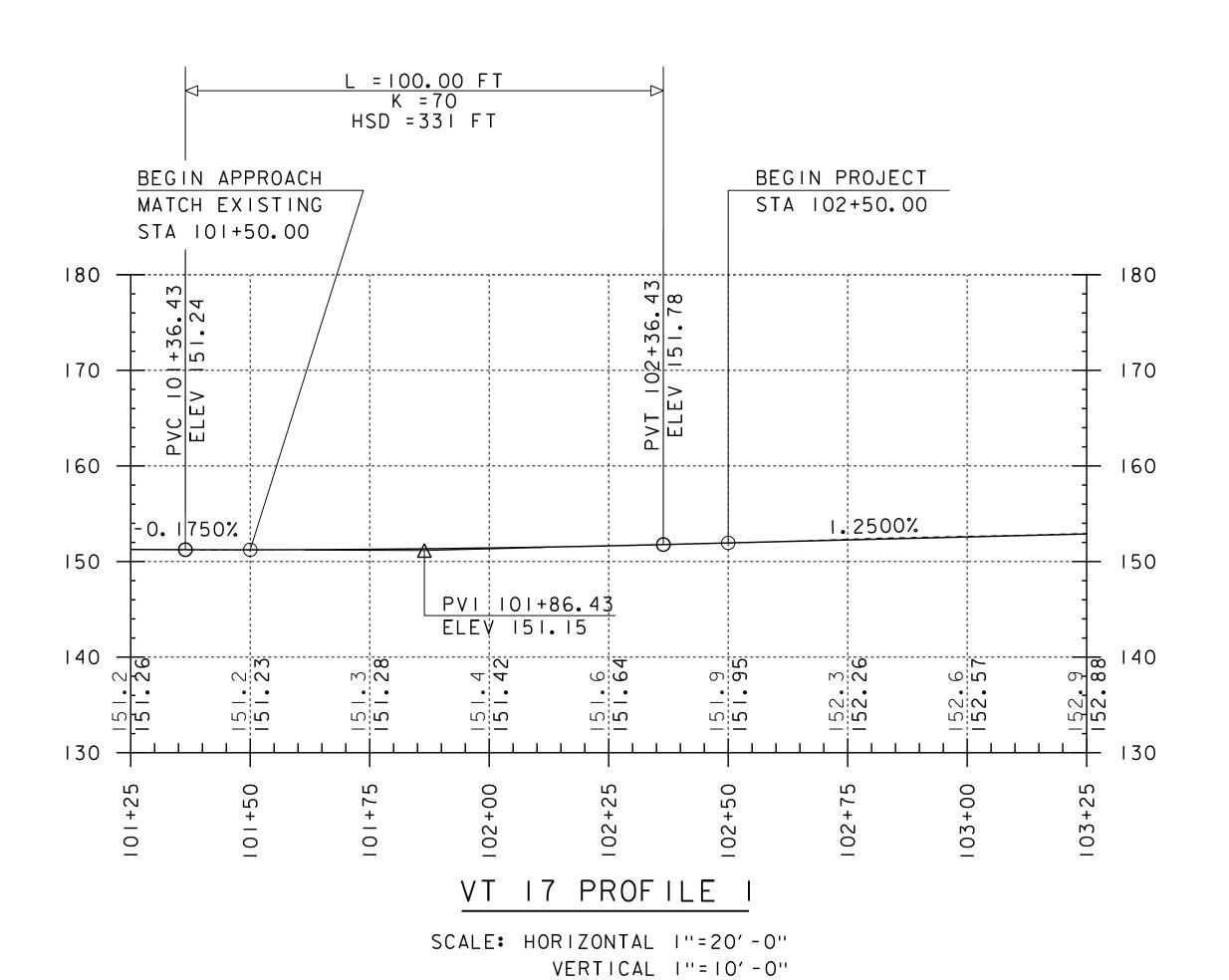
REMOVING AND RESETTING FENCE VT17 STA 107+25.0 - 107+41.4 LT VT17 STA 107+97.8 - 110+75.0 RT

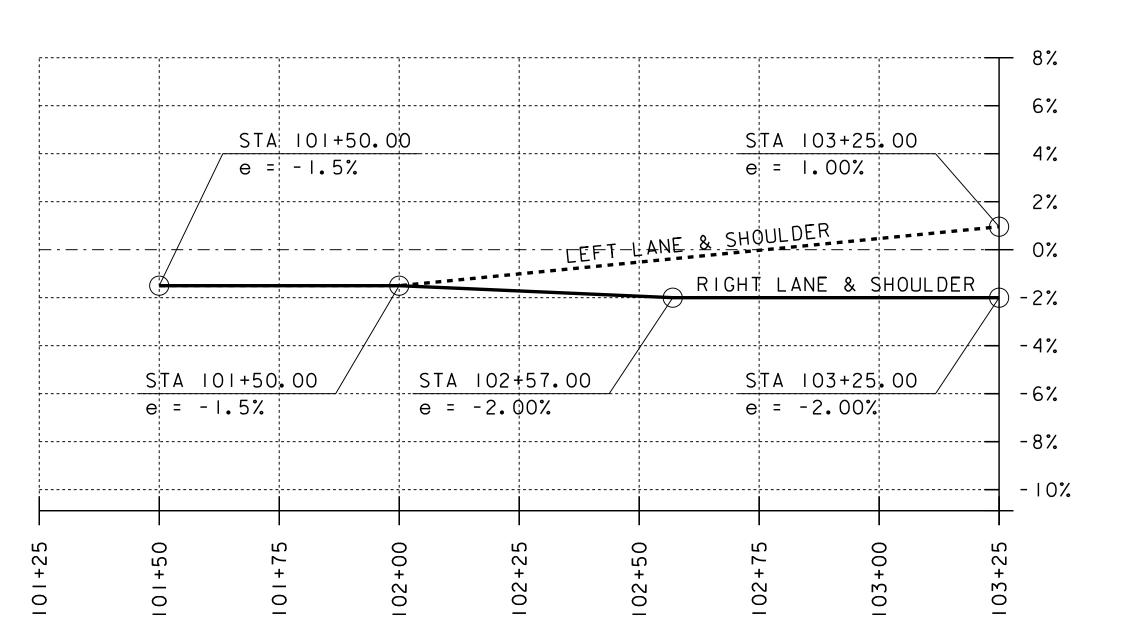


# NOTE:

ELEVATIONS SHOWN TO THE NEAREST TENTH ARE EXISTING GROUND ALONG PROPOSED CENTERLINE.

ELEVATIONS SHOWN TO THE NEAREST HUNDREDTH ARE FINISH GRADES ALONG PROPOSED CENTERLINE.





BANKING DIAGRAM

SCALE: HORIZONTAL I"=20'-0"
VERTICAL I"=4%

PROJECT NAME: WEYBRIDGE-NEW HAVEN PROJECT NUMBER: BF 032-1(19)

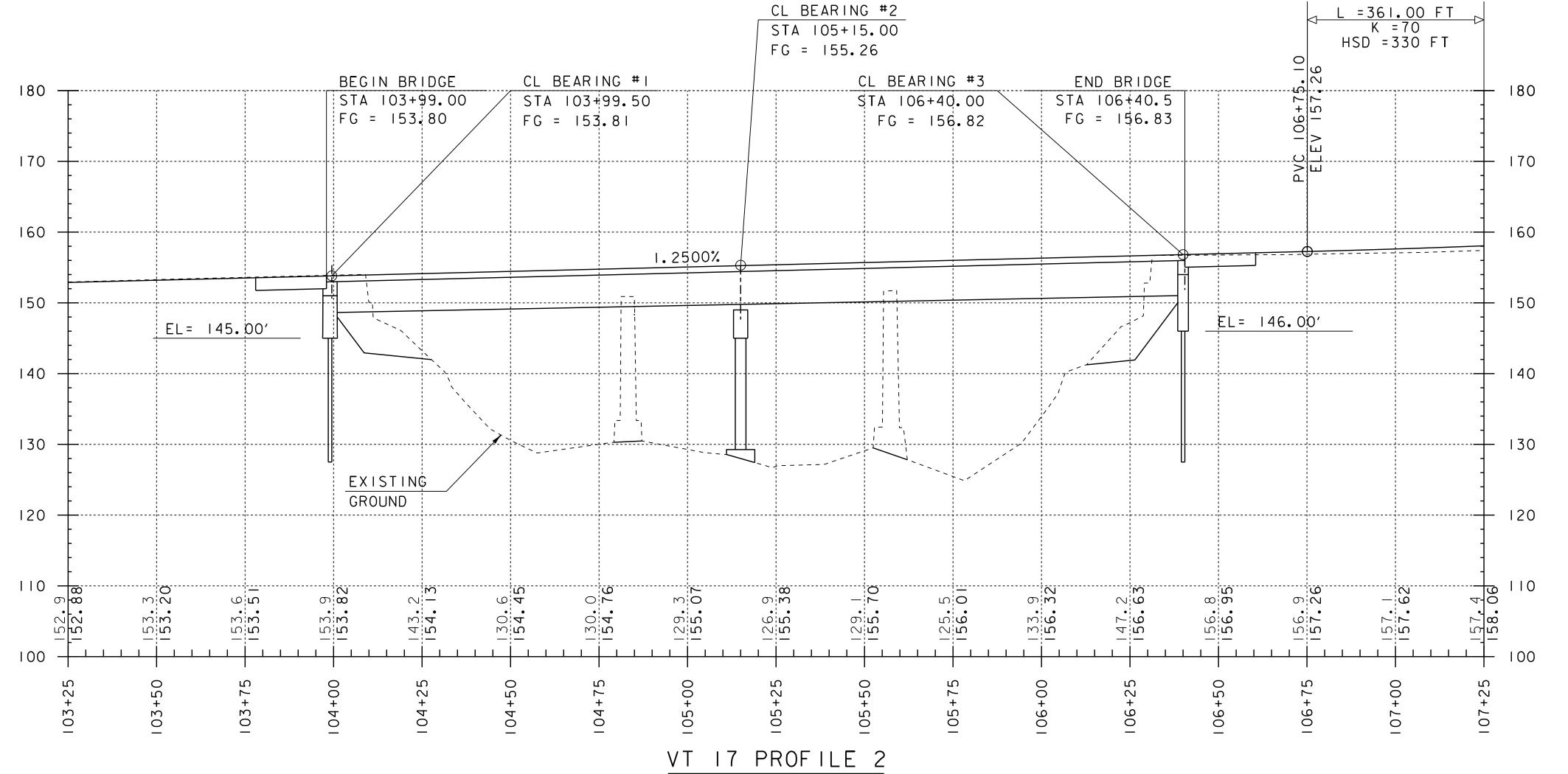
FILE NAME: si2b552pro.dgn
PROJECT LEADER: C.W. CARLSON
DESIGNED BY: D. PETERSON
PROFILE & BANKING I

PLOT DATE: 20-APR-2017
DRAWN BY: M. LONGSTREET
CHECKED BY: D. PETERSON
SHEET 16 OF 85

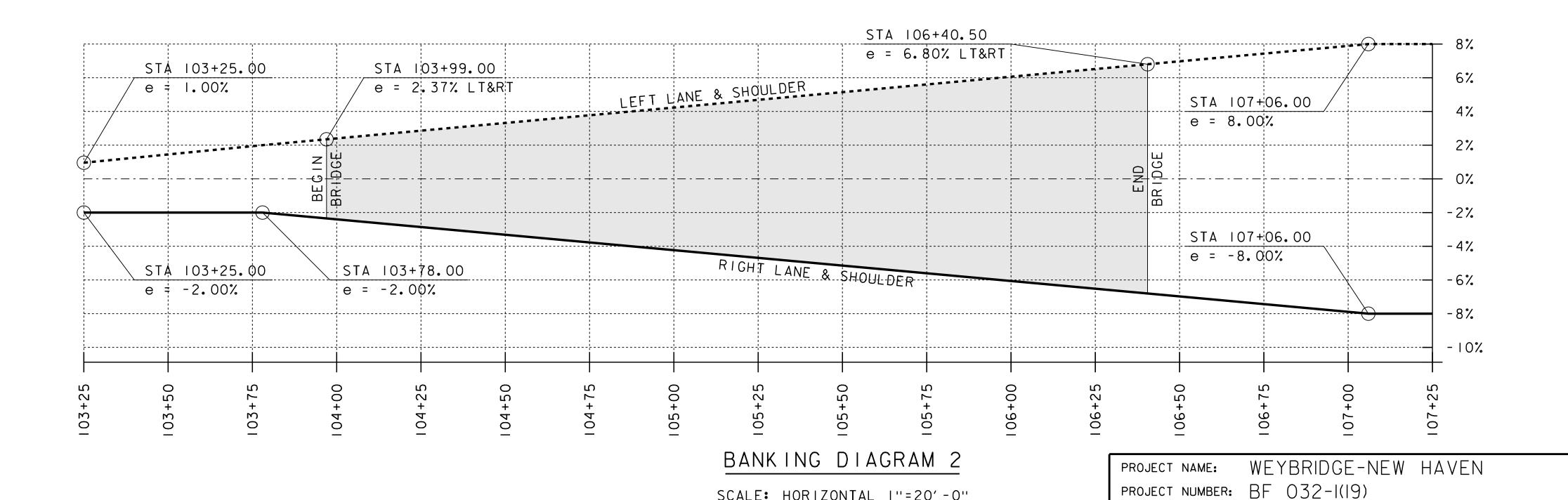
NOTE:

ELEVATIONS SHOWN TO THE NEAREST TENTH ARE EXISTING GROUND ALONG PROPOSED CENTERLINE.

ELEVATIONS SHOWN TO THE NEAREST HUNDREDTH ARE FINISH GRADES ALONG PROPOSED CENTERLINE.



SCALE: HORIZONTAL I"=20'-0" VERTICAL | " = 10' -0"



SCALE: HORIZONTAL I"=20'-0"

VERTICAL I"=4%

PLOT DATE: 20-APR-2017

CHECKED BY: D. PETERSON

SHEET 17 OF 85

DRAWN BY: M. LONGSTREET

FILE NAME: sl2b552pro.dgn

PROFILE & BANKING 2

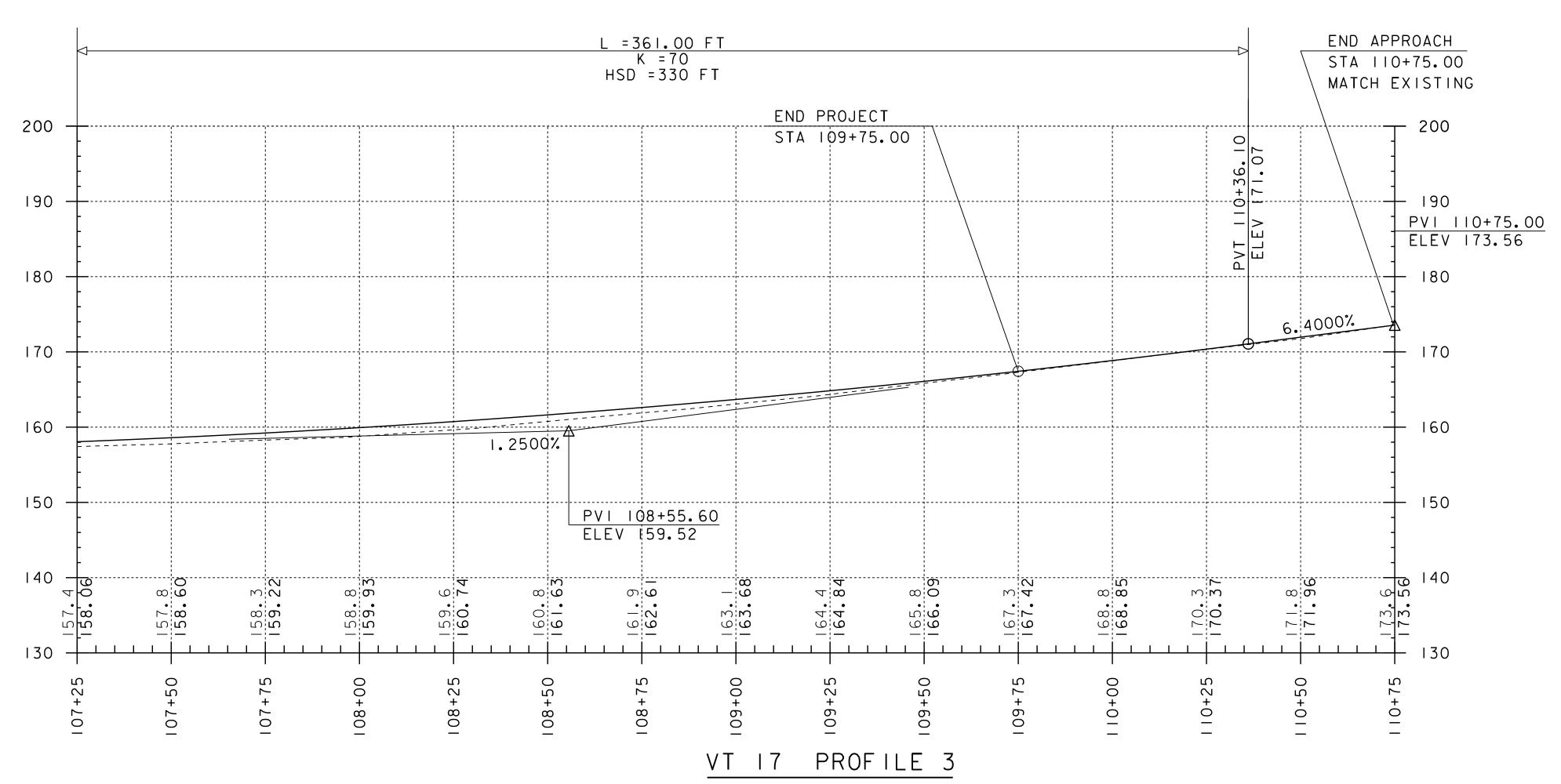
PROJECT LEADER: C.W. CARLSON

DESIGNED BY: D. PETERSON

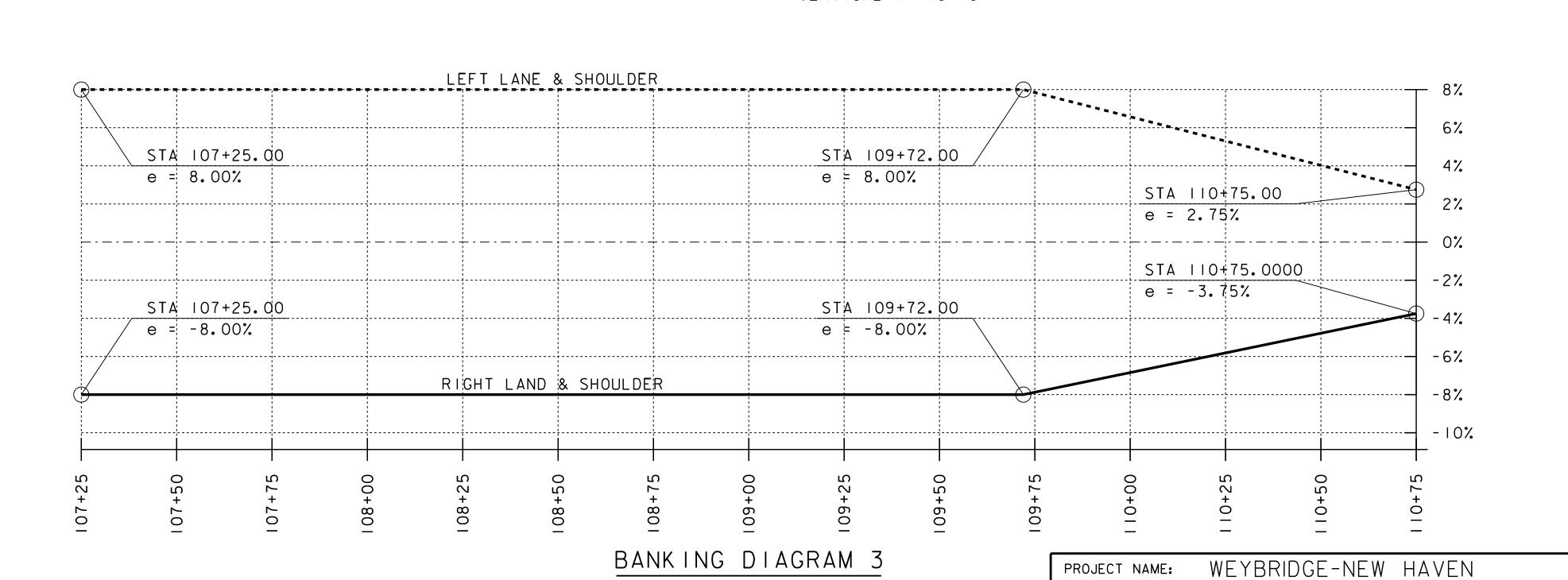
# NOTE:

ELEVATIONS SHOWN TO THE NEAREST TENTH ARE EXISTING GROUND ALONG PROPOSED CENTERLINE.

ELEVATIONS SHOWN TO THE NEAREST HUNDREDTH ARE FINISH GRADES ALONG PROPOSED CENTERLINE.



SCALE: HORIZONTAL I"=20'-0"
VERTICAL I"=10'-0"



SCALE: HORIZONTAL I"=20'-0"

VERTICAL I''=4%

PROJECT NUMBER: BF 032-1(19)

FILE NAME: sl2b552pro.dgn

PROFILE & BANKING 3

PROJECT LEADER: C.W. CARLSON

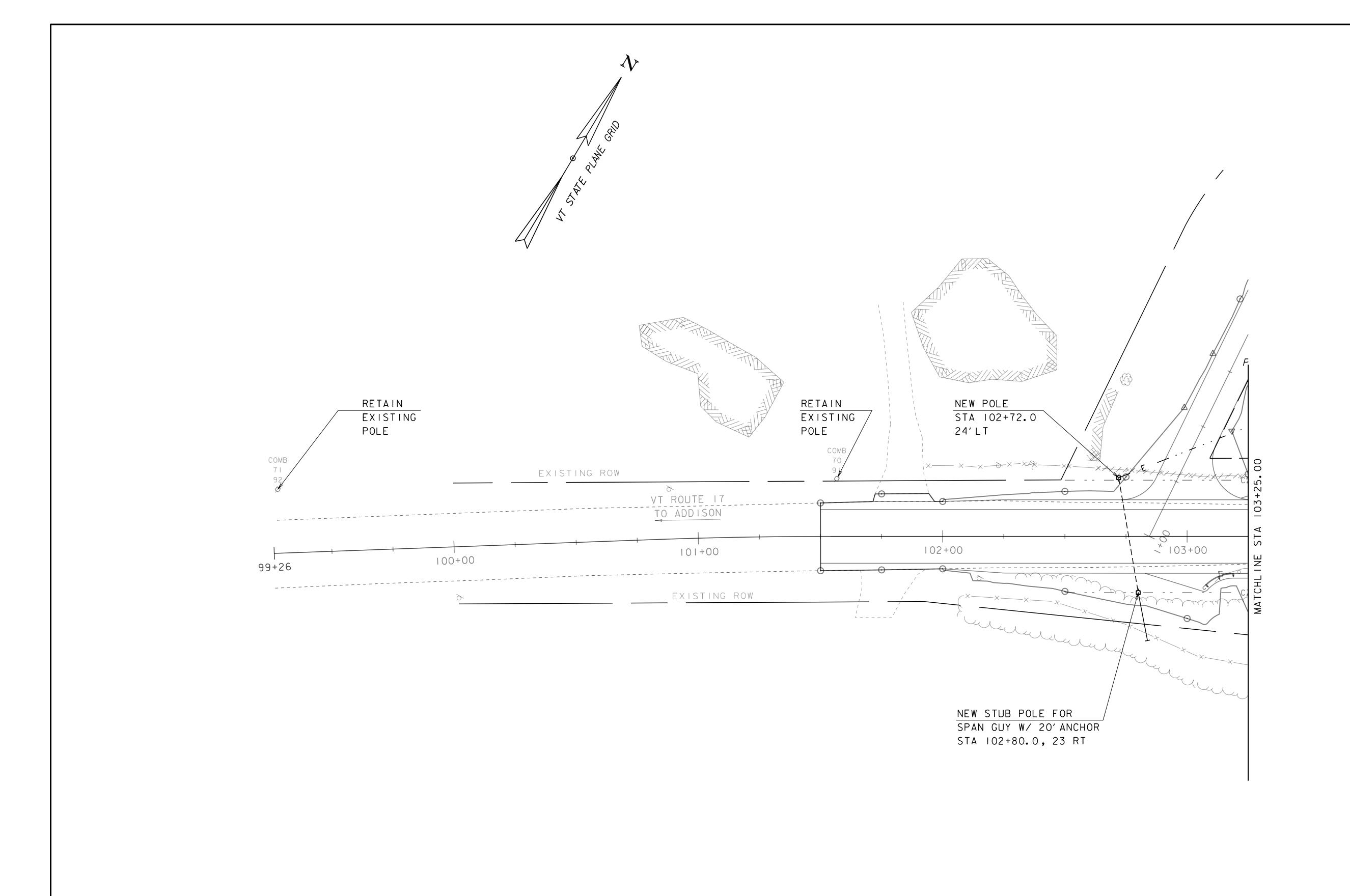
DESIGNED BY: D. PETERSON

PLOT DATE: 20-APR-2017

CHECKED BY: D. PETERSON

SHEET I8 OF 85

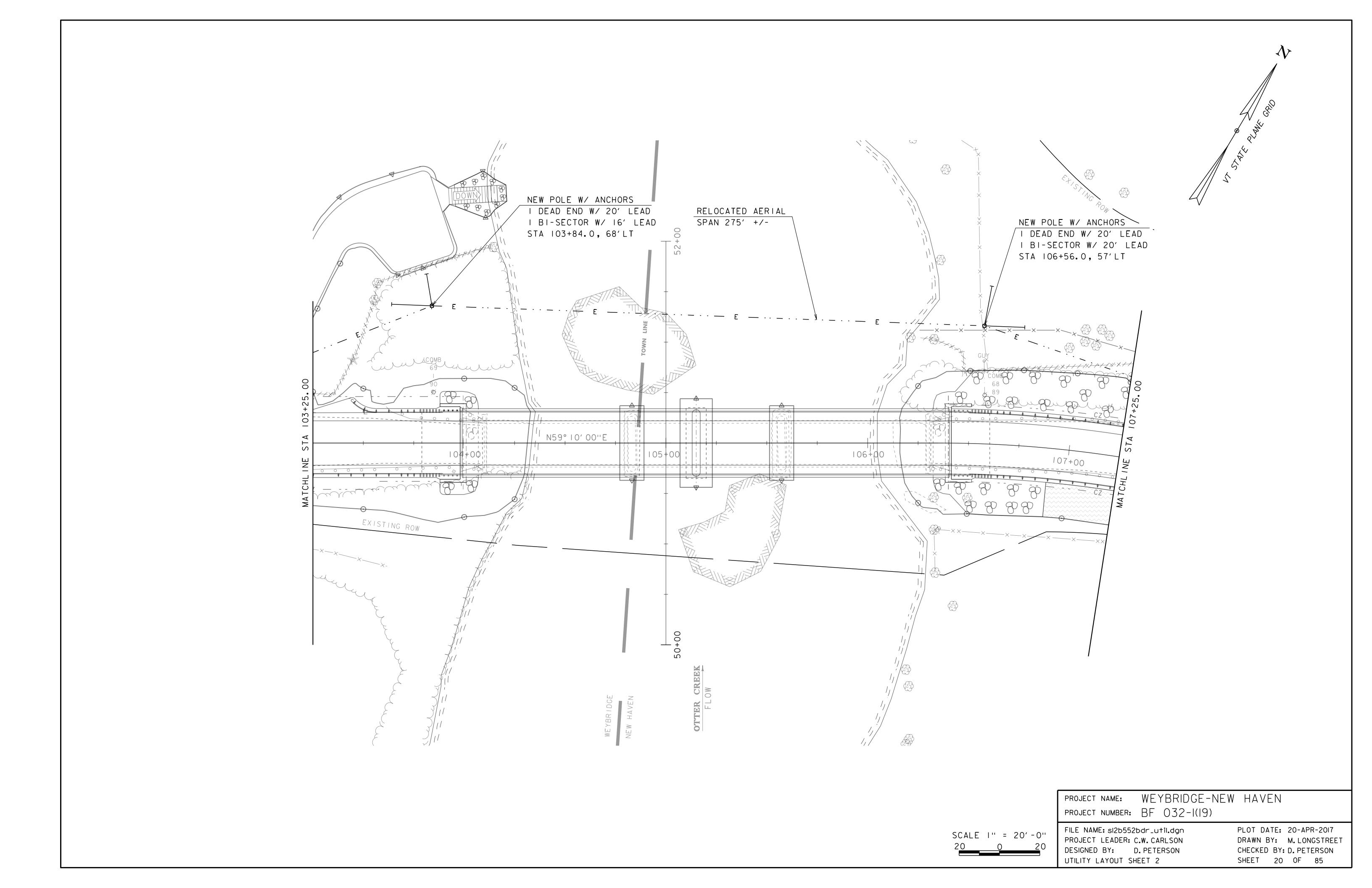
DRAWN BY: M. LONGSTREET

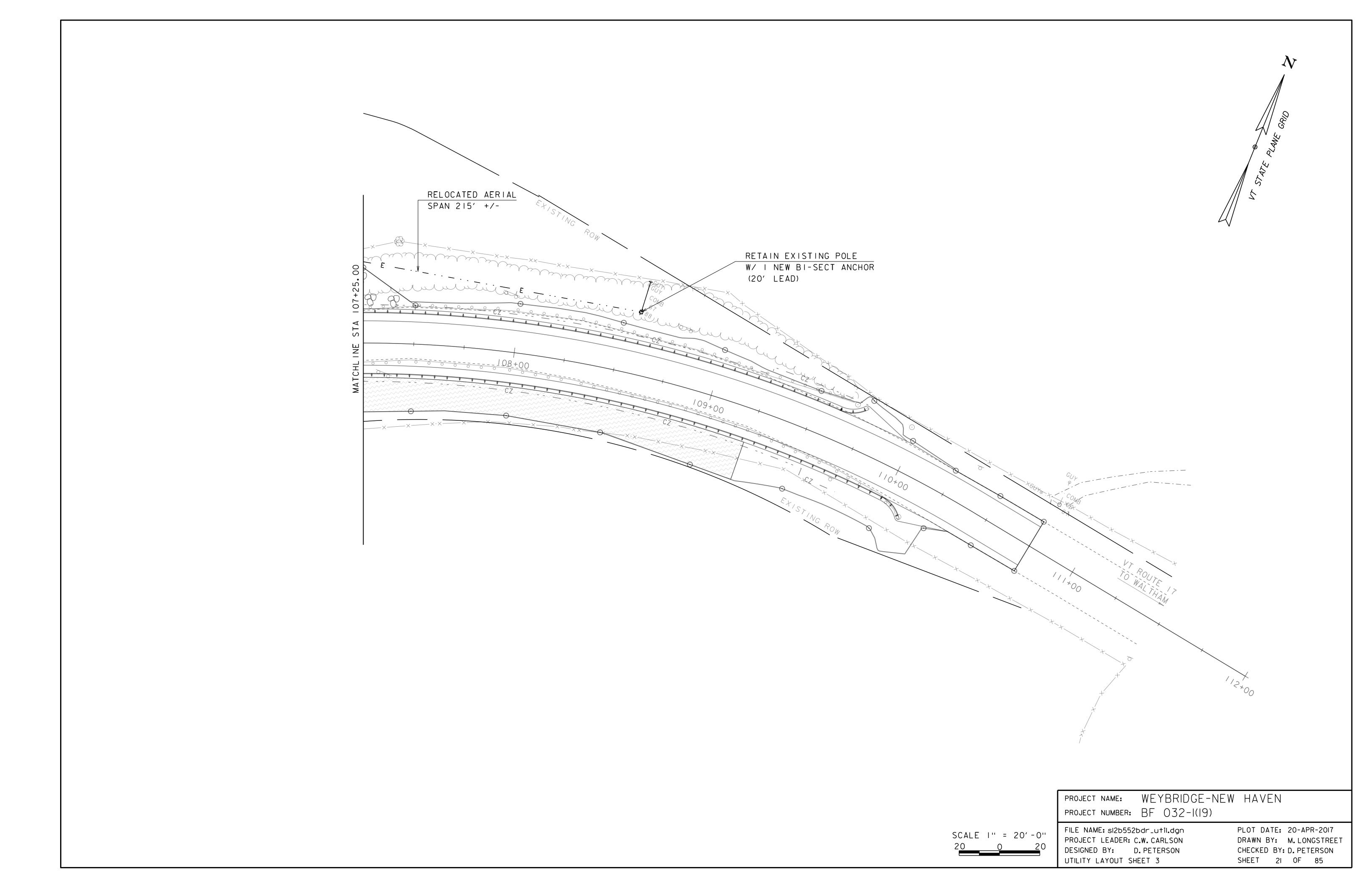


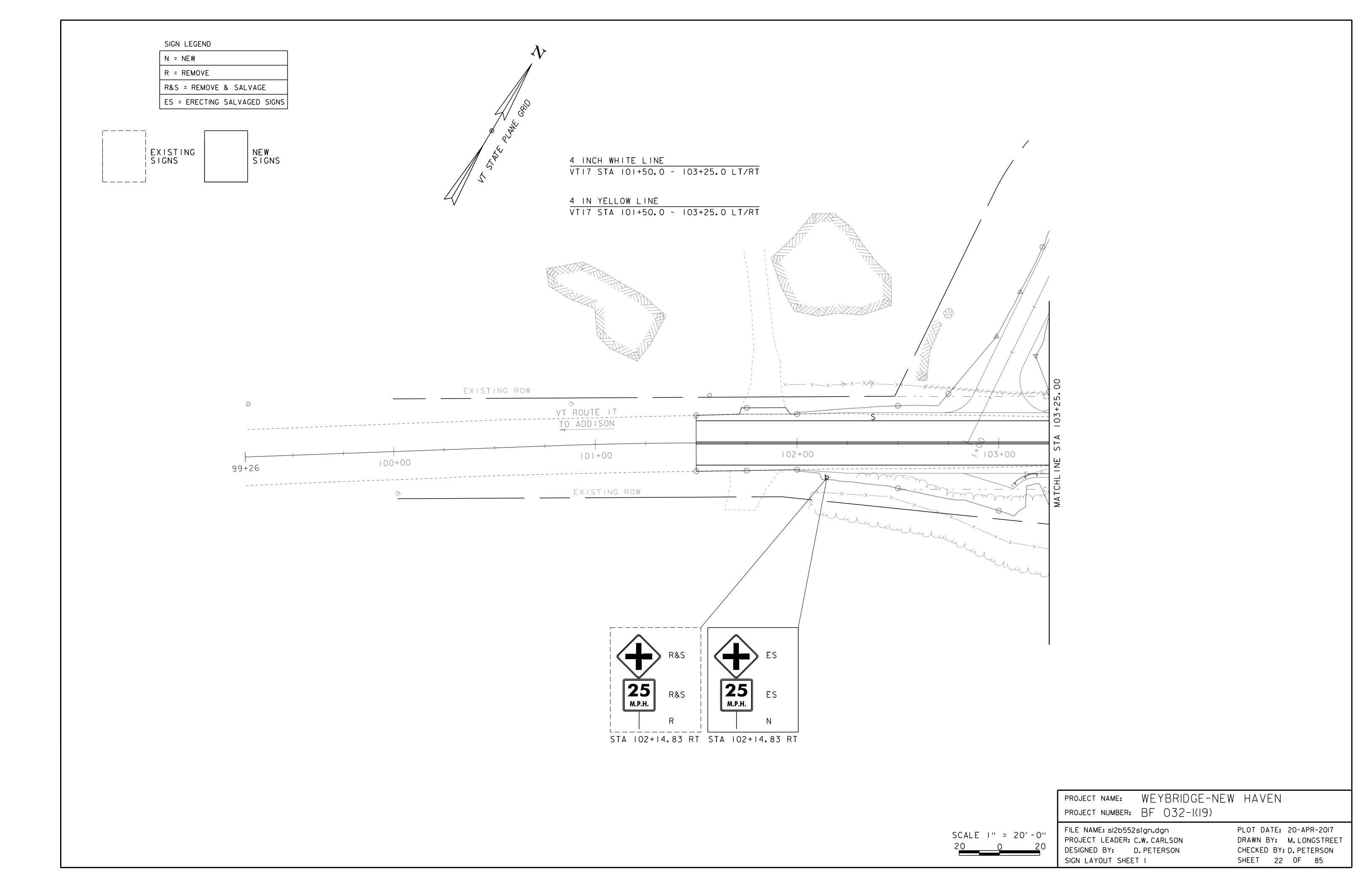
PROJECT NAME: WEYBRIDGE-NEW HAVEN PROJECT NUMBER: BF 032-1(19)

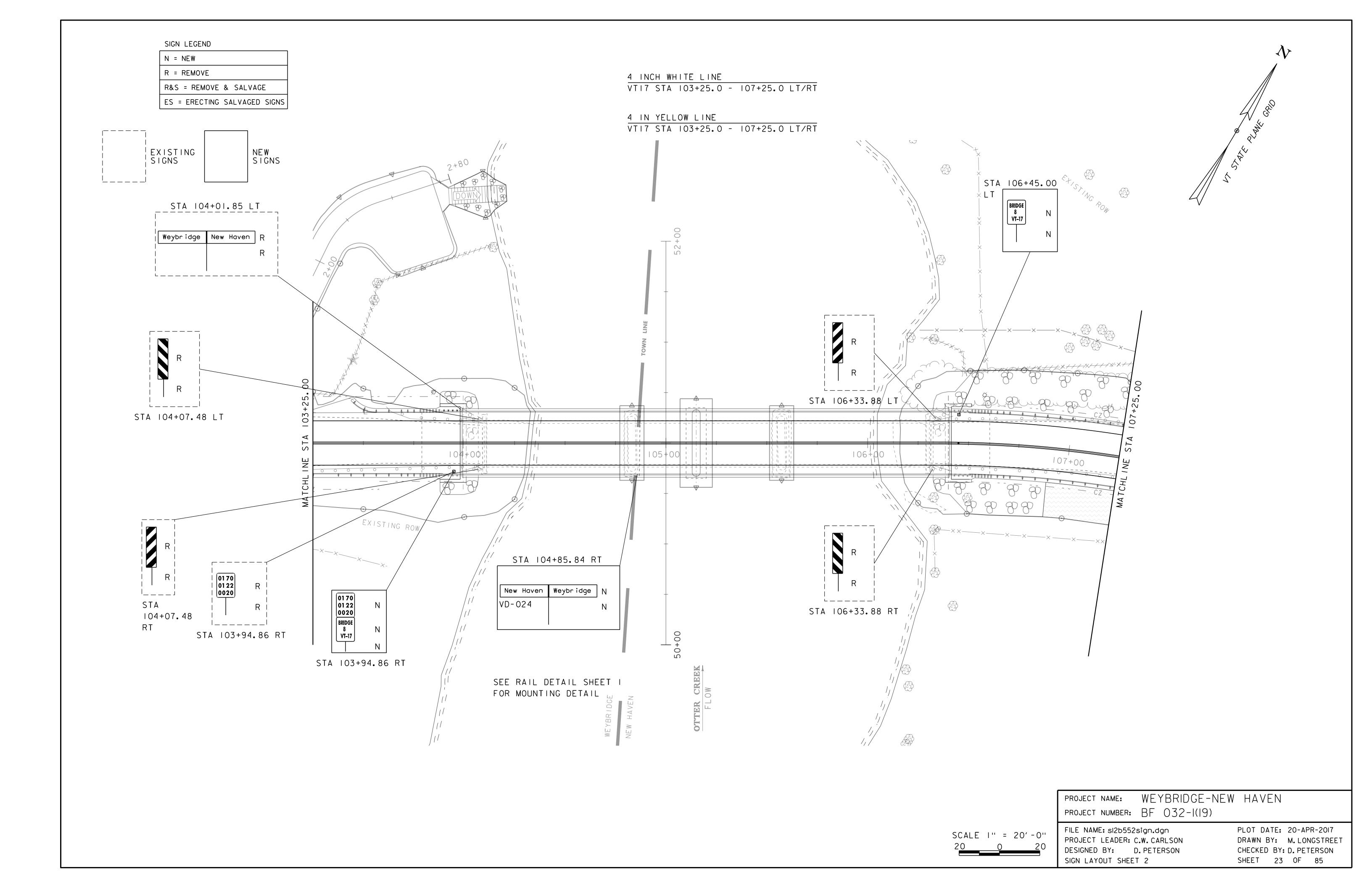
SCALE I'' = 20'-0'' 20 0 20 FILE NAME: sl2b552bdr_util.dgn
PROJECT LEADER: C.W. CARLSON
DESIGNED BY: D. PETERSON
UTILITY LAYOUT SHEET I

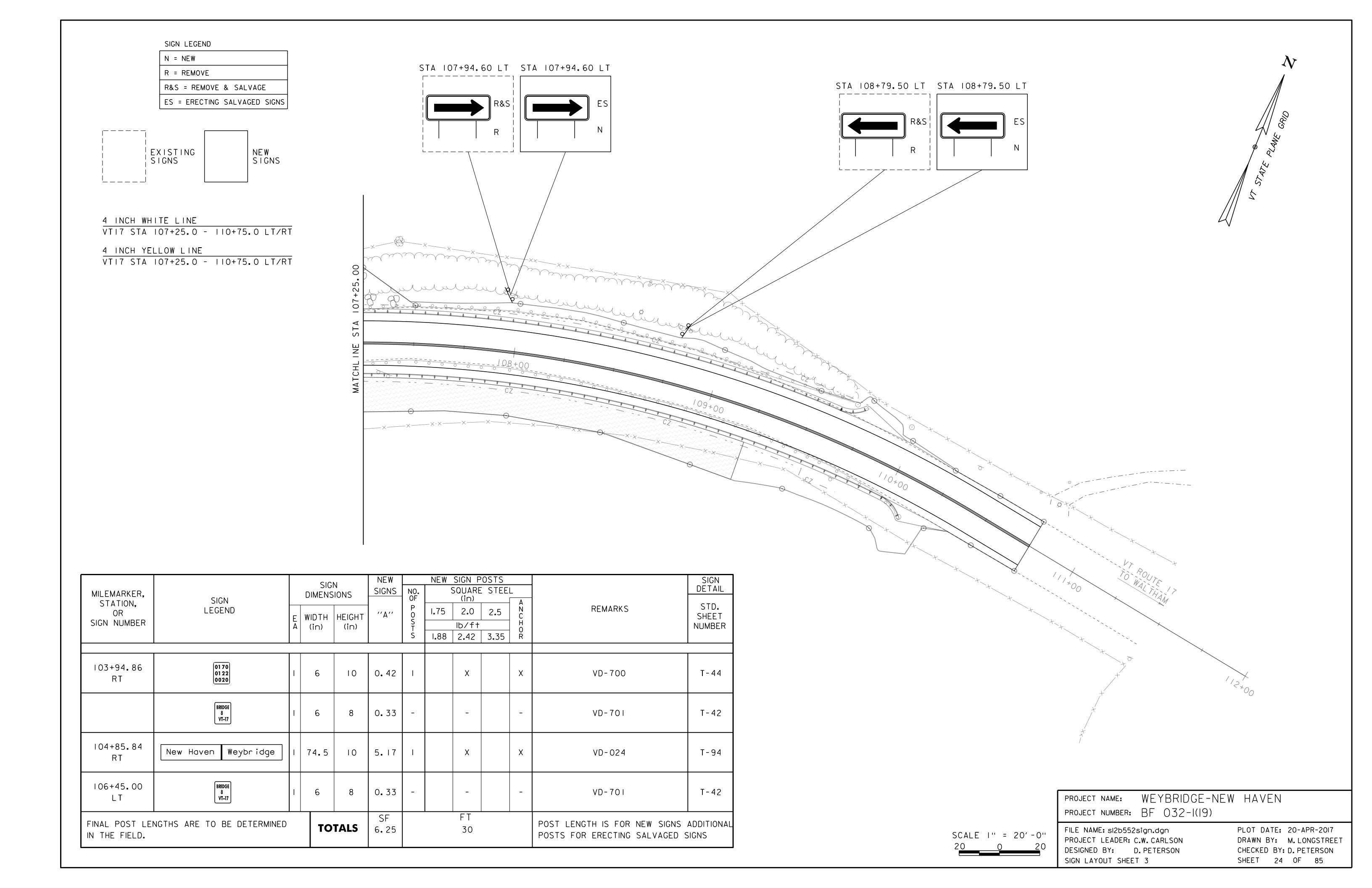
PLOT DATE: 20-APR-2017
DRAWN BY: M. LONGSTREET
CHECKED BY: D. PETERSON
SHEET 19 OF 85

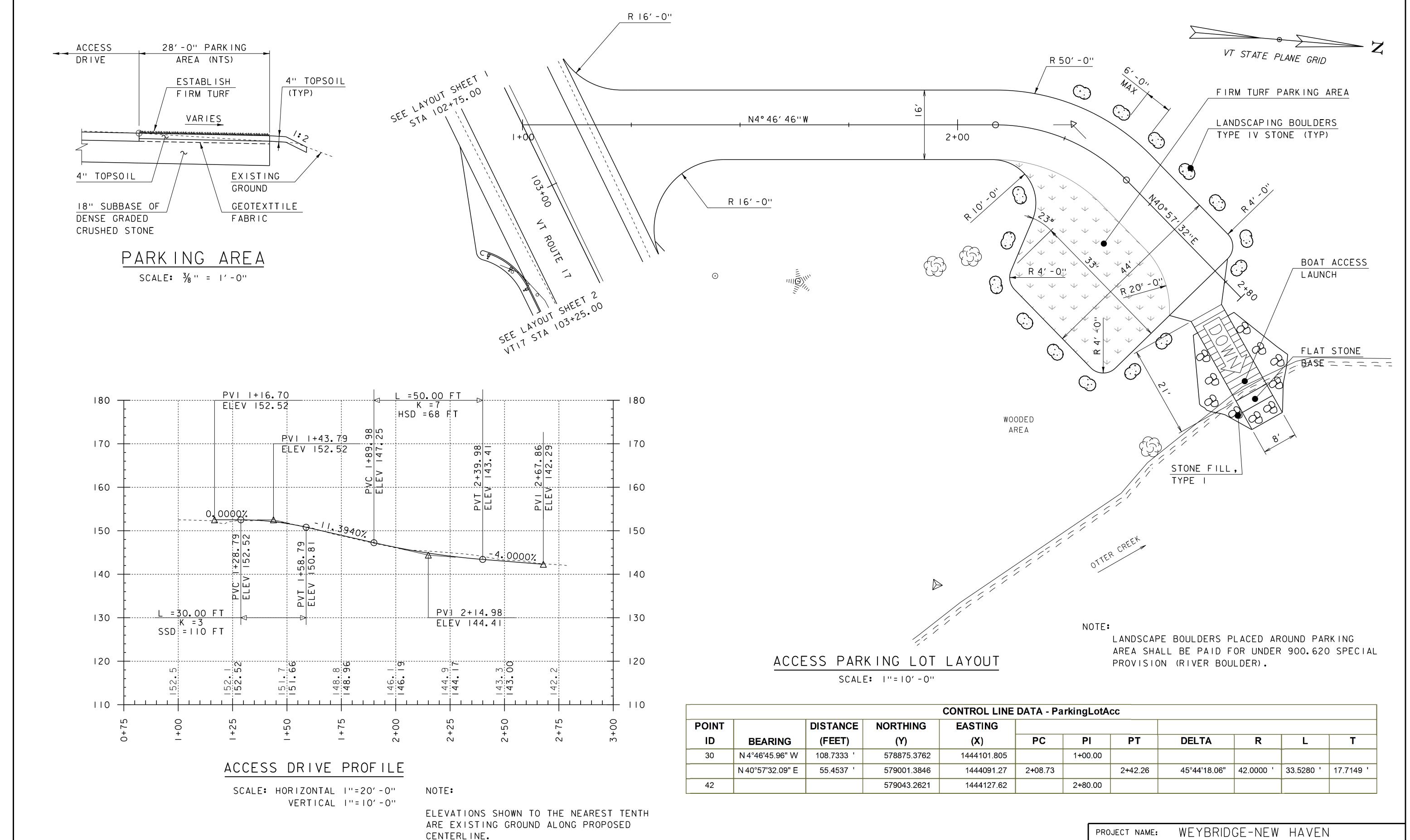












ELEVATIONS SHOWN TO THE NEAREST

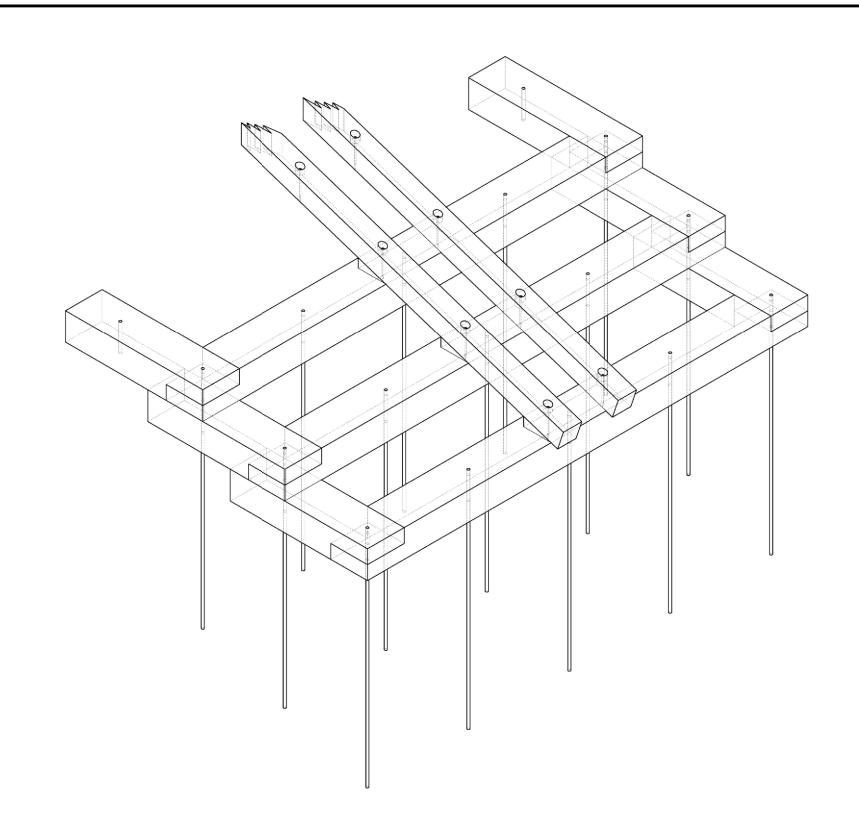
PROPOSED CENTERLINE.

HUNDREDTH ARE FINISH GRADES ALONG

PROJECT NAME: WEYBRIDGE-NEW HAVEN PROJECT NUMBER: BF 032-1(19)

FILE NAME: sI2b552rivAccess.dgn
PROJECT LEADER: C.W. CARLSON
DESIGNED BY: D. PETERSON
ACCESS & PARKING LAYOUT

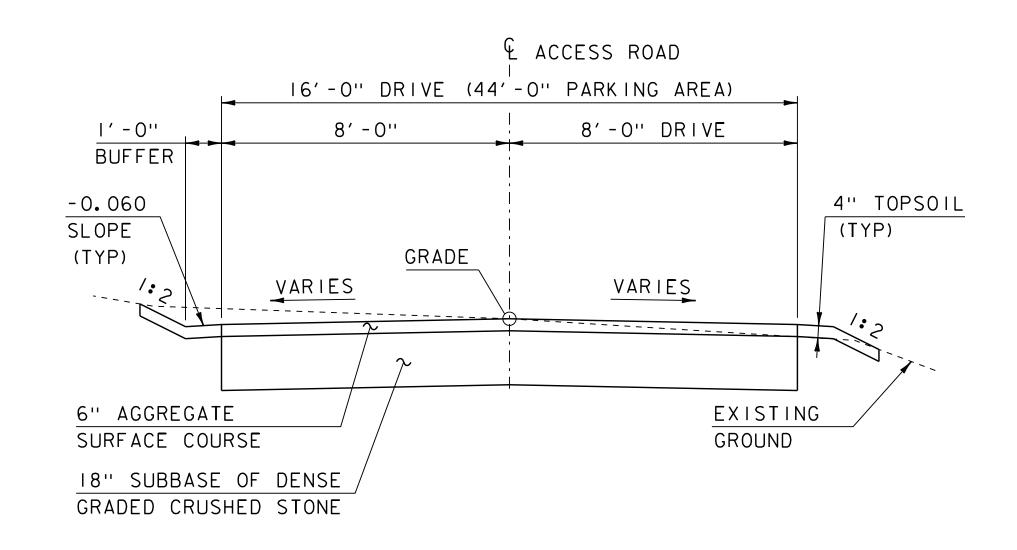
PLOT DATE: 20-APR-2017
DRAWN BY: M. LONGSTREET
CHECKED BY: D. PETERSON
SHEET 25 OF 85



# ISOMETRIC TREAD VIEW

NOT TO SCALE

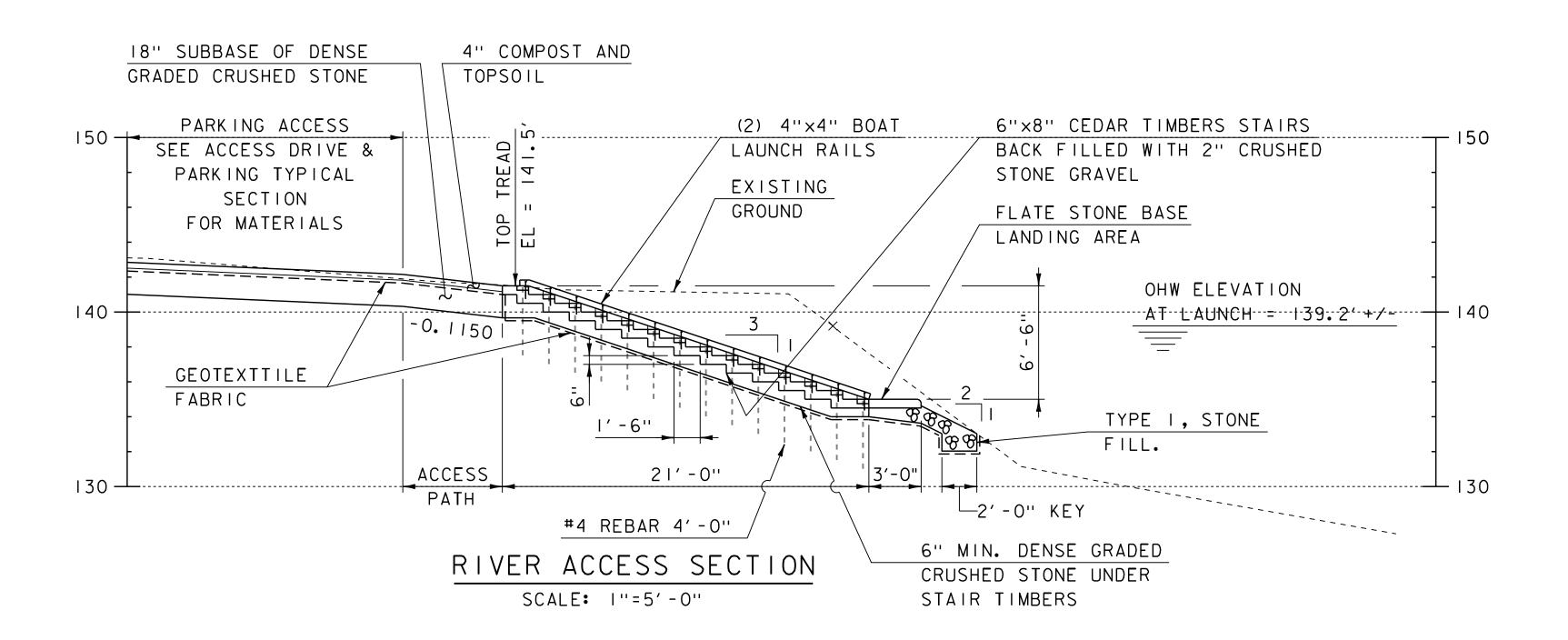
1) BOTTOM THREE TREADS SHOWN.

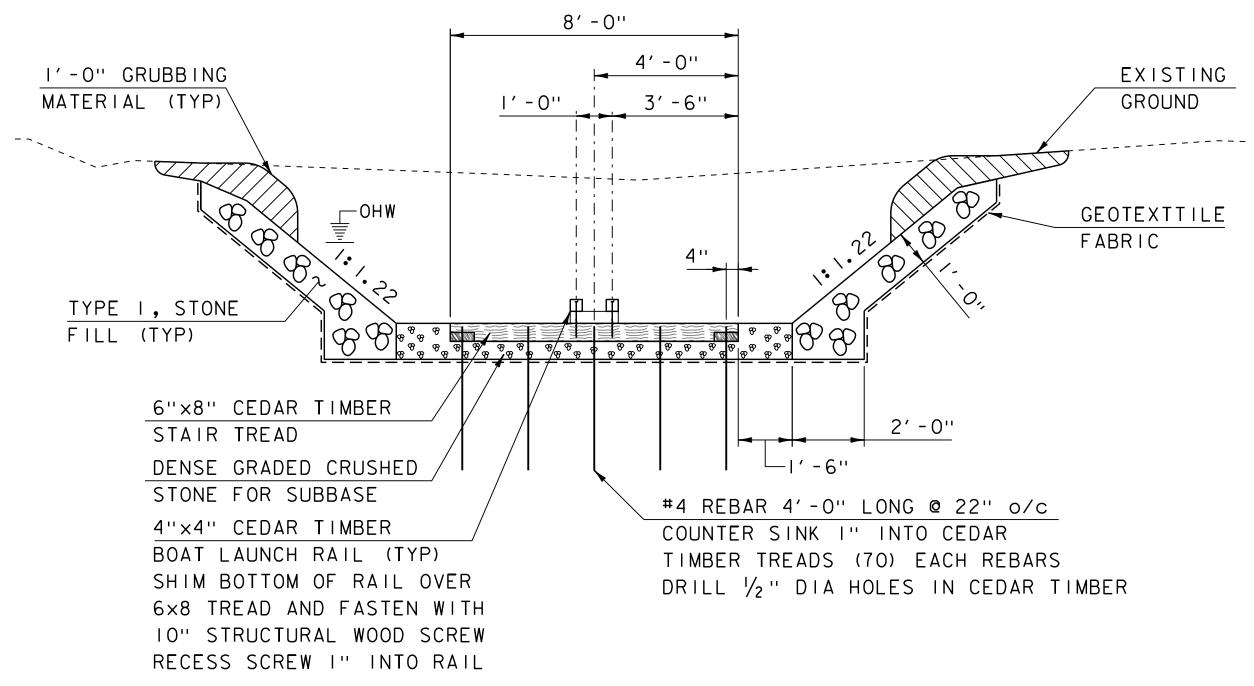


# ACCESS DRIVE & PARKING TYPICAL SECTION

SCALE:  $\frac{3}{8}$ " = 1'-0"

SEE ACCESS DRIVE CROSS SECTIONS FOR CROSS SLOPES OF DRIVE AND PARKING AREA.



