

Inception of Regular Valley Spacing in Fluvial Landscapes: A Linear Stability Analysis

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Abstract

Incipient channelization in mountainous landscapes is often associated with the presence of first-order valleys at a regular wavelength under diverse hydroclimatic forcings. Here we provide a formal linear stability analysis of a landscape evolution model in detachment-limited erosion conditions to quantify the impact of the erosion law on the regular valley formation. The linear stability analysis is conducted for the unchannelized hillslope solutions along a long mountain ridge, where the perturbed equations constitute a third-order differential eigenvalue problem. The solutions to the posed eigenvalue problem are obtained by a spectral Galerkin technique with numerical quadrature. Results reveal the dependence of the erosion threshold and the emergent ridge/valley wavelength on the exponents in the power-law scaling coupling fluvial erosion with specific drainage area (m) and local slope (n). As the exponent m increases for a fixed n , the emergent valley spacing expands and the erosion limit for the first channel instability declines. Conversely, the erosion threshold for the first channelization rises with an increase in n at a particular value of m . We also show that predictions of the stability analysis conform with numerical simulations for different degrees of nonlinearity in the erosion mechanism and agree well with topographic data of a natural landscape.

1 **Inception of Regular Valley Spacing in Fluvial**
2 **Landscapes: A Linear Stability Analysis**

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9 **Key Points:**

- 10 • The linear stability analysis quantifies the critical erosion limit for incipient chan-
11 nelization.
- 12 • Incipient valley spacing widens for higher values of the exponent that couples flu-
13 vial erosion and the specific drainage area.
- 14 • Results from the stability analysis conform with the numerical simulations and
15 data from a natural landscape.

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16 **Abstract**

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 19 Here we provide a formal linear stability analysis of a landscape evolution model in detachment-
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 28 spacing expands and the erosion limit for the first channel instability declines. Con-
 29 versely, the erosion threshold for the first channelization rises with an increase in n at
 30 a particular value of m . We also show that predictions of the stability analysis conform
 31 with numerical simulations for different degrees of nonlinearity in the erosion mechanism
 32 and agree well with topographic data of a natural landscape.

33 **Plain Language Summary**

34 Landscapes tend to exhibit equally spaced valleys at the onset of channelization,
 35 which occurs when the fluvial erosion overcomes the smoothing effects of the hillslope
 36 processes. To theoretically predict the conditions for the first channelization, we study
 37 the growth of very small disturbances added to the landscape forms with no channels.
 38 The results indicate a minimum erosion limit below which no valleys are present. This
 39 critical erosion limit and the emergent valley spacing are determined by the relation be-
 40 tween the specific upstream area and the topographic slope in fluvial erosion law. The
 41 theoretical findings are in good agreement with the numerical simulations and the to-
 42 pographic data from a natural landscape.

43 **1 Introduction**

44 The relative strength of diffusive soil creep and fluvial erosion leads to a distinc-
 45 tive spatial arrangement of interlocked ridges and valleys (Kirkby, 1971; Willgoose et al.,
 46 1991; Rodriguez-Iturbe & Rinaldo, 2001; Birnir et al., 2001; Roering, 2008; Hancock et
 47 al., 2010; Fowler, 2011; Singh et al., 2015; Bonetti et al., 2020). A crucial juncture of this
 48 balance controlling hillslope morphology occurs when erosion is just high enough to over-
 49 come the effect of soil creep and starts carving the surface, thereby leading to the for-
 50 mation of first-order valleys in the landscape. Historically, the presence of regularly-spaced
 51 valleys along mountainous ridges has stimulated efforts to understand the emergence of
 52 such a deterministic behavior of the channelization onset (Gilbert & Dutton, 1880; Shaler,
 53 1899; Hallet, 1990; Talling et al., 1997; Parker & Izumi, 2000; Allen, 2005; Perron, Kirchner,
 54 & Dietrich, 2008).

55 The results from topographic observations in mountainous landscapes with distinct
 56 vegetation cover and climate conditions indicate that the channel initiation tends to oc-
 57 cur at a characteristic spatial scale (Perron, Dietrich, & Kirchner, 2008; Perron et al.,
 58 2009). Even for well-developed channelization regimes with several length scales, in the
 59 power spectrum of the landscape elevation a ‘typical’ wavenumber demarcates around
 60 which most of the energy content is concentrated with a sharp (and power-law) decline
 61 in the energy at high wavenumbers (Hooshyar et al., 2021; Porporato, 2022). These ob-
 62 servations lead to an interesting set of questions regarding the role of the feedback in landscape-
 63 evolution processes in determining the emerging channelization mode as well as the in-
 64 tensity of fluvial erosion needed for the first dissection of the landscape at that scale. A

65 quantitative link between this spectral signature of channelization and the form of the
 66 erosion laws has not yet been completely established. The present work offers a contri-
 67 bution toward this goal. We focus here on a minimalist landscape evolution model (LEM)
 68 (Bonetti et al., 2020) that contains the least amount of complexity to describe the regularly-
 69 spaced channel initiation and pinpoint the corresponding nonlinear feedbacks that in-
 70 duce this phenomenon over geological time scales. While comprehensive landscape evo-
 71 lution modeling studies (e.g., Collins et al. (2004); Van De Wiel et al. (2007); Attal et
 72 al. (2008); Coulthard et al. (2013)), including spatiotemporal heterogeneity and parametriza-
 73 tion for a wide array of geomorphological processes, are a powerful tool to provide the
 74 linkages between distinct processes and the consequent morphological evolution, they tend
 75 to be too involved to allow for theoretical developments that can help isolate the under-
 76 lying basic mechanisms driving the emergence of ubiquitous landscape patterns.

77 1.1 Brief Literature Review

78 Before starting our review of the investigations of landscape stability, it is useful
 79 to orient the reader on the extensive literature on channel formation in natural landscapes
 80 and how different formulations vary in their description of the coupled water and sed-
 81 iment dynamics. Regarding surface water modeling, the more comprehensive approaches
 82 adopt the full-version of shallow-water equations primarily in steady-state conditions,
 83 thereby including the effect of gravity, pressure, and inertial forces on the surface wa-
 84 ter flow (Chen et al., 2014). The next category of approximation dismisses the inertial
 85 effects over long time scales (Leopold & Maddock, 1953; Weinmann & Laurenson, 1979;
 86 Smith, 2010). The minimalist form of water transport assumes a steady-state flow along
 87 the topographic gradient or normal-flow hypothesis. Efforts by Gallant and Hutchinson
 88 (2011), Bonetti et al. (2018) and Porporato (2022) established that this water transport
 89 formalism is analogous to the mathematical equation of the specific drainage area, a , for
 90 the constant flow speed of the water.

91 Regarding the modeling of long-term fluvial erosion processes, LEMs are typically
 92 considered either in the transport-limited (TL) or the detachment-limited (DL) condi-
 93 tions. Some research works have also considered intermediate conditions between these
 94 two regimes (Davy & Lague, 2009; Pelletier, 2012). Under the TL approximation, the
 95 fluvial erosion assumes the form of the divergence of the sediment flux, which in turn,
 96 is related to the shear stress of the surface flow (Willgoose et al., 1991; G. E. Tucker &
 97 Bras, 1998; Hergarten, 2020). The erosion flux is directly related to the shear stress by
 98 flowing water in the DL approximation with the underlying assumption that the surface
 99 resistance to incision is the restricting factor for the erosion rate rather than the haul-
 100 ing capacity of the flow to transport the eroded material (Ahnert, 1987; Howard, 1994).
 101 Hence, the mathematical form of fluvial erosion becomes a sink term in the LEM with
 102 a power-law relation to the specific drainage area and local slope.

103 Within this context, the pioneering work by Smith and Bretherton (1972) provided
 104 the first stability analysis of symmetric hillslopes to small lateral perturbations employ-
 105 ing a continuous model for water under normal-flow hypothesis and sediment fluxes for
 106 TL erosion conditions. This study showed that the concave-up portions of the hillslope
 107 are unstable to lateral perturbations. Nevertheless, the analysis did not predict a char-
 108 acteristic wavelength for the channel instability with an unbound increase in the growth
 109 rate for high-frequency perturbations. Whereas this shortcoming has been attributed to
 110 the use of normal-flow approximation for the water continuity equation (Loewenherz-
 111 Lawrence, 1994; Smith, 2010; Fowler, 2011), here we show that the stability analysis con-
 112 sidering minimalist flow approximation with soil creep and DL erosion condition leads
 113 to a finite channelization mode at a critical threshold of the fluvial erosion. These find-
 114 ings suggest that the assumption of constant coefficients in the perturbed governing equa-
 115 tions in the work of Smith and Bretherton (1972) could be related to the lack of wave-
 116 length selection, as also noted by Fowler (2011).

117 Izumi and Parker (1995) and Izumi and Parker (2000) performed a linear stabil-
 118 ity analysis of the coupled system of shallow water flow in quasi-steady conditions with
 119 the DL approximation for the fluvial erosion. In particular, Izumi and Parker (1995) de-
 120 scribed channelization as the upstream-driven process over an assumed tilted planar sur-
 121 face, where the channels initiate as the surface discharge reaches a critical threshold. The
 122 case of downstream-driven erosion over a steady concave-down erodible surface was an-
 123 alyzed by Izumi and Parker (2000). They focused on deriving finite valley spacing for
 124 the channel initiation but considered constrained assumptions regarding perturbation
 125 structures and the flow boundary conditions. Additionally, Smith (2010) presented a de-
 126 tailed mathematical framework depicting the channel formation with the quasi-steady
 127 flow down the energy-surface gradient in the TL and DL erosion environments. The ini-
 128 tial hillslope was assumed to be a steady planar profile over which small perturbations
 129 could evolve.

130 All the previous theoretical contributions considered perturbations on somewhat
 131 artificial surfaces. The simple hillslope forms used in these studies facilitated analyti-
 132 cal tractability to determine the appearance of well-defined channels, but they are not
 133 necessarily steady-state solutions of LEMs and therefore have limited bearing to natu-
 134 ral landscape morphologies. In this regard, a more realistic starting point to investigate
 135 the conditions of valley formation was pursued by Perron, Dietrich, and Kirchner (2008)
 136 and Perron et al. (2009), who described the evenly-spaced valley formation for the nu-
 137 merical solutions of LEM under the DL fluvial erosion and drainage area field as a proxy
 138 for the water flux. Using numerical simulations, they showed that the relative timescale
 139 of fluvial erosion compared to soil creep controls the valley spacing scale. However, these
 140 analyses did not carry out a formal stability analysis and were limited to numerical sim-
 141 ulations.

