# UDC Appendix

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Wisconsin Division of Safety and Buildings			WISC	ONSIN U PERMIT	NIFORM APPLICA	BUILD TION	ING		Ap	plicat	ion No	).		-(
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### DEPARTMENT OF COMMERCE

The owner, builder or agents shall complete the application form down through the Signature of Applicant block and submit it and building plans and specifications to the enforcing municipality. Permit application data is used for statewide statistical gathering on new one- and two-family dwellings, as well as for local code administration. PERMIT REOUESTED Check off type of Permit Requested, such as structural, HVAC, Electrical or Plumbing. • Fill in owner's current Mailing Address and Telephone Number. PROJECT LOCATION Fill in Building Address (number and street or sufficient information so that the building inspector can locate the construction site. • Fill in Contractor Information. Note, per s. 101.63 (7) Wis. Stats., that the master plumber name and number must be entered before issuing a plumbing permit. • Local zoning, land use and flood plain requirements must be satisfied before a building permit can be issued. County approval may be necessary. • Fill in Zoning District, lot area and required building setbacks. PROJECT DATA - Fill in all numbered project data blocks (1-14) with the required information. All data blocks must be filled in, including the following: 2. Area (involved in project): Basements - include unfinished area only Living area - include any finished area including finished areas in basements Drung area – include any finished area including finished areas in basements Two-family dwellings – include total combined areas
 Occupancy – Check only "Single-Family" or "Two-Family" if that is what is being worked on. In other words, do not check either of these two blocks if only a new detached garage is being built, even if it serves a one or two family dwelling. Instead, check "Garage" and number of stalls. If the project is a community based residential facility serving 3 to 8 residents, it is consid-ended the time. ered a single-family dwelling. HVAC Equipment - Check only the major source of heat, plus central air conditioning if present. Only check "Radiant Baseboard or Panel" if there is no central source of heat. 10. Plumbing – A building permit cannot be issued until a county sanitary permit has been issued for any new of affected existing on-site sewage system. 14. Estimated Cost - Include the total cost of construction, including materials and market rate labor, but not the cost of land or landscaping. SIGNATURE - Sign and date this application form. CONDITIONS OF APPROVAL - The authority having jurisdiction uses this section to state any conditions that must be complied with pursuant to issuing the building permit. ISSUING JURISDICTION: This must be completed by the authority having jurisdiction. Check off Municipality Status, such as town, village or city. Fill in Municipality Name and Municipality Number of inspection authority. Fill in Municipality Number of Dwelling Location if different from municipality where inspection authority is located. (applies to county or state enforcement) Check off type of Permit Issued, such as construction, HVAC, electrical or plumbing. Fill in Wisconsin Uniform Permit Seal Number, if project is a new one- or two-family dwelling. Fill in Name and Inspector Certification Number of person reviewing building plans and date building permit issued. PLEASE RETURN YELLOW COPY WITHIN 30 DAYS AFTER ISSUANCE TO (You may fold along the dashed lines and insert this form into a window envelope.): Safety & Buildings Division P O Box 2509 Madison, WI 53701-2509

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### CAUTIONARY STATEMENT TO OWNERS OBTAINING BUILDING PERMITS

101.65 (1r) of the Wisconsin Statutes requires municipalities that enforce the Uniform Dwelling Code to provide an owner who applies for a building permit with a statement advising the owner that:

If the owner hires a contractor to perform work under the building permit and the contractor is not bonded or insured as required under s. 101.654 (2) (a), the following consequences might occur:

(a) The owner may be held liable for any bodily inquiry to or death of others or for any damage to the property of others that arises out of the work performed under the building permit or that is caused by any negligence of the contractor that occurs in connection with the work performed under the building permit.

(b) The owner may not be able to collect from the contractor damages for any loss sustained by the owner because of a violation by the contractor of the one- and 2- family dwelling code or an ordinance enacted under sub. (1) (a), because of any bodily injury to or death of others or damage to the property of others that arises out of the work performed under the building permit or because of any bodily injury to or death of others or damage to the property of others that is caused by any negligence by the contractor that occurs in connection with the work performed under the building permit.

SBD-5823 (R.05/98)

Register, February, 1999, No. 518

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Register, February, 1999, No. 518

DEPARTMENT OF COMMERCE

Submit to non-enforcing municipalities for new 1and 2- family dwellings

WISCONSIN ADMINISTRATIVE BUILDING PERMIT APPLICATION

State of Wisconsin Safety and Buildings Division

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## (Wis. Stats. 101.63 (7) & 101.65 (3))

EE INSTRUCTIONS ON BACK OF YELLOW COPY.

Personal information you provide may be used for secondary purposes. [Privacy Law 15.04(1)(m)]

PERMIT APPLICAN	VT		的话言题》中 第1993年199				
Last Name	Fir	st Name			Middle Initia	I	
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City		State	Zip Code	Tele	ephone No. (In	clude area c	ode)
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PROJECTLOCATI	ON					Steer State Sciences	
Building Address	<u> </u>	S	ubdivision I	Name	$\overline{a}$	Lot #	Block #
							44 1
Legal Description	· · · · · · · · · · · · · · · · · · ·			Par	el No.		
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□ 1 Family □ 2 Family	Boiler		niral AC	baru or Pane	51 51		rump
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Space Hea	ating						<u> </u>
Water He	ating						
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Living area =	Square Fe	et \$					
I youch that all the ab	ove information is correc	t, and understa	nd that the iss	uance of this	permit is for ad	ministrative p	ourposes only. I
understand that onsite	construction inspections	will not be per	formed by th	e municipalit	y, but that the U	iniform Dwell	ling Code, and that the
issuance of this permi	it does not relieve me of o	compliance with	h other applic	able codes a	d ordinances.	itti. Tulideist	
Applicant's Signati	ure		<del></del>	Date Sign	ied		
MUST BE COMPLETED BY	THE MUNICIPALITY REF	ORE FORWARD	ING PINK PL	TO THE STA	TE DIVISION OF	SAFETY AND	BUILDINGS
TSSUINCE TERISDI		wn 🗆 Villag	e 🗆 City D	County of			
ATTNICTO ATTPINZA		#			136	RES.	
of Dwelling Location	CALDUAN	···					
PERMIT ISSUED F	X	-			D	ATE	
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SBD-8254 (R 01/98)	White - Issuing Jurisdic	tion Pink -	State Withir	30 Days	Yellow -	Applicant	• (

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### INSTRUCTIONS

The owner, builder or agent shall complete and provide all required information on the application form down through the Signature of Applicant block. This data is used for statewide statistical gathering on new one- and two-family dwellings, as well as for local administration. When completed, submit to local municipality having jurisdiction. Plan review or building inspections will not be performed by the municipality.

### PERMIT REQUESTED:

- Fill in building address.
- Fill in legal description of lot, subdivision name, lot number and block number.

### **PROJECT DATA:**

- Fill in **all numbered** project data blocks (1–7) with the required information. All data blocks must be filled in, including the following:
  - 1. **Type** Check only "1–Family" or "2–Family" if that is what is being built. In other words, do NOT use this form if only a new detached garage is being built, even if it serves a one or two family dwelling.
  - HVAC Equipment Check only the major source of heat, not any supplemental sources. Mark central air conditioning if present. Only check "Radiant Baseboard or Panel" if there is no central source of heat.

6. Living Area – Include any finished area including finished areas in basements. For two– family dwellings, include total combined areas.

7. Estimated Cost – Include the total cost of construction, but not cost of land or land-scaping.

### SIGNATURE:

or

• Sign and date application form.

## **ISSUING JURISDICTION – This must be completed by the AUTHORITY HAVING JURISDICTION.**

Check off MUNICIPALITY STATUS of issuing jurisdiction, such as town, village, city county.

Fill in MUNICIPALITY NUMBER OF DWELLING LOCATION. If issued by a county, indicate the specific municipality number where the dwelling will be built.

Fill in name of person issuing permit and date building permit issued.

PLEASE RETURN PINK COPY WITHIN 30 DAYS AFTER ISSUANCE TO (You may fold along the dashed lines and insert this form into a window envelope.):

Safety & Buildings Division P O Box 2509 Madison, WI 53701-2509 ì

	:	NOTICE O	F NONCOMPLIAN	CE	
Report Date:	······································	Inspection Date	Permit No.:	Parcel No:	
Project Addre	55		Subdivision	Lot No.:	Block No.:
Inspection Type(s)	Footing Heat/Vent/AC	Erosion Control	Foundation Electrical	Bsmt Drain Tile Insulation/Energy	Construction
Owner:	······	· ·	Contractor:		
<del></del>		·			
	· .				
AN INSPEC	TION OF THE ABOVE	PREMISES HAS DISCL	- I ALDWIN	GIONCOMPLIANCES:	
NO.	CODE SECTION		TRUNGSAN	DREQUIREMENTS	
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<u> </u>	Ι	RTANT: Please report	when violation are corr	rected. AVOID DELAY	<u></u>
NOTICE O	OF NONCOMPLIANCE	aithin ' 10 days offer y	written notification unless a	n extension of time is granted.	Each day that the violati
continues at	iter notice shall constitute a	separate offense and is su	bject to remedies and pena	Ities by the authority having ju	risdiction.

## INSPECTION REPORT AND

NOTICE OF NONCOMPLIANCE All cited violations shall be corrected within continues after notice shall constitute a separat	<u>30</u> days after written notification unles e offense and is subject to remedies and p	ss an extension of time is granted. Each day that the violation enalties by the authority having jurisdiction.
Enforcement Town Cour Jurisdiction: Village State	nty City OF	Authority By Municipal Ordinance Section::
Inspector's Name:	Violations Explained To:	Compliance Date:
Inspector's Address:	Ö	Office Hours: Telephone No:

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# DO NOT REMOVE OFFICIAL MUNICIPAL NOTICE OF VIOLATION

LOCATION:



SBD-10266 (N.10/95)

DEPARTMENT OF COMMERCE

20 APPENDIX

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Safety and Buildings Division 201 W. Washington Avenue P O Box 7162 Madison, WI 53707–7162 Telephone: (608) 266–3151

### PETITION FOR VARIANCE INFORMATION AND INSTRUCTIONS – ILHR 3

In instances where exact compliance with a particular code requirement cannot be met or alternative designs are desired, the Division has a petition for variance program where it reviews and considers acceptance of alternatives which are not in strict conformance with the letter of the code, but which meet the intent of the code. **A variance is not a waiver from a code requirement.** The petitioner must **provide an equivalency which meets the intent** of the code section petitioned to obtain a variance. Documentation of the rationale for the equivalency is requested below. Failure to provide adequate information may delay your petition. Pictures, sketches, and plans may be submitted to support equivalency. If the proposed equivalency does not adequately safeguard the health, safety, and welfare of building occupants, frequenters, firefighters, etc., the variance request will be denied. NOTE: A SEP-ARATE PETITION IS REQUIRED FOR EACH BUILDING AND EACH CODE ISSUE PETITIONED (i.e., 57.13 window issue cannot be processed on the same petition as 51.16 stair issue). It should be noted that **a petition for variance does not take the place of any required plan review submittal.** 

The Division is unable to process petitions for variance that are not properly completed. Before submitting the application, the following items should be checked for completeness in order to avoid delays:

- Petitioner's name (typed or printed)
- Petitioner's signature
- The Petition For Variance Application must be signed by the owner of the building or project unless a Power of Attorney is submitted.
- Notary Public signature with affixed seal
- Analysis to establish equivalency, including any pictures, illustrations or sketches
  of the existing and proposed conditions to clearly convey your proposal to the
  reviewer.
- Proper fee

Any required position statements by fire chief or municipal official

A position statement from the chief of the local fire department is required for fire safety issues. **No position statement is required for** non-fire safety topics such as <u>sanitary and energy conservation</u>. Position statements for both the fire department and municipality are required for ILHR 69 barrier-free petitions. For rules relating to one- and two-family dwellings, only a position statement from the local enforcing municipality is required. Position statements must be completed and signed by the appropriate <u>fire chief or municipal enforcement official</u>. See the back of SBD-9890, Petition For Variance Application form for these position statement forms. Signatures or seals on all documents must be originals. Photocopies are not acceptable.

SBD-9890 (R.01/98)

Contact numbers and fees for the Division's review of the petition for variance are as follows: Chapters ILHR 20-25, Uniform Dwelling Code ...... (608) 267-5113 ..... \$125.00 Chapters ILHR 67-68, Rental Unit Energy Efficiency Code (608) 266-1930 ..... \$125.00 Chapters ILHR 50-64, Commercial Building Code ...... (608) 266-1835 ..... \$490.00 Chapter ILHR 66, Uniform Multi-Family Dwellings ...... (608) 266-0669 ..... \$490.00 The cities of Milwaukee and Madison may process requests for variances from Chapters ILHR 50 through 64 requirements on projects in their jurisdiction.) Chapter ILHR 66, Multifamily Dwelling ...... (608) 266–1930 ..... \$490.00 \$200.00 Chapter ILHR 70, Historic Building Code ..... (715) 524-3626 ..... \$300.00 All Other Chapters ..... \$200.00 Boilers and Pressure Vessels ...... (414) 548-8617 Priority Review: Does not apply to Uniform Dwelling Code or Historic Building Code issues which already are treated as a priority ..... Double Above Amounts Except for special cases, the Division will review and make a determination on a petition for variance within 30 business days of receipt of all calculations, documents, and fees required for the review. Uniform Dwelling Code petitions will be processed within 5 business days. Priority petitions will be processed within 10 business days. Petitions for variance should be submitted to: Safety and Buildings Division 201 West Washington Avenue P O Box 7162 Madison, Wisconsin 53707 (608) 266-3151 Elevator or barrier-free petitions may be submitted directly to the Waukesha office.

General Plumbing or Private Sewage petitions may be submitted to any of the six full-service offices.

GREEN B.	AY S&BD	HAYWARD S&BD	LACROSSE S&BD	MADISON S&BD	SHAWANO S&BD
2331 San L Green Bay 920–492–5 FAX: 920–	uis Place WI 54304 501 492–5604	15837 USH 63 Hayward, WI 54843 715-634-4870 FAX: 715-634-5150	2226 Rose Street La Crosse, WI 54603 608–785–9334 FAX: 608–785–9330	201 W. Washington Ave. P.O. Box 7162 Madison, WI 53707-7162 608-261-8490 FAX: 608-267-9566	1340 Green Bay St Shawano, WI 54166 715–524–3626 FAX: 715–524–3633

WAUKESHA S&BD 401 Pilot Court Waukesha, WI 53188 414-548-8600 FAX: 414-548-8614

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Dept. Use	e Only		
Plan No.		÷	

### PETITION FOR VARIANCE APPLICATION

Safety and Buildings Division 201 W. Washington Ave. P.O. Box 7162 Madison. WI 53707 Page 1 of

ame ompany Name umber and Street ity, State, Zip Code ontact Person elephone Number FAX Number ) . Plan Review Status On ho eview by Prelin State Municipality Appro State the code section being patitioned.	Building Occupancy Chapter(s) Tenant Name (If any) Building Location (number and s) City Village Tc County of Property ID # (tax parcel # - cor old ninary design oved, requesting revision nitted with petition	and Use street) wmship of htact county) Aiready Built acc brough	Designer Design Firm Number and Street City, State, Zip Code Contact Person Telephone Number ( ) built	Registration No.         FAX Number         ( )
ompany Name umber and Street ity, State, Zip Code ontact Person elephone Number FAX Number ) . Plan Review Status On ho eview by Prelin State Municipality Appro State the code section being patitioned.	Tenant Name (if any) Building Location (number and s City Village To County of Property ID # (tax parcel # - cor old ninary design oved, requesting revision nitted with petition	street) wriship of ntact county) Aiready Built acc	Design Firm Number and Street City, State, Zip Code Contact Person Telephone Number () built	FAX Number
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elephone Number FAX Number ) FAX Number ( ) . Plan Review Status On ho eview by Prelin State Municipality Appro State the code section being patitioned	Property ID # (tax parcel # - cor old ninary design oved, requesting revision nitted with petition	Aiready	Telephone Number	FAX Number ( )
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Reason why compliance with the code of the section period of the section period of the section period of the section petitioned.	AND the specific condition or i cannot be attained without the le of providing equivalent degr	Plan wil	afety, or welfare as add	dressed by the code
<ul> <li>List attachments to be considered as pa expert opinion, previously approved var</li> </ul>	art of the petitioner's statement riances, pictures, plans, sketch	s (i.e., model o ies, etc.).	code sections, test repo	orts, research articles,
/ERIFICATION BY OWNER - PETITION IS VA Section Comm 2.52 for complete fee inform Note: Petitioner must be the owner of the petition unless Power of Attorney is Petitioner's Name (type or print)	LID ONLY IF NOTARIZED WITH mation) building or project. Tenants, a submitted with the Petition for _, being duly sworn, I state as it is true and that I have sig	AFFIXED SEAI ogents, designed Variance App pelitioner that nificant owner	AND ACCOMPANIED E ers, contractors, attorne lication. I have read the forego ship rights to the subject	BY REVIEW FEE (See eys, etc., shall not sign ing petition and I belie t building or project.
Petitioner's Signature	Subscribed and swom to before me this date	Notary Public	······································	My commission expir on

SBD-9890 (R.01/98)

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	Project Location		Plan Number
Page 2 of			
F To be completed for variance	ire Department Position is requested from ILHR 50-64, requirements	<b>Statement</b> ILHR 69, ILHR 10,	and other fire related
have read the application for Approval Condition	or variance and recommend: nal Approval . Denial on including any conflicts with	check appropriate No Comm	e box) ent ulations and
suggested conditions:	on mouting any connots with		
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Fire Department Name and Address	· · · · · · · · · · · · · · · · · · ·		]
Name of Fire Chief or Designee (type or or		Telephone N	umber
Standurg of Fire Chief or Designed	·	Date Signed	· · · · · · · · · · · · · · · · · · ·
Signature of Fire Chief or Designee		Date Signed	
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Signature of Fire Chief or Designee           MUNICIPAL           To be completed for variar           review is by municipality or           I have read the application 1           Approval           Conditio           Explanation for recommendation includ           Municipality Exercising Jurisdiction           Name and Address of Municipal Official (type)	BUILDING INSPECTION nces requested from ILHR 20-2 orders are written on the buildi cases. for variance and recommend anal Approval Denial ing any conflicts with local rules and regu	Date Signed <b>RECOMMEND</b> 23. Also to be used ing under construct : (check appropriat No Comm lations and suggested of Telephone Number of	ATION d if ILHR 50-64 plan tion; optional in other te box) nent onditions:

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### MINIMUM FASTENER SCHEDULE TABLE

### Other interior and exterior panel products and finishes installed per manufacturer requirements. For engineered connectors, use manufacturer's specified fasteners.

Description of Building Materials/Connection	Number and Type of Fas- tener <sup>1 2 3</sup>
Floor Framing	
Joist to sill or girder, toe nail	2–16d, 3–8d
Band or rim joist to joist, end nail	3–16d
Band or rim joist to sill or top plate	2–16d at 16" o.c.
Bridging to joist, toe nail each end	28d
Built-up girder and beams, top loaded	10d at 32" o.c. at top and bot- tom and staggered and two at ends and at each splice
Built-up girder and beams, side-loaded	16d at 16" o.c. at top and bot- tom and staggered and two at ends and at each splice
Ledger strip to beam, face nail	3–16d each joist
Joist on ledger to beam, toe nail	3–8d
Wall Framing	
Sole plate to joist or blocking, face nail	16d at 16" o.c.
Top or sole plate to stud, end nail	2–16d
Stud to sole plate, toe nail	4–8d or 3–16d
Doubled studs, face nail	16d at 24" o.c.
Doubled top plates, face nail	16d at 16" o.c.
Top plates, laps and intersections, face nail	2–16d
Continuous header, two pieces	16d at 16" o.c. along each edge
Continuous header to stud, toe nail	4–8d
1" corner brace to each stud and plate, face nail	2–8d or 2 staples, 1 3/4"
Built-up corner studs	16d at 30" o.c., 16d at 24" o.c.
Roof/Ceiling Framing	
Celling joists to plate, toe nail	2–16d, 3–8d
Ceiling joist, laps over partitions, face nail	3–16d
Celling joist to parallel rafters, face nail	3–16d
Rafter to plate, toe nail (maximum 6' rafter span, engineered connector for longer)	2–16d, 3–8d
Roof rafters to ridge, valley or hip rafters, toe nail	4–16d
Roof rafters to ridge, valley or hip rafters, face nail	3–16d
Collar ties to rafters, face nail	3–8d
Boards and planks	
1" x 6" subfloor or less to each joist, face nail	2-8d or 2 staples, 1 3/4"
Wider than 1" x 6" subfloor toe to each joist, face nail	3-8d or 4 staples 1 3/4"
2" subfloor to joist or girder, blind and face nail	2–16d
1" x 6" roof sheathing to each bearing, face nail	2-8d or 2 staples, 1 3/4"
1" x 8" roof sheathing to each bearing, face nail	2–8d or 3 staples, 1 3/4"
Wider than 1" x 8" roof sheathing to each bearing, face nail	3–8d or 4 staples, 1 3/4"
2-inch planks	2–16d at each bearing

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,	Panel Sheath	ing	· · · · · · · · · · · · · · · · · · ·
		Spa	acing of Fastener
Material	Fastener	Edges	Intermediate Supports
Engineered wood panel for subfloor and roof sheathing and wall corner wind bracing to framing		· · · · · · · · · · · · · · · · · · ·	
5/16-inch to 1/2-inch	6d common or deformed nail or staple, 1 1/2"	6"	12" <sup>4</sup>
5/8-inch to 3/4-inch	8d smooth or common, 6d deformed nail, or staple, 14 ga. 1 ¾"	6"	12" <sup>4</sup>
7/8-inch to 1-inch	8d common or deformed nail	6"	12"
1 1/8-inch to 1 1/4-inch	10d smooth or common, or 8d deformed nail	6"	12"
Combination subfloor/ underlayment to framing		•	
3/4-inch or less	6d deformed or 8d smooth or common nail	6"	12"
7/8-inch to 1-inch	8d smooth, common or deformed nail	6"	12"
1 1/8inch to 1 1/4inch	10d smooth or common or 8d deformed nail	6"	12"
Wood panel siding to fram- ing			
1/2-inch or less	6d corrosion-resistant siding and casing nails	6"	12"
5/8inch	8d corrosion-resistant siding and casing nails	6"	12"

<sup>1</sup>All nails are smooth-common, box or deformed shank except where otherwise stated <sup>2</sup>Nail is a general description and may be T-head, modified round head or round head. <sup>3</sup>Staples are 16-gauge wire, unless otherwise noted, and have a minimum 7/16-inch o.d. crown width. <sup>4</sup>Staples shall be spaced at not more than 10 inches o.c. at intermediate supports for floors.

### UDC Floor & Ceiling Joist and Roof Rafter Span Tables And Design Value Tables

Use the following Span Tables to determine the maximum spans for floor and ceiling joists and roof rafters. These spans are based on:

- simple, single spans (although the tables may be safely used for continuous two-span floor joists)
- uniformly distributed loads
- fully supported members with one edge properly sheathed and nailed
- for floor joists and roof rafters, the top edge shall be properly sheathed and nailed

The criteria for each Span Table is given in the upper left hand corner and is also summarized in the table of Span Tables below. Choose the appropriate Span Table based on the member type and required loading. Select your desired member depth, member spacing and span to determine the minimum Fb value. Note that these tables include recommended deflection criteria. However, for strict code compliance, only the Fb strength requirements must be satisfied. The modulus of elasticity (E) values, would be met for serviceability purposes only.

Note that straight-line interpolation is permitted for intermediate spans and design values. Span is measured from face to face of supports. For sloping rafters, the span is measured along the horizontal projection.

Section Comm 21.27 allows reduction of the snow live load for roof slopes greater than 30 degrees (7/12 slope) based on the formula Cs = 1 - (a-30)/40, where "a" is the slope of the roof expressed in degrees. Following is a table of tabulated values for certain roof slopes.

Slope	Angle in Degrees	Zone 1 Live Load (psf)	Zone 2 Live Load (psf)
7/12	30	40	30
10/12	40	30	22.5
12/12	45	25	18.8
14/12	50	20	15

Use the Design Value tables following the Span Tables to determine the acceptable species and grades to satisfy minimum Fb values obtained from the Span Tables. The Design Value tables assume at least three members spaced no more than 24" on center. Use the Normal Duration column Fb values for joists and the Snow Loading column Fb values for rafters.

See the following examples for further guidance.

Tables are reprinted courtesy of American Forest & Paper Association.

Table	Member	Live	Dead	Condition	(Deflection)*
No.	Туре	Load	Load		
[		(psf)	(psf)		
F-2	Floor Joists	40	10		L/360
C-1	Ceiling Joists	10	5	Drywall ceiling, no attic storage	L/240
C-2	Ceiling Joists	20	10	Attic storage	L/240
R2	Roof Rafters	30 (Zone	10	Maximum 2 layers of asphalt	L/240
		2)		shingles or wood shakes/shingles	
R-3	Roof Rafters	40 (Zone	10	Maximum 2 layers of asphalt	L/240
		1)		shingles or wood shakes/shingles	
R-10	Roof Rafters	30 (Zone	20	Heavy roof covering (clay tile)	L/240
		2)			
R-11	Roof Rafters	40 (Zone	20	Heavy roof covering (clay tile)	L/240
		. 1)			
R–14	Roof Rafters	30 (Zone	10	Maximum 2 layers of asphalt	L/180
		2)		shingles or wood shakes/shingles	
R-15	Roof Rafters	40 (Zone	10	Maximum 2 layers of asphalt	L/180
:		1)		shingles or wood shakes/shingles	
R-22	Roof Rafters	30 (Zone	20	Heavy roof covering (clay tile)	L/180
		2)		·	
R-23	Roof Rafters	40 (Zone	20	Heavy roof covering (clay tile)	L/180
	1	1)			

\*Deflection criteria are optional. For roof rafters with drywall on the underside, use the stricter L/240 tables to limit deflection.

**Example 1. Floor Joists.** Assume a required single span of 12'-9", live load of 10 psf and joists spaced 16 inches on center. Table F-2 (see following highlighted tables) shows that one solution is a grade of 2x8 having an Fb value of 1255 would allow a span of 12'-10 which satisfies the condition. (Note that the recommended E value to limit deflection would be 1,600,000.) Going to the Design Value Tables, we find that as an example, 2x8 Hem Fir grade No.1 has an Fb value of 1310 for normal duration. (It also has an E value of 1,500,000 which satisfies the recommended deflection criteria.)

**Example 2. Rafters.** Assume a horizontal projected span of 13'-0", a live load of 40 psf, dead load of 10 psf, a roof slope of 4/12 and rafters spaced 16 inches on center. Since the slope is shallower than 7/12, there is no allowable reduction of the snow live load. Table R-3 shows that a 2x8 having an Fb value of 1300 would allow a span of 13'-1" which satisfies the condition. (Note that the recommended E value to limit deflection would be 1,120,000.) Going to the Design Value Tables, we find that as an example, 2x8 Douglas Fir-Larch grade No.2 has an Fb value of 1390 for snow loading. (It also has an E value of 1,600,000 which satisfies the recommended deflection criteria.)

### Example 1 TABLE F- 2 FLOOR JOISTS WITH L/360 DEFLECTION LIMITS

### DESIGN CRITERIA:

Deflection - For 40 psf live load. Limited to span in inches divided by 360. Strength - Live load of 40 psf plus dead load of 10 psf determines the required bending design value.

Modulus of Elasticity, E, in 1,000,000 psi Joist Size Spacing (in) {in} 1.6 1.8 1.9 2.0 2.1 2.3 1.4 2.2 2.4 0.8 0.9 1.0 1.1 1.2 1.3 1.5 1.7 9-6 10-0 10-3 10-9 10-11 11-2 11-4 11-7 11-9 11-11 12-1 12-3 12.0 8-6 8-10 9-2 9-9 10-6 7-9 10-2 10-4 10-6 10-8 10-10 11-0 11-2 16.0 8-0 8-4 8-7 8-10 9-1 9-4 9-6 9.9 9-11 8-7 8-9 9-0 9-2 9-4 9-6 9-8 9-10 10-0 10-2 10-4 10-6 7-10 8-1 8-4 2x 6 19.2 7-3 7-7 24.0 6-9 7-0 7-3 7-6 7-9 7-11 8-2 8-4 8-6 8-8 8-10 9-0 9-2 9-4 9-6 9-7 9.9 13-10 15-6 15-9 15-11 16-2 12.0 11-3 11-8 12- I 12-6 12-10 13-2 13-6 2-21 14-5 14-8 15-0 15-3 12-3 12-7 12-10 13-4 13-7 13-10 14-1 14-3 14-6 14-8 11-8 12-0 13-1 16.0 10-2 10-7 11-0 11-4 12-4 13-10 19.2 9-7 10-0 10-4 10-8 11-0 11-3 11-7-11-10 12-1 12-7 12-10 13-0 13-3 13-5 13-8 2x 8 8-11 9-3 9-7 9-11 10-2 10-6 10-9 11-0 11-3 11-5 11-8 11-11 12-1 12-3 12-6 12-8 12-10 24.0 20-4 20-8 12,0 14-4 14-11 15-5 15-11 16-5 16-10 17-3 17-8 18-0 18-5 18-9 19-1 19-5 19-9 20-1 14-6 14-11 15-3 15-8 16-0 16-5 16-9 17-0 17-4 17-8 17-11 18-3 18-6 18-9 13-0 13-6 14-0 16.0 15-5 16-7 17-2 17-5 17-8 14-9 15-1 15-9 16-0 16-4 16-11 2x10 19.2 12-3 12-9 13-2 13-7 14-0 14-5 15-2 15-5 15-8 15-11 16-2 16-5 24.0 11-4 11-10 12-3 12-8 13-0 13-4 13-8 14-0 14-4 14-7 14-11 18-9 19-4 19-11 20-6 21-0 21-6 21-11 22-5 22-10 23-3 23-7 24-0 24-5 24-9 25-1 12.0 17-5 18-1 22-10 19-1 19-6 19-11 20-4 20-9 21-1 21-6 21-10 22-2 22-6 16-5 17-0 17-7 18-1 18-7 16.0 15-10 20-2 20-6 20-10 21-2 21-6 16-0 16-7 17-0 17-6 17-11 18-4 18-9 19-2 19-6 19-10 2x12 19.2 14-11 15-6 15-4 15-10 16-3 16-8 17-0 17-5 17-9 18-1 18-5 18-9 19-1 19-4 19-8 19-11 24.0 13-10 4-4 14-11 833 888 941 993 1043 1092 1140 1187 1233 1278 1323 1367 1410 1452 1494 F, 12.0 718 777 1504 1598 1644 1148 1202 1255 1306 1357 1407 1456 1551 F, 16.0 790 855 917 977 1036 1093 1747 1220 1277 1333 1388 1442 1495 1547 1598 1649 1698 F<sub>b</sub> 840 909 975 1039 1101 1161 19.2 1251 1314 1376 1436 1496 1554 1611 1667 1722 1776 1829 1882 F, 905 979 1050 1119 1186 24.0

Note: The required bending design value, F<sub>x</sub>, in pounds per square inch is shown at the bottom of each table and is applicable to all lumber sizes shown. Spans are shown in feet-inches and are limited to 26' and less. Check sources of supply for availability of lumber in lengths greater than 20'.