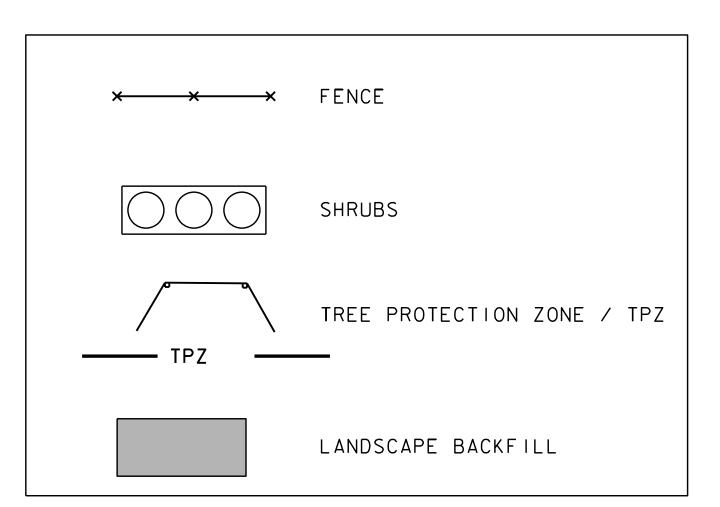


# RIVER ACCESS TYPICAL SECTION

SCALE: 3/8" = 1'-0"

PROJECT NAME: WEYBRIDGE-NEW HAVEN
PROJECT NUMBER: BF 032-1(19)

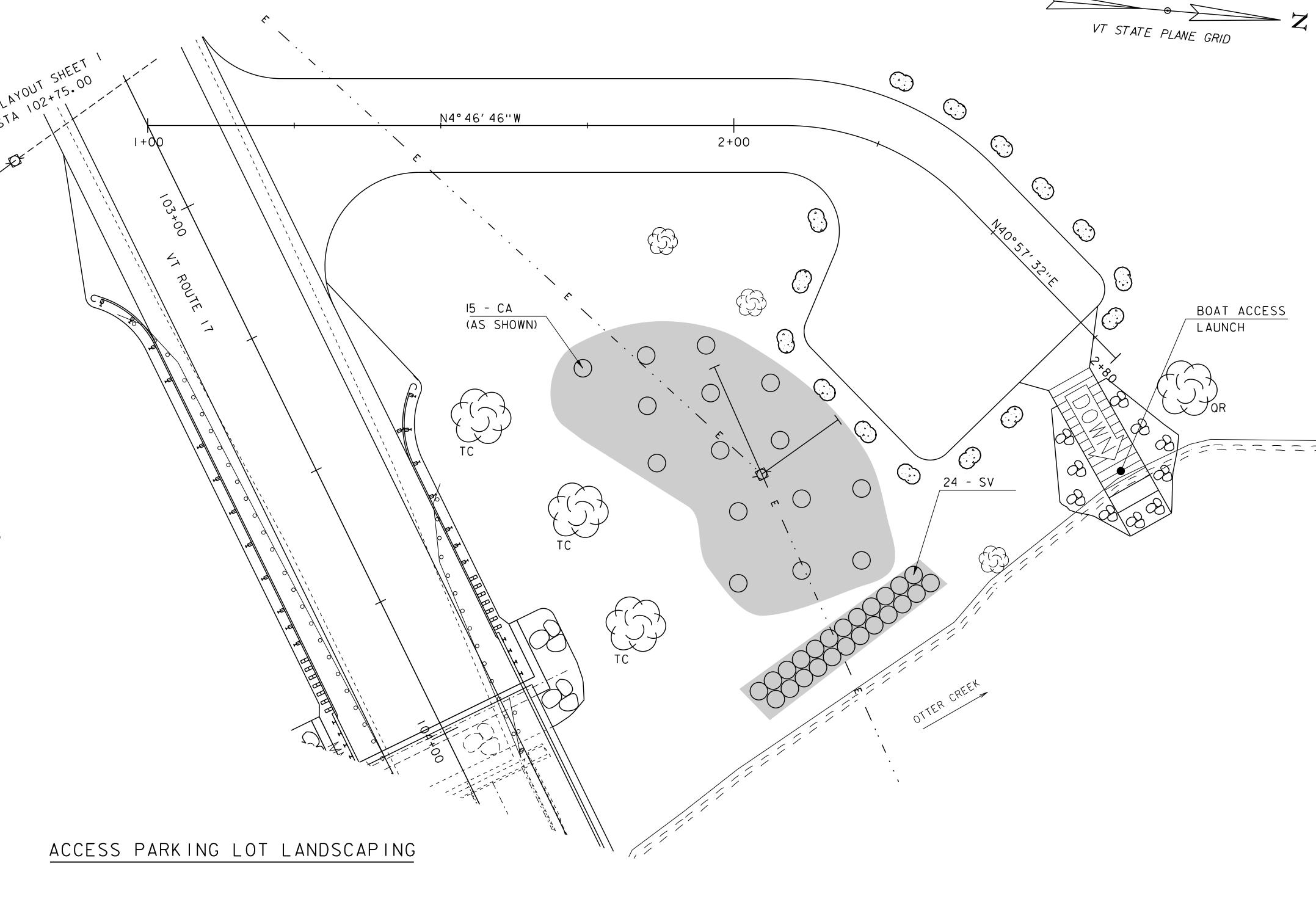
FILE NAME: s12b552rivAccess.dgn PLOT DATE: 20-APR-2017
PROJECT LEADER: C.W. CARLSON DRAWN BY: M. LONGSTREET
DESIGNED BY: D. PETERSON CHECKED BY: D. PETERSON
ACCESS & PARKING DETAILS SHEET 26 OF 85



# SEE LAYOUT SHEET '

# NOTES:

- I. PLANTING BED LOCATED IN UNDISTURBED SOIL BEFORE PLANTING, REMOVE EXISTING GRASS FROM ENTIRE PLANTING BED AREA. PROVIDE CONTINUOUS BARK MULCH FOR BED.
- 2. PLANTING BEDS LOCATED IN DISTURBED AREAS REMOVE COMPACTED FILL AND REPLACE WITH LANDSCAPE BACKFILL AT STATIONING SHOWN ABOVE. PROVIDE CONTINUOUS BARK MULCH FOR ALL SHRUB BEDS.
- 3. LOCATIONS AND BED SHAPES ARE APPROXIMATE AND MAY VARY DUE TO SLOPE. FINAL LOCATION AND ELEVATIONS TO BE DETERMINED BY THE ENGINEER WITH APPROVAL FROM VT FISH AND WILDLIFE FACILITY AND LANDS ADMINISTRATOR.
- 4. GROUPING SHAPES ARE TO BE STAKED AND LAID OUT TO GIVE A NATURAL APPEARANCE.
- 5. TYPICAL GROUPING LAYOUTS SHOWN ARE FOR FLAT AND STEEP SLOPES OF PROJECT WHERE THERE IS NO STONE FILL.
- 6. WATER ALL SHRUBS AND SEEDLING TREES AT TIME OF PLANTING. EACH SHRUB AND SEEDLING TREE SHALL RECEIVE A MINIMUM OF 5 GALLONS OF WATER TWICE A WEEK.

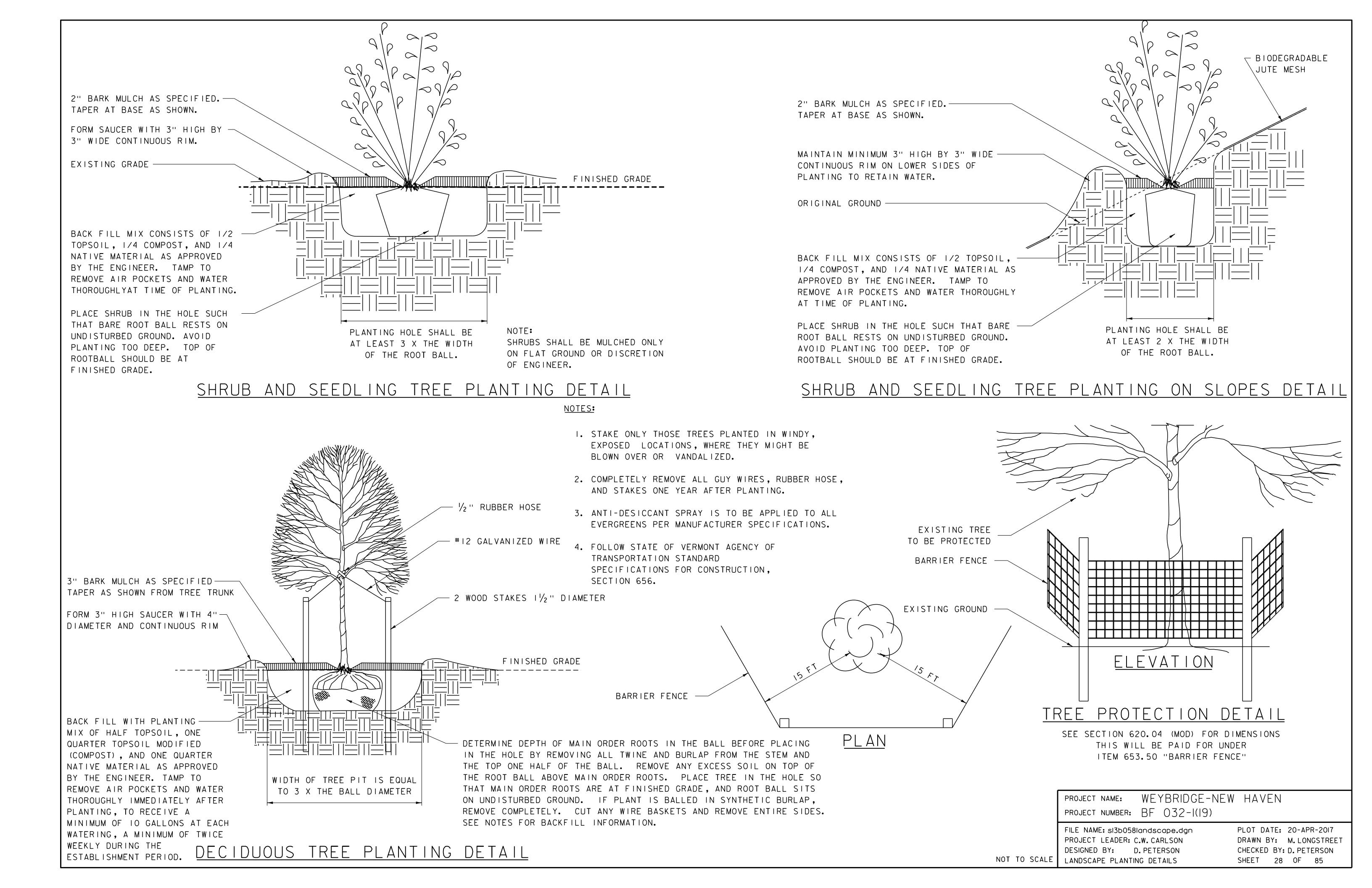


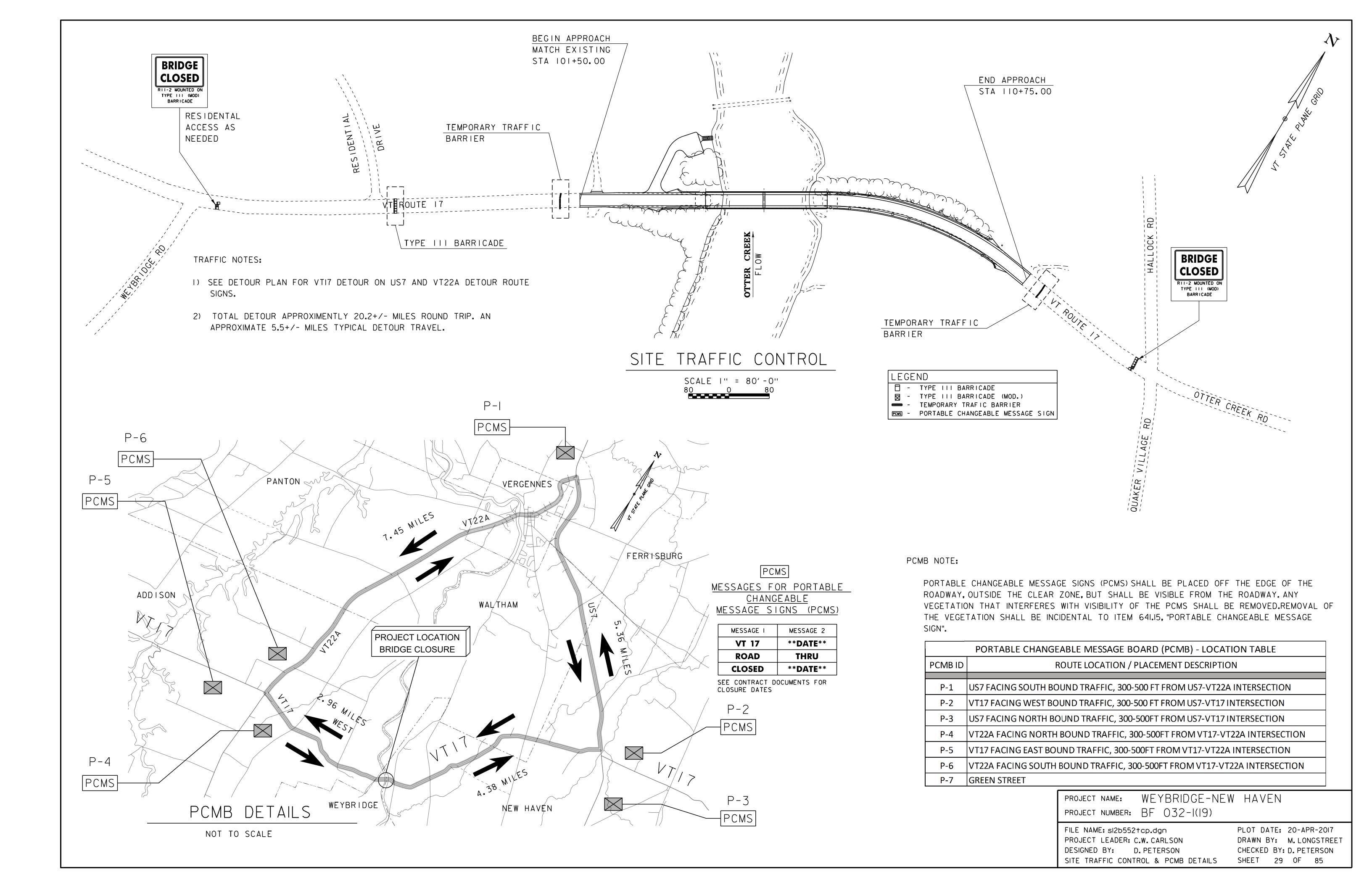
QTY	KEY	BOTANICAL NAME	COMMON NAME	SIZE	CONTAINER	SPACING, REMARKS, ITEMS
		DECIDUOUS SHRUBS				<u>ITEM No. 656.35</u>
15	CA	CORNUS AMOMUM	SILKY DOGWOOD	3'-4' HEIGHT	CONTAINER	5 FT o.c. (min.)
24	SV	CEPHALANTHUS OCCIDEBTALIS	BUTTON BUSH	3'-4' HEIGHT	CONTAINER	5 FT o.c. (min.)
		DECIDUOUS TREES				<u>ITEM No. 656.30</u>
1	QR	QUERCUS BICOLOR	SWAMP WHITE OAK	3'-4' HEIGHT	CONTAINER	AS SHOWN
3	TC	ACER SACCHARINUM	SILVER MAPLES	4'-5' HEIGHT	CONTAINER	AS SHOWN

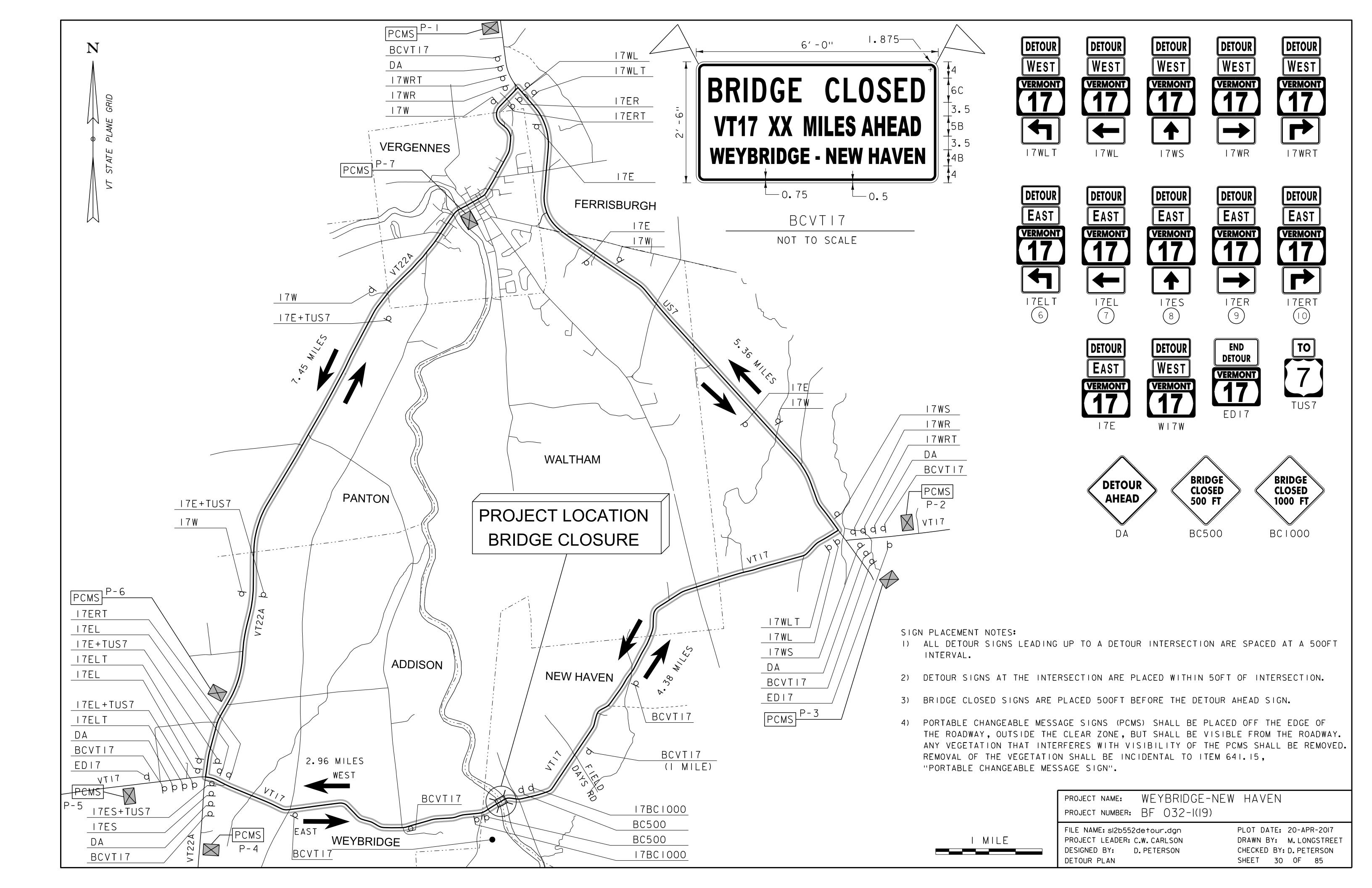
PROJECT NAME: WEYBRIDGE-NEW HAVEN PROJECT NUMBER: BF 032-1(19)

FILE NAME: sl3b058landscape.dgn PROJECT LEADER: C.W. CARLSON DESIGNED BY: D. PETERSON ACCESS & PARKING LANDSCAPING

PLOT DATE: 20-APR-2017 DRAWN BY: M. LONGSTREET CHECKED BY: D. PETERSON SHEET 27 OF 85







# SOIL CLASSIFICATION

# AASHTO

Gravel and Sand
Fine Sand
Silty or Clayey Gravel and Sand
Silty Soil - Low Compressibility

Silty Soil - Highly Compressible
Clayey Soil - Low Compressibility
Clayey Soil - Highly Compressible

# ROCK QUALITY DESIGNATION

R.O.D. (%)	ROCK DESCRIPTION
<25	Very Poor
25 to 50	Poor
51 to 75	Fair
76 to 90	Good
>90	Excellent

# SHEAR STRENGTH

# UNDRAINED SHEAR STRENGTH

| CONSISTENCY | CONSISTENCY | Very Soft | Soft | Soft | Soft | Stiff | Stiff | Very Stiff | Yery Stiff | Yery

# CORRELATION GUIDE OF "N" TO DENSITY/CONSISTENCY

	DENSITY IULAR SOILS)		NSISTENCY ESIVE SOILS)
<u>N</u>	DESCRIPTIVE TERM	<u>N</u>	DESCRIPTIVE TERM
<5 5-10 Ⅱ-24 25-50 >50	Very Loose Loose Med. Dense Dense Very Dense	<2 2-4 5-8 9-15 16-30 31-60 >60	Very Soft Soft Med. Stiff Stiff Very Stiff Hard Very Hard

# COMMONLY USED SYMBOLS

Water Elevation Standard Penetration Boring Auger Boring Rod Sounding Sample Standard Penetration Test Blow Count Per Foot For: 2"O.D. Sampler  $1\frac{3}{8}$ "I.D. Sampler Hammer Weight Of 140 Lbs. Hammer Fall Of 30" Field Vane Shear Test Undisturbed Soil Sample US Blast Diamond Core Mud Drill WΑ Wash Ahead Hollow Stem Auger Core Size 1/8 Core Size 15/8' Core Size 2 1/8" Double Tube Core Barrel Used Liquid Limit Plastic Limit Plasticity Index Non Plastic Moisture Content (Dry Wgt. Basis) Dry Moist Moist To Wet Wet Sat Saturated Boulder Gr Gravel Sa Sand Si Sil+ CI Clay Hardpan Le Ledge No Ledge To Depth Can Not Penetrate Further Top of Ledge Or Boulder No Recovery Rec. Recovery Percent Recovery Rock Quality Designation California Bearing Ratio Less Than

		COLOR	
blk	Black	pnk	Pink
bl	Blue	pu	Purple
brn	Brown	rd	Red
dk	Dark	tn	Tan
gry	Gray	wh	White
gn	Green	yel	Yellow
lt	Light	mltc	Multicolored

Greater Than

VTSPG NAD83 - See Note 7

0range

Refusal (N > 100)

# B-I06 B-IOI **ब** ∄B-I05 B-102 100 05+00 106+d0 B-I03

BORING LAYOUT

SCALE I'' = 30'-0"

**A** ---==';

# DEFINITIONS (AASHTO)

or

BEDROCK (LEDGE) - Rock in its native location of indefinite thickness.

BOULDER - A rock fragment with an average dimension > 12 inches.

COBBLE - Rock fragments with an average dimension between 3 and

GRAVEL - Rounded particles of rock < 3" and > 0.0787" (*10 sieve).

12 inches.

SAND - Particles of rock < 0.0787" (#10 sieve) and > 0.0029" (#200 sieve).

SILT - Soil < 0.0029" (#200 sieve), non or slightly plastic and exhibits no strength when air-dried.

CLAY - Fine grained soil, exhibits plasticity when moist and considerable strength when air-dried.

VARVED - Alternate layers of silt and clay.

HARDPAN - Extremely dense soil, cemented layer, not softened when wet.

MUCK - Soft organic soil (containing > 10% organic material.

MOISTURE CONTENT - Weight of water

divided by dry weight of soil.

FLOWING SAND - Granular soil so saturated (loose) that it flows into drill casing during extraction

of wash rod.

STRIKE - Angle from magnetic north
to line of intersection of bed
with a horizontal plane.

DIP - Inclination of bed with a horizontal plane.

# I. The subsurface explorations shown herein were made between 10/25/16 and 12/21/16 by the Agency.

2. Soil and rock classifications, properties and descriptions are based on engineering interpretation from available subsurface information by the Agency and may not necessarily reflect actual variations in subsurface conditions that may be encountered between individual boring or sample locations.

3. Observed water levels and/or conditions indicated are as recorded at the time of exploration and may vary according to the prevailing rainfall, methods of exploration and other factors.

# GENERAL NOTES

4. Engineering judgment was exercised in preparing the subsurface information presented herein. Analysis and interpretation of subsurface data was performed and interpreted for Agency design and estimating purposes. Presentation of the information in the Contract is intended to provide the Contractor access to the same data available to the Agency. The subsurface information is presented in good faith and is not intended as a substitute for personal investigation, independent interpretation, independent analysis or judgment by the Contractor.

5. Pictorial structure details shown on the boring plan layout or soils profile are for illustrative purposes only and may not accurately portray final contract details.

6. Terminology used on boring logs to describe the hardness, degree of weathering, and spacing of fractures, joints and other discontinuities in the bedrock is defined in the AASHTO Manual on Subsurface Investigations, 1988.

B-109

B-II2

107+00

7. Northing and Easting coordinates are shown in Vermont State Plane Grid North American Datum 1983 in meters and survey feet.

POINT	STATION	OFFSET	ELEVATION	BEDROCK
B-101	103+87.99	-16.75	152.7	141.3
B-102	103+91.00	5.00	153.3	137.6
B-103	103+88.00	16.75	151.7	136.8
B-104	104+01.00	19.50	150.3	131.3
B-105	104+00.85	-5.00	153.7	137.9
B-106	104+04.18	-27.43	144.8	138.8
B-107A	104+87.00	7.00	155.1	130.6
B-108A	105+53.20	-7.00	156.2	128.7
B-109	106+38.41	-15.00	154.5	96.5
B-112	106+38.98	5.65	156.6	94.3
B-113	107+40.40	3.76	157.4	n/a
B-114	108+41.61	4.00	160.0	n/a

STATION | OFFSET | FLEVATION

PROJECT NAME: WEYBRIDGE-NEW HAVEN PROJECT NUMBER: BF 032-1(19)

FILE NAME: sI2b552bor.dgn
PROJECT LEADER: C.W. CARLSON
DESIGNED BY: D. PETERSON
BORING INFORMATION SHEET

108+00

PLOT DATE: 20-APR-2017
DRAWN BY: M. LONGSTREET
CHECKED BY: D. PETERSON
SHEET 31 OF 85

**TOP OF** 

		STATE OF VERMONT			ВС	RING	LOG			Воі	ring N	lo.:	B-1	01
<b>VT</b>	ransw	Orking to Get You There The Construction And Materials Bureau  Orking to Get You There  MATERIALS BUREAU	idge-Ne F 032-1		en					1 of 12b55				
		CENTRAL LABORATORY	•		V	/T 17 Br	. #8			Ch	ecked	d By:	<u>E</u> N	ND.
Boring (	Crew [.]	Judkins, Gomes			Casin	g Sar	npler		Grou	undwa	ater C	) bserva	ations	
Date St		11/17/16 Date Finished: 11/18/16	Type:		WB		SS	Da	te	Dep		N	otes	
	NAD83:		I.D.: Hamm	er Wt:	3 in N.A.		<u>5 in</u> 0 lb.	4 4 4 6	V4.0	(ft)				
Station:		+88.00 Offset: -16.75		er Fall:	N.A.		) in.	11/18	3/16			No W.T	. obse	rve
	Elevation			er/Rod ]	_	Auto/A								
Ground	Lievatioi	1. <u>132.7 IL</u>	Rig: _	Diedr	ich 25		<u>Jnkno</u> v					T		ı
Depth (ft)	Strata (1)	CLASSIFICATION OF MATI (Description)	ERIALS			Run (Dip deg.)	Core Rec. % (RQD %)	Drill Rate minutes/ft	"9/swo	(N Value)	Moisture Content %	Gravel %	Sand %	% Socia
	St.	(= ====================================					Core (R	֡֞֞֞֓֞֟֞֟֓֟֝֟ <u>֟</u>	B	Z)	≱်၀	_ \bar{D}	Š	<u>ت</u> ا
7	X	Field Note:, Fill for drilling pad												
4	<b>* * *  </b>													
┤	X													
6	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	A-1-b, GrSa, brn, Dry, Rec. = 0.6 ft								-3-4	11.5	42.7	44.2	13
2.5									(4	4)				
-0.1	$\bigcup_{i=1}^{\infty} \bigcup_{j=1}^{\infty} o_{ij}$													
		A-1-b, SaGr, brn, Moist, Rec. = 1.4 ft							6-4-	-3-3	7.4	48.8	32.5	   18
5.0	0 - 0								(7					
3.0														
1.0		A-4, GrSiSa, brn, Moist, Rec. = 1.3 ft							3-4-	-3-3	12.2	25.4	37 5	37
1		A-4, GISISA, BITI, MOISI, INCC. – 1.3 II							(7	_ `	12.2	25.4	37.3	
7.5														
7.5		A O A O O'O I AA ' I D O O U							40.4		40.0	00.0	44.0	
		A-2-4, GrSiSa, brn, Moist, Rec. = 0.8 ft							1	8-7- 8	10.3	28.0	41.8	30
-  C									(5	5)				
10.0		A 4 0 1 M 1 4 D 0 7 % 1 1 N 4								4-7	0.0	70.0	45.0	_
-0		A-1-a, Gr, brn, Moist, Rec. = 0.7 ft, Lab Note: was within sample	A lot of b	oroken ro	OCK				3-4 R@ (F	47- 95" २)	9.0	76.8	15.9	7.
-8		11.4 ft - 14.4 ft, No Recovery. BX				1	0	8		Тор	of Be	drock	@ 11.4	4 ft
40.5		·				(0)	(0)			·				
12.5								5						
*								6						
-														
15.0		14.4 ft - 16.4 ft, Blue gray to gray, DOLOMITI Faint yellowish-orange staining on joints Ha			ns.	2 (55)	60 (0)	7						
-5.5		weathered, Poor rock, BX, RMR=33	, v Oi y S	тэнчу				7						
+														
- <u> </u> -		16.4 ft - 18.4 ft, Blue gray, DOLOMITE, with o Brownish-yellow/orange staining on joints H				3 (55)	45	7						
17.5		weathered, Poor rock, BX, RMR=27	aru, VEIY	angridy		(55)	(0)	8						
-														
+	,	Hole stopp	ed @ 18.	4 ft			•	•	•			•		•
20.0		Remarks:												
-		Hole collapsed at 0.0 feet.												
1														
-														
22.5														
22.5														

BOTTOM OF PILE CAP

ELEV. 145.00

**BORING LOG** Boring No.: B-102 STATE OF VERMONT AGENCY OF TRANSPORTATION 1 of 1 Page No.: Weybridge-New Haven CONSTRUCTION AND MATERIALS BUREAU BF 032-1(19) Pin No.: 12b552 CENTRAL LABORATORY VT 17 Br. #8 END Checked By: Casing Sampler Groundwater Observations **Boring Crew:** Judkins, Gomes Type: Depth Date Notes Date Started: ____10/31/16__ Date Finished: ____11/01/16 I.D.: 4 in 1.5 in N.A. 140 lb. Hammer Wt: VTSPG NAD83: N 578925.65 ft E 1444195.78 ft 11/01/16 15.3 W.T. Before Drilling 30 in N.A. Hammer Fall: 103+91.00 Offset: 5.00 Station: Hammer/Rod Type: Auto/AWJ 153.3 ft Ground Elevation: Rig: <u>CME 55 TRACK</u> <u>CE = 1.41</u> Run (Dip deg. CLASSIFICATION OF MATERIALS epth (ff) (Description) Asphalt Pavement, 0.0 ft - 1.0 ft 15-13-26-15 (39) | 6.4 | 54.6 | 33.7 | 11.7 ○ ◯ ◯ ○ ↑ A-1-a, SaGr, brn, Moist, Rec. = 1.1 ft Field Note:, Rollercone, cleaned out casing. A-2-4, SiSaGr, brn, Moist, Rec. = 1.1 ft 12-14-9- | 8.6 | 40.9 | 31.4 | 27.7 | (23)Field Note:, Rollercone, cleaned out casing. Field Note:, No Recovery, Rec. = 0.0 ft 3-5-4-4 A-1-b, SiSaGr, brn, Moist, Rec. = 0.8 ft 2-4-8-7 | 10.2 | 46.0 | 31.9 | 22.1 Field Note:, Rollercone, cleaned out casing. 6-5-6-13 9.3 36.7 38.5 24.8 A-1-b, SiGrSa, brn, Moist, Rec. = 0.6 ft A-2-4, GrSiSa, brn, Moist, Rec. = 0.7 ft, Lab Note: A few | 23-21-4- | 18.3 | 24.4 | 45.5 | 30.1 | pieces of wood and sticks were within sample. Field Note:, NXDC, cleaned out casing. 5-4-3-4 | 27.8 | 1.9 | 30.9 | 67.2 | 28 | 8 A-4, CISi, gry, Moist, Rec. = 1.3 ft, Lab Note: Pieces of decomposing wood were within sample. Field Note:, NXDC, cleaned out casing. | 17.8 | 60.4 | 11.8 | 27.8 A-2-4, SiGr, gry-red, Moist, Rec. = 0.5 ft, Lab Note:  $\setminus$  Broken Rock was within sample. Insufficient sample size  $\mid$ Top of Bedrock @ 15.7 ft R-1 | 100 | 5 (40) |(100)| to perform Atterberg Limit testing. Similar to 13-15 FT ∖sample. 15.7 ft - 20.7 ft, Blue gray to black, Interbedded META-LIMESTONE, and DOLOMITE with calcite veins. Calcite coating and brown staining on joints. Hard, Slightly weathered, Good rock, NX, RMR=69 20 -20.7 ft - 25.7 ft, Blue gray to black, Interbedded R-2 | 100 | 7 META-LIMESTONE, DOLOMITE, and SHALE. Hard, (40-45) (62) Unweathered, Fair rock, NX, RMR=49 25 Hole stopped @ 25.7 ft

Notes: 3. Water level readings have been made at times and under conditions stated. Fluctuations may occur due to other factors than those present at the time measurements were made.

BOTTOM OF

PILE CAP

ELEV. 145.00

Remarks:

Hole collapsed at 7.3 feet.

<<SUB>><<SUB>> is the hammer energy correction factor.

1. Stratification lines represent approximate boundary between material types. Transition may be gradual.

PROJECT NAME: WEYBRIDGE-NEW HAVEN PROJECT NUMBER: BF 032-1(19)

FILE NAME: sl2b552bor.dgn
PROJECT LEADER: C.W. CARLSON
DESIGNED BY: D. PETERSON
BORING LOGS I

PLOT DATE: 20-APR-2017
DRAWN BY: M. LONGSTREET
CHECKED BY: D. PETERSON
SHEET 32 OF 85

	STATE OF VERMONT ICY OF TRANSPORTA				RING				oring N age No		1 of	
VIrans Working to Get You There Vermont Agency of Transportation C	dge-Ne F 032-1 T 17 Br.	(19)	en	Pi	n No.: necked		12b55 EN	2				
Boring Crew: Gomes, Judki	ing Emorgon			Casing	g San	npler		Groundy				
	ished: 11/14/16	Type:	-	WB		<u>ss</u>	Dat	e De	pth	N	otes	
	t E 1444199.23 ft	I.D.: Hamme	er Wt	4 in N.A.	_	<u>5 in</u> 0 lb.		(f				
		Hamme	-	N.A.		) in.	11/14	/16   ′	14.8 V	V.T. af	ter dril	ling
Station: <u>103+88.00</u> Offs	et: <u>16.75</u>		er/Rod Ty									
Ground Elevation: 151.7 ft	_	Rig: _	CME 55 1	RACK	<u>CE</u>	= 1.41				ı	I	
Obepth (ft) (ft) Strata (1)	ASSIFICATION OF MAT (Description)	TERIALS			Run (Dip deg.)	Core Rec. % (RQD %)	Drill Rate minutes/ft	Blows/6" (N Value)	Moisture Content %	Gravel %	Sand %	Fines %
メ メ メ Field Note:, Fill for di	rilling pad											
- * * *												
] * * *												
* * *												
Field Note:, No Reco	overy							1-3-2-2				
-								(5)				
5 Field Note:, No Reco	N/OF/							4-3-4-6				
rield Note., No Necc	overy							(7)				
Field Note:, NXDC, 0	Cleaned out casing											
A-2-4, SiSa, brn, Mo	ist, Rec. = 0.7 ft							5-3-3-3	11.5	19.1	47.2	33.
								(6)				
Field Note:, NXDC, C								6-4-5-3	12.8	30.1	45.3	24
10 – (°) A-2-4, SIGISA, BITI, I	woist, 14ec. – 0.5 it							(9)	12.0	30.1	45.5	~-
	ne, Cleaned out casing											
A-4, SaSi, brn, Moist	, Rec. = 0.1 ft							3-2-3-2 (5)	32.2	0.5	21.3	78.
1///								(0)				
A-4, SaGrSi, brn, Mo	oist Rec = 0.5 ft							5-11-	19.6	38.7	22.3	39
7/// // // // // // // // // // // // //								Ř@1" (R)	1010			
15 / 14 9 ft - 19 9 ft Blue	gray, DOLOMITE, with	calcite vei	ns Yellow	,	1	100	5		of Bed	drock (	ଲ 14 ⁹	9 ft
and tan staining alor	ig joints Hard, Slightly				(40)	(38)		106	Orbot			
/ / NX, RMR=49							4					
							4					
-							4					
-							3					
20 - / 19.9 ft - 24.9 ft Blue	DOLOMITE ::	1-:4		_		0.4						
19.9 It - 24.9 It, Dide	gray, DOLOMITE, with with tan staining. Hard,				2 (50)	94 (72)	13					
Good rock, NX, RMF	R=62						9					
-							7					
-							6					
							5					
25							J					
	Hole stop	ped @ 24.	9 ft									
Remarks: Hole collapsed at 7.0	) feet.											
l I												
-												
1. Stratification lines represent approximat _{&gt;_{&gt; is the hammer energy  Water level readings have been made a}}	e boundary between material ty	ypes. Transitio	n may be gra	idual.								

BOTTOM OF

ELEV. 145.00

PILE CAP

**BORING LOG** Boring No.: B-104 STATE OF VERMONT AGENCY OF TRANSPORTATION Page No.: ___1 of 1 Weybridge-New Haven I rans Working to Get You There Vermont Agency of Transportation CONSTRUCTION AND MATERIALS BUREAU BF 032-1(19) 12b552 Pin No.: CENTRAL LABORATORY VT 17 Br. #8 END Checked By: Casing Sampler Groundwater Observations **Boring Crew:** Judkins, Emerson Type: SS Date Depth Notes Date Started: ___11/14/16__ Date Finished: ___11/16/16_ 1.5 in 4 in N.A. 140 lb. Hammer Wt: VTSPG NAD83: N 578918.32 ft E 1444211.80 ft 16.4 W.T. Before Drilling 11/15/16 N.A. 30 in. Hammer Fall: Offset: 104+1.00 19.50 Station: 11.9 W.T. Before Drilling 11/16/16 Hammer/Rod Type: Auto/AWJ 150.3 ft Ground Elevation: Rig: CME 55 TRACK CE = 1.41 Depth (ft) **CLASSIFICATION OF MATERIALS** (Description) Field Note:, Fill for Pad X X X * * * * * * * * * Field Note:, NXDC, Cleaned out casing 2-2-2-1 Field Note:, No Recovery Field Note:, No Recovery 1-1-1-2 Field Note:, NXDC, Cleaned out casing 4-4-5-4 | 9.3 | 40.3 | 36.1 | 23.6 A-1-b, SiSaGr, brn, Moist, Rec. = 0.7 ft Field Note:, NXDC, Cleaned out casing 5-3-3-7 | 10.1 | 28.5 | 39.0 | 32.5 A-2-4, GrSiSa, brn, Moist, Rec. = 0.8 ft Field Note:, NXDC, Cleaned out casing 9-7-8-5 Field Note:, No Recovery Field Note:, NXDC, Cleaned out casing 5-3-3-3 | 12.2 | 21.3 | 39.9 | 38.8 | A-4, GrSiSa, gry, Moist, Rec. = 1.0 ft Field Note:, No Recovery 4-3-4-3 A-4, SaSiGr, gry-blk, Moist, Rec. = 0.5 ft 19.0 ft - 24.0 ft, Blue gray, DOLOMITE, with calcite veins. Calcite crystals along joint surfaces. Hard, Severely to moderately weathered, Fair rock, NX, RMR=56 (70) (60) 15 10 Hole stopped @ 24.0 ft 25 Remarks: Hole collapsed at 0.0 feet.

Notes: 3. Water level readings have been made at times and under conditions stated. Fluctuations may occur due to other factors than those present at the time measurements were made.

1. Stratification lines represent approximate boundary between material types. Transition may be gradual.

<<SUB>><<SUB>> is the hammer energy correction factor.

BOTTOM OF PILE CAP

ELEV. 145.00

PROJECT NAME: WEYBRIDGE-NEW HAVEN PROJECT NUMBER: BF 032-1(19)

FILE NAME: sl2b552bor.dgn PROJECT LEADER: C.W. CARLSON DESIGNED BY: D. PETERSON BORING LOGS 2

PLOT DATE: 20-APR-2017 DRAWN BY: M. LONGSTREET CHECKED BY: D. PETERSON SHEET 33 OF 85

177	r /	STATE OF VERMONT AGENCY OF TRANSPORTAT	ION			RING			_		ing N		<b>B-1</b> 1 of	
V	Irans	Orking to Get You There  CONSTRUCTION AND  MATERIALS BUREAU  CENTRAL LABORATORY			В	idge-Nev F 032-1( T 17 Br.	19)	en		Pin	No.:		12b55 EN	52
Boring	g Crew:	Garrow, Emerson	_		Casin	_	pler		Grou	ndwa	iter (	Observ	ations	
_		11/04/16 Date Finished: 11/07/16	Type: I.D.:		WB 4 in		<u>S</u> 5 in	Dat	te	Dept	h	N	otes	
	_ G NAD83:		Hamm	er Wt:	N.A.		) lb.	11/07	7/16	(ft)		No M/ T	· to do	n t h
Station		00. 85 Offset: -5.00	Hamm		N.A.		in.	1 1/07	/16			No W.T	. to de	epin
	d Elevation			er/Rod Ty	_	Auto/AV								
Oloun.	la Elevation	<u> </u>	Rig	CME 55	IKACK		= 1.41							
Depth (ft)	Strata (1)	CLASSIFICATION OF MATE (Description)	ERIALS			Run (Dip deg.)	Core Rec. % (RQD %)	Drill Rate minutes/ft	Blows/6"	(N Value)	Moisture Content %	Gravel %	Sand %	Fines %
		Asphalt Pavement, 0.0 ft - 1.15 ft												
-		∖A-1-a, SaGr, brn, Moist, Rec. = 0.9 ft							   11-1	10-	5.7	58.8	31.8	9.
-									11-1					
-		Field Note:, Rollercone, Cleaned out casing									<b>-</b> 1	<b>540</b>	20.0	1
		A-1-a, SaGr, brn, Moist, Rec. = 1.0 ft							8-11-       (20		5.4	54.0	30.6	15
_	000	Field Note:, Rollercone, Cleaned out casing												
5		A-1-a, SaGr, brn, Moist, Rec. = 0.7 ft							6-6-4 (10		5.1	68.7	23.2	8.
+		Field Note:, Rollercone, Cleaned out casing							(10	"				
-	la Constant	A-1-a, SaGr, brn, Moist, Rec. = 0.6 ft							3-6-7		5.8	62.1	26.0	11
		Field Note:, Rollercone, Cleaned out casing							(13 	3)				
		Field Note:, No Recovery							   7-6-3	3_8				
10 —		rield Note., No Necovery							(9)					
		Field Note:, Rollercone, Cleaned out casing												
(		A-1-b, SaGr, brn, Moist, Rec. = 0.4 ft							5-12-   (21		8.3	48.1	32.5	19
		Field Note:, NXDC, Cleaned out casing												
_		Field Note:, No Recovery							2-5-4 (9)					
15		Field Note:, NXDC, Cleaned out casing							 	_	9.0	64.2	18.1	17
<u>}</u>		A-1-b, Gr, brn-gry, Moist, Rec. = 0.3 ft 15.8 ft - 19.8 ft, Blue gray, DOLOMITE, with c	alcito voi	ne Pueta	and	1	72	4	R@3			edrock		
- <del> </del>		tan staining along joints. Hard, Slightly weather				(45-55)	1	3	(1)	,, ,,				
ľ	/ /	RMR=40												
7	7							4						
- <del> </del> -	7 / 7							4						
20 –	/ /	19.8 ft - 20.8 ft, Blue gray, DOLOMITE, with c			IV	2	20	3						
-		staining along joints. Hard, Very slightly weath \RMR=39	iereu, Po	or rock, r	NA,	<u>(45)</u> 100	(35)	4						
_		20.8 ft - 25.8 ft, Blue gray, DOLOMITE, with cand brown staining along joints. Hard, Very sli				(45-50)	(34)	3						
_	4	rock, NX, RMR=49	igility wo	ati ioroa, i	an .			4						
ļ	/ /							3						
25 –														
20	, ,							4						
_		Hole stoppe	ed @ 25.	8 ft										
-		Remarks: Hole collapsed at 5.5 feet.												
20		1.010 001147004 41 0.0 1001.												
30 –														
•	1	on lines represent approximate boundary between material type												

BOTTOM OF PILE CAP

ELEV. 145.00

Boring No.: **BORING LOG** STATE OF VERMONT AGENCY OF TRANSPORTATION Page No.: Weybridge-New Haven CONSTRUCTION AND MATERIALS BUREAU CENTRAL LABORATORY BF 032-1(19) Pin No.: 12b552 VT 17 Br. #8 Checked By: __END Casing Sampler Groundwater Observations Judkins, Gomes Boring Crew: <u>WB</u> 3 in SS Type: Depth Date Date Started: <u>11/02/16</u> Date Finished: <u>11/04/16</u> 1.5 in N.A. 140 lb. 11/04/16 Hammer Wt: N 578960.25 ft E 1444190.48 ft VTSPG NAD83:

BOTTOM OF PILE CAP ELEV. 145.00

VTSP0	G NAD83: n: 104	N 578960.25 ft E 1444190.48 ft -27.43	Hammer Wt: N.A. Hammer Fall: N.A.	30	in.	11/04	/16		lo W.T	. obse	erved
Groun	ıd Elevatioı	n: <u>144.8 ft</u>	Hammer/Rod Type: _ Rig: <u>Diedrich 25</u>	Auto/AV CE = U		'n					
Depth (ft)	Strata (1)	CLASSIFICATION OF MATE (Description)	Run (Dip deg.)	Core Rec. % (RQD %)	Drill Rate minutes/ft	Blows/6" (N Value)	Moisture Content %	Gravel %	Sand %	Fines %	
		A-1-b, SiSaGr, brn, Dry, Rec. = 0.9 ft, Lab Nowere wtihin sample	te: Sticks and twigs				2-2-4-4 (6)	8.1	41.7	37.7	20.6
2.5		A-1-b, SiSaGr, brn, MTD, Rec. = 1.4 ft, Lab N within sample	ote: Broken rock was				6-5-14- 22 (19)	16.0	42.6	33.8	23.6
5.0		A-1-a, SaGr, brn, Moist, Rec. = 0.7 ft					8-18-14- 49 (32)	11.8	66.6	21.3	12.1
-		6.0 ft - 11.0 ft, Blue gray, DOLOMITE, with qu Brown, orange, and rust staining along joints. weathered, Fair rock, BX, RMR=48		1 (70)	86 (50)	8	Тор	of Be	drock	@ 6.0	ft
7.5						12					
10.0						13					
-		11.0 ft - 16.0 ft, Blue gray, DOLOMITE, with c	alcite veins.	2	32	11					
12.5		Yellowish-orange staining along joints. Hard, S Poor rock, BX, RMR=40		(70)	(0)	11					
-) -; -						11					
15.0						11					
1	,	Hole stoppe	ed @ 16.0 ft		l			ı			
17.5 — -		Remarks: Hole collapsed at 4.0 feet.									
20.0											
Notes:	< _{&gt;&lt;&lt;}	on lines represent approximate boundary between material type SUB>> is the hammer energy correction factor. I readings have been made at times and under conditions state		other factor	s than th	ose pres	sent at the tin	ne meas	urement	ts were i	nade.

PROJECT NAME: WEYBRIDGE-NEW HAVEN PROJECT NUMBER: BF 032-1(19)

FILE NAME: sl2b552bor.dgn PROJECT LEADER: C.W. CARLSON DESIGNED BY: D. PETERSON BORING LOGS 3

PLOT DATE: 20-APR-2017 DRAWN BY: M. LONGSTREET CHECKED BY: D. PETERSON SHEET 34 OF 85

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___1 of 1

Notes

				STATE	OF VERMONT			BORII	NG L	.OG		Во	ring No	0.:	B-10	<u>7A</u>		
	AGENCY OF TRANSPORTATION CONSTRUCTION AND MATERIAL S BUREAU							ybridge	e-Nev	v Hav	en	Pa	Page No.: 1 of 1					
			mont Agency of Transportation	MATER	RIALS BUREAU	,		BF 03	-	-		Pir	n No.:		12b55	2		
				CENTRA	AL LABORATOR	1		VT 1	7 Br.	#8		Ch	ecked	Ву:	EN	ID_		
	Boring	Crew:	Gom	nes, Judkins, O	lden				Sam			Groundw	ater O	bserva	ations			
				Date Finished:		Type:		<u>NB</u>	SS		Date			N	otes			
		G NAD83:		973.13 ft E 14		I.D.:		1 in 1.A.	1.5 140			(ft						
				Offset:		Hamme		1.A.	30		12/07/	17 2	0.4 W	/.T. af	ter drill	ing		
	Station		<u>+87.00</u>		7.00		er/Rod Type:		to/AV									
	Ground	d Elevatior	1: 155	5.1 ft		Rig: _	CME 45C SI	KID_	CE =					ı				
	Depth (ft)	Strata (1)			CATION OF MAT (Description)	ERIALS		2	(Dip deg.)	Core Rec. % (RQD %)	Drill Rate minutes/ft	Blows/6" (N Value)	Moisture Content %	Gravel %	Sand %	Fines %		
	\ -			ement, 0.0 ft - 0														
		* A A A	Concrete, 0.1	1 ft - 1.25 ft, (Br	ridge Deck)			_/										
	5 -																	
	_																	
	_																	
	10 -																	
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	-																	
	_																	
	-																	
	15 —																	
	-																	
	-																	
	20 –		Field Note:, 7	Top of Water														
7																		
1/30/			Concrete, 21	.9 ft - 24.5 ft, (F	Footing)													
TOP OF PIER																		
FOOTING			24.5 ft - 29.5	ft, Blue gray, Ir	nterbedded MET	A-LIMEST	ONE, and		1	84	7	Тор	of Bed	lrock (	@ 24.5	5 ft		
•	I -	/			ns. Faint orange/y eathered, Fair roo			( (	50)	(61)	6							
ELEV. 130.50 FA			joints. Hara,	very slightly we		, 1 <b>4</b> /X, 1 XIV	111-40				6							
>		/ /									5							
HAVEN BF032-1(19).GPJ	_										3							
2-1(18	30 -		29.5 ft - 34.5	ft, Dark-gray to	b black, Interbedo	led calcare	eous-sulfidic-		2	84	6							
3F032	-		29.5 feet to 3	0.2 feet. Faint	th calcite veins. S rust and brown/g	ray coating	g on joints		60)	(83)	6							
ш Z Ш	-		Moderately h	ard to hard, Sli	ightly weathered,	Fair rock,	NX, RMR=5	0			6							
											6							
≫ S S Z	-										7							
EYBRIDGE-NEW	35 -				Hole stopp	ped @ 34.	5 ft											
/BRII																		
WE			Remarks:															
06 2					ent grout to top of													
RING L	Notos	1. Stratification	on lines represent a SUB>> is the hamm	ipproximate bounda ner energy correctio	ry between material typ n factor. nd under conditions sta	oes. Transitio	n may be gradual											
$\overline{\alpha}$	Notes:	<ol><li>Water leve</li></ol>	el readings have be	en made at times ar	nd under conditions sta	ted Fluctuation	ons may occur du	ue to other	r factors	s than th	ose prese	ent at the tin	ne measi	urement	s were m	nade.		

OTATE OF VERWICKT					ORING LOG				lo.:	<u>B-108A</u>	
AGENCY OF TRANSPORTATION  CONSTRUCTION AND  We										1 of 1	
MATERIALS BUREAU							F	Pin No.:		12b552	
								Checked By:		END	
na Crow	Carrow Judking Oldon		Casir	ng San	npler		Ground				
_	_	Type:				Date	e D	Depth	Notes		
VTSPG NAD83: N 579019.09 ft E 1444328.91 ft					140 lb.				W.T. above ground		
							′17	7.4 V			round
■ Station 105+5.5.70 Offset =/ 00											
und Elevatio	n: <u>156.2 ft</u>	Rig: _	CME 45C SKIE	<u>CE</u> :	= 1.42						
(1)				( <del>)</del>	%; % %; %	ate s/ft	.e. (e	k e	%	%	%
ata ı		ERIALS		Run p de	A Re	III Re Inte	ows/ Valu	oistu nten	ave	and	Fines
CLASSIFICATION OF MATERIALS (Description)							商区	≥S	Ģ	iš	
	∖Asphalt Pavement, 0.0 ft - 0.15 ft										
Conrete, 0.15 ft - 1.3 ft, (Bridge Deck)											
_											
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- / /				1 (50)	70	15					
<del>                                     </del>		erately nar	a, very		-,	7					
	28.5 ft - 32.5 ft, Gray, DOLOMITE, with calcite					7					
1//	Moderately hard, Unweathered, Good rock, N	IX, RMR=	:69								
7 /	20 5 # . 27 5 # David was a black late the dal	1 DOL O	NAITE		00	1					
	calcite veins and pyrite and sulfidic SHALEY-DOLOMITE with calcite veins. Yellow and gray staining along joints. Sub-vertical weathered				1	1 1					
						1 1					
		Siigntiy we	eatnered, Fair			1 _ 1					
						1 1					
	Hole stopp	ed @ 37.	5 ft	1					1		1
_	7.2.2.2.3 <b>9</b> pp	<u> </u>									
4											
											l l
e :i	ng Crew: _ e Started: _ PG NAD83: ion:	AGENCY OF TRANSPORTAT CONSTRUCTION AND MATERIALS BUREAU CENTRAL LABORATORY  Ing Crew: Garrow, Judkins, Olden  9 Started: 12/20/16 Date Finished: 12/21/16  PG NAD83: N 579019.09 ft E 1444328.91 ft on: 105+53.20 Offset: -7.00  Ind Elevation: 156.2 ft CLASSIFICATION OF MATE (Description)  Asphalt Pavement, 0.0 ft - 0.15 ft Conrete, 0.15 ft - 1.3 ft, (Bridge Deck)  Asphalt Pavement, 0.0 ft - 0.15 ft Conrete, 0.15 ft - 1.3 ft, (Bridge Deck)  32.5 ft - 37.5 ft, Gray, DOLOMITE, with calcit Moderately hard, Unweathered, Good rock, N Starter of Starter	AGENCY OF TRANSPORTATION CONSTRUCTION AND MATERIALS BUREAU CENTRAL LABORATORY  Type: Started: 12/20/16 Date Finished: 12/21/16 PG NAD83: N 579019.09 ft E 1444328.91 ft Hamm. Hamm. Rig: CLASSIFICATION OF MATERIALS (Description)  CLASSIFICATION OF MATERIALS (Description)  Asphalt Pavement, 0.0 ft - 0.15 ft Conrete, 0.15 ft - 1.3 ft, (Bridge Deck)  27.5 ft - 28.5 ft, Gray, /dark gray DOLOMITE, with calc. Trare pyrite. Faint rust staining on joints. Moderately hard slightly weathered, Fair rock, NX, RMR=46  28.5 ft - 32.5 ft, Gray, DOLOMITE, with calc. Trare pyrite. Faint rust staining on joints. Moderately hard slightly weathered, Fair rock, NX, RMR=46  28.5 ft - 32.5 ft, Gray, DOLOMITE, with calc. Trare pyrite. Faint rust staining on joints. Moderately hard slightly weathered, Fair rock, NX, RMR=46  28.5 ft - 32.5 ft, Gray, DOLOMITE, with calc. Trare pyrite. Faint rust staining on joints. Moderately hard slightly weathered, Good rock, NX, RMR=40  32.5 ft - 37.5 ft, Dark-gray to black, Interbedded DOLOM calcite vains and pyrite and sulfictic SHALEY-DOLOMI veins, Yellow and gray staining along joints. Sub-vertic joint at 8.9 feet to 9.5 feet. Moderately hard, Slightly we rock, NX, RMR=44	AGENCY OF TRANSPORTATION CONSTRUCTION AND MATERIALS BUREAU CENTRAL LABORATORY  Type: WB LD: 4 in Hammer Rell: NA Hammer/Rod Type: Rig: CME 45C SKIC  CLASSIFICATION OF MATERIALS  COnrete, 0.15 ft - 1.3 ft, (Bridge Deck)  Asphalt Pavement, 0.0 ft - 0.15 ft Conrete, 0.15 ft - 1.3 ft, (Bridge Deck)  Asphalt Pavement, 0.0 ft - 0.0	AGENCY OF TRANSPORTATION CONSTRUCTION AND MATERIALS BUREAU CENTRAL LABORATORY  Rig Crew: Garrow, Judkins, Olden PS Natred: 12/20/16 Date Finished: 12/20/16 PS Natred: 12/20/16 Date Finished: 12/20/1	AGENCY OF TRANSPORTATION CONSTRUCTION AND MATERIALS BUREAU CENTRAL LABORATORY  BF 032-1(19) VT 17 Br. #8  F0 32-1(19) VT 17 Br. #8  Casing Sampler Type: WB SS I.D.: I.D	AGENCY OF TRANSPORTATION	Agency   A	Page No.	Page No.:   Page	### AGENCY OF TRANSPORTATION CONSTRUCTION AND MATERIALS BUREAU CONSTRUCTION OF MATERIA

1. Stratification lines represent approximate boundary between material types. Transition may be gradual.

Notes:

1. Stratification lines represent approximate boundary between material types. Transition may be gradual.

CSUB>><<SUB>> is the hammer energy correction factor.

3. Water level readings have been made at times and under conditions stated. Fluctuations may occur due to other factors than those present at the time measurements were made.

TOP OF PIER

ELEV. 130.50

FOOTING

PROJECT NAME: WEYBRIDGE-NEW HAVEN PROJECT NUMBER: BF 032-1(19)

FILE NAME: sl2b552bor.dgn PROJECT LEADER: C.W. CARLSON DESIGNED BY: D. PETERSON BORING LOGS 4

PLOT DATE: 20-APR-2017 DRAWN BY: M. LONGSTREET CHECKED BY: D. PETERSON SHEET 35 OF 85

Paris		Vermont Agency of Transportation  MATERIALS BUREAU  CENTRAL LABORATORY	BORING LOG  /eybridge-New Haven BF 032-1(19) VT 17 Br. #8  Casing Sampler  Boring No.: B-109 Page No.: 1 of 2 Pin No.: 12b552 Checked By: END  Groundwater Observations	STATE OF VERMONT AGENCY OF TRANSPORTATION CONSTRUCTION AND MATERIALS BUREAU CENTRAL LABORATORY  BORING LOG  Weybridge-New Haven BF 032-1(19) VT 17 Br. #8  Casing Sampler  Casing Sampler  Groundwater Observations  Groundwater Observations	
Date	g Crew: Started: G NAD83 on: 10		WB         SS         Date         Depth (ft)         Notes           N.A.         140 lb.         12/01/17         No W.T. to 15.5 feet           N.A.         30 in.         12/02/17         19.5         W.T. before drilling	Date Started:   11/17/16   Date Finished:   12/02/16   I.D.:   4 in   1.5 in   Station:   106+38.41   Offset:   -15.00   Hammer/Rod Type:   Auto/AWJ   SS   Date   Depth (ft)   Notes   12/02/17   19.5   W.T. before drilling   Date   Depth (ft)   Notes   12/02/17   19.5   W.T. before drilling   Date   Depth (ft)   Notes   12/01/17   Notes   12/01/17   19.5   W.T. before drilling   Date   Depth (ft)   Notes   12/01/17   Notes	
Ground Elevation: 154.5 ft Rig: CME 45C SKID CE = 1.42			SKID CE = 1.42	Ground Elevation: 154.5 ft Rig: CME 45C SKID CE = 1.42	
Depth (ft)	Strata (1	CLASSIFICATION OF MATERIALS (Description)	Run (Dip deg.) (Core Rec. % (RQD %) Drill Rate minutes/ft (N Value) Moisture Content % Gravel % Sand % Fines %	Strata (1)  Strata	
	* * * *	Field Note:, Fill for drilling pad		Field Note:, NXDC, Cleaned out casing	
5 -	* * *	Field Note:, NXDC, Cleaned out casing	7-4-3-3   12.6   59.1   34.8   6.1	A-3, Sa, blk, Moist, Rec. = 0.9 π (11)	
		A-4, GrSaSi, brn, Moist, Rec. = 0.6 ft  A-4, SaSi, brn, Moist, Rec. = 0.4 ft	4-6-2-3   10.8   23.9   37.0   39.1   4-2-2-1   10.7   16.1   39.0   44.9	45 A-4, SiSa, gry, Moist, Rec. = 0.9 ft, Lab Note: A small amount of clay was within sample. Sample tested non-plastic.  13-10- 11-18  8.9  19.7  43.7  36.6	
10 -		Field Note:, No Recovery, Stone stuck in the end of the sampler	4-13-7- 11 (20)		
15 -		A-2-4, SaSiGr, brn, Moist, Rec. = 0.8 ft Field Note:, Cobbles	11-7-13- 9.9 36.0 31.7 32.3 10 (20)	Field Note:, Cobbles	
		Field Note:, NXDC, Cleaned out casing A-4, GrSaSi, brn, Moist, Rec. = 0.8 ft	2-2-2-2 11.4 21.3 37.6 41.1		
20 -		A-4, SaSi, brn, Moist, Rec. = 1.2 ft  A-2-4, GrSiSa, gry, Moist, Rec. = 0.7 ft	3-3-8-3   14.2   32.0   35.4   32.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6   39.6	A-4, SaGrSi, gry, Moist, Rec. = 1.0 ft, Lab Note: Some clay was  within sample. Sample tested non-plastic.	TID
OI.GDI 1/30/1	0 0			58.0 ft - 63.0 ft, Dark gray, DOLOMITE, with calcite veins.  58.0 ft - 63.0 ft, Dark gray, DOLOMITE, with calcite veins.  Sub-vertical joints at 58.4 to 59.1 feet. Calcite crystals along joint surfaces. Some joints are faded light brown/gray. Hard, Slightly weathered, Fair rock, NX, RMR=47	1111
A VERMON 5	0 0	Field Note:, NXDC, Cleaned out casing A-2-4, SiSa, brn, Moist, Rec. = 0.3 ft	3-2-2-2 30.5 14.9 59.4 25.7	63.0 ft - 68.0 ft, Dark gray, DOLOMITE, with calcite veins. Sub-vertical joints at 58.4 to 59.1 feet. Calcite crystals along joint  5 5 5 100 8 (60-70) (68)	
3F032-1(19).GP	_			Sub-vertical joints at 58.4 to 59.1 feet. Calcite crystals along joint surfaces. Some joints are faded light gray. Hard, Very slightly weathered, Fair rock, NX, RMR=52	
NEW HAVEN BE	0 0	A-2-4, GrSiSa, gry, Moist, Rec. = 2.0 ft, Lab Note: A small amount clay was within sample. Sample tested non-plastic	3-4-3-1 9.8 28.1 41.4 30.5 (7)	Hole stopped @ 68.0 ft	
2 WEYBRIDGE 32 WEYBRIDGE 35		Field Note:, No Recovery	1-5-6-8 (11)	Remarks: Hole collapsed at 4.0 feet.	

1. Stratification lines represent approximate boundary between material types. Transition may be gradual.

Notes:

1. Stratification lines represent approximate boundary between material types. Transition may be gradual.

<SUB>><<SUB>> is the hammer energy correction factor.

3. Water level readings have been made at times and under conditions stated. Fluctuations may occur due to other factors than those present at the time measurements were made.

BOTTOM OF PILE CAP

ELEV. 146.00

PROJECT NAME: WEYBRIDGE-NEW HAVEN
PROJECT NUMBER: BF 032-1(19)

FILE NAME: si2b552bor.dgn PLOT DATE: 20-APR-2017
PROJECT LEADER: C.W. CARLSON DRAWN BY: M. LONGSTREET
DESIGNED BY: D. PETERSON CHECKED BY: D. PETERSON

BORING LOGS 5

SHEET 36 OF 85

1. Stratification lines represent approximate boundary between material types. Transition may be gradual.

Notes:

Notes:

1. Stratification lines represent approximate boundary between material types. Transition may be gradual.

<SUB>><<SUB>> is the hammer energy correction factor.

3. Water level readings have been made at times and under conditions stated. Fluctuations may occur due to other factors than those present at the time measurements were made.