142 Employing unchannelized solutions of LEMs with specific boundary conditions in
 143 a linear stability formulation would help to formulate precisely the criteria for the chan-
 144 nelization onset. A preliminary analysis along these lines was conducted by Bonetti et
 145 al. (2020) using a minimalist DL-LEM for the special case of unitary exponents of the
 146 drainage area and topographic slope. However, a more complete stability analysis that
 147 includes the effect of the nonlinear scaling exponents in the erosion on the channel for-
 148 mation for base-state solutions of the minimalist LEM is still missing and motivates the
 149 work here.

150 1.2 Goal of This Contribution

151 Within the context outlined before, in this paper we focus on a minimalist LEM
 152 in DL conditions and normal-flow approximation for the water flow. We conduct a lin-
 153 ear stability analysis of the unchannelized solutions of the governing equations to iden-
 154 tify the conditions under which an initially smooth surface assumes a morphology sim-
 155 ilar to observed regularly-spaced first-order drainage basins. The DL erosion model is
 156 adopted based on the arguments that the bed erosion for the first channelization over
 157 the hillslope and low-order valleys is bounded by the erosive power/shear stress of the
 158 overland flow rather than the flow capacity to transport the eroded sediments (Howard,
 159 1994; Izumi & Parker, 1995).

160 We consider two symmetric hillslopes along a linear ridgeline as an idealization of
 161 a long mountain ridge in a natural landscape (see Section 3.4) to derive unchannelized
 162 base-state solutions of the governing equations (Bonetti et al., 2019, 2020; Anand et al.,
 163 2020). Differently from previous contributions, the mathematical forms of the unchan-
 164 nelized solutions are obtained by applying the boundary conditions of water and sedi-
 165 ment fluxes in the governing equations and solving for the steady-state (the so-called base-
 166 state profile) rather than assuming an arbitrary initial form of the erodable surface. Since
 167 the solutions are analytically attainable only for m and n equal to 1 (see equation (12)),

168 we adopt a numerical procedure here to compute base-state hillslope profiles for generic
169 values of m and n .

170 The stability problem is solved by utilizing a spectral technique based on the Galerkin
171 projection with numerical quadrature (Canuto et al., 2006), which has been shown to
172 be particularly performant and well suited for morphological problems (Camporeale et
173 al., 2012; Camporeale & Ridolfi, 2012; Camporeale, 2015). Employing this strategy, the
174 impact of nonlinearities present in the erosion law on the hillslope stability and the in-
175 cipient channelization is discussed as erosion gets intensified with respect to soil creep.
176 The predictions of the stability analysis are compared with the numerical simulations
177 in a long rectangular domain for different values of the exponents and also with the to-
178 pographic data of a natural landscape. The obtained results show that the regularly-spaced
179 valleys emerge at a certain proportion of fluvial erosion and soil creep. From the water-
180 flow modeling perspective, our results also show that the minimalist normal-flow hypoth-
181 esis leads to a spatial wavelength preference on the channelization onset under the ac-
182 tion of DL erosion and soil creep.

183 The article is structured as follows. In section 2, we present the coupled govern-
184 ing equations of water and surface elevation in the DL framework, along with domain
185 geometry and boundary conditions used in this study. We further derive the linearized
186 perturbed equations that are recast in terms of a third-order differential eigenvalue prob-
187 lem. In section 3, the results from the linear stability analysis are discussed. The pre-
188 sented method is verified for the special case of unitary exponents in the model. We show
189 the control of the power-law exponents of the specific drainage area and slope in the ero-
190 sion term on the threshold erosion rate for first channelization and incipient valley spac-
191 ing. A comparison between stability analysis predictions and results from numerical sim-
192 ulations is carried out for different values of exponents m and n . We finally show the find-
193 ings of the stability analysis using the topographic data from a natural landscape.

194 2 Linear Stability Analysis

195 This section presents the mathematical equations for the minimalist LEM in DL
196 conditions for fluvial erosion. We define the unchannelized base-state solutions for two
197 symmetric and opposite hillslopes along a long ridge by imposing zero water and sed-
198 iment flux boundary conditions at the ridgeline and fixed-level boundary conditions at
199 the hillslope bases. The stability problem is posed by assuming weak perturbations over
200 the featureless base-state solutions. All arbitrary spatial perturbations are assumed to
201 have very small amplitude compared to the unchannelized solutions, so non-linear (higher-
202 order) interactions do not remain relevant in this regime. Employing normal-mode lin-
203 ear stability analysis, the perturbed governing equations in a linearized form are obtained
204 along with imposed homogeneous boundary conditions.

205 2.1 Governing Equations

206 The coupled dynamics of the landscape elevation and surface water fields can be
207 written in general as

$$208 \frac{\partial z}{\partial t} = U - \nabla \cdot \mathbf{f}_c - \nabla \cdot \mathbf{f}_e, \quad (1)$$

$$209 \frac{\partial h}{\partial t} = R - \nabla \cdot (q\mathbf{n}). \quad (2)$$

210 Equation (1) describes the temporal evolution of the elevation field z under the action
211 of tectonic uplift U , sediment flux due to soil creep \mathbf{f}_c , and the flux transported due to
212 fluvial erosion, \mathbf{f}_e . Soil creep is a term used to represent a combined effect of various bio-
213 physical processes that result in the slow movement of soil over the hillslope. Surface and
214 subsurface processes inducing this movement include animal burrowing, falling trees, wet-

215 ting/drying of the upper soil layer, and freezing/thawing cycle of the pore water in the
 216 subsurface (Carson & Kirkby, 1972; Gabet et al., 2003). The combined effect of these
 217 movements smooths the topography so that the downslope flux can be written as a dif-
 218 fusion term in the average sense, $\mathbf{f}_e = -D_c \nabla z$, where D_c is a coefficient based on com-
 219 bined efficiency of different soil creep processes (Culling, 1963). In the DL approxima-
 220 tion, the fluvial erosion flux is assumed proportional to the shear stress by the flowing
 221 runoff over the surface as $\nabla \cdot \mathbf{f}_e = K'_e q^m |\nabla z|^n$, where K'_e is an erosion coefficient, q is
 222 the specific runoff or the surface flow rate, and m and n are the model exponents (Howard,
 223 1994; Whipple & Tucker, 1999; G. E. Tucker & Hancock, 2010).

224 In equation (2), R represents a runoff-producing rainfall rate, i.e., the amount of
 225 precipitated water contributing to runoff production q in the direction of \mathbf{n} . Under the
 226 quasi-steady-state approximation ($\partial h/\partial t = 0$), with water flowing at a constant speed
 227 in the direction of steepest descent of the topographic surface ($\mathbf{n} = -\nabla z/|\nabla z|$) and a
 228 time-averaged runoff-producing rainfall rate R_0 , the water discharge q is proportional
 229 to the specific drainage area $a (= q/R_0)$. As a result, equation (2) becomes the govern-
 230 ing equation for the specific drainage area (Bonetti et al., 2020; Porporato, 2022). Em-
 231 ploying this proportionality between q and a , the erosion flux is modified as $\nabla \cdot \mathbf{f}_e =$
 232 $K_e a^m |\nabla z|^n$, where $K_e = K'_e R_0^m$.

233 Under these conditions, equations (1) and (2) get simplified as

$$234 \quad \frac{\partial z}{\partial t} = D_c \nabla^2 z - K_e a^m |\nabla z|^n + U, \quad (3)$$

$$235 \quad -\nabla \cdot \left(a \frac{\nabla z}{|\nabla z|} \right) = 1. \quad (4)$$

236 Through the coupling between a and z , the minimalist LEM, given by the system
 237 of equations (3) and (4), captures the essential feedbacks and dynamics of landscapes
 238 evolving over long time scales. Fluvial erosion and soil creep act as sink and diffusion
 239 terms in equation (3), respectively. Erosion excavates sediment at locations where the
 240 accumulation of the specific drainage area is high. This yields a higher surface gradient
 241 at those locations with a further increase in a , thus enforcing the increased erosion and
 242 flow accumulation again. This feedback loop between the emerging topography and the
 243 accumulated specific drainage area can carve a preferential path over time if the surface
 244 smoothing effect by the creep diffusion is not sufficient with the progression of a land-
 245 scape from a smooth topography towards a more dissected one.

246 For a domain with characteristic length l_x , equations (3) and (4) can be non-dimensionalized
 247 to derive a dimensionless quantity

$$248 \quad \mathcal{C}_{\mathcal{I}} = \frac{K_e l_x^{m+n}}{D_c^n U^{1-n}}, \quad (5)$$

249 which determines the relative impact of creep, erosion, and uplift on the first channel-
 250 ization and incipient valley spacing. An increase in $\mathcal{C}_{\mathcal{I}}$ (e.g., increased rainfall, declined
 251 efficiency of the soil creep, reduced resistance to the fluvial erosion) characterizes the ten-
 252 dency of the landscape to branch and form channels and has been therefore called ‘chan-
 253 nelization index’ by Bonetti et al. (2020).

254 2.2 1D Base-State Morphology

255 The landscape geometry considered here consists of two opposite and symmetric
 256 hillslopes along a linear ridge with zero water and sediment flux at the ridgeline and a
 257 fixed-level hillslope base as boundary conditions. These conditions are consistent with
 258 those adopted in earlier studies on the analysis of 1D hillslope morphology (Smith & Brether-
 259 ton, 1972; Loewenherz, 1991; Bonetti et al., 2019). The mathematical form of unchan-
 260 nnelized solutions at steady-state can be obtained by using boundary conditions in equa-
 261 tions (3) and (4) for the 1D transect.

