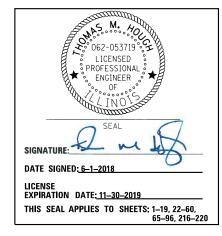
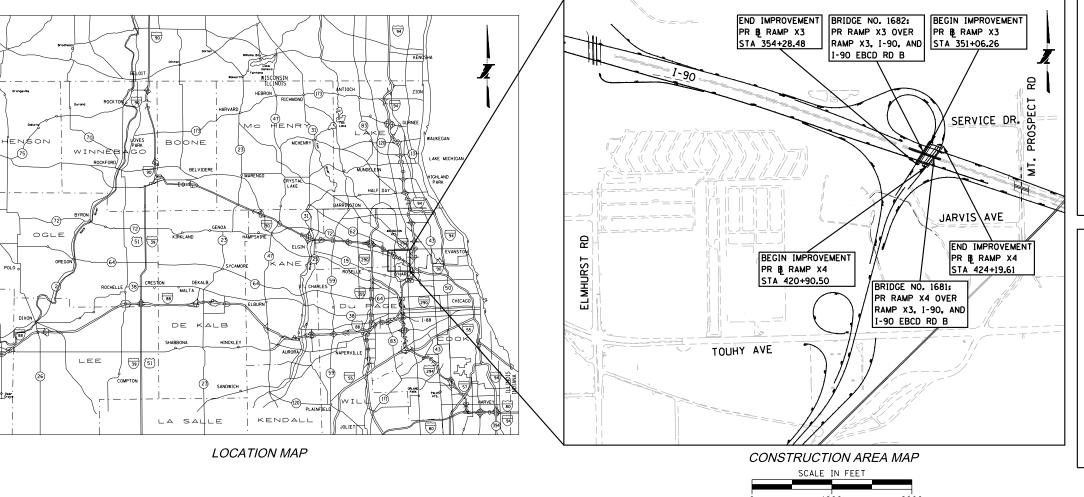


# THE ILLINOIS STATE TOLL HIGHWAY AUTHORITY CONTRACT I-18-4694

ELGIN O'HARE WESTERN ACCESS TOLLWAY (I-490) BRIDGE CONSTRUCTION AND BUILDING DEMOLITION AT JANE ADDAMS MEMORIAL TOLLWAY (I-90)

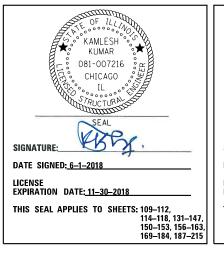
I-90 FROM ELMHURST ROAD TO MOUNT PROSPECT ROAD
I-90 MILE POST 73.5 TO MILE POST 74.4
I-490 MILE POST 6.25

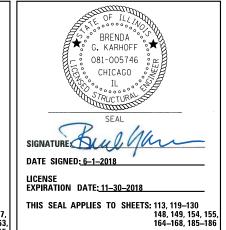












# **INDEX OF DRAWINGS**

DRAWIN	1G	NO.	SHEET	NO.	SHEET DESCRIPTION
		1		COV-1	COVER SHEET
		2		IND-01	INDEX OF DRAWINGS
3	-	4	GEN-01 -	GEN-02	GENERAL NOTES & ROADWAY JURISDICTIONS
5	-	6	GEN-03	GEN-04	SUGGESTED PROGRESS SCHEDULE
7	-	10	S00-01 -	S0Q-04	SUMMARY OF QUANTITIES
		11		EWS-1	EARTHWORK SCHEDULE
12	-	21	ALG-01 -	ALG-10	ALIGNMENT PLANS
22	-	23	TYP-01 -	TYP-02	TYPICAL SECTIONS
24	-	37	MOT-01 -	MOT-14	MAINTENANCE OF TRAFFIC
38	-	39	REM-01 -	REM-02	ROADWAY REMOVALS
40	-	44	REM-03 -	REM-07	BUILDING REMOVALS
45	-	46	TBF-01 -	TBF-02	TRENCH BACKFILL PLANS
		47		PVC-01	PAVEMENT CORE SUMMARY
48	-	49	EW-01 -	EW-02	EARTHWORK PLANS
50	-	53	RDW-01 -	RDW-04	PROPOSED ROADWAY
54	-	56	PRF-01 -		PROPOSED PROFILE
57	-	60	DET-01 -	DET-04	ROADWAY DETAILS
61	-	64	UTL-01 -	UTL-04	EXISTING UTILITY PLANS
65	-	67	DRN-01 -	DRN-03	DRAINAGE PLANS
68	-	84	PVM-01 -	SGN-02	PROPOSED PAVEMENT MARKING & SIGNING
85	-	86	GRD-01 -	GRD-02	PROPOSED GRADING PLANS
87	-	92	BSM-01 -	BSM-06	BORROW SITE PLANS
93	-	96	ERC-01 -	ERC-04	EROSION & SEDIMENT CONTROL PLANS
97	-	102	GN-01 -	EL-05	LIGHTING PLANS
103	-	108	ITS-01 -	ITS-06	ITS PLANS
109	-	215	S-01 -	S-107	STRUCTURAL PLANS
216	-	220	XSC-01 -	XSC-05	CROSS SECTIONS

# **IDOT HIGHWAY STANDARDS**

### STANDARD NO. DESCRIPTION

STANDARD SYMBOLS, ABBREVIATIONS AND PATTERNS 000001-06 001006 DECIMAL OF AN INCH AND OF A FOOT 602001-02 CATCH BASIN TYPE A DRAINAGE STRUCTURES TYPES 4, 5 & 6 602106-01 602601-05 PRECAST REINFORCED CONCRETE FLAT SLAB TOP 604001-04 FRAME AND LIDS TYPE 1 604036-03 GRATE TYPE 8 TRAFFIC CONTROL DEVICES 701901-07 704001-08 TEMPORARY CONCRETE BARRIER 720001-01 SIGN PANEL MOUNTING DETAILS GUARDRAIL AND BARRIER WALL REFLECTOR MOUNTING DETAILS 782006 720011-01 METAL POSTS FOR SIGNS, MARKERS & DELINEATORS

# **TOLLWAY STANDARD DRAWINGS**

### DRAWING NO. DESCRIPTION

SECTION B B1-08 GUTTER AND CURB DETAILS B10-09 SLOPED HEADWALLS TYPE III DETAILS FRAME AND GRATE TYPE 20A B25-01 SECTION C C5-05 CONCRETE BARRIER BASE AND CONCRETE BARRIER, DOUBLE FACE, 42" AND VARIABLE HEIGHT C13-04 CONCRETE MEDIAN BARRIER TRANSITION, TYPE V-F AT BRIDGE PIERS SECTION D D1-05 RIGHT OF WAY FENCE D2-04 SYMBOLS AND PATTERNS

D4-06 ROADWAY DELINEATORS AND REFLECTORS D5-06 PERMANENT PAVEMENT MARKINGS D6-07

PAVEMENT MARKING AND SHOULDER RUMBLE STRIP DETAILS

SECTION E

E1-06 CONSTRUCTION SIGNS LANE CLOSURE DETAILS E2-07 E3-06 SHOULDER CLOSURE DETAILS E5-07 TEMPORARY GORE DETAILS

SECTION F F1-08

OVERHEAD SIGN STRUCTURE SPAN TYPE STRUCTURE DETAILS

SECTION H LIGHTING STANDARD FOUNDATION LIGHTING STANDARD DETAILS H2-06 H3-05 BRIDGE CONDUIT DETAILS

H9-01 UNDERPASS LIGHTING INSTALLATION DETAILS

SECTION K K1-07

TEMPORARY EROSION AND SEDIMENT CONTROLS

DRAWN BY ZAG

CHECKED BY BRH

DATE 06/12/18 DATE 06/12/18 \*\*exp. Chicago, IL BUILDINGS

exp U.S. Services Inc. BUILDINGS • EARTH & ENVIRONMENT • ENERGY INDUSTRIAL • INFRASTRUCTURE • SUSTAINABILITY



		REVISIONS	CONTRACT NO. I-18-4694	IND-01
NO.	DATE	DESCRIPTION	CUNTRACT NO. 1-10-4634	IND-01
			INDEX OF DRAWINGS	DRAWING NO.
				2 220
			AND STANDARDS	2 of 220

# **GENERAL NOTES**

- 1. GENERAL SAFETY PROVISIONS: TO PROVIDE ILLINOIS TOLLWAY AND CROSSROAD PATRONS SAFE TRAVEL CONDITIONS DURING THIS CONSTRUCTION PROJECT, AND TO PROVIDE SAFE WORKING CONDITIONS FOR ALL EMPLOYEES, BOTH OF THE ILLINOIS TOLLWAY AND PRIVATE CONTRACTOR, THE RULES, REGULATIONS, AND CONDITIONS WILL PREVAIL FOR THE DURATION OF THIS CONTRACT.
- 2. THE CONTRACTOR SHALL BE MADE AWARE THAT ALL CONSTRUCTION VEHICLES SHALL BE LIMITED TO 15 FEET ABOVE EXISTING GRADE WHILE CROSSING UNDER COMMONWEALTH EDISON'S
- 3. DISTRIBUTORS: ALL DISTRIBUTORS FOR ASPHALT PAVING OPERATIONS SHALL BE EQUIPPED WITH SHIELDS TO PREVENT DAMAGES TO MOTORISTS' VEHICLES AND TO ADJACENT HIGHWAY APPURTENANCES.
- 4. FENCE: EXISTING FENCE THAT HAS TO BE DISCONNECTED AND/OR REMOVED FOR THE CONTRACTOR'S OPERATION SHALL BE RECONNECTED AND / OR REPLACED BY THE CONTRACTOR IN KIND AT NO ADDITIONAL COST TO THE ILLINOIS TOLLWAY. TEMPORARY FENCE SHOULD BE INSTALLED IF EXISTING FENCE IS TO BE REMOVED BY THE CONTRACTOR IN ACCORDANCE WITH SECTION 664 OF THE STANDARD SPECIFICATIONS. ANY ROW-OF-WAY MARKERS DISTURBED BY HE CONTRACTOR'S OPERATION SHOULD BE REESTABLISHED BY A REGISTERED LAND SURVEYOR AT NO ADDITIONAL COST TO THE ILLINOIS TOLLWAY.
- 5. THE SCALE SHOWN ON THE DRAWINGS APPLIES ONLY TO FULL SIZE PLANS AND NOT TO THE REDUCED SIZE PLANS.
- 6. ALL ELEVATIONS ARE BASED ON UNITED STATES COAST AND GEODETIC SURVEY DATUM. BENCHMARKS FOR THE PROJECT ARE DESCRIBED IN THE PLANS.
- 7. FOR EXISTING PAVEMENT MARKING REMOVALS, SEE SHEETS PVM-05 TO PVM-15.

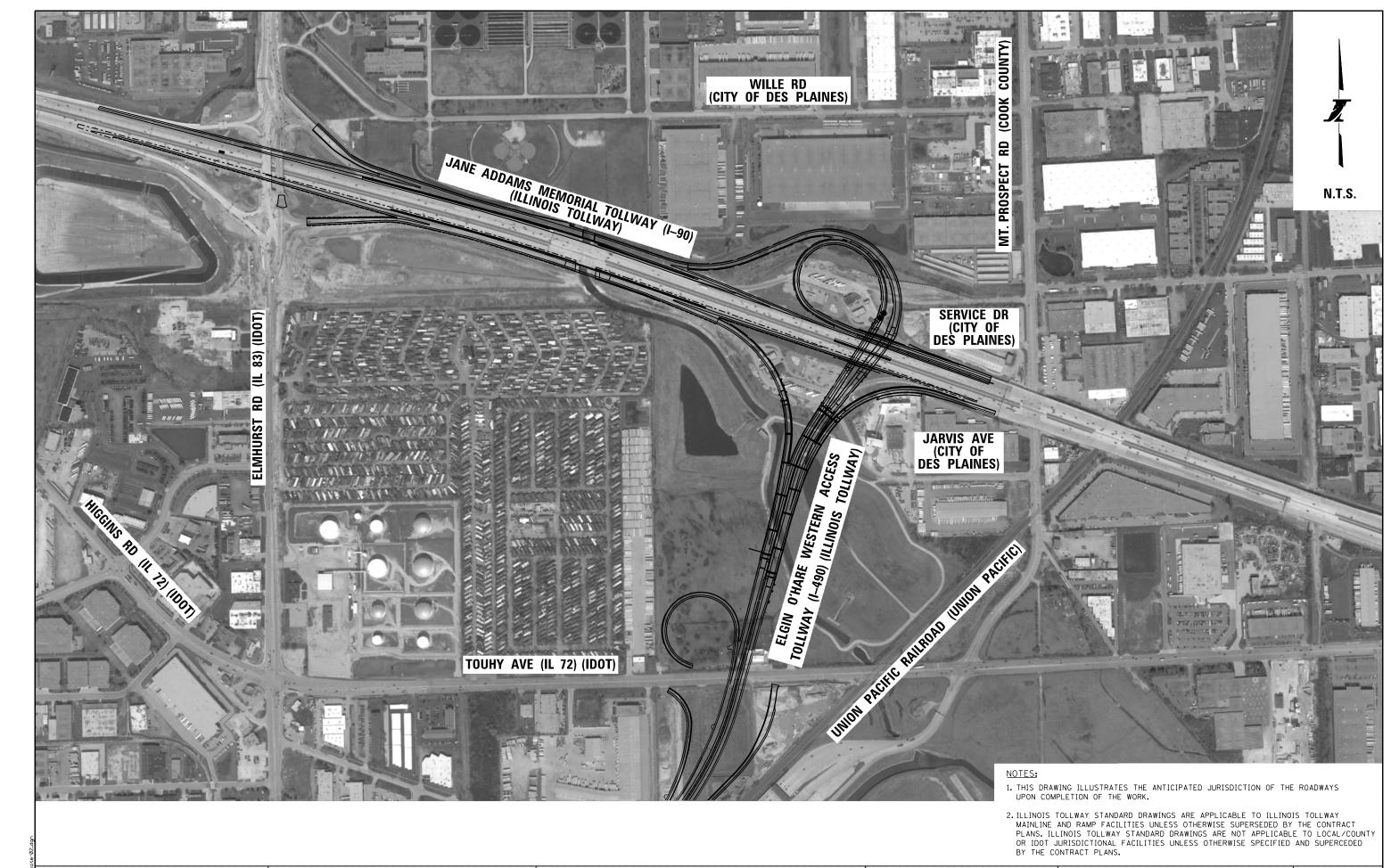
DRAWN BY ZAG DATE 06/12/18 DATE 06/12/18 CHECKED BY BRH



exp U.S. SCIVISES
Chicago, IL
BUILDINGS-EARTH & ENVIRONMENT-ENERGY
INDUSTRIAL-INFRASTRUCTURE-SUSTAINABILITY exp U.S. Services Inc.



		REVISIONS	CONTRACT NO. I-18-4694	GEN-01
NO.	DATE	DESCRIPTION	CONTRACT NO. 1-10-4694	GEN-OI
			GENERAL NOTES	DRAWING NO.
			OLIVERAL NOTES	7 220
				) of 220



CHECKED BY BRH

DATE 06/12/18 DATE 06/12/18 **\***exp.

exp U.S. Services Inc. BUILDINGS • EARTH & ENVIRONMENT • ENERGY INDUSTRIAL • INFRASTRUCTURE • SUSTAINABILITY THE ILLINOIS STATE TOLL HIGHWAY AUTHORITY 2700 OGDEN AVENUE DOWNERS GROVE, ILLINOIS 60515

DESCRIPTION

CONTRACT NO. I-18-4694 GEN-02 ROADWAY JURISDICTIONS

DRAWING NO. 4 of 220

ID	Task Name	Duration	Start	Finish	A 14.0	C 110	O-+ 140 N 140	D 110	1 110	F-1-110	M110	A 14.0	NA 14.0	I 110	1	A 110	C 110	0-+110	N110
1	Contract I-18-4694	370 days	Mon 8/27/18	Thu 10/31/19		Sep 18	Oct '18 Nov '18	Dec 18	Jan 19	Feb 19	Mar 19	Apr 19	May 19	Jun 19	Jul 19	Aug 19	Sep 19	Oct 19	NOV 19
2	Milestones	-		Thu 10/31/19													<del></del>		
3	Notice to Proceed (S.P. 104)	0 days	Mon 8/27/18	Mon 8/27/18	4	8/27													
4	Interim Completion Date: Sign Foundation (S.P. 103.3)	0 days	Wed 10/31/18	Wed 10/31/18			10/31												
5	Interim Completion Date: Stage 1 (S.P. 103.4)	0 days	Fri 12/14/18	Fri 12/14/18				♦ 12/	14										
6	Interim Commencement Date: Stage 2 (S.P. 104.1)	0 days	Fri 3/1/19	Fri 3/1/19						<b>-</b>	3/1								
7	Interim Completion Date: Building Demo (S.P. 103.5)	0 days	Tue 4/30/19	Tue 4/30/19								,	♦ 4/30						
8	Interim Completion Date: S. Abutments (S.P. 103.6)	0 days	Fri 5/31/19	Fri 5/31/19									4	5/31					
9	Interim Commencement Date: Slopewall (S.P. 104.2)	0 days	Mon 9/2/19	Mon 9/2/19													9/2		
10	Substantial Completion Date (S.P. 103.2)	0 days	Mon 9/30/19	Mon 9/30/19													4	9/30	
11	Contract Completion Date (S.P. 103.1)	0 days	Thu 10/31/19	Thu 10/31/19														4	10/31
12	Traffic Control and Protection	324 days	Tue 9/18/18	Mon 9/30/19		9/18												9/30	
13	<b>Temporary Erosion and Sediment Control</b>	316 days	Thu 9/27/18	Mon 9/30/19	=	9/27												9/30	
14	Permits and Mobilization	34 days	Mon 8/27/18	Thu 10/4/18	ı	+	7												
15	Mobilization	19 days	Mon 8/27/18	Mon 9/17/18	8/27	9/	17												
16	Permits Procurement	27 days	Mon 8/27/18	Wed 9/26/18	8/27		9/26												
17	Submittals and Fabrication	156 days	Mon 8/27/18	Sat 2/23/19	ı														
18	Shop Drawings	53 days	Mon 8/27/18	Fri 10/26/18	8/27	•	10/26												
19	Girder Fabrication	103 days	Sat 10/27/18	Sat 2/23/19			10/27				2/23								
20	Stage 1 - I-90 Median Work	76 days	Tue 9/18/18	Fri 12/14/18		<u> </u>		$\neg$											
21	Install Stage 1 Traffic Control	3 days	Tue 9/18/18	Thu 9/20/18		9/18 9	/20												
22	Disconnect Ex. I-90 Median FO Splices (By Others)	2 days	Fri 9/21/18	Sat 9/22/18		9/21	/22												
23	Remove, Test and Salvage I-90 Median FO Cable	2 days	Mon 9/24/18	Tue 9/25/18		9/24	9/25												
24	Median Sign Structures (M.P. 73.5)	31 days	Wed 9/26/18	Wed 10/31/18		r													
25	Median Sign Structure Foundation & Transitions	24 days	Wed 9/26/18	Tue 10/23/18		9/26	10/23	h											
26	Shoulders at Median Sign Structure Foundation	5 days	Wed 10/24/18	Mon 10/29/18			10/24 🕌 10/29												
27	Remove Traffic Control	2 days	Tue 10/30/18	Wed 10/31/18			10/30 10/31												
28	Ramp X3 & X4 Bridges over I-90 (BN 1681 & 1682)	68 days	Thu 9/27/18	Fri 12/14/18		r		$\vdash$											
29	Removal for Median Piers, TSRS and Excavation	12 days	Thu 9/27/18	Wed 10/10/18		9/27	10/10												
30	Test Piles	4 days	Thu 10/11/18	Mon 10/15/18			11 👗 10/15												
31	Procure & Drive Pier Piles	18 days	Tue 10/16/18	Mon 11/5/18		10	11/5												
32	Pier Foundations and Crashwalls	12 days	Tue 11/6/18	Mon 11/19/18			11/6	/19											
33	Median Barrier Wall Transitions	12 days	Tue 11/20/18	Mon 12/3/18			11/20												
34	Reconstruct Shoulders	6 days	Tue 12/4/18	Mon 12/10/18				12/10	)										
35	Pier Columns and Cap	14 days	Tue 11/20/18	Wed 12/5/18			11/20	12/5											
36	Furnish , Install and Test I-90 Median FO Cable	2 days	Tue 12/4/18	Wed 12/5/18			12/4	12/5											
37	Splice I-90 Median FO Cable (By Others)	6 days	Thu 12/6/18	Wed 12/12/18			12/6	12/1	2										

### NOTES:

- 1. THIS IS ONLY A SUGGESTED PROJECT SCHEDULE AND IS NOT TO BE CONSIDERED THE PROGRESS SCHEDULE AS REQUIRED IN TOLLWAY SUPPLEMENTAL SPECIFICATIONS ARTICLE 108.02. THE INTENT OF THIS SUGGESTED PROGRESS SCHEDULE IS TO ILLUSTRATE THE WORK CAN REASONABLY BE PERFORMED WITHIN THE SUGGESTED SCHEDULE DURATION.
- 2. IF ANY DISCREPANCIES EXIST BETWEEN THIS SUGGESTED PROGRESS SCHEDULE AND THE SPECIFICATIONS,
  SPECIAL PROVISIONS OR OTHER CONTRACT DRAWINGS, THE SPECIFICATIONS, SPECIAL PROVISIONS OR OTHER
  CONTRACT DRAWINGS SIALL COVERN
- CONTRACT DRAWINGS SHALL GOVERN.
  3. IT IS THE CONTRACTOR'S RESPONSIBILITY TO PROVIDE MANPOWER AND EQUIPMENT TO MEET THE REQUIREMENTS OF THE CONTRACT DOCUMENTS.

DRAWN BY MDN DATE 06/12/18
CHECKED BY BRH DATE 06/12/18





		REVISIONS	CONTRACT NO. I-18-4694	GEN-03
NO.	DATE	DESCRIPTION	CONTRACT NO. 1-10-4634	GEN-03
			SUGGESTED PROGRESS SCHEDULE	DRAWING NO.
			SOCOLSTED TROOKESS SCHEDOLL	5 220
				5 of 220
				•

ID	Task Name	Duration	Start	Finish	Aug '18	Sep '18	Oct '18   Nov '18	Dec '18	Jan '19	Feb '19	Mar '19	Apr '19	May '19	Jun '19	Jul '19	Aug '19	Sep '19	Oct '19	Nov '19
38	Remove Remaining Stage 1 Traffic Control	2 days	Thu 12/13/18	Fri 12/14/18			12	/13 12/	14										
39	Winter Period	65 days	Sat 12/15/18	Thu 2/28/19	)		1	2/15			2/28								
40	Stage 2 - Remaining Work	210 days	Fri 3/1/19	Thu 10/31/19						i	*								1
41	Install Stage 2 Traffic Control	3 days	Fri 3/1/19	Mon 3/4/19	)					3/1	3/4								
42	Ramp X3 & X4 Bridges over I-90 (BN 1681 & 1682)	173 days	Fri 3/1/19	Wed 9/18/19						1									
43	Test Piles	4 days	Tue 3/5/19	Fri 3/8/19						3/5	3/8								
44	Procure & Drive South Abutment Piles	18 days	Sat 3/9/19	Fri 3/29/19	)					3/	9	3/29							
45	Retaining Wall NW74.41R,EB Construction	20 days	Sat 3/30/19	Mon 4/22/19							3/30		4/22						
46	Construct South Abutment	11 days	Tue 4/23/19	Sat 5/4/19								4/23	5/4						
47	Remove Abandoned Oil Line at N. Abutment	18 days	Fri 3/1/19	Thu 3/21/19						3/1	_3	/21							
48	Drive North Abutment Piles	12 days	Sat 3/30/19	Fri 4/12/19	)						3/30	4/1	.2						
49	Retaining Wall NW74.42R,WB Construction	20 days	Tue 4/23/19	Wed 5/15/19								4/23	5/1	5					
50	Construct North Abutment	11 days	Thu 5/16/19	Tue 5/28/19	)								5/16	5/28					
51	Erect Beams	16 days	Wed 5/29/19	Sat 6/15/19	)								5/29	6/1	5				
52	Construct Superstructure	49 days	Mon 6/17/19	Mon 8/12/19	)								1	6/17		8/12			
53	Approach Slabs	20 days	Tue 8/13/19	Wed 9/4/19											8,	13	9/4		
54	South Abutment Slopewall and Drainage	14 days	Tue 9/3/19	Wed 9/18/19												9/3	9/	18	
55	Bridge Deck Grooving	10 days	Thu 9/5/19	Mon 9/16/19	)											9/5	9/1	16	
56	<b>Des Plaines Oasis Demolition and Site Restoration</b>	59 days	Fri 3/1/19	Wed 5/8/19						1			$\dashv$						
57	Close Des Plaines Oasis	1 day	Fri 3/1/19	Fri 3/1/19						3/1	3/1								
58	I-90 Pavement Marking and Signage	4 days	Sat 3/2/19	Wed 3/6/19	)					3/2	3/6								
59	<b>Building Demolition and Removal</b>	44 days	Sat 3/2/19	Mon 4/22/19						3/2		7	4/22						
60	Oasis Final Grading	7 days	Tue 4/23/19	Tue 4/30/19	)							4/23	4/30						
61	Seeding	7 days	Wed 5/1/19	Wed 5/8/19	)							5/1	5/8						
62	Final Grading and Seeding	10 days	Tue 9/17/19	Fri 9/27/19	)											9	/17	9/27	
63	Remove Stage 2 Traffic Control	2 days	Sat 9/28/19	Mon 9/30/19													9/28	9/30	
64	Clean-Up and Punch List	27 days	Tue 10/1/19	Thu 10/31/19													10/1		10/31
65	Holidays																		
66	Labor Day 2018	1 day	Mon 9/3/18	Mon 9/3/18	9/3	<sub> </sub> 9/3													
67	Thanksgiving 2018	1 day	Thu 11/22/18	Thu 11/22/18	В		11/22												
68	Christmas 2018	1 day	Tue 12/25/18	Tue 12/25/18	3			12/25											
69	New Year's Day 2019	1 day	Tue 1/1/19	Tue 1/1/19	)			1/1	1/1										
70	Easter 2018	1 day	Mon 4/22/19	Mon 4/22/19								4/22	4/22						
71	Memorial Day 2019	1 day	Mon 5/27/19	Mon 5/27/19									5/27	5/27					
72	Independence Day 2019	1 day	Thu 7/4/19	Thu 7/4/19	)									7/4	<sub>1</sub> 7/4				
73	Labor Day 2019	1 day	Mon 9/2/19	Mon 9/2/19												9/2	∣ 9/2		

### NOTE:

1. FOR NOTES SEE SHT. GEN-03.

DRAWN BY MDN DATE 06/12/18

CHECKED BY BRH DATE 06/12/18





		REVISIONS	CONTRACT NO. I-18-4694	GEN-04
10.	DATE	DESCRIPTION	CUNTRACT NO. 1-10-4634	GEN-04
			SUGGESTED PROGRESS SCHEDULE	DRAWING NO.
-			3000E31EB TROOKE33 SCHEBBEE	6 of 220

SP	ITEM NO.	DESCRIPTION	UNIT	QTY.	RECORD OTY.
	20101400	NITROGEN FERTILIZER NUTRIENT	POUND	576	
	20101600	POTASSIUM FERTILIZER NUTRIENT	POUND	1,728	
*	20200100	EARTH EXCAVATION	CU YD	1,895	
*	20400100	BORROW EXCAVATION	CU YD	15,580	
	20800150	TRENCH BACKFILL	CU YD	3,167	
	21001000	GEOTECHNICAL FABRIC FOR GROUND STABILIZATION	SQ YD	389	
	25100630	EROSION CONTROL BLANKET	SQ YD	93,082	
	25100900	TURF REINFORCEMENT MAT	SQ YD	160	
	28100107	STONE RIPRAP, CLASS A4	SQ YD	15	
	28200200	FILTER FABRIC	SQ YD	145	
*	30201500	LIME	TON	248	
	42001300	PROTECTIVE COAT	SQ YD	556	
	44004250	PAVED SHOULDER REMOVAL	SQ YD	785	
	44213200	SAW CUTS	FOOT	964	
	50157300	PROTECTIVE SHIELD	SQ YD	3,562	
	50200100	STRUCTURE EXCAVATION	CU YD	3,764	
	50200450	REMOVAL AND DISPOSAL OF UNSUITABLE MATERIAL FOR STRUCTURES	CU YD	1,643	
	50300225	CONCRETE STRUCTURES	CU YD	839.6	
	50300255	CONCRETE SUPERSTRUCTURE	CU YD	177.7	
	50300260	BRIDGE DECK GROOVING	SQ YD	3,291	
	50300300	PROTECTIVE COAT	SQ YD	4,114	
*	50401340	FURNISHING AND ERECTING PRECAST PRESTRESSED CONCRETE BEAMS, IL63	FOOT	4,231	
	50800205	REINFORCEMENT BARS, EPOXY COATED	POUND	553,030	
	51100100	SLOPE WALL 4 INCH	SQ YD	341	
	51201400	FURNISHING STEEL PILES HP10X42	FOOT	1,900	
	51201900	FURNISHING STEEL PILES HP14X89	FOOT	9,892	
	51202305	DRIVING PILES	FOOT	11,792	
	51203400	TEST PILE STEEL HP10X42	EACH	4	
	51203900	TEST PILE STEEL HP14X89	EACH	6	

SP	ITEM NO.	DESCRIPTION	UNIT	OTY.	RECORD QTY.
	51204650	PILE SHOES	EACH	165	
	52100540	ANCHOR BOLTS, 1 1/2"	EACH	8	
	52200020	TEMPORARY SOIL RETENTION SYSTEM	SQ FT	1,610	
*	550A0380	STORM SEWERS, CLASS A, TYPE 2 18"	FOOT	212	
	55100700	STORM SEWER REMOVAL 15"	FOOT	14	
	33100100	STORM SEREN REMOVAL 13	1001	1.	
	55100900	STORM SEWER REMOVAL 18"	FOOT	4	
	58700300	CONCRETE SEALER	SQ FT	9,505	
	E0100100	CEOCOMPOSITE WALL DRAIN	CO VD	75.4	
	59100100	GEOCOMPOSITE WALL DRAIN	SQ YD	354	
	60200805	CATCH BASINS, TYPE A, 4'-DIAMETER, TYPE 8 GRATE	EACH	2	
	63200310	GUARDRAIL REMOVAL	FOOT	248	
	66900105	UNDERGROUND STORAGE TANK REMOVAL	EACH	8	
*	66900200	NON-SPECIAL WASTE DISPOSAL	CU YD	2,550	
	66900400	SPECIAL WASTE GROUNDWATER DISPOSAL	GALLON	1,000	
	00300400	STECIAL WASTE GROUNDWATER DISTOSAL	OALLON	1,000	
	66900450	SPECIAL WASTE PLANS AND REPORTS	L SUM	1	
	66900530	SOIL DISPOSAL ANALYSIS	EACH	10	
	66000575	DOLODITY DOLLUTANTS ODOLNOWATED ANALYSIS	FACU.	10	
	66900535	PRIORITY POLLUTANTS GROUNDWATER ANALYSIS	EACH	10	
D1	70200100	NIGHTTIME WORK ZONE LIGHTING	L SUM	1	
	70600280	IMPACT ATTENUATORS, TEMPORARY (SEVERE USE, NARROW), TEST LEVEL 3	EACH	6	
	72400320	REMOVE SIGN PANEL - TYPE 2	SQ FT	52	
	72400330	REMOVE SIGN PANEL - TYPE 3	SQ FT	1,714	
	72800100	TELESCOPING STEEL SIGN SUPPORT	FOOT	210	
	73700100	REMOVE GROUND MOUNTED SIGN SUPPORT	EACH	5	
	73700200	REMOVE CONCRETE FOUNDATION - GROUND MOUNT	EACH	5	
	81800400	AERIAL CABLE, 4-1/C NO. 2 WITH MESSENGER WIRE	FOOT	1,690	
	31300400	ALMAL CAULE, 7 1/0 NO. 2 HITH WESSERVEN WINE	1001	1,030	
*	X0320000	DRAINAGE SYSTEM, NO. 1	EACH	1	
*	X0320002	DRAINAGE SYSTEM, NO. 2	EACH	1	

# <u>S.P. LEGEND</u>

- INDICATES SPECIAL PROVISION
   INDICATES TOLLWAY SUPPLEMENTAL SPECIFICATION
   INDICATES IDOT RECURRING SPECIAL PROVISIONS
  BDE INDICATES IDOT BDE SPECIAL PROVISION
  GBSP INDICATES IDOT GBSP SPECIAL PROVISION
  D1 INDICATES IDOT DISTRICT ONE SPECIAL PROVISION

DRAWN BY JP CHECKED BY MDN

DATE 06/12/18 DATE 06/12/18





CONTRACT	REVISIONS		
CUNTRACT	DESCRIPTION	DATE	NO.
SUMMARY O			
) SOMMAN O			
1			

SP	ITEM NO.	DESCRIPTION	TINU	QTY.	RECORD QTY.
*	X0323389	STORM SEWER CONNECTION	EACH	1	
*	X0327009	REMOVE SIGN (SPECIAL)	EACH	2	
*	X0327357	CONSTRUCTION VIBRATION MONITORING	L SUM	1	
	X5860110	GRANULAR BACKFILL FOR STRUCTURES	CU YD	776	
*	X6640535	CHAIN LINK FENCE, 6' ATTACHED TO STRUCTURE	FOOT	399	
*	Z0007601	BUILDING REMOVAL NO. 1	I CIIM	1	
*	20007801	BUILDING REMOVAL NO. 1	L SUM	1	
*	Z0007602	BUILDING REMOVAL NO. 2	L SUM	1	
*	Z0018000	DRAINAGE SCUPPERS (SPECIAL)	EACH	8	
*	Z0018700	DRAINAGE STRUCTURE TO BE REMOVED	EACH	1	
	20010100	BINALINAGE STRUCTURE TO BE NEWOVED	LACIT		
GBSP	Z0046304	PIPE UNDERDRAINS FOR STRUCTURES 4"	FOOT	1,004	
*	JI202210	REMOVAL AND DISPOSAL OF UNSTABLE MATERIAL	CU YD	958	
*	JI209030	POROUS GRANULAR EMBANKMENT	CU YD	899	
*	JI211110	TOPSOIL EXCAVATION AND PLACEMENT	CU YD	230	
*	JI211160	TOPSOIL STRIPPING AND STOCKPILING	CU YD	5,984	
*	JI213004	EXPLORATION TRENCH, UTILITIES (HAND EXCAVATION)	FOOT	1,000	
	11017006	EVEN ADVITANT TOTAL VITA VITA VIVA VIA VIVA VIVA VIVA VIVA	5007	1.000	
*	JI213006	EXPLORATION TRENCH, UTILITIES (VACUUM EXCAVATION)	FOOT	1,000	
*	JI406107	ASPHALT TACK COAT	POUND	353	
*	JI420040	BRIDGE APPROACH SLAB	SQ YD	818	
*	JI440010	CONCRETE MEDIAN BARRIER AND BASE REMOVAL	FOOT	456	
*	31440010	CONCRETE WEDIAN DANNIEN AND DASE NEWOVAL	1001	430	
*	JI482112	WARM-MIX ASPHALT SHOULDERS (9 IN.)	SQ YD	785	
*	JI485020	TEMPORARY PAVEMENT, CLASS 2	SQ YD	155	
*	JI503010	HIGH PERFORMANCE CONCRETE SUPERSTRUCTURE	CU YD	1,202.4	
*	JI522500	MECHANICALLY STABILIZED EARTH RETAINING WALL	SQ FT	10,255	
*	JI599040	FORM LINER TEXTURED SURFACE (SPECIAL)	SQ FT	1,265	
*	JI601298	PIPE UNDERDRAINS, 4" (SPECIAL)	FOOT	103	
	11000740	DRAINAGE CIDUCTURES TYRE A WITH TWO TYRE COA FRANCE AND CRAFF	5400	4	
*	JI602740	DRAINAGE STRUCTURES, TYPE 4 WITH TWO TYPE 20A FRAME AND GRATE	EACH	1	
*	JI606015	GUTTER, TYPE G-2, MODIFIED	FOOT	302	
*	JI606050	CONCRETE GUTTER (SPECIAL)	FOOT	248	
*	JI637003	CONCRETE BARRIER, DOUBLE FACE, 42 INCH	FOOT	109	
*	21021002	CONGRETE DARRIER, DOUBLE FACE, 42 INCH	F 00 1	103	

SP	ITEM NO.	DESCRIPTION	UNIT	OTY.	RECORD QTY.
*	JI637012	CONCRETE BARRIER TRANSITION	FOOT	40	
*	JI637017	CONCRETE BARRIER BASE (SPECIAL)	FOOT	184	
	01031011	CONCRETE BANNEN BASE NO ECIAEN	1001	10 1	
*	JI637052	CONCRETE BARRIER BASE, 7'	FOOT	109	
*	JI664305	RIGHT-OF-WAY FENCE, TYPE 1, 6'	FOOT	198	
*	JI664310	CORNER POST, RIGHT-OF-WAY FENCE, TYPE 1	EACH	9	
*	JI664335	DOUBLE VEHICLE GATE, RIGHT-OF-WAY FENCE, TYPE 1	EACH	2	
*	JI669200	REMOVE ABANDONED OIL PIPELINE	FOOT	2,861	
	01003200	TEMOTE ABANDONES OF THE FINE	1001	2,001	
*	JI669210	FILL ABANDONED OIL PIPELINE	CU YD	8	
*	JI669220	REMOVE OIL SEPARATOR SYSTEM	EACH	4	
*	JI680030	HEADWALL TYPE III, 18", 1:10	EACH	2	
*	JI680120	SLOPED HEADWALL TYPE III, 6", 1:3	EACH	3	
*	JI704000	TEMPORARY CONCRETE BARRIER	FOOT	2,400.0	
*	JI704005	RELOCATE TEMPORARY CONCRETE BARRIER	FOOT	2,075.0	
*	JI782022	BARRIER WALL REFLECTORS, TYPE C	EACH	112	
	01102022	Difficient while the electronic, the c	Enon	112	
*	JI811282	CONDUIT ATTACHED TO STRUCTURE, 4" DIA., STAINLESS STEEL	FOOT	25	
**	JS107361	APPLY DUST SUPPRESSION AGENTS	UNIT	2,234	
**	JS120100	TRAILER MOUNTED FULL MATRIX PORTABLE CHANGEABLE MESSAGE SIGNS	EACH	9	
**	JS120101	TRAILER MOUNTED FULL MATRIX PORTABLE CHANGEABLE MESSAGE SIGNS	EACH/WEEK	12	
	10120102	TRALIED MOUNTED FILL MATRIX PORTABLE CHANCEARLE MESCACE CIONS	E A CIL (A 40 NITII	70	
**	JS120102	TRAILER MOUNTED FULL MATRIX PORTABLE CHANGEABLE MESSAGE SIGNS	EACH/MONTH	32	
**	JS280020	MANAGEMENT OF EROSION AND SEDIMENT CONTROL	CAL. MO.	14	
**	JS280050	SILT FENCE	FOOT	5,885	
	33230030		. 551	5,000	
**	JS280051	RE-ERECT SILT FENCE	FOOT	1,177	
**	JS280070	STABILIZED CONSTRUCTION ENTRANCE	SQ YD	388	
**	JS280100	SUPER SILT FENCE	FOOT	2,081	
-*	33200100	SULLY SILLY LINCE	1 001	2,001	
**	JS280110	TEMPORARY PIPE SLOPE DRAINS	FOOT	393	
**	JS280140	TEMPORARY RIPRAP	TON	60	

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CONTRACT NO. I-18-4694	REVISIONS			
CUNTRACT NO. 1-10-4634	DESCRIPTION	TE	).	NO.
SUMMARY OF QUANTITIES				

SP	ITEM NO.	DESCRIPTION	UNIT	QTY.	RECORD QTY.
**	JS280180	RECTANGULAR INLET PROTECTION	EACH	6	
**	JS280210	FILTER FABRIC INLET PROTECTION, BASKET TYPE	EACH	5	
**	JS280305	TEMPORARY DITCH CHECKS	FOOT	120	
**	JS670CMO	FIELD OFFICE, TYPE C (MODIFIED)	CAL MO	18	
**	JS671010	MOBILIZATION, TOLLWAY	L SUM	1	
*	JS701010	MAINTENANCE OF TRAFFIC	L SUM	1	
**	JS734A10	FOUNDATION FOR OVERHEAD SIGN STRUCTURE, SPAN TYPE	CU YD	108	
**	JS810879	UNDERGROUND CONDUIT, COILABLE NONMETALLIC CONDUIT, 4" DIA.	FOOT	350	
**	JS811051	CONDUIT ATTACHED TO STRUCTURE, 1 1/2" DIA., PVC COATED GALVANIZED STEEL	FOOT	1,000	
**	JS812021	CONDUIT EMBEDDED IN STRUCTURE, 1" DIA., PVC	FOOT	115	
**	JS812023	CONDUIT EMBEDDED IN STRUCTURE, 2" DIA., PVC	FOOT	710	
**	JS812027	CONDUIT EMBEDDED IN STRUCTURE, 4" DIA., PVC	FOOT	240	
**	JS813001	JUNCTION BOX, STAINLESS STEEL, EMBEDDED IN STRUCTURE, 20" X 12" X 8"	EACH	2	
**	JS813014	JUNCTION BOX, STAINLESS STEEL, EMBEDDED IN STRUCTURE, 40" X 14" X 12"	EACH	2	
**	JS813022	JUNCTION BOX, STAINLESS STEEL, ATTACHED TO STRUCTURE, 6" X 6" X 4"	EACH	11	
**	JS813053	JUNCTION BOX, STAINLESS STEEL, ATTACHED TO STRUCTURE, 12" X 10" X 6"	EACH	12	
**	JS813094	JUNCTION BOX, STAINLESS STEEL, ATTACHED TO STRUCTURE, 24" X 24" X 8"	EACH	1	
**	JS816076	UNIT DUCT, WITH 4-1/C NO. 2 AND 1/C NO. 4 GROUND, 600V (XLP-TYPE USE), 2" DIA. CNC	FOOT	870	
**	JS817211	ELECTRIC CABLE IN CONDUIT, 600V (XLP-TYPE USE) 1/C NO. 10	FOOT	5,060	
**	JS817213	ELECTRIC CABLE IN CONDUIT, 600V (XLP-TYPE USE) 1/C NO. 6	FOOT	120	
*	JS821009	TEMPORARY LUMINAIRE, SODIUM VAPOR, HIGH MAST, HORIZONTAL MOUNT, 750WATT	EACH	4	
**	JS821100	LUMINAIRE, LED, HORIZONTAL MOUNT	EACH	2	
**	JS821110	UNDERPASS LUMINAIRE, LED	EACH	25	
**	JS830015	WALL MOUNTED LIGHT POLE, ALUMINUM, 50 FT., TWO 6 FT. MAST ARMS	EACH	1	
**	JS830033	TEMPORARY WOOD POLE, 70 FT., CLASS 3	EACH	6	
**	JS830043	TEMPORARY WOOD POLE, 90 FT., CLASS 2, 15 FT. MAST ARM	EACH	4	
**	JS836006	LIGHT POLE FOUNDATION (ROADWAY) MEDIAN, TYPE 2	EACH	1	
**	JS842080	REMOVAL OF EXISTING LIGHTING UNIT, SALVAGE	EACH	2	
**	JS846001	MAINTAIN LIGHTING SYSTEM	L SUM	1	

SP	ITEM NO.	DESCRIPTION	UNIT	OTY.	RECORD QTY.
*	JT130700	SOLAR POWERED GENERATOR ASSEMBLY	EACH	1	
*	JT130714	REAIMING MVDS UNITS	EACH	1	
*	JT132830	FIDED ODTIC COMMUNICATIONS ITS ASSEMBLY	FACIL	1	
•	31132830	FIBER OPTIC COMMUNICATIONS, ITS ASSEMBLY	EACH	1	
*	JT134000	MAINTAIN INTELLIGENT TRANSPORTATION SYSTEMS	L SUM	1	
*	JT134005	RELOCATE INTELLIGENT TRANSPORTATION SYSTEM ASSEMBLY	EACH	1	
*	JT134037	ITS ELEMENT SITE GROUNDING - POLE MOUNTED ASSEMBLY	EACH	1	
•	JT134039	ITS ELEMENT SITE GROUNDING - SOLAR POWERED GENERATOR ASSEMBLY	EACH	1	
			5.00		
*	JT135042	WEBCAM	EACH	1	
*	JT154002	DISPOSAL OF UNIDENTIFIED HAZARDOUS WASTE	UNIT	60,000	
*	JT154062	CONTRACT ALLOWANCE FOR MAINTAIN INTELLIGENT TRANSPORTATION SYSTEMS REPAIR	UNIT	35,000	
•	JT154112	ALLOWANCE FOR ADDITIONAL ELECTRICAL AND COMMUNICATION WORK	UNIT	40,000	
*	JT154150	ALLOWANCE FOR STEEL COSTS ADJUSTMENT	UNIT	20,000	
*	JT154160	ALLOWANCE FOR FUEL COSTS ADJUSTMENT	UNIT	20,000	
*	JT154168	ALLOWANCE FOR HAUL ROAD MAINTENANCE	UNIT	40,000	
*	JT155001	CONTRACTOR'S QUALITY PROGRAM	L SUM	1	
*	JT155110	WORKFORCE HIRING INCENTIVE	HOUR	1,000	
*	JT160225	CINCLE MODE FIRED ODITIC CARLE DEMOVAL CALVACE	FOOT	26,400	
•	31160225	SINGLE MODE FIBER OPTIC CABLE REMOVAL, SALVAGE	FOOT	26,400	
*	JT160360	FIBER OPTIC CABLE, SINGLE MODE, NON-ARMORED, 36 FIBERS	FOOT	26,400	
*	JT205010	EMBANKMENT UNDER STRUCTURES	CU YD	321	
•	JT211A08	SUBGRADE AGGREGATE, 9"	CU YD	255	
*	JT250432	SEEDING, CLASS 2E SALT TOLERANT ROADSIDE MIX (SPECIAL)	ACRE	1.1	
*	JT250442	SEEDING CLASS OF NATIVE CRASS LOW DROFTLE MIX (SPECIAL)	ACRE	18.1	
*	31230442	SEEDING, CLASS 4F NATIVE GRASS LOW PROFILE MIX (SPECIAL)	ACRE	10.1	
*	JT512300	PILE CASING, CORRUGATED METAL PIPE, 24"	FOOT	1,143	
•	JT637023	CONCRETE MEDIAN BARRIER TRANSITION, TYPE V-F	FOOT	93	
*	JT637026	CONCRETE PIER PROTECTION BARRIER	FOOT	51	
*	JT701030	SUPPLEMENTAL BARRICADE	EACH/DAY	342	

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		REVISIONS	
NO.	DATE	DESCRIPTION	

SP	ITEM NO.	DESCRIPTION	UNIT	QTY.	RECORD QTY.
*	JT701031	SUPPLEMENTAL SIGNING	SQ FT	80	0112
*	JT701032	SUPPLEMENTAL FLASHING ARROW BOARD (PER DAY)	EACH/DAY	10	
*	JT701033	SUPPLEMENTAL FLASHING ARROW BOARD (PER WEEK)	EACH/WEEK	6	
*	JT701034	SUPPLEMENTAL FLASHING ARROW BOARD (PER MONTH)	EACH/MONTH	2	
*	JT701035	SUPPLEMENTAL MAINTENANCE OF TRAFFIC	DAY	30	
			20.57	7.0	
*	JT701050	TEMPORARY INFORMATION SIGNING-GROUND MOUNT, 24 SO FT IN AREA OR LESS	SQ FT	36	
*	JT720110	SIGN INSTALLATION, TYPE 2	SQ FT	140	
*	JT720120	SIGN INSTALLATION, TYPE 3	SQ FT	155	
	JT780300	MULTI DOLVMED DAVENENT MADVING LINE 40	5001	F 20F	
-	31780300	MULTI-POLYMER PAVEMENT MARKING - LINE 4"	FOOT	5,205	
*	JT780305	MULTI-POLYMER PAVEMENT MARKING - LINE 5"	FOOT	912	
*	JT780320	MULTI-POLYMER PAVEMENT MARKING - LINE 10"	FOOT	1,141	
*	JT780355	MULTI-POLYMER PAVEMENT MARKING - SYMBOLS (LARGE)	SQ FT	125	
-	01100333	MODELLI FOR HIGH MANNING STIMBOES VERICE!	30 11	123	
*	JT780JB1	GROOVING FOR RECESSED PAVEMENT MARKING LINES, 6" GROOVE	FOOT	912	
*	JT783005	WATERBLAST PAVEMENT MARKING REMOVAL WITH VACUUM RECOVERY	SQ FT	12,832	
	JT810873	UNDERGROUND CONDUIT, COILABLE NONMETALLIC CONDUIT, SDR 11, 1 1/4" DIA.	FOOT	320	
	JT810876	UNDERGROUND CONDUIT, COILABLE NONMETALLIC CONDUIT, SDR 11, 2" DIA.	FOOT	440	
*	JT836018	ITS ELEMENT POLE FOUNDATION STEEL HELIX (10 FT)	EACH	2	
*	JT836027	ITS CONCRETE SERVICE PAD, TYPE A	EACH	1	
*	JT900088	SETTLEMENT MONITORING	CAL MO	8	
	31300080	SETTLEMENT WONTTONING	CAL INO	- 0	
*	JT900202	TEMPORARY CONSTRUCTION FENCE	FOOT	1,136	
*	JT900521	EMBANKMENT MODIFICATION	CU YD	2,547	
_					

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		REVISIONS	CONTRACT NO. I-18-4694	S0Q-04
ο.	DATE	DESCRIPTION	CUNTRACT NO. 1-10-4634	300-04
			SUMMARY OF QUANTITIES	DRAWING NO.
			301111111111111111111111111111111111111	10 oF 220
				10 of 220

# **EARTHWORK SCHEDULE**

			TOPSOIL STRIPPING	EARTH EXCAVATION (20200100)	STRUCTURE EXCAVATION (50200100)	NON-SPECIAL WASTI DISPOSAL (66900200)	REMOVAL AND DISPOSAL OF UNSUITABLE MATERIAL (JI202210) (CONTINGENCY - 20%) (SEE NOTE 7)	REMOVAL AND DISPOSAL OF UNSUITABLE MATERIALS FOR STRUCTURES (50200450)	TOPSOIL PLACEMENT	EXCAVATION TO BE USED IN EMBANKMENT (8% SHRINKAGE)	EMBANKMENT REQUIRED	EARTHWORK BALANCE WASTE (+) OR SHORTAGE (-)	BORROW EXCAVATION (20400100) (20% SHRINKAGE)
STA FROM	TO STA	LENGTH	А	В	С	D	E = B • 0.20	F	G	H = (B+C)•(1-,08)	I	J = H - I	K = -J / 0.8
		FT	CUYD	CU YD	CU YD	CU YD	CU YD	CU YD	CU YD	CU YD	CU YD	CU YD	CU YD
I-9	90					•	·						
3890+50.00	3891+00.00	50	-	218	-	-	44	-	-	201	0	201	-
3891+00.00	3891+50.00	50	-	687	-	-	137	-	-	632	0	632	-
3891+50.00	3892+00.00	50	-	1,303	-	-	261	-	-	1,198	0	1,198	-
3892+00.00	3892+50.00	50	-	1,277	-	-	255	-	=	1,175	89	1,086	=
3892+50.00	3893+00.00	50	-	674	-	-	135	-	-	620	479	141	-
3893+00.00	3893+21.00	21	-	113	-	-	23	-	-	104	624	-520	-
3893+21.00	3893+50.00	29	-	56	-	-	11	-	-	51	1,129	-1,077	-
3893+50.00	3894+00.00	50	-	73	-	-	15	-	-	67	2,300	-2,232	-
3894+00.00	3894+68.00	68	-	132	-	-	26	-	-	121	4,504	-4,383	-
3894+68.00	3895+00.00	32	-	67	-	-	13	-	-	61	2,165	-2,103	-
3895+00.00	3895+50.00	50	-	78	-	-	16	-	-	72	2,515	-2,443	-
3895+50.00	3896+00.00	50	-	113	-	-	23	-	-	104	1,245	-1,141	-
3896+00.00	END		-	0	-	-	0	-	-	0	0	0	-
		SUB-TOTAL:	0	4,791	0	0	958	0	0	4,407	15,049	-10,642	13,302
ADDITIONAL MSE WALL N	EARTHWORK I	TEMS	0	0	1,360	0		858	0	1,251	0	1,251	_
MSE WALL N			0	0	2,050	0	-	785	0	1,886	0	1,886	
RAMP X4 BN			0	0	211	0	_	0	0	194	0	194	
RAMP X3 BN			0	0	143	0	_	0	0	132	0	132	
		ABUTMENT (SEE NOTE 3)	1,532	-1,532	0	0	_	0	125	-1,409	-125	-1,284	
		ABUTMENT (SEE NOTE 3)	1,364	-1,364	0	0	-	0	105	-1,255	-105	-1,150	
	EMOVAL NO. 1	7.50 . ME. 1. 13EE 1101E 37	1,616	0	0	0	_	0	0	0	0	0	
	EMOVAL NO. 2		1,473	0	0	0	-	0	0	0	0	0	
		SEPARATORS & ABANDONED PIPELINE	0	0	0	2,550	-	0	0	0	2,850	-2,850	_
2.70 122 1	J 55., 51L .	SUB-TOTAL:	5,984	-2,896	3,764	2,550	0	1,643	230	799	2,620	-1,821	2,276
		CDANG TOTAL	F 004	1.005	7.704	2.550	050	1.647	270	F 205	17.670	12.405	15 500
		GRAND TOTAL:	5,984	1,895	3,764	2,550	958	1,643	230	5,205	17,670	-12,465	15,580

OASIS U	OASIS UNDERGROUND STORAGE TANK, OIL SEPARATORS, ABANDONED PIPELINE REMOVAL & NON-SPECIAL WASTE DISPOSAL BACKFILL (66900200)													
	NO.	EXCAVATED SOIL / EACH (CU YD / EACH)	TOTAL EXCAVATED SOIL (CU YD)	ESTIMATED NON-SPL WASTE DISPOSAL (CU YD) (CONTINGENCY - 50%)	CULVE / FACUS	TOTAL REPLACEMENT VOLUME TO GROUND LINE (CU YD)	POROUS GRANULAR EMBANKMENT VOLUME (JI209030) (CU YD)	EMBANKMENT REQUIRED (CU YD)						
	L	М	N = L • M	0 = 0.5 • N	Р	Q = (N - 0) + (L • P)	R	S = Q - R						
EXCAVATION FOR ABANDONED OIL PIPELINE	1	3,349	3,349	1,674	148	1,822	554	1,269						
EXCAVATION FOR UNDERGROUND STORAGE TANKS	2	768	1,536	768	340	1,448	172	1,275						
EXCAVATION FOR OIL SEPARATORS	4	54	214	107	93	478	172	306						
	TOTAL:	-	5,099	2,550	-	3,748	899	2,850						

-	EARTHWORK BILL OF M	MATERIALS	
PAY ITEM NO.	DESIGNATION	TOTAL (CUYD)	CALCULATION NOTES
20200100	EARTH EXCAVATION	1,895	В
20400100	BORROW EXCAVATION	15,580	WHEN K > O, K
50200100	STRUCTURE EXCAVATION	3,764	SEE STRUCTURE PLANS S-11 & S-12
50200450	REMOVAL AND DISPOSAL OF UNSUITABLE MATERIAL FOR STRUCTURES	1,643	SEE STRUCTURE PLANS S-11 & S-12
66900200	NON-SPECIAL WASTE DISPOSAL	2,550	D
JI202210	REMOVAL AND DISPOSAL OF UNSUITABLE MATERIAL	958	E
JI209030	POROUS GRANULAR EMBANKMENT	899	R
JI211110	TOPSOIL EXCAVATION AND PLACEMENT	230	WHEN G < A, THEN G OR WHEN G > A, THEN A
JI211160	TOPSOIL STRIPPING AND STOCKPILING	5,984	Α

### NOTES

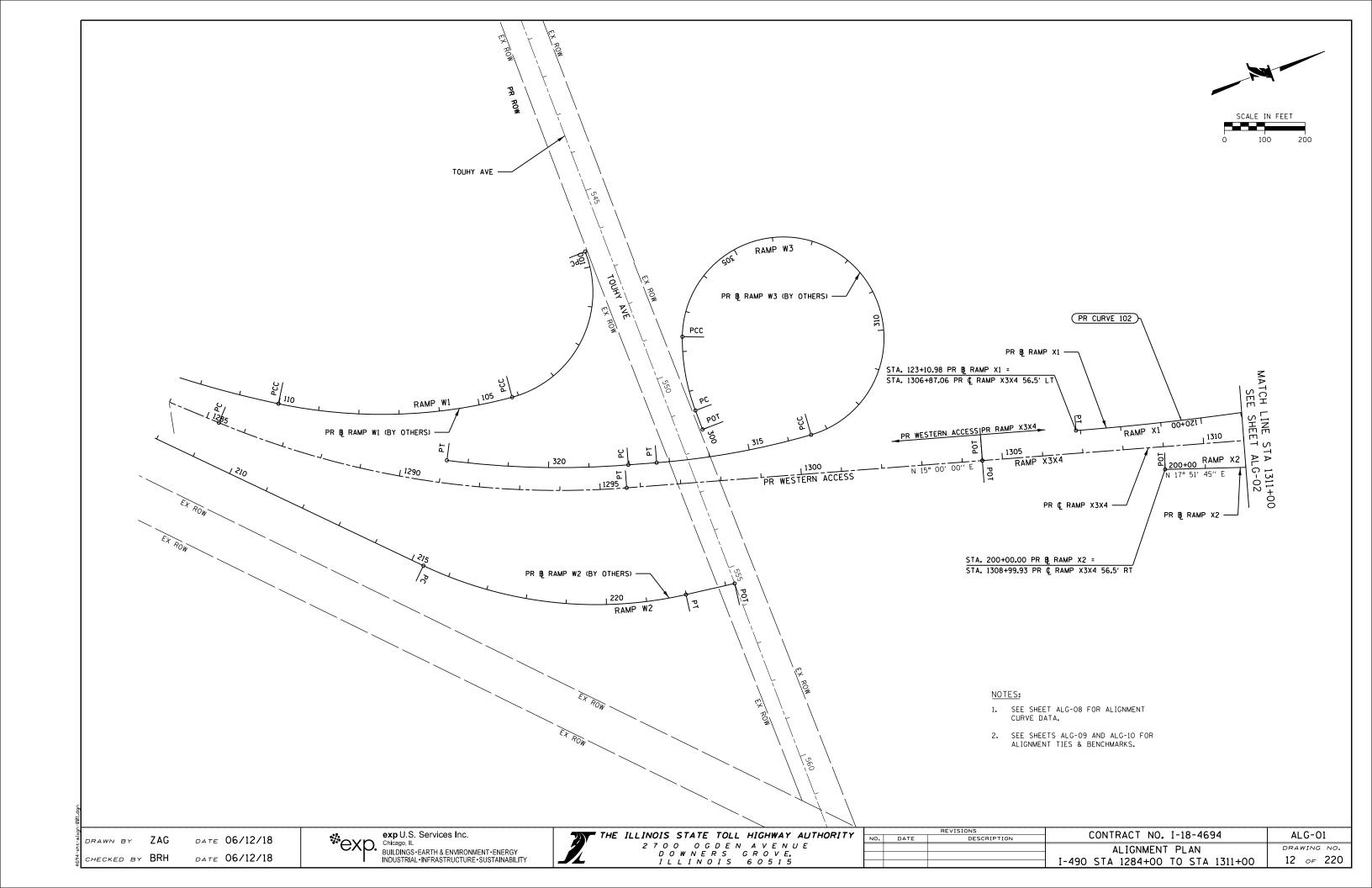
- 1. TOPSOIL WITHIN THE GRADING AND BUILDING DEMO LIMITS SHALL BE STRIPPED AND STOCKPILED AT LOCATIONS SHOWN ON SHEETS ERC-02 AND ERC-03. THIS WORK WILL BE PAID FOR AS TOPSOIL STRIPPING AND STOCKPILING. AN EXISTING TOPSOIL THICKNESS OF 12" WAS USED FOR THE QUANTITY CALCULATIONS.
- 2. TOPSOIL FROM STOCKPILES SHALL BE PLACED AT FINAL LOCATIONS SHOWN ON SHEETS ERC-02 AND ERC-03. THIS WORK WILL BE PAID FOR AS TOPSOIL EXCAVATION AND PLACEMENT.
- 3. I-90 END AREA VOLUMES INCLUDE TOPSOIL. TOPSOIL STRIPPING VOLUMES ARE DEDUCTED FROM EARTH EXCAVATION AND TOPSOIL PLACEMENT VOLUMES ARE DEDUCTED FROM EMBANKMENT REQUIRED.
- 4. UNSUITABLE MATERIAL SHALL BE STOCKPILED AT LOCATIONS SHOWN ON SHEETS ERC-02 AND ERC-03.
- 5. REMOVAL AND DISPOSAL OF UNSUITABLE MATERIAL HAS BEEN INCLUDED IN THE CONTRACT AS A CONTINGENCY, AND HAS BEEN ESTIMATED AS 20% OF THE EARTH EXCAVATION VOLUME. SEE NOTE 7.
- 6. NON-SPECIAL WASTE DISPOSAL HAS BEEN INCLUDED IN THE CONTRACT AS A CONTINGENCY, AND HAS BEEN ESTIMATED AS 50% OF THE EXCAVATED SOILS REQUIRED TO REMOVE ABANDONED OIL PIPELINES, UNDERGROUND STORAGE TANKS AND OIL SEPARATORS.
- 7. REMOVAL AND DISPOSAL OF UNSUITABLE MATERIAL QUANTITIES ARE NOT INCLUDED IN THE OVERALL EARTHWORK BALANCE CALCULATIONS.

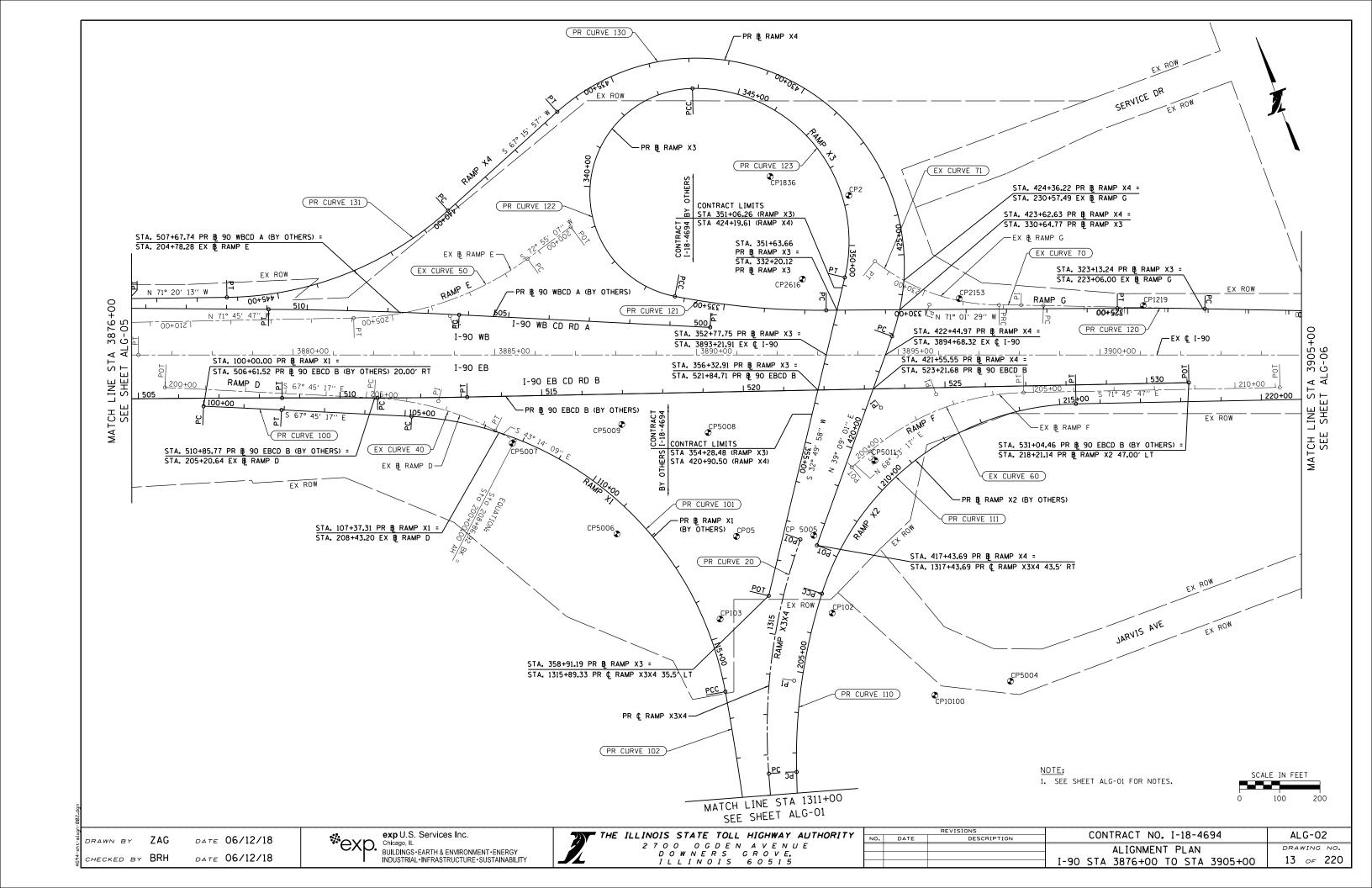
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CHECKED BY BRH DATE 06/12/18

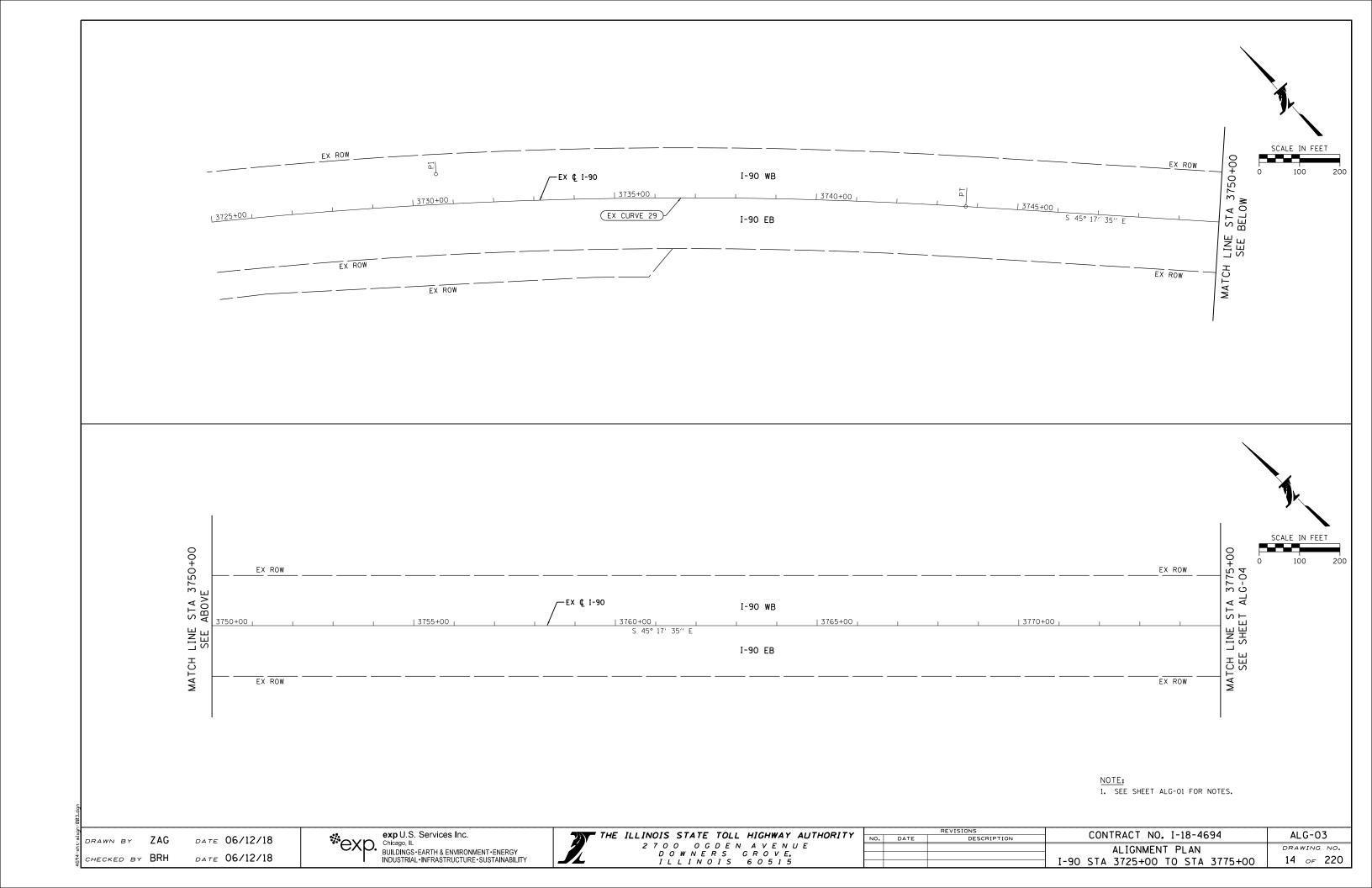


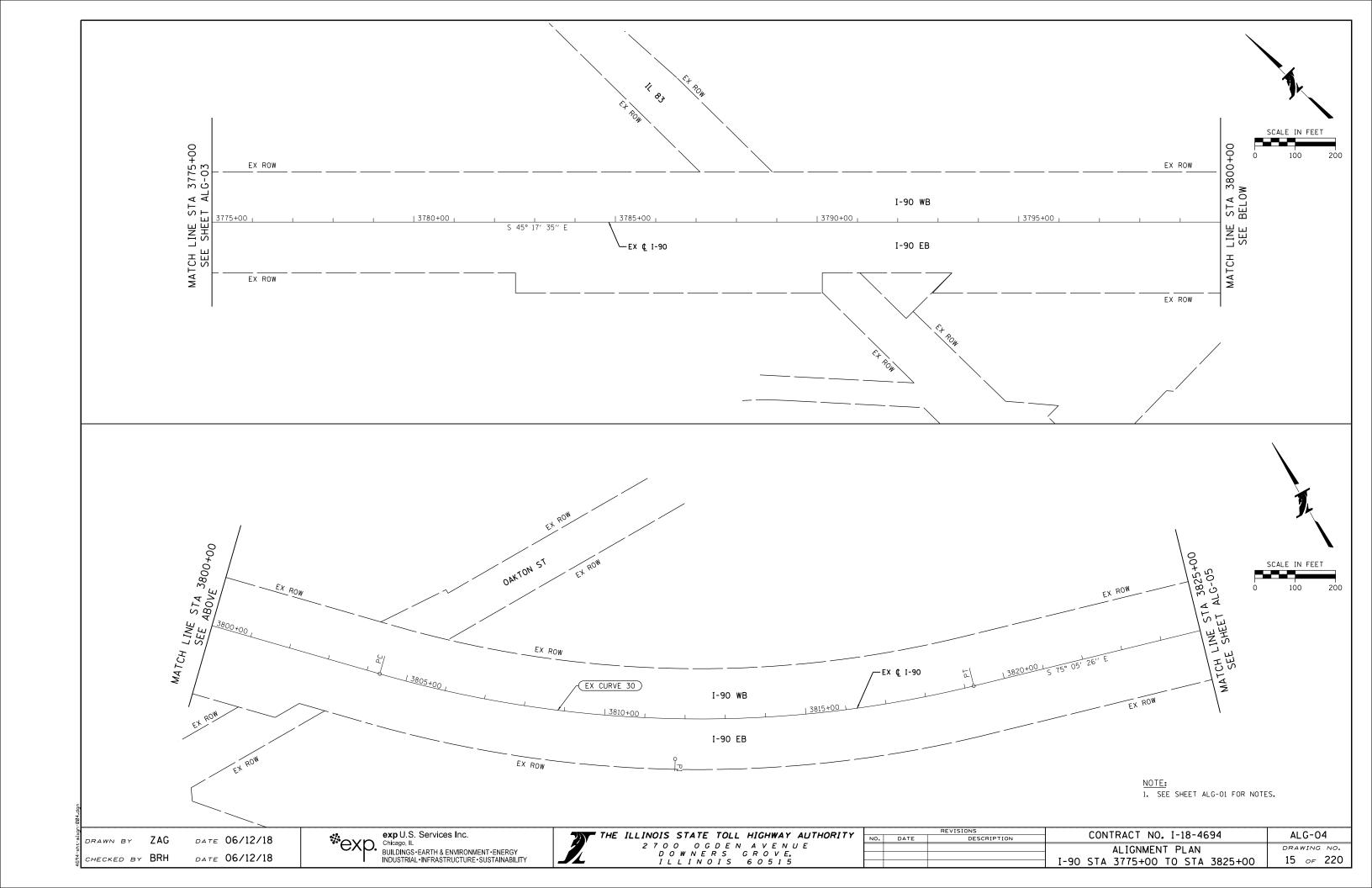


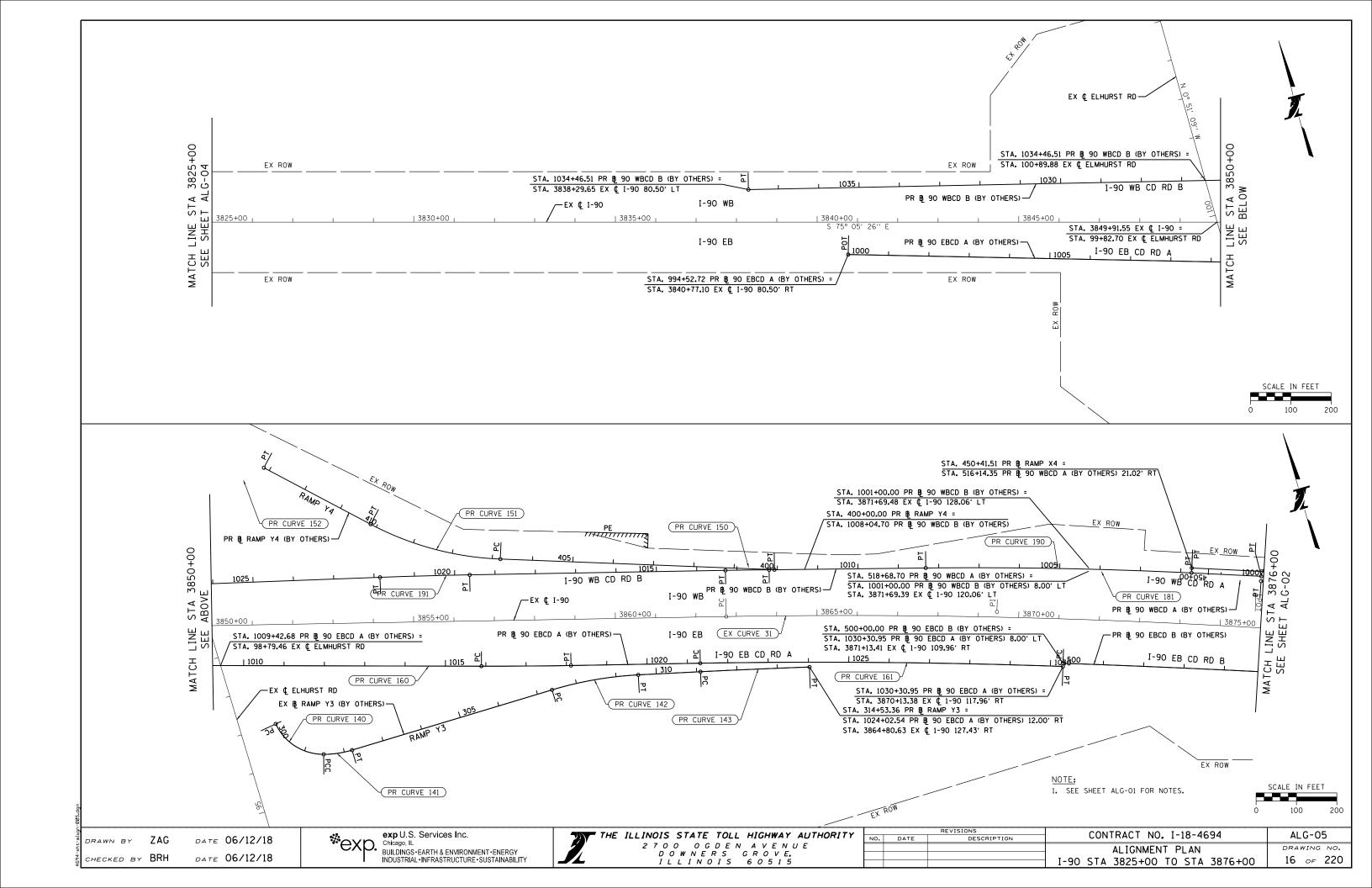
		REVISIONS	CONTRACT NO. I-18-4694	EWS-1
NO.	DATE	DESCRIPTION	CONTRACT NO. 1-10-4034	EM2-1
			EARTHWORK SCHEDULE	DRAWING NO.
			EARTHWORK SCHEDULE	11 220
				11 of 220

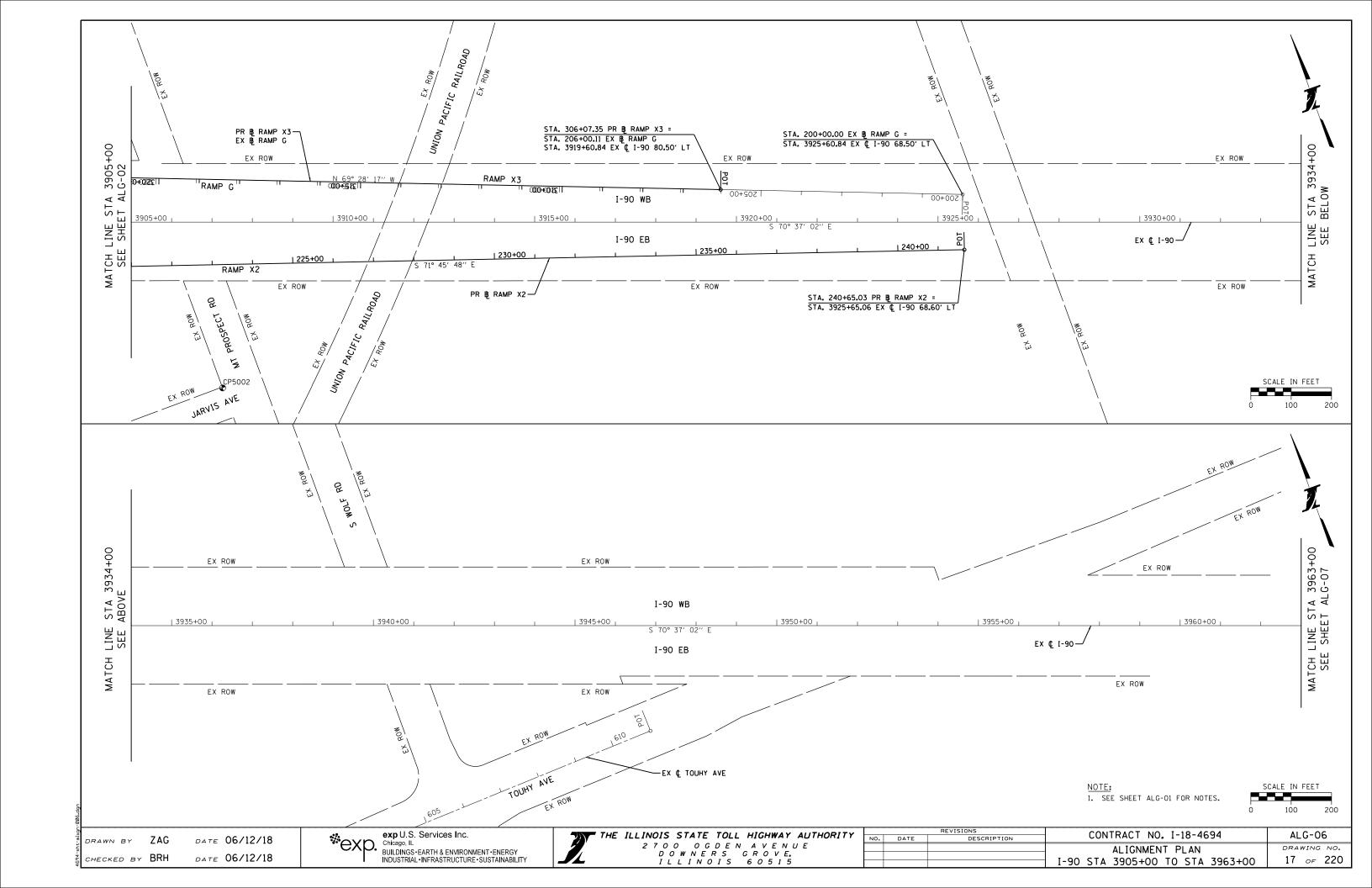


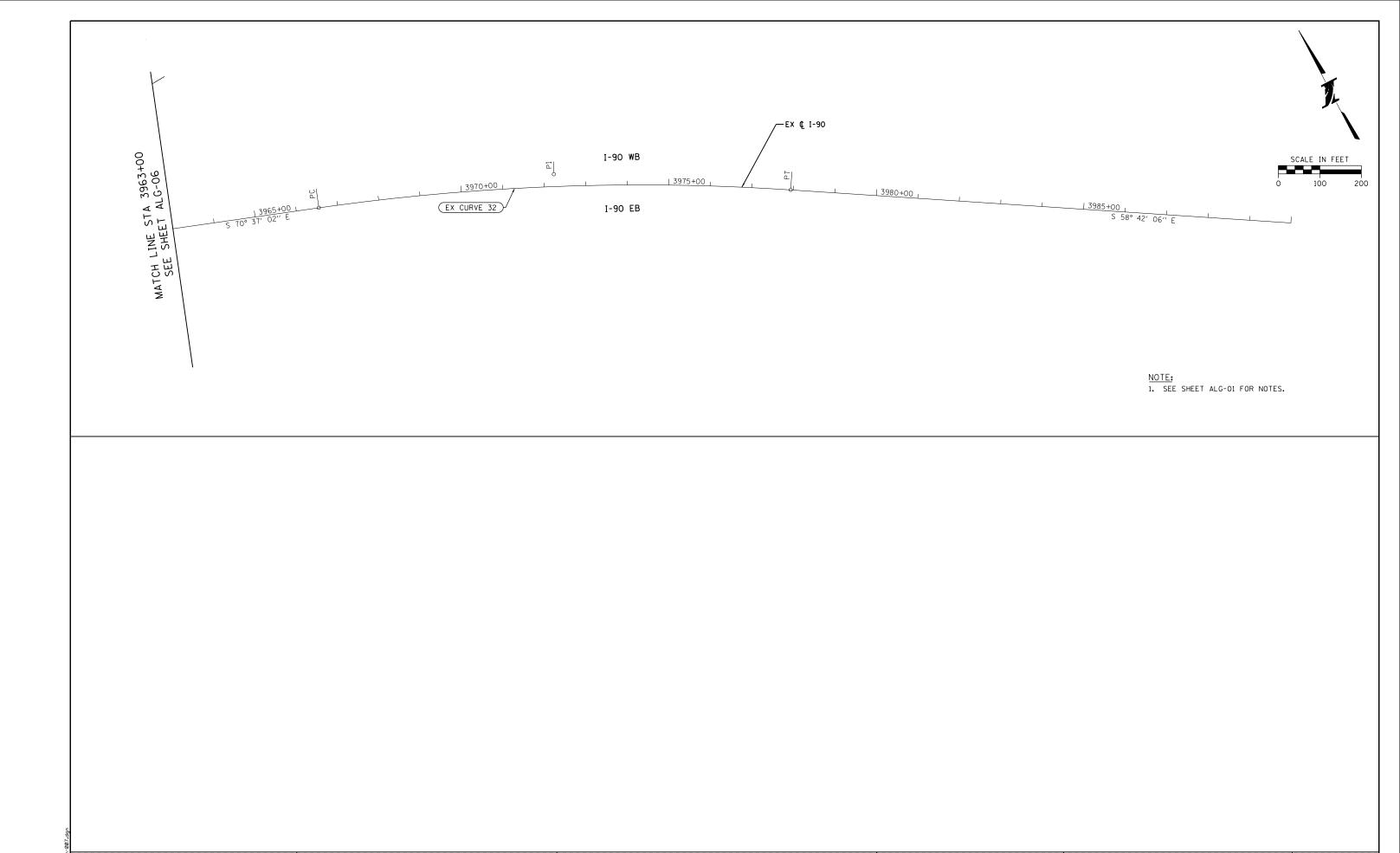












DATE 06/12/18 DRAWN BY ZAG DATE 06/12/18 CHECKED BY BRH

exp U.S. Services Inc.
Chicago, IL
BUILDINGS-EARTH & ENVIRONMENT-ENERGY
INDUSTRIAL-INFRASTRUCTURE-SUSTAINABILITY



		REVISIONS	CONTRACT NO. I-18-4694	ALG-07
	DATE	DESCRIPTION	CUNTRACT NO. 1-10-4634	ALG-07
Ξ			ALIGNMENT PLAN	DRAWING NO.
				1 10 220
			I-90 STA 3963+00 TO STA 3990+00	18 of 220

PROPOSED CURVE AND COORDINATE DATA																						
														DESIGN	P	°C	F	PI .	F	PT T	C	CC
BASELINE	CURVE NAME.	PC STA.	PI STA.	PT STA.		Delta		D		Т	L	R	е	SPEED	NORTH	EAST	NORTH	EAST	NORTH	EAST	NORTH	EAST
WESTERN ACCESS	BY OTHERS	1285+32.52	1290+58.82	1295+65.71	26°	54′ 28.16′′	2°	36′	15.67"	526.30	1,033.19	2,200.00			1,944,789.4636	1,093,770.3864	1,945,181.1482	1,094,121.9214	1,945,689.5168	1,094,258.1383	1,946,258.9187	1,092,133.1015
RAMP X3X4	PR CURVE 20	1311+53.59	1314+53.09	1317+43.69	24°	09′ 00.78′′	4°	05′	33.20′′	299.50	590.10	1,400.00	5.40%	50 MPH	1,947,223.2937	1,094,669.1126	1,947,512.5865	1,094,746.6284	1,947,744.8451	1,094,935.7181	1,946,860.9471	1,096,021.4088
RAMP X1	PR CURVE 100	100+00.00	100+96.78	101+93.57	0°	19′ 33.76′′	0°	10′	06.38"	96.78	193.57	34,015.62	N.C.	50 MPH	1,948,547.1652	1,093,649.4855	1,948,511.0358	1,093,739.2735	1,948,474.3960	1,093,828.8545	1,916,990.4835	1,080,951.5206
RAMP X1	PR CURVE 101	105+16.29	111+76.21	116+32.47	76°	46′ 26.09′′	6°	52'	41.68′′	659.92	1,116.18	833.00	6.00%	40 MPH	1,948,352.2233	1,094,127.5556	1,948,102.3970	1,094,738.3582	1,947,450.6372	1,094,634.9066	1,947,581.2217	1,093,812.2058
RAMP X1	PR CURVE 102	116+32.47	119+72.04	123+10.98	5°	58′ 51.19′′	0°	52′	53.30′′	339.56	678.51	6,500.00	2.00%	50 MPH	1,947,450.6372	1,094,634.9066	1,947,115.2721	1,094,581.6753	1,946,787.2790	1,094,493.7897	1,948,469.6037	1,088,215.2722
RAMP X2	-	-	200+00.00	-	-		-	-	-	-	-	-	-	50 MPH	-	-	1,946,963.6555	1,094,658.0359	-	-	-	-
RAMP X2	PR CURVE 110	202+52.93	204+79.24	207+01.32	19°	09′ 53.66′′	4°	16′	27.15"	226.31	448.38	1,340.50	5.40%	50 MPH	1,947,204.3967	1,094,735.6187	1,947,419.7941	1,094,805.0342	1,947,600.4655	1,094,941.3148	1,946,793,2223	1,096,011.5011
RAMP X2	PR CURVE 111	207+01.32	211+84.65	215+40.23	71°	12′ 33.60′′	8°	29′	17.75′′	483.34	838.92	675.00	6.00%	45 MPH	1,947,600.4655	1,094,941.3148	1,947,986.3359	1,095,232.3774	1,947,835.0797	1,095,691.4362	1,947,193.9835	1,095,480.2001
RAMP X2	-	-	241+65.03	-	-		-	-	-	-	-	-	-	50 MPH	-	-	1,947,013.6684	1,098,184.3986	-	-	-	-
RAMP X3			306+07.35														1,947,354.8456	1,097,663.9108	-	-	-	-
RAMP X3	PR CURVE 120	323+08.16	324+16.61	325+25.05	1°	33′ 11.95″	0°	42'	58.31"	108.45	216.88	8,000.00	N.C.	50 MPH	1,947,951.2771	1,096,071.1035	1,947,989.3073	1,095,969.5415	1,948,024.5704	1,095,866.9860	1,940,459.2972	1,093,265.7092
RAMP X3	PR CURVE 121	332+46.88	334+36.16	336+24.20	11°	19' 07.63''	2°	59′	59.20"	189.28	377.32	1,910.00	3.80%	40 MPH	1,948,259.2821	1,095,184.3758	1,948,320.8272	1,095,005.3849	1,948,416.3052	1,094,841.9546	1,950,065.4911	1,095,805.4306
RAMP X3	PR CURVE 122	336+24.20	360+73.96	343+86.03	167°	53′ 00.25′′	22°	02′	12.62"	2,449.75	761.83	260.00	8.00%	30 MPH	1,948,416.3052	1,094,841.9546	1,949,652.0537	1,092,726.7188	1,948,887.8265	1,095,054.2182	1,948,640.8018	1,094,973.1084
RAMP X3	PR CURVE 123	343+86.03	348+78.19	350+80.14	104°	39′ 19 <b>.</b> 65′′	15°	04'	40.21"	492.15	694.10	380.00	8.00%	35 MPH	1,948,887.8265	1,095,054.2182	1,948,734.2935	1,095,521.8119	1,948,320.7579	1,095,254.9702	1,948,526.7904	1,094,935.6731
RAMP X3			358+91.19	-	-		-	-	-	-	-	-	-		-	-	1,947,639.2618	1,094,815.2220	-	-	-	-
RAMP X4			417+43.69														1,947,717.3811	1,094,969.4521	-	-	-	-
RAMP X4	PR CURVE 130	422+94.32	443+51.03	436+59.52	151°	53′ 03.75′′	11°	07'	31.42"	2,056.71	1,365.20	515.00	6.00%	40 MPH	1,948,144.3890	1,095,317.0938	1,949,739.3532	1,096,615.6089	1,948,944.5258	1,094,718.6899	1,948,469.5372	1,094,917.7147
RAMP X4	PR CURVE 131	440+24.57	443+39.31	446+26.43	41°	23′ 50.26′′	6°	52'	41.68′′	314.74	601.86	833.00	6.00%	50 MPH	1,948,803.4512	1,094,382.0041	1,948,681.8172	1,094,091.7151	1,948,782.5358	1,093,793.5234	1,949,571.7336	1,094,060.0863
RAMP X4			450+41.51	-	-		-	-	-	-	-	-	-		-	-	1,948,915.3625	1,093,400.2707	-	-	-	-

											Е	XISTING (	CURVE AND	COORD	INATE DA	AT.							
															DESIGN	F	PC .	F	PI	F	°T	С	CC
BASELINE	CURVE NAME.	PC STA.	PI STA.	PT STA.		Delta			D		Т	L	R	е	SPEED	NORTH	EAST	NORTH	EAST	NORTH	EAST	NORTH	EAST
I-90 (JANE ADDAMS)			3658+00.00	-	-	_	_		-	-	-	-	-	_		-	-	1,959,966.1578	1,075,341.1124	-	-	-	-
I-90 (JANE ADDAMS)	EX CURVE 29	3717+50.83	3730+66.63	3743+70.94	13°	6′	11.93"	0°	30′	00.38"	1,315.79	2,620.11	11,456.73		70 MPH	1,956,847.6742	1,080,409.3913	1,956,158.1433	1,081,530.0433	1,955,232.5055	1,082,465.1947	1,947,090.0619	1,074,405.5872
I-90 (JANE ADDAMS)	EX CURVE 30	3804+32.72	3811+94.75	3819+22.28	29°	47′	51.32"	2°	00′	01.53"	762.03	1,489.56	2,864.18		70 MPH	1,950,968.1506	1,086,773.3784	1,950,432.0725	1,087,314.9663	1,950,236.0066	1,088,051.3461	1,953,003.7597	1,088,788.2786
I-90 (JANE ADDAMS)	EX CURVE 31	3862+74.94	3869+46.14	3876+16.65	4°	28′	23.97"	0°	20′	00.25"	671.20	1,341.71	17,185.09		70 MPH	1,949,116.1001	1,092,257.4642	1,948,943.4059	1,092,906.0646	1,948,720.6503	1,093,539.2201	1,932,509.5713	1,087,835.8670
I-90 (JANE ADDAMS)	EX CURVE 32	3966+54.85	3972+26.22	3977+93.47	11°	54′	56.17''	1°	02′	47.39"	571.37	1,138.62	5,475.00		70 MPH	1,945,721.0709	1,102,065.1553	1,945,531.4461	1,102,604.1404	1,945,234.6226	1,103,092.3600	1,940,556.3812	1,100,248.1233
			4034+98.76	-	-	-	-	-	-		-	-	-	-		-	-	1,942,270.7513	1,107,967.3781	-	-	-	-
RAMP D			200+00.00	-	-	-	-	-	-	-	-	-	-	-		-	-	1,948,622.8931	1,093,574.5239	-	-	-	-
RAMP D	EX CURVE 40	205+20.00	206+78.63	208+32.40	24°	31′	08.75′′	7°	50′	55.45′′	158.63	312.40	730.00			1,948,426.0361	1,094,055.8215	1,948,365.9849	1,094,202.6412	1,948,250.4192	1,094,311.3002	1,947,750.3684	1,093,779.4645
			208+86.92	-	-	-	-	-	-	-	-	-	-	-		-	-	1,948,210.6927	1,094,348.6525	-	-	-	-
RAMP E			200+00.00															1,948,664.7222	1,094,655.6906	-	-	-	-
RAMP E	EX CURVE 50	201+33.41	203+72.18	205+95.73	35°	19′	06.71′′	7°	38′	21.97"	238.77	462.32	750.00			1,948,625.5347	1,094,528.1628	1,948,555.4009	1,094,299.9272	1,948,630.1232	1,094,073.1524	1,949,342.4506	1,094,307.8642
RAMP E			211+62.63															1,948,807.5349	1,093,534.7250	-	-	-	-
RAMP F			200+00.00	_	-	-	-	-	-	-	=	-	-	-			-	1,947,871.1113	1,095,115.1121	-	-	-	-
RAMP F	EX CURVE 60	200+60.11	202+76.61	204+75.67	39°	40′	56.38′′	9°	32'	57.47''	216.50	415.55	600.00			1,947,893.0896	1,095,171.0637	1,947,972.2448	1,095,372.5751	1,947,904.4913	1,095,578.2006	1,947,334.6293	1,095,390.4312
RAMP F			211+11.08	-	-	-	-	-	-	-	-	-	-	-		-	-	1,947,705.6394	1,096,181.6968	-	-	-	-
RAMP G			200+00.00	-	-	_	-	_	-	-	-	-	-	-		-	-	1,947,144.3976	1,098,225.9153	-	-	-	-
RAMP G	EX CRUVE 70	227+01.28	227+56.26	228+11.23	0°	47′	14.65"	0°	42'	58.31"	54.97	109.94	8,000.00			1,948,091.6708	1,095,696.1698	1,948,110.9481	1,095,644.6888	1,948,129.5160	1,095,592.9477	1,940,599.6932	1,092,890.7692
RAMP G	EX CURVE 71	228+11.23	229+83.82	231+43.61	38°	05'	18.00"	11°	27′	32.96"	172.60	332.38	500.00			1,948,129,5160	1,095,592,9477	1,948,187,8139	1,095,430,4965	1,948,333.9100	1,095,338,6000	1,948,600,1300	1,095,761.8339

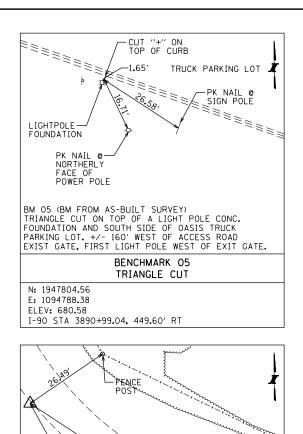
DRAWN BY ZAG

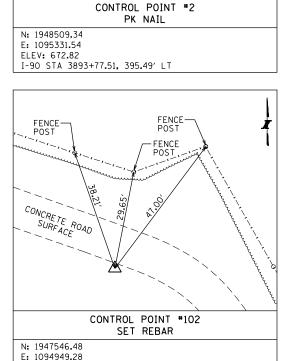
DATE 06/12/18
DATE 06/12/18

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REVISIONS			CONTRACT NO. I-18-4694	ALG-08
NO.	DATE	DESCRIPTION	CUNTRACT NO. 1-10-4634	ALG-06
			ALIGNMENT PLAN	DRAWING NO.
				10 000
			I-90 CURVE DATA	19 of 220