BOTTOM OF PILE CAP

ELEV. 146.00

			STATE OF VERMONT	BOF	RING	LOG		Во	ring N	o.:	B-1	12_	
	I(V	Trancu	AGENCY OF TRANSPORTATION  Orking to Get You There  CONSTRUCTION AND  MATERIAL S BUREAU	Weybrid	_		en		ge No	_	2 of		
		Trans	MATERIALS BUREAU CENTRAL LABORATORY		[∓] 032-1( Γ 17 Br.	-			n No.:		12b55		
4									ecked			ND_	
4	Borin	g Crew: _	Judkins, Gomes Type:	Casing WB		npler SS		Groundw					
	Date	Started: _	10/25/16 Date Finished: 10/27/16 I.D.:	4 in		5 in	Date	e Dep (ft		N	otes		
9	VTSF	PG NAD83:	11 01 000 21 12 11 11 11 10 01 00 11	ner Wt: N.A.		0 lb.	10/26		_	V.T. Be	efore [	Drilling	
9	Statio	on: <u>106</u>	+38 98 ()#661 5 65	ner Fall: <u>N.A.</u> ner/Rod Type: /	<u></u> Auto/A\	in NJ	10/27	/16 2	2.6 V	V.T. Be	efore [	Drilling	
	Grou	nd Elevatio	n: <u>156.6 ft</u> Rig:	CME 45C SKID	CE :	= 1.42							
	Depth (ft)	Strata (1)	CLASSIFICATION OF MATERIALS (Description)		Run (Dip deg.)	Core Rec. % (RQD %)	Drill Rate minutes/ft	Blows/6" (N Value)	Moisture Content %	Gravel %	Sand %	Fines %	
			A-4, SaSi, gray, Moist, Rec. = 1.9 ft, Lab Note: A smal clay was within sample. Sample tested Non-Plastic.	I amount of				3-2-2-3 (4)	10.9	16.8	33.5	49.7	
	50 -		Field Note:, Rollercone, cleaned out casing.  A-2-4, SiSaGr, gray, Moist, Rec. = 0.5 ft, Lab Note: A amount of clay was within sample. Sample tested Nor	very small ı-Plastic.				3-1-2-3 (3)	10.1	36.2	34.3	29.5	
	55 -		Field Note:, NXDC, cleaned out casing. Field Note:, No Recovery					7-6-7-10 (13)					
	60 -		Field Note:, NXDC, cleaned out casing.  A-4, SaSi, gray, Moist, Rec. = 1.8 ft, Lab Note: Some sample. Sample tested Non-Plastic.	clay was within				15-22- 35- R@4" (R <del>)</del> op	8.0	15.3	31.7	53.0	APPROX, PILE TIP
	65 -		62.3 ft - 67.3 ft, Light gray/gray to black, Interbedded META-LIMESTONE, and DOLOMITE with calcite veir staining on joints. Hard, Unweathered, Poor rock, NX,		R-1 (40)	92 (0)	4 4 4 10 4	\ 10p	of Re	grock (	<u>o</u> 62.	3 π	ELEV. 94.3
1/30/17	70 -		67.3 ft - 72.3 ft, Light gray/gray to black, Interbedded META-LIMESTONE, and DOLOMITE with calcite vein and yellow staining on joints. Hard, Unweathered, God RMR=64		R-2 (40)	98 (89)	4 4 4 5 4						
AOT.GDT			Hole stopped @ 72	.3 ft		<u> </u>							
GPJ VERMONT AOT	75 -	- - - -	Remarks: Hole collapsed at 8.9 feet.										
VEN BF032-1(19).G		- - - -											
WEYBRIDGE-NEW HAV	85 -												
(1)		-											
RING LO	Notes:	< _{&gt;&lt;&lt;}	on lines represent approximate boundary between material types. Transition SUB>> is the hammer energy correction factor.		other facto	rs than th	nose pres	ent at the tin	ne meas	surement	s were r	nade.	

PROJECT NAME: WEYBRIDGE-NEW HAVEN
PROJECT NUMBER: BF 032-1(19)

FILE NAME: s12b552bor.dgn PLOT DATE: 20-APR-2017
PROJECT LEADER: C.W. CARLSON DRAWN BY: M. LONGSTREET
DESIGNED BY: D. PETERSON
BORING LOGS 6 CHECKED BY: D. PETERSON
SHEET 37 OF 85

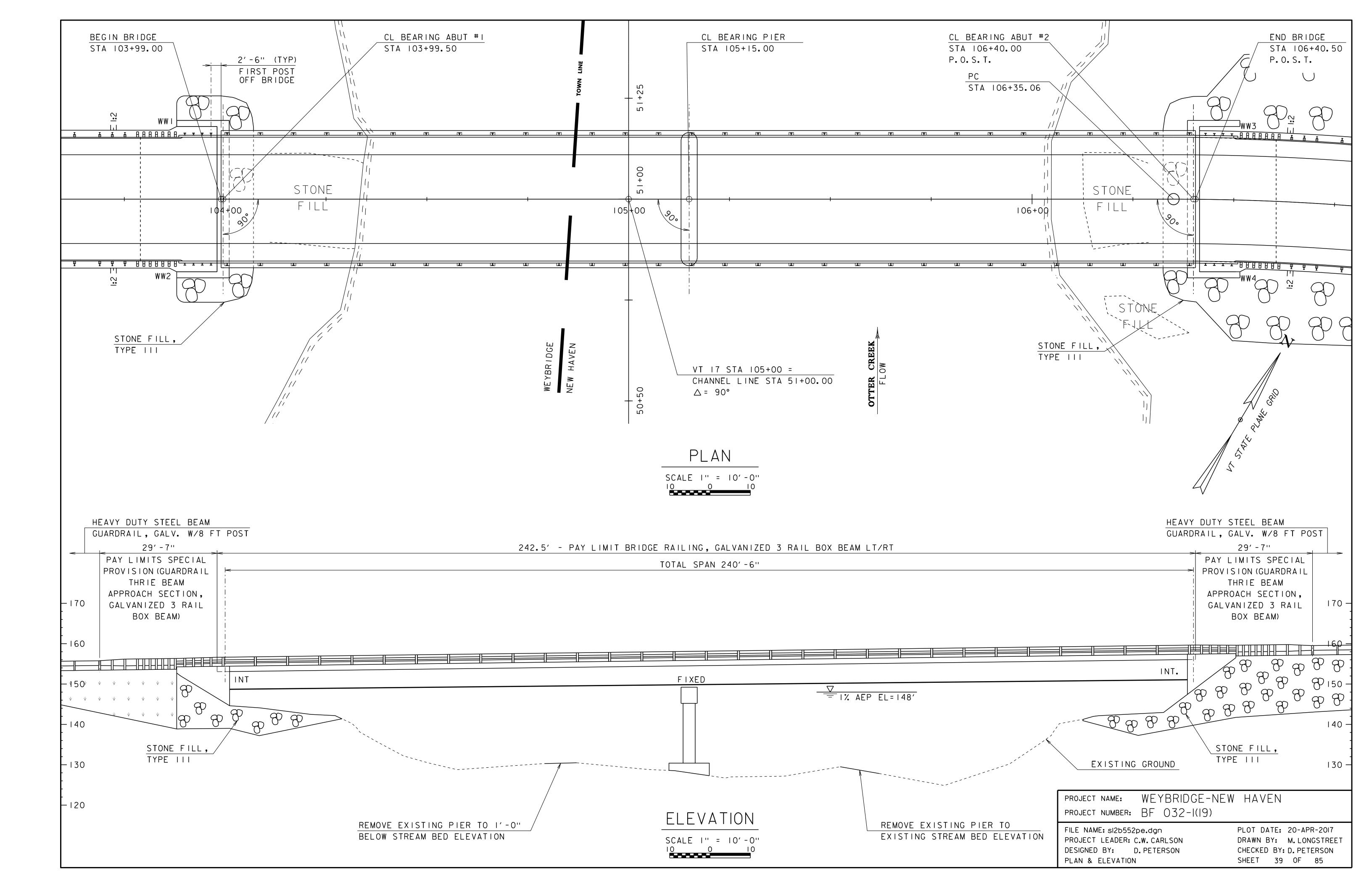
	STATE OF VERMONT AGENCY OF TRANSPORTAT	TION	BOF	RING L	_OG			ring N		<u>B-1</u>	
VTrans	Working to Get You There CONSTRUCTION AND	ION	Weybrid	_		1		ge No		1 of	
110010	MATERIALS BUREAU CENTRAL LABORATORY	,		⁻ 032-1(′ ⁻ 17 Br.	-			No.:		12b55 ^	
			Casing	Sam	pler	Gr		ecked	bserva	A.	
Boring Crew: _	Judkins, Emerson	Type:	WB	S		Date	Dep			otes	
•	11/28/16 Date Finished: 11/30/16	I.D.:	4 in	1.5		Date	(ft				
/TSPG NAD83		Hamme Hamme		<u>140</u> 30		1/30/16	4	.0 V	V.T. du	ıring d	Irillin
	7+40.40 Offset: 3.76	Hamm	er/Rod Type:	Auto/AV							
Ground Elevation	on: <u>157.4 ft</u>	Rig: _	CME 55 TRACK	<u>CE =</u>	<u> 1.41   </u>				<u> </u>	Γ	<del></del>
(ft) Strata (1)	CLASSIFICATION OF N (Description)		LS		Blows/6" (N Value)	Moisture Content %	Gravel %	Sand %	Fines %	% TT	   % d
	Asphalt Pavement, brn, Moist, 0.0 ft - 0.85 ft										
	A-1-b, SaGr, brn, Moist, Rec. = 0.6 ft				9-19-19 14 (38)	9- 6.1	47.5	35.7	16.8		
0 0	A-2-4, SiGrSa, brn, Moist, Rec. = 1.4 ft				9-6-5- (11)	5 9.9	34.0	40.7	25.3		
5					7-6-5- (11)		25.2	43.3	31.5		
	A-4, SiSa, brn, Moist, Rec. = 0.3 ft				5-4-6-1 (10)	11.5	18.7	42.6	38.7		
10 -///	A-2-4, SiSa, brn, Moist, Rec. = 1.0 ft				7-5-8- (13)	6 11.7	19.9	45.4	34.7		
	Field Note:, No Recovery				10-10 10-8						
	Field Note:, No Recovery				(20) 7-2-2-						
15	A-4, Si, gry, Moist, Rec. = 0.7 ft, Lab Note: So Sample tested non-plastic	ome clay	was within sample	).	(4) 7-5-4- (9)	5 32.2	0.7	8.7	90.6		
	Visual Description:, Si, gry, Moist, Field Note:	Pushed	3x30 inch Shelby	Tube							
20 -	A-6, SiCl, gry, Moist, Field Note: Pushed 3x30	0 inch Sh	elby Tube			27.5		6.7	93.3	40	1
-											
25	A-4, Si, gry, Moist, Rec. = 2.0 ft, Lab Note: A sample. Sample tested non-plastic	small am	ount of clay was v	vithin	WH- WH- WH-W (WH)	Ή		16.2	83.8		
30											
	A-4, GrSaSi, gry, Moist, Rec. = 0.9 ft				1-10-1 19		25.2				
0 0	A-2-4, Sa, gry-blk, Moist, Rec. = 0.8 ft	stoner - 1 C	2 2 2 0 tt		(24)	16.0	8.3	72.5	19.2		
-	Hole s	stopped @	y 32.U π								
-											
35 —	Remarks: Hole collapsed at 5.7 feet.										
-	·										

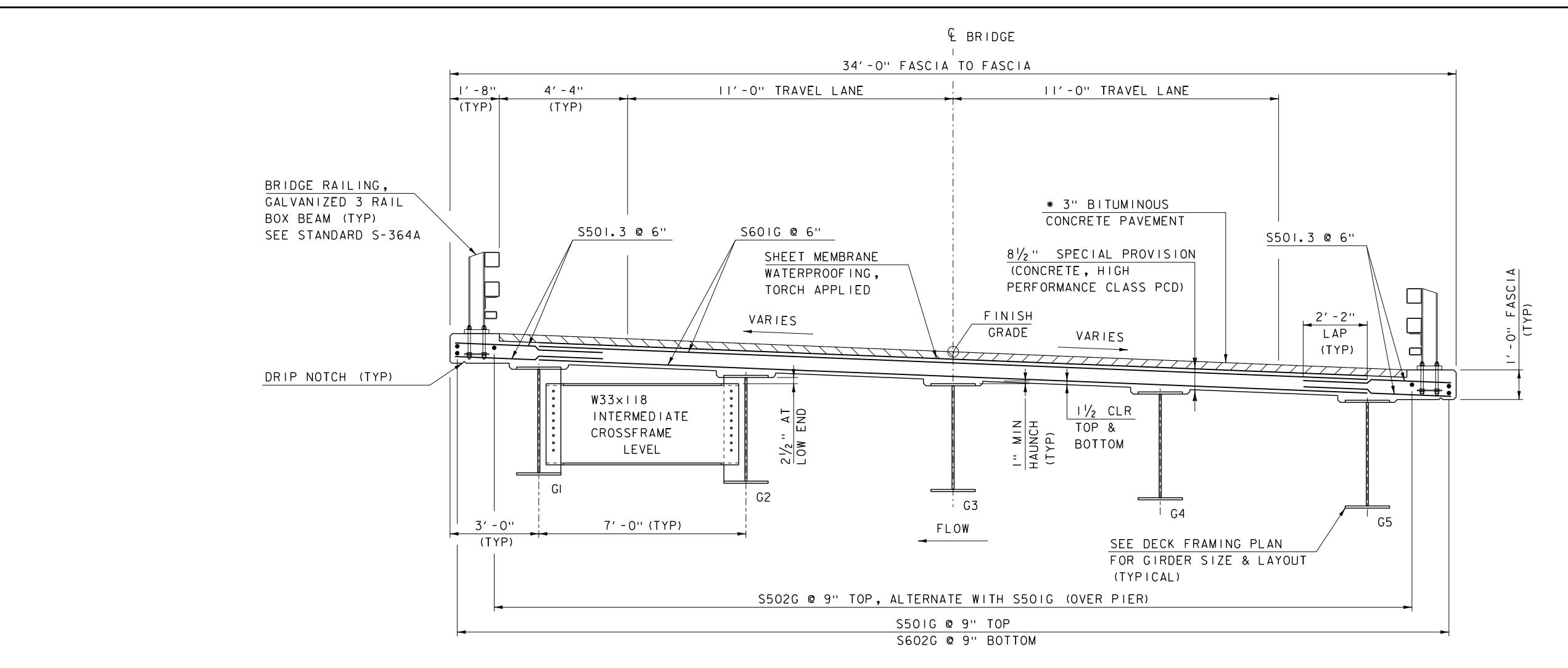
Boring Date S		CONSTRUCTION AND MATERIALS BUREAU CENTRAL LABORATORY		,		ge-Nev )32-1( 17 Br.	19)		Pir	ge No.: No.: ecked		1 of 12b55 	52
Station	Started: 3 NAD83:	+41.61 Offset: 4.00		•		1.5 140 30 uto/AV	S [ ] [ ] [ ] [ ] [ ] [ ] [ ] [ ] [ ] [	Gro Date 30/16	Dep (ft	oth )	Observa No W.T. du	otes	
Depth (ft)	Strata (1)	CLASSIFICATION OF M (Description)		LS			Blows/6" (N Value)	Moisture Content %	Gravel %	Sand %	Fines %	% TT	% Id
		Asphalt Pavement, 0.0 ft - 0.53 ft Reclaimed Stabilised Base, 0.53 ft - 1.5 ft A-1-b, GrSa, brn, Moist, Rec. = 0.8 ft  A-4, GrSiSa, brn, Moist, Rec. = 1.2 ft					11-9-8 (20) 5-6-4-4 (10)	6.6 13.9	44.1 20.7	45.2 42.2			
	0/0/0	A-2-4, SiGrSa, brn, Moist, Rec. = 1.8 ft  A-2-4, GrSiSa, brn, Moist, Rec. = 1.2 ft					6-12-7-8 (19) 4-4-7-10		29.5		26.7 30.6		
10		Field Note:, Rollercone, cleaned out casing Field Note:, No Recovery					(11) 11-9-5-4 (14)						
		A-1-b, SaGr, brn, Moist, Rec. = 0.6 ft  Field Note:, NXDC, cleaned out casing A-2-4, GrSiSa, gry, Moist, Rec. = 0.3 ft					4-3-8-6 (11) 7-1-1-2 (2)	9.9	23.5				
15		Field Note:, NXDC, cleaned out casing Field Note:, No Recovery Field Note:, Rollercone, cleaned out casing	200 = 1.0	) <del>[</del>			4-5-9-10 (14)						
20 -		Field Note:, Pushed 3x30 inch Shelby Tube, Field Note:, Rollercone, cleaned out casing A-7-6, Cl, gry, Moist, Rec. = 1.7 ft	≺ec. = 1.9	<i>σ</i> π			3-6-9-13 (15)	31.8	0.7	2.3	97.0	54	29
25		A-6, SiCl, brn-gry, Moist, Rec. = 2.0 ft					2-3-5-7 (8)	34.8	4.0	8.7	87.3	31	14
30		A-4, GrSaSi, gry, Moist, Rec. = 2.0 ft, Lab Not sample. Sample tested non-plastic	te: Some	clay was	within		4-4-7-8 (11)	10.8	22.4	33.0	44.6		
35 -		Hole s Remarks: Hole collapsed at 6.5 feet.	topped @	) 32.0 ft									

PROJECT	NAME:	WE`	YBRIDGE-NEW	HAVEN
PROJECT	NUMBER:	BF	032-1(19)	

FILE NAME: sl2b552bor.dgn
PROJECT LEADER: C.W. CARLSON
DESIGNED BY: D. PETERSON
BORING LOGS 7

PLOT DATE: 20-APR-2017
DRAWN BY: M. LONGSTREET
CHECKED BY: D. PETERSON
SHEET 38 OF 85





### DECK REINFORCING SECTION

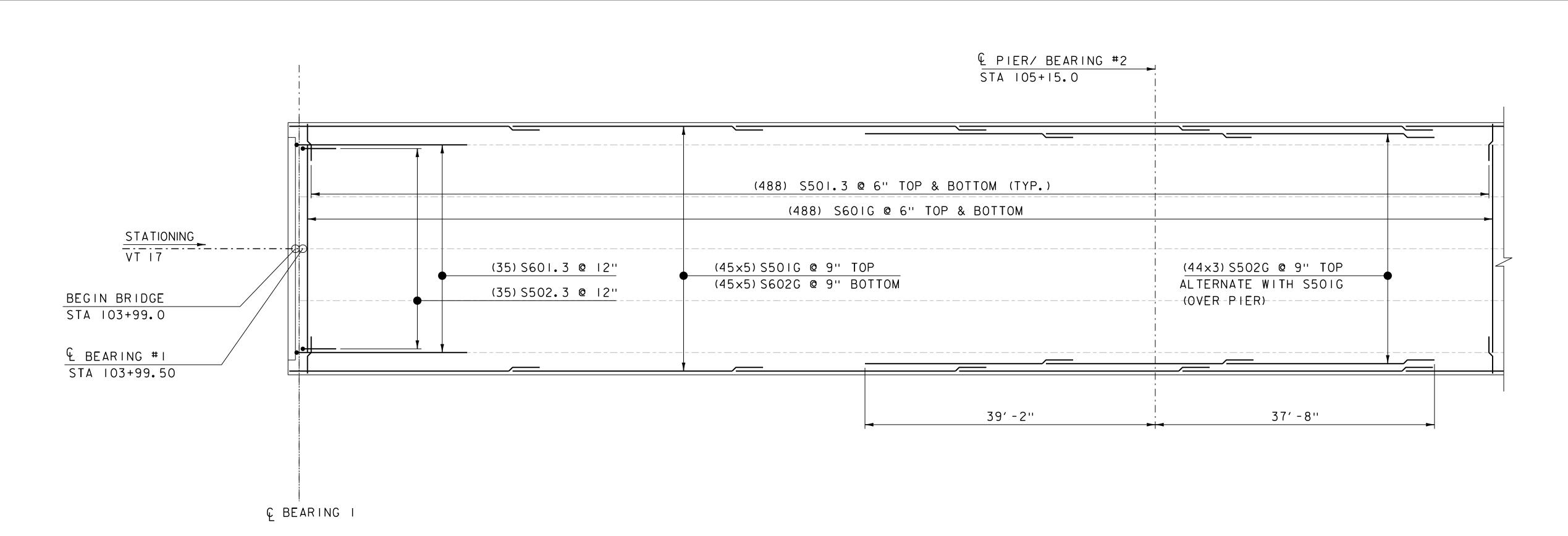
SCALE: 1/2" = 1'-0"

*SEE BRIDGE TYPICAL SECTION FOR PAVEMENT DETAILS

PROJECT NAME: WEYBRIDGE-NEW HAVEN PROJECT NUMBER: BF 032-1(19)

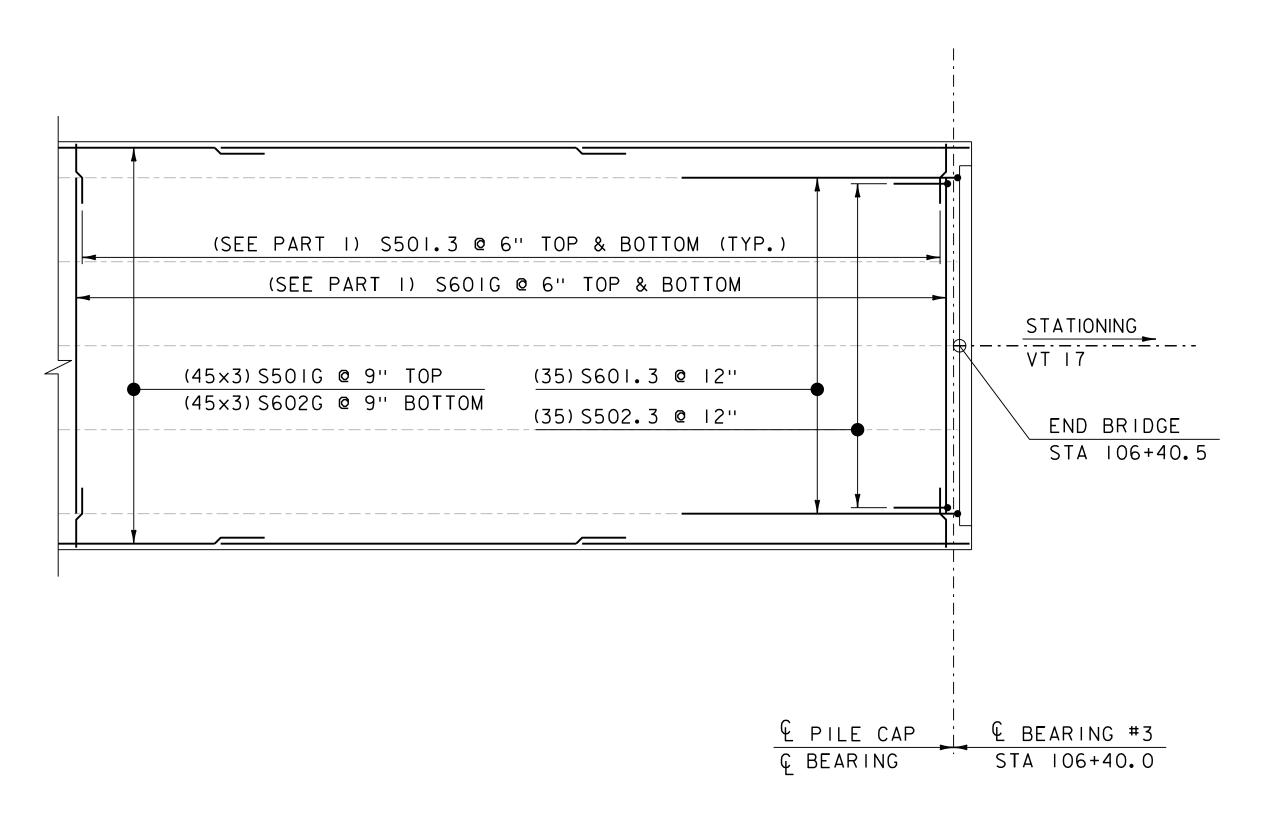
FILE NAME: sl2b552sup.dgn
PROJECT LEADER: C.W. CARLSON
DESIGNED BY: D. PETERSON
DECK TYPICAL

PLOT DATE: 20-APR-2017
DRAWN BY: M. LONGSTREET
CHECKED BY: D. PETERSON
SHEET 40 OF 85



### DECK REINFORCING PLAN (PART I)

SCALE:  $\frac{1}{8}$  " = 1'-0"



### DECK REINFORCING PLAN (PART 2)

SCALE:  $\frac{1}{8}$  " = 1'-0"

#### NOTE:

NF = NEAR FACE FF = FAR FACE EF = EACH FACE

▲ = CUT TO FIT IN FIELD 3" CLEAR, UNLESS OTHERWISE SPECIFIED ON THE PLANS.

#### NOTE:

NF = NEAR FACE
FF = FAR FACE
EF = EACH FACE

▲ = CUT TO FIT IN FIELD

3" CLEAR, UNLESS OTHERWISE

SPECIFIED ON THE PLANS.

2'-2" BAR LAP UNLESS OTHERWISE

SPECIFIED ON THE PLANS.

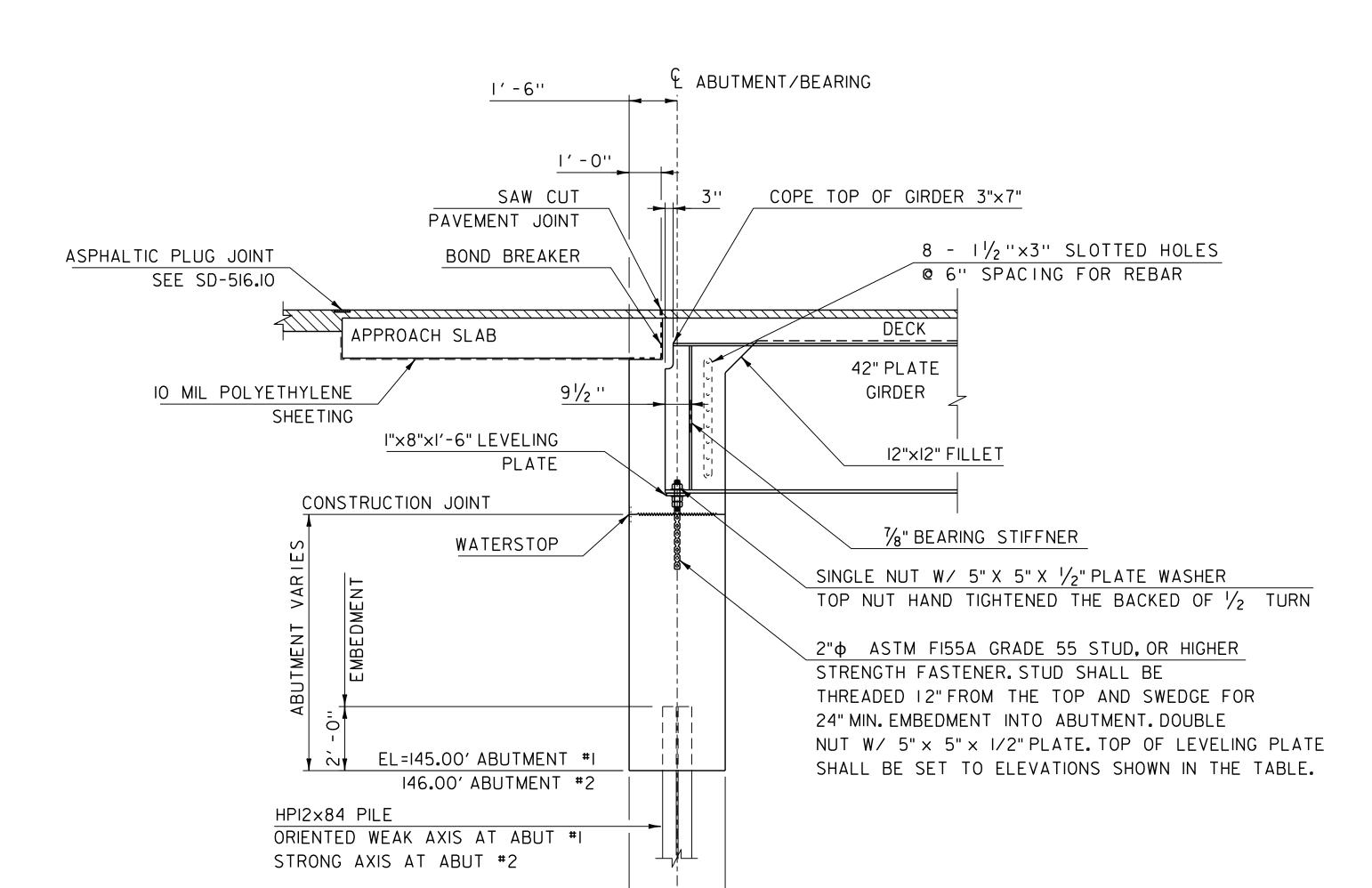
3'-8" BAR LAP FOR GFRP

LONGITUDINAL BARS.

PROJECT NAME: WEYBRIDGE-NEW HAVEN PROJECT NUMBER: BF 032-1(19)

FILE NAME: sl2b552sup.dgn
PROJECT LEADER: C.W. CARLSON
DESIGNED BY: D. PETERSON
DECK REINFORCING PLAN

PLOT DATE: 20-APR-2017
DRAWN BY: M. LONGSTREET
CHECKED BY: D. PETERSON
SHEET 41 OF 85



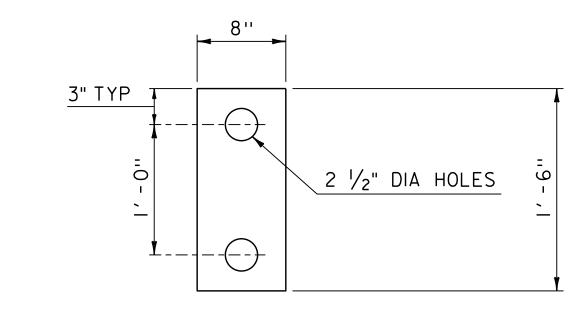
# ABUTMENT END BRIDGE DETAIL SCALE: %" = 1'-0"

1'-6"

THEORETICAL	TOP OF SOLE PI	LATE ELEVATION:
	ABUTMENT #I	ABUTMENT #2
BEAM I	149.24	152.86
BEAM 2	149.07	152.38
BEAM 3	148.91	151.91
BEAM 4	148.74	151.43
BEAM 5	148.58	151.96

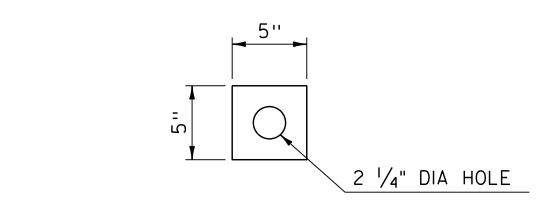
#### NOTES:

- I) PAYMENT FOR LEVELING PLATES, ANCHOR BOLTS, WASHERS, NUTS AND MORTAR TYPE IV SHALL BE INCIDENTAL TO ITEMS 506.55 STRUCTURAL STEEL, PLATE GIRDER.
- 2) ALL STEEL IN THE LEVELING PLATE ASSEMBLY SHALL BE AASHTO M270 GR 50W.



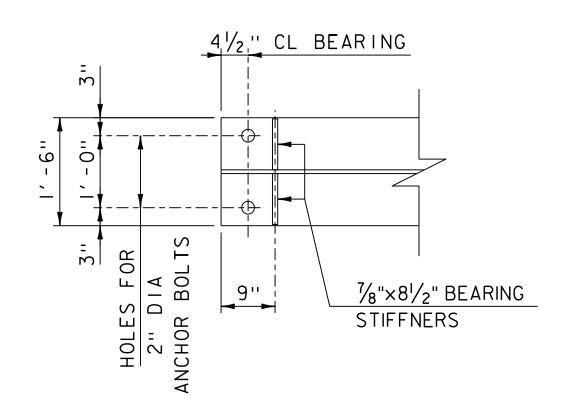
### LEVELING PLATE DETAIL

NOT TO SCALE



1/2" PLATE WASHER DETAIL

NOT TO SCALE



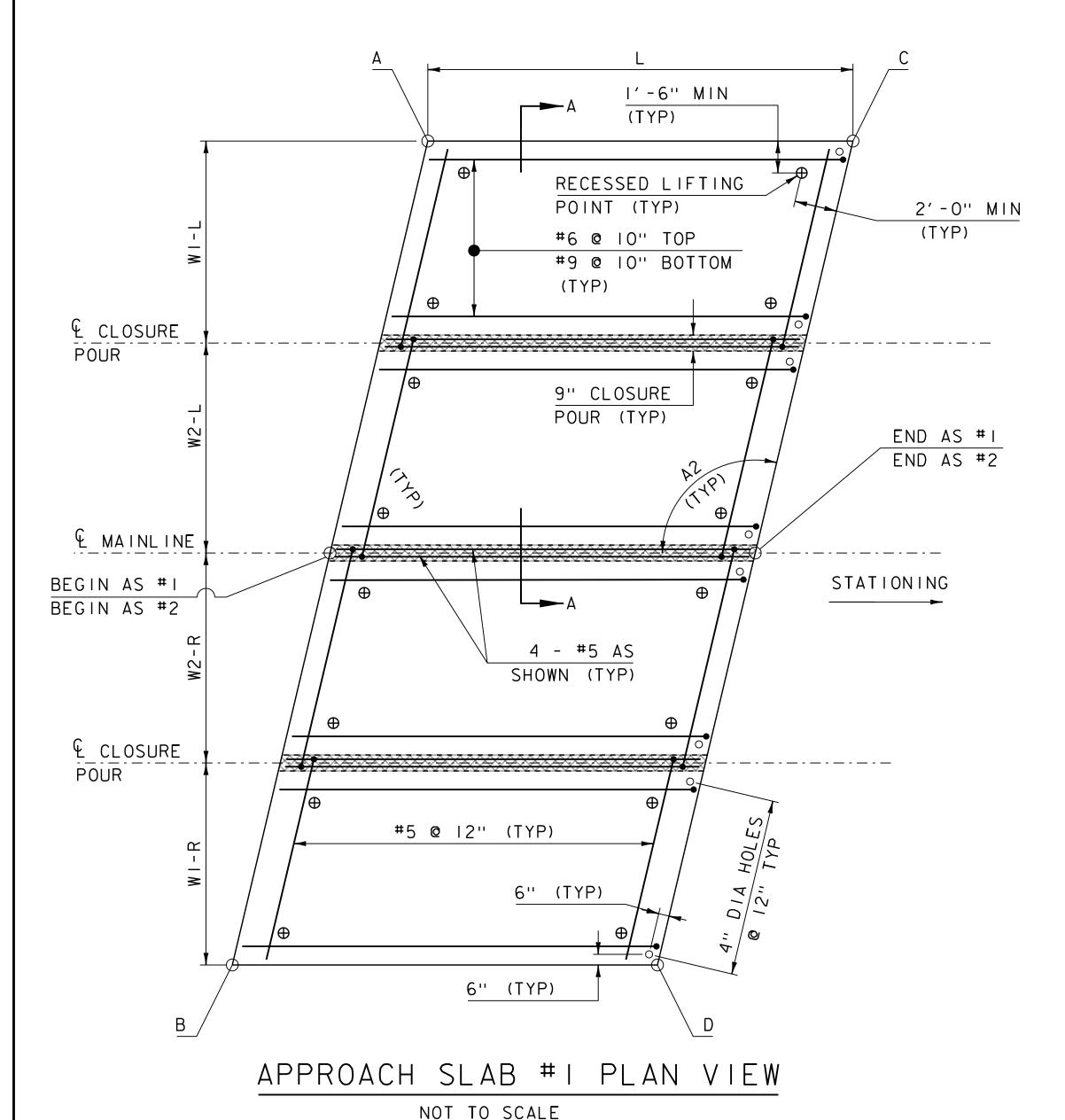
END BEAM BEARING DETAIL

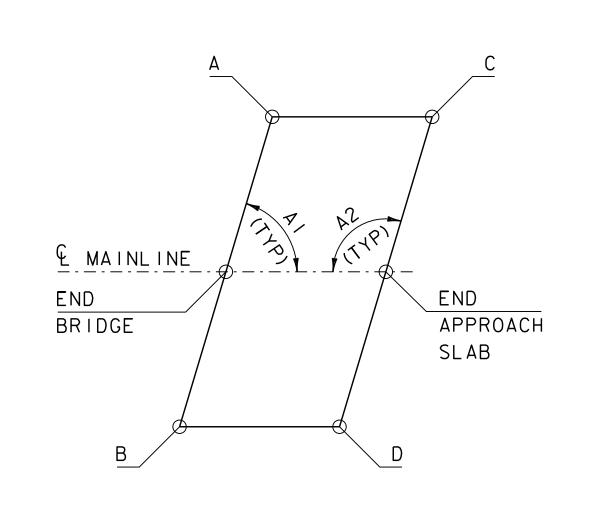
SCALE: 3/4" = 1'-0"

PROJECT NAME: WEYBRIDGE-NEW HAVEN PROJECT NUMBER: BF 032-1(19)

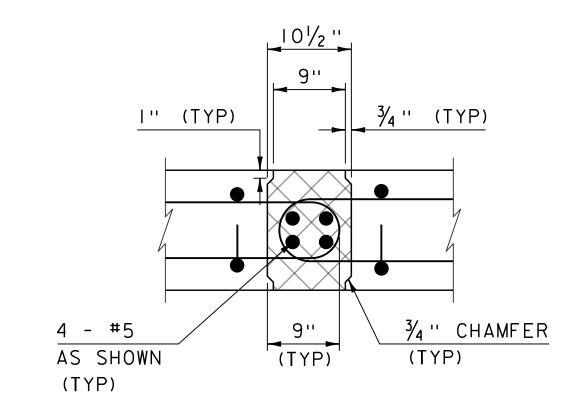
FILE NAME: sl2b552sub.dgn
PROJECT LEADER: C.W. CARLSON
DESIGNED BY: D. PETERSON
BEGIN/END BRIDGE DETAILS

PLOT DATE: 20-APR-2017
DRAWN BY: M. LONGSTREET
CHECKED BY: D. PETERSON
SHEET 42 OF 85





#### APPROACH SLAB #2 PLAN VIEW NOT TO SCALE



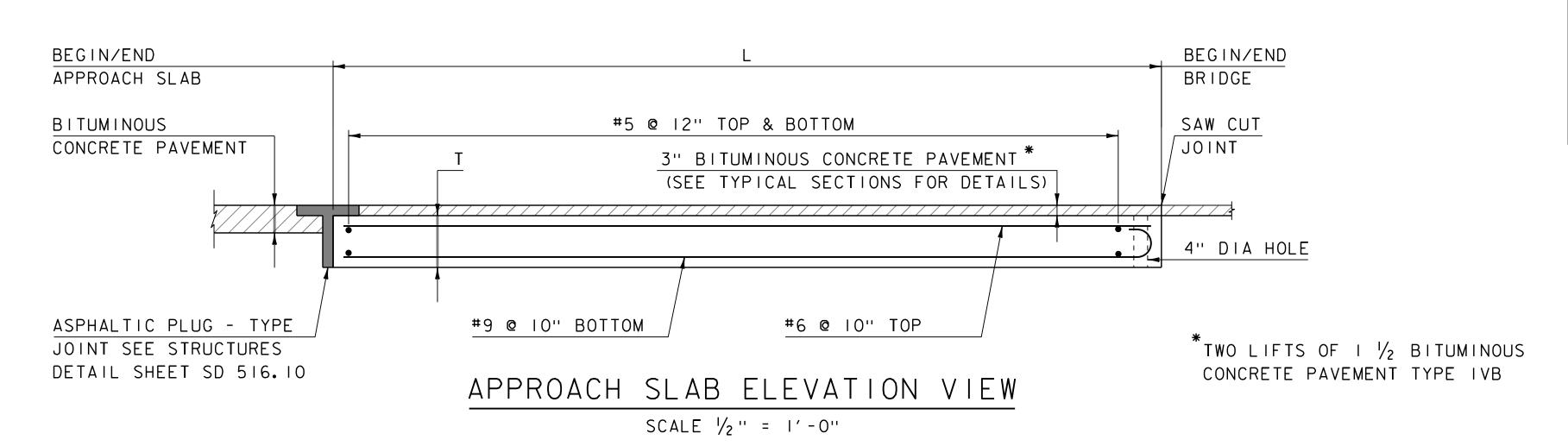
#### CONNECTION DETAIL SECTION SCALE I" = I'-0"

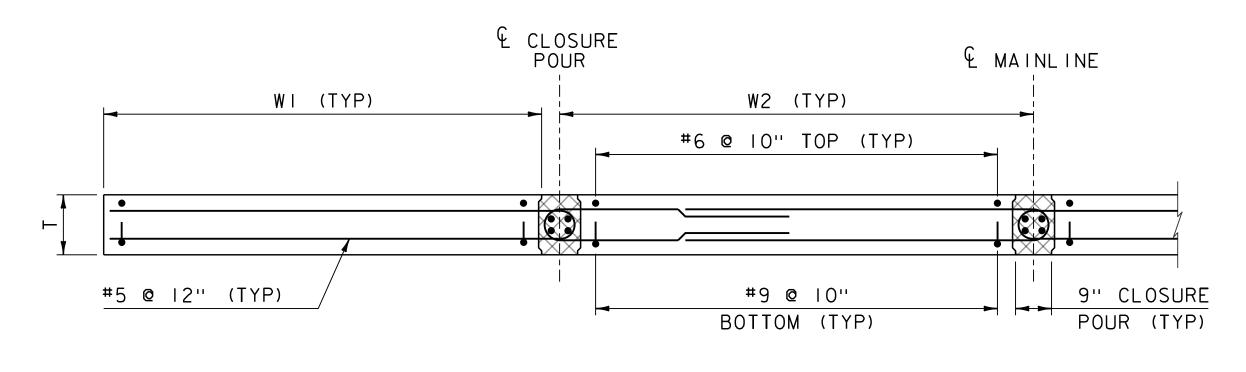
CONNECTION DETAIL PLAN SCALE I'' = I' - O''

I. LIFTING POINTS SHALL BE DESIGNED BY FABRICATOR AND SUBMITTED WITH CALCULATIONS.

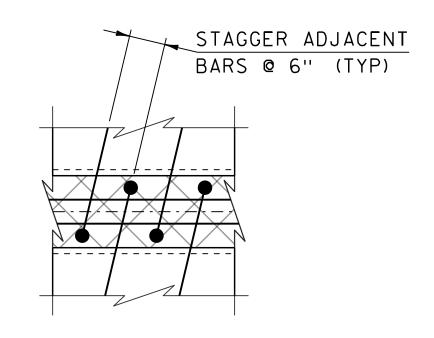
2. REINFORCING STEEL FOR APPROACH SLAB #2 SHALL BE SIMILAR TO THAT SHOWN FOR APPROACH SLAB #1

3. CLOSURE POUR CONCRETE SHALL BE PAID FOR UNDER ITEM 900.608 SPECIAL PROVISION (HIGH PERFORMANCE CONCRETE. RAPID SET) (FPQ)





SECTION A-A SCALE  $\frac{1}{2}$ " = 1'-0"



	STATION	OFFSET	ELEVATION
ΙA	103+79.00	15.33	153.59
BEGIN AS #1	103+79.00	0.00	153.31
ΙB	103+79.00	15.33	152.99
I C	103+99.00	15.33	153.92
END AS #1	103+99.00	0.00	153.56
I D	103+99.00	15.33	153.19

APPROACH SLAB #2

STATION

106+40.36

106+40.50

106+40.65

106+59.86

106+60.51

106+60.50

106+61.19

P. O. S. T.

OFFSET

15.36

0.00

15.31

15.32

0.00

0.00

15.32

ELEVATION

157.66

156.58

154.54

157.87

156.83

156.87

155.74

APPROACH SLAB #1

Т	1'-3"	APPROACH	АΙ	90°
L	20′ -0''	SLAB #1	A2	90°
WI-R	7′ - 4''			
W2-R	8′-0"	•		BASED ON CONTROL
W I - L	7′ - 4''	APPROACH	АΙ	90°

#### APPROACH SLAB DIMENSIONS

SLAB #2

Α2

90°

#### NOTE:

W2-L 8'-0"

NF = NEAR FACE FF = FAR FACE EF = EACH FACE 3" CLEAR, UNLESS OTHERWISE SPECIFIED ON THE PLANS. 2'-7" BAR LAP UNLESS OTHERWISE

SPECIFIED ON THE PLANS.

APPROACH SLAB ELEVATIONS

ALL ELEVATIONS ARE TOP OF SLAB

WEYBRIDGE-NEW HAVEN PROJECT NAME: PROJECT NUMBER: BF 032-1(19)

FILE NAME: sl2b552appslab.dgn PROJECT LEADER: C.W. CARLSON DESIGNED BY: D. PETERSON APPROACH SLAB DETAILS

2 A

BEGIN AS #2

2B

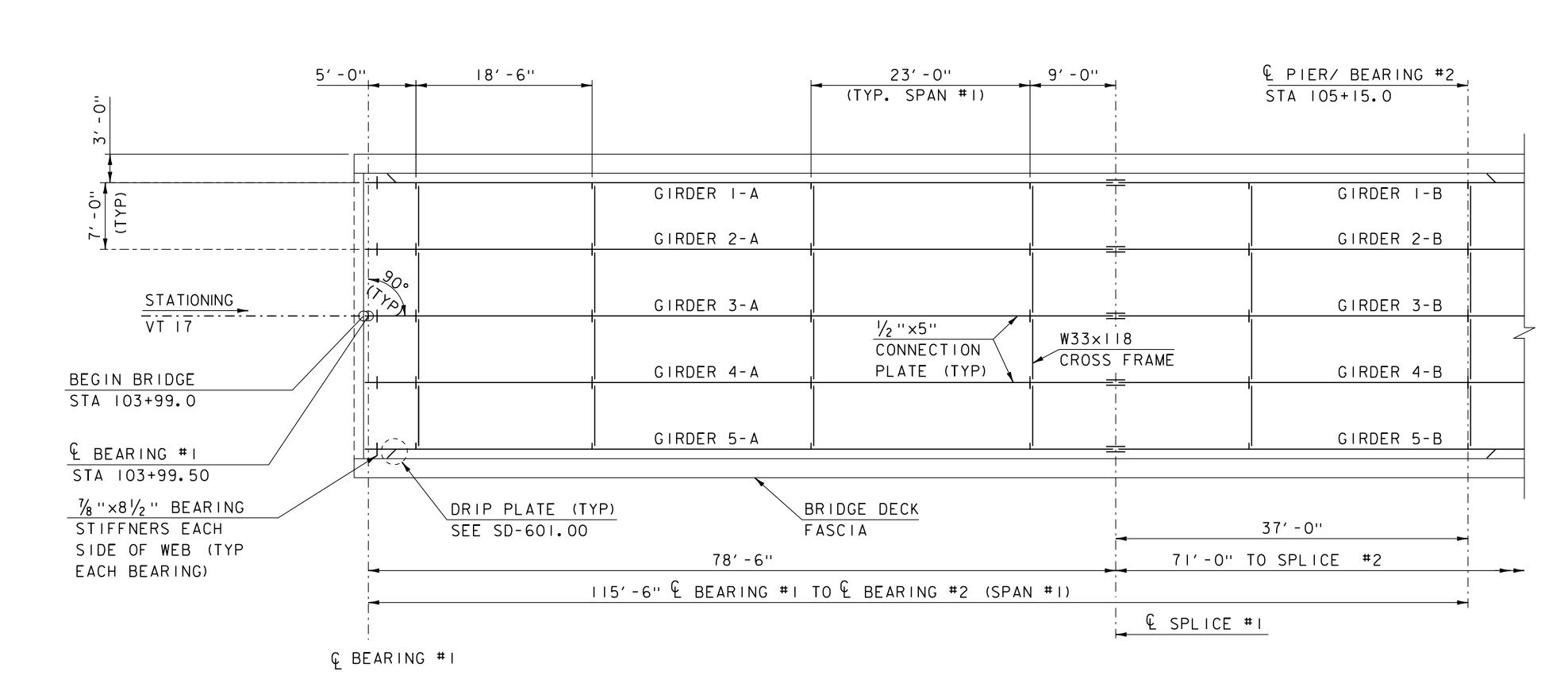
2 C

END AS #2

END AS #2

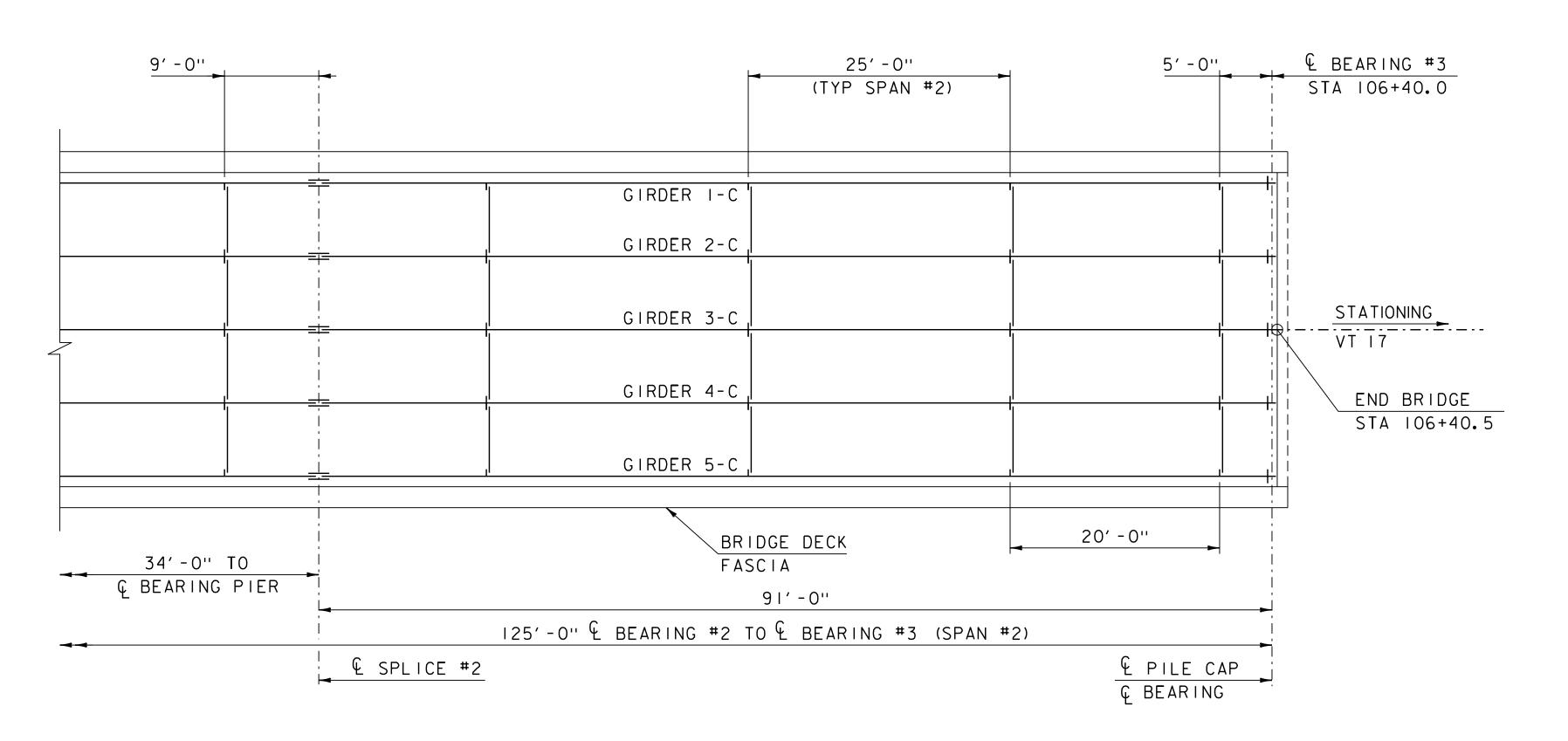
2 D

PLOT DATE: 20-APR-2017 DRAWN BY: M. LONGSTREET CHECKED BY: D. PETERSON SHEET 43 OF 85



### DECK FRAMING PLAN (PART 1)

SCALE:  $\frac{1}{8}$  " = 1'-0"



### DECK FRAMING PLAN (PART 2)

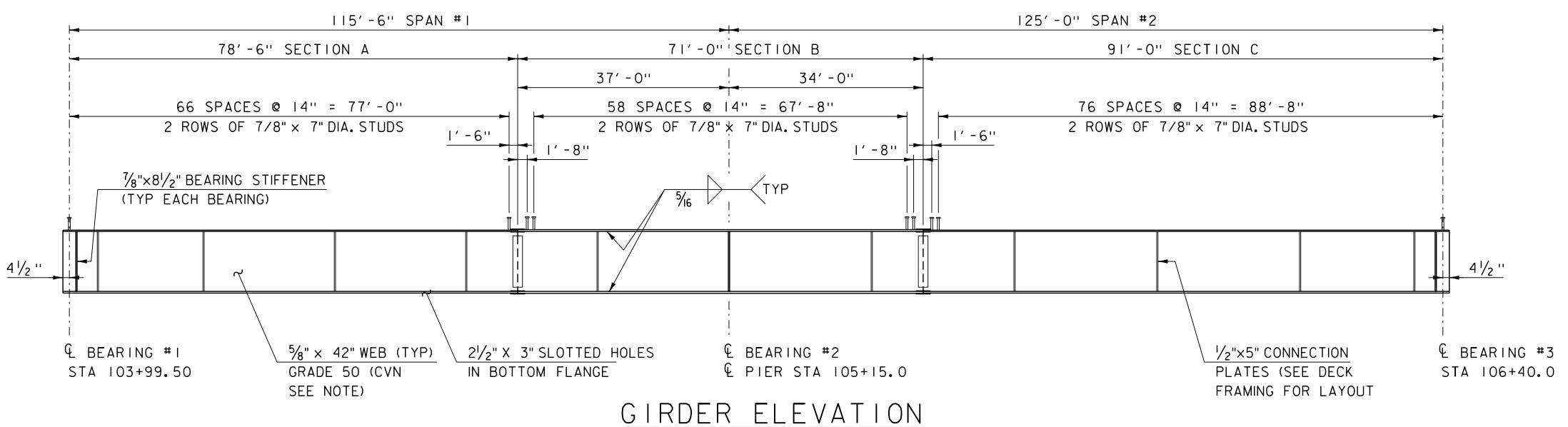
SCALE: 1/8" = 1'-0"

PROJECT NAME: WEYBRIDGE-NEW HAVEN PROJECT NUMBER: BF 032-1(19)

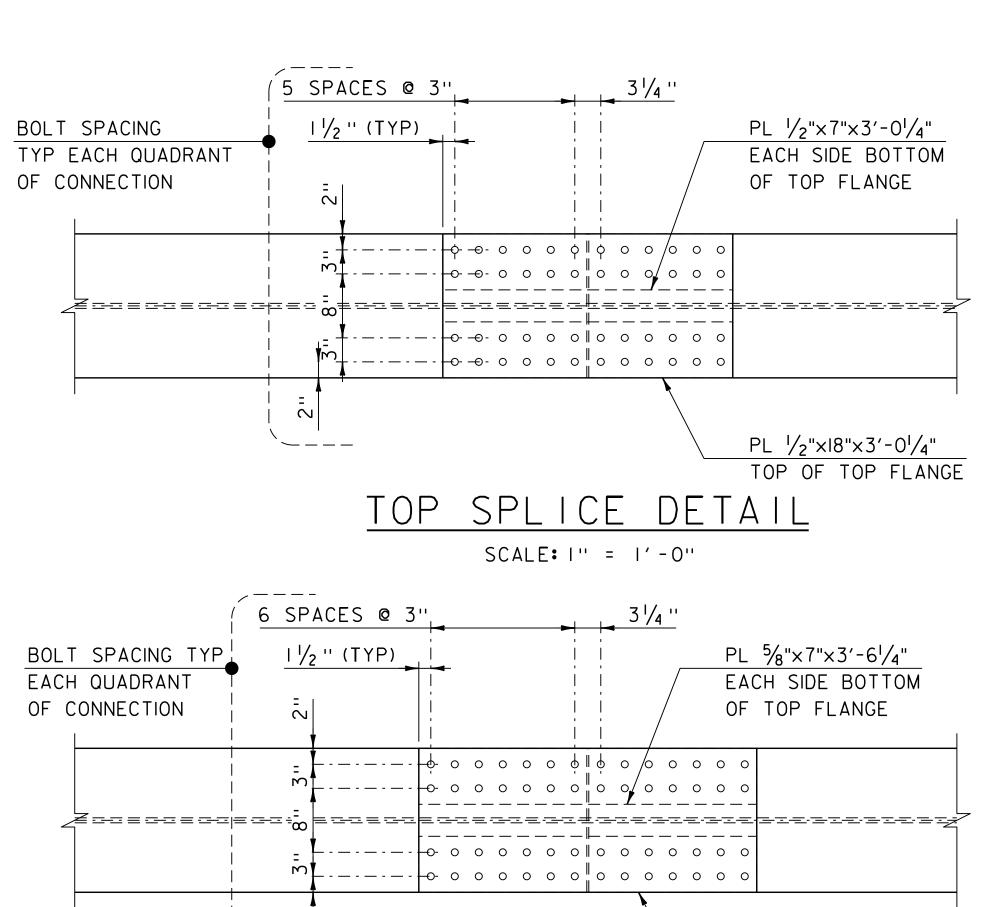
FILE NAME: sl2b552sup.dgn
PROJECT LEADER: C.W. CARLSON
DESIGNED BY: D. PETERSON
DECK FRAMING PLAN

PLOT DATE: 20-APR-2017
DRAWN BY: M. LONGSTREET
CHECKED BY: D. PETERSON
SHEET 44 OF 85

	PLA	TE GIRDER - F	LATE SIZE CF	IAKI	
GIRDER	GIRDER	THICKNESS	WIDTH	CHARPY	STEEL
SECTION	COMPONENT	(IN)	(IN)	V-NOTCH	GRADE
		- 10			
_	TOP FLANGE	7/8	18	YES	50
Α	WEB	5/8	42	YES	50
	BOTTOM FLANGE	1 1/4	18	YES	50
	TOP FLANGE	2	18	YES	50
В	WEB	5/8	42	YES	50
	BOTTOM FLANGE	2	18	YES	50
	TOP FLANGE	7/8	18	YES	50
C	WEB	5/8	42	YES	50
	BOTTOM FLANGE	1 1/4	18	YES	50



N. T. S  $(H = \frac{1}{4} / 3 + V = \frac{1}{4})$ 

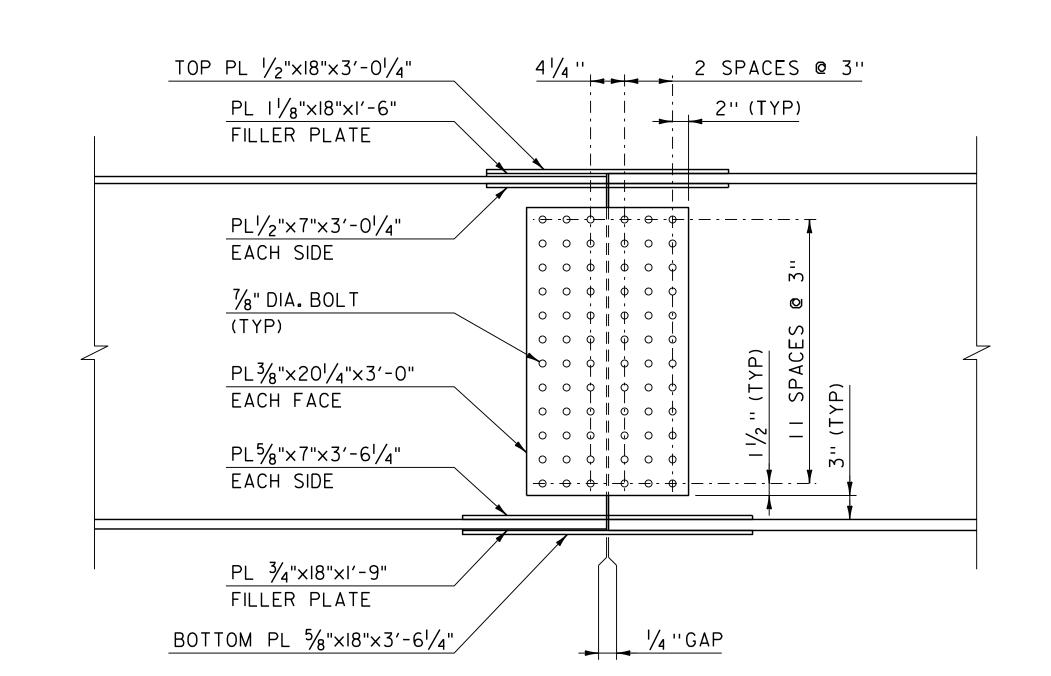


BOTTOM SPLICE DETAIL

SCALE: I" = I'-O"

 $PL \frac{5}{8}$ "×18"×3'-6 $\frac{1}{4}$ "

TOP OF TOP FLANGE



### GIRDER ELEVATION SPLICE DETAIL

SCALE: I" = I'-0"

I.) TYPICAL SPLICE LINE #I DETAIL, MIRROR FOR SPLICE LINE #2.