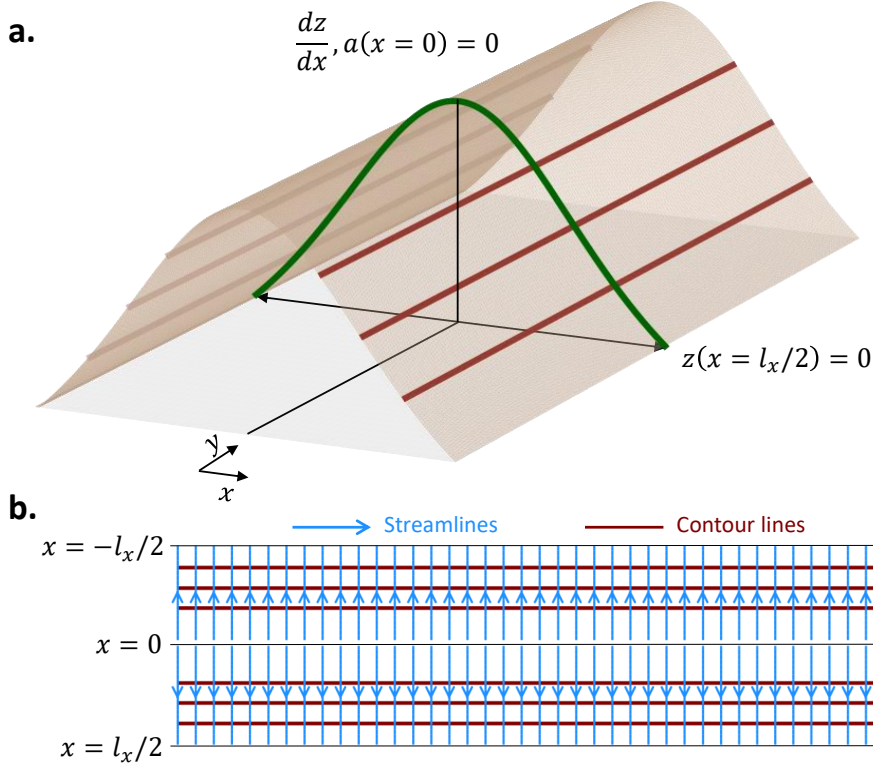


Figure 1. Schematic diagram showing the domain geometry and imposed boundary conditions to compute the unchanneled base-state solutions. (a): A representative 3D steady-state profile is shown, where x -axis points in the direction along the hillslope and y -axis denotes the ridgeline direction normal to the hillslope. The presence of a ridgeline/drainage divide in the center of the domain ensures zero water and sediment flux boundary conditions at $x = 0$. The hillslope baseline is taken fixed at $x = \pm l_x/2$ with zero elevation as the reference level. The green curve shows the unchanneled cross-section profile with l_x as the characteristic length-scale of the domain. (b): The horizontal projection of the landscape is shown with streamlines (in blue) perpendicular to the projected contour lines (in brown).

262 As shown in figure 1(a), the x -axis points in the direction along the hillslope and
 263 the y -axis denotes the direction of the ridgeline/drainage divide. The hillslopes incline
 264 towards a fixed surface level at $x = \pm l_x/2$, which is taken as zero for the elevation refer-
 265 ence ($z = 0$). With sediment and water flux not crossing over the top of the divide,
 266 the boundary conditions for z and a can be written as $dz/dx = 0$ and $a = 0$ at $x =$
 267 0 . With the elevation declining monotonically on either side of the divide, the steady-
 268 state solution for specific drainage area is simply the relation $a_0 = x$ with $x \in [0, l_x/2]$.
 269 Namely, it increases linearly with the distance from the ridgeline, as shown in figure 1(b).
 270 The subscript 0 is used here to represent the base state. The steady-state solution for
 271 the smooth elevation field $z(x)$ depends on the value of exponents m and n . This solu-
 272 tion can be obtained analytically only for $m = n = 1$, where it takes the form of a gen-
 273 eralized hypergeometric function (Bonetti et al., 2019, 2020; Anand et al., 2020), while
 274 it has to be obtained numerically for generic exponents m and n , as discussed in Sec-
 275 tion 3. We refer to this solution as $z_0(x)$ in the stability analysis formulation.

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2.3 Linearized Perturbed Equations

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Having established the base-state solutions, we can now study when they become unstable to small perturbations that lead to the formation of first channels with a specific length scale. A normal-mode linear stability analysis provides a way to systematically detect the inception of this channel instability and the preferential selection of the least stable wavenumber. We refer to the following references for an extensive description of this approach and its applications in various other physical systems (Koch & Meinhardt, 1994; Drazin & Reid, 2004; Chandrasekhar, 2013; Vlase et al., 2019).

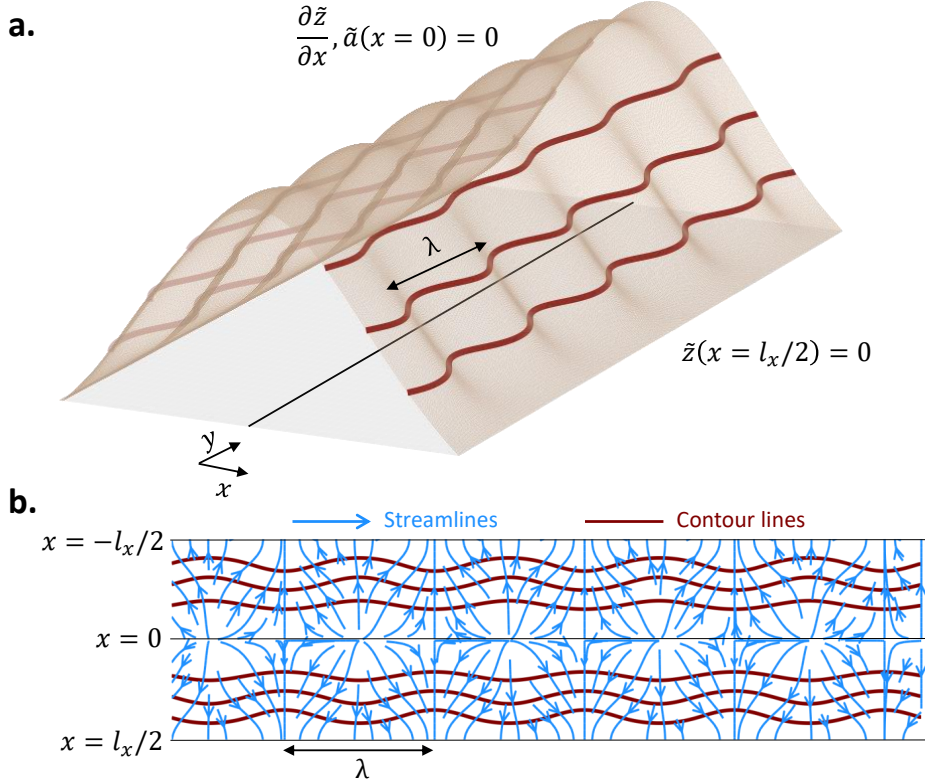


Figure 2. Schematic diagram presenting the perturbed state of the landscape used in the normal-mode analysis and the homogeneous boundary conditions. The weak perturbation \tilde{z} has been exaggerated for better visualization. (a): A representative 3D surface z is displayed, where x -axis/ y -axis denotes the direction along the hillslope/ridgeline. The perturbation with wavenumber k corresponds to the spatial wavelength $\lambda (= 2\pi/k)$. (b): The horizontal projection of the surface is shown with streamlines (in blue) perpendicular to the projected contour lines (in brown). The projected streamlines converge at the equally-spaced emerging valleys and diverge at the corresponding interlocked ridges.

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With infinitesimal perturbations in the base-state solutions, the modified elevation and specific drainage area fields can be written as $z(x, y, t) = z_0(x) + \tilde{z}(x, y, t)$ and $a(x, y, t) = a_0(x) + \tilde{a}(x, y, t)$. Here $z_0(x)$ and $a_0(x)$ are the unchanneled 1D solutions discussed in Section 2.2. $\tilde{z}(x, y, t)$ and $\tilde{a}(x, y, t)$ denote the weak perturbations over the unchanneled solutions. We assume here homogeneous boundary conditions for the weak perturbations, namely $\tilde{z} = 0$ at $x = l_x/2$ and $\partial \tilde{z} / \partial x = \tilde{a} = 0$ at $x = 0$. The mathemati-

cal expressions for \tilde{z} and \tilde{a} are written as

$$\tilde{z} = \psi(x) \exp(iky + \sigma t) + \text{c.c.}, \quad (6)$$

$$\tilde{a} = \phi(x) \exp(iky + \sigma t) + \text{c.c.}, \quad (7)$$

where $\psi(x)$ and $\phi(x)$ represent perturbation amplitudes varying along the hillslope with angular wavenumber k in the y -direction and initial growth rate σ (c.c. refer to complex conjugation). Depending on σ being greater or lower than zero, the perturbation of a particular wavenumber grows or decays over time. A representation of the modified elevation field with the weak perturbation form taken in equations (6) and (7) is displayed in Figure 2.

As shown in Appendix A, substituting the above forms of perturbations for the modified z and a fields in equations (3) and (4) and linearizing, the coupled governing equations for $\psi(x)$ and $\phi(x)$ become

$$\sigma\psi = D_c \frac{d^2\psi}{dx^2} - D_c k^2 \psi - mK_e S_0^n x^{m-1} \phi + nK_e S_0^{n-1} x^m \frac{d\psi}{dx}, \quad (8)$$

$$\frac{d\phi}{dx} = -\frac{k^2 x}{S_0} \psi, \quad (9)$$

where $S_0(x) = \left| \frac{dz_0}{dx} \right|$ is the steady-state unchannelized topographic slope.

To obtain the solutions for the growth of the perturbations using the spectral technique, we recast the reference system from x to \hat{s} ($= 4x/l_x - 1$) to keep the domain between -1 and 1, so that the Legendre polynomials could be used as the basis functions in the spectral solver. By applying this reference-change and non-dimensionalizing equations (8) and (9), a differential equation in terms of $\hat{\phi}$ ($= \phi/l_x$) reads

$$\gamma_1(\hat{s}) \frac{d^3 \hat{\phi}}{d\hat{s}^3} + \gamma_2(\hat{s}) \frac{d^2 \hat{\phi}}{d\hat{s}^2} + \gamma_3(\hat{s}) \frac{d\hat{\phi}}{d\hat{s}} + \gamma_4(\hat{s}) \hat{\phi} = \hat{\sigma} \gamma_5(\hat{s}) \frac{d\hat{\phi}}{d\hat{s}}, \quad (10)$$

where the overhat ($\hat{\cdot}$) refers to the non-dimensional form of the physical quantity. The above-mentioned homogeneous boundary conditions for perturbations can be re-written as $\hat{\phi}(\hat{s} = -1) = \hat{\phi}''(\hat{s} = -1) = \hat{\phi}'(\hat{s} = 1) = 0$. We refer to Appendix B for the derivation of the above equation as well as boundary conditions in terms of $\hat{\phi}(\hat{s})$. The expressions for all coefficients are provided in table B2 (Appendix B).

Equation (10) with the imposed boundary conditions forms an eigenvalue problem, where non-zero solutions $\hat{\phi}(\hat{s})$ exist for unique (eigen)values of the growth rate $\hat{\sigma}$ ($= \sigma l_x^2 / D_c$). This system can be solved to compute the growth rate $\hat{\sigma}$ for perturbation $\hat{\phi}(\hat{s})$ of wavenumber \hat{k} at different $\mathcal{C}_{\mathcal{I}}$ values. By increasing $\mathcal{C}_{\mathcal{I}}$, a critical threshold of this dimensionless quantity, say $\mathcal{C}_{\mathcal{I}cr}$, can be found for which at least one of many possible perturbations starts growing with a positive rate $\hat{\sigma}$. By tracking the wavenumber k_{cr} with the highest positive growth rate at $\mathcal{C}_{\mathcal{I}cr}$, the spacing between emerged first-order valleys $\lambda_{cr} = 2\pi/k_{cr}$ can be computed. Hence, the required proportion of erosion and creep and the resulting valley spacing on channelization onset can be obtained by replicating this approach for different degrees of nonlinearities in m and n .