SERVICE ROAD

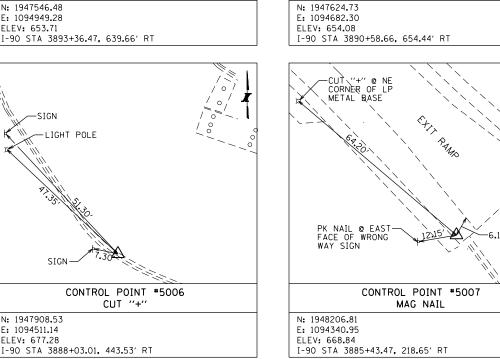
-LIGHT POLE

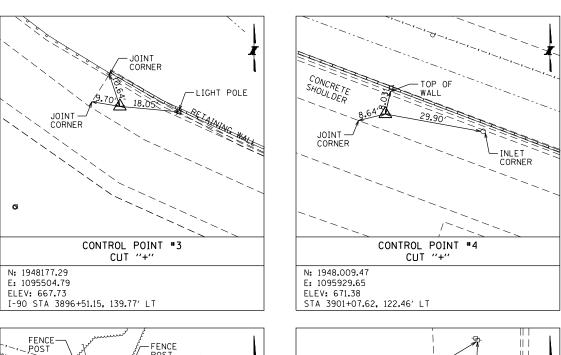
-GUARDRAIL POST CORNER

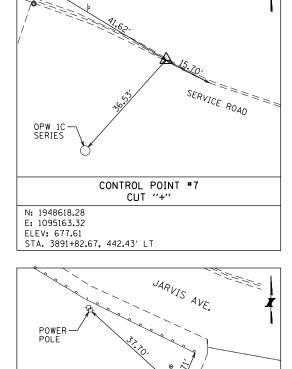
Ø

GUARDRAIL

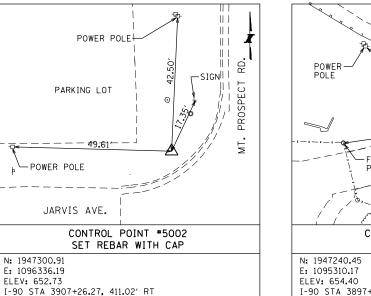
POST

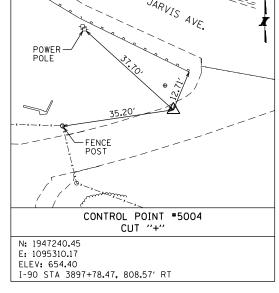


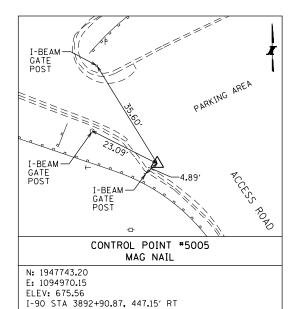




-TRANSFORMER BASE CORNER







— 4′ VENT

I-90 STA 3895+91.11, 843.06' RT

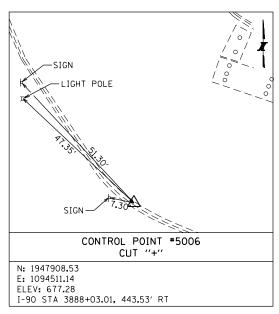
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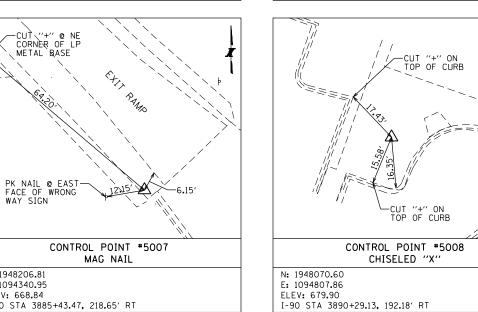
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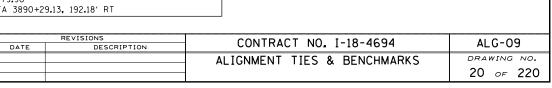
ELEV: 654.01

CONTROL POINT #100

SET REBAR











ELEV: 653.71

-FENCE POST

\$7.30

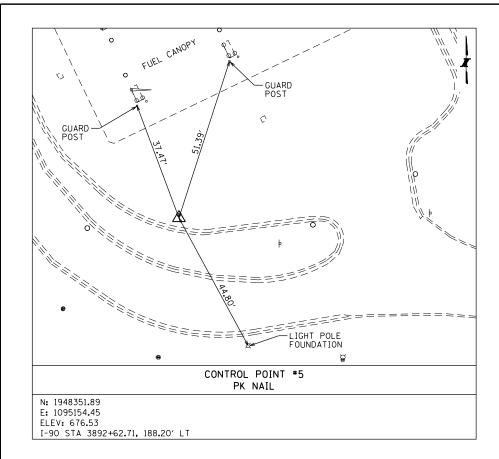
CONTROL POINT #103

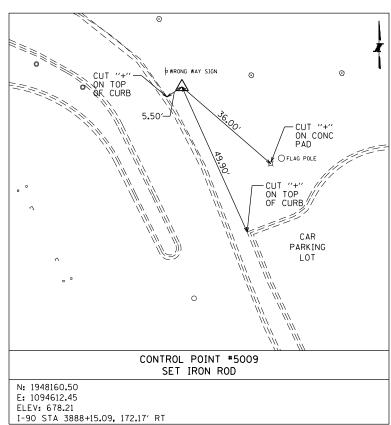
SET REBAR

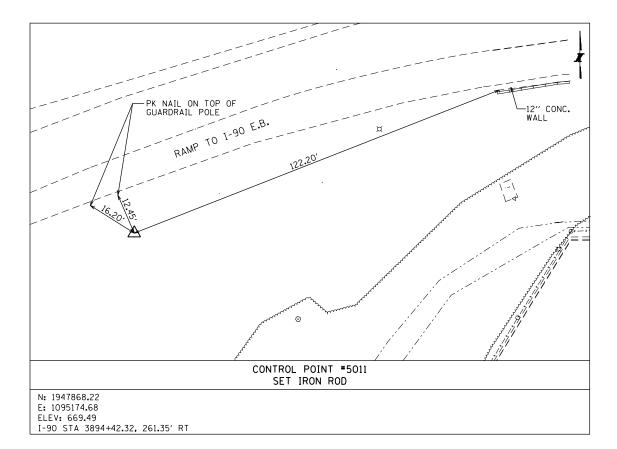
-FENCE

POST

CONCRETE ROAD SURFACE





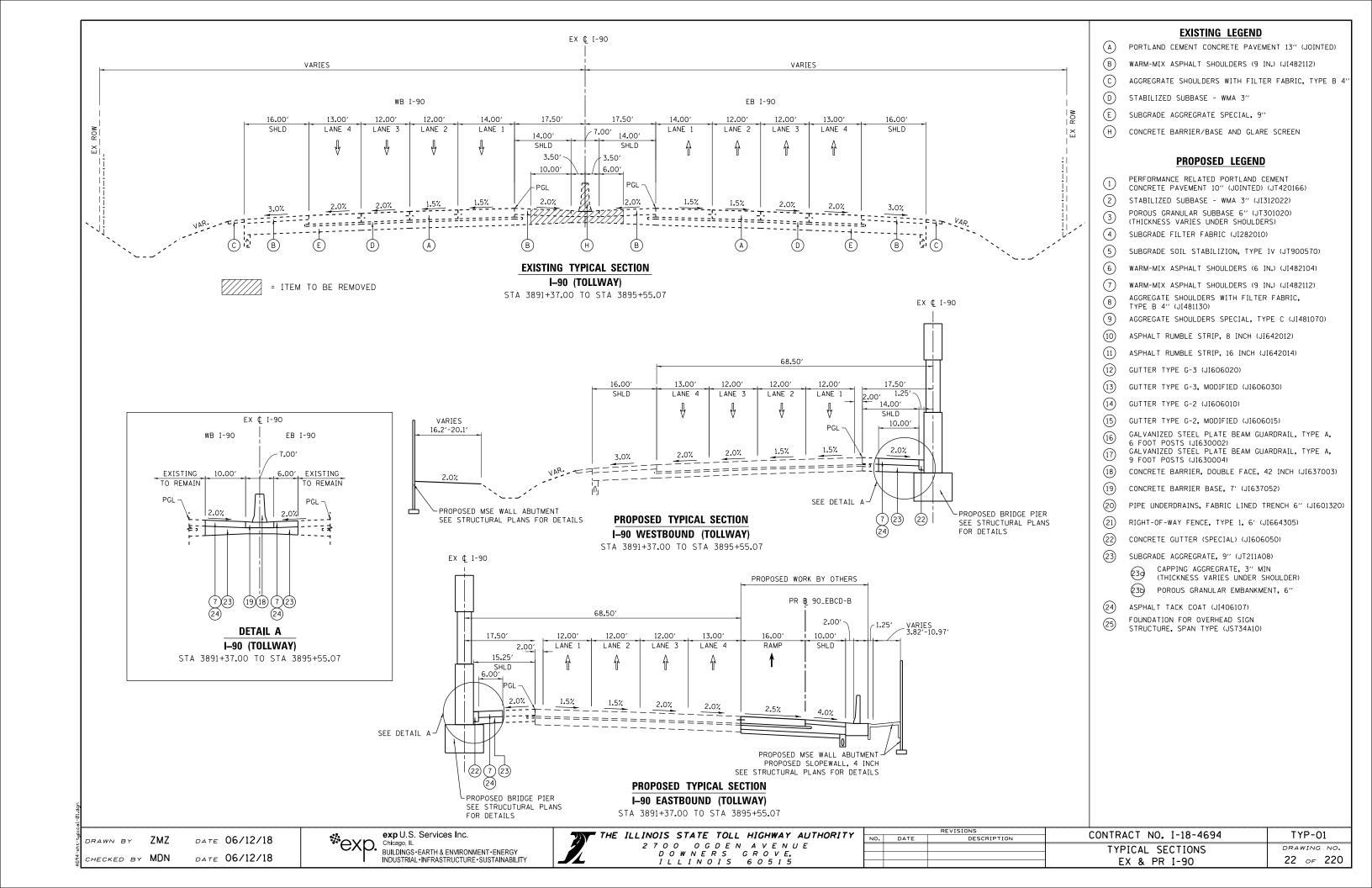


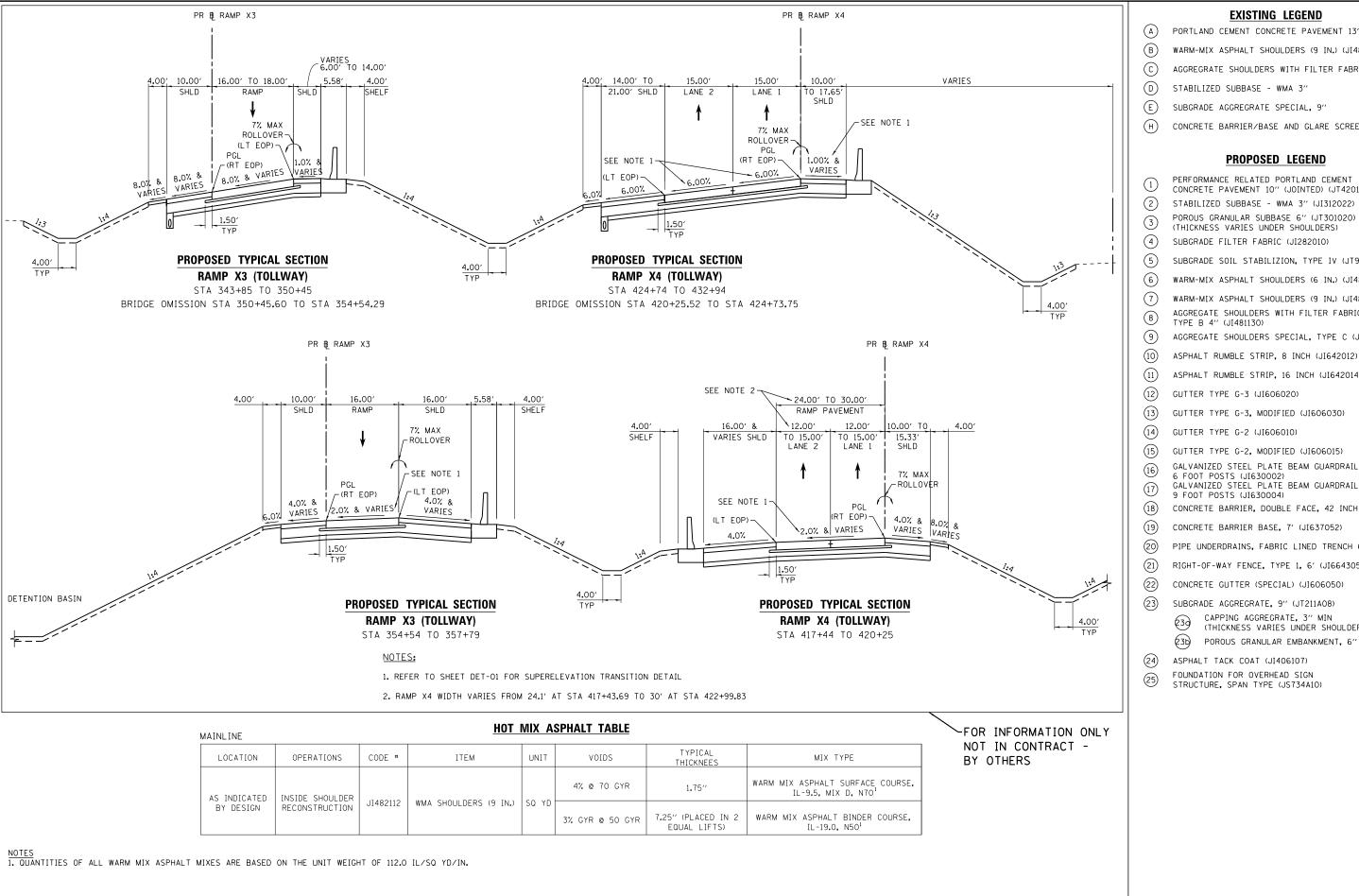
DRAWN BY ZAG DATE 06/12/18
CHECKED BY BRH DATE 06/12/18





		REVISIONS	CONTRACT NO. I-18-4694	ALG-10		
NO.	DATE	DESCRIPTION	CUNTRACT NO. 1-10-4634	ALG-10		
			ALIGNMENT TIES & BENCHMARKS	DRAWING NO.		
			ALIGNMENT TIES & BENGINMANNS	21 of 220		





### **EXISTING LEGEND**

- PORTLAND CEMENT CONCRETE PAVEMENT 13" (JOINTED)
- WARM-MIX ASPHALT SHOULDERS (9 IN.) (JI482112)
- AGGREGRATE SHOULDERS WITH FILTER FABRIC, TYPE B 4
- STABILIZED SUBBASE WMA 3"
- SUBGRADE AGGREGRATE SPECIAL, 9"
- CONCRETE BARRIER/BASE AND GLARE SCREEN

## PROPOSED LEGEND

- PERFORMANCE RELATED PORTLAND CEMENT CONCRETE PAVEMENT 10" (JOINTED) (JT420166)
- STABILIZED SUBBASE WMA 3" (JI312022)
- POROUS GRANULAR SUBBASE 6" (JT301020)
- (THICKNESS VARIES UNDER SHOULDERS)
- SUBGRADE FILTER FABRIC (JI282010)
- SUBGRADE SOIL STABILIZION, TYPE IV (JT900570)
- WARM-MIX ASPHALT SHOULDERS (6 IN.) (JI482104)
- WARM-MIX ASPHALT SHOULDERS (9 IN.) (JI482112)
- AGGREGATE SHOULDERS WITH FILTER FABRIC,
- TYPE B 4" (JI481130)
- AGGREGATE SHOULDERS SPECIAL, TYPE C (JI481070)
- ASPHALT RUMBLE STRIP, 16 INCH (JI642014)
- GUTTER TYPE G-3 (JI606020)
- GUTTER TYPE G-3, MODIFIED (JI606030)
- GUTTER TYPE G-2 (JI606010)
- GUTTER TYPE G-2, MODIFIED (JI606015)
- GALVANIZED STEEL PLATE BEAM GUARDRAIL, TYPE A, 6 FOOT POSTS (JI630002) GALVANIZED STEEL PLATE BEAM GUARDRAIL, TYPE A,
- 9 FOOT POSTS (JI630004)
- CONCRETE BARRIER, DOUBLE FACE, 42 INCH (JI637003)
- CONCRETE BARRIER BASE, 7' (JI637052)
- PIPE UNDERDRAINS, FABRIC LINED TRENCH 6" (JI601320)
- RIGHT-OF-WAY FENCE, TYPE 1, 6' (JI664305)
- CONCRETE GUTTER (SPECIAL) (JI606050)
- SUBGRADE AGGREGRATE, 9" (JT211AO8)
- CAPPING AGGREGRATE, 3" MIN
- (THICKNESS VARIES UNDER SHOULDER)
- POROUS GRANULAR EMBANKMENT, 6"
- ASPHALT TACK COAT (JI406107)
- FOUNDATION FOR OVERHEAD SIGN STRUCTURE, SPAN TYPE (JS734A10)

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DESCRIPTION

CONTRACT NO. I-18-4694 TYP-02 TYPICAL SECTIONS DRAWING NO. 23 of 220 PR RAMPS X3 & X4

ZMZ CHECKED BY MDN

DATE 06/12/18 DATE 06/12/18

\*\*exp. Chicago, IL BUILDINGS

### MAINTENANCE OF TRAFFIC GENERAL NOTES

- TWO TRAILER MOUNTED FULL MATRIX PORTABLE CHANGEABLE MESSAGE SIGNS SHALL BE INSTALLED ON THE APPROACHES TO THE WORK ZONE AT LOCATIONS DETERMINED BY THE ENGINEER PRIOR TO ANY CONSTRUCTION ACTIVITIES AND SHALL REMAIN IN PLACE FOR THE DURATION OF STAGE I AND STAGE II CONSTRUCTION, MESSAGING TO BE DETERMINED BY THE ENGINEER
- 2. FIVE TRAILER MOUNTED FULL MATRIX PORTABLE CHANGEABLE MESSAGE SIGNS SHALL BE INSTALLED IN ADVANCE OF STAGE II CONSTRUCTION AS SPECIFIED IN THE SPECIAL PROVISION "COORDINATION OF DES PLAINES OASIS CLOSURE".
- CONSTRUCTION SIGNS SHALL BE POST MOUNTED OR ATTACHED TO PORTABLE SUPPORTS AND SHALL BE INSTALLED 8' TO 12' FROM THE ADJACENT TRAVEL LANE WHEREVER POSSIBLE. UNDER NO CONDITIONS SHALL SIGNS BE LOCATED TO PROVIDE LESS THAN 2' CLEARANCE BETWEEN THE EDGE OF SIGN AND THE ADJACENT TRAVEL LANE.
- 4. ALL SIGNS IN PLACE LONGER THAN 4 DAYS SHALL BE POST-MOUNTED IN THE GROUND WHERE POSSIBLE.
- 5. ALL SIGNS SHALL BE BOLTED TO SIGN SUPPORTS, UNLESS OTHERWISE NOTED.
- 6. ONE TYPE "A" WARNING LIGHT SHALL BE INSTALLED ABOVE EACH OF THE ADVANCE WARNING SIGNS.
- 7. TOLLWAY CONSTRUCTION SIGNS SHALL BE IN ACCORDANCE WITH TOLLWAY STANDARD E1.
- TEMPORARY LANE AND SHOULDER CLOSURES, AS NECESSARY, SHALL BE IN ACCORDANCE WITH TOLLWAY STANDARDS E2 & E3 RESPECTIVELY AND WILL ONLY BE PERMITTED DURING ALLOWABLE LANE AND SHOULDER CLOSURE TIMES, SEE CONTRACT SPECIAL PROVISION MAINTENANCE OF TRAFFIC, LANE AND SHOULDER CLOSURES SHALL BE APPROVED BY THE TOLLWAY, LANE AND SHOULDER CLOSURE REQUESTS SHALL BE IN ACCORDANCE WITH THE TRAFFIC CONTROL COMMUNICATION MANUAL, LATEST EDITION.
- 9. DELINEATORS ON TEMPORARY CONCRETE BARRIER SHALL BE INSTALLED IN ACCORDANCE WITH TOLLWAY STANDARD D4.
- 10. TEMPORARY GORE DETAILS SHALL BE IN ACCORDANCE WITH TOLLWAY STANDARD E5 AND AS SHOWN ON THE PLANS
- 11. EMERGENCY PULL-OUTS ARE NOT INCLUDED IN THE WORK.
- 12. THE EXISTING TRAFFIC PATTERN OF FOUR LANES OF TRAFFIC IN EACH DIRECTION ALONG I-90 (AS SHOWN IN PLANS) SHALL BE MAINTAINED AT ALL TIMES EXCEPT DURING APPROVED TEMPORARY LANE CLOSURE HOURS.
- 13. TEMPORARY CONCRETE BARRIER SHALL BE ANCHORED TO THE SHOULDER IN ACCORDANCE WITH THE IDOT HIGHWAY STANDARD 704001 AND TOLLWAY SPECIAL PROVISION FOR TEMPORARY CONCRETE BARRIER, BARRIER ANCHORED TO ASPHALT SHOULDERS SHALL USE THE DETAIL SHOWN ON SHEET MOT-02.
- 14. FULL CLOSURE OF THE I-90 TOLLWAY WILL BE REQUIRED FOR GIRDER ERECTION. THIS WORK SHALL BE COMPLETED DURING THE TIME PERIOD SPECIFIED IN THE SPECIAL PROVISION MAINTENANCE OF TRAFFIC.
- 15. CONTRACTOR ACCESS TO THE WORK AREA FOR TRUCKS, EQUIPMENT AND MATERIAL DELIVERIES SHALL BE AS SHOWN ON SHEET MOT
- 16. THE FOLLOWING IS LIST OF MAINTENANCE OF TRAFFIC ASSOCIATED ITEMS, FOR WHICH NOMINAL CONTINGENCY QUANTITIES HAVE BEEN PROVIDED, MUST BE APPROVED BY THE ENGINEER BEFORE USE:

JT701030 - SUPPLEMENTAL BARRICADE

JT701031 - SUPPLEMENTAL SIGNING

JT701032 - SUPPLEMENTAL FLASHING ARROW BOARD (PER DAY)

JT701033 - SUPPLEMENTAL FLASHING ARROW BOARD (PER WEEK)

JT701034 - SUPPLEMENTAL FLASHING ARROW BOARD (PER MONTH

JT701035 - SUPPLEMENTAL MAINTENANCE OF TRAFFICE (PER DAY)

## SUGGESTED STAGING

# STAGE I

### TRAFFIC CONTROL AND PROTECTION

- 1. ERECT ADVANCE SIGNING AND TRAILER MOUNTED FULL MATRIX PORTABLE CHANGEABLE MESSAGE SIGNS IN EACH DIRECTION ALONG I-90 AS SHOWN ON THE PLANS AND DIRECTED BY THE ENGINEER.
- 2. PROVIDE PERMANENT INSIDE SHOULDER CLOSURE IN BOTH EASTBOUND AND WESTBOUND DIRECTION AT THE MEDIAN WORK AREAS AS SHOWN ON THE PLANS, AND AT JUNCTION BOXES AS REQUIRED PER TOLLWAY STANDARDS.
- 3. PROVIDE TEMPORARY INSIDE ONE LANE CLOSURE AND ANCHOR TEMPORARY CONCRETE BARRIER ON EASTBOUND AND WESTBOUND INSIDE SHOULDER AT THE MEDIAN WORK AREAS SHOWN ON THE PLANS.
- 4. PROVIDE TEMPORARY INSIDE ONE LANE CLOSURE FOR REMOVAL OF TEMPORARY CONCRETE BARRIER FROM EASTBOUND AND WESTBOUND
- 5. AFTER COMPLETION OF ALL WORK, REMOVE WARNING SIGNS, ARROW BOARDS, AND ALL OTHER TEMPORARY TRAFFIC CONTROL DEVICES.

### CONSTRUCTION

- 1. DISCONNECT EXISTING SPLICE (BY OTHERS), PULL, STORE AND PROTECT I-90 MEDIAN FIBER OPTIC CABLE.
- 2. REMOVE EXISTING MEDIAN BARRIER WITHIN CONSTRUCTION LIMITS SHOWN ON THE PLANS.
- 3. CONSTRUCT MEDIAN SIGN TRUSS FOUNDATIONS AT STA. 3846+20 (M.P. 73.5) AND STA. 3891+79 (M.P. 74.4).
- 4. CONSTRUCT RAMP X3 (BRIDGE NO. 1682) AND RAMP X4 (BRIDGE NO. 1681) PIERS.
- 5. CONSTRUCT CONDUITS FOR ROADWAY LIGHTING AND FIBER OPTIC CABLE AS SHOWN ON THE PLANS.
- 6. CONSTRUCT MEDIAN BARRIER AND PULL I-90 MEDIAN FIBER OPTIC CABLE BACK. RESPLICE CABLE AT JUNCTION BOX (BY OTHERS).
- 7. RECONSTRUCT EASTBOUND AND WESTBOUND SHOULDER WITHIN CONSTRUCTION LIMITS SHOWN ON THE PLANS.

### WINTER STAGE

### TRAFFIC CONTROL AND PROTECTION

- 1. REMOVE ALL LANE AND SHOULDER CLOSURE WARNING SIGNS, ARROW BOARDS AND ALL OTHER TEMPORARY TRAFFIC CONTROL DEVICES.
- 2. REMOVE OR COVER ADVANCE SIGNING AND REMOVE PORTABLE CHANGEABLE MESSAGE SIGNS.
- 3. MAINTAIN EXISTING TRAFFIC PATTERN OF FOUR LANES OF TRAFFIC IN EACH DIRECTION ALONG I-90 WITH OPEN INSIDE AND OUTSIDE SHOULDERS.

### CONSTRUCTION

NONE

### TRAFFIC CONTROL AND PROTECTION

- 1. ERECT ADVANCE SIGNING AND TRAILER MOUNTED FULL MATRIX PORTABLE CHANGEABLE MESSAGE SIGNS IN EACH DIRECTION ALONG I-90 AS SHOWN ON HE PLANS AND DIRECTED BY THE ENGINEER.
- 2. PROVIDE AUXILIARY LANE CLOSURE OF THE WESTBOUND EXIT TO THE DES PLAINES OASIS AND TEMPORARY OUTSIDE ONE LANE CLOSURES TO RESTRIPE EXIT RAMPS AND INSTALL BARRICADES OR DRUMS TO CLOSE 1-90 EASTBOUND AND WESTBOUND EXIT RAMPS TO THE DES PLAINES OASIS AS SHOWN ON
- 3. PROVIDE TEMPORARY LANE CLOSURES FOR REMOVAL OF "DES PLAINES OASIS" OVERHEAD TRUSS MOUNTED SIGN PANELS AND INSTALLATION OF "RIGHT LANE ENDS" OVERHEAD TRUSS MOUNTED SIGN PANELS.
- PROVIDE TEMPORARY SHOULDER CLOSURE, AS NECESSARY, FOR REMOVAL OF "DES PLAINES OASIS" ASSOCIATED GROUND MOUNTED SIGNS AND FOR REMOVAL OF EXISTING & INSTALLATION OF PROPOSED SIGNS.
- 4. PROVIDE TEMPORARY OUTSIDE ONE LANE CLOSURE AND INSTALL TEMPORARY CONCRETE BARRIER ON OUTSIDE EDGE OF EASTBOUND AND WESTBOUND OUTSIDE SHOULDER AT BRIDGE WORK AREA ADJACENT TO I-90 AS SHOWN ON THE PLANS.
- 5. PROVIDE FULL I-90 CLOSURE WITH STATE TROOPER ASSISTANCE FOR GIRDER ERECTION.
- 6. AFTER COMPLETION OF BRIDGE WORK ADJACENT TO I-90, PROVIDE TEMPORARY OUTSIDE ONE LANE CLOSURE FOR REMOVAL OF TEMPORARY CONCRETE BARRIER FROM OUTSIDE SHOULDER.
- 7. AFTER COMPLETION OF ALL WORK, REMOVE WARNING SIGNS, ARROW BOARDS, AND ALL OTHER TEMPORARY TRAFFIC CONTROL DEVICES.

### CONSTRUCTION

- 1. REMOVE EXISTING PAVEMENT MARKING AND INSTALL PERMANENT PAVEMENT MARKING TO RESTRIPE I-90 EASTBOUND AND WESTBOUND EXIT RAMPS TO THE DES PLAINES OASIS AS SHOWN ON THE PLANS.
- 2. REMOVE "DES PLAINES OASIS" OVERHEAD TRUSS MOUNTED SIGN PANELS AT THE FOLLOWING LOCATIONS AS SHOWN ON THE PLANS:

EASTBOUND STA. 3691+00 (M.P. 70.6) "DES PLAINES OASIS 3 MILES" CLOSE TWO LEFT LANES

FASTBOUND STA. 3794+88 (M.P. 72.6) "DES PLAINES OASIS 1 MILE" CLOSE FOUR LEFT LANES

WESTBOUND STA. 3917+50 (M.P. 74.9) "DES PLAINES OASIS" CLOSE TWO RIGHT LANES

WESTBOUND STA. 3942+00 (M.P. 75.4) "DES PLAINES OASIS" CLOSE TWO RIGHT LANES

WESTBOUND STA. 3975+00 (M.P. 76.0) "DES PLAINES OASIS 1 MILE" CLOSE TWO RIGHT LANES

INSTALL "RIGHT LANE ENDS" OVERHEAD TRUSS MOUNTED SIGN PANELS AT THE FOLLOWING LOCATIONS AS SHOWN ON THE PLANS:

WESTBOUND STA. 3942+00 (M.P. 75.4) "RIGHT LANE ENDS 1/2 MILE" CLOSE TWO RIGHT LANES

WESTBOUND STA. 3975+00 (M.P. 76.0) "RIGHT LANE ENDS 1 MILE" CLOSE TWO RIGHT LANES

REMOVE "DES PLAINES OASIS" ASSOCIATED GROUND MOUNTED SIGNS AT THE FOLLOWING LOCATIONS AS SHOWN ON THE PLANS:

EASTBOUND STA. 3846+00 (M.P. 73.5) "MOBILE ELECTRIC GAS SIGN"

EASTBOUND STA. 3853+00 (M.P. 73.7) "DES PLAINES OASIS NEXT RIGHT"

EASTBOUND STA. 3877+65 (M.P. 74.1) "DES PLAINES OASIS EXIT

WESTBOUND STA. 3986+00 (M.P. 76.2) "MOBILE ELECTRIC GAS SIGN" WESTBOUND STA. 3988+00 (M.P. 76.2) "DES PLAINES OASIS 1 1/2 MILES"

REMOVE EXISTING & INSTALL PROPOSED EASTBOUND AND WESTBOUND GROUND & POLE MOUNTED SIGNS AT THE LOCATIONS AS SHOWN ON THE PLANS.

- 3. DEMOLISH EASTBOUND AND WESTBOUND CASIS, REMOVE EXISTING EASTBOUND AND WESTBOUND CASIS RAMP PAVEMENT AND SHOULDERS WITHIN CONSTRUCTION LIMITS SHOWN ON THE PLANS.
- 4. RELOCATE WESTBOUND I-90 TOLLWAY ITS CCTV CAMERA.
- 5. CONSTRUCT MECHANICALLY STABILIZED EARTH RETAINING WALLS, EARTH EMBANKMENTS, DRAINAGE STRUCTURES, AND COLLATERAL ITEMS.
- 6. CONSTRUCT SOUTHBOUND RAMP X3 (BRIDGE NO. 1682) AND NORTHBOUND RAMP X4 (BRIDGE NO. 1681) ABUTMENTS,
- 7. ERECT BRIDGE GIRDERS AND CONSTRUCT BRIDGE SUPERSTRUCTURES.

DATE 06/12/18 DRAWN BY CHECKED BY BRH DATE 06/12/18



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		REVISIONS	CONTRACT NO. I-18-4694	MOT-01
10.	DATE	DESCRIPTION	CONTRACT NO. 1-10-4694	MOT-OI
			MAINTENANCE OF TRAFFIC	DRAWING NO.
				24 of 220
			GENERAL NOTES AND STAGING	27 OF 220



SHOULDER WORK AHEAD









R1-2-48

W21-5a(0)-48

W21-5bL(0)-48

W21-5aL(0)-48

W21-1(0)-48

W21-1a(0)-48

TYPE "A" WARNING LIGHT (TYP)



ROAD CONSTRUCTION AHEAD

W20-1a(0)-48



W20-3(0)-48



HIT A WORKER \$10,000 FINE 14 YRS JAIL

W21-I116-4860 WORK ZONE PUBLIC INFORMATION SIGN

WORK ZONE

SPEED LIMIT

BEGINS

\$XXX FINE MINIMUM

END WORK ZONE SPEED LIMIT

W20-7(0)-48

ROAD CLOSED

RAMP CLOSED

TS-6

WILL BE CLOSED



G20-I103-3660

R11-2-4830

TS-4

(SEE NOTE 1)

WORK ZONE SPEED LIMIT SIGN ASSEMBLY



WARNING SIGN (0)-48 LETTERING: 8" (CAPS) (SEE NOTE 2)

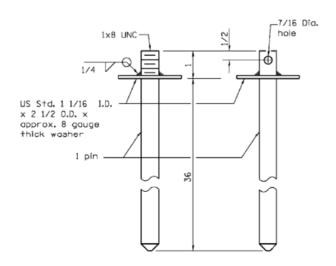
### NOTES:

- 1. THE SIGN IS REQUIRED ON BOTH SIDES OF THE RAMP, MINIMUM OF 1 WEEK IN ADVANCE OF THE CLOSURE.
- 2. THE SIGN IS REQUIRED ALONG HIGGINS RD AT MWRDGC STOCKPILE B BORROW SITE (PLACED 200' IN ADVANCE OF ENTRANCE IN BOTH DIRECTIONS).

# TEMPORARY CONCRETE BARRIER INSTALLATION SCHEDULE

MOT SHEET/	CTACE		TEMPORARY CONCRETE	RELOCATE TEMPORA BARRIER (JI704005)		ANCHORED/	NUMBER OF PINNS	NON-ANCHORED /NON-REST- RAINED LENGTH (FT)	TOTAL TEMPORARY CONCRETE BARRIER IN THIS STAGE LENGTH (FT)
LOCATION	STAGE	LOCATION	BARRIER (JI704000) LENGTH (FT)	WITHIN WORK ZONE	TO STORAGE WITHIN WORK ZONE	RESTRAINED LENGTH (FT)*	(EA)		
MOT-05 - MOT-06	I	I-90 EB	787.5		512.5	787.5	195	0.0	787.5
MOT-06	I	I-90 WB	787.5		525.0	787.5	195	0.0	787.5
SIGN STA 3846+20	I	I-90 EB	412.5			412.5	105	0.0	412.5
SIGN STA 3846+20	I	I-90 WB	412.5			412.5	105	0.0	412.5
MOT-10 - MOT-11	II	I-90 EB		512.5		75	18	437.5	
MOT-11	II	I-90 WB		525.0		75	18	450.0	
	TOTAL		2,400.0	1,037.5	1,037.5		636		

• SEE SPECIAL PROVISION TEMPORARY CONCRETE BARRIER FOR ANCHORING REQUIREMENTS.



# TEMPORARY CONCRETE BARRIER ANCHOR PINS ON ASPHALT

(End may be beveled 1/4 max.)

# ASPHALT PAVEMENT ANCHOR PIN DETAIL **FOR TEMPORARY CONCRETE BARRIER**

DETAIL NOTES:

1. ALL DIMENSIONS ARE IN INCHES UNLESS OTHERWISE SHOWN.

CHECKED BY BRH

JΡ

DATE 06/12/18

DATE 06/12/18

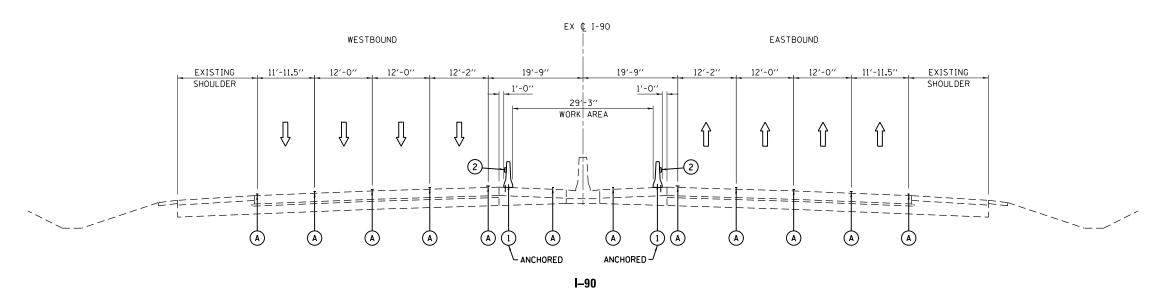


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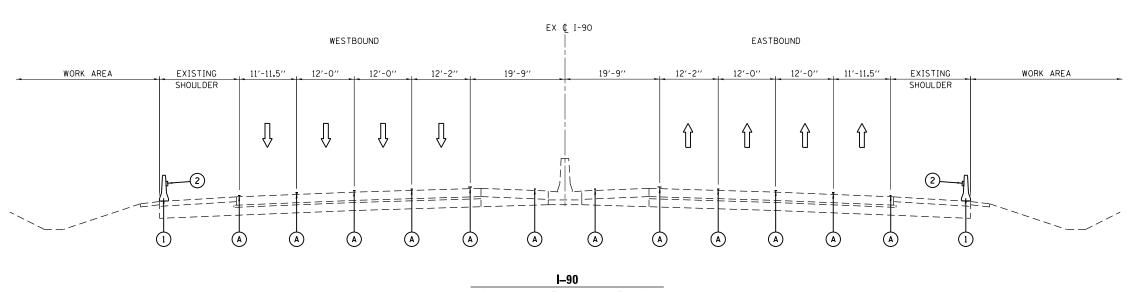


		REVISIONS	CONTRACT NO. I-18-4694	M0T-02		
NO.	DATE	DESCRIPTION	CONTRACT NO. 1-10-4634	MO1-02		
			MAINTENANCE OF TRAFFIC	DRAWING NO.		
			SIGN LOG AND TCB SCHEDULE	25 OF 220		
			SIGN LOG AND ICD SCHEDULE	23 07 220		



# MOT TYPICAL SECTION - STAGE I

FROM STA 3891+12 TO STA 3895+80 SIGN FOUNDATION STA 3846+20 (SEE NOTE 1)



MOT TYPICAL SECTION - STAGE II FROM STA 3890+27 TO STA 3897+51

**LEGEND** 

NOTES:

1. FOR MEDIAN SIGN FOUNDATION CONSTRUCTION STAGING PLAN SEE SHEET MOT-14.

EXISTING TRAFFIC

EXISTING CENTER OF PAVEMENT MARKING LINE TO REMAIN

1 TEMPORARY CONCRETE BARRIER (J1704000)

BARRIER WALL REFLECTORS, TYPE C (JI782022) (SEE TOLLWAY STANDARD D4)

CHECKED BY BRH

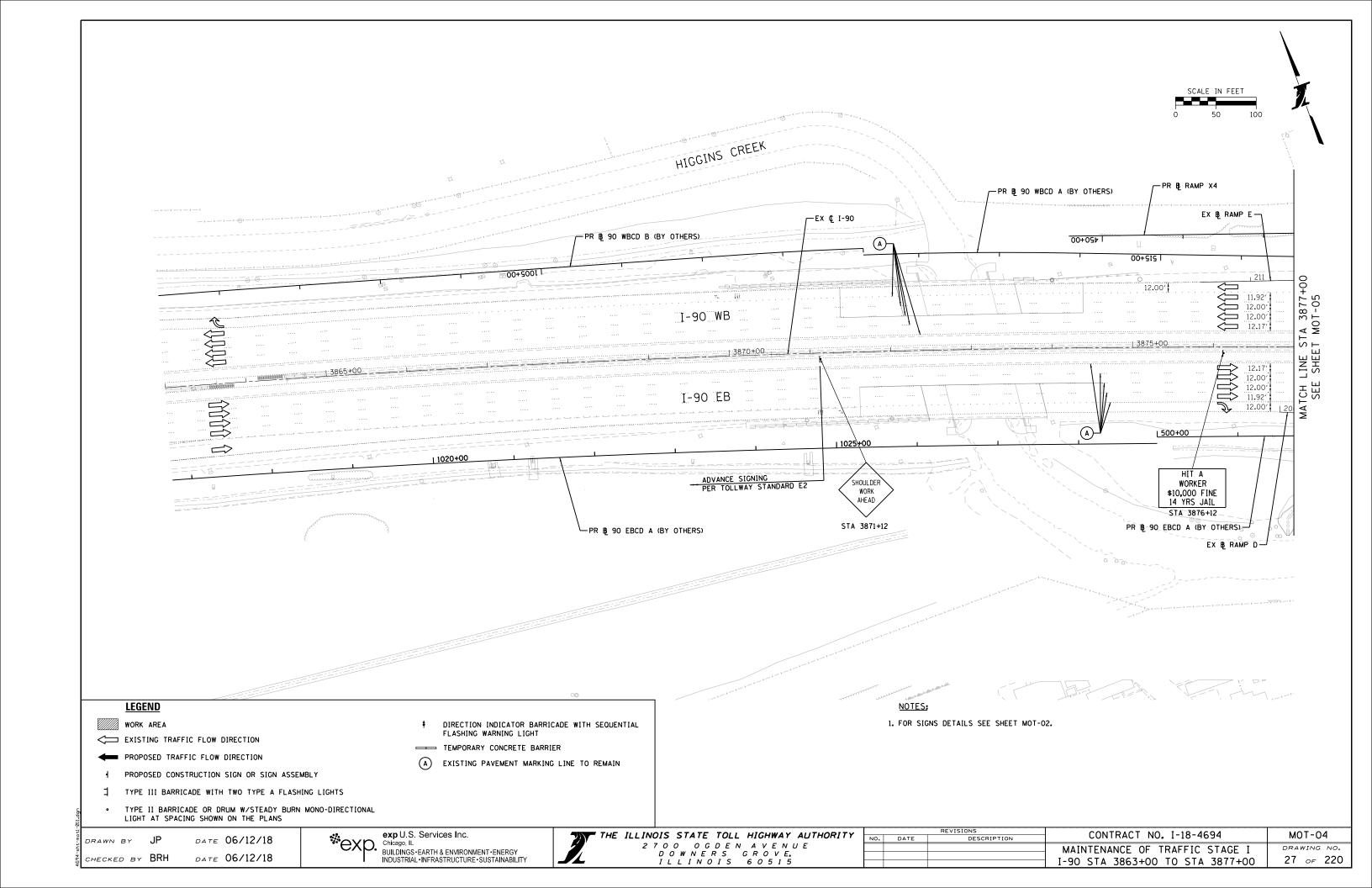
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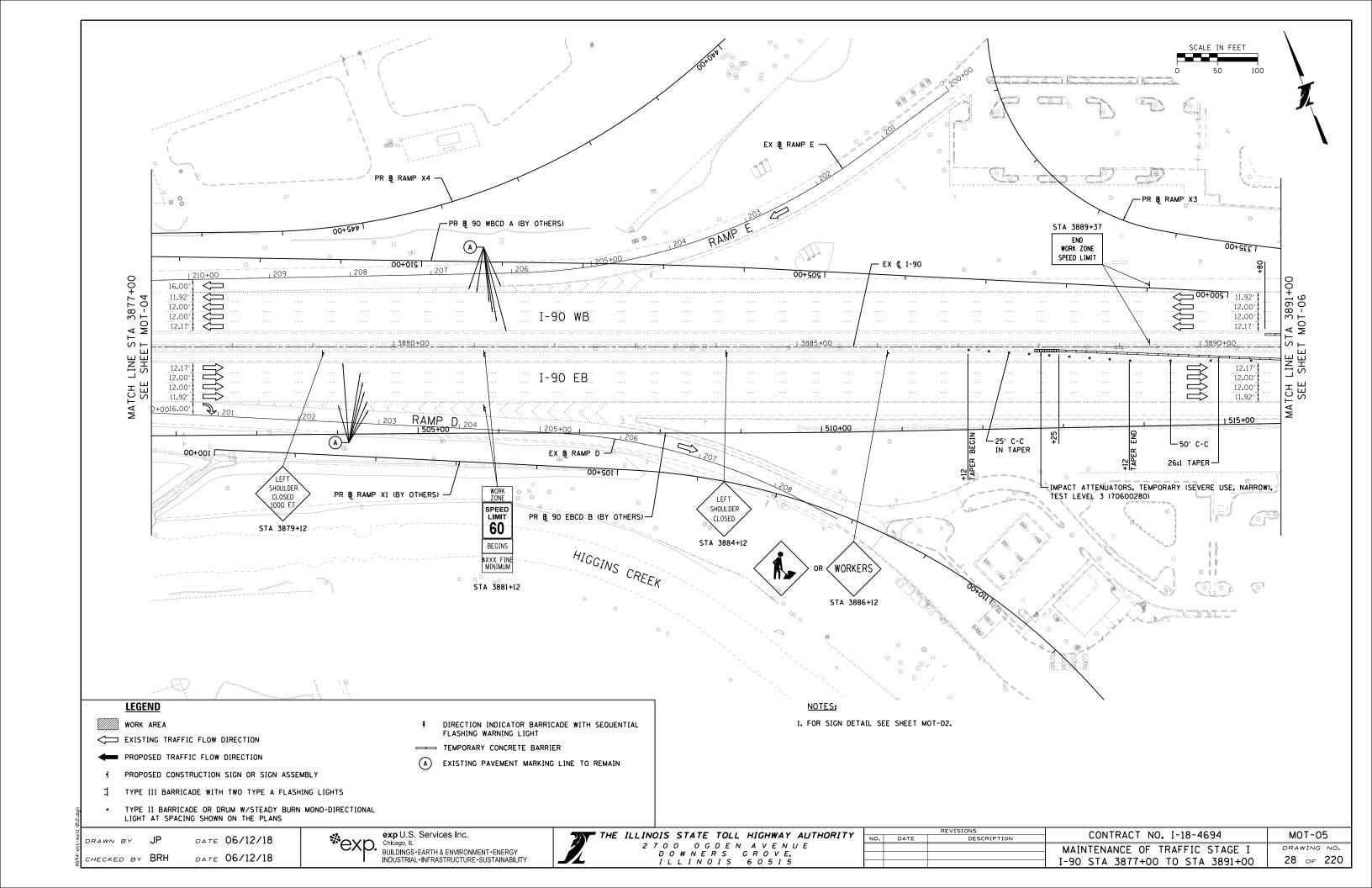
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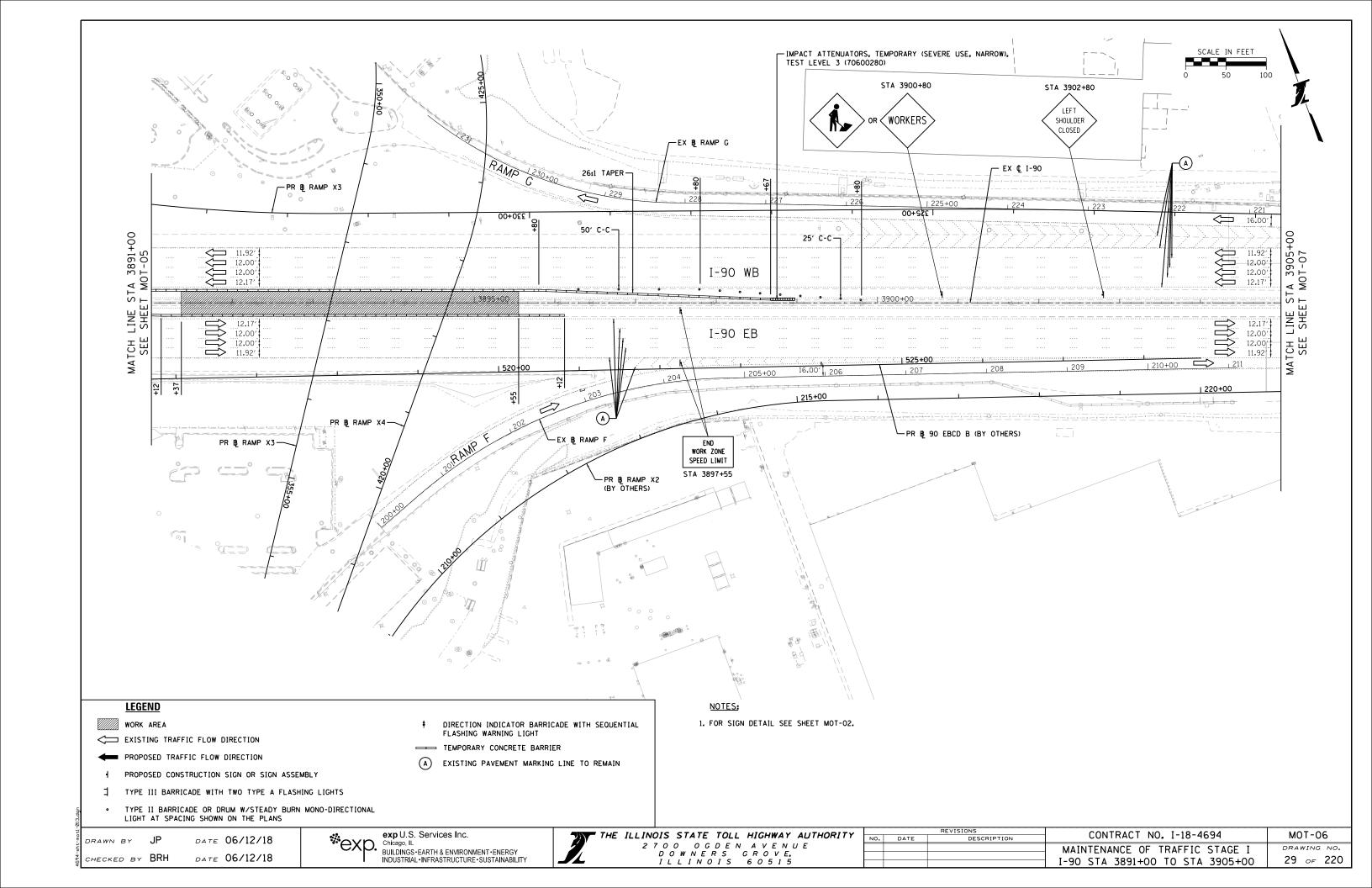
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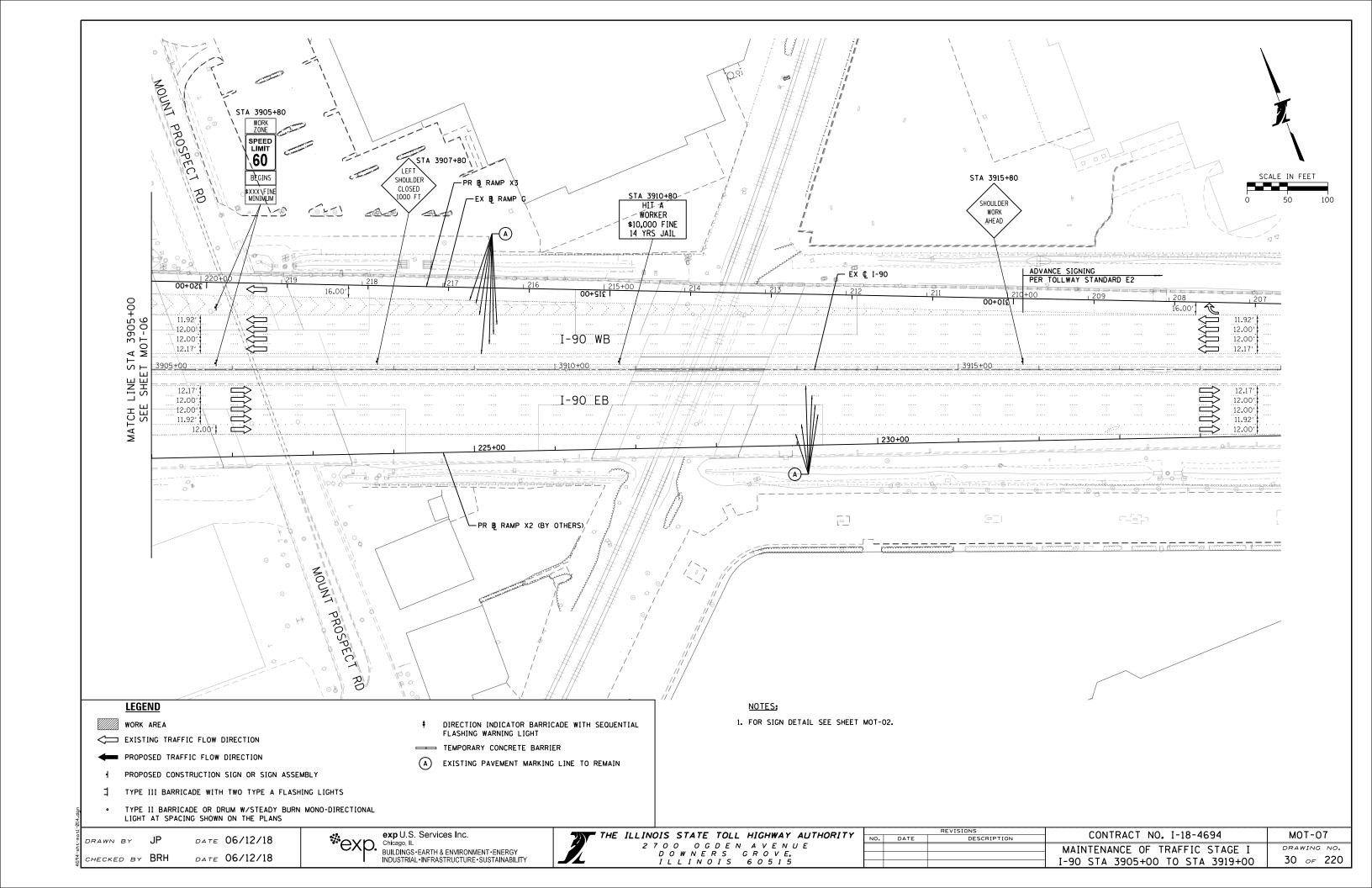


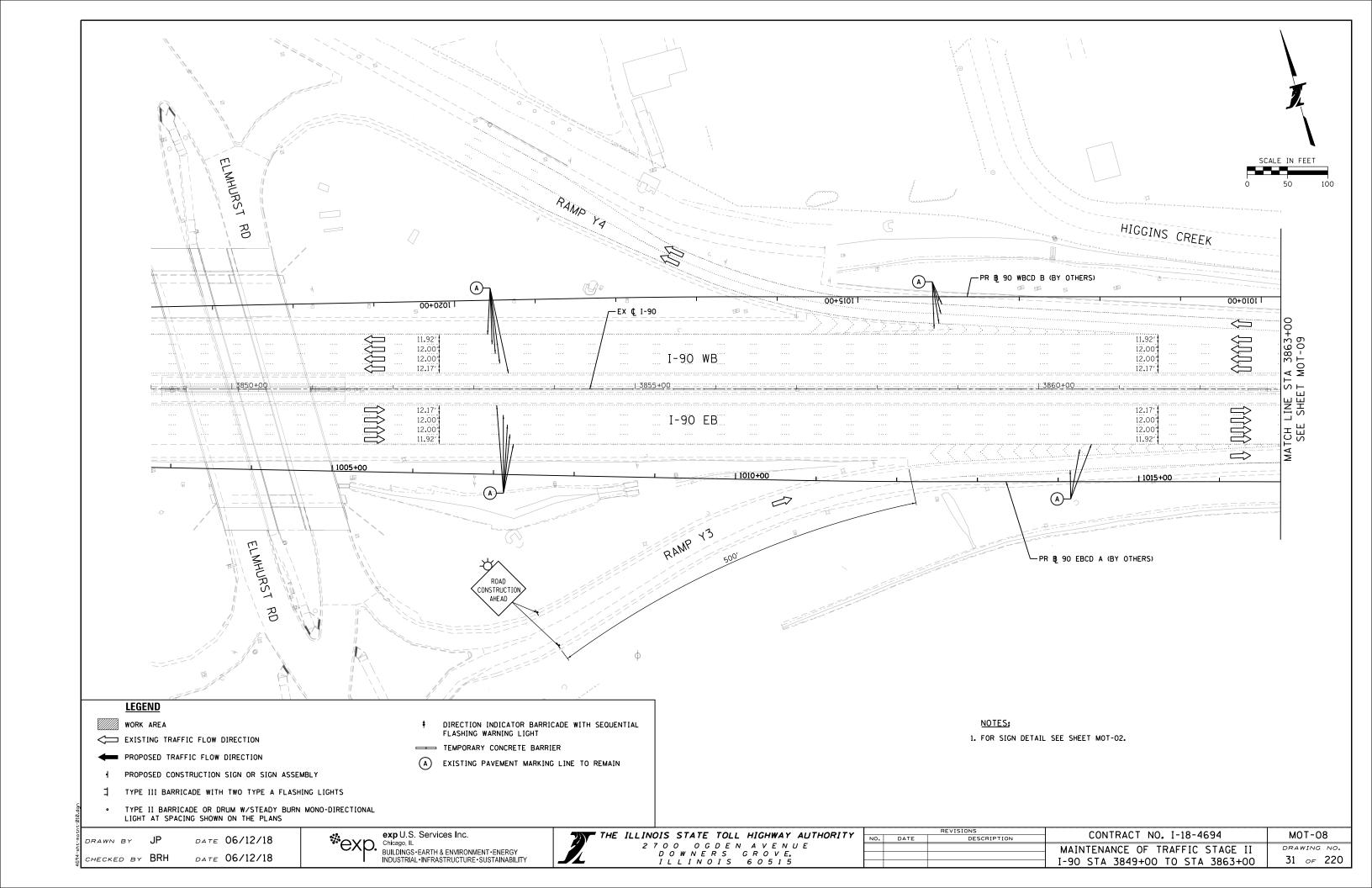
REVISIONS			CONTRACT NO 1 19 4CO4	MOT-03		
0.	DATE	DESCRIPTION	CONTRACT NO. I-18-4694	MO1-03		
			MAINTENANCE OF TRAFFIC	DRAWING NO.		
				26 220		
			TYPICAL SECTIONS	26 of 220		
			·			

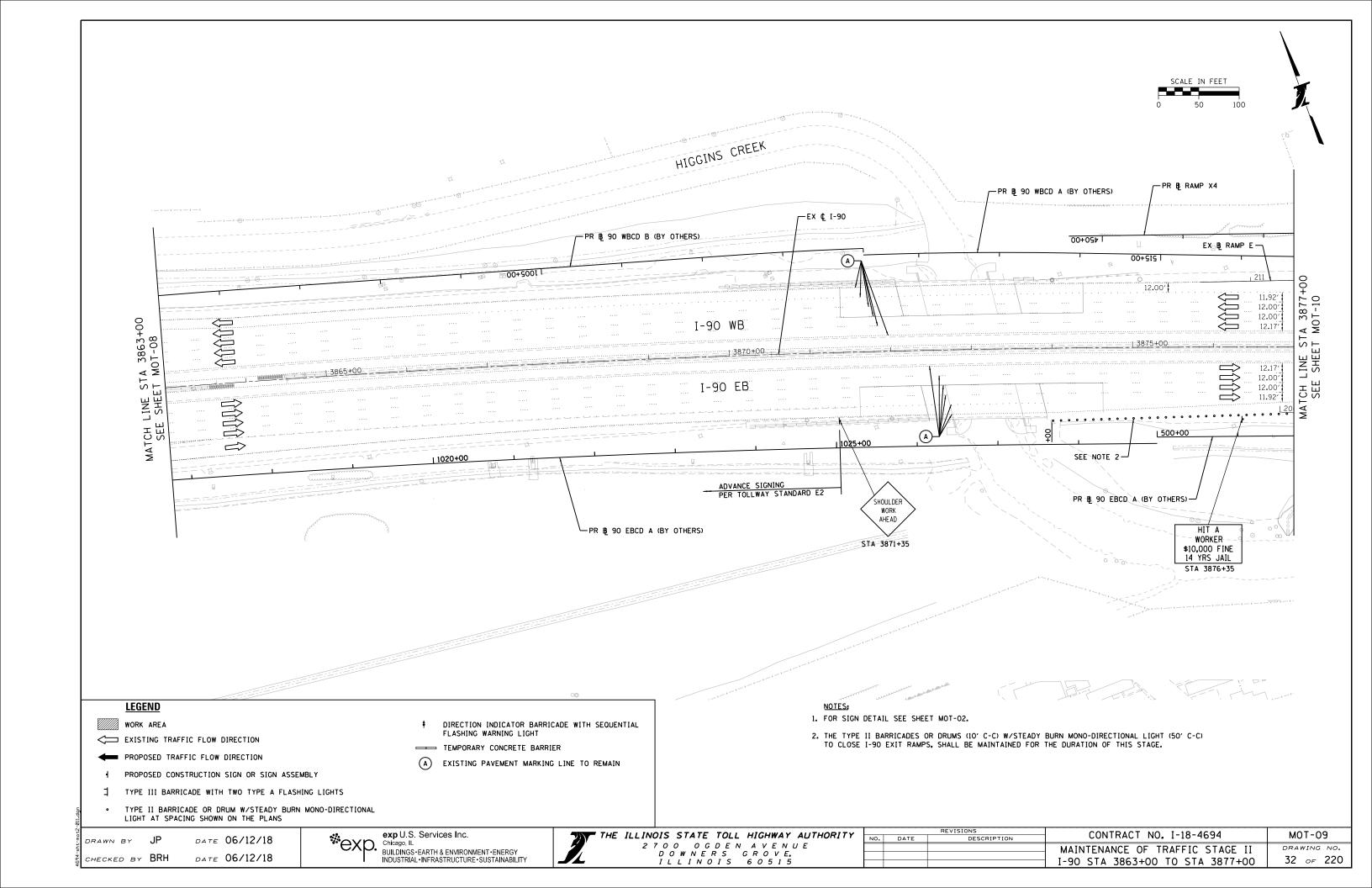


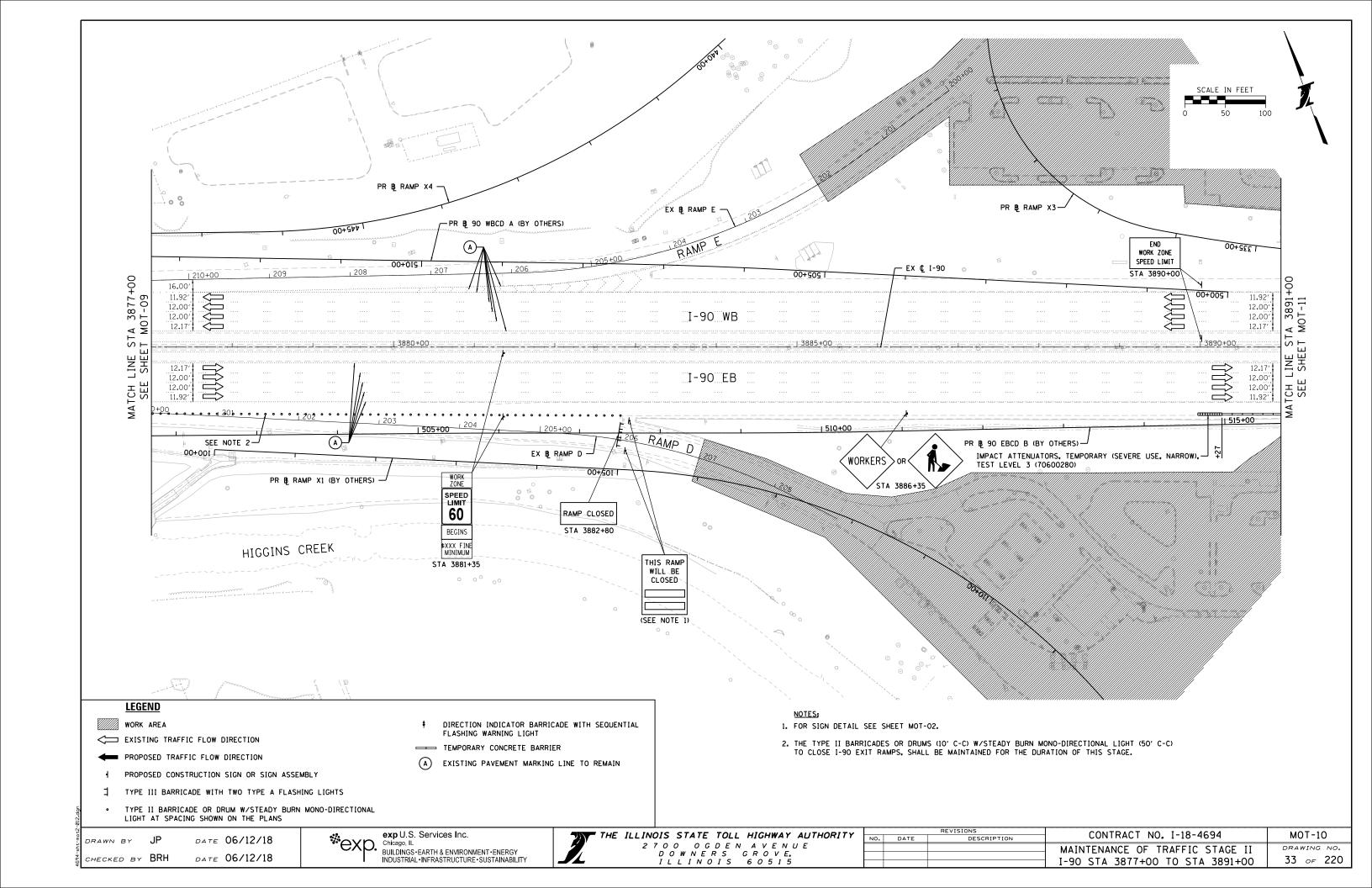


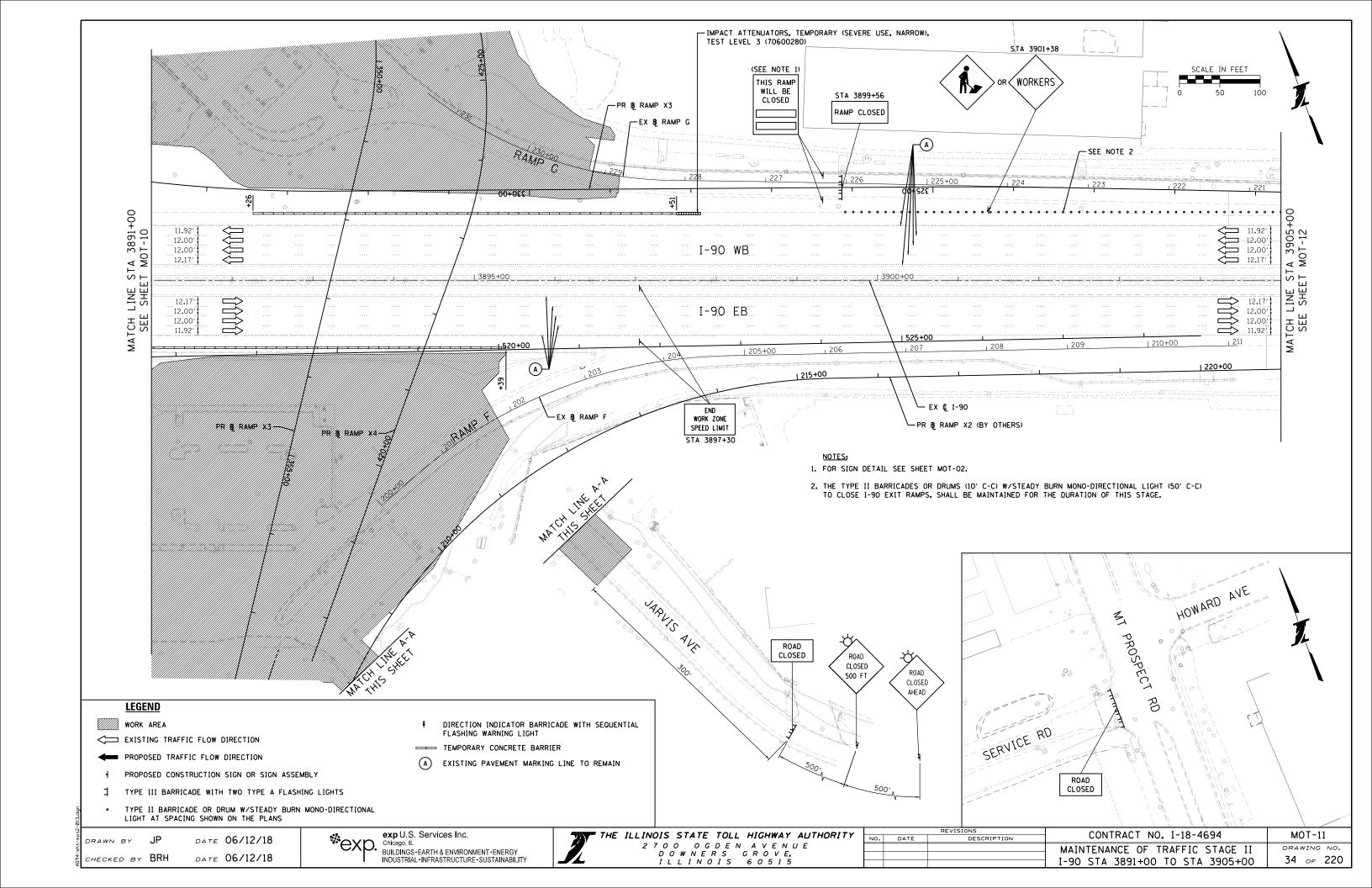


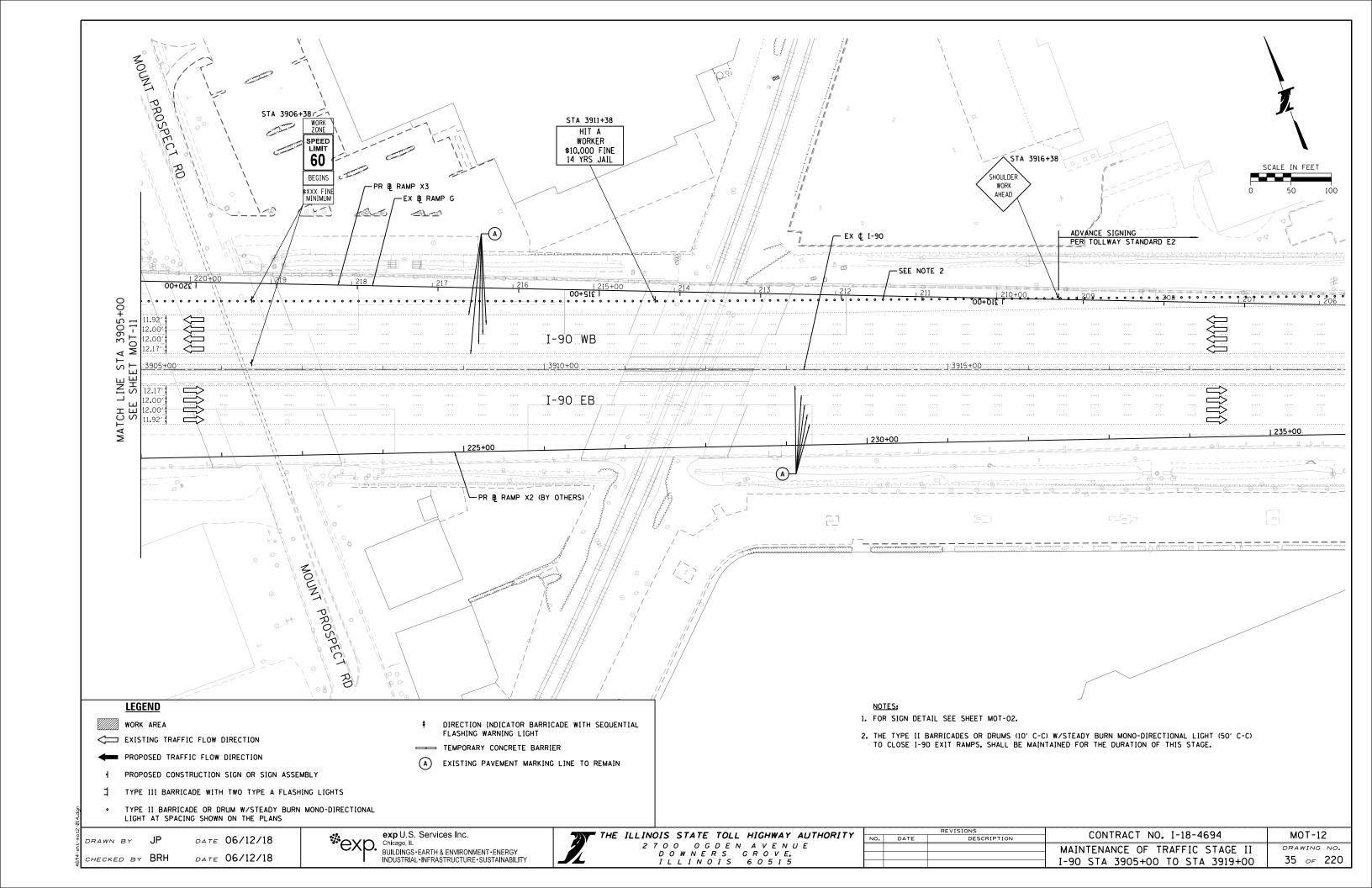


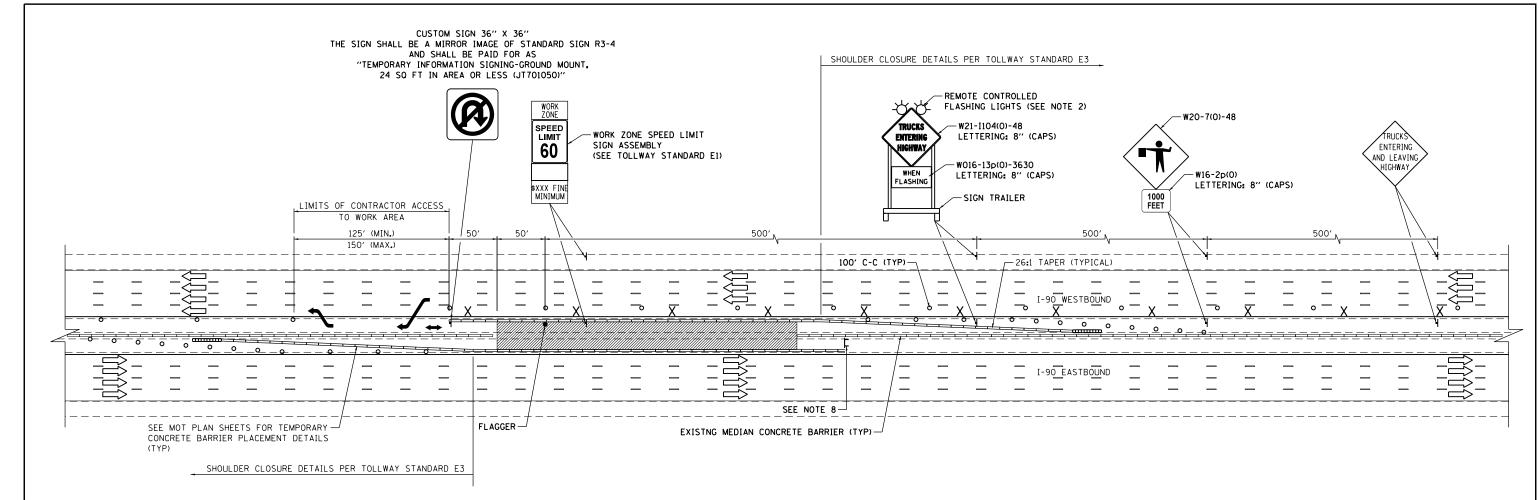












## CONTRACTOR ACCESS TO MEDIAN WORK AREA DETAIL



WORK AREA

EXISTING TRAFFIC FLOW DIRECTION

TEMPORARY CONCRETE BARRIER

- PROPOSED CONSTRUCTION SIGN ON SUPPORT PER ILLINOIS TOLLWAY STANDARD UNLESS NOTED
- DRUM WITH STEADY BURN MONO-DIRECTIONAL LIGHT
  THE IMPACT ATTENUATOR, TEMPORARY (SEVERE USE, NARROW)

TEST LEVEL 3

→ TRUCK MOVEMENT DIRECTION

- X TEMPORARY CLOSURE DURING ALLOWABLE LANE CLOSURE TIMES
- TYPE III BARRICADE

### NOTES:

- SIGNS DESIGNATED FOR THIS ACCESS TO WORK AREA SHALL BE COVERED OR TURNED AWAY FROM THE TRAFFIC WHEN THE FLAGGER IS NOT ON STATION AND THE ACCESS OPENINGS ARE NOT IN USE.
- THE FLASHING WARNING LIGHT SHALL MEET THE REQUIREMENTS OF ILLINOIS TOLLWAY SUPPLEMENTAL SPECIFICATIONS AND BE OPERATED BY THE FLAGGER REMOTELY. THE LIGHTS SHALL BE FLASHING ONLY WHEN A VEHICLE IS ENTERING THE ILLINOIS TOLLWAY.
- 3. WHEN THREE LANES OR MORE ARE OPENED TO TRAFFIC, ADVANCE WARNING SIGNS AND ASSEMBLIES SHALL BE PROVIDED ON BOTH SIDES OF TRAVELED WAY.
- FOR NIGHT TIME OPERATIONS, TEMPORARY LIGHTING OF CONSTRUCTION ACCESS TO WORK AREA SHALL BE PROVIDED AND SHALL BE PAID FOR AS "NIGHTTIME WORK ZONE LIGHTING (70200100).
- 5. TEMPORARY LANE CLOSURE DURING ALLOWABLE LANE CLOSURE TIMES SHALL BE UTILIZED TO ELIMINATE THE MERGING OF CONSTRUCTION TRAFFIC INTO THROUGH TRAFFIC LANES.
- A 1'-0" MINIMUM/2'-0" DESIRABLE SHY DISTANCE SHALL BE PROVIDED, MEASURED BETWEEN EDGE OF PAVEMENT MARKING TO THE EDGE OF TRAFFIC CONTROL DEVICES.
- 7. "TRUCKS ENTERING HIGHWAY" SIGN MAY BE SUPPORTED BY OPTIONAL POST OR STAND MOUNTED DEVICES WHEN POSITIONED BEHIND TEMPORARY CONCRETE BARRIER.
- 8. THE "CONTRACTOR ACCESS TO MEDIAN WORK AREA DETAIL" IS APPLICABLE TO ACCESS FROM ONLY ONE DIRECTION OF TRAFFIC. BARRICADE SHALL BE IN PLACE WHEN ACCESS TO MEDIAN IS FROM ONLY ONE DIRECTION OF TRAFFIC.
- ACCESS TO MEDIAN WORK AREA FROM BOTH DIRECTIONS OF TRAFFIC SHALL REQUIRE THE "CONTRACTOR ACCESS TO MEDIAN WORK AREA DETAIL" TO BE UTILIZED FOR BOTH DIRECTIONS OF TRAFFIC.

CHECKED BY BRH

DATE 06/12/18

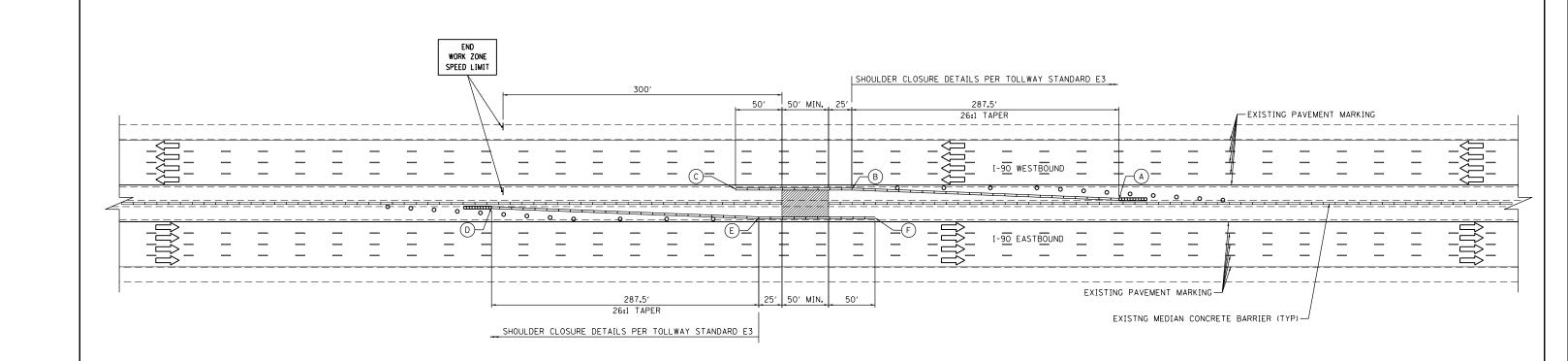
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		REVISIONS	CONTRACT NO. I-18-4694	MOT-13		
o. 2	DATE	DESCRIPTION	CONTRACT NO. 1-10-4694			
			MAINTENANCE OF TRAFFIC	DRAWING NO.		
			- 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1	36 <i>o</i> 220		
			WORK AREA ACCESS DETAIL	36 of 220		



#### MEDIAN SIGN FOUNDATION CONSTRUCTION STAGING PLAN

SIGN FOUNDATION STA 3846+20

SIGN STA.	Α	В	C	D	Ē	F
3846+20	3849+57.5	3846+70.0	3845+45.0	3842+82.5	3845+70.0	3846+95.0

## <u>LEGEND</u>

WORK AREA

DATE 06/12/18

TEMPORARY CONCRETE BARRIER (JI704000)

O DRUM WITH STEADY BURN MONO-DIRECTIONAL LIGHT

IMPACT ATTENUATOR, TEMPORARY (SEVERE USE, NARROW) TEST LEVEL 3 (70600280)

- ACCESS TO MEDIAN WORK AREA SHALL BE IN ACCORDANCE WITH "CONTRACTOR ACCESS TO MEDIAN WORK AREA DETAIL".
- 2. SEE SHEET MOT-13 FOR "CONTRACTOR ACCESS TO MEDIAN WORK AREA DETAIL".
- 3. SEE SHEET MOT-02 FOR TEMPORARY CONCRETE BARRIER INSTALLATION SCHEDULE. ANCHOR PIN DETAIL AND SIGNS DETAILS.

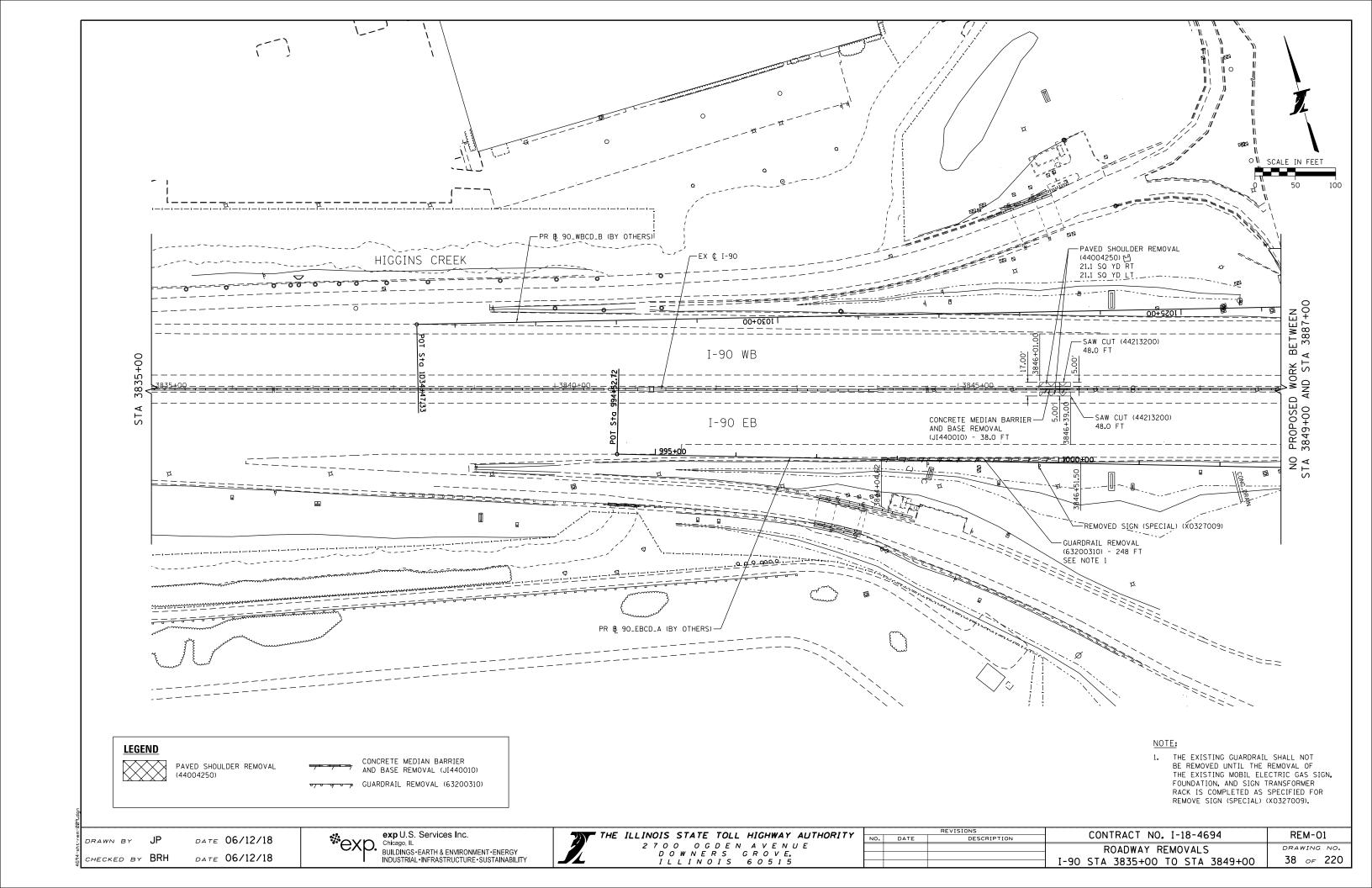
DATE 06/12/18

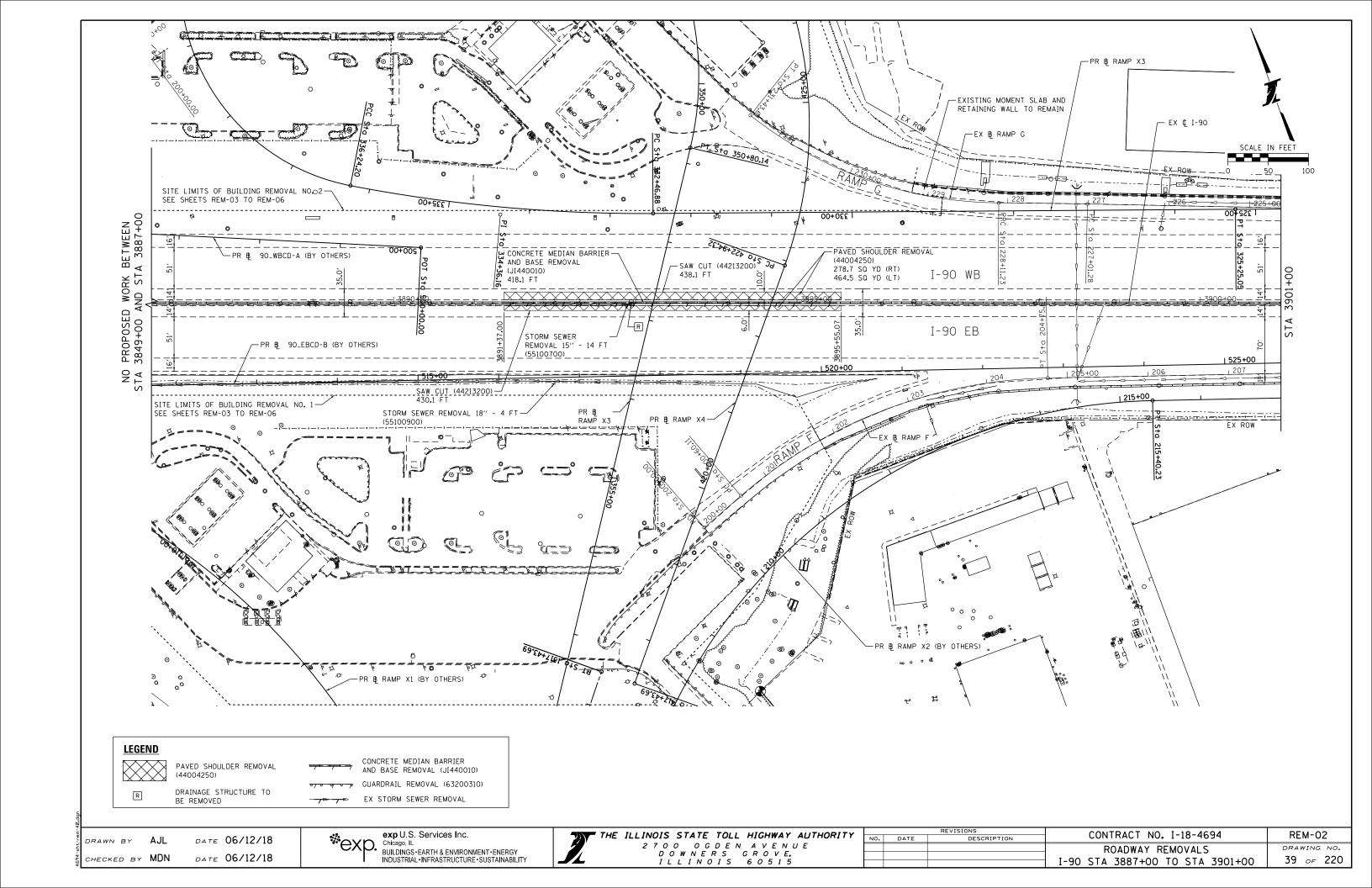
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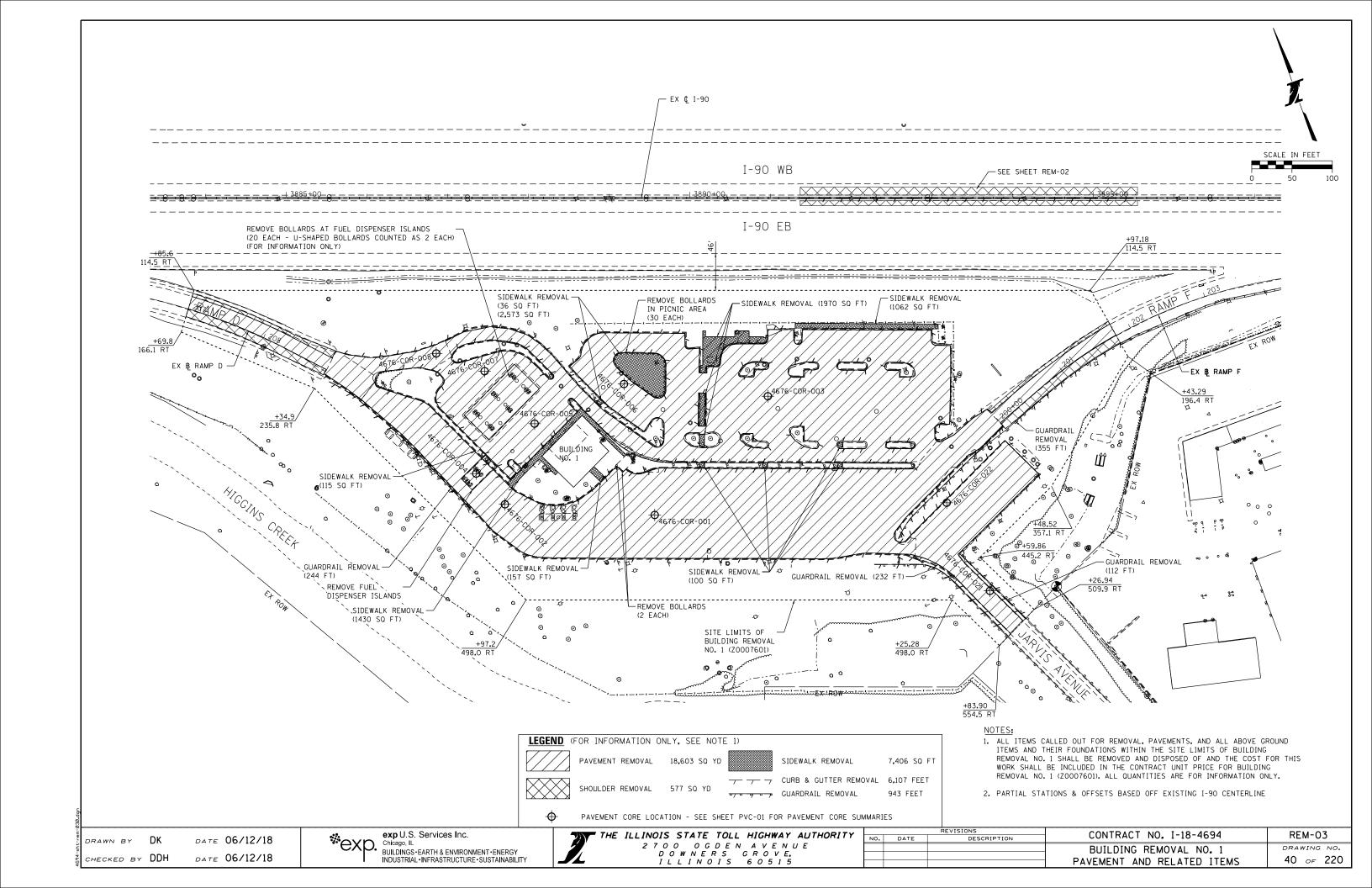
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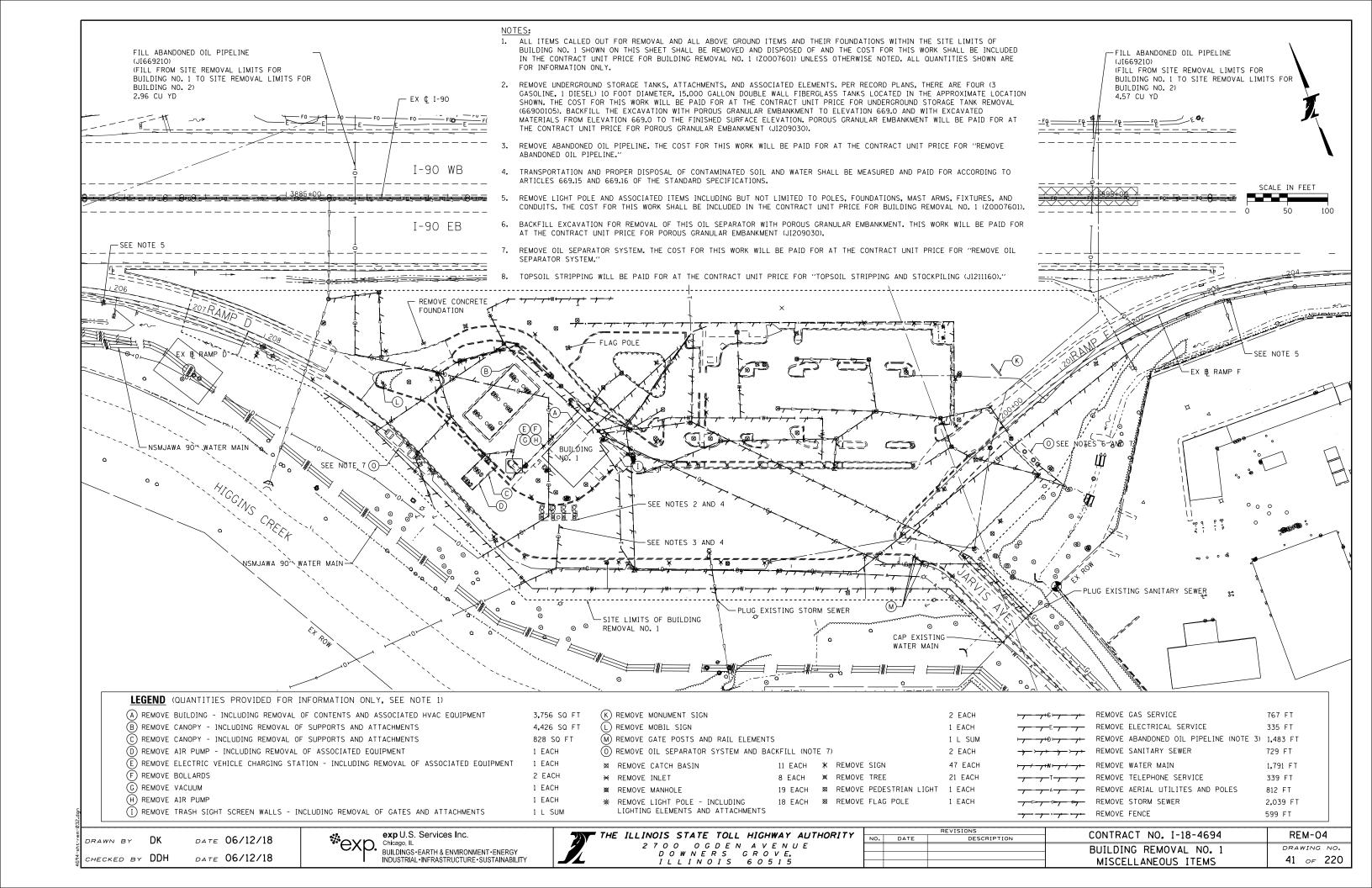
THE ILLINOIS STATE TOLL HIGHWAY AUTHORITY 2700 OGDEN AVENUE DOWNERS GROVE, ILLINOIS 60515

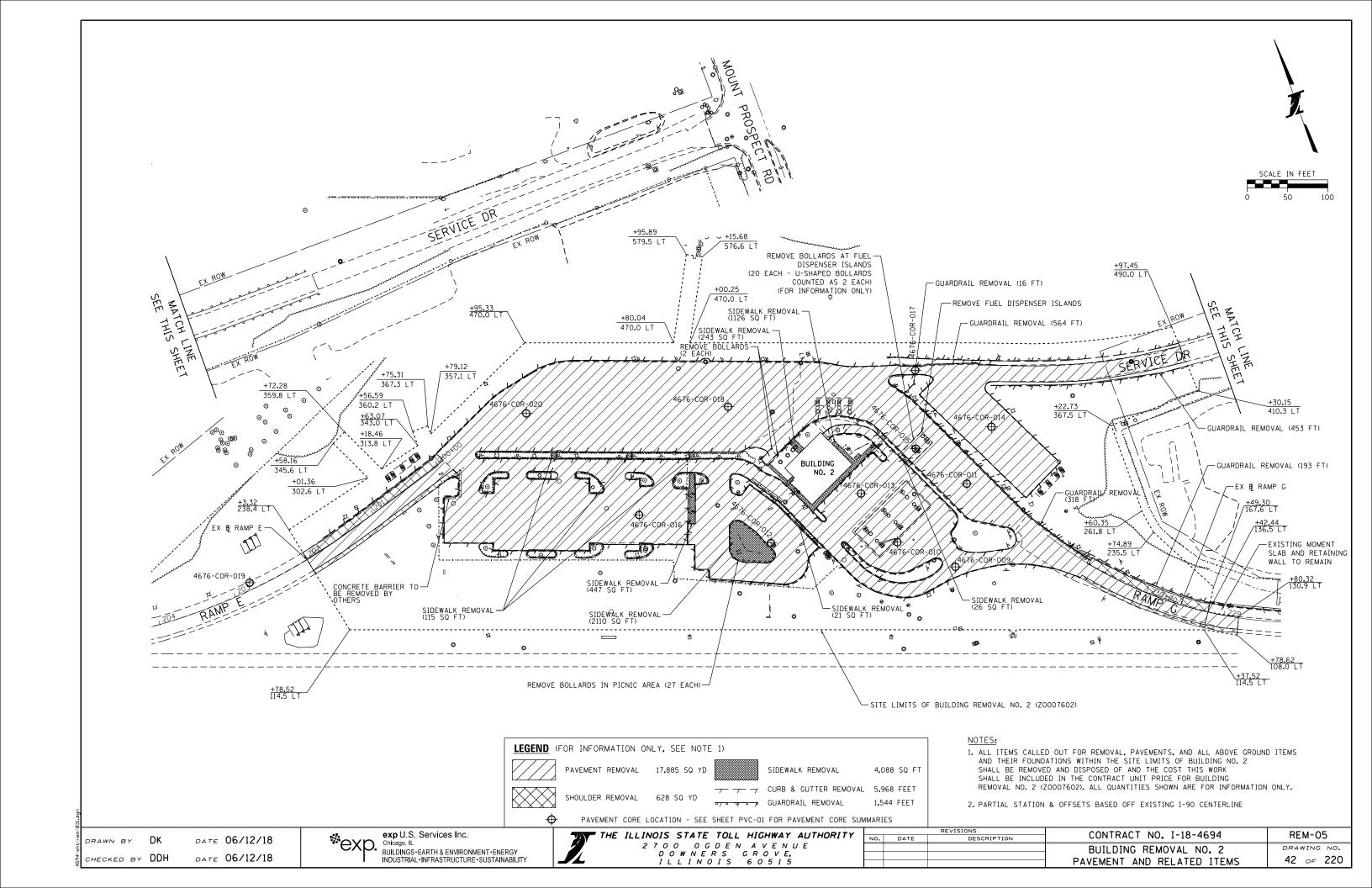
CONTRACT NO. I-18-4694 MOT-14 DESCRIPTION DRAWING NO. MAINTENANCE OF TRAFFIC 37 of 220 AT MEDIAN SIGN FOUNDATION