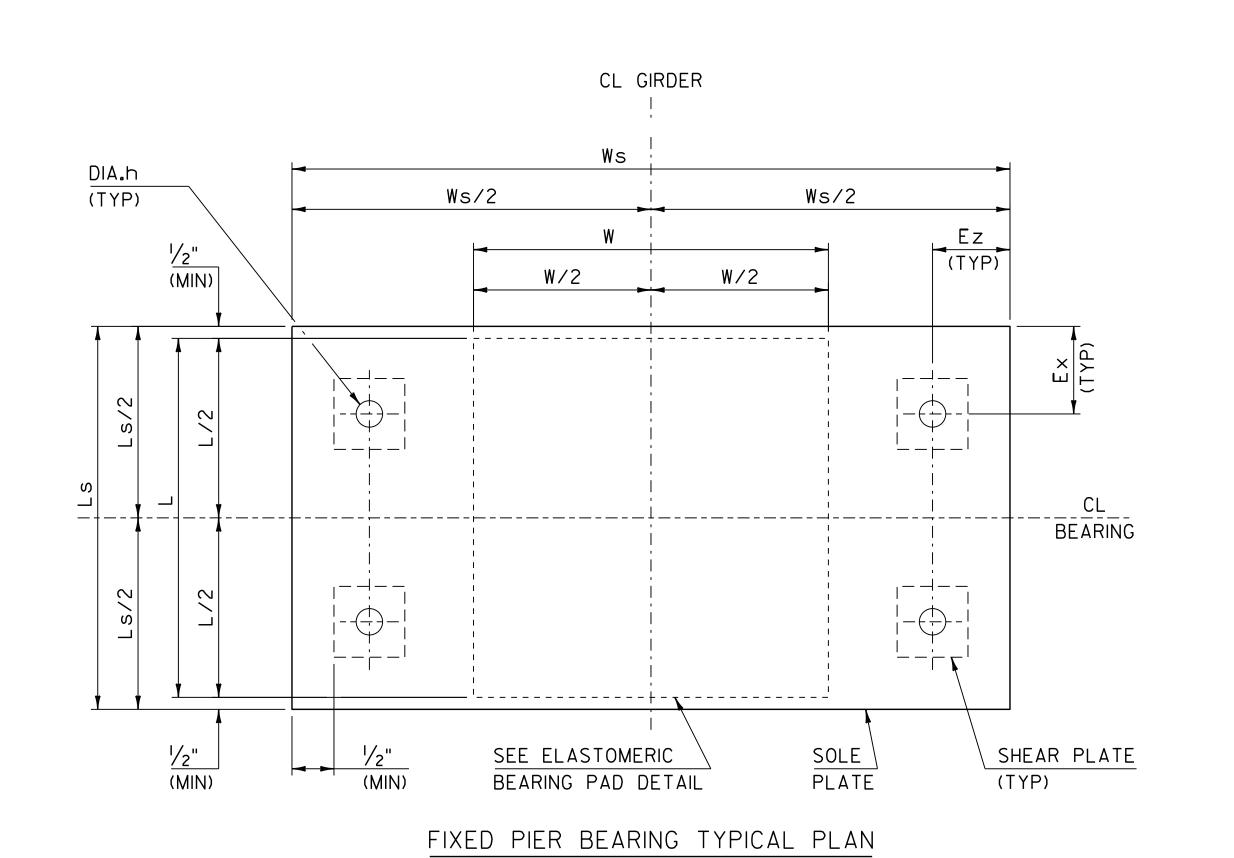
#### NOTE:

CVN - SHALL MEET CHARPY V-NOTCH REQUIREMENTS FOR MAIN MEMBERS AS INDICATED IN SECTION 714 OF THE STANDARD SPECIFICATION

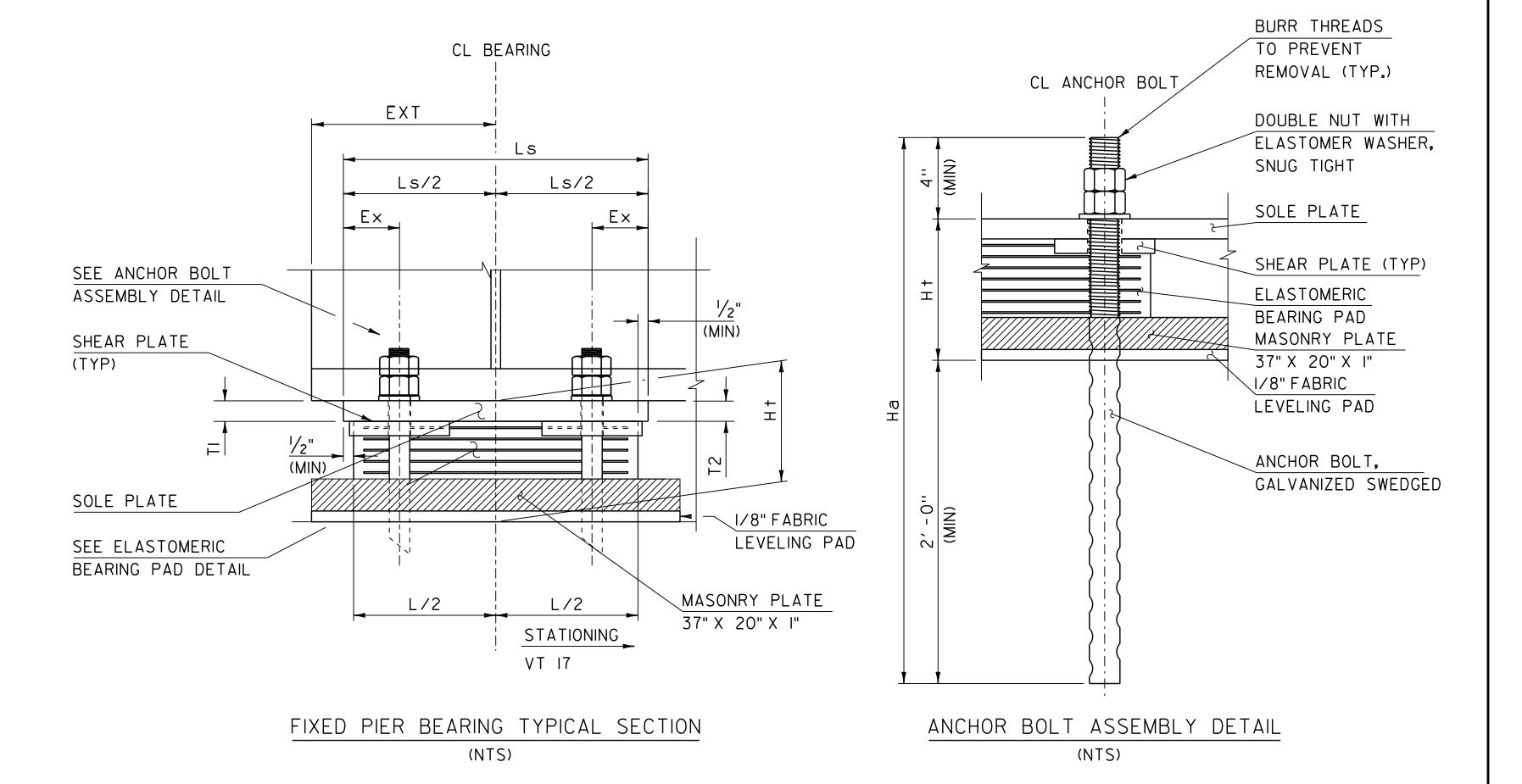
PROJECT NAME: WEYBRIDGE-NEW HAVEN PROJECT NUMBER: BF 032-1(19)

FILE NAME: sl2b552sup.dgn
PROJECT LEADER: C.W. CARLSON
DESIGNED BY: D. PETERSON
GIRDER DETAILS

PLOT DATE: 20-APR-2017
DRAWN BY: M. LONGSTREET
CHECKED BY: D. PETERSON
SHEET 45 OF 85



(NTS)



#### NOTES

- I. THE TOTAL HEIGHT OF THE ELASTOMER LAYERS AND STEEL LAMINATIONS: Hp=2(Hc)+Ns(Ts)+Ne(Te)
- SHEAR PLATES (TO REINFORCE SOLE PLATE) ARE 5"×5"×3".

#### CL GIRDER Ws Ws/2 Ws/2 CL BEARING SEE ANCHOR BOLT Ez Εz ASSEMBLY DETAIL NS EQUALS LAYERS OF STEEL @ THICKNESS OF Ts SOLE PLATE 1 | 1 SHEAR PLATE (TYP) Ne EQUALS LAYERS (SEE NOTE 2) OF ELASTOMER @ SEE ELASTOMERIC THICKNESS OF Te 1/4" (TYP) BEARING PAD DETAIL MASONRY PLATE 37" X 20" X I" W/2 W/2 I/8" FABRIC LEVELING PAD ELASTOMERIC BEARING PAD DETAIL

## FIXED PIER BEARING TYPICAL ELEVATION (NTS)

										ELA	STOM	IERIC	BEARIN	G TABLE	- FIXED	)											
		UN	IFACTOR	ED LOADS																							
LOCATION	QUANTITY	DL+SDL	LL W/O	TOTAL DESIGN			El	LASTOM	ERIC PA	VD			A۱	ICHOR BOL	TS	ANCH	IOR BOLT I	HOLES	WELD SIZE		SOLE PI	LATE		MAS	ONRY P	LATE	BRG. Ht
LOCATION	REQUIRED	(kips)	IMP. (kips)	REACTION (kips)	L	W	Ns	Ts	Ne	Te	Нс	Нр	PER/BRG.	DIA.a	На	DIA.h	Ez	Ex	F	Ws	Ls	T1	T2	W	L	Т	DNG. HI
PIER	5	216.6	142.1	358.7	18"	22"	6	0.125"	5	0.5"	0.25"	3.75"	4	1 1/2"	35"	1 5/16"	3"	3"	5/16"	37	19	1.5"	1.875"	37"	20"	1"	6.5625"

#### **ELASTOMERIC BEARING NOTES**

- BOLTS FURNISHED FOR BEARINGS SHALL CONFORM TO ASTM F1554 GRADE 105.
- PRIOR TO WELDING BEAMS/GIRDERS TO SOLE PLATES AT THE PIERS, THE CONCRETE DECK SHALL BE PLACED AND CURED, AND THE BEAMS/GIRDERS SHALL BE RAISED TO ALLOW RELEASE OF INITIAL BEARING DEFORMATION DUE TO BEAM/GIRDER CAMBER RELAXATION. THE CONTRACTOR SHALL MEET ALL OF THE REQUIREMENTS OF SECTION 502, INCLDUING THE SUBMITTAL OF CONSTRUCTION DRAWINGS. ALL MATERIALS AND WORK REQUIRED IN ORDER TO RESET BEARINGS SHALL BE INCIDENTAL TO THE STRUCTURAL STEEL ITEM IN THE CONTRACT.

THE STEEL SOLE PLATES SHALL BE HOT BONDED TO THE REINFORCED ELASTOMERIC PAD DURING THE VULCANIZATION PROCESS. THE STEEL SURFACES TO BE BONDED TO THE PAD SHALL NOT BE GALVANIZED/METALIZED.

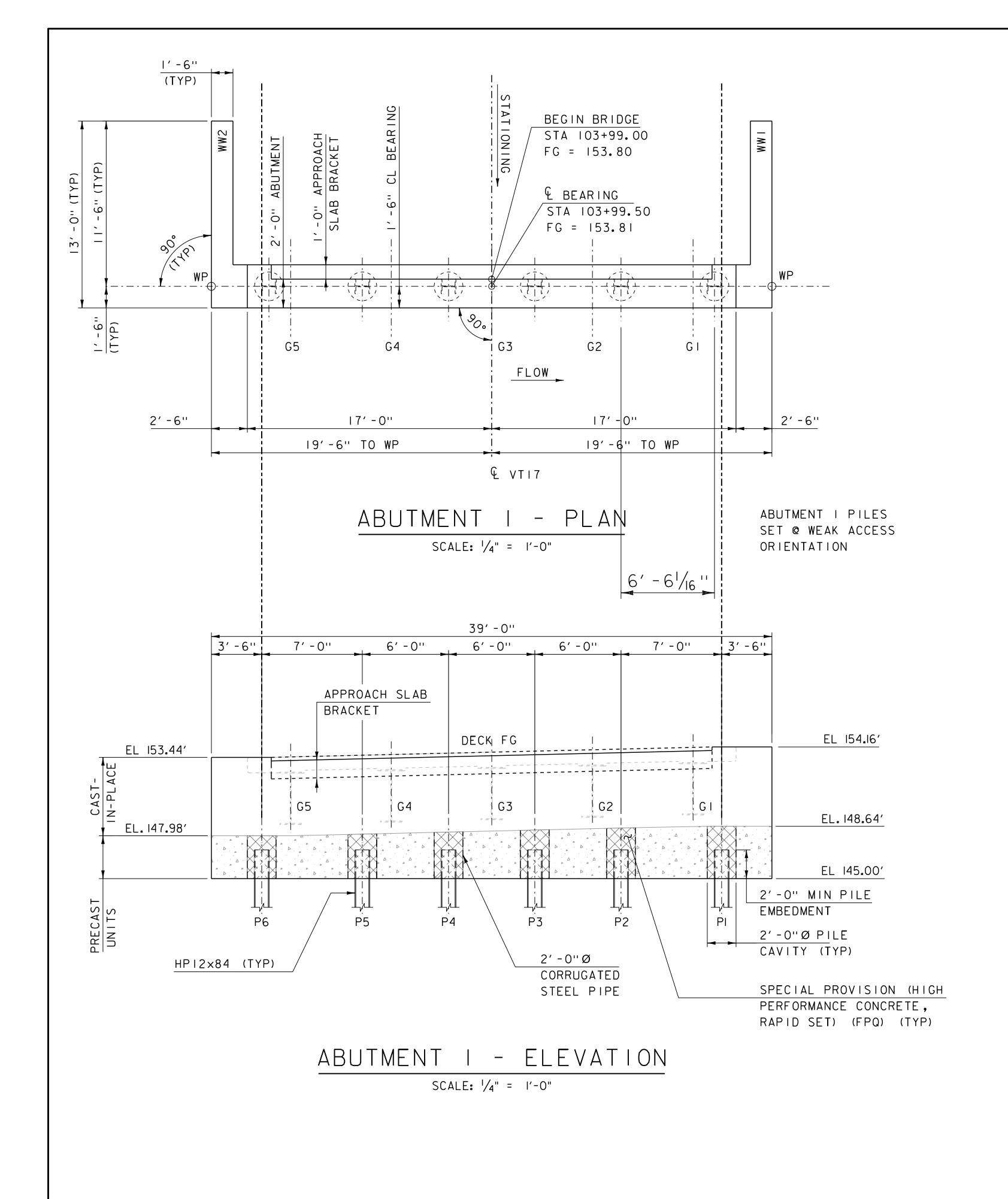
#### DESIGN CRITERIA:

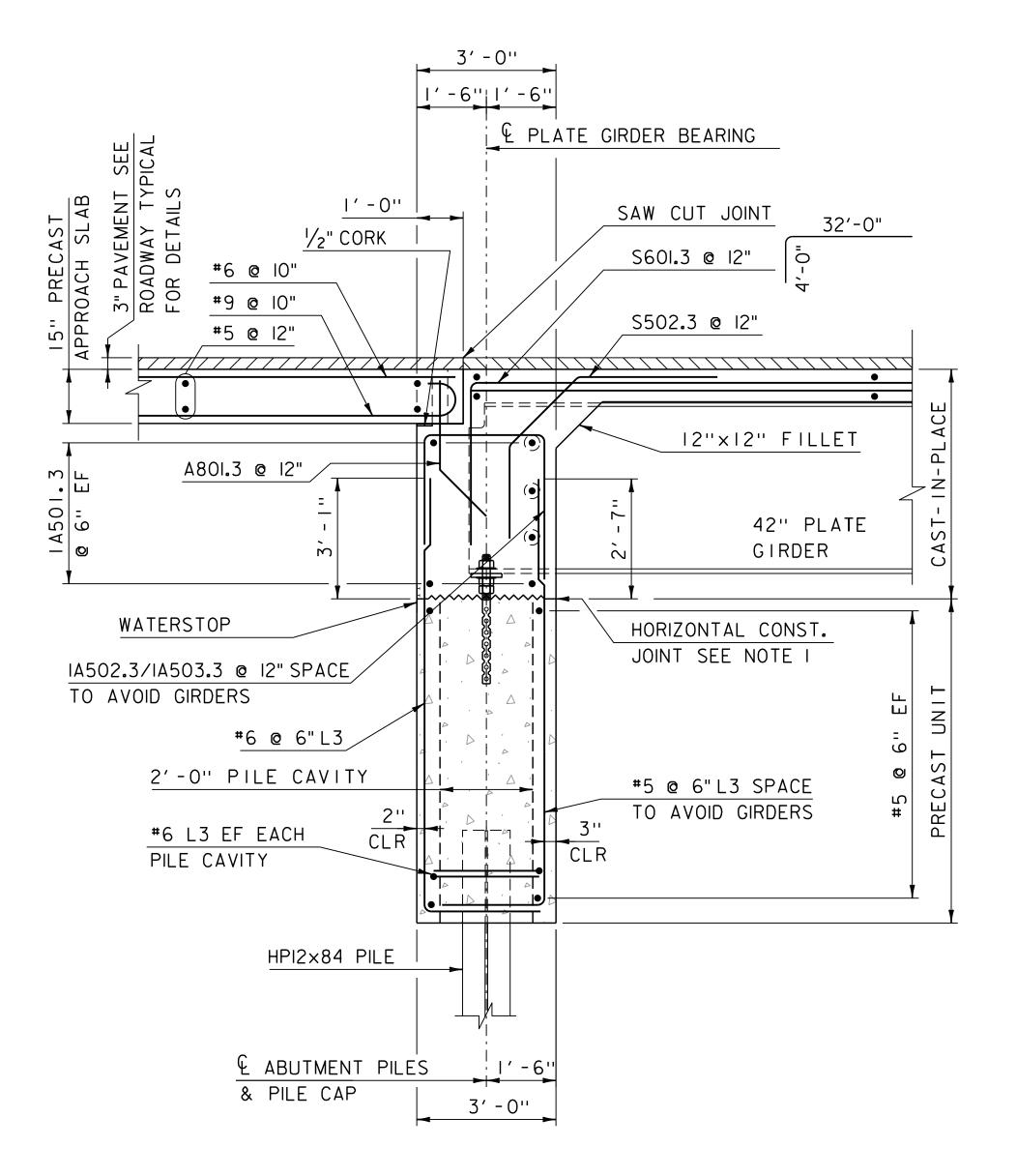
(NTS)

- A) DESIGN ROTATION PIER
- = 0.016 RAD
- 3) HORIZONTAL CAPACITY SHALL BE MINIMUM OF 25% VERTICAL LOAD IN ANY UNRESTRAINED DIRECTION.
- DESIGN LOAD PER BEARING : SEE TABLE
- D) NO FABRIC REINFORCEMENT WILL BE ALLOWED IN ELASTOMERIC PADS

PROJECT NAME: WEYBRIDGE-NEW HAVEN
PROJECT NUMBER: BF 032-1(19)

FILE NAME: si2b552sup.dgn PLOT DATE: 20-APR-2017
PROJECT LEADER: C.W. CARLSON DRAWN BY: M. LONGSTREET
DESIGNED BY: D. PETERSON CHECKED BY: D. PETERSON
PIER BEARINGS DETAIL SHEET SHEET 46 OF 85





### ABUTMENT I - TYPICAL

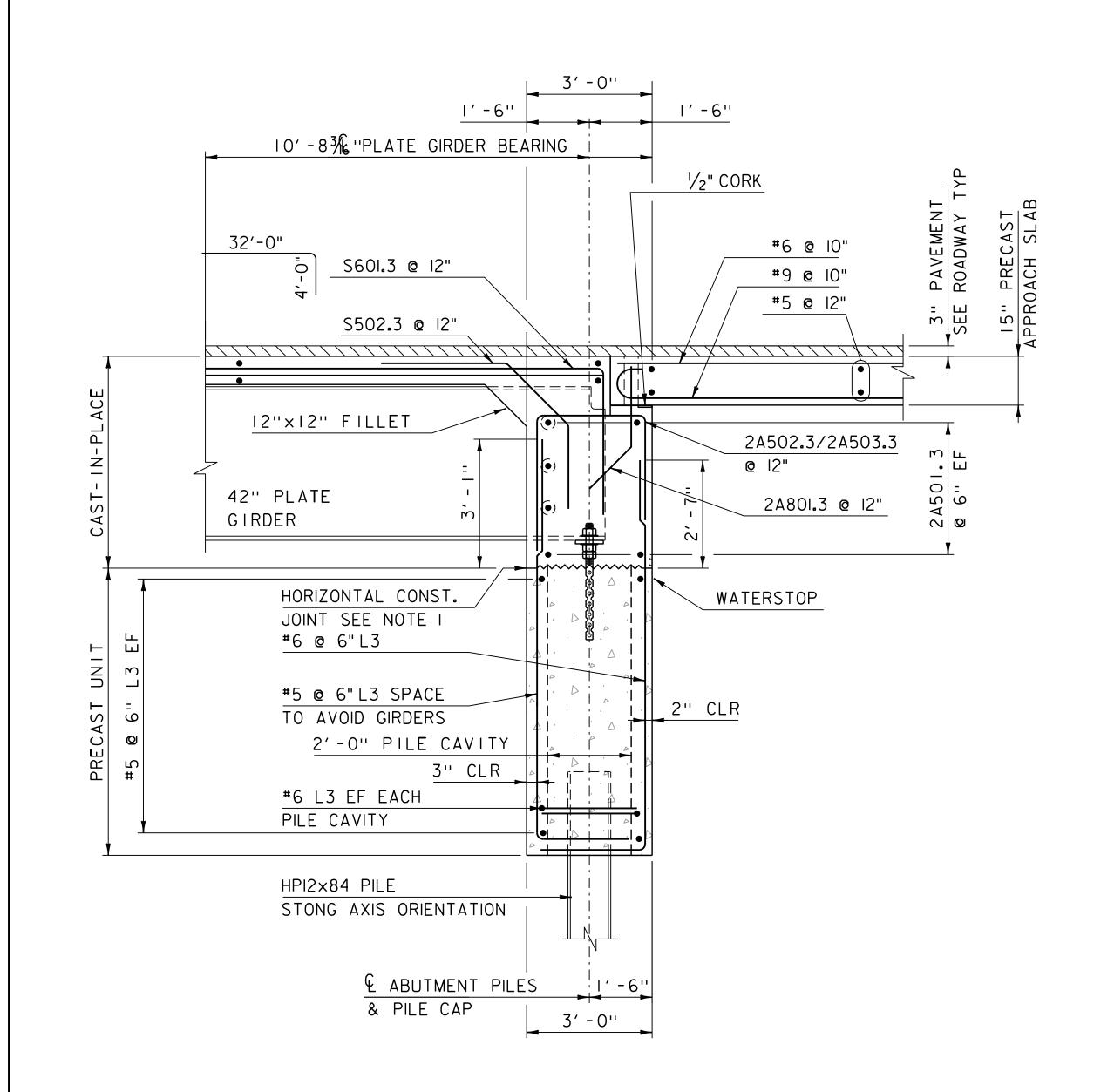
SCALE:  $\frac{1}{2}$ " = 1'-0"

- I) HORIZONTAL CONSTRUCTION JOINT SHALL BE ROUGHENED AS SHOWN IN SD-501.00. SURFACE SHALL BE ROUGHENED TO WITHIN 3" OF EACH FACE OF CONCRETE.
- 2) CUT TABS ON WATERSTOP AND ADHERE TO PRECAST.

PROJECT NAME: WEYBRIDGE-NEW HAVEN PROJECT NUMBER: BF 032-1(19)

FILE NAME: si2b552sub.dgn
PROJECT LEADER: C.W. CARLSON
DESIGNED BY: D. PETERSON
ABUTMENT I PLAN & ELEVATION

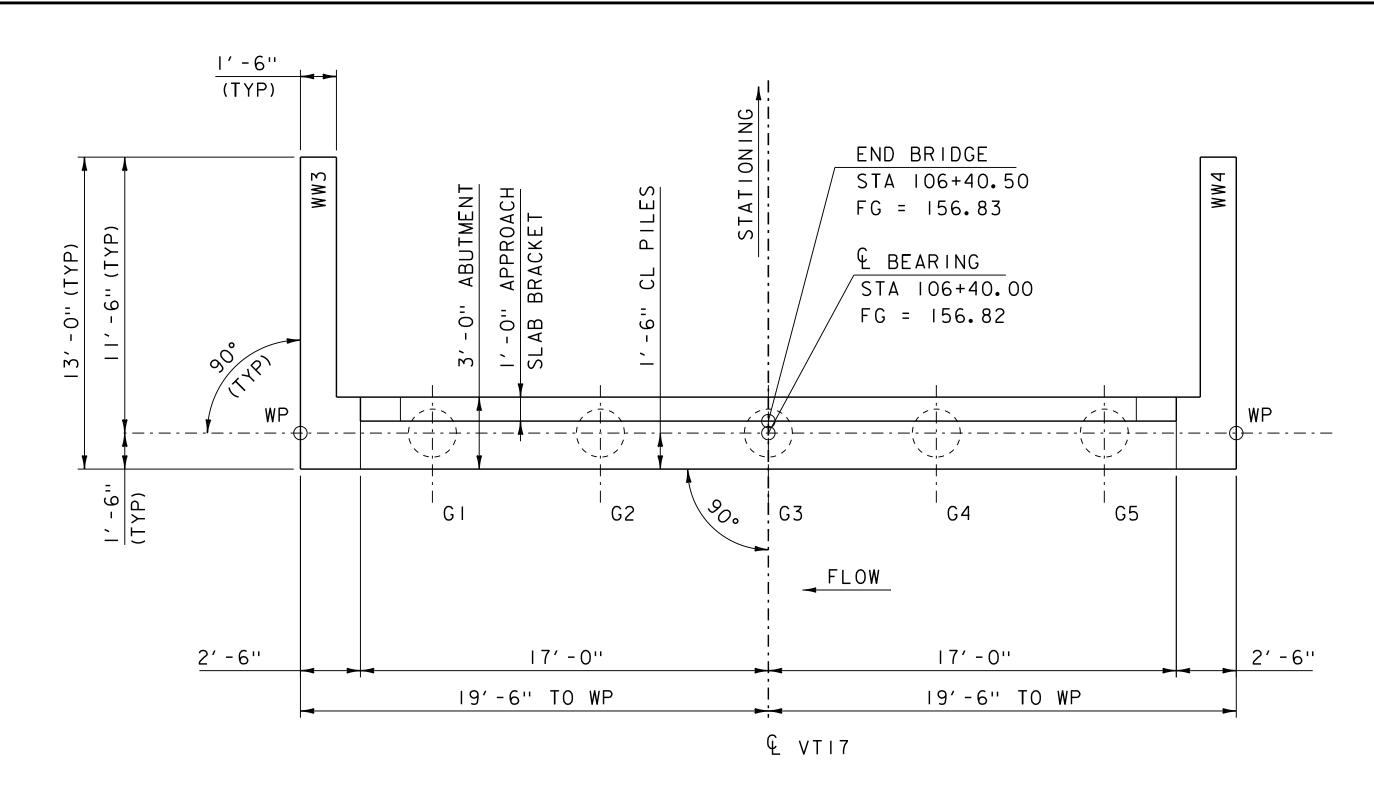
PLOT DATE: 20-APR-2017
DRAWN BY: M. LONGSTREET
CHECKED BY: D. PETERSON
SHEET 47 OF 85



### ABUTMENT 2 - TYPICAL

SCALE:  $\frac{1}{2}$ " = 1'-0"

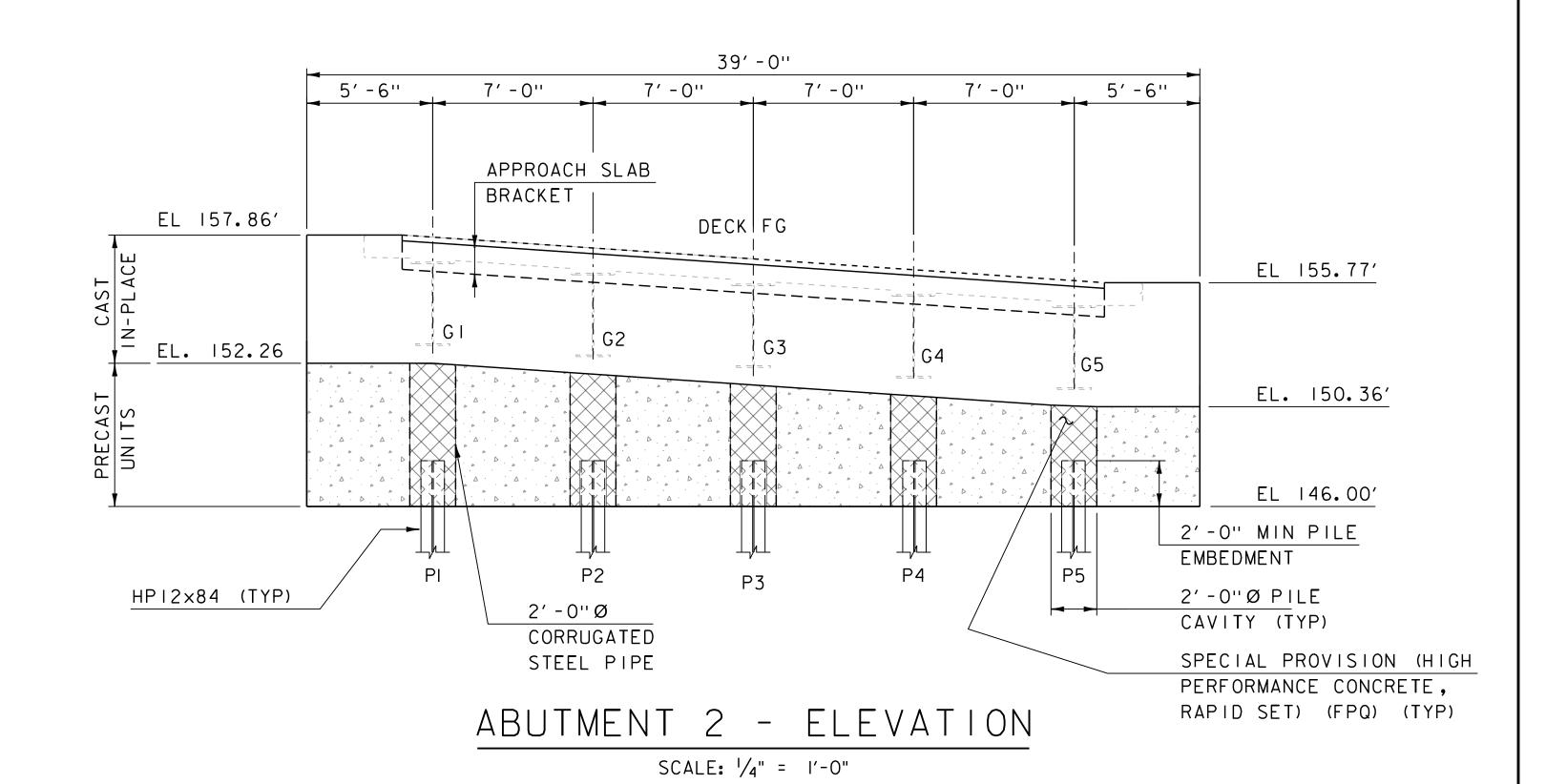
- I) HORIZONTAL CONSTRUCTION JOINT SHALL BE ROUGHENED AS SHOWN IN SD-501.00. SURFACE SHALL BE ROUGHENED TO WITHIN 3" OF EACH FACE OF CONCRETE.
- 2) CUT TABS ON WATERSTOP AND ADHERE TO PRECAST.



ABUTMENT 2 - PLAN

SCALE: 1/4" = 1'-0"

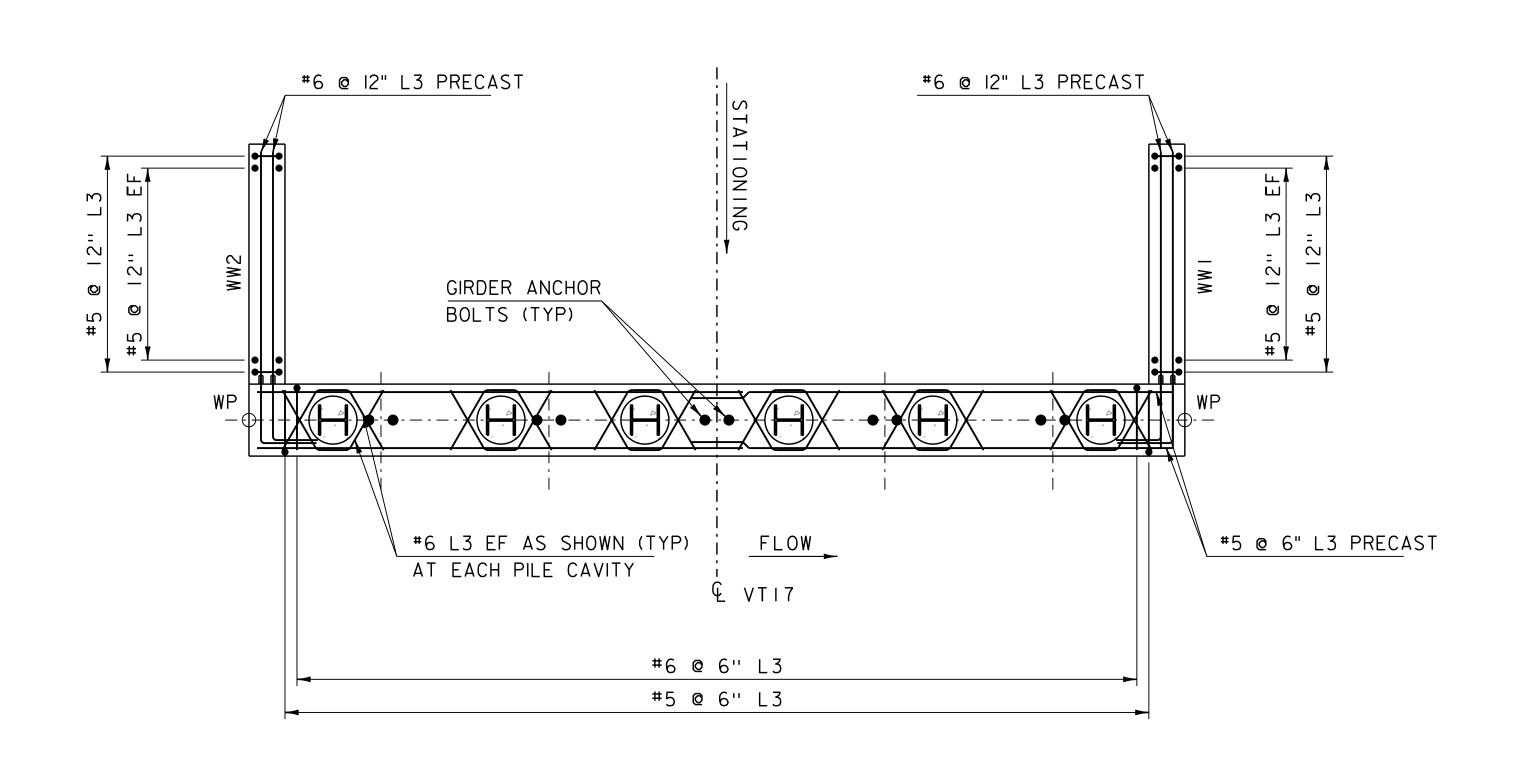
ABUTMENT 2 PILES
SET @ STRONG ACCESS
ORIENTATION



PROJECT NAME: WEYBRIDGE-NEW HAVEN PROJECT NUMBER: BF 032-1(19)

FILE NAME: sI2b552sub.dgn
PROJECT LEADER: C.W. CARLSON
DESIGNED BY: D. PETERSON
ABUTMENT 2 PLAN & ELEVATION

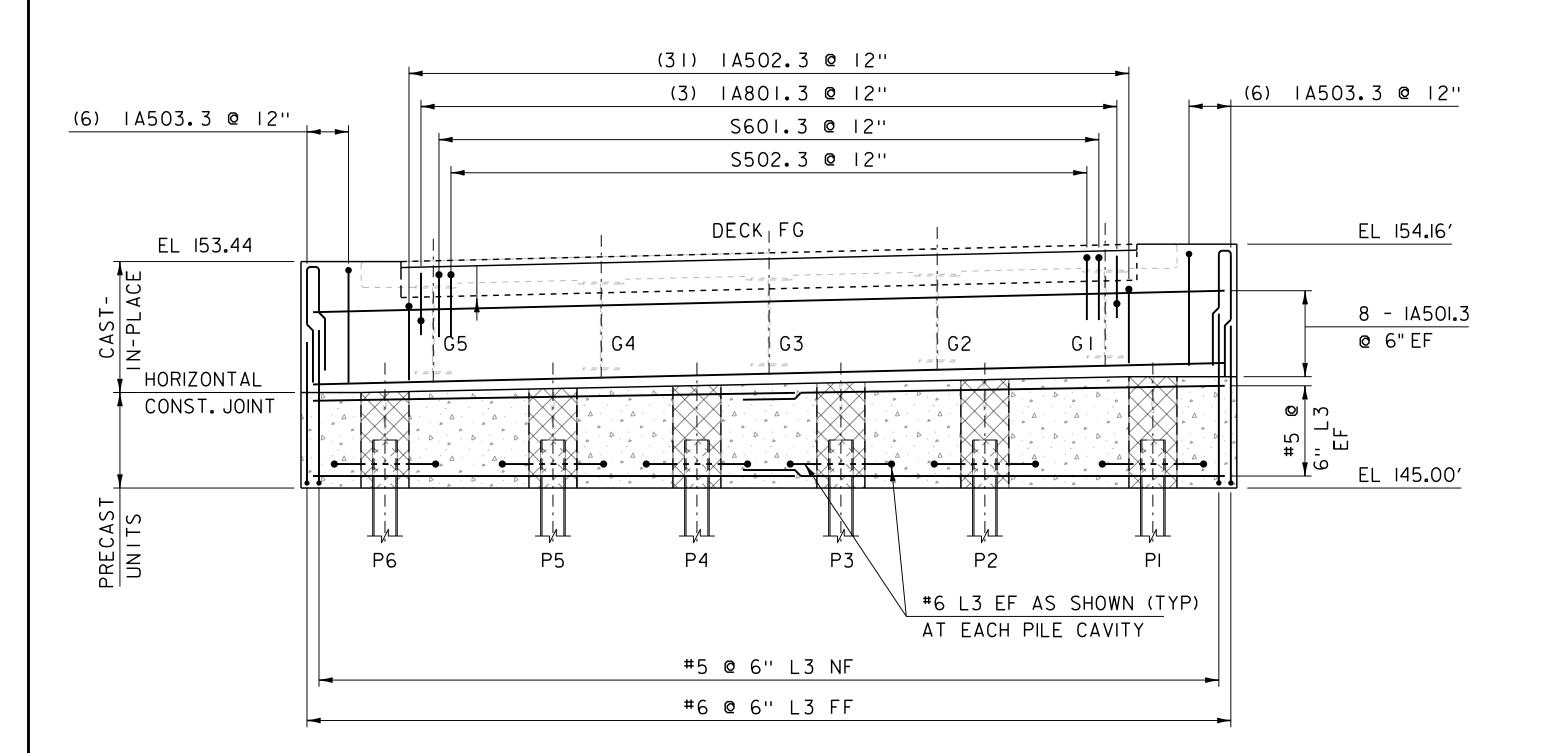
PLOT DATE: 20-APR-2017
DRAWN BY: M. LONGSTREET
CHECKED BY: D. PETERSON
SHEET 48 OF 85



### ABUTMENT I PRECAST - REINFORCING PLAN

SCALE: 1/4" = 1'-0"

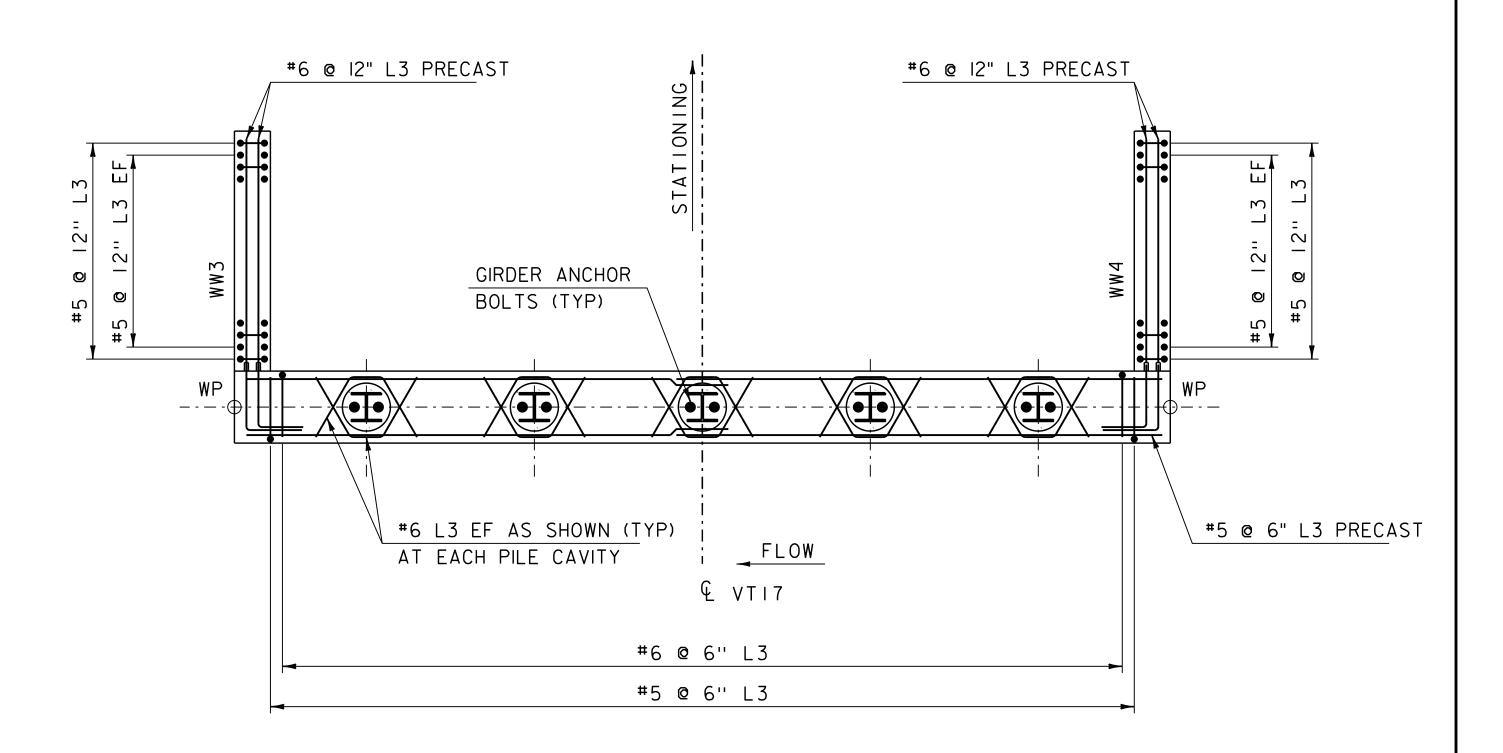
ABUTMENT I PILES
SET @ WEAK ACCESS
ORIENTATION



### ABUTMENT I - ELEVATION

SCALE: 1/4" = 1'-0"

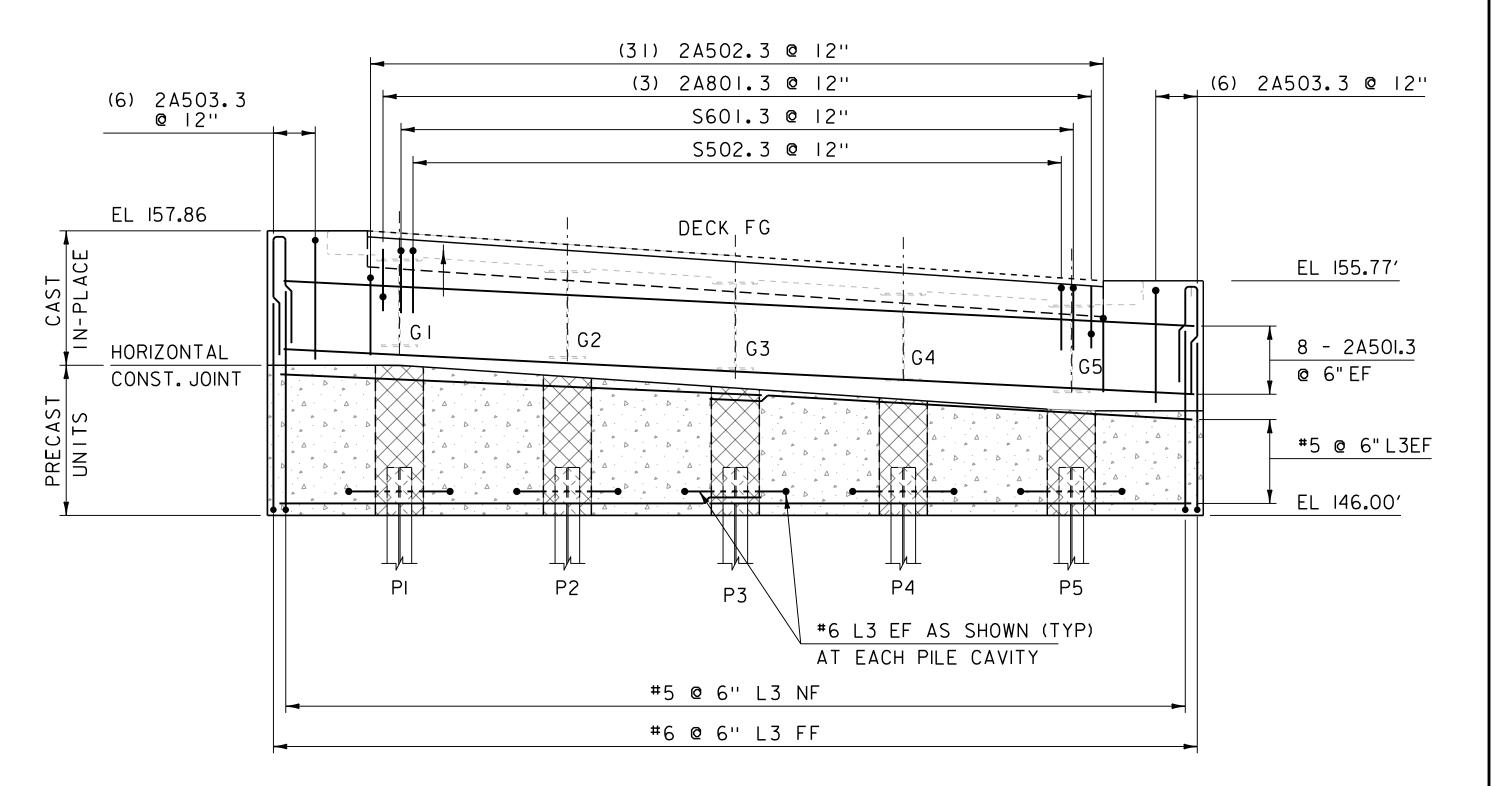
	GIRDER SEAT EI	LEVATIONS
	TOP OF LEVELIN	NG PLATE
	ABUTMENT #I	ABUTMENT #2
GI	149.22	152.84
G2	149.05	152.36
G3	148.89	151.89
G4	148.72	151.41
G5	148.56	150.94



### ABUTMENT 2 PRECAST - REINFORCING PLAN

SCALE:  $\frac{1}{4}$ " = 1'-0"

ABUTMENT 2 PILES SET @ STRONG ACCESS ORIENTATION



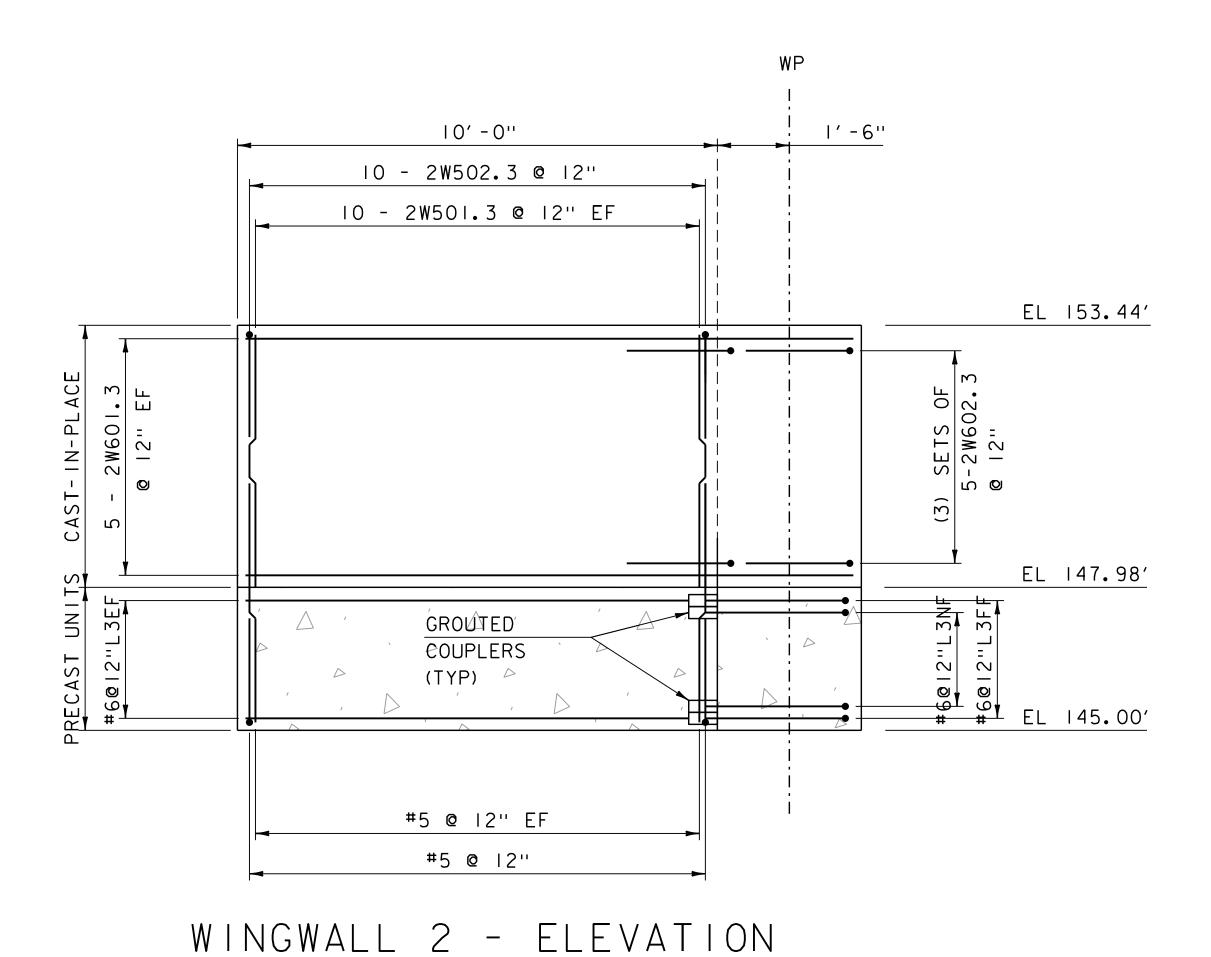
### ABUTMENT 2 - ELEVATION

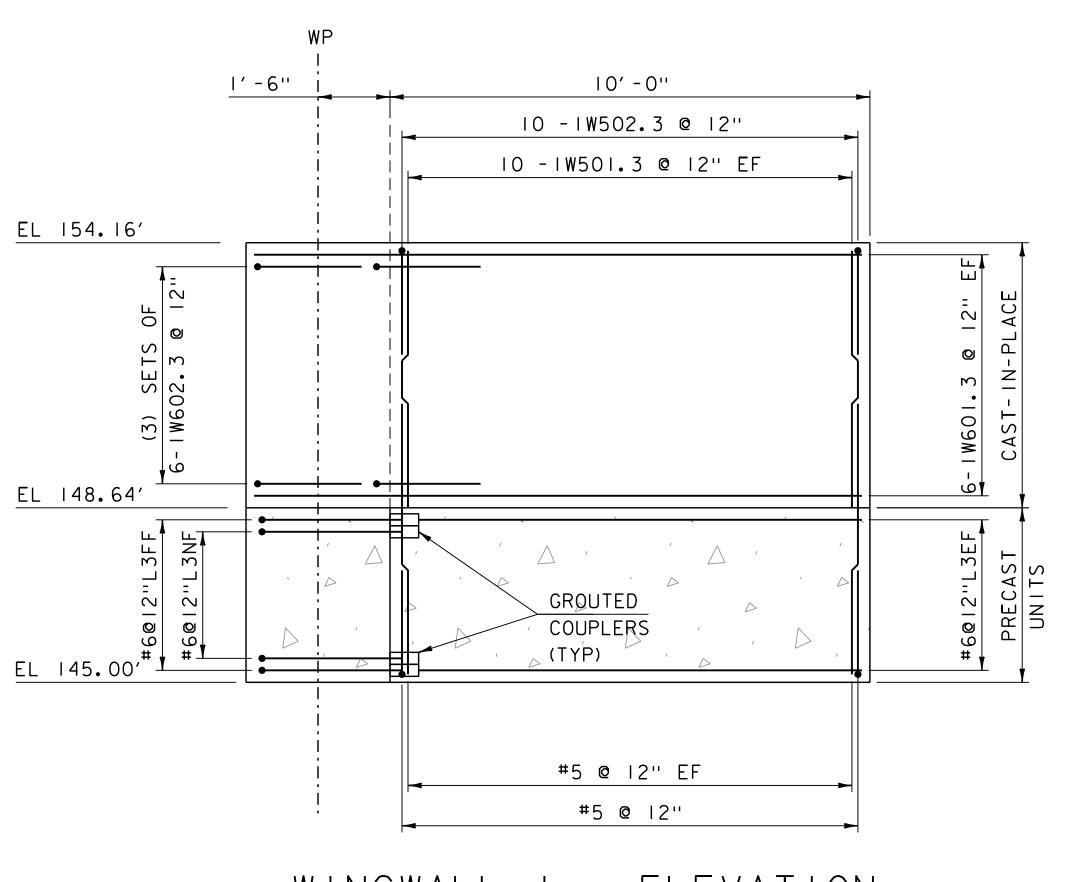
SCALE: 1/4" = 1'-0"

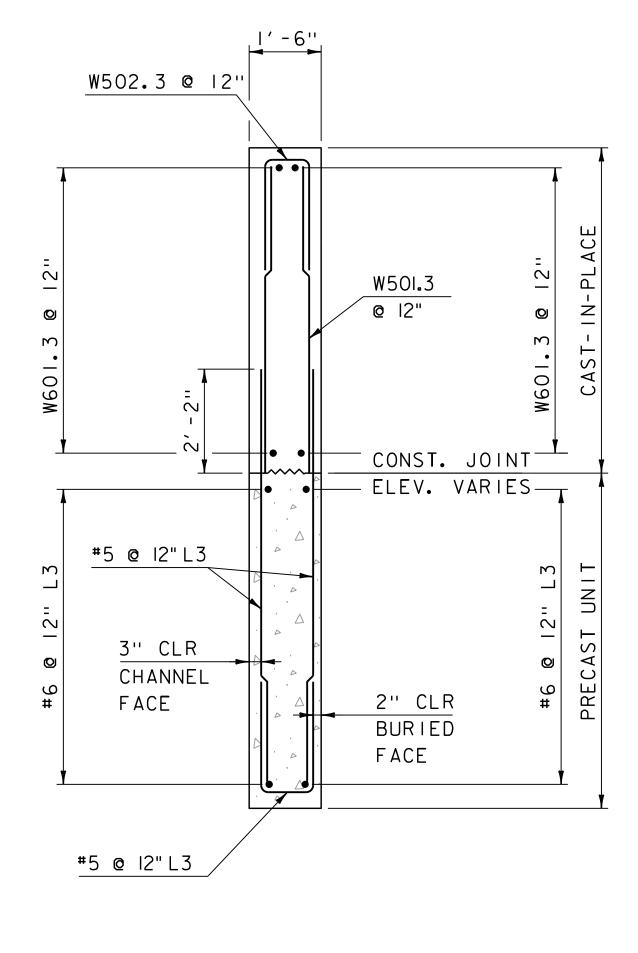
PROJECT NAME: WEYBRIDGE-NEW HAVEN PROJECT NUMBER: BF 032-1(19)

FILE NAME: sl2b552sub.dgn
PROJECT LEADER: C.W. CARLSON
DESIGNED BY: D. PETERSON
ABUTMENT REINFORCING DETAILS

PLOT DATE: 20-APR-2017
DRAWN BY: M. LONGSTREET
CHECKED BY: D. PETERSON
SHEET 49 OF 85

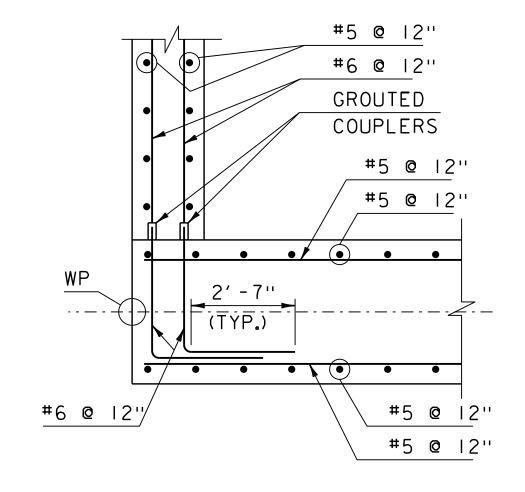






- ELEVATION WINGWALL SCALE:  $\frac{1}{2}$ " = 1'-0"

-4 - TYPICAL WINGWALL I SCALE:  $\frac{1}{2}$ " = 1'-0"



SCALE:  $\frac{1}{2}$ " = 1'-0"

#### WWI-WW4 CORNER DETAIL PCU

SCALE:  $\frac{1}{2}$ " = 1'-0" (BELOW SEAT)

I) WING WALL CORNER DETAILS SIMILAR BUT OPOSITE HAND FOR WWI AND WW4.

W501.3 W601.3 A501.3 #6 @ 6" L3 • • / • • • W602.3 2'-7" #5 @ 6" L3 (TYP.) A501.3

#### WWI-WW4 CORNER DETAIL CIP

SCALE:  $\frac{1}{2}$ " = 1'-0" (ABOVE SEAT)

I) WING WALL CORNER DETAILS SIMILAR BUT MIRRORED FOR WWI AND WW4.

NOTE:

NF = NEAR FACE FF = FAR FACE

EF = EACH FACE ▲ = CUT TO FIT IN FIELD 3" CLEAR, UNLESS OTHERWISE SPECIFIED ON THE PLANS. 2'-2" BAR LAP UNLESS OTHERWISE

SPECIFIED ON THE PLANS.

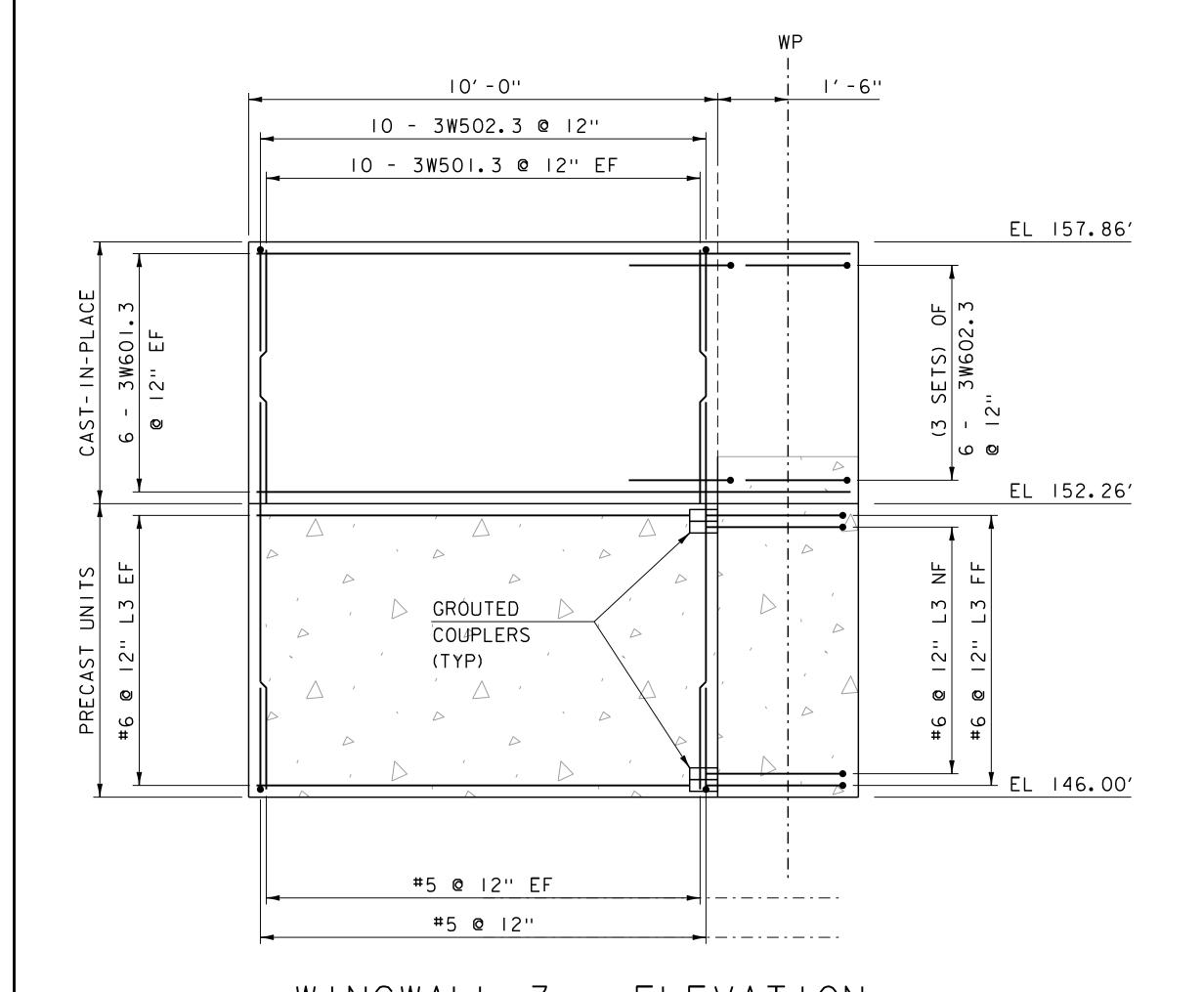
WEYBRIDGE-NEW HAVEN PROJECT NAME: PROJECT NUMBER: BF 032-1(19)

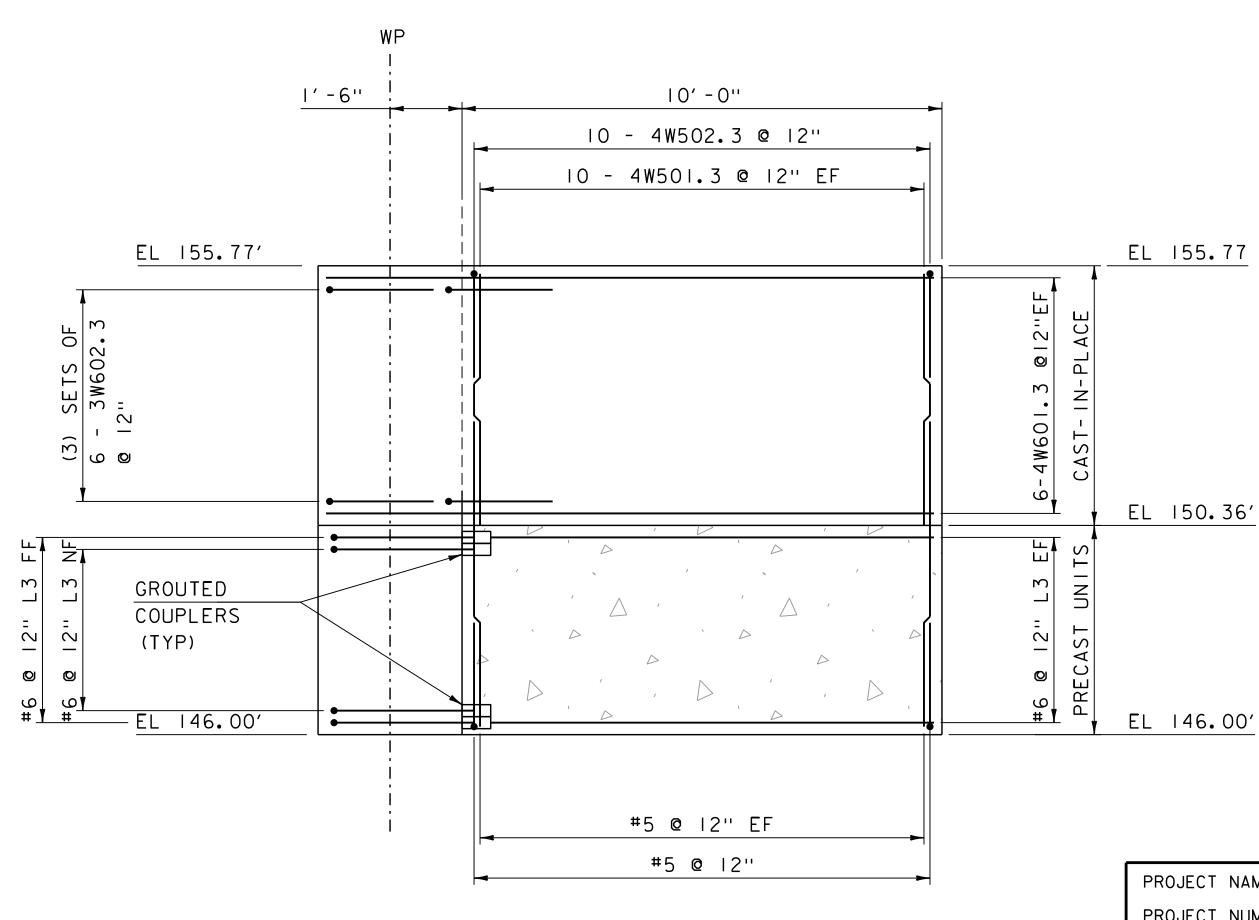
FILE NAME: sl2b552sub.dgn PROJECT LEADER: C.W. CARLSON DESIGNED BY: D. PETERSON WINGWALL DETAILS I

PLOT DATE: 20-APR-2017 DRAWN BY: M. LONGSTREET CHECKED BY: D. PETERSON SHEET 50 OF 85

PRECAST ABUTMENT FABRI	CATION TOLERANCES
LENGTH (OVERALL)	+/-  /4 ''
WIDTH (OVERALL)	*/- ¹ / ₄ ''
DEPTH (OVERALL)	+/-  / ₄ ''
VARIATION FROM SPECIFIED	½ 1/8" PER 12" WIDTH
END SQUARENESS OR SKEW	½ ½" MAXIMUM
LOCATION OF MECHANICAL SPLICE CONNECTORS MEASURED FROM COMMON REFERENCE POINT	+/- ¹ / ₄ ''
LOCATION OF PROJECTING REINFORCING MEASURED FROM COMMON REFERENCE POINT	+/- ¹ / ₄ ''
LOCAL SMOOTHNESS OF ANY SURFACE	½ ¼" IN IO FEET
LOCATION OF POST TENSIONING CONDUITS	+⁄- ¹ / ₄ ''
LOCATION OF PILE CAVITIES	<del>'</del> /-   ''

PRECAST ABUTMENT ERI	ECTION TOLERANCE
VARIATION FROM SPECIFIED BRIDGE SEAT ELEVATION	½ ½ 1, ½ 1 MAXIMUM BETWEEN ADJACENT UNITS
PLAN LOCATION OF ANY POINT MEASURED FROM COMMON REFERENCE POINT	+/_ ¹ / ₂ ''
PLUMB	½ ¼" IN IO FEET
	½ ½" MAXIMUM





WINGWALL 4 - ELEVATION

SCALE:  $\frac{1}{2}$ " = 1'-0"

NOTE:

NF = NEAR FACE FF = FAR FACE EF = EACH FACE

EF = EACH FACE

A = CUT TO FIT IN FIELD

ZH CLEAR LINESS OTHERWIS

3" CLEAR, UNLESS OTHERWISE SPECIFIED ON THE PLANS.