To proceed toward a solution, we converted the differential problem of equation (10) into an integral form. This is usually referred to as a weak formulation of the problem due to a reduction in the differentiability constraint of the solution. The weak formulation was then solved by utilizing a spectral technique based on the Galerkin projection with numerical quadrature (Canuto et al., 2006). We employed the algorithm proposed by Swarztrauber (2003) to compute quadrature points and weights for the numerical quadrature. To guarantee an acceptable spectral accuracy, we used 200 points between -1 and 1 for the presented results. A detailed explanation of the developed methodology and the spectral solver is provided in Appendix C.

3 The Emergence of First Channel Instability

The unchannelized slope $\hat{S}_0(\hat{s})$ as well as its first and second derivatives for different values of $\mathcal{C}_{\mathcal{I}}$ are needed to finalize the form of non-constant coefficients and solve the eigenvalue problem posed in equation (10); see table B2. These expressions are analytically attainable only for the case of unitary exponents m and n , where the unchannelized slope and its derivatives take the form of Dawson functions (see equation (12)). For any other values of m and n , these derivatives have to be obtained numerically. \hat{S}_0 ($= |d\hat{z}_0/d\hat{s}|$) can be computed by first recasting the 1D form of equation (3) in terms of \hat{S}_0 at steady-state as

$$\frac{d\hat{S}_0}{d\hat{s}} = \frac{1}{16} \left[1 - 4^{n-m} \mathcal{C}_{\mathcal{I}} (\hat{s} + 1)^m \hat{S}_0^n \right], \quad (11)$$

which then can be integrated numerically with appropriate boundary conditions for any m and n values.

We solved here the differential equation (11) with the initial value $\hat{S}_0(\hat{s} = -1) = 0$. Once the numerical solution of \hat{S}_0 was obtained, the form of \hat{S}_0' was computed by using equation (11) at the discrete quadrature points. \hat{S}_0'' was then obtained by using second-order accurate central difference of \hat{S}_0' at the interior quadrature points and first-order accurate finite difference at the boundary points.

3.1 Verification of the Numerical Solver

We performed a code verification to ensure that the developed spectral eigenvalue solver (i.e., using the numerical form of slope and its derivatives) correctly solves the stability problem, without any programming/numerical error (Roache, 1998; Oberkampf & Roy, 2010). For that, the results using numerical integration of \hat{S}_0 (and its derivatives) were compared with the linear stability results employing the analytical solution for \hat{S}_0 for the case of $m = n = 1$. The analytical expression for the slope in the case of the unitary exponents is

$$\hat{S}_0 = \mathcal{D} \left(\frac{(\hat{s} + 1)\sqrt{\mathcal{C}_{\mathcal{I}}}}{4\sqrt{2}} \right) / (2\sqrt{2\mathcal{C}_{\mathcal{I}}}), \quad (12)$$

where $\mathcal{D}(\cdot)$ is the Dawson function (Bonetti et al., 2019, 2020; Anand et al., 2020).

All the predictions from the linear stability analysis are shown for the length scale $l_x = 100$ m. Figure 3 shows the stability analysis results using numerical integration of equation (11) for the base-state. In panel a, each curve represents the growth rate for different wavenumbers of lateral perturbations at a given $\mathcal{C}_{\mathcal{I}}$. The red curve in the panel shows the critical value $\mathcal{C}_{\mathcal{I}cr} \approx 37$ for which the fastest growth rate becomes positive for the intermediate wavenumber $k_{cr} \approx 0.153 \text{ m}^{-1}$. This numerical prediction of channel initiation matches with the predictions using the Dawson functions in Bonetti et al. (2020). The marginal/neutral stability curve is also shown in Figure 3b to present the transition of an unchannelized hillslope from a stable to unstable state as the critical value of the channelization index is reached. The system here displays a type I linear instability, which is similar to the Orr–Sommerfeld stability problem for the plane Poiseuille flow (Cross & Hohenberg, 1993).

3.2 The Influence of Different m Values

The values of m and n describe the coupling of the specific drainage area and local slope in the fluvial erosion mechanism. A thorough review of these power-law relationships derived from either shear stress or unit stream power law compared with the evidence from field studies is discussed in Whipple and Tucker (1999) and Lague (2014). In modeling studies of landscape evolution, the value of n is typically taken as unity with a usual range of the exponent reported between 0.67 and 1.67 (Seidl et al., 1992; Per-

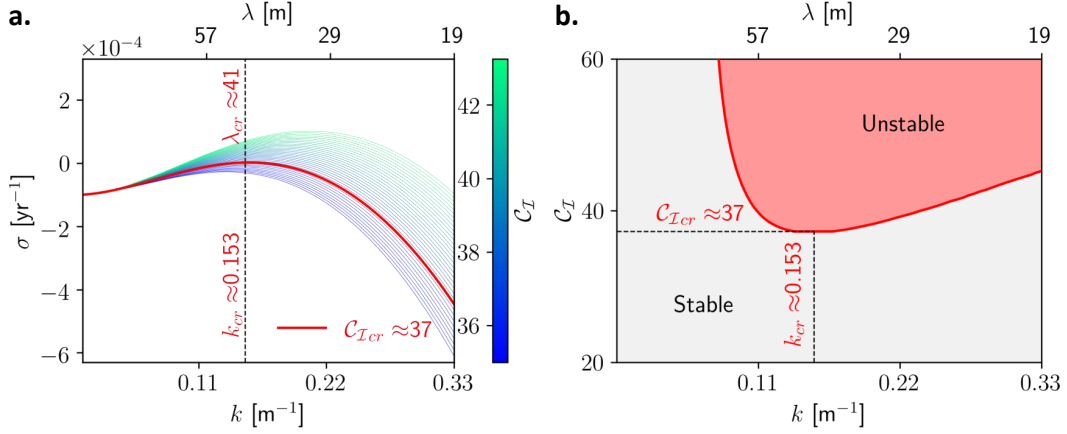


Figure 3. Linear stability analysis for the exponents $m = n = 1$ and the domain length-scale $l_x = 100$ m. (a) Growth rate σ as a function of wavenumber k for different values of C_I . The red curve corresponds to $C_I \approx 37$ with first positive growth rate for $k_{cr} \approx 0.153$ m⁻¹, which is equivalent to a characteristic incipient valley spacing $\lambda_{cr} = 2\pi/k_{cr} \approx 41$ m. (b) Marginal stability curve (the solid red curve) characterizes the instability of the base-state to the lateral perturbations. The red region designates the unstable wavenumbers with positive growth rate and the gray region describes the stable wavenumbers for distinct values of C_I .

ron et al., 2009). The value of the exponent m is usually between 0.3 and 0.8 in literature based on the analysis of the stream profiles from digital elevation models, field and map studies (Flint, 1974; Tarboton et al., 1991; Slingerland et al., 1998; Snyder et al., 2000; G. Tucker & Whipple, 2002; Bonetti et al., 2019). Exponent m equal to 0.5 is generally taken as the base case in the Optimal Channel Network (OCN) theory due to its close resemblance with scaling laws obtained in fluvial landscapes with negligible diffusive soil creep, i.e., $C_I \rightarrow \infty$ (Banavar et al., 1997; Rodriguez-Iturbe & Rinaldo, 2001; Rinaldo et al., 2014; Hooshyar et al., 2020).

We show here the role of the exponent m on the emergence of first-order valleys for $n = 1$, while the non-unity value of n is further examined in Section 3.3. Figure 4a displays the marginal stability curves obtained for eight values of m between 0.125 (red curve) and 1 (blue curve), where the corresponding horizontal lines represent the channelization threshold $C_{I,cr}$ and the vertical lines mark the fastest-growing wavenumber k_{cr} . Figure 4 b,c display the dependency of the channelization threshold and emerging valley spacing on the value of m . Specifically, as m decreases, an increase in the critical C_I value is observed together with the formation of narrower valleys at the instance of channelization.

Using the definition of the channelization index C_I given in equation (5), it can be seen that the system's behavior evolves independent of the uplift rate for $n = 1$ and is primarily governed by the ratio of the coefficient of erosion (K_e) to the soil creep (D_c) for the same length-scale of the domain. Describing the above results for channel instability in terms of $K_e/D_{c,cr}$ ($= C_{I,cr}/l_x^{m+1}$) provides insight on the efficiency level of erosion needed to initiate the channelization in natural landscapes. As displayed in the inset of panel b, the ratio of K_e to D_c grows by a factor of almost 100 for the reduction in the value of m by a factor of 8 from 1.0 to 0.125. This increase in the erosion threshold for the appearance of first-order valleys as m approaches zero reveals the importance of non-locality conveyed by a in the erosion mechanism on the development of channels in fluvial landscapes.

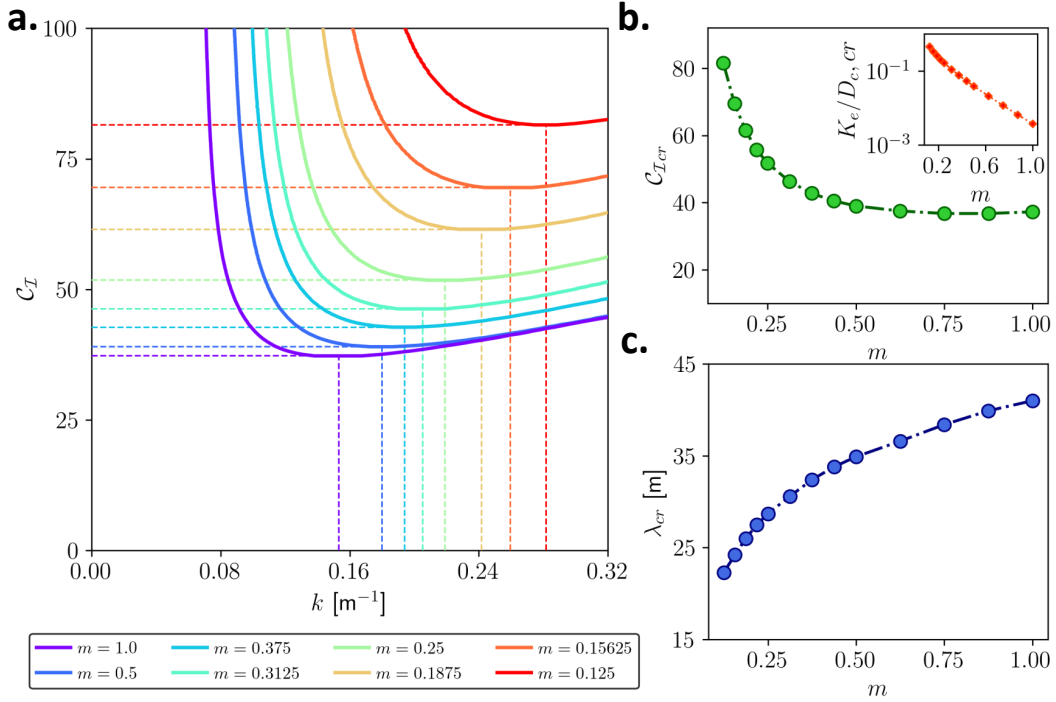


Figure 4. The effect of the drainage area exponent m on threshold of erosion intensity for the channelization and incipient valley spacing. (a): Marginal stability curves for exponent m values varying from 0.125 to 1, keeping $n = 1$. Each stability curve of a distinct color designates a particular value of m with the same-colored vertical line denoting the most unstable wavenumber, k_{cr} , and a corresponding horizontal line of the same color indicating the critical value of channelization index, C_{Icr} , for the first channelization. (b): Plot of critical channelization index C_{Icr} versus m . The inset displays the relation between K_e/D_c and m for channelization onset. (c): Variation of the incipient valley spacing at the channelization threshold λ_{cr} as a function of m .