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**Comm 20 APPENDIX** 

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## Example 1

Species and Grade	Size	Design V Bendin	Value in	Modulus	Grading
		Denam	5, <b>F</b> D	Flasticity	Agency
		Normai Duration	Snow	"E"	rigency
	L	Duration	Loading	<u> </u>	
Eastern White Pine		2155	2490	1 200 000	
Select Structural	4	1225	2400	1,200,000	
No.2	ł ·	1333	1140	1,100,000	
No 3	2-4	605	605	900,000	
Sud	201	570	655	900,000	F
Construction		775	895	1.000.000	
Standard		430	495	900.000	
Utility	1	200	230	800,000	
Select Structural		1870	2150	1,200,000	-
No.1	1	1160	1330	1,100,000	
No.2	2x6	860	990	1,100,000	
No.3	]	525	600	900,000	
Stud		520	595	900,000	NELMA
Select Structural	ļ	1725	1985	1,200,000	NSLB
No.1	2x8	1070	1230	1,100,000	
No.2		795	915	1,100,000	
No.3	1	485	555	900,000	
Select Structural		1580	1820	1,200,000	
No.1	2x10	980	1123	1,100,000	
No.2	-	123	<u> </u>	1,100,000	
NO.3		1445	1655	1 200,000	
No.1	2012	800	1035	1,200,000	
No 2	2414	660	760	1,100,000	
No 3	-	405	465	900,000	
Hom Fir	1	1. 100			·
Caleat Structural		2415	2775	1 600 000	ו
No.1 & Ptr	-	1810	2085	1,000,000	{ !
No 1	4	1640	1885	1,500,000	· .
No 2		1465	1685	1,300,000	
No 3	2x4	865	990	1.200.000	1
Stud	-	855	980	1,200,000	1
Construction	1	1120	1290	1,300,000	
Standard	1	635	725	1,200,000	
Utility		290	330	1,100,000	1
Select Structural		2095	2405	1,600,000	
No.1 & Btr	] .	1570	1805	1,500,000	
No.1	2x6	1420	1635	1,500,000	1
No.2	_	1270	1460	1,300,000	
No.3	4	750	860	1,200,000	1
Stud		775	895	1,200,000	1
Select Structural	4	1930	2220	1,600,000	WCLIB
No.1 & Btr		1450	1665	1,500,000	WWPA
No.1	2x8	1310	1510	1,500,000	1
No.2		1175	1350	1,300,000	1
No.3		690	795	1,200,000	4
Select Structural		1770	2035	1,600,000	_
No.1 & Btr	4 .	1330	1525	1,500,000	4
No.1	2x10	1200	1380	1,500,000	4
No.2	-	1075	1235	1,300,000	4
No.3		635	725	1,200,000	-
Select Structural	4	1610	1850	1,600,000	4
No.1 & Btr		1210	1390	1,500,000	4
No.1	2XI2	1095	1255	1,500,000	4
No.2		980	1125	1,300,000	4
L 110.3		1 375	1 000	1,200,000	1

### **Example 2** TABLE R-3 RAFTERS WITH L/240 DEFLECTION LIMITATION

DESIGN CRITERIA:

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Strength - Live Load of 40 psf plus Dead Load of 10 psf determines the required bending design value. Deflection - For 40 psf live load. Limited to span in inches divided by 240.

	· · · ·		•					-1	R	after	Bendin	ig Design	Value,	F <sub>b</sub> , (psi)			• •						
Size (in)	Spacing (in)					•		· ·	•.					-		n at ng b							•
		300	400	500	600	700	800	900	1000	1100	1200		1400	1500	1600	1700	1800	1900	2000	2100	2200	2300	2400
	12.0	5.6	6.4	7- 1	70	8.5	9.0	9.6	10- 0	10.6	11.0	11-5	11.11	12-4	12.8	13., 17	13-6	13.10	14-7	•			
	16.0	4-9	5-6	6-2	6-9	7-3	7-9	8-3	8-8	9-1	9-6	9-11	10-3	10-8	11-0	11-4	11-8	12-0	12-4	12-7	12-11		
2x 6	19.2 24.0	4-4 3-11	5-0 4-6	5-7 5-0	6-2 5-6	6-8 5-11	7-1 6-4	7-6 6-9	7-11 7-1	8-4 7-5	8-8 7-9	9- 1 8- 1	9- 5 8- 5	9-9 8-8	10-0 9-0	10-4 9-3	10-8 9-6	10-11 9-9	11-3 10-0	-6  ()-3	[1-9 10-6	12-0 10-9	12-4 11-0
	12.0	7-3	8-4	9-4	10-3	11-1	11-10	12-7	13-3	13-11	14-6	لمعلم	15-8	16-3	16-9	17-3	17-9	18-3	18-9				
2x 8	16.0 19.2	6-3 5-9	7-3 6-7	8-1 7-5	8-11 8-1	9-7 8-9	10-3 9-4	10-10 9-11	11-6 10-6	12-0 11-0	12-7		13-7	14-0	14-6 13-3	14-11 13-8	15-5 14-0	15-10	16-3 14-10	16-7 15-2	17-0	15-10	16-3
	24.0	5-2	5-11	6-7	7-3	7-10	8-4	8-11	9-4	9-10	10-3	10-8	11-1	11-6	11-10	12-2	12-7	12-11	13-3	13-7	13-11	14-2	14-6
	12.0	9-3	10-8	11-11	13-1	14-2	15-1	16-0	16-11	17-9	18-6	19-3	20-0	20-8	21-4	22- 0	22-8	23-3	23-11				
2210	16.0	8-0 7-4	9-3 8-5	10-4 9-5	11-4 10-4	12-3	13-1	13-10	14-8 13-4	15-4 14-0	16-0 14-8	16-8	17-4	17-11	18-6 16-11	19-1 17-5	19-7 17-11	20-2 18-5	20-8 18-11	21-2 19-4	21-8 19-10	20-3	20-8
2410	24.0	6-6	7-7	8-5	9-3	10-0	10-8	11-4	11-11	12-6	13-1	13-7	14-2	14-8	15-1	15-7	16-0	16-6	16-11	17-4	17-9	18-1	18-6
	12.0	11-3	13.0	14-6	15-11	17. 7	18-4	19-6	20-6	21.7	22-6	23-5	24-4	25-2	26-0	<u>.</u>							
	16.0	9-9	11-3	12-7	13-9	14-11	15-11	16-10	17-9	18-8	19-6	20-3	21-1	21-9	22-6	23-2	23-10	24-6	25-2	25-9			
2x12	19.2 24.0	8-11 7-11	10-3 9-2	11-6 10-3	12-7 11-3	13-7 12-2	14-6 013-0	15-5 13-9	16-3 14-6	17-0 15-3	17-9 15-11	18-6 16-7	19-3 17-2	19-11 17-9	20-6 18-4	21-2 18-11	21-9 19-6	22-5 20-0	23- 0 20- 6	23-6 21-1	24-   21- 7	24-8 22-0	25- 2 22- 6
																						••	
E	12.0	0.14	0.22	0.31	0.41	0.51 0.44	0.63 0.54	0.75	0.88 0.76	1.01	1.15	L30	1.45	1.61 1.39	1.77 1.54	1.94 1.68	2.12 1.83	2.30 1.99	2.48 2.15	2.31	2.48		
Ē	19.2	0.11	0.18	0.24	0.32	0.41	0.50	0.59	0.69	0.80	0.91	1.03	1.15	1.27	1.40	1.54	1.67	1.81	1.96	2.11	2.26	2.42	2.58
Е	24,0	0.10	0.16	0.22	0.29	0.36	0,44	0.53	0.62	0.71	0.81	0.92	1.03	1.14	L25	1,37	1.50	1.62	1.75	1.89	2.02	2.16	2.30

Note: The required modulus of clasticity, E, in 1,000,000 pounds per square inch is shown at the bottom of each table, is limited to 2.6 million psi and less, and is applicable to all lumber sizes shown. Spans are shown in feet-inches and are limited to 26' and less. Check sources of supply for availability of lumber in lengths greater than 20'.

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### Example 2

Species and Grade	Size	Design Bendin	Value in g, "Fb"	Modulus of	Grading Rules
		Normal	Snow	Elasticity	Agency
		Duration	Loading	"E.	
Cottonwood				• • • • • • • • • • • • • • • • • • • •	
Select Structural		1510	1735	1.200.000	
No.1	1	1080	1240	1,200,000	
No.2	1	1080	1240	1,100,000	
No.3	2x4	605	695	1,000,000	
Stud	1	600	690	1,000,000	
Construction	1	805	925	1,000,000	
Standard	1	460	530	900,000	1
Utility		200	230	900,000	
Select Structural		1310	1505	1,200,000	
No.1	]	935	1075	1,200,000	
No.2	] 2x6	935	1075	1,100,000	
No.3	ļ	525	600	1,000,000	
Stud		545	630	1,000,000	
Select Structural		1210	1390	1,200,000	NSLB
No.1	2x8	865	990	1,200,000	
No.2	1	865	990	1,100.000	
No.3		485	555	1,000,000	
Select Structural		1105	1275	1,200,000	
No.1	[ 2x10	790	910	1,200,000	
No.2		790	910	1,100.000	
No.3	l	445	510	1,000,000	
Select Structural	l	1005	1155	1,200,000	
No.1	2x12	720	825	1,200,000	
No.2	4	720	825	1,100,000	
No.3		405	465	1,000,000	
Douglas Fir-Larch	<b></b>	F	r ·· ·		
Select Structural	4	2500	2875	1,900,000	
No.1 & Btr	Į	1985	2280	1,800,000	
No.1	1	1725	1985	1,700,000	
No.2		1510	1735	1,600,000	
No.3	2x4	865	990	1,400,000	
Stud	1	855	980	1,400,000	
Construction	-	1150	1325	1,500,000	
Standard	ł	635	725	1,400,000	
Utility		315	365	1,300,000	
Select Structural	4	2170	2495	1,900,000	
No.1 & Btr		1720	1975	1.800,000	
No.1	2x6	1495	1720	1,700,000	
No.2	-	1310	1505	1,600,000	
N0.3	-	750	860	1,400,000	
Stud	<u>                                     </u>	775	895	1,400,000	WOUT
No. 1 & Sta	4	2000	2300	1,900,000	WUCLIB
110,1 & SIF	<u></u>	<u> 5851</u>	1825	1,800,000	WWPA
No.1	2x8	1380	1585	1,700,000	
No.2		1210	1390	1,600,000	
No.3	T	690	795	1,400,000	ſ
Select Structural		1835	2110	1,900,000	
No.1 & Btr	]	1455	1675	1,800,000	]
No.1	2x10	1265	1455	1,700,000	
No.2		1105	1275	1,600,000	
No.3	]	635	725	1,400,000	}
Select Structural	]	1670	1920	1,900,000	
No.1 & Btr	]	1325	1520	1,800,000	]
No.1	2x12	1150	1325	1,700,000	]
No.2		1005	1155	1,600,000	
No.3		575	660	1,400,000	

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## TABLE F- 2 FLOOR JOISTS WITH L/360 DEFLECTION LIMITS

### **DESIGN CRITERIA:**

Strength - Live load of 40 psf plus dead load

of 10 psf determines the required bending design value.

Joist Size	Spacin	g						Modulu	s of Elastic	ity, E, in 1,	,000,000 ps	i						
(in)	(in)	0,8	0.9	1.0	1.1	1.2	1.3	1.4	1.5	1.6	1.7	1.8	1.9	2.0	2.1	2.2	2.3	2.4
2x 6	12.0	8 6	8-10	9-2	9-6	9 9	10- 0	10 3	10 6	10-9	10-11	11–2	11-4	11-7	11-9	11-11	12– 1	12-3
	16.0	7 9	80	8-4	8-7	810	9- 1	9 4	9- 6	9-9	9-11	10–2	10-4	10-6	10-8	10-10	11– 0	11-2
	19.2	7 3	7-7	7-10	8-1	8 4	8- 7	8 9	9- 0	9-2	9-4	9–6	9-8	9-10	10-0	10-2	10– 4	10-6
	24.0	6 9	7-0	7-3	7-6	7 9	7-11	8 2	8- 4	8-6	8-8	8–10	9-0	9-2	9-4	9-6	9– 7	9-9
2x 8	12.0	11-3	11-8	12 1	12 6	1210	13-2	13-6	13-10	14-2	145	14 8	15-0	15-3	156	15-9	15-11	16-2
	16.0	10-2	10-7	11- 0	11 4	11 8	12-0	12-3	12-7	12-10	131	13 4	13-7	13-10	14-1	14-3	14-6	14-8
	19.2	9-7	10-0	10 4	10 8	11 0	11-3	11-7	11-10	12-1	12-4	12 7	12-10	13-0	13-3	13-5	13-8	13-10
	24.0	8-11	9-3	9 7	911	10 2	10-6	10-9	11-0	11-3	11-5	11 8	11-11	12-1	12-3	12-6	12-8	12-10
2x10	12.0	14-4	14–11	15-5	15–11	16-5	16–10	17 3	17 8	18– 0	18-5	18-9	19– 1	19 5	19-9	20 1	20- 4	20- 8
	16.0	13-0	13– 6	14-0	14– 6	14-11	15– 3	15- 8	16 0	16– 5	16-9	17-0	17– 4	17- 8	17-11	18 3	18 6	18- 9
	19.2	12-3	12– 9	13-2	13– 7	14-0	14– 5	14 9	15 1	15– 5	15-9	16-0	16– 4	16 7	16-11	17 2	17- 5	17- 8
	24.0	11-4	11–10	12-3	12– 8	13-0	13– 4	13 8	14 0	14– 4	14-7	14-11	15– 2	15- 5	15-8	1511	16- 2	16- 5
2x12	12.0	17- 5	18 I	18-9	19 4	1911	20 6	21-0	21- 6	2111	225	2210	23-3	23 7	24-0	24 5	24 9	25- 1
	16.0	15-10	16 5	17-0	17 7	18 1	18 7	19-1	19- 6	1911	204	209	21-1	21 6	21-10	22 2	22 6	22-10
	19.2	14-11	15 6	16-0	16 7	17 0	17 6	17-11	18- 4	18 9	19-2	196	19-10	20 2	20-6	2010	21 2	21- 6
	24.0	13-10	14 4	14-11	15 4	1510	16 3	16-8	17- 0	17 5	17-9	181	18-5	18 9	19-1	19 4	19 8	19-11
F⊾ F⊾ Fs	12.0 16.0 19.2 24.0	718 790 840 905	777 855 909 979	833 917 975 1050	888 977 1039 1119	941 1036 1101 1186	993 1093 1161 1251	1043 1148 1220 1314	1092 1202 1277 1376	1140 1255 1333 1436	1187 1306 1388 1496	1233 1357 1442 1554	1278 1407 1495 1611	1323 1456 1547 1667	1367 1504 1598 1722	1410 1551 1649 1776	1452 1598 1698 1829	1494 1644 1747 1882

Note: The required bending design value,  $F_m$  in pounds per square inch is shown at the bottom of each table and is applicable to all lumber sizes shown. Spans are shown in feet-inches and are limited to 26' and less. Check sources of supply for availability of lumber in lengths greater than 20'.

**Comm 20 APPENDIX** 

 TABLE C-1

 CEILING JOISTS WITH L/240 DEFLECTION LIMITS

DESIGN CRITERIA:

Deflection – For 10 psf live load. Limited to span in inches divided by 240. Strength – Live Load of 10 psf plus dead load of 5 psf determines the required fiber stress value.

Joist Size	Spacing	g						Modulus	of Elastici	ty, E, in 1,0	00,000 psi							
(in)	(in)	0.8	0.9	1.0	1.1	1.2	1.3	1.4	1.5	1.6	1.7	1.8	1.9	2.0	2.1	2.2	2.3	2.4
2x 4	12.0 16.0 19.2 24.0	9–10 8–11 8–5 7–10	10 3 9 4 8 9 8 1	10 7 9 8 9 1 8 5	10-11 9-11 9-4 8-8	11-3 10-3 9-8 8-11	11-7 10-6 9-11 9-2	11-10 10-9 10-2 9-5	12-2 11-0 10-4 9-8	12-5 11-3 10-7 9-10	12 8 11 6 1010 10 0	1211 11-9 11-0 10-3	13-2 11-11 11-3 10-5	13-4 12-2 11-5 10-7	13-7 12-4 11-7 10-9	13-9 12-6 11-9 10-11	140 129 120 111	14-2 12-11 12-2 11-3
2x 6	12.0 16.0 19.2 24.0	15 6 14 1 13 3 12 3	16 1 14 7 13 9 12 9	16 8 15 2 14 3 13 3	17-2 15-7 14-8 13-8	17-8 16-1 15-2 14-1	18 2 16 6 15 7 14 5	18– 8 16–11 15–11 14– 9	19- 1 17- 4 16- 4 15- 2	19 6 17 8 16 8 15 6	19-11 18-1 17-0 15-9	20- 3 18- 5 17- 4 16- 1	20 8 18 9 17 8 16 4	21-0 19-1 17-11 16-8	21-4 19-5 18-3 16-11	21-8 19-8 18-6 17-2	22- 0 20- 0 18-10 17- 5	22 4 20 3 19 1 17 8
2x 8	12.0 16.0 19.2 24.0	20– 5 18– 6 17– 5 16– 2	21-2 19-3 18-1 16-10	21-11 19-11 18-9 17-5	22 8 20 7 19 5 18 0	23-4 21-2 19-11 18-6	24- 0 21- 9 20- 6 19- 0	24- 7 22- 4 21- 0 19- 6	25-2 22-10 21-6 19-11	25- 8 23- 4 21-11 20- 5	23–10 22– 5 20–10	24 3 22-10 21- 2	24 8 23 3 21 7	25-2 23-8 21-11	25~ 7 24- 0 22- 4	25-11 24-5 22-8	24-9 23-0	25– 2 23– 4
2x10	12.0 16.0 19.2 24.0	26 0 23 8 22 3 20 8	24– 7 23– 1 21– 6	25-5 23-11 22-3	24-9 22-11	25 5 23 8	24 3	24–10	25-5	26-0								
F, F, F, F,	12.0 16.0 19.2 24.0	711 783 832 896	769 847 900 969	825 909 965 1040	880 968 1029 1108	932 1026 1090 1174	983 1082 1150 1239	1033 1137 1208 1302	1082 1191 1265 1363	1129 1243 1321 1423	1176 1294 1375 1481	1221 1344 1429 1539	1266 1394 1481 1595	1310 1442 1533 1651	1354 1490 1583 1706	1396 1537 1633 1759	1438 1583 1682 1812	1480 1629 1731 1864

The required bending design value, F<sub>n</sub>, in pounds per square inch is shown at the bottom of each table and is applicable to all lumber sizes shown. Spans are shown in feet-inches and are limited to 26' and less. Check sources of supply for availability of lumber in lengths greater than 20'.

Note:

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## TABLE C-2 CEILING JOISTS WITH L/240 DEFLECTION LIMITS

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Limite Streng dead I	ed to spa gth – Liv oad of 1	in in inches /e Load of : 0 psf deter	divided by 20 psf plus mines the r	v 240. equired ben	ding desigr	ı value.												
							Jois	Modulus o	f Elasticity	, E, in 1,0	00,000 psi				-			
Size	Spacia (in)	ng																
(111)	(111)	0.8	0.9	1.0	1,1	1.2	1.3	1.4	1.5	1.6	1.7	1.8	1.9	2.0	2.1	2.2	2.3	2.4
	12.0	7–10	8-1	8-5	8 8	8-11	92	9 5	9– 8	9–10	10-0	10-3	10-5	10-7	10~ 9	10-11	11-1	[]-3
	16.0	7–1	7-5	7-8	7-11	8- I	8-4	8-7	8-9	8-11	9-1	9-4	9-6	9-8	99	9-11	10-1	10-3
2x 4	19.2	6-8	6-11	7-2	7-5	7-8	7~10 7 2	8–1 7–6	83 7-8	8-5 7-10	8-7 8-0	8-9 8-1	8-11	9-1 8-5	9-3 8-7	9-4 8-8	9-0 8-10	8-11 8-11
	24.0	0-2	0 3	0-0	0-11	/- 1	/- 3	)=0	7 0	7-10	00	0 1	0-5	0.5	0.1		• • •	•
	12.0	12-3	12-9	13-3	13 8	14-1	14-5	149	15-2	15 6	159	16 1	16-4	16 8	16-11	17-2	17-5	17 8
	16.0	11-2	11-7	12-0	12 5	12-9	13-1	13-5	139	14-1	14-4	14-7	14-11	15-2	15-5	157	15-10	16 1
2x 6	19.2	10 6	10-11	11-4	11-8	12-0	124	12-8	12-11	13-3	13-6	13-9	14-0	14-3	14-6	14-8	14-11	15-2
	24.0	99	10-2	10-6	10-10	11-2	11-5	11-9	12-0	12-3	12-6	12-9	13-0	10-0	13-3	13-8	13-10	14 1
	12.0	16-2	16-10	17-5	18 0	186	19-0	19–6	19–11	20-5	20-10	21-2	217	21-11	22 4	22-8	23 0	23-4
	16.0	14 8	15-3	15-10	16-4	16-10	17-3	17-9	18-1	18-6	18-11	19-3	19-7	19-11	20-3	20-7	20-11	21-2
2x 8	19.2	13-10	14-5	14-11	15-5	15-10	16-3	16-8	17-1	17-5	17-9	18-1	185	18-9	19 1	19 5	19 8 18 3	19-11
	24.0	12-10	1,3-4	13-10	14-5	14-0	13-1	15-0	15-10	10-2	10-0	10-10	11-2	17-5	11-1	10-0	, 10 0	10.0
	12.0	20- 8	21_6	22_3	22-11	23 <u>-</u> 8	24-3	24-10	25-5	260								
	16.0	18-9	21- 0 19- 6	20-2	20-10	21-6	22-1	22-7	23-1	23-8	24-1	24-7	25-0	25 5	25-10			
2x10	19.2	17-8	18-4	19-0	19-7	20-2	20-9	21-3	21-9	22-3	22-8	23-1	23-7	23-11	244	24-9	25-1	25-5
	24.0	16 5	17 0	17 8	18-3	189	19–3	19-9	20-2	20-8	21-1	21-6	21-10	22-3	22-7	22-11	23-4	23 8
÷	12.0	896	969	1040	1108	1174	1239	1302	1363	1423	1481	1539	1595	1651	1706	1759	1812	1864
F.	16.0	986	1067	1145	1220	1293	1364	1433	1500	1566	1631	1694	1756	1817	1877	1936	1995	2052
F.	19.2	1048	1134	1216	1296	1374	1449	1522	1594	1664	1733	1800	1866	1931	1995	2058	2120	2181
F.	24.0	1129	1221	1310	1396	1480	1561	1640	1717	1793	1800	1939	2010	2080	2149	2217	4460	2349

Note: The required bending design value, F<sub>st</sub> in pounds per square inch is shown at the bottom of each table and is applicable to all lumber sizes shown. Spans are shown in fect-inches and are limited to 26' and less. Check sources of supply for availability of lumber in lengths greater than 20'.

TABLE R-2 **RAFTERS WITH L/240 DEFLECTION LIMITATION** 

DESIGN CRITERIA: Strength - Live Load of 30 psf plus Dead Load of 10 psf determines the required bending design value. Deflection - For 30 psf live load. Limited to span in inches divided by 240.

									Raffi	ending )	Design V	alue, F.,	, (psi)										
Size (in)	Spacing (in)	300	400	500	600	700	800	900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300	2400
2x 6	12.0 16.0 19.2 24.0	6-2 5-4 4-10 4-4	7 1 6 2 5 7 5 0	7-11 6-10 6-3 5-7	8-8 7-6 6-10 6-2	9 5 8 2 7 5 6 8	10-0 8-8 7-11 7-1	10 8 9 3 8 5 7 6	11-3 9-9 8-11 7-11	11-9 10-2 9-4 8-4	12- 4 10- 8 9- 9 8- 8	12-10 11- 1 10- 1 9- 1	13-3 11-6 10-6 9-5	13-9 11-11 10-10 9-9	14-2 12-4 11-3 10-0	14 8 12 8 11 7 10 4	15 1 13 1 1111 10 8	156 135 123 1011	15-11 13-9 12-7 11-3	14- 1 12-10 11- 6	14-5 13-2 11-9	13-6 12-0	12 4
2x 8	12.0 16.0 19.2 24.0	8 1 7 0 6 5 5 9	9-4 81 7-5 6-7	10- 6 9- 1 8- 3 7- 5	11-6 9-11 9-1 8-1	12- 5 10- 9 9- 9 8- 9	13-3 11-6 10-6 9-4	14-0 12-2 11-1 9-11	14-10 12-10 11- 8 10- 6	15-6 13-5 12-3 11-0	16–3 14–0 12–10 11–6	16–10 14– 7 13– 4 11–11	17 6 15 2 13-10 12 5	18– 1 15– 8 14– 4 12–10	18-9 16-3 14-10 13-3	19– 4 16– 9 15– 3 13– 8	19-10 17-2 15-8 14-0	20– <sup>°</sup> 5 17– 8 16– 2 14– 5	20–11 18 1 16- 7 14–10	18 7 16-11 15- 2	19 0 17 4 15 6	17-9 15-10	16-3
2x10	12.0 16.0 19.2 24.0	10– 4 8–11 8– 2 7– 4	11-11 10-4 9-5 8-5	13–4 11–7 10–7 9–5	14 8 12 8 11 7 10 4	15-10 13-8 12-6 11-2	16-11 14-8 13-4 11-11	17–11 15– 6 14– 2 12– 8	18–11 16– 4 14–11 13– 4	19–10 17–2 15–8 14–0	20- 8 17-11 16- 4 14- 8	21-6 18-8 17-0 15-3	22-4 19-4 17-8 15-10	23-1 20-0 18-3 16-4	23–11 20– 8 18–11 16–11	24– 7 21– 4 19– 6 17– 5	25-4 21-11 20-0 17-11	26- 0 22- 6 20- 7 18- 5	23- 1 21- 1 18-11	23-8 21-8 19-4	24-3 22-2 19-10	22 8 20 3	20 8
2x12	12.0 16.0 19.2 24.0	12 7 10-11 9-11 8-11	146 127 116 103	16-3 14-1 12-10 11-6	17-9 15-5 14-1 12-7	19-3 16-8 15-2 13-7	20- 6 17- 9 16- 3 14- 6	21 9 18-10 17 3 15 5	23 0 19-11 18 2 16 3	24 1 2010 19 0 17 0	25-2 21-9 19-11 17-9	22 8 20 8 18 6	23- 6 21- 6 19- 3	24 4 22 3 19-11	25-2 23-0 20-6	2511 23- 8 21- 2	24 4 21 9	25- 0 22- 5	25 8 23 0	23 6	24 1	24-8	25-2
E E E E	12.0 16.0 19.2 24.0	0.15 0.13 0.12 0.11	0.23 0.20 0.18 0.16	0.32 0.28 0.26 0.23	0.43 0.37 0.34 0.30	0.54 0.47 0.43 0.38	0.66 0.57 0.52 0.46	0.78 0.68 0.62 0.55	0.92 0.80 0.73 0.65	1.06 0.92 0.84 0.75	1.21 1.05 0.95 0.85	1.36 1.18 1.08 0.96	1.52 1.32 1.20 1.08	1.69 1.46 1.33 1.19	1.86 1.61 1.47 1.31	2.04 1.76 1.61 1.44	2.22 1.92 1.75 1.57	2.41 2.08 1.90 1.70	2.60 2.25 2.05 1.84	2.42 2.21 1.98	2.60 2.37 2.12	2.53 2.27	2.41

The required modulus of elasticity, E, in 1,000,000 pounds per square inch is shown at the bottom of each table, is limited to 2.6 million psi and less, and is applicable to all lumber sizes shown. Spans are shown in feet-inches and are limited to 26' and less. Check sources of supply for availability of lumber in lengths greater than 20'. Note:

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### TABLE R-3 RAFTERS WITH L/240 DEFLECTION LIMITATION

DESIGN CRITERIA:

Strength – Live Load of 40 psf plus Dead Load of 10 psf determines the required bending design value.

Deflection - For 40 psf live load.

Limited to span in inches divided by 240.

									Rafile	ending I	Design V	alue, F	, (psi)										
Size (in)	Spacing (in)	300	400	500	600	700	800	900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300	2400
2x 6	12.0 16.0 19.2 24.0	5–6 4–9 4–4 3–11	6 4 5 6 5 0 4 6	7 1 6 2 5 7 5 0	7-9 6-9 6-2 5-6	8 5 7 3 6 8 511	9-0 7-9 7-1 6-4	9 6 8 3 7 6 6 <u>9</u>	10– 0 8– 8 7–11 7– 1	10- 6 9- 1 8- 4 7 5	11-0 9-6 8-8 7-9	11- 5 911 9- 1 8 1	11-11 10-3 9-5 8-5	12 4 10 8 9 9 8 8	12-8 11-0 10-0 9-0	13– 1 11– 4 10– 4 9– 3	13-6 11-8 10-8 9-6	13-10 12-0 10-11 9-9	14-2 12-4 11-3 10-0	12-7 11-6 10-3	12-11 11-9 10-6	12 0 10 9	12 4 11 0
2x 8	12.0 16.0 19.2 24.0	7-3 6-3 5-9 5-2	84 7-3 6-7 5-11	9– 4 8– 1 7– 5 6– 7	10-3 8-11 8-1 7-3	11 1 9 7 8 9 7-10	11-10 103 94 84	12- 7 10-10 9-11 8-11	13-3 11-6 10-6 9-4	1311 12- 0 11- 0 9-10	14-6 12-7 11-6 10-3	15–1 13–1 11–11 10–8	15 8 13 7 12 5 11 1	16-3 14-0 12-10 11-6	16–9 14–6 13–3 11–10	17– 3 14–11 13– 8 12– 2	17-9 15-5 14-0 12-7	18-3 15-10 14-5 12-11	18~9 16-3 14-10 13-3	16-7 15-2 13-7	17– 0 15– 6 13–11	15–10 14–2	16-3 14-6
2x10	12.0 16.0 19.2 24.0	9- 3 8- 0 7- 4 6- 6	10- 8 9- 3 8- 5 7- 7	11-11 10-4 9-5 8-5	13 1 11 4 10 4 9 3	14-2 12-3 11-2 10-0	15- 1 13- 1 11-11 10- 8	16-0 13-10 12-8 11-4	16–11 14– 8 13– 4 11–11	17– 9 15– 4 14– 0 12– 6	18-6 16-0 14-8 13-1	19– 3 16– 8 15– 3 13– 7	20-0 17-4 15-10 14-2	20- 8 17-11 16- 4 14- 8	21 4 18 6 1611 15 1	22- 0 19- 1 17- 5 15- 7	22 8 19 7 1711 16 0	23- 3 20- 2 18- 5 16- 6	23-11 20-8 18-11 16-11	21-2 19-4 17-4	21 8 1910 17 <del></del> 9	20-3 18-1	20 8 18 6
2x12	12.0 16.0 19.2 24.0	11-3 99 8-11 7-11	13-0 11-3 10-3 9-2	14 6 12 7 11 6 10 3	15–11 13–9 12–7 11–3	17-2 14-11 13-7 12-2	18-4 15-11 14-6 13-0	19 6 1610 15 5 13 9	20 6 17 9 16 3 14 6	21-7 18-8 17-0 15-3	22- 6 19- 6 17- 9 15-11	23-5 20-3 18-6 16-7	24 4 21 1 19 3 17 2	25-2 21-9 19-11 17-9	26 0 22- 6 20- 6 18- 4	23-2 21-2 18-11	23–10 21– 9 19– 6	24– 6 22 5 20– 0	25– 2 23– 0 20– 6	25 9 23- 6 21- 1	24 1 21 7	24– 8 22– 0	25 2 22 6
E E E E	12.0 16.0 19.2 24.0	0.14 0.12 0.11 0.10	0.22 0.19 0.18 0.16	0.31 0.27 0.24 0.22	0.41 0.35 0.32 0.29	0.51 0.44 0.41 0.36	0.63 0.54 0.50 0.44	0.75 0.65 0.59 0.53	0.88 0.76 0.69 0.62	1.01 0.88 0.80 0.71	1,15 1.00 0.91 0.81	1.30 1.12 1.03 0.92	1.45 1.26 1.15 1.03	1.61 1.39 1.27 1.14	1.77 1.54 1.40 1.25	1.94 1.68 1.54 1.37	2.12 1.83 1.67 1.50	2.30 1.99 1.81 1.62	2.48 2.15 1.96 1.75	2.31 2.11 1.89	2.48 2.26 2.02	2.42 2.16	2.58 2.30

Note: The required modulus of elasticity, E, in 1,000,000 pounds per square inch is shown at the bottom of each table, is limited to 2.6 million psi and less, and is applicable to all lumber sizes shown. Spans are shown in feet-inches and are limited to 26' and less. Check sources of supply for availability of lumber in lengths greater than 20'.