2'-2" BAR LAP UNLESS OTHERWISE SPECIFIED ON THE PLANS.

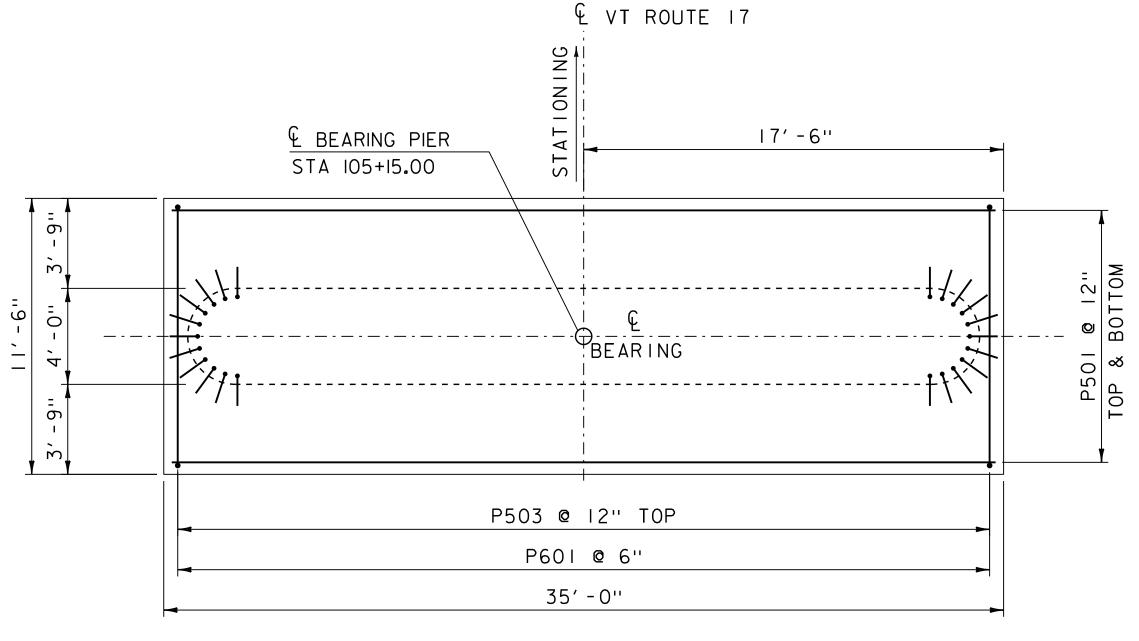
PROJECT NAME: WEYBRIDGE-NEW HAVEN PROJECT NUMBER: BF 032-1(19)

FILE NAME: cl2bE52cub dag

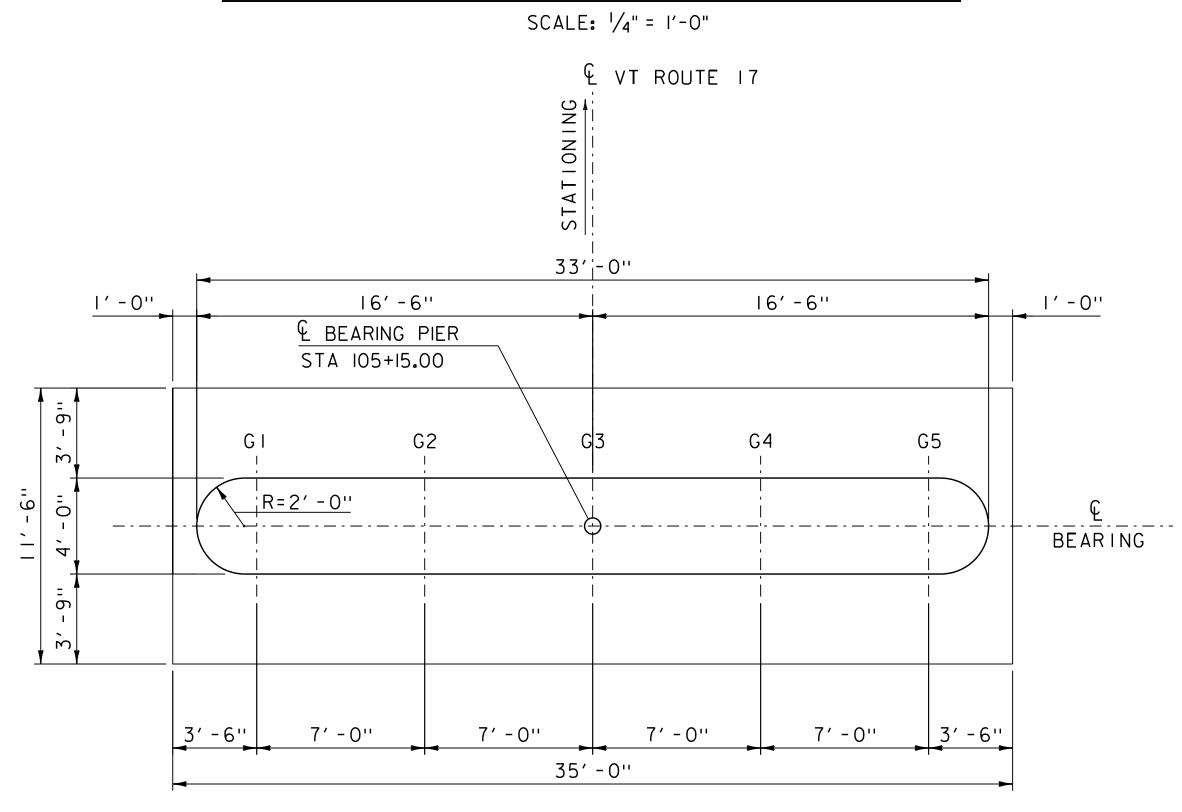
FILE NAME: si2b552sub.dgn PLOT DATE: 20-APR-2017
PROJECT LEADER: C.W. CARLSON DRAWN BY: M. LONGSTREET
DESIGNED BY: D. PETERSON CHECKED BY: D. PETERSON
WINGWALL DETAILS 2 SHEET 51 OF 85

WINGWALL 3 - ELEVATION

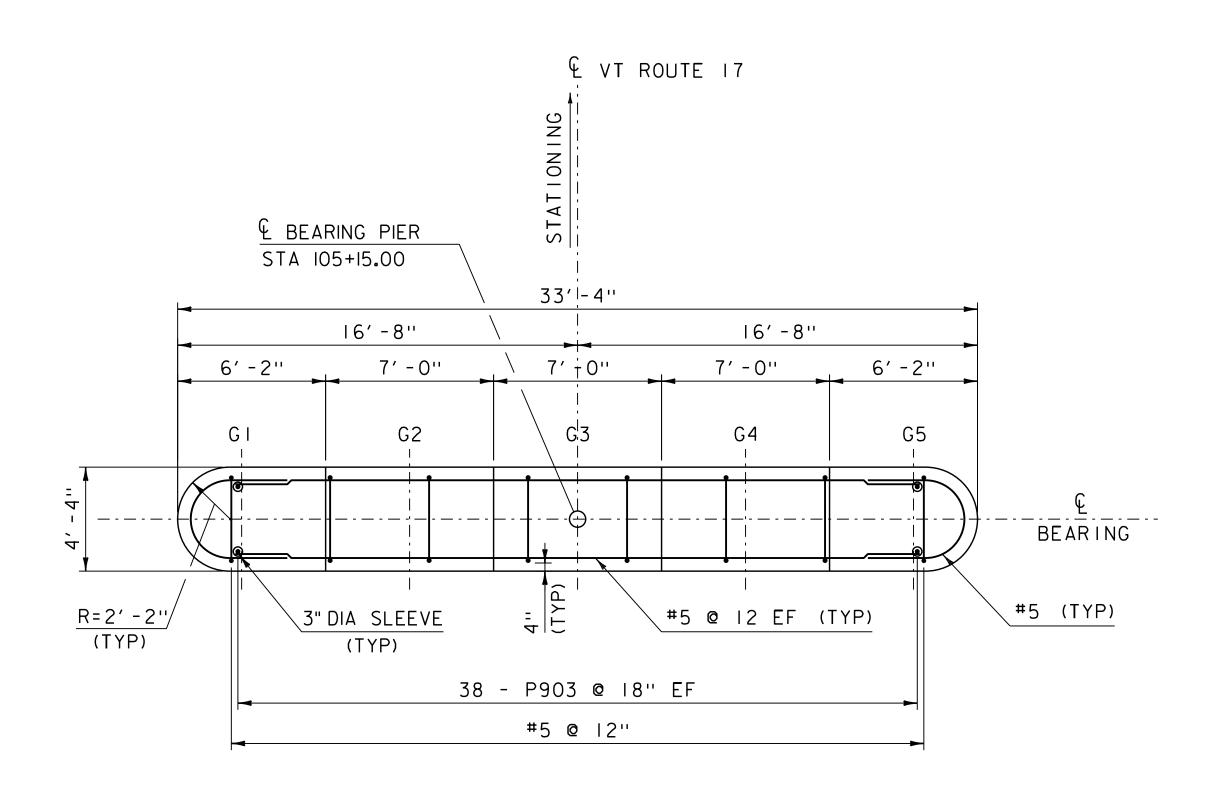
SCALE:  $\frac{1}{2}$ " = 1'-0"



### PIER FOUNDATION REINFORCING PLAN







## PRECAST PIER CAP PLAN

SCALE:  $\frac{1}{4}$ " = 1'-0"

#### NOTE:

NF = NEAR FACE
FF = FAR FACE
EF = EACH FACE

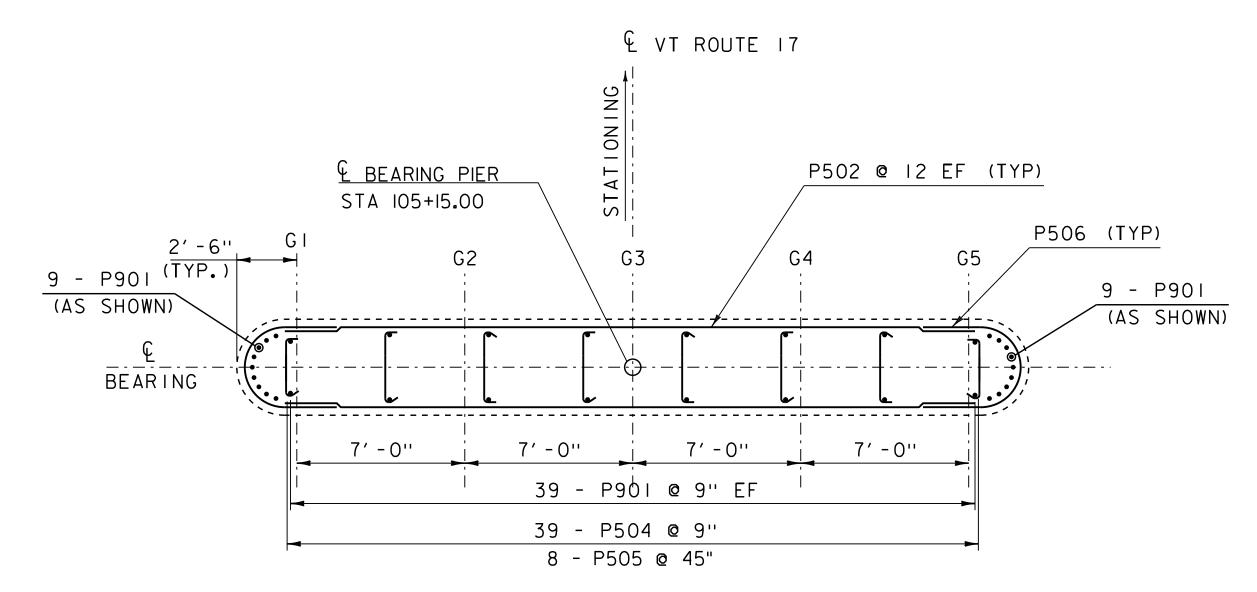
A = CUT TO FIT IN FIELD
4" CLEAR, UNLESS OTHERWISE
SPECIFIED ON THE PLANS.

2'-2" BAR LAP UNLESS OTHERWISE SPECIFIED ON THE PLANS.

PROJECT NAME: WEYBRIDGE-NEW HAVEN PROJECT NUMBER: BF 032-1(19)

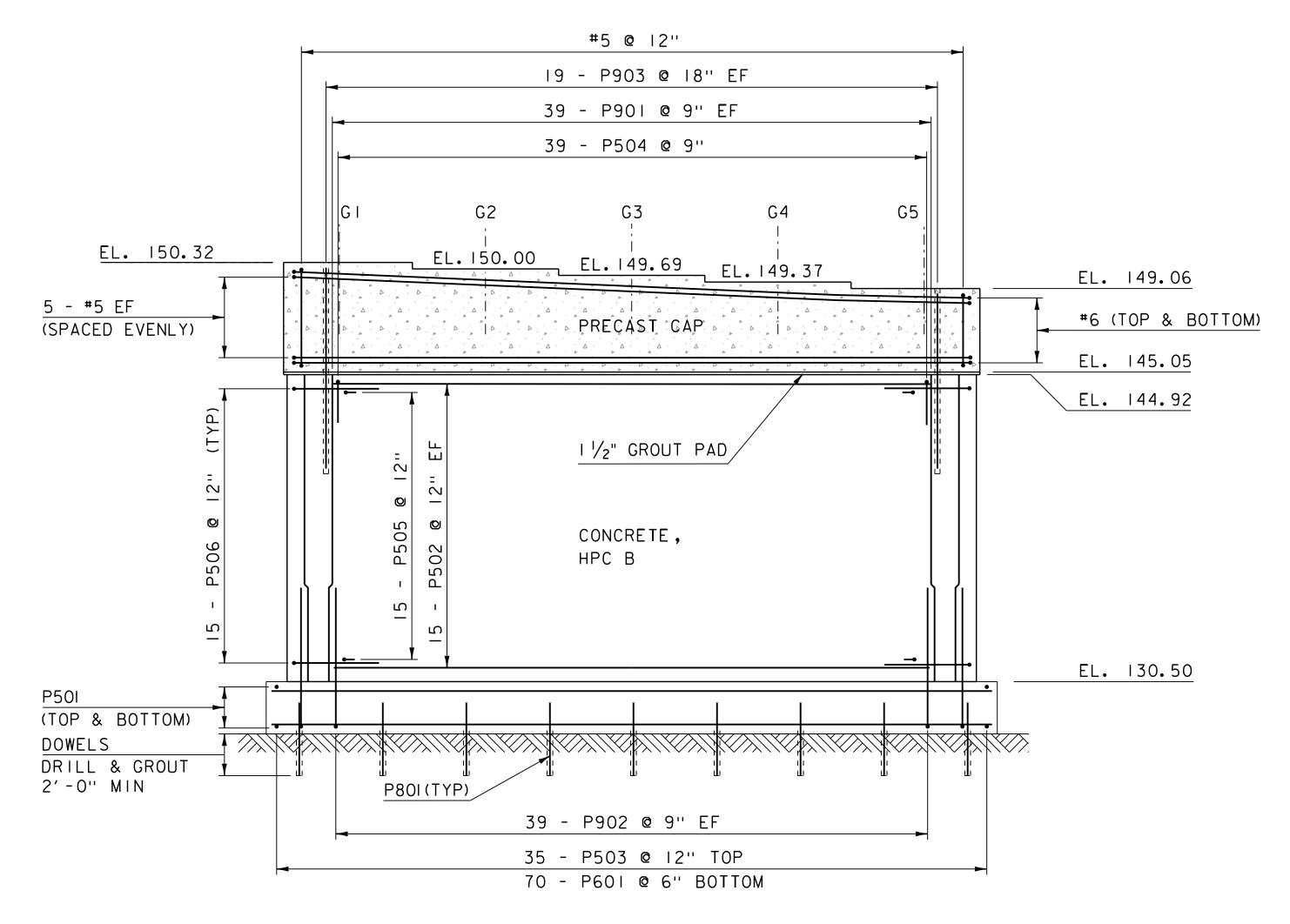
FILE NAME: sl2b552sub.dgn
PROJECT LEADER: C.W. CARLSON
DESIGNED BY: D. PETERSON
PIER DETAILS I

PLOT DATE: 20-APR-2017
DRAWN BY: M. LONGSTREET
CHECKED BY: D. PETERSON
SHEET 52 OF 85



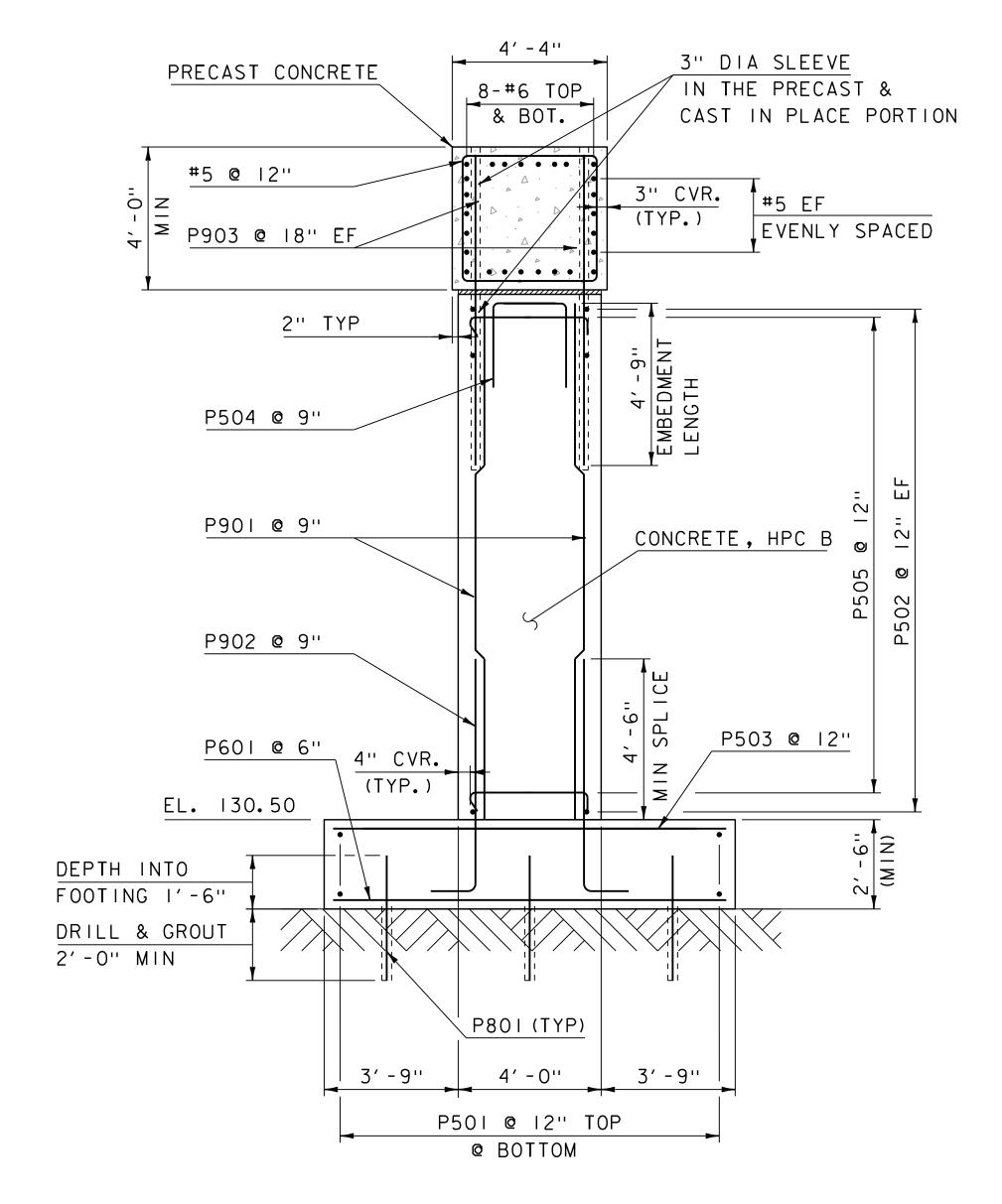
### PIER STEM REINFORCING PLAN

SCALE:  $\frac{1}{4}$ " = 1'-0"



## PIER ELEVATION

SCALE:  $\frac{1}{4}$ " = 1'-0"



### PIER SECTION (TYP)

SCALE:  $\frac{3}{8}$ " = 1'-0"

#### NOTE:

NF = NEAR FACE
FF = FAR FACE
EF = EACH FACE

▲ = CUT TO FIT IN FIELD
4" CLEAR, UNLESS OTHERWISE
SPECIFIED ON THE PLANS.
2'-2" BAR LAP UNLESS OTHERWISE

SPECIFIED ON THE PLANS.

PROJECT NAME: WEYBRIDGE-NEW HAVEN PROJECT NUMBER: BF 032-1(19)

FILE NAME: sl2b552sub.dgn
PROJECT LEADER: C.W. CARLSON
DESIGNED BY: D. PETERSON
PIER DETAILS 2

PLOT DATE: 20-APR-2017
DRAWN BY: M. LONGSTREET
CHECKED BY: D. PETERSON
SHEET 53 OF 85

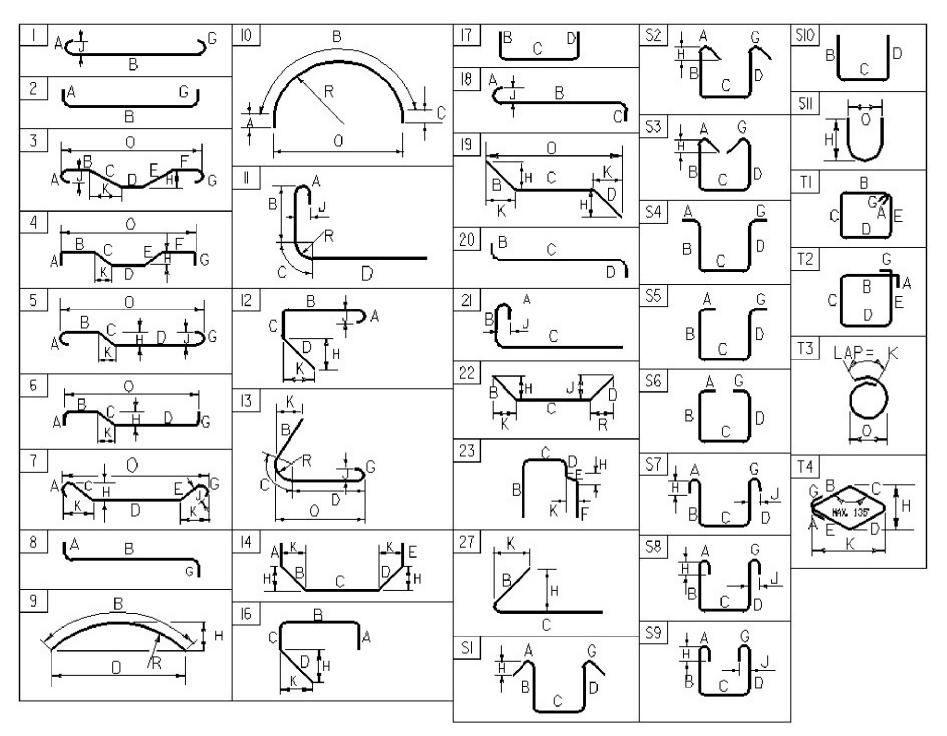
## STATE OF VERMONT AGENCY OF TRANSPORTATION

## REINFORCING STEEL SCHEDULE

AGEN	NCY	OF T	<b>RANS</b>	PORTA'	TION							K		N	L	ノト	くし	IN	G	<b>5</b>				<b>3</b> 6	П	
TEM EACH	SIZE	LENGTH	MARK	TYPE A	В	С	D E	E F	G	H J	К		T		EACH SIZE	1		TYPE A	В		D	E F	G	Н Ј	К	R O
DEC	K																									
	5 5		S501G S502G																							
△ 1954		4'- 11"	S501.3	STR																						
					2'- 2"	2'- 0"	2'- 2"			1'- 6" 1'-	6" 1'- 6	1'- 6"														
	6 3	28'- 1" 32'- 3"	S601G S602G																							
* 71	6	36'- 0"	S601.3	17	32'- 0"	4'- 0"																				
ABL	JTME	NT #1																								
* 17	5	38'- 1"	1A501.3	STR																						
			1A502.3 1A503.3				3'- 9" 5'- 1"																			
△ 33	8	4'- 0"	1A801.3	22	2'- 0"	2'- 0"				1'- 5"	. 1'- 5															
ABL	JTME	NT #2																								
			2A501.3	STR																						
			2A502.3 2A503.3				3'- 9" 5'- 1"																			
			2A801.3			2'- 0"				1'- 5"	. 1'- 5	3"														
DIE																										
PIEI		34'- 5"	P501	STR																						
30		29'- 0"	P502 P503	STR STR																						
* 71	6	11'- 0"	P601	STR																						
△ 29	8	3'- 6"	P801	STR																						
	9		P901 P903	STR STR																						
39 120		7'- 7" 4'- 4"	P504 P505	S10 S3 0'- 6'	2'- 2"	3'- 3"	2'- 2"			0'- 4"																
△ 32		9'- 6"	P506	S11	3-4	0-0				3'- 10"			3'- 4"													
* 97	9	8'- 0"	P902	2 1'- 3'	6'- 9"																					
WIN	IGWA	LL #1																								
20 12	5	5'- 3" 12'- 6"	1W501.3 1W601.3	STR STR																						
10	5	5'- 5"	1W502.3	S10	2'- 2"	1'- 1"	2'- 2"																			
18	6	5- 2"	1W602.3	510	2-1	2'- 7"																				
	IGWA																									
			2W501.3 2W601.3																							
			2W502.3 2W602.3			1'- 1" 2'- 7"	2'- 2"																			
	IGWAI		3W501.3	STR																						
12	6	12'- 6"	3W601.3	STR																						
			3W502.3 3W602.3			1'- 0" 2'- 7"	2'- 2"																			
WIN	IGWAI	LL #4																								
20	5	5'- 1"	4W501.3																							
			4W601.3		01 0"	4! 0"	21 211																			
			4W502.3 4W602.3		2'- 2"	2'- 7"																				

#### ~ NOTES ~

- 1. UNLESS OTHERWISE DESIGNATED, ALL BAR REINFORCEMENT FOR CONCRETE IN SIZES UP TO AND INCLUDING NO. 18 SHALL CONFORM TO THE REQUIREMENTS OF THE "SPECIFICATIONS FOR DEFORMED BILLET-STEEL BARS FOR CONCRETE REINFORCEMENT", AASHTO M 31 (ASTM A 615-SI). ALL BARS SHALL BE GRADE 60, UNLESS OTHERWISE DESIGNATED.
- 2. FOR TYPICAL BENDING DETAILS, RECOMMENDED PIN DIAMETER "D" OF BENDS AND HOOKS, AND OTHER STANDARD PRACTICE, SEE CURRENT CONCRETE REINFORCING STEEL INSTITUTE "MANUAL OF STANDARD PRACTICE".
- 3. BARS WHICH REQUIRE MORE ACCURATE BENDING THAN STANDARD PRACTICES SHOULD HAVE LIMITS INDICATED.
- 4. ALL DIMENSIONS ARE OUT TO OUT OF BAR EXCEPT "A" AND "G" ON STANDARD 180 DEGREE AND 135 DEGREE HOOKS.
- 5. "J" DIMENSION ON 180 DEGREE HOOKS TO BE SHOWN ONLY WHERE NECESSARY TO RESTRICT HOOK SIZE. OTHERWISE, STANDARD HOOKS ARE TO BE USED.
- 6. "H" DIMENSION ON STIRRUPS TO BE SHOWN ONLY WHEN NECESSARY TO MAINTAIN CLEARANCES.
- 7. WHERE SLOPE DIFFERS FROM 45 DEGREES, DIMENSIONS "H" AND "K" MUST BE SHOWN.
- 8. A DENOTES BARS TO BE CUT IN FIELD.
- 9. * DENOTES ONE EXTRA BAR ADDED FOR TESTING PURPOSES.
- 10.  $\triangle$  DENOTES TWO EXTRA BARS ADDED FOR TESTING PURPOSES.
- 11. E IN BAR MARK PREFIX DENOTES EPOXY COATED REINFORCING STEEL.



### ASTM STANDARD

REINFORCING BARS									
B A = 817 F D C = T N A -	W FIR H T P D U N D C P E - F L L	DAMETER ACHE	A D C 4	TO SELTION  TO NIM OTO N  IN C - FS					
#3	0.376	0.375	0.11	1.178					
[#] 4	0.668	0.500	0.20	1.571					
[#] 5	1.043	0.625	0.31	1.963					
[#] 6	1.502	0.750	0.44	2.356					
<b>#</b> 7	2.04	0.875	0.60	2.749					
[#] 8	2.670	1.000	0.79	3.14					
[#] 9	3.400	1.13	1.00	3.54					
[#] 10	4.3	1.270	1.27	3.990					
[#] 11	5.31	1.410	1.56	4.430					
[#] 14	7.65	1.69	2.25	5.32					
[#] 18	13.60	2.26	4.00	7.09					

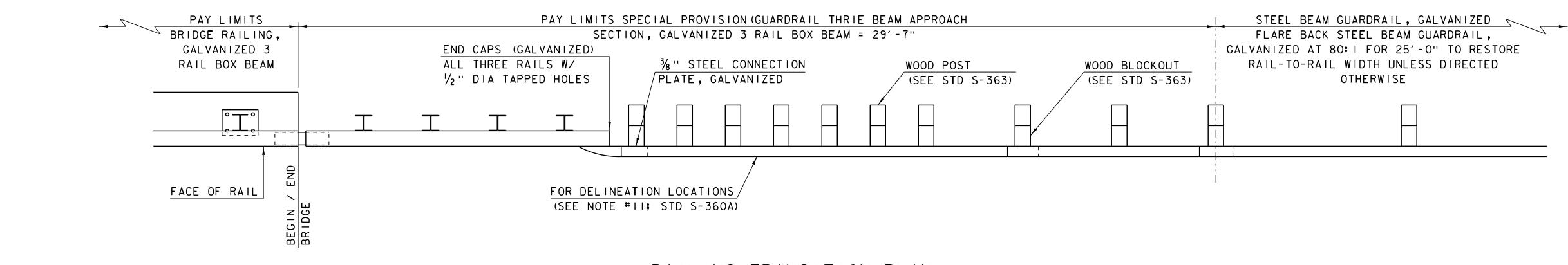
#### ~ REINFORCING STEEL CORROSION RESISTANCE LEVEL ~

THE REINFORCING STEEL MARKS IN THIS SCHEDULE INDICATE THE REQUIRED BAR CORROSION RESISTANCE LEVEL. CORROSION RESISTANCE LEVEL IS DENOTED WITH A .2 FOR LEVEL TWO SUFFIX OR .3 FOR LEVEL THREE SUFFIX, .1 FOR LEVEL ONE IS TO BE OMITTED. THE BAR MATERIAL TYPE AND BAR STEEL GRADE PROVIDED FOR EACH CORROSION LEVEL WILL BE RECORDED ON THE PLAN SET PI SHEET FOR AS-BUILT RECORD PLAN ARCHIVES.

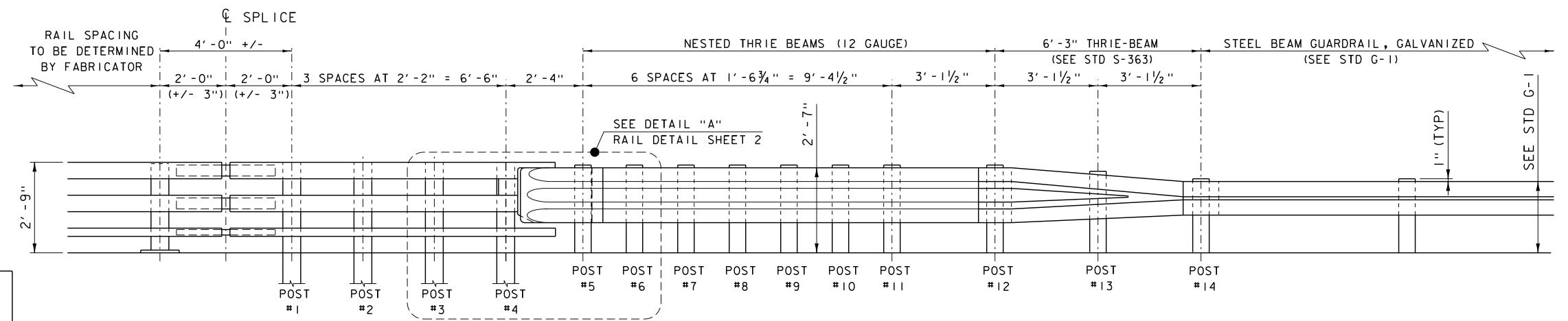
PROJECT NAME: WEYBRIDGE - NEW HAVEN
PROJECT NUMBER: BF 032-1(19)

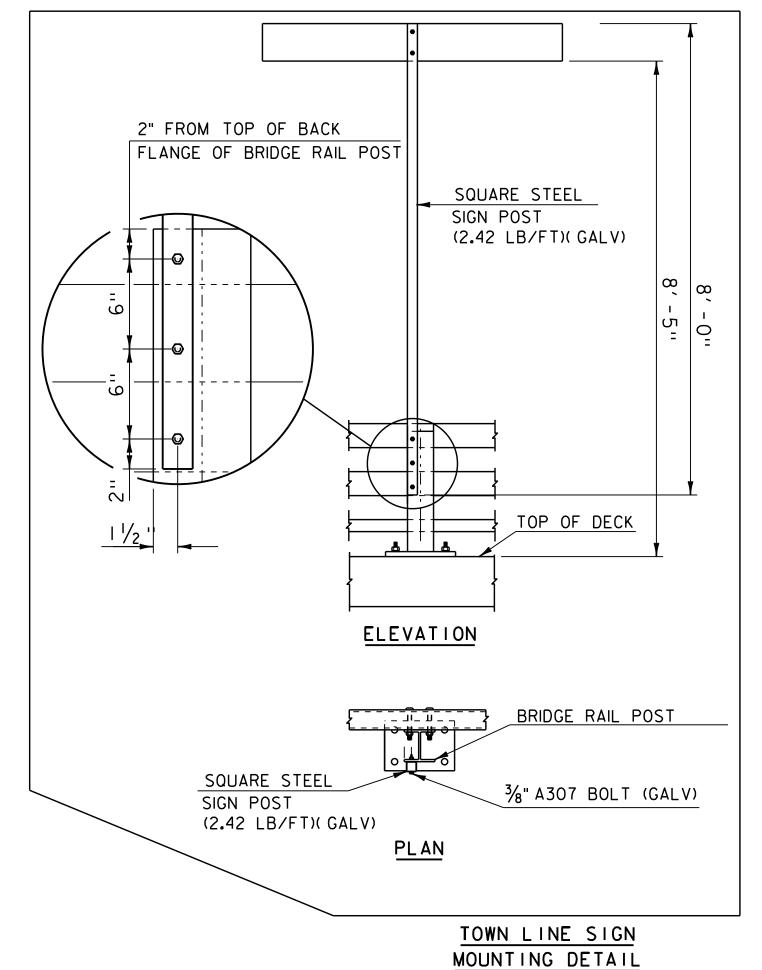
FILE NAME: 12B552rss.dgn
PROJECT MANAGER: C. CARLSON
DESIGNED BY: D. PETERSON
REINFORCING STEEL SCHEDULE SHEET #1

PLOT DATE: 2/26/2013
DRAWN BY: D. KARABEGOVIC
CHECKED BY: D. PETERSON
SHEET 54 OF 85



#### RAILING TRANSITION PLAN





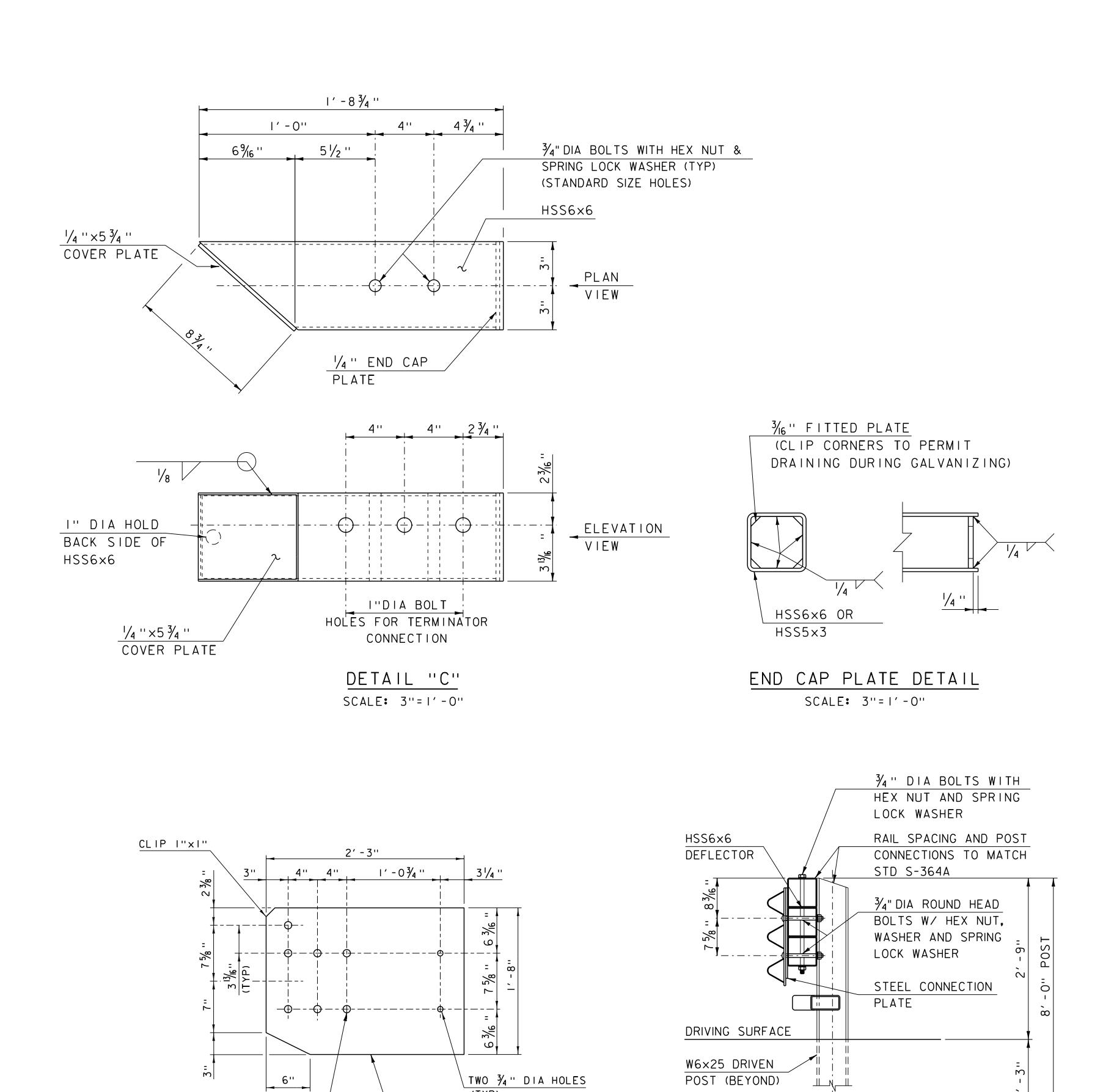
#### RAILING TRANSITION ELEVATION

#### NOTES:

- I. ALL APPROACH RAIL SPLICES SHALL BE LAPPED IN THE DIRECTION OF TRAFFIC FLOW.
- 2. TUBE AND STEEL POST MATERIALS, DIMENSION SIZES AND NOTES SHALL BE THE SAME AS THOSE OF THE BRIDGE RAIL, UNLESS OTHERWISE NOTED.
- 3. APPROACH RAIL BOLTS SHALL BE ASTM A307 GRADE A AND NUTS SHALL BE AASHTO M291 (ASTM A563 GRADE A OR BETTER) (GALVANIZED). WASHERS SHALL BE ASTM F844.
- 4. PRIOR TO GALVANIZING, GRIND ALL EDGES TO A MINIMUM RADIUS OF 1/16".

PROJECT NAME: WEYBRIDGE-NEW HAVEN
PROJECT NUMBER: BF 032-1(19)

FILE NAME: s12b552rail.dgn PLOT DATE: 20-APR-2017
PROJECT LEADER: C.W. CARLSON DRAWN BY: M. LONGSTREET
DESIGNED BY: D. PETERSON CHECKED BY: D. PETERSON
RAIL DETAIL SHEET 1 SHEET 55 OF 85



(TYP)

AASHTO M 270 GRADE 36

CONNECTION PLATE

SCALE: 1/2"=1'-0"

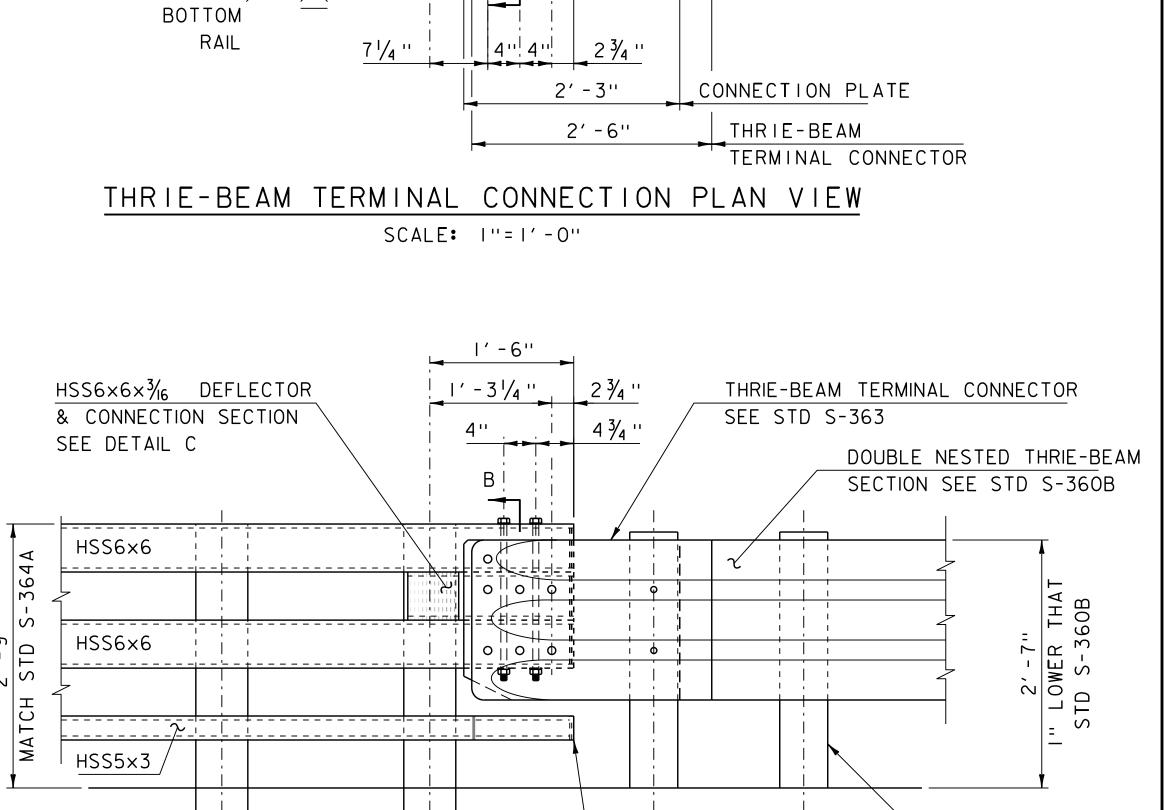
(GALV)  $(1'-8" \times 2'-3")$ 

I" DIA HOLES

%" STEEL CONNECTION PLATE

SECTION B-B

SCALE: | "= | '-0"



BOTTOM HSS5x3 BENT

POST #6

WAY FROM TRAFFIC

POST #5

POST #3

W6×25

DRIVEN

POST

POST

B-U2a

DETAIL A - TERMINAL CONNECTION ELEVATION VIEW

SCALE: | " = | ' - 0"

POST

#4

BROJECT NAME: WEVRRINGE_NEW

POST #5

PROJECT NAME: WEYBRIDGE-NEW HAVEN

PROJECT NUMBER: BF 032-1(19)

FILE NAME: s12b552rail.dgn PLOT DATE: 20-APR-2017

PROJECT LEADER: C.W. CARLSON DRAWN BY: M. LONGSTREET

DESIGNED BY: D. PETERSON CHECKED BY: D. PETERSON

RAIL DETAIL SHEET 2 SHEET 56 OF 85

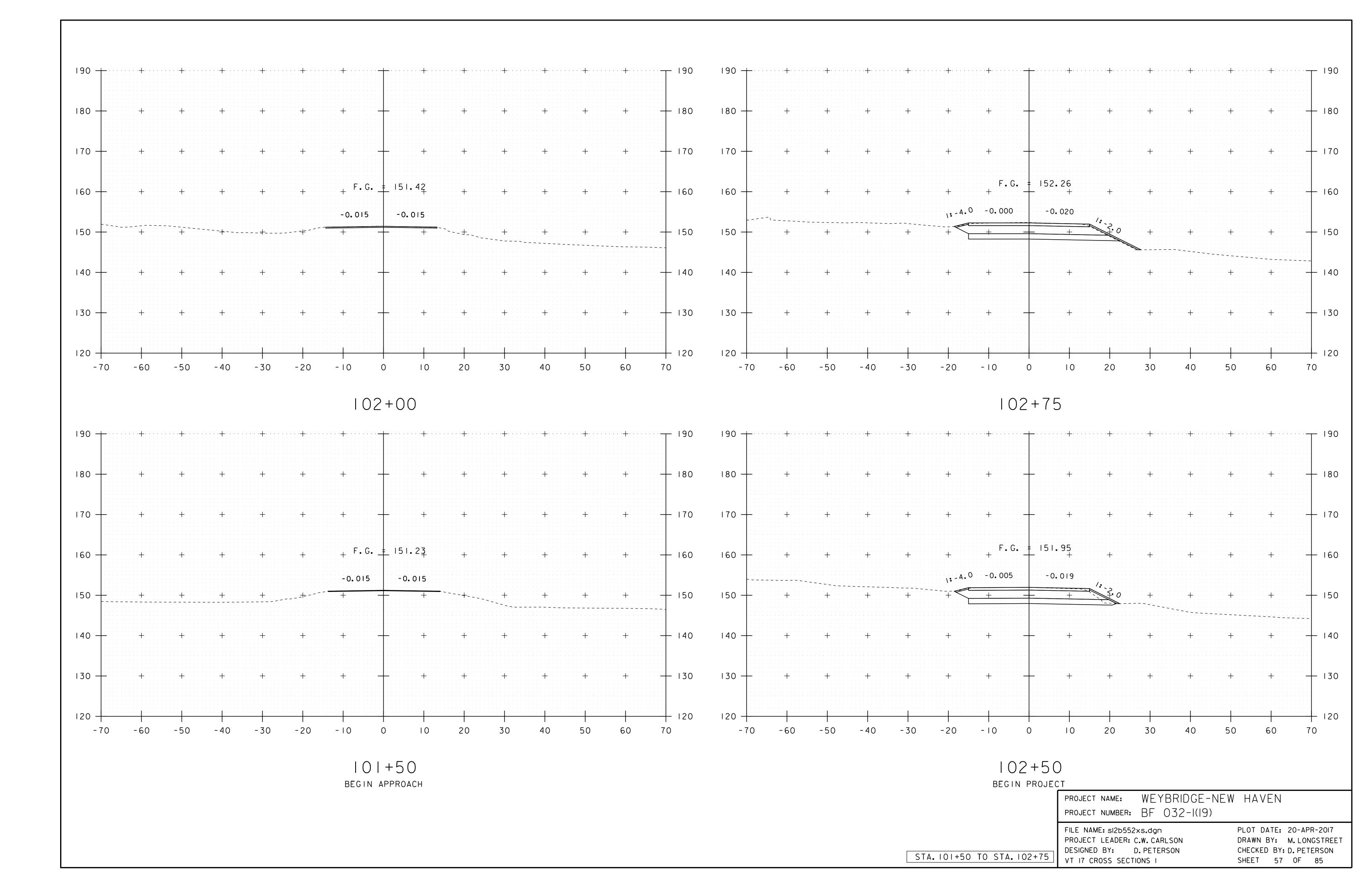
3/6" COVER PLATE EACH

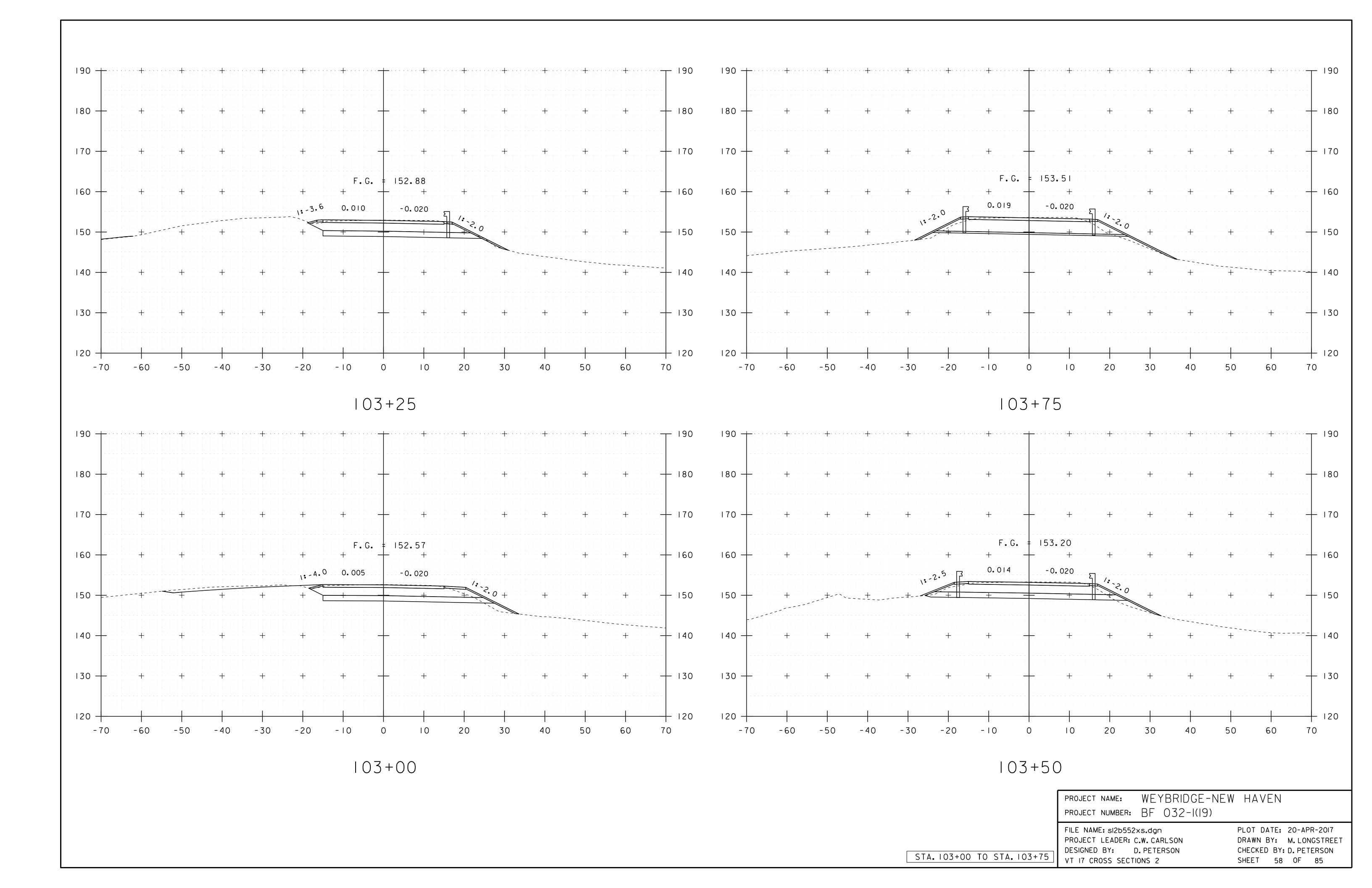
END OF TS RAILS (TYP)