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3.3 Numerical Simulations for Generic m and n

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We compared the predictions of the linear stability analysis with the instance of the first channelization by using the numerical algorithm introduced in Anand et al. (2020) for the simulations of the complete LEM. This efficient algorithm provides an order of traversing nodes in the discrete domain so that the erosion term can be computed implicitly as an upper/lower triangular matrix system with the time complexity of the algorithm varying linearly with the number of nodes in the domain. The solutions obtained using this algorithm were verified and tested carefully against analytical predictions in Anand et al. (2020).

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For these numerical simulations, we considered a rectangular domain with a high aspect ratio to numerically replicate the instability onset in the linear ridge with symmetric hillslopes, as considered in the stability analysis formulation. For all the simulations, the width and length of the domain were kept equal to 100 m and 500 m, respectively, with unit grid spacing. Fixed zero elevation boundary conditions were used and the solutions were analyzed in the middle 300 m to reduce the effect of lateral sides on the channel spacing. We used two values of $n = 0.75$ and 1.0 with $m = 0.125, 0.25, 0.375, 0.5, 0.625, 0.75$, and 1.0. The value of C_I was increased for each scenario of m and n till first-order channels were observed in the domain.

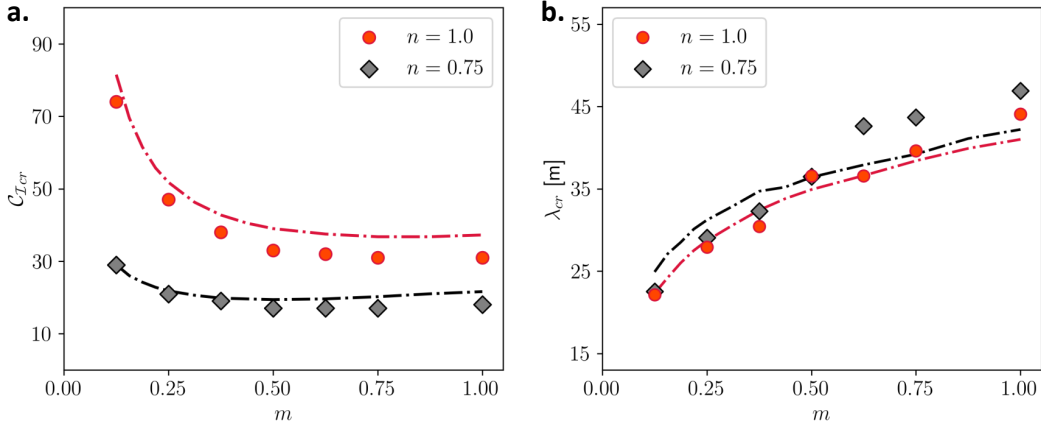


Figure 5. Comparison of predictions from linear stability analysis versus numerical simulation results in a rectangular domain (width = 100 m, length = 500 m, and 1 m grid spacing) for varying values of m with $n = 0.75$ (gray) and $n = 1.0$ (red). (a): Plot of the channelization threshold \mathcal{C}_{Icr} versus exponent m . (b): Variation of λ_{cr} as a function of m . Dashed curves represent stability analysis predictions for a given n , while symbols show results obtained from numerical simulations. A good agreement between predictions of channel initiation from the two approaches is observed across different m and n values.

427 Figure 5 compares the instance of the first channelization obtained using numerical
 428 simulations with results from the stability analysis. The comparison shows that the
 429 stability analysis agrees fairly well with the occurrence of first channelization and incip-
 430 ient valley spacing obtained in the steady-state solutions from the numerical modeling.
 431 The slight difference in \mathcal{C}_{Icr} and λ_{cr} values for the numerical model and the stability anal-
 432 ysis hints at the nonlinear interactions (higher-order terms in the governing equations
 433 of the perturbations) discounted in the linear stability formulation that, despite being
 434 small, are present in the numerical simulations of the governing equations (3) and (4).

435 The channelization threshold \mathcal{C}_{Icr} increases with the lowering of the exponent m
 436 at a particular value of n . On the contrary, the value of \mathcal{C}_{Icr} grows with an increase in
 437 n at a fixed value of m . The emergent valley spacing widens for high values of m (at $n =$
 438 0.75 and 1.0), while the exponent n has little bearing on the preferential scale of chan-
 439 nelization at a given m . The formation of narrower valleys with the decrease in m is vis-
 440 ible from the hillslope morphologies at the first channel instability obtained using nu-
 441 merical simulations (figure 6).

442 3.4 Comparison with Regular Valley Spacing in a Natural Landscape

443 We also compared the predictions from the linear stability analysis with the obser-
 444 vations of first-order valley formation in a mountainous landscape dominated by dif-
 445 fusive creep and fluvial erosion. The landscape examined here is a portion of the Gabi-
 446 lan Mesa in California characterized by a Mediterranean climate and oak-savanna (lightly
 447 forested grassland) vegetation cover, previously investigated in Perron, Kirchner, and Di-
 448 etrich (2008) and Perron et al. (2009). The displayed terrain has NE-SW trending prin-
 449 cipal channels (green) and evenly-spaced intervening hillslopes along the ridges (brown)
 450 as shown in figure 7(a,b) with the distance between the two prominent channels to be
 451 roughly 550 m and the valley spacing along the ridges around 163 ± 11 m (Perron, Kirchner,
 452 & Dietrich, 2008; Perron et al., 2009). Assuming exponent $n = 1$, the values of $D_c/K_e =$
 453 124 ± 3 and $m = 0.35 \pm 0.003$ were computed by using the shapes of hilltops and stream

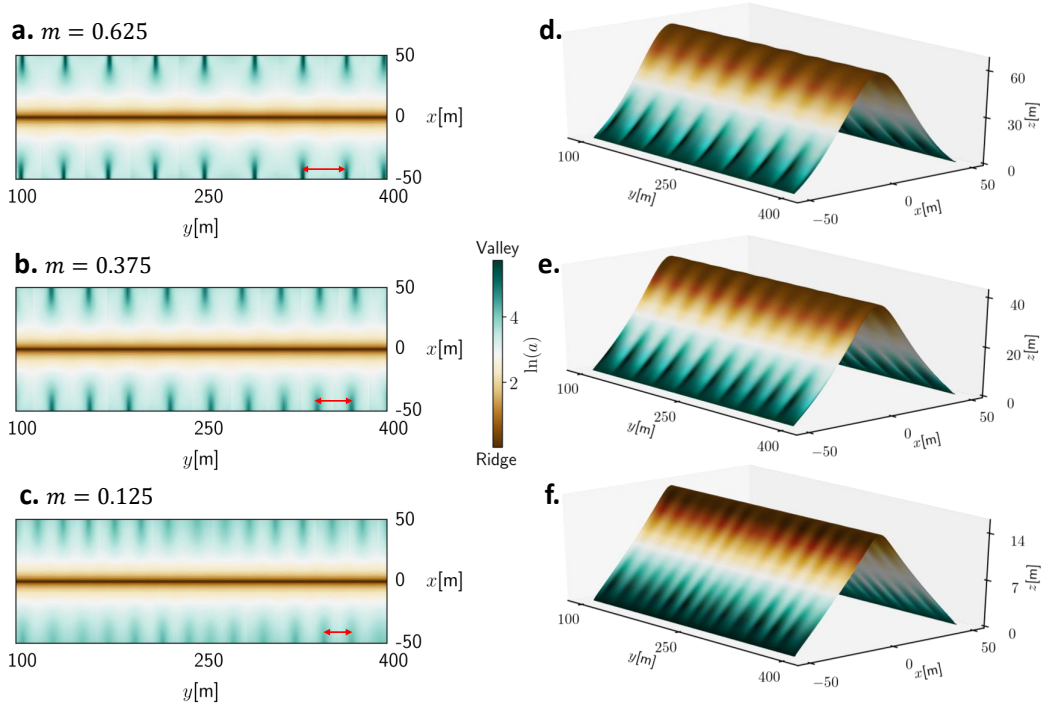


Figure 6. First channel instability observed in numerical simulations over a rectangular domain (width = 100 m, length = 500 m, and 1 m grid spacing) with $n = 1$. Plots of specific drainage area (a) field are shown for the middle 300 m (i.e., neglecting the last 100 m of the domain on both sides) for (a) $m = 0.625$ at $\mathcal{C}_{\mathcal{I}} = 32$, (b) $m = 0.375$ at $\mathcal{C}_{\mathcal{I}} = 38$, and (c) $m = 0.125$ at $\mathcal{C}_{\mathcal{I}} = 74$. The color-scale to display a field is kept the same for the presented cases to highlight the effect of an increase in the value of m with wider and larger flow accumulating in first-order valleys. Red arrows in each plot indicate typical valley spacing. (d, e, f): 3D steady-state surface profiles for the solutions shown in panels a, b, and c, respectively.

454 profiles for the given topography in Perron et al. (2009). Employing these values of the
 455 parameters and $l_x = 550$ m with relative uncertainties l_x and n assumed to be 2.5%,
 456 we estimated the value of $\mathcal{C}_{\mathcal{I}}$ to be 40.4 ± 7.3 .