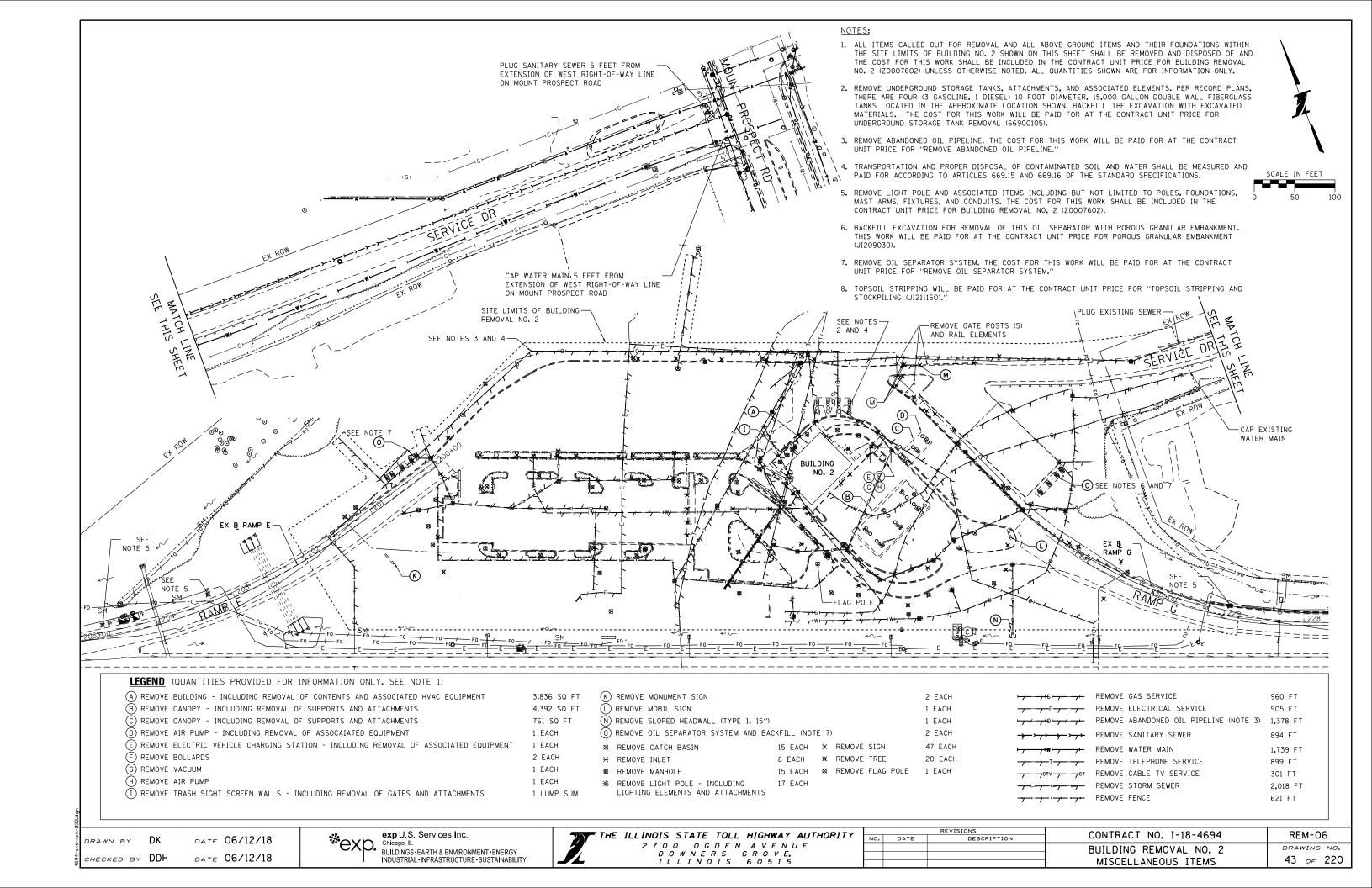


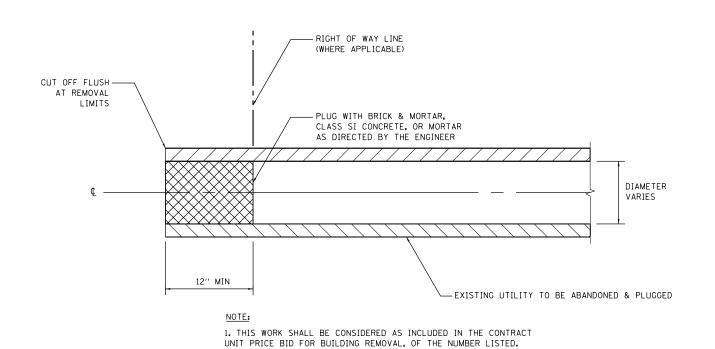












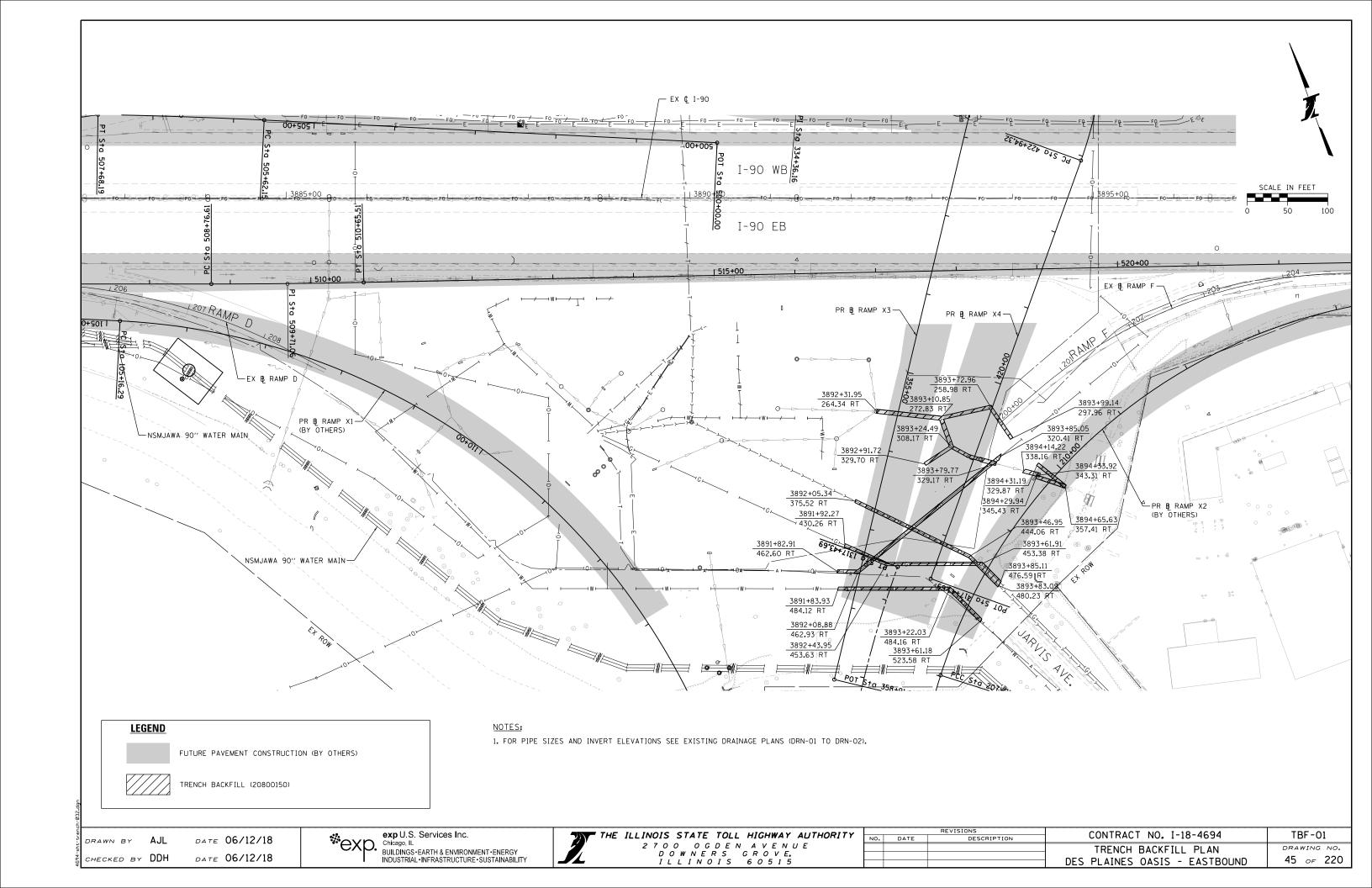
**UTILITY PLUG** 

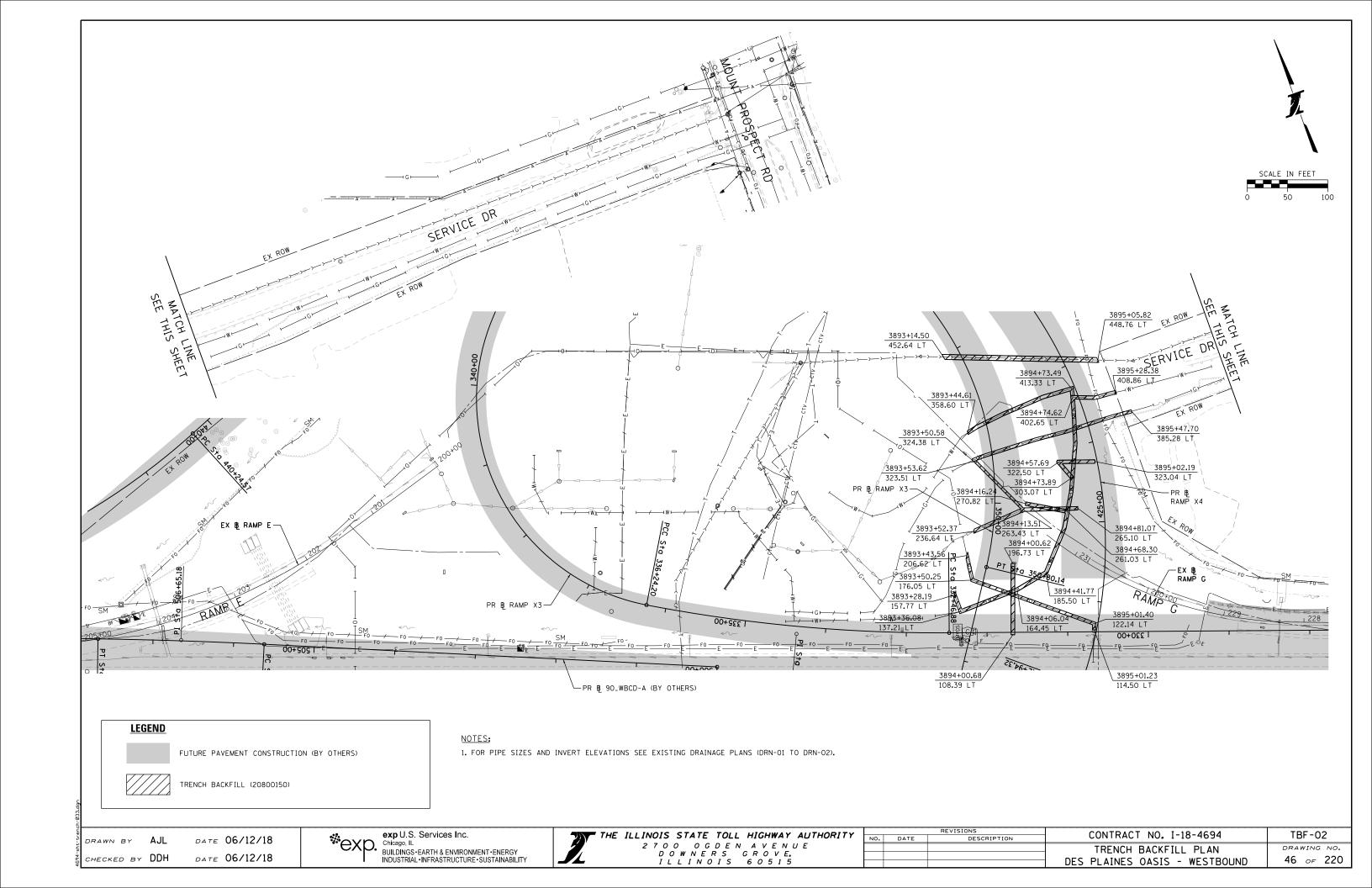
DRAWN BY AJL DATE 06/12/18 CHECKED BY MDN DATE 06/12/18

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INDUSTRIAL INFRASTRUCTURE SUSTAINABILITY



		REVISIONS	CONTRACT NO. I-18-4694	REM-07
10.	DATE	DESCRIPTION	CONTRACT NO. 1-10-4634	KEM-07
			REMOVAL DETAILS	DRAWING NO.
			NEWOYAL BETALLS	44 of 220



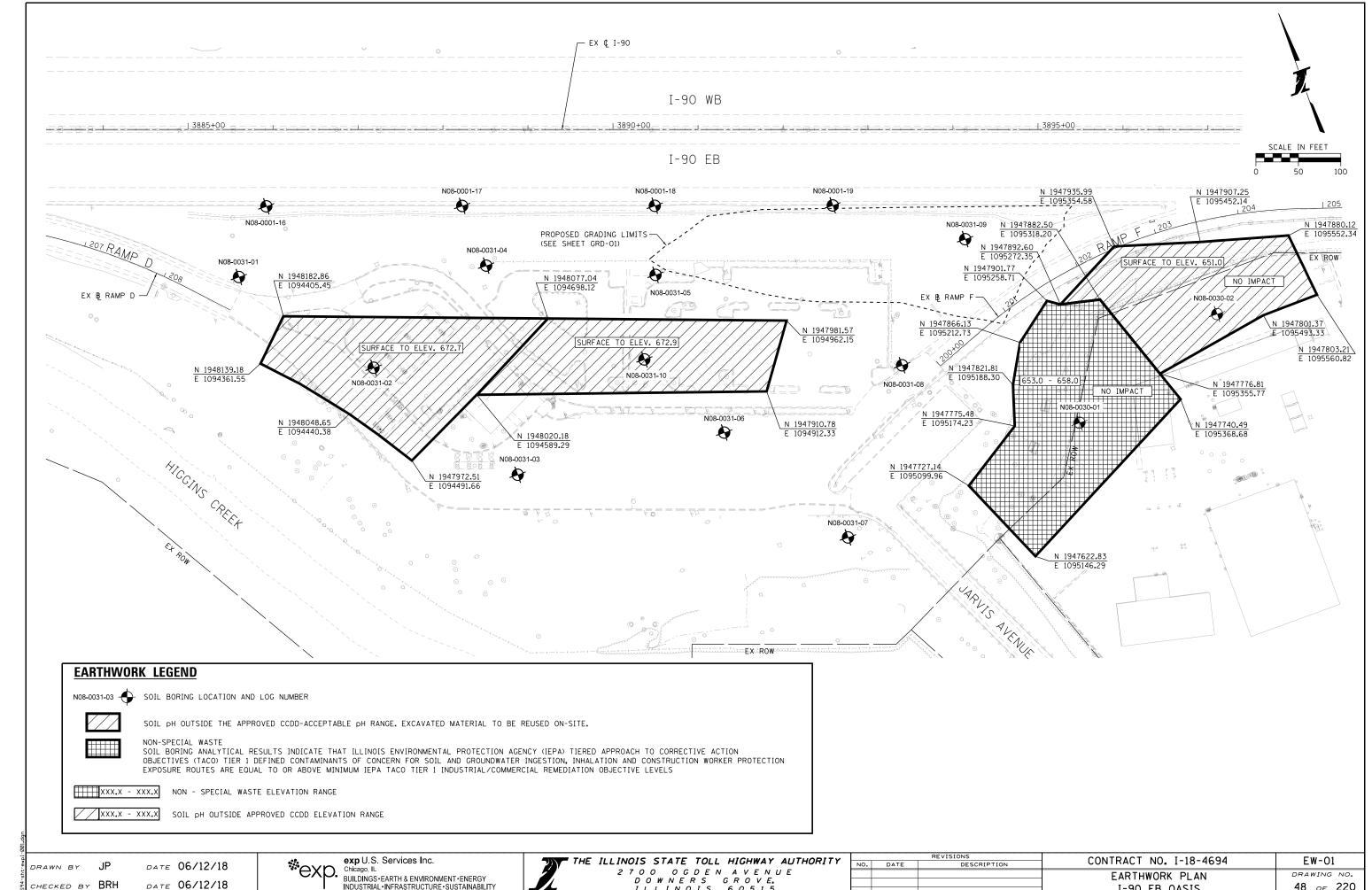


No.	Services, Environmental & Cl Amberat Court, Sulte opervile, Illinois 605 (630) 355+2838	Inc. PAVEMENT CORE Of Displaceting SUMMARY	
Project: <u>IS</u>	THA Contract	-17-4676 Elgin-O'Hare Western Access (EOWA)	
_	90 & 1-490 lr	terchange	Date:4/16/201
County: <u>Co</u> Client: <u>E</u>			Cored By: T
Cijant. <u>C</u>	NF.		Checked by. Ad
CORE NO.	THICKNESS (in.)	MATERIAL DESCRIPTION	
<b>4</b> 676		Station: 3889+56 Offset: 392.8' Right	
COR-001	7.5 6.5 <b>+</b>	CONCRETE—well consolidated SAND, GRAVEL & STONE	
4676		Station: 3887+71 Offset: 379.6' Right	
COR-002		CONCRETE—well consolidated CRUSHED STONE	
<b>4</b> 676		Station: 3890+97 Offset: 245.6' Right	
COR-003	1.5 1.75 8.75+	ASPHALT—well consolidated, fine to medium coarse aggregate. ASPHALT—well consolidated, fine to medium coarse aggregate. SAND & CRAVEL.	
		Station: 3887+35 Offset: 341.5' Right	
4676 COR-004	8.75 9.25	CONCRETE—well consolidated CRUSHED STONE SANDY GRAVELLY CLAY	
		Station: 3888+10 Offset: 279.6' Right	
4676 COR-005	2.0 2.0 3.0 11.0+	ASPHALT-well consolidated, fine to medium coarse aggregate. ASPHALT-well consolidated, fine to medium coarse aggregate. ASPHALT-well consolidated, fine to medium coarse aggregate. SAND, CRAYEL & STONE	
		Station: 3889+18 Offset: 230.5' Right	
4676 COR-006	2.0 11.5+	ASPHALT—well consolidated, fine to medium coarse aggregate. SAND & GRAVEL	
4070		Station: 3887+46 Offset: 214.6' Right	
<b>4</b> 676 COR-007	1.5 1.75 3.0 7.75 4.0 6.0+	ASPHALT—well consolidated, fine to medium coarse aggregate. ASPHALT—well consolidated, fine to medium coarse aggregate. ASPHALT—well consolidated, fine to medium coarse aggregate. SAND, GRAYEL & STONE TOPSOIL—black SILTY SAND	
4070		Station: 3886+86 Offset: 192.7' Right	
4676 COR-008	11.75 12.25+	CONCRETE—well consolidated CRUSHED STONE	
<b>4</b> 676		Station: 3893+29 Offset: 192.5' Left	
COR-009	11.0 6.0 9.0+	CONCRETE—well consolidated CRUSHED STONE CLAYEY GRAVEL	
		Station: 3892+58 Offset: 223.1' Left	
<b>4</b> 676 COR <b>—</b> 010	2.25 2.25 3.25 8.25 2.0+	ASPHALT—well consolidated, fine to medium coarse aggregate. ASPHALT—slightly porous, fine to medium coarse aggregate. ASPHALT—slightly porous, fine to medium coarse aggregate. SAND, GRAVEL & STONE—dark brown GRAVEL	

Geotechnica 805	O Services, Environmental & Ch Amberta Court. Suite topervile, Illinois 805 (630) 355-2838	PAVEMENT CORE  If Digineering SUMMARY  SO SUMMARY	Page: 2 of 3
Project: <u>I</u>	STHA Contract	-17-4676 Elgin-O'Hare Western Access (EOWA)	GSI Job No.: 170
Location: <u>I</u> -	-90 & <b>J−4</b> 90 Ir	nterchange	Date:4/16/20
County: C	ook		Cored By:
Client: <u>E</u>	XP		Checked By:A
CORE NO.	THICKNESS (in.)	MATERIAL DESCRIPTION	
<b>4</b> 676		Station: 3893+43 Offset: 295.6' Left	
COR-011	8.25 9.75 <b>+</b>	CONCRETE—well consolidated CRUSHED STONE	
<b>4</b> 676		Station: 3891+01 Offset: 222.0' Left	
COR-012	1.5 2.5 17.0 3.0+	ASPHALT—well consolidated, fine to medium coarse aggregate. ASPHALT—well consolidated, fine to medium coarse aggregate. SAND & GRAYEL CLAYEY SILT	
<b>4</b> 676		Station: 3892+12 Offset: 283.3' Left	
COR-013	1.5 2.75 2.5 11.25+	ASPHALT—well consolidated, fine to medium coarse aggregate. ASPHALT—slightly porous, fine to medium coarse aggregate. ASPHALT—well consolidated, fine to medium coarse aggregate. SAND, GRAVEL & STONE	
<b>4</b> 676		Station: 3893+74 Offset: 366.1' Left	
COR-014	1.5 1.75 16.75 <b>+</b>	ASPHALT—well consolidated, fine to medium coarse aggregate. ASPHALT—well consolidated, medium coarse aggregate. SAND & GRAVEL	
		Station: 3892+80 Offset: 338.9' Left	
<b>4</b> 676 COR <b>—</b> 015	9.25 6.0 8.75 <b>→</b>	CONCRETE—well consolidated GRAYEL & STONE CLAYEY SAND & GRAYEL	
4070		Station: 3889+37 Offset: 256.8' Left	
4676 COR-016	2.0 1.5 16.5 <b>+</b>	ASPHALT—slightly porous, fine to medium course aggregate. ASPHALT—well consolidated, medium course aggregate. SAND & GRAVEL	
<b>4</b> 676		Station: 3892+80 Offset: 436.0' Left	
COR-017	3.0 1.25 2.75 17.0 <del>1</del>	ASPHALT-slightly porous, fine to medium coarse aggregate. ASPHALT-slightly porous, fine to medium coarse aggregate. ASPHALT-well consolidated, medium coarse aggregate. SAND & GRAVEL	
4676		Station: 3890+48 Offset: 390.9' Left	
COR-018	8.25 10.25 <b>+</b>	CONCRETE—slightly porous. CRUSHED STONE	
<b>4</b> 676		Station: 3884+55 Offset: 172.9' Left	
COR-019	1.75 3.75 11.5 <del>+</del>	ASPHALT—slightly porous, fine to medium course aggregate. ASPHALT—slightly porous, fine to medium course aggregate. CRUSHED CONCRETE, GRAVEL & BRICK	
<b>4</b> 676		Station: 3887+97 Offset: 382.7' Left	
COR-020	7.5 12.5 <b>+</b>	CONCRETE—well consolidated CRUSHED CONCRETE & GRAVEL	
<b>4</b> 676		Station: 3893+72 Offset: 486.8' Right	
COR-021	2.75 1.5 7.75 <b>+</b>	ASPHALT—slightly porous, fine to medium coarse aggregate. ASPHALT—well consolidated, fine to medium coarse aggregate. SAND & GRAVEL	

			Page: 3 of 3
Ge	Services,	nc. PAVEMENT CORE	
Geotechnical 805	O Services, Environmental & Cl Amnerat Court, Sulte lepetville, Illinois 605 (630) 355-2838	IDigitieering SUMMARY	
	(630) 355-2838	60	
roject: <u>I</u> S	THA Contract	-17-4676 Elgin-O'Hare Western Access (EOWA)	GSI Job No.: 1703
ocation: <u>l-</u>	-90 & <b>J−4</b> 90 Ir	nterchange	Date:4/16/2018
ounty: <u>C</u>			Cored By:T
ient: <u>E</u>	XP		Checked By:AJ
OR <b>E</b> NO.	THICKNESS (in.)	MATERIAL DESCRIPTION	
NO.	(111.)	Station: 3893+19 Offset: 377.6' Right	
676	2.25	-	
-022	1.5 13.25 <b>+</b>	ASPHALT—well consolidated, fine to medium coarse aggregate. ASPHALT—well consolidated, fine to medium coarse aggregate. SAND & GRAVEL	
	I		

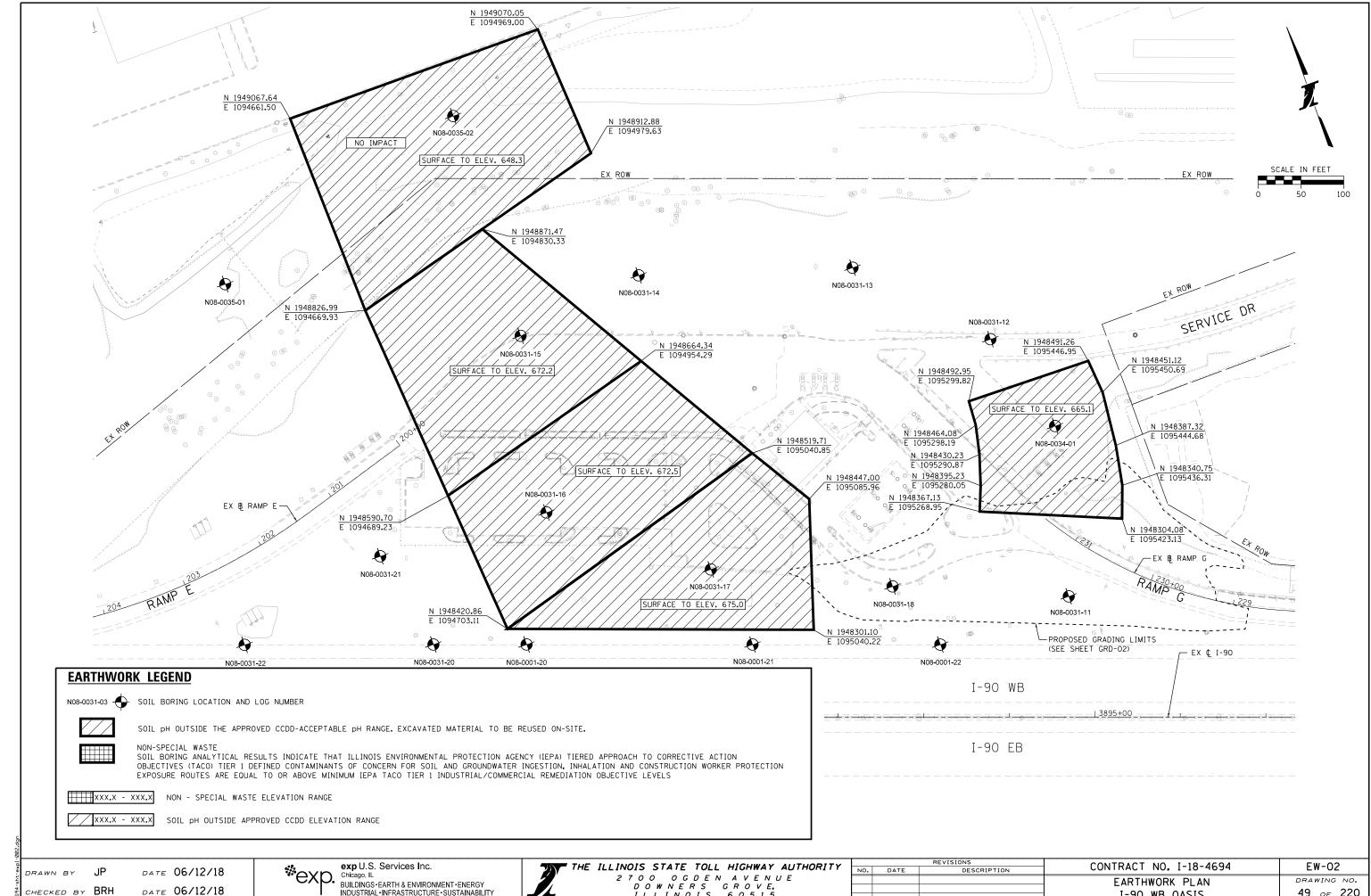




BUILDINGS EARTH & ENVIRONMENT ENERGY INDUSTRIAL INFRASTRUCTURE SUSTAINABILITY

2700 OGDEN AVENUE DOWNERS GROVE, ILLINOIS 60515

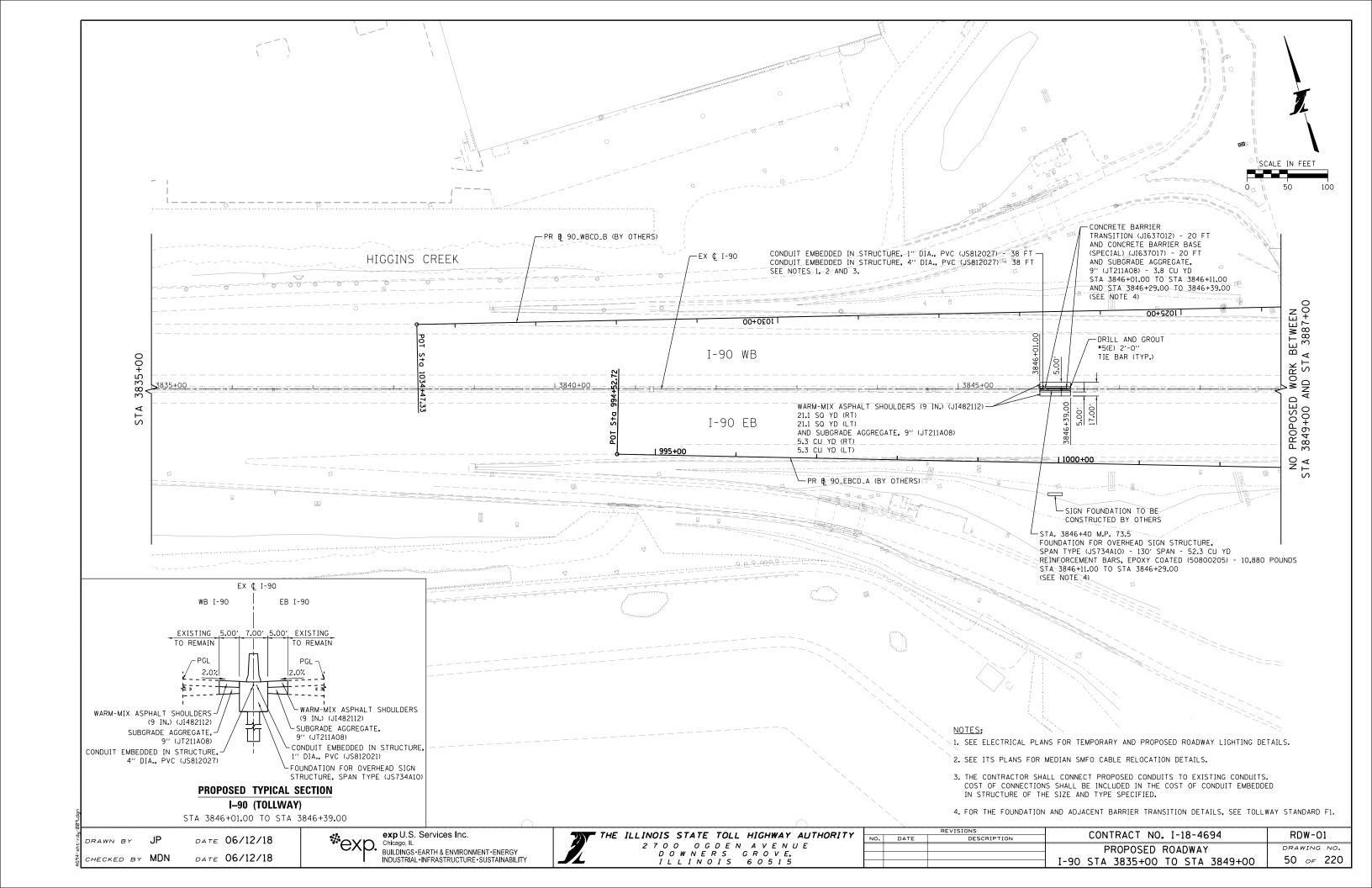
		REVISIONS	CONTRACT NO. I-18-4694	EW-O1
10.	DATE	DESCRIPTION	CUNTRACT NO. 1-10-4634	EM-OI
			EARTHWORK PLAN	DRAWING NO.
				48 of 220
			I-90 EB OASIS	48 of 220
				•

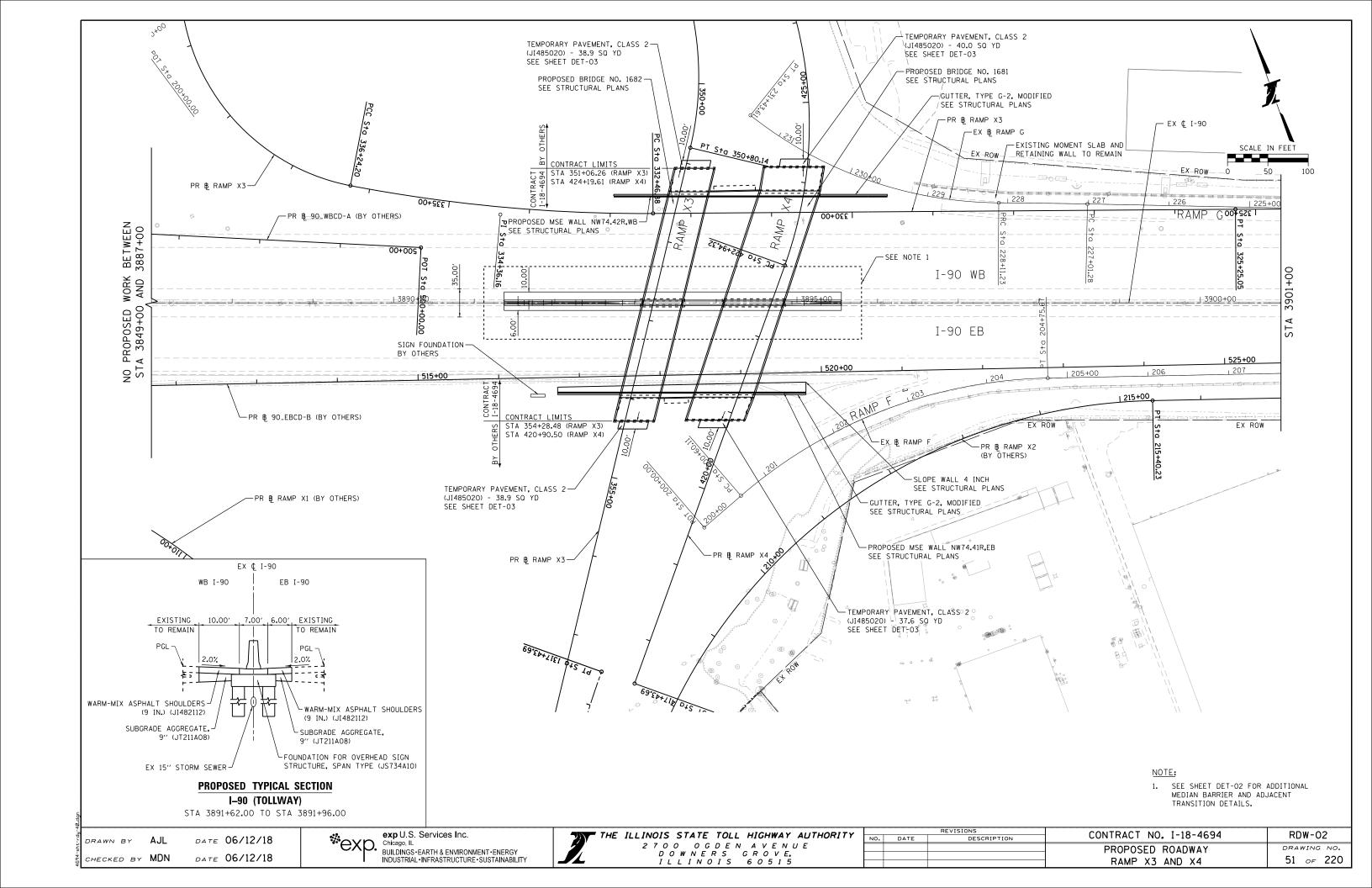


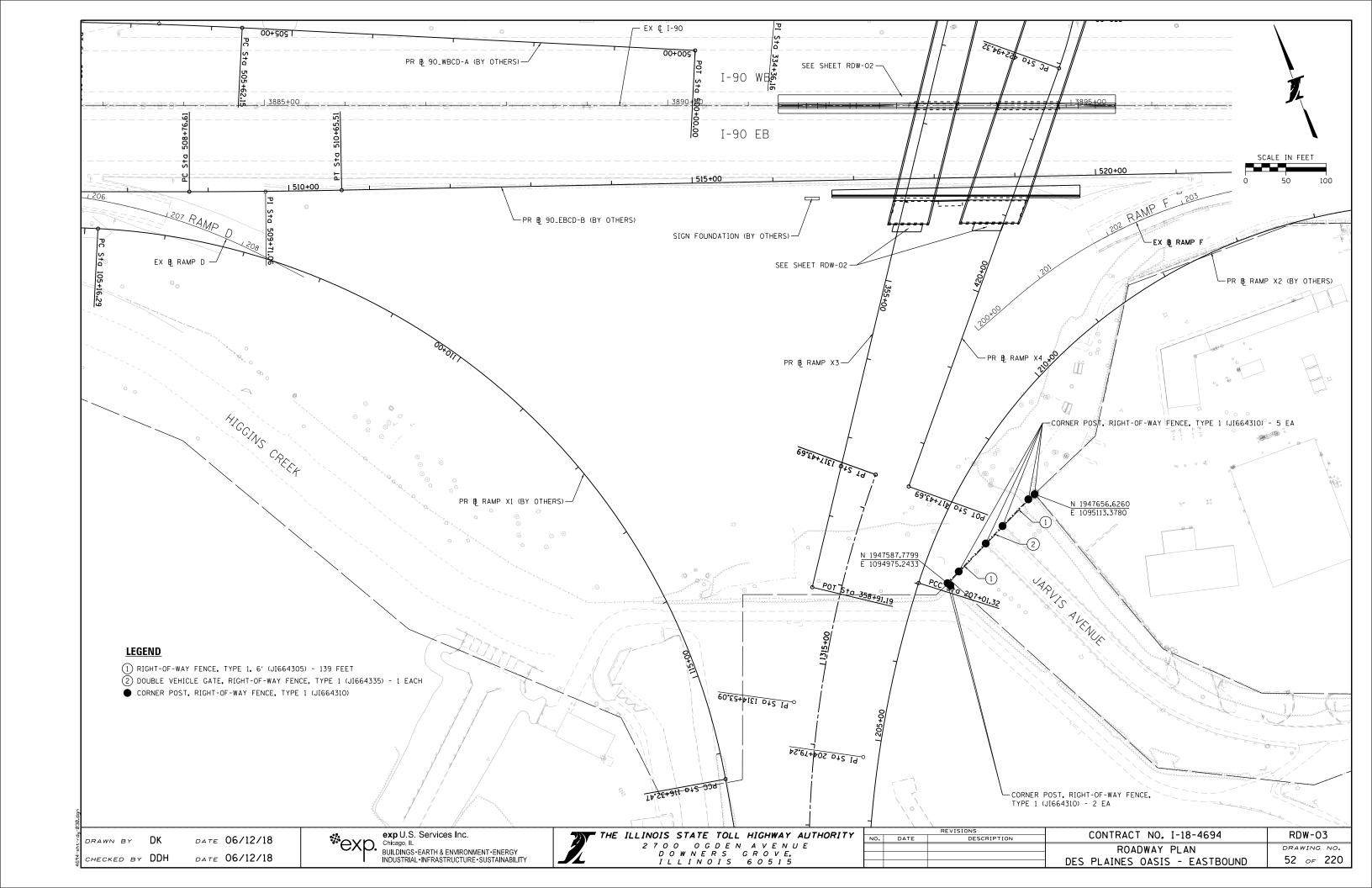
BUILDINGS EARTH & ENVIRONMENT ENERGY INDUSTRIAL INFRASTRUCTURE SUSTAINABILITY

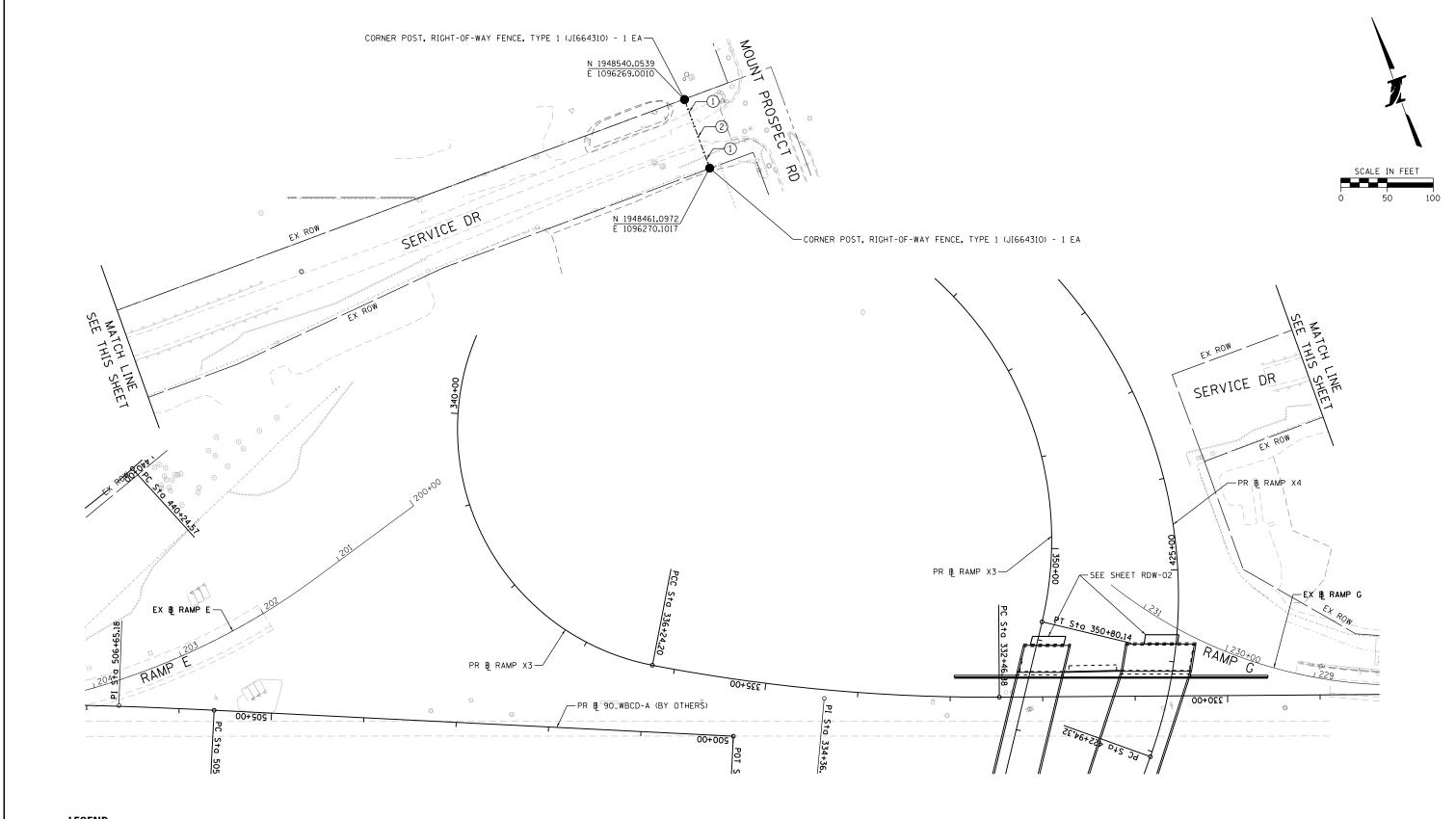
2700 OGDEN AVENUE DOWNERS GROVE, ILLINOIS 60515

49 of 220 I-90 WB OASIS









# **LEGEND**

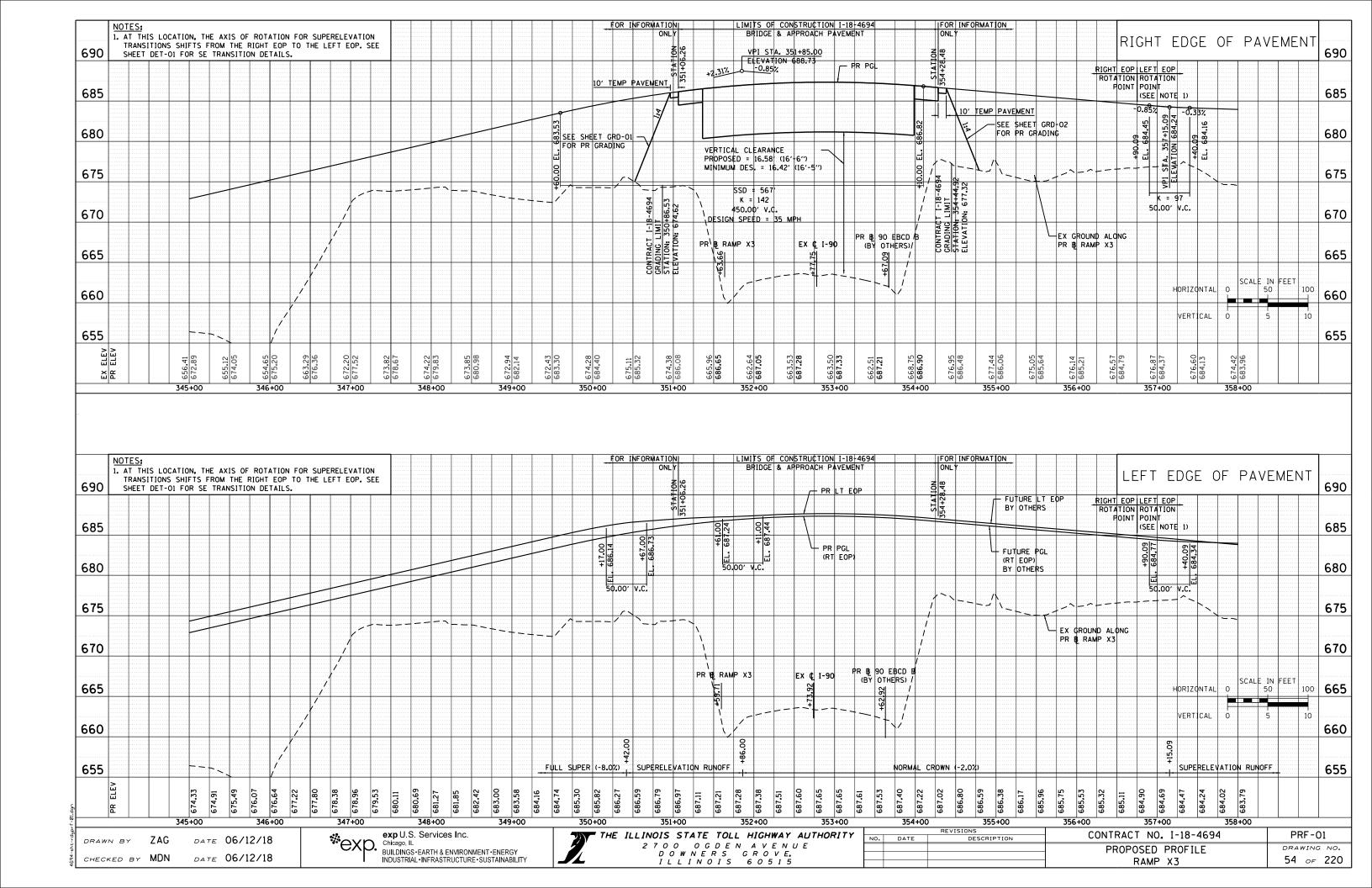
- 1 RIGHT-OF-WAY FENCE, TYPE 1, 6' (JI664305) 59 FEET
- 2 DOUBLE VEHICLE GATE, RIGHT-OF-WAY FENCE, TYPE 1 (J1664335) 1 EACH
- CORNER POST, RIGHT-OF-WAY FENCE, TYPE 1 (JI664310)

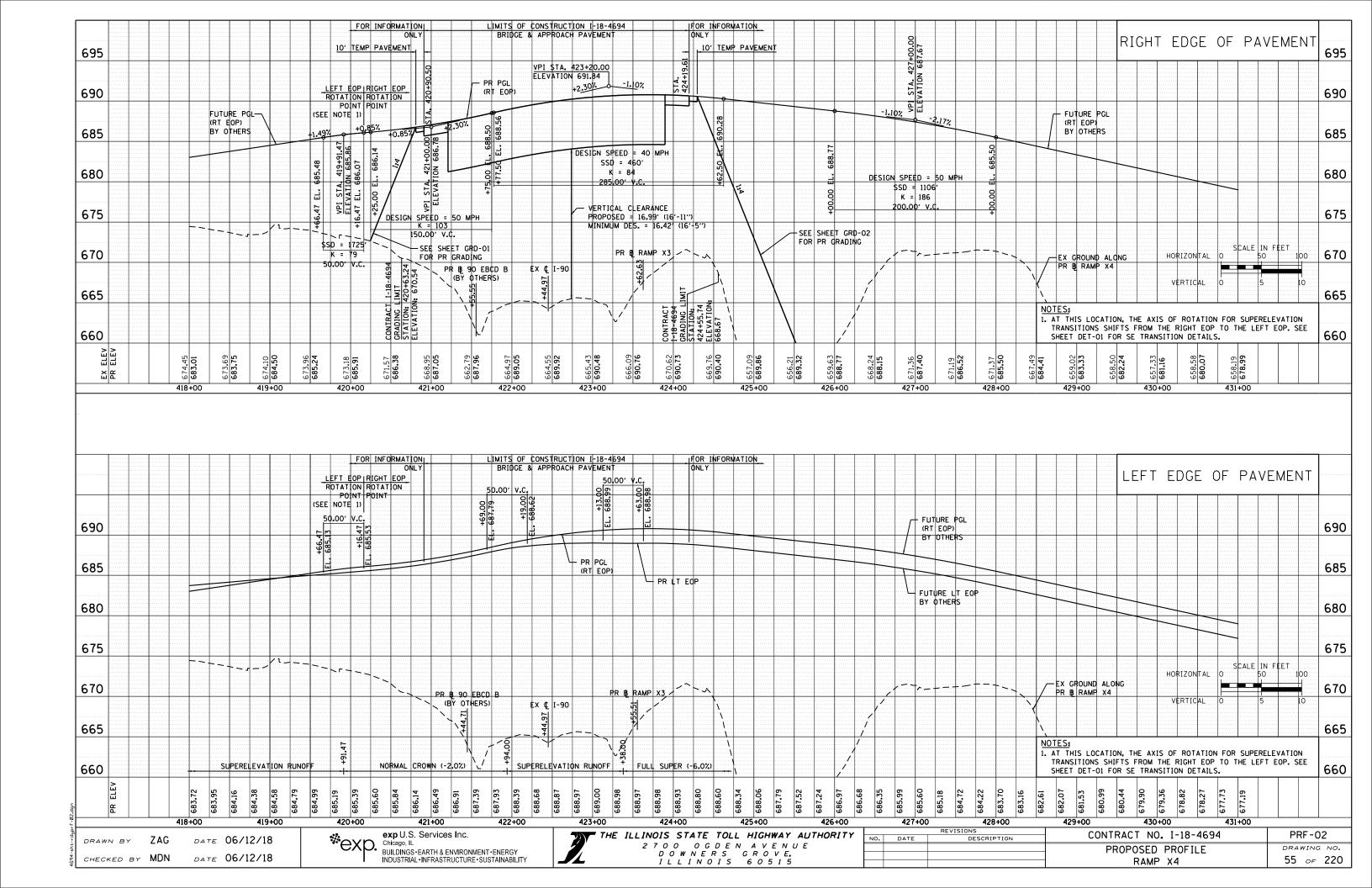
DRAWN BY DK DATE 06/12/18 CHECKED BY DDH DATE 06/12/18

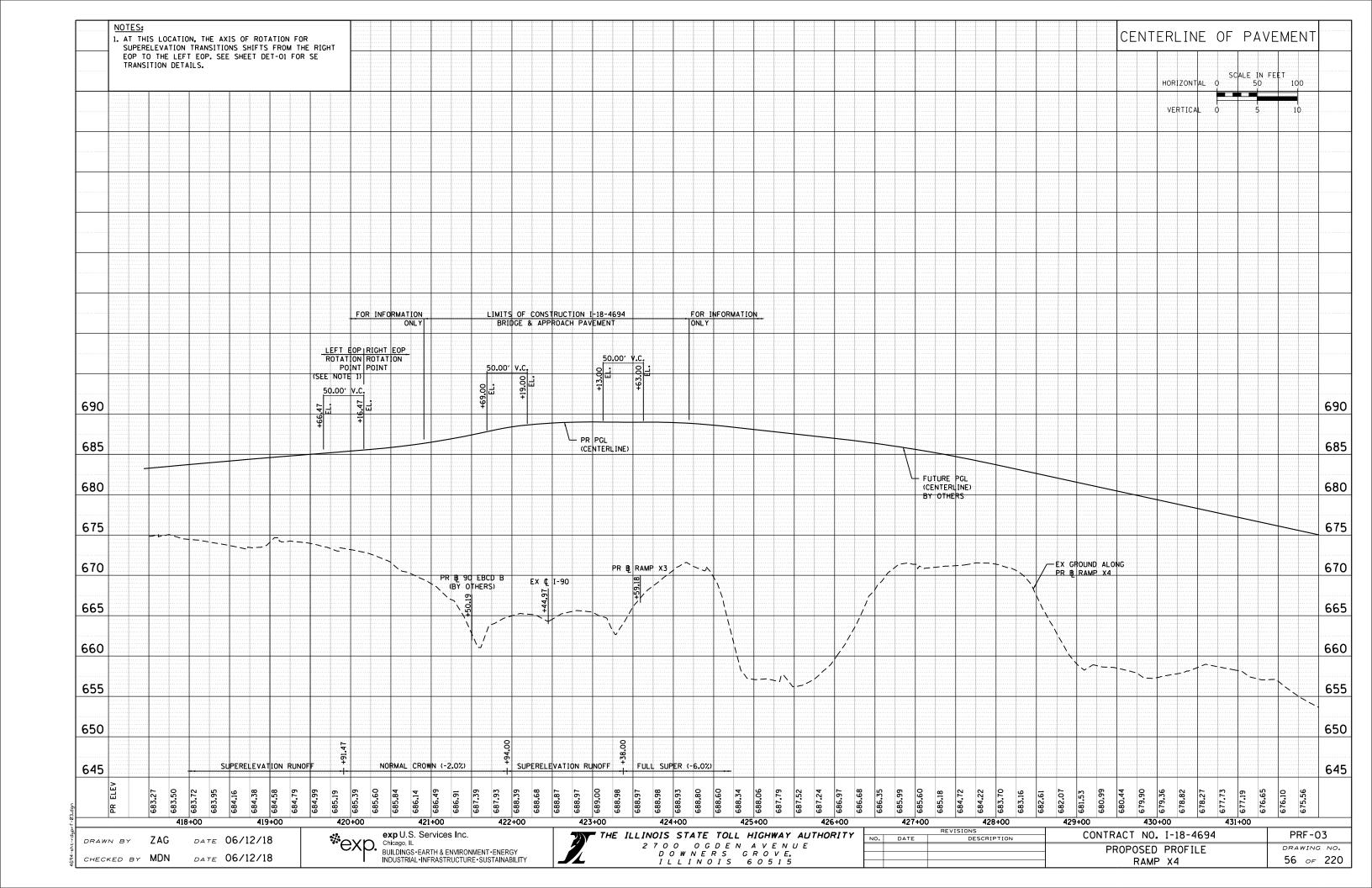
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INDUSTRIAL-INFRASTRUCTURE-SUSTAINABILITY

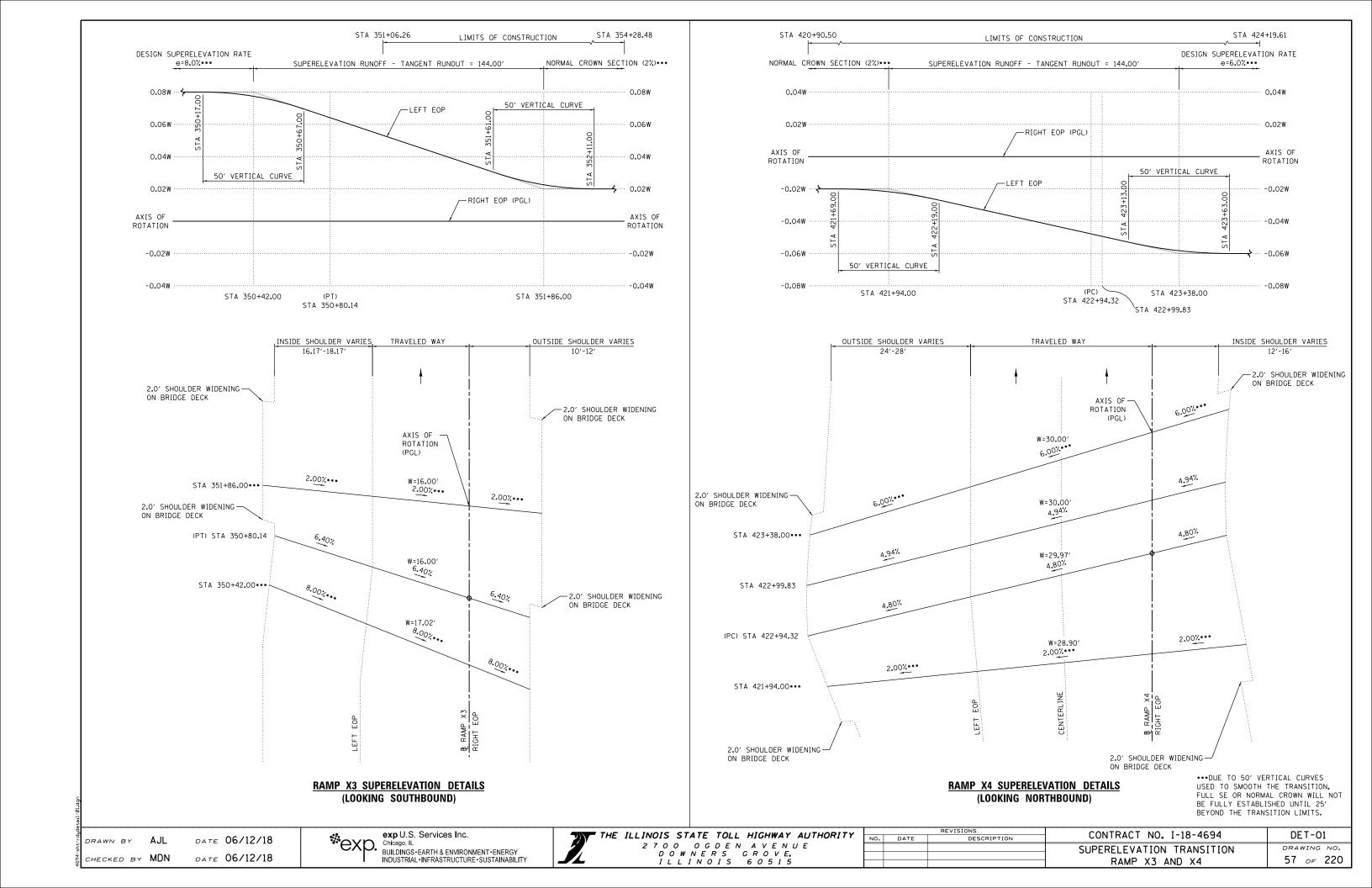


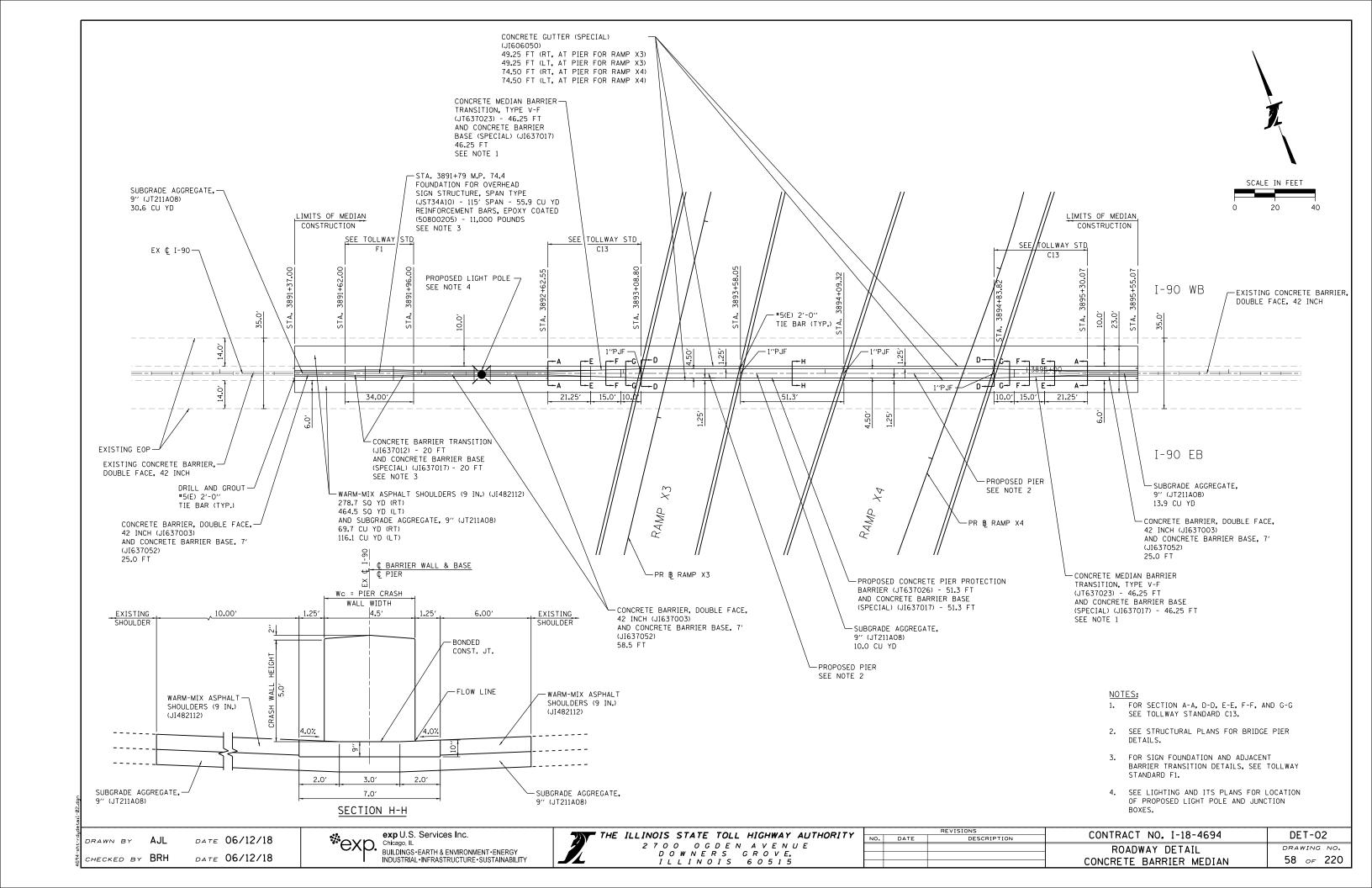
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NO.	DATE	DESCRIPTION	CUNTRACT NO. 1-10-4634	KDW-04
			ROADWAY PLAN	DRAWING NO.
			DES PLAINES OASIS - WESTBOUND	53 of 220
			DES PLAINES DASIS - WESTBOUND	33 OF 220

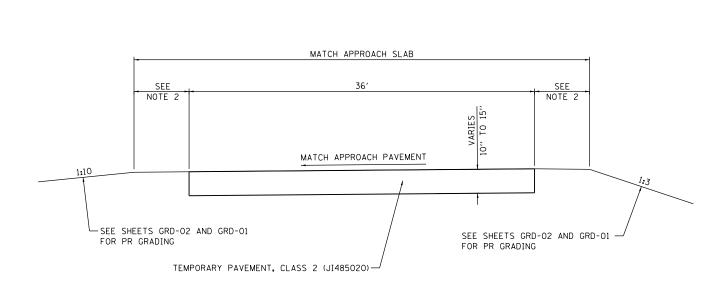




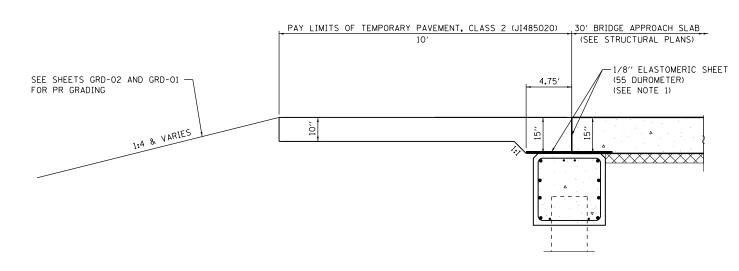






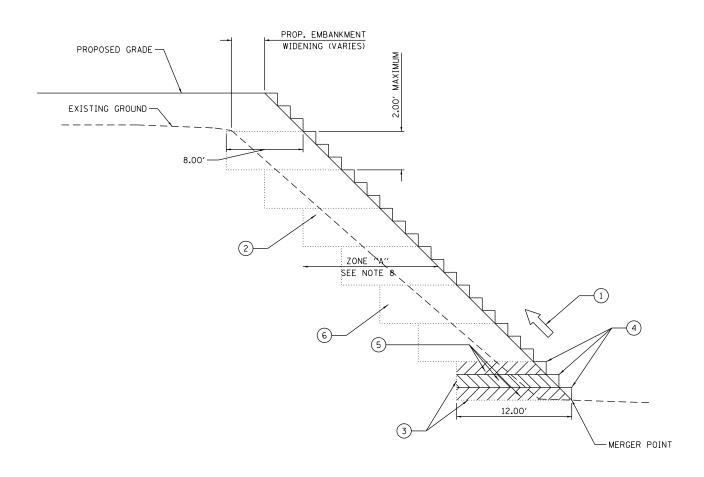


### TYPICAL SECTION **TEMPORARY PAVEMENT**



# **ELEVATION TEMPORARY PAVEMENT**

- BOND BREAKER IS INCLUDED IN THE COST OF TEMPORARY PAVEMENT,
- 2. OFFSET TEMPORARY PAVEMENT TO BE CENTERED BETWEEN EDGES OF



# TYPICAL BENCHING DETAIL FOR EMBANKMENT

#### NOTES:

- CONSTRUCT SUCCEEDING BENCH CUTS AND EMBANKMENT PLACEMENT AND COMPACTION FROM BOTTOM TO TOP IN STAIRSTEP FASHION.
- EXISTING FORESLOPE PREPARED IN ACCORDANCE WITH ARTICLE 205.03 OF THE STANDARD SPECIFICATIONS.
- (3) BENCH CUT EXISTING SLOPE TYPICAL FOR EACH STEP.
- 4 TRIM TO FINAL SLOPE.
- EQUAL 8-INCH LIFTS OF EMBANKMENT COMPACTED IN ACCORDANCE WITH ARTICLE 205.06 OF THE STANDARD SPECIFICATIONS.
- EXCAVATION OF BENCH CUTS WITHIN EXISTING EMBANKMENT WILL BE PAID FOR AT THE CONTRACT UNIT PRICE PER CUBIC YARD FOR "EARTH EXCAVATION". THIS PRICE WILL INCLUDE ALL LABOR AND MATERIAL, NO ADDITIONAL COMPENSATION WILL BE ALLOWED.
- SLOPES SHALL BE BENCHED ACCORDING TO THIS DETAIL WHEN THE SLOPE IS STEEPER THAN 1:3 AND THE HEIGHT IS GREATER THAN 5'.
- EMBANKMENT FOR FORESLOPES INSIDE THE 1:2 INFLUENCE AREA SHALL BE COMPACTED TO 95% OF STANDARD LABORATORY DENSITY.

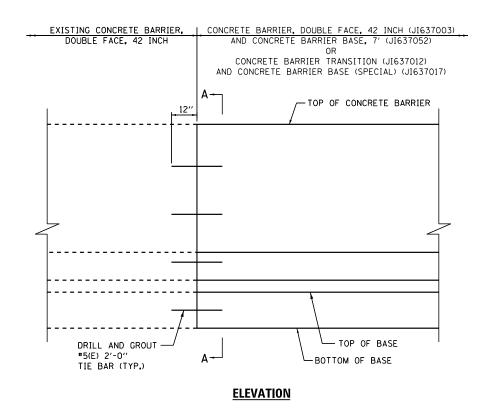
APPROACH SLAB.

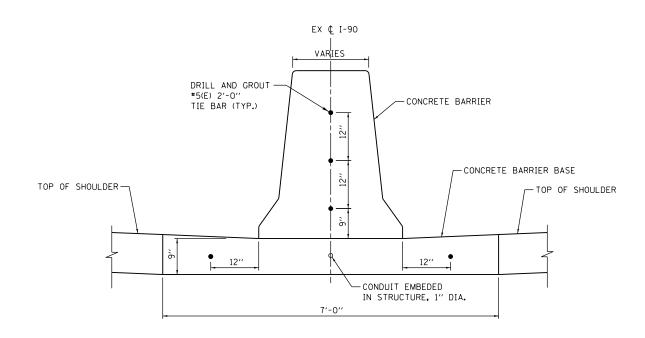
DRAWN BY AJL DATE 06/12/18 DATE 06/12/18 CHECKED BY BRH

\*\*exp. Chicago, IL BUILDINGS

exp U.S. Services Inc. BUILDINGS EARTH & ENVIRONMENT ENERGY INDUSTRIAL INFRASTRUCTURE SUSTAINABILITY THE ILLINOIS STATE TOLL HIGHWAY AUTHORITY 2700 OGDEN AVENUE DOWNERS GROVE, ILLINOIS 60515

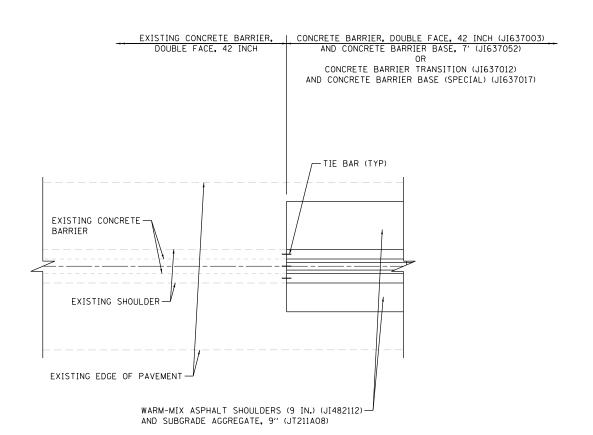
		REVISIONS	CONTRACT NO 1 19 4CO4	DET-03
NO.	DATE	DESCRIPTION	CONTRACT NO. I-18-4694	DE 1-03
			ROADWAY DETAILS	DRAWING NO.
				59 <i>o</i> 220
			TEMPORARY PAVEMENT AND BENCHING	59 of 220





# SECTION A-A

(SEE TOLLWAY STANDARDS FOR ADDITIONAL INFORMATION)



## **PLAN VIEW**

THE ILLINOIS STATE TOLL HIGHWAY AUTHORITY CONTRACT NO. I-18-4694 DET-04 NO. DATE DESCRIPTION 2700 OGDEN AVENUE DOWNERS GROVE, ILLINOIS 60515 DRAWING NO. ROADWAY DETAIL 60 of 220 CONCRETE BARRIER TRANSITION DETAIL

DRAWN BY JP DATE 06/12/18 CHECKED BY BRH DATE 06/12/18

exp U.S. Services Inc. exp. Chicago, IL BUILDINGS BUILDINGS • EARTH & ENVIRONMENT • ENERGY INDUSTRIAL • INFRASTRUCTURE • SUSTAINABILITY

		UTILITY INFORMAT	ION					Ş	STATUS	OF UTILI	TIES						WORK OR	RDER/PE	RMITS				
					CROSSROAD OR PARALLE UTILITY CONFLICT	CON SHO	STING ILITY FLICT WN ON .ANS	EXIST UTILI CONFL SHOWN CROS SECTIO	TY ICT ON SS	TEMPORAR' STAGE CONSTRUC		COMMENTS	TOLLWAY UTILITY CONFLICT NUMBER	TOLLWAY UTILITY WORK ORDER NUMBER	ORDER FOR UTILITY WORK AND PLANS APPROVED AND AVAILABLE	NO RELOCATIO - WATCH AND PROTECT	NRELOCATION FOR	N BY UT This C	ILITY COMPANY	RELOCATION BY CONTRACTOR UNDER THIS CONTRACT	UTILITY SERVICE AGREEMENT PROCESSED	LAGREEMENT	SPECIAL REQUIREMENTS OR COMMENTS
	UTILITY COMPANY	UTILITY COMPANY ADDRESS	UTILITY COMPANY CONTACT PERSON	CONTACT PERSON TELEPHONE NUMBER	CROSSROAD (C), PARALLEL (F	YES/ NO	SHEET NO.	YES/ S NO	HEET ,	YES/NO SH	HEET	PROPOSED UTILITY RELOCATIONS ARE SHOWN ON PLANS INCLUDED AS PART OF THE UTILITY WORK ORDER AND SHALL BE MADE AVAILABLE TO THE CONTRACTOR. ONLY EXISTING UTILITY FACILITIES ARE SHOWN ON THE CONTRACT DRAWINGS.			YES/NO	YES/NO	YES/ EST NO RELC	IMATED ART OF OCATION	ESTIMATED COMPLETION OF RELOCATION	YES/NO	YES/NO	YES/NO	
ABOVE GROUND																							
ELECTRICAL	COMED	1910 SOUTH BRIGGS STREET JOLIET, IL 60433	JOSE MALAGON	815-724-5065	Р	YES	UTL-03	NO	N/A	NO N	I/A	AERIAL ELECTRIC SERVICE LINES (12Kv) AND 2 POLES	1			NO	NO I	N/A	N/A	YES		YES	SEE NOTE 1
BELOW GROUND-DRY  ELECTRICAL	COMED	1910 SOUTH BRIGGS STREET JOLIET, IL 60433	JOSE MALAGON	815-724-5065	С	YES	UTL-03, UTL-04	NO	N/A	NO N	I/A	ACTIVE AND ABANDONED UNDERGROUND ELECTRIC SERVICE LINES (12kV)	2			NO	NO I	N/A	N/A	YES		YES	SEE NOTE 1
	ISTHA (MANAGED BY ILLINOIS	TBD	TBD	TBD	С	YES	UTL-04	NO	N/A	NO N	N	TOLLWAY ITS FIBER OPTIC AND ELECTRIC: 144 FIBERS IN CONDUIT; 2-1/C 350 MCM AND 1-1/C NO. 250MCM CABLES IN CONDUIT; 1-1/4" DIA. CONDUIT	9			YES	NO I	N/A	N/A	NO			
FIBER OPTIC	TOLLWAY'S FIBEF MANAGEMENT COMPANY)	₹55			С	YES	UTL-02, UTL-03	NO	N/A	NO N	I/A	TOLLWAY ITS FIBER OPTIC CABLE, 36 FIBERS IN CONDUIT, WITHIN I-90 MEDIAN	3			NO	NO I	N/A	N/A	YES			SEE NOTE 2
	COMCAST	688 INDUSTRIAL DRIVE ELMHURST, IL 60126 ATTN: BOB SHULTER (REGIONAL ROW MANAGER	THOMAS MUNAR )	224-229-5851	С	YES	UTL-04	NO	N/A	NO N	I/A	UNDERGROUND SERVICE POSSIBLE ALREADY ABANDONED OR REMOVED WITHIN BUILDING REMOVAL AREA	10			NO	NO I	N/A	N/A	YES			SEE NOTE 1
TELEPHONE	AT&T DISTRIBUTION (LEGACY)	LEGAL MANDATE GROUP 1000 COMMERCE DRIVE OAK BROOK, IL 60523	ALEX BRYANT	630-272-9010	С	YES	UTL-03, UTL-04	NO	N/A	NO N	I/A	ACTIVE AND ABANDONED UNDERGROUND TELEPHONE LINE SERVICES	4			NO	NO 1	N/A	N/A	YES		YES	SEE NOTE 1
BELOW GROUND-WET																							
GAS	NICOR	1844 FERRY ROAD NAPERVILLE, IL 60563	BRUCE KOPPANG	708-243-5136	C&P	YES	UTL-03, UTL-04	NO	N/A	NO N	I/A	ACTIVE AND ABANDONED UNDERGROUND GAS LINE SERVICES	5			NO	NO I	N/A	N/A	YES		YES	SEE NOTE 1
OIL	UNKNOWN	TBD	TBD	TBD	C&P	YES	UTL-03, UTL-04	NO	N/A	NO N	I/A	2" & 8" ABANDONED PETROLEUM PIPELINE SEE PERMIT NO. 59-29	6			NO	NO I	N/A	N/A	YES			SEE NOTE 1
SANITARY SEWER	CITY OF DES PLAINES	1420 MINER STREET DES PLAINES, IL 60016	JON DUDDLES	847-391-6127	C&P	YES	UTL-03, UTL-04	NO	N/A	NO N	I/A	ACTIVE AND ABANDONED 8" DIA. C-900 & PVC SDR-26 AND CLAY SANITARY SEWER SERVICES	7			NO	NO I	N/A	N/A	YES			SEE NOTE 1
WATER MAIN	NSMJAWA	900 WELLINGTON AVENUE ELK GROVE VILLAGE, IL 60007		847-981-4083 R 773-686-0077	Р	YES	UTL-03	NO	N/A	NO N	I/A	90" UNDGERGOUND TRANSMISSION MAIN	11			YES	NO I	N/A	N/A	NO			
WATER MAIN	CITY OF DES PLAINES	1420 MINER STREET DES PLAINES, IL 60016	JON DUDDLES	847-391-6127	C&P	YES	UTL-03, UTL-04	NO	N/A	NO N	1/A [	10" AND 12" DIA. DUCTILE IRON WATER MAIN SERVICES AND 1-1/2" DIA. ACTIVE AND ABANDONED WATER MAIN SERVICES.	8			NO	NO I	N/A	N/A	YES			SEE NOTE 1

# NOTES:

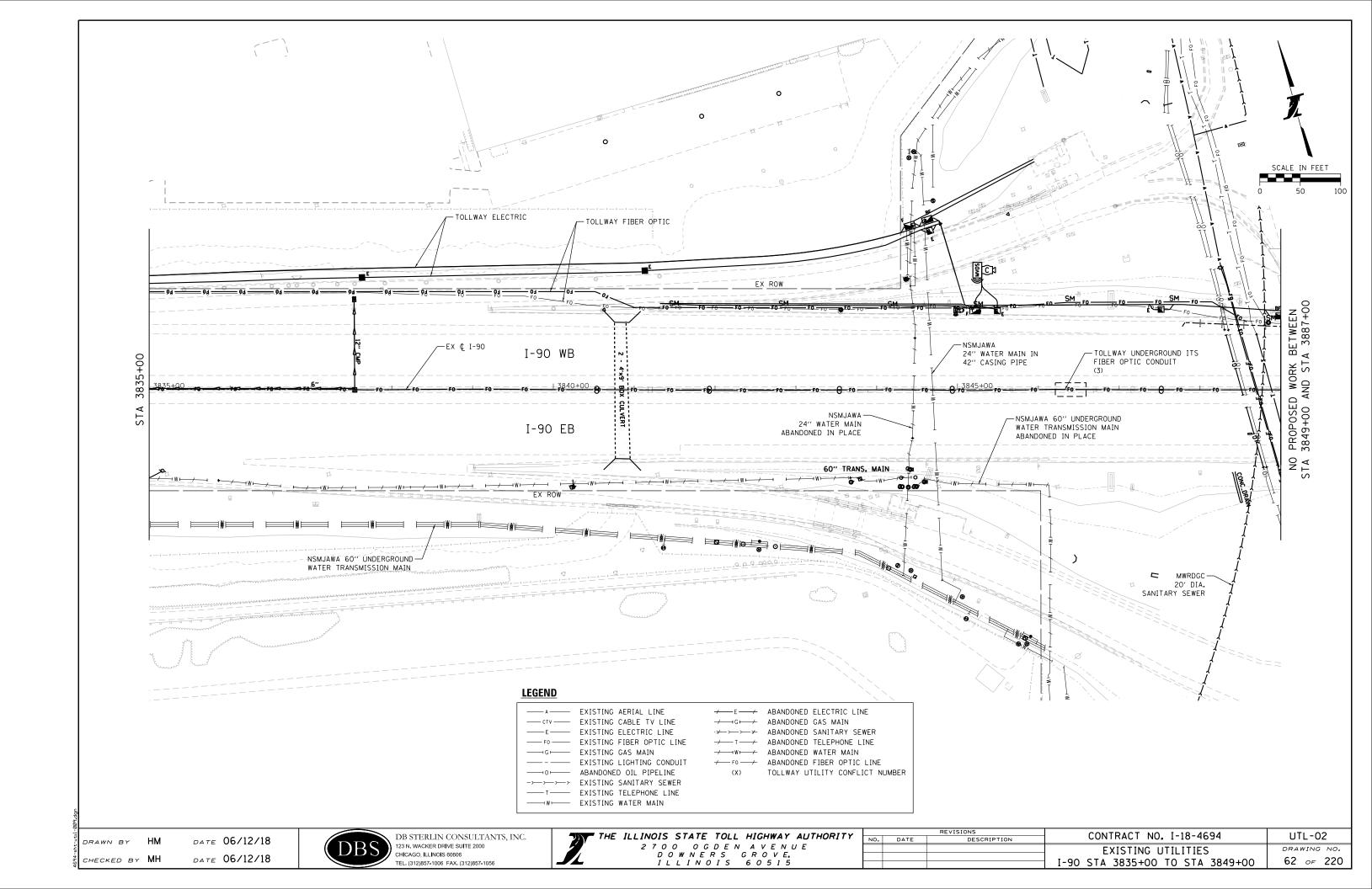
- 1. REMOVAL OF EXISTING SERVICE LINES SERVING THE BUILDING AND ABANDONED UTILITIES IS PART OF BUILDING DEMOLITION. FOR REMOVAL LIMITS, SEE SHEETS REM-04 THROUGH REM-06.
- 2. FOR RELOCATION OF TOLLWAY FIBER OPTIC LINE, SEE ITS PLANS.

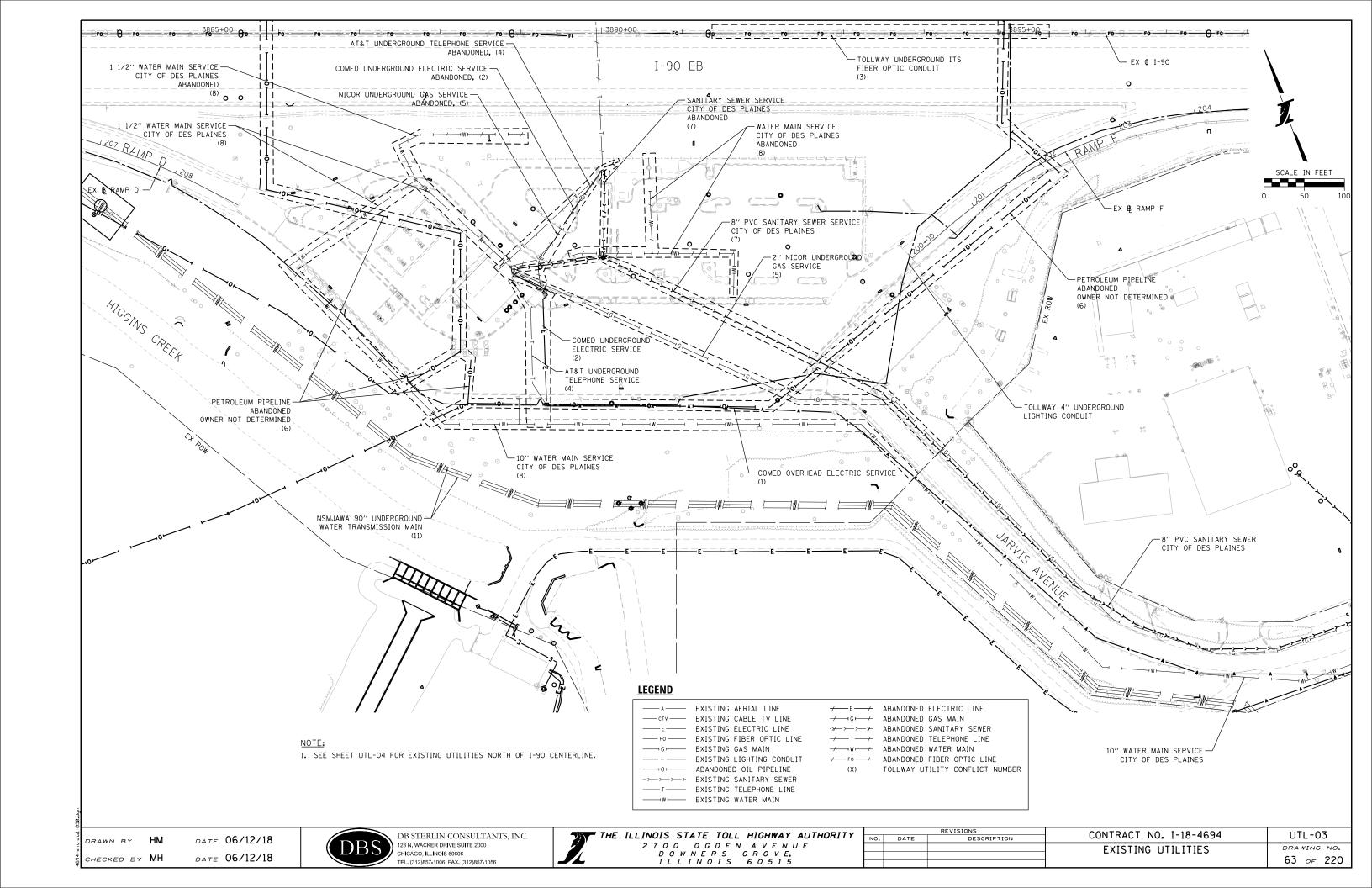
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CHECKED BY MH DATE 06/12/18

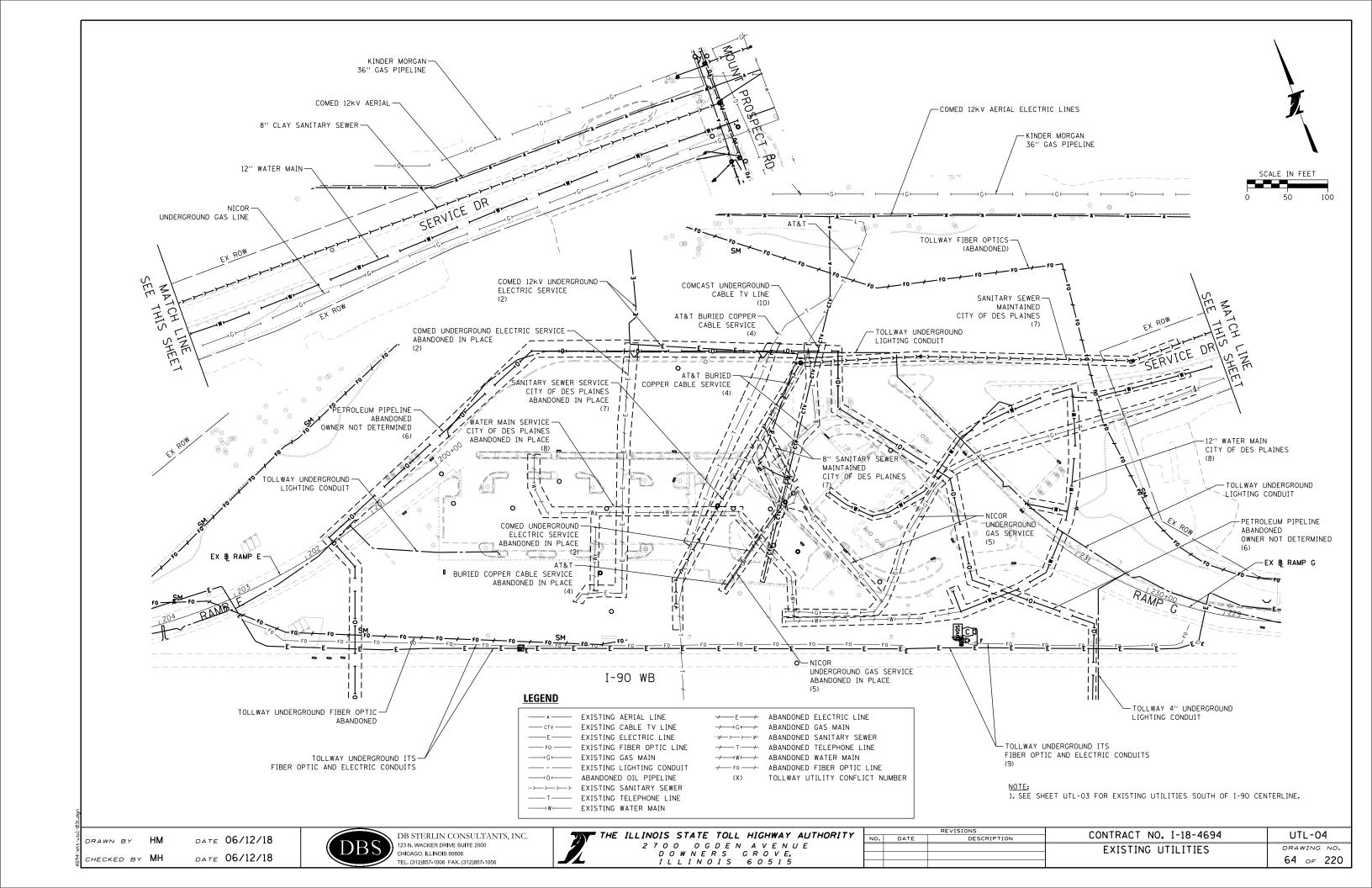


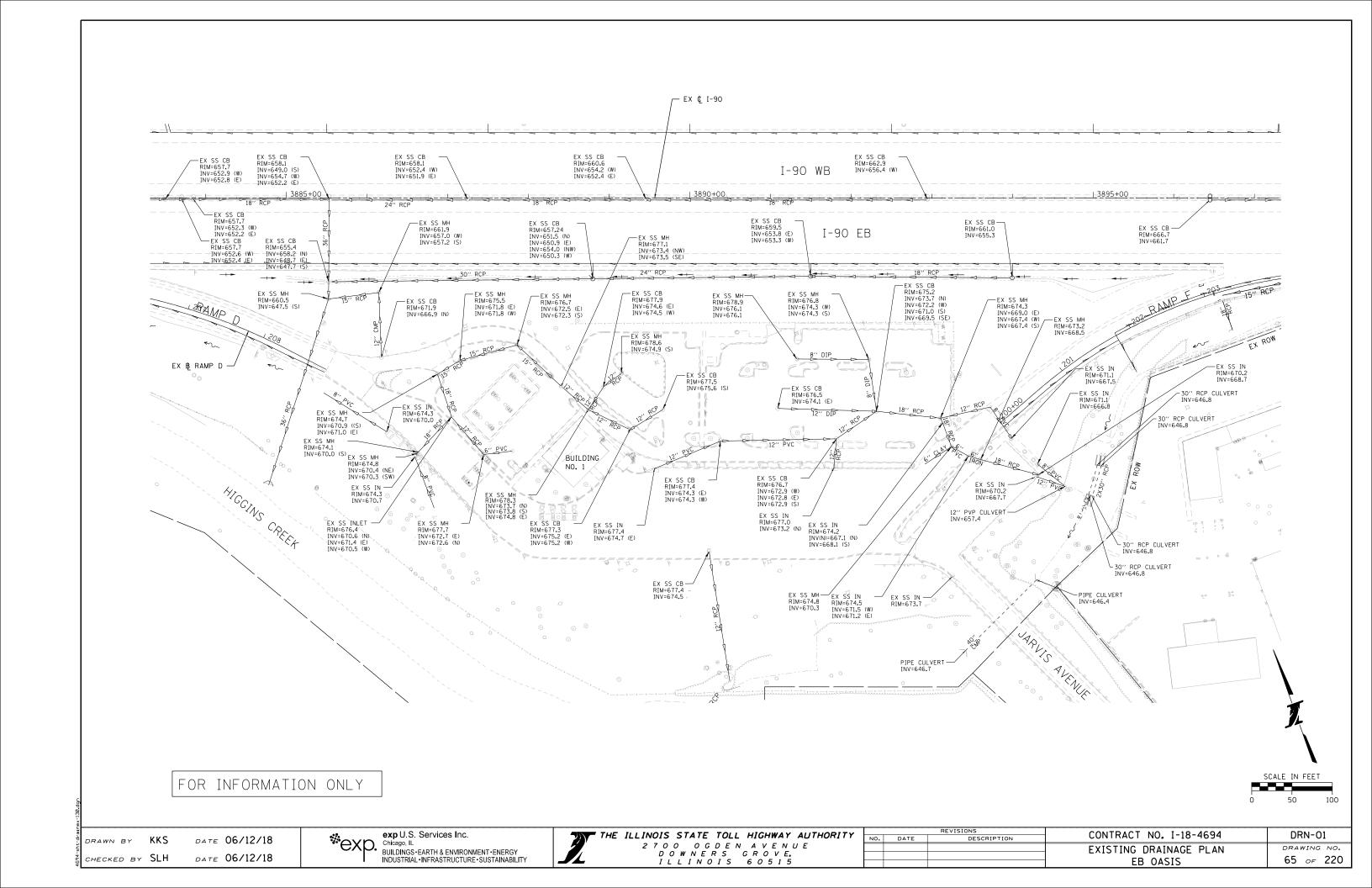
THE	ILLINOIS STATE TOLL HIGHWAY AUTHORITY
<b>9</b>	ILLINOIS STATE TOLL HIGHWAY AUTHORITY 2700 OGDEN AVENUE DOWNERS GROVE, ILLINOIS 60515
	DOWNERS GROVE, ILLINOIS 60515

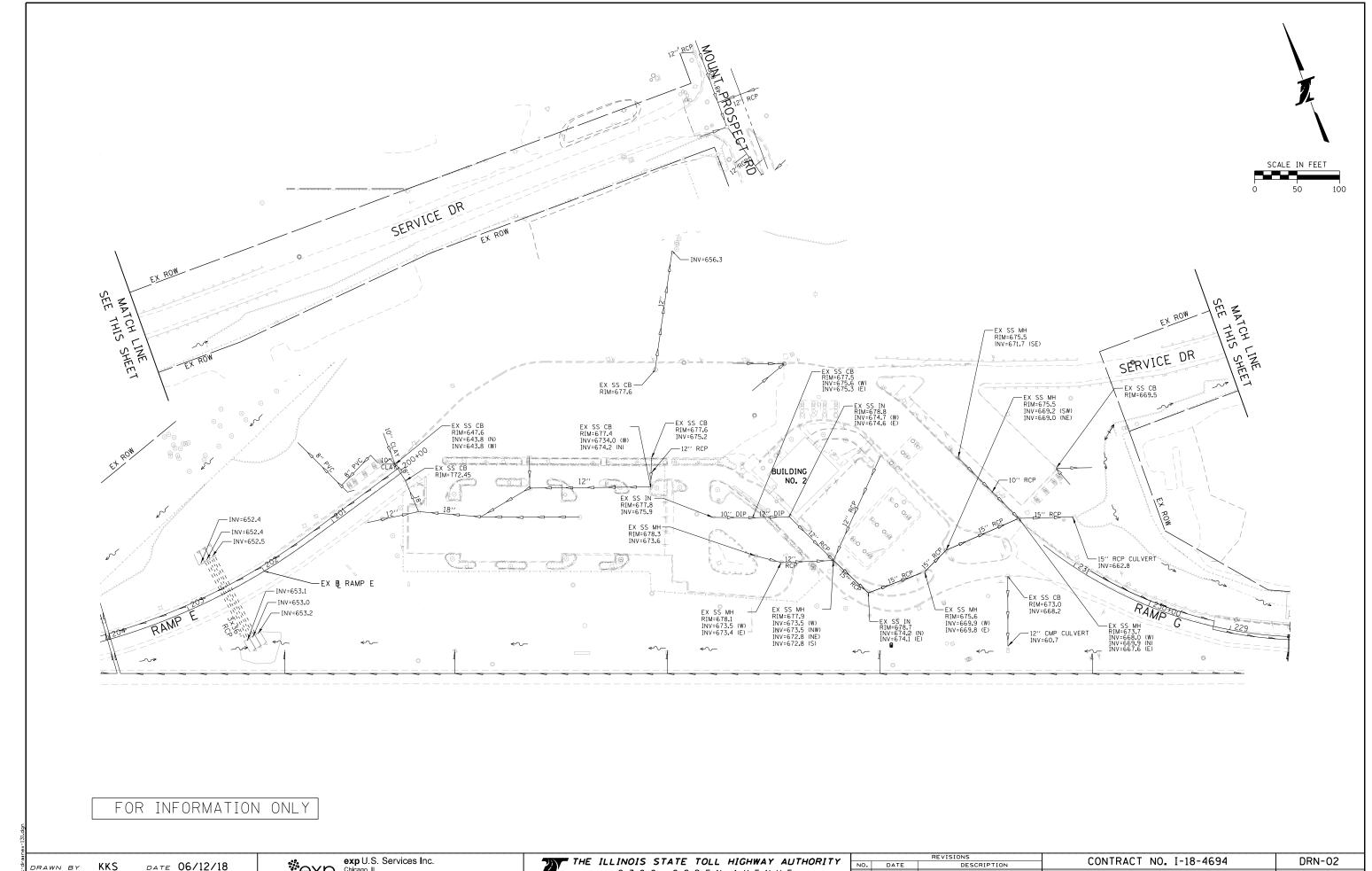
		REVISIONS	CONTRACT NO. I-18-4694	UTL-01
NO.	DATE	DESCRIPTION	CUNTRACT NO. 1-10-4634	016-01
			UTILITY MATRIX	DRAWING NO.
			OTIETT WATER	61 of 220