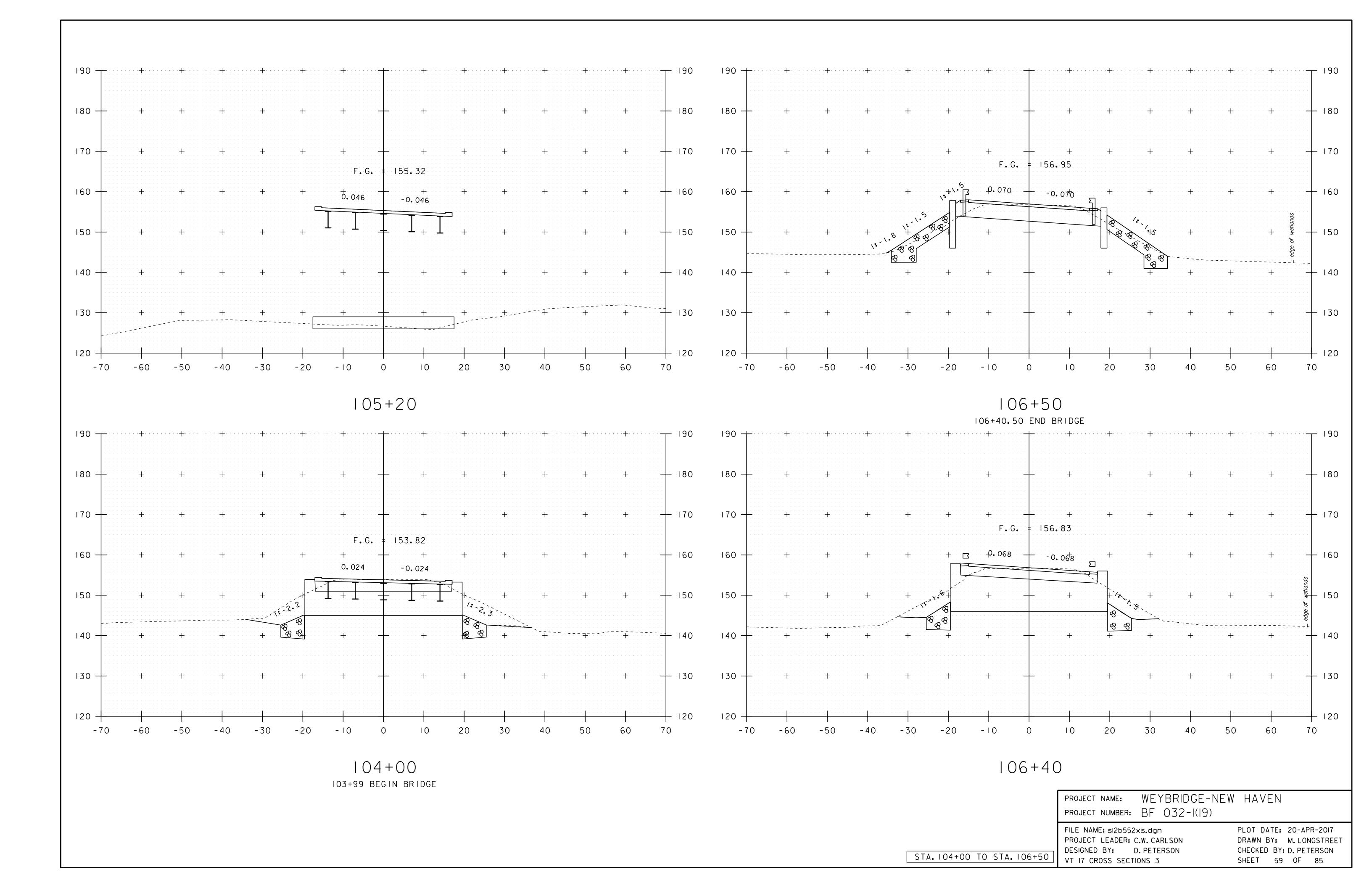
POST #6

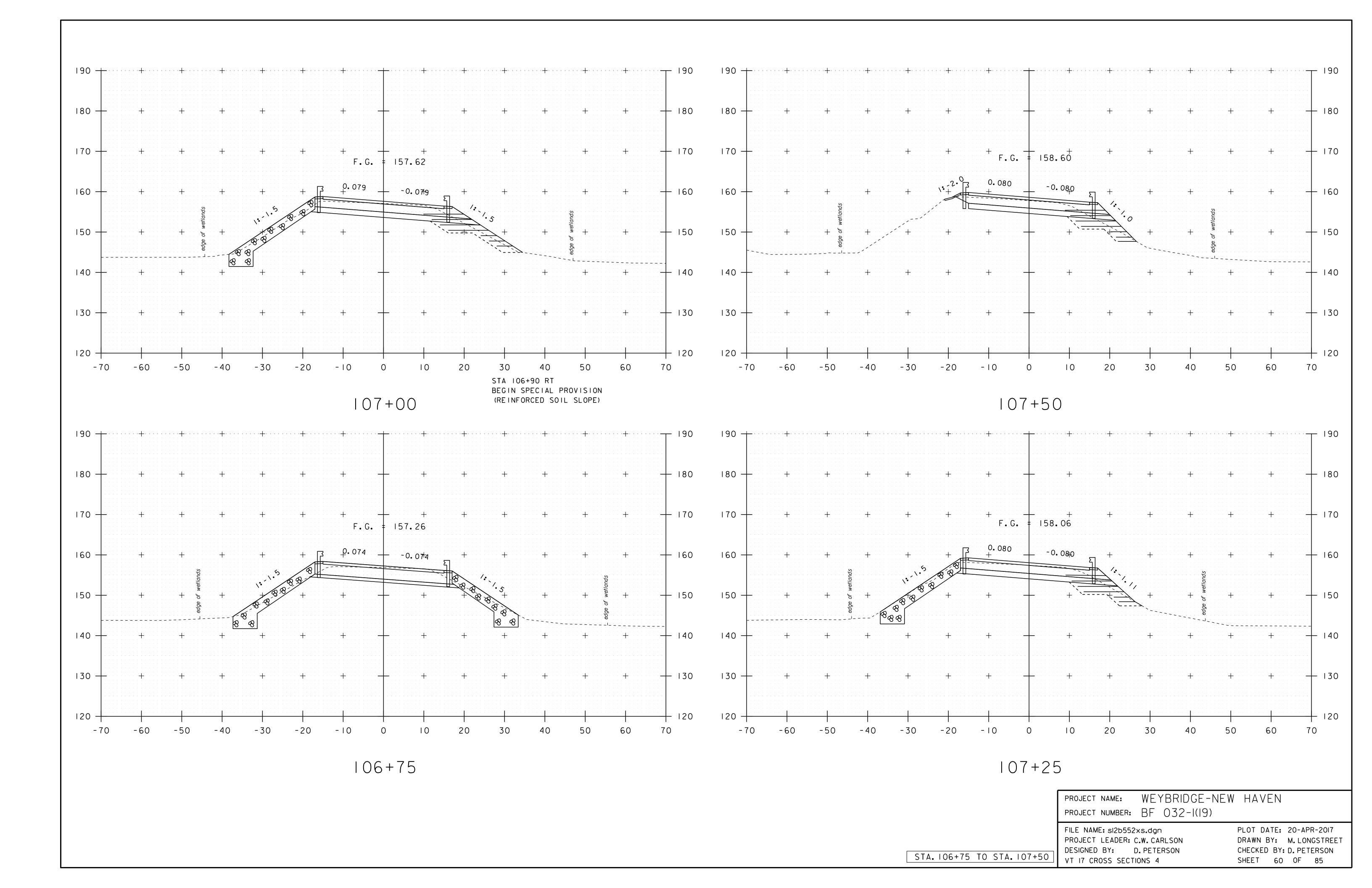
WOOD POST

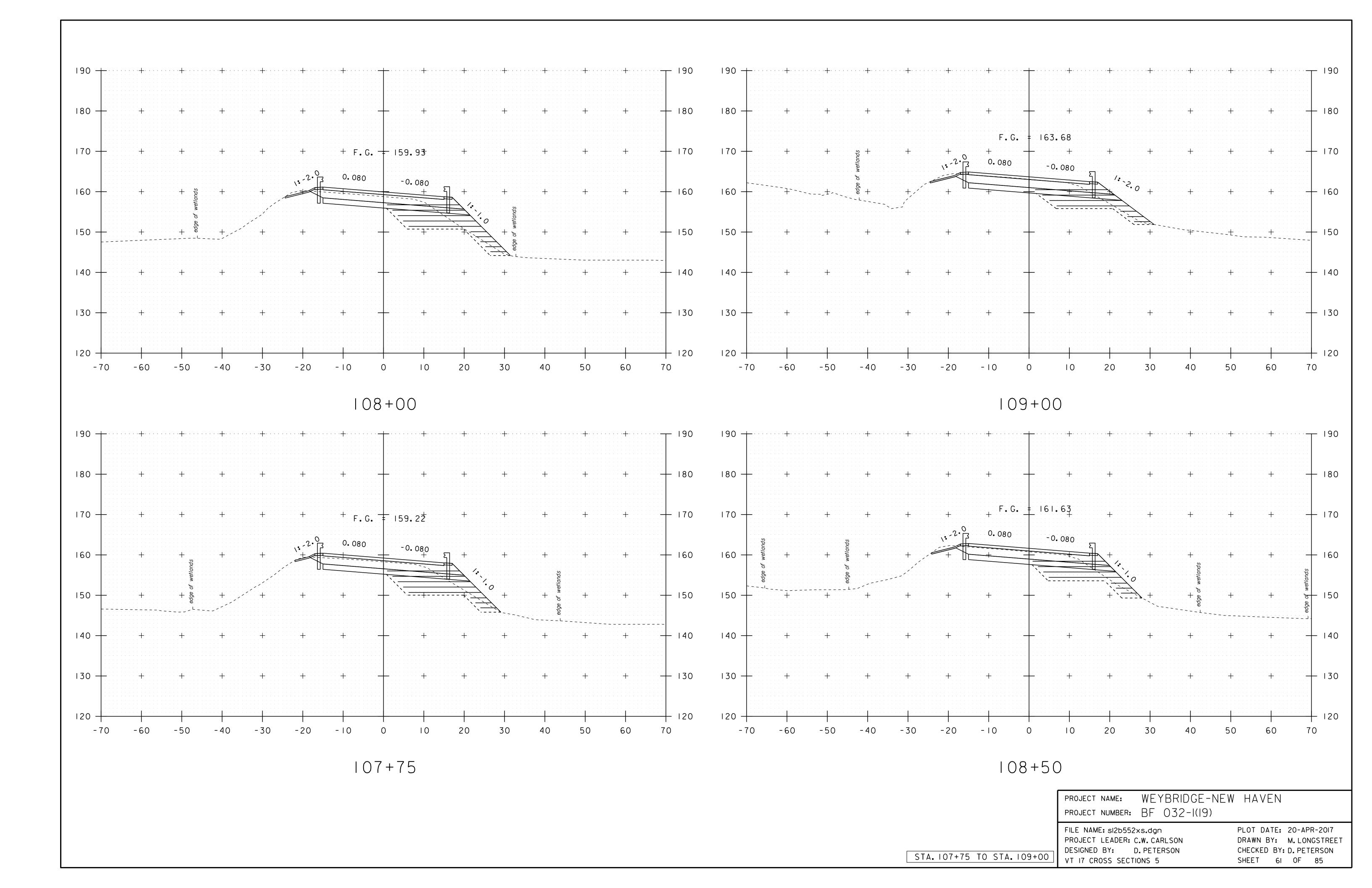
SEE G-I

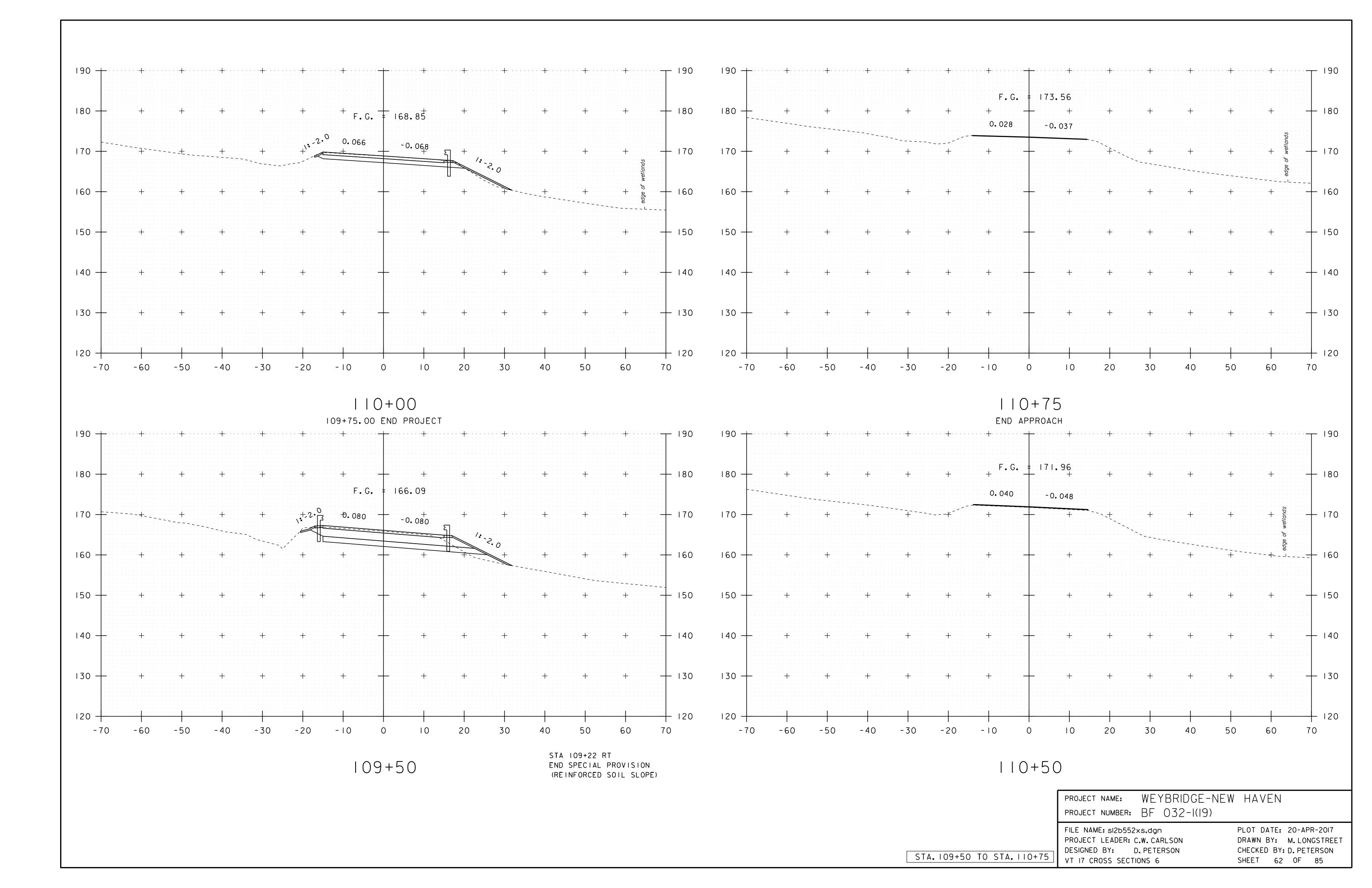


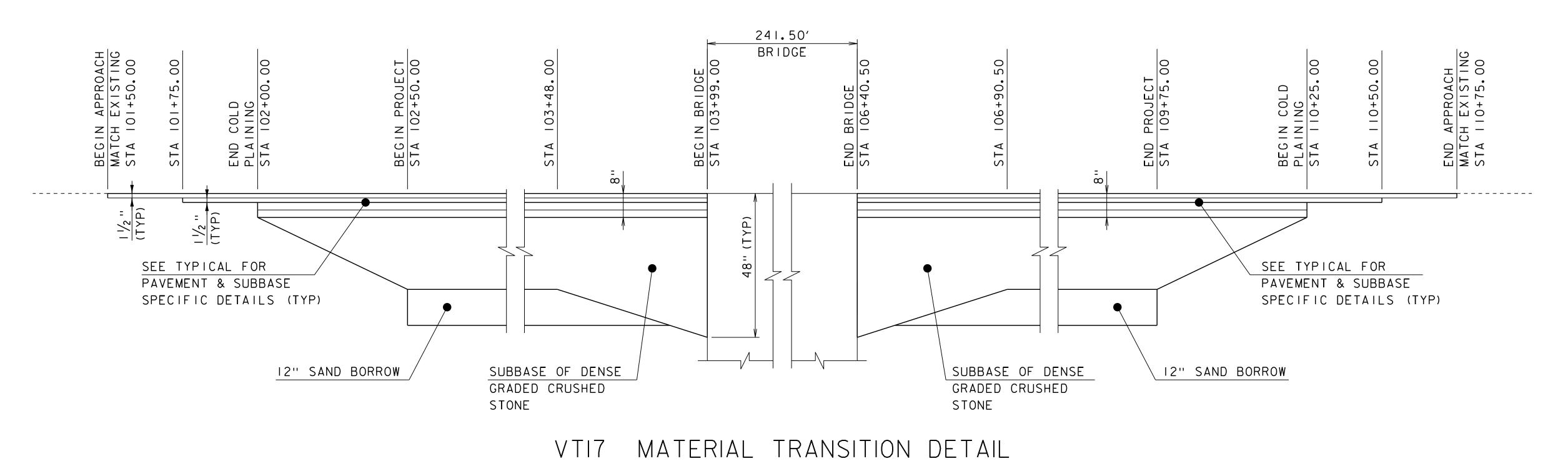












HORIZONTAL SCALE: I" = 20'-0"

NO VERTICAL SCALE

PROJECT NAME:

PROJECT NUMBER: BF 032-1(19)

FILE NAME: sl2b552pro.dgn

PROJECT LEADER: C.W. CARLSON DESIGNED BY: D. PETERSON

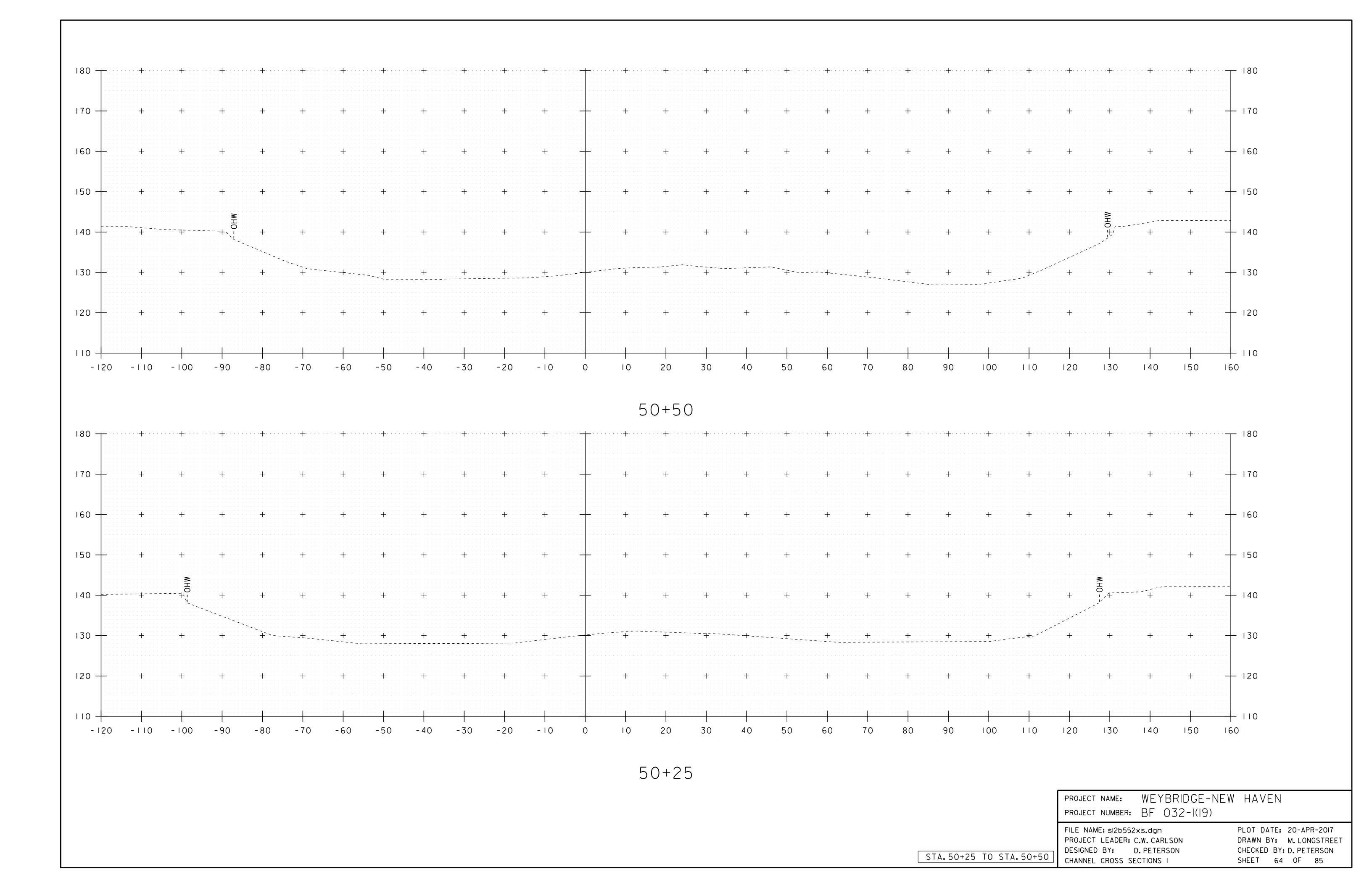
MATERIAL TRANSITION & DETAILS

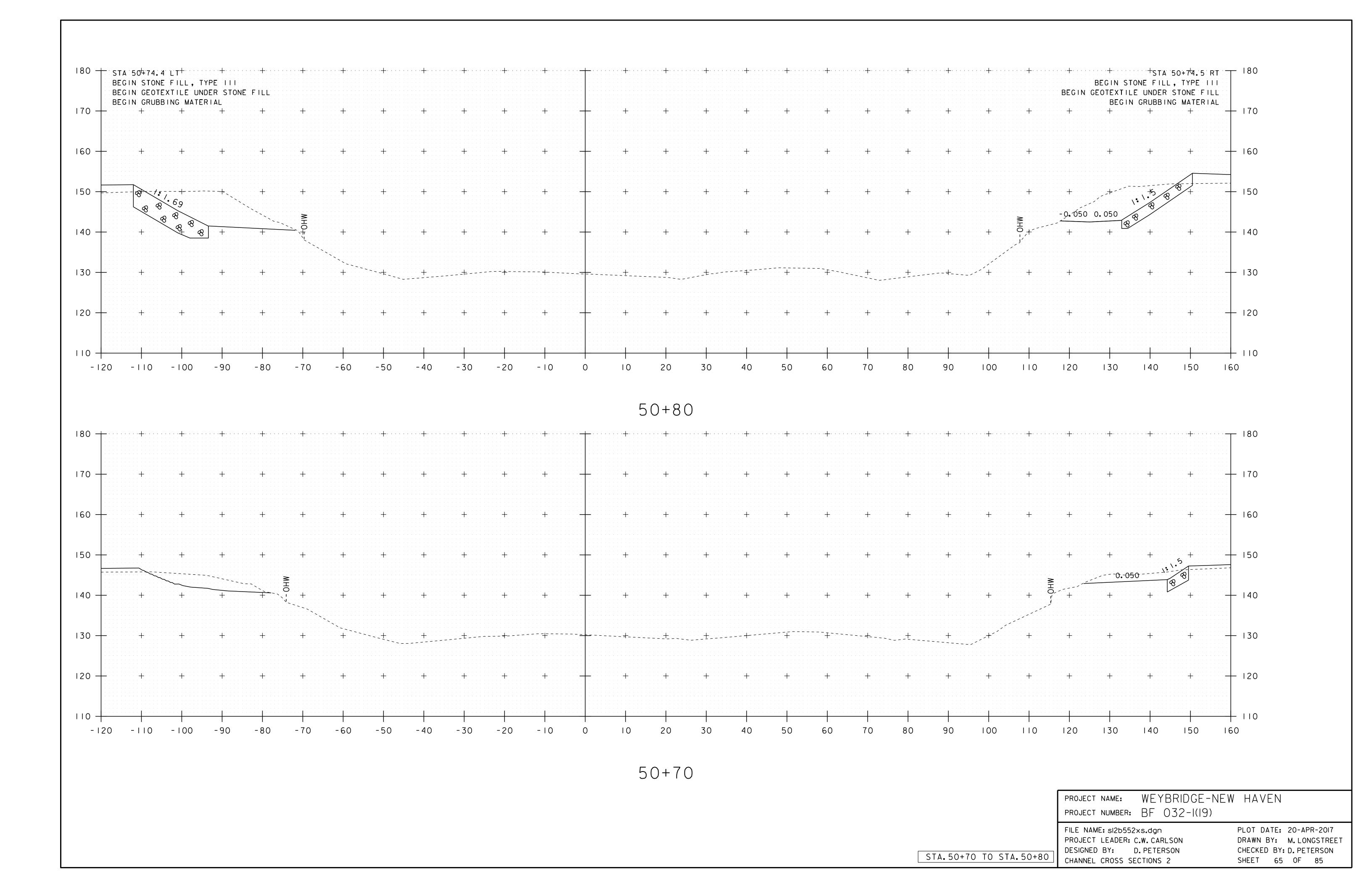
WEYBRIDGE-NEW HAVEN

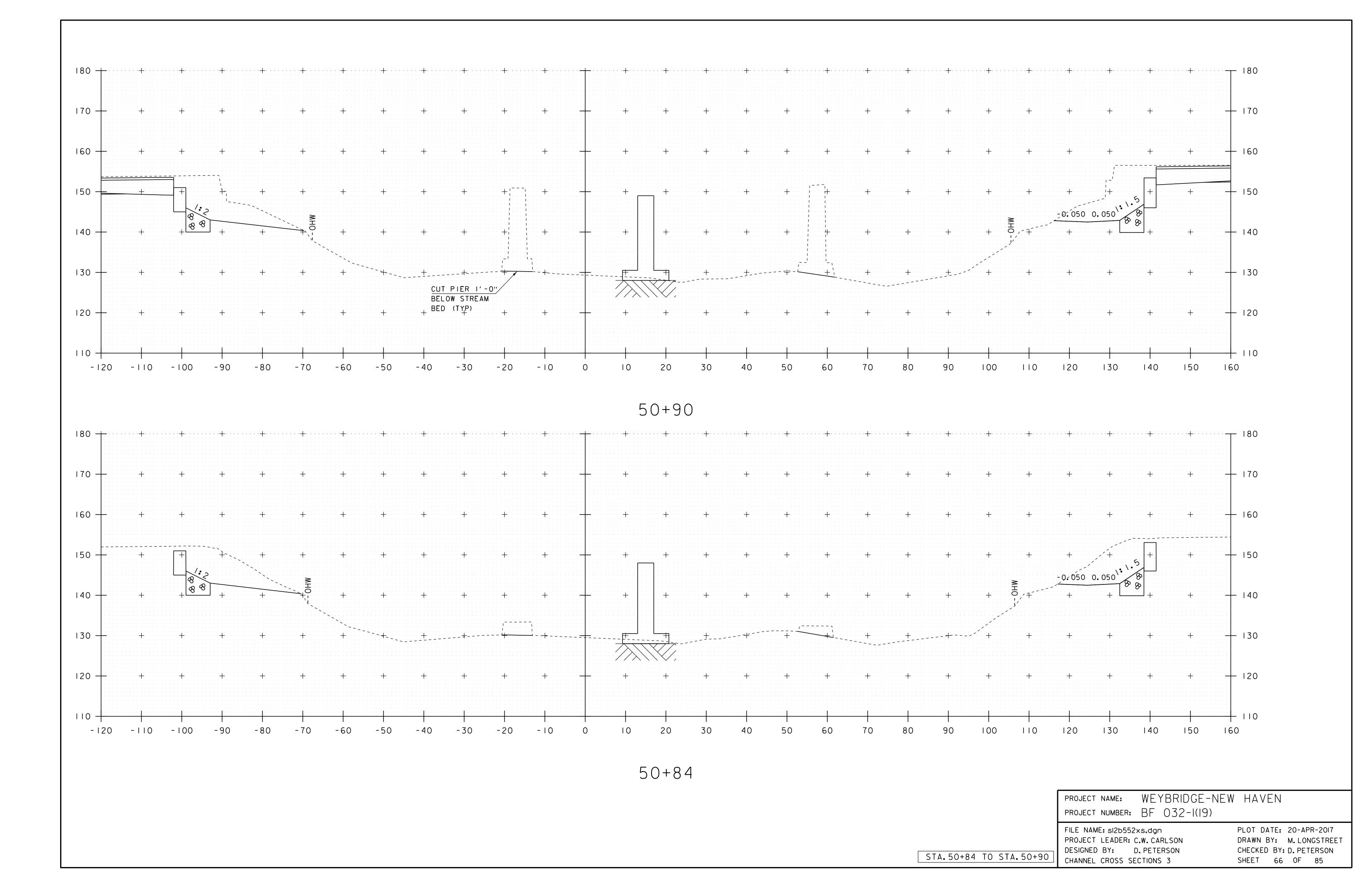
PLOT DATE: 20-APR-2017
DRAWN BY: M. LONGSTREET

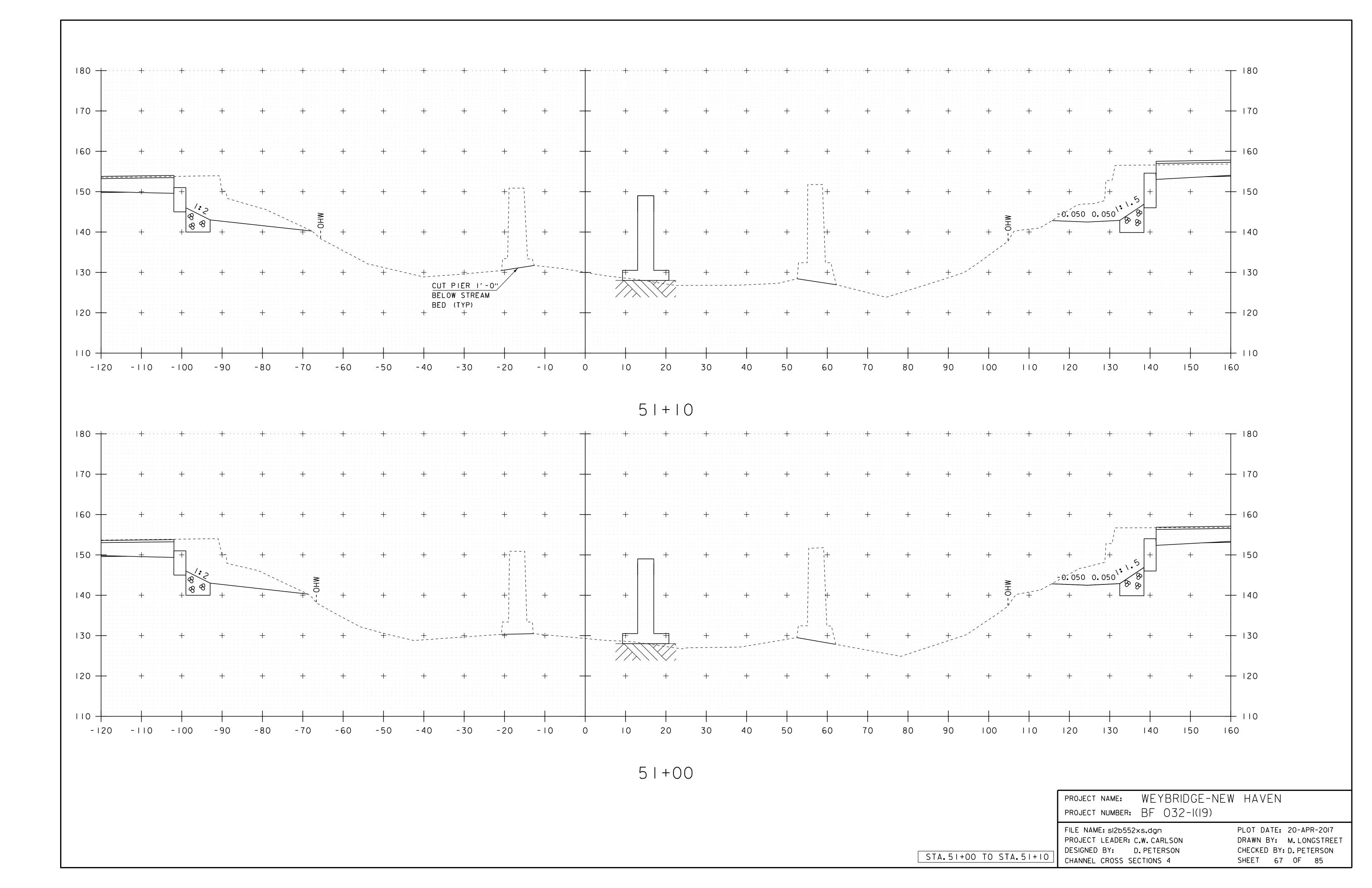
CHECKED BY: D. PETERSON

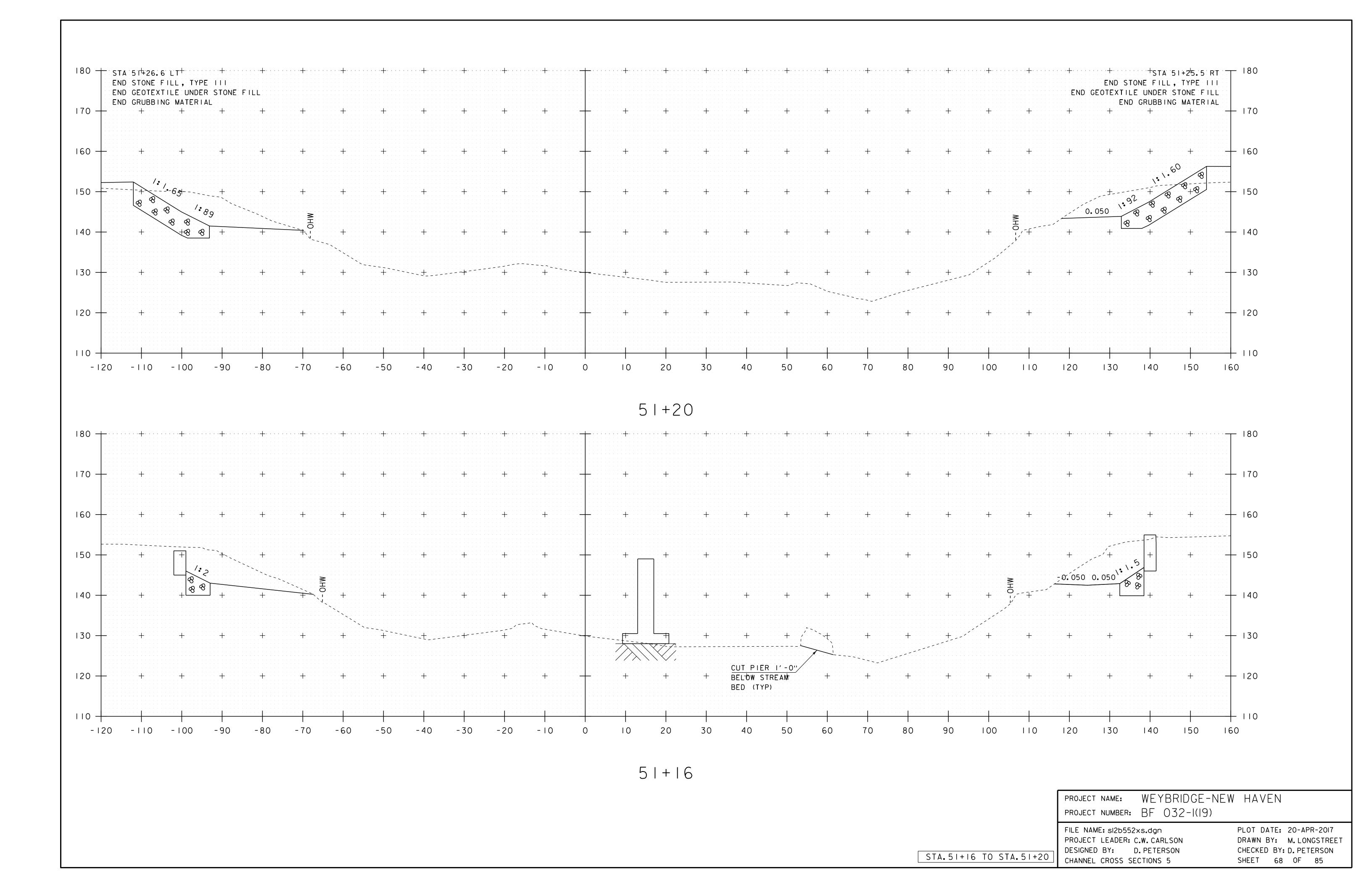
SHEET 63 OF 85

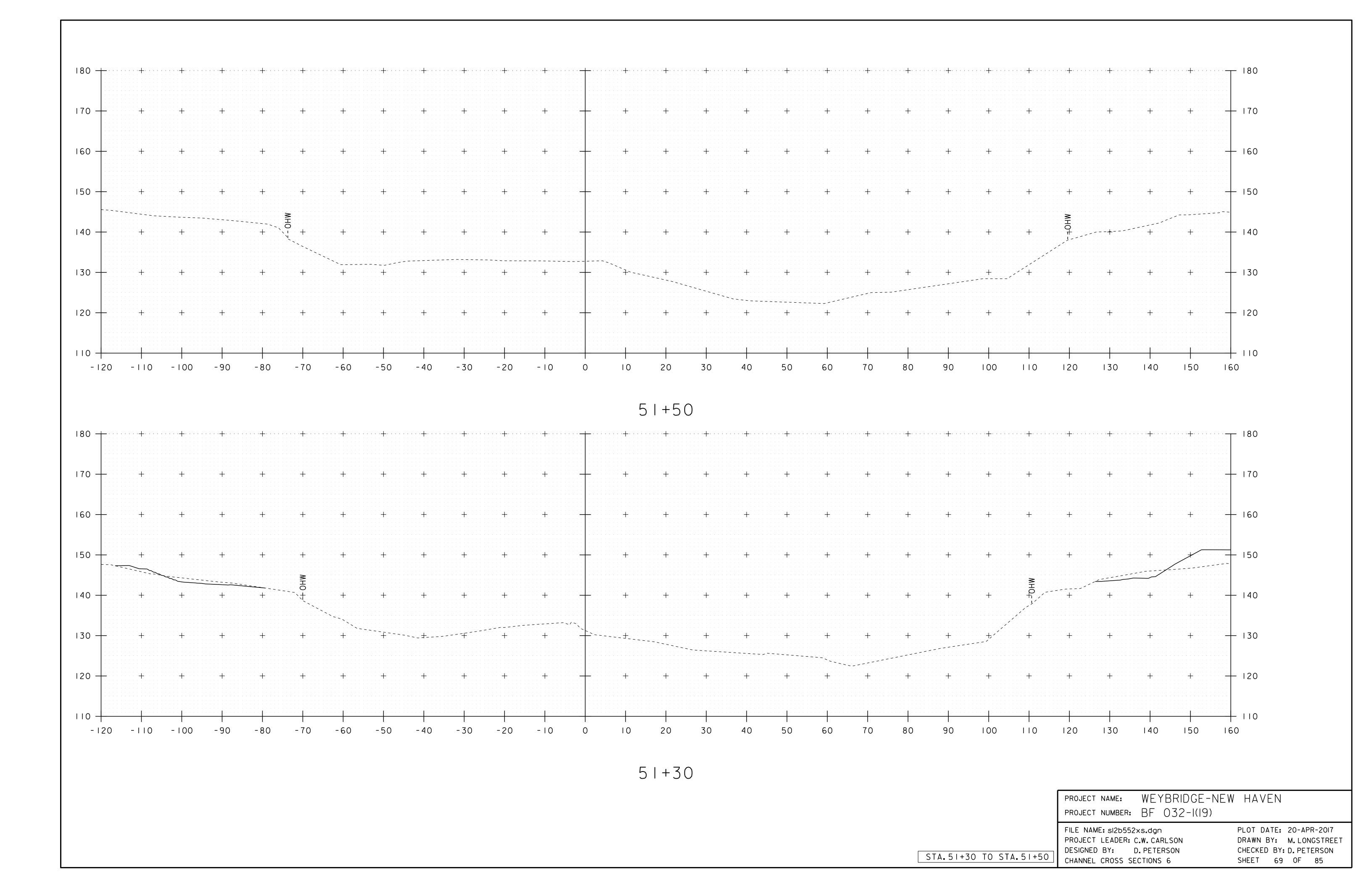


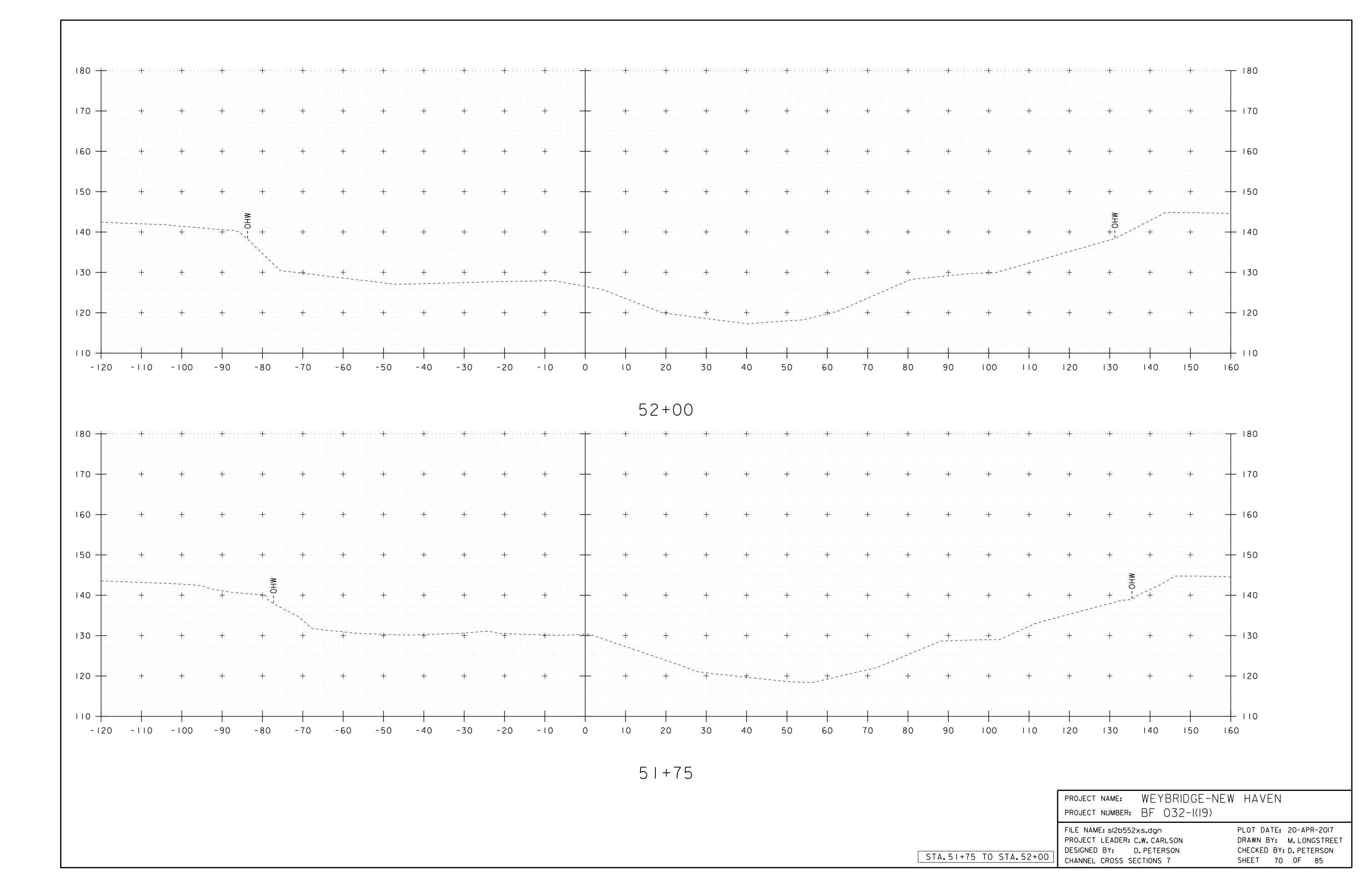


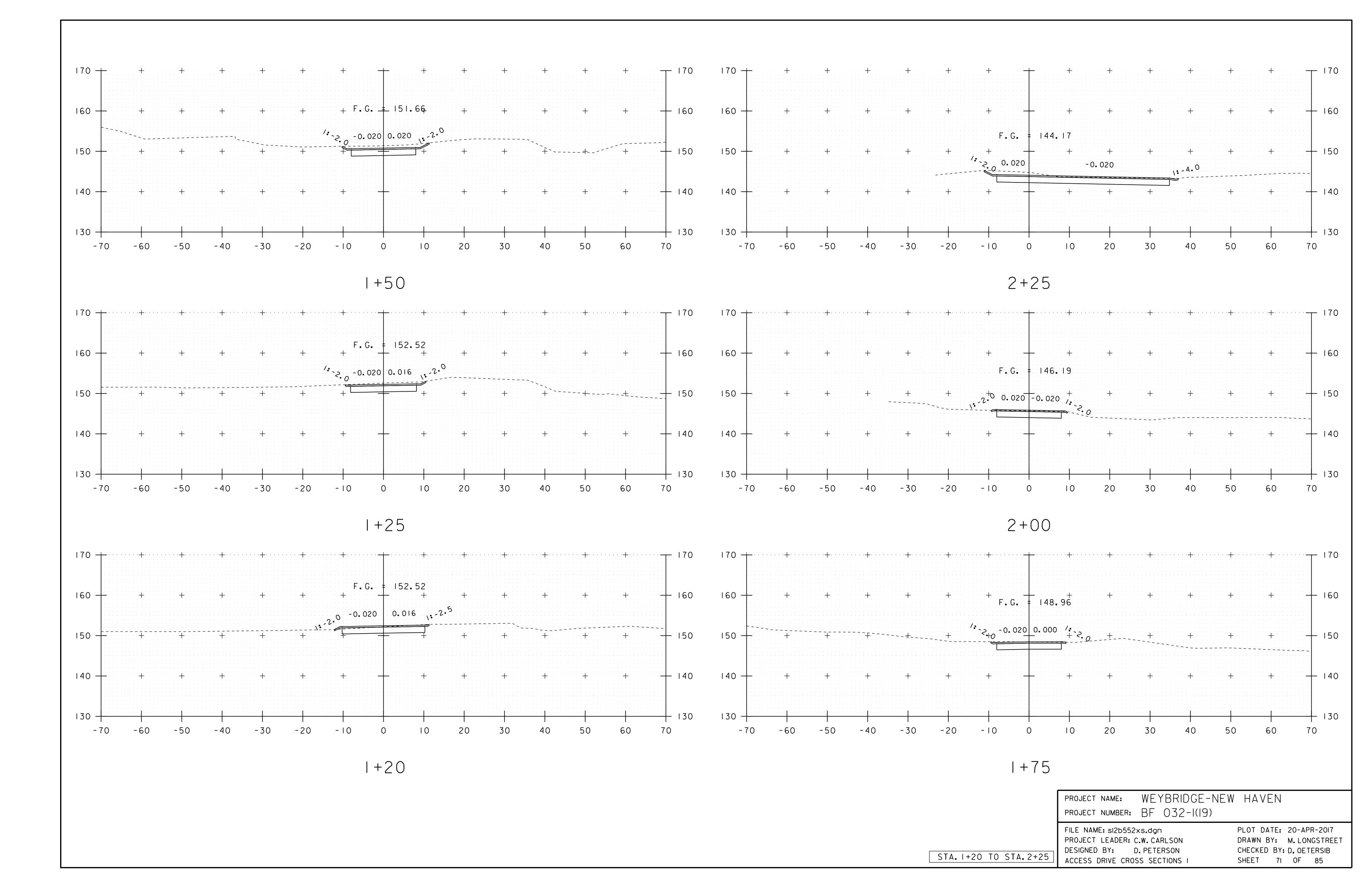


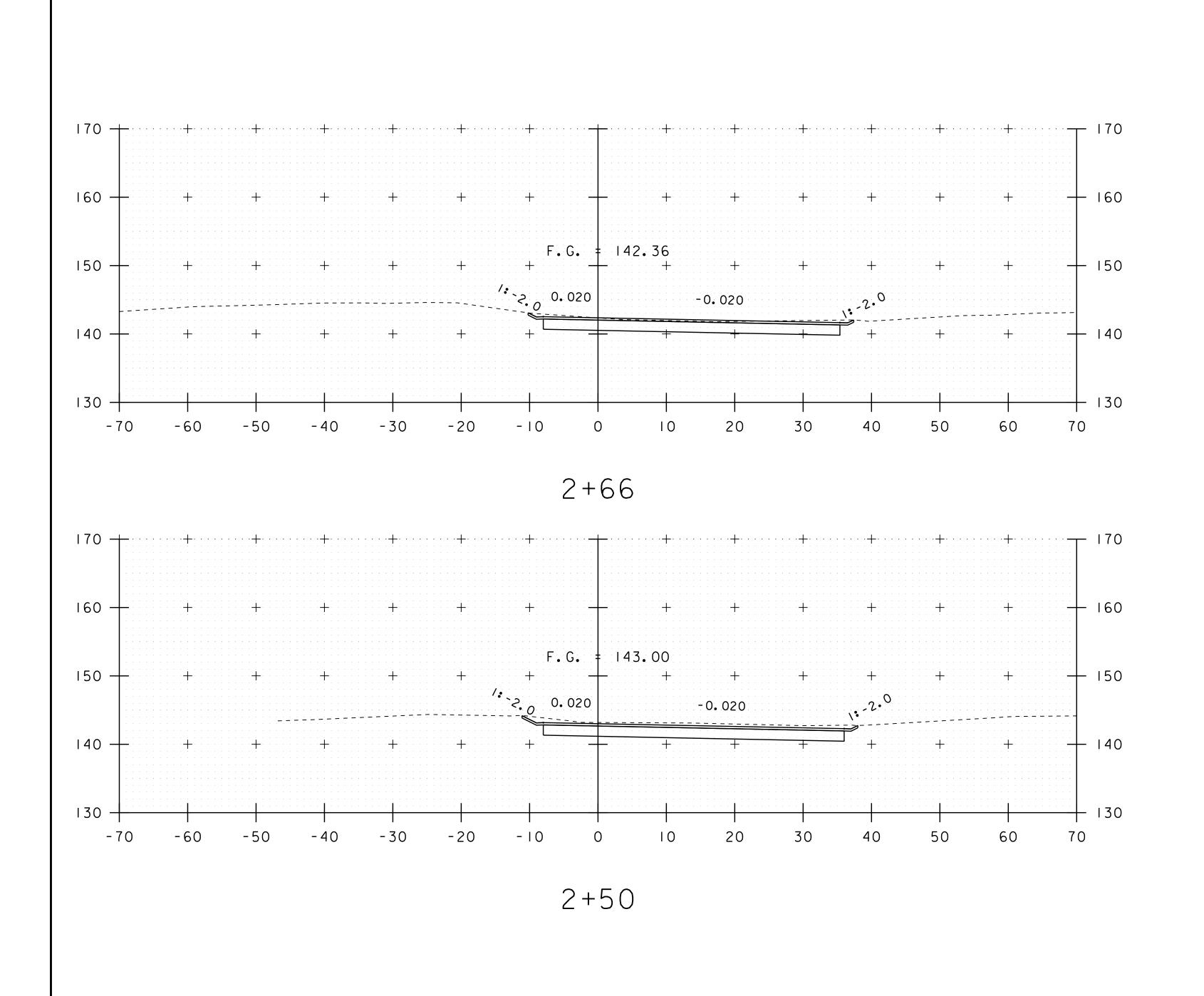












PROJECT NAME: WEYBRIDGE-NEW HAVEN PROJECT NUMBER: BF 032-1(19)

FILE NAME: si2b552xs.dgn
PROJECT LEADER: C.W. CARLSON
DESIGNED BY: D. PETERSON
ACCESS DRIVE CROSS SECTIONS 2

PLOT DATE: 20-APR-2017
DRAWN BY: M.LONGSTREET
CHECKED BY: D. PETERSON
SHEET 72 OF 85

#### **EPSC PLAN NARRATIVE**

#### 1.1 PROJECT DESCRIPTION

THIS PROJECT INVOLVES THE REMOVAL AND REPLACEMENT OF BRIDGE NUMBER 8 OVER THE OTTER CREEK. THE BRIDGE IS ON VT ROUTE 17, ON THE WEYBRIDGE - NEW HAVEN TOWN BOUNDARY. THE PROJECT IS APPROXIMATELY 3.0 MILES EAST OF THE JUNCTION WITH VT ROUTE 22A. THE SPAN OF THE PROPOSED BRIDGE IS 241.50 FEET. THE TOTAL PROJECT IS 725.00 FEET IN LENGTH INCLUDING THE BRIDGE.

NOTE: AREA OF DISTURBANCE INCLUDES LIMITS OF EARTH DISTURBANCE WITHIN THE PROJECT AREA, AS WELL AS WASTE, BORROW AND STAGING AREAS, AND OTHER EARTH DISTURBING ACTIVITIES WITHIN OR DIRECTLY ADJACENT TO THE PROJECT LIMITS AS SHOWN ON THE ATTACHED EPSC PLAN.

TOTAL AREA OF DISTURBANCE AS SHOWN ON THE ATTACHED EPSC PLAN IS APPROXIMATELY 1.78 ACRES.

IT IS ANTICIPATED THAT THIS PROJECT WILL LAST ONE CONSTRUCTION SEASON.

#### 1.2 SITE INVENTORY

#### 1.2.1 TOPOGRAPHY

THE TOPOGRAPHY OF THE AREA IS RELATIVELY FLAT TERRAIN WITH CULTIVATED FARM LAND AND PASTURES. THERE IS LOW BRUSH AND SMALL TREE VEGETATION ON THE BANKS OF THE CREEK AND ALL FOUR CORNERS OF THE BRIDGE AS WELL AS ALONG SIDE VT 17 SURROUNDING THE BRIDGE. THERE ARE NO RESIDENTIAL STRUCTURES WITHIN THE PROJECT LIMITS. THERE IS ONE STORAGE BARN WITH A GRAVEL DRIVE LOCATED NEAR THE BEGIN PROJECT LOCATION SOUTH EAST OF THE BRIDGE.

### 1.2.2 DRAINAGE, WATERWAYS, BODIES OF WATER, AND PROXIMITY TO NATURAL OR MAN-MADE WATER FEATURES

THE OTTER CREEK IS THE ONLY WATER SOURCE ON THE PROJECT SITE. THE CREEK IS CLASSIFIED CLASS B, IT IS MEDIUM SIZE, HAS LEDGE AND SILT BOTTOM, THE WATER IS SLOW MOVING WITH A WIDE FLOOD PLAIN, THE BANKS ARE EQUIWIDTH WITH A SINUOSITY RATED AS IRREGULAR, WANDERING.

#### 1.2.3 VEGETATION

THE VEGETATION IN THE PROJECT AREA CONSISTS OF SMALL TREES AND UNDERGROWTH. THE IMPACT TO VEGETATION WILL BE LIMITED TO THAT WHICH IS DIRECTLY AFFECTED BY REPLACEMENT OF BRIDGE. UPON PROJECT COMPLETION, THE CHANNEL WILL BE ARMORED WITH STONE FILL TYPE III AS SPECIFIED ON THE PLANS. DISTURBED VEGETATION WILL BE REESTABLISHED WITH STANDARD SEED AND MULCH PRACTICES.

#### 1.2.4 SOILS

ALL SOIL DATA CAME FROM THE U.S. DEPARTMENT OF AGRICULTURE SOIL CONSERVATION SERVICE FOR THE COUNTY OF ADDISON, VERMONT. SOILS ON THE PROJECT SITE ARE:

- Cw—Covington and Panton silty clays
- FaC—Farmington extremely rocky silt loam
- Lf-Limerick silt loam, very wet
- Wo-Winooski very fine sandy loam

NOTE: K-VALUES GENERALLY INDICATE THE FOLLOWING: 0.0-0.23 = LOW EROSION POTENTIAL 0.24-0.36 = MODERATE EROSION POTENTIAL 0.37 AND HIGHER = HIGH EROSION POTENTIAL

#### 1.2.5 SENSITIVE RESOURCE AREAS

CRITICAL HABITATS: NO

HISTORICAL OR ARCHEOLOGICAL AREAS: YES, ON 3 CORNERS (SEE PLANS FOR LOCATION)
PRIME AGRICULTURAL LAND: NO

THREATENED AND ENDANGERED SPECIES: YES, FRESHWATER MUSSELS, NORTHERN LONG-EARED BAT AND INDIANA BAT

WATER RESOURCE: OTTER CREEK

WETLANDS: YES, ON (2) CORNERS, NO PERMANENT IMPACTS IN WETLANDS.

#### 1.3 RISK EVALUATION

THIS PROJECT FALLS UNDER THE JURISDICTION OF GENERAL PERMIT 3-9020 FOR STORMWATER RUNOFF FROM CONSTRUCTION SITES FOR LOW RISK PROJECTS. ANY MODIFICATIONS TO THE PROJECT THAT INCREASE THE RISK TO ENVIRONMENTAL RESOURCES SHALL BE EVALUATED IN ACCORDANCE WITH THE PERMIT REQUIREMENTS. THE CONTRACTOR WILL BE RESPONSIBLE FOR ANY ADDITIONAL PERMITTING.

#### 1.4 EROSION PREVENTION AND SEDIMENT CONTROL

THE EROSION CONTROL PLANS ARE MEANT AS A GUIDELINE FOR PREVENTING EROSION AND CONTROLLING SEDIMENT TRANSPORT. THE PRINCIPLES OUTLINED IN THIS NARRATIVE CONSIST OF APPLYING MEASURES THROUGHOUT CONSTRUCTION OF THE PROJECT IN ORDER TO MINIMIZE SEDIMENT TRANSPORT TO THE RECEIVING WATERS. THE MEASURES INCLUDE STABILIZATION AND STRUCTURAL PRACTICES, STORM WATER CONTROLS AND OTHER POLLUTION PREVENTION PRACTICES. THEY HAVE BEEN PROPOSED BY THE DESIGNER AS A BASIS FOR PROTECTING RESOURCES AND WILL NEED TO BE BUILT UPON BASED ON THE SPECIFIC MEANS AND METHODS OF THE CONTRACTOR. REFER TO THE LOW RISK SITE HANDBOOK AND APPROPRIATE DETAIL SHEETS FOR SPECIFIC GUIDANCE AND CONSTRUCTION DETAILING.

ALL MEASURES SHALL BE REGULARLY MAINTAINED AND SHALL BE CHECKED FOR SEDIMENT BUILD-UP. SEDIMENT SHALL BE DISPOSED OF AT AN APPROVED SITE WHERE IT WILL NOT BE SUBJECT TO EROSION.

#### 1.4.1 MARK SITE BOUNDARIES

SITE BOUNDARIES AND AREAS CONSTRUCTION EQUIPMENT CAN ACCESS SHALL BE DELINEATED.

PROJECT DEMARCATION FENCING (PDF) SHALL BE USED TO PHYSICALLY MARK SITE BOUNDARIES. BECAUSE THIS PROJECT FALLS UNDER THE CGP 3-9020, BARRIER FENCE SHALL BE USED INSTEAD OF PROJECT DEMARCATION FENCE WITHIN 100 FEET OF A WATER RESOURCE (STREAM, BROOK, LAKE, POND, WETLAND, ETC).

#### 1.4.2 LIMIT DISTURBANCE AREA

PREVENTING INITIAL SOIL EROSION BY MINIMIZING THE EXPOSED AREA IS MUCH MORE EFFECTIVE THAN TREATING ERODED SEDIMENT. EARTH DISTURBANCE CAN BE MINIMIZED THROUGH CONSTRUCTION PHASING BY ONLY OPENING UP EARTH AS NECESSARY. THIS CAN LIMIT THE AREA THAT WILL BE DISTURBED AND EXPOSED TO EROSION. EMPLOY TEMPORARY CONSTRUCTION STABILIZATION PRACTICES IN INCREMENTAL STAGES AS PHASES CHANGE. FOR PROJECTS, WHICH FALL UNDER THE CONSTRUCTION GENERAL PERMIT, ONLY THE ACREAGE LISTED ON THE PERMIT AUTHORIZATION MAY BE EXPOSED AT ANY GIVEN TIME.

MAINTAINING VEGETATED BUFFERS ALONG STREAM BANKS, WETLANDS OR OTHER SENSITIVE AREAS IS A CRUCIAL EROSION AND SEDIMENT CONTROL MEASURE THAT SHOULD BE ESTABLISHED WHEREVER POSSIBLE.

#### 1.4.3 SITE ENTRANCE/EXIT STABILIZATION

TRACKING OF SEDIMENT ONTO PUBLIC HIGHWAYS SHALL BE MINIMIZED TO REDUCE THE POTENTIAL FOR RUNOFF ENTERING RECEIVING WATERS. INSTALLATION SHALL COINCIDE WITH THE CONTRACTORS PROGRESS SCHEDULE.

STABILIZED CONSTRUCTION ENTRANCES SHALL BE INSTALLED AS PROPOSED ON THE EPSC PLAN AND ANYWHERE EQUIPMENT WILL BE GOING FROM AREAS OF EXPOSED SOILS TO PAVED SURFACES.

#### 1.4.4 INSTALL SEDIMENT BARRIERS

SEDIMENT BARRIERS SHALL BE UTILIZED TO INTERCEPT RUNOFF AND ALLOW SUSPENDED SEDIMENT TO SETTLE OUT. THEY SHALL BE INSTALLED PRIOR TO ANY UP-SLOPE WORK.

SILT FENCE WILL BE INSTALLED AS PROPOSED ON THE EPSC PLAN. BECAUSE THIS PROJECT FALLS UNDER THE CGP 3-9020, WOVEN WIRE REINFORCED SILT FENCE SHALL BE USED INSTEAD OF SILT FENCE WITHIN 100 FEET UPSLOPE OF RECEIVING WATERS.

FILTER CURTAIN WILL BE INSTALLED AS PROPOSED ON THE EPSC PLAN.

#### 1.4.5 DIVERT UPLAND RUNOFF

DIVERSIONARY MEASURES SHALL BE USED TO INTERCEPT RUNOFF FROM ABOVE THE CONSTRUCTION AND DIRECT IT AROUND THE DISTURBED AREA SO THAT CLEAN WATER DOES NOT BECOME MUDDIED WHILE TRAVELING OVER EXPOSED SOILS ON THE CONSTRUCTION SITE.

THE PROJECT AREA IS RELATIVELY FLAT. THEREFORE, IT IS NOT ANTICIPATED THAT DIVERSION MEASURES WILL BE NECESSARY.

#### 1.4.6 SLOW DOWN CHANNELIZED RUNOFF

CHECK STRUCTURES SHALL BE UTILIZED TO REDUCE THE VELOCITY, AND THUS THE EROSIVE POTENTIAL, OF CONCENTRATED FLOW IN CHANNELS. STONE CHECK DAMS ARE NOT ANTICIPATED AT THIS TIME.

#### 1.4.7 CONSTRUCT PERMANENT CONTROLS

PERMANENT STORMWATER TREATMENT DEVICES SHALL BE INSTALLED AS SHOWN ON THE PLANS AND IN ACCORDANCE WITH PERMIT CONDITIONS. PERMANENT STORMWATER TREATMENT NOT ANTICIPATED ON THIS PROJECT.

#### 1.4.8 STABILIZE EXPOSED SOILS DURING CONSTRUCTION

ALL AREAS OF DISTURBANCE MUST HAVE TEMPORARY STABILIZATION IN PLACE WITHIN 48 HOURS OF DISTURBANCE OR IN ACCORDANCE WITH THE CONSTRUCTION GENERAL PERMIT 3-9020 AUTHORIZATION.

SURFACE ROUGHENING OF ALL EXPOSED SLOPES, COMBINED WITH TEMPORARY MULCHING, SHALL BE UTILIZED ON A REGULAR BASIS. BIODEGRADABLE EROSION CONTROL MATTING OR AN EQUIVALENT SHALL BE USED TO STABILIZE ALL SLOPES STEEPER THAN 1:3.

THE FORECAST OF RAINFALL EVENTS SHALL TRIGGER IMMEDIATE PROTECTION OF EXPOSED SOILS.

#### 1.4.9 WINTER STABILIZATION

VARIOUS MEASURES SPECIFIC TO WINTER MAY BE NECESSARY SHOULD THE PROJECT EXTEND INTO WINTER (OCTOBER 15 THROUGH APRIL 15). THE PIER WILL BE CONSTRUCTED IN THE FALL OF ONE CONSTRUCTION SEASON AND REMAINDER OF THE BRIDGE WILL BE CONSTRUCTED THE FOLLOWING CONSTRUCTION SEASON. ANY CHANNEL OR CAUSEWAY EXCAVATION SHALL BE PREPARED FOR WINTER STABILIZATION. REFER TO THE LOW RISK SITE HANDBOOK FOR GUIDANCE.

#### 1.4.10 STABILIZE SOIL AT FINAL GRADE

EXPOSED SOIL MUST BE STABILIZED WITHIN 48 HOURS OF REACHING FINAL GRADE.

SEED, MULCH, FERTILIZER AND LIME SHALL BE USED TO ESTABLISH PERMANENT VEGETATION. FOR SLOPES STEEPER THAN 1:3, BIODEGRADABLE EROSION CONTROL MATTING OR AN EQUIVALENT SHALL BE USED INSTEAD OF MULCH.

#### 1.4.11 DE-WATERING ACTIVITIES

DISCHARGE FROM DEWATERING ACTIVITIES THAT FLOWS OFF OF THE CONSTRUCTION SITE MUST NOT CAUSE OR CONTRIBUTE TO A VIOLATION OF THE VERMONT WATER QUALITY STANDARDS.

TREATMENT OF DEWATERING COFFERDAM IS ANTICIPATED. A LOCATION FOR TREATMENT HAS BEEN PROPOSED AND IS SHOWN ON THE PLANS. HOWEVER, THE SPECIFIC MEANS FOR TREATMENT OF DISCHARGE SHALL BE PROVIDED BY THE CONTRACTOR.

#### 1.4.12 INSPECT YOUR SITE

INSPECT THE PROJECT SITE BASED ON SPECIAL PROVISION REQUIREMENTS OR CONSTRUCTION GENERAL PERMIT AUTHORIZATION STIPULATIONS. NO KNOW SPECIAL SITE SPECIFIC OR GENERAL PERMIT INSPECTION REQUIREMENTS AT THIS TIME.

#### 1.5 SEQUENCE AND STAGING

THIS SECTION WILL BE DEVELOPED BY THE CONTRACTOR USING THE GUIDANCE OUTLINED IN THE VTRANS EPSC PLAN CONTRACTOR CHECKLIST.