457 A long ridge between two main channels resembles the domain geometry used in
 458 the stability analysis formulation. In this correspondence, fixed elevation boundary con-
 459 ditions at the hillslope base used in the stability analysis agree with the base level set
 460 for the Mesa landscape by the Salinas River. We conducted the linear stability analy-
 461 sis for $m = 0.35 \pm 0.003$ and $n = 1.0 \pm 0.025$ and tracked the instance of first channel-
 462 ization along with the dominant channelization mode in the calculated $\mathcal{C}_{\mathcal{I}}$ range for the
 463 landscape. The stability analysis results predict the value of $\mathcal{C}_{\mathcal{I}cr} \approx 44 \pm 3.5$, which
 464 falls in the estimated $\mathcal{C}_{\mathcal{I}}$ range for the landscape. The dominant valley spacing is com-
 465 puted to be 175^{+6}_{-38} m, which is also in line with the measured spacing around 163 ± 11
 466 m in the landscape from Gabilan Mesa. Figure 7(c) shows the stability analysis result
 467 for average values of the parameters $m = 0.35$, $n = 1$, and $l_x = 550$ m.

468 A satisfactory agreement between the first-order valley spacing obtained from the
 469 stability analysis with those acquired by the topographic measurements of the landscape
 470 suggests that the linear stability formulation of the minimalist LEM captures well the
 471 feedback between the competing diffusive creep and fluvial erosion for the first-order chan-

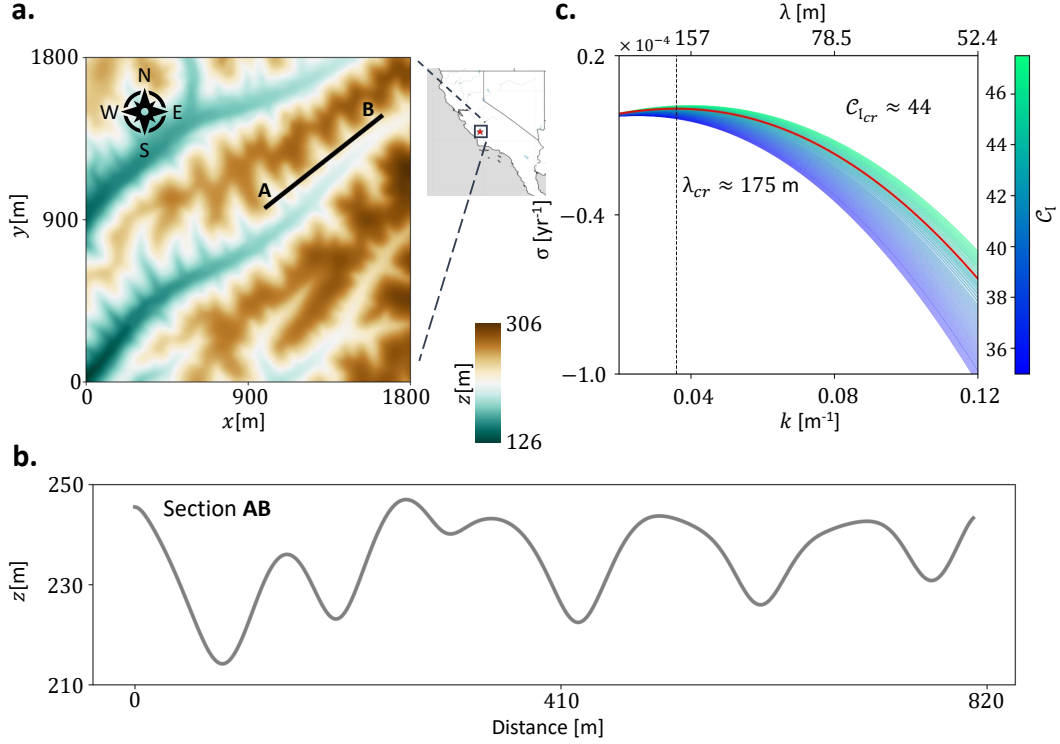


Figure 7. Comparison of the stability analysis results with the topographic data of a natural landscape. (a): 2D color-plot (top view) of the landscape covering approximately 3.25 km^2 area in Gabilan Mesa (California), where evenly-spaced green valleys appear along the brown mountain ridge. (b): Plot of the elevation field along the cross-section AB highlighted in panel a. The topographic data was obtained from the National Center for Airborne Laser Mapping. (c): Result of the linear stability analysis using the spectral solver for the exponents $m = 0.35$ and $n = 1$.

472 nelization. This is a promising result as the constant average values assumed for the
 473 parameters and the nonlinear interactions neglected in the stability analysis are approx-
 474 imations of the heterogeneous and noisy reality.

475 4 Discussion and Conclusions

476 We conducted a linear stability analysis of the governing equations of a minimal-
 477 ist DL-LEM and quantified the role of different formulations of fluvial erosion on the for-
 478 mation of the evenly-spaced valleys. The use of the spectral method made it feasible to
 479 compute solutions to the posed stability problem (Canuto et al., 2006; Camporeale et
 480 al., 2012), in the presence of a differential equation where non-constant coefficients elude
 481 analytical tractability. The flexibility provided by the spectral method can be extended
 482 further to quantify the effect of factors such as erosion threshold, spatially varying pa-
 483 rameterization, etc., on the channelization in the natural landscapes.

484 The results have shown that the first-order valleys with spacing λ_{cr} emerge at a
 485 specific proportion of fluvial erosion and soil creep given by the critical value of the non-
 486 dimensional index $\mathcal{C}_{I_{cr}}$. We obtained the dependency of λ_{cr} and $\mathcal{C}_{I_{cr}}$ on the exponent
 487 m and n in the erosion mechanism. In particular, a reduction in m for a fixed value of
 488 n increases $\mathcal{C}_{I_{cr}}$ threshold, which means that higher erosion potential is required to carve

489 the hillslope for channelization as the relative importance of the specific drainage area
 490 in the erosion mechanism diminishes. Conversely, the threshold for channel formation
 491 rises with an increase in the value of n for a particular m value. The exponent m fur-
 492 ther impacts the selection of a characteristic valley spacing with progressively narrower
 493 valleys appearing for the declining value of m .

494 We compared the results of the linear stability analysis with the numerical simu-
 495 lations of the LEM in a long rectangular domain. A close agreement between the two
 496 approaches was observed for the inception of the regularly-spaced valleys at a certain
 497 erosion threshold. Prediction of the stability analysis was further validated by using the
 498 topographic data of a natural landscape. The present analysis of the effect of the ero-
 499 sion law on the channelization also agrees with the observations from numerical simu-
 500 lations in a square domain discussed in Bonetti et al. (2020). For example, the simulated
 501 landscapes in figure 3 of Bonetti et al. (2020) show channelization and subsequent branch-
 502 ing at higher $\mathcal{C}_{\mathcal{I}}$ values as n increases (0.7, 1.0, and 1.3) at $m = 0.5$. Similarly, the ap-
 503 pearance of narrower primary valleys with smaller junction angles at secondary branch-
 504 ing has been noted in the study for lower m values at a given n .

505 The main result of our linear stability theory is that a normal-flow hypothesis in
 506 detachment-limited conditions constitutes the minimalist water-flow model, and the coun-
 507 teracting diffusive creep and fluvial erosion for sediment transport create a simple sys-
 508 tem for channel formation. Numerical solutions to the posed stability problem demon-
 509 strate that preserving the spatial variability of the base-state solutions and the coeffi-
 510 cients of the final eigenvalue problem allow a characteristic wavenumber selection based
 511 on the erosion law, providing an explicit linkage between the nonlinear erosion feedbacks
 512 and the spectral signature of channelization in the natural landscapes.

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 524 oratory at Princeton University.

525 Open Research

526 The Python code developed for the linear stability analysis is available at [https://](https://github.com/ShashankAnand1996/LEM_Stability_Analysis)
 527 github.com/ShashankAnand1996/LEM_Stability_Analysis. Well-commented Python
 528 code for the simulation results is also accessible at [https://github.com/ShashankAnand1996/](https://github.com/ShashankAnand1996/LEM)
 529 [LEM](https://github.com/ShashankAnand1996/LEM).

Appendix A Linearized Perturbed Equations and Boundary Conditions

The modified elevation and specific drainage area fields with weak perturbations can be written as

$$z(x, y, t) = z_0(x) + \tilde{z}(x, y, t), \quad (\text{A1})$$

$$a(x, y, t) = a_0(x) + \tilde{a}(x, y, t), \quad (\text{A2})$$

where $z_0(x)$ and $a_0(x)$ are the steady-state unchanneled solutions; \tilde{z} and \tilde{a} denote perturbation fields.

Using equation (A1), the updated topographic gradient vector becomes

$$\nabla z = \left(-S_0 + \frac{\partial \tilde{z}}{\partial x} \right) \mathbf{i} + \frac{\partial \tilde{z}}{\partial y} \mathbf{j}, \quad (\text{A3})$$

where $S_0(x) = \left| \frac{dz_0}{dx} \right|$ is the unchanneled local slope, \mathbf{i} is the unit vector in x -axis direction, and \mathbf{j} is the unit vector in the direction of y -axis. Employing this form of the gradient, the linearized expression for the updated topographic slope is written as

$$\begin{aligned} |\nabla z| &= \sqrt{S_0^2 + \left(\frac{\partial \tilde{z}}{\partial x} \right)^2 - 2S_0 \frac{\partial \tilde{z}}{\partial x} + \left(\frac{\partial \tilde{z}}{\partial y} \right)^2} \\ &= S_0 \left[1 + \frac{1}{2} \left(-\frac{2}{S_0} \frac{\partial \tilde{z}}{\partial x} \right) \right] = S_0 - \frac{\partial \tilde{z}}{\partial x}. \end{aligned} \quad (\text{A4})$$

The governing equation for the updated elevation field $z(x, y, t)$ is

$$\begin{aligned} \frac{\partial z}{\partial t} &= D_c \nabla^2 z - K_e a^m |\nabla z|^n + U, \\ \frac{\partial \tilde{z}}{\partial t} &= D_c \nabla^2 z_0 + D_c \nabla^2 \tilde{z} - K_e (a_0 + \tilde{a})^m \left(S_0 - \frac{\partial \tilde{z}}{\partial x} \right)^n + U, \end{aligned} \quad (\text{A5})$$

where $(a_0 + \tilde{a})^m = a_0^m + m a_0^{m-1} \tilde{a}$ for small perturbation \tilde{a} . Writing $\left(S_0 - \frac{\partial \tilde{z}}{\partial x} \right)^n = S_0^n \left(1 - \frac{1}{S_0} \frac{\partial \tilde{z}}{\partial x} \right)^n$ and performing series expansion for small $\frac{\partial \tilde{z}}{\partial x}$ modifies the term as $S_0^n - n S_0^{n-1} \frac{\partial \tilde{z}}{\partial x}$. Using these expressions and $a_0 = x$, the linearized equation for $\tilde{z}(x, y, t)$ reads

$$\frac{\partial \tilde{z}}{\partial t} = D_c \nabla^2 \tilde{z} - m K_e S_0^n x^{m-1} \tilde{a} + n K_e x^m S_0^{n-1} \frac{\partial \tilde{z}}{\partial x}. \quad (\text{A6})$$