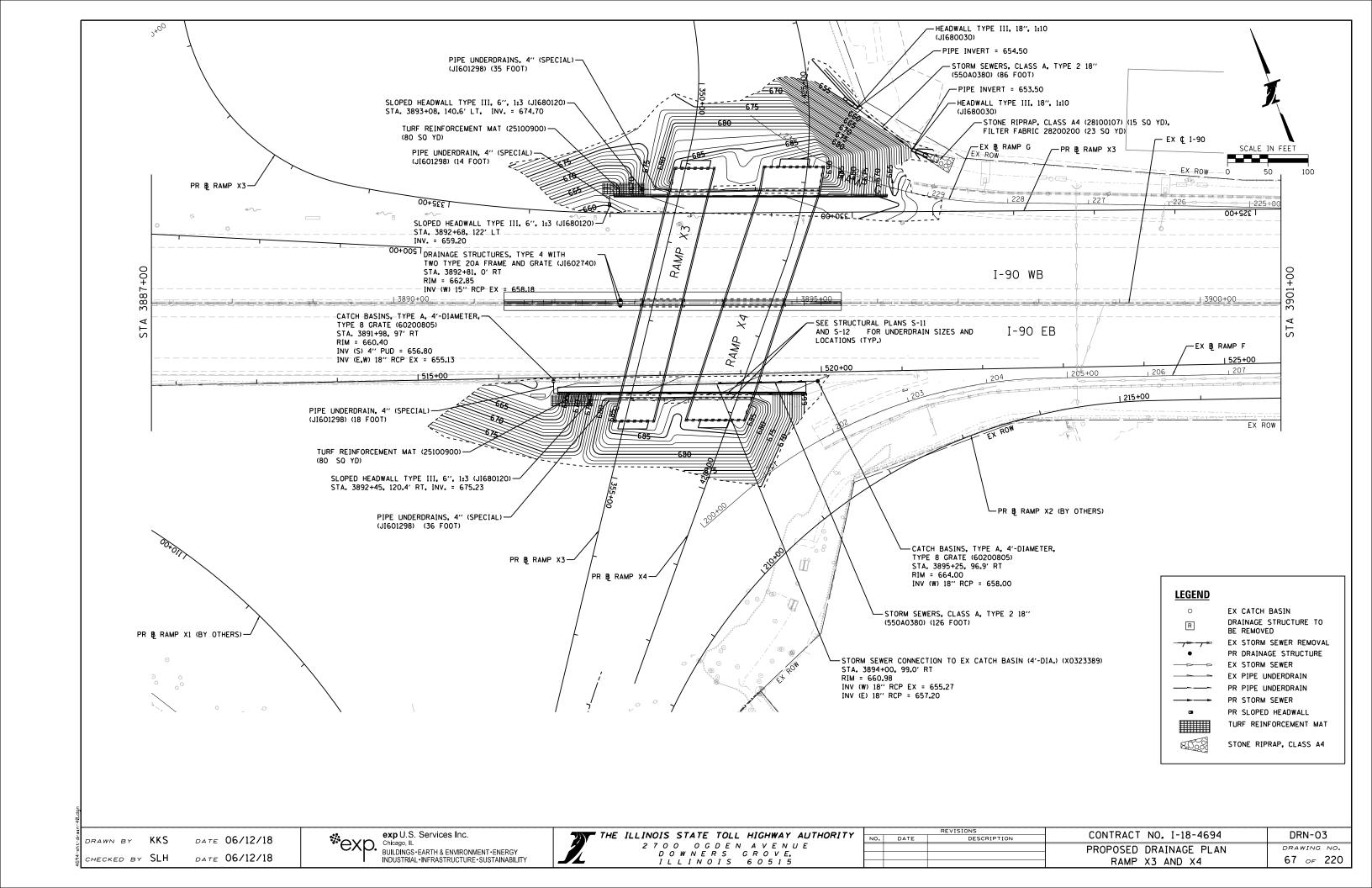
CHECKED BY SLH

DATE 06/12/18 DATE 06/12/18

exp U.S. Services Inc.
Chicago, IL
BUILDINGS EARTH & ENVIRONMENT ENERGY
INDUSTRIAL INFRASTRUCTURE SUSTAINABILITY

THE ILLINOIS STATE TOLL HIGHWAY AUTHORITY
2700 OGDEN AVENUE
DOWNERS GROVE,
ILLINOIS 60515

EXISTING DRAINAGE PLAN WB OASIS DRAWING NO. 66 of 220



#### SIGNING LEGEND

## GUIDE TO SEQUENCE NUMBERING CODE

MS - MISCELLANEOUS

### ITEMS TO BE SALVAGED - SEE S.P. 114

SIGN SHEETS REMOVED STRUCTURAL STEEL BREAKAWAY POSTS

EB - 90 - TR - 101 (P) PROPOSED SIGN, GROUND MOUNTED EXISTING SIGN, GROUND MOUNTED DIRECTION OF TRAFFIC/RAMP ACTION EB - EASTBOUND (R) - REMOVE EXISTING OVERHEAD SIGN STRUCTURE, SPAN TYPE WB - WESTBOUND (P) - PROPOSED (FIO) - FOR INFORMATION ONLY ROADWAY OR INTERCHANGE SIGN PANEL NUMBER EXISTING SIGN PANEL AND POST TO BE REMOVED/RELOCATED 90 - IL 90 (JANE ADDAMS MEMORIAL EXPRESSWAY) XXX - SIGN PANELS MOUNTING TYPE TR - TRUSS PROPOSED SIGN PANEL WP - WOOD POST TS - TELESCOPING STEEL → PROPOSED SIGN, LIGHT-POLE MOUNTED BS - BREAKAWAY STEEL LP - LIGHT POLE

DRAWN BY DK DATE 06/12/18

CHECKED BY DDH DATE 06/12/18





				SIGN	REMOVAL SCHE	DULE						
					EXI	ISTING SIGN S	SIZE	73700100	73700200	72400320	72400330	X0327009
LOCATION	EX. STATION	SIGN PANEL NO.	SIGN DESCRIPTION	EXISTING MOUNTING	HEIGHT	WIDTH	AREA	REMOVE GROUND MOUNTED SIGN SUPPORT	REMOVE CONCRETE FOUNDATION - GROUND MOUNT	REMOVE SIGN PANEL - TYPE 2	REMOVE SIGN PANEL - TYPE 3	REMOVE SIGN (SPECIAL)
					FOOT	FOOT	SQ FT	EACH	EACH	SQ FT	SO FT	EACH
EB 90	3691+00	EB-90-TR-001(R)	DES PLAINES OASIS	TRUSS	10	15	150				150	
EB 90	3794+88	EB-90-TR-002(R)	DES PLAINES OASIS	TRUSS	10	15	150				150	
EB 90	3846+00	EB-90-MS-003(R)	MOBIL ELECTRIC GAS SIGN	MISCELLANEOUS								1
EB 90	3853+00	EB-90-BS-004(R)	DES PLAINES OASIS	BREAKAWAY STEEL	12	18	216	2	2		216	
EB 90	3877+40	EB-90-BS-005(R)	DES PLAINES OASIS	BREAKAWAY STEEL	11	20	220	2	2		220	
EB 90	3880+60	EB-90-LP-006(R)	EXIT 20 MPH	LIGHT POLE	5	4	20			20		
EB 90	3882+85	EB-90-WP-007(R)	OASIS	WOOD POST	5	6	30				30	
EB 90	3896+50	EB-90-WP-008(R)	MERGE	WOOD POST	4	4	16			16		
WB 90	3883+20	WB-90-WP-009(R)	MERGE	WOOD POST	4	4	16			16		
WB 90	3899+27	WB-90-WP-012(R)	OASIS	WOOD POST	5	6	30				30	
WB 90	3917+50	WB-90-TR-013(R)	DES PLAINES OASIS	TRUSS	13	18	234				234	
WB 90	3942+00	WB-90-TR-014(R)	DES PLAINES OASIS	TRUSS	13	18	234				234	
WB 90	3975+00	WB-90-TR-015(R)	DES PLAINES OASIS	TRUSS	13	18	234				234	
WB 90	3986+00	WB-90-MS-016(R)	MOBIL ELECTRIC GAS SIGN	MISCELLANEOUS								1
WB 90	3988+00	WB-90-BS-017(R)	DES PLAINES OASIS	BREAKAWAY STEEL	12	18	216	1	1		216	
			<u>'</u>	ı		1	TOTAL	5	5	52	1,714	2

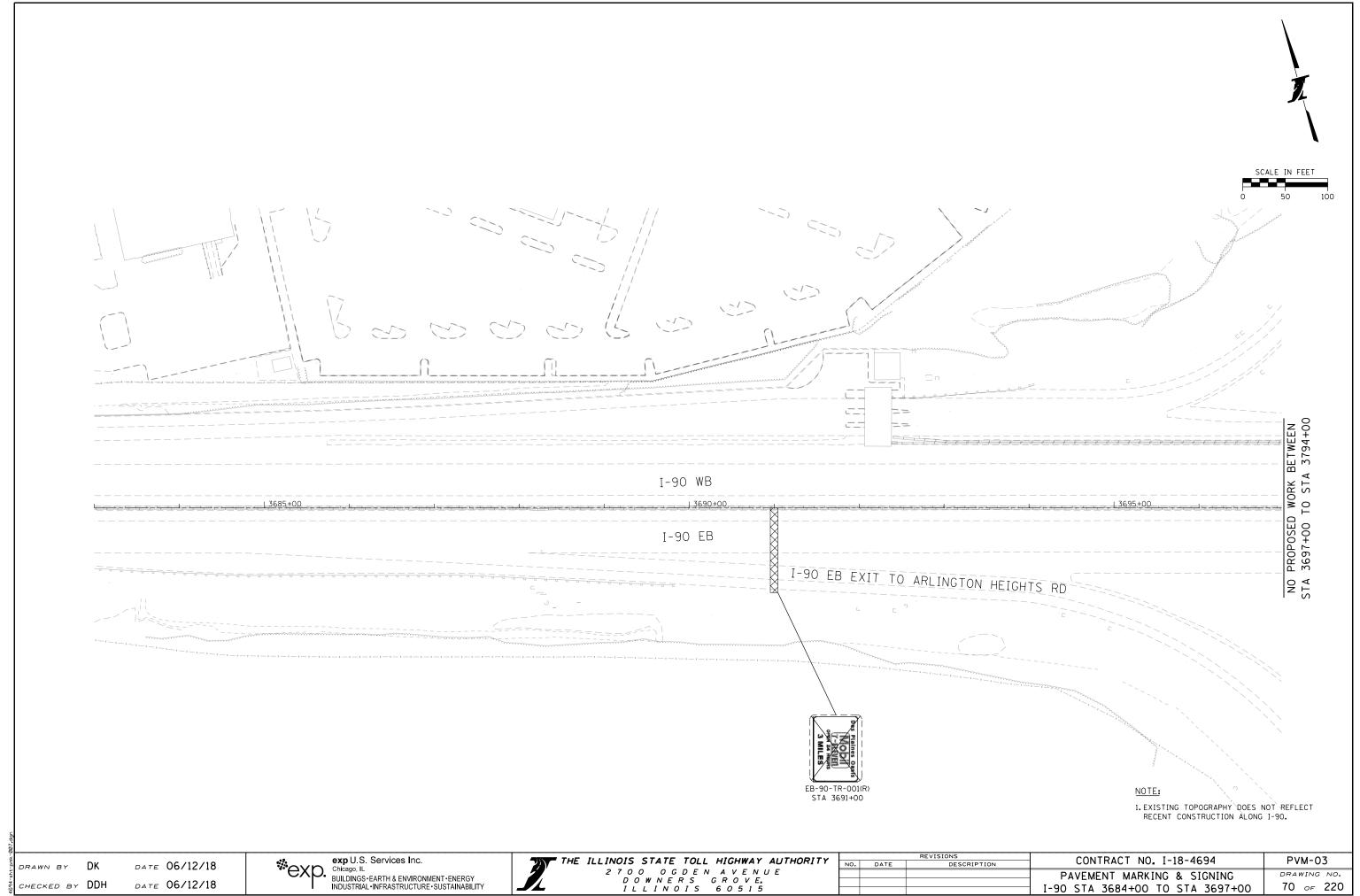
					PROPOSED SIGN SCHEDULE							
LOCATION	EX. STATION		SIGN PANEL NO.	SIGN DESCRIPTION	PROPOSED MOUNTING	MUTCD CODE (TOLLWAY STANDARD E1)	PROPOSED SIGN SIZE		JT720110	JT720120	72800100	
		OFFSET LT/RT					HEIGHT WI	WIDTH	AREA	SIGN INSTALLATION -	SIGN INSTALLATION - TYPE 3	TELESCOPING STEEL SIGN SUPPORT
							FOOT	FOOT	SQ FT	SQ FT	SQ FT	FT
EB 90 RAMP D	207+00	VARIES LT/RT	EB-90-TS-103(P)	OBJECT MARKER	TELESCOPING STEEL	OM4-2	1.5	3(1.5)	2.25	6.75		30
EB 90 RAMP D	201+00	25 FT RT	EB-90-TS-109(P)	RAMP CLOSED AHEAD	TELESCOPING STEEL	(TS-9)	5	4	20	20		36
EB 90 RAMP D	207+00	VARIES LT/RT	EB-90-TS-110(P)	RAMP CLOSED	TELESCOPING STEEL	(TS-6)	2	2(5)	20	20		52
EB 90 JARVIS ROAD	3896+15	704 FT RT	EB-90-TS-111(P)	DEAD END	TELESCOPING STEEL	W14-1	1.5	1.5	2.25	2.25		10
EB 90 JARVIS ROAD	3894+05	532 FT RT	EB-90-MS-112(P)	DO NOT ENTER	FENCE	R-IT1	4	3	12	12		
WB 90	3925+26	94 FT LT	WB-90-LP-105(P)	LANE ENDS	LIGHT POLE	W4-2	4	4	16	16		
WB 90	3929+66	95 FT LT	WB-90-LP-106(P)	RIGHT LANE ENDS	LIGHT POLE	W9-1	4	4	16	16		
WB 90	3942+00	96 FT LT	WB-90-TR-107(P)	RIGHT LANE ENDS 1/2 MILE	TRUSS	-	6.5	11.5	74.75		74.75	
WB 90	3975+00	80 FT LT	WB-90-TR-108(P)	RICHT LANE ENDS 1 MILE	TRUSS	-	7	11.5	80.5		80.5	
WB 90 RAMP G	229+18	VARIES LT/RT	WB-90-TS-105(P)	OBJECT MARKER	TELESCOPING STEEL	OM4-2	1.5	3(1.5)	2.25	6.75		30
WB 90 RAMP G	222+35	13 FT RT	WB-90-LP-111(P)	RAMP CLOSED AHEAD	LIGHT POLE	(TS-9)	5	4	20	20		
WB 90 RAMP G	229+18	VARIES LT/RT	WB-90-TS-112(P)	RAMP CLOSED	TELESCOPING STEEL	(TS-6)	2	2(5)	20	20		52
									TOTAL	140	155	210

DRAWN BY DK DATE 06/12/18
CHECKED BY DDH DATE 06/12/18

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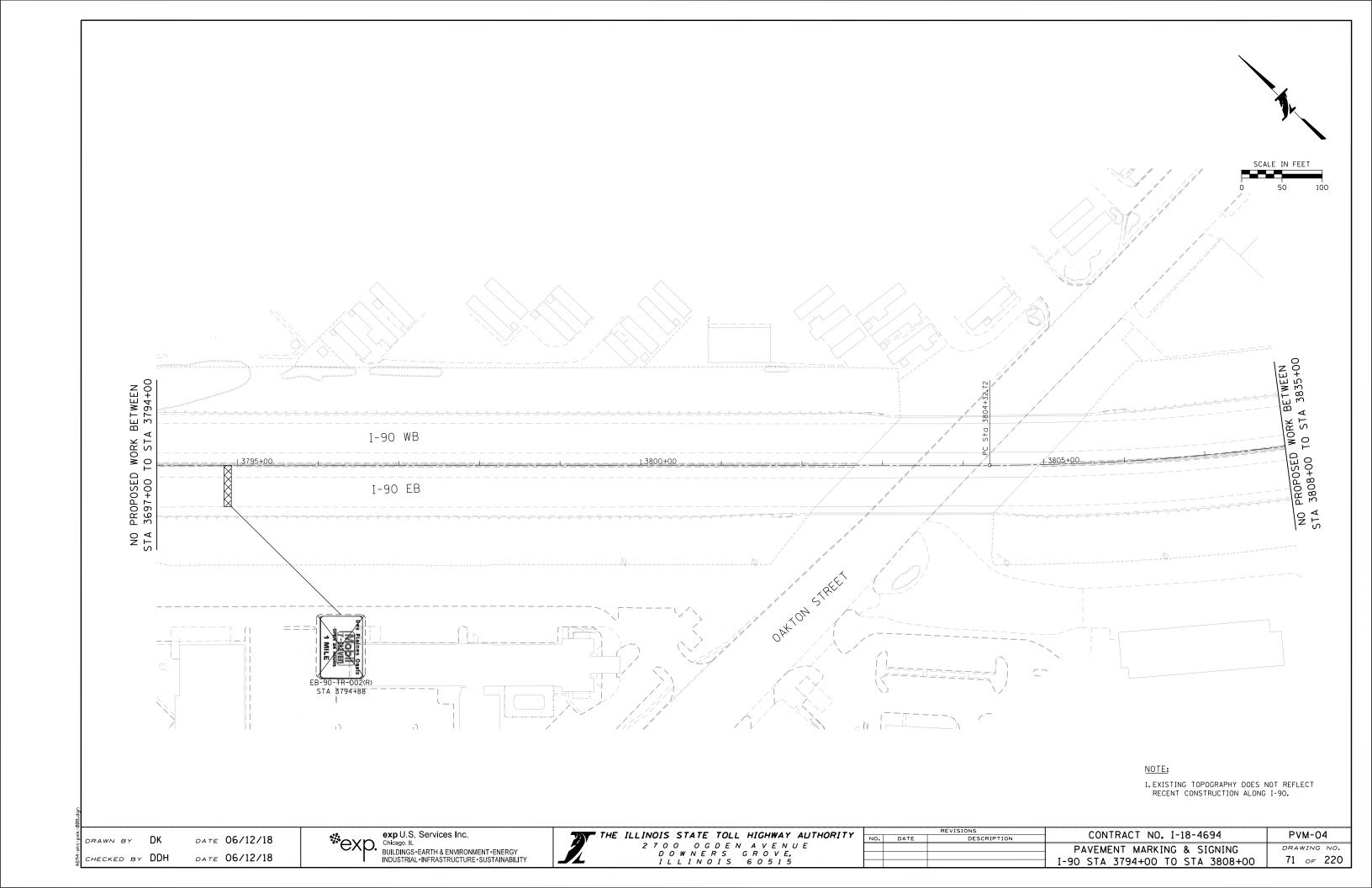
	THE	ILLINOIS STATE TOLL HIGHWAY AUTHORITY 2 7 0 0 0 0 D E N A V E N U E D 0 W N E R S G R O V E, I L L I N O I S 6 0 5 1 5
<b>4</b>		2700 OGDEN AVENUE
		DOWNERS GROVE,
1		ILLINOIS 60515

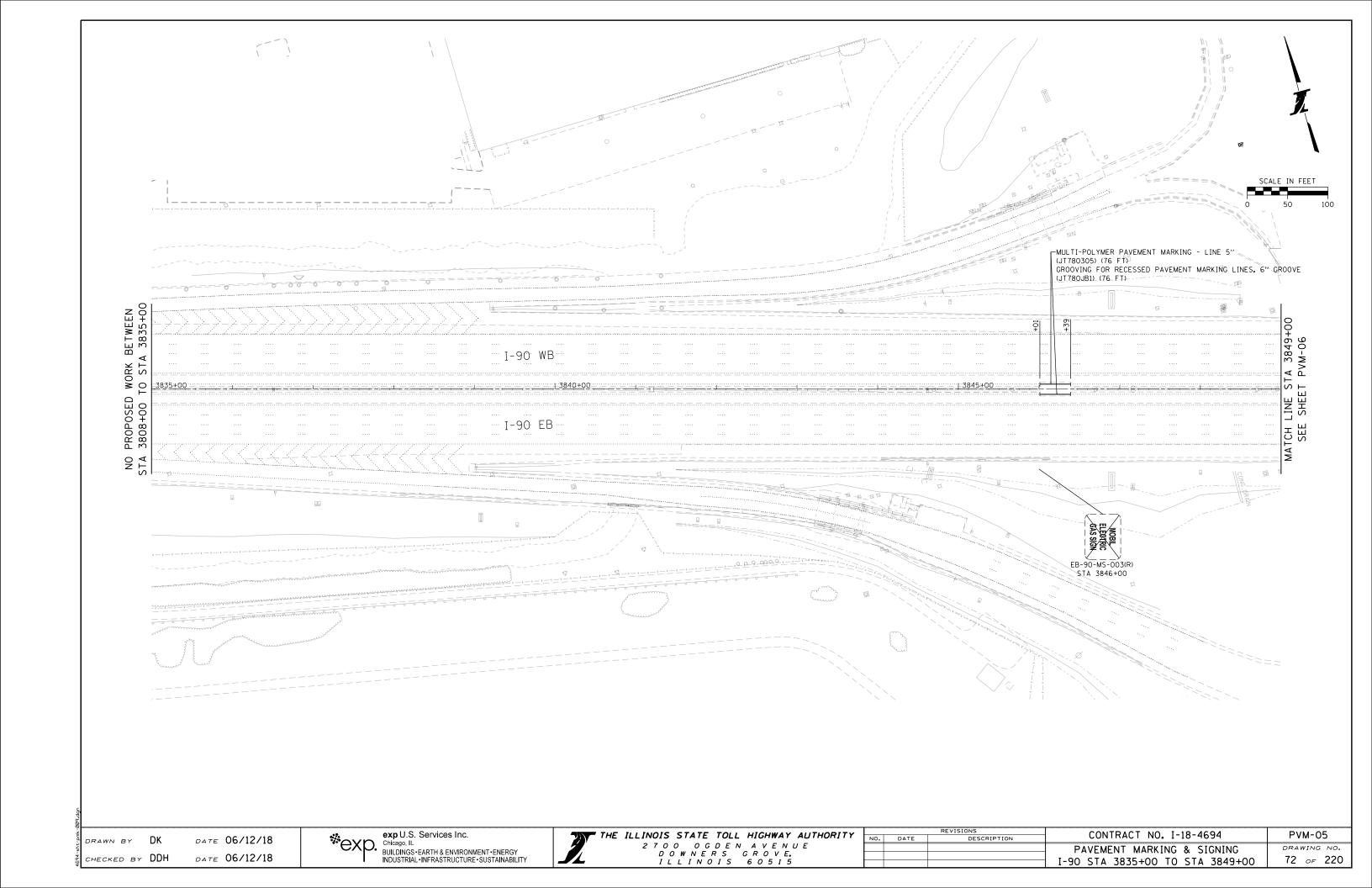
		REVISIONS	CONTRACT NO. I-18-4694	PVM-02		
١٥.	DATE	DESCRIPTION	CONTRACT NO. 1-10-4634	P V M - U Z		
			SIGN REMOVAL AND	DRAWING NO.		
				69 <i>o</i> 220		
			PROPOSED SIGNING SCHEDULES	69 of 220		

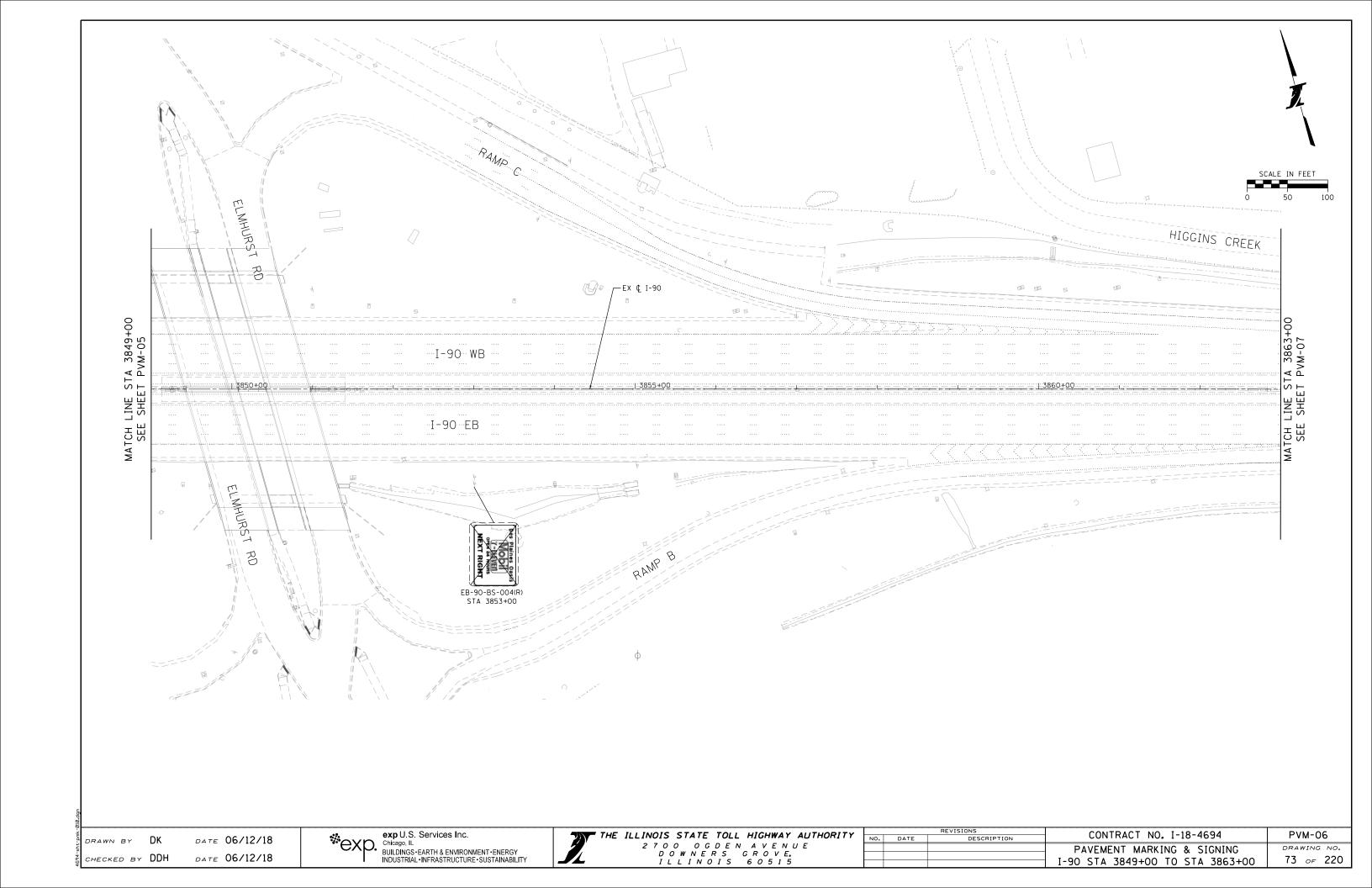


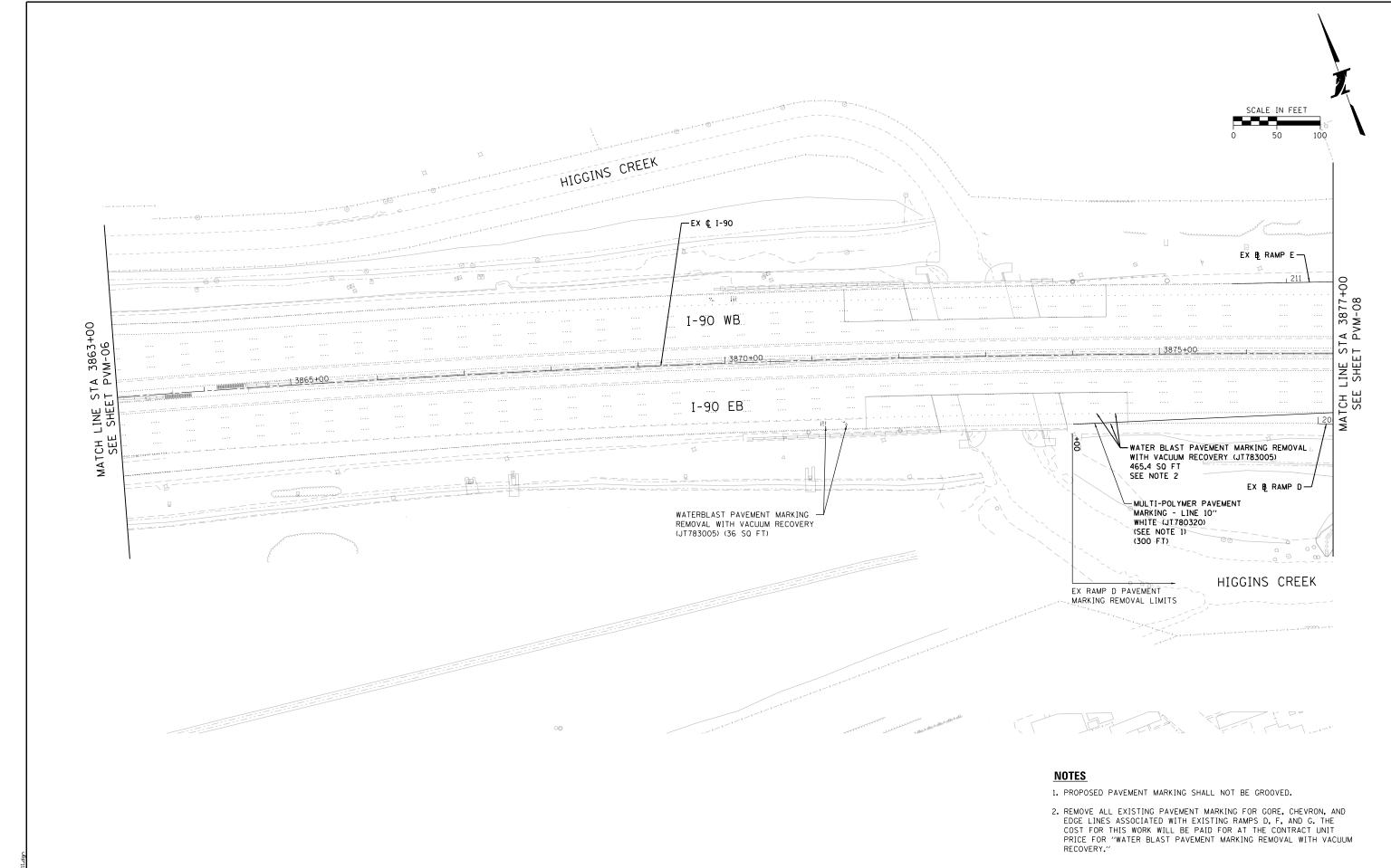
PAVEMENT MARKING & SIGNING I-90 STA 3684+00 TO STA 3697+00

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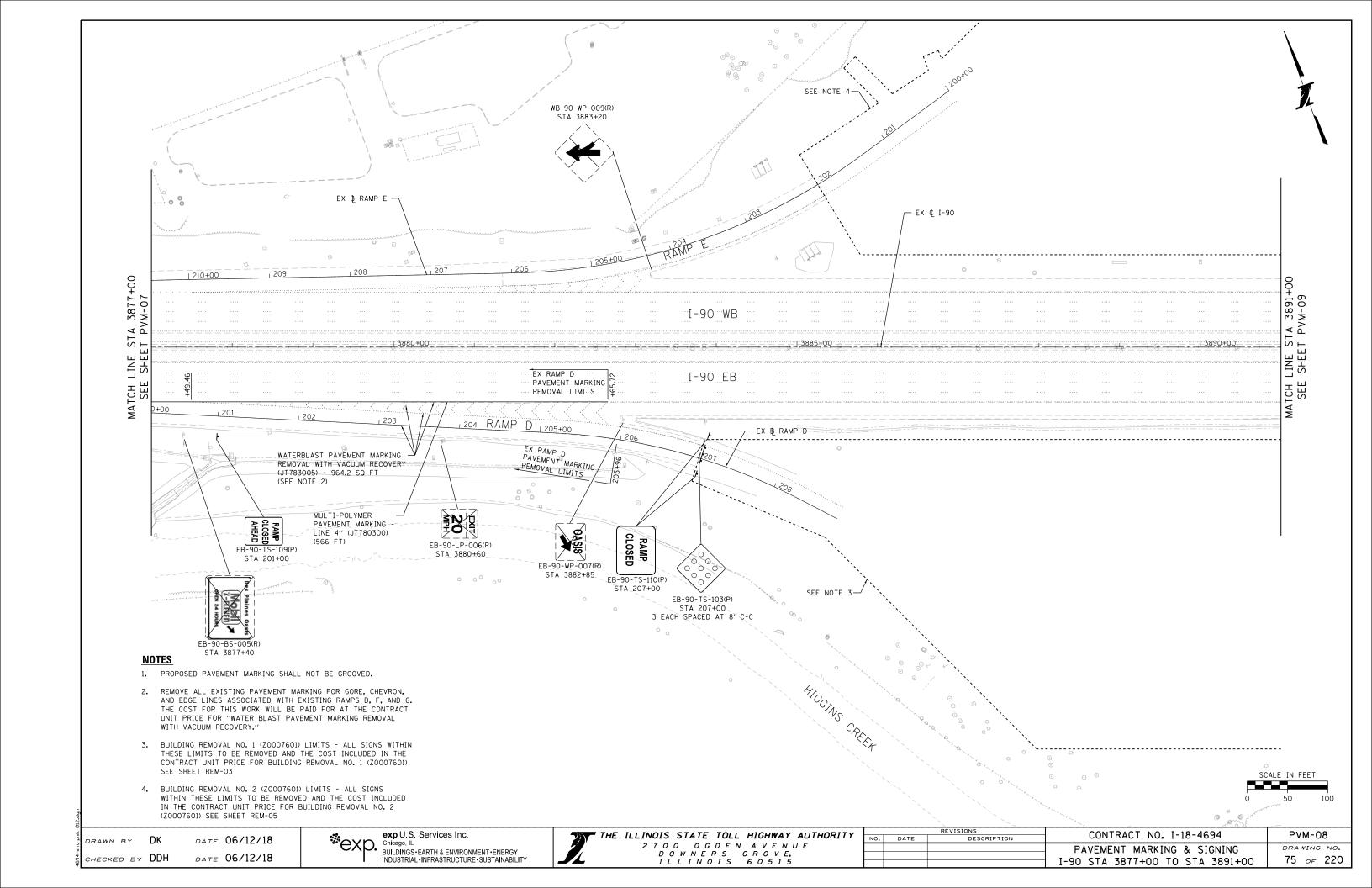


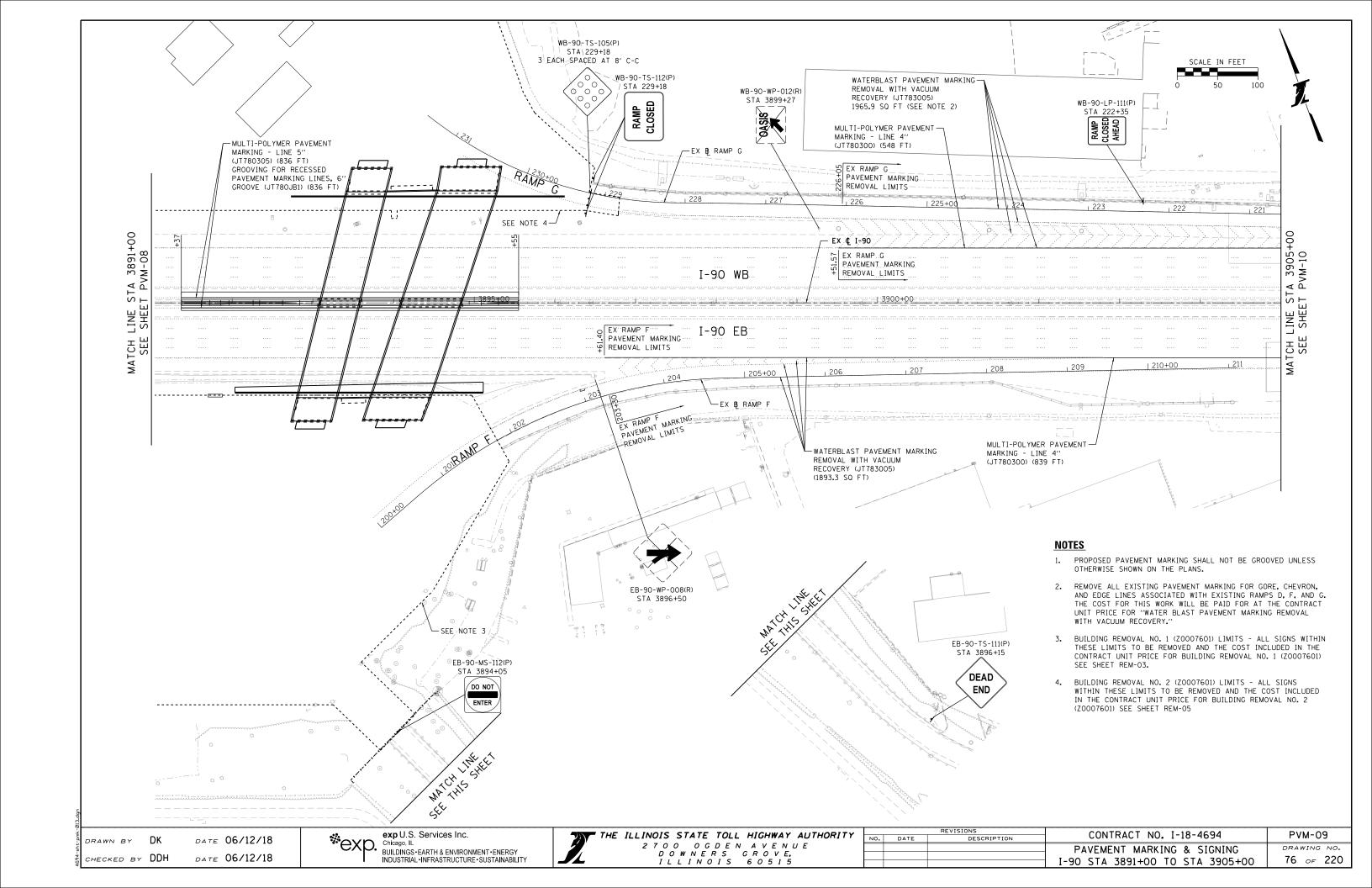
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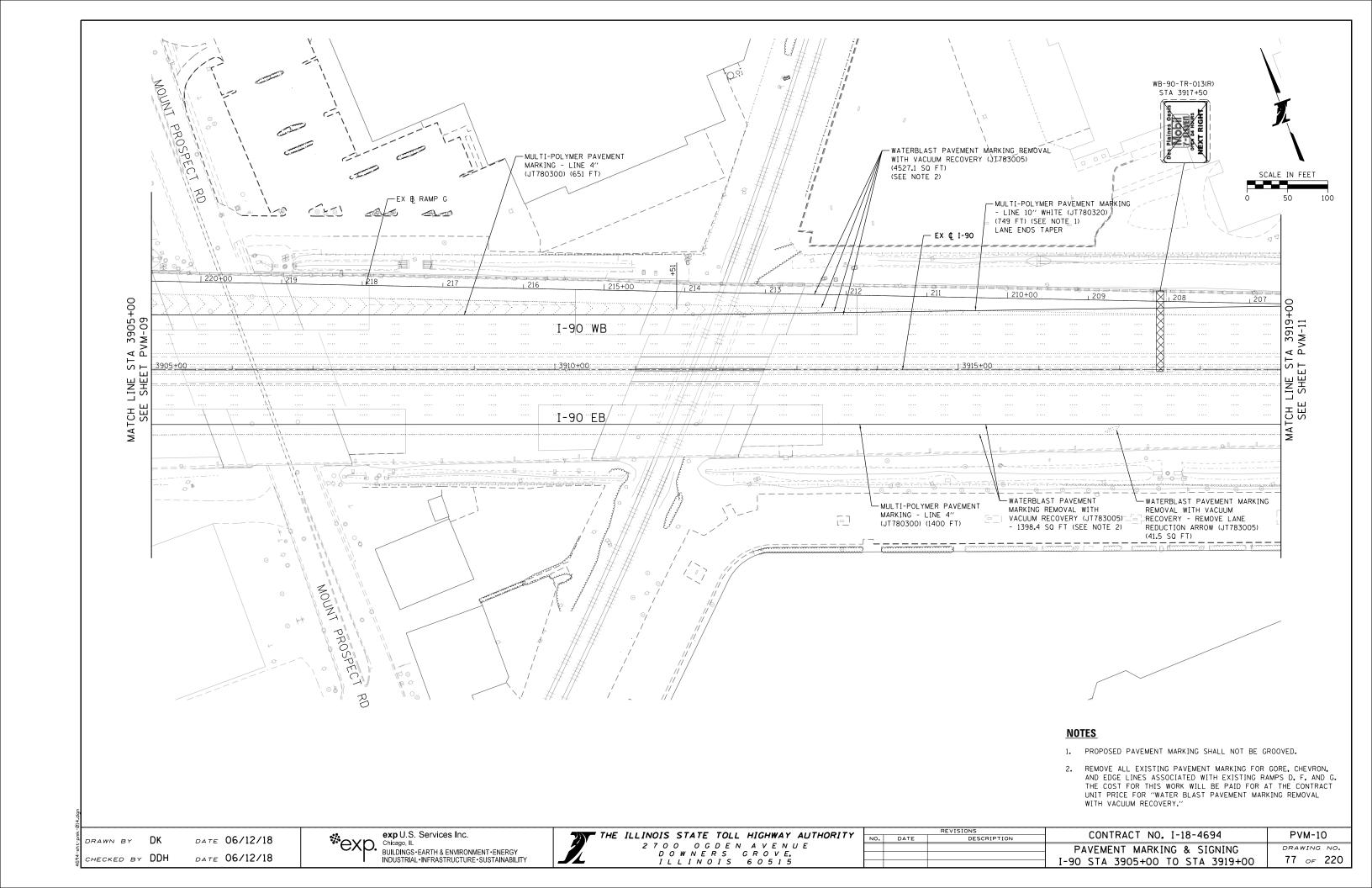
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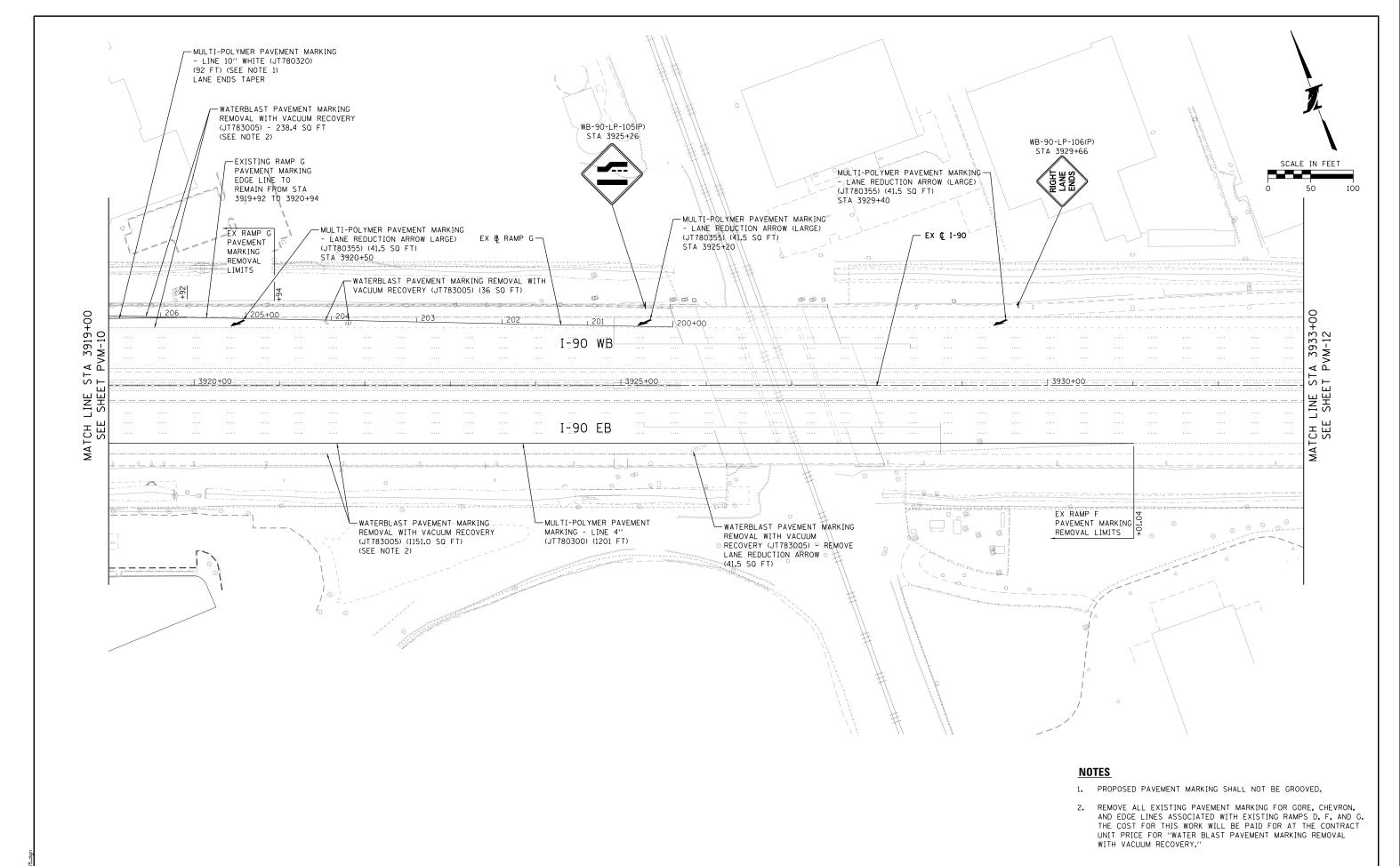
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CONTRACT NO. I-18-4694 PVM-07 DESCRIPTION DRAWING NO. PAVEMENT MARKING & SIGNING 74 of 220 I-90 STA 3863+00 TO STA 3877+00







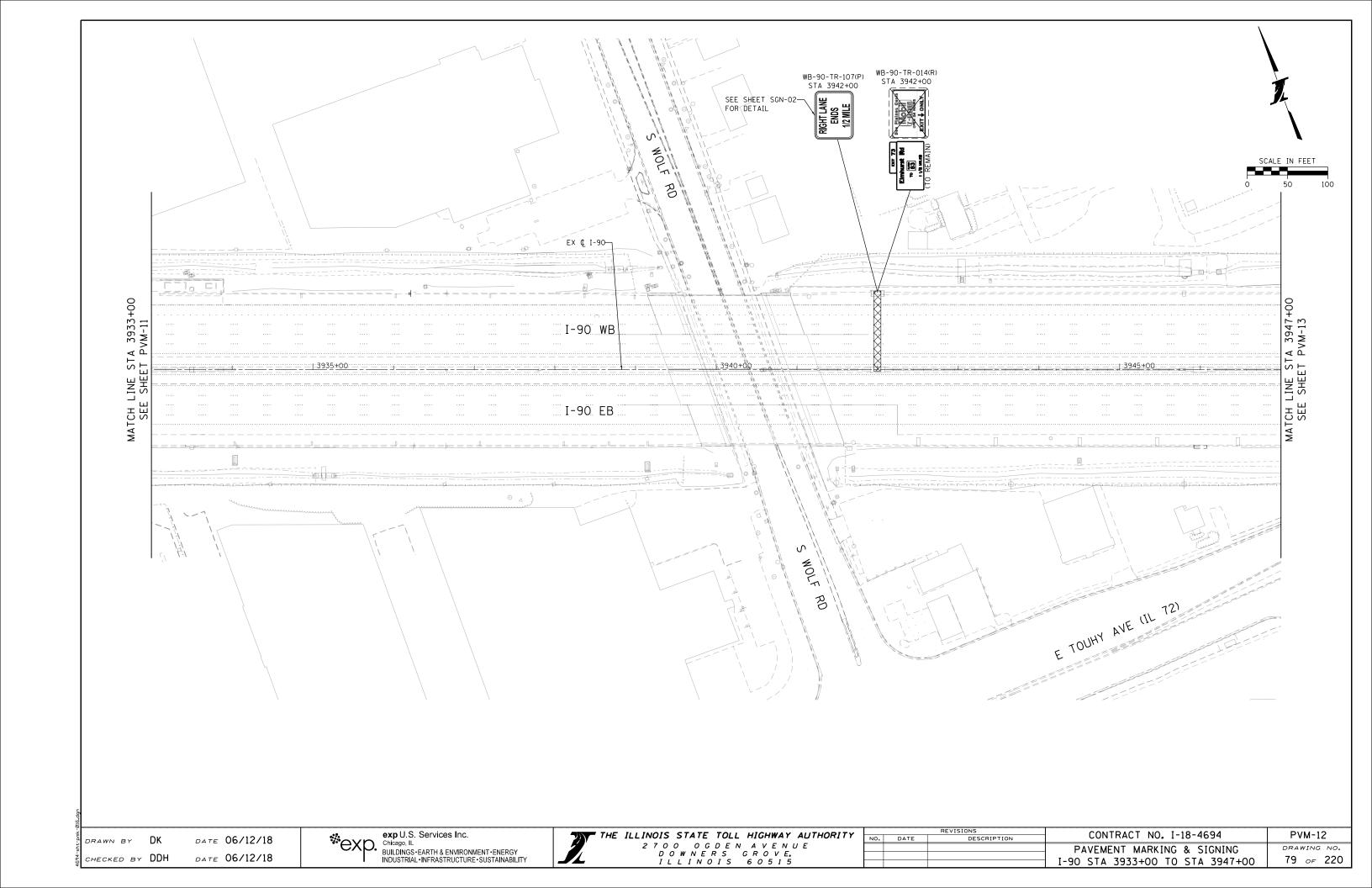


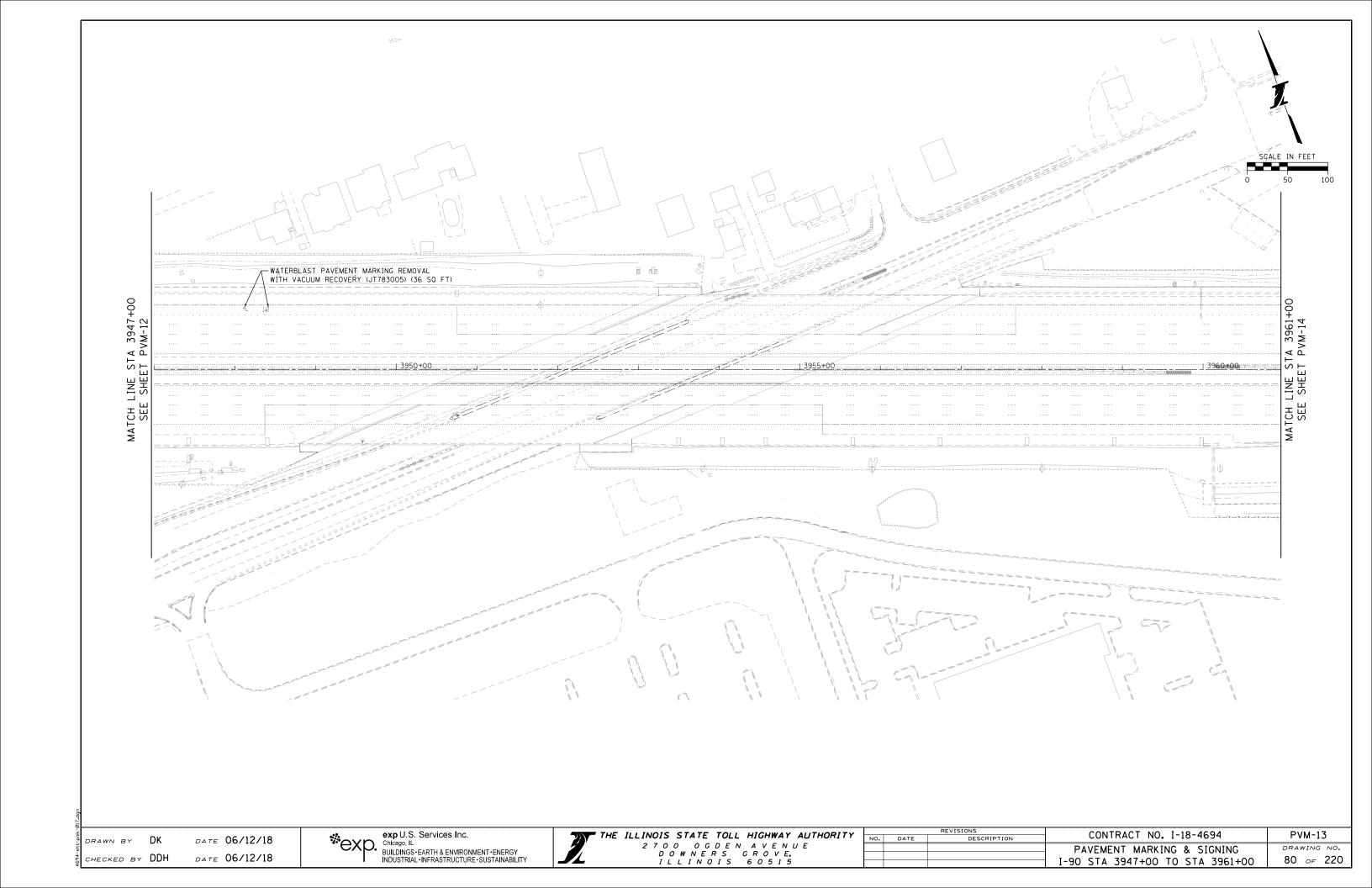
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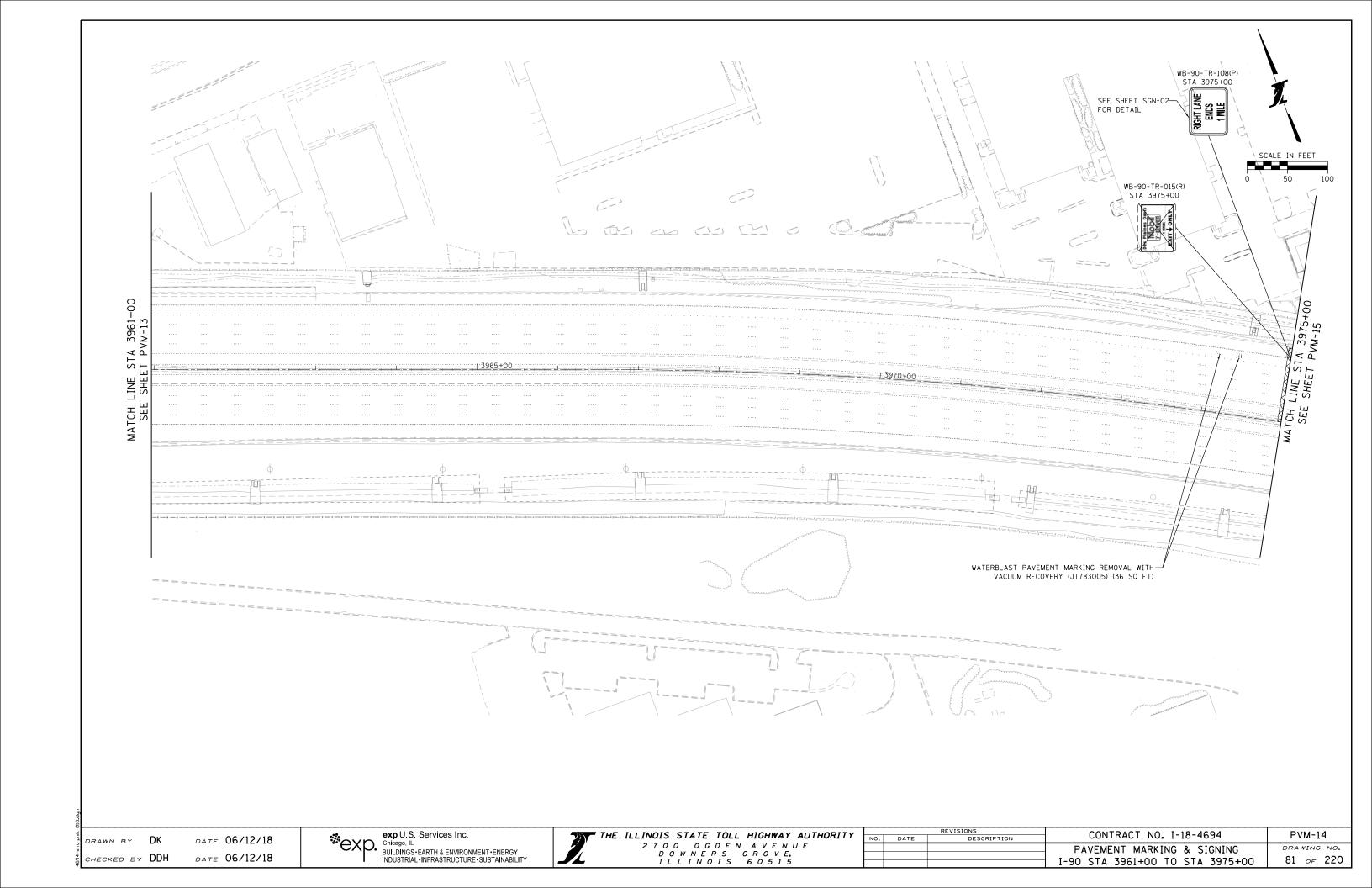
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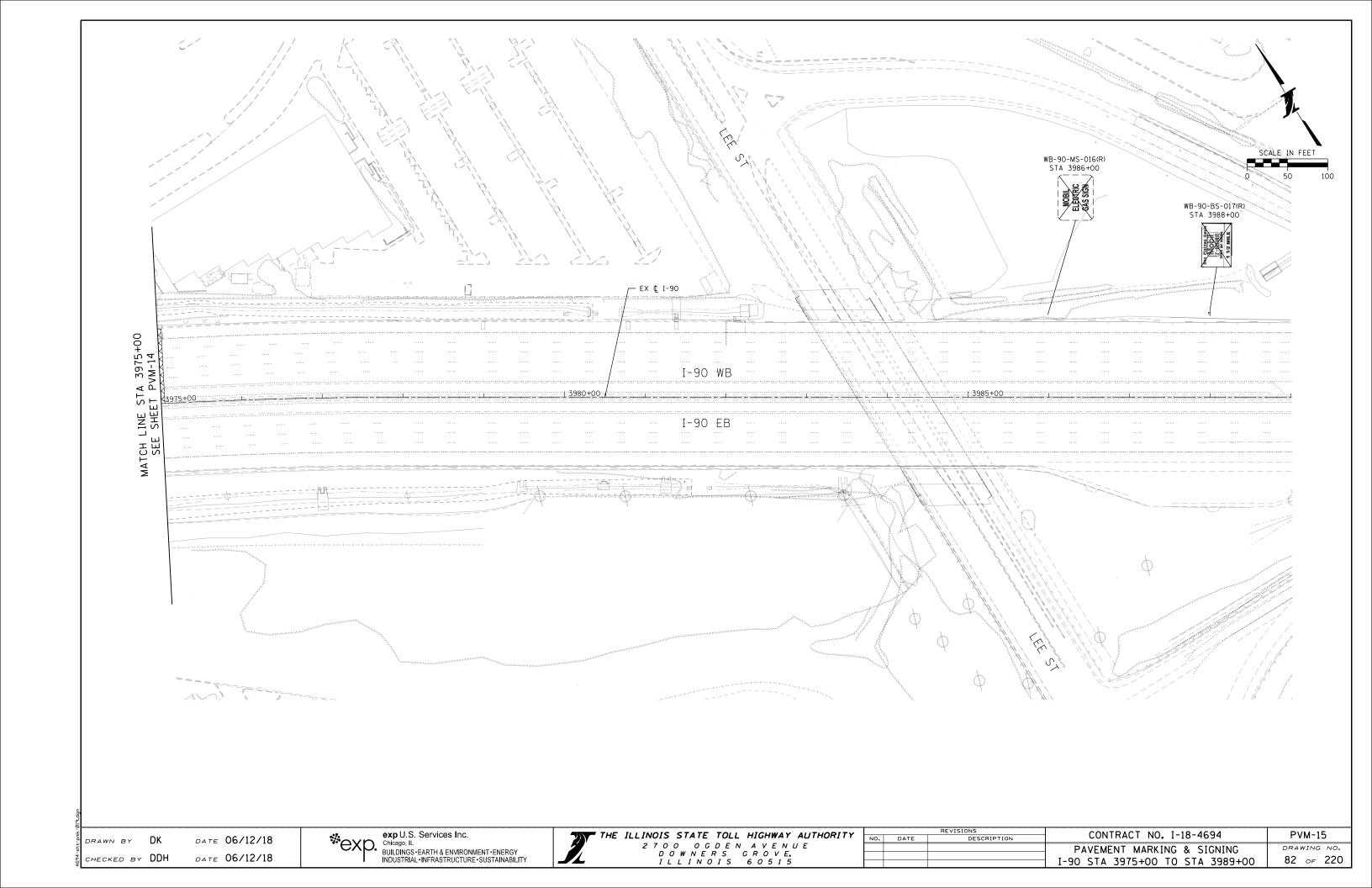
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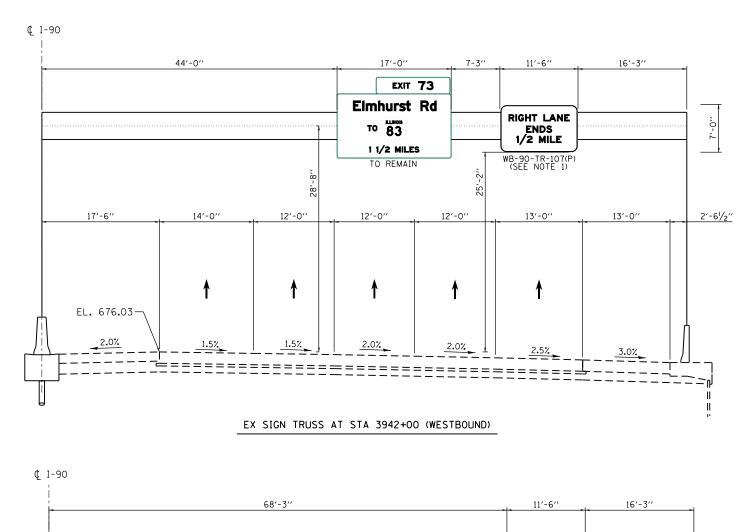
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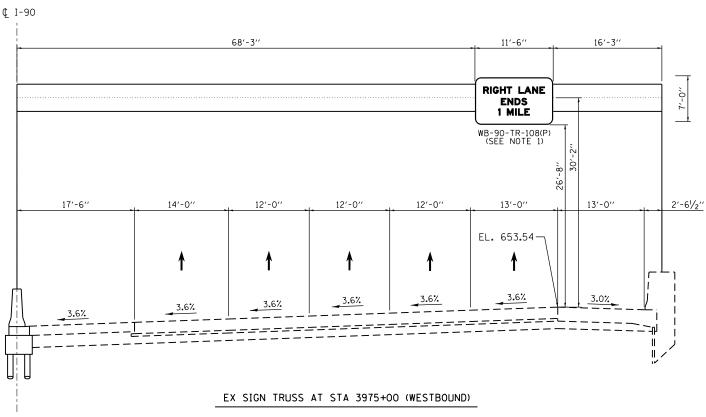












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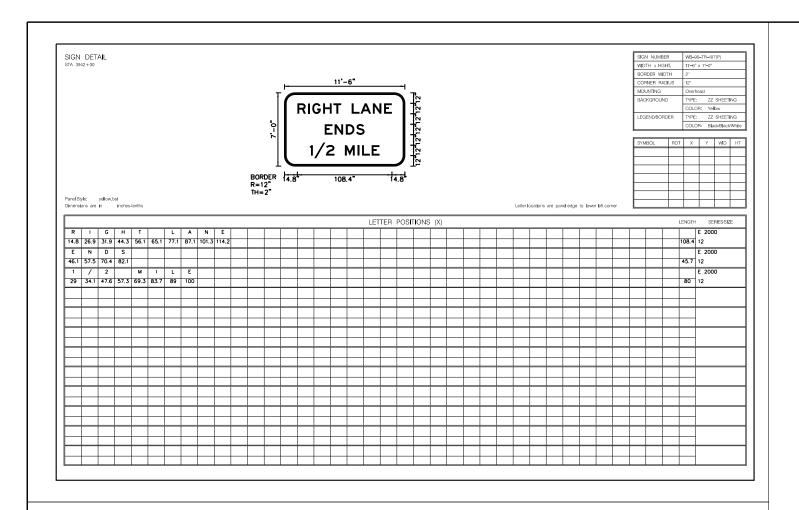
1. SEE SHEET SGN-02 FOR SIGN DETAILS.

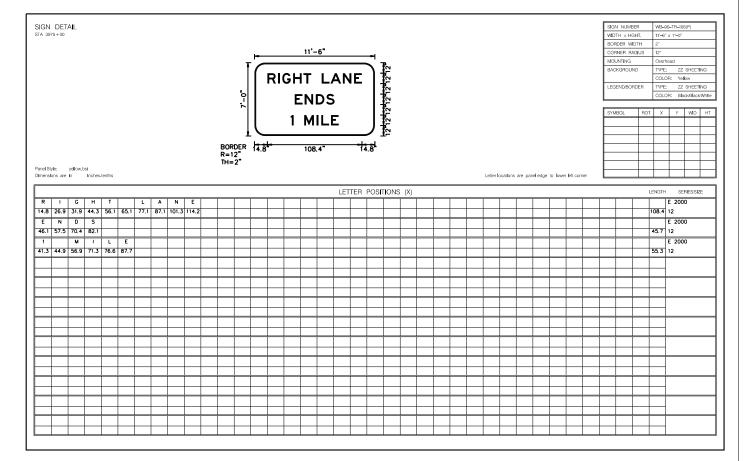
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INDUSTRIAL INFRASTRUCTURE SUSTAINABILITY



	REVISIONS	CONTRACT NO. I-18-4694	SGN-01
DATE	DESCRIPTION	CONTRACT NO. 1-10-4634	3GN-01
		OVERHEAD SIGN DETAILS	DRAWING NO.
		OVERHEAD STON DETAILS	83 <i>o</i> ≥ 220
			83 of 220



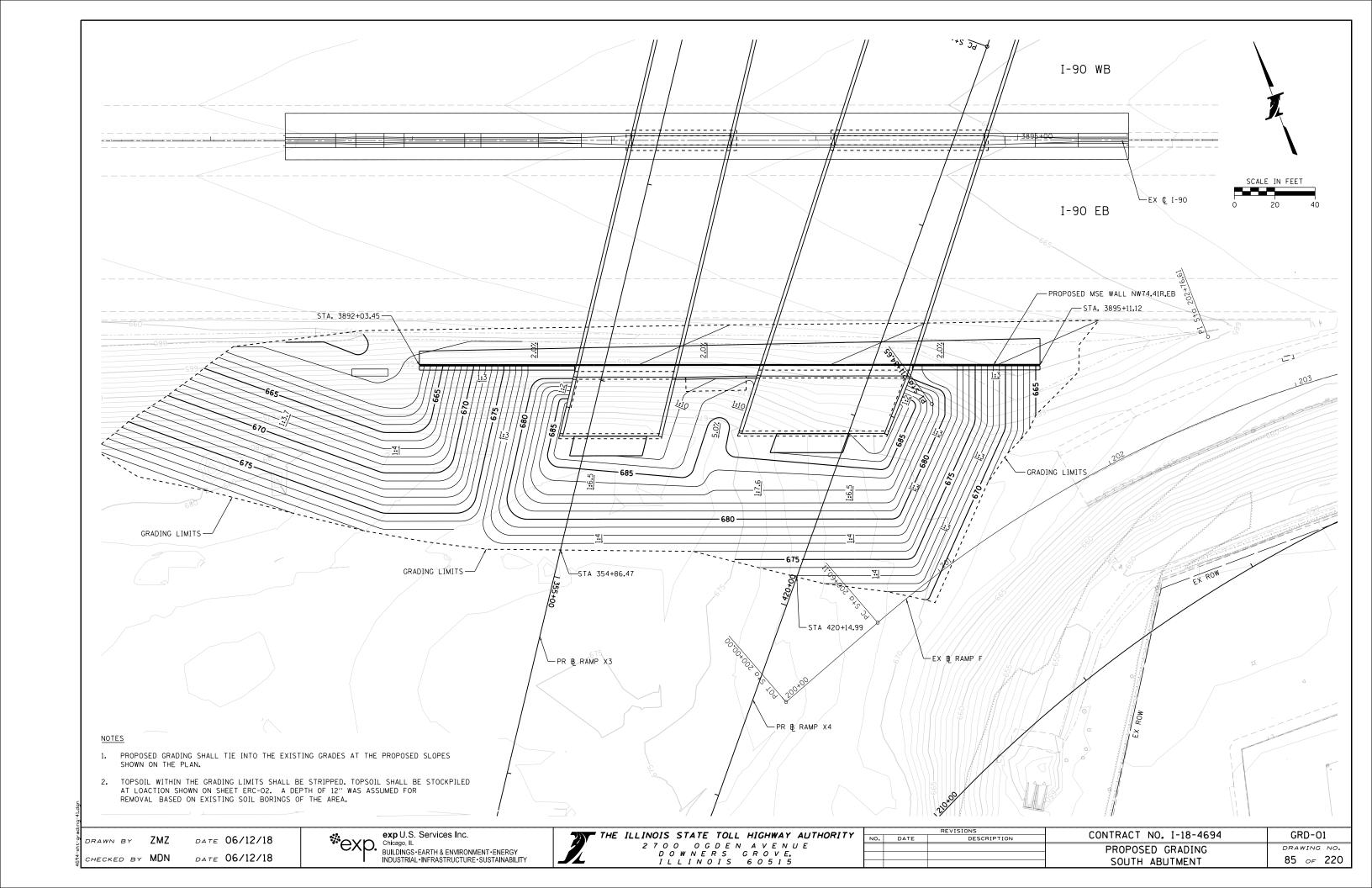


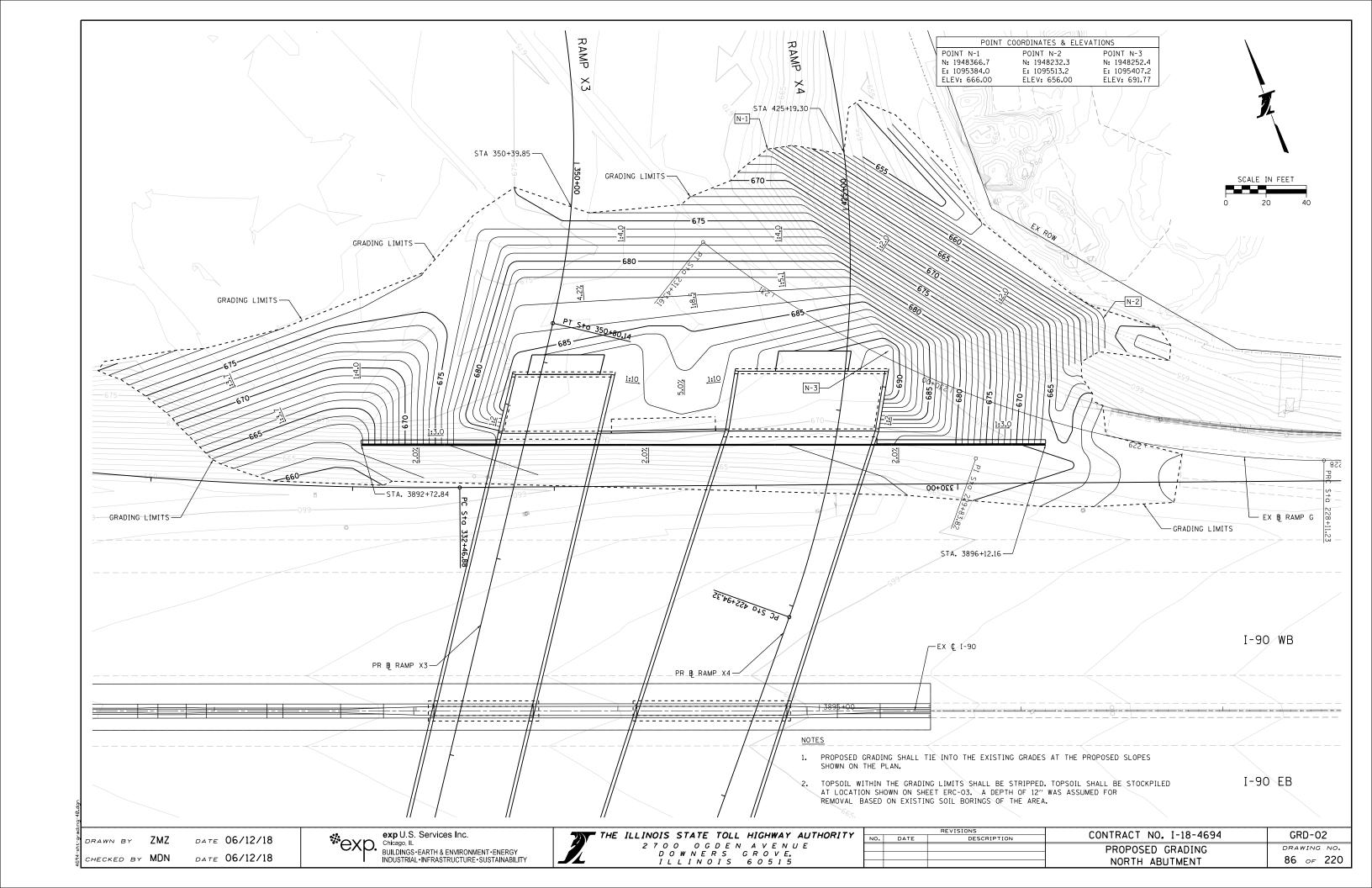
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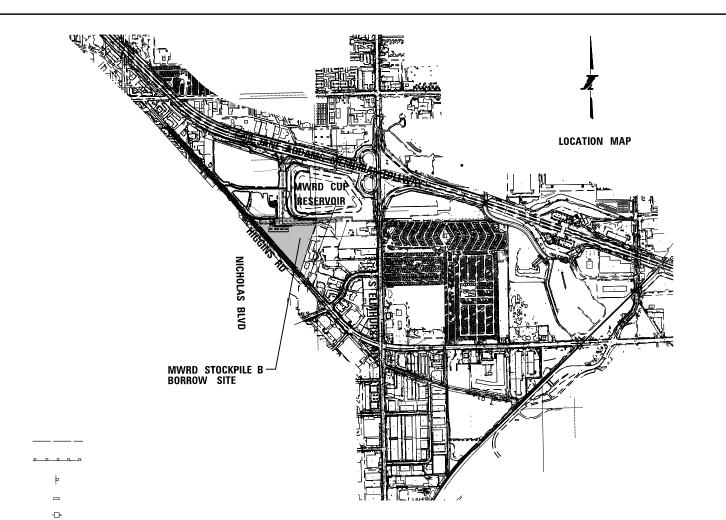
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INDUSTRIAL-INFRASTRUCTURE-SUSTAINABILITY









# **LOCATION MAP**

## INDEX OF DRAWINGS

SHEET NO.	SHEET DESCRIPTION
BSM-01	LEGEND, INDEX OF SHEETS
BSM-02	EXISTING TYPICAL SECTIONS AND DOUBLE CRANE MAT DETAIL
BSM-03	HAUL ROAD ALIGNMENT, TIES AND BENCHMARKS
BSM-04	EXISTING BORROW SITE HAUL ROAD AND SITE PLAN
BSM-05	MWRD BORROW SITE EROSION CONTROL AND GRADING PLAN
BSM-06	MWRD BORROW SITE EROSION CONTROL AND GRADING PLAN

## **BILL OF MATERIALS**

PAY ITEM NO.	DESCRIPTION	UNIT	QUANTITY
20101400	NITROGEN FERTILIZER NUTRIENT	POUND	93
20101600	POTASSIUM FERTILIZER NUTRIENT	POUND	279
20400100	BORROW EXCAVATION	CU YD	15,580
25100630	EROSION CONTROL BLANKET	SQ YD	15,110
JS107361	APPLY DUST SUPPRESSION AGENTS	UNIT	363
JT154168	ALLOWANCE FOR HAUL ROAD MAINTENANCE	UNIT	40,000
JT250442	SEEDING CLASS 4F NATIVE GRASS LOW PROFILE MIX (SPECIAL)	ACRE	3.1
JT900088	SETTLEMENT MONITORING	CAL MO	8

DRAWN BY SB DATE 06/12/18 CHECKED BY BRH DATE 06/12/18

**LEGEND** 

EXISTING ROW EXISTING GUARD RAIL EXISTING SIGN EXISTING INLET EXISTING POWER POLE EXISTING SPLICE BOX

EXISTING CONTROLLER BOX EXISTING FIRE HYDRANT EXISTING HANDHOLE EXISTING LIGHT POLE EXISTING MANHOLE

EXISTING OVERHEAD UTILITY

EXISTING BOX CULVERT HEADWALL

EXISTING GAS LINE EXISTING SANITARY LINE EXISTING WATER LINE EXISTING TRAFFIC SIGNAL

EXISTING DITCH CHECK EXISTING FIELD VENT PIPE EXISTING PAVEMENT UNDERDRAINS EXISTING VEGETATION OUTLINE EXISTING SINGLE TREE OR BUSH

EXISTING BUILDINGS

EXISTING FENCE

EXISTING WETLAND

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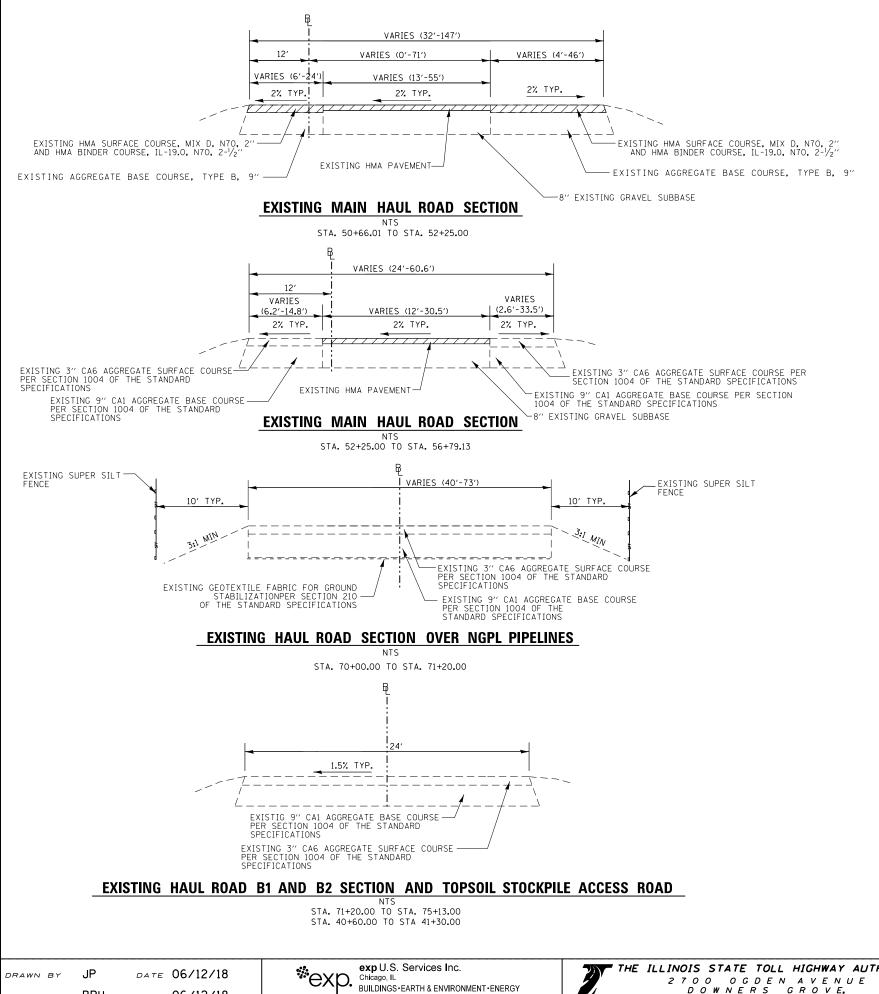
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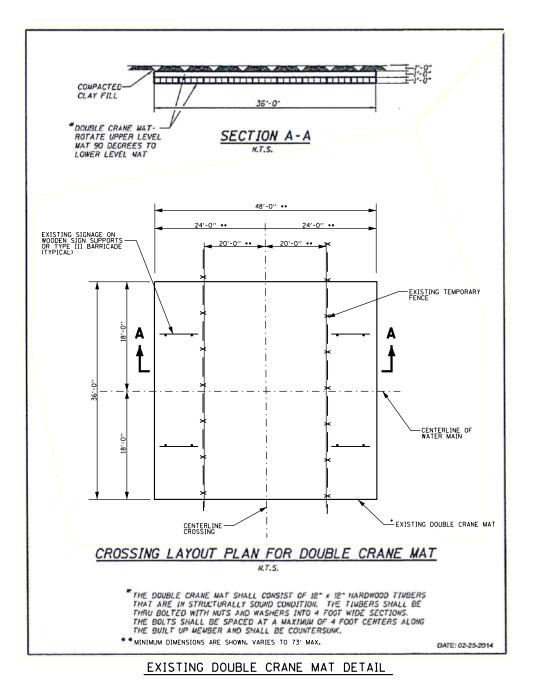
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		REVISIONS	CONTRACT NO. I-18-4694	DCM-01
NO.	DATE	DESCRIPTION	CUNTRACT NO. 1-10-4634	BSM-01
			LEGEND. INDEX OF SHEETS.	DRAWING NO.
			•	87 of 220
			AND BILL OF MATERIALS	01 OF 220



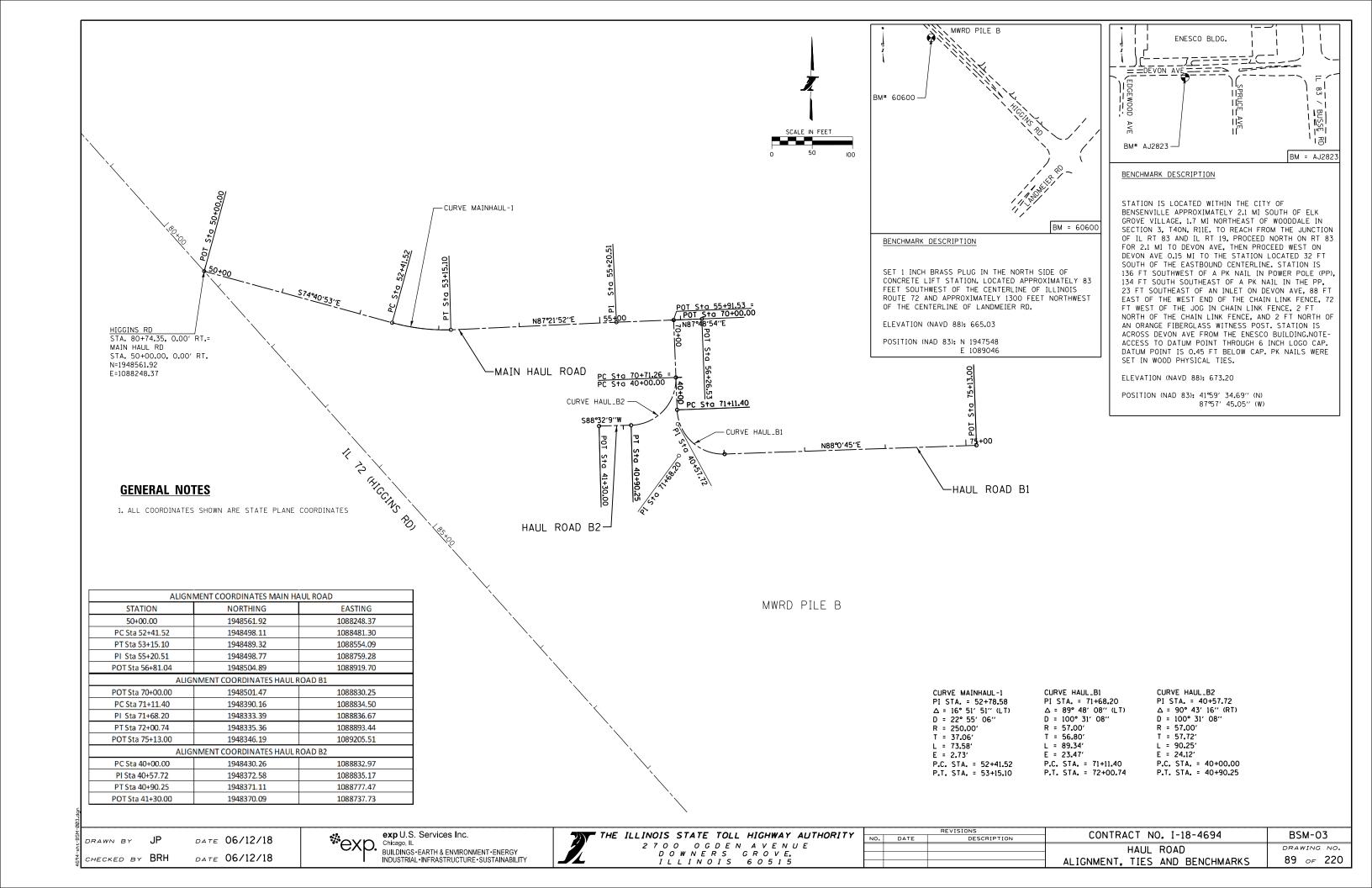


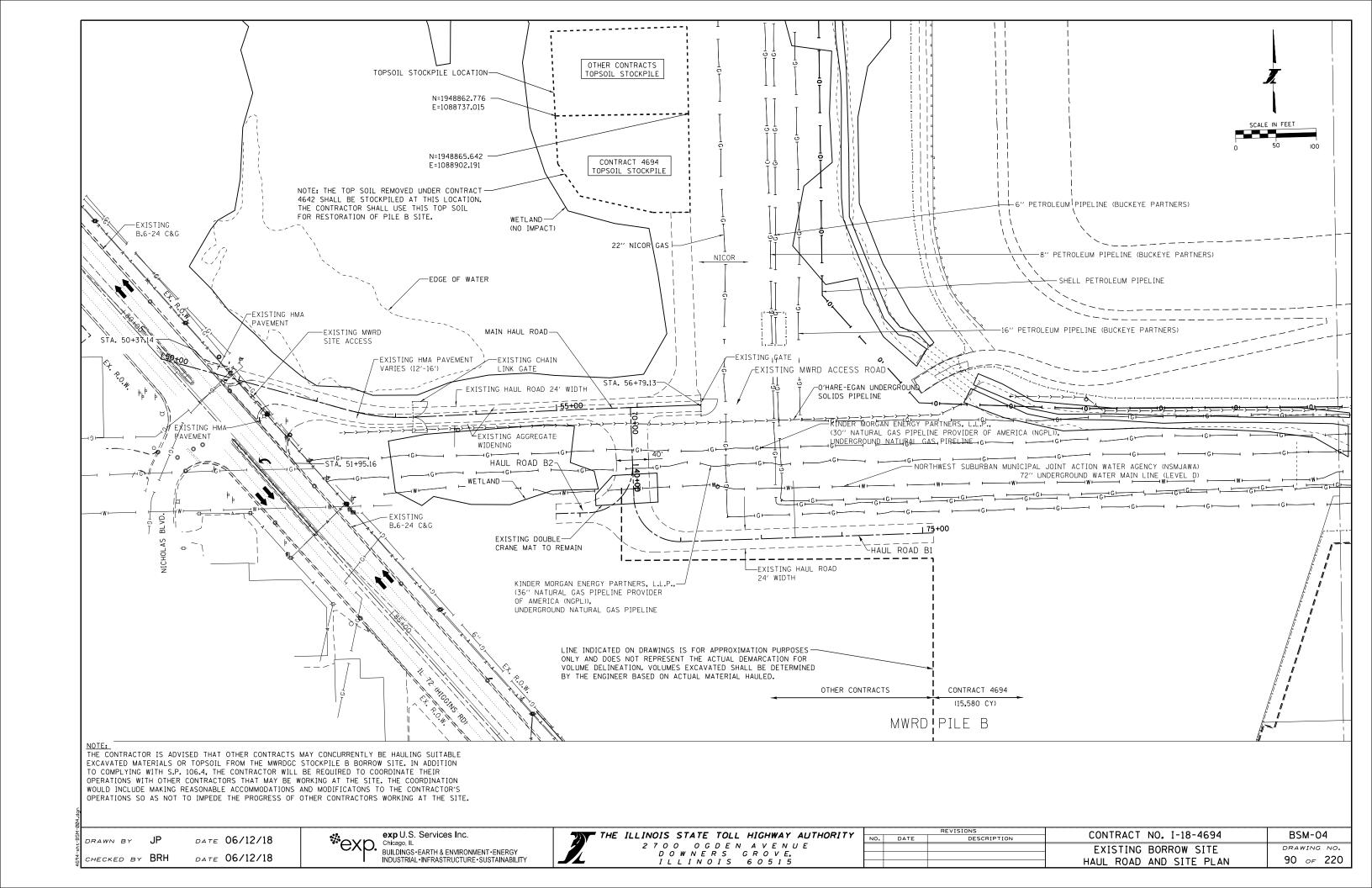
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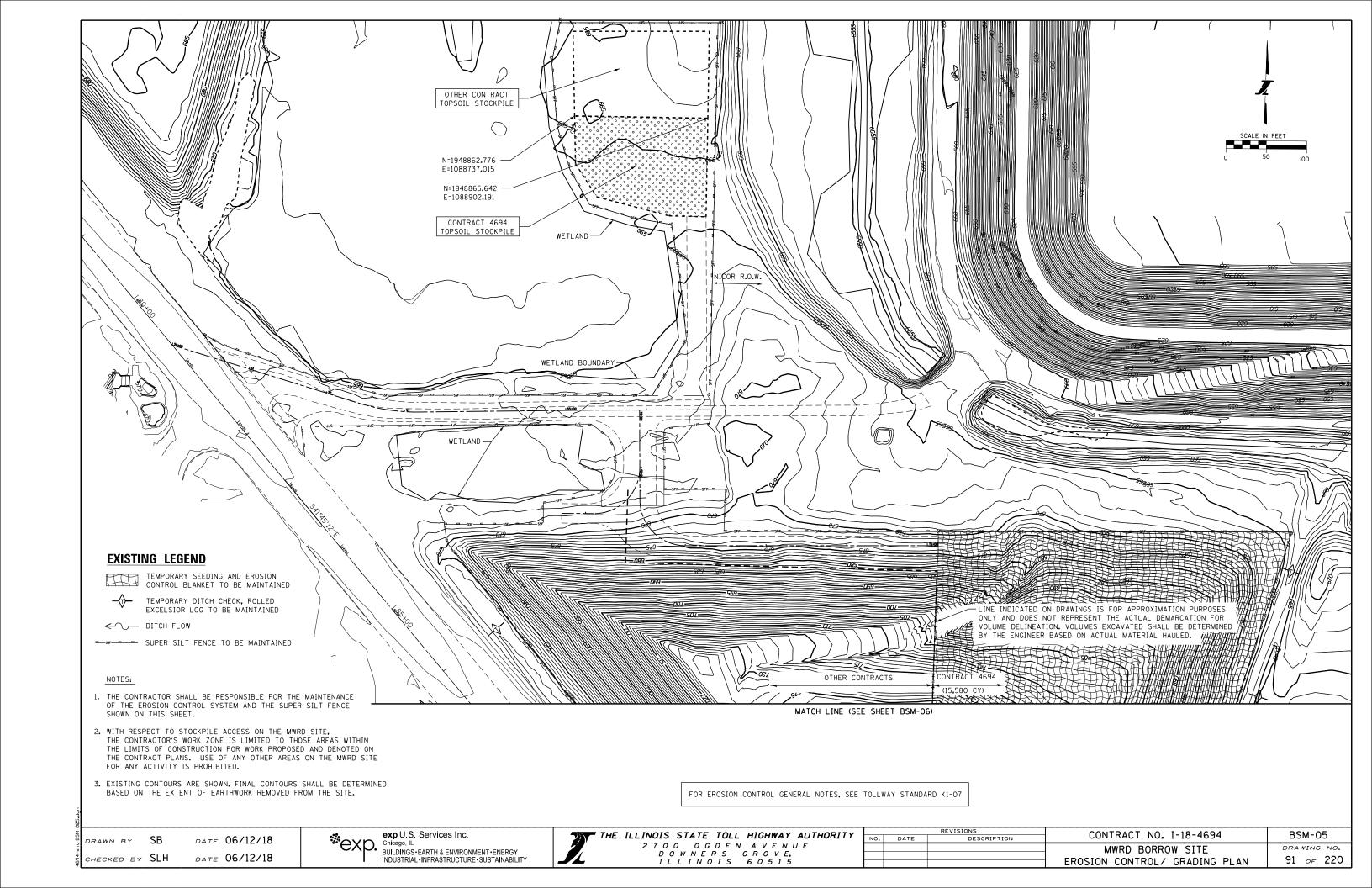
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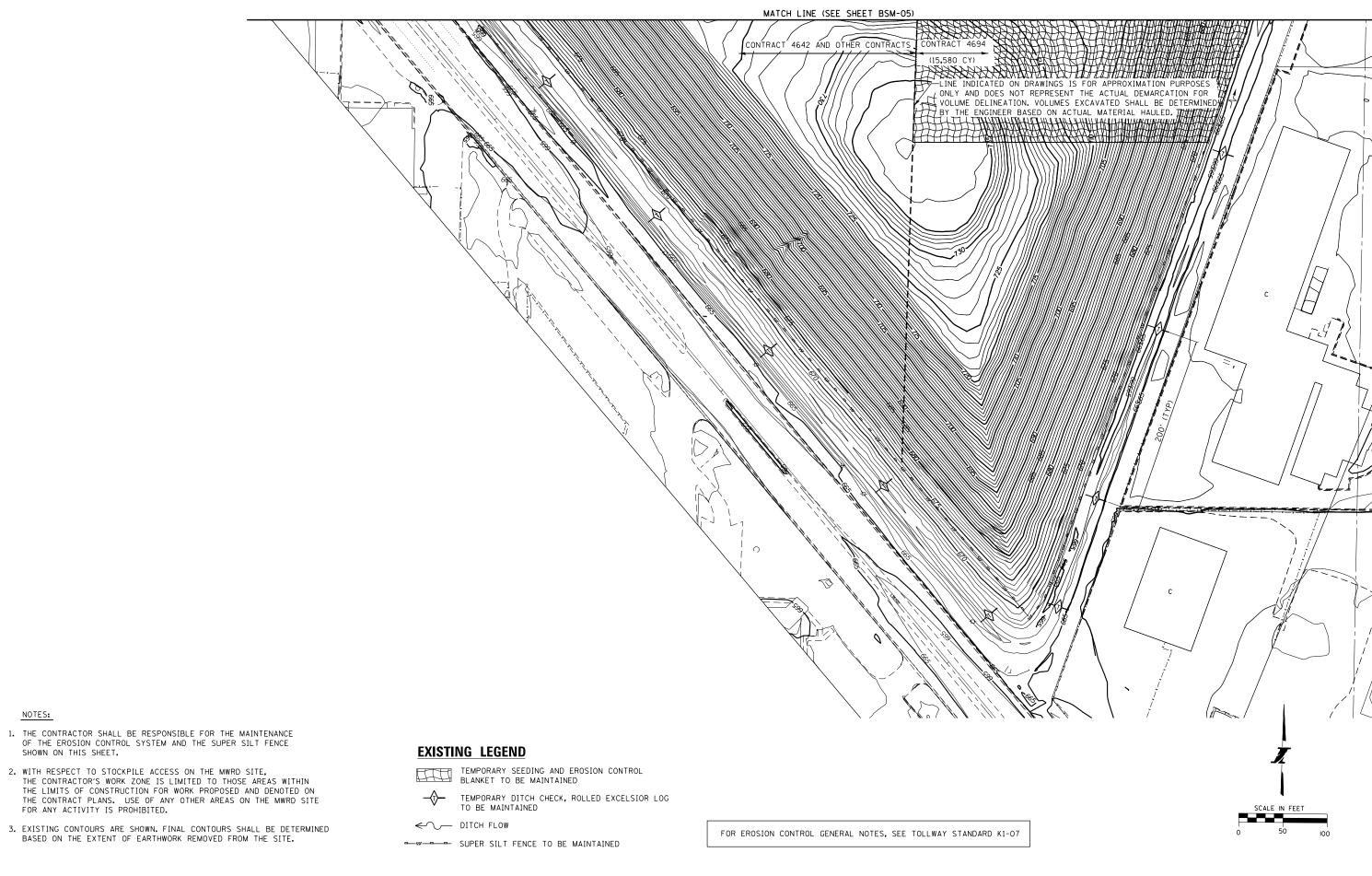


REVISIONS			CONTRACT NO. I-18-4694	BSM-02		
ο.	DATE	DESCRIPTION	CUNTRACT NO. 1-10-4634	D3M-02		
			EXISTING TYPICAL SECTIONS	DRAWING NO.		
				88 <i>o</i> 220		
			AND DOUBLE CRANE MAT DETAIL	00 OF 220		









DRAWN BY SB DATE 06/12/18 DATE 06/12/18

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THE ILLINOIS STATE TOLL HIGHWAY AUTHORITY 2700 OGDEN AVENUE DOWNERS GROVE, ILLINOIS 60515

		REVISIONS	CONTRACT NO. I-18-4694	BSM-06	
NO. DATE DESCRIPTION			CONTRACT NO. 1-10-4634	D2M-06	
			MWRD BORROW SITE	DRAWING NO.	
				92 <i>o</i> 220	
			EROSION CONTROL / GRADING PLAN	92 of 220	

#### **EROSION AND SEDIMENT CONTROL AND SEEDING SCHEDULE**

	NITROGEN FERTILIZER NUTRIENT	POTASSIUM FERTILIZER NUTRIENT	GEOTECHNICAL FABRIC FOR GROUND STABILIZATION	FILTER FABRIC	EROSION CONTROL BLANKET	STABILIZED CONSTRUCTION ENTRANCE	SILT FENCE	RE-ERECT SILT FENCE	SUPER SILT FENCE	TEMPORARY PIPE SLOPE DRAINS	TEMPORARY RIPRAP	RECTANGULAR INLET PROTECTION	FILTER FABRIC INLET PROTECTION, BASKET TYPE	TEMPORARY DITCH CHECKS	SEEDING, CLASS ZE SALT TOLERANT ROADSIDE MIX (SPECIAL) (JT250432)	SEEDING, CLASS 4F NATIVE GRASS, LOW PROFILE MIX (SPECIAL)	TEMPORARY CONSTRUCTION FENCE
SHEET	20101400 POUND	20101600 POUND	21001000 S0 YD	28200200 SQ YD	25100630 SQ YD	JS280070 SQ YD	JS280050 F00T	JS280051 F00T	JS280100 F00T	JS280110 F00T	JS280140 TON	JS280180 EACH	JS280210 EACH	JS280305 F00T	JT250432 ACRE	JT250442 ACRE	JT900202 F00T
ERC-02	240	720	30 10	48	38674	194	2508	502	1374	169	23	6	5	10	0.4	7.6	1 001
ERC-03	243	729	389	74	39298	194	3377	675	707	224	37	0	0	110	0.7	7.4	1136
TOTAL	483	1449	389	122	77972	388	5885	1177	2081	393	60	6	5	120	1.1	15.0	1136

#### **GENERAL NOTES**

- 1) FOR EROSION AND SEDIMENT CONTROL GENERAL NOTES, SEE STANDARD K1-07 SHEETS.
- 2) THE PERMANENT SEEDING SHALL BE USED ON ALL DISTURBED AREAS WHENEVER POSSIBLE.
- ALL TEMPORARY STOCKPILES SHALL HAVE SILT FENCE AT THE PERIMETER OF THE STOCKPILE, STOCKPILES SHALL NOT BE LOCATED CLOSER THAT 25 FEET TO A PAVED ROADWAY OR 100 FEET TO A DRAINAGE CHANNEL. STOCKPILES SHALL NOT BE LOCATED IN THE FLOODPLAIN, OVERFLOW ROUTES, RIPARIAN AREAS (VEGETATED FLOODPLAINS), WETLANDS, WATERS OF THE U.S., OR AREAS SUBJECT TO INUNDATION. TEMPORARY STOCKPILE LOCATIONS SHALL BE APPROVED BY THE ENGINEER. SEDIMENT CONTROL MEASURES MUST BE IN PLACE PRIOR TO THE BUILDING OR REMOVAL OF ANY STOCKPILE.
- RUNOFF LEAVING THE JOB SITE MUST PASS THROUGH AN EROSION AND SEDIMENT CONTROL SYSTEM FOLLOWING TOLLWAY STANDARD K1-07 "TEMPORARY EROSION AND SEDIMENT CONTROLS" AND AS SHOWN IN THE PLANS.
- THE CONTRACTOR WILL ASSUME RESPONSIBILITY FOR MAINTENANCE OF ALL SOIL EROSION AND SEDIMENT CONTROL DURING CONSTRUCTION.
- GRAVELED ROADS, ACCESS DRIVES, PARKING AREAS OF SUFFICIENT WIDTH AND LENGTH, AND VEHICLE WASH DOWN OR UNDER CARRIAGE WASH OFF FACILITIES IF NECESSARY, SHALL BE PROVIDED TO PREVENT THE DEPOSIT OF SOIL FROM BEING TRACKED ONTO PUBLIC OR PRIVATE ROADWAYS. ANY SOIL REACHING A ROADWAY SHALL BE CLEANED TO THE SATISFACTION OF THE ENGINEER.
- SHOULD IT BE NECESSARY TO REMOVE ANY EROSION CONTROL DEVICES FOR CONSTRUCTION REASONS, THE CONTRACTOR SHALL FIRST OBTAIN PERMISSION AND SHALL REPAIR OR REPLACE THE REMOVED DEVICES THE SAME DAY. THE COST OF REMOVING AND REPLACING THE DEVICE SHALL BE INCIDENTAL TO THE CONTRACT.
- SILT FENCE SHALL BE USED AS A PERIMETER SEDIMENT BARRIER TO FILTER RUNOFF LEAVING THE PROJECT LIMITS AS INDICATED ON THE EROSION AND SEDIMENT CONTROL PLAN. THE RESIDENT ENGINEER SHALL MAKE THE FINAL DETERMINATION ON PLACEMENT AND LOCATION OF THE PERIMETER EROSION BARRIER.
- EXISTING AND PROPOSED DRAINAGE STRUCTURES RECEIVING RUNOFF SHALL BE PROTECTED BEFORE CONSTRUCTION COMMENCES UPSTREAM.
- THE CONDITION OF THE CONSTRUCTION SITE FOR WINTER SHUTDOWN SHALL BE ADDRESSED EARLY IN THE FALL GROWING SEASON SO THAT SLOPES AND OTHER EARTH AREAS MAY BE STABILIZED WITH FINAL VEGETATIVE COVER FOR PROPER EROSION AND SEDIMENT CONTROL. ALL OPEN AREAS THAT ARE TO REMAIN IDLE THROUGHOUT THE WINTER SHALL RECEIVE TEMPORARY EROSION CONTROL MEASURES INCLUDING FINAL SEEDING AND EROSION CONTROL BLANKET PRIOR TO THE END OF THE FALL GROWING SEASON. THE AREAS TO BE WORKED BEYOND THE END OF THE GROWING SEASON MUST INCORPORATE SOIL STABILIZATION MEASURES THAT DO NOT RELY ON VEGETATIVE COVER SUCH AS EROSION CONTROL BLANKET.
- 11) THE CONTRACTOR SHALL REFER TO SECTION 280.02 OF THE ILLINOIS TOLLWAY SUPPLEMENTAL SPECIFICATIONS FOR PENALTIES FOR NON-CONFORMANCE.
- EROSION AND SEDIMENT CONTROL ITEMS ARE CONSIDERED TO BE HIGH PRIORITY ITEMS ON THIS CONTRACT. THE ENGINEER WILL IMPLEMENT ALL PROVISIONS OF THE SPECIFICATION NECESSARY TO ENSURE THAT SOIL EROSION AND SEDIMENT CONTROL ITEMS ARE CONSTRUCTED AND MAINTAINED TO CONTROL OFF-SITE SEDIMENT DISCHARGES.
- 13) TEMPORARY EROSION AND SEDIMENT CONTROL MEASURES ARE TO BE INSTALLED, EFFECTIVE, AND MAINTAINED THROUGHOUT ALL PHASES OF CONSTRUCTION, INCLUDING SHUTDOWN
- 14) THE CONTRACTOR SHALL CONFINE CONSTRUCTION ACTIVITIES WITHIN THE CONSTRUCTION LIMITS AS SHOWN ON THE PLANS. AREAS OUTSIDE THE SHOWN CONSTRUCTION LIMITS DISTURBED BY THE CONTRACTOR SHALL BE RESTORED AND STABILIZED AS DIRECTED BY THE ENGINEER AT THE CONTRACTOR'S EXPENSE.