#### 1.5.1 CONSTRUCTION SEQUENCE

#### 1.5.2 OFF-SITE ACTIVITIES

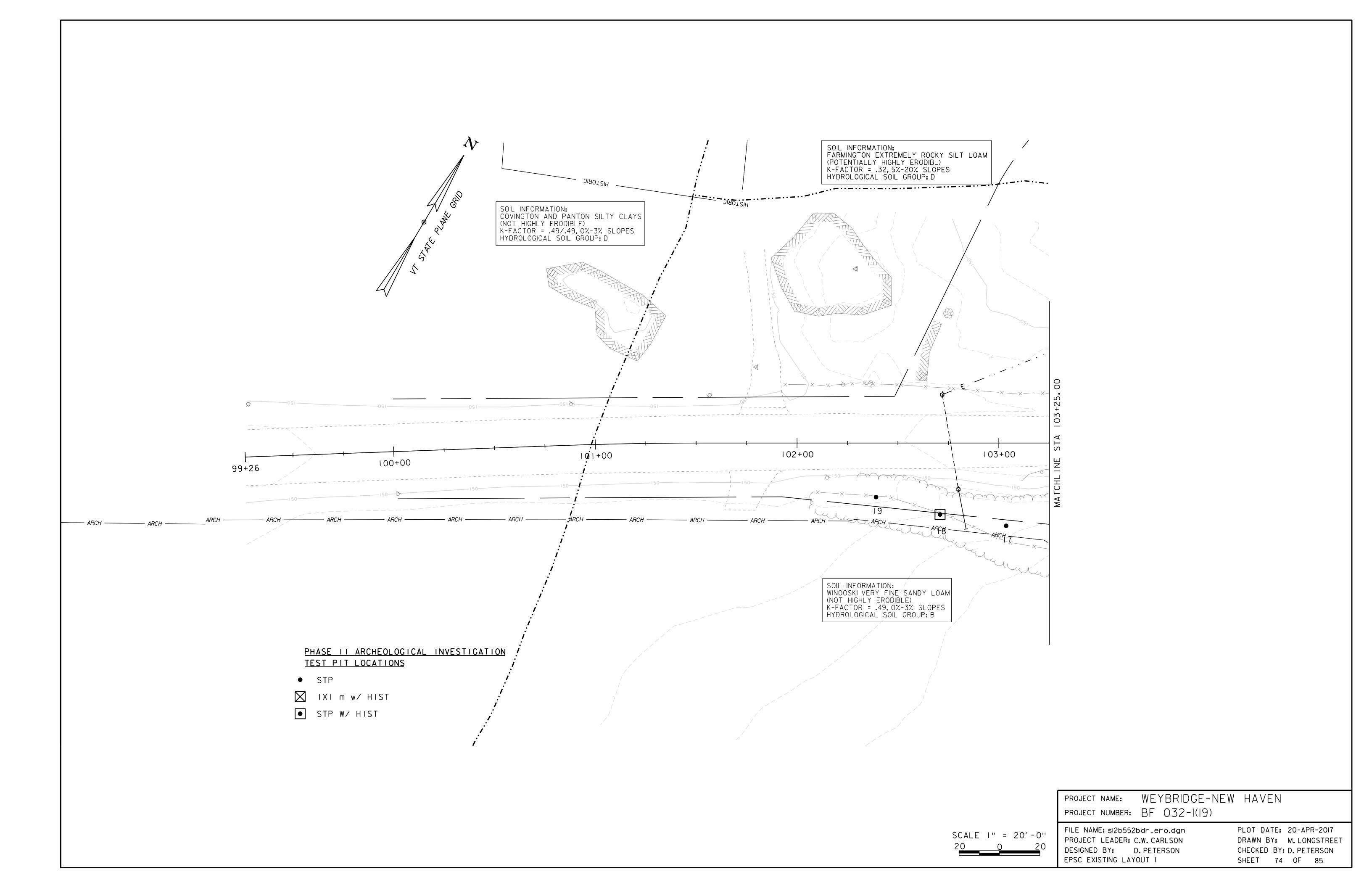
IN ADDITION TO THE CONTRACTOR CHECKLIST ANY ACTIVITIES OUTSIDE THE CONSTRUCTION LIMITS SHALL FOLLOW SPECIFICATION 105.25- 105.29 OF THE STANDARD SPECIFICATIONS FOR CONSTRUCTION.

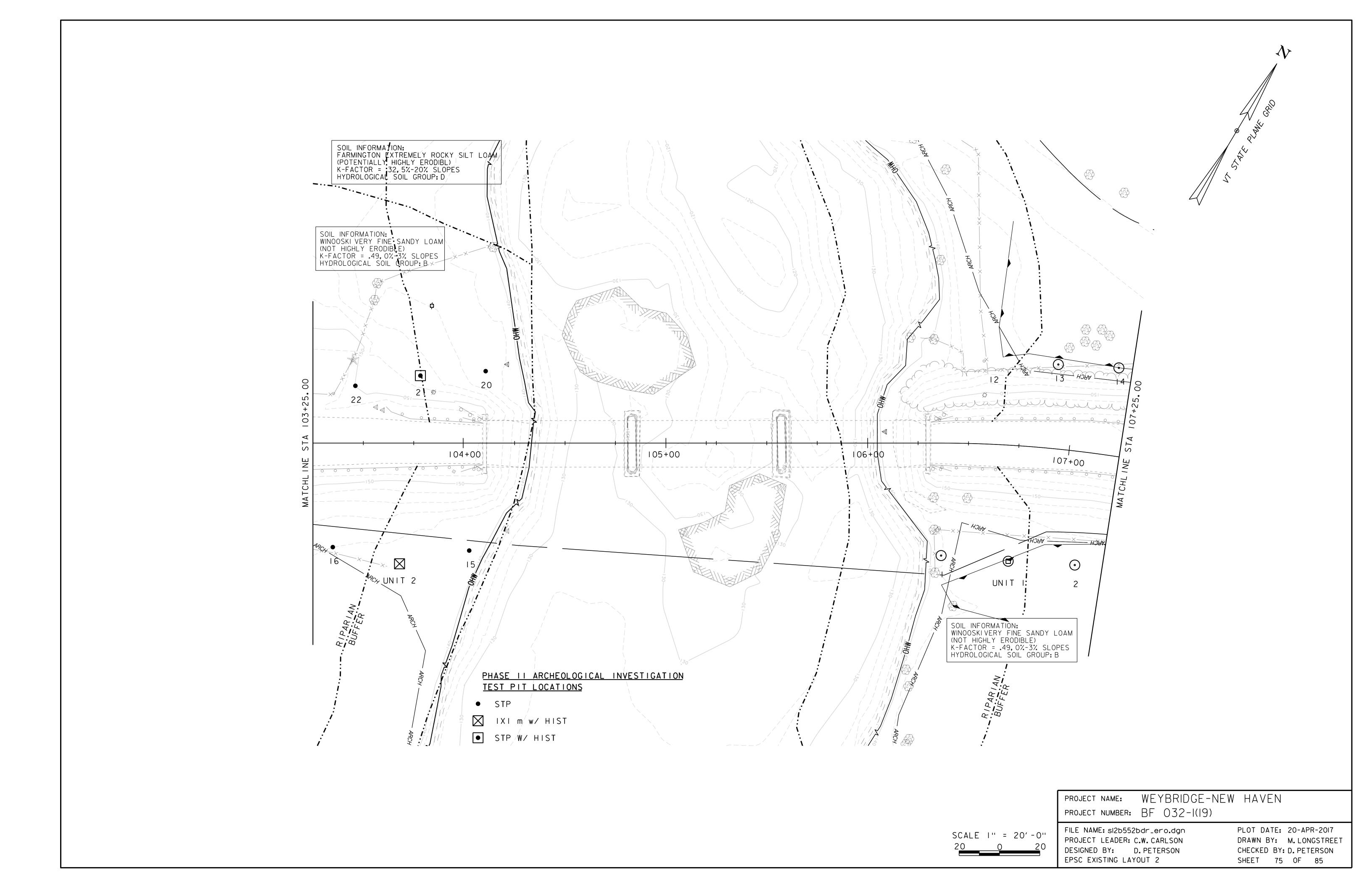
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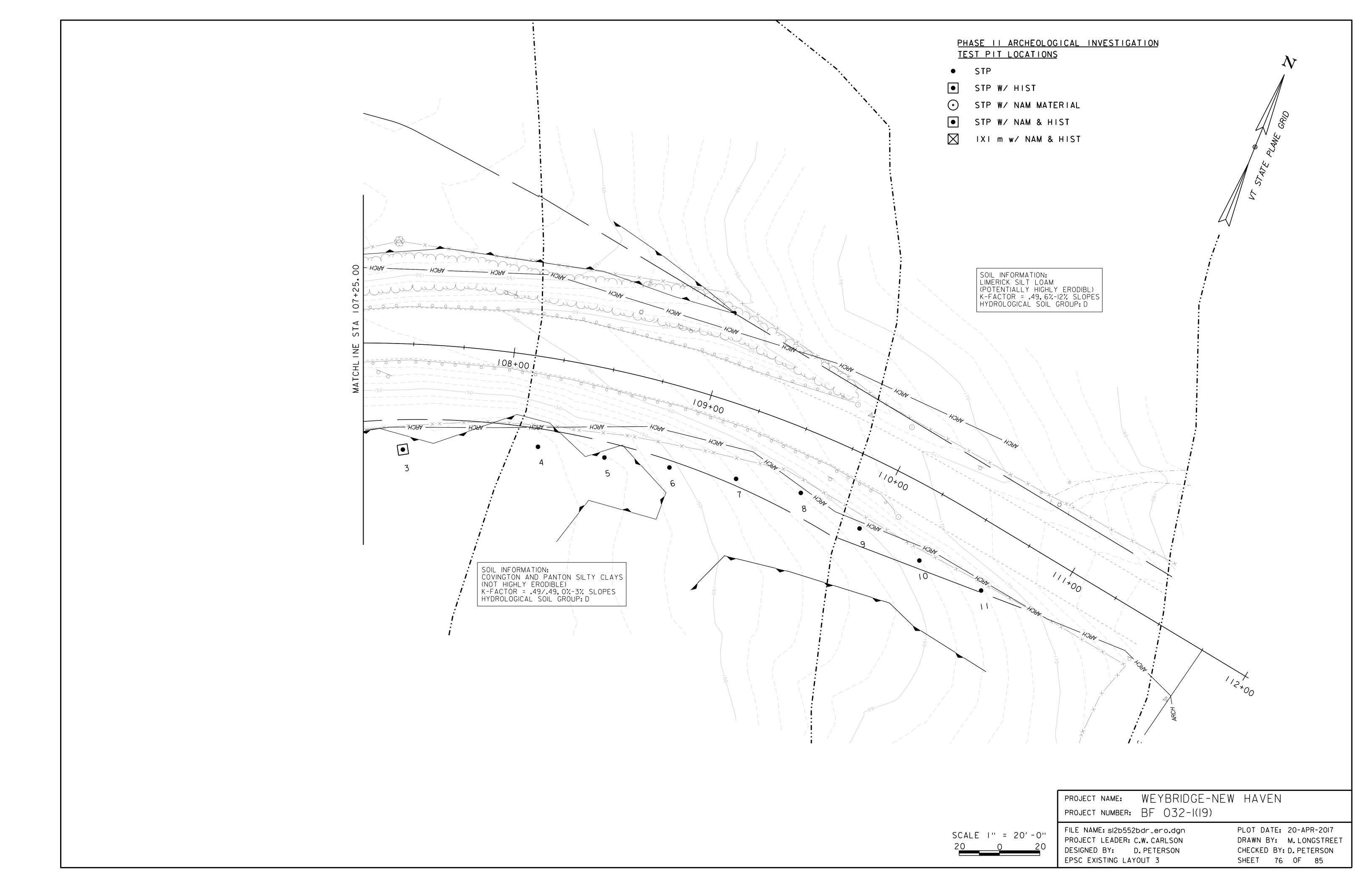
FILE NAME: sl2b552eronar.dgn PROJECT LEADER: C.W. CARLSON DESIGNED BY: D. PETERSON

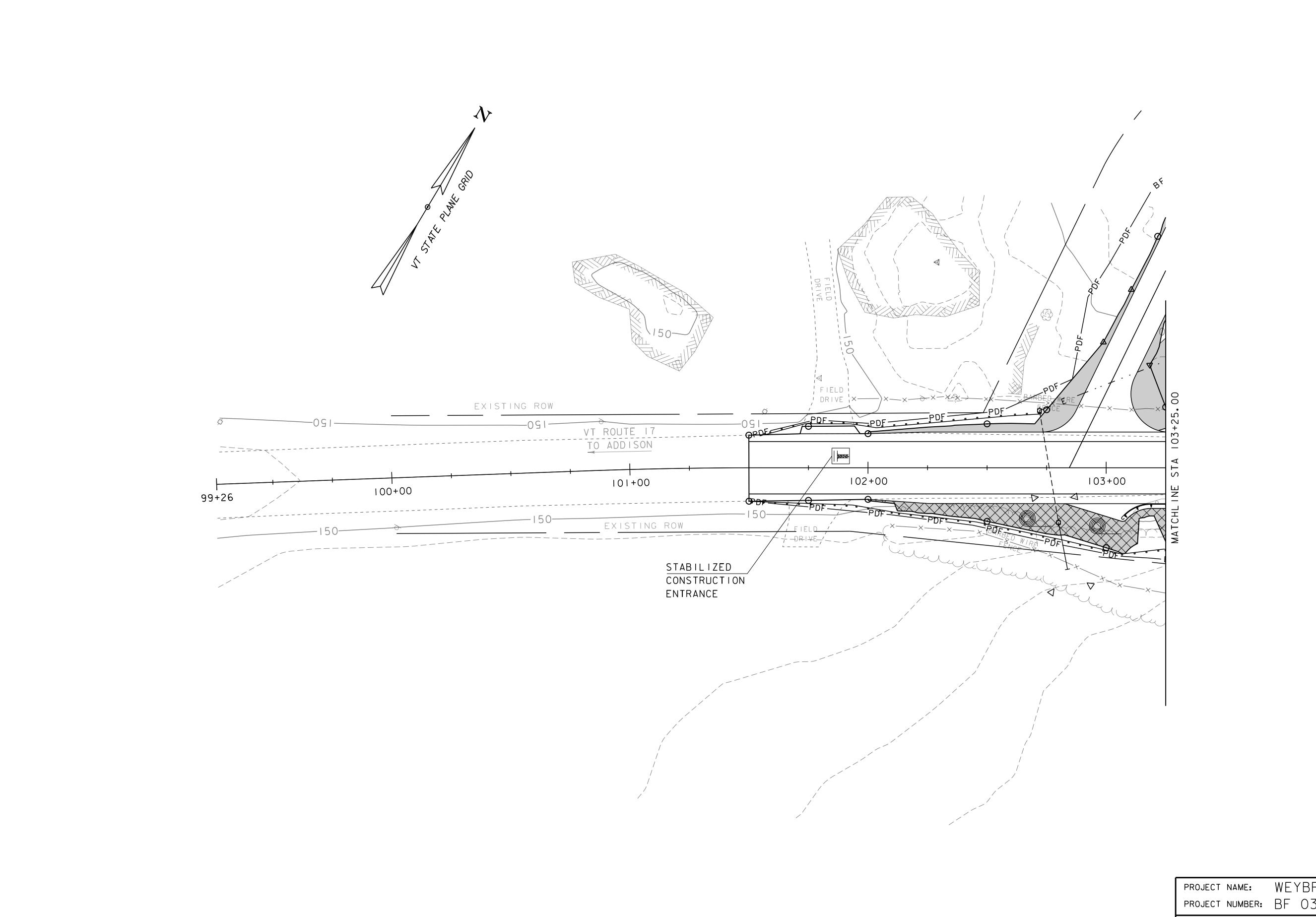
EPSC NARRATIVE

PLOT DATE: 20-APR-2017
DRAWN BY: M. LONGSTREET
CHECKED BY: D. PETERSON
SHEET 73 OF 85







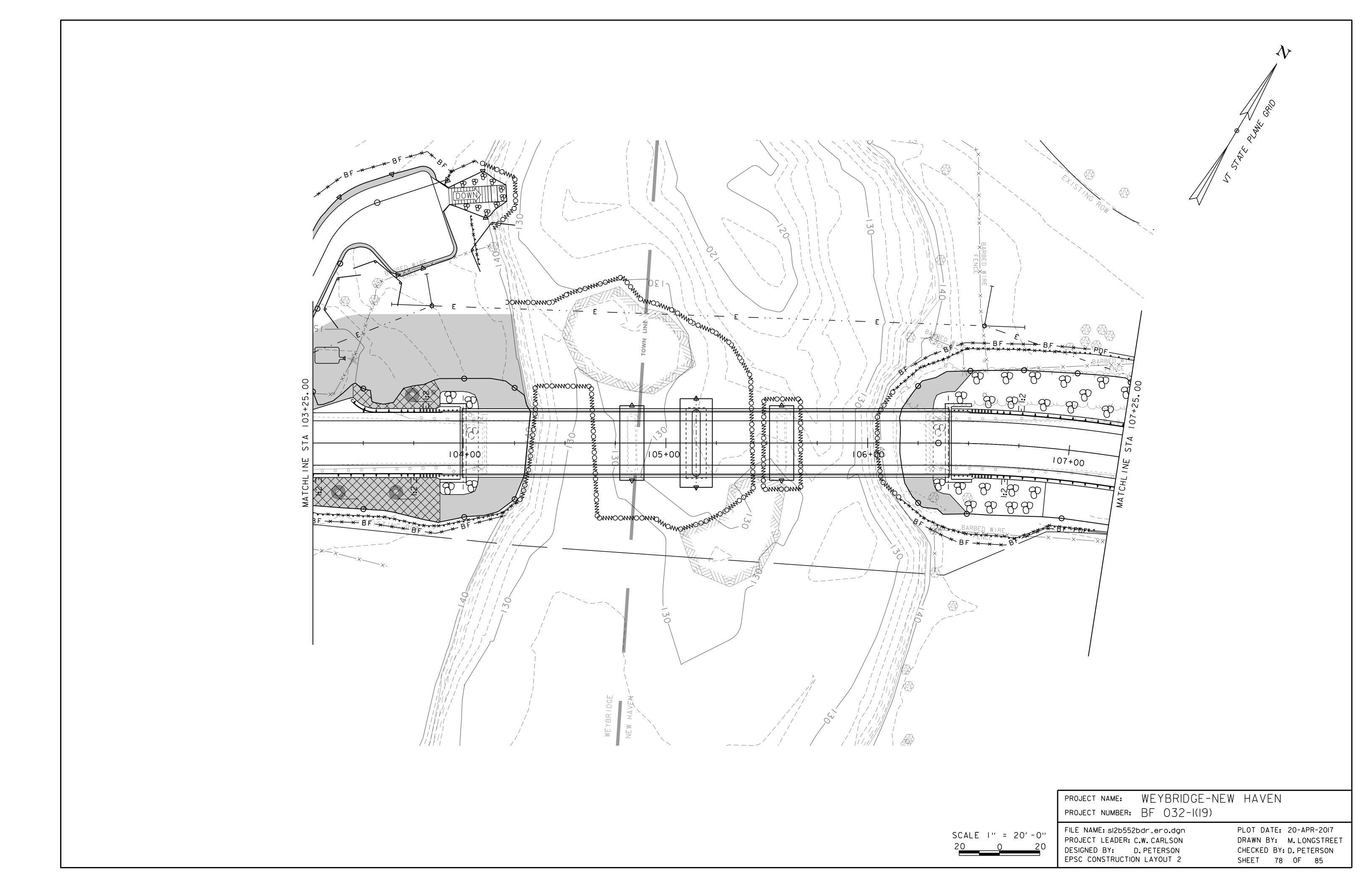


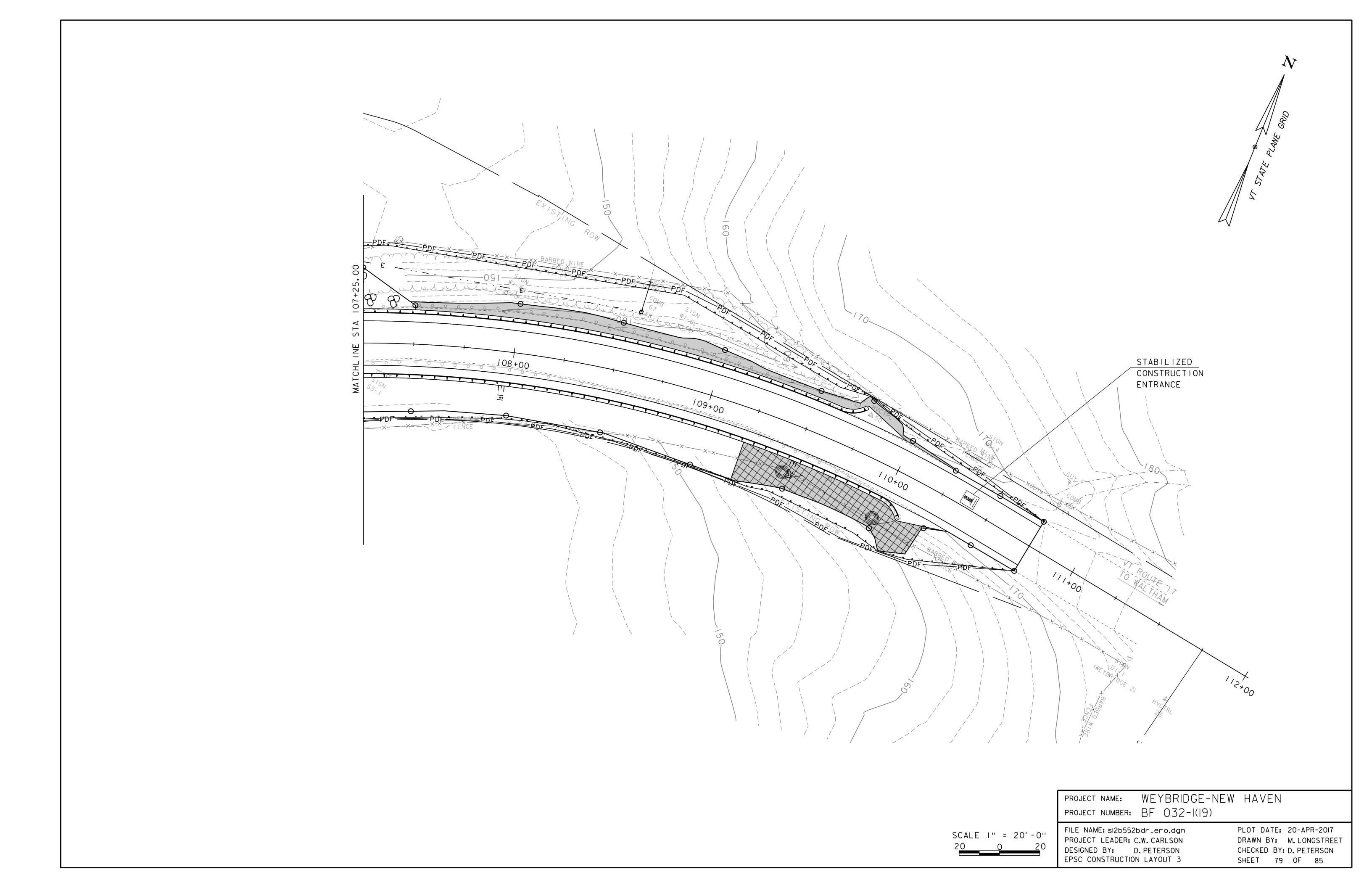
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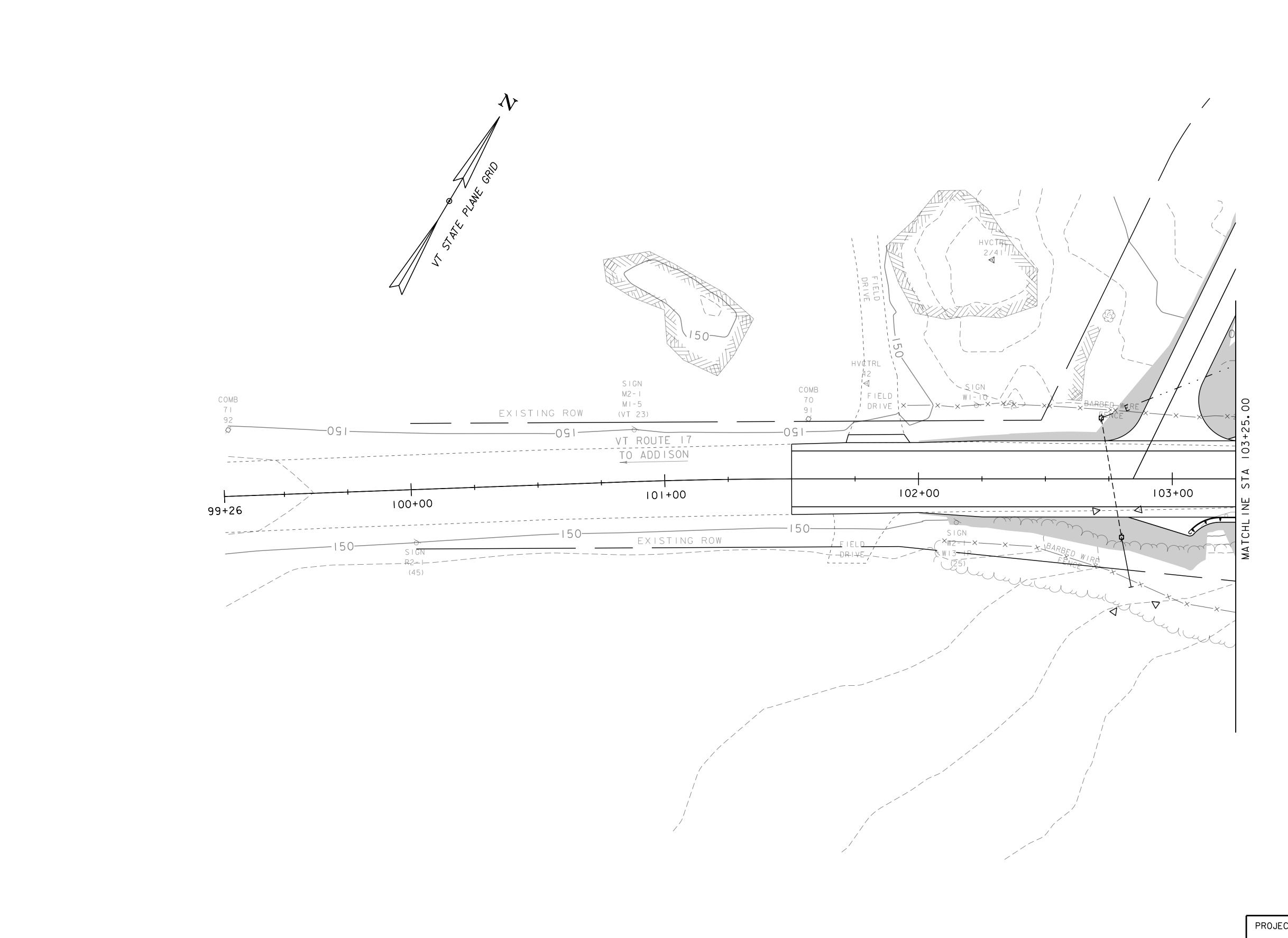
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SCALE I'' = 20'-0'' 20 0 20

PLOT DATE: 20-APR-2017 DRAWN BY: M. LONGSTREET CHECKED BY: D. PETERSON SHEET 77 OF 85



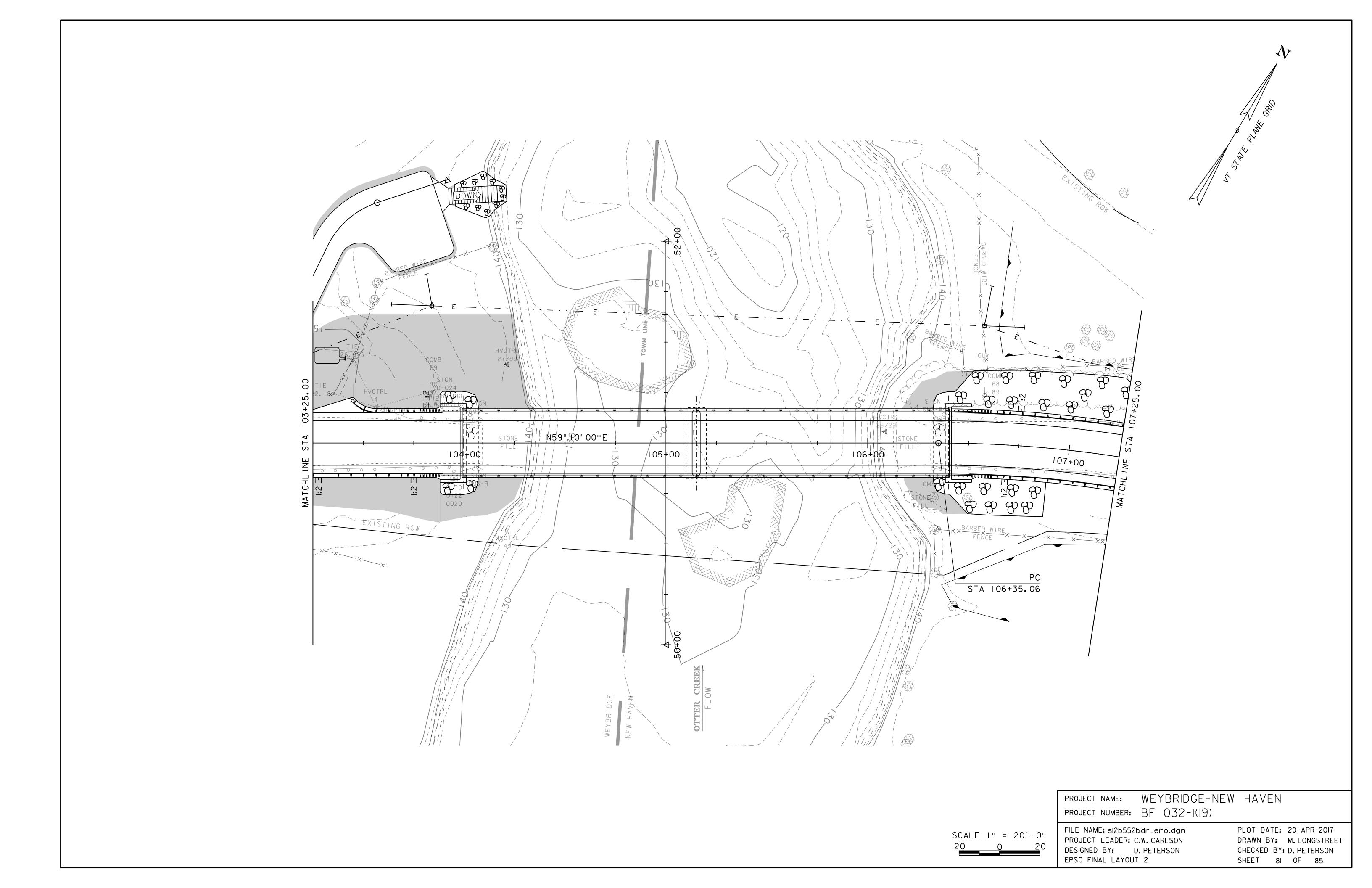


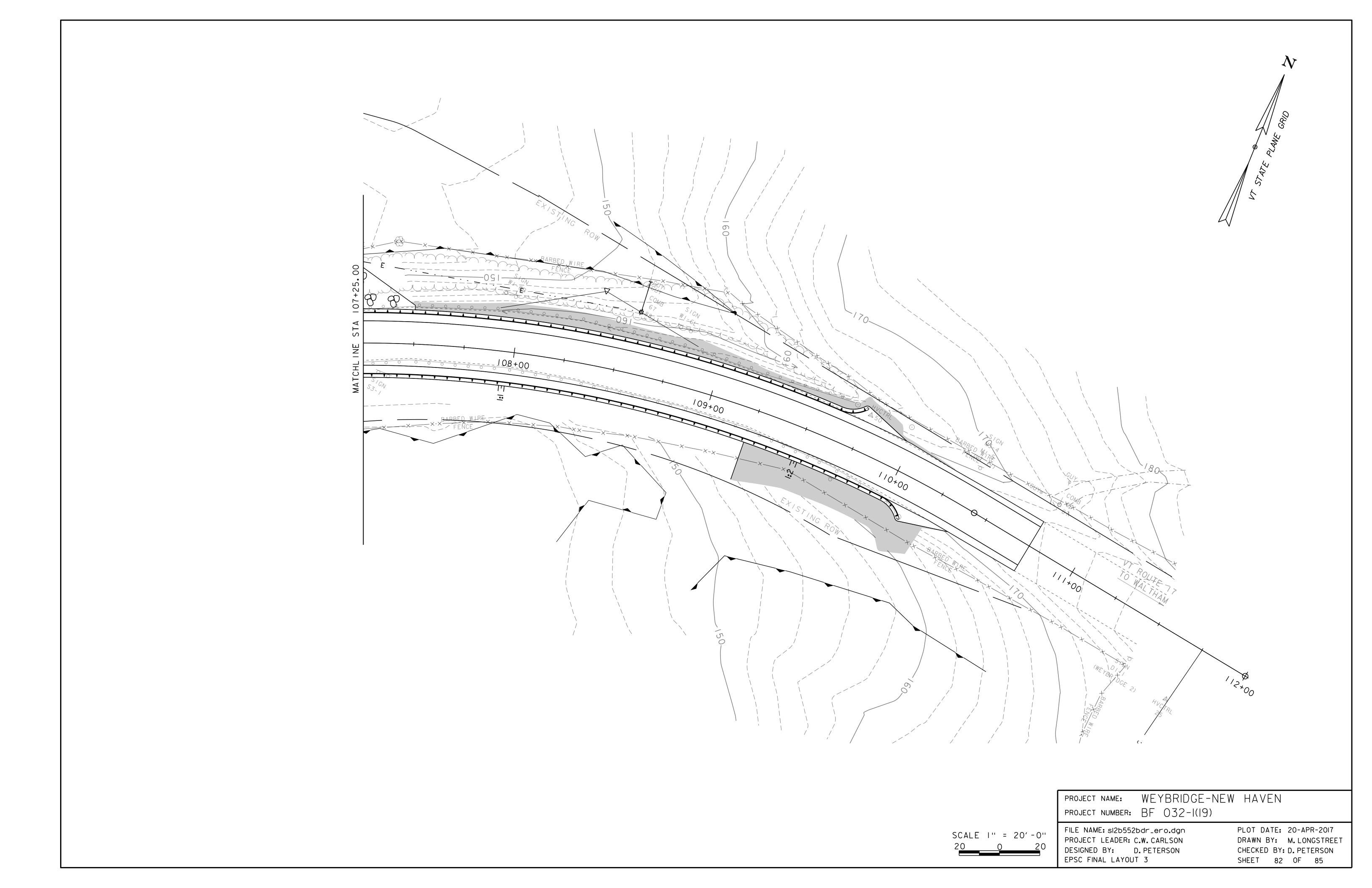


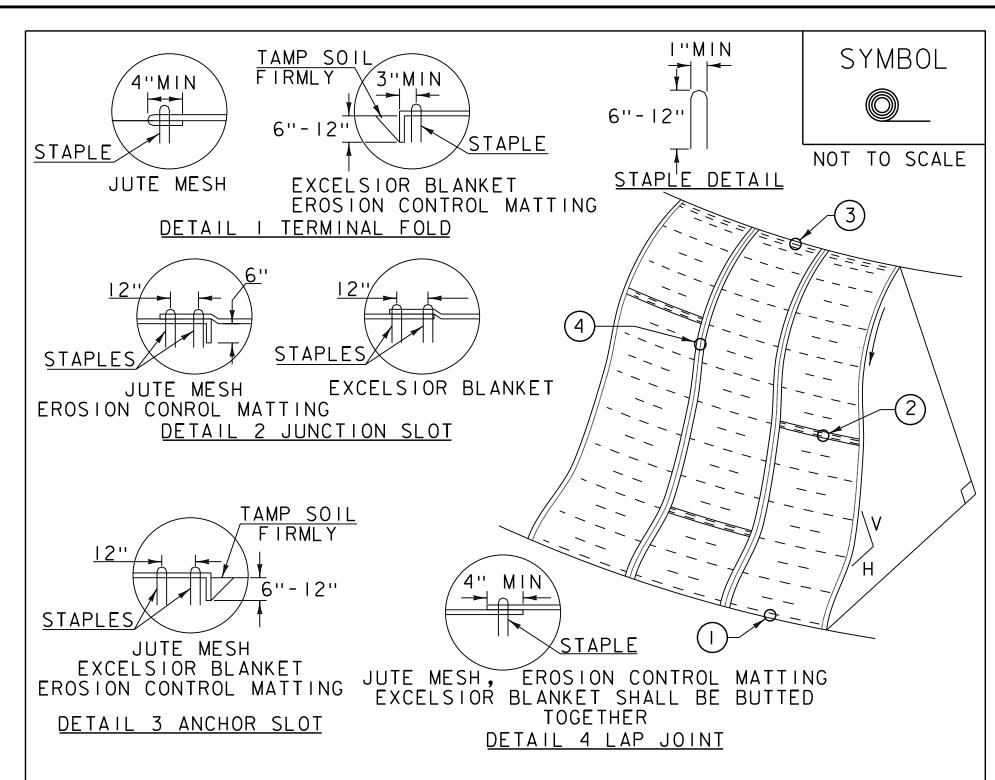
PROJECT NAME: WEYBRIDGE-NEW HAVEN PROJECT NUMBER: BF 032-1(19)

FILE NAME: sI2b552bdr_ero.dgn
PROJECT LEADER: C.W. CARLSON
DESIGNED BY: D. PETERSON
EPSC FINAL LAYOUT I

SCALE I'' = 20'-0'' 20 0 20 PLOT DATE: 20-APR-2017
DRAWN BY: M. LONGSTREET
CHECKED BY: D. PETERSON
SHEET 80 OF 85







#### CONSTRUCTION SPECIFICATIONS

- I. APPLY TO SLOPES GREATER THAN 3H: IV OR WHERE NECESSARY TO AID IN ESTABLISHING VEGETATION.
- 2. APPLY FERTILIZER, LIME SEED PRIOR TO PLACING MATTING.
- 3. STAPLES ARE TO BE PLACED ALTERNATELY, IN COLUMNS APPROXIMATELY 2' APART AND IN ROWS APPROXIMATELY 3' APART. APPROXIMATELY 175 STAPLES ARE REQUIRED PER 4'X225' ROLL OF MATERIAL AND 125 STAPLES ARE REQUIRED PER 4'X150' ROLL OF MATERIAL.
- 4. DISTURBED AREAS SHALL BE SMOOTHLY GRADED. EROSION CONTROL MATERIAL SHALL BE PLACED LOOSELY OVER GROUND SURFACE. DO NOT STRETCH.
- 5. ALL TERMINAL ENDS AND TRANSVERSE LAPS SHALL BE STAPLED AT APPROXIMATELY 12" INTERVALS.

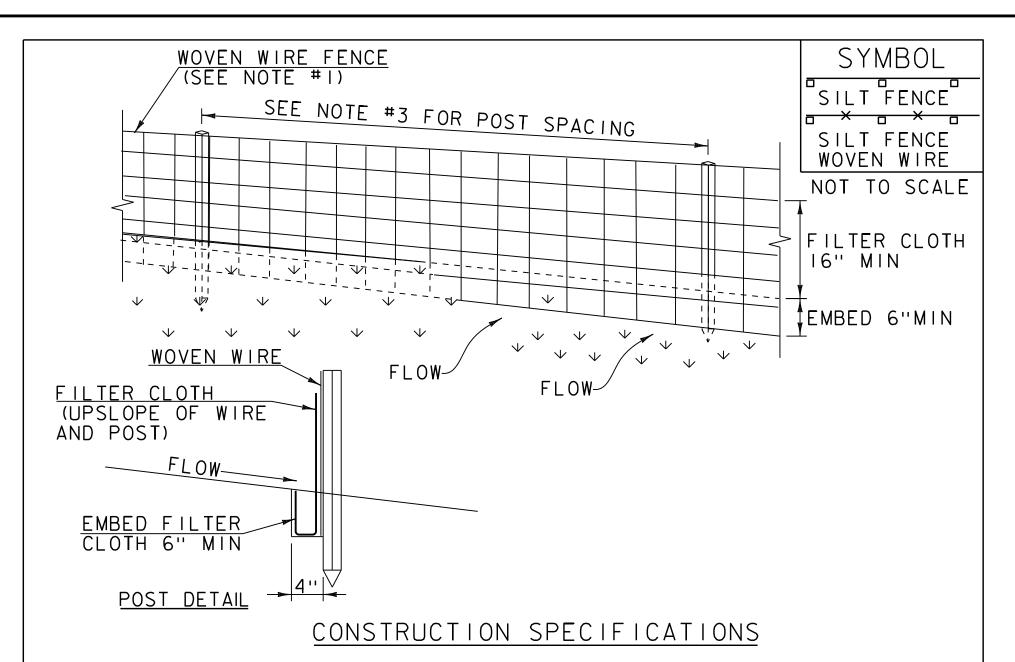
ADAPTED FROM DETAILS PROVIDED BY: NEW YORK STATE DEC ORIGINALLY DEVELOPED BY USDA-NRCS VERMONT DEPARTMENT OF ENVIRONMENTAL CONSERVATION ROLLED EROSION CONTROL PRODUCT (RECP) SIDE SLOPE

NOTES:

REFER TO "THE VERMONT STANDARDS & SPECIFICATIONS FOR EROSION PREVENTION & SEDIMENT CONTROL -2006- "FROM THE VT AGENCY OF NATURAL RESOURCES FOR ADDITIONAL GUIDANCE.

THIS WORK SHALL BE PERFORMED IN ACCORDANCE WITH SECTION 653 AND AS SHOWN IN THE PLANS FOR TEMPORARY EROSION MATTING (PAY ITEM 653.20) OR PERMANENT EROSION MATTING (PAY ITEM 653.21).

REVISIONS
APRIL 16, 2007 JMF
JANUARY 13, 2009 WHF



- . WOVEN WIRE REINFORCED FENCE IS REQUIRED WITHIN 100' UPSLOPE OF RECEIVING WATERS WHEN THE PROJECT FALLS UNDER A CONSTRUCTION STORMWATER PERMIT. WOVEN WIRE SHALL BE A MIN. 14 GAUGE WITH A 6" MAX. MESH OPENING.
- 2. FILTER CLOTH SHALL BE EITHER FILTER X, MIRAFIIOOX, STABILINKA TI40N OR APPROVED EQUIVALENT.
- 3. POST SPACING FOR WIRE-BACKED FENCE SHALL BE 10' MAXIMUM. FOR FILTER-CLOTH FENCE, WHEN ELONGATION IS >50%, POST SPACING SHALL NOT EXCEED 4' AND WHEN ELONGATION IS <50%, POST SPACING SHALL NOT EXCEED 6'.
- 4. WOVEN WIRE FENCE IS TO BE FASTENED SECURELY TO FENCE POSTS WITH WIRE TIES. FILTER CLOTH IS TO BE FASTENED SECURELY TO WOVEN WIRE FENCE WITH TIES SPACED EVERY 24" AT TOP AND MID SECTION.
- 5. WHEN TWO SECTIONS OF FILTER CLOTH ADJOIN EACH OTHER THEY SHALL BE OVER-LAPPED BY 6" AND FOLDED.
- 6. MAINTENANCE SHALL BE PERFORMED AS NEEDED AND MATERIAL REMOVED WHEN SEDIMENT REACHES HALF OF FABRIC HEIGHT.

ADAPTED FROM DETAILS PROVIDED BY: NEW YORK STATE DEC ORIGINALLY DEVELOPED BY USDA-NRCS VERMONT DEPARTMENT OF ENVIRONMENTAL CONSERVATION

SILT FENCE

NOTES:
REFER TO "THE VERMONT STANDARDS & SPECIFICATIONS FOR EROSION PREVENTION & SEDIMENT CONTROL -2006- "FROM THE VT AGENCY OF NATURAL RESOURCES FOR ADDITIONAL GUIDANCE.

THIS WORK SHALL BE PERFORMED IN ACCORDANCE WITH SECTION 649 AND AS SHOWN IN THE PLANS FOR GEOTEXTILE FOR SILT FENCE (PAY ITEM 649.51) OR GEOTEXTILE FOR SILT FENCE, WOVEN WIRE REINFORCED (PAY ITEM 649.515).

REVISIONS	
MARCH 21, 2008	WHF
DECEMBER II, 2008	WHF
JANUARY 13. 2009	WHF

	VAOT LOW GROW/FINE FESCUE MIX					
	LBS/AC					
WEIGHT	BROADCAST	HYDROSEED	NAME	LATIN NAME	GERM	PURITY
38%	57	95	CREEPING RED FESCUE	FESTUCA RUBRA VAR. RUBRA	90%	98%
29%	43.5	72.5	HARD FESCUE	FESTUCA LONGIFOLIA	85%	95%
15%	22.5	37.5	CHEWINGS FESCUE	FESTUCA RUBRA VAR. COMMUTATA	87%	95%
15%	22.5	37.5	ANNUAL RYEGRASS	LOLIUM MULTIFLORUM	90%	95%
3%	4.5	7.5	INERTS			
100%	150	250				

				·		
VAOT RURAL AREA MIX						
LBS/AC						
VEIGHT	BROADCAST	HYDROSEED	NAME	LATIN NAME	GERM	PURITY
37.5%	22.5	45	CREEPING RED FESCUE	FESTUCA RUBRA VAR. RUBRA	85%	98%
37.5%	22.5	45	TALL FESCUE	FESTUCA ARUNDINACEA	90%	95%
5.0%	3	6	RED TOP	AGROSTIS GIGANTEA	90%	95%
15.0%	9	18	WHITE FIELD CLOVER	TRIFOLIUM REPENS	85%	98%
5.0%	3	6	ANNUAL RYE GRASS	LOLIUM MULTIFLORUM	85%	95%
100%	60	120				

GENERAL AMENDMENT GUIDANCE				
FERTILIZER	L	IME		
10/20/10	AG LIME	PELLITIZED		
500 LBS/AC	2 TONS/AC	1 TONS/AC		

#### CONSTRUCTION GUIDANCE

- I.SEED MIX: THE CONTRACTOR SHALL COORDINATE WITH THE RESIDENT ENGINEER ON WHICH SEED MIX TO USE.
- 2.SEED MIX: USE AS INDICATED IN THE PLANS AND/OR FOR ALL ESTABLISHED UPLAND (NON WETLAND) AREAS DISTURBED BY THE CONTRACTOR.
- 3.ALL SEED MIXTURES: SHALL NOT HAVE A WEED CONTENT EXCEEDING 0.40% BY WEIGHT AND SHALL BE FREE OF ALL NOXIOUS SEED.
- 4.FERTILIZER AND LIMESTONE: SHALL FOLLOW RATES SHOWN ON PLAN OR AS DIRECTED BY THE ENGINEER.
- 5. HAY MULCH: TO BE PLACED ON EARTH SLOPES AT THE RATE OF 2 TONS/ACRE, ACHIEVE 90% GROUND COVER OR AS DIRECTED BY THE ENGINEER.
- 6. HYDROSEEDING: ALTHOUGH GUIDANCE IS GIVEN ABOVE THE SITE CONDITIONS AND THE TYPE OF HYDROSEED PROPOSED FOR USE WILL ULTIMATELY DICTATE THE AMOUNTS AND TYPES OF SOIL AMENDMENTS TO BE APPLIED.
- 7.TURF ESTABLISHMENT: PLACING SEED, FERTILIZER, LIME AND MULCH PRIOR
  TO SEPTEMBER 15 AND AFTER APRIL 15 CAN BETTER ENSURE A VIGOROUS
  GROWTH OF GRASS.

DAPTED FROM VTRANS TECHNICAL LANDSCAPE MANUAL FOR ROADWAYS AND TRANSPORTATION FACILITIES	TURF ESTABLISHMENT
	DEVICIONS

THIS WORK SHALL BE PERFORMED IN ACCORDANCE WITH SECTION 651 FOR SEED (PAY ITEM 651.15)

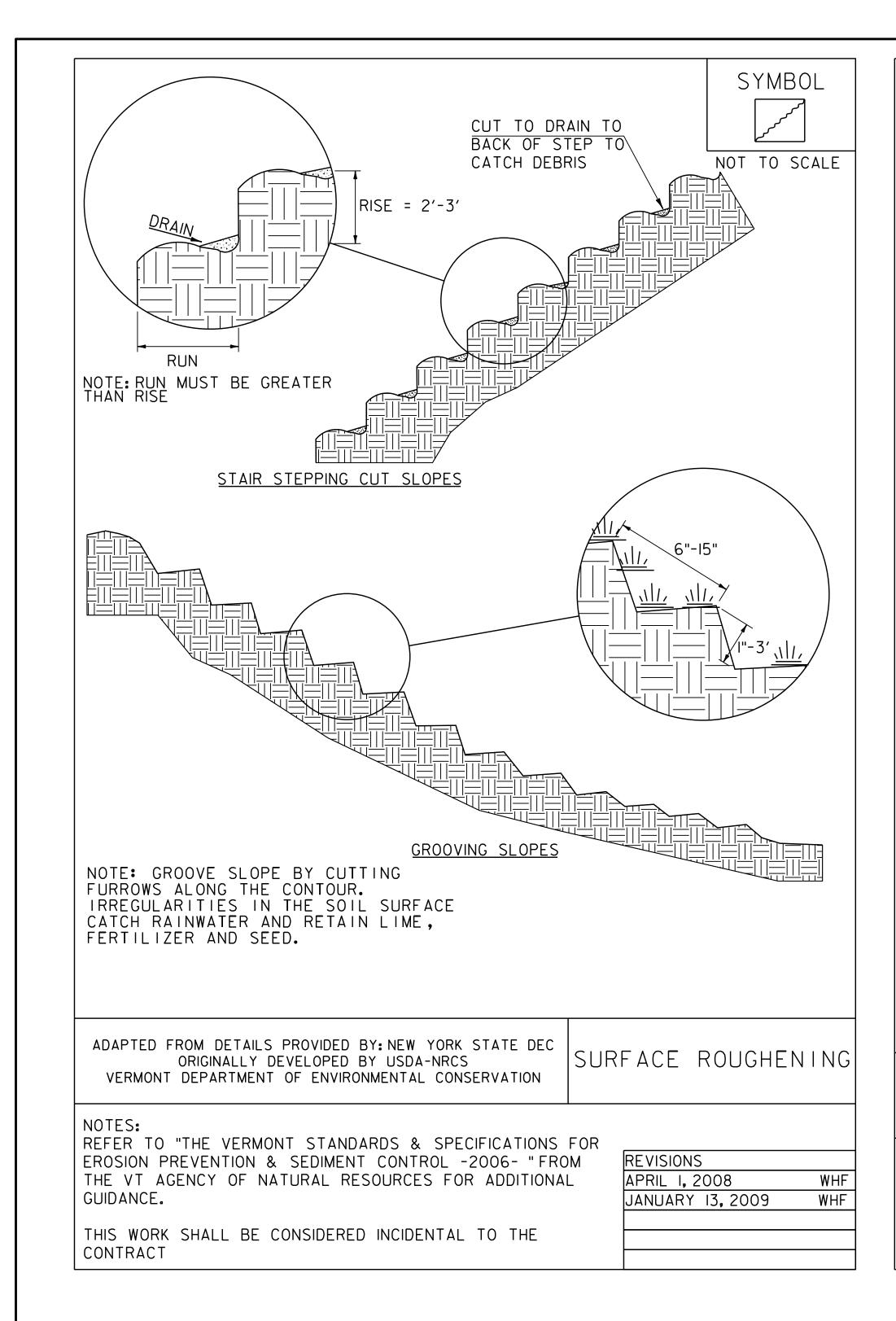
REVISIONS

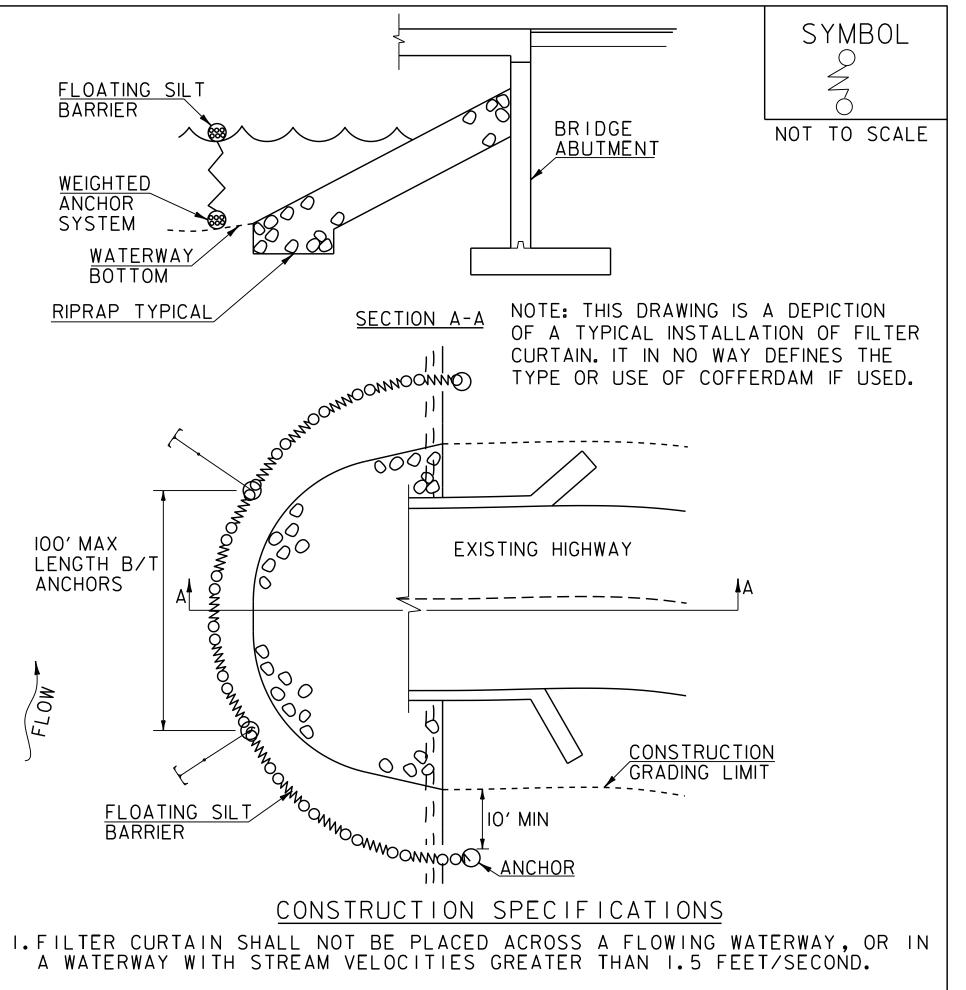
JANUARY 12, 2015 WHF

PROJECT NAME: WEYBRIDGE-NEW HAVEN PROJECT NUMBER: BF 032-1(19)

FILE NAME: sl2b552eronar.dgn
PROJECT LEADER: C.W. CARLSON
DESIGNED BY: D. PETERSON
EPSC DETAILS I

PLOT DATE: 20-APR-2017
DRAWN BY: M. LONGSTREET
CHECKED BY: D. PETERSON
SHEET 83 OF 85





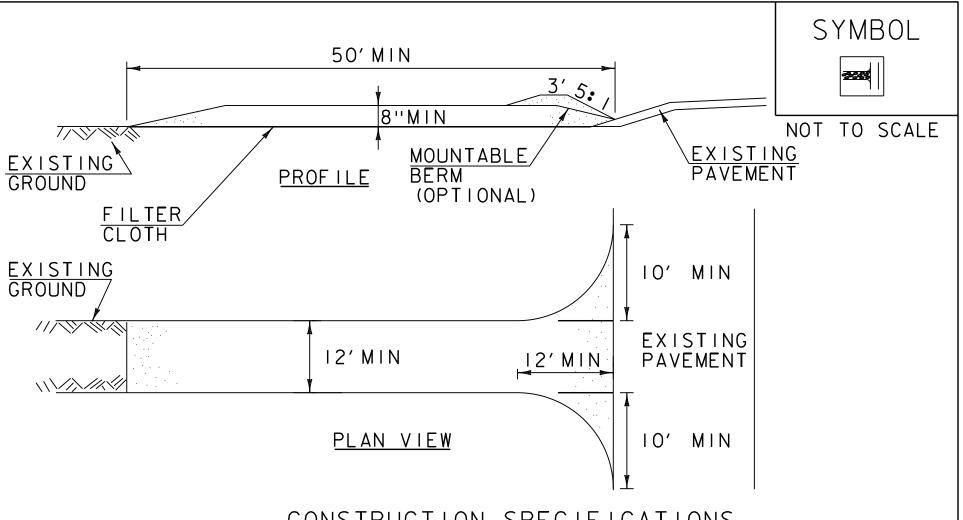
- 2. MAXIMUM 100' LENGTH BETWEEN ANCHORS.
- 3. LAST SECTION SHALL TERMINATE A MINIMUM OF 10' BEYOND LIMIT OF DISTURBANCE.
- 4. THE WEIGHTED ANCHOR SYSTEM SHALL BE A TYPE WHICH ALLOWS THE CURTAIN TO CONFORM TO THE BOTTOM OF THE WATERWAY.
- 5. THE CURTAIN SHALL BE REMOVED BY SLOWLY PULLING TOWARD THE SHORE MINIMIZING THE ESCAPE OF SEDIMENTS INTO WATERWAY.

FILTER CURTAIN

REVISIONS

THIS WORK SHALL BE PERFORMED IN ACCORDANCE WITH SECTION 649 FOR GEOTEXTILE FOR FILTER CURTAIN (PAY ITEM 649.61).

APRIL 1, 2008 WHF JANUARY 13, 2009 WHF SEPTEMBER 4.2009 WHF



#### CONSTRUCTION SPECIFICATIONS

- I.STONE SIZE- USE 1-4" STONE, RECLAIMED OR RECYCLED CONCRETE EQUIVALENT.
- 2.LENGTH- NOT LESS THAN 50' (EXCEPT ON A SINGLE RESIDENCE LOT WHERE A 30' MINIMUM LENGTH APPLIES).
- 3. THICKNESS- NOT LESS THAN 8".
- 4.WIDTH- 12' MINIMUM, BUT NOT LESS THAN THE FULL WIDTH AT POINTS WHERE INGRESS OR EGRESS OCCURS. 24' IF SINGLE ENTRANCE TO SITE.
- 5. GEOTEXTILE MUST BE PLACED OVER THE ENTIRE AREA PRIOR TO PLACING STONE.
- 6. SURFACE WATER- ALL SURFACE WATER FLOWING OR DIVERTED TOWARD CONSTRUCTION ENTRANCES SHALL BE PIPED BENEATH THE ENTRANCE. IF PIPING IS IMPRACTICAL, A MOUNTABLE BERM WITH 5: I SLOPES WILL BE PERMITTED.
- 7. MAINTENANCE THE ENTRANCE SHALL BE MAINTAINED IN A CONDITION WHICH WILL PREVENT TRACKING OR FLOWING OF SEDIMENT ONTO PUBLIC RIGHTS-OF-WAY, ALL SEDIMENT SPILLED, DROPPED, WASHED OR TRACKED ONTO PUBLIC RIGHTS-OF-WAY MUST BE REMOVED IMMEDIATELY.
- 8. WHEN WASHING IS REQUIRED, IT SHALL BE DONE ON AN AREA STABILIZED WITH STONE AND WHICH DRAINS INTO AN APPROVED SEDIMENT TRAPPING DEVICE.
- 9. PERIODIC INSPECTION AND NEEDED MAINTENANCE SHALL BE PROVIDED ACCORDING TO PERMIT REQUIREMENTS.

ADAPTED FROM DETAILS PROVIDED BY: NEW YORK STATE DEC ORIGINALLY DEVELOPED BY USDA-NRCS VERMONT DEPARTMENT OF ENVIRONMENTAL CONSERVATION

STABILIZED CONSTRUCTION ENTRANCE

NOTES:

REFER TO "THE VERMONT STANDARDS & SPECIFICATIONS FOR EROSION PREVENTION & SEDIMENT CONTROL -2006- "FROM THE VT AGENCY OF NATURAL RESOURCES FOR ADDITIONAL GUIDANCE.

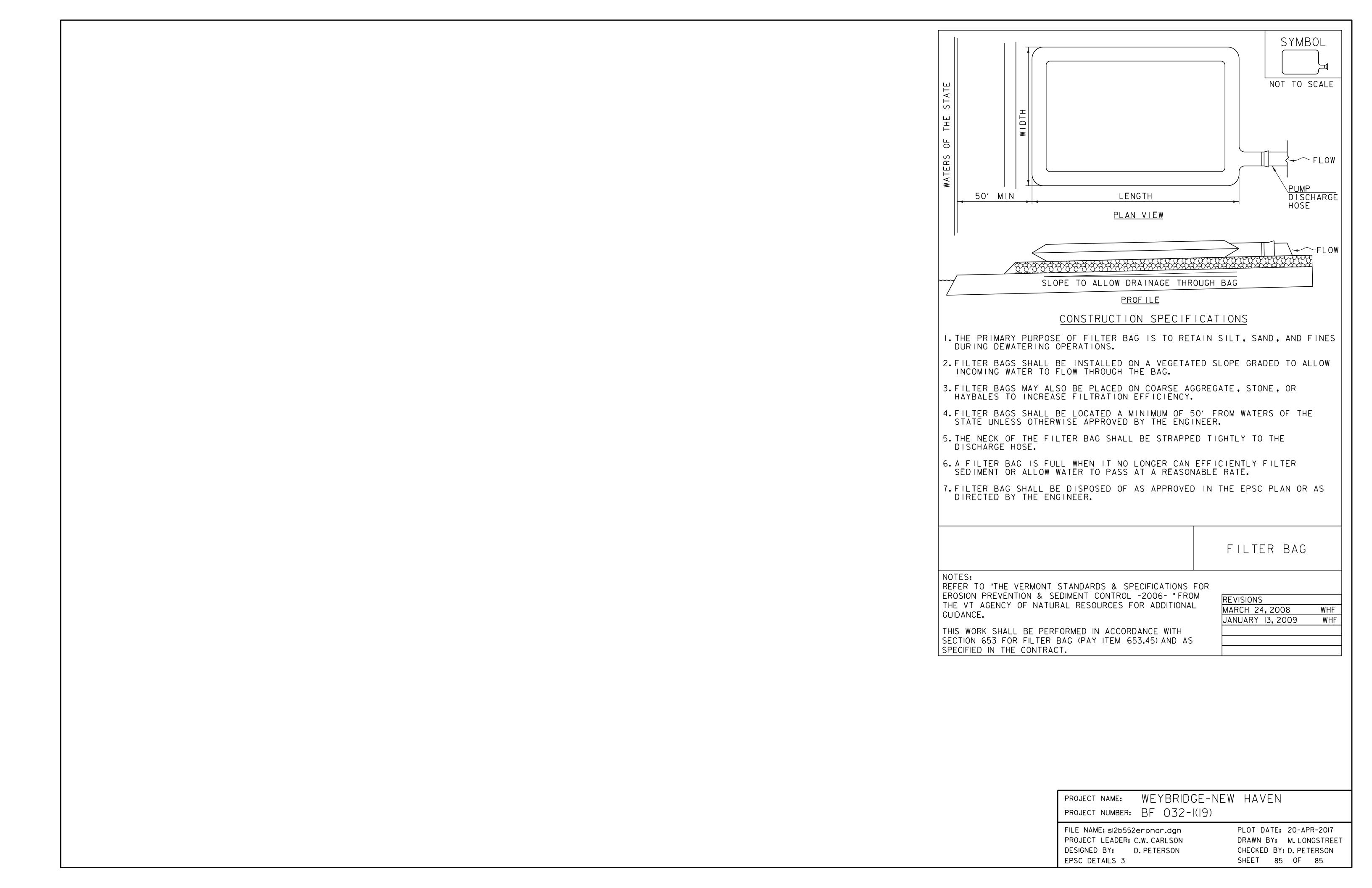
THIS WORK SHALL BE PERFORMED IN ACCORDANCE WITH SECTION 653 FOR VEHICLE TRACKING PAD (PAY ITEM 653.35) OR AS SPECIFIED IN THE CONTRACT.