Using equations (A3) and (A4), the unit vector in the direction of steepest descent of the updated elevation field is

$$\begin{aligned} \mathbf{n} &= -\frac{\nabla z}{|\nabla z|} = -\frac{\left(-S_0 + \frac{\partial \tilde{z}}{\partial x} \right)}{\left(S_0 - \frac{\partial \tilde{z}}{\partial x} \right)} \mathbf{i} - \frac{\frac{\partial \tilde{z}}{\partial y}}{\left(S_0 - \frac{\partial \tilde{z}}{\partial x} \right)} \mathbf{j} = \mathbf{i} - \left(\frac{S_0 - \frac{\partial \tilde{z}}{\partial x}}{\frac{\partial \tilde{z}}{\partial y}} \right)^{-1} \mathbf{j} \\ &= \mathbf{i} - \left(\frac{S_0}{\frac{\partial \tilde{z}}{\partial y}} - \frac{\frac{\partial \tilde{z}}{\partial x}}{\frac{\partial \tilde{z}}{\partial y}} \right)^{-1} \mathbf{j} = \mathbf{i} - \frac{\frac{\partial \tilde{z}}{\partial y}}{S_0} \mathbf{j}. \end{aligned} \quad (\text{A7})$$

556 The linearized governing equation for $\tilde{a}(x, y, t)$ can be obtained using equation (A7)
 557 as

$$\begin{aligned}
 558 \quad & \nabla \cdot (\mathbf{a}\mathbf{n}) = 1 \\
 559 \quad & \nabla \cdot \left((a_0 + \tilde{a})\mathbf{i} - \frac{1}{S_0} \frac{\partial \tilde{z}}{\partial y} (a_0 + \tilde{a})\mathbf{j} \right) = 1 \\
 560 \quad & \frac{\partial(x + \tilde{a})}{\partial x} - \frac{1}{S_0} \frac{\partial \tilde{z}}{\partial y} \frac{\partial(x + \tilde{a})}{\partial y} - \frac{1}{S_0} \frac{\partial^2 \tilde{z}}{\partial y^2} (x + \tilde{a}) = 1 \\
 561 \quad & \frac{\partial \tilde{a}}{\partial x} - \frac{1}{S_0} \frac{\partial \tilde{z}}{\partial y} \frac{\partial \tilde{a}}{\partial y} - \frac{x}{S_0} \frac{\partial^2 \tilde{z}}{\partial y^2} - \frac{\tilde{a}}{S_0} \frac{\partial^2 \tilde{z}}{\partial y^2} = 0 \\
 562 \quad & \frac{\partial \tilde{a}}{\partial x} = \frac{x}{S_0} \frac{\partial^2 \tilde{z}}{\partial y^2} + h.o.t. = \frac{x}{S_0} \frac{\partial^2 \tilde{z}}{\partial y^2}. \tag{A8}
 \end{aligned}$$

563 We employ the mathematical expressions for $\tilde{z}(x, y, t)$ and $\tilde{a}(x, y, t)$ as

$$564 \quad \tilde{z} = \psi(x) \exp(iky + \sigma t) + c.c., \tag{A9}$$

$$565 \quad \tilde{a} = \phi(x) \exp(iky + \sigma t) + c.c., \tag{A10}$$

566 where $\psi(x)$ and $\phi(x)$ denote perturbation amplitudes varying along the hillslope with
 567 angular wavenumber k in the y -direction and initial growth rate σ . Substituting these
 568 in equations (A6) and (A8), we write the coupled equations for $\psi(x)$ and $\phi(x)$ as

$$569 \quad \sigma\psi = D_c \frac{d^2\psi}{dx^2} - D_c k^2 \psi - mK_e S_0^n x^{m-1} \phi + nK_e S_0^{n-1} x^m \frac{d\psi}{dx}, \tag{A11}$$

$$570 \quad \frac{d\phi}{dx} = -\frac{k^2 x}{S_0} \psi. \tag{A12}$$

571 In this work, we assume homogeneous boundary conditions for the weak pertur-
 572 bations, namely $\tilde{a} = 0$ at $x = 0$, $\tilde{z} = 0$ at $x = l_x/2$, and $\partial\tilde{z}/\partial x = 0$ at $x = 0$. These
 573 three conditions are re-written in terms of only $\phi(x)$ to proceed towards a solution, as
 574 shown below.

575 Using equation (A10), $\tilde{a} = 0$ at $x = 0$ becomes $\phi(x = 0) = 0$. The condition
 576 $\tilde{z} = 0$ at $x = l_x/2$ gives $\psi(x = l_x/2) = 0$ by using equation (A9). Substituting this
 577 relation in equation (A12) provides $d\phi/dx(x = l_x/2) = 0$. Finally, $\frac{\partial\tilde{z}}{\partial x} = 0$ at $x = 0$
 578 gets translated into $\frac{d\psi}{dx} = 0$ at $x = 0$. Imposing this requirement in equation (A12)
 579 gives $\frac{d}{dx} \left(\frac{S_0}{x} \frac{d\phi}{dx} \right) \Big|_{x=0} = 0$. Under the assumption that $S_0(x)$ behaves linearly in the limit
 580 $x \rightarrow 0$, we get the boundary condition $\frac{d^2\phi}{dx^2}(x = 0) = 0$.

581 Appendix B Non-dimensionalization and Eigenvalue Problem Formu- 582 lation

583 The physical problem posed here has three primary dimensions, namely vertical
 584 direction (Z) for the elevation field, horizontal direction (X) for spatial extent of the so-
 585 lution domain, and time (T) for the rate of evolution. Using l_x as the horizontal scale,
 586 $\frac{Ul_x^2}{D_c}$ as the vertical scale, and $\frac{l_x^2}{D_c}$ as the time scale, perturbed equations (A11) and (A12)
 587 can be recast in the following dimensionless form

$$588 \quad \hat{\sigma}\hat{\psi} = -\hat{k}^2\hat{\psi} + \frac{d^2\hat{\psi}}{d\hat{x}^2} - m\mathcal{C}_{\mathcal{I}}\hat{S}_0^n \hat{x}^{m-1} \hat{\phi} + n\mathcal{C}_{\mathcal{I}}\hat{S}_0^{n-1} \hat{x}^m \frac{d\hat{\psi}}{d\hat{x}}, \tag{B1}$$

$$589 \quad \frac{d\hat{\phi}}{d\hat{x}} = -\frac{\hat{k}^2 \hat{x}}{\hat{S}_0} \hat{\psi}, \tag{B2}$$

$$590 \quad \mathcal{C}_{\mathcal{I}} = \frac{K_e l_x^{m+n}}{D_c^n U^{1-n}}, \tag{B3}$$

591 where the overhat notation ($\hat{\cdot}$) refers to the non-dimensional form of the physical quantities.
 592 The channelization index $\mathcal{C}_{\mathcal{I}}$ is the non-dimensional quantity that represents the
 593 tendency of the system to form channels. Table B1 presents the non-dimensionalized forms
 594 of the variables involved in the above equations.

Table B1. Variables present in the perturbed equations (A11) and (A12) along with their dimension functions and non-dimensionalized forms used in equations (B1) and (B2).

Variable	Dimension Function	Non-dimensionalized Form
σ	T^{-1}	$\hat{\sigma} = \sigma \frac{l_x^2}{D}$
ψ	Z	$\hat{\psi} = \psi \frac{\hat{D}_c}{Ul_x^2}$
ϕ	L	$\hat{\phi} = \frac{\phi}{l_x}$
k	L^{-1}	$\hat{k} = kl_x$
S_0	ZL^{-1}	$\hat{S}_0 = S_0 \frac{D_c}{Ul_x}$
x	L	$\hat{x} = \frac{x}{l_x}$

595 Combining equations (B1) and (B2) and changing the reference variable from \hat{x} to
 596 \hat{s} ($= 4\hat{x} - 1$), the final form of the perturbed equation in terms of only $\hat{\phi}(\hat{s})$ reads

$$597 \quad \gamma_1(\hat{s})\hat{\phi}''' + \gamma_2(\hat{s})\hat{\phi}'' + \gamma_3(\hat{s})\hat{\phi}' + \gamma_4(\hat{s})\hat{\phi} = \hat{\sigma}\gamma_5(\hat{s})\hat{\phi}', \quad (\text{B4})$$

598 where the prime ($'$) refers to the derivative with respect to \hat{s} . The expressions for coef-
 599 ficients are specified in table B2.

Table B2. Constants and expressions for the coefficients in the differential eigenvalue problem (equation (10) in the main text). The prime ($'$) refers to the derivative with respect to \hat{s} .

Name	Form of the constant/expression
a_1	$\frac{n\mathcal{C}_{\mathcal{I}}}{4^{m-n}}$
a_2	\hat{k}^2
a_3	$\frac{m\mathcal{C}_{\mathcal{I}}\hat{k}^2}{4^{2+m-n}}$
$\hat{S}_0(\hat{s})$	$ dz_0/d\hat{s} $
$\gamma_1(\hat{s})$	$16\hat{S}_0^2(\hat{s}+1)^2$
$\gamma_2(\hat{s})$	$-32\hat{S}_0^2(\hat{s}+1) + 32\hat{S}_0\hat{S}_0'(\hat{s}+1)^2 + a_1\hat{S}_0^{n+1}(\hat{s}+1)^{m+2}$
$\gamma_3(\hat{s})$	$16\hat{S}_0\hat{S}_0''(\hat{s}+1)^2 - 32\hat{S}_0\hat{S}_0'(\hat{s}+1) + 32\hat{S}_0^2 - a_2\hat{S}_0^2(\hat{s}+1)^2 - a_1\hat{S}_0^{n+1}(\hat{s}+1)^{m+1} + a_1\hat{S}_0^n\hat{S}_0'(\hat{s}+1)^{m+2}$
$\gamma_4(\hat{s})$	$a_3\hat{S}_0^{n+1}(\hat{s}+1)^{m+2}$
$\gamma_5(\hat{s})$	$\hat{S}_0^2(\hat{s}+1)^2$

600 Finally, the boundary conditions for $\hat{\phi}(\hat{s})$ in the changed reference variable \hat{s} are

$$601 \quad \hat{\phi} = 0 \quad (\hat{s} = -1), \quad (\text{B5})$$

$$602 \quad \hat{\phi}' = 0 \quad (\hat{s} = 1), \quad (\text{B6})$$

$$603 \quad \hat{\phi}'' = 0 \quad (\hat{s} = -1). \quad (\text{B7})$$

Appendix C Weak Formulation and Galerkin Discretization

Equation (B4) along with boundary conditions mentioned in equations (B5), (B6), and (B7) constitute an eigenvalue problem, which is solved here by transforming the final equation into an integral form (weak formulation). The dependency on \hat{s} has been omitted hereafter in the expressions for the ease of notation.