### TABLE R-10 RAFTERS WITH L/240 DEFLECTION LIMITATION

DESIGN CRITERIA:

Strength – Live Load of 30 psf plus Dead Load of 20 psf determines the required bending design value. Deflection – For 30 psf live load. Limited to span in inches divided by 240.

Size	Spacing																									
(in)	(in)											Raf	ter Bendi	ng Design	Value, Ft	" (psi)										
		300	400	500	600	700	800	900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300	2400	2500	2600	2700
		ŀ										h								1			1			
	12.0	5-6	6-4	7-1	7-9	8-S	9-0	9-6	10-0	9-7	10-0	10-5	10-10	11-3	11-7	11-11	12-4	128	13-0	13-3	137	13-11	14-2			
	16.0	4-9	5-6	6-2	6-9	7-3	7-9	8-3	88	8-4	8-8	9-1	y~5	9-9	10-0	10-4	10-8	10-11	11-3	11-6	11-9	12-0	12-4	12-7	12-10	13-1
2x6	19,2	4-4	3-0	5-7	6-2	6-8	7-1	7-6	7~≀1	7-7	7-11	8-3	8-7	8-11	9-2	9-5	9-9	40-0	10-3	10~6	10-9	11-0	11-3	11-3	11-8	11-11-
	24.0	311	4-6	5-0	5-6	5-11	6-4	.6-9	7-1	6-10	.7-1	7-5	7~8	7-11	8-2	8-5	88	8-11	9-2	9-S	9-7	9-10	10-0	10-3	10-5	10-8
		1			1	· ·	1	•			•			•	н. н.				l	1			1			
	12.0	7-3	8-4	9-4	t0-3	11-1	11-10	12-7	13-3	12-8	t3-3	13-9	14-4	14-40	15-3	15-9	16-3	16-8	17-1	17-6	17-11	18-4	18-9			}
	16,0	6-3	7-3	8-1	8-11	9-7	10-3	10-10	11-6	11-0	11~6	11-11	12-3	12-10	13-3	13-8	14-0	14-5	14-10	15-2	15-6	15-10	16-3	16-7	16-10	17-2
2x8	19.2	5-9	6-7	7-3	8-1	8-9	9-4	9-11	10-6	10-0	10-6	10-11	11-4	11-8	12-1	12-5	12-10	13-2	13-6	13-10	14-2	14-6	14-10	15-1	15-5	158
	24.0	5-2	5-11	6-7	7-3	7-10	8-4	8-11	9-4	9-0	9-4	9-9	10-1	10-6	10-10	11-2	11-6	11-9	12-1	12-5	12-8	12-11	13-3	13-6	13-9	14-4)
								<u>.</u>		F	1				[						· .			·		
	12.0	9-3	10-8	1 n-n	13-1	14-2	15-1	16-0	16-11	16-2	16-11	17-7	18-3	18-11	196	20-1	20-8	21-3	21-10	22-4	22-10	23-5	23-11			
	16.0	8-0	9-3	10-4	11-4	12-3	13-1	13-10	14-8	14-0	14-8	15-3	15-10	16-4	16-11	17-5	17-13	18-5	18-11	19-4	19-10	20-3	20-8	21-1	21-6	21-11
2x10	19.2	7-4	8-5	9-3	10-4	11-2	11-11	128	13-4	12-9	13-4	13-11	14-5	14-11	13-3	13-11	16-4	1610	17-3	17-8	18-1	18-6	18-11	19-3	19-8	20-0
	24.0	6-6	7-7	85	9-3	10-0	10-8	11-4	11-11	11-5	11-11	12-5	12-11	13-4	13-9	14-3	14-8	15-0	15-5	15-10	16-2	16-6	16-1	17-3	17-7	17-11
								1					1		1		1				1					
	120	n_ <b>x</b>	13-0	14.6	15-11	17-2	28-4	19-6	20-6	19_8	20-6	21-5	22-2	23-0	23-9	24-5	25-2	25-10		·						
	16.0	0_9	11-3	12_2	13.0	14-11	15-11	16-10	17-9	12-0	17-9	18-6	19-3	19-11	20-6	21-2	21-9	22-5	23-0	23-6	24-1	24-8	23-2	25-8		
2 12	10.0	8-11	10-3	12=1	12-7	13-7	14-6	15-5	16-3	15-7	16-3	16-11	17-6	18-2	18-9	19-4	19-11	20-5	21-0	21-6	22-0	22-6	23-0	23-5	23-11	24-4
2.412	24.0	2-11	9-2	10-3	11-3	12-2	13-0	13-9	14-6	13-11	14-6	15-1	15-8	16-3	16-9	17-3	17-9	18-3	18-9	19-3	19-8	20-1	20-6	21-0	21-5	21-9
	14.0	7-11		10.5	+									1	<u></u>										1	
- ·	10.0	a	1.10	0.02	0.21	11.20	0.47	0.56	0.66	0.77	1.88	0.99		1 22	135	148	1.61	1.75	1.89	2.03	2.18	2.33	2.48			
E.	12.0	0.11	0.17	10.20	1.56	0.30	0.41	0.30	0.57	0.67	0.76	0.86	0.96	1 06	1.17	1.28	1.39	1.51	1.63	1.76	1.88	2.01	2.15	2.28	2.42	2.56
5	10,0	0.09	0.14	0.20	0.20	0.33	0.71	0.49	0.52	0.61	0.69	0.78	0.87	0.97	1.07	1.17	1.27	1.38	1.49	1.60	1.72	1.84	1.96	2.08	2.21	2,34
÷	17.4	0.02	0.13	0,10	6.22	0.22	0.33	0.40	0.46	0.54	0.62	0.70	0.78	0.87	0.95	1.04	1.14	1.23	1.33	1.43	1.54	1.64	1.75	1,86	1.98	2.09
	47,17	w,wo	W. 1.4	1	1	1	L		1												· · · · · · · · · · · · · · · · · · ·			<u>.</u>	· · · · · ·	

Note: The required modulus of elasticity, E, in 1,000,000 pounds per square inch is shown at the bottom of each table, is limited to 2.6 million psi and less, and is applicable to all lumber sizes shown. Spans are shown in feet-inches and are limited to 26' and less. Check sources of supply for availability of lumber in lengths greater than 20'.

DEPARTMENT OF COMMERCE

**Comm 20 APPENDIX** 

### TABLE R--11 RAFTERS WITH L/240 DEFLECTION LIMITATION

### DESIGN CRITERIA: Strength – Live Load of 40 psf plus Dead Load of 20 psf determines the required bending design value. Deflection – For 40 psf live load. Limited to span in inches divided by 240.

Size	Spacing	Rofter Bending Design Value Fu (nsi)																								
(in)	(in)											Rafi	er Bendin	g Design	Value, F <sub>h</sub> ,	(psi)										
		300	400	500	60X)	700	800	900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300	24(X)	2500	26610	2700
															19.0			12.10				10.0	1.0 4			
	12,0	5-0	2-10	6-6	7-1	/8	8~2	8-8	9-2	10-0	11-0	11-5	11-11	12-4	12-8	13-1	13-0	13-10	14-2	14-1	14-11	15-5	15-7	10-11		
	16,0	4-4	5-0	5-7	6-2	6-8	7-1	7-6	7-11	9-1	9-6	9-11	10-3	108	11-0	11-4	118	12-0	12-4	12-7	12-11	13-2	13-6	13-9	14-17	14-3
2x6	19,2	4-0	4-7	5-1	5-7	6-1	6-6	6-10	7-3	X-4	8-8	9-1	9-5	9-9	10-0	10-4	10-8	10-11	11-3	11-6	11-9	12-0	12-4	12-7	12-10	13-1
	24,0	3-7	4-1	4-7	5-0	<u>&gt;-&gt;</u>	5-10	02	0-0	1-2	7-9	8-1	8-3	8-8	9-0	9-3	9-0	9-9	10-0	10-3	10-0	10-9	11-0	11-3	11-2	11-8
	12.0	6-7	7-8	8-7	9.4	10-1	10-10	11-6	12~1	13-11	14-6	15–1	15-8	16-3	16-9	17-3	17-9	18-3	18-9	19-2	198	20-1	20-6	20-51		
· · · · ·	16.0	5-9	6-7	7-5	8-1	8-9	9-4	9-11	10-6	12-0	12-7	13-1	137	14-0	14-6	14-11	15-5	15-10	16-3	16-7	17-0	17-5	17-9	18-1	18-6	18-10
2x8	19.2	5-3	6-0	6-9	7-5	8-0	8-7	9-1	97	11-0	11-6	11-11	12-5	12-10	13-3	13-8	14-0	14-5	14-10	15-2	15-6	15-10	16-3	16-7	16-10	17-2
	24.0	4-8	5-5	6-0	6-7	7-2	7-9	8-1	8-7	9-10	10-3	10-8	11-1	116	11-10	122	12-7	12-11	13-3	13-7	13-11	14-2	14-6	14-10	15-1	15-5
i					· · · · · · · · · · · · · · · · · · ·																· · · · ·					
	12.0	8-5	9-9	10-11	11-11	12-11	13-9	14-8	15-5	17-9	18-6	19-3	20-0	20-8	21-4	22-0	228	23-3	23-11	24-6	25-1	257				
	16.0	7-4	8-5	9-5	10-4	11-2	11-11	12~8	13-4	15-4	16-0	16-8	17-4	17-11	18-6	19-1	19-7	20-2	20-8	21-2	21-8	22-2	22-8	23-1	23-7	24-0
2×10	19.2	6-8	7-8	8-7	9-5	10-2	10-11	11-7	12-2	14-0	14-8	15-3	15-10	16-4	16-11	17-3	17-11	18-5	18-11	19-4	19-10	20-3	20-8	21-1	21-6	21-11
	24.0	6-0	6-11	7-8	8-5	9-1	9_9	10-4	10-11	12-6	13-1	13-7	14-2	14-8	15-1	15-7	16-0	16-6	16-11	17-4	17-9	18-1	18-6	18-11	19-3	19-7
		<u>.</u>				<u> </u>											<u> </u>							i —		<b></b>
	12.0	10-3	11-10	13-3	14-6	158	-16-9	.17-9 .	18-9	21-7	22-6	23-5	24-4	25-2	26-0				·							
	16.0	8-11	10-3	11-6	12-7	13-7	14-6	15-5	16-3	18-8	19-6	20-3	21-1	21-9	22-6	23-2	23-10	24-6	25-2	25-9						
2x12	19.2	8-1	9-4	10-6	11-6	12-5	13-3	14-1	14-10	17-0	17-9	18-6	19-3	19-11	20-6	21-2	21-9	22-5	23-0	23-6	24-1	25-2	25-8			
	24,0	7-3	8-5	9-4	10-3	11-1	11-10	12-7	13-3	15-3	75-11	16-7	17-2	17-9	18-4	18-11	19-6	20-0	20-6	21-1	21-7	22-0	22-6	23-0	23-5	23-10
												-					r									
Е	12.0	0.11	0.17	0,24	0.31	0.39	0.48	0,57	0.67	0.76	0,86	0.97	1,09	1.21	1.33	1.46	1.59	1.72	1.86	2.00	2.14	2.29	2.44	2,60		1
E	16.0	0.09	0,15	0.20	0.27	0,34	0.41	0.49	0,58	0.66	0.75	0.84	0.94	1.05	1.15	1.26	1.37	1,49	1.61	1.73	1.86	1.99	2,12	2.25	2.39	2.53
E	19.2	0,09	0.13	0.19	0.24	0,31	0.38	0.45	0,53	0.60	0.68	0.77	0.86	0.95	1.05	1.15	1.25	1.36	1.47	1.58	1.70	1.81	1,93	2.05	2.18	2.31
E	24.0	0.08	0.12	0.12	0.22	0,28	0.34	0,40	0.47	0.54	0,61	0.69	0.77	0.85	0.94	2.03	1.12	1.22	1.31	1.41	1.52	1.62	1,73	1.84	1.95	2.06
	001			6.1	the T	- 1 00/	000	undo no		منطمحن	ahauun	at the b	ottom of	coach to	bla ic li	mitad to	26 mi	llion nei	and les	e and i	e applie	ablato	all form	borciza	chown	Spane

Note: The required modulus of elasticity, E, in 1,000,000 pounds per square inch is shown at the bottom of each table, is limited to 2.6 million psi and less, and is applica are shown in feet-inches and are limited to 26' and less. Check sources of supply for availability of lumber in lengths greater than 20'.

**Comm 20 APPENDIX** 

TABLE R–14 RAFTERS WITH L/180 DEFLECTION LIMITATION

DESIGN CRITERIA: Strength – Live Load of 30 psf plus Dead Load of 10 psf determines the required bending design value. Deflection – For 30 psf live load. Limited to span in inches divided by 180.

Size	Spacing	-																												
(in)	(In)													Ra	ter Hendla	ig Deskyn	Value, F <sub>h</sub> ,	(psł)												
		200	300	400	300	600	700	800	900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300	2400	2500	2000	2700	2800	2900	3000
	12.0	3-2	3-11	4-6	5-1	5-6	6-0	6-5	6-9	7-2	7-6	7-10	8-2	8-5	8-9	9-0	9-4	9-7	9-10	10-1	10-4	15-3	10-10	11-1	1				[	1 1
	16.0	2-9	3-5	3-11	4-4	4-10	5-2	5-6	310	0-2	6-6	6-9	7-1	7-4	7-7	7-10	8-1	8-40	8-6	8-9	9-0	13-2	9-5	9-7	9-9	10-0				
2x4	19.2	2-6	3-1	3-7	4-0	14	4-9 -	5-1	5-4	5-8	3-11	6-2	6-5	6-8	6-61	7-2	7-4	7-7	7-9	8-0	8-2	12-0	8-7	8-9	8-11	9-i	9-3	9-5		
	24.6	2-3	2-9	3-2	3-7	3-11	4-3	4-6	4-10	5-1	5-4	5-6	3-9	6-0	6-2	6-5	6-7	6-9	7-0	7-2	74	10-9	7-8	7-10	8-0	8-2	8-4	8-5	8-1	8-9
								<u> </u>																						
	12.0	5-0	6-2	7~1	7-11	8-8	9-5	10-0	10-8	11-3	11-9	12-4	12-10	13-3	13-9	14-2	14-8	15-1	15-6	15-11	16-3	15-3	17-0	17-5						
	16.0	4-4	54	6-2	0-10	7-6	8-2	8-8	9-3	9-9	10-3	10-8	11-1	11-6	11-11	72-4	12-8	13-1	13-5	13-9	74-1	13-2	14-9	13-1	13-4	15-8			F	
2x6	19,2	4-0	4-10	3-7	6-3	6-10	7-5	7-11	8-5	8-11	.9-4	9-9	10-1	10-6	30-10	71-3	11-7	11-11-	12-3	12-7	12-10	12-0	13-6	13-9	14-0	14-4	14-7	14-10		
	24.0	3-7	4.4	5-0	3-7	<del>0-2</del>	6-8	7-1	7-6	7-11	8-3	8-8	9-1	9-5	9_9	10-0	10-4	10-8	10-11	11-3	11-6	10-9	12-0	12-4	12-7	12-10	13-1	13-3	13-6	13-9
																											<u> </u>			
	12.0	6-7	8-1	9-4	10-6	11-6	12-5	13-3	14-0	14-10	15-6	16-3	16-10	17-6	181	18-9	19-4	19-10	20-5	20-11	21-5	20-1	22-5	22-11						
	16,0	5-9	7-0	8-1	y_1	9-11	10-9	11-6	42-2	12-10	13-5	14-0	14-7	15-2	15-8	16-3	10-9	17-2	17-8	18-1	18-7	17-5	19-3	19-10	20-3	208			-	
2x8	19.2	5-3	6-3	7-5	8-3	9-1	9-9	10-6	11-A	11-8	12-3	12-10	13-4	13-10	14-4	14-10	13-3	15-8	16-2	16-7	11-11	13-10	17-9	18-1	18-6	78-10	19-3	19-7		
	24.0	4-8	5-9	o-7	7-5	8-1	8-9	9-4	9-11	10-6	11::0.	11-6	11-11	125	12-10	13-3	13-8	14-0	14-3	14-10	15-2 -	14-2	15-10	16-3	16-7	10-10	17-2	17-6	17-10	18-1
	12,0	8-5	10-4	12-21	13-4	14-8	15-10	16-11	17-11	18-11	19-10	208	21-6	22-4	23-1	23-11	24-7	25-4	26-0			257					4.1			
	16.0	7-4	<u>я–</u> 11	10-4	11-7	12-8	13-8	14-8	15-6	<b>16</b> -4	17-2	11-11	18-8	19-4	20-0	20-8	21-4	21-11	22-6	23-1	23-8	22-2	24-10	25-4	25-10				-	
2x10	19,2	6-8	8-2	9-5	10-7	11-7	12-0	13-4	14-2	14-11	138	16-4	17-0	17-8	18-3	18-11	19-6	20-0	20-7	21-1	21-8	20-3	22-8	23-1	23-7	24-1	24-0	25-0	• ·	
	24.0	6-0	7-4	8-5	9-5	10-4	11-2	11-11	128	73:4	14-0	14-8	15-3	15-10	16-4	16-11	17\$	17-11	18-3	18-11	19-4	18-1	20-3	20-8	21~1	21-6	21-13	22-4	22-9	23-1
																														í T
Е	12,0	0.06	0.11	0.17	0,24	0.32	0,40	0,49	0.59	0.69	0.79	0.91	1.02	1.14	1.27	1.39	1.53	1.66	1,80	1.95	2,10	2.29	2,40	2.56						( .
Б	10,0	0.05	0.10	0,15	0,21	0.28	0,35	0,43	0.51	0.60	0.69	0.78	0,88	0.99	7.10	1.21	1.32	1,44	1.56	1.69	1.82	1.99	2,08	2.22	2.36	2.50		· · · ·		
E	19.2	0.05	0,09	0.14	0,19	0.25	0,32	0.39	0.47	0.54	0,63	0,72	0.81	0.90	1.00	1,10	1.21	1.32	1.43	1.54	1,66	1.78	1.90	2.03	2.15	2,28	2,42	2.55		
E	24,0	0,04	0.08	0,12	0.17	0.23	0.29	0.35	0,42	0.49	0.56	0,64	0,72	0.81	0.89	0.99	1.08	1.18	1.28	1.38	1,48	1.59	1.70	1.81	1.93	2.04	2.16	2,28	2,41	2.53

Note: The required modulus of elasticity, E, in 1,000,000 pounds per square inch is shown at the bottom of each table, is limited to 2.6 million psi and less, and is applicable to all lumber sizes shown. Spans are shown in feet-inches and are limited to 26' and less. Check sources of supply for availability of lumber in lengths greater than 20'.

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DEPARTMENT OF COMMERCE

### TABLE R-15 RAFTERS WITH L/180 DEFLECTION LIMITATION

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DESIGN CRITERIA:

Register, February, 1999, No. 518

Strength – Live Load of 40 psf plus Dend Load of 10 psf determines the required bending design value. Deflection – For 40 psf live load. Limited to span in inches divided by 180.

	· · ·																													· · · · · ·
Size	Spacing																													
(in)	(in)													Ra	tter Rendl	ng Design	Value, Fin	(psi)												
		200	300	400	500	700	700	800	900	1000	1300	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300	2400	2500	2600	2700	2800	2900	3000
		1								ŀ							ł			1			1				1			
	12,0	2-10	3-6	4-0	4-6	4-11	5-4	5-9	6-1	6-5	68	7-0	7-3	77	7-10	8-1	8-4	8-7	8-10	9-1	9-3	9-6	9-8	9-11	10-1	4	1			1 1
	16,0	2-6	3-0	3-6	3-11	43	48	4-11	5-3	5-6	3-10	6-1	6-4	6-7	6-9	7-0	7-3	7-5	7-8	7-10	8-0	8-2	8-5	8-7	8-9	8-11	9-1	1		
2x4	19.2	2-3	2-9	3-2	3-7	3-11	4-3	4-6	<del>4-10</del>	5-1	3-4	3-6	5-9	6-0	6-2	6-3	6-7	6-9	7-0	7-2	7-4	7-6	7-8	7-10	8-0	8-2	8-4	8-3	87	
	24.0	2-0	2-0	2-10	3-2	3-0	3-9	4-0	1-3	4-6	4.9	4-11	5-2	5-4	5-0	5-9	5-11	6-1	6-3	6-5	6-7	6-8	6-10	7-0	7-2	7-3	7-5	7-7	78	7-10
		<u> </u>		·	-			-	l								1	1	1											
	120	4.4	5.6	6.4 ·	7-1	7_0	x5	9-0	9-6	10-0	10-6	11-0	11-5	11-11	12-4	128	13-1	13-6	13-10	14-2	14-7	14-11	15-3	15-7	15-11					
ļ	16.0	5.0	3-0	<u>.</u>	7.2	~ *	7.3	7-9	2.3	8-8	9-1	9-0	9-11	10-3	10-8	11-0	11-4	11-8	12-0	12-4	12-7	12-11	13-2	13-6	13-9	14-0	14-3			
102	16.0	1-11	34	5-0	5.7	6-2	6-8	7-1	7-6	3-0 7-11	8-4	8-8	9-1	9-5	9-9	10-0	10-4	<b>\$0-8</b>	10-11	11-3	11-6	11-9	12-0	12-4	12-7	12-10	11-1	13-3	13-6	
	24.0		3-11	4-6	3-0	5-6	3-11	11-14	6-9	7-1	7-5	7-9	8-1	8-5	8-8	9-0	9-3	9-6	9-9	10-0	10-3	10-6	10-9	11-0	11-3	11-3	11-8	11-11	12-1	12-4
	14.0	·-~				- · · ·			<u> </u>					<del> </del>									1		· · · · ·					
																		17 0	14.7	10.0	10.1	10.9	20.1	20.6	20.11					1. 1
	12.0	5-11	73	8-4	9-	10-3	11-1	11-10	2-1	13-3	13-11	14-0	13-1	10-0	40-5	10-7	11-3	17-7	10-2	10-7	12 1	12.0	12.5	12.0	18-1	18-6	18-30			┝───┥
	16.0	5-2	6-3		X-1	8-11	9-7	10-3	10-10	11-0	12-0	12-7	13-1	13-7	14-0 15 18-	14-0	14-11	13-3	13-10	10-3	15.2	17-0		16-1	16-1	10-0	17_2	17-6	17-10	
2x8	19.2	4-8	5-9	6-7	7-5	8-1	8-9	<u><u><u></u></u></u>	9-11	10-0	11-0	11-0	11.41	12-3	14-10	13-3	15-5	13-7	19-51		127-	10-0	14-2	14-6	14-10	15-1	155	15-8	15-10-	16-3
	24,0	+-2	3-+2	>-11	°−/	7-3	7-10	8-4 	3-11	9-4	9-10	10-0	10~0	11-1	11-0	11-10	112-1	12-1	12-11		1.0-1					1.5=1	12-2	12 4		<u> </u>
	l																								ŀ					ŀ I
	12.0	7-7	9-3	108	11-11	13-1	14-2	15-1	16-0	16-11	17-9	18-6	19-3	200	208	21-4	22-0	22-8	23-3	23-11	24-6	25-1	25-7							
	16,9	6-6	8-0	9-3	10-1	11-1	12-3	11-1-	13-10	14-8	15-4	16-0	16~8	17-4	17-11	18-6	19-1	197	20-2	20-8	21-2	21-8	22-2	22-8	23-1	23-7	24-0			
2x10	19,2	6-0	7-4	8-5	9-5	10-1	11-2	11-11	12-8	13-4	14-0	14-8	13-3	15-10	16-4	10-11	17-3	17-11	18-5	18-11	19-4	19-10	26-3	208	21-1	21-0	21-11	22-4	22-9	
	24,0	3-4	0-0	7-7	8-5	9-3	10-0	10-8	11-4	11-11	12-6	13-11	13-7	14-2	14-8	15-1	15-7	16-0	16-6	10-11	17-4	17-9	18-1	18-6	18-11	19-3	19-7	20-0	20-4	20-8
R	12.0	0.05	611	0.17	0.23	0.31	0.38	0.47	0.56	0.66	0.76	0,86	0.97	1,09	1.21	1.33	1.46	1.59	1.72	1,86	2,00	2.14	2.29	2.44	2.60					
<u> </u>	10.0	0.05	0.04	014	0.20	0.26	0.33	0.41	0.49	0.57	0.66	0.75	0.84	0.94	1.05	1.13	1,26	137	1.49	1,61	1,73	1.86	1.99	2,12	2.25	2.39	2.53			
F	19.5	0.05	0.09	813	0.18	0.24	0.30	0.37	0.44	0.52	0.60	0.68	0.77	0.80	0,95	1.05	1.15	1,25	1.36	1.47	1.58	1.70	1.81	1.93	2,05	2,18	2,31	2,43	2.57	
-	24.0	0.04	80.0	0.12	0.16	0.22	0.27	0.33	0.40	0.46	0.54	0,61	0.69	0,77	0.85	0.94	1.03	1,12	1.22	131	1.41	1.52	1.62	1.73	1,84	1,95	2,06	2.18	2.30	2.41

Note: The required modulus of elasticity, E, in 1,000,000 pounds per square inch is shown at the bottom of each table, is limited to 2.6 million psi and less, and is applicable to all lumber sizes shown. Spans are shown in feet-inches and are limited to 26' and less. Check sources of supply for availability of lumber in lengths greater than 20'.

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	TA	BLE R-22	
RAFTERS	WITH L/180	DEFLECTION	LIMITATION

DESIGN CRITERIA: Strength – Live Load of 30 psf plus Dead Load of 20 psf determines the required bending design value. Deflection – For 30 psf live load. Limited to span in inches divided by 180.

Size	Spacing																													
(ln)	(ln)						_							Red	ter Bendl	ng Design	Vulue, F <sub>bs</sub>	(psi)							_					
		200	300	400	500	600	700	800	900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300	2400	2300	2600	2700	2800	2900	3000
															[															
	12.0	2-10	3-6	4-0	4-6	4-11	5-4	5-9	6-1	6-5	68	7-0	7-3	7-7	7-10	8-1	8-4	8-7	8-10	90	9-3	9-6	9-8	9-11	10-1	10-4	10-6	108	10-13	11-1
	16,0	2-6	3-0	3-6	3-11	4-3	4-8	τu	3-3	5-6	3-10	6-1	6-4	o-7	0~9	7-0	7-3	75	7-8	7-10	8-0	8-2	8-5	8-7	8-9	8-11	9-1	9-3	9-5	9-7
2x4	19.2	2-3	2-9	3-2	3-7	3-11	4-3	4-0	4-10	5-l	54	5-6	3-9	6-0	6-2	6-5	6-7	0-9	7-0	7-2	7-4	7-6	7-8	7-10	8-0	8-2	8-4	85	8-7	8-9
	24,0	20	20	2-10	3-2	3-6	3-9	4-0	43	4-6	4.9	4-11	\$2	5-4	5-6	5-9	5-11	6-l	6-3	6-5	6-7	6-8	6−10	7-0	7-2	7-3	7-3	7-7	78	7-10
													·																	
	12.0	4-6	5-6	6-4	7-1	7-9	8-5	9-0	9-6	10-0	10-6	11-0	11-5	11-11	12-4	12*	. 13-1	136	13-10	14-2	14-7	14-11	15-3	15-7	15-11	16-2	16-6	16-10	17-1	17 <u>-</u> 5
	16,0	3-11	40	5-0	0-2	6-9	7-3	7-9	\$-3	8-8	9-1	9-6	9-11	10-3 -	10-8	11-0	្រាណ	11-8	12-0	12-4	12-7	12-11	13-2	11-6	13-9	14-0	14-3	14-7	14-10	15-1
2x6	19.2	3-7	4-4		5-7	6-2	64	7-1	7-6	7-11	8-4	8-8	9-1	9-5	9-9	10-0	10-4	10-8	10-11	11-3	11-6	11-9	12-0	12-4	12-7	12-10	13-1	73-3	1.1-0	13-9
	24.0	3-2	3-11	4-0	5-0	5-0	5-11	<del>، ۱</del>	6-9	7-1	7-5	7_9	8-1	8-5	8-8	9-0	9-3	9-6	9-9	0-01	10-3	10-0	10-9	11-0	11-3	11-5	11-8	11-11	12-1	12-4
														[											1					
	12,0	5-11	7-3	84	9-4	103	11-1	11-10	12-7	13-3	13-11	146	15-1	15-8	26-3	16-9	17-3	17-9	18-3	18-9	19-2	19-8	20-1	20-6	20-11	21-4.5	21-9	22-2	22-6	22-11
	16.0	5-2	6-3	7-3	8-1	8-11	9-7	10-3	10-10	11-0	12-0	12-7	3	13-7	14-0	14-0	14-11	15~5	15-10	63	16-7	17-0	17-5	17-9	.18-1	18-0	18-10	19-2	19-6	19-10
2x8	19,2	4-8	5-9	n~7	7-5	8-1	8-9	9-4	9-11	10-6	11-0	11-6	11-11	12-5	12-10	13-3	138	440	14-3	14-10	15-2	13-6	15-10	16-3	16-7	10-10	17-2	17-6	17-10	18-1
	24,0	4-2	5-2	5-11	6-7	7-3	7-10	8-4	8-11	9-4	9-10	10-3	108	11-1	11-6	11-10	12-2	12-7	12-11	13-3	13-7	13-11	14-2	14-6	1410	15-1	15-5	15-8	15~[1	46-3
	12,0	7-7	9-3	108	11-11	13-1	14-2	15-1	16-0	16-11	17-9	18-6	19-3	20-0	20~8	21-4	22-0	228	23-3	23-11	24-6	25-1	25-7				-			
	16,0	6-6	8-0	9-3	10-4		12-3	13-1	13-10	14-8	15-4	16-0	16-8	17-4	17-0	18-6	19-1	19-7	202	20-8	21-2	21-8	22-2	22-8	23-6	23-7	24-0	24-6	24-11	25-4
2x10	19.2	6-0	7-4	8-5	95	10-4	11-2	11-11	12-8	13-4	14-0	74-8	15-3	13-10	10-4	16-11	17-5	17-11	18-5	18-11	19-4	19-10	20-3	20-8	21-1	21-6 -	21-11	22-4	22-9	23-1
	24.0	5-4	60	7_7	8-5	9-3	10-0	10-8	114	11-11	12-6	13-1	13-7	14-2	<sup></sup> 14-8	15-1	15-7	16-0	16-6	10-11	17-4	17-9	18-1	18-6	18-11	19-3	19-7	20-0	20-4	20-8
Ë	\$2.0	0,04	0,08	0.12	0.17	0.23	0.29	0,35	0.42	0.49	0.57	0.65	0,73	0,82	0,91	1.00	1.09	l,19… ·	1,29	1.39	1.50	L61 -	1.72	- 1,83	1.95 -	2,07	2.19	2.31	2,43	2.56
E	16,0	0.04	0.07	0.11	0,15	0,20	0.25	0,31	0.36	0.43	0,49	0.\$6	0.63	0.71	0.78	0,86	0.95	1.03	1.12	1.21	1.30	1.39	1.49	1.59	1.69	1.79	1,89	2,00	2.11	2,22
E	19.2	0.03	0,06	0.10	0.14	0.18	0.23	0,28	0,33	0.39	0.45	0.51	0.58	0,65	0.72	0,79	0.86	0,94	1.02	1,10	1.19	1.27	1,36 .	1.45	1.54	1,63	1.73	1,83	1.92	2,03
£	24.0	0.03	0,06	0.09	0.12	0.16	0,20	0.25	0,30	0.35	0.40	0,46	0.52	0,58	0,64	0.71	0.77	0,84	0.91	1,99	1.06	1.14	1.22	1.30	1,38	1.46	1,35	1.63	1.72	1,81

Note: The required modulus of elasticity, E, in 1,000,000 pounds per square inch is shown at the bottom of each table, is limited to 2.6 million psi and less, and is applicable to all lumber sizes shown. Spans are shown in feet-inches and are limited to 26' and less. Check sources of supply for availability of lumber in lengths greater than 20'.