- TEMPORARY EROSION AND SEDIMENT CONTROL DEVICES SHALL BE CONSTRUCTED AS SHOWN ON THE PLANS OR AS DIRECTED BY THE ENGINEER. ANY DEVIATION FROM THE TEMPORARY EROSION CONTROL PLAN OR SCHEDULE SHALL BE AT THE DISCRETION OF THE ENGINEER.
- IN CASE OF CONFLICT BETWEEN THE EROSION CONTROL TABLES, EROSION CONTROL PLAN AND OVERVIEW DRAWINGS, CONTRACTOR SHALL NOTIFY THE ENGINEER AND RECEIVE CLARIFICATIONS BEFORE PROCEEDING WITH THE WORK.
- THE CONTRACTOR SHALL SUBMIT AS PART OF THEIR SIGNED CONTRACTOR CERTIFICATION STATEMENT THE ITEMS SPECIFIED IN S.P. 111.2, STORM WATER POLLUTION PREVENTION
- FOR THE DURATION OF THE PROJECT, THE CONTRACTOR SHALL PROTECT ALL ON-SITE, ADJACENT AND/OR DOWNSTREAM SEWERS, DITCHES, AND WATERCOURSES FROM CONTAMINATION BY WATERBORNE SILTS, SEDIMENTS, FUELS, SOLVENTS, LUBRICANTS, OR OTHER POLLUTANTS ORIGINATING FROM ANY WORK DONE ON OR IN SUPPORT OF THE PROJECT.
- THE CONTRACTOR SHALL BE REQUIRED TO TREAT TRAVELED AND OTHER PROJECT AREAS TO CONTROL DUST. WATER SHALL BE APPLIED TO SUCH AREAS AS DIRECTED BY THE ENGINEER, CALCIUM CHLORIDE SHALL NOT BE USED FOR THIS PURPOSE. DUST SHALL BE CONTROLLED THROUGH A UNIFORM APPLICATION OF SPRAYED WATER IN A MANNER MEETING ENGINEER APPROVAL AND IN ACCORDANCE WITH THE CONTRACTOR'S DUST CONTROL PLAN SUBMITTED IN ACCORDANCE WITH ARTICLE 107.36 OF THE TOLLWAY SUPPLEMENTAL SPECIFICATIONS. THE NUMBER OF APPLICATIONS AND THE AMOUNT OF WATER SHALL BE BASED ON FIELD AND WEATHER CONDITIONS.
- ALL TEMPORARY EROSION AND SEDIMENT CONTROLS SHALL BE REMOVED ONCE CONSTRUCTION IS COMPLETE AND DISTURBED AREAS ARE PERMANENTLY STABILIZED.

#### **SEQUENCING NOTES:**

- 1) REFER TO GEN-03 AND GEN-04 FOR SUGGESTED CONSTRUCTION SEQUENCING.
- CONSTRUCTION ACTIVIES SHALL BE SCHEDULED TO MINIMIZE THE TIME THE SOIL IS EXPOSED AND UNPROTECTED. IN NO CASE SHALL THE EXISTING VEGETATION BE DESTROYED, REMOVED, OR DISTRUBED MORE THAN FOURTEEN (14) CALENDAR DAYS PRIOR TO THE INITIATION OF IMPROVEMENTS.
- ALL EROSION AND SEDIMENT CONTROL FEATURES SHALL BE PHASED OR IMPLEMENTED PRIOR TO COMMENCEMENT OF UPLAND DISTURBANCE. SOIL DISTURBANCE SHALL BE CONSTRUCTED IN SUCH A MANNER AS TO MINIMIZE EROSION. SOIL STABILIZATION MEASURES SHALL CONSIDER THE TIME OF YEAR, SITE CONDITIONS AND THE USE OF TEMPORARY AND/OR PERMANENT MEASURES.
- THE CONTRACTOR SHALL BE REQUIRED TO INSTALL ALL EROSION AND SEDIMENT CONTROL MEASURES PRIOR TO BEGINNING ANY CONSTRUCTION ACTIVITIES WHICH WILL POTENTIALLY CREATE ERODIBLE CONDITIONS.
- PERMANENT SEEDING SHALL BE PROVIDED AT ALL EXPOSED EARTH DURING CONSTRUCTION STAGES, AND AT ALL LOCATIONS AS DIRECTED BY THE ENGINEER.

#### **INSPECTION AND MAINTENANCE:**

- THE CONTRACTOR SHALL ASSIGN AN ESCM TO THE PROJECT. THIS PERSON IS REQUIRED TO HAVE TAKEN AN APPROVED SEDIMENT AND EROSION CONTROL TRAINING COURSE. THE ESCM WILL BE RESPONSIBLE FOR SUPERVISING THE MAINTENANCE OF EROSION & SEDIMENT CONTROL MEASURES AND IMPLEMENTATION OF THIS PLAN.
- 2) A MAINTENANCE INSPECTION REPORT SHALL BE PREPARED AFTER EACH INSPECTION AND RETAINED FOR REVIEW BY THE IEPA OR OTHER REGULATORY AGENCIES. SEE NPDES GENERAL

- INSPECTION SHALL BE CONDUCTED AT LEAST ONCE EVERY SEVEN (7) CALENDAR DAYS AND WITHIN 24 HOURS OF THE END OF A STORM THAT IS 0.5 INCHES OR GREATER OR THE EQUIVALENT SNOWFALL. INSPECTIONS MAY BE REDUCED TO ONCE PER MONTH WHEN EQUIVALENT SNOWFALL. INSPECTIONS MAY BE REDUCED TO ONCE PER MONTH WHEN CONSTRUCTION ACTIVITIES HAVE CEASED DUE TO FROZEN CONDITIONS. WEEKLY INSPECTIONS SHALL RECOMMENCE WHEN CONSTRUCTION ACTIVITIES ARE RESUMED
- ALL CONTROLS SHALL BE MAINTAINED IN GOOD WORKING ORDER BY THE GENERAL CONTRACTOR OR SUBCONTRACTOR, IF REPAIR IS WARRANTED IT SHALL BE INITIATED WITHIN 24 HOURS.
- NEW CONTROL MEASURES NEEDED OR CONTROLS NEEDING MODIFICATION AS A RESULT OF AN INSPECTION SHALL BE IMPLEMENTED AS SOON AS PRACTICAL BUT NO LATER THAN 7
- REQUESTS FOR REPAIRS TO EXISTING CONTROLS OR NEW CONTROL MEASURES REQUESTED BY A REGULATORY AGENCY SHALL BE INITIATED WITHIN 24 HOURS.
- INLET PROTECTION: REMOVE SEDIMENT FROM INLET FILTER BASKETS WHEN BASKET IS 25% FULL OR 50% OF THE FABRIC PORES ARE COVERED WITH SILT. CLEAN FILTER IF STANDING WATER IS PRESENT LONGER THAN ONE HOUR AFTER A RAIN EVENT. CLEAN SEDIMENT OR REPLACE SILT FENCE WHEN SEDIMENT ACCUMULATES TO ONE-THIRD THE HEIGHT OF THE FABRIC. REMOVE TRASH ACCUMULATED AROUND OR ON TOP OF PRACTICE. WHEN FILTER IS REMOVED FOR CLEANING, REPLACE FABRIC IF ANY TEAR IS PRESENT.
- OUTLET PROTECTION/TEMPORARY RIPRAP: RESTORE DISLODGED PROTECTION AND CORRECT EROSION THAT MAY OCCUR. REMEDY DEFICIENT AREAS PRONE TO INCREASED EROSION IMMEDIATELY TO PREVENT GREATER DEFICIENCIES.
- TEMPORARY DITCH CHECKS: REMOVE SEDIMENT FROM UPSTREAM SIDE OF DITCH CHECKS WHEN SEDIMENT HAS REACHED 50% OF HEIGHT OF STRUCTURE. REPAIR OR REPLACE DITCH CHECKS WHENEVER TEARS, SPLITS, UNRAVELING OR COMPRESSED EXCELSIOR IS APPARENT. REPLACE TORN FABRIC MAT THAT MAY ALLOW WATER TO UNDERMINE DITCH CHECK. REMOVE DEBRIS (GARBAGE, CROP RESIDUE, ETC.) WHEN OBSERVED. REESTABLISH THE FLOW OVER THE CENTER OF THE DITCH CHECK. WATER OR SEDIMENT GOING AROUND THE DITCH CHECK INDICATES INCORRECT INSTALLATION. DEVICE NEEDS LENGTHENING OR THE SELECTED DEVICE IS INAPPROPRIATE FOR THE SITE CONDITIONS. REMOVE DITCH CHECKS ONCE ALL UPSLOPE AREAS ARE STABILIZED AND SEED OR OTHERWISE STABILIZE TEMPORARY DITCH CHECK AREAS.
- SILT FENCE: REPAIR TEARS, GAPS OR UNDERMINING. RESTORE LEANING SILT FENCE AND ENSURE TAUT. REPAIR OR REPLACE ANY MISSING OR BROKEN STAKES IMMEDIATELY. CLEAN FENCE LINE IF SEDIMENT REACHES ONE-THIRD HEIGHT OF BARRIER. REMOVE FENCE ONCE FINAL STABILIZATION IS ESTABLISHED. REPAIR FENCE IF UNDERMINING OCCURS ANYWHERE ALONG ITS ENTIRE LENGTH.
- 11) TEMPORARY STABILIZED CONSTRUCTION ENTRANCES: REPLENISH STONE OR REPLACE EXIT IF VEHICLES CONTINUE TO TRACK SEDIMENT ONTO THE ROADWAY FROM THE CONSTRUCTION SITE. SWEEP SEDIMENT ON ROADWAY FROM CONSTRUCTION ACTIVITIES IMMEDIATELY. ENSURE CULVERTS ARE FREE FROM DAMAGE.
- STOCKPILE MANAGEMENT: REPAIR AND/OR REPLACE PERIMETER CONTROLS AND STABILIZATION MEASURES WHEN STOCKPILE MATERIAL HAS POTENTIAL TO BE DISCHARGED OR LEAVE THE LIMITS OF THE PROTECTION. REMOVE ALL OFF-TRACKED MATERIAL BY SWEEPING OR OTHER METHODS. UPDATE THE SWPPP ANY TIME A STOCKPILE LOCATION HAS BEEN REMOVED, RELOCATED, ADDED OR REQUIRED MAINTENANCE. DURING SUMMER MONTHS, STOCKPILES SHOULD BE WATERED TO MAINTAIN THE COVER CROP.
- 13) TEMPORARY SLOPE DRAINS: FILL ERODED AREA AT INLET WITH WELL-COMPACTED SOIL. STABILIZE OUTFALL TO ELIMINATE SCOUR. REPAIR LEAKS ALONG LENGTH OF PIPE AND RE-COMPACT SOIL TO STABILIZE PIPE. RECONNECT PIPE AT JOINTS WHEN SEPARATION OCCURS. RESTORE OR INCREASE ANCHORS ALONG LENGTH OF PIPE TO ENSURE PIPE STABILITY. IF SLOPE DRAIN WASHES OUT IT MAY BE NECESSARY TO USE AGGREGATE-LINED CHANNELS OR ADDITIONAL DRAINS.
- 14) LOCATIONS WHERE VEHICLES ENTER AND EXIT THE SITE INSPECT FOR EVIDENCE OF OFF SITE SEDIMENT TRACING, REMOVE SEDIMENT AS NECESSARY.

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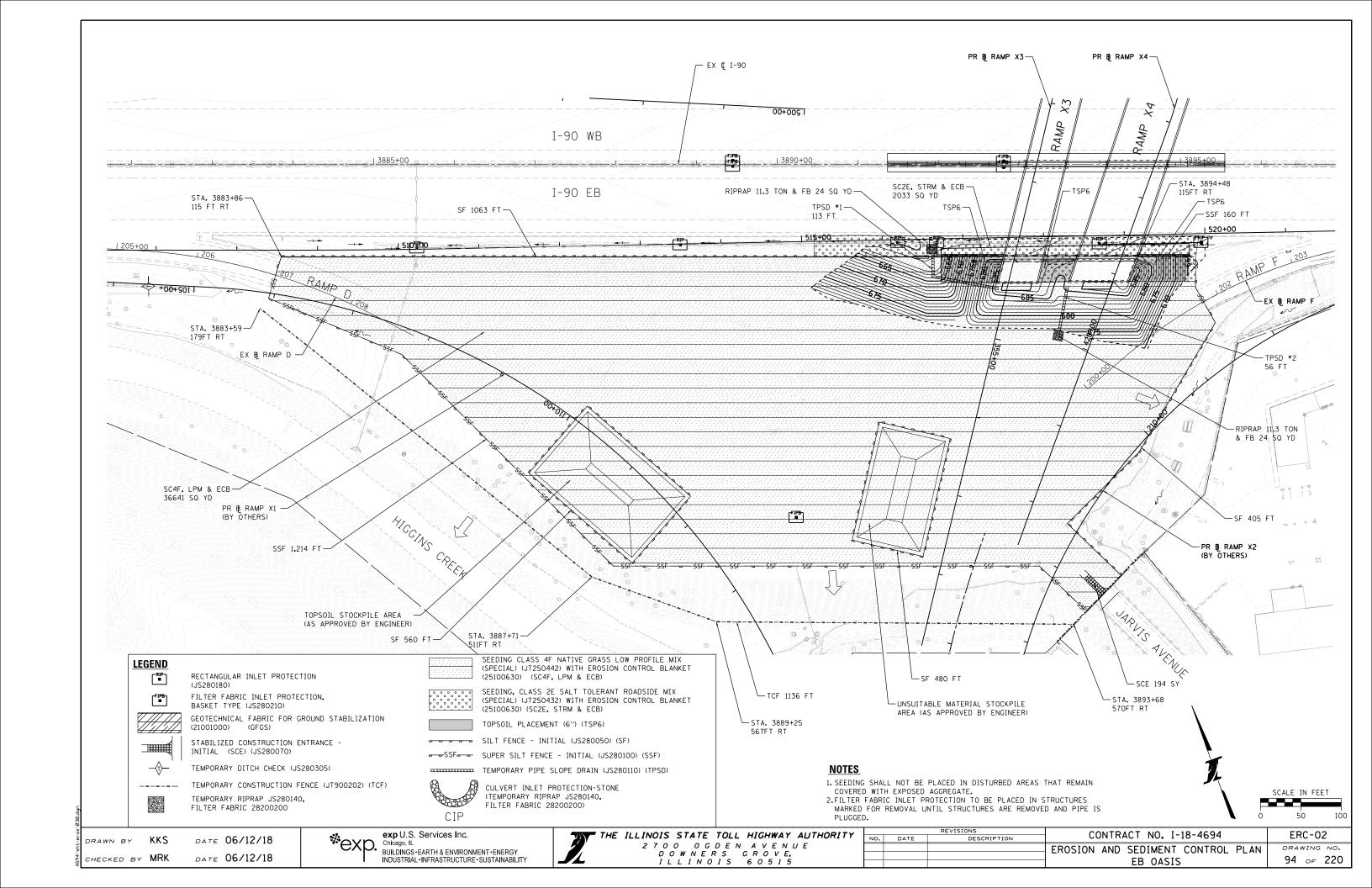
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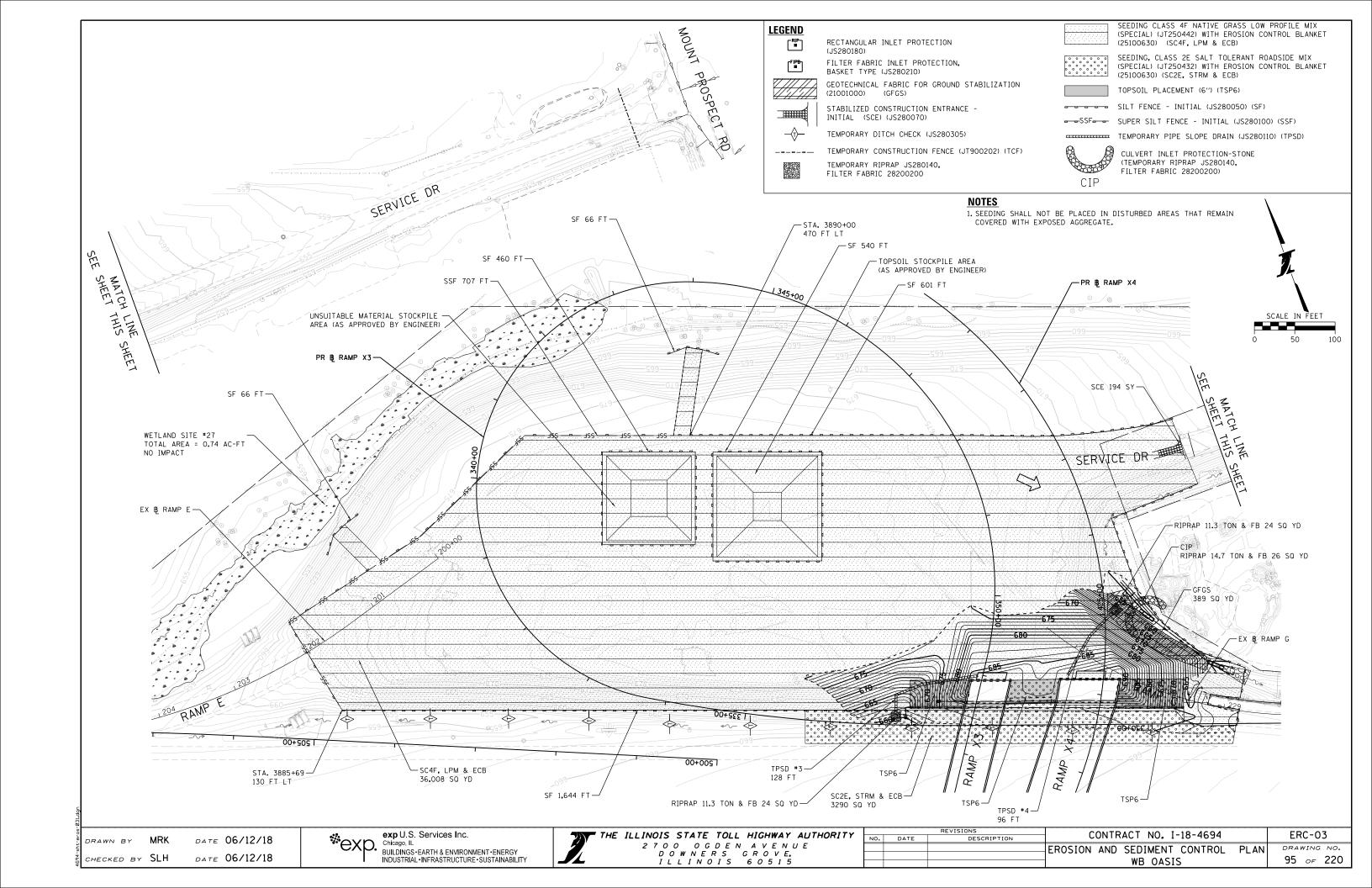
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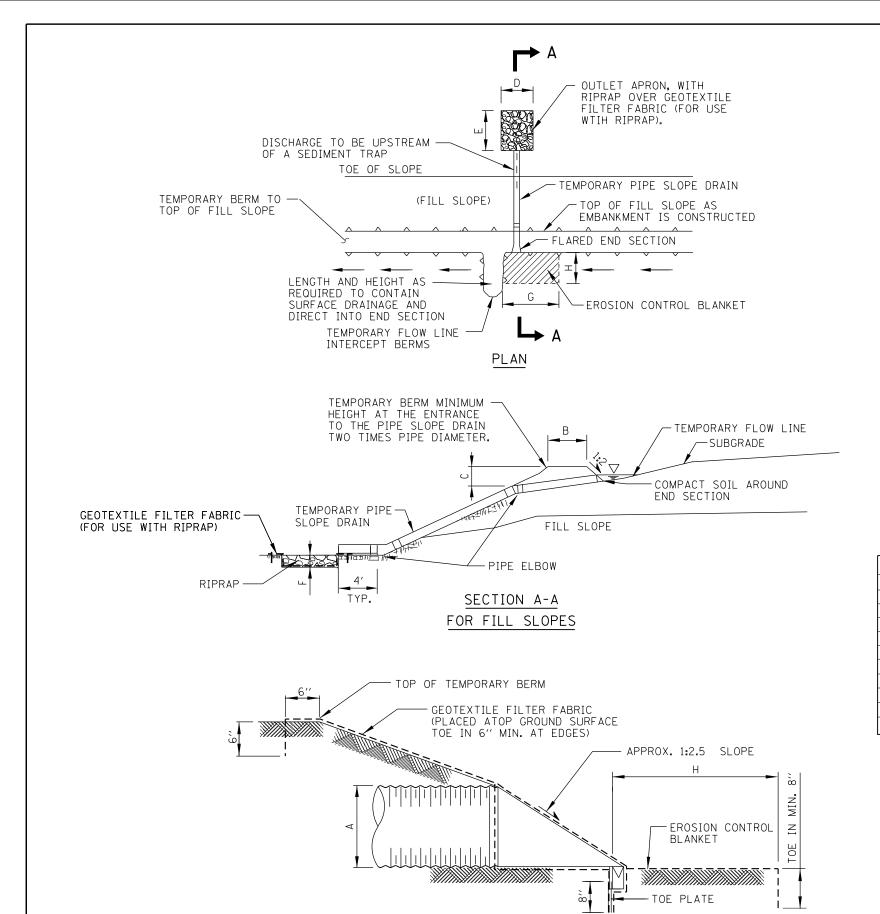
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THE ILLINOIS STATE TOLL HIGHWAY AUTHORITY 2 7 0 0 O G D E N A V E N U E DOWNERS GROVE. ILLINOIS 60515

			REVISIONS	CONTRACT NO. I-18-4694	ERC-01
	NO.	DATE	DESCRIPTION	CUNTRACT NO. 1-10-4634	ERC-01
				EROSION AND SEDIMENT CONTROL	DRAWING NO.
					07 220
				SCHEDULE AND GENERAL NOTES	93 of 220
_				SCHEDULE AND GENERAL NOTES	93 of 22







## TEMPORARY PIPE SLOPE DRAIN

#### NOTES:

- 1. ALL TEMPORARY PIPE SLOPE DRAINS TO DISCHARGE INTO THE BACK OF SEDIMENT TRAPS, INTO SEDIMENT BASINS OR DITCHES DISCHARGING INTO TRAPS OR BASINS.
- 2. GEOTEXTILE SHALL BE PLACED AROUND THE FLARE END SECTION.
- AN EROSION CONTROL BLANKET TO BE INSTALLED AT THE FLARE END SECTION EXTENDING ALONG THE TEMPORARY FLOW LINE.
- 4. TEMPORARY PIPE SLOPE DRAINS WILL BE SIZED AND SPACED ALONG THE FILL TO ADEQUATELY HANDLE THE RUNOFF FROM THE CONTRIBUTING AREA. A MINIMUM TWO TEMPORARY PIPE SLOPE DRAINS WILL BE PLACED IN EVERY SAG.
- THE PIPE SHALL BE INSTALLED WITH WATER-TIGHT CONNECTING BANDS AND SHALL BE SECURELY ANCHORED BY HOLD DOWN STAKES AND CABLES.
- 6. STAPLES SHALL BE USED TO ANCHOR GEOTEXTILE AND EROSION CONTROL BLANKET IN CONFORMANCE TO MANUFACTURER'S REQUIREMENTS.
- 7. THE OUTLET RIPRAP APRON PROTECTION SHALL BE BASED ON THE PIPE DIAMETER AND DISCHARGE VELOCITY OF STORMWATER FLOWS.
- REFERENCE DESIGN CRITERIA: ILLINOIS URBAN MANUAL AND IDOT BUREAU OF DESIGN AND ENVIRONMENTAL MANUAL.
- 9. ALL SLOPES ARE EXPRESSED AS UNITS OF VERTICAL DISPLACEMENT TO UNITS OF HORIZONTAL DISPLACEMENT (V:H).

DESIGN ELEMENTS		#1	#2	#3	#4
DRAINAGE AREA/SLOPE DRAIN	X (ACRES)	0.33	0.25	0.15	0.34
PIPE SLOPE DRAIN DIAMETER	A (INCHES)	12	12	12	12
PIPE SLOPE DRAIN SPACING	S (FEET)	-	-	-	-
BERM AT INLET TOP WIDTH	B (FEET)	4	4	4	4
BERM AT INLET HEIGHT	C (FEET)	2	2	2	2
OUTLET APRON WIDTH	D (FEET)	12	12	12	12
OUTLET APRON LENGTH	E (FEET)	12	12	12	12
OUTLET APRON DEPTH	F (FEET)	1.25	1.25	1.25	1.25
OUTLET APRON RIPRAP	GRADATION	RR-3	RR-3	RR-3	RR-3
EROSION CONTROL BLANKET LENGTH	G (FEET)	10	10	10	10
EROSION CONTROL BLANKET WIDTH	H (FEET)	5	5	5	5

KKS DATE 06/12/18 DATE 06/12/18 CHECKED BY SLH

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TYPICAL CROSS SECTION FLARED END SECTION

THE	ILLINOIS STATE TOLL HIGHWAY AUTHORITY
	2700 OGDEN AVENUE DOWNERS GROVE.
	ILLINOIS 60515

REVISIONS			CONTRACT NO. I-18-4694	ERC-04		
NO.	DATE	DESCRIPTION	CONTRACT NO. 1-10-4694	ERC-04		
			EROSION AND SEDIMENT CONTROL	DRAWING NO.		
				96 OF 220		
			DETAILS	96 of 220		

### SYMBOL LEGEND:

EXISTING TOLLWAY LIGHTING UNIT, TO REMAIN

EXISTING TOLLWAY LIGHTING UNIT, TO BE REMOVED, R-0-R

AND SALVAGED

\_\_\_

EXISTING CONDUIT EBMEDDED IN STRUCTURE

PROPOSED JUNCTION BOX, SIZE AS NOTED ON THE PLANS J

LINIT DUCT LINDERGROUND

UNDERGROUND CONDUIT WITH UNIT DUCT. CONDUIT EBMEDDED IN STRUCTURE, PVC.

SIZE AS NOTED ON THE PLANS.

PROPOSED DUAL ARM WALL MOUNTED LIGHTING UNIT, 50' M.H., 6' M.A., LED LUMINAIRE (CONTRACTOR SHALL MATCH EXISTING LUMINARE)

TEMPORARY WOOD POLE, 90 FT., CLASS 2, WITH 15 FT. MAST ARM,

65 FT. M.H., AND 750 WATTS HPS LUMINAIRE

TEMPORARY WOOD POLE, 70 FT., CLASS 3

AERIAL CABLE, 4-1/C NO. 2 WITH MESSENGER WIRE

UNDERPASS LUMINAIRE LED, WALL MOUNTED, WITH 16 FT., MINIMUM, MOUNTING HEIGHT

CONDUIT ATTACHED TO STRUCTURE, PVC COATED RGS,

SIZE AS NOTED.

## **SUMMARY OF QUANTITY**

PAY ITEM	DESCRIPTION	UNIT	ОТУ
81800400	AERIAL CABLE, 4-1/C NO. 2 WITH MESSENGER WIRE	FOOT	1690
JI811282	CONDUIT ATTACHED TO STRUCTURE, 4" DIA., STAINLESS STEEL	FOOT	25
JS810879	UNDERGROUND CONDUIT, COILABLE NONMETALLIC CONDUIT, 4" DIA.	FOOT	350
JS811051	CONDUIT ATTACHED TO STRUCTURE, 1 1/2" DIA., PVC COATED GALVANIZED STEEL	FOOT	1000
JS812023	CONDUIT EMBEDDED IN STRUCTURE, 2" DIA., PVC	FOOT	710
JS812027	CONDUIT EMBEDDED IN STRUCTURE, 4" DIA., PVC	FOOT	240
JS813001	JUNCTION BOX, STAINLESS STEEL, EMBEDDED IN STRUCTURE, 20" X 12" X 8"	EACH	2
JS813022	JUNCTION BOX, STAINLESS STEEL, ATTACHED TO STRUCTURE, 6" X 6" X 4"	EACH	11
JS813053	JUNCTION BOX, STAINLESS STEEL, ATTACHED TO STRUCTURE, 12" X 10" X 6"	EACH	12
JS813094	JUNCTION BOX, STAINLESS STEEL, ATTACHED TO STRUCTURE, 24" X 24" X 8"	EACH	1
JS816076	UNIT DUCT, WITH 4-1/C NO. 2 AND 1/C NO. 4 GROUND, 600V (XLP-TYPE USE), 2" DIA CNC	FOOT	870
JS817211	ELECTRIC CABLE IN CONDUIT, 600V (XLP-TYPE USE) 1/C NO. 10	FOOT	5000
JS821009	TEMPORARY LUMINAIRE, SODIUM VAPOR, HORIZONTAL MOUNT, 750WATT	EACH	4
JS821100	LUMINAIRE, LED, HORIZONTAL MOUNT	EACH	2
JS821110	UNDERPASS LUMINAIRE, LED	EACH	25
JS830015	WALL MOUNTED LIGHT POLE, ALUMINUM, 50 FT., TWO 6 FT. MAST ARM	EACH	1
JS830033	TEMPORARY WOOD POLE, 70 FT., CLASS 3	EACH	5
JS830043	TEMPORARY WOOD POLE, 90 FT, CLASS 2, 15 FT MAST ARM	EACH	4
JS836006	LIGHT POLE FOUNDATION (ROADWAY) MEDIAN, TYPE 2	EACH	1
JS842080	REMOVAL OF EXISTING LIGHTING UNIT, SALVAGE	EACH	2
JS846001	MAINTAN LIGHTING SYSTEM	L SUM	1

### **GENERAL NOTES:**

- 1. ELECTRICAL WORK SHALL CONFORM WITH THE NATIONAL ELECTRIC CODE AND NATIONAL ELECTRIC SAFETY CODE, AND LOCAL CODES.
- 2. THE CONTRACTOR SHALL VERIFY ALL OF THE DATA SHOWN ON THE CONTRACT PLANS AND REFERENCE DRAWINGS WHICH WILL AFFECT HIS WORK UNDER THIS CONTRACT AND THE OPERATION OF THE EXISTING ROADWAY LIGHTING AND SIGN LIGHTING SYSTEMS.
- 3. ALL ELECTRICAL MATERIALS SHALL BE NEW AND OF THE TYPE AND KINDS APPROVED BY THE ENGINEER AND OF THE FOLLOWING ORGANIZATIONS AS

NATIONAL ELECTRICAL MANUFACTURERS ASSOCIATION INSTITUTE OF ELECTRICAL AND ELECTRONICS ENGINEERS ILLUMINATING ENGINEERING SOCIETY OF NORTH AMERICA AMERICAN ASSOC. OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS U.S. DEPARTMENT OF TRANSPORTATION UNDERWRITERS LABORATORIES AMERICAN NATIONAL STANDARD INSTITUTE INSULATED CABLE ENGINEERS ASSOCIATION

4. THIS CONTRACT INCLUDES PROPOSED ROADWAY LED LUMINAIRES THE ROADWAY LUMINAIRE TABLE SHALL APPLY FOR TYPICAL

IF THE CONTRACTOR ELECTS TO FURNISH GENERAL ELECTRIC LUMINAIRES. ALL LUMINAIRES LABELED TYPE 23E1 DISTRIBUTION FILE NAME: ERS2\_23E1X40.ies

IF THE CONTRACTOR ELECTS TO FURNISH AMERICAN ELECTRIC LUMINAIRES, ALL LUMINAIRES LABELED TYPE II DISTRIBUTION, FILE NAME: ATBL\_D\_XXXXX\_N2.ies

IF THE CONTRACTOR ELECTS TO FURNISH PHILIPS LUMINAIRES. ALL LUMINAIRES SHALL BE LABELED TYPE R2M DISTRIBUTION, FILE NAME: RFL-215W96LED4K-T-R2M (S1410224m).ies

5. THIS CONTRACT INCLUDES THE PROPOSED UNDERPASS LED LUMINAIRES, THE UNDERPASS LUMINAIRE TABLE SHALL APPLY FOR TYPICAL DESIGN SPACING AND TILT ANGLE.

IF THE CONTRACTOR ELECTS TO FURNISH CREE LUMINAIRES, ALL LUMINAIRES LABELED TYPE 3M AND TYPE N6 DISTRIBUTION File Name: FLD-304-3M-\_--04-E-UH-700-40K-CONFIGURED.ies File Name: FLD-304-N6-\_\_-04-E-UL-700-40K-CONFIGURED.ies

IF THE CONTRACTOR ELECTS TO FURNISH KENALL LUMINAIRES, ALL LUMINAIRES LABELED TYPE V-VS-C DISTRIBUTION, File Name: DLD1220-108L40K-DCC-DV.ies

IF THE CONTRACTOR ELECTS TO FURNISH PHILIPS LUMINAIRES, ALL LUMINAIRES SHALL BE LABELED TYPE 4 DISTRIBUTION. FILE NAME: FX180-FNA5-R-12.ies

- 6. NO MATERIAL OR EQUIPMENT SHALL BE DELIVERED TO THE JOB SITE PRIOR TO INSPECTION AND APPROVAL BY THE TOLLWAY ENGINEER. ANY MATERIAL OR EQUIPMENT DELIVERED TO THE JOB SITE VIOLATING THIS PROCEDURE SHALL BE REMOVED AT THE CONTRACTOR'S EXPENSE.
- 7. CROSS STREETS AND HIGH MAST LIGHTING ARE NOT INCLUDED IN THIS CONTRACT.
- 8. ALL NEW CABLE DUCT, CONDUITS, JUNCTION BOXES AND APPURTENANCES ARE ILLUSTRATED DIAGRAMMATICALLY. THE ACTUAL LOCATION IN THE FIELD SHALL MEET WITH THE APPROVAL OF THE ENGINEER.

#### **TOLLWAY STANDARDS:**

### SECTION H - LIGHTING

H1-07 LIGHT STANDARD FOUNDATION LIGHT STANDARD DETAILS H2-06 BRIDGE CONDUIT DETAILS H3-05

H9-01 UNDERPASS LIGHTING INSTALLATION DETAILS

- 9. ALL SINGLE RUNS OF CABLE DUCT SHALL BE PLOWED IN, MULTI- CABLES DUCT RUNS ARE TO BE INSTALLED IN COMMON TRENCH AND BACKFILLED.
- 10. CONDUIT AND CABLE DUCT SHALL BE POSITIONED IN THE FIELD TO AVOID CONFLICTS WITH UNDERDRAINS AND OTHER EXISTING UNDERGROUND FACILITIES.
- 11. INSTALL CONDUIT EXPANSION FITTINGS AT ALL BRIDGE EXPANSION JOINT. THE COST OF THIS WORK SHALL BE INCIDENTAL TO THE CONDUIT INSTALLATION.
- 12. REMOVAL OF TEMPORARY WOOD POLES AND AERIAL CABLES INSTALLED IN THIS CONTRACT IS INCLUDED IN THE PRICE OF THE ITEM "MAINTAIN LIGHTING SYSTEM."
- 13. ALL TEMPORARY CONNECTIONS REQUIRED TO MAINTAIN OPERATION OF THE EXISTING LIGHTING SYSTEM ARE INCLUDED IN THE PRICE OF THE ITEM "MAINTAIN LIGHTING SYSTEM."

## **ROADWAY LUMINAIRE TABLE (LED)**

			PHOTOMETRIC DISTRIBUTION TYPE			
TYPICAL ROADWAY SECTION	RADIUS OF ROADWAY LOOP	LUMINAIRE PLACEMENT	NOMINAL LIGHT STANDARD SPACING	AEL	GE	PHILIPS
TYPE A	N/A	OUTSIDE SHOULDER	220′	TYPE II	TYPE 23E1	TYPE R2M
TYPE E	N/A	MEDIAN	190′	TYPE II	TYPE 23E1	TYPE R2M

## UNDERPASS LUMINAIRE TABLE (LED)

		PHOTOMETRIC DISTRIBUTION TYPE/TILT ANGLE		
TYPICAL ROADWAY	LAYOUT TYPE	CREE (Wall Mount)	KENALL	PHILIPS
TYPE E	OPPOSITE	TYPE N6 / 70° TILT	TYPE V-VS-C 70° TILT	TYPE 4 / 10° TILT

#### CABLE AND CONDUIT TAGS

- A AERIAL CABLE, 4-1/C NO. 2 WITH MESSENGER WIRE
- UNIT DUCT, WITH 4-1/C NO. 2 AND 1/C NO. 4 GROUND, 600V (XLP-TYPE USE)
- (C) 4-1/C NO. 10, 1C NO. 10 GRD IN 1 1/2"C PVC COATED RGS
- (D) 2-1/C NO. 10, 1/C NO. 10 GRD IN 1 1/2"C PVC COATED RGS
- (1) CONDUIT EMBEDDED IN STRUCTURE 4" DIA. PVC
- UNDERGROUND CONDUIT, COILABLE NONMETALIC CONDUIT, 4" DIA.
- (3) CONDUIT ATTACHED TO STRUCTURE 4" DIA. PVC COATED RGS.
- CONDUIT EMBEDDED IN STRUCTURE 2" DIA. PVC
- (5) CONDUIT ATTACHED TO STRUCTURE 4" DIA., STAINLESS STEEL.

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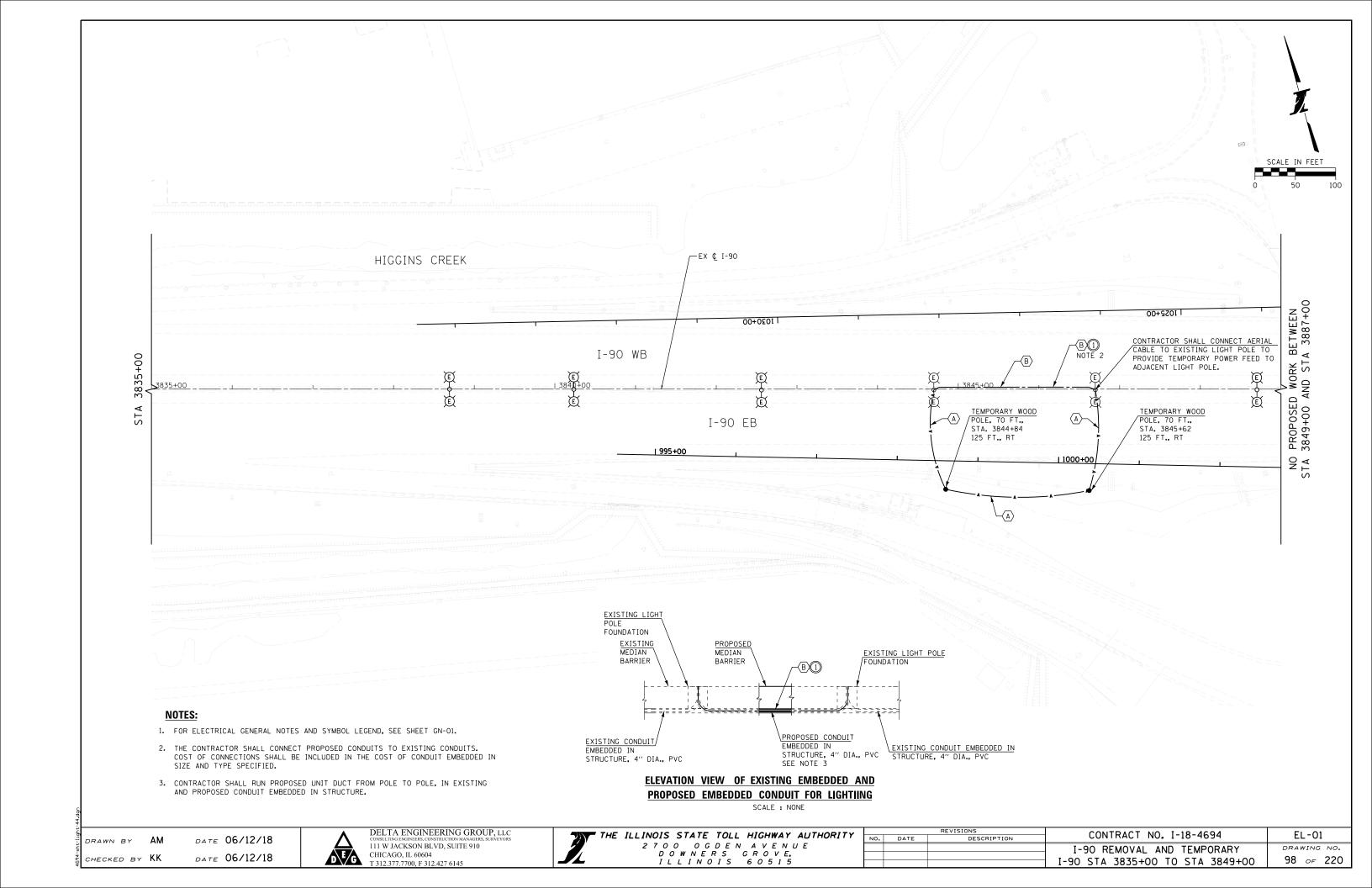


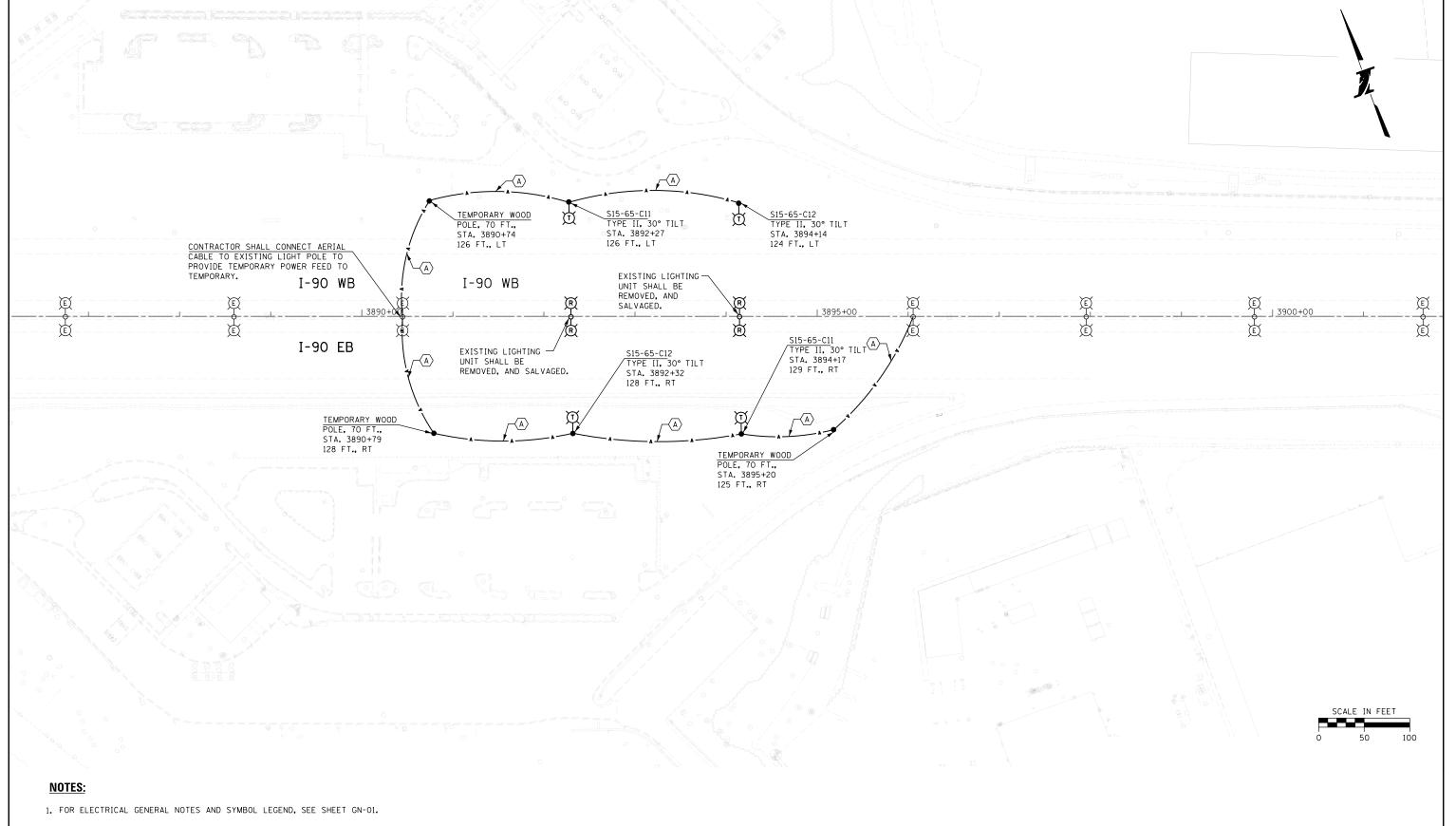
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				LEGENDS. STANDARDS. SYMBOLS	DRAWING NO.
				·	97 <sub>05</sub> 220
				AND GENERAL NOTES	91 of 220





- 2. THE COST OF THE JUNCTION BOX AND EMBEDDED CONDUIT REMOVAL SHALL BE INCLUDED IN THE UNIT PRICE OF CONCRETE MEDIAN BARRIER AND BASE REMOVAL (JI440010).
- 3. TEMPORARY LIGHTING SHALL BE INSTALLED AND MADE OPERATIONAL BEFORE REMOVING EXISTING LIGHTS FROM SERVICE. TEMPORARY LIGHTING SHALL BE REMOVED WHEN NO LONGER REQUIRED AT THE END OF STAGE 1.

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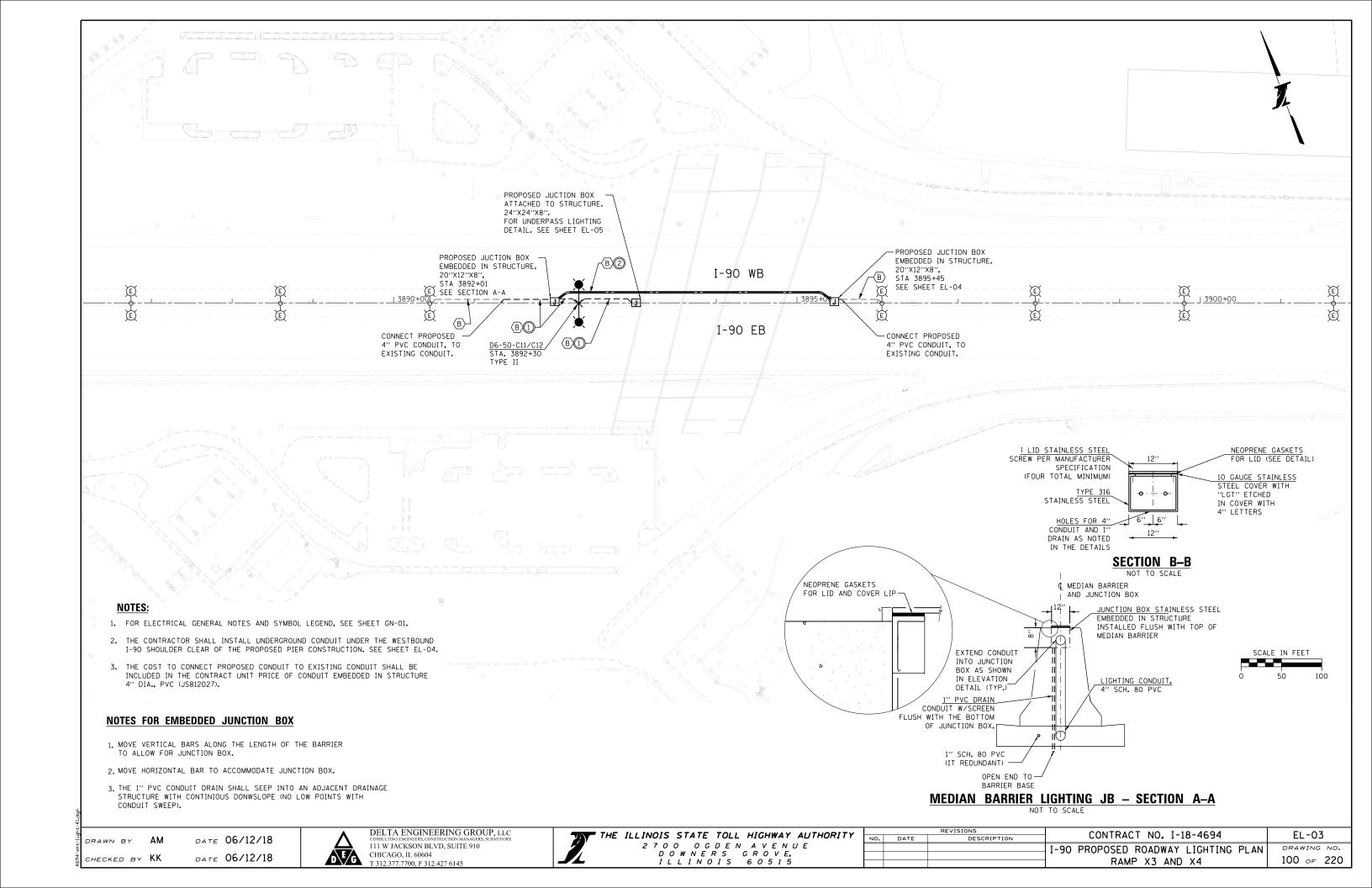
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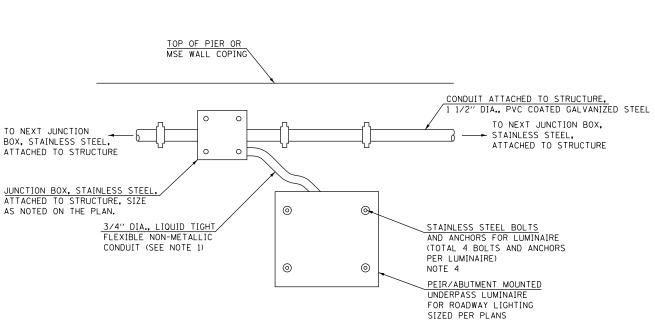
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			I-90 REMOVAL AND TEMPORARY	DRAWING NO.	
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			ROADWAY LIGHTING PLAN	99 of 220	

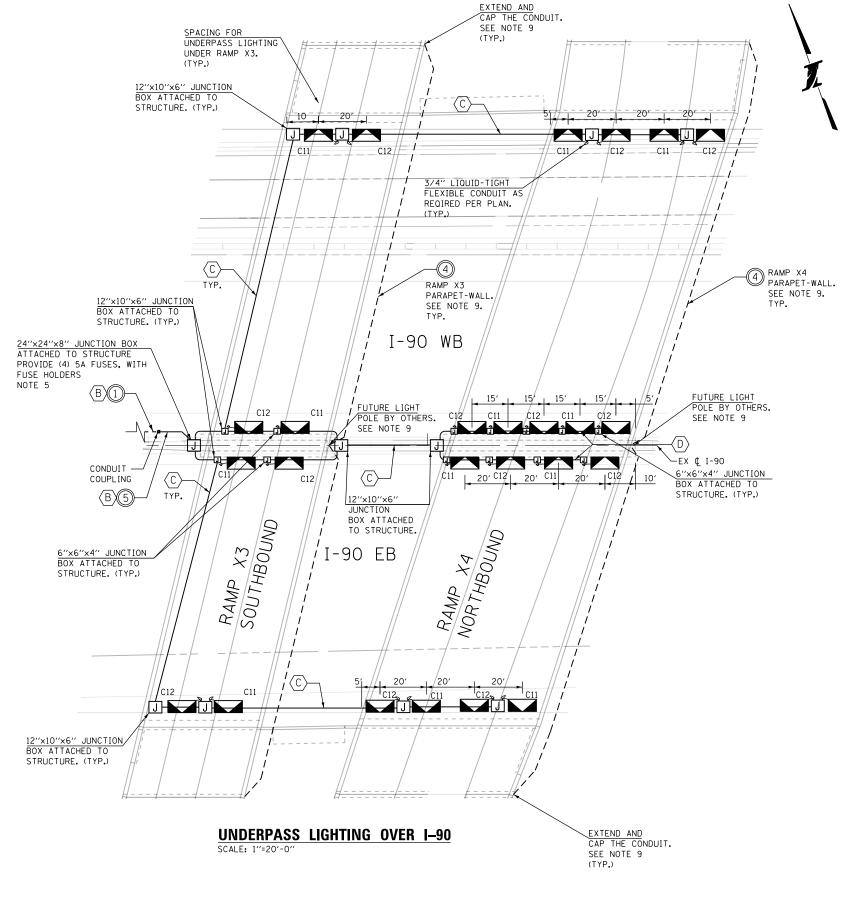




# UNDERPASS LUMINAIRE ELEVATION DETAIL

## NOTES:

- 1. FOR ELECTRICAL GENERAL NOTES AND SYMBOL LEGEND, SEE SHEET GN-01.
- THE CONTRACTOR SHALL INSTALL THE UNDERPASS LUMINAIRES ON THE PROPOSED PIERS AND MSE WALLS. LUMINAIRES ON THE BRIDGE PIERS SHALL BE OPERATIONAL BEFORE REMOVING THE TEMPORARY ROADWAY LIGHTING AT THE END OF STAGE I.
- 3. THE COST OF THE LUMINAIRE ASSEMBLY COMPLETE, INCLUDING ANCHORS AND ALL APPLICABLE HARDWARE, SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE OF UNDERPASS LUMINAIRE, LED (JS821110).
- 4. THE CONTRACTOR SHALL FURNISH AND INSTALL 3/8" DIA., ADHESIVE ANCHORS WITH 2-3/8" MINIMUM EMBEDMENT (HILITI HIT HY-200 OR EQUIVALENT) AS APPROVED BY THE ENGINEER.
- 5. FURNISH AND INSTALL FOUR (4) 5A FUSES WITH FUSE HOLDERS PER TOLLWAY STANDARD H9-01. THE CONTRACTOR SHALL VERIFY THE FUSE SIZE WITH THE LED FIXTURE MANUFACTURE. THE COST OF THE FUSES AND HOLDERS SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE OF JUNCTION BOX, STAINLESS STEEL, ATTACHED TO STRUCTURE, 24"X 24"X 8"(JS813094).
- 6. CONDUITS AND JUNCTION BOXES SHALL BE MOUNTED OFFSET FROM THE STRUCTURE BY MEANS OF STAINLESS STEEL C-CHANNELS. JUNCTION BOXES SHALL BE INSTALLED IN A MANNER SUCH THAT THE BOXES OPEN TOWARDS THE ROADWAY.
- 7. FURNISHING AND INSTALLING LIQUID TIGHT METALLIC CONDUITS AND \*10 AWG WIRING SHALL BE INCLUDED IN THE UNIT PRICE FOR UNDERPASS LUMINAIRE, LED (JS821110).
- 8. FOR UNDERPASS LIGHTING INSTALLATION, SEE ILLINOIS TOLLWAY STANDARD H9.
- 9. THE CONTRACTOR SHALL THREAD AND CAP THE EXPOSED ENDS OF THE EMBEDDED CONDUIT AT FUTURE LIGHT POLE LOCATIONS AND AT THE ENDS OF THE BARRIER WALL ON THE BRIDGE APPROACH PAVEMENTS. THE CONDUIT SHALL EXTEND A MINIMUM OF 7" BEYOND THE CONCRETE LIMITS. THE COST OF THIS WORK SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE OF CONDUIT EMBEDDED IN STRUCTURE, 2" DIA., PVC (JS812023).
- 10. CONTRACTOR SHALL INSTALL CONDUIT PLUGS IN JUNCTION BOXES TO PREVENT WATER/DIRT ENTRY PRIOR TO INSTALLATION. PULL TAPE SHALL BE ACCESSIBLE AND SECURED INSIDE JUNCTION BOXES.



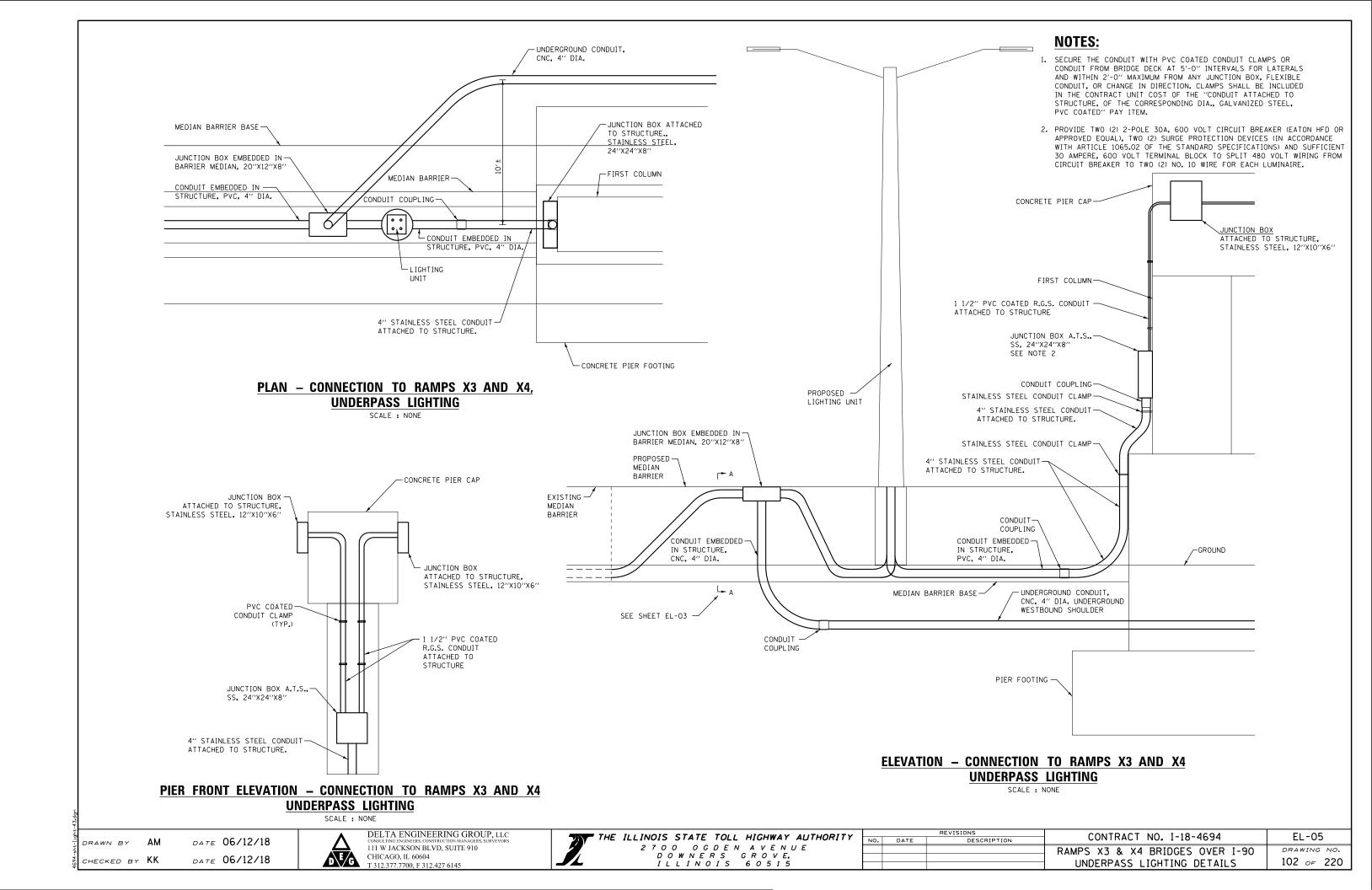
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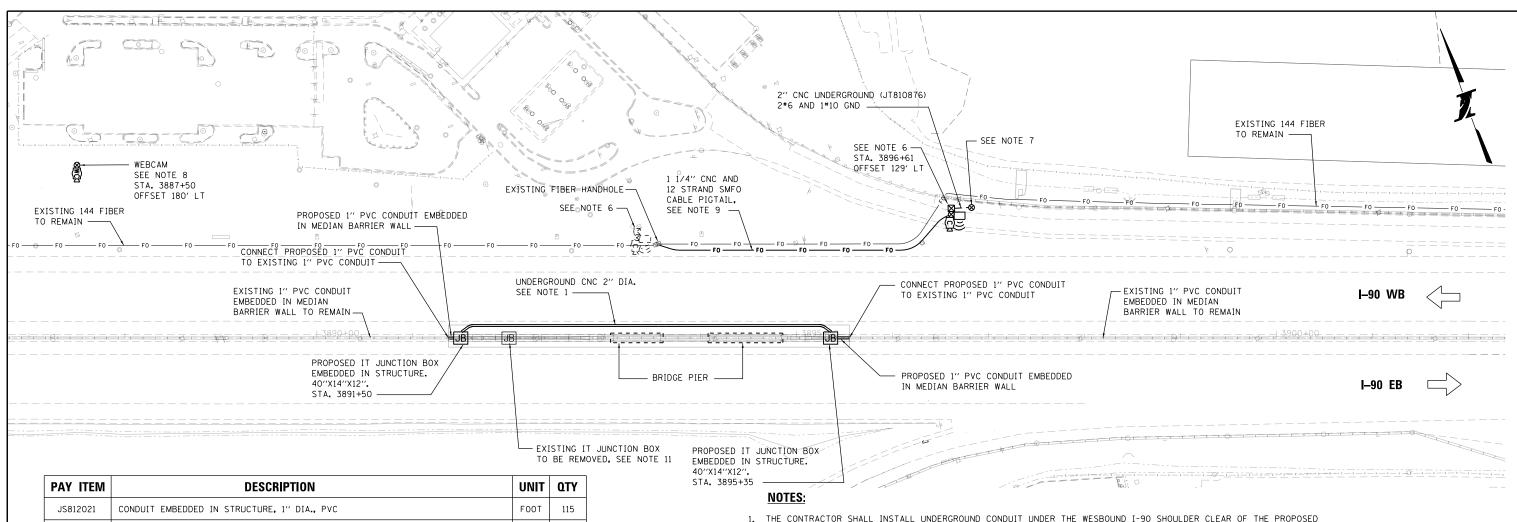


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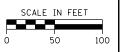
REVISIONS		REVISIONS	CONTRACT NO. I-18-4694	EL 04
	DATE	DESCRIPTION	CUNTRACT NO. 1-10-4634	EL-04
			RAMPS X3 & X4 BRIDGES OVER I-90	DRAWING NO.
				101 OF 220
			UNDERPASS LIGHTING PLAN	101 0+ 220





JUNCTION BOX, STAINLESS STEEL, EMBEDDED IN STRUCTURE, 40" X 14" X 12" JS813014 EACH ELECTRIC CABLE IN CONDUIT, 600V (XLP-TYPE USE) 1/C NO. 10 EACH 60 JS817211 JS817213 ELECTRIC CABLE IN CONDUIT, 600V (XLP-TYPE USE) 1/C NO. 6 EACH 120 JS830033 TEMPORARY WOOD POLE, 70 FT., CLASS 3 FACH JT130700 SOLAR POWERED GENERATOR ASSEMBLY EACH JT130714 REAIMING MVDS UNITS EACH JT132830 FIBER OPTIC COMMUNICATIONS. ITS ASSEMBLY EACH MAINTAIN INTELLIGENT TRANSPORTATION SYSTEMS JT134000 I SUM JT134005 RELOCATE INTELLIGENT TRANSPORTATION SYSTEM ASSEMBLY EACH JT134037 ITS ELEMENT SITE GROUNDING - POLE MOUNTED ASSEMBLY EACH ITS ELEMENT SITE GROUNDING - SOLAR POWERED GENERATOR ASSEMBLY JT134039 EACH JT135042 EACH JT154062 CONTRACT ALLOWANCE FOR MAINTAIN INTELLIGENT TRANSPORTATION SYSTEM REPAIR UNIT 35,000 JT154112 ALLOWANCE FOR ADDITIONAL ELECTRICAL AND COMMUNICATIONS WORK UNIT 40,000 JT160225 SINGLE MODE FIBER OPTIC CABLE REMOVAL. SALVAGE FOOT 26,400 JT160360 FIBER OPTIC CABLE, SINGLE MODE NON-ARMORED, 36 FIBERS FOOT 26,400 JT810873 UNDERGROUND CONDUIT, COILABLE NONMETALLIC CONDUIT, SDR 11, 1 1/4" DIA. FOOT 320 JT810876 UNDERGROUND CONDUIT, COILABLE NONMETALLIC CONDUIT, SDR 11, 2" DIA. FOOT 440 ITS ELEMENT POLE FOUNDATION STEEL HELIX (10 FT) EACH JT836018

- 1. THE CONTRACTOR SHALL INSTALL UNDERGROUND CONDUIT UNDER THE WESBOUND I-90 SHOULDER CLEAR OF THE PROPOSED PIER CONSTRUCTION. SEE SHEET ITS-02.
- 2. THE EXISTING WESTERN SPLICE FOR THE 36 SINGLE MODE FIBER OPTIC (SMFO) CABLE IS LOCATED WEST OF BUSSE ROAD IN A JUNCTION BOX EMBEDDED IN THE I-90 MEDIAN BARRIER WALL NEAR STATION 3790+00. THE CONTRACTOR SHALL CONFIRM THE SPLICE LOCATION PRIOR TO STARTING WORK, OTHERS (THE ILLINOIS TOLLWAY'S FIBER MAINTENANCE MANAGER) WILL DISCONNECT THE EXISTING SPLICE.
- 3. THE EXISTING EASTERN SPLICE FOR THE 36 SINGLE MODE FIBER OPTIC (SMFO) CABLE IS LOCATED INSIDE PLAZA 17 (DEVON PLAZA). THE CONTRACTOR SHALL CONFIRM THE SPLICE LOCATION PRIOR TO STARTING WORK, OTHERS (THE ILLINOIS TOLLWAY'S FIBER MAINTENANCE MANAGER) WILL DISCONNECT THE EXISTING SPLICE.
- 4. AFTER THE 36 SMFO CABLE IS DISCONNECTED, THE CONTRACTOR SHALL REMOVE AND SPOOL THE EXISTING CABLE CONTRACTOR SHALL PERFORM OTDR TEST ON THE REMOVED 36 SMFO CABLE BEFORE DELIVERING IT TO THE ILLINOIS TOLLWAY'S FIBER MANAGEMENT COMPANY.
- 5. AFTER THE MEDIAN BARRIER WALL AND UNDERGROUND CONDUIT CONSTRUCTION IS COMPLETE, THE CONTRACTOR SHALL INSTALL THE NEW 36 SMFO CABLE INTO THE MEDIAN BARRIER WALL AND UNDERGROUND CONDUITS FROM PLAZA 17 (DEVON PLAZA) TO JUNCTION BOX EMBEDDED IN THE I-90 MEDIAN BARRIER WALL NEAR STATION 3790+00. THE CONTRACTOR SHALL COIL 50' OF SLACK IN EACH JUNCTION BOX EMBEDDED IN THE MEDIAN BARRIER WALL. OTHERS (THE TOLLWAY'S FIBER MANAGEMENT COMPANY) WILL SPLICE AT THE EXISTING JUNCTION BOXES NEAR STATION 3790+00 AND AT PLAZA 17 (DEVON PLAZA).
- 6. THE CONTRACTOR SHALL REMOVE AND REINSTALL THE EXISTING ITS CAMERA, MVDS, AND ITS CABINET ON AN ITS ELEMENT POLE FOUNDATION STEEL HELIX (10 FT) (JT836018) WITH AN ITS ELEMENT SITE GROUNDING POLE MOUNTED ASSEMBLY (JT134037). THE CONTRACTOR SHALL REMOVE THE EXISTING FOUNDATION.
- 7. THE CONTRACTOR SHALL LOCATE THE GROUND MOUNTED SOLAR POWERED GENERATOR ASSEMBLY (JT130700) WITHIN 20' OF THE POLE MOUNTED ITS SYSTEM AND ENUSRE THAT THE SOLAR PANELS HAVE UNOBSTRUCTED SUN EXPOSURE.
- 8. THE CONTRACTOR SHALL INSTALL THE WEBCAM (JT135042) ON THE TEMPORARY WOOD POLE, 70 FT., CLASS 3 (JS830033). THE WEBCAM SHALL BE INSTALLED AND MADE OPERATIONAL WITHIN THREE (3) WEEKS FROM NOTICE TO PROCEED (NTP). ON COMPLETION OF THE CONTRACT, THE WOOD POLE AND THE WEBCAM SHALL REMAIN IN PLACE AND BECOME PROPERTY OF THE ILLINOIS TOLLWAY.
- 9. THE COST OF FURNISHING AND INSTALLING THE 12 STRAND SMFO CABLE, PIGTAIL SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE FIBER OPTIC COMMUNICATIONS. ITS ASSEMBLY (JT132830)
- 10. THE CONTRACTOR SHALL NOTIFY THE ENGINEER 14 DAYS PRIOR TO WHEN SPLICING WORK (DISCONNECTING OR CONNECTING) IS REQUESTED TO BE PERFORMED. THE ENGINEER WILL CONTACT THE ILLINOIS TOLLWAYS FIBER MAINTENANCE MANAGER TO SCHEDULE SPLICING WORK.
- 11. THE COST OF THE JUNCTION BOX AND EMBEDDED CONDUIT REMOVAL SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE OF CONCRETE MEDIAN BARRIER AND BASE REMOVAL (J1440010).



DRAWN BY AM DATE 06/12/18

ITS CONCRETE SERVICE PAD, TYPE A

DATE 06/12/18

JT836027

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DELTA ENGINEERING GROUP, LLC
CONSULTING ENGINEERS, CONSTRUCTION MANAGERS, SURVEYORS
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EACH

THE ILLINOIS STATE TOLL HIGHWAY AUTHORITY

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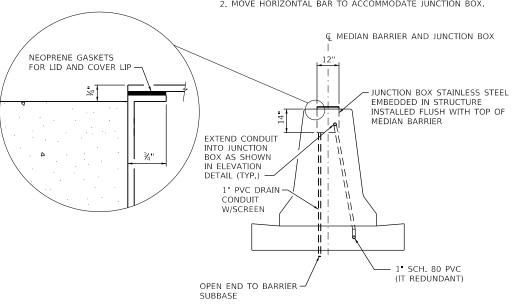
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REVISIONS CONTRACT NO. I-18-4694 I	
	ITS-01
NO. DATE DESCRIPTION CONTINACT NO. 1 TO NO.	2-01
I-90 ITS DEVICE / MEDIAN FIBER	WING NO.
	of 220
RELOCATION PLAN 103	of <b>220</b>

#### **NOTES:** JUNCTION BOX, STAINLESS STEEL, JUNCTION BOX, STAINLESS STEEL, 1. THE COST OF THIS WORK SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE OF JUNCTION BOX, STAINLESS EMBEDDED IN STRUCTURE, EMBEDDED IN STRUCTURE, 40" X 14" X 12" — 40" X 14" X 12" -STEEL, EMBEDDED IN STRUCTURE, 40" x 14" x 12" (JS813014). EXISTING -PROPOSED -MEDIAN 2. CONTRACTOR SHALL INSTALL CONDUIT PLUGS IN IT JUNCTION BOXES TO PREVENT WATER/DIRT ENTRY PRIOR TO INSTALLATION. MEDIAN BARRIER BARRIER PULL TAPE SHALL BE SECURED AND ACCESSIBLE INSIDE JUNCTION EXTEND CONDUIT INTO BASE OF JUNCTION BOX MIN. 3" (TYP.) ------±---t-------EXISTING CONDUIT EMBEDDED IN EXISTING CONDUIT EMBEDDED IN STRUCTURE, 1" DIA., PVC - STRUCTURE, 1" DIA., PVC FOR SMFO CABLE CONDUIT EMBEDDED IN CONDUIT EMBEDDED IN STRUCTURE, 1" DIA., PVC STRUCTURE, 1" DIA., PVC UNDERGROUND CONDUIT, FOR SMFO CABLE FOR SMFO CABLE COILABLE NONMETALLIC CONDUIT, SDR 11, 2" DIA. 1" PVC CONDUIT WITH UNDER WESTBOUND SHOULDER SCREEN (DRAIN) FOR SMFO CABLE SEE NOTE 1 ↓ LID STAINLESS STEEL SCREW PER MANUFACTURER SPECIFICATION (FOUR TOTAL MINIMUM) - NEOPRENE GASKETS FOR LID (SEE DETAIL) **ELEVATION VIEW OF EMBEDDED AND** 10 GAUGE STAINLESS STEEL COVER WITH "IT" ETCHED UNDERGROUND CONDUIT FOR SMFO CABLE IN COVER WITH 4" LETTERS SCALE : NONE TYPE 316 STAINLESS STEEL -HOLES FOR CONDUIT AND 1" DRAIN AS NOTED IN THE DETAILS 12" **JUNCTION BOX** NOT TO SCALE

#### NOTES FOR EMBEDDED JUNCTION BOX

- 1. MOVE VERTICAL BARS ALONG THE LENGTH OF THE BARRIER TO ALLOW FOR JUNCTION BOX.
- 2. MOVE HORIZONTAL BAR TO ACCOMMODATE JUNCTION BOX.



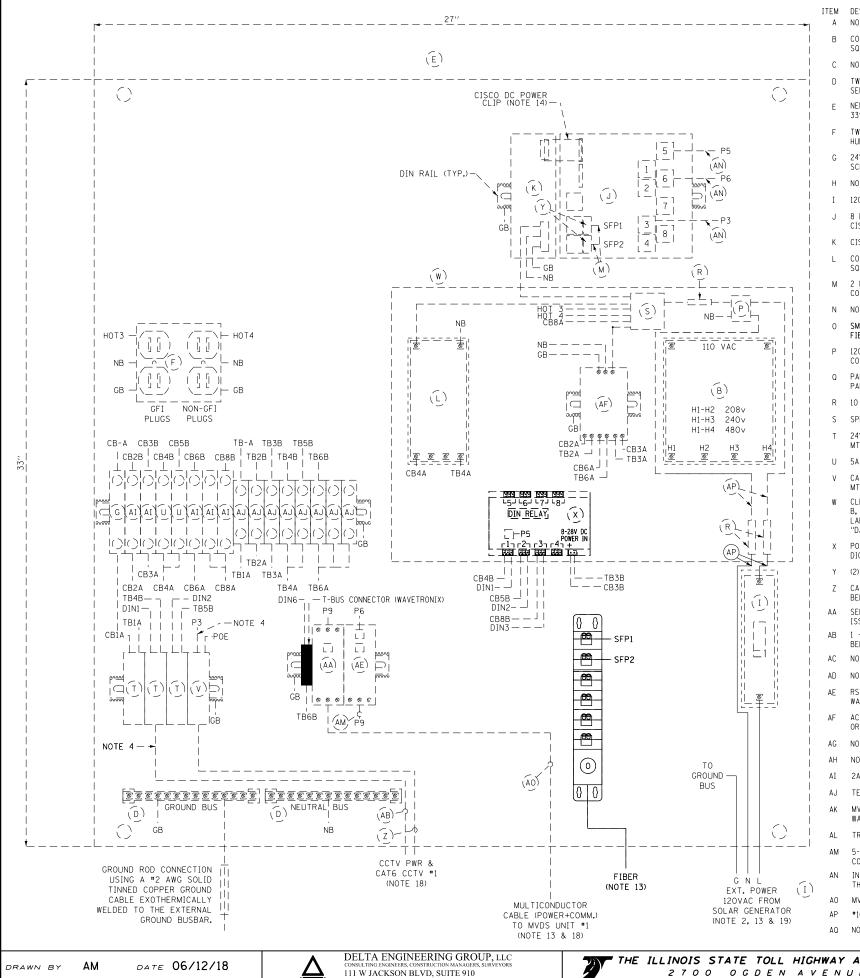
## MEDIAN BARRIER AT IT DATA JB - SECTION A-A

AM DATE 06/12/18 DRAWN BY DATE 06/12/18 CHECKED BY KK

DELTA ENGINEERING GROUP, LLC 111 W JACKSON BLVD, SUITE 910 CHICAGO, IL 60604 T 312.377.7700, F 312.427 6145

THE ILLINOIS STATE TOLL HIGHWAY AUTHORITY 2700 OGDEN AVENUE DOWNERS GROVE, ILLINOIS 60515

REVISIONS		REVISIONS	CONTRACT NO. I-18-4694	ITS-02
NO.	DATE	DESCRIPTION	CONTRACT NO. 1-10-4634	113-02
			ELEVATION PLAN FOR	DRAWING NO.
			MEDIAN FIBER CONDUIT	104 of 220
			MEDIAN FIDER CONDUIT	101 07 220



CHICAGO, IL 60604

T 312.377.7700, F 312.427 614

CHECKED BY KK

DATE 06/12/18

- ITEM DESCRIPTION
  - NOT USED FOR THIS SHEET APPLICATION
  - CONTROL POWER TRANSFORMER, 1000VA, 208/240/480-120VAC, 1PH SOUARE D/CLASS 9070 - T1000 D95
  - NOT USED FOR THIS SHEET APPLICATION
  - TWO (2) GROUNDING BAR SYSTEM HOFFMAN/PGS2K. BONDED OR SEPARATED AS REQUIRED.
  - NEMA 4X STAINLESS STEEL, 36"H X 30"W X 12"D ENCLOSURE WITH 33"X27" PANEL, HOFFMAN/A36H3012SS6LP & A36P30
  - TWO DUPLEX 120V RECEPTACLES, ONE GFCI AND NON-GFI (SEE NOTE 9) HUBBELL/GFR5362 & BR20WR
  - 24VDC, 1P, 15A CIRCUIT BREAKER SCHNEIDER ELECTRIC/MGN61510
  - NOT USED FOR THIS SHEET APPLICATION
  - 120VAC, 1P, 30A CIRCUIT BREAKER WITH TERMINAL SHIELD
  - 8 ELECTRICAL PORT AND TWO FOC PORT SWITCH CISCO MODEL CISCO/IE-3000-8TC-E
  - CISCO POWER SUPPLY, CISCO/PWR-IE-3000-AC=
  - CONTROL POWER TRANSFORMER, 250VA, 120-24VAC, 1PH SQUARE D/CLASS 9070-T250D13
  - 2 METER SMFO LC-LC DUPLEX JUMPERS, CORNING/040402R5Z20002M
  - NOT USED FOR THIS SHEET APPLICATION
  - SMF PATCH PANEL WITH LC CONNECTORS FIBER CONNECTIONS G620U012LAN-200-0
  - 120VAC SURGE SUPPRESSOR, MOUNTED ON DIN RAIL
  - COOPER CROUSE HINDS/MA15/D/1/SI OR APPROVED EQUAL PANDUIT WIRING DUCT (OR EQUIVALENT)
  - PANDUIT/FIX1LG6 WITH COVER-C1LG6
  - 10 AMP FUSE, GOULD (MERSEN)/ATM-10
  - SPLICE BLOCK, ALTECH/38041
  - 24VAC/VDC SURGE SUPPRESSOR, MOUNTED ON DIN RAIL MTL INSTRUMENTS/ZB24580
  - 5A CIRCUIT BREAKER, ALLEN BRADLEY/1492-SPM1B050
  - CAT6 PoE+ SURGE SUPRESSOR, MOUNTED ON COMMON DIN RAIL MTL INSTRUMENTS/ZB24597 OR APPROVED EQUAL
  - CLEAR PLEXIGLASS SAFETY COVER ENCOMPASSING ITEMS L, R, S, B, P, X & AF. (THE INSTALLER SHALL PERMANENTLY AFFIX A LABEL STATING "DANGER 480 VAC" OR "DANGER 240 VAC" OR "DANGER 120 VAC" FOR 120 VAC AS FIELD CONDITIONS WARRANT.)
  - POWER CONTROLLER, 8-CHANNEL DIN ETHERNET RELAY DIGITAL LOGGERS/DIN 4
  - (2) CISCO GLC-LX-SM-RGD = 1 GBPS SM SFP MODULES
  - CATEGORY 6 CABLE, 23AWG, OUTDOOR RATED CABLE
  - SENSOR SURGE SUPPRESSION, WAVETRONIX CLICK-200 OR ISS ZONE BARRIER ZB 24510
  - 1 3/C \*16 CCTV POWER CABLE, OUTDOOR RATED CABLE BELDEN/1034A OR APPROVED EQUAL
  - NOT USED FOR THIS SHEET APPLICATION
  - NOT USED FOR THIS SHEET APPLICATION
  - RS-232 / RS-485 TO ETHERNET CONVERTOR WAVETRONIX - CLICK-301 OR ISS-MOXA P5150A, OK-35A
  - AC/DC POWER SUPPLY, 24VDC WAVETRONIX CLICK-204 OR ISS LAMBDA DSP100-24
  - NOT USED FOR THIS SHEET APPLICATION
  - NOT USED FOR THIS SHEET APPLICATION
  - 2A CIRCUIT BREAKER, ALLEN BRADLEY/1492-SPM1B020
  - TERMINAL BLOCK, ALLEN BRADLEY/1492-CD8
  - MVDS ASSEMBLY (NOT SHOWN), SEE SPECIAL PROVISIONS WAVETRONIX (SMART SENSOR HDSS-126) OR ISS (SX-300)
  - TRANSFORMER COVERS, SQUARE D/9070FSC2
  - 5-CONDUCTOR JUMPER (Tx, Rx, GND, RTS, CTS), RS-232 SERIAL COMMUNICATIONS (APPLICABLE TO ISS/MOXA)
  - INDOOR/OUTDOOR RATED CAT6 (1000MBS, TEMPERATURE HARDENED) THESE ARE THE CAT6 CABLES ROUTED INSIDE CABINET
  - AO MVDS CABLE, WAVETRONIX WX-SS-706-60 OR ISS G4-CBL-60

  - AO NOT USED FOR THIS SHEET APPLICATION

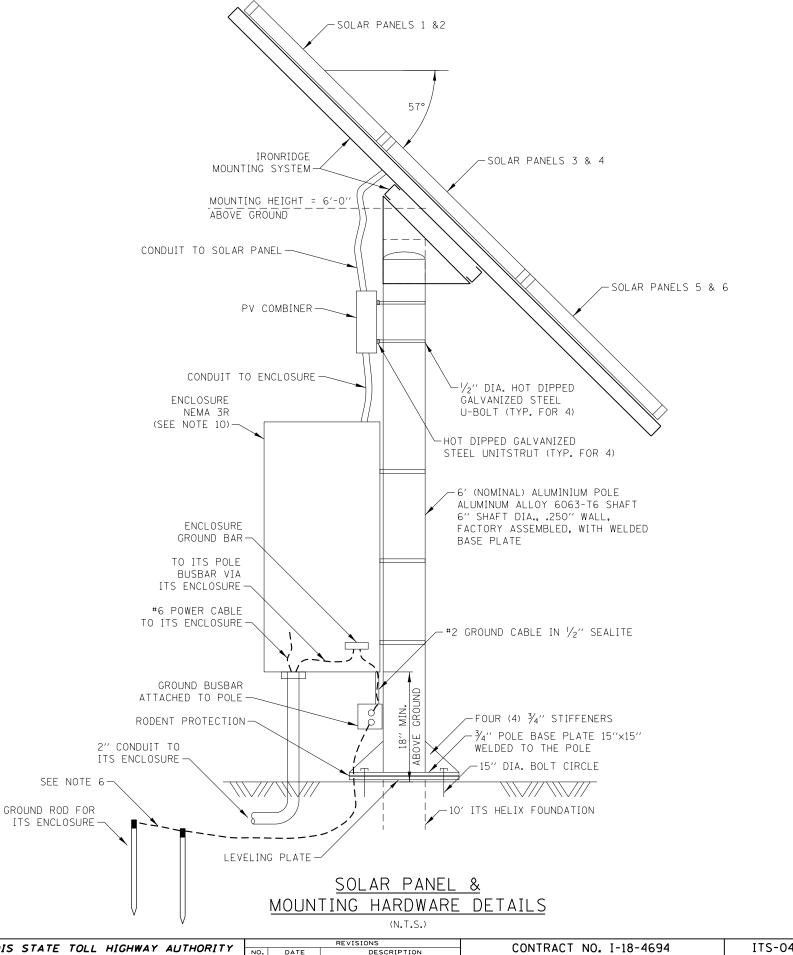
- NOTES:
- 1. ALL POWER WIRING SHALL BE RHH/RHW WITH WIRE TERMINALS OR TINNED.
- 2. CONTRACTOR TO VERIFY CORRECT TRANSFORMER TAPS ARE USED BASED ON INCOMING
- 3. ALL CABLES AND EQUIPMENT SHALL BE PROPERLY DRESSED AND LABELED. ALL CONDUITS SHALL BE PROPERLY PLUGGED WITH DUCT SEAL PUTTY (RAINBOW
- 4. SHEET SHOWS BOTH 24VAC AND POE OPTIONS. CONNECTIONS REQUIRED FOR 24VAC OPTION ONLY ARE DENOTED WITH A DASHED LINE.
- 5. EACH 120VAC OUTLET, PS OR TRANSFORMER (ITEM F, K, L, & AF) SHALL BE FED FROM A SEPARATE INPUT LINE.
- 6. MOUNT ITEMS J & K ON A 15 INCH CONTINUOUS SECTION OF DIN RAIL. THE DIN RAIL SHALL BE INSTALLED WITH THE CENTER LINE NO LESS THAN 5 INCHES FROM ANY OBSTACLE ABOVE AND NO LESS THAN 4 INCHES FROM ANY OBSTACLE BELOW. ALL DIN RAIL SHALL BE GROUNDED.
- 7. ALL CABLES INSTALLED WITHIN THE CABINET AND POLE SHALL BE OUTDOOR RATED.
- 8. WIFI COMMUNCATION SHALL BE DISABLED ON DIN ETHERNET RELAY.
- 9. THE GFI OUTLETS LOAD SHALL NOT BE CONNECTED TO ANY OTHER LOAD IN THE ENCLOSURE. THE 1900 QUAD BOX GFI'S ARE INTENDED TO BE UTILIZED FOR EXTERNAL EQUIPMENT ONLY. EACH OUTLETS TAB SHALL BE BROKEN SO THEY ARE INDEPENDENT.
- 10. ALL BREAKERS SHALL BE LABELED (e.g. CAMERA-AC, CAMERA-DC, DIN RELAY-AC, DIN RELAY-DC, CELL MODEM-AC ETC.).
- 11. THE GROUND WIRE IN THE 3/C #16 CCTV POWER CABLE SHALL BE TAPED GREEN.
- 12. USE THE MOUNTING TABS ON THE IP RELAY UNIT TO MOUNT THE UNIT DIRECTLY TO THE BACK PLATE. REFER TO THE IP RELAY WIRING TABLE FOR WIRING DETAILS.
- 13. ALL CABLES SHALL ENTER THE ENCLOSURE FROM THE BOTTOM.
- 14. POWER FEED TO THE CISCO IE3000 SWITCH SHALL BE FROM THE 120VAC INPUT WHEN THE ENCLOSURE IS AC POWERED.
- 15. NOT USED FOR THIS SHEET APPLICATION
- 16. IF A SOLAR GENERATOR IS CONNECTED, THEN ITEM P AND THE SECONDARY SIDE OF ITEM B SHALL BE CONNECTED UNTIL A FINAL AC CONNECTION IS MADE.
- 17. ITEM X IS USED TO CONTROL POWER TO THE CAMERAS AND DETECTORS. ALL 120VAC CONNECTIONS ON ITEM X SHALL BE PROTECTED.
- 18. CABLES TO BE ROUTED THROUGH POLE.
- 19. WHEN A 24VDC TO 120VAC POWER GENERATOR IS CONNECTED, THEN THE 480VAC TO 120VAC STEP DOWN TRANSFORMER IS BYPASSED.
- 20. NOT USED FOR THIS SHEET APPLICATION
- 21. CUT AND STRIP MANUFACTURER-SUPPLIED POWER CORD AS REQUIRED TO MAKE TERMINATIONS.
- 22. DIN RAIL SHALL BE INSTALLED AS ILLUSTRATED ON DRAWING. DIN RAIL SHALL BE GROUNDED TO THE GROUND BUS.
- 23. TIE THE ENCLOSURE INTO THE GROUND BUS.
- 24. ITEM W SHALL BE FORMED AND MOLDED TO FIT AROUND THE AREA DENOTED BY THE DASHED LINE. THE PLEXIGLASS SHALL BE MOUNTED TO THE BACKPLATE WITH SUFFICIENT AIR HOLES TO ALLOW HEAT TO ESCAPE THE AREA. THERE SHALL ALSO BE OPENINGS ON THE BOTTOM TO ALLOW CABLES TO BE PASSED FROM THE AC SECTION TO THE OTHER SECTIONS OF THE

ITS-03

- 25. ITEM AL SHALL BE PLACED ON ITEMS B AND L.
- 26. ALL INTERNAL ENCLOSURE ROUTED AND TERMINATED CAT6 CABLE SHALL BE
- 27. ALL INTERNAL 24VAC, 120VAC (STARTING ON SECONDARY SIDE OF ITEM B) AND ANY DC VOLTAGE POWER FEEDS USE \*16 AWG CABLE.
- 28. ITEM SHOWN IN DASHED LINE ARE EXISTING EQUIPMENT AND SYSTEM COMPONENTS RELOCATED FROM EXISTING CCTV+MVDS INSTALLATION AT 3893+38, 112' LT.