A weak formulation is obtained by multiplying both sides of equation (B4) by a generic L^2 -test function v_i (with $i \in 1, N$) and integrating over the interval $(-1, 1)$ as

$$\left(\gamma_1 \hat{\phi}''', v_i\right) + \left(\gamma_2 \hat{\phi}'', v_i\right) + \left(\gamma_3 \hat{\phi}', v_i\right) + \left(\gamma_4 \hat{\phi}, v_i\right) = \hat{\sigma} \left(\gamma_5 \hat{\phi}', v_i\right), \quad (\text{C1})$$

where $(f, g) := \int_{-1}^1 f(\hat{s}')g(\hat{s}')d\hat{s}'$ defines the inner product between two functions. The numerical approximation of inner products in the above equation can be computed by interpolatory Legendre-Gauss quadrature formula, which approximates the integration of a generic function f in the domain $[-1, 1]$ through the use of weights w_k computed at discrete (Gauss-Lobatto) nodes \hat{s}_k as

$$\int_{-1}^1 f(\hat{s})d\hat{s} \approx \sum_{k=0}^{k=K} f(\hat{s}_k)w_k. \quad (\text{C2})$$

In the numerical solver developed for this work, \hat{s}_k and w_k are computed using the algorithm provided by Swarztrauber (2003).

Based on previous works on spectral solutions of eigenvalue problems in shear flows (Shen, 1994), we seek a solution of $\hat{\phi}$ in the form

$$\hat{\phi} = u_{-1}(\hat{s})\alpha_{-1} + u_0(\hat{s})\alpha_0 + \sum_{j=1}^N u_j(\hat{s})\alpha_j = \sum_{j=-1}^N u_j(\hat{s})\alpha_j, \quad (\text{C3})$$

where α_j are the unknown coefficients of the linear expansion and u_j reads

$$u_{-1}(\hat{s}) = \frac{1 + \hat{s}}{2} \quad (\text{C4})$$

$$u_0(\hat{s}) = -\frac{\hat{s}^2}{4} + \frac{\hat{s}}{2} + \frac{3}{4} \quad (\text{C5})$$

$$u_j(\hat{s}) = \frac{L_{j+2}(\hat{s}) - L_j(\hat{s})}{\sqrt{2(2j+3)}} \quad (j \in [1, N]). \quad (\text{C6})$$

In the above expressions, L_j represents the Legendre polynomial of degree j . So, $u_j(\pm 1) = 0$ for $j \geq 1$ with $u_{-1}(-1) = u_0(-1) = 0$. The additional functions u_{-1} and u_0 have been added to the basis to accommodate the non-vanishing boundary conditions.

Finally, from equation (C6) and using the properties of the Legendre polynomial (Szegő, 1939; Pólya & Szegő, 1972), one obtains

$$u_j'(\hat{s}) = \sqrt{\frac{2j+3}{2}} L_{j+1}(\hat{s}). \quad (\text{C7})$$

Taking these particular forms of trial functions, the boundary condition $\phi(-1) = 0$ gets implicitly imposed in the formulation. The remaining two boundary conditions (equations (B6) and (B7)) have to be applied explicitly in the strong form, as described later. Test functions (v_i for $i \in [1, N]$) are chosen by integrating twice each Legendre poly-

637 nomial as

$$638 \quad v_i = \sqrt{i + \frac{3}{2}} \left(\frac{L_{i+3} - L_{i+1}}{(2i+3)(2i+5)} - \frac{L_{i+1} - L_{i-1}}{(2i+1)(2i+3)} \right), \quad (\text{C8})$$

$$639 \quad v'_i = \frac{L_{i+2} - L_i}{\sqrt{2(2i+3)}}, \quad (\text{C9})$$

$$640 \quad v''_i = \sqrt{\frac{2i+3}{2}} L_{i+1}, \quad (\text{C10})$$

641 where these functions satisfy homogeneous boundary conditions as $v_i(\pm 1) = v'_i(\pm 1) =$
642 0 .

643 Using integration by parts, the third-order term in equation (C1) can be written
644 as

$$645 \quad \begin{aligned} & \left(\hat{\phi}''' \gamma_1, v_i \right) = \hat{\phi}''(1) \gamma_1(1) v_i(1) - \hat{\phi}''(-1) \gamma_1(-1) v_i(-1) \\ & - \hat{\phi}'(1) [\gamma_1'(1) v_i(1) + \gamma_1(1) v'_i(1)] + \hat{\phi}'(-1) [\gamma_1'(-1) v_i(-1) + \gamma_1(-1) v'_i(-1)] \\ & + \left(\hat{\phi}', \gamma_1'' v_i + \gamma_1' v'_i \right) + \left(\hat{\phi}', \gamma_1' v'_i + \gamma_1 v''_i \right). \end{aligned} \quad (\text{C11})$$

648 The above expression gets simplified using $v_i(\pm 1) = 0$ and $v'_i(\pm 1) = 0$ as

$$649 \quad \left(\hat{\phi}''' \gamma_1, v_i \right) = \left(\hat{\phi}', \gamma_1'' v_i + \gamma_1' v'_i \right) + \left(\hat{\phi}', \gamma_1' v'_i + \gamma_1 v''_i \right). \quad (\text{C12})$$

650 Similarly, the second-order term in equation (C1) is simplified to

$$651 \quad \left(\hat{\phi}'' \gamma_2, v_i \right) = - \left(\hat{\phi}', \gamma_2' v_i + \gamma_2 v'_i \right). \quad (\text{C13})$$

652 Using equations (C12) and (C13) and the property of symmetry for the inner product
653 $((f, g) = (g, f))$, the weak formulation becomes

$$654 \quad \left(\gamma_1'' v_i + 2\gamma_1' v'_i + \gamma_1 v''_i - \gamma_2' v_i - \gamma_2 v'_i + \gamma_3 v_i, \hat{\phi}' \right) + \left(\gamma_4 v_i, \hat{\phi} \right) = \hat{\sigma} \left(\gamma_5 v_i, \hat{\phi}' \right). \quad (\text{C14})$$

655 The final form of the weak formulation in terms of trial (u_j) and test functions (v_i)
656 is obtained as

$$657 \quad \sum_{j=-1, N} \left(\gamma_1'' v_i + 2\gamma_1' v'_i + \gamma_1 v''_i - \gamma_2' v_i - \gamma_2 v'_i + \gamma_3 v_i, u'_j \right) \alpha_j + \left(\gamma_4 v_i, u_j \right) \alpha_j = \sum_{j=-1}^{j=N} \hat{\sigma} \left(\gamma_5 v_i, u'_j \right) \alpha_j, \quad (\text{C15})$$

658 for $i = 1, N$. The system shown by equation (C15) consists of N equations with $N +$
659 2 unknowns ($\alpha_j, j \in [-1, N]$). This can also be represented in matrix notation as $\mathbf{A}\boldsymbol{\alpha} =$
660 $\hat{\sigma}\mathbf{B}\boldsymbol{\alpha}$, where the matrix entries can be written as

$$661 \quad A_{ij} = \sum_{k=0}^K \left[\left(\left(\gamma_1''(\hat{s}_k) v_i(\hat{s}_k) + 2\gamma_1'(\hat{s}_k) v'_i(\hat{s}_k) + \gamma_1(\hat{s}_k) v''_i(\hat{s}_k) - \gamma_2'(\hat{s}_k) v_i(\hat{s}_k) \right. \right. \right. \\ 662 \quad \left. \left. \left. - \gamma_2(\hat{s}_k) v'_i(\hat{s}_k) + \gamma_3(\hat{s}_k) v_i(\hat{s}_k) \right) u'_j(\hat{s}_k) + \gamma_4(\hat{s}_k) v_i(\hat{s}_k) u_j(\hat{s}_k) \right) w_k \right], \quad (\text{C16})$$

$$663 \quad B_{ij} = \sum_{k=0}^K \left(\gamma_5(\hat{s}_k) v_i(\hat{s}_k) u'_j(\hat{s}_k) w_k \right). \quad (\text{C17})$$

664 The next two equations are obtained by imposing boundary conditions $\phi''(-1) = 0$ and
665 $\phi'(1) = 0$ in the strong form. Using the trial functions (and $u'_0(1) = u''_{-1}(-1) = 0$),
666 we can write

$$667 \quad u''_0(-1) \alpha_0 + u''_1(-1) \alpha_1 + u''_2(-1) \alpha_2 + \dots = 0, \quad (\text{C18})$$

$$668 \quad u'_{-1}(1) \alpha_{-1} + u'_1(1) \alpha_1 + u'_2(1) \alpha_2 + \dots = 0. \quad (\text{C19})$$

669 The relationship between the coefficients for the imposed boundary conditions can
670 be obtained as

$$671 \quad \alpha_0 = -\frac{u_1''(-1)}{u_0''(-1)}\alpha_1 - \frac{u_2''(-1)}{u_0''(-1)}\alpha_2 - \dots = p_1\alpha_1 + p_2\alpha_2 + \dots \quad (\text{C20})$$

$$672 \quad \alpha_{-1} = -\frac{u_1'(1)}{u_{-1}'(1)}\alpha_1 - \frac{u_2'(1)}{u_{-1}'(1)}\alpha_2 - \dots = q_1\alpha_1 + q_2\alpha_2 + \dots \quad (\text{C21})$$

673 with $p_j = -\frac{u_j''(-1)}{u_0''(-1)} = -\frac{u_j''(-1)}{-1/2}$ and $q_j = -\frac{u_j'(1)}{u_{-1}'(1)} = -\frac{u_j'(1)}{1/2}$ for $j = 1, N$.

674 Applying this relation among the coefficients, the modified left-hand and right-hand
675 matrix entries read

$$676 \quad A'_{i,j} = A_{i,0}p_j + A_{i,-1}q_j + A_{i,j}, \quad B'_{i,j} = B_{i,0}p_j + B_{i,-1}q_j + B_{i,j}, \quad (i, j \in [1, N]). \quad (\text{C22})$$

677 The algebraic system, $\mathbf{A}'\boldsymbol{\alpha} = \hat{\sigma}\mathbf{B}'\boldsymbol{\alpha}$, now consists of N equations in N unknowns
678 ($\alpha_j, j \in [1, N]$), which can be solved as a generalized eigenvalue problem to compute
679 the growth rate ($\hat{\sigma}$) for different values of \hat{k} and $\mathcal{C}_{\mathcal{I}}$.

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