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### TABLE R-23 RAFTERS WITH L/180 DEFLECTION LIMITATION

DESIGN CRITERIA:

Strength ~ Live Load of 40 psf plus Dead Load of 20 psf determines the required bending design value. Deflection – For 40 psf live load. Limited to span in inches divided by 180.

Silven	Spacing																													
(in)	(in)													Ra	ter Bendi	ng Design	Value, F <sub>b</sub> ,	(psi)												
		200	300	400	500	600	700	800	900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300	2400	2500	2600	2700	2800	2900	3000
																			T											
	12.0	2_7	2.2	2.8	ا بد	4-6	4-11	51	5-6	5-10	6-1	6-5	6-8	6-11	7-2	2-5	7-7	7-10	8-0	8-3	8-5	8-8	8-10	9-0	9-3	9-5	9-7	9-9	9-11	10-1
L	16.0	7-3	7_0	100	27		43	4-6	10	3-1	<u> </u>	3-6	5.4	6-0	5-2	6-5	6-7	6-9	7-0	7-2	7-4	7-6	7-8	7-10	80-	8-2	8-4	8-3	87	8-9-1
214	19.2	2-1	2-6	2-11	3-3	3-7	3-10	4-1	1-1	4-7	1-10	5-1	3-3	5-5	3-8	5-10	6-0	6-2	6-4	6-0	6-8	6-10	7-0	7-2	7-3	7-3	7-7	7_9	7-10	8-0
	24.0	1-10	2.3	2-7	2-11	3-2	3-5	3-8	3-11	4-1	4.4	4-6	48	4-11	3-1	5-3	5-5	5-6	3-8	5-10	6-0	6-1	6-3	6-5	6-6	6-8	6-9	0-11	7-0	12
				· · · ·													·		<u> </u>	<u> </u>										
	-			6.00				<b>v</b> n			0.2	10.0	10.4	10.10	11_1	11.7	11-11	17.4	12.4	100	12.3	13.7	11-11	14.2	14.6	14.0	15.1	16-1	16.7	1511
	12.0	4-1	30	5-10	0-0	11	1/-0 	7 1	2.6	7 31	9-7	10-0	0_1	10-10	0_0	10-0	10-4	10.8	12-0	10-1	1.5-0	11.5	12-0	12-4	17-7	12-10	12.1	11-3	1.5-/	
244	10,0	3-1	4-4	11	3-7	-5 9	(	1-1	6.10	7-3	2.7	7_11	1	8.2	8-11	6_2	0.5	0_0	10-0	I MLT	10-6	10-0	11-0	11-1	11:3	11-8	11-11	12-2	12-4	12-7
23.0	19,2	3-3	3.9	4-1	<u></u>	30	<u> </u>	C10	4-10	<u> </u>	~ 10	1-1-1	7-5	2-8	7-11	100	8-5	8-8	2.1	4.2	6.5	9_7	9_10	10.0	10-3	10-5	10-3	10-10	11-0	
	24.0	2~11	Ţ		<i>+,</i>	~	~~		1-2	,-,		·	~	,0		0.2	<u> </u>								<u></u>					
																				1										
	12.0	5-5	6-7	7-8	8-7	9-1	10-1	10-10	11-6	12-1	128	13-3	13-9	14-4	14-10	15-3	15-9	16-3	16-8	17-4	17-6	17-11	18-4	18-9	19-1	19-6	19-10	20-3	20-7	20-11
	16.0	44	3	o=7 <sup>−</sup>	7-5	8-1	8-9	9~4	9-11	10–6	п <u>-</u> 0	11-0	11-11	12-5	12-10	13-3	13-8	14-0	14-5	14-10	15-2	12-6	15-10	16-3	10-7	16-10	17-2	17-6	17-10	18-1
2x8	19,2	4-3	3-3	\$	6-9	7-5	8-0	8-7	9-1	97	10-0	10-6	10-11	11-4	11-8	12-1	12-\$	12-10	13-2	15~6	13-10	14-2	14-6	14-10	12-1	15-5	15-8	16-0	10-3	16-7
	24,0	3-30	4-8	5-5	69	6-7	7-2	7-8	8-1	8-7	9-0	9-4	9-9	10-1	10-6	10-10	11-2	11-6	11-9	12-1	12-5	12-8	12-11	13-3	1,5-0	13-9	14-0	14-4	14-7	14-10
																			1											
•	12.0	6-11	8-5	9.9	10-11	16-11	12-11	13-9	14-8	15-5	16-2	16-11	17-7	18-3	18-11	19-6	20-1	208	21-3	21-10	22-4	22-10	23-5	23-61	24-5	24-10	25-4	25-10		
	16.0	<del>6-0</del>	7-4	8-5	9-5	10-4	11-2	าม-ม	12-8	13-4	14-0	14-8	13-3	15-10	16-4	16-11	17-5	17-11	18-5	18-11	19-4	<u>19</u> =10	20-1	20-8	21-1	21-6	21-11	22-4	22-9	2,31
2x10	19,2	5-5	6-8	7-8	8-7	9-5	10-2	10-11	71-7	12-2	129	13-4	13-11	14-5	14-11	<sup></sup> 15-5	-13-11	16-4	16-10	17-3	178	18-1	18-6	18-11	19-3	19-8	20-0	20-5	20-9	21-1
	24,0	4-11	6-0	<u>6-11</u>	7-8	8-5	9-1	9-9	10-4	10-11	11-3	11-11	12-5	12-11	11-4	13-9	14-3	14-8	13-0	13-3	15-10.	16-2	16-6	16-11	12-3	17-7	17-11	18~3	18-7	18-11
																				}									· ·	
Е	12.0	0.04	0.08	0.13	0.18	0.23	0.29	0.36	0.43	0.50	0,58	0.66	0.74	0.83	0.92	1.01	1.11	1,21	1.31	1.41	1,52	1.63	1.74	1.86	. 1,98	2.10	2.22	2.34	2.47	2.60
E	16,0	0.04	0.07	0.11	0.15	0,20	0.25	0.31	0.37	0.43	030	0.57	0.64	0.72	0.80	0.88	0.96	1.05	1.6	1.22	1.32	7.41	1,31	1.61	1.71	1.82	1,92	2.03	2.14	2.25
Æ	19.2	0.04	70.06	0.10	0.14	81,0	0.23	0.28	0.14	0,40	0.46	0.52	0,59	0.65	0.73	0.80	0.88	0.95	1.04	1.12	1.20	1.29	1.38	1,47	1.36	1,60	1.75	1,85	1.93	2.05
E	24,0	0,03	0.06	0.09	0,13	0,16	0.21	0.25	0.30	0.35	0,41	0,46	0.52	0,39	0.65	0,72	0.78	0,85	0.93	1.00	1,08	1.15	1.23	1.31	1.40	1.48	1.37	1,66	1,75	1.84

Note: The required modulus of elasticity, E, in 1,000,000 pounds per square inch is shown at the bottom of each table, is limited to 2.6 million psi and less, and is applicable to all lumber sizes shown. Spans are shown in feet-inches and are limited to 26' and less. Check sources of supply for availability of lumber in lengths greater than 20'.

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**Comm 20 APPENDIX** 

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### DEPARTMENT OF COMMERCE

noisinte content in use.	-	Design Value in J	Bending, "Fb"		
Species and Grade	Size	Normal Duration	Snow Loading	Modulus of Flasticity "F"	Grading Rules
Cottonwood			Show Soluting	modulus of Elasticity 12	ingency
Select Structural		1510	1735	1.200.000	
No.I		1080	1240	1.200.000	
No.2		1080	1240	1,100,000	
No.3	2x4	605	695	1.000.000	
Stud		600	690	1.000.000	
Construction		805	925	1.000.000	
Standard		460	530	900.000	
Utility		200	230	900.000	
Select Structural		1310	1505	1.200.000	
No.1		935	1075	1,200,000	
No.2	2x6	935	1075	1,100.000	
No.3		525	600	1.000.000	
Stud		545	630	1.000.000	
Select Structural		1210	1390	1,200,000	NSLB
No.1	2x8	865	990	1,200,000	
No.2		865	990	1,100,000	
No.3		485	555	1,000,000	
Select Structural		1105	1275	1,200,000	
No.1	2x10	790	910	1,200,000	
No.2		790	910	1,100,000	
No.3		445	510	1,000,000	1
Select Structural		. 1005	1155	1,200,000	
No.1	2x12	720	825	1,200,000	r
No.2		720	825	1,100,000	
No.3		405	465	1,000,000	
Douglas Fir-Larch					•
Select Structural		2500	2875	1,900,000	
No.1 & Btr		1985	2280	1,800,000	
No.1		1725	1985	1,700,000	1
No.2		1510	1735	1,600,000	1
No.3	2x4	865	990	1,400,000	1
Stud		855	980	1,400,000	1
Construction		· 1150	1325	1,500,000	
Standard		635	725	1,400,000	1
Utility		315	365	1,300,000	1
Select Structural		2170	2495	1,900,000	
No.1 & Btr		1720	1975	1,800,000	
No.1	2x6	1495	1720	1,700,000	
No.2		1310	1505	1,600,000	
No.3		750	860	1,400,000	-
Stud		775	895	1,400,000	
Select Structural		2000	2300	1,900,000	WCLIB
No.1 & Str		1585	1825	1,800,000	WWPA
No.1	2x8	1380	1585	1,700,000	]
No.2		1210	1390	1,600,000	]
No.3		690	795	1,400,000	]
Select Structural		1835	2110	1,900,000	]
No.1 & Btr		1455	1675	1,800,000	]
No.1	2x10	1265	1455	1,700,000	]
No.2		1105	1275	1,600,000	1

635

1670

1325

1150

1005

575

2x12

725

1920

1520

1325

1155

660

Design Values for Joists and Rafters These "Fb" values are for use where repetitive members are spaced not more than 24 inches. Values for surfaced dry or surfaced green lumber apply at 19% maximum moisture content in use

1,400,000

1,900,000

1,800,000

1,700,000

1,600,000

1,400,000

No.3

No.1

No.2

No.3

ţ

Select Structural

No.1 & Btr

Spectes and Grade         Size         Normal Duration         Snow Leading         Modulus of Elasticity "P"         Grading Rules"         Agency           Douglas Fir-Lack (North)         2245         2580         1/90,000         Agency           No.1 No.2         2425         1633         1600,000         Agency           No.1 No.2         2424         2580         1/90,000         Agency           Standard         2424         2530         940         1/400,000           Standard         063         1/930,000         1/930,000         Standard         1/945         1/930,000           Standard         1945         2235         1/900,000         NLGA         1/945         1/960,000         NLGA           No.1 No.2         2x8         1/140         1/310         1.600,000         NLGA           No.3         2x8         1/140         1/310         1.600,000         NLGA           No.3         2x8         1/405         1/900         1.600,000         NLGA           No.3         2x8         1/405         1/900         1.600,000         NLGA           No.3         2x8         1/405         1/900,000         1.600,000         NO.3         1.600         6600			Design Value in	Bending, "Fb"		
Species and Grade         Nize         Normal Duration         Snow Loading         Modulus of Elasticity "E"         Agency           Select Structural $2245$ 2530 $1,900,000$ $8104$ $2245$ 2530 $1,900,000$ No.1 /No.2 $1425$ $1603$ $1400,000$ $1400,000$ Stand $2x4$ $820$ 943 $1,400,000$ Costruction $1035$ $1225$ $1,500,000$ Standard $2x6$ $1235$ $1,500,000$ No.1 /No.2 $2x6$ $1235$ $1,600,000$ No.1 /No.2 $2x6$ $1735$ $2665$ $1,900,000$ No.1 /No.2 $2x6$ $1735$ $2665$ $1,900,000$ No.1 /No.2 $2x6$ $1140$ $1100$ $1600,000$ No.1 /No.2 $2x10$ $1645$ $1800$ $1,900,000$ No.1 /No.2 $2x10$ $1645$ $1800$ $1,900,000$ No.1 /No.2 $2x10$ $1645$ $1800,000$ $1,800,000$ No.1 /No.2 $2x10$						Grading Rules
	Species and Grade	Size	Normal Duration	Snow Loading	Modulus of Elasticity "E"	Agency
Select Structural         2245         2580         1,900,000           No.1 /No.2         1,425         1,630,000         1,400,000           Stud         220         940         1,400,000           Organization         1985         1,235         1,500,000           Studard         1985         1,235         1,500,000           Organization         605         665         1,400,000           No.1 /No.2         2x6         1235         1,400,000           No.1 /No.2         2x8         1140         1310         1,600,000           No.1 /No.2         2x8         1442         1500         1,900,000           No.1 /No.2         2x10         1045         1200         1,600,000           No.1 /No.2         2x10         1045         1200         1,600,000           No.1 /No.2         2x10         1045         1200         1,600,000           No.1 /No.2         2x10         1645         1890         1,400,000           No.	Douglas Fir-Larch (North)					
No.1 No.2         1425         1635         1,600,000           Stud         2x4         \$20         940         1,400,000           Stud         2x4         \$20         945         1,400,000           Stud         1955         1255         1,500,000           Stud         290         330         1,500,000           Stud         290         330         1,500,000           No.1 No.2         2x6         1235         1420         1,600,000           No.1 No.2         2x6         1235         1420         1,600,000           Stud         750         860         1,400,000         NLGA           Select Structural         1945         1200         1,600,000           No.1 No.2         2x8         1140         1160         1,600,000           No.1 No.2         2x8         1425         1635         1,900,000           No.1 No.2         2x10         1045         1200         1,600,000           No.1 No.2         2x12         350         1,400,000         1,600,000           Select Structural         1495         1720         1,900,000         1,600,000           No.1 No.2         2x14         820 <td< td=""><td>Select Structural</td><td></td><td>2245</td><td>2580</td><td>1,900,000</td><td>· · ·</td></td<>	Select Structural		2245	2580	1,900,000	· · ·
No.3         2x4         220         940         1,400,000           Construction         1955         1255         1,500,000           Stadard         065         695         1,400,000           Othily         290         330         1,500,000           Select Structural         1945         2235         1,900,000           No.1 No.2         2x60         1225         1420         1,600,000           No.1 No.2         2x60         1710         815         1,400,000           No.1 No.2         2x65         1700         815         1,400,000           No.1 No.2         2x8         1140         1310         1,600,000           No.3         655         755         1,400,000           No.1 No.2         2x10         1645         1890         1,900,000           No.3         600         660         1,400,000         1,600,000           No.3         2x10         1645         1890         1,900,000           No.3         2x44         235         1,400,000         1,600,000           No.1 No-2         2x12         555         1,600,000         1,600,000           No.1 No-2         2x12         545         <	No.1 /No.2		1425	1635	1,600,000	, i i i i i i i i i i i i i i i i i i i
Stud         2x4         820         945         1,400,000           Construction         1955         1255         1,500,000           Standard         290         330         1,300,000           Q90         330         1,300,000           Standard         290         330         1,300,000           No.1/No.2         2x6         1235         1420         1,600,000           No.1/No.2         2x6         1235         1420         1,600,000           Stud         750         860         1,406,000         NLGA           No.1/No.2         2x8         1140         1310         1,600,000           No.1/No.2         2x8         1645         1890         1,900,000           No.1/No.2         2x10         1645         1890         1,900,000           No.1/No.2         2x10         1645         1890         1,900,000           No.1/No.2         2x10         1645         1890         1,900,000           No.1/No.2         2x12         950         1090         1,400,000           No.1/No.2         2x12         1555         1755         1,400,000           No.1/No.2         2x12         1555         1755 <td>No.3</td> <td></td> <td>820</td> <td>940</td> <td>1,400,000</td> <td>· 1</td>	No.3		820	940	1,400,000	· 1
Construction         1995         1,255         1,500,000           Sindard         605         695         1,400,000           Select Structural         1945         2235         1,906,000           No.1 No.2         2x6         1235         1420         1,600,000           No.3         1705         2665         1,900,000         NLGA           Select Structural         2x8         1140         1310         1,600,000           No.1 No.2         2x8         1140         1310         1,600,000           No.1 No.2         2x8         1140         1310         1,600,000           No.1 No.2         2x8         1140         1310         1,600,000           No.3         655         755         1,400,000           Select Structural         1645         1890         1,900,000           No.1 No.2         2x10         1645         1300         1,900,000           No.1 No.2         2x12         550         1,400,000         1,400,000           No.1 No.2         2x42         535         1,200,000         1,400,000           No.1 No.2         2x42         530         1,400,000         1,400,000           No.2         1,2245<	Stud	2x4	820	945	1,400,000	
Standard         605         695         1,400,000           Select Structural         290         330         1,300,000           Select Structural         1945         2235         1,960,000           No.1 /No.2         2x6         1235         1420         1,660,000           Stud         750         860         1,400,000         NLGA           Stud         750         860         1,400,000         NLGA           No.1 /No.2         2x8         1140         1310         1,600,000           No.3         655         755         1,400,000         Select Structural         1643         1890         1,900,000           No.3         660         690         1,400,000         1,600,000         Select Structural         1445         1600         1,400,000           No.3         2x12         950         1090         1,600,000         Select Structural         1,400,000         1,600,000           No.3         2x42         950         1090         1,600,000         Select Structural         1,400,000         1,400,000           No.3         2x42         2580         1,400,000         Select Structural         1,400,000         Select Structural         1,400,000 <td< td=""><td>Construction</td><td></td><td>1095</td><td>1255</td><td>1,500,000</td><td></td></td<>	Construction		1095	1255	1,500,000	
Utility         290         330         1,300,000           Select Structural         1945         2235         1,900,000           No.1 No.2         2x6         1945         2235         1,900,000           No.3         710         815         1,400,000         NLGA           Select Structural         750         860         1,400,000         NLGA           No.1 No.2         2x8         1140         1310         1,600,000           No.1 No.2         2x8         1440         1310         1,600,000           No.1 No.2         2x8         1645         1890         1,900,000           No.1 No.2         2x10         1645         1890         1,900,000           No.3         2x12         1995         1720         1,900,000           No.1 No.2         2x12         1995         1720         1,900,000           No.1 No.2         2x12         1555         1785         1,300,000           No.2         2x12         1955         1785         1,300,000           No.3         2x4         2245         2580         1,400,000           Stad         1345         1425         1200,000           No.3         2x4 </td <td>Standard</td> <td></td> <td>605</td> <td>695</td> <td>1,400,000</td> <td></td>	Standard		605	695	1,400,000	
Select Structural         1945         2235         1,900,000           No.1 /No.2         2x6         1235         1420         1,600,000           No.3         750         860         1,400,000         NLGA           Stud         750         860         1,400,000         NLGA           Solext Structural         1795         2605         1,900,000         NLGA           No.1 /No.2         2x8         1140         1310         1,600,000           No.3         655         755         1,400,000         Selext Structural         1645         1890         1,900,000           No.3         650         0.90         1,400,000         Selext Structural         1495         1,200         1,600,000           No.3         2x12         950         1090         1,600,000         Selext Structural         1495         1,200,000         Selext Structural         1,400,000         Structural         1,400,000         Selext Structural         1,400,000         Selext Structural         1,400,000         Selext Structural         1,400,000         Selext Structural         Selext Structural         1,400,000         Structural         Selext Structural         Selext Structural         Selext Structural         Selext Structural         Selext St	Utility		290	330	1,300,000	
No.1 /No.2         2x6         1235         1420         1,600,000           Sid         710         815         1,400,000           Sud         750         860         1,400,000           No.1 /No.2         2x8         1195         2065         1,560,000           No.1 /No.2         2x8         1140         1310         1,600,000           No.3         2x8         1645         1890         1,900,000           No.1 /No.2         2x10         1645         1200         1,600,000           No.3         655         755         1,400,000         1,600,000           No.3         1645         1200         1,600,000         1,600,000           No.3         1495         1720         1,900,000         No.6           No.1 /No.2         2x12         505         1600         1,600,000           No.3         1405         1720         1,900,000         No.6           No.1 /No.2         2x12         510         1000         1,600,000           No.1 /No.2         2x42         5230         1,400,000           No.1         105         1,200,000         1,000,000           Stud         2x4         820	Select Structural		1945	2235	1,900,000	
No.3         710         8.15         1,400,000           Suid         750         8.66         1,400,000           Select Structural         1795         2065         1,960,000           No.1 No.2         2x8         1140         1310         1,600,000           No.1 No.2         2x8         1140         1310         1,600,000           Select Structural         1645         1890         1,900,000           No.1 No-2         2x10         1645         1200         1,600,000           No.3         1495         1720         1,900,000         1,400,000           Select Structural         1495         1720         1,900,000         1,600,000           No.3         2x12         950         1090         1,600,000         1,600,000           No.3         2x42         250         1,400,000         1,400,000         1,400,000           No.1         155         1735         1,300,000         1,400,000         1,400,000           No.1         155         1735         1,300,000         1,400,000         1,400,000         1,400,000         1,400,000         1,400,000         1,400,000         1,400,000         1,400,000         1,400,000         1,400,0000	No.1 /No.2	2x6	1235	1420	1,600,000	
Stud         750         860         1.400.000         NLGA           Select Structural         1795         2065         1.900.000           No.1 /No.2         2x.8         1140         1310         1.600.000           No.3         655         755         1.400.000           Select Structural         1645         1890         1.900.000           No.1 /No.2         2x.10         1045         1200         1.600.000           No.3         0.00         600         690         1.400.000           Select Structural         1495         1720         1.900.000           No.3         2x12         950         1060         1.600.000           No.3         2x42         950         1.400.000         1.600.000           No.1         1425         1635         1.200.000         1.600.000           No.1         1425         1635         1.200.000         1.000.000           No.1         12245         2580         1.400.000         1.000.000           No.1         1245         1635         1.200.000         1.000.000         1.000.000           Stud         20         940         1.100.000         1.000.000         1.000.000	No.3		710	815	1,400,000	1
Select Structural         1795         2065         1,900,000           No.1 No.2         2x8         1140         1310         1,600,000           No.3         655         755         1,400,000           Select Structural         1645         1890         1,900,000           No.3         1645         1890         1,600,000           No.3         060         660         1,400,000           No.3         2x10         1495         1720         1,900,000           No.1 No.2         2x12         950         1090         1,600,000           No.1 No.2         2x12         255         1,200,000         1,600,000           No.1         2x42         245         2580         1,400,000           No.1         1555         1785         1,300,000         1,100,000           No.2         2x4         820         945         1,100,000           Stud         2x4         820         945         1,100,000           Standard         1945         2235         1,400,000           No.1         2x6         1235         1420         1,200,000           No.1         2x6         1235         1,400,000         1,000,000 <td>Stud</td> <td></td> <td>750</td> <td>860</td> <td>1,400,000</td> <td>NLGA</td>	Stud		750	860	1,400,000	NLGA
No.1 No.2         2x8         1140         1310         1,600,000           No.3         655         755         1,400,000           Select Structural         1645         1890         1,900,000           No.3         600         6690         1,600,000           No.3         1495         1720         1,900,000           No.3         1495         1720         1,900,000           No.3         2x12         950         1090         1,600,000           No.1         1555         1785         1,300,000           No.1         1555         1785         1,300,000           No.1         2x44         820         940         1,100,000           Stud         2x0         945         1,100,000           Stud         2x35         1,400,000         1065           No.1         1345         1545         1,300,000           No.1         1945         2235         1,400,000	Select Structural		1795	2065	1,900,000	1 1
No.3         655         755         1.400,000           Select Structural         1645         1890         1.900,000           No.3         2x10         1045         1200         1.600,000           No.3         600         690         1.400,000           Select Structural         1495         1720         1,900,000           No.3         2x12         950         1090         1.600,000           No.3         2x12         950         1090         1.600,000           No.3         2x41         545         630         1,400,000           Dauglas Fir-South         2245         2580         1,400,000           No.1         1555         1785         1,300,000           No.1         2x4         820         940         1,100,000           Stud         1065         1225         1,200,000           Stundard         065         695         1,100,000           Utility         290         330         1,000,000           No.1         1345         1545         1,300,000           No.2         2x6         1235         1,400,000           No.1         2x6         1240         1,200,000	No.1 /No.2	2x8	1140	1310	1,600,000	}
Select Structural         1645         1890         1,900,000           No.1 No-2         2x10         1045         1200         1,600,000           No.3         600         690         1,400,000         1,900,000           Select Structural         1495         1720         1,900,000           No.1 No.2         2x12         950         1090         1,600,000           No.3         545         630         1,400,000           Douglas Fir-South         555         1785         1,300,000           No.1         1555         1785         1,300,000           No.2         1425         1635         1,200,000           No.3         2x4         820         940         1,100,000           Stud         605         695         1,100,000         1065           Stud         1945         2235         1,400,000           Vility         290         330         1,000,000           No.1         1945         1235         1,400,000           No.1         2x6         1235         1,400,000           No.1         2x6         1345         1,500,000           No.1         2x6         1235         1,400,000	No.3		655	755	1,400,000	1
No.1 /No-2         2x10         1045         1200         1,600,000           No.3         600         690         1,400,000           Select Structural         1495         1720         1900,000           No.1 /No.2         2x12         950         1090         1,600,000           No.3         545         630         1,400,000           Douglas Fir-South         5         1785         1,200,000           No.1         1425         1635         1,200,000           No.1         1425         1635         1,200,000           No.1         1425         1635         1,200,000           No.1         2x4         820         940         1,100,000           Stud         1065         1225         1,200,000           Stud         1065         1225         1,200,000           Villity         290         330         1,000,000           No.1         2x6         1345         1545         1,300,000           No.1         2x6         1235         1,400,000         1,00,000           No.1         2x6         1345         1,400,000         1,00,000           No.1         2x6         1235         1,	Select Structural		1645	1890	1,900,000	1 [
No.3         600         600         600         1400,000           Select Structural         2x12         950         1090         1,600,000           No.3         2x12         950         1090         1,600,000           No.3         2x12         950         1090         1,600,000           No.3         2x12         950         1090         1,600,000           Donglas Fir-South         2245         2580         1,400,000           Select Structural         1355         1785         1,200,000           No.2         1425         1635         1,200,000           No.3         2x4         820         940         1,100,000           Stud         1065         1225         1,200,000           Construction         1065         1225         1,200,000           Standard         1945         2235         1,400,000           No.1         2x6         1235         1420         1,200,000           No.1         1145         1,300,000         1,000,000         1,000,000           No.1         2x6         1235         1420         1,200,000           No.1         2x10         1795         2665         1,400,00	No.1 /No-2	2x10	1045	1200	1,600,000	
Select Structural         1495         1720         1,900,000           No.1 No.2 $2xl2$ 950         1090         1,600,000           No.3         545         630         1,400,000           Douglas Fir-South         2         2245         2580         1,400,000           No.1         No.1         1555         1785         1,300,000           No.2         2x44         820         940         1,100,000           Stud         2x44         820         940         1,100,000           Stud         2x44         820         940         1,100,000           Stud         2x44         820         940         1,100,000           Standard         605         695         1,200,000           Utility         290         330         1,000,000           Standard         1945         2235         1,400,000           No.1         2x66         1235         1420         1,200,000           No.2         2x66         1235         1420         1,200,000           No.1         2x8         170         815         1,100,000           No.1         2x8         1240         1430         1,300,000 <td>No.3</td> <td></td> <td>600</td> <td>690</td> <td>1,400,000</td> <td></td>	No.3		600	690	1,400,000	
No.1 /No.2         2xl2         950         1090         1,600,000           No.3         545         630         1,400,000           Douglas Fir-South         2245         2580         1,400,000           No.1         1555         1785         1,300,000           No.1         1555         1785         1,300,000           No.1         1555         1785         1,000,000           No.3         2x4         820         940         1,100,000           Stud         820         940         1,100,000         1065           Studad         820         940         1,000,000         1065         1225         1,200,000           Standard         0605         695         1,100,000         1000,000         1000,000         1000,000         1000,000           Stelect Structural         1345         1545         1,300,000         100,000         1000,000         100,000         NwPA           Stud         2x6         1235         1420         1,200,000         100,000         NWPA           Stud         2x8         1240         1430         1,300,000         1,000,000         Select Structural         1400         1310         1,200,000	Select Structural		1495	1720	1,900,000	
No.3         545         630         1,400,000           Dauglas Fir-South $2245$ 2580         1,400,000           Sclect Structural         1555         1785         1,300,000           No.1         1555         1785         1,300,000           No.2         1425         1635         1,200,000           No.3         2x4         820         940         1,100,000           Stud         20         945         1,100,000         800           Studard         1065         1225         1,200,000         1005         1,100,000           Studard         1945         2335         1,400,000         100,000         WWPA         100,000         100,000         WWPA         100,000         100,000         100,000         100,000         100,000         100,000         100,000	No.1 /No.2	2x12	950	1090	1,600,000	
Douglas Fir-South           Select Structural         1,400,000           No.1         1555         1785         1,300,000           No.2         1425         1635         1,200,000           No.3         2x4         820         940         1,100,000           Stud         820         945         1,100,000         1065         1225         1,200,000           Standard         6605         695         1,100,000         1065         1225         1,200,000           Villity         290         330         1,000,000         1065         1225         1,200,000           Standard         1945         2235         1,400,000         1,000,000         1065         1235         1420         1,200,000           No.1         1945         2235         1,400,000         1,000,000         WWPA           Select Structural         1795         2065         1,400,000         WWPA           Select Structural         1795         2065         1,400,000         WWPA           No.2         2x8         1240         1430         1,300,000           No.1         2x10         1140         1310         1,200,000           No.3         0	No.3		545	630	1,400,000	1
Select Structural         2245         2580         1,400,000           No.1         1555         1785         1,300,000           No.2         1425         1635         1,200,000           No.3         2x4         820         940         1,100,000           Stud         20         940         1,100,000         1065         1225         1,200,000           Standard         1065         1225         1,200,000         100,000         1011         100,000           Standard         1065         1225         1,200,000         100,000         100,000         1000,000         100,000         100,000         100,000         100,000         100,000         100,000         100,000         100,000         100,000         100,000         1,400,000         100,000         100,000         WWPA         1345         1545         1,300,000         100,000         WWPA         1345         1420         1,200,000         1,100,000         WWPA         1995         2065         1,400,000         WWPA         1455         1,200,000         1,400,000         1,400,000         1,400,000         1,400,000         1,400,000         1,400,000         1,400,000         1,400,000         1,400,000         1,400,000         1,	Douglas Fir-South	I	•			
No.1         1555         1785         1,300,000           No.2         1425         1635         1,200,000           No.3         2x4         820         940         1,100,000           Stud         820         945         1,100,000           Construction         1065         1225         1,200,000           Studard         065         693         1,100,000           Utility         290         330         1,000,000           Select Structural         1945         2235         1,400,000           No.2         2x6         1235         1420         1,200,000           No.3         710         815         1,100,000         1,300,000           No.2         2x6         1235         1420         1,200,000           No.3         710         815         1,100,000         WWPA           Select Structural         7750         860         1,100,000           No.2         1140         1310         1,200,000           No.2         1140         1310         1,200,000           No.2         1140         1310         1,200,000           No.1         2x10         1645         1890         1,4	Select Structural		2245	2580	1,400,000	}
$\begin{array}{ c c c c c c c c c c c c c c c c c c c$	No.1		1555	1785	1,300,000	-
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	No.2		1425	1635	1,200,000	1
Stud         820         945         1,100,000           Construction         1065         1225         1,200,000           Standard         605         695         1,100,000           Utility         290         330         1,000,000           Select Structural         1945         2235         1,400,000           No.1         2x6         13345         15455         1,300,000           Stud         710         815         1,100,000         WWPA           Select Structural         750         860         1,100,000         WWPA           No.1         2x8         1795         2065         1,400,000           No.2         2x8         1240         1430         1,300,000           No.2         655         755         1,100,000         WWPA           Select Structural         2x10         1645         1890         1,400,000           No.1         2x10         1645         1890         1,400,000           No.2         1045         1200         1,200,000           No.1         2x10         1140         1310         1,300,000           No.2         600         690         1,100,0000         1,400,000 </td <td>No.3</td> <td>2x4</td> <td>820</td> <td>940</td> <td>1,100,000</td> <td>1</td>	No.3	2x4	820	940	1,100,000	1
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	Stud		820	945	1,100,000	1 (
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	Construction		1065	1225	1,200,000	1 . ` .
$\begin{array}{ c c c c c c c c c c c c c c c c c c c$	Standard		605	695	1,100,000	
Select Structural         1945         2235         1,400,000           No.1         1345         1545         1,300,000           No.2         2x6         1235         1420         1,200,000           No.3         710         815         1,100,000           Stud         750         860         1,100,000           Select Structural         2x8         1795         2065         1,400,000           No.1         2x8         1140         1310         1,300,000           No.2         655         755         1,100,000           No.1         2x10         1645         1890         1,400,000           No.1         2x10         1140         1310         1,200,000           No.1         2x10         1645         1890         1,400,000           No.2         0         1140         1310         1,200,000           No.2         1045         1200         1,200,000           Select Structural         1495         1720         1,400,000           No.2         1495         1720         1,400,000           No.3         545         630         1,100,000	Utility		290	330	1,000,000	
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	Select Structural		1945	2235	1,400,000	
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	No.1		1345	1545	1,300,000	1 i
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	No.2	2x6	1235	1420	1,200,000	
Stud750 $860$ $1,100,000$ WWPASelect Structural1795 $2065$ $1,400,000$ No.1 $2x8$ $1240$ $1430$ $1,300,000$ No.21140 $1310$ $1,200,000$ No.3 $655$ $755$ $1,100,000$ Select Structural $2x10$ $1645$ $1890$ $1,400,000$ No.1 $2x10$ $1140$ $1310$ $1,300,000$ No.2 $000$ $600$ $690$ $1,100,000$ No.3 $2x12$ $1035$ $1190$ $1,300,000$ No.1 $2x12$ $1035$ $1190$ $1,300,000$ No.3 $545$ $630$ $1,100,000$	No.3		710	815	1,100,000	
$ \begin{array}{c c c c c c c c c c c c c c c c c c c $	Stud		750	860	1,100,000	WWPA
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	Select Structural		1795	2065	1,400,000	
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	No,1	2x8	1240	1430	1,300,000	
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	No.2		1140	1310	1,200,000	
$ \begin{array}{c c c c c c c c c c c c c c c c c c c $	No.3		655	755	1,100,000	1
No.1         2x10         1140         1310         1,300,000           No.2         1045         1200         1,200,000           No.3         600         690         1,100,000           Select Structural         1495         1720         1,400,000           No.1         2x12         1035         1190         1,300,000           No.2         950         1090         1,200,000           No.3         545         630         1,100,000	Select Structural		1645	1890	1,400,000	- ·
No.2         1045         1200         1,200,000           No.3         600         690         1,100,000           Select Structural         1495         1720         1,400,000           No.1         2x12         1035         1190         1,300,000           No.2         950         1090         1,200,000           No.3         545         630         1,100,000	No.1	2x10	1140	1310	1,300,000	1
No.3         600         690         1,100,000           Select Structural         1495         1720         1,400,000           No.1         2xl2         1035         1190         1,300,000           No.2         950         1090         1,200,000           No.3         545         630         1,100,000	No.2		1045	1200	1,200,000	1
Select Structural         1495         1720         1,400,000           No.1         2xl2         1035         1190         1,300,000           No.2         950         1090         1,200,000           No.3         545         630         1,100,000	No.3		600	690	1,100,000	
No.1         2xl2         1035         1190         1,300,000           No.2         950         1090         1,200,000           No.3         545         630         1,100,000	Select Structural		1495	1720	1,400,000	
No.2         950         1090         1,200,000           No.3         545         630         1,100,000	No.1	2x12	1035	1190	1,300,000	1
No.3 545 630 1,100,000	No.2		950	1090	1,200.000	1
	No.3		545	630	1,100,000	