## NOTES:

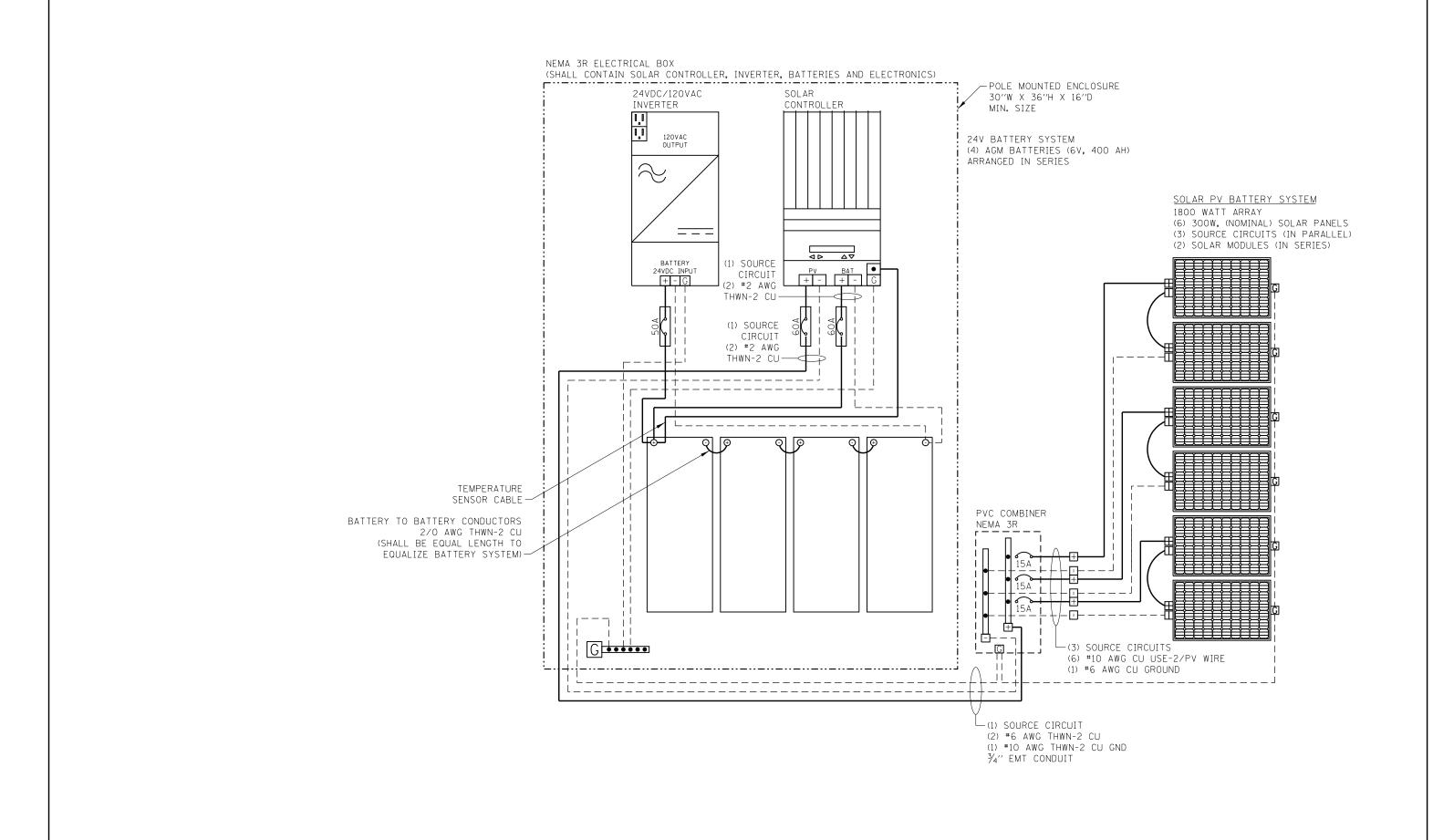
- 1. SOLAR POWER GENERATOR TO INCLUDE PANEL, BRACKETS, CABINET, CHARGER REGULATOR, BATTERIES, AND CABLES. STRUCTURE TO BE DESIGNED TO MEET STRUCTURAL DESIGN CRITERIA IN SPECIFICATION.
- 2. THE BATTERIES SHALL BE WIRED TO PROVIDE 24V DC POWER TO AN INVERTER FOR 120V AC DELIVERY TO ITS ENCLOSURE.
- 3. CONTRACTOR SHALL LOCATE THE GROUND MOUNTED SOLAR PANEL SYSTEM LESS THAN 20' FROM THE POLE-MOUNTED ITS SYSTEM AND ENSURE THAT THE SOLAR PANELS HAVE UNOBSTRUCTED SUN EXPOSURE.
- 4. GROUND MOUNTED SOLAR PANEL POLES INSTALLED WITHIN THE CLEAR ZONE SHALL BE SHIELDED BY BARRIER, LOCATED A MINIMUM OF 5' BEHIND THE PLANE OF ANY GUARDRAIL POSTS. SEE ILLINOIS TOLLWAY GUARDRAIL STANDARD (SECTION C OF STANDARDS) FOR MORE INFORMATION. ALL OTHER POLES SHALL BE LOCATED OUTSIDE THE CLEAR ZONE OR AS DIRECTED BY THE ENGINEER. FINAL LOCATION TO BE APPROVED BY THE ENGINEER.
- 5. ALL EQUIPMENT MUST BE CONNECTED TO A COMMON GROUND THROUGH THE ADJACENT ITS POLE BUSBAR, CONNECT A #2 AWG GROUND CABLE FROM THE EXTERNAL SOLAR POLE MOUNTED GROUND BUSBAR TO THE GROUND BAR IN THE SOLAR ENCLOSURE, ANY GROUND CONNECTED TO THE EXTERNAL GROUND BUSBAR SHALL BE EXOTHERMIC WELDED TO THE BUSBAR, SEALTITE CONDUIT SHOULD BE GROMMETTED ON END GOING TO BUSBAR TO PREVENT RODENTS AND INSECTS FROM ENTERING, A #2 AWG GROUND CABLE SHALL BE ATTACHED TO THE GROUND BUSBAR ATTACHED TO THE ADJACENT ITS POLE AND ROUTED THROUGH THE CONDUIT CONNECTING THE TWO ENCLOSURES AND ATTACHED TO THE GROUND BUSBAR ATTACHED TO THE SOLAR POLE. THE GROUND BUSBAR SHALL CONNECT TO A GROUND ROD (IN AN INSPECTION WELL) FOR THE SOLAR GENERATOR.
- 6. THE SOLAR POWER GENERATOR GROUND ROD SHALL BE CONNECTED TO THE GROUND ROD FOR THE ITS ENCLOSURE VIA A #2 AWG BARE GROUND CABLE EXOTHERMIC WELDED TO BOTH GROUND RODS.
- 7. CONTRACTOR TO PROVIDE ALL POWER AND GROUND WIRING REQUIRED FOR SYSTEM OPERATION WITHIN AND OUTSIDE THE ENCLOSURE.
- 8. BACKFILL HELIX FOUNDATION TO THE TOP OF THE POLE BASE ON ALL SIDES.
- ALL CABLING (INCLUDING CABLING INSIDE THE ENCLOSURE) SHALL BE OUTDOOR RATED. THE GROUND WIRE (WHITE) IN THE POWER CABLE SHALL BE TAPED GREEN.
- 10. ENCLOSURE SHALL BE VENTED AND CONTAIN BATTERIES AND SOLAR CONTROLLER.
- 11. SOLAR PANELS SHALL FACE 186 DEGREES FROM MAGNETIC NORTH AND SHALL BE TILTED 57 DEGREES FROM THE HORIZON.



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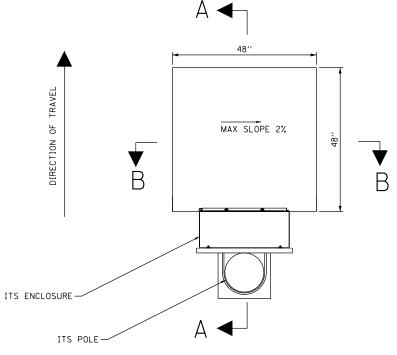
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REVISIONS		REVISIONS	CONTRACT NO. I-18-4694	ITS-05
٥.	DATE	DESCRIPTION	CUNTRACT NO. 1-10-4694	113-05
			SOLAR POWER GENERATOR	DRAWING NO.
				107 of 220
			CABINET 1-LINE ELECTRICAL DIAGRAM	101 6F 220

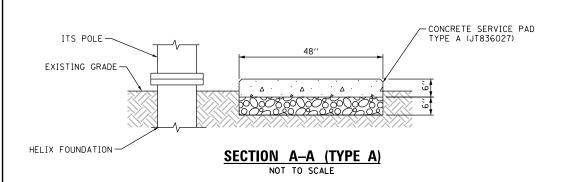
## NOTES:

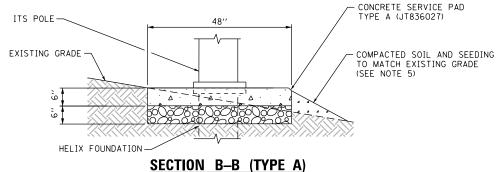
- 1. TYPE A SERVICE PADS SHALL BE INSTALLED ON SLOPES UP TO AND INCLUDING 1:6 (V:H).
- 2. CONCRETE SHALL BE IDOT CLASS SI.
- 3. ALL EXPOSED CONCRETE EDGES SHALL HAVE A 1" MINIMUM CHAMFER.
- 4. CONTRACTOR SHALL TAKE PRECAUTIONS TO STABALIZE EXISTING ITS POLES AND HELIX FOUNDATIONS WHILE EXCAVATING SOIL FOR INSTALLATION OF CONCRETE SERVICE PADS.
- 5. COMPACTED SOIL SHALL BE PLACED TO BE LEVEL WITH THE SERVICE PAD. CONTRACTOR MAY USE EXCVATED SOIL FROM PLACING THE PAD'S AGGREGATE BASE FOR GRADING PURPOSES WITH APPROVAL OF THE ENGINEER. SEEDING AND EROSION CONTROL SHALL BE PER THE GENERAL NOTES, SEE BELOW.
- 6. SOIL EXCAVATED FOR THE PURPOSE OF MAINTAINING A STABLE WORKING SLOPE WHILE INSTALLING THE SERVICE PAD SHALL BE REPLACED. BACKFILL SHALL BE EARTH WHICH IS FREE FROM DEBRIS, CINDERS, AND ROCKS MEASURING 2" OR GREATER IN DIAMETER. IN THE EVENT THAT EXCAVATED MATERIAL IS UNSUITABLE FOR USE AS BACKFILL, THE CONTRACTOR SHALL USE A CLEAN, NATURAL SAND. THIS SUBSTITUTE BACKFILL SHALL BE INCIDENTAL TO THE SERVICE PAD INSTALLATION AND WILL NOT BE PAID FOR SEPARATELY. ALL BACKFILL MATERIALS SHALL BE COMPACTED TO THE SATISFACTION OF THE
- 7. THE TOP SURFACE OF SOIL DISTURBED BY EXCAVATION FOR PLACING THE SERVICE PADS SHALL BE SEEDED AND AND PROTECTED WITH EROSION CONTROL MEASURES PER THE GENERAL NOTES, SEE BELOW.



## **CONCRETE SERVICE PAD FOR ITS POLE PLAN VIEW**

NOT TO SCALE

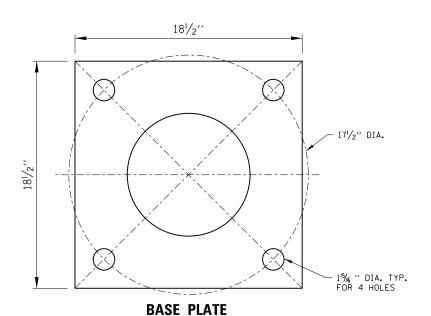




NOT TO SCALE

#### **EROSION AND SEDIMENT CONTROL GENERAL NOTES**

- PRIOR TO PERFORMING CONSTRUCTION ACTIVITIES RESULTING IN LAND DISTURBANCE SUCH AS EXCAVATION, TRENCHING, ETC., CONTRACTOR SHALL INSTALL EROSION CONTROL DEVICES TO PREVENT SEDIMENT FROM LEAVING THE CONSTRUCTION SITE AND ENTERING THE STORM DRAINAGE SYSTEM. THESE COMPONENTS SHALL BE IN PLACE BEFORE CONSTRUCTION ACTIVITIES TAKE PLACE. THIS PAYMENT WILL BE INCLUDED IN THE CONTRACT UNIT PRICE FOR FOUNDATION, HANDHOLE, SERVICE CONCRETE PAD AND CONDUIT INSTALLATION OR REMOVAL.
- 2. AFTER EARTH DISTURBING WORK IS COMPLETED, THE DISTURBED AREAS SHALL BE SEEDED AND COVERED WITH EROSION CONTROL BLANKET. BLANKET SHOULD BE KNITTED STRAW BLANKET PER IDOT STANDARD SPECIFICATION 1081.10(b).
- THE EXISTING GROUND COVER OVER ALL DISTURBED AREAS SHALL BE RESTORED USING EROSION CONTROL BLANKET AND SEEDING, CLASS 2E. THIS PAYMENT WILL BE INCLUDED IN THE CONTRACT UNIT PRICE FOR FOUNDATION, HANDHOLE, SERVICE CONCRETE PAD AND CONDUIT INSTALLATION OR REMOVAL.
- CONTRACTOR SHALL RESTORE TO THEIR ORIGINAL CONDITION AREAS THAT HAVE BEEN DISTURBED OR GRADED FOR TEMPORARY ACCESS TO THE JOB SITE, STORAGE AREAS, AND TEMPORARY VEHICULAR PARKING. NO ADDITIONAL COMPENSATION WILL BE PAID FOR RESTORING AREAS DISTURBED FOR THIS WORK.
- ALL EROSION CONTROL AND SEEDING FOR RESTORATION SHALL BE COORDINATED WITH AND COMPLETED TO THE SATISFACTION OF THE ENGINEER.



-HEX NUT

HELIX FOUNDATION PLATE

SEE BASE PLATE DETAIL

- 1023/32" DIA. CENTER HOLE

103/4" DIA. O.D. x 1/4"

15" DIA. HELIX 3" PITCH

−1¾′′ DIA.×10′′

WALL THICKNESS (MIN.)

TWO (2)  $3\frac{1}{2}$ "×21" SLOTS 180° APART

(FOR HOLE SPACING)

**BASE ATTACHMENT DETAIL** 

17 1/2" BASE DIA.

NOT TO SCALE

**HELIX FOUNDATION – ISOMETRIC VIEW** NOT TO SCALE

LOCK WASHER-

FLAT WASHER

HOT DIPPED GALVANIZED

13#4" DIAMETER BOLT WITH HEX HEAD (4 REQ'D PER FOUNDATION)-

## **HELIX FOUNDATION DETAILS**

NOT TO SCALE

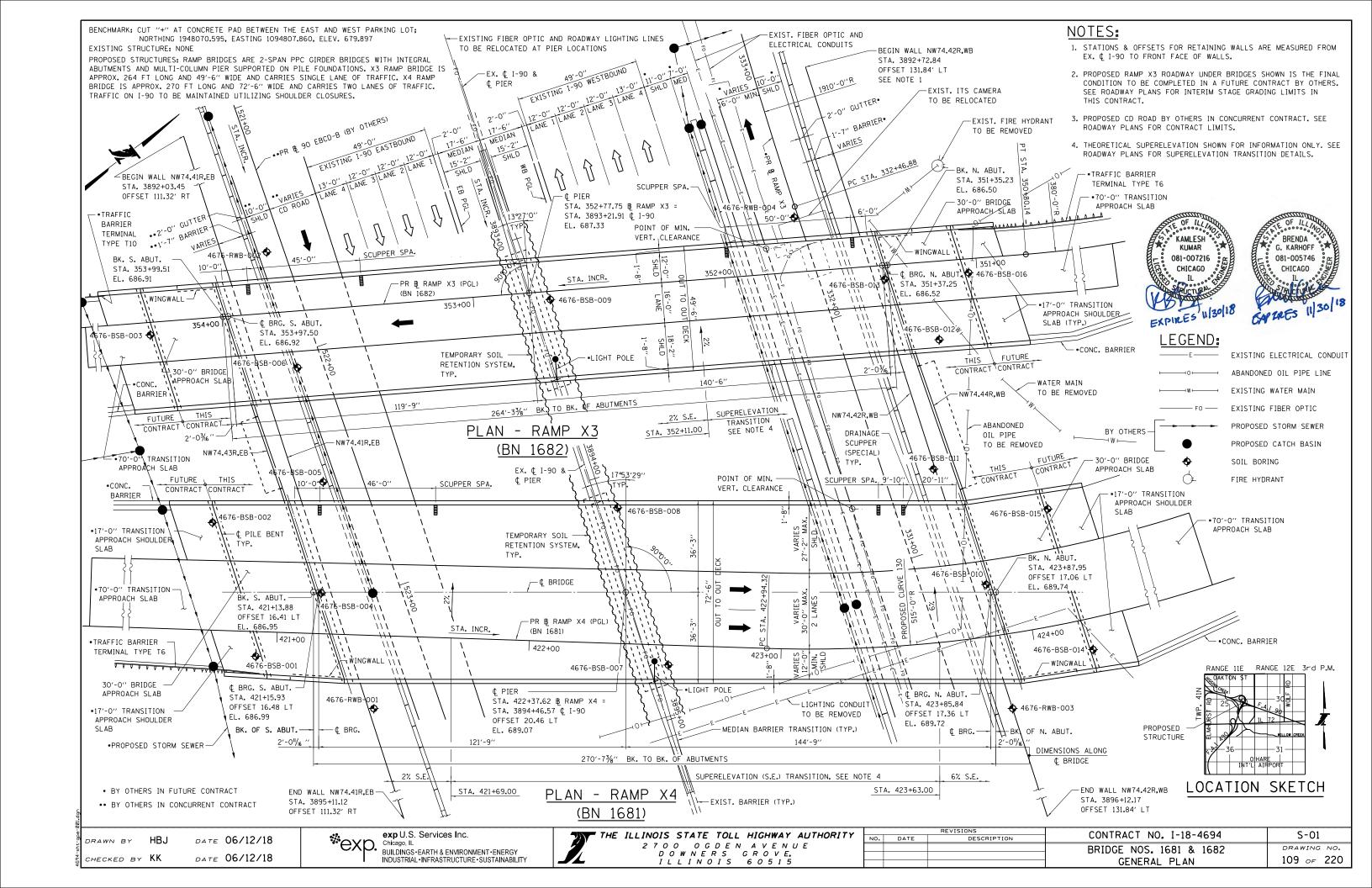
AM DATE 06/12/18 CHECKED BY KK DATE 06/12/18

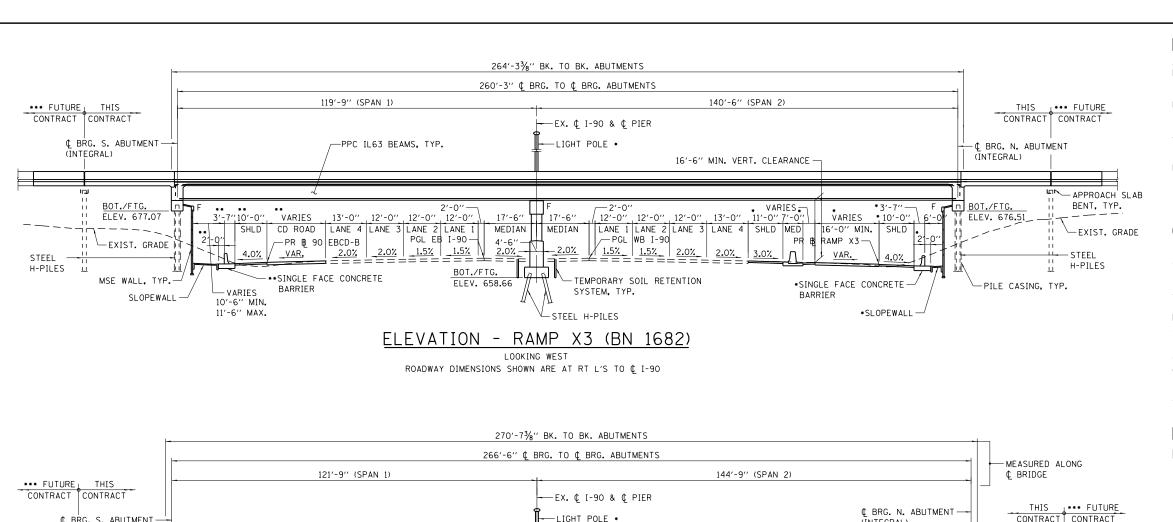
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REVISIONS		REVISIONS	CONTRACT NO. 1.10.4CO4	ITS-06
NO.	DATE	DESCRIPTION	CONTRACT NO. I-18-4694	113-06
			ITS POLE HELIX FOUNDATION AND	DRAWING NO.
				100 000
			ITS CONCRETE SERVICE PAD DETAILS	108 <i>o</i> ⊧ 220





#### CONTRACT CONTRACT ¢ BRG. S. ABUTMENT (INTEGRAL) -PPC IL63 BEAMS. TYP. (INTEGRAL) 16'-11" MIN. VERT. CLEARANCE -- WINGWALL WINGWALL -BOT./FTG. BOT./FTG. VARIES ELEV. 676.50 !! 3'-7"10'-0" 13'-0" 12'-0" 12'-0" 12'-0" 12'-0" , 12'-0" , 12'-0" , 13'-0" VARIES \* 10'-0" ELEV. 679.52 () CD ROAD LANE 4 LANE 3 LANE 2 LANE 1 LANE 1 LANE 2 LANE 3 LANE 4 MEDIAN | MEDIAN 16'-0" MIN. EXIST. GRADE ++ SHLD SHLD MED SHLD -EXIST. GRADE 1.1 [11] PGL WB I-90 -PR # 90 EBCD-B 4'-6" RAMP X3-() 2.0% 1.5% 1.5% 1.5% 1.5% 2.0% VAR. 4.0% VAR. ~|+ 1'! -STEEL STEEL H-PILES BOT./FTG. H-PILES \*SINGLE FACE CONCRETE TEMPORARY SOIL RETENTION MSE WALL, TYP.-ELEV. 659.47 \*SINGLE FACE CONCRETE BARRIER ─ PILE CASING, TYP. SYSTEM, TYP. VARIES BARRIER SLOPEWALL 10'-4" MIN •SLOPEWALL STEEL H-PILES 13'-9" MAX.

# ELEVATION - RAMP X4 (BN 1681)

LOOKING WEST

ROADWAY DIMENSIONS SHOWN ARE AT RT L'S TO ¢ I-90

# HIGHWAY CLASSIFICATION

FAI 90 - I-90 FUNCTIONAL CLASS: INTERSTATE ADT: 175,900 (EXIST) 181,000 (2030) ADTT: 14,480

SPEED: 65 M.P.H. (POSTED); 70 M.P.H. (DESIGN) DIRECTIONAL DISTRIBUTION: 48%/52% (EB/WB - AM PEAK) 53%/47% (FB/WB - PM PFAK)

> WEST BYPASS (RAMP X3, BN 1682) FUNCTIONAL CLASS: INTERSTATE EXPRESSWAY ADT: 14.000 (2030) ADTT: 1,120

SPEED: 25/45 M.P.H. (POSTED); 30/50 M.P.H. (DESIGN)

WEST BYPASS (RAMP X4, BN 1681) FUNCTIONAL CLASS: INTERSTATE EXPRESSWAY ADT: 19,000 (2030)

ADTT: 1.520 SPEED: 35/45 M.P.H. (POSTED): 40/50 M.P.H. (DESIGN)

DATE 06/12/18

DATE 06/12/18

HBJ

CHECKED BY KK

# MAXIMUM LIVE LOAD PLUS IMPACT DEFLECTION ≤ SPAN LENGTH/800 **\***ехр.

SOIL SITE CLASS = D

DESIGN LOADS

LIVE LOAD: HL-93 & IL-120

exp U.S. Services Inc. Chicago, IL

ALLOW 50 PSF FOR FUTURE WEARING SURFACE

SEISMIC PERFORMANCE ZONE (SPZ) = 1 (LRFD)

SEISMIC CRITERIA

BUILDINGS · EARTH & ENVIRONMENT · ENERGY INDUSTRIAL • INFRASTRUCTURE • SUSTAINABILITY

LIVE LOAD DEFLECTION CRITERIA

DESIGN SPECTRAL ACCELERATION AT 1.0 SEC.  $(S_{D1}) = 0.084g$ 

DESIGN SPECTRAL ACCELERATION AT 0.2 SEC. (Sps) = 0.144g

# $\odot \odot \odot$ 00 22'-0" 4'-0'10'-0"

IL-120 DESIGN TRUCK AXLE LOADS AND SPACING

THE ILLINOIS STATE TOLL HIGHWAY AUTHORITY

2 7 0 0 0 G D E N A V E N U E

DOWNERS GROVE. ILLINOIS 60515

#### \* BY OTHERS IN FUTURE CONTRACT

- \*\* BY OTHERS IN CONCURRENT CONTRACT
- \*\*\* SEE ROADWAY PLANS FOR TEMPORARY PAVEMENT & INTERIM GRADING.

#### DESIGN SPECIFICATIONS

2017 AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, 8TH

ILLINOIS STATE TOLL HIGHWAY AUTHORITY STRUCTURE DESIGN MANUAL, MARCH 2018.

ILLINOIS DEPARTMENT OF TRANSPORTATION BRIDGE MANUAL, JANUARY 2012.

ILLINOIS STATE TOLL HIGHWAY AUTHORITY GEOTECHNICAL ENGINEER'S MANUAL, MARCH 2018

ILLINOIS DEPARMENT OF TRANSPORTATION ALL BRIDGE DESIGNERS MEMORANDUMS, ABD 12.3 & 15.2

#### CONSTRUCTION SPECIFICATIONS

ILLINOIS DEPARTMENT OF TRANSPORTATION GUIDE BRIDGE SPECIAL PROVISIONS (GBSPs), 2018

ILLINOIS STATE TOLL HIGHWAY AUTHORITY SUPPLEMENTAL SPECIFICATIONS TO THE ILLINOIS DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION (CURRENT AT THE TIME OF CONTRACT ADVERTISEMENT)

ILLINOIS DEPARTMENT OF TRANSPORTATION SUPPLEMENTAL SPECIFICATIONS AND RECURRING SPECIAL PROVISIONS (CURRENT AT THE TIME OF CONTRACT ADVERTISEMENT)

ILLINOIS DEPARMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION, 2016

### DESIGN STRESSES

#### REINFORCED CONCRETE

f'c = 3,500 PSI (CLASS SI - SUBSTRUCTURE)

f'c = 4,000 PSI (CLASS BS - PARAPETS & BARRIERS)

f'c = 4,000 PSI (PERFORMANCE MIX - DECKS, DIAPHRAGMS, APPROACH SLABS, TRANSITION APPROACH

SLABS & TRANSITION APPROACH SHOULDER SLABS)

#### PRECAST CONCRETE

f'c = 4,500 PSI (CLASS PC - MSE WALL PANELS)

#### PRESTRESSED CONCRETE

f'c = 8,500 PSI (CLASS PS) f'ci = 7,000 PSI (CLASS PS)

#### REINFORCEMENT

fy = 60,000 PSI

#### PRESTRESSING STRANDS

fpu = 270,000 PSI (0.6" Ø LOW RELAXATION STRANDS) fpb+ = 202,500 PSI (0.6" Ø LOW RELAXATION STRANDS)

#### HORIZONTAL CURVE DATA

PROPOSED CURVE 130 PI STA. = 443+51.03  $\triangle$  = 151° 53′ 03.75′

D = 11° 07′ 31.42′

R = 515.00'T = 2.056.71'

L = 1,365.20' E = 1,605.21

e = 6% T.R. = N/A

S.E. RUN = 216.00' P.C. STA. = 422+94.32

LIMIT STATE INVENTORY OPERATING B.N. 1681 1682 1681 1682 FLEXURE 1.43 1.56 1.86 2.03 STRENGTH SHEAR 1.47 1.44 1.91 1.87 SERVICE III FLEXURE 1.55 1.46

STRUCTURE RATING (HL-93)

DESIGN LOAD RATING

P.T. STA. = 436+59.52 CONTRACT NO. I-18-4694 S-02 DESCRIPTION O. DATE BRIDGE NOS. 1681 & 1682 DRAWING NO. 110 of 220 GENERAL ELEVATION

# INDEX OF SHEETS

# TOTAL BILL OF MATERIAL - BRIDGE NOS. 1681 & 1682

S-01	BRIDGE NOS. 1681 & 1682 GENERAL PLAN
S-02	BRIDGE NOS. 1681 & 1682 GENERAL ELEVATION
S-03	BRIDGE NOS. 1681 & 1682 INDEX OF SHEETS & TOTAL BO
S-04	BRIDGE NOS. 1681 & 1682 GENERAL NOTES
S-05 S-06	BRIDGE NOS. 1681 & 1682 BRIDGE AESTHETIC DETAILS BRIDGE NOS. 1681 & 1682 PROTECTIVE SHIELD
S-07	BRIDGE NO. 1681 SUBSTRUCTURE LAYOUT
	BRIDGE NOS. 1681 & 1682 TEMPORARY SOIL RETENTION
S-08	SYSTEM
S-09	BRIDGE NO. 1681 PILE DRIVING RECORDS
S-10	BRIDGE NOS. 1681 & 1682 PILE DETAILS
S-11	SOUTH MSE WALL - NW74.41R,EB GENERAL PLAN &
	ELEVATION
S-12	NORTH MSE WALL - NW74.42R,WB GENERAL PLAN & ELEVATION
S-13	MSE WALL SECTIONS & DETAILS 1
S-14	MSE WALL SECTIONS & DETAILS 2
S-15	MSE WALL SECTIONS & DETAILS 3
S-16	BRIDGE NO. 1681 SOUTH ABUTMENT PLAN & ELEVATION
S-17	BRIDGE NO. 1681 SOUTH ABUTMENT DETAILS
S-18 S-19	BRIDGE NO. 1681 NORTH ABUTMENT PLAN & ELEVATION BRIDGE NO. 1681 NORTH ABUTMENT DETAILS
S-20	BRIDGE NO. 1681 WINGWALL DETAILS
S-21	SOUTH L-WALL - NW74.43R,EB ELEVATION & DETAILS
S-22	NORTH L-WALL - NW74.44R,WB ELEVATION & DETAILS
S-23	BRIDGE NO. 1681 PIER PLAN & ELEVATION
S-24	BRIDGE NO. 1681 PIER DETAILS
S-25	BRIDGE NO. 1681 FRAMING PLAN
S-26 S-27	BRIDGE NO. 1681 PPC IL63 BEAM DETAILS 1 BRIDGE NO. 1681 PPC IL63 BEAM DETAILS 2
S-28	BRIDGE NO. 1681 PPC IL63 BEAM DETAILS 3
S-29	BRIDGE NO. 1681 PPC IL63 BEAM DETAILS 4
S-30	BRIDGE NO. 1681 TOP OF SLAB PLAN
S-31	BRIDGE NO. 1681 TOP OF SLAB ELEVATIONS 1
S-32	BRIDGE NO. 1681 TOP OF SLAB ELEVATIONS 2
S-33 S-34	BRIDGE NO. 1681 TOP OF SLAB ELEVATIONS 3 BRIDGE NO. 1681 TOP OF SLAB ELEVATIONS 4
S-35	BRIDGE NO. 1681 DECK PLAN 1
S-36	BRIDGE NO. 1681 DECK PLAN 2
S-37	BRIDGE NO. 1681 DECK PLAN 3
S-38	BRIDGE NO. 1681 DECK PLAN 4
S-39	BRIDGE NO. 1681 DECK CROSS SECTION
S-40 S-41	BRIDGE NO. 1681 SOUTH ABUTMENT DIAPHRAGM DETAILS BRIDGE NO. 1681 NORTH ABUTMENT DIAPHRAGM DETAILS
S-42	BRIDGE NO. 1681 PIER DIAPHRAGM DETAILS
S-43	BRIDGE NO. 1681 SUPERSTRUCTURE DETAILS 1
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S-45	BRIDGE NO. 1681 SUPERSTRUCTURE DETAILS 3
S-46	BRIDGE NOS. 1681 & 1682 BRIDGE DRAINAGE DETAILS
S-47 S-48	BRIDGE NOS. 1681 & 1682 DRAINAGE SCUPPER DETAILS BRIDGE NO. 1681 TOP OF APPROACH SLAB ELEVATIONS
S-49	BRIDGE NO. 1681 SOUTH APPROACH SLAB
S-50	BRIDGE NO. 1681 NORTH APPROACH SLAB
S-51	BRIDGE NO. 1681 APPROACH SLAB DETAILS
S-52	BRIDGE NO. 1681 SOUTH APPROACH PILE BENT
S-53	BRIDGE NO. 1681 NORTH APPROACH PILE BENT
S-54 S-55	BRIDGE NO. 1682 SUBSTRUCTURE LAYOUT BRIDGE NO. 1682 PILE DRIVING RECORDS
S-56	BRIDGE NO. 1682 SOUTH ABUTMENT PLAN & ELEVATION
S-57	BRIDGE NO. 1682 SOUTH ABUTMENT DETAILS
S-58	BRIDGE NO. 1682 NORTH ABUTMENT PLAN & ELEVATION
S-59	BRIDGE NO. 1682 NORTH ABUTMENT DETAILS
S-60	BRIDGE NO. 1682 WINGWALL DETAILS
S-61	BRIDGE NO. 1682 PIER PLAN & ELEVATION BRIDGE NO. 1682 PIER DETAILS
S-62 S-63	BRIDGE NO. 1682 PIER DETAILS BRIDGE NO. 1682 FRAMING PLAN
S-64	BRIDGE NO. 1682 PPC IL63 BEAM DETAILS 1
S-65	BRIDGE NO. 1682 PPC IL63 BEAM DETAILS 2
S-66	BRIDGE NO. 1682 PPC IL63 BEAM DETAILS 3
S-67	BRIDGE NO. 1682 PPC IL63 BEAM DETAILS 4
S-68	BRIDGE NO. 1682 TOP OF SLAB PLAN
S-69 S-70	BRIDGE NO. 1682 TOP OF SLAB ELEVATIONS 1 BRIDGE NO. 1682 TOP OF SLAB ELEVATIONS 2
S-70	BRIDGE NO. 1682 TOP OF SLAB ELEVATIONS 2  BRIDGE NO. 1682 TOP OF SLAB ELEVATIONS 3

S-72 BRIDGE NO. 1682 DECK PLAN 1
S-73 BRIDGE NO. 1682 DECK PLAN 2
S-74 BRIDGE NO. 1682 DECK PLAN 3
S-75 BRIDGE NO. 1682 DECK PLAN 4
S-76 BRIDGE NO. 1682 DECK CROSS SECTION
S-77 BRIDGE NO. 1682 SOUTH ABUTMENT DIAPHRAGM DETAILS
S-78 BRIDGE NO. 1682 NORTH ABUTMENT DIAPHRAGM DETAILS
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S-80 BRIDGE NO. 1682 SUPERSTRUCTURE DETAILS 1
S-81 BRIDGE NO. 1682 SUPERSTRUCTURE DETAILS 2
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S-83 BRIDGE NO. 1682 TOP OF APPROACH SLAB ELEVATIONS
S-84 BRIDGE NO. 1682 SOUTH APPROACH SLAB
S-85 BRIDGE NO. 1682 NORTH APPROACH SLAB
S-86 BRIDGE NO. 1682 APPROACH SLAB DETAILS
S-87 BRIDGE NO. 1682 SOUTH APPROACH PILE BENT
S-88 BRIDGE NO. 1682 NORTH APPROACH PILE BENT
S-89 BRIDGE NOS. 1681 & 1682 BORING LOG
S-90 BRIDGE NOS. 1681 & 1682 BORING LOG
S-91 BRIDGE NOS. 1681 & 1682 BORING LOG
S-92 BRIDGE NOS. 1681 & 1682 BORING LOG
S-93 BRIDGE NOS. 1681 & 1682 BORING LOG
S-94 BRIDGE NOS. 1681 & 1682 BORING LOG
S-95 BRIDGE NOS. 1681 & 1682 BORING LOG
S-96 BRIDGE NOS. 1681 & 1682 BORING LOG
S-97 BRIDGE NOS. 1681 & 1682 BORING LOG
S-98 BRIDGE NOS. 1681 & 1682 BORING LOG
S-99 BRIDGE NOS. 1681 & 1682 BORING LOG
S-100 BRIDGE NOS. 1681 & 1682 BORING LOG
S-101 BRIDGE NOS. 1681 & 1682 BORING LOG
S-102 BRIDGE NOS. 1681 & 1682 BORING LOG
S-103 BRIDGE NOS. 1681 & 1682 BORING LOG
S-104 BRIDGE NOS. 1681 & 1682 BORING LOG
S-105 BRIDGE NOS. 1681 & 1682 BORING LOG S-106 BRIDGE NOS. 1681 & 1682 BORING LOG
S-107 BRIDGE NOS. 1681 & 1682 BORING LOG

	T			DN 1	C 01	П		DN 1	C02	
PAY ITEM			FSTIM	BN 1		RECORD QTY.	BN 1682 ESTIMATED QUANTITY			RECORD QTY.
NUMBER	ITEM	UNIT	SUPER	SUB	TOTAL	TOTAL	SUPFR	SUB	TOTAL	TOTAL
50157300	PROTECTIVE SHIELD	SQ YD	2.117	-	2.117	101112	1.445	-	1.445	
50200100	STRUCTURE EXCAVATION	CU YD		211	211		-	143	143	
50300225	CONCRETE STRUCTURES	CU YD		462.9	462.9		_	296.9	296.9	
50300225	CONCRETE SUPERSTRUCTURE	CU YD	89.7	-	89.7		88	-	88	
50300260	BRIDGE DECK GROOVING	SQ YD	2.004	_	2,004		1.287	_	1.287	
50300300	PROTECTIVE COAT	SQ YD	2,386	_	2,386		1,660	_	1,660	
50401340	FURNISHING AND ERECTING PRECAST PRESTRESSED CONCRETE BEAMS, IL63	FOOT	2,668	-	2,668		1,563	-	1,563	
50800205	REINFORCEMENT BARS, EPOXY COATED	POUND	244,090	60,970	305,060		176,550	39,980	216,530	
51201400	FURNISHING STEEL PILES HP10X42	FOOT	-	1,135	1,135		-	765	765	
51201900	FURNISHING STEEL PILES HP14X89	FOOT	-	5,924	5,924		-	3,968	3,968	
51202305	DRIVING PILES	FOOT	-	7,059	7,059		-	4,733	4,733	
51203400	TEST PILE STEEL HP10X42	EACH	-	2	2		-	2	2	
51203900	TEST PILE STEEL HP14X89	EACH	-	3	3		-	3	3	
51204650	PILE SHOES	EACH	-	97	97		-	68	68	
52100540	ANCHOR BOLTS, 1 1/2"	EACH	-	4	4		-	4	4	
52200020	TEMPORARY SOIL RETENTION SYSTEM	SQ FT	-	947	947		-	663	663	
58700300	CONCRETE SEALER	SQ FT	-	5,282	5,282		-	3,454	3,454	
59100100	GEOCOMPOSITE WALL DRAIN	SQ YD	-	161	161		-	107	107	
JI420040	BRIDGE APPROACH SLAB	SQ YD	488	-	488		330	-	330	
JI503010	HIGH PERFORMANCE CONCRETE SUPERSTRUCTURE	CU YD	733	-	733		469.4	-	469.4	
JI599040	FORM LINER TEXTURED SURFACE (SPECIAL)	SQ FT	544	109	653		532	80	612	
JT512300	PILE CASING, CORRUGATED METAL PIPE, 24"	FOOT	_	687	687		_	456	456	
X0320000	DRAINAGE SYSTEM, NO. 1	EACH	-	1	1		-	-	-	
X0320002	DRAINAGE SYSTEM, NO. 2	EACH	-	-	-		-	1	1	
X0327357	CONSTRUCTION VIBRATION MONITORING	L SUM	-	0.5	0.5		-	0.5	0.5	
X5860110	GRANULAR BACKFILL FOR STRUCTURES	CU YD	-	412	412		-	248	248	
Z0018000	DRAINAGE SCUPPERS (SPECIAL)	EACH	4	-	4		4	-	4	
Z0046304	PIPE UNDERDRAINS FOR STRUCTURES 4"	FOOT	-	162	162		-	112	112	

# TOTAL BILL OF MATERIAL - NW74.41R,EB, NW74.42R,WB, NW74.43R,EB & NW74.44R,WB

			S. MSE WALL	- NW74.41R,EB	N. MSE WALL -	NW74.42R,WB	S. L-WALL -	NW74.43R,EB	N. L-WALL -	NW74.44R,WB
PAY ITEM			ESTIMATED QTY.	RECORD QTY.	ESTIMATED QTY.	RECORD QTY.	ESTIMATED QTY.	RECORD QTY.	ESTIMATED QTY.	RECORD QTY.
NUMBER	ITEM	UNIT	TOTAL	TOTAL	TOTAL	TOTAL	TOTAL	TOTAL	TOTAL	TOTAL
50200100	STRUCTURE EXCAVATION	CU YD	1,360		2,050		-		-	
50200450	REMOVAL AND DISPOSAL OF UNSUITABLE MATERIAL FOR STRUCTURES	CU YD	858		785		-		-	
50300225	CONCRETE STRUCTURES	CU YD	-		-		28.5		51.3	
50300300	PROTECTIVE COAT	SQ YD	33		35		-		-	
50800205	REINFORCEMENT BARS, EPOXY COATED	POUND	-		-		3,460		6,100	
51100100	SLOPE WALL 4 INCH	SQ YD	341		-		-		-	
58700300	CONCRETE SEALER	SQ FT	-		-		266		503	
59100100	GEOCOMPOSITE WALL DRAIN	SQ YD	-		-		30		56	
JI522500	MECHANICALLY STABILIZED EARTH RETAINING WALL	SQ FT	4,922		5,333		-		-	
JI606015	GUTTER, TYPE G-2, MODIFIED	FOOT	146		156		-		-	
JT205010	EMBANKMENT UNDER STRUCTURES	CU YD	174		147		-		-	
X5860110	GRANULAR BACKFILL FOR STRUCTURES	CU YD	-		-		41		75	
X6640535	CHAIN LINK FENCE, 6' ATTACHED TO STRUCTURE	FOOT	191		208		-		-	
Z0046304	PIPE UNDERDRAINS FOR STRUCTURES 4"	FOOT	308		340		30		52	·

DATE 06/12/18

DATE 06/12/18

DRAWN BY EG

CHECKED BY KK



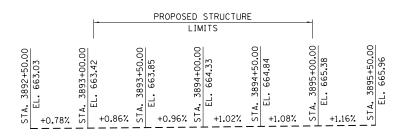
		REVISIONS	CONTRACT NO. I-18-4694	S-03		
NO.	DATE	DESCRIPTION	PER OI I TON IDANIMOD	3 03		
			BRIDGE NOS. 1681 & 1682	DRAWING NO.		
				111 220		
			INDEX OF SHEETS & TOTAL BOM	III of 220		

#### GENERAL NOTES

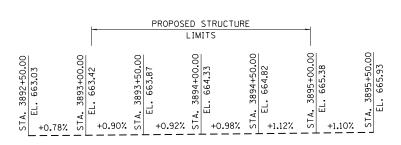
- ALL EXPOSED CONCRETE EDGES SHALL HAVE A 3/4" X 45° CHAMFER, EXCEPT WHERE SHOWN OTHERWISE. CHAMFER ON VERTICAL EDGES SHALL BE CONTINUED A MINIMUM OF ONE FOOT BELOW FINISHED GROUND LEVEL.
- 2. REINFORCEMENT BARS, INCLUDING EPOXY-COATED REINFORCEMENT BARS, SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M-31 OR ASTM A706, GRADE 60, DEFORMED BARS.
- 3. REINFORCEMENT BARS DESIGNATED "(E)" SHALL BE EPOXY COATED.
- 4. REINFORCEMENT BAR BENDING DETAILS SHALL BE IN ACCORDANCE WITH THE LATEST "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES", ACI 315.
- 5. REINFORCEMENT BAR BENDING DIMENSIONS ARE OUT TO OUT.
- 6. BARS NOTED THUS, 3x2-#5 INDICATES 3 LINES OF BARS WITH 2 LENGTHS OF BARS PER LINE.
- 7. COVER FROM THE FACE OF CONCRETE TO FACE OF REINFORCEMENT BARS SHALL BE 3" FOR SURFACES FORMED AGAINST EARTH AND 2" FOR ALL OTHER SURFACES UNLESS OTHERWISE SHOWN.
- 8. CONTRACTOR SHALL NOT SCALE DIMENSIONS FROM THE CONTRACT PLANS FOR CONSTRUCTION PURPOSES. SCALES SHOWN ARE FOR INFORMATION ONLY.
- 9. NO CONSTRUCTION JOINTS EXCEPT THOSE SHOWN ON THE PLANS WILL BE ALLOWED UNLESS APPROVED BY THE ENGINEER.
- 10. THE PROTECTIVE SHIELD SYSTEM SHALL EXTEND A MINIMUM OF 2' BEYOND THE OUTSIDE OF PROPOSED PARAPETS FOR THE FULL LENGTH OF BRIDGE.
- 11. CONCRETE SEALER SHALL BE APPLIED TO THE EXPOSED SURFACES OF CRASHWALL, COLUMNS, MEDIAN PIER CAPS, FRONT FACES OF ABUTMENTS, AND WINGWALLS,
- 12. AFTER THE BEAMS (GIRDERS) ARE SET, ALL ELEVATIONS FOR DETERMINING FILLET HEIGHTS SHALL BE TAKEN AT ONE TIME.
- 13. UPON COMPLETION OF EACH STRUCTURE, THE CONTRACTOR SHALL MEASURE THE RESULTING HORIZONTAL AND VERTICAL CLEARANCES AND SUBMIT THEM TO THE ENGINEER FOR REVIEW AND INCLUSION IN THE AS BUILT PLANS (RECORD DRAWINGS). COST IS INCLUDED IN THE UNIT PRICE OF FURNISHING AND ERECTING PRECAST PRESTRESSED CONCRETE BEAMS IL63.
- 14. THE EMBANKMENT CONFIGURATION SHOWN SHALL BE THE MINIMUM THAT MUST BE PLACED AND COMPACTED PRIOR TO CONSTRUCTION OF THE ABUTMENTS AND BRIDGE APPROACH SLABS.
- 15. THE SOIL BORING LOGS REPRESENT POINT INFORMATION. PRESENTATION OF THIS INFORMATION IN NO WAY IMPLIES THAT SUBSURFACE CONDITIONS ARE THE SAME AT LOCATIONS OTHER THAN THE EXACT LOCATION OF THE BORING.
- 16. A MINIMUM PERIOD OF 60 DAYS BETWEEN CASTING OF THE BEAM AND PLACING OF THE CONCRETE DECK SHALL BE PROVIDED.
- 17. FOR ALL CONCRETE BEAMS LONGER THAN 120 FOOT, THE CONTRACTOR SHALL SUBMIT CALCULATIONS FOR LATERAL STABILITY DURING SHIPPING, HANDLING, AND ERECTION TO THE ENGINEER FOR APPROVAL PRIOR TO SHIPPING. THE CALCULATIONS SHALL BE SEALED AND SIGNED BY AN ILLINOIS LICENSED STRUCTURAL ENGINEER.
- 18. ALL SIDE RETAINERS SHALL BE INSTALLED AND BOLTED DOWN PRIOR TO FORMING AND POURING THE DECK SLAB.
- 19. FOLLOWING THE PLACEMENT OF EACH BEAM AND PRIOR TO THE PLACEMENT OF ADJACENT BEAM THE CONTRACTOR SHALL SURVEY THE HORIZONTAL AND VERTICAL POSITION OF EACH BEAM AT THE CENTER LINE OF ALL BEARINGS. THE RECORDED STATIONS AND ELEVATIONS SHALL BE COMPARED TO THE CONTRACT PLANS AND/OR THE ERECTION PLAN AND SIGNIFICANT DEVIATIONS FROM THE PLANS SHALL BE REPORTED TO THE ENGINEER. COST TO BE INCLUDED WITH UNIT PRICE OF FURNISHING AND ERECTING PRECAST PRESTRESSED CONCRETE BEAMS 1163.
- 20. SLIPFORMING OF CONCRETE PARAPETS IS NOT PERMITTED.

DATE 06/12/18

21. CALCULATED WEIGHT OF STRUCTURAL STEEL AASHTO M270 GR. 36 = 30770 LB. INCLUDED IN THE COST OF FURNISHING AND ERECTING PRECAST PRESTRESSED CONCRETE BEAMS, IL63.



EXIST. PROFILE GRADE - I-90 EB



EXIST. PROFILE GRADE - I-90 WB

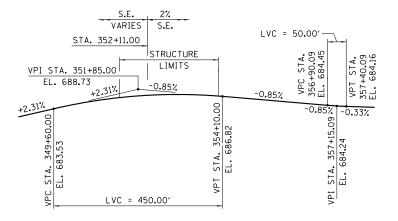
ALONG WB PGL

S.E. VARIES S.E. 2% TO 6% S.E. STA. 421+69.00 STA. 423+63.00 STRUCTURE LIMITS VPI STA. 421+00.00 VPI STA. 423+20.00 EL. 686.78 EL. 691.84 +2.30% -1.10% VPI STA. 419+91.47 EL. 685.86 VPC STA. 419+66.47 EL. 685.48 STA. EL. 6 VPC 9 VPT STA. 420+16.47 STA EL. FL 686-07 LVC = 285.00' VΡΤ LVC=50.00' LVC=150.00'

# THEORETICAL PROFILE GRADE - RAMP X4

ALONG B RAMP X4

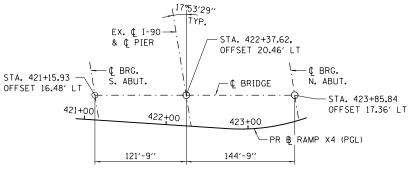
THEORETICAL PROFILE GRADE & SUPERELEVATION SHOWN FOR INFORMATION ONLY. SEE ROADWAY PLANS SUPERELEVATION TRANSITION FOR ELEVATION DETAILS AT TRANSITION.



# THEORETICAL PROFILE GRADE - RAMP X3

ALONG B RAMP X3

THEORETICAL PROFILE GRADE & SUPERELEVATION SHOWN FOR INFORMATION ONLY. SEE ROADWAY PLANS SUPERELEVATION TRANSITION FOR ELEVATION DETAILS AT TRANSITION.



<u> OFFSET SKETCH - RAMP X4</u>

DRAWN BY EG DATE 06/12/18

CHECKED BY KK

**\***ехр.

exp U.S. Services Inc.
Chicago, IL
BUILDINGS-EARTH & ENVIRONMENT-ENERGY
INDUSTRIAL -INFRASTRUCTURE - SUSTAINABILITY

THE ILLINOIS STATE TOLL HIGHWAY AUTHORITY

2 7 0 0 0 G D E N A V E N U E

D O W N E R S G R O V E.

I L I N O I S 6 0 5 1 5

CONTRACT NO. I-18-4694

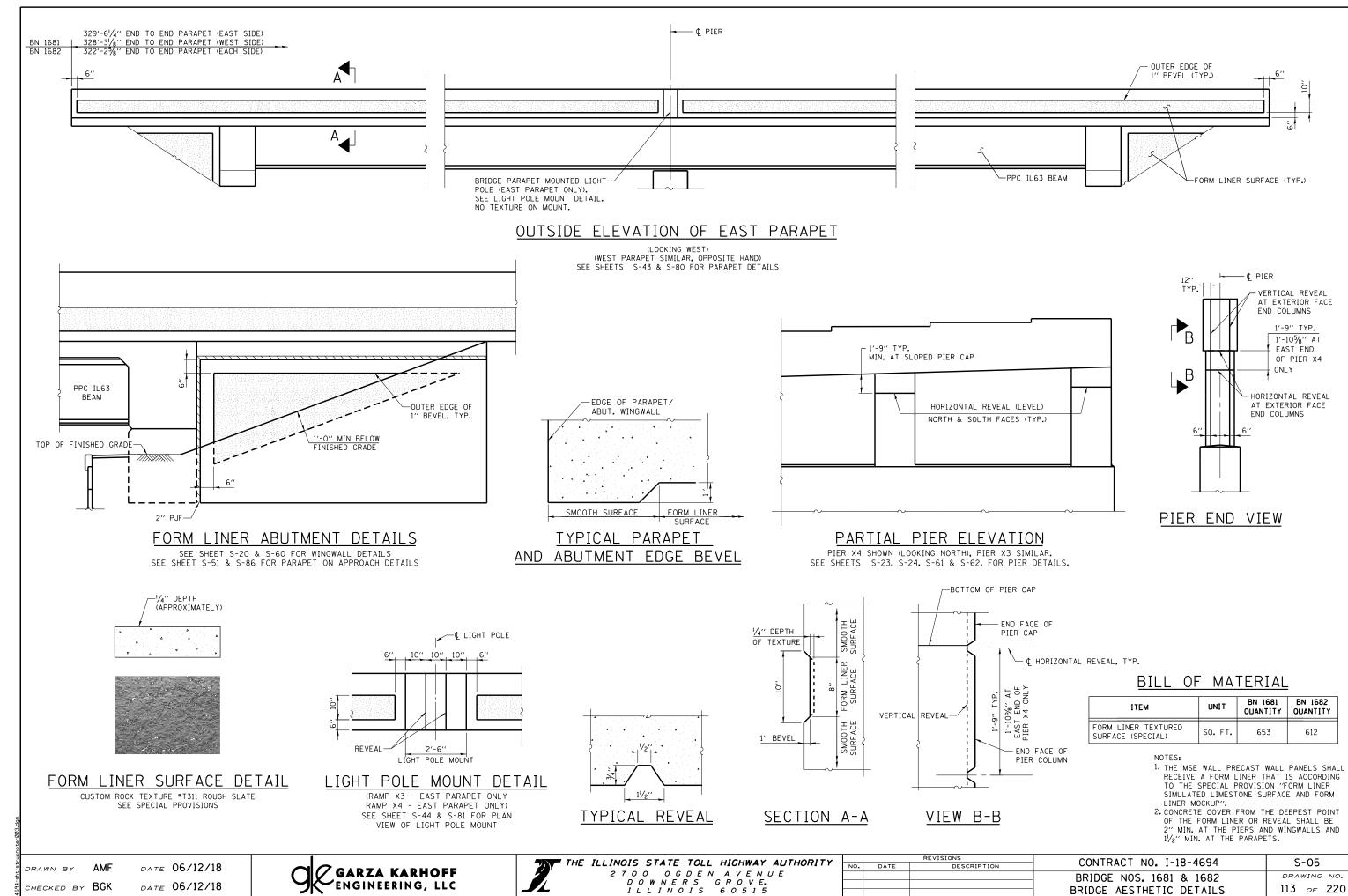
DESCRIPTION

BRIDGE NOS. 1681 & 1682

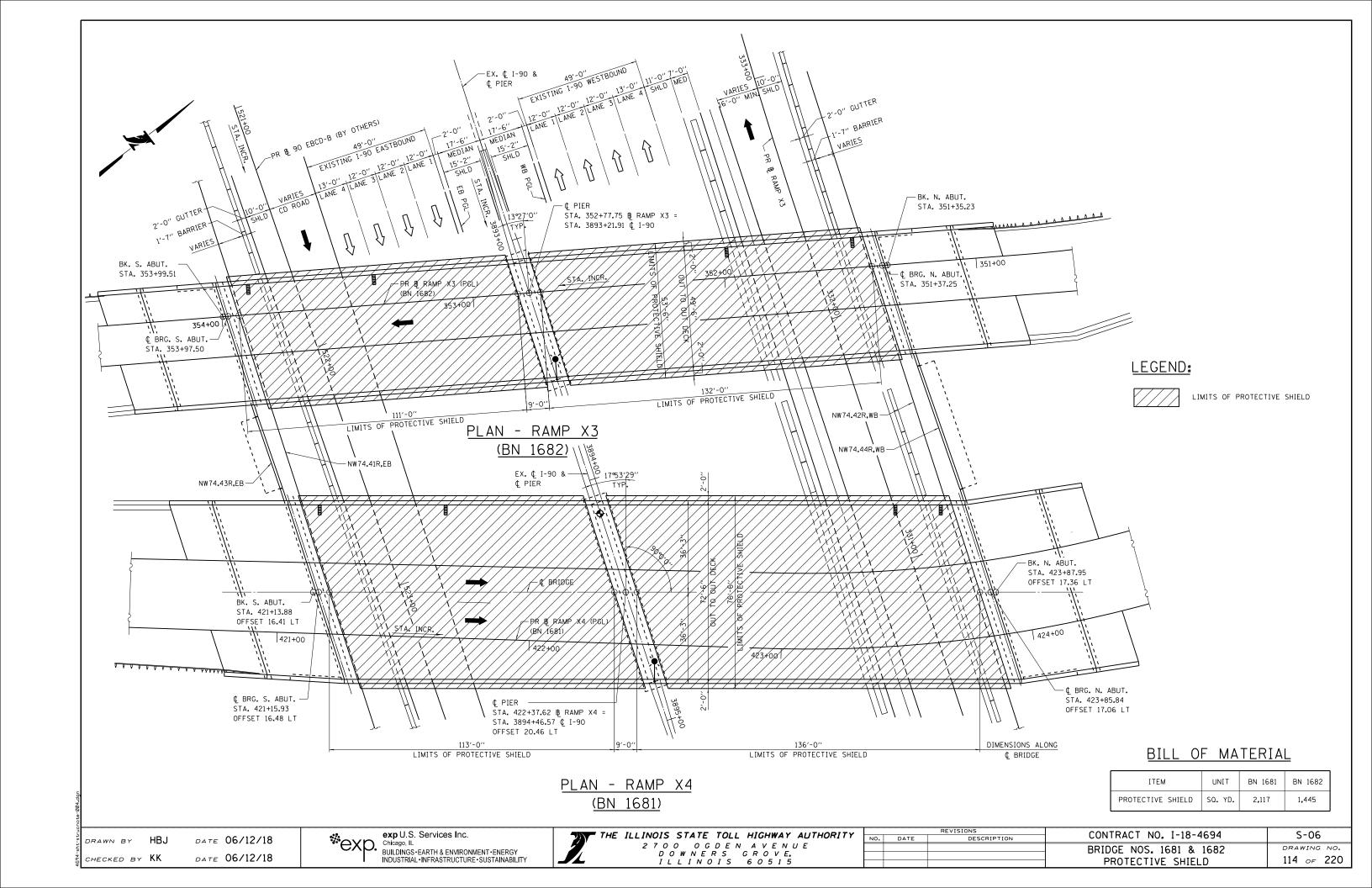
GENERAL NOTES

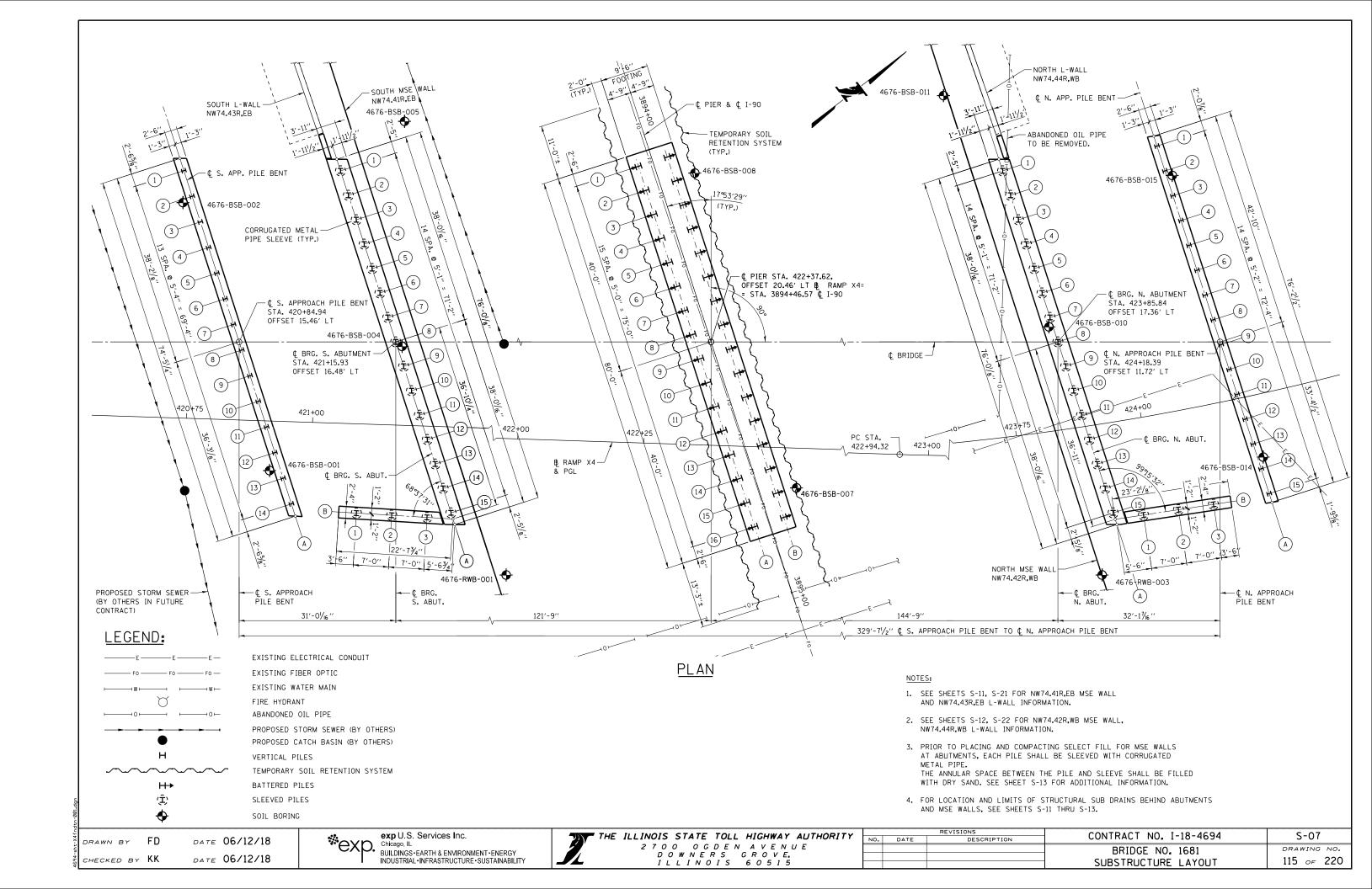
DRAWING NO.

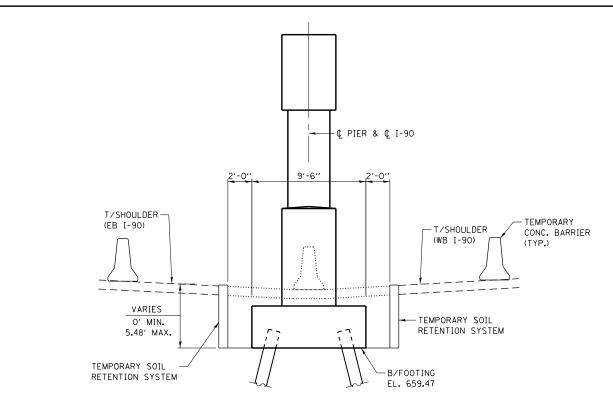
112 OF 220



7 T V			112131313
111	NO.	DATE	DESCRIPTION







# TEMPORARY SOIL RETENTION SYSTEM SECTION AT RAMP X4 PIER

(LOOKING WEST)

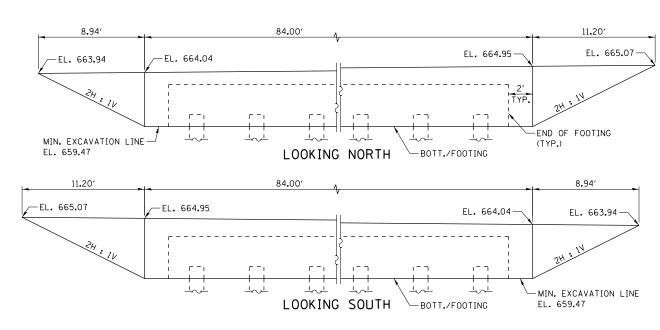
#### NOTE:

THE INFORMATION SHOWN FOR TSRS IS ESTIMATED. IT IS THE CONTRACTOR'S RESPONSIBILITY TO PROVIDE A DESIGN AND DETAILS FOR EACH TSRS, COMPLETE WITH CALCULATIONS, SIGNED AND SEALED BY AN ILLINOIS LICENSED STRUCTURAL ENGINEER, FOR THE ENGINEER'S REVIEW AND ACCEPTANCE BEFORE STARTING WORK.

# -¢ PIER & ¢ I-90 T/SHOULDER-(EB I-90) T/SHOULDER TEMPORARY CONC.-(WB I-90) BARRIER (TYP.) VARIES TEMPORARY SOIL O' MIN. RETENTION SYSTEM 5.02' MAX. TEMPORARY SOIL -B/FOOTING RETENTION SYSTEM EL. 658.66

# TEMPORARY SOIL RETENTION SYSTEM SECTION AT RAMP X3 PIER

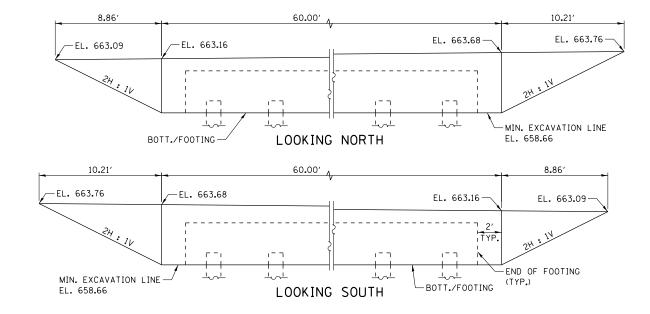
(LOOKING WEST)



# TEMPORARY SOIL RETENTION SYSTEM INSIDE ELEVATIONS AT RAMP X4 PIER (BN 1681)

# BILL OF MATERIAL

ITEM	UNIT	BN 1681 QUANTITY	BN 1682 QUANTITY	
TEMPORARY SOIL RETENTION SYSTEM	SQ. FT.	947	663	



# TEMPORARY SOIL RETENTION SYSTEM INSIDE ELEVATIONS AT RAMP X3 PIER (BN 1682)

DRAWN BY FD DATE 06/12/18

CHECKED BY KK DATE 06/12/18

\*exp. exp Chica BUILE INDU

exp U.S. Services Inc.
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BUILDINGS • EARTH & ENVIRONMENT • ENERGY
INDUSTRIAL • INFRASTRUCTURE • SUSTAINABILITY

		REVISIONS	CONTRACT NO I-19-4604	S-08
NO.	DATE	DESCRIPTION	CONTRACT NO. I-18-4694	3-08
			BRIDGE NOS. 1681 & 1682	DRAWING NO.
				116 oF 220
			TEMPORARY SOIL RETENTION SYSTEM	116 of 220

# PILE DRIVING RECORD - BN 1681

DATE PILE DRIVEN:(MONTH YEAR)		
TYPE & SIZE PILE USED:		
PILE DRIVING EQUIPMENT USED:		ENERGY RATING:
HAMMER USED: TYPE:	STROKE	WEIGHT
FORMULA USED TO CALCULATE CAPACITY:		
PILE DRIVING CONTRACTOR:		CM:

GROUND DRIVING DATA FOR THE FINAL 5 FT BLOWS											
PILE LOCATION	PILE NUMBER	SURFACE	CUT-OFF ELEVATION	PENETRATED LENGTH, FT.	4′ TO 3′	3′ T0 2′	2′ T0 1′	12" TO 6" ••	6" TO	CAPACITY TONS	REMARKS
PIER										TUNS	REWARNS

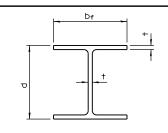
GROUND DRIVING DATA FOR THE FINAL 5 FT BLOWS													
PILE LOCATION	PILE NUMBER	SURFACE	CUT-OFF ELEVATION	PENETRATED LENGTH, FT.	5' TO 4'	4′ TO 3′	3′ T0 2′	2′ T0 1′	1′ T0 0′	12" TO 6" ••	6" TO 0" ••	CAPACITY TONS	REMARKS
S. ABUT.													
S. APP.													
PILE													
BENT													
N. ABUT.													
N. ADUT.													
N. APP. PILE													
BENT													
		I	1		I	1	1	1	ı	1	ı	1	

<sup>••</sup> FOR PILES DRIVEN TO REFUSAL, BLOW COUNT FOR THE LAST FOOT SHALL BE RECORDED IN 6 INCHES INCREMENTS. PILE DAMAGE, OBSTRUCTION, PILE REJECTION, TEST PILES ETC. SHALL BE RECORDED IN REMARKS COLUMN.

DRAWN BY FD CHECKED BY KK DATE 06/12/18 DATE 06/12/18

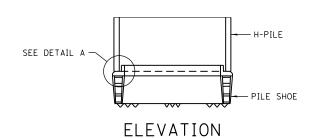


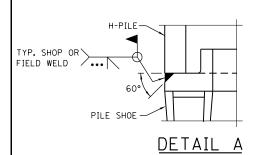
		REVISIONS	CONTRACT NO. I-18-4694	S-09
NO.	DATE	DESCRIPTION	CONTRACT NO. 1-10-4694	3-09
			BRIDGE NO. 1681	DRAWING NO.
			PILE DRIVING RECORDS	117 of 220



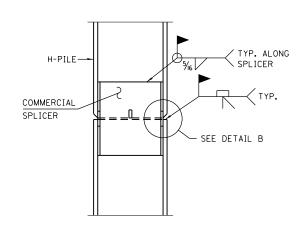
# STEEL PILE TABLE

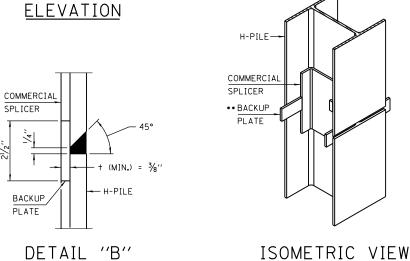
DESIGNATION	DEPTH d	FLANGE WIDTH b <sub>f</sub>	WEB AND FLANGE THICKNESS †	ENCASEMENT DIAMETER A
HP 14×117	141/4′′	147/8′′	13/16 ′′	30′′
×102	14''	143/4′′	11/16 ′′	30′′
×89	137/8′′	143/4′′	5/8′′	30′′
×73	135/8′′	145/8′′	1/2"	30′′
HP 12×84	121/4′′	121/4′′	11/16 ′′	24′′
×74	121/8''	121/4′′	5/8′′	24''
×63	12''	121/8′′	1/2"	24′′
×53	113/4′′	12''	7/6 ′′	24''
HP 10×57	10′′	101/4′′	9/16 ′′	24''
×42	9¾''	101/8''	%6 ′′	24′′
HP 8×36	8′′	81/8''	7/6 ′′	18''



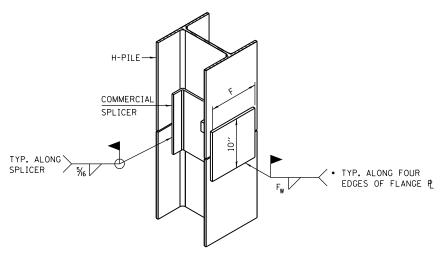


H-PILE SHOE ATTACHMENT

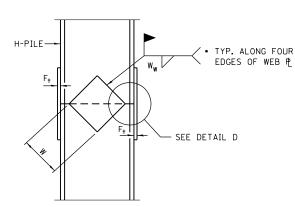




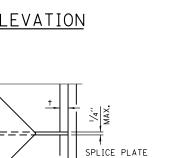
WELDED COMMERCIAL SPLICE



ISOMETRIC VIEW



ELEVATION



THICKNESS F.

DETAIL D

DESIGNATION	F	F <sub>†</sub>	F <sub>w</sub>	w	W <sub>+</sub>	w <sub>w</sub>
HP 14×117	121/2''	1''	7/8′′	7¾′′	5/8′′	1/2"
×102	121/2''	7/8′′	3/4′′	73/4''	5/8′′	1/2"
×89	121/2"	3/4′′	11/16 ''	73/4′′	5/8′′	1/2"
×73	121/2''	5/8′′	% ''	73/4′′	5/8′′	1/2"
HP 12×84	10′′	7/8′′	11/16 ''	61/2"	5/8′′	1/2"
×74	10′′	7/8′′	11/16 ′′	61/2"	5/8′′	1/2"
×63	10′′	5/8′′	1/2"	61/2''	1/2"	3/8′′
×53	10′′	5/8′′	1/2"	61/2"	1/2"	3/8′′
HP 10×57	8′′	3/4′′	% ''	51/4′′	1/2"	3/8′′
×42	8′′	5/8′′	% ''	51/4"	1/2"	3/8′′
HP 8×36	7′′	5/8′′	⅓ <sub>6</sub> ′′	41/4''	1/2"	3/8′′

II

END VIEW

TYP. ALONG FOUREDGES OF FLANGE ₱

# WELDED PLATE FIELD SPLICE

# WELDED COMMERCIAL SPLICE ALTERNATE

- \* INTERRUPT WELDS 1/4" FROM END OF WEB AND/OR EACH FLANGE.
- \*\* REMOVE PORTIONS OF BACKUP PLATES THAT EXTEND OUTSIDE THE FLANGES.
- ••• WELD SIZE PER PILE SHOE MANUFACTURER (5/6" MIN.).

THE STEEL H-PILES SHALL BE ACCORDING TO AASHTO M270 GRADE 50.

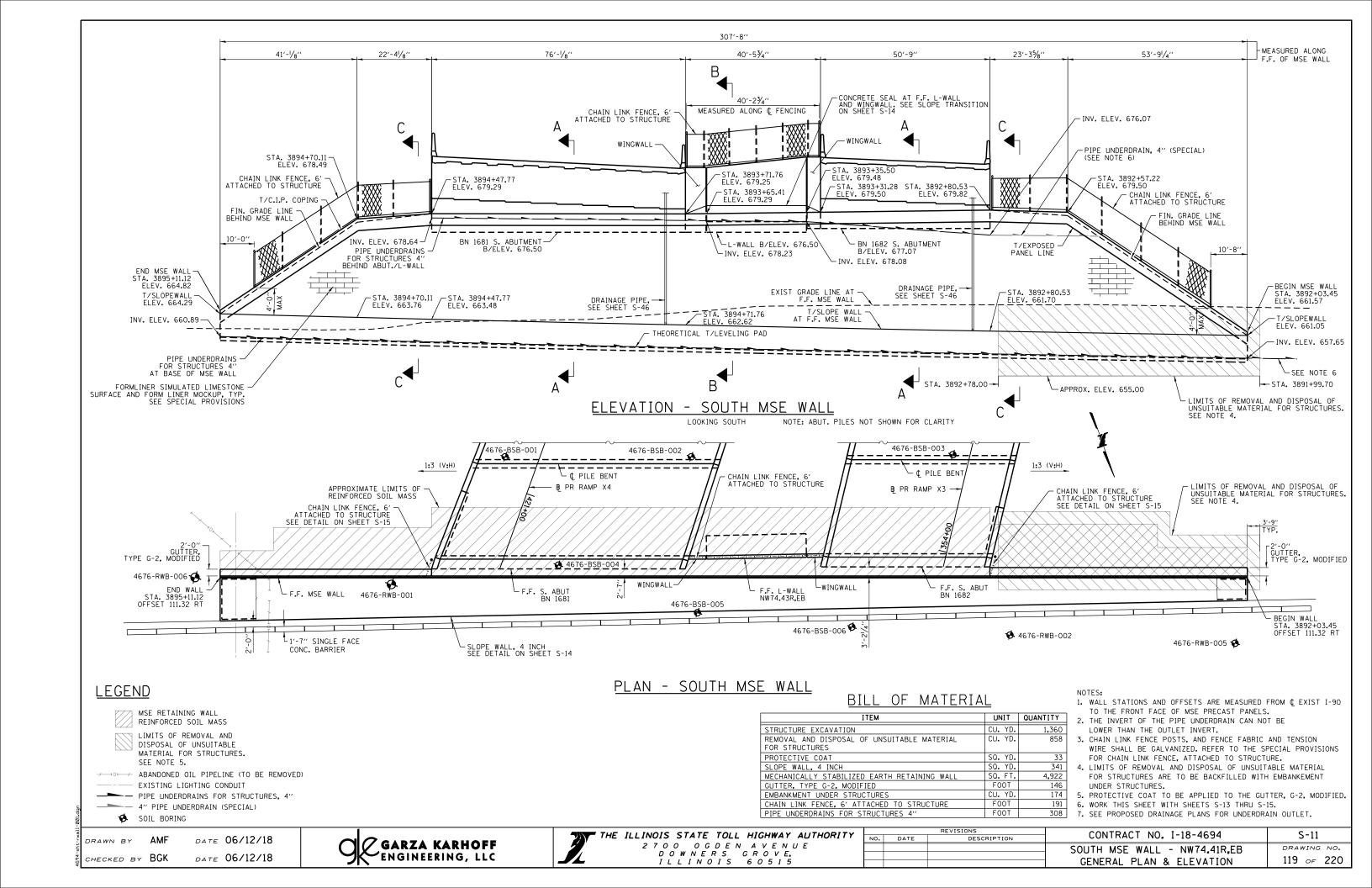
DRAWN BY FD DATE 06/12/18 DATE 06/12/18 CHECKED BY KK

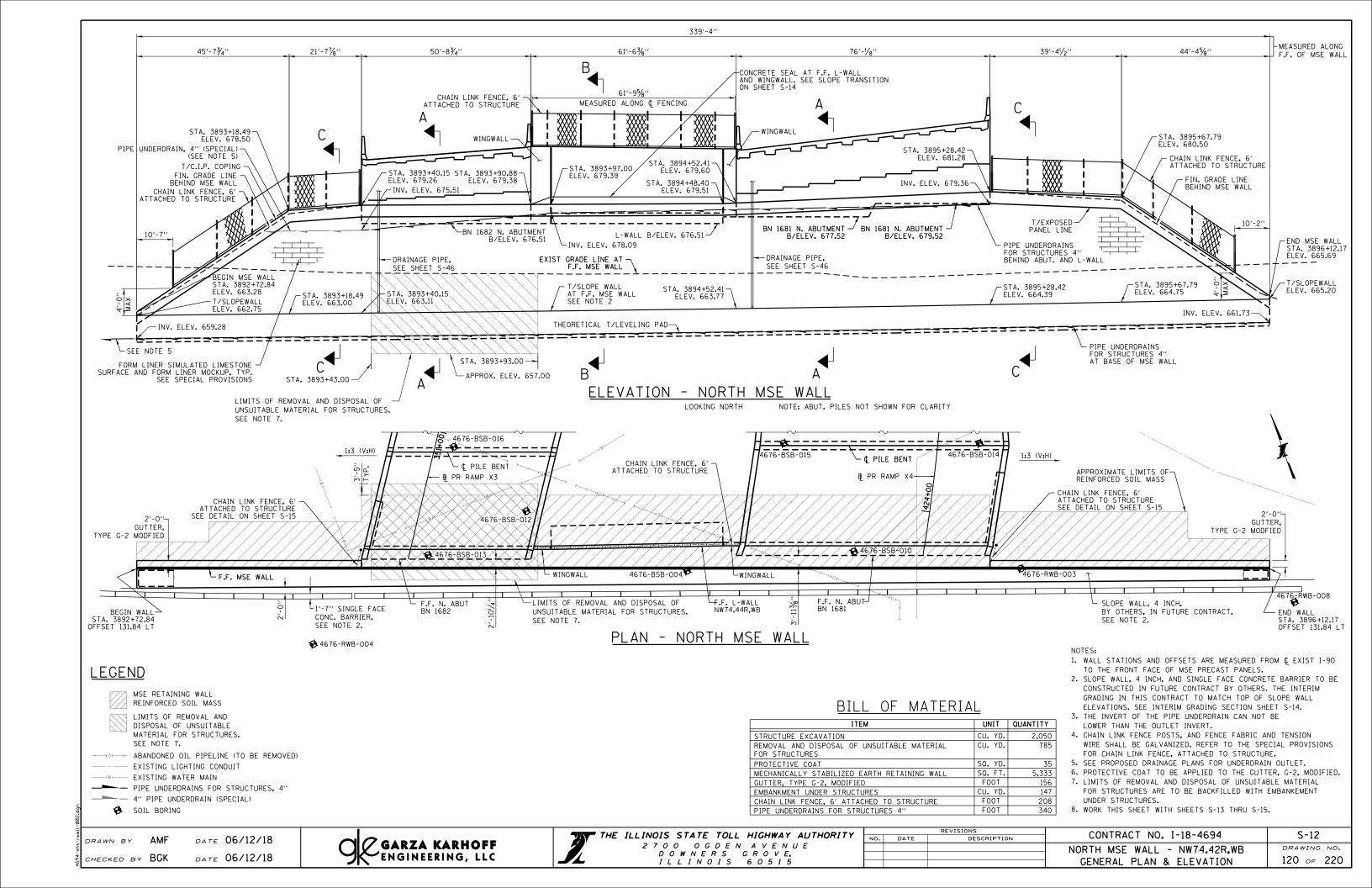


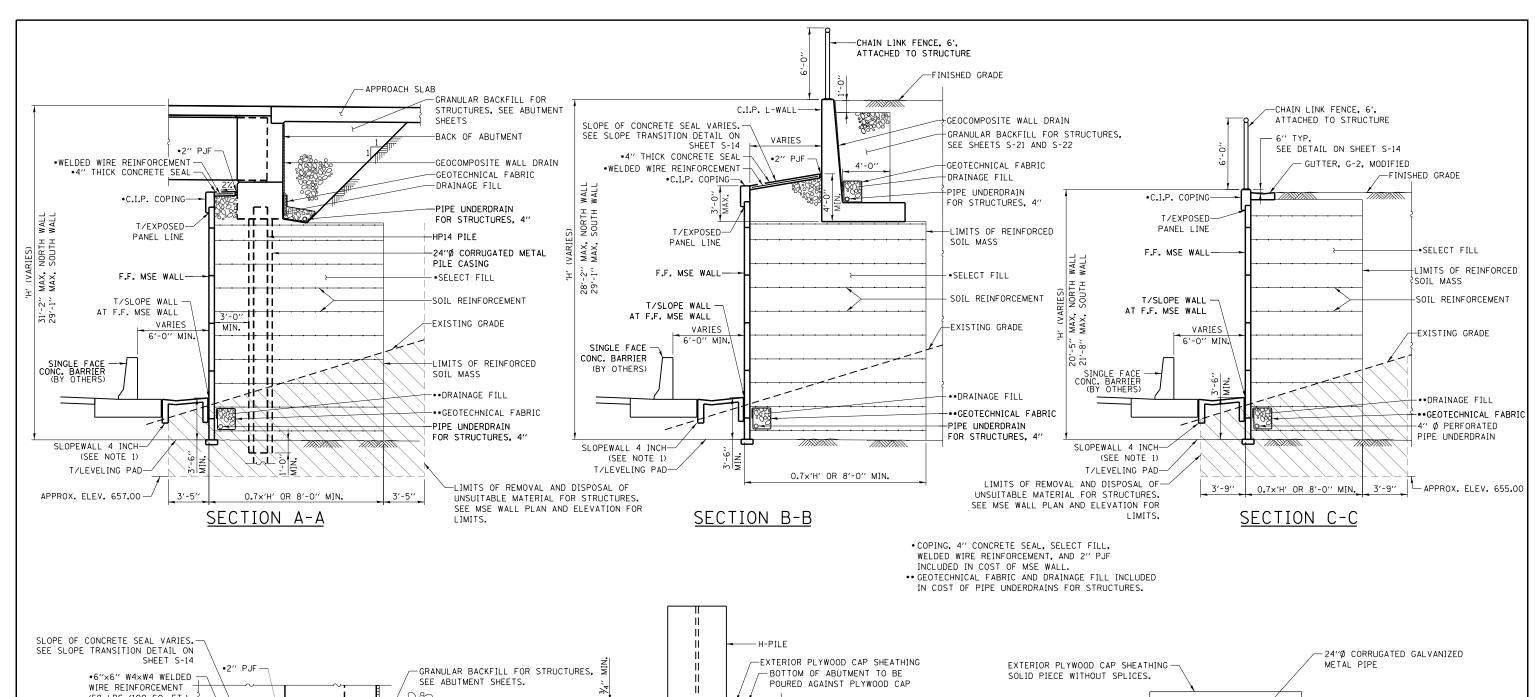
exp U.S. Services Inc. BUILDINGS • EARTH & ENVIRONMENT • ENERGY INDUSTRIAL • INFRASTRUCTURE • SUSTAINABILITY

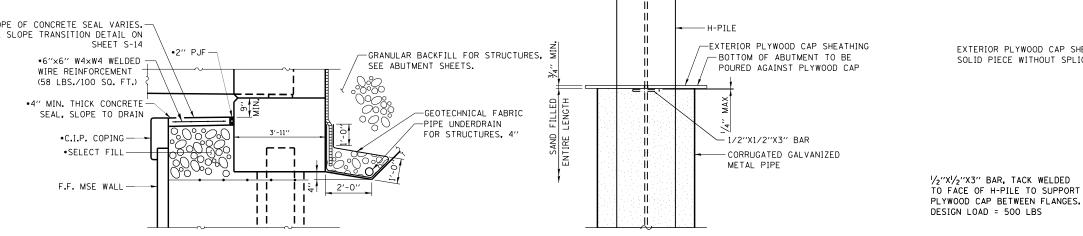


NO. DATE DESCRIPTION CONTRACT NO. I-18-4694 S-10  BRIDGE NOS. 1681 & 1682  DRAWING DESCRIPTION CONTRACT NO. I-18-4694	
BRIDGE NOS 1681 & 1682 DRAWING	
	NO.
110	220
PILE DETAILS 118 OF 2	220









PILE CASING - PLAN

NOTES:

1. CONCRETE SLOPEWALL AT NORTH MSE WALL AND SINGLE FACE BARRIER WALLS ARE TO BE CONSTRUCTED IN A FUTURE CONTRACT. SEE INTERIM GRADING DETAIL ON SHEET S-14.

1/8" MAX GAP SEALED WITH

SILICONE CAULK, TYP.

2" MIN. CLEARANCE, TYP.

-H-PILE

DRAWN BY AMF DATE 06/12/18
CHECKED BY BGK DATE 06/12/18

GARZA KARHOFF ENGINEERING, LLC

SECTION THRU ABUTMENT

THE ILLINOIS STATE TOLL HIGHWAY AUTHORITY

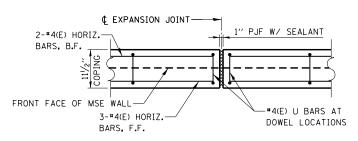
2 7 0 0 0 G D E N A V E N U E

D 0 W N E R S G R O V E,

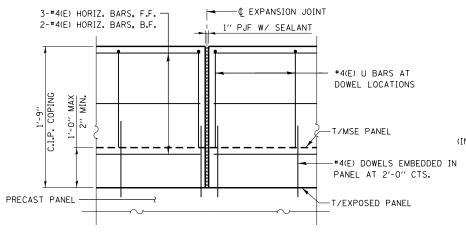
I L L I N O I S 6 0 5 1 5

PILE CASING - ELEVATION

		REVISIONS	CONTRACT NO. I-18-4694	S-13
NO.	DATE	DESCRIPTION	CUNTRACT NO. 1-10-4694	3-13
			MSE WALL	DRAWING NO.
			SECTIONS & DETAILS 1	121 of 220
			020110110 01 02111120 1	

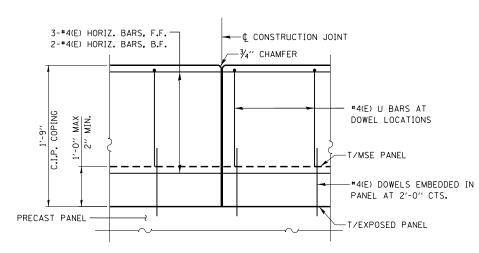


## COPING EXPANSION JOINT - PLAN



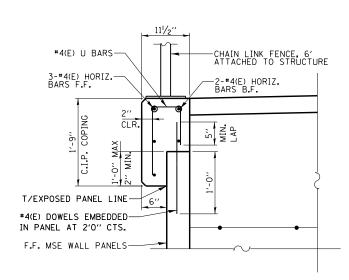
# COPING EXPANSION JOINT - ELEVATION

PROVIDE EXPANSION JOINTS AT MAX SPACING OF 90'-0"



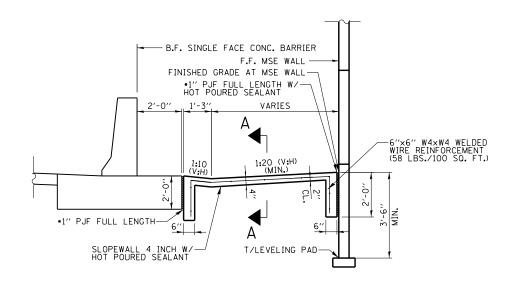
# COPING CONSTRUCTION JOINT - ELEVATION

PROVIDE CONSTRUCTION JOINTS AT MAX SPACING OF 30'-0"



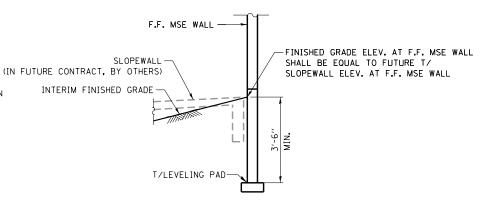
# C.I.P. COPING DETAIL

SEE NOTES 1 AND 2

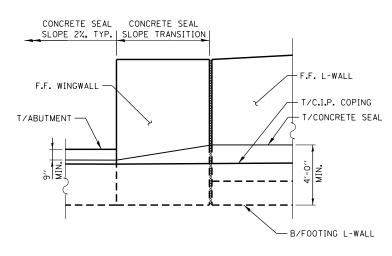


## SECTION THROUGH SLOPEWALL

HORIZONAL DIMENSIONS MEASURED NORMAL TO CD ROAD
• 1" PJF COST INCLUDED WITH SLOPEWALL 4 INCH

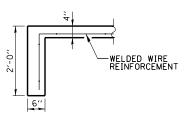


# INTERIM GRADING AT NORTH MSE WALL



# CONCRETE SEAL SLOPE TRANSITION

NOT TO SCALE



SECTION A-A
AT ENDS OF SLOPEWALL

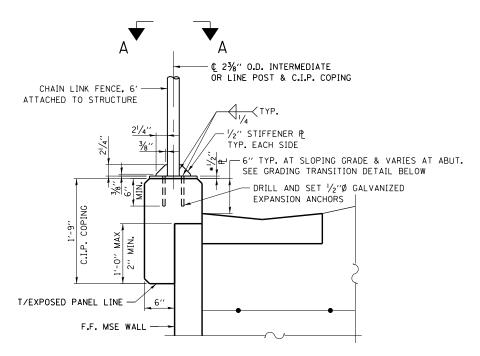
#### NOTES

- 1. THE MSE WALL SUPPLIER IS RESPONSIBLE FOR THE DESIGN OF THE REINFORCING BARS IN C.I.P. COPING. THE REINFORCEMENT IN C.I.P. COPING SHOWN IS MINIMUM REQUIRED.
- 2. COST OF CAST-IN-PLACE COPING, CONCRETE SEAL, AND REINFORCING BARS IS INCLUDED WITH THE COST OF MECHANICALLY STABILIZED EARTH RETAINING WALL.

DRAWN BY AMF DATE 06/12/18
CHECKED BY BGK DATE 06/12/18

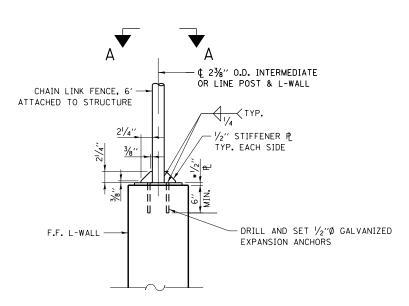
GARZA KARHOFF ENGINEERING, LLC

		REVISIONS	CONTRACT NO. I-18-4694	S-14
NO.	DATE	DESCRIPTION	CUNTRACT NO. 1-10-4634	3-14
			MSE WALL	DRAWING NO.
			- = - =	122 220
			SECTIONS & DETAILS 2	122 of 220

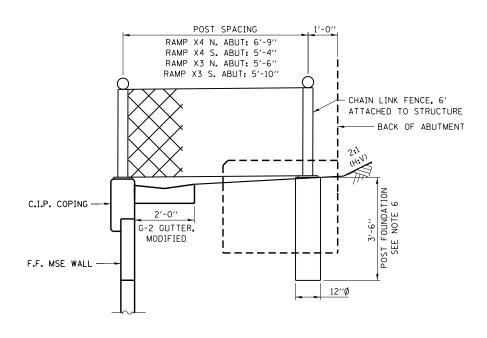


\*SHIM AS REQUIRED

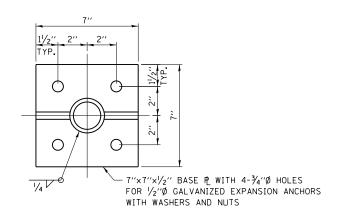
# CHAIN LINK FENCE ATTACHED TO COPING



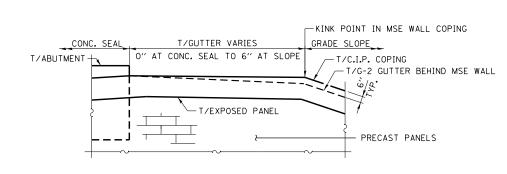
CHAIN LINK FENCE ATTACHED TO L-WALL



CHAIN LINK FENCE AT ABUTMENT WINGWALLS - ELEVATION



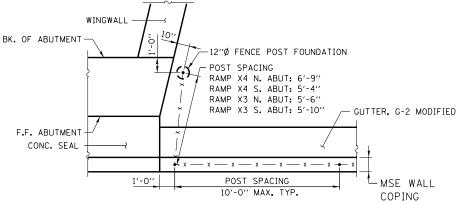
SECTION A-A



# GRADING TRANSITION BEHIND MSE WALL COPING

NOT TO SCALE CHAIN LINK FENCE NOT SHOWN FOR CLARITY

- 1. IF NECESSARY, THE SIZE OF THE BASE PLATE AND LOCATION OF
- THE EXPANSION ANCHORS MAY BE ADJUSTED TO MISS THE WALL REINFORCEMENT. 2. BASE PLATES AND STIFFENERS SHALL BE FABRICATED FROM MATERIAL MEETING THE REQUIREMENTS OF AASHTO GRADE 36.
- 3. BASE PLATES, STIFFENERS, AND POSTS SHALL BE HOT DIPPED GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH AASHTO M111.
- MSE WALLS. ANCHOR BOLTS SHALL BE A MINIMUM 6 INCHES FROM JOINTS. POST SPACING SHALL BE NO LONGER THAN 10 FEET. CONTRACTOR SHALL SUBMIT FENCE



CHAIN LINK FENCE AT ABUTMENT WINGWALLS - PLAN

- 5. COST OF BASE PLATES, STIFFENERS, AND EXPANSION ANCHORS FOR MOUNTING FENCE ONTO C.I.P. COPING, L-WALL, WINGWALL, AND CONCRETE POST FOUNDATION IS
- INCLUDED IN COST OF CHAIN LINK FENCE, 6' ATTACHED TO STRUCTURE. 6. CONCRETE POST FOUNDATION SHALL BE OF CLASS SI CONCRETE MEETING THE REQUIREMENTS OF SECTION 1020 OF THE STANDARD SPECIFICATIONS. THE TOP OF FOUNDATION SHALL BE SLIGHTLY ABOVE THE GROUND LINE AND SHALL BE TROWELED TO A SMOOTH FINISH. POST SHALL BE CENTERED ON THE FOUNDATION. COST OF POST FOUNDATION IS INCLUDED IN THE COST OF CHAIN LINK FENCE, 6' ATTACHED TO STRUCTURE.

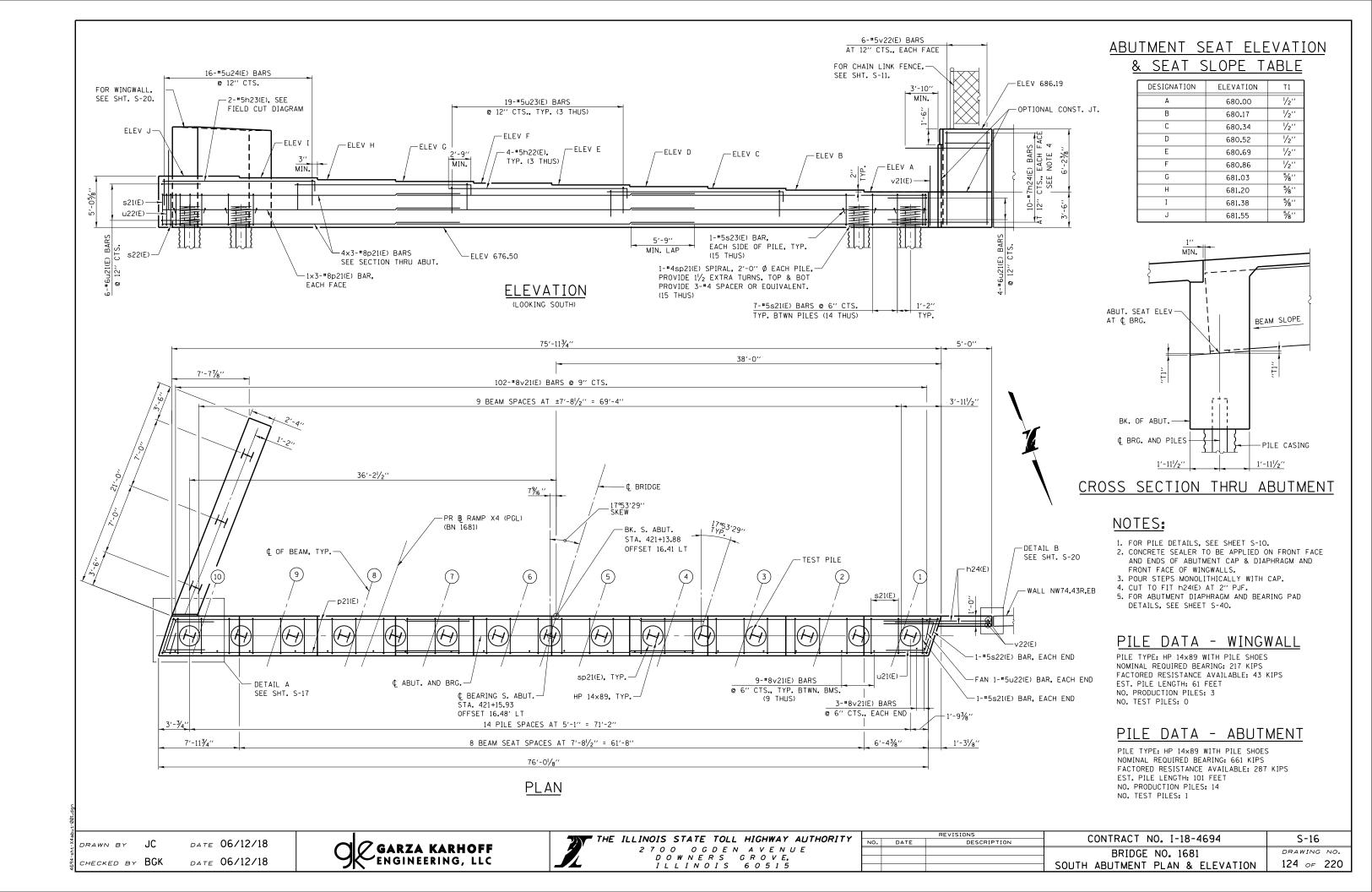
4. CHAIN LINK FENCE SHALL BE MOUNTED TO THE TOP OF WINGWALLS, L-WALLS, AND

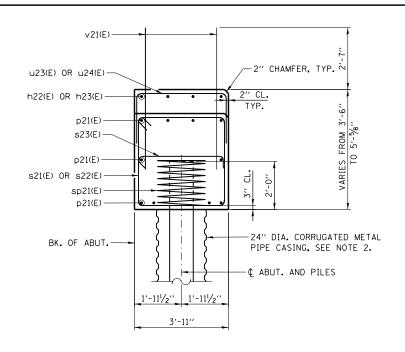
SHOP DRAWINGS FOR REVIEW AND APPROVAL.

AMF DATE 06/12/18 DATE 06/12/18 CHECKED BY BGK

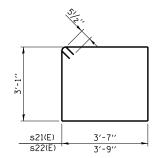
**GARZA KARHOFF** ENGINEERING, LLC

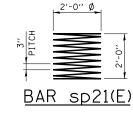
CONTRACT NO. I-18-4694	S-15
SCRIPTION CUNTRACT NO. 1-10-4694	3-15
MSE WALL	DRAWING NO.
	127 220
SECTIONS & DETAILS 3	123 of 220



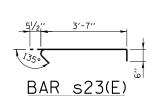


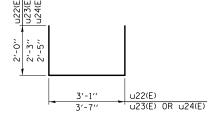
# CROSS SECTION THRU ABUT. (DIMENSIONS AT RIGHT ANGLE TO ABUTMENT)



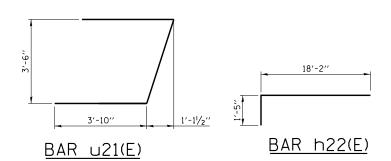


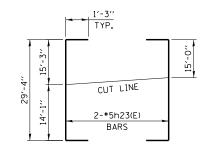
BAR s21(E) OR s22(E)





BAR u22(E), u23(E) OR u24(E)





1'-13/8'

# BAR h23(E)

# FIELD CUTTING DIAGRAM ORDER h23(E) BARS FULL LENGTH.

# BILL OF MATERIAL

	BAR	No.	SIZE	LENGTH	SHAPE
	h21(E)	14	#7	20'-6''	
	h22(E)	12	#5	19'-7''	1
L	h23(E)	2	#5	31′-10′′	
L	h24(E)	20	#7	9'-0''	
L	h25(E)	14	#7	19′-7′′	
L					
ŀ	p21(E)	30	#8	29′-1′′	
t	s21(E)	100	#5	14'-3''	
Г	s22(E)	2	#5	14'-7''	
F	s23(E)	30	#5	4'-7''	
ŀ	sp21(E)	15	#4	2'-0''	₩
ŀ	u21(E)	10	#6	11'-4''	<del></del>
t	u22(E)	2	#5	7'-1''	_=
t	u23(E)	57	#5	8'-1''	
t	u24(E)	16	#5	8'-5''	
r					
r	v21(E)	189	#8	5′-11′′	
	v22(E)	12	#5	9'-4''	
L	v23(E)	52	#5	9'-4''	
F		<u> </u> DESCRIPTIO	<u> </u> N	UNIT	QUANTITY
Ī	CONCRET	E STRUCTU	RES	CU YD	66.7
	REINFORCEMENT BARS, EPOXY			POUND	10,280
	FURNISHI HP14X89	NG STEEL	PILES	FOOT	1,597
Г	DRIVING	PILES		FOOT	1,597
L	TEST PIL	LE STEEL H	HP14X89	EACH	1
Ĺ	PILE SHO	DES		EACH	18
	CONCRET	E SEALER		SQ FT	1,027
	GEOCOMPOSITE WALL DRAIN			SQ YD	80
	GRANULAR BACKFILL FOR STRUCTURES			CU YD	190
	PIPE UNDERDRAINS FOR STRUCTURES 4"			FOOT	81
	PILE CAS METAL P	SING, CORR IPE, 24"	UGATED	FOOT	342

\* LENGTH IS HEIGHT OF SPIRAL.