REVISIONS MARCH 24, 2008 WHF JANUARY 13, 2009 WHF

WEYBRIDGE-NEW HAVEN PROJECT NAME: PROJECT NUMBER: BF 032-1(19)

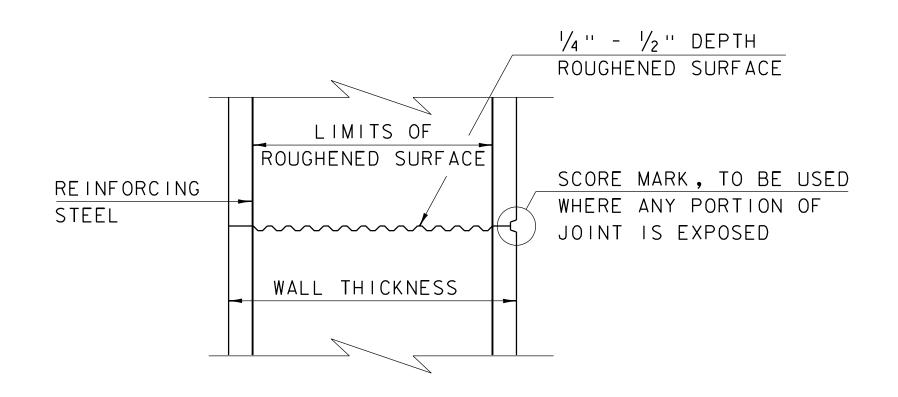
FILE NAME: sI2b552eronar.dgn PROJECT LEADER: C.W. CARLSON DESIGNED BY: D. PETERSON EPSC DETAILS 2

PLOT DATE: 20-APR-2017 DRAWN BY: M. LONGSTREET CHECKED BY: D. PETERSON SHEET 84 OF 85



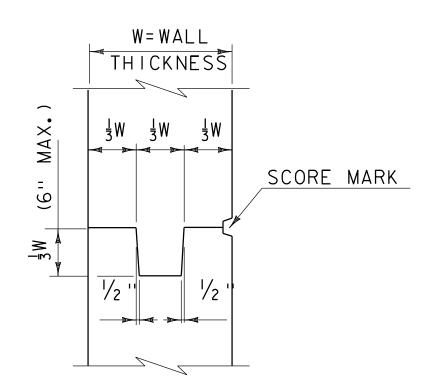
#### CONCRETE GENERAL NOTES

- I. ALL EXPOSED EDGES OF CONCRETE SHALL BE CHAMFERED I'' x I''
- 2. REINFORCING STEEL SIZE AND SPACING SHOWN IN THE PLANS IS BASED ON 60 KSI STEEL, UNLESS NOTED OTHERWISE. WITH THE ENGINEER'S PERMISSION, BAR SIZE AND SPACING MAY BE MODIFIED ACCORDING TO THE LATEST AASHTO LRFD BRIDGE DESIGN SPECIFICATION AND STRUCTURES DESIGN MANUAL WHEN USING HIGHER STRENGTH STEEL.

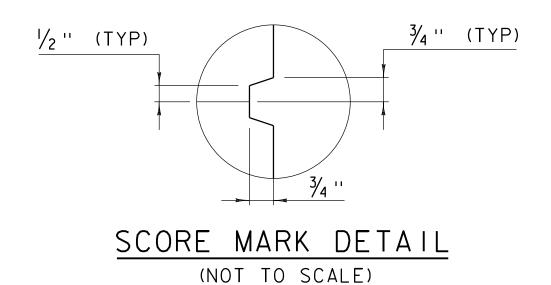


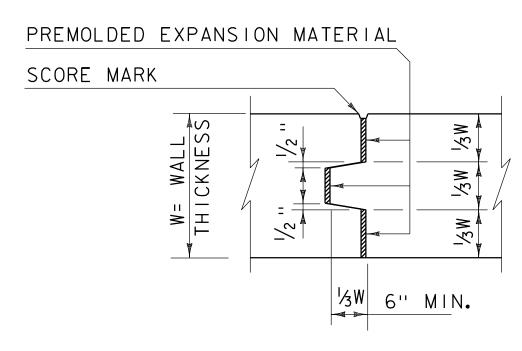
# TYPICAL HORIZONTAL CONSTRUCTION JOINT

- I. THE SURFACE OF THE CONCRETE CONSTRUCTION JOINTS SHALL BE CLEANED AND FREE OF LAITANCE.
- 2. IMMEDIATELY BEFORE NEW CONCRETE
  IS PLACED, ALL CONSTRUCTION JOINTS SHALL
  BE WETTED AND STANDING WATER REMOVED.

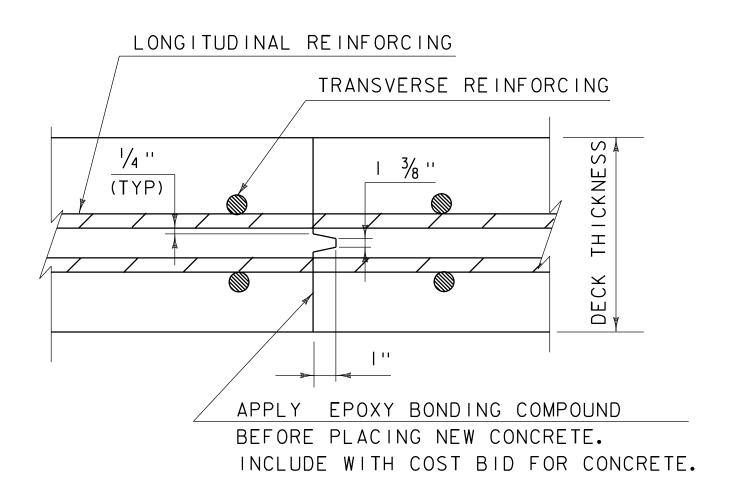


TYPICAL CONCRETE CONSTRUCTION JOINT
(NOT TO SCALE)





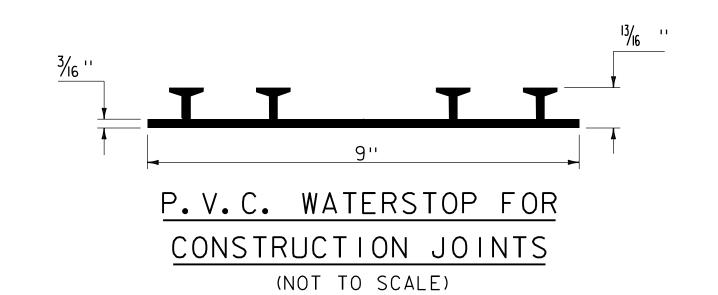
TYPICAL CONCRETE EXPANSION JOINT
(NOT TO SCALE)



TRANSVERSE BRIDGE SLAB

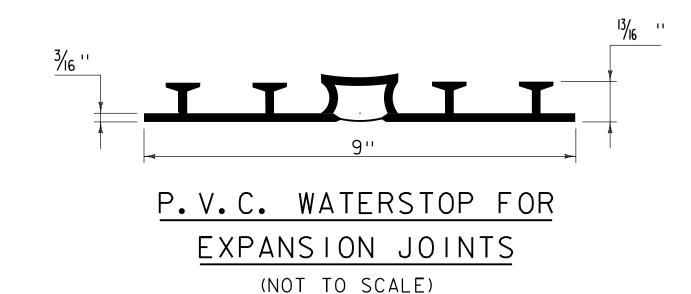
CONSTRUCTION JOINT DETAILS

(NOT TO SCALE)



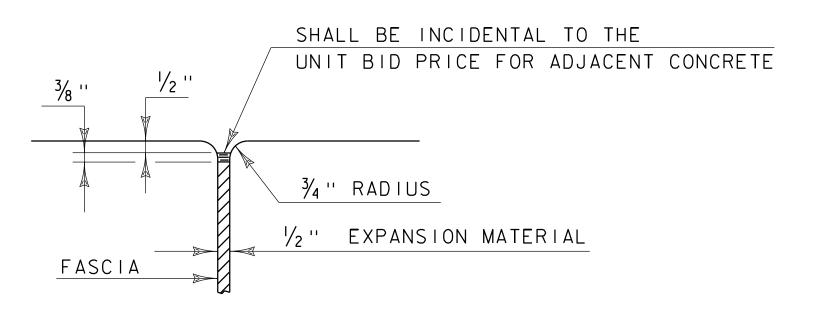
PAYMENT FOR THE P.V.C. WATERSTOP SHALL BE INCIDENTAL TO THE UNIT BID PRICE FOR THE ADJACENT CONCRETE.

OTHER CONFIGURATIONS OF WATERSTOP MAY BE USED UPON APPROVAL OF THE ENGINEER.



PAYMENT FOR THE P.V.C. WATERSTOP SHALL BE INCIDENTAL TO THE UNIT BID PRICE FOR THE ADJACENT CONCRETE.

OTHER CONFIGURATIONS OF WATERSTOP MAY BE USED UPON APPROVAL OF THE ENGINEER.



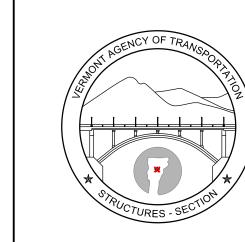
JOINT BETWEEN FASCIA

AND WINGWALL

(NOT TO SCALE)

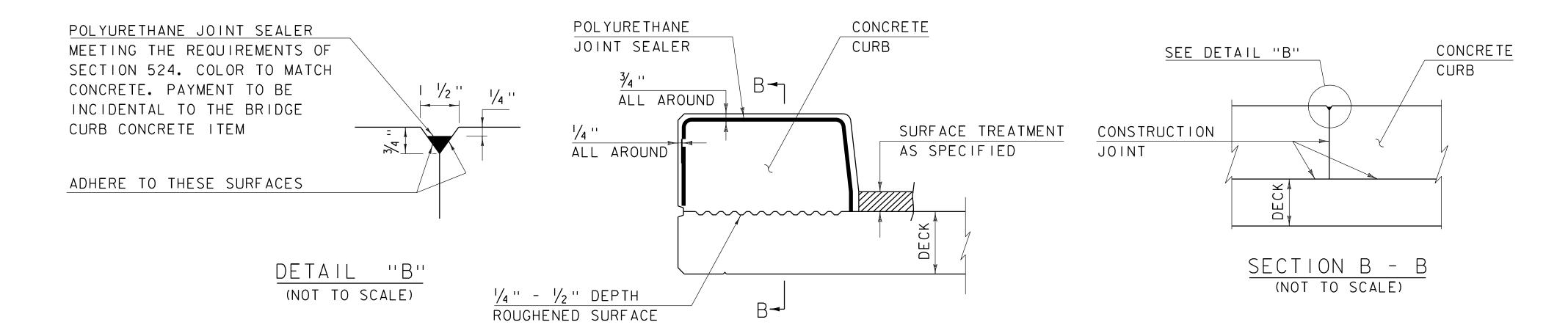
	REVISIONS
MAY 7, 2010	APPROVED FOR USE BY VAOT STRUCTURES SECTION
FEBRUARY 9, 2012	REBAR SUBSTITUTION ALLOWANCE ADDED TO CONCRETE GENERAL NOTES.

CONCRETE
DETAILS AND NOTES



# STRUCTURES DETAIL

SD-501.00

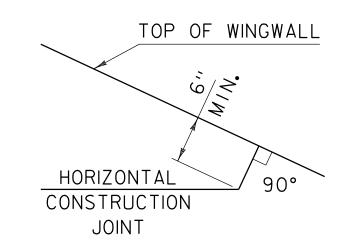


# CONCRETE CURB JOINT SECTION (NOT TO SCALE)

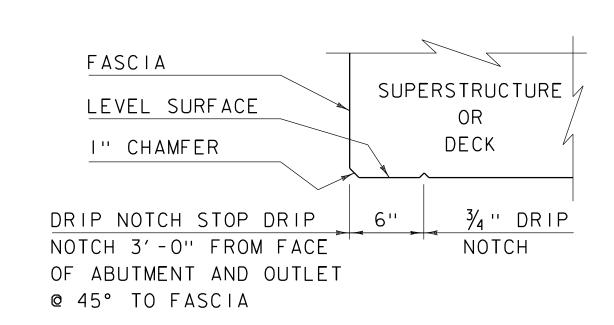
I. SEE TYPICAL HORIZONTAL CONSTRUCTION JOINT DETAIL FOR ADDITIONAL INFORMATION

#### CONCRETE CURB JOINT NOTES

- I. CONCRETE CURBS MAY BE PLACED IN ONE CONTINUOUS OPERATION IF AN APPROVED SHRINKAGE REDUCING ADMIXTURE LISTED IN THE SPECIAL PROVISIONS IS USED WITH THE CONCRETE MIX DESIGN. PAYMENT FOR THE SHRINKAGE REDUCING ADMIXTURE WILL BE INCIDENTAL TO THE BRIDGE CURB CONCRETE ITEM.
- 2. IF THE CONTRACTOR CHOOSES NOT TO USE AN APPROVED SHRINKAGE REDUCING ADMIXTURE, THE CURBS SHALL BE CONSTRUCTED WITH CONSTRUCTION JOINTS SPACED AT A MAXIMUM OF 15'-O" CENTER TO CENTER AND 2'-O" MINIMUM FROM THE CENTER OF NEAREST BRIDGE RAILING POST.
- 3. ON MULTI-SPAN CONTINUOUS SUPERSTRUCTURES, REGARDLESS OF WHETHER APPROVED SHRINKAGE REDUCING ADMIXTURE IS USED, CURB JOINTS SHALL BE LOCATED OVER THE CENTERLINE OF PIERS AND 7'-O" EACH SIDE OF THE CENTERLINE OF EACH PIER.
- 4. WHEN CURB JOINTS ARE USED THE CURBS SHALL BE PLACED IN ALTERNATE SECTIONS WITH A MINIMUM OF 48 HOUR DELAY BETWEEN ADJACENT PLACEMENTS.
- 5. LONGITUDINAL REINFORCING SHALL BE CONTINUOUS THROUGH CURB CONSTRUCTION JOINTS. CURB STIRRUP BARS SHALL BE TURNED AS NECESSARY TO MAINTAIN COVER IN THE FLARED CURB ENDS.
- 6. THE JOINT SPACING AND DETAILS SHOWN SHALL APPLY TO SIDEWALKS WHEN SHOWN IN THE PLANS.

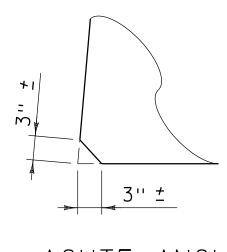


HORIZONTAL WINGWALL
CONSTRUCTION JOINT
(NOT TO SCALE)



DRIP NOTCH DETAIL

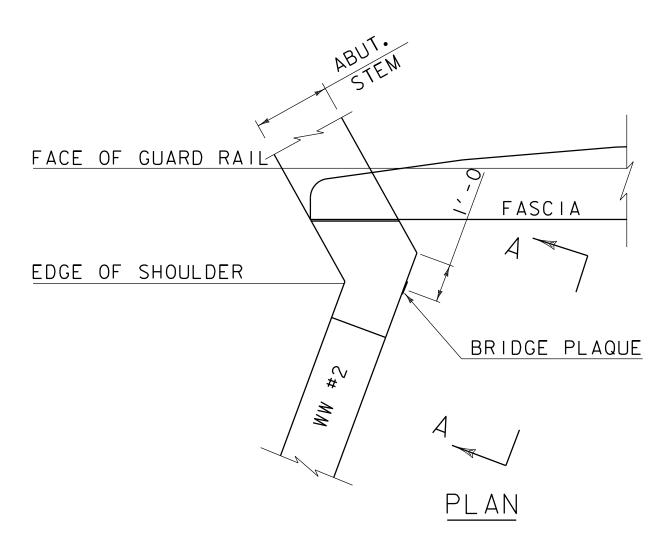
(NOT TO SCALE)

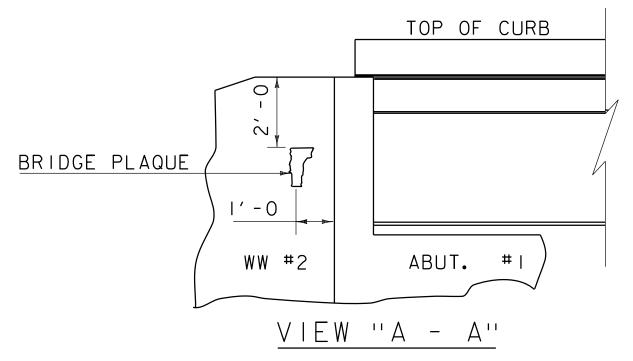


ACUTE ANGLE

CLIP DETAIL

(NOT TO SCALE)





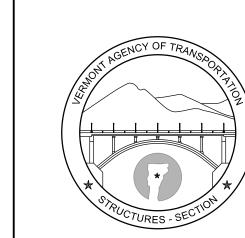
BRIDGE PLAQUE (NOT TO SCALE)

THE BRIDGE PLAQUE WILL BE SUPPLIED BY THE AGENCY OF TRANSPORTATION AND SHALL BE INSTALLED BY THE CONTRACTOR AT ABUTMENT #1 ON THE RIGHT SIDE AS SHOWN OR AS DIRECTED BY THE ENGINEER.

PAYMENT FOR INSTALLATION OF THE BRIDGE PLAQUE SHALL BE INCIDENTAL TO THE ADJACENT CONCRETE.

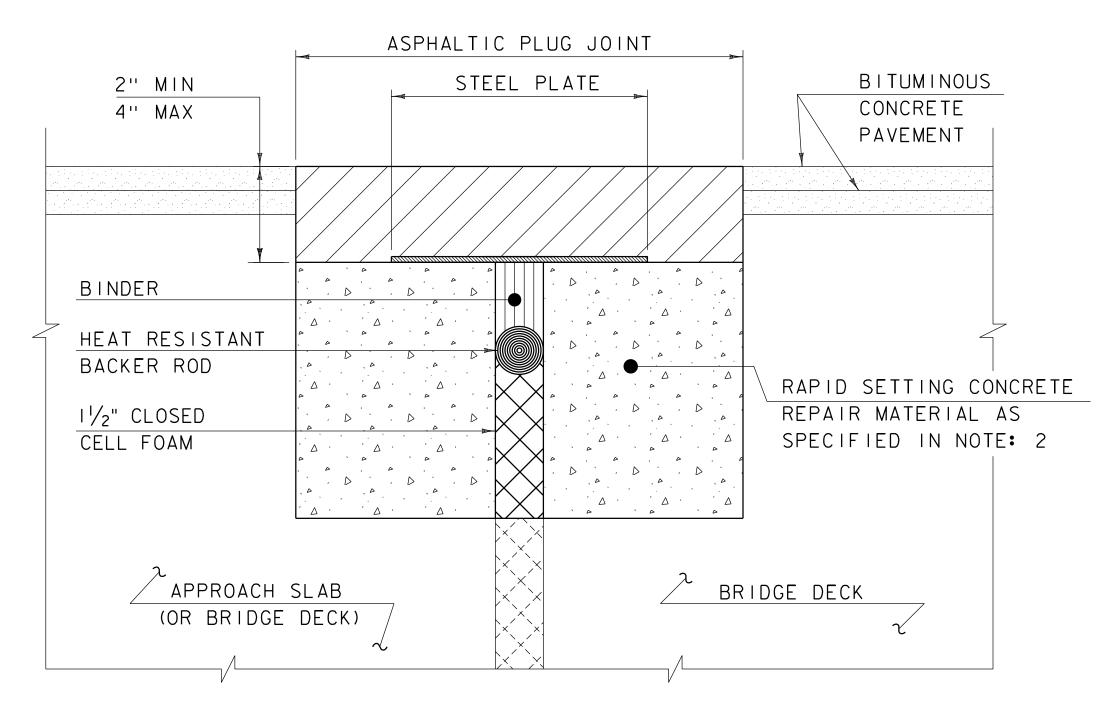
# MAY 7, 2010 APPROVED FOR USE BY VAOT STRUCTURES SECTION JUNE 4, 2010 MODIFIED AND ADDED TWO DETAILS OCTOBER 10, 2012 MODIFIED HORZ. JOINT WINGWALL ADD 6" MIN. DIMENSION

CONCRETE
DETAILS AND NOTES



# STRUCTURES DETAIL

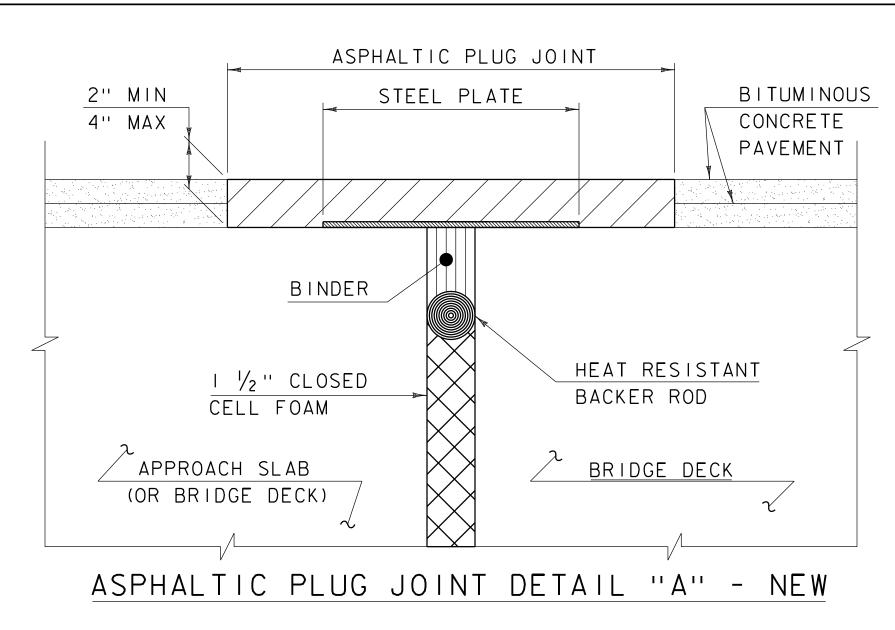
SD-502.00



#### ASPHALTIC PLUG JOINT DETAIL - REHAB

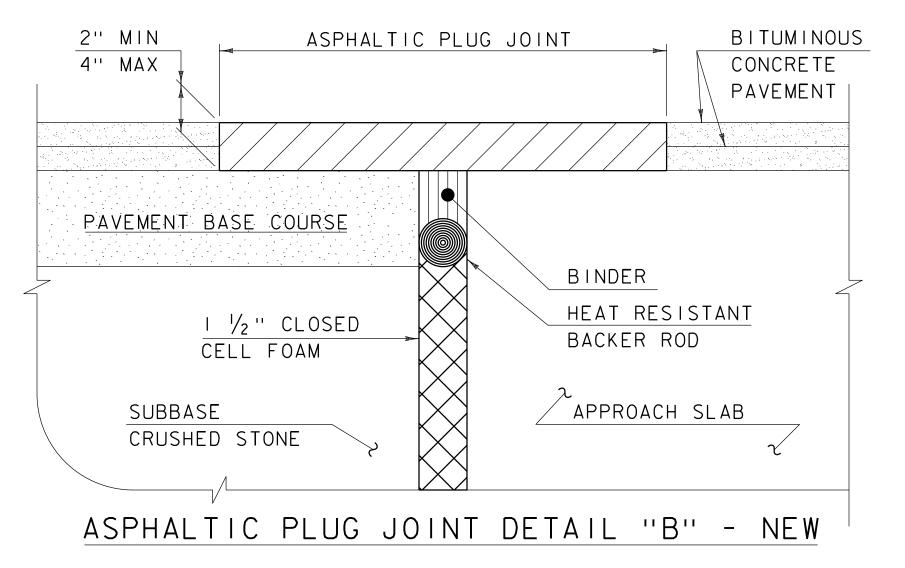
#### NOTES:

- I. THE CONTRACTOR SHALL REMOVE ALL ASPHALTIC PLUG JOINT MATERIAL AND DETERIORATED CONCRETE AS DIRECTED BY THE ENGINEER. REMOVAL OF THE FIRST 4 INCHES OF MATERIAL SHALL BE INCLUDED IN THE BID PRICE FOR ITEM 516.10 BRIDGE EXPANSION JOINT, ASPHALTIC PLUG. ANY REMOVAL OF MATERIAL GREATER THAN 4 INCHES SHALL BE INCLUDED IN THE BID PRICE OF ITEM 580.20 RAPID SETTING CONCRETE REPAIR MATERIAL WITH COARSE AGGREGATE.
- 2. THE CONTRACTOR SHALL REPLACE REMOVED MATERIAL THAT IS LESS THAN 4" FROM FINISHED GRADE WITH ASPHALTIC PLUG JOINT MATERIAL MEETING THE REQUIREMENTS OF SUBSECTION 707.15. ALL REMOVED MATERIAL THAT IS GREATER THAN 4 INCHES FROM FINISHED GRADE SHALL BE REPLACED WITH RAPID SETTING CONCRETE REPAIR MATERIAL WITH COARSE AGGREGATE MEETING THE REQUIREMENTS OF SUBSECTION 780.04.
- 3. REINFORCING STEEL NOT SHOWN FOR CLARITY.
- 4. PLACE 1/4" THICK BY 8" WIDE SECTIONS OF STEEL PLATE OVER THE CENTER OF THE MOVEMENT GAP. SECURE THE PLATES FROM MOVING BY INSERTING LOCATING PINS THROUGH THE PRE-STAMPED HOLES INTO BACKER ROD AND COVER WITH HOT BINDER. THE STEEL PLATES MAY BE OMITTED WHERE THE ENGINEER DETERMINES THAT THE APPROACH SLAB OR BRIDGE DECK WILL PROVIDE INADEQUATE SUPPORT AND WHERE VERTICAL MOVEMENT OF THE PLATES MIGHT OCCUR.



# NOTE: PLACE 1/4" THICK BY 8" WIDE SECTIONS OF STEEL PLATE OVER THE CENTER OF THE MOVEMENT GAP. SECURE THE PLATES FROM MOVING BY INSERTING LOCATING PINS THROUGH THE PRE-STAMPED

HOLES INTO BACKER ROD AND COVER WITH HOT BINDER.



#### ASPHALTIC PLUG JOINT NOTES

#### **INSTALLATION:**

- 1. LOCATE THE JOINT CENTRALLY OVER THE DECK OVERLAY EXPANSION GAP OR FIXED JOINT, MARKED OUT TO THE MANUFACTURER'S RECOMMENDED WIDTH.
- 2. REMOVE THE BITUMINOUS CONCRETE PAVEMENT FULL DEPTH AS SHOWN ON THE PLANS. THE PAVEMENT SHALL BE DRY AND SAW CUT TO THE LIMITS REQUIRED TO PLACE THE JOINT. A PNEUMATIC HAMMER AND CHISEL MAY BE USED ADJACENT TO THE CURB ONLY WHEN SAW CUTTING IS NOT POSSIBLE.
- BLAST CLEAN THE JOINT AREA OF DEBRIS, ASPHALT AND SHEET MEMBRANE.
  THOROUGHLY DRY THE JOINT AREA WITH COMPRESSED AIR PRIOR TO APPLYING BINDER MATERIAL.
- PLACE PROPERLY SIZED HEAT RESISTANT BACKER ROD IN THE MOVEMENT GAP ALLOWING FOR 1" +/- OF BINDER ABOVE THE ROD.
- 5. HEAT AND PLACE THE BINDER MATERIAL AS RECOMMENDED BY THE MANUFACTURER.
- 6. IMMEDIATELY AFTER TOP COATING, CAST AN ANTI-SKID MATERIAL OVER THE JOINT TO REDUCE THE RISK OF TRACKING.

#### WEATHER LIMITATIONS

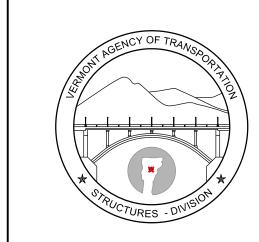
APPLY BINDER MATERIAL ONLY WHEN THE FOLLOWING CONDITIONS PREVAIL OR AS RECOMMENDED BY THE MANUFACTURER:

- 1. THE AMBIENT AIR TEMPERATURE IS AT LEAST 10 DEG C (50 DEG F) AND RISING.
- 2. THE ROAD SURFACE IS DRY.
- 3. WEATHER CONDITIONS OR OTHER CONDITIONS ARE FAVORABLE AND ARE EXPECTED TO REMAIN SO FOR THE PERFORMANCE OF SATISFACTORY WORK.

DETAILS ON THIS SHEET ARE NOT TO SCALE.

	REVISIONS
MAY 7, 2010	APPROVED FOR USE BY VAOT STRUCTURES SECTION
AUGUST 29, 2011	ADD DETAIL "B" AND REV. NOTES

BRIDGE JOINT
ASPHALTIC PLUG

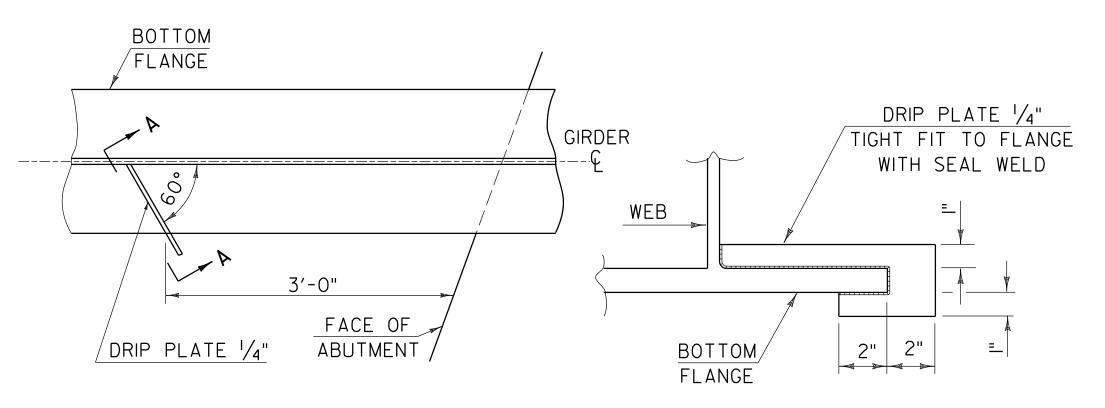


STRUCTURES DETAIL

SD-516.10

#### STRUCTURAL STEEL GENERAL NOTES:

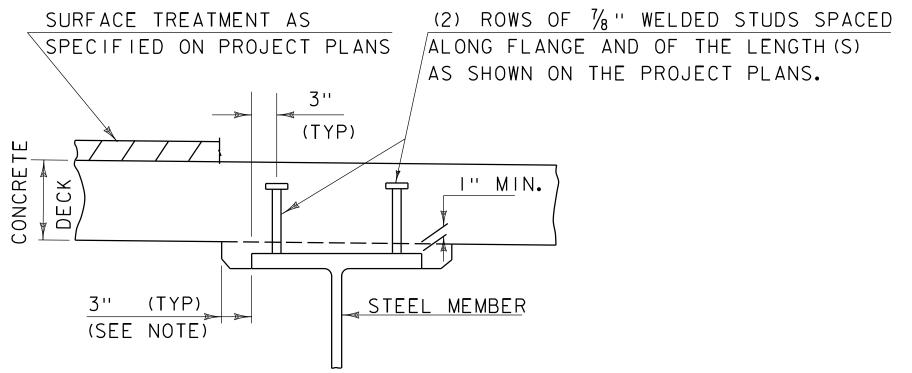
- I. ALL FIELD CONNECTIONS SHALL BE MADE WITH 7/8" DIAMETER HIGH-STRENGTH BOLTS IN 15/16" DIAMETER HOLES, PER SUBSECTION 506.19, UNLESS OTHERWISE SPECIFIED.
- 2. ALL HOLES IN THE WEBS OF THE FASCIA GIRDERS THAT ARE NOT OTHER-WISE FILLED, SHALL BE FILLED WITH EITHER BUTTON HEAD OR HEX HEAD BOLTS. THESE BOLTS SHALL BE TIGHTENED IN ACCORDANCE WITH SUBSECTION 506.19.
- 3. ALL WELDING SHALL CONFORM TO THE PROVISIONS OF SUBSECTION 506.10.
- 4. ANY CONNECTIONS THAT ARE NOT DETAILED ON THE PLANS SHALL BE DETAILED BY THE FABRICATOR AND SUBMITTED TO THE STRUCTURES ENGINEER FOR APPROVAL.
- 5. STRUCTURAL STEEL MEMBERS DESIGNATED "CVN" IN THE PLANS SHALL BE CHARPY V-NOTCH TESTED IN ACCORDANCE WITH SUBSECTION 714.01 OF THE STANDARD SPECIFICATIONS.
- 6. ENDS OF GIRDERS ARE TO BE VERTICAL IN THEIR FINAL POSITION.
- 7. AFTER SUPERSTRUCTURE STEEL HAS BEEN ERECTED, ELEVATIONS ALONG THE TOP OF THE GIRDERS SHALL BE TAKEN AS DIRECTED BY THE RESIDENT ENGINEER FOR USE IN DETERMINING FINISHED GRADES.



#### PLAN DRIP PLATE

SECTION A - A

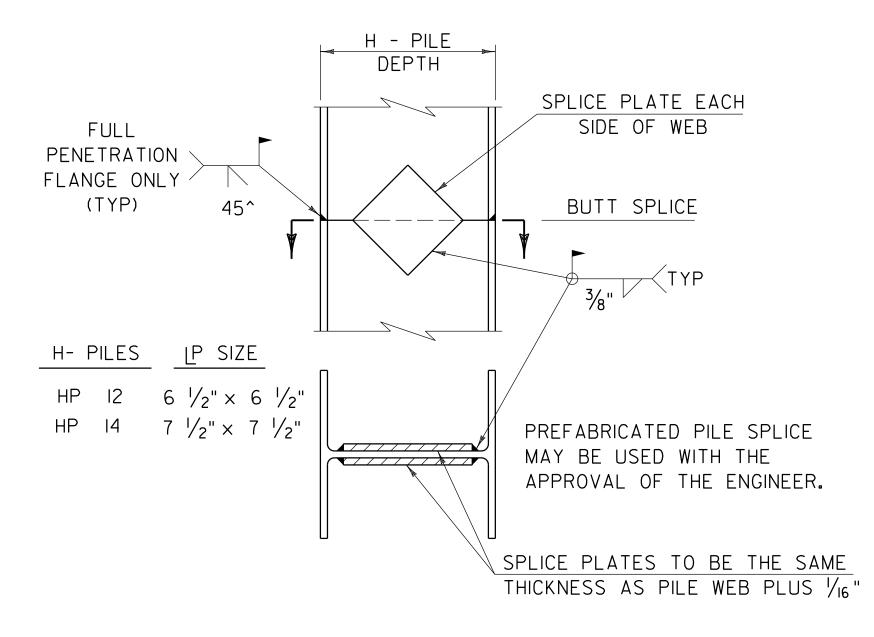
NOTE: DRIP PLATES SHALL BE PLACED ON OUTSIDE EDGE OF FASCIA GIRDERS ON THE HIGH SIDE OF ALL PIERS AND ABUTMENTS OR AS INDICATED ON PROJECT PLANS.



#### NOTE:

THE 3" HORIZONTAL SECTION MAY BE ELIMINATED FOR FORMING SYSTEMS DESIGNED FOR THE CONSTRUCTION OF VERTICAL HAUNCHES. ANY VOIDS RESULTING FROM FORMING SYSTEM ELEMENTS SHALL BE FILLED WITH JOINT SEALER, POLYURETHANE MEETING THE REQUIREMENTS OF SECTION 524. THE COST OF THE JOINT SEALER, POLYURETHANE SHALL BE INCIDENTAL TO THE ADJACENT CONCRETE.

HAUNCH AND SHEAR CONNECTOR DETAIL

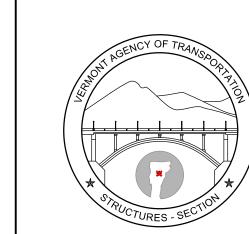


#### DETAIL OF PILE SPLICE

DETAILS ON THIS SHEET ARE "NOT TO SCALE" UNLESS NOTED OTHERWISE

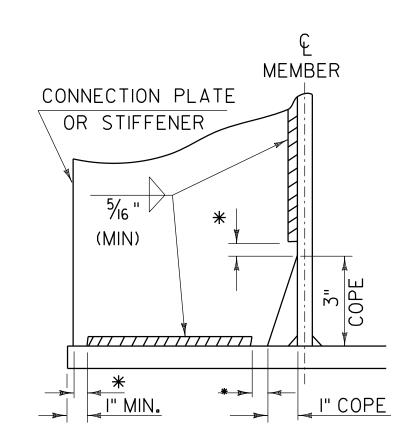
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# STRUCTURAL STEEL DETAILS & NOTES



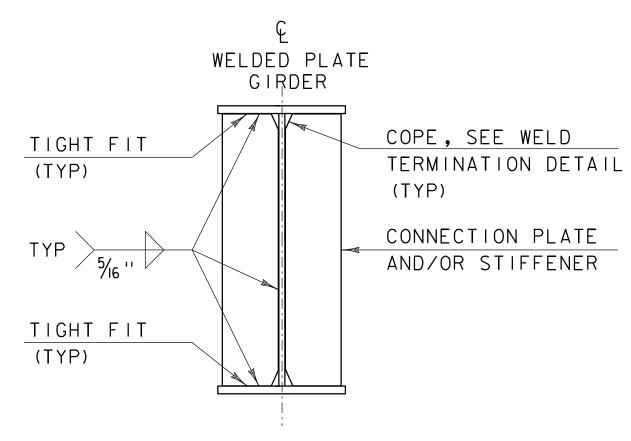
# STRUCTURES DETAIL

SD-601.00



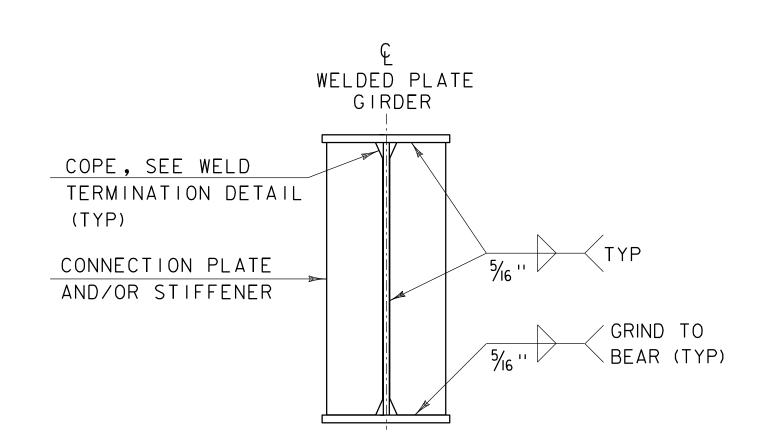
# WELD TERMINATION AND COPING DETAILS FOR STEEL MEMBERS

*NO WELD FOR 3/8" MIN. 7/8" MAX. (EXCEPT MUST MAINTAIN I" MINIMUM FROM EDGE OF FLANGE)



# INTERMEDIATE CONNECTION PLATES AND/OR STIFFENERS FOR WELDED PLATE GIRDERS

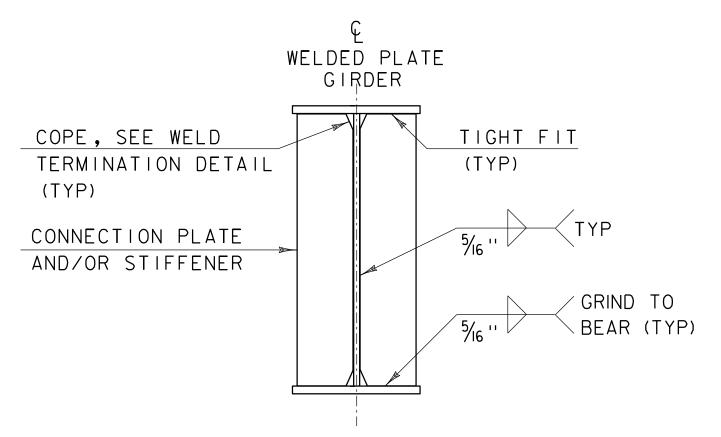
INTERMEDIATE DETAIL IS ONLY USED WHEN PLATE DOES NOT OCCUR AT AN ABUTMENT OR PIER.



ABUTMENT BEARING STIFFENERS

AND/OR CONNECTION PLATES

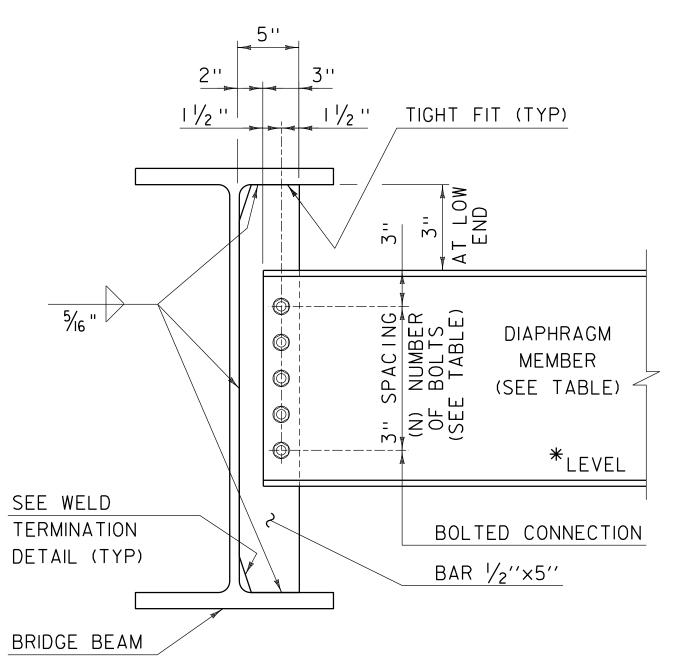
FOR WELDED PLATE GIRDERS



PIER BEARING STIFFENERS

AND/OR CONNECTION PLATES

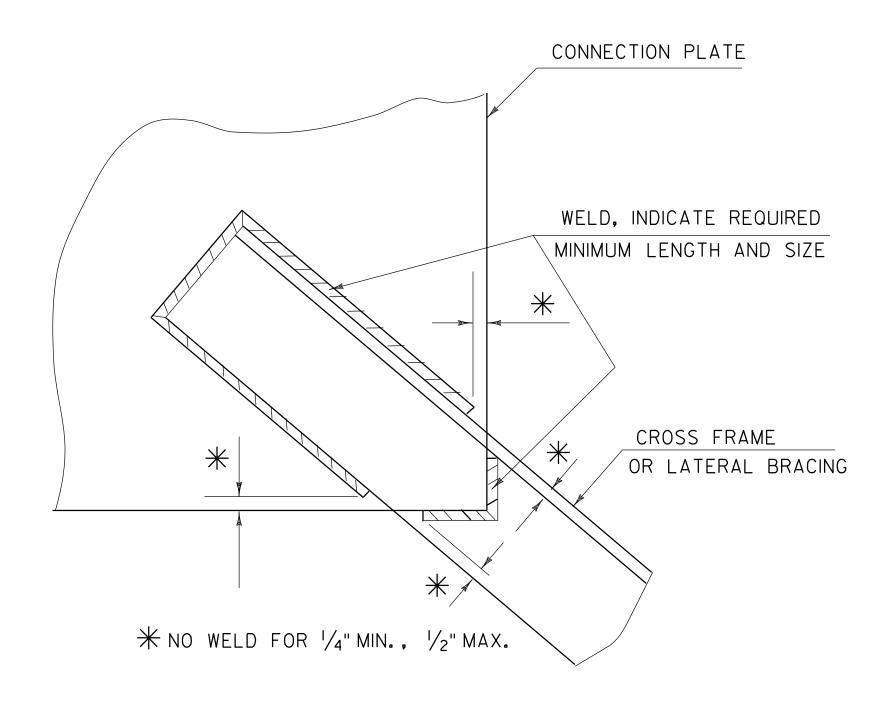
FOR WELDED PLATE GIRDERS



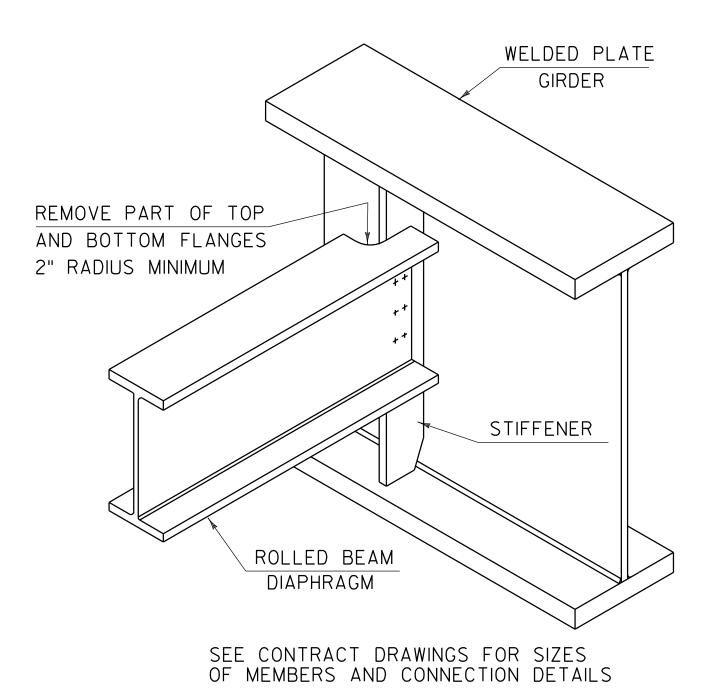
	DEPTH		DIAPHRAGM MEMBER	(N) BOLTS
	SEAM	24'' 30''}	CI5×33.9	4
	ROLLED BEAM	3l'' 36''}	MCI8×42.7	5
	ROL	37'' 42''}	W2I×44	6
	PLATE GIRDER WEB	31'' 36''	W27×84	7
		37'' 42''}	W33×118	9
		43'' 48''	W36×I35	10



* IF CLEARANCE CANNOT BE MET, DIAPHRAGM MAY BE SLOPED.



WELD LOCATION DETAIL AT CROSS FRAMES AND LATERAL BRACING

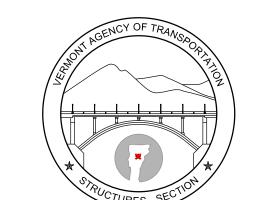


ROLLED BEAM USED AS DIAPHRAGM

DETAILS ON THIS SHEET ARE "NOT TO SCALE" UNLESS NOTED OTHERWISE.

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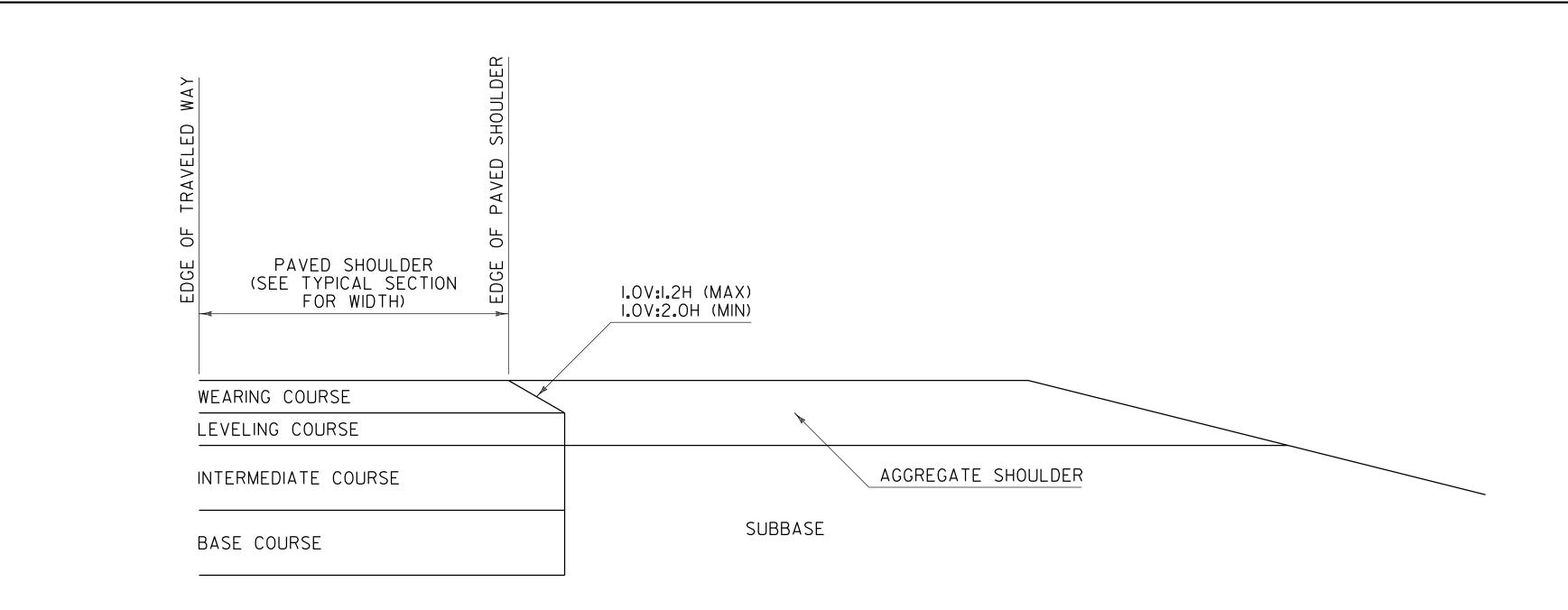
STRUCTURAL STEEL PLATE
GIRDER DETAILS AND NOTES



# STRUCTURES

DETAIL

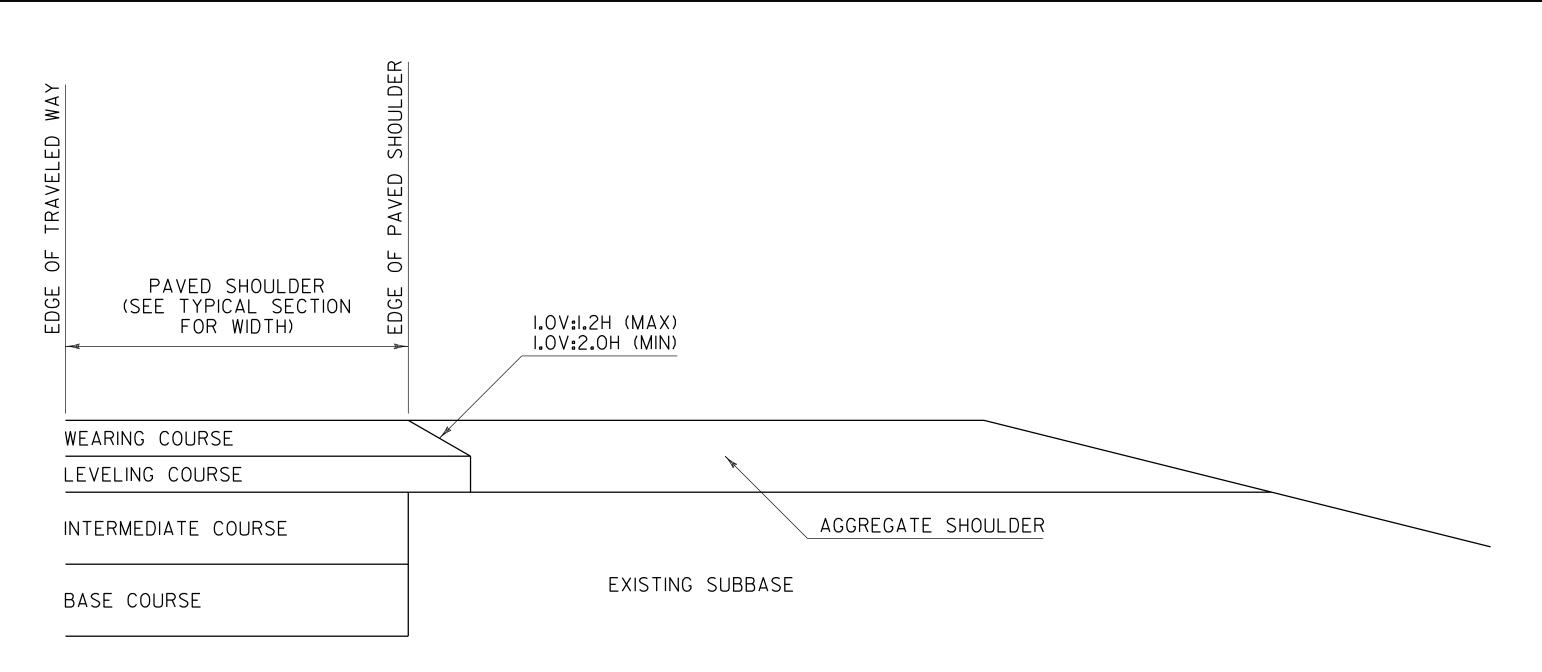
SD-602.00



#### **NOTES:**

### SAFETY EDGE DETAIL FOR PAVING BELOW WEARING COURSE

- I. THIS DETAIL IS INTENDED FOR WHEN PAVING EXTENDS BELOW THE WEARING COURSE.
- 2. PRIOR TO PLACEMENT OF THE LEVELING AND/OR WEARING COURSE, THE SUBBASE LOCATED BENEATH THE AGGREGATE SHOULDER SHALL BE PREPARED FLUSH WITH THE BOTTOM OF THE LEVELING COURSE.
- 3. BASE COURSE LIMITS MAY VARY, SEE TYPICAL SECTIONS FOR WIDTH.



### SAFETY EDGE DETAIL FOR PAVING WEARING COURSE ONLY

#### **NOTES:**

- I. THIS DETAIL IS INTENDED FOR WHEN ONLY THE LEVELING AND/OR WEARING COURSE IS TO BE PLACED.
- 2. PAVEMENT COURSES MAY VARY, SEE TYPICAL SECTIONS FOR ACTUAL PAVEMENT COURSES REQUIRED.

REV.	DATE	DESCRIPTION
0	MAR. 29, 2016	ORIGINAL APPROVAL
OTHER	DETAILS REQUIRED	NONE
	DETAILS APPROVED	FOR USE BY HIGHWAY SAFETY & DESIGN

## SAFETY EDGE DETAILS

SAFETY EDGE WIDTH COURSE THICKNESS	BASED ON WEARING AND A IV:1.6H SLOPE
WEARING COURSE THICKNESS (INCHES)	NOMINAL SAFETY EDGE WIDTH (INCHES)
I <b>.</b> 25	2.000
I <b>.</b> 50	2.375
I <b>.</b> 75	2.750
2.00	3.125
2.25	3.500
2.50	4.000

#### **GENERAL NOTES:**

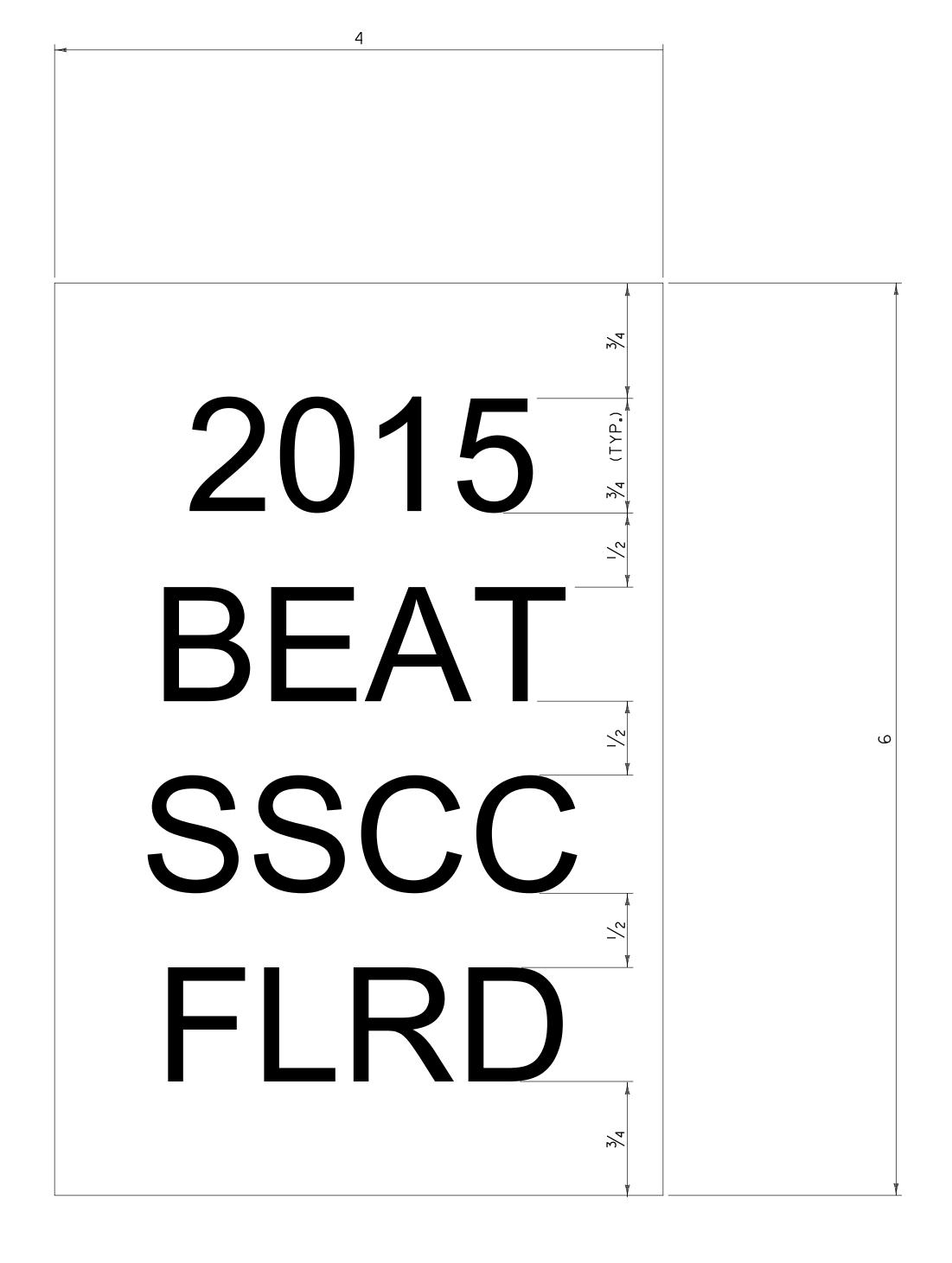
- I. PLACEMENT OF THE WEARING COURSE SHALL INCLUDE THE SAFETY EDGE, UNLESS THE FOLLOWING APPLIES:
  - A. THE ADJACENT SLOPE IS STEEPER THAN THE SAFETY
  - THE EDGE OF PAVEMENT BEING PLACED ABUTS BOUND MATERIAL.
  - C. VEHICLES ARE RESTRICTED FROM LEAVING THE PAVED SURFACE (EXAMPLE: GUARDRAIL).
- 2. THE SAFETY EDGE SHALL BE FORMED IN SUCH A WAY THAT THE BITUMINOUS CONCRETE PAVEMENT IS EXTRUDED OR COMPRESSED TO FORM THE SLOPE. DEVICES THAT SIMPLY STRIKE-OFF THE MIX WITHOUT PROVIDING ANY COMPACTIVE EFFORT WILL NOT BE ALLOWED.
- 3. THE SAFETY EDGE SHALL NOT BE CONSIDERED PART OF THE PAVED SHOULDER.
- 4. THIS WORK SHALL BE INCIDENTAL TO THE RESPECTIVE BITUMINOUS CONCRETE PAVEMENT ITEM.



HIGHWAY SAFETY

& DESIGN DETAIL

HSD-400.01



#### **GENERAL NOTES:**

- I. LINE ONE SHALL INDICATE THE INSTALLATION YEAR (YYYY).
- 2. LINE TWO SHALL INDICATE THE MODEL AS IDENTIFIED ON THE APPROVED PRODUCTS LIST. FOR GENERIC INSTALLATIONS THE STANDARD DRAWING DESIGNATION OR NAME AS IDENTIFIED IN THE FHWA ELIGIBILITY LETTER SHALL BE USED.
- 3. LINE THREE SHALL SHALL INDICATE ADDITIONIAL MODEL INFORMATION IF NECESSARY.
- 4. LINE FOUR SHALL INDICATE FLARED (FLRD) OR TANGENT (TANG).
- 5. LEGEND SHALL BE ONE ARIEL FONT.
- 6. LEGEND SHALL BE BLACK ON A WHITE BACKGROUND, LENGEND AND BACKGROUND SHALL NOT BE REFLECTIVE.
- 7. SUITABLE MATERIAL SHALL BE USED SO AS TO NOT DETERIORATE DURING EXPOSURE TO WEATHER.
- 8. LABELS SHALL BE APPLIED IN SUCH A WAY THAT THEY REMAIN INTACT DURING THE LIFE OF THE TERMINAL.
- 9. FOR W-BEAM GUARDRAIL, LABEL SHALL BE PLACED ON THE TOP OF POST ONE FACING AWAY FROM TRAFFIC.
- IO. FOR BOX BEAM GUARDRAIL, LABEL SHALL BE PLACED ON THE BOX BEAM ADJACENT TO POST ONE FACING AWAY FROM TRAFFIC.
- II. PAYMENT SHALL BE INCIDENTAL TO OTHER TRAFFIC BARRIER ITEMS.
- 12. ALL DIMENSIONS IN INCHES.

HIGHWAY SAFETY

& DESIGN DETAIL

HSD-621.06

DESCRIPTION DATE NOV. 3, 2015 ORIGINAL APPROVAL OTHER DETAILS REQUIRED: NONE DETAILS APPROVED FOR USE BY HIGHWAY SAFETY & DESIGN

GUARDRAIL TERMINAL LABEL DETAIL