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### DEPARTMENT OF COMMERCE

Modulus of Elasticity "E"

1,200,000

1,100,000

1,100,000

900,000

900,000

900,000

800,000

1,200,000

1,100,000

1,100,000

900,000

900,000

1,100,000

1,100,000

1,200,000

1,100,000

1,100,000

900,000

1,200,000

900,000

1,200,000 NSLB

NELMA

1,000,000

2480

1535

1140

695

655

895

495

230

2150

1330

990

600

595

1985

1230

915

555

1820

1125

835

510

1655

Grading Rules Agency

· · · · · · · · · · · · · · · · · · ·	1	Design Value in	Bending, "Fb"
Species and Grade	Size	Normal Duration	Snow Loading
Eastern Hemlock-Tamarack		1 1	·····
Select Structural		2155	248
No.1		1335	153
No.2		990	114
No.3	2x4	605	69
Stud		570	
Construction		775	89
Standard		430	49
Utility		200	23
Select Structural		1870	215
No.1		1160	133
No.2	2x6	860	99
No.3		525	60
Stud		520	59
Select Structural		1725	198
No.1	2x8	1070	123
No.2		795	91
No.3		485	55
Select Structural		1580	182
No.1	2x10	980	112
No.2		725	83
No.3		445	51
Select Structural		1440	165
No.1	2x12	890	102
No.2		660	76

				-4	
No.1	2xl2	890	1025	1,100,000	
No.2		660	760	1,100,000	
No.3		405	465	900,000	
Eastern Softwoods	· · · · · · · ·				
Select Structural		2155	2480	1,200,000	
No.1		1335	1535	1,100,000	
No.2		990	1140	1,100,000	1
No.3	2x4	605	695	900,000	
Stud	·	570	655	900,000	
Construction		775	895	1,000,000	
Standard		430	495	900,000	
Utility		200	230	800,000	
Select Structural		1870	2150	1,200,000	
No.1		1160	1330	1,100,000	1
No.2	2x6	860	990	1,100,000	1
No.3		525	600	900,000	
Stud		520	595	900,000	NELMA
Select Structural		1725	1985	1,200,000	NSLB
No.1	2x8	1070	1230	1,100,000	1
No.2		795	915	1,100,000	1
No.3		485	555	900,000	1
Select Structural		1580	1820	1,200,000	1
No.1	2x10	980	1125	1,100,000	1
No.2		725	835	1,100,000	1
No.3		445	510	900,000	1
Select Structural		1440	1655	1,200,000	1
No.1	2x12	890	1025	1,100,000	
No.2		660	760	1,100,000	1
No.3		405	465	900,000	1

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	····	Design Value in	Bending, "Fb"		7
Species and Grade	Size	Normal Duration	Snow Loading	Modulus of Elasticity "E"	Grading Rules Agency
Eastern White Pine				a de la constantina d	
Select Structural		2155	2480	1,200,000	
No.I		1335	1535	1,100,000	
No.2		990	1140	1,100,000	
No.3	2x4	605	695	900,000	
Stud		570	655	900,000	
Construction		775	895	1,000,000	
Standard		430	495	900,000	
Utility		200	230	800,000	
Select Structural		1870	2150	1,200,000	
No.1		1160	1330	1,100,000	
No.2	2x6	860	990	1,100,000	
No.3		525	600	900,000	
Stud		520	595	900,000	NELMA
Select Structural		1725	1985	1,200,000	NSLB
No.1	2x8	1070	1230	1,100,000	
No.2	-	795	915	1,100,000	í I
No.3		485	555	900,000	
Select Structural		1580	1820	1,200,000	
No.1	2x10	980	1125	1,100,000	
No.2		725	835	1,100,000	
No.3	-	445	510	900.000	
Select Structural		1440	1655	1,200,000	
No.1	2x12	890	1025	1,100,000	
No.2		660	760	1,100,000	{
No.3	-	405	465	900.000	/
Hem Fir					<u> </u>
Select Structural	1	2415	2775	1.600.000	1 1
No.1 & Btr		1810	2085	1,500,000	
No.1	-	1640	1885	1,500,000	
No.2	-	1465	1685	1 300 000	{
No.3		865	990	1 200,000	
Stud		855	980	1 200,000	4
Construction		1120	1290	1300,000	-
Standard		635	725	1 200 000	-
Utility		290	330	1 100 000	{
Select Structural	-	2095	2405	1 600 000	
No.1 & Btr		1570	1805	1 500,000	
No.1	- 2x6	1420	1635	1,500,000	{
No.2		1270	1460	1 300,000	-
No 3		750	860	1,300,000	-
Stud		775	205	1,200,000	
Select Structural		1030	220	1,200,000	WCLIB
No.1 & Btr		1250	1665	1,000,000	WWPA
No1	2*8	1310	1005	1,500,000	
No 2		1310	1310	1,300,000	-
No 3		600	1550	1,500,000	
Salaat Structural		1770	193	1,200,000	4
No L& Bir	_	17/0	2030	1,000,000	4 ł
No t		1330	1525	1,500,000	4
No.1		1200	1380	1,500,000	4
No.2		10/5	1235	1,300,000	4
NU.3		635	725	1,200,000	1
Select Structural		1610	1850	1,600,000	1 /
INO.1 & BIT		1210	1390	1,500,000	1 · ( ·
N0.1	2x12	1095	1255	1,500,000	1 <u>`</u> ı
No.2		980	1125	1,300,000	1
No.3		575	660	1,200,000	

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### DEPARTMENT OF COMMERCE

		Design Value in	Bending, "Fb"		a
Species and Grade	Siza	Normal Duration	Snow Loading	Modulue of Florisity (F)	Grading Rules
Hem-Fir (North)	. 5120	Trofmar Docation	Show Loading	Wooding of Elasticity E	Agency
Select Structural		2245	2580	1 700 000	ľ
No.1 /No.2	— ,	1725	1985	1,600,000	
No.3		990	1140	1,000,000	
Stud	2x4	980	1125	1,400,000	i.
Construction		1325	1520	1,500,000	
Standard		720	825	1,400,000	
Utility		345	395	1,300,000	
Select Structural		1945	2235	1,700,000	
No.1 /No.2	2x6	1495	1720	1.600.000	
No.3		860	990	1,400,000	
Stud		890	1025	1,400,000	NLGA
Select Structural		1795	2065	1,700,000	
No.1 /No.2	2x8	1380	1585	1,600,000	
No.3		795	915	1,400,000	
Select Structural		1645	1890	1,700,000	
No.1 /No.2	2x10	1265	1455	1,600,000	
No.3		725	835	1,400,000	
Select Structural		1495	1720	1,700,000	
No.1 /No.2	2x12	1150	1325	1,600,000	
No.3		660	760	1,400,000	
Mixed Maple		· · · · ·		• ·	•
Select Structural		1725	1985	1,300,000	]
No.1		1250	1440	1,200,000	]
No.2		1210	1390	1,100,000	]
No.3	2x4	690	795	1,000.000	]
Stud		695	B00	1,000,000	
Construction		. 920	1060	1,100,000	
Standard		520	595	1,000,000	
Utility		260	300	900,000	]
Select Structural		1495	1720	1,300,000	
No.1		1085	1245	1,200,000	-
No.2	2x6	1045	1205	1,100,000	
INO.3		600	690	1,000,000	
Stud		630	/25	1,000,000	NELMA
No 1		1380	1383	1,300,000	4
No 2		1000	1150	1,200,000	-
No 3		500	1110	1,100,000	-
Select Structural		1245	1455	1,000,000	ł
No 1	2v10	015	1433	1,300,000	4
No 2		825	1000	1,200,000	4
No 3	<u> </u>	505	580	1,100,000	-
Select Structural		1150	1225	1 300 000	4
No.1	2x12	835	060	1,00,000	4
No.2	2012	805	025	1 100 000	4
No.3		460	530	1 000 000	4
1	I	-100	1 220	1,000,000	4

	Design Value in Bending, "Fb"				(
Species and Grade	Size	Normal Duration	Snow Loading	Modulus of Elasticity "E"	Grading Rules Agency
Mixed Oak				e di seco	
Select Structural		1985	2280	1,100,000	
No.1		1425	1635	1,000,000	
No.2		1380	1585	900,000	
No.3	2x4	820	940	800,000	
Stud		790	910	800,000	
Construction		1065	1225	900,000	
Standard		605	695	800,000	
Utility		290	330	800,000	
Select Structural		1720	1975	1,100,000	
No.1		1235	1420	1,000,000	
No.2	2x6	1195	1375	900,000	
No.3	-	710	815	800,000	
Stud		720	825	800,000	NELMA
Select Structural		1585	1825	1,100,000	)
No.1	2x8	1140	1310	1,000,000	
No.2		1105	1270	900,000	1
No.3		655	755	800,000	
Select Structural		1455	1675	1,100,000	
No.1	2x10	1045	1200	1,000,000	1
No.2		1010	1165	900,000	
No.3		600	690	800,000	
Select Structural		1325	1520	1,100,000	1
No.1	2x12	950	1090	1,000,000	
No.2		920	1060	900,000	
No.3		545	630	800,000	1
Mixed Southern Pine				<b>.</b>	(
Select Structural		2360	2710	1,600,000	1 <u>`</u>
No.1		1670	1920	1,500,000	
No.2		1500	1720	1,400,000	
No.3	2x4	865	990	1,200,000	
Stud		890	1020	1,200,000	
Construction		1150	1320	1,300,000	
Standard		635	725	1,200,000	
Utility		315	365	1,100,000	
Select Structural		2130	2450	1,600,000	
No.1		1490	1720	1,500,000	1
No.2	2x6	1320	1520	1,400,000	-
No.3	······	775	895	1,200,000	1
Stud		775	895	1,200,000	\$PIB
Select Structural		2010	2310	1,600,000	1
No.1	2x8	1380	1590	1,500,000	1
No.2		1210	1390	1,400,000	1
No.3		720	825	1,200,000	1
Select Structural		1730	1980	1,600,000	<b>j</b>
No.1	2x10	1210	1390	1,500.000	1
No.2		1060	1220	1,400.000	1
No.3	'	605	695	1.200.000	-
Select Structural		1610	1850	1.600.000	1
No.1	2x12	1120	1290	1,500,000	4
No.2		1010	1160	1,000,000	1
No 3		575	660	1 200 000	4
			1	1,200,000	1

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## DEPARTMENT OF COMMERCE

		Design Value in Bending, "Fb"			
Species and Grade	Size	Normal Duration	Snow Londing	Modulus of Floatisity (F)	Grading Rules
Northern Red Oak	- Oiec	Tiorman Duration	Show Loading	would of Elasticity E	Agency
Select Structural		2415	2775	1 400 000	l
No.1		1725	1985	1,100,000	
No.2		1680	1935	1,300,000	
No.3		950	1090	1,200,000	
Stud		950	1090	1,200,000	
Construction		1265	1455	1,200,000	
Standard		720	825	1,100,000	
Utility		345	395	1.000.000	
Select Structural		2095	2405	1,400,000	
No.1		1495	1720	1,400,000	
No.2	2x6	1460	1675	1,300,000	
No.3	·	820	945	1,200,000	
Stud		865	990	1,200,000	NELMA
Select Structural		1930	2220	1,400,000	
No.1	2x8	1380	1585	1,400,000	
No.2		1345		1,300,000	
No.3		760	875	1,200,000	
Select Structural		1770	2035	1,400,000	
No.1	2x10	1265	1455	1,400,000	
No.2		1235	1420	1,300,000	
No.3		695	800	1,200,000	
Select Structural		1610	1850	1,400,000	
No.1	2x12	1150	1325	1,400,000	
No.2		1120	1290	1,300,000	1
No.3		635	725	1,200,000	
Northern Species					_
Select Structural		1640	1885	1,100,000	
No.1 /No.2		990	1140	1,100,000	
No.3		605	695	1,000,000	
Stud	2x4	570	655	1,000,000	
Construction		775	895	1,000,000	-
Standard		430	495	900,000	
Colliny		200	230	900,000	
		1420	1635	1,100,000	
No. 17 No.2	<sup>2xo</sup>	860	990	1,100,000	
NO.3		525	600	1,000,000	
Colort Structurel		520	595	1,000,000	NLGA
		1310	1510	1,100,000	
No 3		/95	915	1,100,000	4
Select Structural		483	232	1,000,000	4
No 1/No 2		1200	1580	1,100,000	4
No 3		125	510	1,100,000	}
Select Structural		440	310	1,000,000	4
No.1/No.2		640	1200	1,100,000	ł
No 3		000	100	1,100,000	4
1.0%		405	465	1,000,000	1

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	Design Value in Bending, "Fb"				(
Species and Grade	Size	Normal Duration	Snow Loading	Modulus of Elasticity "E"	Grading Rules Agency
Northern White Cedar					
Select Structural		1335	1535	800,000	
No.1		990	1140	700,000	
No.2		950	1090	700,000	
No.3	2x4	560	645	600,000	
Stud		540	620	600,000	
Construction		720	825	700,000	
Standard		405	465	600,000	
Utility		200	230	600,000	
Select Structural		1160	1330	800,000	
No.I		860	990	700,000	
No.2	2x6	820	945	700,000	
No.3		485	560	600,000	
Stud	_	490	560	600,000	NELMA
Select Structural		1070	1230	800.000	
No.1	2x8	795	915	700.000	
No.2		760	875	700,000	
No.3		450	515	600,000	1
Select Structural		980	1125	800.000	
No.1	2x10	725	835	700.000	
No 2		695	800	700,000	{
No 3		410	475	600,000	
Select Structural		800	1025	800,000	1
No I	2,12	660	760	700,000	- I
No.2	2712	625	700	700,000	-
No.3		033	125	600,000	
Red Manle		575	450		└ <u></u> ,
Salaat Structural	1	2245	2500	1 700 000	1 <sup>\</sup> .
No 1		1505	1925	1,700,000	-
No.1		1555	1033	1,000,000	- 1
No 3		1555	1765	1,300,000	- !
Stud	2X++	905	1040	1,500,000	- 1
Carateurian		1010	1020	1,500,000	
Construction		1210	1390	1,400,000	-
Standard		000	700	1,500,000	4
Salast Structural		313	303	1,200,000	- !
Select Structural		1945	2235	1,700,000	-
No.1		1385	1590	1,600,000	4
No.2	2x6	1345	1545	1,500,000	-
N0.3		/85	905	1,300,000	
Stud		805	925	1,300,000	NELMA
Select Structural		1795	2065	1,700,000	4
No.1	2x8	1275	1470	1,600,000	· ·
No.2		1240	1430	1,500,000	
No.3		725	835	1,300,000	_i
Select Structural		1645	1890	1,700,000	
No.1	2x10	1170	1345	1,600,000	
No.2		1140	1310	1,500,000	]
No.3	· ·	665	765	. 1,300,000	]
Select Structural		1495	1720	1,700,000	1 1
No.1	2x12	1065	1225	1,600,000	1
No.2		1035	1190	1,500,000	1
No.3		605	695	1,300,000	1

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## DEPARTMENT OF COMMERCE

		Design Value in	Bending, "Fb"		
Species and Grade	Size	Normal Duration	Snow Loading	Modulus of Elasticity "E"	Grading Rules Agency
Red Oak				· ·	
Select Structural		1985	2280	1,400,000	
No.1		1425	1635	1,300,000	
No.2	· ·	1380	1585	1,200,000	
No.3	2x4	820	940	1,100,000	
Stud		790	910	1,100,000	
Construction		1065	1225	1,200,000	
Standard		605	695	1,100,000	
Utility		290	330	1,000,000	
Select Structural		1720	1975	1,400,000	
No.1		1235	1420	1,300,000	
No.2	2x6	1195	1375	1,200,000	
No.3		710	815	1,100,000	
Stud		720	825	1,100,000	NELMA
Select Structural		1585	1825	1,400,000	
No.1	2x8	1140	1310	1,300,000	1
No.2		1105	1270	1,200,000	
No.3		655	755	1,100,000	
Select Structural		1455	1675	1,400,000	1
No.I	2x10	1045	1200	1,300,000	1
No.2		1010	1165	1,200,000	1
No.3		600	690	1,100,000	1
Select Structural		1325	1520	1,400,000	
No.1	2x12	950	1090	1,300,000	1
No.2		920	1060	1,200,000	1
No.3		545	630	1,100,000	1

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	T	Design Value in	Bending, "Fb"		
Species and Grade	Size	Normal Duration	Snow Loading	Modulus of Elasticity "E"	Grading Rules Agency
Redwood		· · · · · · · · · · · · · · · · · · ·		· · · · · · · · · · · · · · · · · · ·	
Clear Structural		3020	3470	1,400,000	
Select Structural	1	2330	2680	1,400,000	
Select Structural, open grain	1	1900	2180	1,100,000	
No.1	1	1680	1935	1,300,000	
No.1, open grain	1	1335	1535	1,100,000	
No.2	1	1595	1835	1,200,000	
No.2, open grain	2x4	1250	1440	1,000,000	
No.3	1	905	1040	1,100,000	
No.3, open grain	1	735	845	900,000	
Stud		725	835	• 900,000	
Construction	-	950	1090	900,000	
Standard	1	520	595	900,000	
Utility	1	260	300	800,000	1
Clear Structural	1	2615	3010	1,400,000	1
Select Structural	1	2020	2320	1,400,000	1
Select Structural, open grain	1	1645	1890	1,100,000	
No.1	1	1460	1675	1,300,000	
No.1, open grain	2x6	1160	1330	1,100,000	
No.2		1385	1590	1,200,000	
No.2, open grain	1	1085	1245	1,000,000	1
No.3	1	785	905	1,100,000	•
No.3, open grain	1	635	730	900,000	
Stud	1	660	760	900,000	1
Clear Structural	1	2415	2775	1,400,000	]
Select Structural	1	1865	2140	1,400,000	RIS (
Select Structural, open grain	1	1520	1745	1,100,000	]
No.1	1	1345	1545	1,300,000	]
No.1, open grain	2x8	1070	1230	1,100,000	1
No.2	1	1275	1470	1,200,000	]
No.2, open grain	1	1000	- 1150	1,000,000	]
No.3	1	725	835	1,100,000	
No.3, open grain	1	585	675	900,000	]
Clear Structural		2215	2545	1,400,000	]
Select Structural	7	1710	1965	1,400,000	]
Select Structural, open grain	]	1390	1600	1,100,000	
No.1		1235	1420	1,300,000	
No.1, open grain	2x10	980	1125	1,100,000	
No.2	]	1170	1345	1,200,000	
No.2, open grain	7	915	1055	1,000,000	
No.3	7	665	765	1,100,000	
No.3, open grain	1	540	620	900,000	
Clear Structural	]	2015	2315	1,400,000	
Select Structural		1555	1785	1,400,000	_
Select Structural, open grain	]	1265	1455	1,100,000	
No.1	]	1120	1290	1,300,000	
No.1, open grain	2x12	890	1025	1,100,000	
No.2	]	1065	1225	1,200,000	]
No.2, open grain	]	835	960	1,000,000	
No.3	]	605	695	1,100,000	]
No.3, open grain	7	490	560	900,000	

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### DEPARTMENT OF COMMERCE

Design Value in Bending, "Fb"					
Species and Grade	Size	Normal Duration	Snow Loading	Modulus of Elasticity "E"	Grading Rules Agency
Southern Pine	I	·			
Dense Select Structural		3510	4030	1,900,000	
Select Structural		3280	3770	1,800,000	
Non-Dense Select Structural		3050	3500	1,700,000	
No.1 Dense		2300	2650	1,800,000	
No.1		2130	2450	1,700,000	
No.1 Non-Dense		1950	2250	1,600,000	
No.2 Dense	2x4	1960	2250	1,700,000	
No.2		1720	1980	1,600,000	
No.2 Non-Dense		1550	1790	1,400,000	
No.3		980	1120	1,400,000	
Stud		1010	1160	1,400,000	
Construction		1270	1450	1,500,000	
Standard		720	825	1,300,000	
Utility		345	395	1,300,000	
Dense Select Structural		3100	3570	1,900,000	
Select Structural		2930	3370	1,800,000	
Non-Dense Select Structural		2700	3110	1,700,000	
No.1 Dense		2010	2310	1,800,000	
No.1		1900	2180	1,700,000	1
No.1 Non-Dense	2x6	1720	1980	1,600,000	
No.2 Dense		1670	1920	1,700,000	
No.2		1440	1650	1,600,000	
No.2 Non-Dense		1320	T520	1,400,000	
No.3		865	990	1,400,000	
Stud		890	1020	1,400,000	
Dense Select Structural		2820	3240	1,900,000	
Select Structural		2650	3040	1,800,000	
Non-Dense Select Structural		2420	2780	1,700,000	SPIB
No.1 Dense		1900	. 2180	1,800,000	1
No.1	2x8	1730	1980	1,700,000	1
No.1 Non-Dense		1550	1790	1,600,000	1
No.2 Dense		1610	1850	1,700,000	
No.2		1380	1590	1,600,000	
No.2 Non-Dense		1260	1450	1,400,000	1
No.3		805	925	1,400,000	
Dense Select Structural		2470	2840	1,900,000	1
Select Structural	-	2360	2710	1,800,000	
Non-Dense Select Structural		2130	2450	1,700,000	1
No.1 Dense		1670	1920	1,800,000	4
No.1	2x10	1500	1720	1,700,000	1
No.1 Non-Dense		1380	1590	1,600,000	1
No.2 Dense	<del></del>	1380	1590	1,700,000	1
No.2		1210	1390	1,600,000	1
No.2 Non-Dense		1090	1260	1,400,000	1
No.3		690	795	1,400,000	4
Dense Select Structural		2360	2710	1,900,000	1
Select Structural		2190	2510	1.800.000	1
Non-Dense Select Structural		2010	2310	1.700.000	1
No.1 Dense		1550	1790	1.800.000	4
Nol	2 12	1440	1,770	1.700.000	4
No I Non-Dense		1320	1520	1 600 000	4
No 2 Dense	_	1320	1520	1,000,000	4
No 2		1120	1320	1,700,000	-
No 2 Non-Dense		1040	1290	1,000,000	4
No 3	_	660	760	1 400 000	-
11012		1000	1700	1,700,000	1

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	1	Design Value in	Bending, "Fb"		(	
Species and Grade	Size	Normal Duration	Snow Loading	Modulus of Elasticity "E"	Grading Rules Agency	
Spruce-Pine-Fir			8			
Select Structural		2155	2480	1,500,000	]	
No.1 /No.2		1510	1735	1,400,000		
No.3	·	865	990	1,200,000		
Stud	2x4	855	980	1,200,000		
Construction		1120	1290	1,300,000		
Standard		635	725	1,200,000		
Utility		290	330	1,100,000		
Select Structural		1870	2150	1,500,000		
No.1 /No.2	2x6	1310	1505	1,400,000		
No.3		750	860	1,200,000		
Stud		775	895	1,200,000	NLGA	
Select Structural	·   ·	1725	1985	1,500,000	1	
No. 1 / No.2	2x8	1210	1390	1,400,000		
No.3		690	795	1,200,000	1	
Select Structural	2x10	1580	1820	1,500,000	1	
No.1/No.2		1105	1275	1,400,000	1	
No.3		635	725	1,200,000		
Select Structural		1440	1655	1,500,000	1	
No.1 /No.2	2x12	1005	1155	1,400,000	-	
No.3		575	660	1,200,000	-	
Spruce-Pine-Fir (South)						
Select Structural	1	2245	2580	1,300,000	]	
No.1		1465	1685	1,200,000	1	
No.2		1295	1490	1,100,000		
No.3	2x4	735	845	1,000,000	1	
Stud		725	835	1,000,000	1 (	
Construction		980	1125	1,000,000		
Standard		545	630	900,000		
Utility		260	300	900,000		
Select Structural		1945	2235	1,300,000		
No.1		1270	1460	1,200,000		
No.2	2x6	1120	1290	1,100,000		
No.3		635	730	1000,000	NELMA	
Stud		660	760	1,000,000	NSLB	
Select Structural		1795	2065	1,300,000	WCLIB	
No.1	2x8	1175	1350	1,200:000	WWPA	
No.2		1035	1190	1,100,000	]	
No.3		585	675	1,000,000	]	
Select Structural		1645	1890	1,300,000	]	
No.1	2x10	1075	1235	1,200,000	7	
No.2		950	1090	1,100,000	]	
No.3		540	620	1,000,000		
Select Structural		1495	1720	1,300,000	]	
No.I	2x12	980	1125	1,200,000	]	
No.2		865	990	1,100,000	]	
No.3		490	560	1,000,000	7	

## DEPARTMENT OF COMMERCE

		Design Value in	Bending, "Fb"		
Species and Grade	Siza	Normal Duration	Snow Loading	Modulue of Flasticity "F"	Grading Rules
Western Cedars	3120	Normar Duration	Show Loading	Would of Elasticity 15	ABenej
Select Structural		1725	1985	1,100,000	
No.1		1250	1440	1,000,000	
No.2		1210	1390	1.000.000	1
No.3	2x4	690	795	900.000	
Stud	24.1	695	800	900.000	
Construction		920	1060	900.000	
Standard		520	595	800,000	
Utility		260	300	800,000	
Select Structural		1495	1720	1,100,000	
No.1		1085	1245	1,000,000	
No.2	2x6	1045	1205	1,000,000	
No.3		600	690	900,000	
Stud		635	725	900,000	WCLIB
Select Structural		1380	1585	1,100,000	WWPA
No.I	2x8	1000	1150	1,000,000	
No.2		965	1110	1,000,000	
No.3		550	635	900,000	
Select Structural		1265	1455	1,100,000	
No.1	2x10	915	1055	1,000,000	
No.2	1	885	1020	1,000,000	
No.3		505	580	900,000	
Select Structural		1150	1325	1,100,000	
No.1	2x12	. 835	960	1,000,000	
No.2		805	925	1,000,000	
No.3	l	460	530	900,000	
Western Woods				1	1
Select Structural		1510	1735	1,200,000	
No.I		1120	1290	1,100,000	
No.2		1120	1290	1,000,000	
N0.5	2x4	643	745	900,000	
Stud	-	033	125	900,000	1
Standard	-	633	520	1,00,000	-
	ł .	220	265	800,000	4
Select Structural	1	1310	1505	1 200 000	4
No l	4	970	1305	1 100 000	-
No 2	286	970	1120	1,000,000	ł
No 3	2.00	560	645	900,000	
Stud	{	575	660	900,000	WCLIB
Select Structural	<u> </u>	1210	1390	1,200,000	WWPA
No.1	2x8	895	1030	000.001.1	1
No.2		895	1030	1.000.000	-
No.3	-	520	595	900.000	1
Select Structural	1	110	1275	1,200,000	1
No.1	2x10	820	945	1,100,000	1
No.2	1 .	820	945	1,000,000	1
No.3	1.	475	545	900,000	1
Select Structural	1	1005	1155	1,200,000	1
No.1	2x12	750	860	1,100,000	1
No.2	1	750	860	1,000,000	
No.3	1	430	495	900,000	]

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		Design Value in	Bending, "Fb"		(
Species and Grade	Size	Normal Duration	Snow Loading	Modulus of Elasticity "E"	Grading Rules Agency
White Oak	<u> </u>				
Select Structural		2070	2380	1,100,000	
No.1		1510	1735	1,000,000	
No.2		1465	1685	900,000	
No.3	2x4	820	940	800,000	
Stud .		820	945	800,000	
Construction		1095	1255	900,000	
Standard		605	695	800,000	
Utility		290	330	800,000	
Select Structural		1795	2065	1,100,000	
No.1		1310	1505	1,000,000	
No.2	2x6	1270	1460	900,000	
No.3		710	815	800,000	
Stud .		750	860	800,000	NELMA
Select Structural		1655	1905	1,100,000	
No.1	2x8	1210	1390	1,000,000	
No.2		1175	1350	900,000	
No.3		655	755	800,000	
Select Structural		1520	1745	1,100,000	
No.1	2x10	1105	1275	1,000,000	
No.2		1075	1235	900,000	
No.3		600	690	800,000	
Select Structural		1380	1585	1,100,000	
No.1	2xi2	1005	1155	1,000,000	
No.2		980	1125	900,000	
No.3		545	630	800,000	
Yellow Poplar				· · · · · · · · · · · · · · · · · · ·	(
Select Structural		1725	1985	1,500,000	
No.1		1250	1440	1,400,000	1
No.2		1210	1390	1,300,000	
No.3	2x4	690	795	1,200,000	
Stud		695	800	1,200,000	1 ·
Construction		920	1060	1,300,000	1
Standard		520	595	1,100,000	
Utility		230	265	1,100,000	1
Select Structural		1495	1720	1,500,000	1
No.1		1055	1245	1,400,000	1 1
No.2	2x6	1045	1205	1,300,000	
No.3		600	690	1,200,000	
Stud		635	725	1,200,000	NSLB
Select Structural		1380	1585	1,500,000	n
No.I	2x8	1000	1150	1,400,000	1
No.2		965	1110	1,300,000	
No.3		550	635	1,200,000	1
Select Structural		1265	1455	1,500,000	1
No.1	2x10	915	1055	1,400,000	1
No.2		885	1020	1.300.000	1
No.3		505	580	1.200.000	1
Select Structural		1150	1325	1.500.000	4
No.1	2x12	835	960	1,400,000	4
No.2		805	925	1.300.000	1
No.3		460	530	1.200.000	1

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# APPENDIX EROSION CONTROL PROCEDURES EXAMPLES, ILLUSTRATIONS AND GUIDELINES

The following examples and illustrations of some erosion control procedures are provided for your information. Many of these examples can be found in the "Wisconsin Construction Site Best Management Practices Handbook", developed by the Wisconsin department of natural resources. Note: The Handbook is available from Document sales, 202 South Thornton Avenue, P.O. Box 7840, Madison, WI 53707–8480; phone (608) 266–3358.