## NOTES:

- 1. SEE MSE WALL SHEETS S-11 TO S-13 FOR ABUTMENT DRAINAGE DETAILS.
- 2. SEE MSE WALL SHEET S-13 FOR TYPICAL SECTION THRU ABUTMENT AND MSE WALL.

DRAWN BY JC DATE 06/12/18
CHECKED BY BGK DATE 06/12/18





-WINGWALL

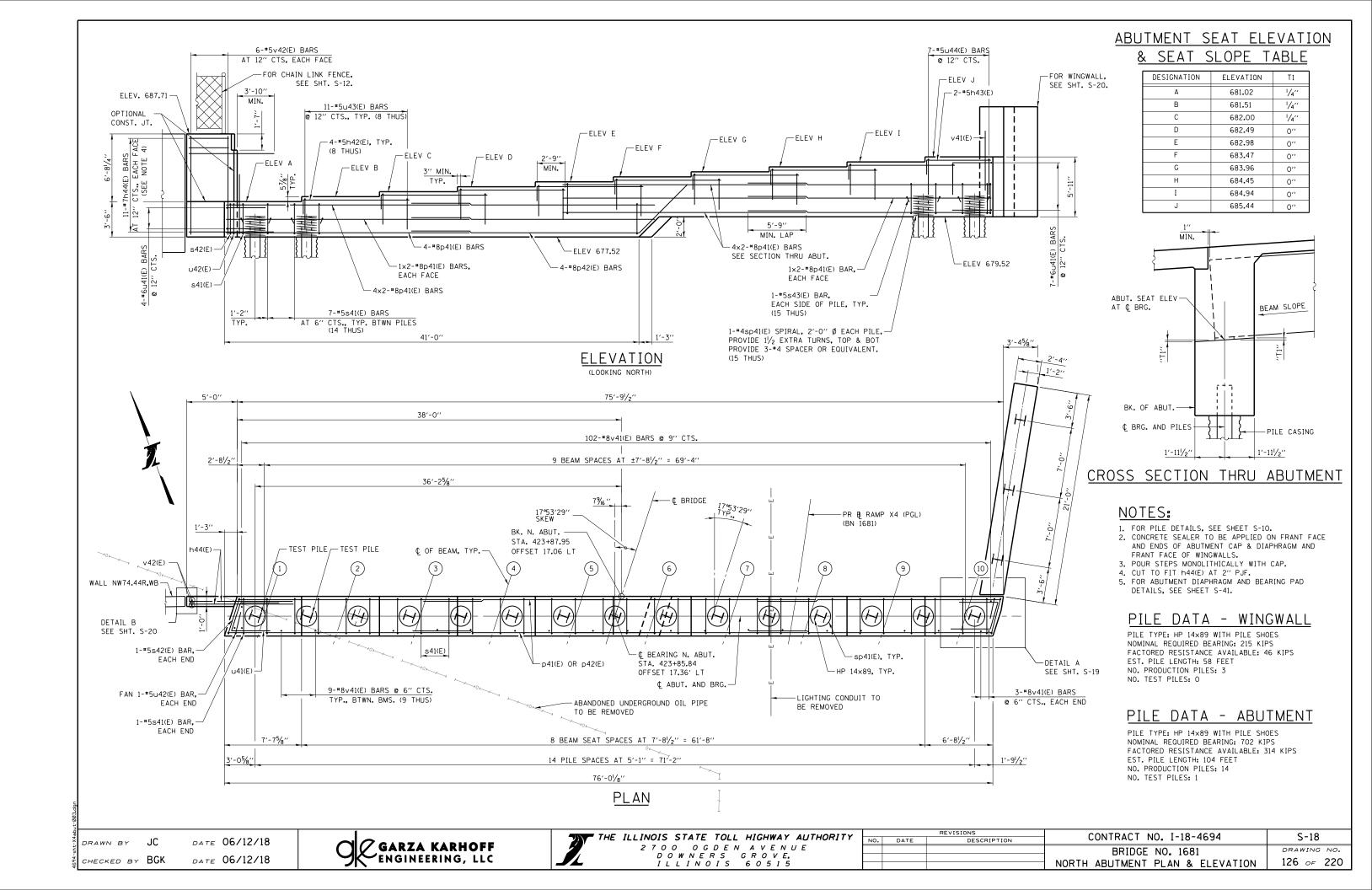
-¢ BRG. AND PILES

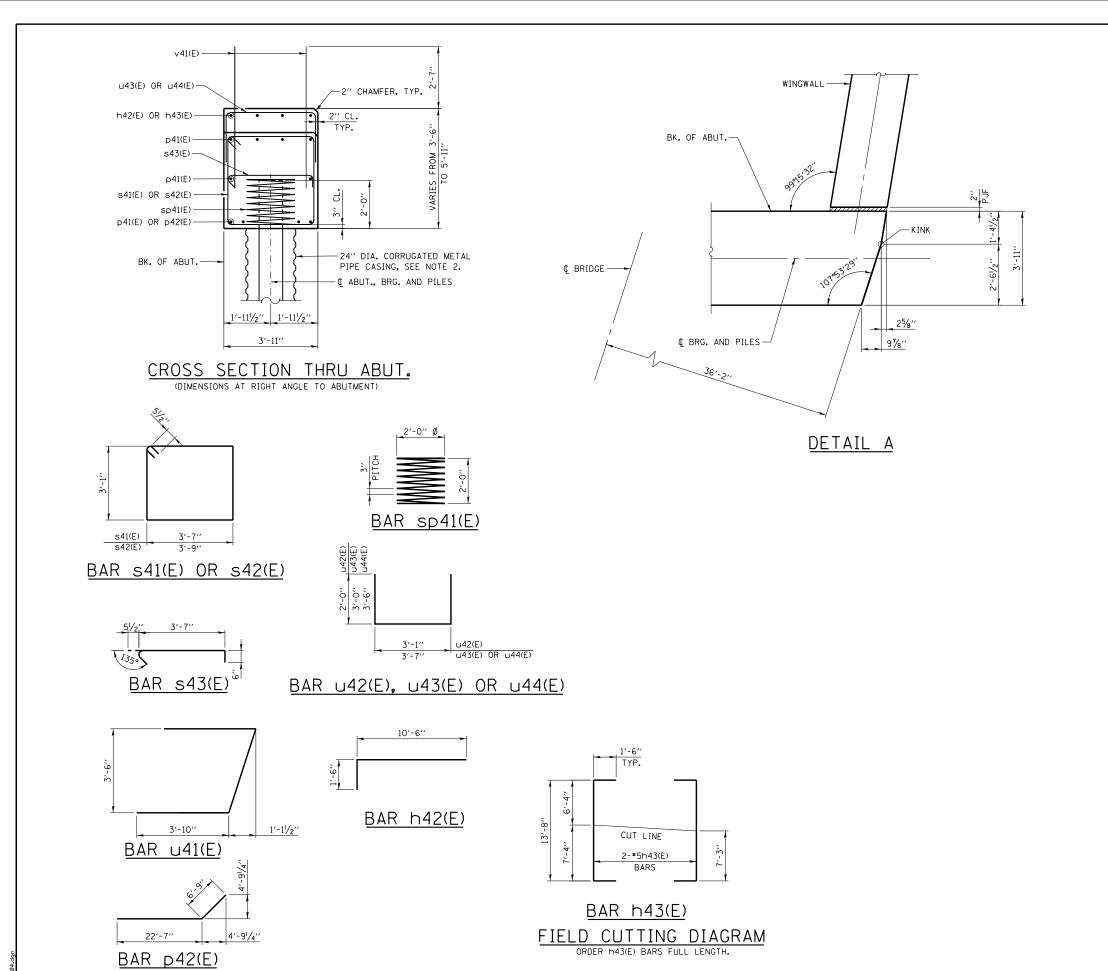
DETAIL A

-BK. OF ABUT.

-¢ BRIDGE

	REVISIONS		CONTRACT NO. I-18-4694	l 5-17 l
NO.	DATE	DESCRIPTION	CONTRACT NO. 1-10-4634	3-11
			BRIDGE NO. 1681	DRAWING NO.
				125 oF 220
			SOUTH ABUTMENT DETAILS	125 of 220





#### BILL OF MATERIAL

	BAR	No.	SIZE	LENGTH	SHAPE
١	h41(E)	16	#7	20'-6''	
	h42(E)	32	<b>#</b> 5	12'-0''	
-	h43(E)	2	#5	16′-8′′	
-	h44(E)	22	#7	8′-8′′	
	h45(E)	16	#7	20'-10''	
١					
	p41(E)	36	#8	24'-1''	
١	p42(E)	4	#8	29'-4''	
١	s41(E)	100	#5	14'-3''	<u>I</u>
	s42(E)	2	#5	14'-7''	<u> </u>
-	s43(E)	30	#5	4'-7''	
١	44.5	4.5		0. 0	=
*	sp41(E)	15	#4	2'-0''	₹
-		11	#6	11/ ///	
-	u41(E) u42(E)	11	#5	11'-4'' 7'-1''	=
-	u43(E)	88	#5	9'-7''	=
-	u44(E)	7	#5	10'-7''	=
-	U44(E)	1		10 - 1	
ł	v41(E)	189	#8	5′-11′′	
1	v42(E)	12	#5	9'-10''	
ı	∨43(E)	52	#5	10'-6''	
1	V 1012	32		10 0	
ı		ESCRIPTIO	N	UNIT	QUANTITY
١	CONCRET	E STRUCTU	RES	CU YD	75.1
	REINFORG COATED	CEMENT BAI	RS, EPOXY	POUND	11,350
	FURNISHI HP14X89	NG STEEL	PILES	FOOT	1,630
-	DRIVING	PILES		FOOT	1,630
-	TEST PIL	E STEEL H	HP14X89	EACH	1
	PILE SHO	)ES		EACH	18
	CONCRET	E SEALER		SQ FT	1,133
	GEOCOMP	OSITE WAL	L DRAIN	SQ YD	81
	GRANULAR BACKFILL FOR STRUCTURES			CU YD	222
	PIPE UNI STRUCTU	DERDRAINS RES 4"	FOOT	81	
	PILE CAS METAL P	SING, CORR IPE, 24"	UGATED	FOOT	345

\* LENGTH IS HEIGHT OF SPIRAL.

# NOTES:

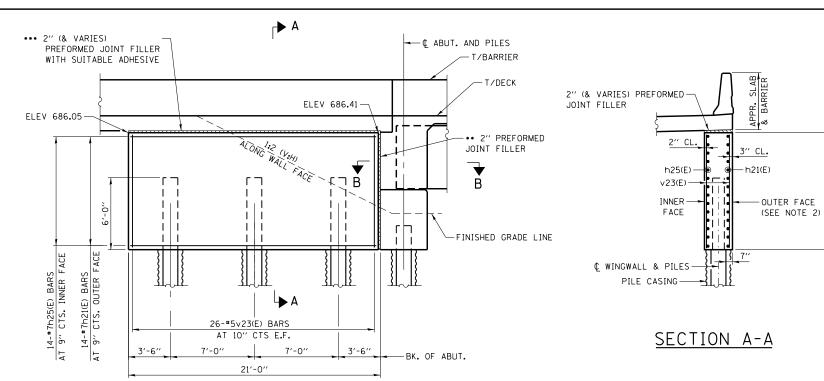
- SEE MSE WALL SHEETS S-11 TO S-13 FOR ABUTMENT DRAINAGE DETAILS.
- 2. SEE MSE WALL SHEET S-13 FOR TYPICAL SECTION THRU ABUTMENT AND MSE WALL.

DRAWN BY JC DATE 06/12/18
CHECKED BY BGK DATE 06/12/18

GARZA KARHOFF ENGINEERING, LLC



		REVISIONS	CONTRACT NO. I-18-4694	S-19
NO.	DATE	DESCRIPTION	CUNTRACT NO. 1-10-4634	3-19
			BRIDGE NO. 1681	DRAWING NO.
				127 of 220
			NORTH ABUTMENT DETAILS	121 of 220



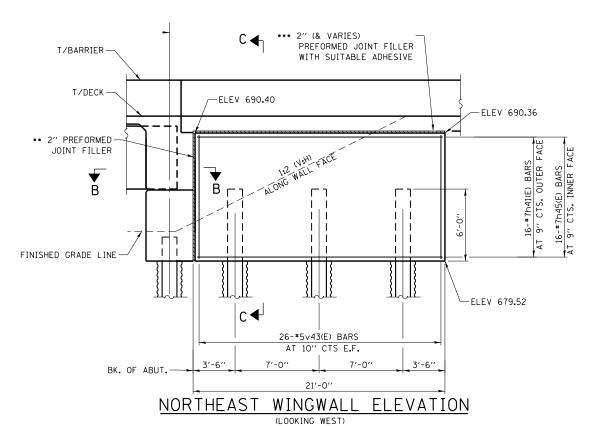
GEOCOMPOSITE WALL DRAIN-3/8"×1'-4" NEOPRENE SHEET (55 DUROMETER) ATTACHED -FULL HEIGHT AT EDGES TO THE END DIAPHRAGM/BACKWALL AND WINGWALL WITH A  $\frac{3}{8}$ "x5" STEEL PLATE AND  $\frac{1}{2}$ " Ø ANCHOR BOLTS, NUTS AND WASHERS AT 12" CTS. VERTICALLY. INCLUDED WITH THE COST OF HIGH PERFORMANCE CONCRETE SUPERSTRUCTURE. 2" MIN. PREFORMED JOINT FILLER WINGWALL END DIAPHRAGM/ BACKWALL

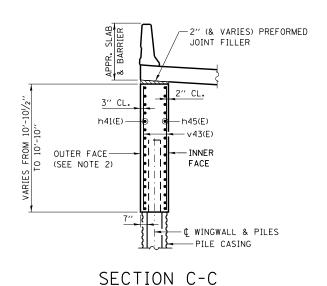
# SECTION B-B

SOUTH ABUTMENT SECTION SHOWN, NORTH ABUTMENT SECTION SIMILAR.

#### SOUTHEAST WINGWALL ELEVATION (LOOKING WEST)

- PREFORMED JOINT FILLER WITH CONCRETE FLAT HD. C.S. 21/2" LONG NAILS @ 12" STAGGERED CTS. VERTICALLY.
- \*\*\* ADHESIVE MUST BE COMPATIBLE WITH PREFORMED JOINT FILLER MATERIAL AND CONCRETE. SURFACE PREPARATION SHALL BE CONDUCTED IN ACCORDANCE WITH MANUFACTURER'S GUIDELINE.





6" HOLLOW BULB DUMBBELL TYPE NON-METALLIC WATER SEAL -GEOCOMPOSITE 6" FROM TOP OF WALL TO BOTTOM WALL DRAIN (COST INCLUDED WITH CONCRETE STRUCTURES) FRONT FACE CEMENT NAILS FLAT HD. C.S. 3" LONG — AT 12" CTS. VERTICAL EACH FACE (COST INCLUDED WITH CONCRETE STRUCTURES) 3/4" CHAMFER, TYP. 2" MIN. PREFORMED JOINT FILLER WINGWALL RETAINING WALL

#### DETAIL B

SOUTH ABUTMENT DETAIL SHOWN, NORTH ABUTMENT DETAIL SIMILAR.

# NOTES:

- 1. FOR REBAR DETAILS AND BILL OF MATERIAL, SEE SHEETS S-17 AND S-19.
- 2. FOR FORM LINER DETAILS, SEE SHEET S-05.
- 3. FOR PILE CASING DETAILS, SEE SHEET S-13.

\*\* PREFORMED JOINT FILLER WITH CONCRETE FLAT HD. C.S. 21/2" LONG NAILS @ 12" STAGGERED CTS. VERTICALLY.

\*\*\* ADHESIVE MUST BE COMPATIBLE WITH PREFORMED JOINT FILLER MATERIAL AND CONCRETE, SURFACE PREPARATION SHALL BE CONDUCTED IN ACCORDANCE WITH MANUFACTURER'S GUIDELINE.

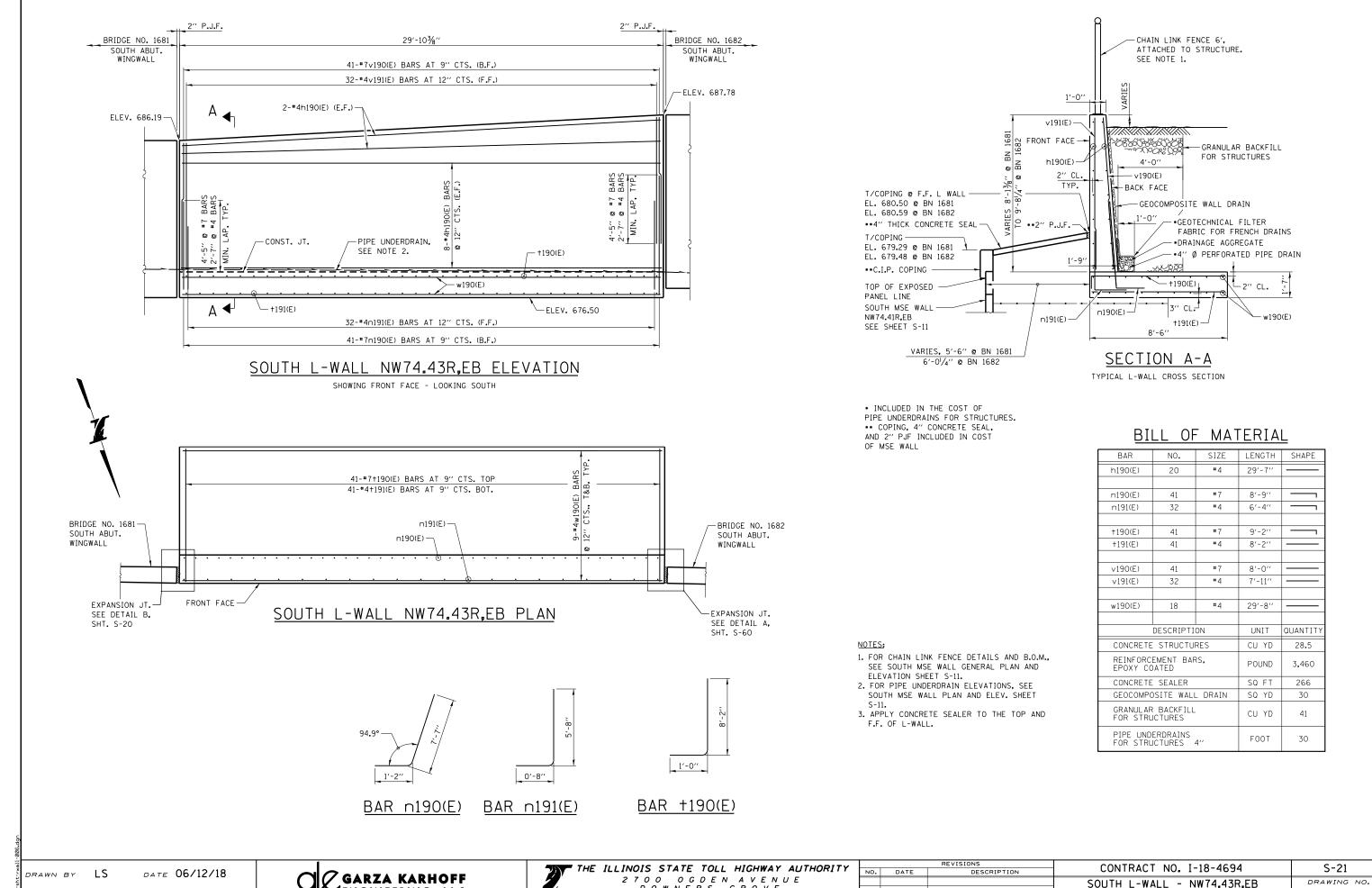
DATE 06/12/18 **GARZA KARHOFF** DATE 06/12/18

JC

CHECKED BY BGK



		REVISIONS	CONTRACT NO. I-18-4694	S-20
NO.	DATE	DESCRIPTION	CONTRACT NO. 1-10-4634	3-20
			BRIDGE NO. 1681	DRAWING NO.
				128 <i>o</i> ≥ 220
			WINGWALL DETAILS	128 of 220



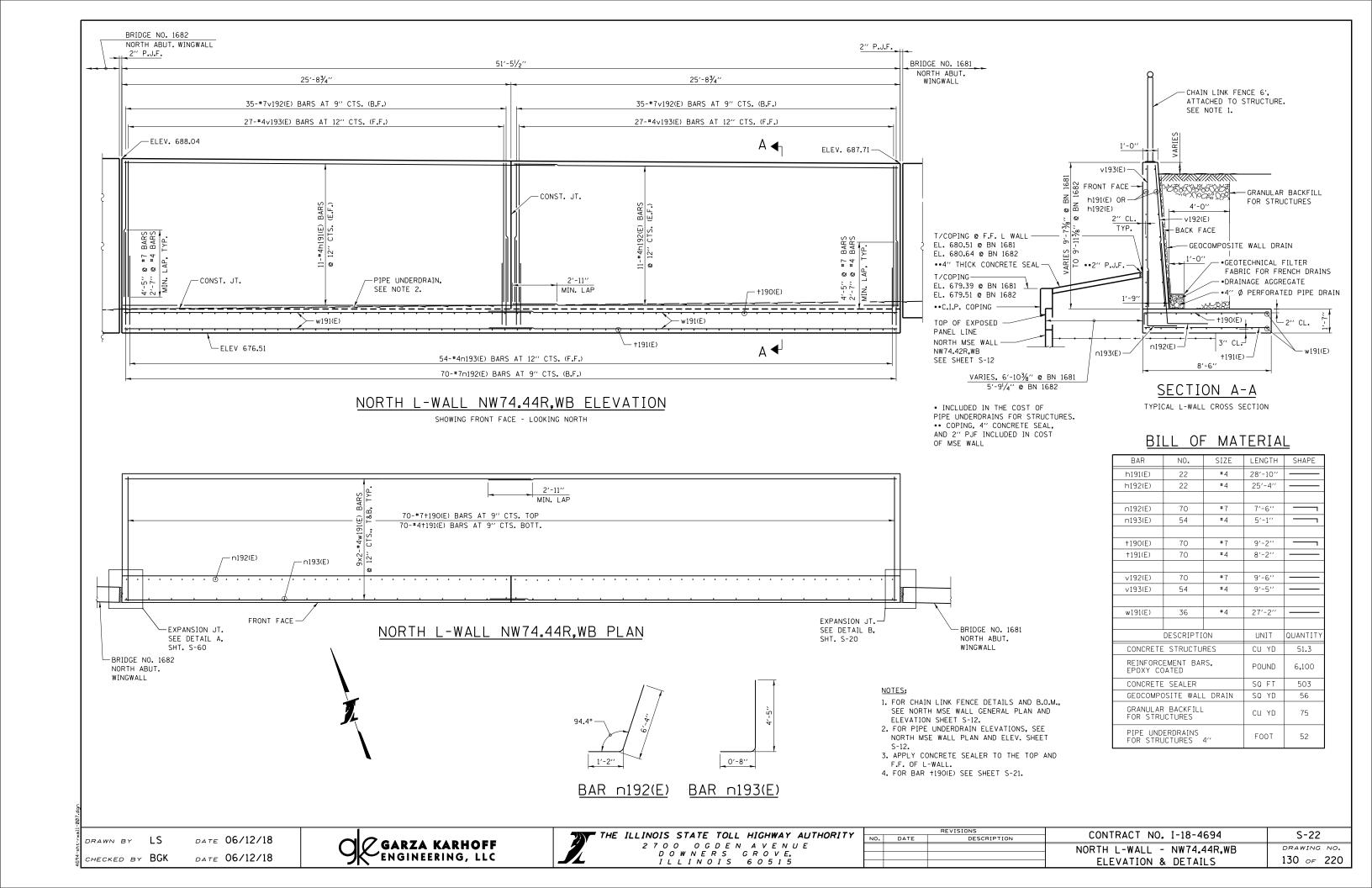
CHECKED BY BGK

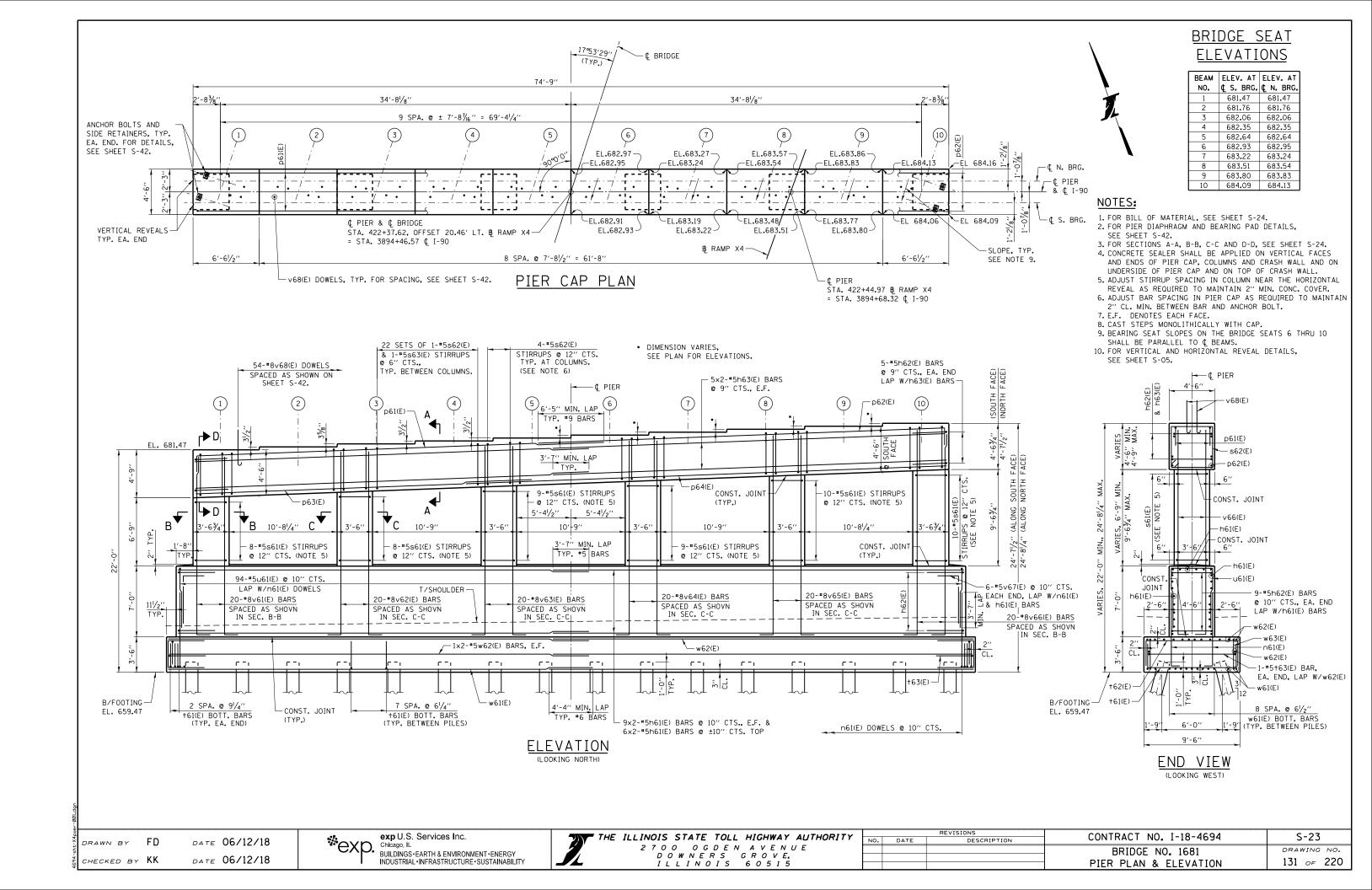
DATE 06/12/18

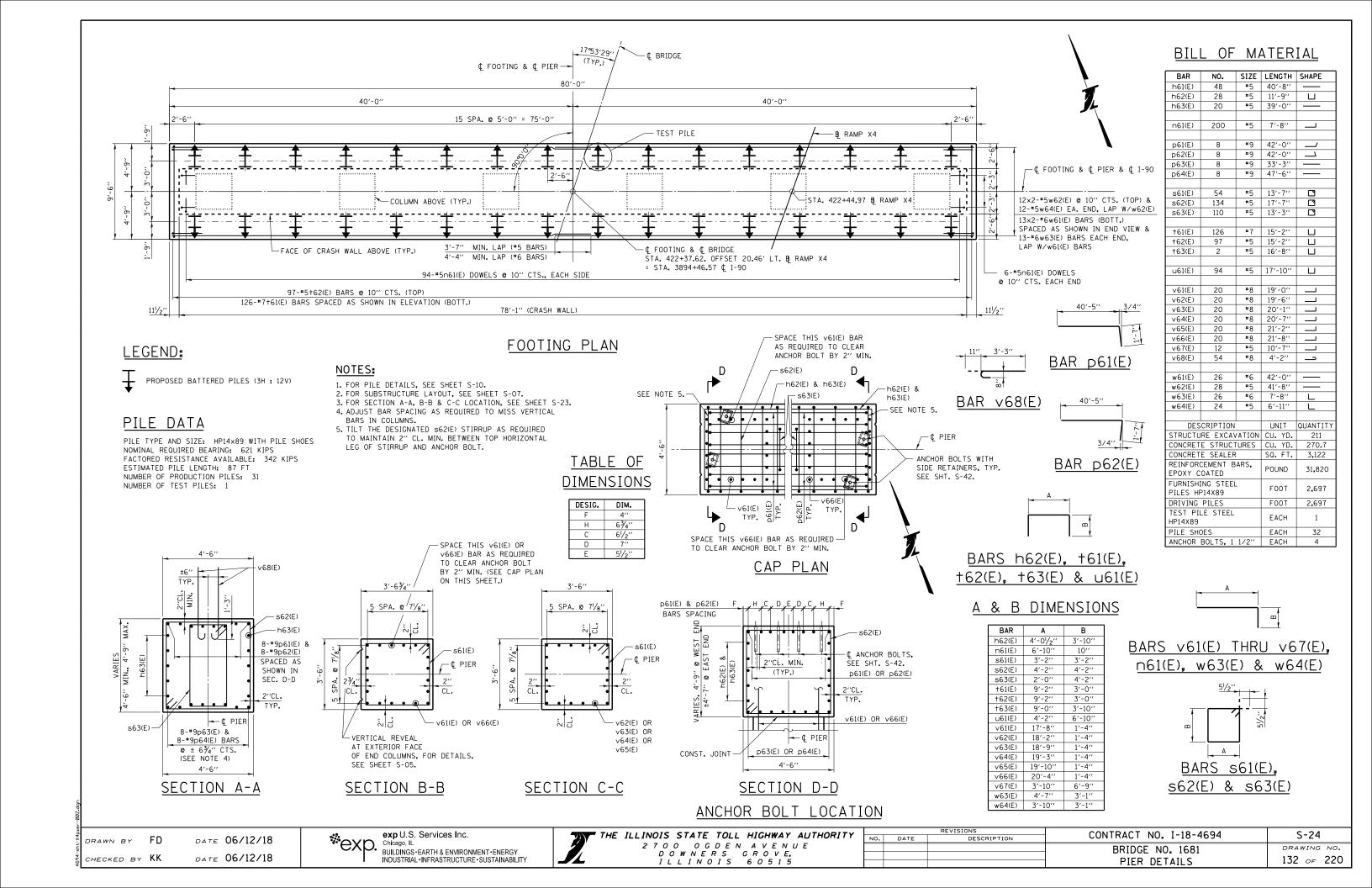
TENGINEERING, LLC

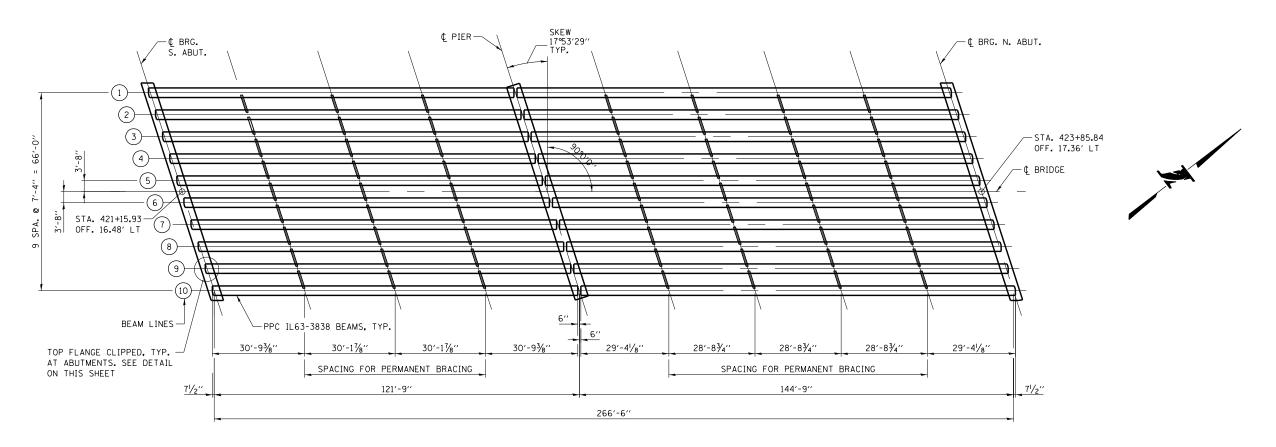
2700 OGDEN AVENUE DOWNERS GROVE, ILLINOIS 60515

		REVISIONS	CONTRACT NO. I-18-4694	l S-21
NO.	DATE	DESCRIPTION	CONTRACT NO. 1-10-4634	3-21
			SOUTH L-WALL - NW74.43R.EB	DRAWING NO.
				129 of 220
			ELEVATION & DETAILS	123 0+ 220









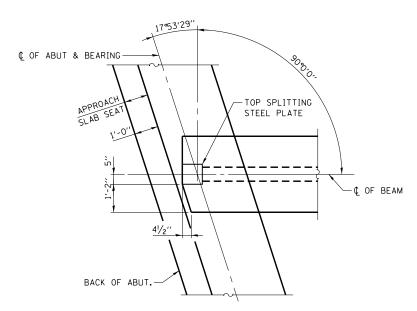
# FRAMING PLAN - RAMP X4

INTERIOR BEAM MOMENT TABLE						
0.45 SPAN 1 PIER 0.55 SPAN						
I	(IN.4)	527,741	527,741	527,741		
I,	(IN.4)	1,018,719	1,018,719	1,018,719		
Sb	(IN.3)	18,688	18,688	18,688		
Sb'	(IN.3)	24,901	24,901	24,901		
S†	(IN.3)	15,182	15,182	15,182		
S†'	(IN.3)	46,117	46,117	46,117		
DC1	(K/′)	2.021	2.021	2.021		
M DC1	(′K)	3,707	0	5,240		
DC2	(K/')	0.110	0.110	0.110		
M DC2	(′K)	89	-250	173		
DW	(K/′)	0.367	0.367	0.367		
M DW	(′K)	298	-833	577		
M LL + IM	(′K)	1,923	-2,159	2,251		

	INTERIOR BEAM REACTION TABLE						
			S. ABUT.	PIER	PIER	N. ABUT.	
				SPAN 1	SPAN 2		
	R DC1	(K)	123.0	123.0	146.3	146.3	
••	R DC2	(K)	4.6	9.2	9.2	6.2	
••	R DW	(K)	15.5	30.7	30.7	20.8	
**	R LL + IM	(K)	96.5	99.8	99.8	102.0	
	R TOTAL	(K)	239.6	262.7	286.0	275.3	

- BASED ON HL-93.
- \*\* AT CONTINUOUS PIER(S), REACTIONS FROM COMPOSITE LOADS ARE ASSUMED TO BE EQUALLY DISTRIBUTED TO EACH BEARING LINE.

- I NON-COMPOSITE MOMENT OF INERTIA OF BEAM SECTION (IN.4).
- I' COMPOSITE MOMENT OF INERTIA OF BEAM SECTION (IN.4).
- Sb NON-COMPOSITE SECTION MODULUS FOR THE BOTTOM FIBER OF THE PRESTRESSED BEAM (IN.3).
- Sb' COMPOSITE SECTION MODULUS FOR THE BOTTOM FIBER OF THE PRESTRESSED BEAM (IN. 3).
- S+ NON-COMPOSITE SECTION MODULUS FOR THE TOP FIBER OF THE PRESTRESSED BEAM (IN.3).
- COMPOSITE SECTION MODULUS FOR THE TOP FIBER OF THE PRESTRESSED BEAM (IN.3).
- DC1 UN-FACTORED NON-COMPOSITE DEAD LOAD (KIPS/FT.).
- M DC1 UN-FACTORED MOMENT DUE TO NON-COMPOSITE DEAD LOAD (KIP-FT.).
- DC2 UN-FACTORED LONG-TERM COMPOSITE (SUPERIMPOSED EXCLUDING FUTURE WEARING SURFACE) DEAD LOAD (KIPS/FT.).
- M DC2 UN-FACTORED MOMENT DUE TO LONG-TERM COMPOSITE (SUPERIMPOSED EXCLUDING FUTURE WEARING SURFACE) DEAD LOAD (KIP-FT.).
- DW UN-FACTORED LONG-TERM COMPOSITE (SUPERIMPOSED FUTURE WEARING SURFACE ONLY) DEAD LOAD (KIPS/FT.).
- M DW UN-FACTORED MOMENT DUE TO LONG-TERM COMPOSITE (SUPERIMPOSED FUTURE WEARING SURFACE ONLY) DEAD LOAD (KIP-FT.).
- M LL + IM UN-FACTORED LIVE LOAD MOMENT PLUS DYNAMIC LOAD ALLOWANCE (IMPACT) (KIP-FT.).

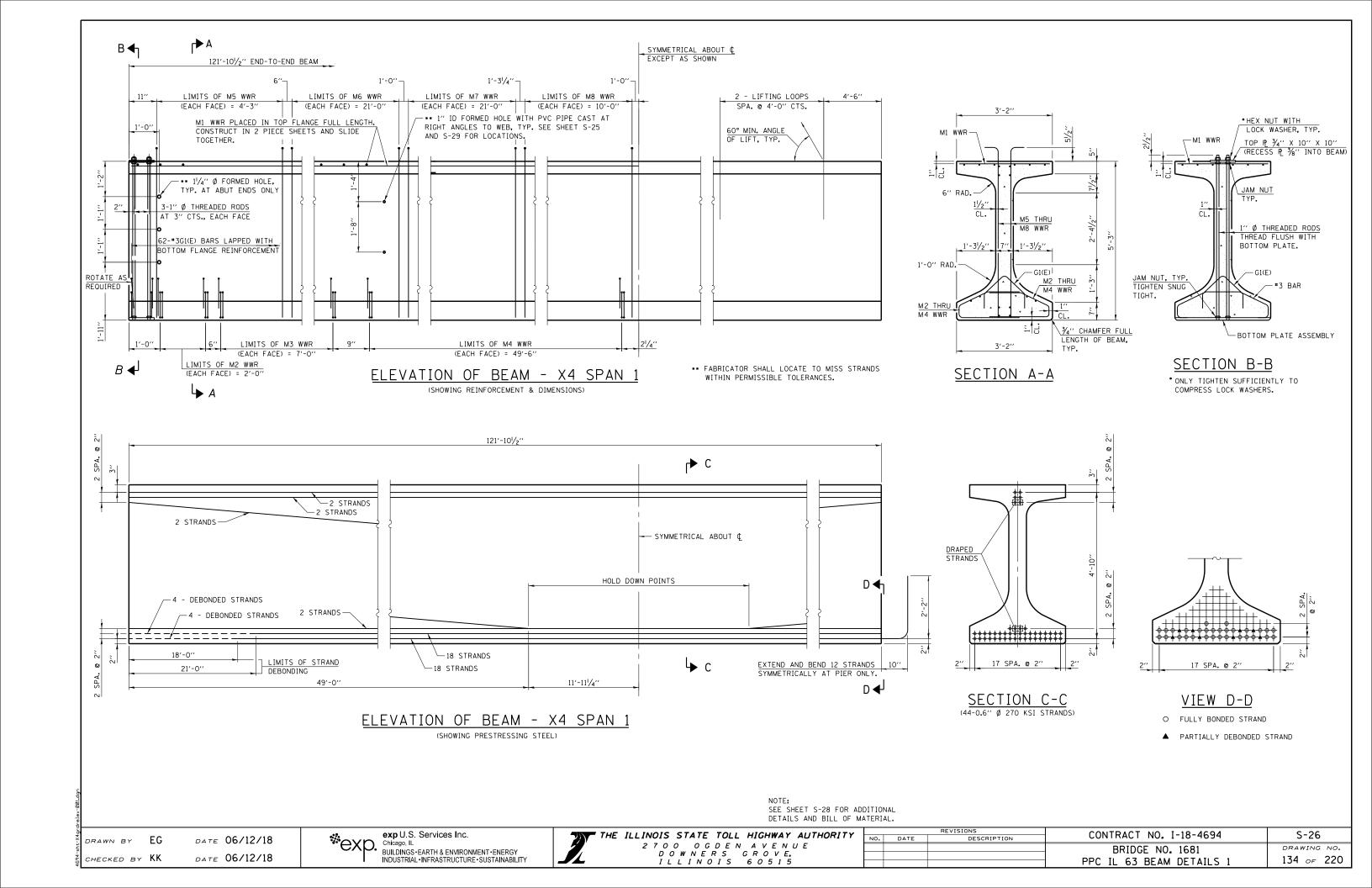


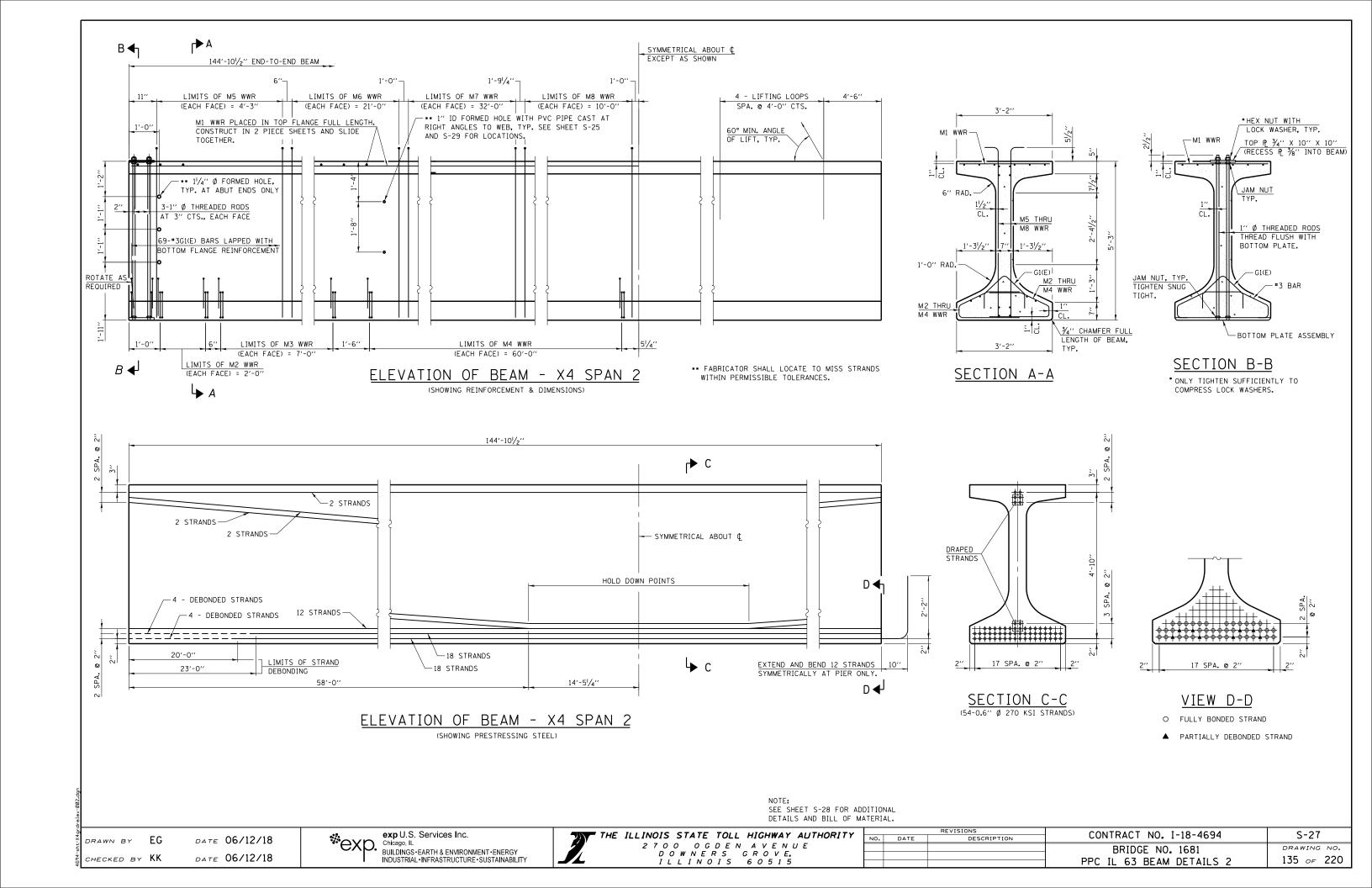
TOP FLANGE PLAN - CLIPPED

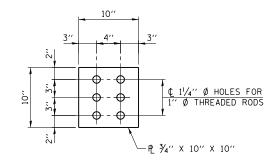
DRAWN BY EG DATE 06/12/18 DATE 06/12/18 CHECKED BY KK

exp U.S. Services Inc. \*\*exp. Chicago, IL BUILDINGS BUILDINGS EARTH & ENVIRONMENT ENERGY INDUSTRIAL INFRASTRUCTURE SUSTAINABILITY

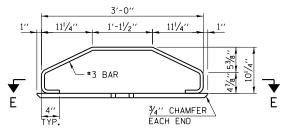
		REVISIONS	CONTRACT NO. I-18-4694	S-25
o. 2	DATE	DESCRIPTION	CUNTRACT NO. 1-10-4694	3-25
			BRIDGE NO. 1681	DRAWING NO.
			FRAMING PLAN	133 of 220



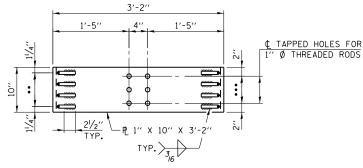




# PLAN - TOP PLATE



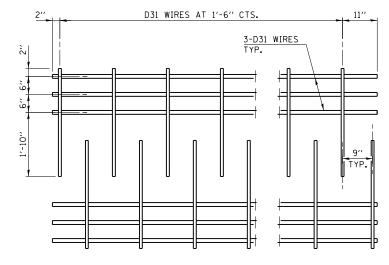
ELEVATION - BOTTOM PLATE ASSEMBLY



# SECTION E-E

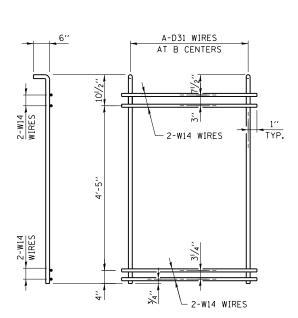
\*\* 3 SPACES AT 21/2" = 71/2"

\*\*\* 2 SPACES AT 3" = 6"



# M1 WWR DETAIL

WHEN MULTIPLE SHEETS OF MI WWR ARE REQUIRED ALONG THE BEAM LENGTH, #5(E) BARS (5'-0" LONG) SHALL BE USED TO SPLICE THE LONGITUDINAL D31 WIRES TOGETHER (MIN. LAP 2'-2").



#### M5 THRU M8 WWR DETAIL (SEE TABLE OF DIMENSIONS)

# TABLE OF DIMENSIONS

<u>SPAN 1</u>				
WWR	A	В		
M2	9	3''		
М3	15	6′′		
M4	34	1'-6''		
М5	18	3′′		
М6	43	6''		
М7	22	1'-0''		
М8	6	2'-0''		

SPAN 2					
WWR	Α	В			
M2	9	3''			
М3	15	6′′			
M4	41	1'-6''			
M5	18	3′′			
М6	43	6′′			
М7	33	1'-0''			
М8	6	2'-0''			

# 1'-03/4" A-D11 WIRES AT 111/4" B CENTERS 2-W4.5 WIRES-2-W4.5 WIRES TYP.

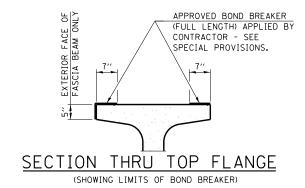
# M2 THRU M4 WWR DETAIL

# NOTES:

- 1. INSERTS FOR 3/4" Ø THREADED DOWEL RODS, WHEN SPECIFIED, ARE TO BE TWO STRUT, FERRULE TYPE FOR INTERIOR BEAMS AND SINGLE FERRULE, FLARED LOOP TYPE FOR EXTERIOR BEAMS.
- 2. PRESTRESSING STEEL SHALL BE UNCOATED HIGH STRENGTH, LOW RELAXATION 7-WIRE STRAND, GRADE 270. THE NOMINAL DIAMETER FOR BEAM STRANDS SHALL BE 0.6" AND THE NOMINAL CROSS-SECTIONAL AREA SHALL BE 0.217 SO. IN. THE NOMINAL DIAMETER FOR LIFTING LOOPS SHALL BE 1/2" AND THE NOMINAL CROSS SECTIONAL AREA SHALL BE 0.153 SQ. IN.
- 3. THE BEAMS SHALL HAVE A FINAL CONCRETE COMPRESSIVE STRENGTH, f'c, OF 8500 PSI AND A RELEASE CONCRETE COMPRESSIVE STRENGTH, f'ci, OF 7000 PSI.

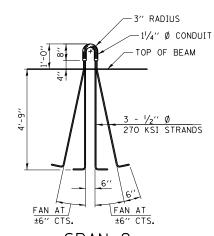
  4. A MINIMUM 21/2" Ø LIFTING PIN SHALL BE USED TO ENGAGE THE LIFTING LOOPS DURING
- HANDI ING.
- 5. BEND THE EXTENDED STRANDS INWARD ON THE FASCIA BEAMS TO MAINTAIN 11/2" CLEARANCE INSIDE THE PIER DIAPHRAGM.
- 6. THE TOP AND BOTTOM PLATES SHALL BE AASHTO M270 GRADE 50.
  7. THE TOP PLATES AND BOTTOM PLATE ASSEMBLIES SHALL BE GALVANIZED ACCORDING TO AASHTO M111. THE THREADED RODS, NUTS AND WASHERS SHALL BE GALVANIZED ACCORDING TO AASHTO M232.
- 8. THREADED RODS SHALL BE ASTM F 1554 GRADE 55.

  9. BEAMS SHALL NOT BE RELEASED FROM THE FABRICATOR UNTIL THEY HAVE ATTAINED 45 DAYS OF AGE OR OLDER.
- 10. WELDED WIRE REINFORCEMENT (WWR) SHALL CONFORM TO ASTM A884 WITH A CLASS A. TYPE 1 EPOXY COATING.



3" RADIUS -11/4" Ø CONDUIT TOP OF BEAM 270 KSI STRANDS FAN AT FAN AT ±6" CTS

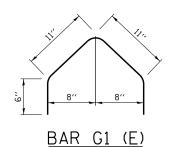
SPAN 1 LIFTING LOOP DETAIL



SPAN 2 LIFTING LOOP DETAIL

# BILL OF MATERIAL

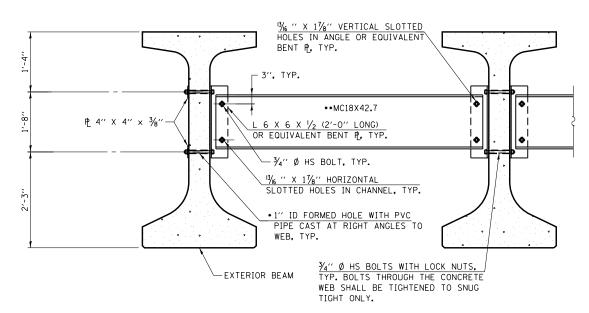
ITEM	UNIT	QUANTITY
FURNISHING AND ERECTING PRECAST PRESTRESSED CONCRETE BEAMS, IL63	FOOT	2668



\*\*exp. Chicago, IL BUILDINGS

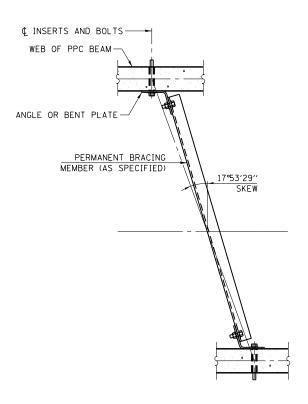
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		REVISIONS	CONTRACT NO. I-18-4694	S-28
NO.	DATE	DESCRIPTION	CONTRACT NO. 1-10-4634	3-20
			BRIDGE NO. 1681	DRAWING NO.
			BN1B0E 1402 1601	
			PPC IL 63 BEAM DETAILS 3	136 oF 220 l
			TIC IL OJ DLAM DETAILS J	100 0, 220



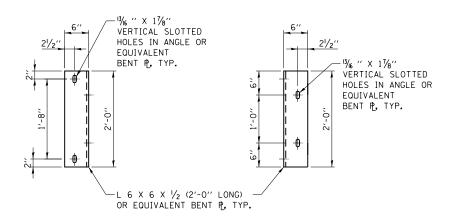
# NOTES:

- 1. ALL MATERIAL FOR BRACING SHALL BE HOT DIP GALVANIZED ACCORDING TO AASHTO M111 UNLESS OTHERWISE NOTED.
  2. TWO HARDENED WASHERS ARE REQUIRED FOR EACH SET OF
- OVERSIZED HOLES.
- 3. ALL HOLES SHALL BE 1% " Ø UNLESS OTHERWISE NOTED. 4. %6" X 3" X 3" PLATE WASHERS ARE REQUIRED OVER ALL
- SLOTTED HOLES.
- 5. ALL BOLTS SHALL BE GALVANIZED ACCORDING TO AASHTO M232.
  6. BRACING SHALL BE INSTALLED AS BEAMS ARE ERECTED AND TIGHTENED AS SOON AS POSSIBLE DURING ERECTION.
- 7. PERMANENT BRACING SHALL NOT BE PAID FOR SEPARATELY, BUT SHALL BE INCLUDED IN THE COST OF FURNISHING AND ERECTING PRECAST PRESTRESSED CONCRETE BEAMS.
- \* FABRICATOR SHALL LOCATE TO MISS STRANDS WITHIN PERMISSIBLE TOLERANCES.
- \*\* ALTERNATE MC18X45.8 CHANNELS ARE PERMITTED TO FACILITATE MATERIAL ACQUISITION.



PLAN

# PERMANENT BRACING DETAILS



BEAM FACE

DIAPHRAGM FACE

DIAPHRAGM SUPPORT

DRAWN BY EG

CHECKED BY KK

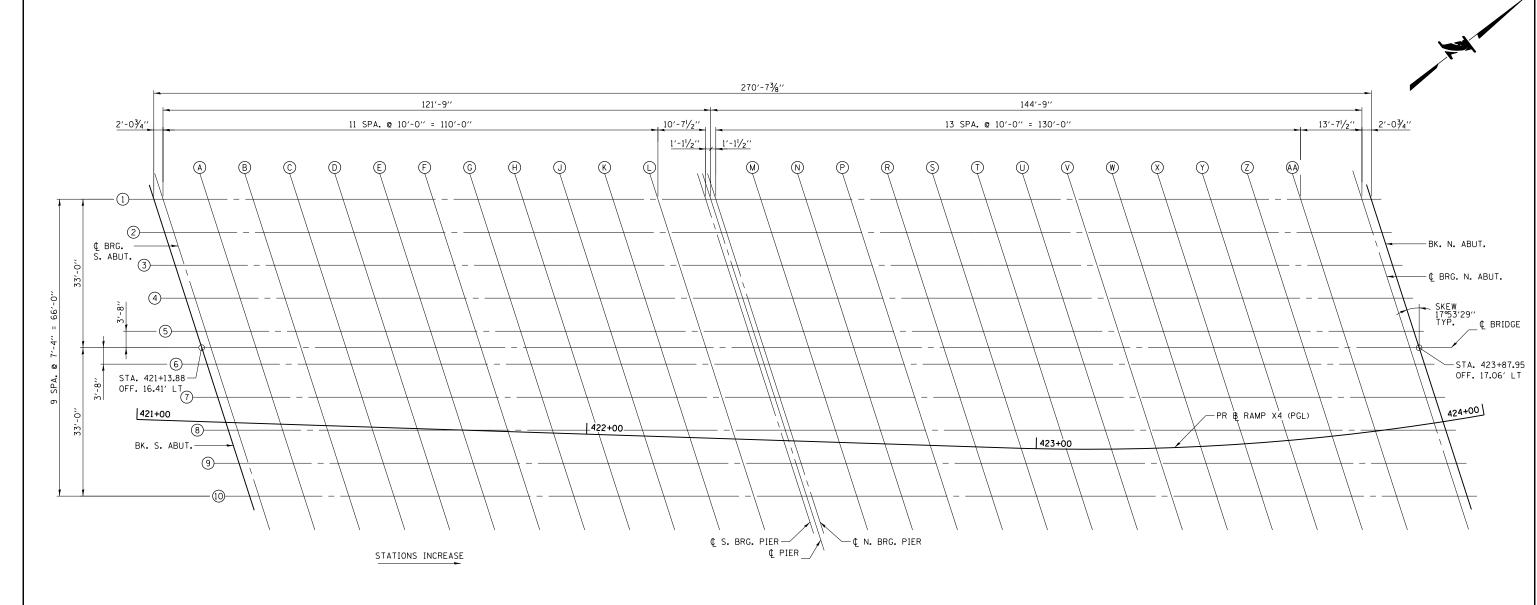
DATE 06/12/18

\*\*exp. Chicago, IL BUILDINGS

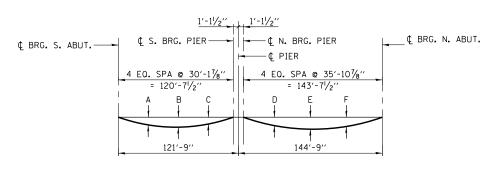
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		REVISIONS	CONTRACT NO. I-18-4694	5-29
NO.	DATE	DESCRIPTION	CONTRACT NO. 1-10-4634	3-29
			BRIDGE NO. 1681	DRAWING NO.
				137 220
			PPC IL 63 BEAM DETAILS 4	137 of 220



# PLAN - TOP OF SLAB ELEVATIONS



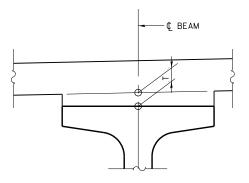
# DEAD LOAD DEFLECTION DIAGRAM

(INCLUDES WEIGHT OF CONCRETE ONLY.)

NOTE:
THE ABOVE DEFLECTIONS ARE NOT TO BE USED IN THE
FIELD IF THE ENGINEER IS WORKING FROM THE GRADE
ELEVATIONS ADJUSTED FOR DEAD LOAD DEFLECTIONS AS
SHOWN ON SHEETS S-31 TO S-34.

## DEAD LOAD DEFLECTIONS

BEAM	Α	В	С	D	E	F
1-2	1''	11/2"	1''	21/8′′	3′′	21/8′′
3-5	1''	13/8′′	1''	2''	2%"	2"
6-9	1''	13/8′′	1''	11/8′′	2¾′′	2"
10	1''	13/8′′	⅓"	2''	21/8′′	2''



TO DETERMINE "T": AFTER ALL BEAMS HAVE BEEN ERECTED, ELEVATIONS OF THE TOP FLANGES OF THE BEAMS SHALL BE TAKEN AT INTERVALS SHOWN ABOVE. THESE ELEVATIONS SUBTRACTED FROM THE "THEORETICAL GRADE ELEVATIONS ADJUSTED FOR DEAD LOAD DEFLECTION" SHOWN ON SHEETS S-31 TO S-34, MINUS SLAB THICKNESS, EQUALS THE FILLET HEIGHTS "T" ABOVE TOP FLANGE OF BEAMS.

# FILLET HEIGHTS

DRAWN BY EG DATE 06/12/18
CHECKED BY CCE DATE 06/12/18

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THE ILLINOIS STATE TOLL HIGHWAY AUTHORITY

2 7 0 0 0 G D E N A V E N U E

D O W N E R S G R O V E,

I L L I N O I S 6 0 5 1 5

	REVISIONS		CONTRACT NO. I-18-4694	C-30
NO.	DATE	DESCRIPTION	CUNTRACT NO. 1-10-4634	S-30
			BRIDGE NO. 1681	DRAWING NO.
			BINIDGE NO. 1001	
		<u> </u>	TOP OF SLAB PLAN	l 138 <i>o</i> ⊧ 220 l
			TOT OF SEAD LEAR	

BEAM 2 BEAM 3 BEAM 1

LOCATION	STATION	OFFSET	THEORETICAL GRADE ELEVATIONS	THEORETICAL GRADE ELEVATIONS ADJUSTED FOR DEAD LOAD DEFLECTION
BK. S. ABUT.	421+02.15	-49.04	686.11	686.11
CL BRG. S. ABUT.	421+04.21	-49.11	686.14	686.14
А	421+14.20	-49.44	686.30	686 <b>.</b> 33
В	421+24.20	-49.77	686.47	686 <b>.</b> 53
С	421+34.19	-50.09	686.65	686 <b>.</b> 73
D	421+44.18	-50.42	686.84	686.94
Е	421+54.18	-50.75	687.03	687.15
F	421+64.17	-51.07	687.24	687.36
G	421+74.17	-51.40	687.45	687 <b>.</b> 57
Н	421+84.16	-51.73	687.64	687.74
J	421+94.16	-52.06	687.80	687.88
K	422+04.15	-52.38	687.90	687.96
L	422+14.15	-52.71	687.97	688.00
CL S. BRG. PIER	422+24.77	-53.06	688.00	688.00
CL PIER	422+25.89	-53.10	688.00	688.00
CL N. BRG. PIER	422+27.02	-53.13	688.00	688.00
М	422+37.01	-53.46	688.01	688.06
N	422+47.00	-53.79	688.00	688.11
Р	422+57.00	-54.11	687.98	688.13
R	422+66.99	-54.44	687.95	688.14
S	422+76.99	-54.77	687.90	688.12
Т	422+86.98	-55.10	687.84	688.08
U	422+97.30	-55.42	687.76	688.01
٧	423+08.51	-55.58	687.67	687.91
W	423+19.71	-55.52	687.59	687.81
X	423+30.92	-55.25	687.53	687.73
Υ	423+42.10	-54.75	687.52	687.68
Z	423+53.25	-54.05	687.54	687.66
AA	423+64.36	-53.12	687.59	687.66
CL BRG. N. ABUT.	423+79.43	-51.52	687.68	687.68
BK. N. ABUT.	423+81.69	-51.24	687.70	687.70

LOCATION	STATION	OFFSET	THEORETICAL GRADE ELEVATIONS	THEORETICAL GRADE ELEVATIONS ADJUSTED FOR DEAD LOAD DEFLECTION
BK. S. ABUT.	421+04.76	-41.79	686.29	686.29
CL BRG. S. ABUT.	421+06.81	-41.86	686.33	686 <b>.</b> 33
A	421+16.81	-42.19	686.49	686.52
В	421+26.80	-42.51	686.66	686.72
С	421+36.80	-42.84	686.84	686.93
D	421+46.79	-43.17	687.03	687.14
E	421+56.79	-43.50	687 <b>.</b> 23	687.35
F	421+66.78	-43.82	687.44	687 <b>.</b> 57
G	421+76.78	-44.15	687.65	687.77
Н	421+86.77	-44.48	687.84	687.94
J	421+96.76	-44.80	687.99	688.08
K	422+06.76	-45.13	688.10	688.16
L	422+16.75	-45.46	688.17	688.20
CL S. BRG. PIER	422+27.37	-45.81	688 <b>.</b> 22	688.22
CL PIER	422+28.50	-45.84	688 <b>.</b> 22	688.22
CL N. BRG. PIER	422+29.62	-45.88	688.23	688.23
М	422+39.62	-46.21	688.25	688 <b>.</b> 30
N	422+49.61	-46.54	688.26	688.37
Р	422+59.61	-46.86	688.26	688.41
R	422+69.60	-47.19	688.24	688.43
S	422+79 <b>.</b> 59	-47 <b>.</b> 52	688.21	688.43
Т	422+89.59	-47.84	688.16	688.40
U	423+00.13	-48.14	688.10	688.35
V	423+11.16	-48.25	688.04	688.28
w	423+22.19	-48.14	687.98	688.21
X	423+33.21	-47.82	687.95	688.15
Y	423+44.22	-47.29	687.95	688.12
Z	423+55.19	-46.54	687.98	688.11
AA	423+66.12	-45.58	688.04	688.12
CL BRG. N. ABUT.	423+80.93	-43.94	688.14	688.14
BK. N. ABUT.	423+83.16	-43.65	688.15	688.15

LOCATION	STATION	OFFSET	THEORETICAL GRADE ELEVATIONS	THEORETICAL GRADE ELEVATIONS ADJUSTED FOR DEAD LOAD DEFLECTION
BK. S. ABUT.	421+07.36	-34.54	686.48	686.48
CL BRG. S. ABUT.	421+09.42	-34.61	686.51	686.51
A	421+19.41	-34.93	686.68	686.71
В	421+29.41	-35.26	686.85	686.91
С	421+39.40	-35.59	687.04	687.12
D	421+49.40	-35.92	687.23	687.33
E	421+59.39	-36.24	687.43	687.55
F	421+69.39	-36.57	687.65	687.76
G	421+79.38	-36.90	687.86	687.97
н	421+89.38	-37.23	688.04	688.14
J	421+99.37	-37.55	688.19	688.27
К	422+09.37	-37.88	688.30	688.36
L	422+19.36	-38.21	688.39	688.42
CL S. BRG. PIER	422+29.98	-38.56	688.45	688.45
CL PIER	422+31.10	-38.59	688.45	688.45
CL N. BRG. PIER	422+32.23	-38.63	688.46	688.46
M	422+42.22	-38.96	688.50	688.55
N	422+52.22	-39.28	688.53	688.63
Р	422+62.21	-39.61	688.54	688.68
R	422+72.21	-39.94	688.54	688.72
S	422+82.20	-40.27	688.52	688.73
Т	422+92.20	-40.59	688.49	688.72
U	423+02.87	-40.85	688.45	688.69
V	423+13.73	-40.91	688.41	688.65
W	423+24.59	-40.76	688.38	688.60
X	423+35.44	-40.39	688.37	688.57
Y	423+46.27	-39.81	688.39	688.55
Z	423+57.06	-39.03	688.43	688.55
AA	423+67.82	-38.04	688.50	688.57
CL BRG. N. ABUT.	423+82.39	-36.35	688.59	688.59
BK. N. ABUT.	423+84 <b>.</b> 58	-36.06	688.60	688.60

DRAWN BY EG

CHECKED BY CCE

DATE 06/12/18 DATE 06/12/18



	REVISIONS		CONTRACT NO. I-18-4694	S-31
NO.	DATE	DESCRIPTION	CUNTRACT NO. 1-10-4034	2-21
			BRIDGE NO. 1681	DRAWING NO.
				170 220
			TOP OF SLAB ELEVATIONS 1	139 of 220

BEAM 4 BEAM 5 BEAM 6

LOCATION	STATION	OFFSET	THEORETICAL GRADE ELEVATIONS	THEORETICAL GRADE ELEVATIONS ADJUSTED FOR DEAD LOAD DEFLECTION
BK. S. ABUT.	421+09.97	-27.29	686.67	686.67
CL BRG. S. ABUT.	421+12.02	-27.35	686.70	686.70
А	421+22.02	-27.68	686.87	686.90
В	421+32.01	-28.01	687.05	687.11
С	421+42.01	-28.34	687.23	687 <b>.</b> 32
D	421+52.00	-28.66	687.43	687 <b>.</b> 53
Е	421+62.00	-28.99	687.64	687 <b>.</b> 75
F	421+71.99	-29.32	687.85	687.97
G	421+81.99	-29.65	688.06	688.17
Н	421+91.98	-29.97	688.24	688.34
J	422+01.98	-30.30	688.39	688.47
К	422+11.97	-30.63	688.51	688.57
L	422+21.97	-30.96	688.61	688.64
CL S. BRG. PIER	422+32.59	-31.30	688.69	688.69
CL PIER	422+33.71	-31.34	688.69	688.69
CL N. BRG. PIER	422+34.83	-31.38	688.70	688.70
М	422+44.83	-31.70	688.76	688.81
N	422+54.82	-32.03	688.80	688.90
P	422+64.82	-32.36	688.83	688.98
R	422+74.81	-32.69	688.85	689.03
S	422+84.81	-33.01	688.85	689.06
T	422+94.84	-33.34	688.84	689.07
U	423+05.53	-33 <b>.</b> 55	688.82	689.06
٧	423+16.22	-33.56	688.80	689.03
W	423+26.92	-33.36	688.79	689.01
X	423+37.60	-32.95	688.80	689.00
Υ	423+48.25	-32.33	688.83	688.99
Z	423+58.88	-31.51	688.88	689.00
AA	423+69.46	-30.48	688.95	689.02
CL BRG. N. ABUT.	423+83 <b>.</b> 80	-28.76	689.04	689.04
BK. N. ABUT.	423+85.96	-28.46	689.06	689.06

LOCATION STATION OFFSET THEORETICAL GRADE ELEVATIONS ADJUSTED FOR LOAD DEFLECT CAL GRADE ELEVATIONS ADJUSTED FOR LOAD DEFLECT CALCALLY ADJUSTED FOR LOAD TO CALCALLY ADJUSTED FOR LOAD TO CALCALLY ADJUSTED FOR LOAD TO CALCALLY ADJUSTED FO	R DEAL
CL BRG. S. ABUT. 421+14.63 -20.10 686.89 686.89  A 421+24.63 -20.43 687.06 687.06  B 421+34.62 -20.76 687.24 687.30  C 421+44.62 -21.08 687.43 687.53  D 421+54.61 -21.41 687.63 687.73  E 421+64.60 -21.74 687.84 687.93  F 421+74.60 -22.07 688.05 688.17  G 421+84.59 -22.39 688.26 688.3  H 421+94.59 -22.72 688.44 688.5  J 422+04.58 -23.05 688.60 688.61  K 422+14.58 -23.38 688.73 688.73  L 422+24.57 -23.70 688.84 688.8	
A 421+24.63 -20.43 687.06 687.07 687.	
B 421+34.62 -20.76 687.24 687.30 C 421+44.62 -21.08 687.43 687.53 D 421+54.61 -21.41 687.63 687.73 E 421+64.60 -21.74 687.84 687.93 F 421+74.60 -22.07 688.05 688.17 G 421+84.59 -22.39 688.26 688.33 H 421+94.59 -22.72 688.44 688.54 J 422+04.58 -23.05 688.60 688.61 K 422+14.58 -23.38 688.73 688.73 L 422+24.57 -23.70 688.84 688.83	
C 421+44.62 -21.08 687.43 687.53 D 421+54.61 -21.41 687.63 687.73 E 421+64.60 -21.74 687.84 687.93 F 421+74.60 -22.07 688.05 688.17 G 421+84.59 -22.39 688.26 688.33 H 421+94.59 -22.72 688.44 688.54 J 422+04.58 -23.05 688.60 688.61 K 422+14.58 -23.38 688.73 688.73 L 422+24.57 -23.70 688.84 688.8	
D 421+54.61 -21.41 687.63 687.73 E 421+64.60 -21.74 687.84 687.93 F 421+74.60 -22.07 688.05 688.17 G 421+84.59 -22.39 688.26 688.33 H 421+94.59 -22.72 688.44 688.54 J 422+04.58 -23.05 688.60 688.61 K 422+14.58 -23.38 688.73 688.73 L 422+24.57 -23.70 688.84 688.8	
E 421+64.60 -21.74 687.84 687.99 F 421+74.60 -22.07 688.05 688.17 G 421+84.59 -22.39 688.26 688.37 H 421+94.59 -22.72 688.44 688.54 J 422+04.58 -23.05 688.60 688.60 K 422+14.58 -23.38 688.73 688.79 L 422+24.57 -23.70 688.84 688.8	
F 421+74.60 -22.07 688.05 688.15 G 421+84.59 -22.39 688.26 688.3 H 421+94.59 -22.72 688.44 688.5 G 688.60 G 688	
G 421+84.59 -22.39 688.26 688.31 H 421+94.59 -22.72 688.44 688.54 J 422+04.58 -23.05 688.60 688.60 K 422+14.58 -23.38 688.73 688.73 L 422+24.57 -23.70 688.84 688.81	
H 421+94.59 -22.72 688.44 688.54 J 422+04.58 -23.05 688.60 688.66 K 422+14.58 -23.38 688.73 688.73 L 422+24.57 -23.70 688.84 688.8	
J     422+04.58     -23.05     688.60     688.60       K     422+14.58     -23.38     688.73     688.73       L     422+24.57     -23.70     688.84     688.8	
K 422+14.58 -23.38 688.73 688.79 L 422+24.57 -23.70 688.84 688.8	
L 422+24.57 -23.70 688.84 688.8	
CL S. BRG. PIER 422+35.19 -24.05 688.93 688.93	
1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	
CL PIER 422+36.32 -24.09 688.94 688.94	
CL N. BRG. PIER 422+37.44 -24.13 688.95 688.99	
M 422+47.43 -24.45 689.03 689.08	
N 422+57.43 -24.78 689.09 689.19	
P 422+67.42 -25.11 689.13 689.28	
R 422+77.42 -25.43 689.17 689.35	
S 422+87.41 -25.76 689.18 689.40	
T 422+97.57 -26.08 689.19 689.47	
U 423+08.11 -26.24 689.19 689.43	
V 423+18.64 -26.20 689.19 689.43	
W 423+29.17 -25.95 689.21 689.43	
X 423+39.69 -25.50 689.23 689.43	
Y 423+50.18 -24.85 689.28 689.44	
Z 423+60.64 -23.99 689.34 689.46	
AA 423+71.06 -22.93 689.41 689.48	
CL BRG. N. ABUT. 423+85.17 -21.16 689.50 689.50	
BK. N. ABUT. 423+87.30 -20.86 689.51 689.5	

LOCATION	STATION	OFFSET	THEORETICAL GRADE ELEVATIONS	THEORETICAL GRADE ELEVATIONS ADJUSTED FOR DEAD LOAD DEFLECTION
BK. S. ABUT.	421+15.18	-12.78	687.05	687.05
CL BRG. S. ABUT.	421+17.24	-12.85	687.08	687.08
A	421+27.23	-13.18	687 <b>.</b> 25	687.28
В	421+37.23	-13.51	687.44	687.49
С	421+47.22	-13.83	687.63	687.71
D	421+57.22	-14.16	687.83	687.93
E	421+67.21	-14.49	688.04	688.15
F	421+77.21	-14.81	688.26	688.37
G	421+87.20	-15.14	688.46	688.57
н	421+97.19	-15.47	688.65	688.75
J	422+07.19	-15.80	688.81	688.89
К	422+17.18	-16.12	688.95	689.01
L	422+27.18	-16.45	689.08	689.11
CL S. BRG. PIER	422+37.80	-16.80	689.19	689.19
CL PIER	422+38.92	-16.84	689.20	689.20
CL N. BRG. PIER	422+40.05	-16.87	689.21	689.21
М	422+50.04	-17.20	689.31	689.35
N	422+60.04	-17 <b>.</b> 53	689.38	689.48
Р	422+70.03	-17.86	689.45	689.58
R	422+80.03	-18.18	689.50	689.67
S	422+90.02	-18.51	689.53	689.73
Т	423+00.23	-18.80	689.56	689.78
U	423+10.61	-18.92	689.58	689.80
V	423+20.99	-18.83	689.60	689.82
W	423+31.36	-18.54	689.63	689.84
X	423+41.72	-18.04	689.67	689.86
Y	423+52.05	-17.35	689.73	689.88
Z	423+62.35	-16.46	689.79	689.90
AA	423+72.60	-15.37	689.86	689.93
CL BRG. N. ABUT.	423+86.50	-13.56	689.95	689.95
BK. N. ABUT.	423+88.59	-13.26	689.97	689.97

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DATE 06/12/18 DATE 06/12/18



BEAM 8 BEAM 7 X4 PGL

LOCATION	STATION	OFFSET	THEORETICAL GRADE ELEVATIONS	THEORETICAL GRADE ELEVATIONS ADJUSTED FOR DEAD LOAD DEFLECTION
BK. S. ABUT.	421+17.79	-5.53	687.24	687.24
CL BRG. S. ABUT.	421+19.84	-5 <b>.</b> 60	687.27	687.27
А	421+29.84	-5.93	687.45	687.48
В	421+39.83	-6.25	687.63	687.69
С	421+49.83	-6.58	687.83	687.90
D	421+59.82	-6.91	688.03	688.13
Е	421+69.82	-7.24	688.24	688.35
F	421+79.81	-7.56	688.46	688.57
G	421+89.81	-7.89	688.67	688.78
Н	421+99.80	-8.22	688.86	688.96
J	422+09.80	-8.55	689.03	689.11
K	422+19.79	-8.87	689.19	689.24
L	422+29.78	-9.20	689.33	689.36
CL S. BRG. PIER	422+40.40	-9 <b>.</b> 55	689.46	689.46
CL PIER	422+41.53	-9.58	689.47	689.47
CL N. BRG. PIER	422+42.65	-9.62	689.49	689.49
М	422+52.65	-9 <b>.</b> 95	689.59	689.64
N	422+62.64	-10.28	689.69	689.78
Р	422+72.64	-10.60	689.77	689.91
R	422+82.63	-10.93	689.83	690.01
S	422+92.63	-11.26	689.89	690.09
T	423+02.81	-11.52	689.93	690.15
U	423+13.04	-11.58	689.97	690.20
٧	423+23.27	-11.44	690.01	690.24
W	423+33.49	-11.11	690.06	690.27
X	423+43.69	-10.58	690.12	690.30
Υ	423+53.86	-9.85	690.18	690.33
Z	423+64.00	-8.92	690.24	690.36
AA	423+74.11	-7.80	690.31	690.38
CL BRG. N. ABUT.	423+87.79	-5.96	690.41	690.41
BK. N. ABUT.	423+89.85	-5.65	690.42	690.42

LOCATION	STATION	OFFSET	THEORETICAL GRADE ELEVATIONS	THEORETICAL GRADE ELEVATIONS ADJUSTED FOR DEAD LOAD DEFLECTION
BK. S. ABUT.	421+20.39	1.72	687.43	687.43
CL BRG. S. ABUT.	421+22.45	1.65	687.46	687.46
A	421+32.44	1.33	687.64	687 <b>.</b> 67
В	421+42.44	1.00	687.83	687.88
С	421+52.43	0.67	688.02	688.10
D	421+62.43	0.34	688.23	688.33
E	421+72.42	0.02	688.45	688.55
F	421+82.42	-0.31	688.67	688.78
G	421+92.41	-0.64	688.88	688.99
Н	422+02.41	-0.97	689.08	689.17
J	422+12.40	-1.29	689.26	689.34
К	422+22.40	-1.62	689.43	689.49
L	422+32.39	-1.95	689.59	689.62
CL S. BRG. PIER	422+43.01	-2.30	689.74	689.74
CL PIER	422+44.13	-2.33	689.75	689.75
CL N. BRG. PIER	422+45.26	-2.37	689.77	689.77
М	422+55.25	-2.70	689.89	689.94
N	422+65.25	-3.02	690.00	690.10
Р	422+75.24	-3.35	690.10	690.24
R	422+85.24	-3.68	690.18	690.36
S	422+95.24	-4.01	690.25	690.45
Т	423+05.32	-4.22	690.31	690.53
U	423+15.40	-4.23	690.37	690.60
V	423+25.48	-4.05	690.43	690.66
W	423+35.55	-3.68	690.50	690.71
x	423+45.60	-3.11	690 <b>.</b> 56	690.75
Y	423+55.62	-2.34	690 <b>.</b> 63	690.78
Z	423+65.61	-1.38	690.70	690.81
AA	423+75.56	-0.23	690.77	690.83
CL BRG. N. ABUT.	423+89.04	1.64	690.86	690.86
BK. N. ABUT.	423+91.07	1.96	690.87	690.87

LOCATION	STATION	OFFSET	THEORETICAL GRADE ELEVATIONS	THEORETICAL GRADE ELEVATIONS ADJUSTED FOR DEAD LOAD DEFLECTION
BK. S. ABUT.	421+19.77	0.00	687.38	687.38
CL BRG. S. ABUT.	421+21.86	0.00	687.42	687.42
A	421+31.97	0.00	687.60	687.63
В	421+42.08	0.00	687.80	687.86
С	421+52.19	0.00	688.01	688.09
D	421+62.30	0.00	688.22	688.32
E	421+72.42	0.00	688.45	688.55
F	421+82.53	0.00	688.68	688.79
G	421+92.64	0.00	688.90	689.00
н	422+02.75	0.00	689.11	689.20
J	422+12.87	0.00	689.30	689.38
К	422+22.98	0.00	689.49	689.54
L	422+33.09	0.00	689.66	689.69
CL S. BRG. PIER	422+43.84	0.00	689.83	689.83
CL PIER	422+44.97	0.00	689.84	689.84
CL N. BRG. PIER	422+46.11	0.00	689.86	689.86
М	422+56.22	0.00	690.00	690.05
N	422+66.33	0.00	690.14	690.23
P	422+76.45	0.00	690.25	690.39
R	422+86.56	0.00	690.36	690 <b>.</b> 54
S	422+96.67	0.00	690.46	690.66
Т	423+06.73	0.00	690.54	690.76
U	423+16.73	0.00	690.61	690.84
V	423+26.66	0.00	690.67	690.89
W	423+36.55	0.00	690.71	690.92
X	423+46.38	0.00	690.75	690.93
Y	423+56.16	0.00	690.77	690.92
Z	423+65.90	0.00	690.78	690.89
АА	423+75.61	0.00	690.78	690.85
CL BRG. N. ABUT.	423+88.77	0.00	690.76	690.76
BK. N. ABUT.	423+90.76	0.00	690.76	690.76

DRAWN BY EG

CHECKED BY CCE

DATE 06/12/18 DATE 06/12/18



		REVISIONS			
١٥.	DATE	DESCRIPTION	CONTRACT NO. I-18-4694	S-33	
			BRIDGE NO. 1681	DRAWING NO.	
			TOP OF SLAB ELEVATIONS 3	141 of 220	

# BEAM 9

LOCATION	STATION	OFFSET	THEORETICAL GRADE ELEVATIONS	THEORETICAL GRADE ELEVATIONS ADJUSTED FOR DEAD LOAD DEFLECTION
BK. S. ABUT.	421+23.00	8.97	687.62	687.62
CL BRG. S. ABUT.	421+25.06	8.90	687.65	687.65
Α	421+35.05	8.58	687.83	687 <b>.</b> 86
В	421+45.05	8.25	688.02	688.08
С	421+55.04	7.92	688.22	688.30
D	421+65.03	7.60	688.43	688.53
E	421+75.03	7.27	688.65	688.76
F	421+85.02	6.94	688.88	688.99
G	421+95.02	6.61	689.09	689.20
Н	422+05.01	6.29	689.30	689.40
J	422+15.01	5.96	689.50	689.58
K	422+25.00	5.63	689.68	689.74
L	422+35.00	5.30	689.85	689.89
CL S. BRG. PIER	422+45.62	4.96	690.02	690.02
CL PIER	422+46.74	4.92	690.04	690.04
CL N. BRG. PIER	422+47.87	4.88	690.06	690.06
М	422+57.86	4.56	690.20	690.25
N	422+67.85	4.23	690.33	690.42
Р	422+77.85	3.90	690.44	690.58
R	422+87.84	3 <b>.</b> 57	690.54	690.71
S	422+97.82	3 <b>.</b> 26	690.63	690.83
T	423+07.75	3.09	690.71	690.93
U	423+17.69	3.12	690.79	691.01
٧	423+27.63	3.35	690.86	691.09
W	423+37.55	3.76	690.94	691.15
Χ	423+47.46	4.37	691.01	691.20
Y	423+57.33	5.17	691.08	691.24
Z	423+67.17	6.16	691.15	691.26
AA	423+76.98	7.34	691.22	691.29
CL BRG. N. ABUT.	423+90.25	9.25	691.31	691.31
BK. N. ABUT.	423+92.25	9 <b>.</b> 57	691.33	691.33

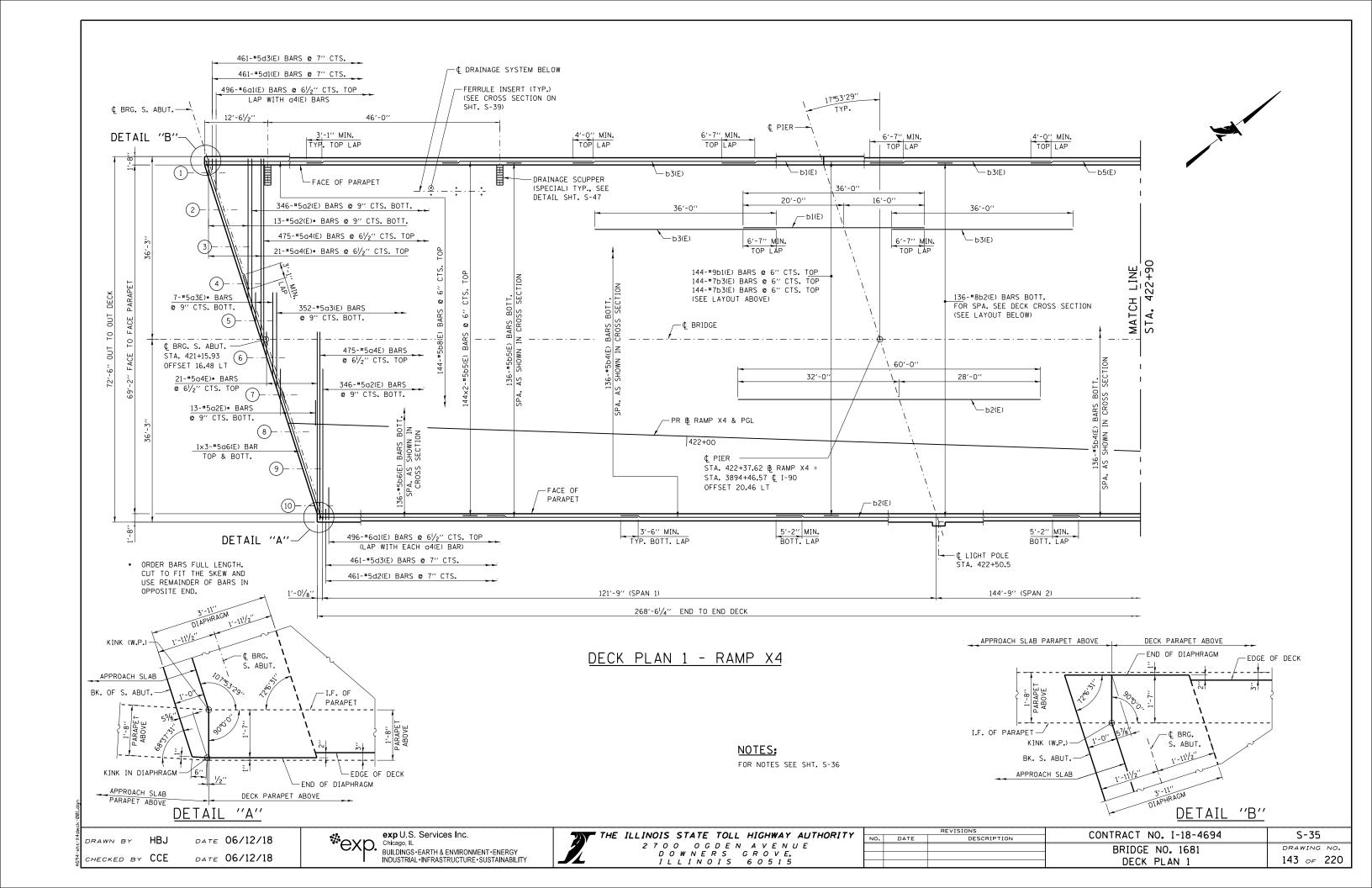
# <u>BEAM 10</u>

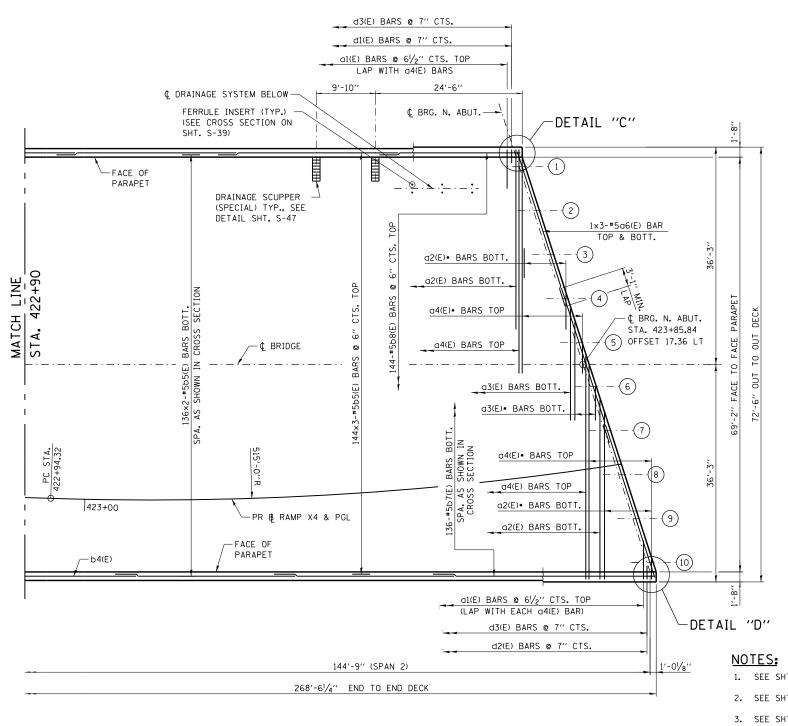
LOCATION	STATION	OFFSET	THEORETICAL GRADE ELEVATIONS	THEORETICAL GRADE ELEVATIONS ADJUSTED FOR DEAD LOAD DEFLECTION
BK. S. ABUT.	421+25.61	16.22	687.81	687.81
CL BRG. S. ABUT.	421+27.66	16.16	687.85	687.85
A	421+37.66	15.83	688.03	688.06
В	421+47.65	15 <b>.</b> 50	688.22	688.28
С	421+57.65	15.17	688.42	688.50
D	421+67.64	14.85	688.63	688.73
E	421+77.64	14.52	688.86	688 <b>.</b> 97
F	421+87.63	14.19	689.09	689 <b>.</b> 20
G	421+97.62	13.87	689.31	689.42
Н	422+07.62	13.54	689.53	689.63
J	422+17.61	13.21	689.74	689.82
К	422+27.61	12.88	689.94	690.00
L	422+37.60	12.56	690.13	690.16
CL S. BRG. PIER	422+48.22	12.21	690.32	690.32
CL PIER	422+49.35	12.17	690.34	690.34
CL N. BRG. PIER	422+50.47	12.13	690.36	690.36
M	422+60.47	11.81	690.51	690.57
N	422+70.46	11.48	690.66	690.76
Р	422+80.46	11.15	690.79	690.93
R	422+90.45	10.83	690.91	691.09
S	423+00.32	10.53	691.01	691.22
Т	423+10.12	10.42	691.11	691.34
U	423+19.92	10.49	691.20	691.44
V	423+29.72	10.76	691.29	691.53
W	423+39.50	11.21	691.38	691.60
X	423+49.26	11.86	691.46	691.65
Y	423+58.99	12.69	691.53	691.69
Z	423+68.69	13.71	691.60	691.72
AA	423+78.35	14.92	691.67	691.74
CL BRG. N. ABUT.	423+91.43	16.86	691.77	691.77
BK. N. ABUT.	423+93.40	17.18	691.78	691.78

DRAWN BY EG CHECKED BY CCE DATE 06/12/18 DATE 06/12/18



REVISIONS			CONTRACT NO. I-18-4694	S-34
NO.	DATE	DESCRIPTION	CUNTRACT NO. 1-10-4634	3-34
			BRIDGE NO. 1681	DRAWING NO.
				142 220
			TOP OF SLAB ELEVATIONS 4	142 of 220





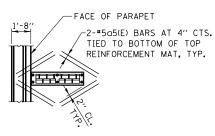
APPROACH SLAB PARAPET ABOVE DECK PARAPET ABOVE O.F. OF KINK IN PARAPET END OF DIAPHRAGM-PARAPET (W.P.) EDGE OF DECK APPROACH SLAB -BK. OF N. ABUT. ¢ BRG. N. ABUT. DETAIL "C" 1'-11/2" 1'-111/2'' -BK. OF N. ABUT. APPROACH SLAB ¢ BRG. N. ABUT. -KINK (W.P.) - I.F. OF PARAPET I.F. OF PARAPET -KINK IN DIAPHRAGM EDGE OF DECK APPROACH SLAB PARAPET ABOVE END OF DIAPHRAGM-DECK PARAPET ABOVE

# DETAIL "D"

# DECK PLAN 2 - RAMP X4



- 1. SEE SHT. S-39 FOR DECK CROSS SECTION.
- 2. SEE SHT. S-44 FOR SUPERSTRUCTURE DETAILS.
- 3. SEE SHT. S-45 FOR BAR LIST AND SUPERSTRUCTURE BILL OF MATERIAL.
- 4. SEE SHT. S-40, S-41 AND S-42 FOR DIAPHRAGM DETAILS.
- 5. SEE SHT. S-43 FOR PARAPET REINFORCEMENT.
- 6. SEE SHT. S-44 FOR PARAPET DETAILS AT LIGHT POLES.
- 7. SEE SHT. S-46 AND S-47 FOR DRAINAGE AND SCUPPER DETAILS.
- 8. BARS INDICATED THUS 69x3-#5 ETC. INDICATES 69 LINES OF BARS WITH 3 LENGTHS PER LINE.
- 9. LAP SPLICES SHALL BE AS FOLLOWS UNLESS NOTED OTHERWISE: #5 BARS TOP: 3'-1" #5 BARS BOTTOM: 3'-6"



NOTE: CUT LONGITUDINAL REINFORCEMENT TO CLEAR DRAINAGE SCUPPERS.

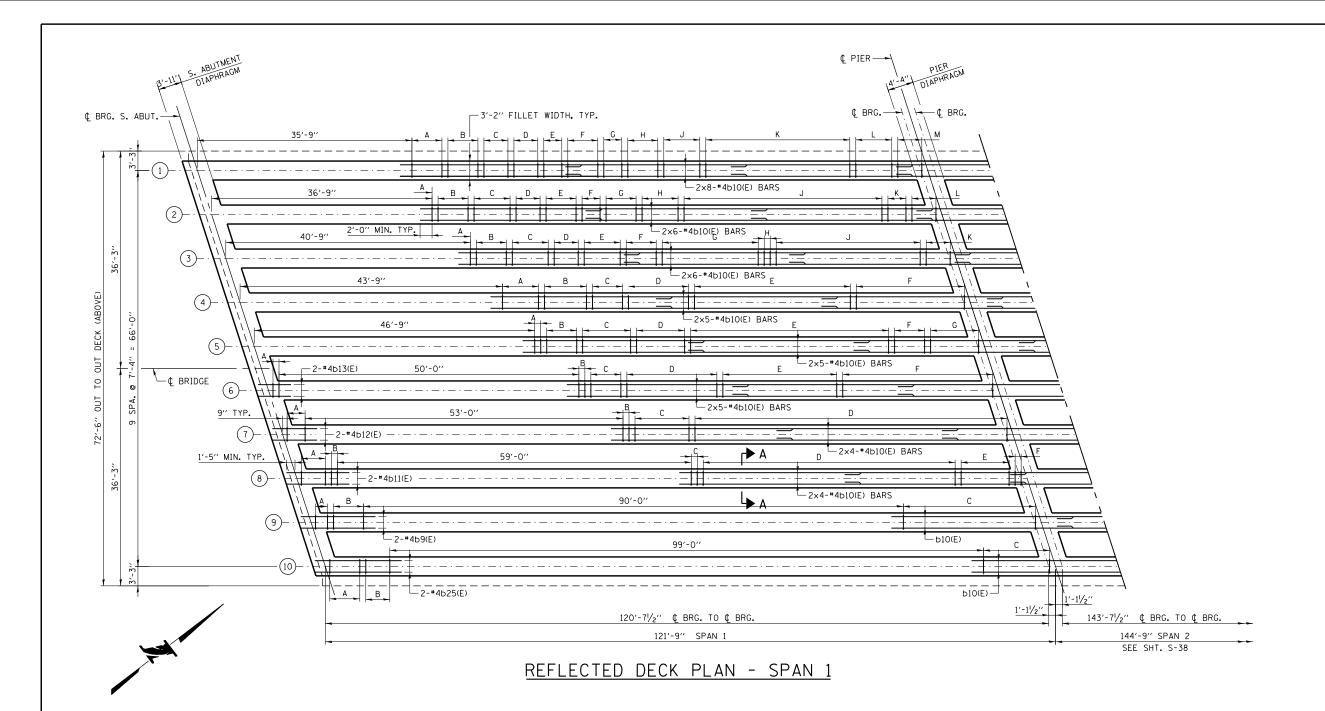
DETAIL "E" - ADD'L REINF. AT SCUPPER

HBJ DATE 06/12/18 DATE 06/12/18 CHECKED BY CCE

\*\*exp. Chicago, IL BUILDINGS

exp U.S. Services Inc. BUILDINGS EARTH & ENVIRONMENT ENERGY INDUSTRIAL INFRASTRUCTURE SUSTAINABILITY

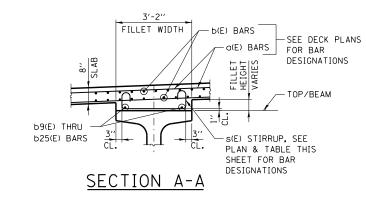
REVISIONS	CONTRACT NO. I-18-4694	S-36
O. DATE DESCRIPTION	CONTRACT NO. 1-10-4694	3-36
	BRIDGE NO. 1681	DRAWING NO.
		144 OF 220
	DECK PLAN 2	177 OF 220



#### TABLE OF STIRRUPS - SPAN 1 \*

BEAM	Α	В	C	D	E	F	G	н	J	К	L	М
1	6-#4s2(E)	6-#4s3(E)	5-#4s4(E)	5-#4s5(E)	4-#4s6(E)	6-#4s7(E)	4-#4s8(E)	6-#4s9(E)	7-#4s10(E)	25-#4s11(E)	7-#4s10(E)	5-#4s9(E)
2	2-#4s1(E)	5-#4s2(E)	7-#4s3(E)	5-#4s4(E)	6-#4s5(E)	4-#4s6(E)	6-#4s7(E)	7-#4s8(E)	34-#4s9(E)	4-#4s8(E)	5-#4s7(E)	-
3	2-#4s1(E)	5-#4s2(E)	7-#4s3(E)	5-#4s4(E)	7-#4s5(E)	6-#4s6(E)	17-#4s7(E)	2-#4s8(E)	25-#4s7(E)	5-#4s6(E)	-	-
4	7-#4s2(E)	8-#4s3(E)	6-#4s4(E)	11-#4s5(E)	27-#4s6(E)	19-#4s5(E)	-	-	-	-	-	-
5	2-#4s1(E)	6-#4s2(E)	9-#4s3(E)	9-#4s4(E)	34-#4s5(E)	6-#4s4(E)	7-#4s3(E)	-	-	-	-	-
6	2-#4s1(E)	2-#4s1(E)	6-#4s2(E)	16-#4s3(E)	20-#4s4(E)	26-#4s3(E)	-	-	-	-	-	-
7	4-#4s2(E)	2-#4s1(E)	10-#4s2(E)	53-#4s3(E)	-	-	-	-	-	-	-	-
8	5-#4s2(E)	2-#4s1(E)	2-#4s1(E)	43-#4s2(E)	9-#4s1(E)	2-#4s2(E)	-	-	-	-	-	-
9	3-#4s3(E)	6-#4s2(E)	23-#4s1(E)	-	-	-	-	-	-	-	-	-
10	6-#4s3(E)	5-#4s2(E)	12-#4s1(E)	-	-	-	-	-	-	-	-	-

• STIRRUPS SPACED @ 12" CTS. TYP.