Figures E-1 to E-11, depict the materials and installation of some erosion control procedures.

Also included in the appendix are examples of plot plans depicting the best management practices that will help meet the requirements of the performance standards in this code.

**Figure E** – 12 is an example of a site with slopes of 12 % or less and also simple slopes, i.e. all slopes occurring in one general direction. Downslope measures are required, to reduce maintenance of these measures, the upslope diversion is recommended.

**Figure E** – 13 is an example of a site with complex slopes (slopes occurring in more than one direction). This site also has an area where slopes that are 12-20% are going to be disturbed. The location of the erosion control procedures are clearly indicated on the plot plan, including narratives that indicated methods of permanent stabilization.

Figure E - 14 is an example of a large lot, greater than 5 acres, with slopes greater than 12% and where the area of land disturbing activity is indicated. This plan indicates the use of vegetative barriers.

**Figure E – 15** explains how to determine and calculate % slopes.

Guidelines for timing the implementation of the erosion control practices and procedures in order to stabilize areas disturbed during construction of one and 2-family dwellings are included in this appendix. Dormant seeding, the guidelines for the use of vegetative buffers and the recommended maintenance for erosion control practices are also included.

For sites using either straw bales or silt fences as a perimeter control, <u>Table E-1</u> is included as a guide for determining the distance between parallel fences constructed on various slopes. Perimeter measures should be installed at right angles to the direction of flow. Drainage area is to be no more than 1/4 acres (approx. 10,000 square feet) per 100 feet of perimeter control.

## TABLE E-1 DISTANCE BETWEEN PARALLEL STRAW BALES OR SILT FENCES

Slope	Slope Distance
Percent	(feet)
< 2%	100 feet
2 to 5%	75 feet
5 to 10%	50 feet
10 to 20%	25 feet
> 20 %	15 feet

# **VEGETATIVE BARRIERS**

Vegetative barriers may be used as a perimeter measure if disturbed areas above consist of slopes no greater than 6% and barriers are on a grade no steeper than 5%. Vegetative barriers are to be a minimum of 10' wide for every 50 feet of open ground draining to them. These barriers must be maintained, i.e. not driven on or destroyed. If the barriers become covered with silt or otherwise destroyed, additional perimeter measures may be required.

# **TEMPORARY STABILIZATION OR MULCH CROP**

It is much easier to control erosion than to control sediment. Temporary stabilization helps to minimize erosion and therefore the need for long term maintenance of silt fences and straw bales. Annual rye grass may be planted as a temporary cover between April 1 and September 15. If seeding is done in the spring or late summer seeding dates and slopes are 6% or less, mulch may not be necessary.

Winter rye may be planted between July 15 and October 15. These seedings should be mulched.

# LATE SEASON CONSTRUCTION MULCHING/DORMANT SEEDING

If ground is broken after September 15, mulch should be applied as soon as a rough grade is established, unless final grade and landscaping is to be completed before the next growing season. Mulch will help to reduce the raindrop impact. Seeding should not be done between September 15 and November 1 as the weather is warm enough for the seed to germinate but it will not have an opportunity to establish a root system strong enough to survive the winter. A dormant seeding may be done OVER the mulch after November 1. These seedings are risky. A split application of seed may also be made, using half in November and balance early in spring.

# WINTER CONSTRUCTION

In areas with course soils, (sands) if excavation is possible most likely a trencher can be used to install the necessary silt fence. If at all possible leave the perimeter of the site undisturbed (this is assuming the site had vegetation present prior to frost); this may be the easiest erosion control for flat sites (6% or less).

In areas that have heavy soils, (clays) close attention should be paid to the try to get perimeter measures installed prior to frost penetrating greater than 6". If ground is solidly frozen, perimeter measures that need to be trenched may have to wait to be installed when the frost first starts to come out in the spring. This does not eliminate the need to keep sediment from leaving the site. Alternate methods for controlling erosion should be considered such as the use of soil stabilizers.

# MAINTENANCE OF THE MOST COMMONLY USED EROSION CONTROL PROCF DURES

#### SILT FENCES

Repair or replacement should be done within 24 hours if fencing is torn, sagging, overtopped, blown over (laying down), shows a lack of integrity, or in any way is not functioning as designed. Sediment deposits should be removed after each storm event. Sediment deposits shall be removed when deposits reach 0.5 the above ground height of the fence. Silt fence should be removed after upland areas have been stabilized. Any sediment deposits remaining in place after the silt fence is no longer required should be dressed to conform to the existing grade, prepared and stabilized.

#### STRAWBALES

Replacement of broken or torn bales should be done within 24 hours. Sediment deposits should be removed when deposits reach 0.5 the height of the bales. Strawbales should be removed after upland areas have been stabilized. Any sediment deposits remaining in place after the strawbale barrier is no longer required should be dressed to conform to the existing grade, prepared and stabilized.

#### MULCHING

Additional mulch or matting should be applied when rills develop (rill - small, eroded ditch measuring 1" or less width).

#### **TEMPORARY DIVERSION**

Any breaks or eroded areas of a diversion should be repaired within 24 hours.

#### SEDIMENT TRAP

Any structural deficiencies should be repaired within 24 hours. Sediment should be removed when it reaches half of the outlet height of trap.

#### SODDING

Repair or replacement of sod that has been destroyed in an area of channelized flow should be done within 24 hours after the rain event.

### **INLET PROTECTION BARRIERS**

Sediment deposits should be removed when deposits reach 0.5 the height of the fence. Repair or replacement should be made to damaged barriers within 24 hours.

## **TEMPORARY GRAVEL CONSTRUCTION ENTRANCE**

Rock should be maintained to meet the design criteria of 2-3" aggregate stone; 12 feet wide and 50 feet long or the distance to the foundation, whichever is less; and maintained at a depth of 6". Filter fabric (geotextile) should be used as a separation barrier between the rock and soil if soils are mainly clay or silt.



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### **PLAN**

#### NOTES:

- 1. THE STRAW BALES SHALL BE PLACED ON SLOPE CONTOUR WITH ENDS OF STRAW BALE FENCE TURNED UPSLOPE TO PREVENT FLANKING
- 2. BALES TO BE PLACED SO THAT BINDINGS ARE ORIENTED AROUND THE SIDES RATHER THAN ALONG THE TOPS AND BOTTOMS OF THE BALES.
- 3. BALES TO BE PLACED IN A ROW WITH THE ENDS TIGHTLY ABUTTING.
- 4. KEY IN BALES 4" INTO SOIL TO PREVENT EROSION OR FLOW UNDER BALES

**FIG. E - 2** STRAW BALE FENCE











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## PURPOSE

To divert runoff around disturbed areas to a location where the clean water can be discharged to existing vegetation in such a way as to prevent any negative offsite impacts.

## CONDITIONS WHERE PRACTICE APPLIES

1. Where drainage areas do not exceed 3 acres.

2. Upslope of disturbed areas where erosion is likely to occur.

3. Upslope of soil piles.

4. Above steep cut or fill slopes.

#### STABILIZATION

Diversions side slopes, ridge, downslope side of the berm and channel should be stabilized within 7 days of final grading by:

1. Sodding;

2. seeding and mulching in combination with filter fabric barriers or straw bale barriers;

3. covering with suitable geotextile;

4. covering with 6 mil polyethylene sheeting. (vegetation should be used as the stabilization method if diversion is to be in place 30 days or longer)

# FIG. E – 10

TEMPORARY DIVERSION













Register, February, 1999, No. 518

DEPARTMENT OF COMMERCE

# s. Comm 21.16 Frost Protected Shallow Footings

In lieu of frost walls, the following is an acceptable method.

### **Minimum Ground Insulation Requirements (1)**

		Mean Annual Temperature (2, 6)			Minimum Footing Depth (7, 8)	
Air Freezing Index (F- days) (3)	W-Insulation Width from Edge of Footing (4, 5)	38	40	≥41	D- Concrete Depth	G-Granular Base Thickness
2250 or less	63"	NA	NA	2.5"	10"	6"
2251 - 3000	79"	4"	3.5"	3.5"	10"	6"
3001 - 3750	91"	5"	NA	NA	10"	6"

Notes:

1. Recommendations are based on information found in "Design Guide for Frost-Protected shallow Foundations" prepared for the U. S. Department of Housing and Urban Development by NAHB Research Center (Instrument 3: DU100K000005987, dated June 1994

2. Units are degrees Fahrenheit. See estimate provided on Mean Annual Temperature Contour Map.

3. Air freezing index shall be based on maximum year expected for a 100 year return period. See estimate provided on AFI Contour Map.

4. Ground insulation to the building interior can be extended beneath the entire slab where it is desired to protect the entire slab from frost heave action.

5. Ground insulation to the building interior can be in one horizontal plane (as shown in the detail) and covered with non frost-susceptible fill or the insulation maybe placed directly beneath the slab.

6. Insulation thickness recommendations are for extruded polystyrene (XPS) insulation.

7. The minimum depth of concrete footing and horizontal insulation is 10". A 6" drainage layer is required under the insulation.

8. Insulation placed directly beneath the footing shall be Type IV or Type VI XPS in accordance with ASTM C578. Maximum deadload placed on the Type IV insulation shall be 1200 pounds/square foot. Maximum deadload placed on Type VI shall be 1900 psf.



Plan View

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**Air-Freeze Index Contour Map** 



Mean Annual Temperature Contour Map

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## **UDC Energy Worksheet**

The UDC Energy Worksheet is required to be submitted with building plans for plan review prior to issuance of a building permit. Following is a sample dwelling and completed Energy Worksheet and a blank worksheet after that. The sample completed worksheet has been completed for both the Prescriptive Package and System Design Methods for demonstration purposes. Normally only one method is required to be completed for showing code compliance.

Sample dwelling: Non-Electrically heated single-family dwelling located in Dane County (Zone 3). Has 1,500 square feet and 186 linear feet of perimeter building thermal envelope. Garage is not heated. Estimated infiltration rate is .3 air changes per hour. There will be 170 cfm of installed exhaust ventilation.

Gross Above-Foundation Walls: Wall $= 8.002(07\% 1/0\%) \times 186$ linear feet $= 1.504$ square feet	
Box sill = 0.81 feet (9-3/4 inches deep: sill, header, subfloor) x 186 linear feet = $151$	square feet
Wood 1 x 8-inch drop siding	$\hat{R} = 0.79$
1-inch extruded polystyrene sheathing	R = 5
R13 batt insulation	R = 13
2 x 4 framing, 16 inches O.C.	R = 4.4
1/2-inch drywall finish	R = 0.56
Door area = 38 sq ft	
Insulated steel doors	U = 0.35
Windows:	
Above-Foundation Windows - 150 sq ft	
Wood, low-E, argon-filled, double-pane with 1/2" air space, rated by NFRC	U = 0.35
Foundation wall window area $= 20$ square feet	X1 0.97
Operable metal w/o thermal break, double pane	U = 0.8/
Foundation - 8 ft high, 1 ft exposed	ter A
8-inch poured concrete	R = 0.8
1-inch extruded polystyrene for full height	$\mathbf{R} = 5$
Ceiling - 1,500 square feet, standard roof trusses (no raised heel)	•
2 x 4 trusses, 24 inches O.C.	R = 4.4
Blown fiberglass insulation	R/inch = 2.5
Insulation in cavity, 16 inches	$\mathbf{R} = 40$
Insulation over framing, 12.5 inches	R = 31.25
5/8-inch drywall finish	R = 0.56
Haating Blant	
CossFired Hot Air 90% AFUE	High Efficiency
Gas-rited Hot All, 50 % Ar 02	g
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Sample - Zone 3         Project Address:         Owner:
Builder:       Owner:         Worksheet Completed By:       Date:         Does dwelling unit have three kilowatts or more input capacity of permanently installed electrical space heating equipment?         Dives (see below)       NO         You will need to apply the stricter standards shown for electrically-heated homes if you answered "YES" to the above question.         A. Area Calculations       Enter appropriate dimensions to obtain area values. Some calculations will not be necessary depending on home design or calculation method. These calculated areas are referenced elsewhere on this worksheet, for example, "(A.1.)".         1. Window, Skylight & Patio Door Area (overall unit area)       2. Opaque Door Area         a. In Above- Foundation Walls       b. In Foundation Walls         a. In Above- Foundation Walls       b. In Foundation Walls $\frac{150}{c. Total (a. + b.) =}$ $\frac{20}{170}$ sq. ft. $\frac{38}{c. Total (a. + b.) =}$ $\frac{38}{20}$ or $\frac{5q. ft.}{c. Total (a. + b.) =}$ $3.$ Gross Exposed Basement Wall Area       4. Basement Wall Area Below Grade $1'x \ 186'$ $1302$ sq. ft. $5.$ Opaque [1] Basement Wall Area (A.3. + A.4 A.1.bA.2.b.)       1504 + 151 $186 + 1302 - 20 - 0$ $1468$ sq. ft. $14655 - sq. ft.$ $1655 - 150 - 38$ $1655 + 20 + 0$ $1655 - 150 - 38$
Date: <th< th=""></th<>
Does dwelling unit have three kilowatts or more input capacity of permanently installed electrical space heating equipment? U YES (see below) NO You will need to apply the stricter standards shown for electrically-heated homes if you answered "YES" to the above question. A. Area Calculations Enter apportate dimensions to obtain area values. Some calculations will not be necessary depending on home design or calculation method. These calculated areas are referenced elsewhere on this worksheet, for example, "(A.I.)". 1. Window, Skylight & Patio Door Area (overall unit area) a. In Above- Foundation Walls b. In Foundation Walls 150 sq. ft. 20 sq. ft. 38 on sq. ft. C. Total (a. + b.) = 38 on sq. ft. 3. Gross Exposed Basement Wall Area 1'x 186' 7'x 186' 7'x 186' 5. Opaque [1] Basement Wall Area (A.3. + A.4 A.1.b A.2.b.) 186 + 1302 - 20 - 0 1468 sq. ft. 7. Above Foundation Code Wall Area (A.6. + A1.b. + A.2.b.) 1655 + 20 + 0 1050 -
You will need to apply the stricter standards shown for electrically-heated homes if you answered "YES" to the above question. A. Area Calculations Enter appropriate dimensions to obtain area values. Some calculations will not be necessary depending on home design or calculation method. These calculated areas are referenced elsewhere on this worksheet, for example, "(A.1.)". 1. Window, Skylight & Patio Door Area (overall unit area) a. In Above- Foundation Walls b. In Foundation Walls 2. Opaque Door Area a. In Above- Foundation Walls b. In Foundation Walls 2. Opaque Door Area a. In Above- Foundation Walls b. In Foundation Walls 2. Opaque Door Area a. In Above- Foundation Walls b. In Foundation Walls 3. Gross Exposed Basement Wall Area 1' x 186' 5. Opaque [1] Basement Wall Area (A.3. + A.4 A.1.b A.2.b.) 186 + 1302 - 20 - 0 1468 sq. ft. 7. Above Foundation Code Wall Area (A.6. + Al.b. + A.2.b.) 1655 + 20 + 0 1655 - 150 - 38 1467
A. Area Calculations         Enter appropriate dimensions to obtain area values. Some calculations will not be necessary depending on home design or calculation method. These calculated areas are referenced elsewhere on this worksheet, for example, "(A.1.)".         1. Window, Skylight & Patio Door Area (overall unit area)       2. Opaque Door Area         a. In Above- Foundation Walls       2. Opaque Door Area         a. In Above- Foundation Walls       a. In Above- Foundation Walls         b. In Foundation Walls       a. In Above- Foundation Walls         b. In Foundation Walls       a. In Above- Foundation Walls         b. In Foundation Walls       a. In Above- Foundation Walls         c. Total (a. + b.) =       38         g. c. Total (a. + b.) =       38         d. Gross Exposed Basement Wall Area       4. Basement Wall Area Below Grade         1'x 186'       7' x 186'         186 + 1302 - 20 - 0       1504 + 151         186 + 1302 - 20 - 0       1504 + 151         186 + 1302 - 20 - 0       1504 + 151         186 + 1302 - 20 - 0       1655 - 150 - 38         1655 + 20 + 0       1655 - 150 - 38
1. Window, Skylight & Patio Door Area (overall unit area) a. In Above-Foundation Walls2. Opaque Door Area a. In Above-Foundation Walls5. In Foundation Walls $150$ c. Total $(a. + b.) =$ sq. ft. $38$ c. Total $(a. + b.) =$ $0$ sq. ft.sq. ft. $150$ c. Total $(a. + b.) =$ $170$ $20$ sq. ft.sq. ft. $38$ c. Total $(a. + b.) =$ $0$ sq. ft. $3.$ Gross Exposed Basement Wall Area $4.$ Basement Wall Area Below Grade $0$ sq. ft.sq. ft. $1' \times 186'$ $186$ sq. ft. $7' \times 186'$ $5.$ Opaque [1] Basement Wall Area (A.3. + A.4 A.1.b A.2.b.) $6.$ Gross Heated Above-Foundation Wall Area, including boxsill $186 + 1302 - 20 - 0$ $1468$ sq. ft. $1504 + 151$ $186 + 1302 - 20 - 0$ $1468$ sq. ft. $1655 - 150 - 38$ $1655 + 20 + 0$ $1075$ $1075$
a. In Above- Foundation waits $\frac{150}{c. Total (a. + b.) =} sq. ft.$ $\frac{20}{sq. ft.} sq. ft.$ $\frac{38}{c. Total (a. + b.) =} sq. ft.$ $\frac{1302}{sq. ft.} sq. ft.$ $\frac{1302}{sq. ft.} sq. ft.$ $\frac{1302}{sq. ft.} sq. ft.$ $\frac{1302}{sq. ft.} sq. ft.$ $\frac{1468}{sq. ft.} sq. ft.$ $\frac{1655}{sq. ft.} sq. ft.$ $\frac{1655}{sq. ft.} sq. ft.$ $\frac{1655 - 150 - 38}{1467}$
$\frac{150}{c. \operatorname{Total}(a.+b.) =} \operatorname{sq. ft.} \frac{20}{170} \operatorname{sq. ft.} \frac{38}{c. \operatorname{Total}(a.+b.) =} \operatorname{sq. ft.} \frac{4. \operatorname{Basement Wall Area Below Grade}}{4. \operatorname{Basement Wall Area Below Grade}}$ $\frac{1' \times 186'}{186} \frac{7' \times 186'}{1302} \operatorname{sq. ft.} \frac{1302}{sq. ft.} \operatorname{sq. ft.} \frac{1302}{sq. ft.} \operatorname{sq. ft.} \frac{1302}{sq. ft.} \operatorname{sq. ft.} \frac{1}{sq. ft.} \frac{1}{1504 + 151}$ $\frac{186 + 1302 - 20 - 0}{1468} \operatorname{sq. ft.} \frac{1}{1504 + 151} \frac{1655}{sq. ft.} \operatorname{sq. ft.} \frac{1}{sq. ft.} \frac{1}$
$\frac{150}{c. \text{ Total } (a. + b.) =} \frac{20}{170} \frac{sq. ft.}{c. \text{ Total } (a. + b.) =} \frac{38}{sq. ft.} \frac{0}{c. \text{ Total } (a. + b.) =} \frac{sq. ft.}{38} \frac{0}{c. \text{ Total } (a. + b.) =} \frac{sq. ft.}{38} \frac{0}{c. \text{ Total } (a. + b.) =} \frac{sq. ft.}{38} \frac{0}{c. \text{ Total } (a. + b.) =} \frac{sq. ft.}{38} \frac{0}{c. \text{ Total } (a. + b.) =} \frac{sq. ft.}{38} \frac{0}{c. \text{ Total } (a. + b.) =} \frac{sq. ft.}{38} \frac{0}{c. \text{ Total } (a. + b.) =} \frac{sq. ft.}{38} \frac{0}{c. \text{ Total } (a. + b.) =} \frac{sq. ft.}{38} \frac{0}{c. \text{ Total } (a. + b.) =} \frac{sq. ft.}{38} \frac{0}{c. \text{ Total } (a. + b.) =} \frac{sq. ft.}{38} \frac{0}{c. \text{ Total } (a. + b.) =} \frac{sq. ft.}{38} \frac{1}{c. \text{ Total } (a. + b.) =} \frac{sq. ft.}{38} \frac{0}{c. \text{ Total } (a. + b.) =} \frac{sq. ft.}{38} \frac{1}{c. \text{ Total } $
3. Gross Exposed Basement Wall Area       4. Basement Wall Area Below Grade         1'x 186'       7'x 186'         186       sq. ft.         5. Opaque [1] Basement Wall Area (A.3. + A.4 A.1.b       6. Gross Heated Above-Foundation Wall Area, including boxsill         A.2.b.)       186 + 1302 - 20 - 0         186 + 1302 - 20 - 0       1504 + 151         186 + 1302 - 20 - 0       1504 + 151         186 + 1302 - 20 - 0       1504 + 151         186 + 1302 - 20 - 0       1504 + 151         186 + 1302 - 20 - 0       1504 + 151         186 + 1302 - 20 - 0       1655         186 + 1302 - 20 - 0       1504 + 151         186 + 1302 - 20 - 0       1504 + 151         186 + 1302 - 20 - 0       1655         186 + 1302 - 20 - 0       1655         186 + 1302 - 20 - 0       1655         186 + 1302 - 20 - 0       1655         186 + 1302 - 20 - 0       1655         186 + 1302 - 20 - 0       1655         197 - 38       1655
1' x 186'7' x 186'186sq. ft.5. Opaque [1] Basement Wall Area (A.3. + A.4 A.1.b A.2.b.)6. Gross Heated Above-Foundation Wall Area, including boxsill186 + 1302 - 20 - 01504 + 151186 + 1302 - 20 - 01504 + 151186 + 1302 - 20 - 01504 + 151186 + 1302 - 20 - 01504 + 151186 + 1302 - 20 - 01655186 + 1302 - 20 - 01655186 + 1302 - 20 - 01655186 + 1302 - 20 - 01655186 + 1302 - 20 - 01655186 + 1302 - 20 - 016551655 + 20 + 01655 - 150 - 381655 + 20 + 01655 - 150 - 38
1861302sq. ft.5. Opaque [1] Basement Wall Area (A.3. + A.4 A.1.b A.2.b.)6. Gross Heated Above-Foundation Wall Area, including boxsill186 + 1302 - 20 - 01504 + 151186 + 1302 - 20 - 01504 + 151186 + 1302 - 20 - 01504 + 151186 + 1302 - 20 - 01655186 + 1302 - 20 - 01504 + 151186 + 1302 - 20 - 01655186 + 1302 - 20 - 01504 + 151186 + 1302 - 20 - 01655186 + 1302 - 20 - 01655186 + 1302 - 20 - 01655186 + 1302 - 20 - 016551655 + 20 + 01655 - 150 - 381655 - 150 - 381467
5. Opaque [1] Basement Wall Area (A.3. + A.4 A.1.bA.2.b.)       6. Gross Heated Above-Foundation Wall Area, including boxsill         186 + 1302 - 20 - 0       1468         186 + 1302 - 20 - 0       1504 + 151         186 + 1302 - 20 - 0       1504 + 151         186 + 1302 - 20 - 0       1655         186 + 1302 - 20 - 0       1655         186 + 1302 - 20 - 0       1655 - 150 - 38         1655 + 20 + 0       1655 - 150 - 38
186 + 1302 - 20 - 0       1504 + 151         186 + 1302 - 20 - 0       1468         1468       sq. ft.         If the exposed area of A.3. is greater than the below grade area of A.4., add A.5. to A.7 and cross out the number in this cell.       1655         7. Above Foundation Code Wall Area (A.6. + A1.b. + A.2.b.)       8. Opaque [1] Above-Foundation Wall Area (A.6 A1.a A.2.a.)         1655 + 20 + 0       1655 - 150 - 38
$ \begin{array}{c} 1468 \\  sq. ft. \\ 1655 \\ 1655 + 20 + 0 \end{array} $ $ \begin{array}{c} 1468 \\  sq. ft. \\ 1655 \\ 1655 - 150 - 38 \end{array} $ $ \begin{array}{c} 1655 \\ 1655 \\ 1467 \end{array} $
sq. ft.       1655         If the exposed area of A.3. is greater than the below grade area of A.4., add A.5. to A.7 and cross out the number in this cell.       sq. ft.         7. Above Foundation Code Wall Area (A.6. + A1.b. + A.2.b.)       8. Opaque [1] Above-Foundation Wall Area (A.6 A1.a A.2.a.)         1655 + 20 + 0       1655 - 150 - 38
A.4., add A.5. to $A_{1,2}$ and cross out the number in this content of the foundation o
1655 + 20 + 0 1655 - 150 - 38
1000 + 20 + 0 4467
10/5 m A
9. Floor Area Over Interior Unconditioned Spaces Less Than 10. Insulated Roof Or Ceiling (less skylights)
<sup>50°</sup> 28 x 45 = 1260
$12 \times 20 = 240$
0 so ft 1500
11. Exterior Floor Area (Overhangs) 12. Crawl Space Wall Area
0 sq. ft. 0 sq. ft.
13. Slab On Grade (above or less than 12 inches below grade)       14. Total Heated Envelope Area (A.5 + A.7 + A.9 + A.10 + A.11 + A.12 + (A.13. X 2'))
1468 + 1675 + 0 + 1500 + 0 + 0 + 0
0 lineal feet of slab perimeter 4643 sq. ft.
15. Percent Glazing (for Prescriptive Package Method, 16. Windows Description - Above-Foundation Windows:
Section B, only) (A.I.C. + A./. X 100%) Frame type: Wood of wood of wood of a wood
170 + 1675 x 100%       Dual-Glazing Air Space: □ 1/4' □ 3/8" x 1/2" or more         10.2       %         10.2       %

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B. Prescriptive Package Method (Skip this section if using the System Design Method of Sections C-F) The prescriptive package method is the simplest method for determining compliance with the UDC insulation and window requirements. To use the prescriptive package method, enter your actual design values in the "Actual" row below. For a component,

with two or more areas of different insulation levels, such as windows, either use the least insulating value for both areas or use the Weighted Average tables below. Multiply your % glazing by the glazing U-value to obtain your "Glazing Factor". Find the Prescriptive Table that applies to your space heating fuel and sheathing type. Select a package from the table that most closely matches the construction indicated on your plans. Do not exceed the package U-values or glazing factor or fall below the package R-values with your design. Transfer the R-Values and U-values to the blank table below in the "Allowed" row. Then proceed to Section F. See page 2 for detailed instructions for this section.

	Package #	% glazing	U glazing	Glazing Factor (% glazing × U	R wall	R ceiling	R Bsmt Crawl Space, Slab or	U door	U overall	Equip. Eff.
	1	and the second second		glazing)			Floor			
Actual		10.2 % (A.15)	0.41	0.042	R13 + 5	R40	R5	0.35		High
Allowed	45		*******	0.0504 Max	R18, I Min	<b>R40</b> Min	R5 Min	0.35 Max	0.086	High

(Please go to Section F.)

Optional R-Value/U-Value Weighted Average Table for Component: Windows

Component Constructi	on Description	R Value	U-Value (1+R Value)	Area (sq ft)	U-Value × Area (UA)
Basement windows	· · ·		0.87	20	17.4
Above-foundation wind	dows		0.35	150	52.5
			· · · · · · · · · · · · · · · · · · ·	Total Area = 170	Total UA = 69.9
69.9 .	170	0	.41		

(Total UA) (Total Area)

(Weighted Average U-Value (for windows or doors))

(Total Area) (Total UA) (Weighted Average R-Value (for all other components))

Optional R-Value/U-Value Weighted Average Table for Component:

Component Construction Description	R Value	U-Value (1+R Value)	Area (sq ft)	U-Value × Area (UA)
			Total Area =	Total UA =

(Weighted Average U-Value (for windows or doors)) (Total UA) (Total Area)

(Total Area) (Total UA) (Weighted Average R-Value (for all other components))

Because the sample house fit a Package, you would normally skip ahead to Section F. For demonstration purposes here, the System Design Method is also completed.

C. Code-Allowed Heat Loss For System Design Method

Enter area values from Section A as notated and temperature differences per footnote 2 into this table and then multiply across by the electric or non-electric code-required U-value. Total the right column to find the total allowed heat loss factor.

Component	Area Component From Sect A. × Required U-Value			
		NON-ELEC	D ELECTRIC	
1. Opaque Basement Wall [2]	1468 (A.5.)	0.077 [3]	0.077 [3]	113
2. Above Foundation Code Wall	1675 (A.7.)	0.110	0.080	184
3. Floor Over Interior Unconditioned Space	(A.9.)	0.050	0.050	
4. Roof or Ceiling	1500 (A.10.)	0.026	0.020	39
5. Floor Over Exterior	(A.11.)	0.033	0.033	
6. Crawl Space Wall	(A.12.)	0.060	0.060	
7. Slab On Grade Unheated Heated [3]	(A.13.) Lin. ft.	0.72 'F' 0.70 'F'	0.68 'F' 0.68' F'	
8. Subtotal	336			
<ol> <li>Credit for High Efficiency Heating Plant: 1.18 for Otherwise use 1.0</li> </ol>	× 1.18			
10.	396.5			

#### D. System Design Method - Actual 'U' Values Of Your Home's Components

**D.1.** Above-Foundation Components - If applicable, check the appropriate typical component constructions listed below, and use the pre-calculated U values. If your wall construction is not listed, you may obtain a pre-calculated U value from the default U-Value tables in the UDC Appendix. (Note that the default Table 2 Wood Frame U-values assume no insulating sheathing which penalizes you if your wall does have insulating sheathing, then you may need to use the Manual Calculation section below.) If you are using exterior metal framing, then you must use the Metal-Frame Wall U-Values of the UDC Appendix. If your component construction is not listed here or in the default tables, you need to use the Manual Calculation section below.) If you are using in the listed here or in the default tables, you need to use the Manual Calculation section below to manually enter R-values for the different layers of building materials from the Typical Thermal Properties of Building Materials Table of the UDC Appendix, ASHRAE Fundamentals Manual or manufacturer's specifications. Total them across and then obtain the U-value by taking the reciprocal (1/R) of the total R-value.

Above-Foundation Walls 2X4, 16" O.C., R-13 batt, R-1 board: U079 2X4, 16" O.C., R-13 batt, R-5 board: U061											
	2X6, 16" O.C., R-19 batt, R-1 board; U059								5 board: L	I <del>-</del> .049	
□ Other - describe:							U		from D	efault Tabl	e
Roof or Ceiling	🗆 2X4	truss, 24"	O.C., with	R-38 insulation	n: U03	0 🗆 2X4	truss, 24" O.C	., with R-5	2 insulatio	on: U02	:5
9	🖸 2X I	2 cathedra	l ceiling, l	6" O.C., with R	-38 insula	tion U02	7				
Other - describe: F	140 with regu	ılar truss	es				U	- <i>0.029</i>	from D	efault Tab.	le 1
Floor Over Exterior	or Unconditio	ned Space	3	2X10 joists,	16" O.C.,	R-19 batt: L	J047				
🗖 Other - describe:							t	J	from D	efault Tab	le
		Option	al Manu	al U-Value Calo	ulation (i	f assembly r	tot listed abov	e)			· ·
	Cavity Or	Ext.	Ext.	Insulation	Shea-	Framing	Insulation	Inter-	Int.	Total	U-Value
Component	Solid If	Air	Finish	Over	thing	Or Solid	Within	ior	Air	R-'	
Name	Applicable	Film*		Framing			Cavity	Finish	Film*	Value	
Above Foundation	Cavity	.17	0.79	5.0			13	0.56	.68	20.2	.050
Wall	Solid	.17	0.79	5.0		4.4		0.56	.68	11.6	.086
	Cavity										L
	Solid					l	<u> </u>				<u> </u>

* Air Film R-Values								
Location	Heat Flow Direction							
	Upwards	Horizontal	Downwards					
Exterior	.17	.17	.17					
Interior	,61	.68	.92					

**D.2. Foundation And Slab-On-Grade Components -** Check appropriate boxes for planned type of construction to determine precalculated overall 'U-value' including air films, wall, insulation, soil and cavity/solid differences. Slab on grade F-values are per lineal foot of slab perimeter.

Component Type	U-V	alue
Foundation Wall	Basement	Crawl Space
□ Masonry or concrete wall without insulation	0.360	0.477
Masonry or concrete wall with R-5 insulation board for full height	0.115	0.136
Masonry or concrete wall with R-10 insulation board or R-11 insulation batt and 2X4's for full height	0.072	0.081
Permanent wood foundation with R-19 batt for full height	0.054	0.059
Basement or crawl space floor without insulation	0.025	0.025
Basement floor with R-5 insulation	0.022	0.022
Slab-On-Grade (or within 12" of grade)	F-V	alue
□ Slab-on-grade without insulation	1.	04
□ Slab-on-grade with R-5 insulation for 48" total horizontal and vertical application	0.	74
□ Slab-on-grade with R-10 insulation board for 48" total application	0.	68

**D.3. Windows And Doors -** Use manufacturer's specifications for window and glazed door values, if they were determined per NFRC Std 100, to enter into Table E. Otherwise see default tables of UDC s. Comm 22.05 for U-values.

### E. System Design Method - Calculated Envelope Heat Loss Factor Of Your Home

Enter values into table from elsewhere on this worksheet and multiply across to find the actual heat loss factor of each component. If using pre-calculated component U-values, do not calculate separate cavity and solid figures or apply wood frame factors. Total component heat loss factors in right column to find total envelope heat loss factors.

	Cavity Or	Area	×	×	
Component	Solid H	From Sect	WOOD Frame	Actual 'U' Value	TIAN CITAN
	Appneable	Jeck A		From Sect. D	(UA)
Above-Foundation Windows		750 (A.1.a.)		0.35	52.5
Foundation Windows		20 (A.1.b)		0.87	17.4
Doors		38 (A.2.c)	******	0.35	13.3
Opaque Basement Wall		1468 (A.5.)		0.115	168.8
Opaque Above-Foundation Wall	Cavity		.75	.050	55
• •	Solid	1467 (A.8.)	.25	.086	31.5
Floor Over Unconditioned Spaces	Cavity				
	Solid	(A.9.)			
Roof or Ceiling	Cavity				
	Solid	1500 (A.10.)		0.029	43.5
Floor Over Exterior	Cavity				
	Solid	(A.11.)			
Crawl Space Wall		(A.12.)			
	╉━━━━╋	· · ·		<u> </u>	
Slab On Grade		(A.13.)Lin. ft.		F-Value	· · · · · · · · · · · · · · · · · · ·
Total Calculated Envelope I	382				
ractor of the 10 of Section C	. (Emer nere:	)[	oy more mail I	/0	

\*\* Adjustment Factors For Wood-Framed Components - Do not apply if your are using a pre-calculated or default U-Value.

Spacing Of Framing	Stud	Walls	Joists/Rafters			
Members	Cavity	Solid	Cavity	Solid		
12"	.70	30	.86	.14		
16"	- 75	25	.90	.10		
24"	.78	.22	.93	.07		

### F. Heat Loss Factor Due to Air Infiltration (for heating equipment sizing)

Enter appropriate values. A maximum infiltration air change rate of 0.5 per hour is allowed in addition to ventilation losses.

Floor Level	Area (sq ft)	× Height (ft)	Fan Capacity (cfm)	× Constant	× Air Changes Per Hour	= Heat Loss Factor(UA)
Basement	1500	8		.018	0.3	64.8
Level 1	1500	8		.018	0.3	64.8
Level 2				.018		
Level 3				.018		
Ventilation			170	.432		73.4
	203					

#### G. Heating Equipment Sizing

Enter appropriate value to determine the maximum and minimum allowable heating equipment capacity in BTUs/HR. A more detailed calculation may be submitted to the local code official. [4]

1	Prescriptive 0.086 4643							
N	Method: U overall from selected Prescriptive Total Envelope Area							
	Package of Section B (A.14.)		399.3					
	OR System Design Method: Calculated Heat Loss Factor from Sect. E.	_						
	Infiltration & Ventilation Heat Loss Factor (from Sect. F.)	+	203					
	Total Heat Loss Factor (UA)	1	602.3					
	Temperature Difference from Zone Table on page 1	×	85					
	Minimum Heating Equipment Output	=	51,196					
1	Allowable Heating Equipment Size Margin Multiplier	×	1.15					
	Maximum Allowable Heating Equipment Output [5]	=	58,875					
Ì	Planned Furnace Output Or Boiler IBR Rating		60,000					
	Make & Model if High Efficiency Credit has been taken: Acme XLH60K							
(See notes on page 2 of Energy Worksheet; I = insulating sheathing, RT = raised heel roof truss) Table B.1 Prescriptive packages, Non-electric Heat, Structural Sheathing only								
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Package	Glazing Factor	R wall	R ceiling	R basement	Udoor	Uoverall	HVAC Equipment Efficiency	
1	0.0370	R21	R42	R7	0.35	0.073	Normal	
2 ·	0.0264	R21	R51, RT	R5	0.35	0.073	Normal	
3	0.0333	R15	R42	R10	0.35	0.073	Normal	
4	0.0440	R19	R33	RIO	0.35	0.073	Normal	
5	0.0330	R13	R42	RII	0.35	0.073	Normal	
6	0.0480	<b>R</b> 19	R33	R11	0.35	0.073	Normal	
7	0.0600	R2i	R47	RII	0.35	0.073	Normal	
8	0.0407	R13	R44	R13	0.35	0.073	Normat	
9	0.0600	R 19	R42	R13	0.35	0.073	Normal	
10	0.0680	R21	R38 RT	R13	0.35	0.073	Normal	
11	0.0296	RI3	R49	R5	0.35	0.086	High	
12	0.0440	R19	830	R5	0.35	0.086	High	
13	0.0520	R21	R33	R5	0.35	0.086	High	
14	0.0720	R13	R47	RIO	0.35	0.086	High	
15	0.0784	.R19	R 38	RIO	0.47	0.086	High	
16	0.0640	R13	R33	RII	0.47	0.086	High	
17	0.0896	R10	RAQ	R11	0.35	0.086	High	
19	0.0896	P21	P34	R11	0.35	0.086	High	
10	0.0000	D10	P34	RII	0.42	0.086	High	
19	0.0720	D12	P40		0.35	0.086	High	
20	0.0840	810	P20	P12	0.33	380.0	High	
- 21	0,0840	R17 D01	R30	R13	0.47	0.086	High	
44	0.0690	R41	R51	R15	U,47	U.Va0	HVAC Fouinment Efficience	
Package	Glazing Factor	K Wall	R celling	R Crawi	0 4001	0 overall	Normal	
- 23	0.0320	R19	R34	<u>N10</u>	0.47	0.070	High	
24	0.0072	R13	022	R17	0.47	0.003	High	
Dealiana	Ciertos Kaston	R13	P calling	Relah	TI door	1 overall	HVAC Equipment Efficiency	
Package	Giazing Pactor	1 Wau 1 1 2 2 1	N Cenning	DC	0.47	0.103	Normal	
20	0.0300	D12	D26	R.7	0.47	0.103	High	
21	0.0740	RD 012	R30	<u>NJ</u>	0.47	0.121	High	
20	Charles Faster	R15	D selling	D bested slob	U.47	11 overall	HVAC Equipment Efficiency	
Package	Giazing Factor	R WAB	K ceiling	R neated-slad	0.42	0 101	Normal	
	0,0560	R21	R47	R3	0.47	0.100	High	
30	0.0728	<u> </u>	R42	<u>K5</u>	0.47	0.120	tiga Uiab	
	0.0760	K13	K38	<u>кэ</u>	0.47	U.120	Inigii	
Package	Glazing Factor	R wall	R ceiling	K floor	U door	0 overail	HVAC Equipment Enterency	
32	0.0480	<u>R19</u>	R47	R19	0.33	0.005		
33	0.0728	<u></u>	K30	R19	0.47	0.077	Tugi Uish	
	0.0560	<b>K13</b>	<u>  K39</u>	RI9	0.47	0.077	nigu	
		Table B-2 I	Prescriptive pa	ckages, Non-elec	tric Heat, Ir	sulating Sheat	ung	
rackage	Glazing Pactor	R Wall	R ceiing	K Dasement	0.26	0.072	Normal	
35	0.0370	R20,1	K42	R/	0.35	0.073	Normal	
30	0.0363	K28, 1	K38, K1	K3	0.35	0.073	Normal	
37	0.0552	K18.1	K44		0.35	0.073	Internet	
38	0.0560	R20, 1	R47	<u></u>	0.35	0.073	ivormat	
39	0.0560	R23, 1	R34	<u></u>	0.35	0.073	Normat	
40	0.0560	R 18, 1	R47	<u></u>	0.35	0.073	Normai	
41	0.0616	R23, 1	R42	RII	0.35	0.073	Normal	
42	0.0546	R18, 1	R44	RII	0.35	0.073	Normat	
43	0.0672	R23.1	1 R40	[ R13	0.35	0.073	l Normal	

Prescriptive Package Tables (Corrected) s on page 2 of Energy Worksheet; I = insulating sheathing, RT = raised heel roo

40	0.0560	R18,1	R47	- R11	0.35	0.073	Normal	
41	0.0616	R23, 1	R42	RII	0.35	0.073	Normal	•
42	0.0546	R18,1	R44	RH	0.35	0.073	Normat	
43	0.0672	R23,1	R40	R13	0.35	0.073	Normal	
44	0.0720	R25.1	R36	R13	0.35	0.073	Normal	
45	0.0504	R18, 1	R40	R5	0.35	0.086	High	
46	0.0560	R19,1	R47	R5	0.35	0.086	High	
47	0.0560	R23, 1	R38	R5	0.47	0.086	High	
48	0.0600	R25, I	R38	R5	0.47	0.086	High	
49	0.0680	R26, 1	R42	R5	0.35	0.086	High	
50	0.0680	R28, 1	R47	R5	0,47	0.086	High	
51	0.0672	R26, 1	R47	R5	0.35	0.086	High	
52	0.0672	R28, 1	R38	R5 -	0.35	0.086	High	
53	0.0720	R20, 1	R42	R7	0.47	0.086	High	
54	0.0855	R18, 1	R36	RII	0.35	0.086	High	

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## Wisconsin Uniform Dwelling Code Energy Worksheet

Instructions: This worksheet is a Safety & Buildings Division (S&BD)-approved method of manually showing compliance with the energy conservation and heating equipment sizing requirements of the Uniform Dwelling Code (UDC), for new dwelling permits submitted on or after February 1, 1999. It may be necessary for the user to purchase a copy of the UDC from State Document Sales, (608)266-3358. Additional information is printed in the UDC Commentary, which is available for a fee, as are blank copies of this form, from S&BD at POB 2509, Madison, WI 53701, Tel. 608-267-4405. Earlier editions of this worksheet may NOT be used. Numbers in brackets, [1], refer to the footnotes printed on page 2.

You may also submit completed worksheets from the computer program WIScheck, which is available for free download from http://www.energycodes.org/ on the Internet.

A required U-value is the maximum acceptable heat transmittance for an element. A required insulation R-value is the minimum acceptable level of resistance to heat transmittance. (U-values and R-values are reciprocals of each other.) If a component includes two or more areas of different insulation levels, either use the less insulating value for both areas, or use the Optional Weighted Average table in the Prescriptive Package Method section or enter separate areas and insulation values in the System Design Method. All "U" values must be carried to four places after the decimal point, rounded to three places. Other values may be rounded to the whole number.

Window and door U-values must be tested and documented by the manufacturer in accordance with the National Fenestration Rating Council (NFRC) test procedures or be taken from the glazing U-value table in s. Comm 22.05. Center-of-glass U-values cannot be used. If a door contains glass and an aggregate U-value rating for that door is not available, include the glass area of the door with your windows and use the opaque door U-value to determine compliance of the door.

The code gives credit for high-efficiency heating equipment. "High-Efficiency" means a furnace with an AFUE of 90% or more, or a heat pump with an HSPF of 7.8 or more without the use of electric resistance backup heat of greater than 3 kilowatts. If you plan to install more than one piece of heating equipment, the equipment with the lowest efficiency must exceed the efficiency required by the selected package.

Choice of Method: You have the choice of using the Prescriptive Package Method or the System Design Method to show code compliance. For the simpler Prescriptive Package Method, which is recommended for standard designs, complete Sections A., B., F., and G. Instructions are on page 2. You will be first calculating component areas, then comparing your planned insulation levels to the required insulation levels of the Prescriptive Packages. You will then calculate infiltration and ventilation heat losses to size your heating equipment. If you cannot comply with one of the prescriptive packages, you may be able to show compliance by the System Design Method.

For the System Design Method, which is recommended for alternative designs in which more insulation is installed in one component to offset less in another, complete Sections A., C., D., E., F. and G. You will be first calculating component areas, then a code-allowed heat loss factor, then component U- and R-values and then your calculated heat loss factor which you will compare to the code-allowed heat loss factor. You will then calculate infiltration and ventilation heat losses to size your heating equipment.

The **County Zone Table** below is use for determining the temperature difference for sizing your heating plant in Section G. You may submit to your local code official more exact calculations to size your heating equipment.

Zone 1 - 95 degrees	Zone 2 - 90 degrees	Zone 3 - 85 degrees	Zone 4 - 80 degrees
Ashland, Barron, Bayfield,	Adams, Buffalo, Clark, Eau Claire,	Brown, Calumet, Columbia, Crawford,	Jefferson, Kenosha,
Burnett, Chippewa, Douglas,	Jackson, Juneau, LaCrosse, Langlade,	Dane, Dodge, Door, Fond du Lac,	Milwaukee, Ozaukee,
Dunn, Florence, Forest, Iron,	Marathon, Marinette, Menominee,	Grant, Green, Green Lake, Iowa,	Racine, Rock.
Lincoln, Oneida, Pierce, Polk,	Monroe, Portage, Shawano, Oconto,	Kewaunee, LaFayette, Manitowoc,	Walworth,
Price, Rusk, Saint Croix,	Pepin, Trempeleau, Vernon,	Marquette, Outagamie, Richland, Sauk,	Washington,
Sawyer, Taylor, Vilas, Washburn	Waupaca, Wood	Sheboygan, Waushara, Winnebago	Waukesha

SBD-5518 (R, 12/98) Corr.

#### **Detailed Instructions for Section B. Prescriptive Package Method:**

R-value requirements are for insulation only and do not include structural components.

For a component with two or more areas of different insulation levels, either use the least insulating value for both areas or use the Weighted Average tables on page 4.

Wall R-values represent the sum of the wall cavity insulation plus insulating sheathing, if used. Do not include exterior siding, structural sheathing or interior drywall. For example, an R-20 requirement could be met *EITHER* by R-15 cavity insulation plus R-5 sheathing *OR* R-13 cavity insulation plus R-7 sheathing. Note that there are separate tables for walls with structural sheathing only and for walls with insulating sheathing. To use a table for insulating sheathing, the sheathing used must be at least R-4, except that at least R-2 insulation may be provided over corner bracing. Table wall R-Values apply to wood-frame or mass (concrete, masonry, log) wall assemblies, but not to metal-frame construction. If metal frame is planned, use the adjusted R-Values from the Metal-Frame Wall Tables of the UDC Appendix. Table wall values apply to boxsills.

Ceiling R-values represent the sum of the cavity insulation plus insulating sheathing, if used. For ventilated ceilings, any insulating sheathing must be placed between the conditioned space and the ventilated portion of the roof. Ceiling R-values with "RT" indicates that a raised-heel truss or oversized truss construction must be used so that the insulation achieves the full insulation thickness over the exterior walls.

Floor requirements apply to floors over unconditioned spaces (such as un-insulated crawlspaces, basements and garages). Floors over outside air shall have a Uoverall = 0.033 or R-30 added insulation.

"Heated-Slab" requirements apply to slabs that contain heat ducts or pipes. All slab insulation must extend at least 48 inches either 1) down from the top of the slab, or 2) down from the top of the slab to the bottom of the slab and then horizontally underneath the slab, or 3) down from the top of the slab to the bottom of the slab and then horizontally away from the slab, with pavement or at least 10 inches of soil covering the horizontal insulation.

Walls of basements below un-insulated floors must be insulated from the top of the basement wall to the level of the basement floor. Conditioned basement windows and glass doors must be included with the other glazing. Exterior basement doors must meet the door U-value requirements. If more than 50% of the basement is exposed, then all of the basement walls must instead meet the above-foundation wall requirements.

**Crawl space wall R-value requirements** are for walls of unventilated crawlspaces. The crawlspace wall insulation must extend from the top of the wall (including the sill plate) to at least 12 inches below the outside finished grade. If the distance from the outside finished grade to the top of the footing is less than 12 inches, the insulation must extend a total vertical plus horizontal distance of 24 inches from the outside finished grade.

#### Footnotes for worksheet:

- [1] Opaque wall area is wall area minus opening areas of doors and windows.
- [2] These below-grade U-values have the insulating value of the soil added to the code-required U-values which apply to the building materials only. See Sect. D.2. for typical insulated component U-values.
- [3] These slab-on-grade F-values are derived from the code-required U-values and include the heat loss through the edge and body of the slab. See Sect. D.2. Temperature difference is the same as for above-grade spaces.

[4] For building additions, show that the existing heating equipment, if used to heat the addition, is large enough. To do so, you must calculate the heat loss of the whole building.

[5] If desired manufacturer does not have a furnace of this size, then a designer may select the manufacturer's next larger size.

#### Submit completed worksheet pages 3-6 with dwelling plans to local enforcing municipality.

Project Address:

Builder: \_\_\_\_\_ Owner: \_\_\_\_\_

You will need to apply the stricter standards shown for electrically-heated homes if you answered "YES" to the above question.

#### A. Area Calculations

Enter appropriate dimensions to obtain area values. Some calculations will not be necessary depending on home design or calculation method. These calculated areas are referenced elsewhere on this worksheet, for example, "(A.1.)".

1. Window, Skylight & Patio Door Area (overall unit area)	2. Opaque Door Area
a. In Above-Foundation Walls b. In Foundation Walls	a. In Above- Foundation Walls b. In Foundation Walls
sq. ft.	sq. ft sq. ft.
c. Total (a. + b.) =	c. Total $(a. + b.) =$
3. Gross Exposed Basement Wall Area	4. Basement Wall Area Below Grade
:	
sq. ft.	<u>sq. ft.</u>
5. Opaque [1] Basement Wall Area (A.3. + A.4 A.1.b	6. Gross Heated Above-Foundation Wall Area, including boxsill
A.2.b.)	(1, 1) is the set of the set o
	and the state of the second
	na terre a construction de la const No
sq. ft.	
If the exposed area of A.3. is greater than the below grade area of	sq. ft.
A.4., and A.5. to A.7 and cross out the number in this cell. 7. Above Foundation Code Wall Area $(A 6 + A1 h + A2 h)$	8 Opaque [1] Above-Foundation Wall Area (A.6 A1.a A.2.a.)
7. Above Foundation Cone Wait Area (1969 - 1986)	
	and the state of the
sq. ft.	sq. ft.
50°	10. Insulated Root of Centing (1635 skylights)
sa ft	ra ft
11 Exterior Floor Area (Overhangs)	12. Crawl Space Wall Area
Sq. II.	$\frac{5q. II.}{14 \text{ Total Heated Envelope Area (A 5 + A 7 + A 0 + A 10 + A 11 + 11 + 11)}$
15. Stab On Grade (above of less than 12 menes below grade)	A.12 + (A.13, x 2'))
lineal feet of slab perimeter	sq. ft.
15. Percent Glazing (for Prescriptive Package Method,	Ib. Windows Description - Above-Foundation Windows:
Section B, omy) (A.I.C. ÷ A.7. × 100%)	Glazing type: Dual Drible Dual w/storm panel
	Dual-Glazing Air Space: 1/4' 3/8" 1/2" or more
%	Features: Low-E Argon-filled Suspended film (
	Foundation Windows:  Vinyl Metal

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# B. Prescriptive Package Method (Skip this section if using the System Design Method of Sections C-F)

The prescriptive package method is the simplest method for determining compliance with the UDC insulation and window requirements. To use the prescriptive package method, enter your actual design values in the "Actual " row below. For a component, with two or more areas of different insulation levels, such as windows, either use the least insulating value for both areas or use the Weighted Average tables below. Multiply your % glazing by the glazing U-value to obtain your "Glazing Factor". Find the Prescriptive Table that applies to your space heating fuel and sheathing type. Select a package from the table that most closely matches the construction indicated on your plans. Do not exceed the package U-values or glazing factor or fall below the package R-values with your design. Transfer the R-Values and U-values to the blank table below in the "Allowed" row. Then proceed to Section F. See page 2 for detailed instructions for this section.

	Package #	% glazing	U glazing	Glazing Factor (% glazing × U glazing)	R wall	R ceiling	R Bsmt, Crawl Space, Slab or Floor	U door	U overall	Equip. Eff.
Actual		% (A.15)				10 C				1
Allowed				Max	Min	Min	Min	Max		I

(Please go to Section F.)

(Total Area)

#### Optional R-Value/U-Value Weighted Average Table for Component:

Component Construction Description	R Value	U-Value (1+R Value)	Area (sq ft)	U-Value × Area (UA)
· · · · · · · · · · · · · · · · · · ·				
		· · · · · · · · · · · · · · · · · · ·	Total Area =	Total UA =

	+		
(Total UA)	(Total Area)	(Weighted Average U-Value (for windows or doors))	
	÷		
(Total Area)	(Total UA)	(Weighted Average R-Value (for all other components))	

#### Optional R-Value/U-Value Weighted Average Table for Component:

Component Construction Description	R Value	U-Value (1+R Value)	Area (sq ft)	U-Value × Area (UA)
	<u> </u>			
			Total Area =	Total UA =
(Total UA) + (Total Area)	(Weighted a	Average U-Value	(for windows or doo	rs))

(Total UA)

#### C. Code-Allowed Heat Loss For System Design Method

Enter area values from Section A as notated and temperature differences per footnote 2 into this table and then multiply across by the electric or non-electric code-required U-value. Total the right column to find the total allowed heat loss factor.

(Weighted Average R-Value (for all other components))

	Area			= Heat Loss
Component	From Sect A.	× Requi	UA	
		I NON-ELEC	<b>D</b> ELECTRIC	
1. Opaque Basement Wall [2]	(A.5.)	0.077 [3]	0.077 [3]	
2. Above Foundation Code Wall	(A.7.)	0.110	0.080	
3. Floor Over Interior Unconditioned Space	(A.9.)	0.050	0.050	
4. Roof or Ceiling	(A.10.)	0.026	0.020	
5. Floor Over Exterior	(A.11.)	0.033	0.033	
6. Crawl Space Wall	(A.12.)	0.060	0.060	
7. Slab On Grade  Unheated Heated [3]	(A.13.) Lin. ft,	0.72 'F' 0.70 'F'	0.68 'F' 0.68' F'	
8. Subtotal				
<ol> <li>Credit for High Efficiency Heating Plant: 1.18 for Otherwise use 1.0</li> </ol>	furnace or boiler >90% AFU	E; 1.15 for heat put	mp> 7.8 HPSF,	×
10.	Total Co	de-Allowed He	at Loss Factor	

#### D. System Design Method - Actual 'U' Values Of Your Home's Components

D.1. Above-Foundation Components - If applicable, check the appropriate typical component constructions listed below, and use the pre-calculated U values. If your wall construction is not listed, you may obtain a pre-calculated U value from the default U-Value tables in the UDC Appendix. (Note that the default Table 2 Wood Frame U-values assume no insulating sheathing which penalizes you if your wall does have insulating sheathing, then you may need to use the Manual Calculation section below.) If you are using exterior metal framing, then you must use the Metal-Frame Wall U-Values of the UDC Appendix. If your component construction is not listed here or in the default tables, you need to use the Manual Calculation section below. If you component construction is not listed here or in the default tables, you need to use the Manual Calculation section below to manually enter R-values for the different layers of building materials from the Typical Thermal Properties of Building Materials Table of the UDC Appendix, ASHRAE Fundamentals Manual or manufacturer's specifications. Total them across and then obtain the U-value by taking the reciprocal (I/R) of the total R-value.

Above-Foundation Walls 2X4, 16" O.C., R-13 batt, R-1 board: U079						i board: U	1061				
□ 2X6, 16" O.C., R-19 batt, R-1 board: U059					🗆 2X6.	16" O.C., R-1	9 batt, R-5	i board: U	l049		
D Other - describe: U - from Default Table											
Roof or Ceiling	Roof or Ceiling 2X4 truss, 24" O.C., with R-38 insulation; U030 2X4 truss, 24" O.C., with R-52 insulation; U025								5		
	🗆 2X I	2 cathedra	l ceiling, I	16" O.C., with I	R-38 insula	tion U02	7				:
Other - describe:							U	-	from De	fault Table	
Floor Over Exterior	or Uncondition	ned Space	2	2X10 joists	, 16" O.C.,	R-19 batt: L	)047			-	
Other - describe:		•					UU	-	from De	fault Table	
		Ň	lanual U-	Value Calcula	tion (if ass	embly not li	sted above)		1.16.1.1		:
	Cavity Or	Ext.	Ext.	Insulation	Shea-	Framing	Insulation	Inter-	Int.	Total	U-Value
Component	Solid If	Air	Finish	Over	thing	Or Solid	Within	ior	Air	R-	
Name	Applicable	Film*		Framing			Cavity	Finish	Film*	Value	
	Cavity										
	Solid										
	Cavity										
1	Solid										

* Air Film R-Values								
Location	Heat Flow Direction							
	Upwards	Horizontal	Downwards					
Exterior	.17	.17	.17	1				
Interior	.61	.68	.92	1				

**D.2. Foundation And Slab-On-Grade Components -** Check appropriate boxes for planned type of construction to determine precalculated overall 'U-value' including air films, wall, insulation, soil and cavity/solid differences. Slab on grade F-values are per lineal foot of slab perimeter.

Component Type	U-Value		
Foundation Wall	Basement	Crawl Space	
Masonry or concrete wall without insulation	0.360	0.477	
□ Masonry or concrete wall with R-5 insulation board for full height	0.115	0.136	
C Masonry or concrete wall with R-10 insulation board or R-11 insulation batt and 2X4's for full height	0.072	0.081	
C Permanent wood foundation with R-19 batt for full height	0.054	0.059	
Basement or crawl space floor without insulation	0.025	0.025	
Basement floor with R-5 insulation	0.022	0.022	
Slab-On-Grade (or within 12 " of grade)	F-V	alue	
□ Slab-on-grade without insulation	1.	04	
□ Slab-on-grade with R-5 insulation for 48" total horizontal and vertical application	0.74		
□ Slab-on-grade with R-10 insulation board for 48" total application	0.	68	

**D.3. Windows And Doors** - Use manufacturer's specifications for window and glazed door values, if they were determined per NFRC Std 100, to enter into Table E. Otherwise see default tables of UDC s. Comm 22.05 for U-values.

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# E. System Design Method - Calculated Envelope Heat Loss Factor Of Your Home

Enter values into table from elsewhere on this worksheet and multiply across to find the actual heat loss factor of each component. If using pre-calculated component U-values, do not calculate separate cavity and solid figures or apply wood frame factors. Total component heat loss factors in right column to find total envelope heat loss factors.

Component	Cavity Or Solid If Applicable	Area From Sect. A	× Wood Frame Factor**	× Actual 'U' Value From Sect. D	Heat Loss Factor (UA)		
Above-Foundation Windows		(A.1.a.)		1			
Foundation Windows	********	(A.1.b)	**	· · · · · · · · · · · · · · · · · · ·			
Doors		(A.2.c)	م جب خط <u>با م در و م</u>				
Opaque Basement Wall		(A.5.)		· · · · · · · · · · · · · · · · · · ·			
Opaque Above-Foundation Wall	Cavity						
	Solid	(A.8.)					
Floor Over Unconditioned Spaces	Cavity				· · · ·		
	Solid	(A.9.)					
Roof or Ceiling	Cavity						
	Solid	(A.10.)		· · · · · · · · · · · · · · · · · · ·			
Floor Over Exterior	Cavity						
	Solid	(A.11.)		· · · · · · · · · · · · · · · · · · ·			
Crawl Space Wall		(A.12.)	********				
Slab On Grade	********	(A.13.)Lin. ft.		F-Value			
Total Calculated Envelope Heat Loss Factor- Not to exceed Total Code Allowed Heat Loss Factor of line 10 of Section C. (Enter here: )by more than 1%							

\*\* Adjustment Factors For Wood-Framed Components - Do not apply if your are using a pre-calculated or default U-Value.

Spacing Of Framing	Stud V	Valls	Joists/Rafters		
Members	Cavity	Solid	Cavity	Solid	
12"	.70	.30	.86	.14	
16°	.75	.25	.90	.10	
24"	.78	.22	.93	.07	

F. Heat Loss Factor Due to Air Infiltration (for heating equipment sizing)

Enter appropriate values. A maximum infiltration air change rate of 0.5 per hour is allowed in addition to ventilation losses.

Floor Level	Area (sq ft)	× Height (ft)	Fan Capacity (cfm)	× Constant	× Air Changes Per Hour	= Heat Loss Factor(UA)		
Basement				.018				
Level 1		+		.018				
Level 2				.018	· .			
Level 3				.018				
Ventilation				.432				
Total Infiltration & Ventilation Heat Loss Factor								

#### G. Heating Equipment Sizing

Enter appropriate value to determine the maximum and minimum allowable heating equipment capacity in BTUs/HR. A more detailed calculation may be submitted to the local code official. [4]

Prescriptive			
Package	×		
Method:	U overall from selected Prescriptive Total Envelope Area		
	Package of Section B (A.14.)		
OR System D	esign Method: Calculated Heat Loss Factor from Sect. E.		
Infiltration &	Ventilation Heat Loss Factor (from Sect. F.)	+	
Total Heat Los	s Factor (UA)	8	
Temperature L	Difference from County Zone Table on page 1	×	
	Minimum Heating Equipment Output	n	
Allowable Hea	ting Equipment Size Margin Multiplier		× 1.15
	Maximum Allowable Heating Equipment Output [5]	=	
Planned Furna	ce Output Or Boiler IBR Rating		
Make & Mode	l if High Efficiency Credit has been taken:		

Package	<b>Glazing Factor</b>	R wall	R ceiling	R basement	U door	U overail	<b>HVAC Equipment Efficiency</b>	
1	0.0370	R21	R42	R7	0.35	0.073	Normal	
2	0.0264	R21	R51, RT	R5	0.35	0.073	Normał	
3	0.0333	R15	R42	R10	0.35	0.073	Normal	
4	0.0440	R19	R33	R10	0.35	0.073	Normal	
5	0.0330	R13	R42	RH	0.35	0.073	Normal	
6	0.0480	R19	R33	R11	0.35	0.073	Normal	
7	0.0600	R21	R47	RIL	0.35	0.073	Normal	
8	0.0407	R13	R44	R13	0.35	0.073	Normal	
9	0.0600	R19	R42	R13	0.35	0.073	Normal	
10	0.0680	R21	R38, RT	R13	0.35	0.073	Normal	
11	0.0296	R13	R49	R5	0.35	0.086	High	
12	0.0440	R19	R30	R5	0.35	0.086	High	
13	0,0520	R21	R33	R5	0.35	0.086	High	
14	0.0720	R13	R47	R10	0.35	0.086	High	
15	0.0784	R19	R38	R10	0.47	0.086	High	
16	0.0640	RI3	R33	RII	0.47	0.086	High	
17	0.0896	RI9	R49	RII	0.35	0.086	High	
18	0.0896	R21	R34	R11	0.35	0,086	High	
19	0.0920	R19	R34	RII	0.47	0.086	High	
20	0.0840	<b>R</b> 13	R49	R13	0.35	0.086	High	
21	0.0840	R19	R30	· R13	0.47	0.086	High	
22	0.0896	R21	R31	R13	0.47	0.086	High	
Package	<b>Glazing Factor</b>	R wall	R celling	R crawl	U door	Uoverall	HVAC Equipment Efficiency	
23	0.0520	R19	R34	R19	0.47	0.070	Normal	
24	0.0672	R13	R36	R19	0,47	0.083	High	
25	0.0720	R13	R33	R19	0.47	0.083	High	
Package	<b>Glazing Factor</b>	R wall	R ceiling	R slab	U door	U overall	HVAC Equipment Efficiency	
26	0,0560	R21	R36	R5	0.47	0.103	Normal	
27	0.0728	R13	R36	R5	0,47	0.121	High	
28	0.0760	R13	R34	R5	0.47	0.121	High	
Package	Glazing Factor	R wall	R ceiling	R heated-slab	U door	U overall	HVAC Equipment Efficiency	
29	0.0560	R21	R47	R5	0.47	0.101	Normal	
30	0.0728	R13	R42	R5	0.47	0.120	High	
31	0.0760	R13	R38	R5	0.47	0,120	High	
Package	Giazing Factor	R wall	R ceiling	R floor	U door	Uoverall	HVAC Equipment Efficiency	
32	0.0480	R19	R47	R19	0.35	0.065	Normal	
33	0.0728	R19	R36	R19	0.47	0.077	High	
34	0.0560	R13	R34	R19	0.47	0.077	High	

Prescriptive Package Tables (Corrected) (See notes on page 2 of Energy Worksheet: i = insulating sheathing, RT = raised heel roof truss) Table B-1 Prescriptive packages, Non-electric Heat, Structural Sheathing only

Table B-2 Prescriptive packages, Non-electric Heat, Insulating Sheathing

Package	Glazing Factor	R wall	R ceiling	R basement	nt Udoor Uoverall		HVAC Equipment Efficiency	
35	0.0370	R20, 1	R42	R7	0.35	0.073	Normal	
36	0.0363	R28, I	R38, RT	R5	0.35	0.073	Normal	
37	0.0552	R18, I	R44	RIO	0.35	0.073	Normat	
38	0.0560	R20, J	R47	R10	0.35	0.073	Normal	
39	0.0560	R23, I	R34	R10	0.35	0.073	Normal	
40	0.0560	R18, I	R47	R11	0.35	0.073	Normal	
41	0.0616	R23, I	R42	R11	0.35	0.073	Normal	
42	0.0546	R18, 1	R44	R11	0.35	0.073	Normał	
43	0.0672	R23, I	R40	R13	0.35	0.073	Normal	
44	0.0720	R25, 1	R36	R13	0.35	0.073	Normai	
45	0.0504	R18, 1	R40	R5	0.35	0.086	High	
46	0.0560	R19, I	R47	RS	0.35	0.086	High	
47	0.0560	R23, 1	R 38	R5	0.47	0.086	High	
48	0.0600	R25, 1	R38	R5	0.47	0.086	High	
49	0.0680	R26, I	R42	R5	0.35	0.086	High	
50	0.0680	R28, I	R47	R5	0.47	0.086	High	
51	0.0672	R26, I	R47	R5	0.35	0.086	High	
52	0.0672	R28, I	R38	R5	0.35	0.086	High	
53	0.0720	R20, 1	R42	R7	0.47	0.086	High	
54	0.0855	R18.1	R36	R11	0.35	0.086	High	

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		_					
55	0.0896	R23, 1	R33	R11	0.47	0.086	High
56	0.0861	R 18, 1	R36	RI3	0.47	0.086	Hìgh
57	0.1000	R23, 1	R33	R13	0.47	0.086	High
Package	<b>Glazing Factor</b>	R wall	R ceiling	R crawl	U door	U overall	HVAC Equipment Efficiency.
58	0.0546	R18,1	R38	R19	0.47	0.070	Normal
59	0.0784	R15,1	R30	R19	0.47	0.083	High
60	0.0880	R15,1	R38	R19	0.47	0.083	High
Package	<b>Glazing Factor</b>	R wall	R ceiling	R slab	U door	U overall	HVAC Equipment Efficiency
61	0.0640	R23, I	R36	R5	0.47	0.103	Normal
62	0.0896	R15,1	R36	R5	0.47	0.121	: High
63	0.0960	R15, I	R38	R5	0.47	0.121	High
Package	<b>Glazing Factor</b>	R wall	R ceiling	R heated-slab	U door	U overall	HVAC Equipment Efficiency
64	0.0640	R23.1	R34	R5	0.47	0.101	Normai
65	0.0840	R15, I	R31	R5	0.47	0.121	High
66	0.0920	R15,1	R33	R5	0.47	0.121	High
Package	<b>Glazing Factor</b>	R wall	R ceiling	R floor	U door	U overali	HVAC Equipment Efficiency
67	0.0480	R20, 1	R44	R19	0.35	0.065	Normai
68	0.0728	R20, I	R36	R19	0.47	0.077	High
69	0,0560	R14, 1	R38	R19	0.47	0.078	High

Table B-3 Prescriptive packages, Electric Heat, Structural Sheathing Only

Package	Glazing Factor	R wall	R ceiling	R basement	U door	U overall	<b>HVAC</b> Equipment Efficiency
E 70	0.0396	R21	R37, RT	R19	0.35	0.059	Normal
E 71	0.0429	R21	R42, RT	R19	0.35	0.059	Normal
E 72	0.0520	R21	R49	R13	0.35	0.068	High
E 73	0.0640	R19	R42, RT	R19	0.35	0.068	High
E 74	0.0693	R21	R49, RT	R19	0.47	0,068	High
Package	<b>Glazing Factor</b>	R wall	R ceiling	R crawl	U door	U overail	HVAC Equipment Efficiency
E 75	0.0429	R21	R54, RT	R30	0.35	0.054	Normal
E 76	0.0480	R21	R45, RT	R19	0.35	0.062	High
E 77	0.0627	R21	R54, RT	R30	0.47	0.062	High
Package	<b>Glazing Factor</b>	R wall	R ceiling	R slab	U door	U overall	HVAC Equipment Efficiency
E 78	0.0396	R26	R51, RT	R10	0.35	0.083	Normal
E 79	0.0480	R21	R49	R7	0.35	0.095	High
E 80	0.0528	R21	R49, RT	R5	0.35	0.095	High
Package	<b>Glazing Factor</b>	R wall	R ceiling	R floor	U door	U overall	HVAC Equipment Efficiency
E 81	0.0363	R21	R54, RT	R30	0,35	0.052	Normal
E 82	0.0520	R21	R49	R30	0.35	0.060	High
E 83	0.0528	R21	R44, RT	R30	0.47	0.060	High

1.11		Table B-4 Prescriptive packages, Electric Heat, Insulating Sheathing							
Package	Glazing Factor	R wall	R ceiling	R basement	U door	U overall	HVAC Equipment Efficiency		
E 84	0.0480	R25,1	R48, RT	R16	0.35	0.059	Normal		
E 85	0.0495	R25, 1	R48, RT	R16	0.35	0.059	Normał		
E 86	0.0462	R28, 1	R40	R16	0.35	0.059	Normal		
E 87	0.0429	R25, I	R36	R18	0.35	0.059	Normal		
E 88	0.0528	R23.1	R58, RT	R18	0.35	0.059	Normal		
E 89	0.0462	R25, I	R42	R18	0.35	0.059	Normal		
E 90	0.0560	R25, 1	R46, RT	RIO	0.35	0,068	High		
E 91	0.0640	R23, 1	R48, RT	R13	0.35	0.068	High		
Ē 92	0.0600	R25,1	R42	R13	0.35	0.068	' High		
E 93	0.0600	R23,1	R37	R18	0.47	0.068	High		
E 94	0.0759	R25, I	R46, RT	R18	0.47	0.068	High		
Package	Glazing Factor	R wall	R ceiling	R crawl	U door	U overall	HVAC Equipment Efficiency		
E 95	0.0429	R25,1	R48, RT	R23	0,35	0.054	Normal		
E 96	0.0520	R23, I	R38	R23	0.35	0.062	High		
E 97	0.0561	R25, I	R44	R23	0.47	0.062	High		
Package	Glazing Factor	R wall	R ceiling	R slab	U door	U overall	HVAC Equipment Efficiency		
E 98	0.0396	R25.1	R48, RT	RIO	0.35	0.083	Normal		
E 99	0.0560	R23, 1	R44	R7	0,35	0.095	High		
E 100	0.0594	R25, I	R46, RT	R5	0.47	0.095	High		
Package	Glazing Factor	R wall	R celling	R floor	U door	U overall	HVAC Equipment Efficiency		
E 101	0.0429	R25, 1	R46, RT	R30	0.35	0.052	Normal		

R30

R30

0.35

0.47

0.060

0.060

0.0560

0.0627

R23, I

R25, 1

R44

R44, RT

E 102

E 103

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High

High

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# Default Assembly R and U Value Tables

(All U-values include framing factors, finish materials and air films.)

Insulation	Standard	Raised	Insulation	Standard	Raised
K-value	Iruss	Truss(0)	R-vame	Iruss	Truss
	U-Value	U-Value	~	U-Value	U-Value
R-0	0.568	0.568	R-33	0.033	0.029
R–7	0.119	0.119	R-34	0.032	0.028
R-8	0.108	0.108	R-35	0.032	0.028
R-9	0.098	0.098	R-36	0.031	0.027
R-10	0.089	0.089	R-37	0.031	0.026
R-11	0.082	0.082	R-38	0.030	0.025
R–12	0.076	0.076	R-39	0.030	0.025
R-13	0.070	0.070	R-40	0.029	0.024
R-14	0.066	0.066	R-41	0.029	0.024
R-15	0.062	0.061	R-42	0.028	0.023
R-16	0.059	0.058	R-43	0.028	0.023
R–17	0.056	0.055	R-44	0.027	0.022
R-18	0.053	0.052	R-45	0.027	0.022
R–19	0.051	0.049	R-46	0.027	0.021
R-20	0.048	0.047	R-47	0.026	0.021
R-21	0.047	0.045	R-48	0.026	0.020
R-22	0.045	0.043	R-49	0.026	0.020
R-23	0.043	0.041	R-50	0.026	0.020
R–24	0.042	0.040	R51	0.025	0.019
R–25	0.040	0.038	R-52	0.025	0.019
R26	0.039	0.037	R-53	0.025	0.019
R-27	0.038	0.035	R54	0.025	0.018
R-28	0.037	0.034	R-55	0.024	0.018
R-29	0.036	0.033	R-56	0.024	0.018
R-30	0.035	0.032	R-57	0.024	0.018
R-31	0.034	0.031	R-58	0.024	0.017
R-32	0.034	0.030	R-59	0.024	0.017

Table 1. Ceiling U-Values<sup>(a)</sup>

(a) R-values represent the sum of the ceiling cavity insulation plus the R-value of insulating sheathing (if used). For example, R-19 cavity insulation plus R-2 sheathing is reported as R-21 ceiling insulation. For ventilated ceilings, insulating sheathing must be placed between the conditioned space and the ventilated portion of the roof (typically applied to the trusses or rafters immediately behind the drywall or other ceiling finish material).

(b) To receive credit for a raised truss, the insulation must achieve its full insulation thickness over the exterior walls.

Insulation R–Value <sup>(c)</sup>	16in. O.C. Wall UValue	24-in. O.C. Wall U-Value		
R-0	0.238	0,241		
R7	0.105	0.104		
R-8	0.099	0.097		
R–9	0.094	0.092		
R-10	0.090	0.088		
R-11	0.089	0.087		
R-12	0.085	0.083		
R-13	0.082	0.080		
R-14	0.079	0.077		
R15	0.077	0.074		
R-16	0.066	0.064		
R-17	0.064	0.062		
R18	0.062	0.060		
R-19	0.060	0.059		
R-20	0.059	0.057		
R-21	0.057	0.056		
R-22	0.056	0.054		
R23	0.055	0.053		
R-24	0.054	0.052		
R-25	0.053	0.051		
R-26	0.052	0.050		
R–27	0.051	0.049		
R-28	0.050	0.048		

Table 2. Wood–Frame Wall U–Values<sup>(a,b)</sup>

(a) U-values are for uncompressed insulation.

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(b) U-values in this Table were developed for wood-frame walls, but the 16-in. O.C. Wall U-Value column can also be used for above-grade concrete, masonry, and log walls. Mass wall R-value to U-value conversion tables are planned for future versions of the MECcheck Manual. TM

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(c) Wall R-values are the sum of the cavity insulation plus insulating sheathing (if used).

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Table 3. 16-in. O.C. Metal-Frame Wall U-Values and Equivalent Prescriptive Package Wall R-Values (Use the U-values below for the System Design Method of the Energy Worksheet. Use the equivalent R-value below to choose an Energy Worksheet Prescriptive Package with a wall R-value that is less than or equal to it. If you have an equivalent R-value without an "I" listed after it, then you must use a Package wall R-value without an "I" listed after it, then you must use a Package wall R-value without an "I" designation.)

Covity	Insulating Sheething D. Value										27. E. 1. B
Cavity				1	ពទធាឧណារ្ទ	g Sheathi	ng K-va	iue			
R–Value											
	R-0	R-1	R-2	R-3	R-4	R-5	R-6	R–7	R-8	R–9	<b>R-10</b>
R-0	U-0.270	U-0.258	U-0.205	U-0.170	U-0.146	U-0.127	U-0.113	U-0.101	U-0.092	U-0.084	U-0.078
R–11	U-0.120	U-0.118	U-0.106	U-0.096	U-0.087	U0.080	U–0.074 R15	U-0.069 R151	U-0.065 R16I	U-0.061 R18I	U-0.057 R20I
R-13	U-0.114	U-0.111	U0.100	U-0.091	U-0.084	U0.077 R15	U-0.072 R15	U-0.067 R15I	U-0.063 R17I	U-0.059 R19I	U-0.056 R22I
R-15	Ü-0.109	U-0.107	U-0.096	U-0.088	U~0.081	U0.075 R15	U-0.070 R15	U-0.065 R16I	U-0.061 R18I	U-0.058 R19I	U-0.054 R22I
R–19	U-0.101	U-0.099	U-0.090	U-0.083	U0.077 R15	U-0.071 R15	U-0.066 R15I	U-0.062 R17I	U-0.059 R191	U-0.055 R20I	U-0.052 R22I
R-21	U-0.098	U-0.096	U-0.088	U-0.081 R13	U-0.075 R15	U-0.070 R15	U-0.065 R16I	U-0.061 R18I	U-0.058 R19I	U-0.054 R20I	U-0.052 R22I
R-25	U-0.094	U-0.093	U0.085	U–0.078 R13	U-0.073 R15	U-0.068 R15I	U-0.063 R17I	U0.060 R19I	U-0.056 R20I	U-0.053 R20I	U-0.051 R23I

Table 4. 24-in. O.C. Metal-Frame Wall U-Values and Equivalent Prescriptive Package Wall R-Values (Use the U-values below for the System Design Method of the Energy Worksheet. Use the equivalent R-value below to choose an Energy Worksheet Prescriptive Package with a wall R-value that is less than or equal to it. If you have an equivalent R-value without an "I" listed after it, then you must use a Package wall R-value without an "I" listed after it, then you must use a Package wall R-value without an "I" designation.)

Cavity		Insulating Sheathing R-Value									
R–Value											
	R-0	R-1	R-2	R3	R4	R5	R6	R–7	R-8	R-9	<b>R-10</b>
R-0	U-0.270	U-0.258	U-0.205	U-0.170	U-0.146	U–0.127	U-0.113	U-0.101	U-0.092	U0.084	U-0.078 R13
R-11	U-0.106	U-0.104	U-0.095	U-0.086	U-0.080 R13	U-0.074 R15	U-0.069 R15I	U-0.064 R17I	U-0.060 R18I	U-0.057 R20I	U-0.054 R20I
R-13	U-0.100	U-0.098	U-0.090	U-0.082 R13	U-0.076 R15	U–0.071 R15	U0.066 R15I	U-0.062 R17I	U-0.058 R19I	U-0.055 R20I	U-0.052 R22I
R–15	U-0.094	U-0.093	U-0.085	U–0.078 R13	U-0.073 R15	U-0.068 R151	U-0.063 R17I	U-0.060 R19I	U0.056 R20I	U-0.053 R20I	U-0.051 R23I
R–19	U-0.088	U-0.086	U-0.080 R13	Ŭ0.074 R15	U~0.069 R15I	U-0.064 R17I	U-0.060 R191	U-0.057 R20I	U-0.054 R20I	U-0.051 R23I	U-0.049 R241
R-21	U-0.085	U-0.084	U~0.077 R15	U–0.072 R15	U–0.067 R15I	U-0.063 R17I	U-0.059 R19I	U-0.056 R20I	U-0.053 R20I	U0.050 R23I	U-0.048 R24I
R-25	U-0.081 R13	U-0.080 R13	U0.074 R15	U-0.069 R15	U-0.064 R17I	U-0.060 R19I	U-0.057 R20I	U-0.054 R201	U-0.051 R23I	U-0.049 R23I	U-0.046 R241

Insulation R–Value	Floor U-Value
R-0	0.249
R-7	0.096
R-11	0.072
R-13	0.064
R-15	0.057
R-19	0.047
R-21	0.044
R-26	0.037
R-30	0.033

### Table 5. Floor U-Values

# Table 6. Basement U-Values<sup>(a)</sup>

Insulation R–Value	Basement Wall U–Value	Insulation R–Value	Basement Wall U–Value
R-0	0.360	R-10	0.072
R-1	0.244	R11	0.067
R-2	0.188	R-12	0.062
R-3	0.155	R-13	0.059
R-4	0.132	R-14	0.055
R5	0.115	R-15	0.052
R6	0.102	R-16	0.050
R-7	0.092	R-17	0.047
R-8	0.084	R-18	0.045
R-9	0.077	R–19	0.043
		R-20	0.041

(a) Insulation R-values represent the sum of exterior and/or interior insulation. Basement walls must be insulated from the top of the basement wall to 10 ft below ground level or to the floor of the basement, whichever is less.

Perimeter Insulation R-Value	Slab F-Value			
······································	24-in. Insulation Depth	48-in. Insulation Depth		
R-0	1.04	1.04		
R-1	0.91	0.89		
R-2	0.86	0.83		
R-3	0.83	0.79		
R-4	0.82	0.76		
R-5	0.80	0.74		
R6	0.79	0.73		
R-7	0.79	0.71		
R8	0.78	0.70		
R-9	0.77	0.69		
R-10	0.77	0.68		
R-11		0.68		
R-12		0.67		
R-13		0.66		
		0.66		
R-15		0.65		
R-16		0.65		
R-17		0.65		
R-18		0.64		
R-19		0.64		
R–20		0.64		

Table 7. Slab F--Values

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Insulation R-Value	Crawl Space Wall U-Value
R-0	0.477
R-1	0.313
R-2	0.235
R-3	0.189
R-4	0.158
R-5	0.136
R-6	0.120
R-7	0.107
R-8	0.096
R-9	0.088
R-10	0.081
R-11	0.075
R-12	0.069
R-13	0.065
R-14	0.061
R-15	0.057
R–16	0.054
R–17	0.051
R-18	0.049
R-19	0.047
R-20	0.045

# Table 8. Crawl Space Wall U-Values

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Frame/Glazing Features	Single Pane	Double Pane				
Metal Without Thermal Break	Metal Without Thermal Break					
Operable	1.27	0.87				
Fixed	1.13	0.69				
Garden Window	2.60	1.81				
Curtain Wall	1.22	0.79				
Door	1.26	0.80				
Skylight	1.98	1.31				
Site-Assembled Skylight	1.36	0.82				
Metal With Thermal Break						
Operable	1.08	0.65				
Fixed	1.07	0.63				
Curtain Wall	1.11	0.68				
Door	1.10	0.66				
Skylight	1.89	1.11				
Site-Assembled Skylight	1.25	0.70				
Reinforced Vinyl or Metal-Clad Wood						
Operable	0.90	0.57				
Fixed	0.98	0.56				
Door	0.99	0.57				
Skylight	1.75	1.05				
Wood/Vinyl/Fiberglass						
Operable	0.89	0.55				
Fixed	0.98	0.56				
Garden Window	2.31	1.61				
Door	0.98	0.56				
Skylight	1.47	0.84				
Glass Block Assemblies		0.60				

Table 9. U--Values for Windows, Glazed Doors, and Skylights<sup>(a)</sup>

(a) The U-values in these tables can be used in the absence of test U-values. The product cannot receive credit for a feature that cannot be clearly detected. Where a composite of materials from two different product types is used, the product must be assigned the higher U-value.

Table 10.	U-Value	Table for	Non-Glazed	Doors <sup>(a)</sup>

Steel Doors	,	
Without Foam Core	0.6	0
With Foam Core	0.3	5
Wood Doors	Without Storm	With Storm
Panel With 7/16-in. Panels	0.54	0.36
Hollow Core Flush	0.46	0.32
Panel With 1–1/8–in. Panels	0.39	0.28
Solid Core Flush	0.40	0.26

(a) The U-values in these tables can be used in the absence of test U-values. The product cannot receive credit for a feature that cannot be clearly detected. Where a composite of materials from two different product types is used, the product must be assigned the higher U-value.

# DEPARTMENT OF COMMERCE

			Resistance (R)		
•	Description	Density, lb/ft <sup>3</sup>	Per Inch Thickness °F . ft <sup>2</sup> . h	For Thickness Listed	
	SHEATHING				
	Gypsum or plaster board	50	·	0.45	
	Gypsum or plaster board	50		0.56	· · · ·
	Plywood (Douglas Fir) 1/2".	34		0.62	<i>1</i>
	Plywood (Douglas Fir)	34	<u> </u>	0.77	
	Plywood or wood panels	34	_	0.93	14
	Vegetable fiber board				
	Sheathing, regular density	18	_	1.32	
	Hardboard				· 3
	Medium density	50	1.37	·	
	Particleboard				
	Medium density	50	1.06	_	
	FINISH FLOORING MATERIALS				<del>-</del> /
	Carriet and rubber pad	_	_	1 23	
	INSULATING MATERIALS		······.		
	Righter and Batt				1.1
	Mineral filter filtroue form processed from rock slag, or glass				
	anneral noci, norods form processed from rock, stag, or glass	04.20		11	f
	approx. 3-4 m	0.4-2.0		11	
	approx. 5.5 m	10.4-2.0	— .	15	
	approx. 5.5 in	1.2-1.0		15	
	approx. 5.5–6.5 in.	0.4-2.0		19	
	approx, 5.5 in.	0.6-1.0		21	
	approx. 6–7.5 in.	0.4–2.0	—	22	
	approx. 8.25–10 in.	0.4-2.0		30	
	approx. 10–13 in	0.4-2.0	6 <b>2</b>	38	
	Board and Slabs				
	Glass fiber, organic bonded	4.0-9.0	4.00	—	·
	Expanded polystyrene, extruded (smooth skin surface)	1.83.5	5.00	_	
	Expanded polystyrene, molded beads	1.0	3.85	;	
		1.25	4.00		
		1.5	4.17	_	
		1.75	4.17	41 <sup>4</sup> 1114	
		2.0	4.35		
	Cellular polyurethane/polyisocyanurate	1.5	6.25-5.56		
	Cellular polyisocyanurate (CFC-11 exp.) (gas-impermeable facers)	2.0	7.04		
	Mineral fiberboard, wet felted				
	Acoustical tile	18.0	2.86	—	
	Loose Fill				
	Cellulosic insulation (milled paper or wood pulp)	2.3-3.2	3.70-3.13		
	Perlite, expanded	2.0-4.1	3.7-3.3	<u> </u>	
		4.1–7.4	3.3-2.8	—	
		7.4-11.0	2.8-2.4	—	
	Mineral fiber (rock, slag, or glass)				
	approx. 3.755 in,	0.6-2.0	_	11.0	
	арргох. 6.5–8,75 in.	0.6-2.0	_	19.0	
	approx. 7.5–10 in.	0.6-2.0	_	22.0	
	approx. 10.25–13.75 in.	0.6-2.0	_	30.0	
	Mineral fiber (rock, slag, or glass)				
	approx. 3.5 in. (closed sidewall application)	2.0-3.5	_	12 0-14 0	
	Vermiculite exfoliated	70-9.9	212	12.0-14.0	
	formatume, on the first of the	40-60	2.13		
	Spray Amiliad	4.0-0.0	<i>4.61</i>		
	opras pipinta Dalaurathana foom :	15 36	675 551		
	i viyuvuuniiv ivalli Uraafarmaldabuda faam	1.3-2.3	0.23-3.30	_	
	Callulasia fibas	0./-1.0	4.33-3.3/		
		3.3-6.0	3.45-2.94		
	GIASS HDEF	3.3-4.5	3.85-3.70	*****	

# Typical Thermal Properties of Building Materials-Design Values<sup>a</sup>

ROOFING				
Asphalt shingles	70	_	0.44	
PLASTERING MATERIALS				_
Cement plaster, sand aggregate	116	0.20	_	
0.75 in.	_		0.15	
MASONRY MATERIALS				_
Masonry Units				
Brick, fired clay	150	0.12-0.10	<u> </u>	
Concrete blocks				
Normal weight aggregate (sand and gravel)				
8 in., 33-36 lb, 126–136 lb/ft <sup>3</sup> concrete, 2 or 3 cores	_	_	1.11-0.97	
Same with perlite filled cores	—	_	2.0	
Same with vermiculite filled cores	_	—	1.92-1.37	
12 in., 50 lb, 125 lb/ft <sup>3</sup> concrete, 2 cores			1.23	
Concretes				
Sand and gravel or stone aggregate concretes	150	0.10	_	a e t
SIDING MATERIALS (on flat surface)				-
Siding				
Asphalt roll siding			0.15	4
Hardboard siding, 7/16"		<u> </u>	0.67	
Wood, drop, 1 by 8 in.	_	_	0.79	
Aluminum, steel, or vinyl, over sheathing				
Hollow-backed	. <b></b> -	_	0.61	
Insulating-board backed nominal 3/8"	_	_	1.82	
Insulating-board backed nominal 3/8", foil backed	—	_	2.96	
WOOD				
Maples, oak and similar materials	45	0.91		
Fir, pine and similar materials	32	1.25		
3/4''	32	0.94		
1–1/2"	32	1.9		
3-1/2"	32	4.4		
5–1/2"	32	6.9		
7–1/4"	32	9.1		
91/4"	32	11.6		
11-1/4"	32	14.1		

<sup>a</sup>Values are for a mean temperature of 75°F. Representative values for dry materials are intended as design (not specification) values for materials in normal use. Thermal values of insulating materials may differ from design values depending on their in-situ properties (e.g., density and moisture content, orientation, etc.) and variability experienced during manufacture. For properties of a particular product, use the value supplied by the manufacturer or by unbiased tests in accordance with s. Comm 22.31.

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s. Comm 22.26 Slab-On-Grade Insulation Details

Insulation shall extend vertically and horizontally for a total of 48". In all cases the insulation shall insulate to the top edge of the floor perimeter. The last diagram is not an acceptable method.

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