#### NOTES:

- 1. STIRRUPS SPACED @ 12" CTS. TYP.
- 2. SEE SHT. S-35 FOR DECK SLAB REINFORCEMENT.
- 3. SEE SHT. S-45 FOR SUPERSTRUCTURE BILL OF MATERIAL.
- 4. BARS INDICATED THUS 2×3-\*5 ETC. INDICATES 2 LINES OF BARS WITH 3 LENGTHS PER LINE.
- 5. LAP SPLICES SHALL BE AS FOLLOWS UNLESS NOTED OTHERWISE: #4 BARS: 2'-5"

DRAWN BY HBJ DATE 06/12/18
CHECKED BY CCE DATE 06/12/18

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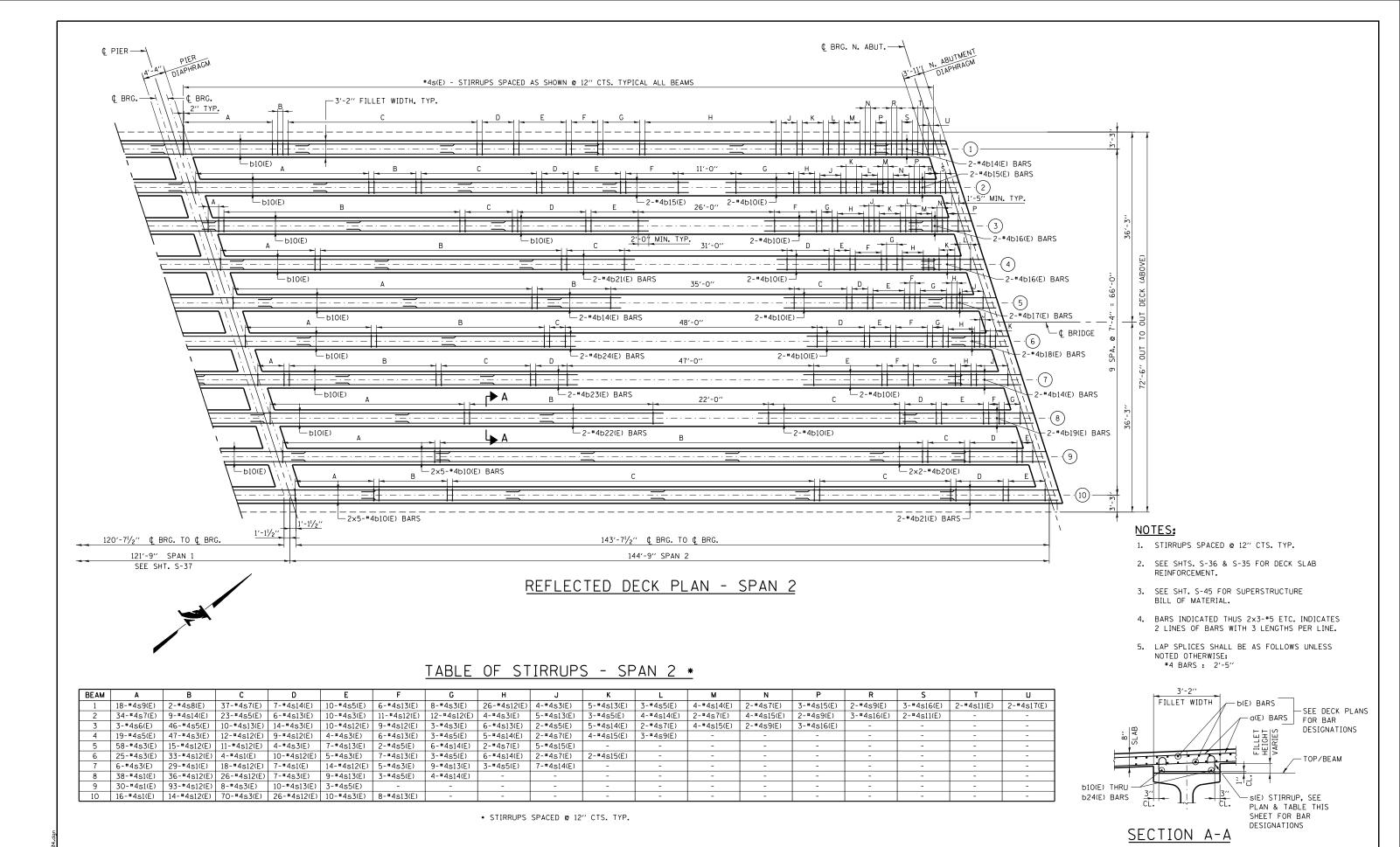
THE ILLINOIS STATE TOLL HIGHWAY AUTHORITY

2 7 0 0 0 G D E N A V E N U E

D O W N E R S G R O V E.

I L L I N O I S 6 0 5 1 5

REVISIONS			00NITD40T NO 1 10 4004	6 37
ο.	DATE	DESCRIPTION	CONTRACT NO. I-18-4694	S-37
			BRIDGE NO. 1681	DRAWING NO.
				145 OF 220
			DECK PLAN 3	145 of 220



DRAWN BY HBJ DATE 06/12/18
CHECKED BY CCE DATE 06/12/18

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INDUSTRIAL-INFRASTRUCTURE-SUSTAINABILITY

THE ILLINOIS STATE TOLL HIGHWAY AUTHORITY

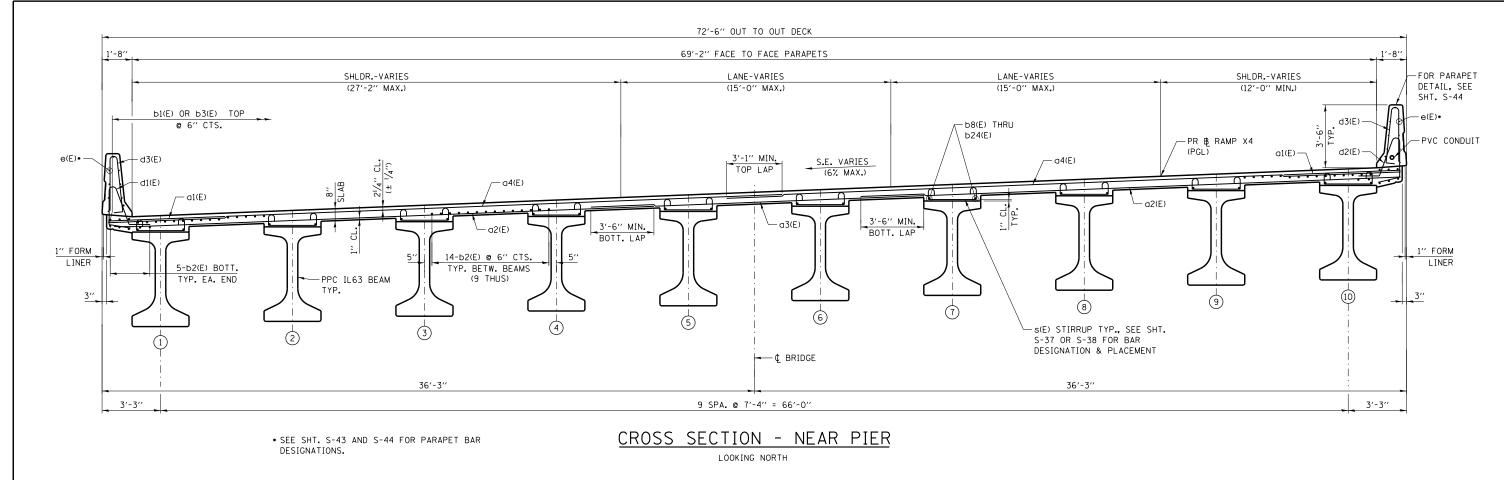
2 7 0 0 0 G D E N A V E N U E

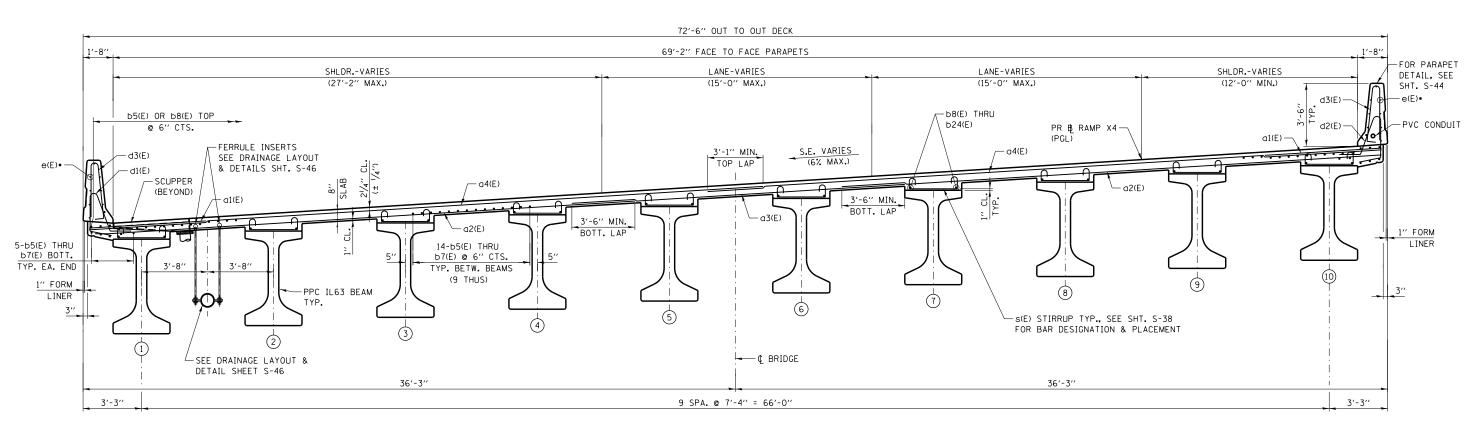
D 0 W N E R S G R O V E,

I L L I N O I S 6 0 5 1 5

REVISIONS CONTRACT NO. I-18-4694 S-38

BRIDGE NO. 1681 DRAWING NO.
DECK PLAN 4 146 of 220





CROSS SECTION - NEAR ABUTMENT

THE ILLINOIS STATE TOLL HIGHWAY AUTHORITY

2700 OGDEN AVENUE DOWNERS GROVE, ILLINOIS 60515 DATE

DESCRIPTION

CONTRACT NO. I-18-4694

BRIDGE NO. 1681

DECK CROSS SECTION

S-39

DRAWING NO.

147 OF 220

exp U.S. Services Inc.

BUILDINGS EARTH & ENVIRONMENT ENERGY INDUSTRIAL INFRASTRUCTURE SUSTAINABILITY

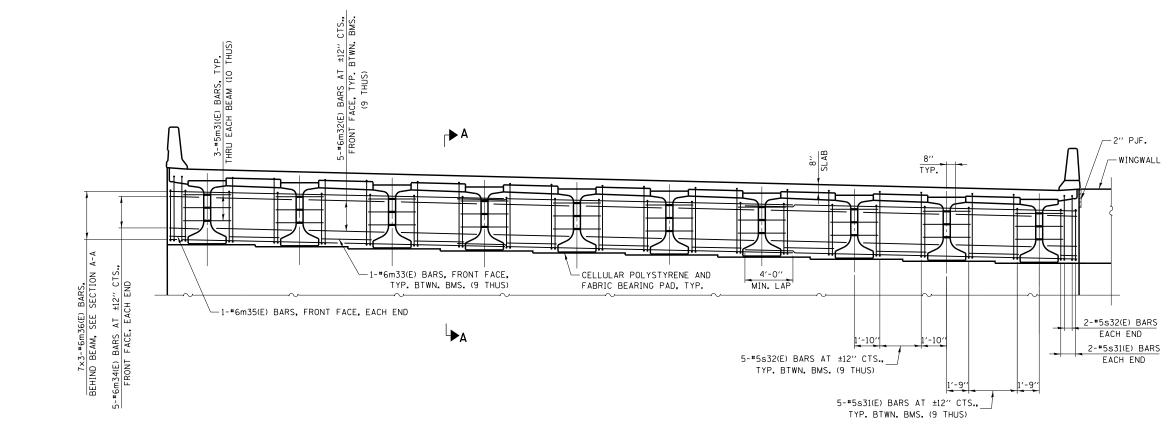
\*\*exp. Chicago, IL BUILDINGS

HBJ

CHECKED BY CCE

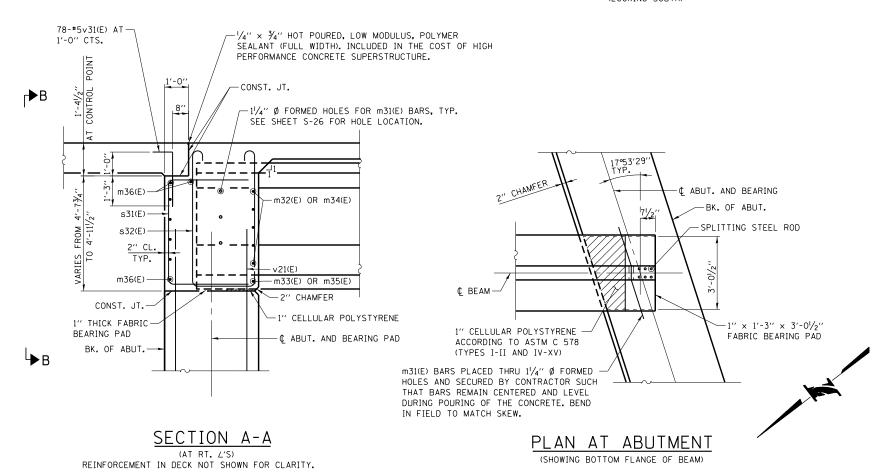
DATE 06/12/18

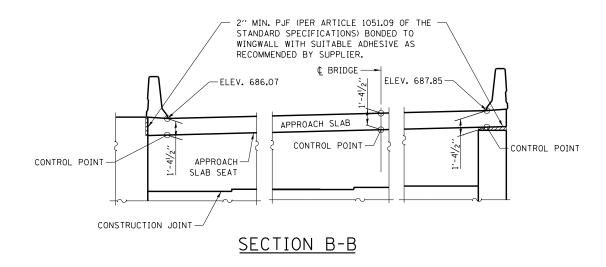
DATE 06/12/18



#### DIAPHRAGM ELEVATION AT SOUTH ABUTMENT

(LOOKING SOUTH)





#### NOTES:

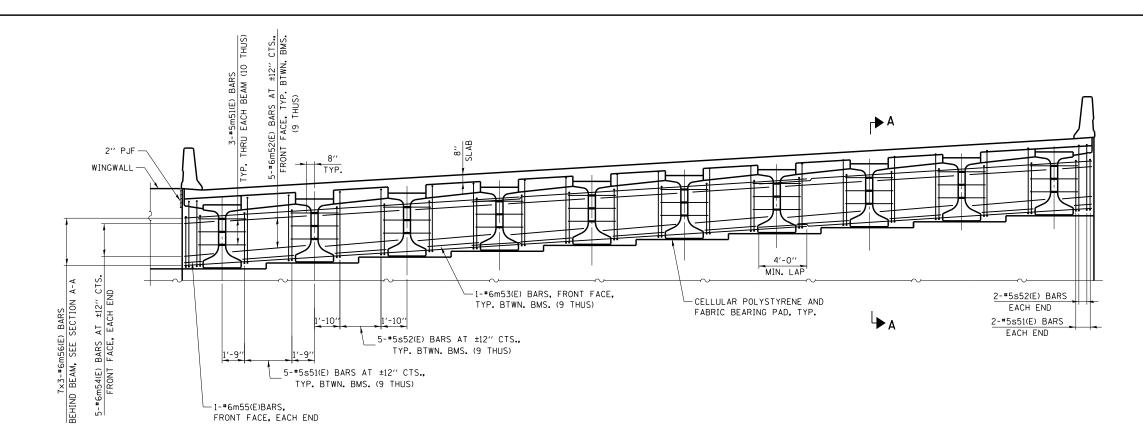
- 1. FOR BAR LIST IN DIAPHRAGM SEE SHEET S-45.
- 2.FOR BILL OF MATERIAL SEE SHEET S-45.
- 3. FOR DETAILS OF BARS s31(E), s32(E) AND v31(E) SEE SHEET S-45.
- 4. THE s31(E), AND s32(E) BARS SHALL BE PLACED PARALLEL TO THE BEAMS. SPACING FOR THESE BARS SHALL BE RIGHT ANGLES TO THE BEAMS.
- 5.THE APPROACH SLAB SEAT SHALL HAVE A CONSTANT SLOPE DETERMINED FROM THE CONTROL POINTS SHOWN.
- 6.COST OF CELLULAR POLYSTYRENE IS INCLUDED WITH HIGH PERFORMANCE CONCRETE SUPERSTRUCTURE.

DRAWN BY JC DATE 06/12/18
CHECKED BY BGK DATE 06/12/18

GARZA KARHOFF ENGINEERING, LLC



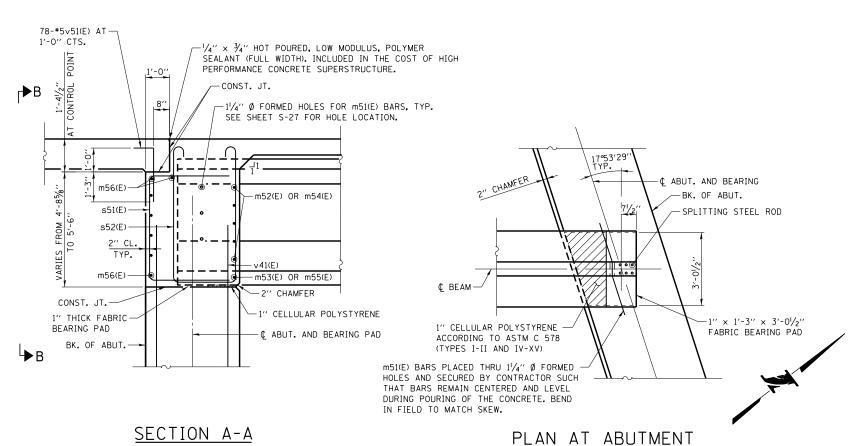
REVISIONS			CONTRACT NO. I-18-4694	S-40
ο.	DATE	DESCRIPTION	CUNTRACT NO. 1-10-4634	3-40
			BRIDGE NO. 1681	DRAWING NO.
			SOUTH ABUTMENT DIAPHRAGM DETAILS	148 <i>o</i> ≥ 220
			SOUTH ADDITIONAL DIALLINAGIN DETAILS	1.0 0. 220

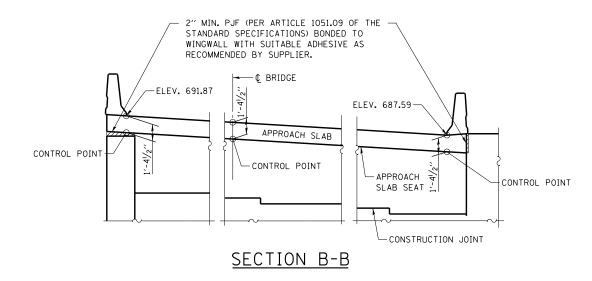


#### DIAPHRAGM ELEVATION AT NORTH ABUTMENT

(LOOKING NORTH)

(SHOWING BOTTOM FLANGE OF BEAM)





#### NOTES:

- 1. FOR BAR LIST IN DIAPHRAGM SEE SHEET S-45.
- 2. FOR BILL OF MATERIAL SEE SHEET S-45.
- 3. FOR DETAILS OF BARS s51(E), s52(E) AND v51(E) SEE SHEET S-45.
- 4. THE s51(E), AND s52(E) BARS SHALL BE PLACED PARALLEL TO THE BEAMS. SPACING FOR THESE BARS SHALL BE RIGHT ANGLES TO THE BEAMS.
- 5. THE APPROACH SLAB SEAT SHALL HAVE A CONSTANT SLOPE DETERMINED FROM THE CONTROL POINTS SHOWN.
- 6. COST OF CELLULAR POLYSTYRENE IS INCLUDED WITH HIGH PERFORMANCE CONCRETE SUPERSTRUCTURE.

DRAWN BY JC DATE 06/12/18
CHECKED BY BGK DATE 06/12/18

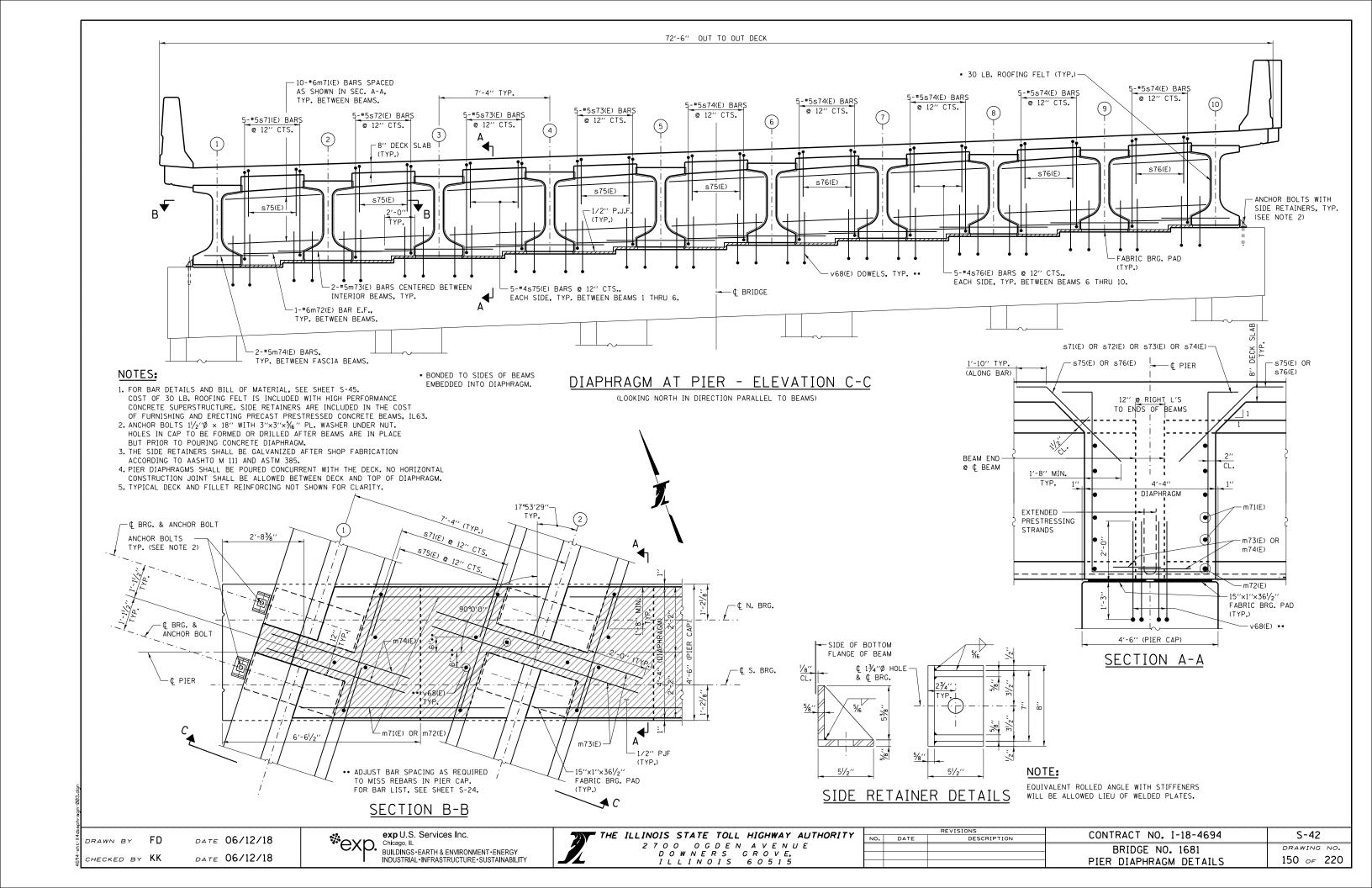
(AT RT. L'S)

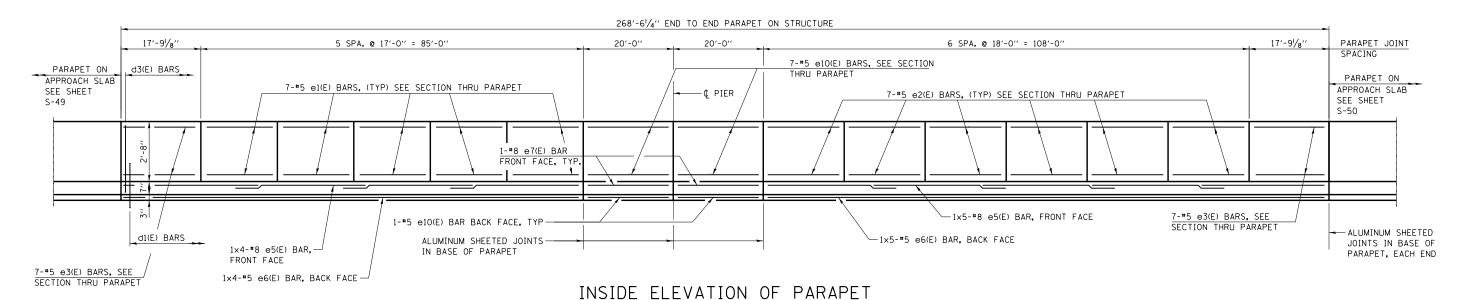
REINFORCEMENT IN DECK NOT SHOWN FOR CLARITY.

GARZA KARHOFF ENGINEERING, LLC

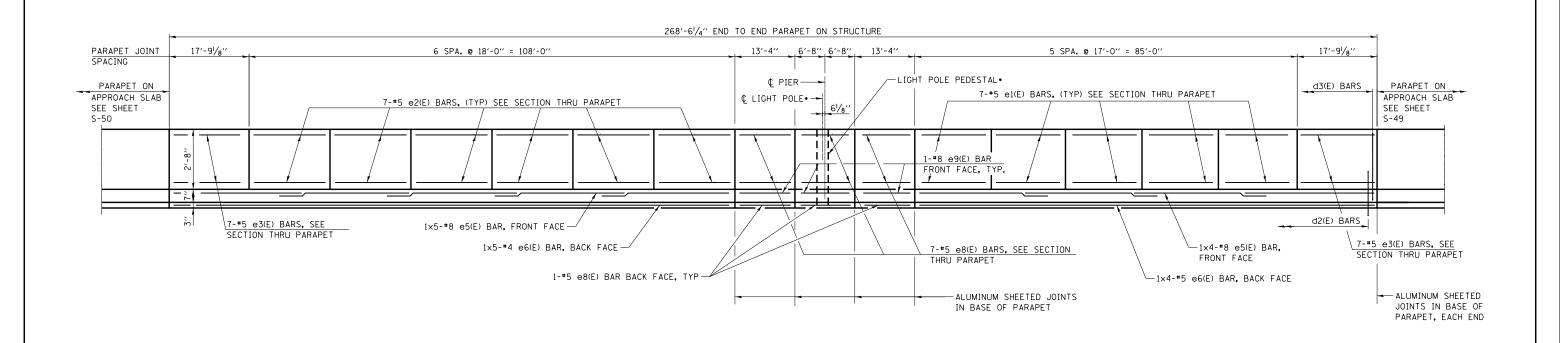


REVISIONS			CONTRACT NO. I-18-4694	S-41
ΝО.	DATE	DESCRIPTION	CUNTRACT NO. 1-10-4634	3-41
			BRIDGE NO. 1681	DRAWING NO.
				149 of 220
			NORTH ABUTMENT DIAPHRAGM DETAILS	149 of 220





(WEST PARAPET, LOOKING WEST)



#### INSIDE ELEVATION OF PARAPET

(EAST PARAPET, LOOKING EAST)

MIN LAP #8 BARS = 5'-11" #5 BARS = 3'-0"

\* FOR ADDITIONAL LIGHT POLE PEDESTAL REINFORCEMENT SEE SHEET S-44.

DRAWN BY EG DATE 06/12/18

CHECKED BY KK DATE 06/12/18

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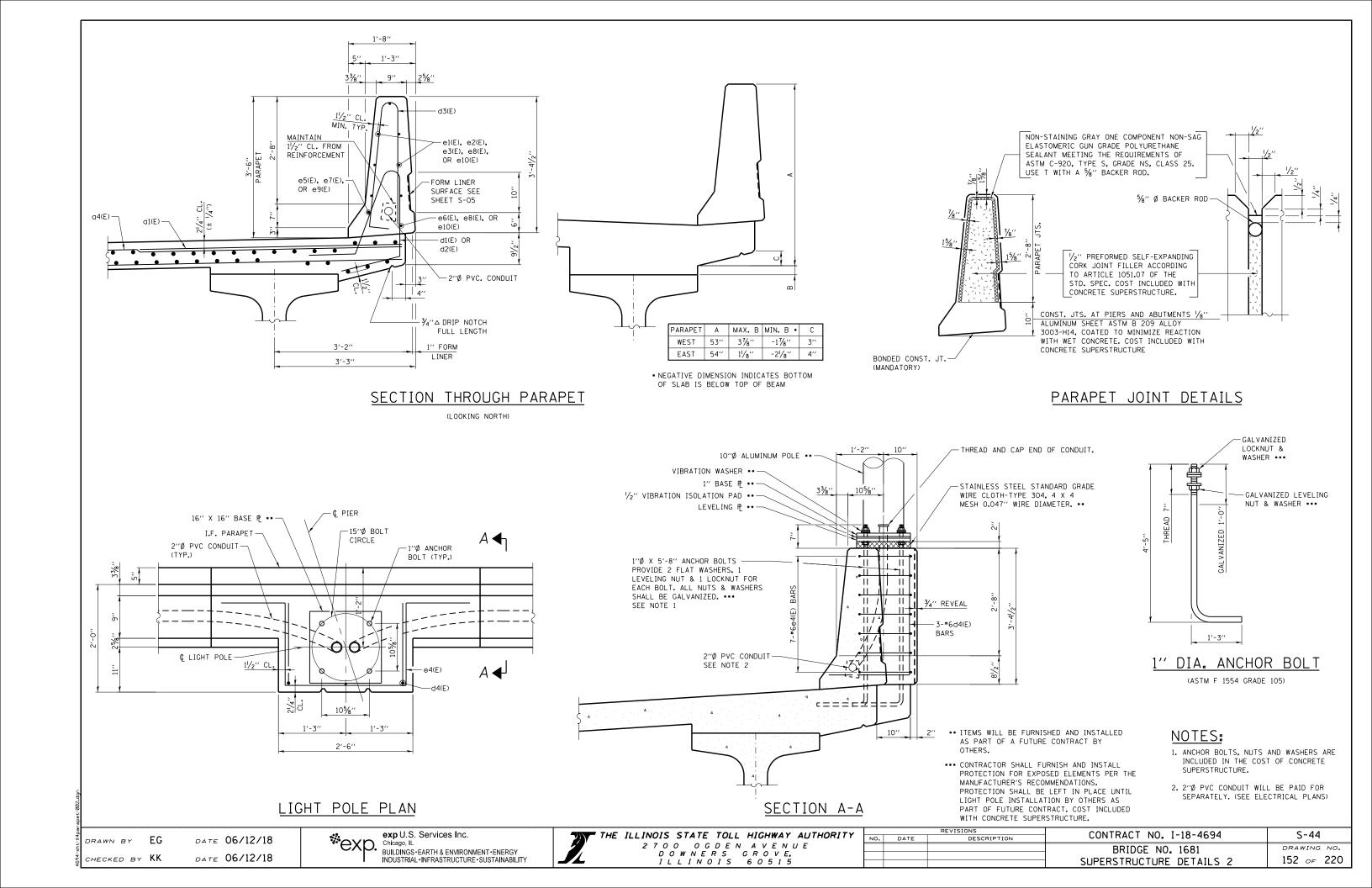
THE ILLINOIS STATE TOLL HIGHWAY AUTHORITY

2 7 0 0 0 G D E N A V E N U E

D O W N E R S G R O V E,

I L L I N O I S 6 0 5 1 5

REVISIONS			CONTRACT NO. I-18-4694	S-43
NO.	DATE	DESCRIPTION	CUNTRACT NO. 1-10-4634	3-43
			BRIDGE NO. 1681	DRAWING NO.
				151 220
			SUPERSTRUCTURE DETAILS 1	151 of 220



## BILL OF MATERIAL

#### DECK

5.5		6175		655
BAR	No.	SIZE	LENGTH	SHAPE
a1(E)	992	#6	6′-6′′	
a2(E)	718	#5	30′-0′′	
a3(E)	359	<b>#</b> 5	18'-8''	
a4(E)	992	<b>#</b> 5	37'-5''	
a5(E)	32	#5	4'-0''	
a6(E)	6	#5	27'-0''	
33121				
b1(E)	144	#9	36'-0''	
b2(E)	136	#8	60'-0''	
b3(E)	288	#7	36′-0″	
64(E)	272	#5	36′-0″	
b5(E)	1128	<b>#</b> 5	30′-0′′	
b6(E)	136	#5	37'-0''	
67(E)	136	<b>#</b> 5	37'-6''	
Ь8(E)	288	#5	24'-0''	
Ь9(E)	2	#4	12'-7''	
b10(E)	120	#4	30'-0''	
b11(E)	2	#4	10'-7''	
b12(E)	2	#4	7'-7''	
b13(E)	2	#4	5'-7''	
b14(E)	6	#4	14'-0''	
b15(E)	4	#4	17'-0''	
b16(E)	4	#4	12'-0''	
			13'-0''	
b17(E)	2	#4		
b18(E)	2	#4	11'-0''	
b19(E)	2	#4	25′-0″	
b20(E)	4	#4	18'-0''	
b21(E)	4	#4	22'-8''	
b22(E)	2	#4	23'-8''	
b23(E)	2	#4	18'-8''	
b24(E)	2	#4	25′-8′′	
b25(E)	2	#4	14'-7''	
d1(E)	461	<b>#</b> 5	6'-11''	<u> </u>
d2(E)	461	#5	6'-11''	<u> </u>
d3(E)	922	#5	6′-10′′	Ŋ
s1(E)	184	#4	4'-9''	ட
s2(E)	110	#4	4'-10''	டி
s3(E)	414	#4	4'-11''	ը
s4(E)	56	#4	5′-0′′	ը
s5(E)	231	#4	5′-1′′	<u> </u>
s6(E)	49	#4	5′-2′′	<u> </u>
s7(E)	142	#4	5′-3′′	<u> </u>
s8(E)	19	#4	5'-4''	<u> </u>
s9(E)	72	#4	5′-5′′	<u> </u>
s10(E)	14	#4	5′-6″	
\$10(E)	29	#4	5'-7"	<u>ը 1</u>
		#4	4'-10''	
s12(E)	385			
s13(E)	94	#4	5′-0′′	ر م
s14(E)	57	#4	5′-2′′	<u></u>
s15(E)	22	#4	5′-4′′	ر _
s16(E)	9	#4	5'-6''	<u></u>
s17(E)	2	#4	5′-8′′	டி
	DESCRIPTION	N	UNIT	QUANTITY
HIGH PER SUPERSTE	FORMANCE RUCTURE	CONCRETE	CU.YD.	565
REINFORC COATED	EMENT BAR	S, EPOXY	POUND	224440
PROTECT			SQ.YD.	2,064
BRIDGE D	ECK GROOV	ING×	SQ.YD.	2,004

#### PARAPETS

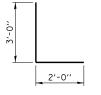
BAR	No.	SIZE	LENGTH	SHAPE
e1(E)	70	<b>#</b> 5	16'-8''	
e2(E)	84	#5	17'-8''	
e3(E)	28	<b>#</b> 5	17′-5′′	
e4(E)	7	#6	8'-11''	-
e5(E)	18	#8	30'-0''	
e6(E)	18	<b>#</b> 5	27'-10''	
e7(E)	2	#8	19'-8''	
e8(E)	24	<b>#</b> 5	13′-0′′	
e9(E)	3	#8	13'-0''	
e10(E)	16	<b>#</b> 5	19'-8''	
d4(E)	3	#6	5′-0′′	L
DESCRIPTION			UNIT	QUANTITY
CONCRETE SUPERSTRUCTURE		CU YD	73.3	
REINFORCEMENT BARS, EPOXY COATED			POUND	6220
PROTEC	TIVE COA	Т	SQ YD	263

#### PIER DIAPHRAGM

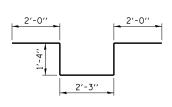
BAR	No.	SIZE	LENGTH	SHAPE
m71(E)	90	#6	6'-9''	
m72(E)	18	#6	4'-0''	
m73(E)	16	<b>#</b> 5	7'-3''	
m74(E)	4	#5	5′-0′′	
s71(E)	5	#5	17'-6''	U
s72(E)	5	#5	17'-4''	U
s73(E)	10	#5	17'-3''	L
s74(E)	25	#5	17'-1''	L
s75(E)	50	#4	6'-1''	
s76(E)	40	#4	5′-8′′	
[	ESCRIPTION	V	UNIT	QUANTITY
HIGH PER SUPERSTE	FORMANCE RUCTURE	CU YD	57.3	
REINFORC COATED	EMENT BAR	S, EPOXY	POUND	2320

#### SOUTH ABUTMENT DIAPHRAGM

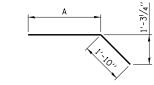
BAR	No.	SIZE	LENGTH	SHAPE
m31(E)	30	#5	4'-0''	
m32(E)	45	#6	6'-4''	
m33(E)	9	#6	4'-2''	
m34(E)	10	#6	2'-6''	
m35(E)	2	#6	1'-5''	
m36(E)	21	#6	28'-0''	
s31(E)	49	#5	11'-4''	
s32(E)	49	#5	14'-10''	二
v31(E)	78	#5	3'-1"	
DESCRIPTION			UNIT	QUANTITY
HIGH PERFORMANCE CONCRETE SUPERSTRUCTURE			CU YD	52.9
REINFOR EPOXY (		POUND	3120	



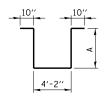
BAR d4(E)



BAR e4(E)



BARS s75(E) & s76(E)



BARS s71(E) THRU s74(E)

BAR	Α
s71(E)	5′-10′′
s72(E)	5′-9′′
s73(E)	5'-81/2"
s74(E)	5'-71/2"
s75(E)	4'-3''
s76(E)	3'-10''

#### NORTH ABUTMENT DIAPHRAGM

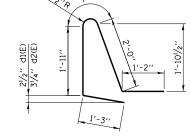
BAR	No.	SIZE	LENGTH	SHAPE
m51(E)	30	#5	4'-0''	
m52(E)	45	#6	6'-4''	
m53(E)	9	#6	4'-2''	
m54(E)	10	#6	2'-6''	
m55(E)	2	#6	1′-5′′	
m56(E)	21	#6	28'-0''	
s51(E)	49	#5	11'-4''	
s52(E)	49	#5	15'-0''	<u> </u>
v51(E)	78	#5	3'-1''	
D	ESCRIPTIO	UNIT	QUANTITY	
	RFORMANCE E SUPERS	CU YD	57.8	
REINFOR EPOXY C		ARS,	POUND	3130



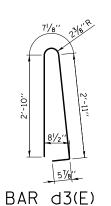
#### BARS v51(E) & v31(E)

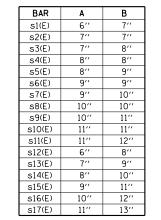


BARS s52(E) & s32(E) BARS s51(E) & s31(E)



BARS d1(E & d2(E)





2'-8''

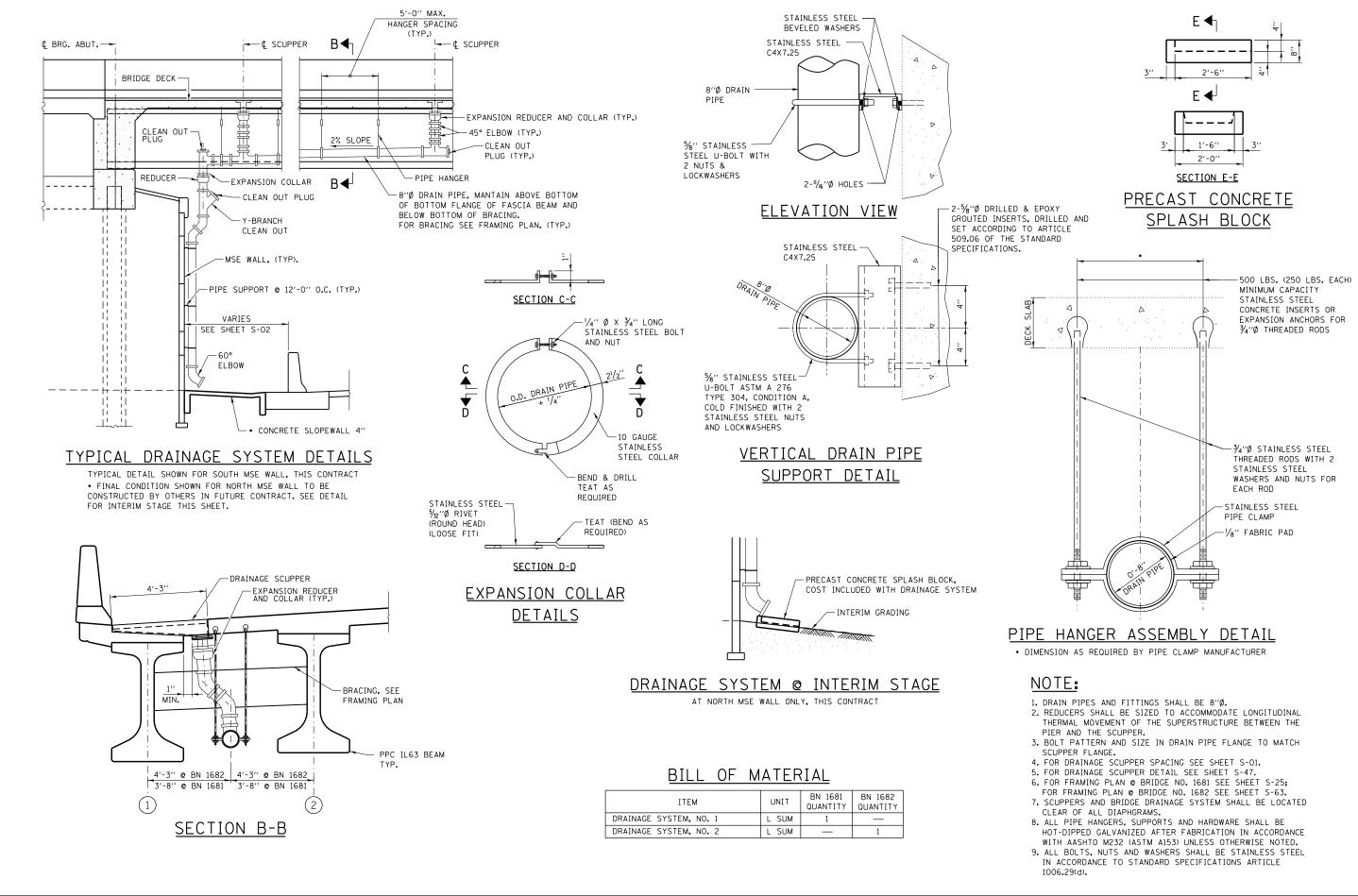
BARS s1(E) THRU s17(E)

DATE 06/12/18 DRAWN BY EG CHECKED BY KK DATE 06/12/18

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REVISIONS			CONTRACT NO. I-18-4694	S-45
NO.	DATE	DESCRIPTION	CUNTRACT NO. 1-10-4634	3-43
			BRIDGE NO. 1681	DRAWING NO.
				153 <i>o</i> ≥ 220
			SUPERSTRUCTURE DETAILS 3	155 of 220



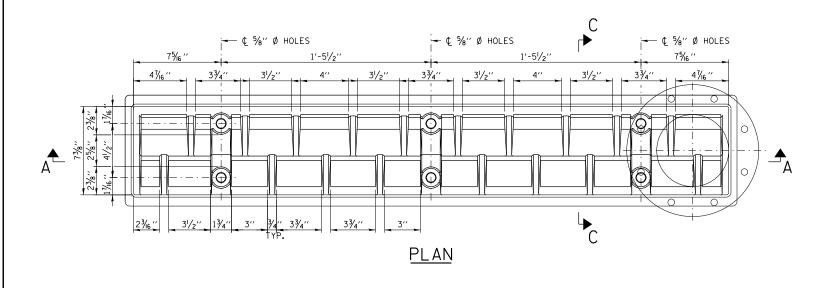
DRAWN BY LS CHECKED BY BGK

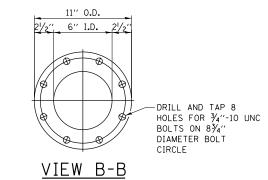
DATE 06/12/18
DATE 06/12/18

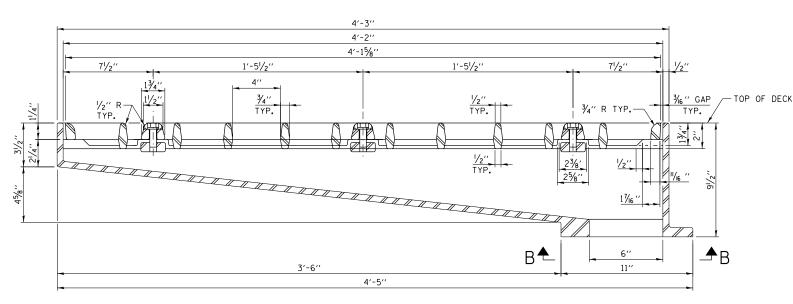
GARZA KARHOFF ENGINEERING, LLC

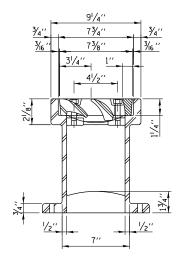


REVISIONS		REVISIONS	CONTRACT NO. I-18-4694	S-46
10.	DATE	DESCRIPTION	CUNTRACT NO. 1-10-4634	3-46
			BRIDGE NOS. 1681 & 1682	DRAWING NO.
			BRIDGE DRAINAGE DETAILS	154 of 220
			BRIDGE BRAINAGE BETAILS	10 1 11 220





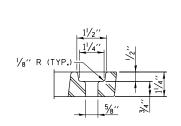




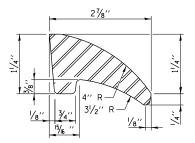
SECTION C-C

## SECTION A-A

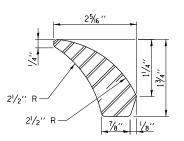
SEE SHEET S-46 FOR SCUPPER LOCATION RELATIVE TO PARAPET







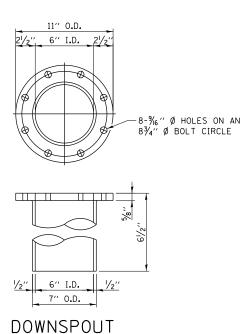
FIRST VANE DETAIL



SECOND VANE DETAIL

#### NOTES:

- ALL CAST IRON PARTS SHALL BE GRAY IRON CONFORMING TO THE REQUIREMENTS OF AASHTO M 105, CLASS 35B.
- 2. BOLTS, ANCHOR STUDS, WASHERS AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A 307 AND SHALL BE GALVANIZED ACCORDING TO AASHTO M 232.
- 3. AS AN ALTERNATE, BOLTS, ANCHOR STUDS, WASHERS AND NUTS MAY BE STAINLESS STEEL ACCORDING TO ARTICLE 1006.29(D) OF THE STANDARD SPECIFICATIONS.
- 4. STRUCTURAL STEEL WELDMENTS OF EQUAL SECTIONS AND OF THE SAME CONFIGURATION MAY BE SUBSTITUTED FOR THE CAST IRON SCUPPER FRAME. FILLET OR FULL PENETRATION WELDS SHALL BE USED FOR THE WELDMENTS. DETAILS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL. STRUCTURAL STEEL WELDMENTS SHALL NOT BE SUBSTITUTED FOR THE CAST IRON SCUPPER GRATE. STRUCTURAL STEEL FRAMES AND DOWNSPOUTS SHALL BE GALVANIZED ACCORDING TO AASHTO MIII.
- 5. THE CONTRACTOR SHALL TAKE APPROPRIATE MEASURES TO ASSURE THAT PROTECTIVE COAT IS NOT APPLIED TO THE SCUPPER.
- 6. COST OF THE GRATE, FRAME, DOWNSPOUT, ANCHOR STUDS, BOLTS, WASHERS AND NUTS INCLUDING COMPLETE INSTALLATION OF THE SCUPPER SHALL BE PAID FOR AT THE CONTRACT UNIT PRICE EACH FOR DRAINAGE SCUPPER.
- 7. ALTERNATE FIBERGLASS DOWNSPOUT CONFORMING TO ASTM D 2996 WITH A SHORT-TIME RUPTURE STRENGTH HOOP TENSILE STRESS OF 30,000 PSI MIN. MAY BE USED IN LIEU OF THE CAST IRON OR STEEL EQUIVALENT.



## BILL OF MATERIAL

ITEM	UNIT	BN 1681 QUANTITY	BN 1682 QUANTITY
DRAINAGE SCUPPERS (SPECIAL)	EACH	4	4

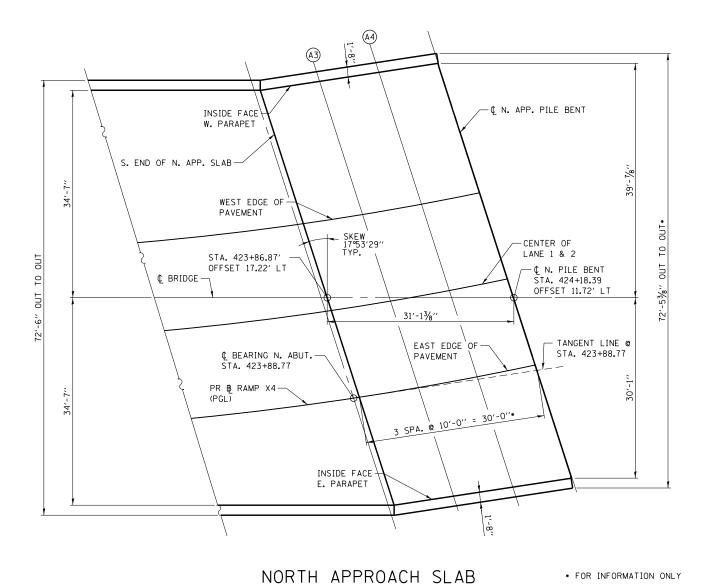
DRAWN BY LS DATE 06/12/18
CHECKED BY BGK DATE 06/12/18

GARZA KARHOFF ENGINEERING, LLC



		REVISIONS	CONTRACT NO. I-18-4694	S-47
١٥.	DATE	DESCRIPTION	CONTRACT NO. 1-10-4634	3-41
			BRIDGE NOS. 1681 & 1682	DRAWING NO.
				155 <i>o</i> ≥ 220
			DRAINAGE SCUPPER DETAILS	133 OF 220

INSIDE FACE W. PARAPET WEST EDGE OF PAVEMENT SKEW 17°53′29″ TYP. 70'-7%" OUT TO OUT• 3 SPA. @ 10'-0" = 30'-0" 72'-6" OUT TO OUT — STA. 421+14.93 ¢ BRIDGE — OFFSET 16.44' LT © S. PILE BENT -STA. 420+84.94 OFFSET 15.46' LT -CENTER OF LANE 1 & 2 EAST EDGE OF-PR B RAMP X4 PAVEMENT (PGL) ¢ S. APP. PILE BENT -N. END OF S. APP. SLAB -INSIDE FACE E. PARAPET



SOUTH APPROACH SLAB (SHOWING GEOMETRY AND SCREED LINES)

PLAN

#### INSIDE FACE WEST PARAPET

LOCATION	STATION	OFFSET	THEORETICAL GRADE ELEVATIONS
C/L S. APP. PILE BENT	420+72.65	-49.66	685.67
A1	420+82.65	-49.99	685.80
A2	420+92.64	-50.32	685.93
N. END S. APP. SLAB	421+02.64	-50.64	686.08
S. END N. APP. SLAB	423+80.21	-53.02	687.59
A3	423+91.36	-53.06	687.57
Α4	424+02.50	-52.90	687.54
C/L N. APP. PILE BENT	424+13.64	-52.53	687.52

#### WEST EDGE OF PAVEMENT

LOCATION	STATION	OFFSET	THEORETICAL GRADE ELEVATIONS
C/L S. APP. PILE BENT	420+80.55	-27.68	686.21
A1	420+90.63	-27.79	686.35
A2	421+01.04	-27.90	686.51
N. END S. APP. SLAB	421+10.77	-28.01	686.67
S. END N. APP. SLAB	423+84.61	-30.00	688.97
A3	423+95.22	-30.00	688.94
Д4	424+05.80	-30.00	688.91
C/L N. APP. PILE BENT	424+16.36	-30.00	688.85

## CENTER OF LANE 1 & 2

LOCATION	STATION	OFFSET	THEORETICAL GRADE ELEVATIONS
C/L S. APP. PILE BENT	420+85.47	-14.01	686.55
A1	420+95.58	-14.01	686.70
A2	421+05.69	-14.01	686.86
N. END S. APP. SLAB	421+15.80	-14.01	687.03
S. END N. APP. SLAB	423+87.25	-15.00	689.86
А3	423+97.54	-15.00	689.84
Α4	424+07.80	-15.00	689.80
C/L N. APP. PILE BENT	424+18.03	-15.00	689.75

#### EAST EDGE OF PAVEMENT & PR B RAMP X4 (PGL)

(SHOWING GEOMETRY AND SCREED LINES)

LOCATION	STATION	OFFSET	THEORETICAL GRADE ELEVATIONS
C/L S. APP. PILE BENT	420+90.50	0.00	686.91
A1	421+00.62	0.00	687.06
A2	421+10.72	0.00	687.23
N. END S. APP. SLAB	421+20.84	0.00	687.40
S. END N. APP. SLAB	423+89.74	0.00	690.76
А3	423+99.73	0.00	690.73
Α4	424+09.68	0.00	690.69
C/L N. APP. PILE BENT	424+19.61	0.00	690.64

### INSIDE FACE EAST PARAPET

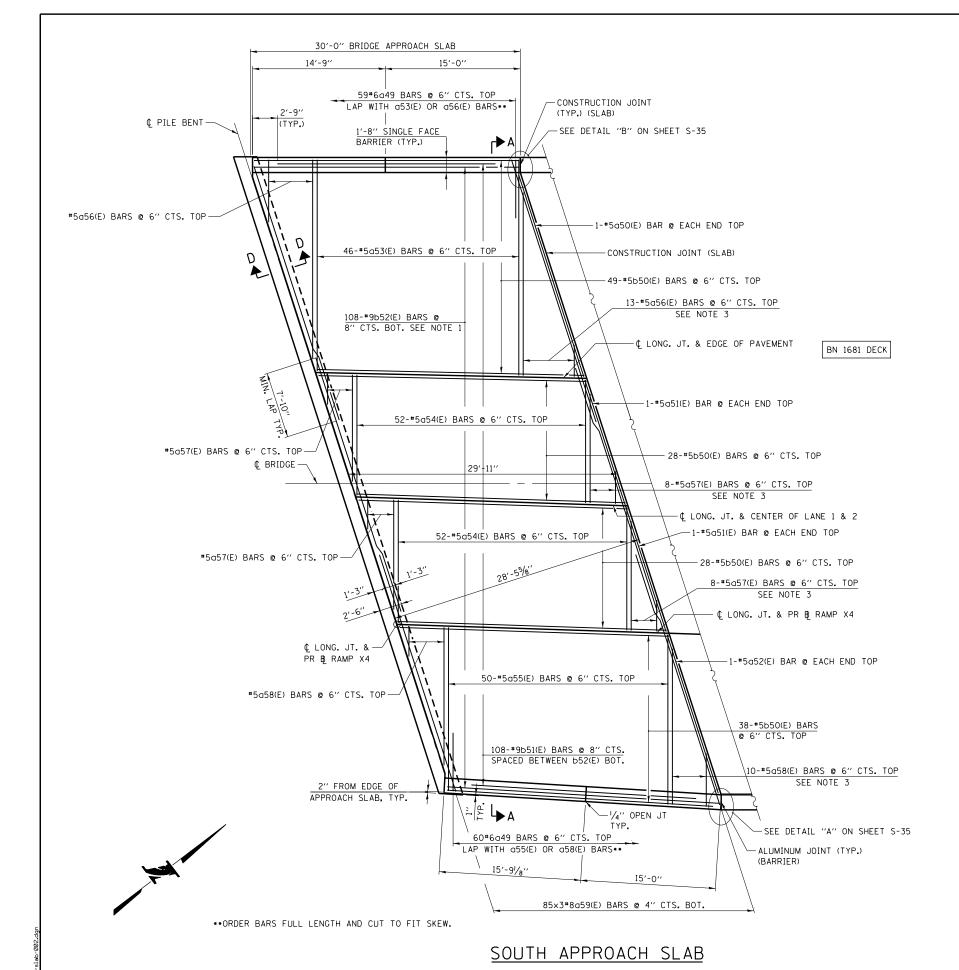
LOCATION	STATION	OFFSET	THEORETICAL GRADE ELEVATIONS
C/L S. APP. PILE BENT	420+96.57	16.90	687.34
A1	421+06.79	17.18	687.50
A2	421+17.01	17.47	687.68
N. END S. APP. SLAB	421+27.22	17.76	687.87
S. END N. APP. SLAB	423+92.64	18.66	691.87
А3	424+02.29	18.83	691.85
Α4	424+11.93	19.19	691.83
C/L N. APP. PILE BENT	424+21.55	19.73	691.81

DRAWN BY EG CHECKED BY CCE DATE 06/12/18 DATE 06/12/18

\*\*exp. Chicago, IL BUILDINGS

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		REVISIONS	CONTRACT NO. I-18-4694	S-48
NO.	DATE	DESCRIPTION	CUNTRACT NO. 1-10-4634	3-40
			BRIDGE NO. 1681	DRAWING NO.
				150 220
			TOP OF APPROACH SLAB ELEVATIONS	156 <i>o</i> ≥220



exp U.S. Services Inc.

BUILDINGS EARTH & ENVIRONMENT ENERGY INDUSTRIAL INFRASTRUCTURE SUSTAINABILITY

\*\*exp. Chicago, IL BUILDINGS

DRAWN BY EG

CHECKED BY KK

DATE 06/12/18

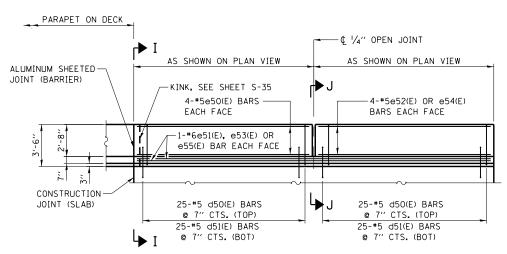
DATE 06/12/18

#### BILL OF MATERIAL

BILL (	OF MATER	IAL FOR	SOUTH APP.	SLAB
BAR	No.	SIZE	LENGTH	SHAPE
a49(E)	119	#6	6′-6′′	
a50(E)	2	#5	23'-8''	
a51(E)	4	#5	14'-6''	
a52(E)	2	#5	18'-6''	
a53(E)	46	<b>#</b> 5	24'-0''	
a54(E)	104	#5	13'-8''	
a55(E)	50	#5	18'-10''	
a56(E)	13	#5	23′-3′′	
a57(E)	16	#5	13'-7''	
a58(E)	10	#5	19'-6''	
a59(E)	255	#8	30′-0′′	
b50(E)	143	<b>#</b> 5	29′-7′′	
b51(E)	108	#9	24′-5′′	
b52(E)	108	#9	32'-0''	
	SCRIPTION		UNIT	QUANTITY
BRIDGE AF	PROACH S	SLAB	SQ.YD.	238
REINFORCE EPOXY CO		RS,	LBS.	51230
PROTECTIV	/E COAT		SQ.YD.	228
BRIDGE DE	CK GROOV	/ING	SQ.YD.	220

ВІ	LL OF MA	TERIAL F	OR BARRIER	S
BAR	No.	SIZE	LENGTH	SHAPE
d50(E)	100	#5	6′-10′′	1
d51(E)	100	#5	8'-4''	Ĺ
F0/F)	16		14/ 0//	
e50(E)	16	#5	14'-8''	
e51(E)	4	#6	29′-8′′	
e52(E)	8	#5	14'-5''	
e53(E)	2	#6	29′-5′′	
e54(E)	8	#5	15'-5''	
e55(E)	2	#6	30′-5′′	
DE:	SCRIPTION	l	UNIT	QUANTITY
CONCRETE SUPERSTRUCTURE			CU.YD.	8.2
REINFORCEMENT BARS, EPOXY COATED			LBS.	2430
PROTECTIV	/E COAT		SQ.YD.	30

\* FOR INFORMATION ONLY



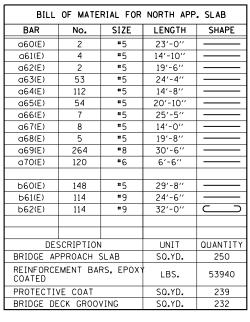
#### SOUTH APPROACH SLAB BARRIER INSIDE ELEVATION

(EAST BARRIER SHOWN, WEST BARRIER SIMILAR)

#### NOTES:

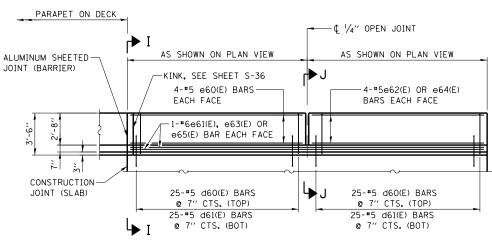
- 1. TILT HOOK OF #9 BARS FOR MINIMUM 21/4" CLEARANCE.
- 2. USE 4'-0" MIN. LAP FOR #5 BARS. USE 7'-10" MIN. LAP FOR #8 BARS.
- 3. CUT REINFORCEMENT IN THE FIELD TO FIT THE SKEW AND USE REMAINDER IN OPPOSITE END. COAT CUT ENDS WITH EPOXY.
- 4. FOR SECTIONS A-A AND D-D SEE SHEET S-51.
- 5. PROTECTIVE COAT SHALL BE APPLIED TO TOP AND TRAFFIC FACES OF BARRIERS.
- 6. TOOL EDGES OF EXPANSION JOINTS TO 1/4" RADIUS.
- 7. CONCRETE BARRIERS SHALL BE CONSTRUCTED & PAID FOR IN ACCORDANCE WITH SECTIONS 503 AND 508 OF THE STANDARD SPECIFICATIONS.
- 8. FOR BARS b52(E), d50(E), AND d51(E), SEE SHEET S-51.

#### BILL OF MATERIAL



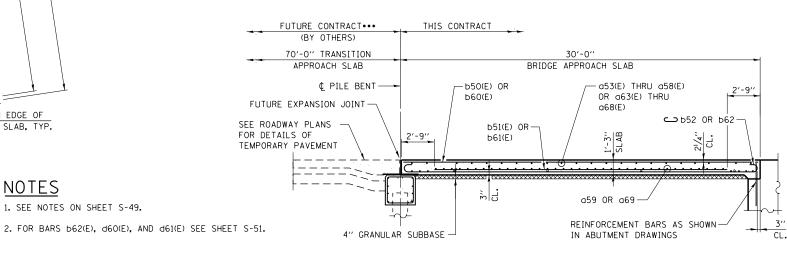
BILL OF MATERIAL FOR BARRIERS						
BAR	No.	SIZE	LENGTH	SHAPE		
d60(E)	100	#5	6'-10''	1		
d61(E)	100	<b>#</b> 5	8'-4''	L		
e60(E)	16	#5	14'-8''			
e61(E)	4	#6	29'-8''			
e62(E)	8	<b>#</b> 5	14'-6''			
e63(E)	2	#6	29'-6''			
e64(E)	8	#5	14'-7''			
e65(E)	2	#6	29'-7''			
DES	SCRIPTION	I	UNIT	QUANTITY		
CONCRETE	SUPERST	RUCTURE	CU.YD.	8.2		
REINFORCEMENT BARS, EPOXY COATED			LBS.	2430		
PROTECTIV	/E COAT		SQ.YD.	29		

\* FOR INFORMATION ONLY



#### NORTH APPROACH SLAB BARRIER INSIDE ELEVATION

(WEST BARRIER SHOWN, EAST BARRIER SIMILAR)



#### LONGITUDINAL CROSS SECTION

\*\*\* SEE ROADWAY PLANS FOR TEMPORARY PAVEMENT AND INTERIM GRADING

EG DATE 06/12/18 DRAWN BY DATE 06/12/18 CHECKED BY KK

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60-#6070(E) BARS @ 6" CTS. TOP LAP WITH 063(E) OR 066(E) BARS...

7-#5066(E) BARS @ 6" CTS. TOP

SEE NOTE 3 ON SHEET S-49

SEE DETAIL "C"

ON SHEET S-36

1-#5a60(E) BAR @-

¢ LONG. JT. & EDGE

1-#5a61(E) BAR @

¢ BRIDGE

1-#5a61(E) BAR @ EACH END TOP

CONSTRUCTION JOINT (SLAB)

¢ LONG. JT. &

PR B RAMP X4

1-#5a62(E) BAR @ EACH END TOP

SEE DETAIL "D" ON SHEET S-36

NORTH APPROACH SLAB

EACH END TOP

¢ LONG. JT. & CENTER OF LANE 1 & 2-

EACH END TOP

OF PAVEMENT

BN 1681 DECK

1'-8" SINGLE FACE
BARRIER (TYP.)

53-\*5a63(E) BARS @ 6" CTS. TOP

29'-61/4"

4-#5067(E) BARS @ 6" CTS. TOP

56-#5a64(E) BARS @ 6" CTS. TOP

31'-03/8

-1-#5b60(E) BARS TOP\*\*

56-#5a64(E) BARS @ 6" CTS. TOP

1-#5b60(E) BARS TOP\*\*-

6-\*5068(E) BARS @ 6" CTS. TOP SEE NOTE 3 ON SHEET S-49

54-#5065(E) BARS @ 6" CTS, TOP

60-#6070(E) BARS @ 6" CTS. TOP

LAP WITH G65(E) AND G68(E) BARS.

4-#5067(E) BARS @ 6" CTS. TOP SEE NOTE 3 ON SHEET S-49

SEE NOTE 3 ON SHEET S-49

1-#5b60(E) BARS TOP\*\*

- ¢ PILE BENT

\_#5a66(E) BARS @ 6" CTS. TOP

#5067(E) BARS @

6" CTS. TOP

1'-3'

V4" OPEN JT TYP.

14'-10"

88×3\*8069(E) BARS @ 4" CTS. BOT.

#5067(E) BARS @

6" CTS. TOP

¢ LONG. JT. &

PR B RAMP X4

114#9b61(E) BARS @ 8"

CTS.

SPACED BETWEEN

<u>662(E)</u>

2" FROM EDGE OF

APPROACH SLAB, TYP.

\*\* REBAR TO BE SPRUNG IN PLACE TO FOLLOW SAW CUT JOINT.

5 b60 (E)

#5068(E) BARS @

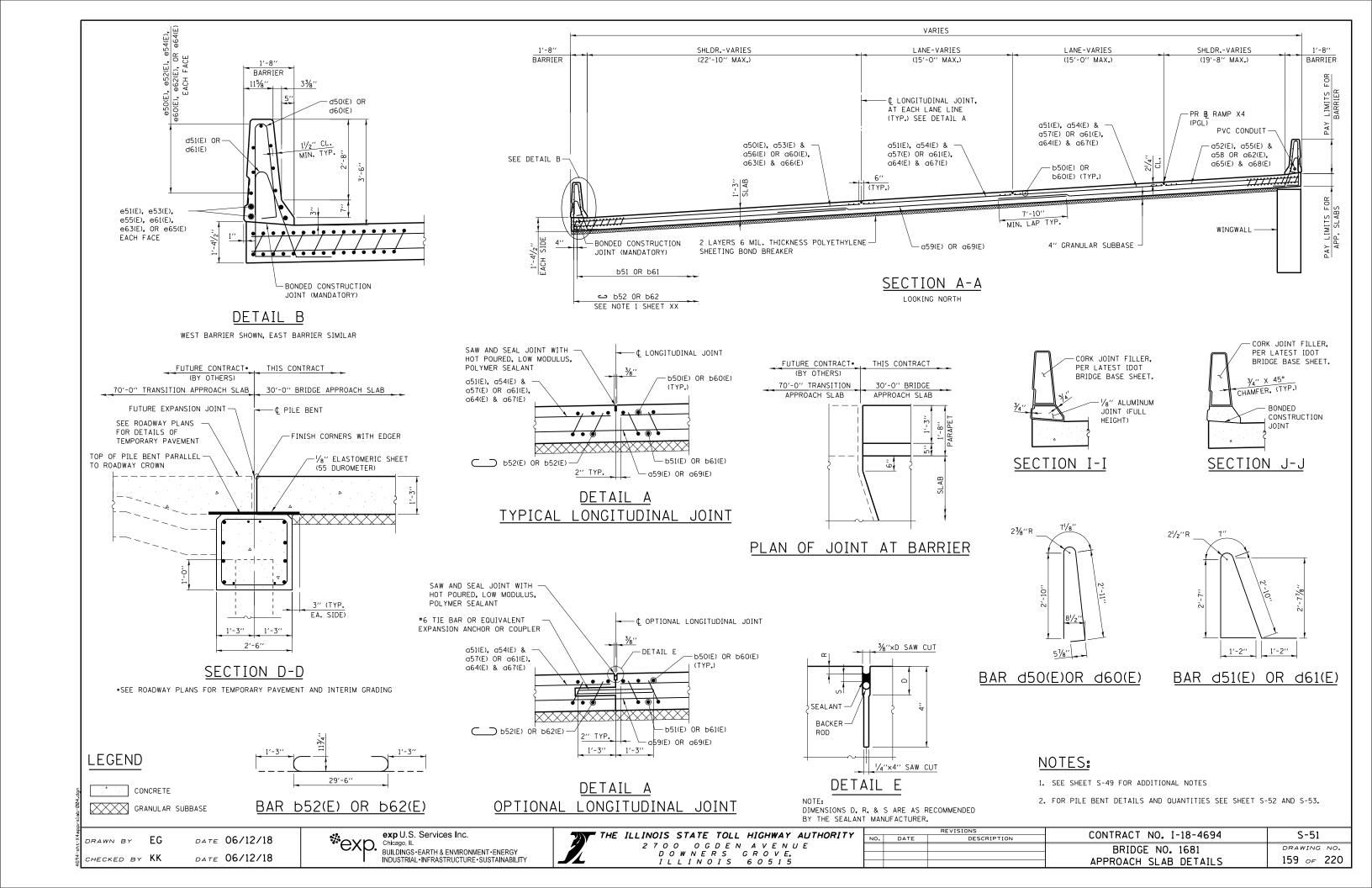
6" CTS. TOP

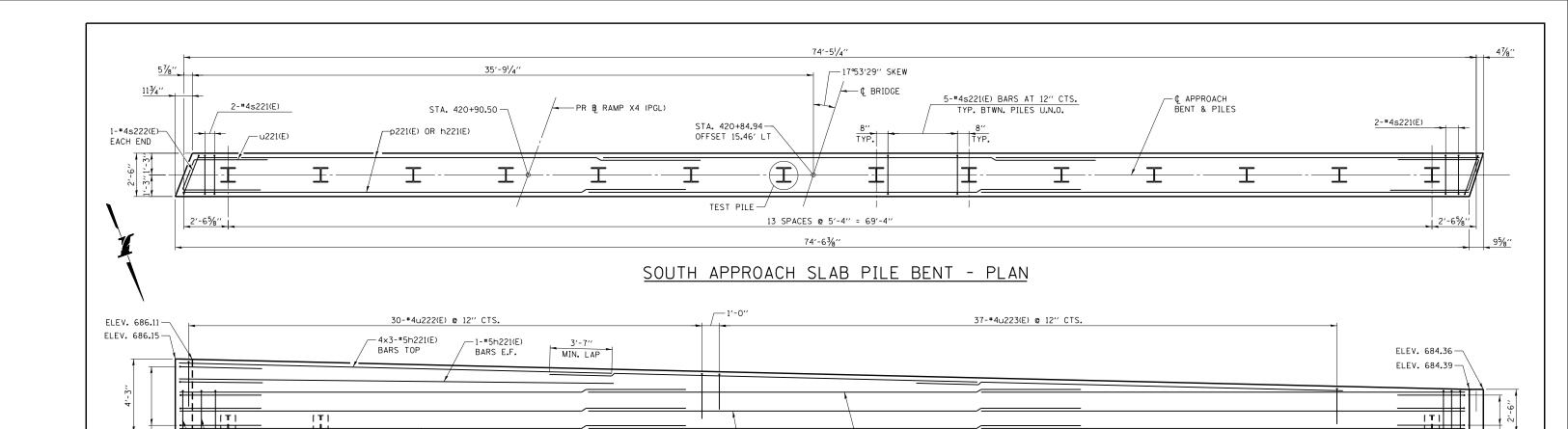
THE ILLINOIS STATE TOLL HIGHWAY AUTHORITY 2700 OGDEN AVENUE DOWNERS GROVE, ILLINOIS 60515

NOTES

1. SEE NOTES ON SHEET S-49.

		REVISIONS	CONTRACT NO. I-18-4694	S-50
NO.	DATE	DESCRIPTION	CONTRACT NO. 1-10-4634	3-30
			BRIDGE NO. 1681	DRAWING NO.
				158 OF 220
			NORTH APPROACH SLAB	158 of 220





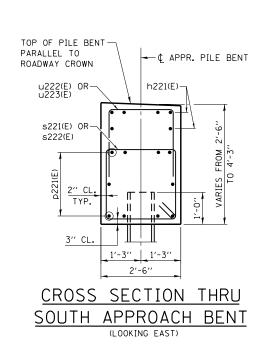
-1×3-#8p221(E) BARS E.F.

SOUTH APPROACH SLAB PILE BENT - ELEVATION
(LOOKING SOUTH)

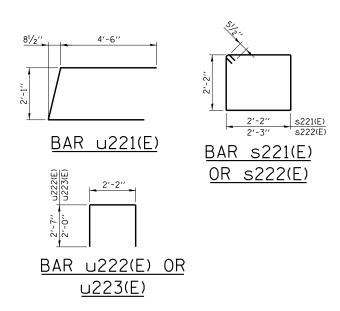
## BILL OF MATERIAL

└─ s221(E) - s222(E)

BAR	No.	SIZE	LENGTH	SHAPE
h221(E)	14	#5	25′-0′′	
p221(E)	30	#8	28'-7''	
s221(E)	69	#4	9'-7''	
s222(E)	2	#4	9'-9''	
u221(E)	8	#6	11'-3''	
u222(E)	30	#4	7'-4''	
u223(E)	37	#4	6′-2′′	
l	DESCRIPTION	١	UNIT	QUANTITY
CONCRETE	STRUCTUR	ES	CU YD	23.3
REINFORC EPOXY CO	EMENT BAR	POUND	3550	
FURNISHI PILES HP	NG STEEL 10×42	FOOT	533	
DRIVING	PILES	FOOT	533	
TEST PIL	E STEEL HF	EACH	1	
PILE SHO	ES	EACH	14	



ELEV. 681.88



-4×3-#8p221(E) BARS TOP

MIN. LAP TYP.

#### PILE DATA

TYPE: HP10×42 WITH PILE SHOES NOMINAL REQUIRED BEARING: 178 KIPS FACTORED RESISTANCE AVAILABLE: 98 KIPS EST. LENGTH: 41' NO. PRODUCTION PILES: 13 NO. TEST PILES: 1

#### NOTES

FOR PILE DETAILS, SEE SHEET S-10.
 FOR SUBSTRUCTURE LAYOUT, SEE SHEET S-07.
 TOP OF CAP ELEVATION GIVEN AT ¢ APPROACH BENT.

DRAWN BY EG DATE 06/12/18
CHECKED BY KK DATE 06/12/18

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INDUSTRIAL-INFRASTRUCTURE-SUSTAINABILITY

-4×3-#8p221(E)

BARS BOTT.

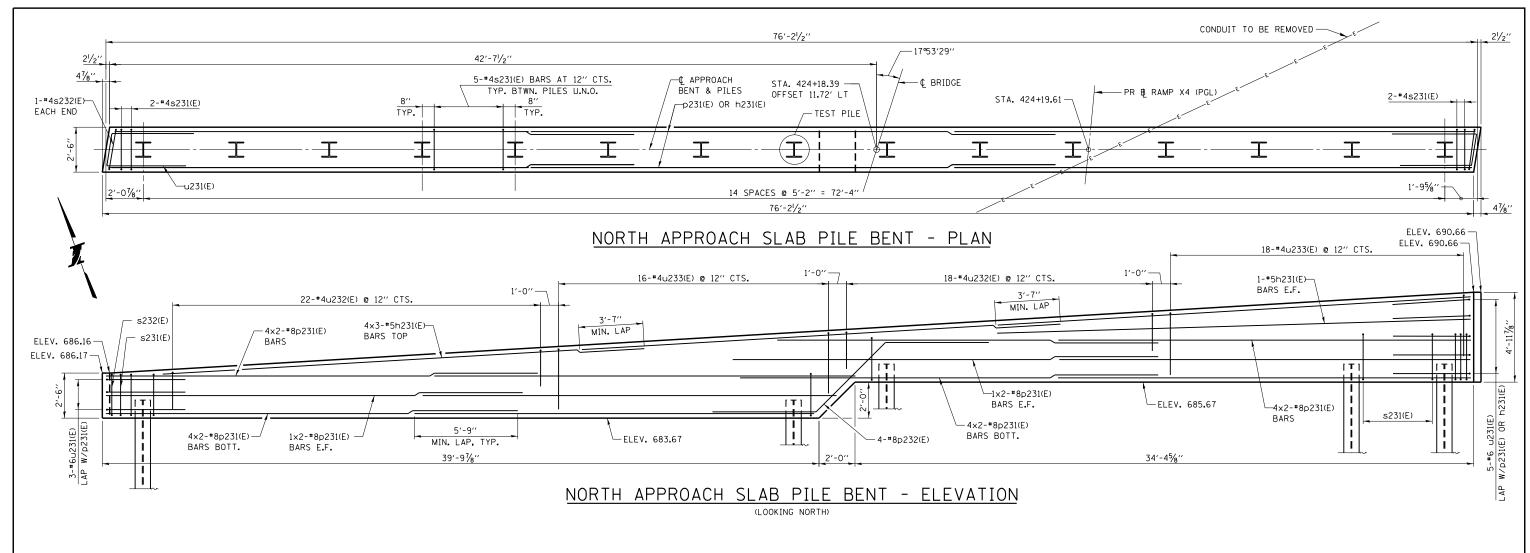
THE ILLINOIS STATE TOLL HIGHWAY AUTHORITY

2 7 0 0 0 G D E N A V E N U E

D O W N E R S G R O V E.

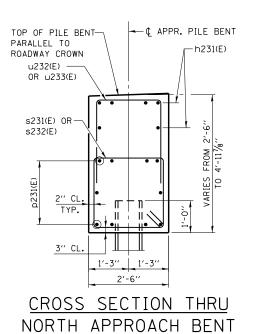
I L L I N O I S 6 0 5 1 5

Τ			REVISIONS	CONTRACT NO. I-18-4694	S-52
E	NO.	DATE	DESCRIPTION	CUNTRACT NO. 1-10-4634	3-52
Γ				BRIDGE NO. 1681	DRAWING NO.
Γ					100 000
Γ				SOUTH APPROACH PILE BENT	160 of 220

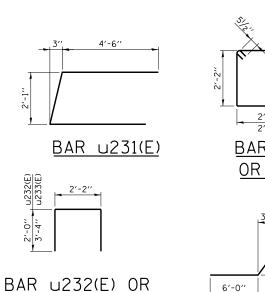


#### BILL OF MATERIAL

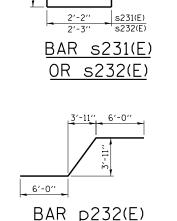
BAR	No.	SIZE	LENGTH	SHAPE
h231(E)	14	#5	26'-10"	
1123I(E)	14	73	26 -10	
- 071/5\	40	**0	201 611	
p231(E)	40	#8	22′-6′′	
p232(E)	4	#8	17′-5′′	
s231(E)	74	#4	9′-7′′	
s232(E)	2	#4	9′-9′′	
u231(E)	8	#6	11'-3''	
u232(E)	40	#4	6′-2′′	
u233(E)	34	#4	8'-10''	
I	DESCRIPTION	V	UNIT	QUANTITY
CONCRETE	STRUCTUR	ES	CU YD	27.1
REINFORC EPOXY CO	EMENT BARS	S,	POUND	3970
FURNISHI PILES HP		FOOT	602	
DRIVING	PILES	FOOT	602	
TEST PIL	E STEEL HE	EACH	1	
PILE SHO	ES	EACH	15	



(LOOKING EAST)



u233(E)



## PILE DATA

TYPE: HP10×42 WITH PILE SHOES NOMINAL REQUIRED BEARING: 173 KIPS FACTORED RESISTANCE AVAILABLE: 95 KIPS EST. LENGTH: 43' NO. PRODUCTION PILES: 14 NO. TEST PILES: 1

#### **NOTES**

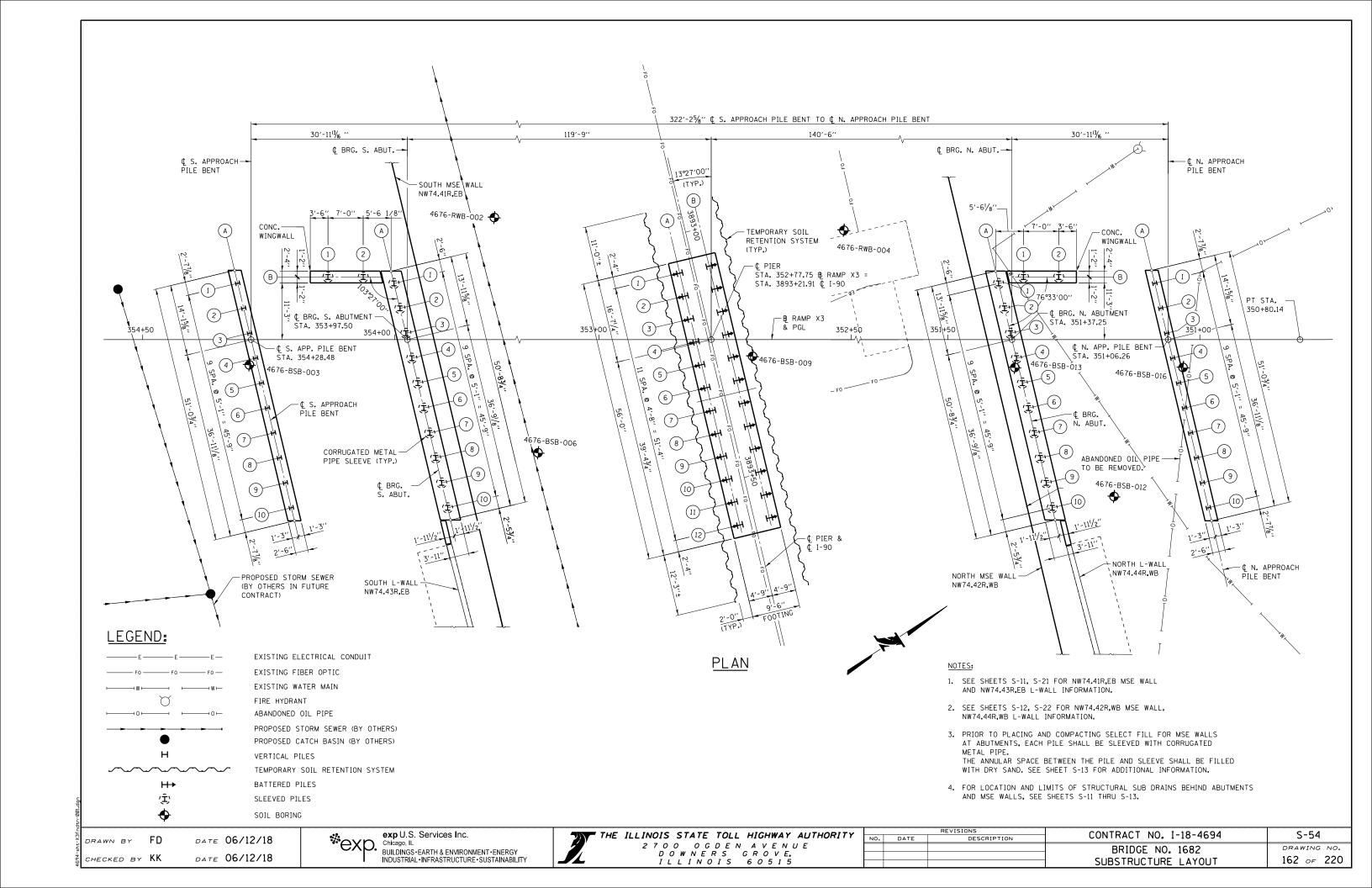
- 1. FOR PILE DETAILS, SEE SHEET S-10.
- 2. FOR SUBSTRUCTURE LAYOUT, SEE SHEET S-07.

3. TOP OF CAP ELEVATION GIVEN AT ¢ APPROACH BENT.

EG DATE 06/12/18 CHECKED BY KK DATE 06/12/18

exp U.S. Services Inc. \*\*exp. Chicago, IL BUILDINGS BUILDINGS • EARTH & ENVIRONMENT • ENERGY INDUSTRIAL • INFRASTRUCTURE • SUSTAINABILITY THE ILLINOIS STATE TOLL HIGHWAY AUTHORITY 2700 OGDEN AVENUE DOWNERS GROVE, ILLINOIS 60515

REVISIONS S-53 CONTRACT NO. I-18-4694 DATE DESCRIPTION BRIDGE NO. 1681 DRAWING NO. 161 <sub>OF</sub> 220 NORTH APPROACH PILE BENT



#### PILE DRIVING RECORD - BN 1682

DATE PILE DRIVEN:(MONTH YEAR)	
TYPE & SIZE PILE USED:	
PILE DRIVING EQUIPMENT USED:	ENERGY RATING:
HAMMER USED: TYPE: STROKE	WEIGHT
FORMULA USED TO CALCULATE CAPACITY:	
PILE DRIVING CONTRACTOR:	CM:

		GROUND			DI	RIVING D	ATA FOR	THE FIN	NAL 5 FT	BLOW	IS		
PILE LOCATION	PILE NUMBER	SURFACE	CUT-OFF ELEVATION	PENETRATED LENGTH, FT.		4′ TO 3′	3′ T0 2′	2' T0 1'	1′ TO 0′	12" TO 6" ••		CAPACITY TONS	REMARKS
PIER													

	GROUND DRIVING DATA FOR THE FINAL 5 FT BLOWS						Г						
PILE	PILE	GROUND SURFACE ELEVATION	CUT-OFF	PENETRATED LENGTH, FT.	5' TO 4'		3' TO 2'					CAPACITY TONS	REMARKS
LOCATION	NOWBER	LLLVATION	LLLVATION	LENOTH, 112	7	3		1	-	0	0	10113	TLIMATUS
S. ABUT.													
S. APP. PILE													
BENT													
N. ABUT.													
									<del>                                     </del>				
													<u> </u>
N. APP. PILE									-				
BENT													

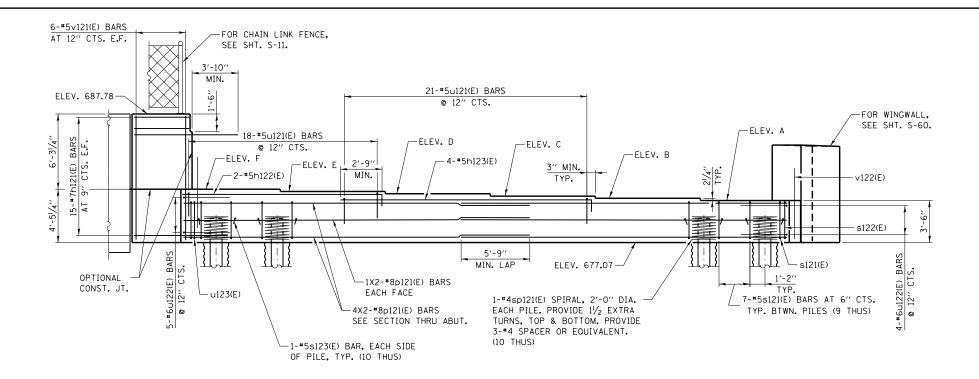
•• FOR PILES DRIVEN TO REFUSAL, BLOW COUNT FOR THE LAST FOOT SHALL BE RECORDED IN 6 INCHES INCREMENTS. PILE DAMAGE, OBSTRUCTION, PILE REJECTION, TEST PILES ETC. SHALL BE RECORDED IN REMARKS COLUMN.

DRAWN BY FD CHECKED BY KK DATE 06/12/18 DATE 06/12/18

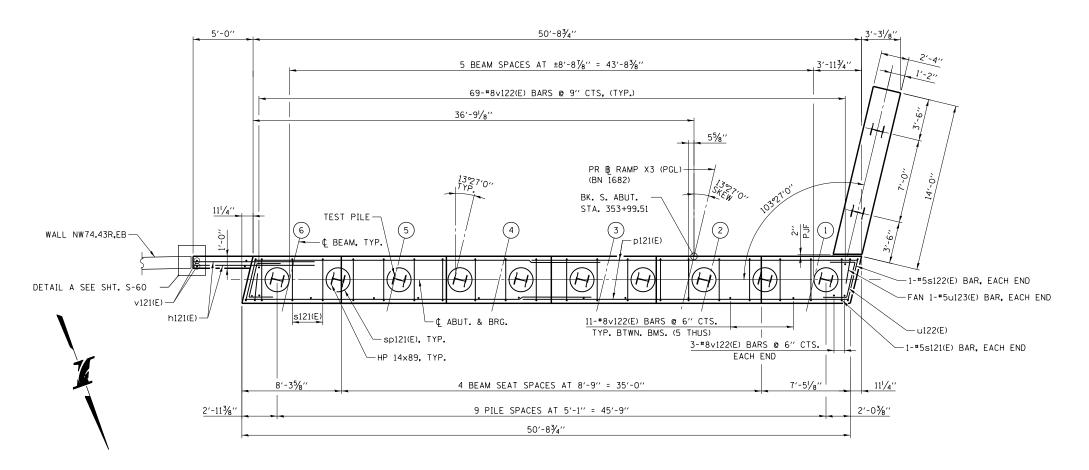
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Chicago, IL
BUILDINGS EARTH & ENVIRONMENT ENERGY
INDUSTRIAL INFRASTRUCTURE SUSTAINABILITY



REVISIONS			CONTRACT NO. I-18-4694	C_EE
NO.	DATE	DESCRIPTION	CUNTRACT NO. 1-10-4694	S-55
			BRIDGE NO. 1682	DRAWING NO.
				167 220
			PILE DRIVING RECORDS	163 of 220



#### ELEVATION LOOKING SOUTH

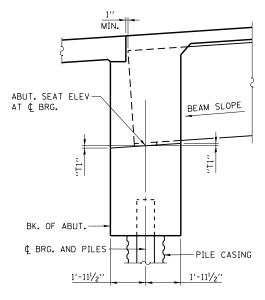


#### PLAN

## ABUTMENT SEAT ELEVATION

### & SEAT SLOPE TABLE

DESIGNATION	ELEVATION	T1
А	680.57	1/4"
В	680.76	1/4′′
С	680.94	0′′
D	681.13	0′′
E	681.32	0′′
F	681.51	0′′



#### CROSS SECTION THRU ABUTMENT

#### NOTES:

- 1. FOR PILE DETAILS, SEE SHEET S-10.
- 2. CONCRETE SEALER TO BE APPLIED ON FRANT FACE AND ENDS OF ABUTMENT CAP & DIAPHRAGM AND FRANT FACE OF WINGWALLS.
- 3. POUR STEPS MONOLITHICALLY WITH CAP.
- 4. CUT TO FIT h121(E) AT 2" PJF.
- 5. FOR ABUTMENT DIAPHRAGM AND BEARING PAD DETAILS, SEE SHEET S-77.

#### PILE DATA - WINGWALL

PILE TYPE: HP 14×89 WITH PILE SHOES NOMINAL REQUIRED BEARING: 187 KIPS FACTORED RESISTANCE AVAILABLE: 47 KIPS EST. PILE LENGTH: 65 FEET NO. PRODUCTION PILES: 2 NO. TEST PILES: 0

#### PILE DATA - ABUTMENT

PILE TYPE: HP 14x89 WITH PILE SHOES NOMINAL REQUIRED BEARING: 600 KIPS FACTORED RESISTANCE AVAILABLE: 274 KIPS EST. PILE LENGTH: 100 FEET NO. PRODUCTION PILES: 9 NO. TEST PILES: 1

JC DATE 06/12/18 DATE 06/12/18 CHECKED BY BGK

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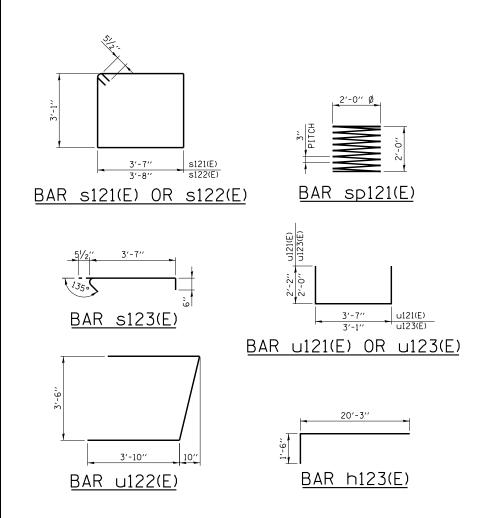
THE ILLINOIS STATE TOLL HIGHWAY AUTHORITY 2700 OGDEN AVENUE DOWNERS GROVE, ILLINOIS 60515

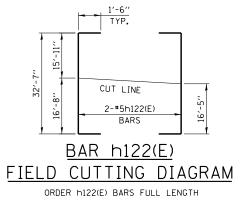
CONTRACT NO. I-18-4694	REVISIONS	S-56
CONTRACT NO. 1-18-4694	DESCRIPTION	3-36
BRIDGE NO. 1682		DRAWING NO.
		164 of 220
SOUTH ABUTMENT PLAN & ELEVA	50	164 of 220

#### v122(E) --2" CHAMFER, TYP. u121(E) h122(E) OR h123(E) p121(E) s123(E) p121(E) s121(E) OR s122(E)sp121(E)p121(E) --24" DIA. CORRUGATED METAL BK. OF ABUT.-PIPE CASING, SEE NOTE 2. ¢ ABUT. AND PILES

#### CROSS SECTION THRU ABUT.

(DIMENSIONS AT RIGHT ANGLE TO ABUTMENT)





#### BILL OF MATERIAL

BAR	No.	SIZE	LENGTH	SHAPE
h121(E)	30	#7	8'-8''	
h122(E)	h122(E) 2 #5		35'-7''	
h123(E)	h123(E) 4 #5		21'-9''	
h124(E)	12	#7	13′-6′′	
h125(E)	12	#7	14'-0''	
p121(E)	20	#8	28′-1′′	
s121(E)	65	#5	14'-3''	<b>□</b> 7
s122(E)	2	#5	14'-5''	7
s123(E)	20	#5	4'-7''	
sp121(E)	10	#4	2'-0''	₹
u121(E)	39	#5	7'-11''	
u122(E)	9	#6	11'-4''	
u123(E)	2	#5	7'-1''	
v121(E)	12	#5	10'-4''	
v122(E)	130	#8	5′-11′′	
v123(E)	36	#5	7'-9''	
[	ESCRIPTIO	N	UNIT	QUANTITY
CONCRET	E STRUCTU	RES	CU YD	41.4
REINFOR( COATED	CEMENT BAI	RS, EPOXY	POUND	6,980
FURNISHI HP14X89	ING STEEL	PILES	FOOT	1,030
DRIVING	PILES		FOOT	1,030
TEST PII	LE STEEL H	HP14X89	EACH	1
PILE SHO	DES		EACH	12
CONCRET	E SEALER	SQ FT	678	
GEOCOMP	OSITE WAL	SQ YD	52	
GRANULA STRUCTU	R BACKFILL RES	CU YD	120	
PIPE UNI STRUCTU	DERDRAINS RES 4"	FOOT	56	
PILE CAS METAL P	SING, CORR IPE, 24"	FOOT	240	

\* LENGTH IS HEIGHT OF SPIRAL.

#### NOTES:

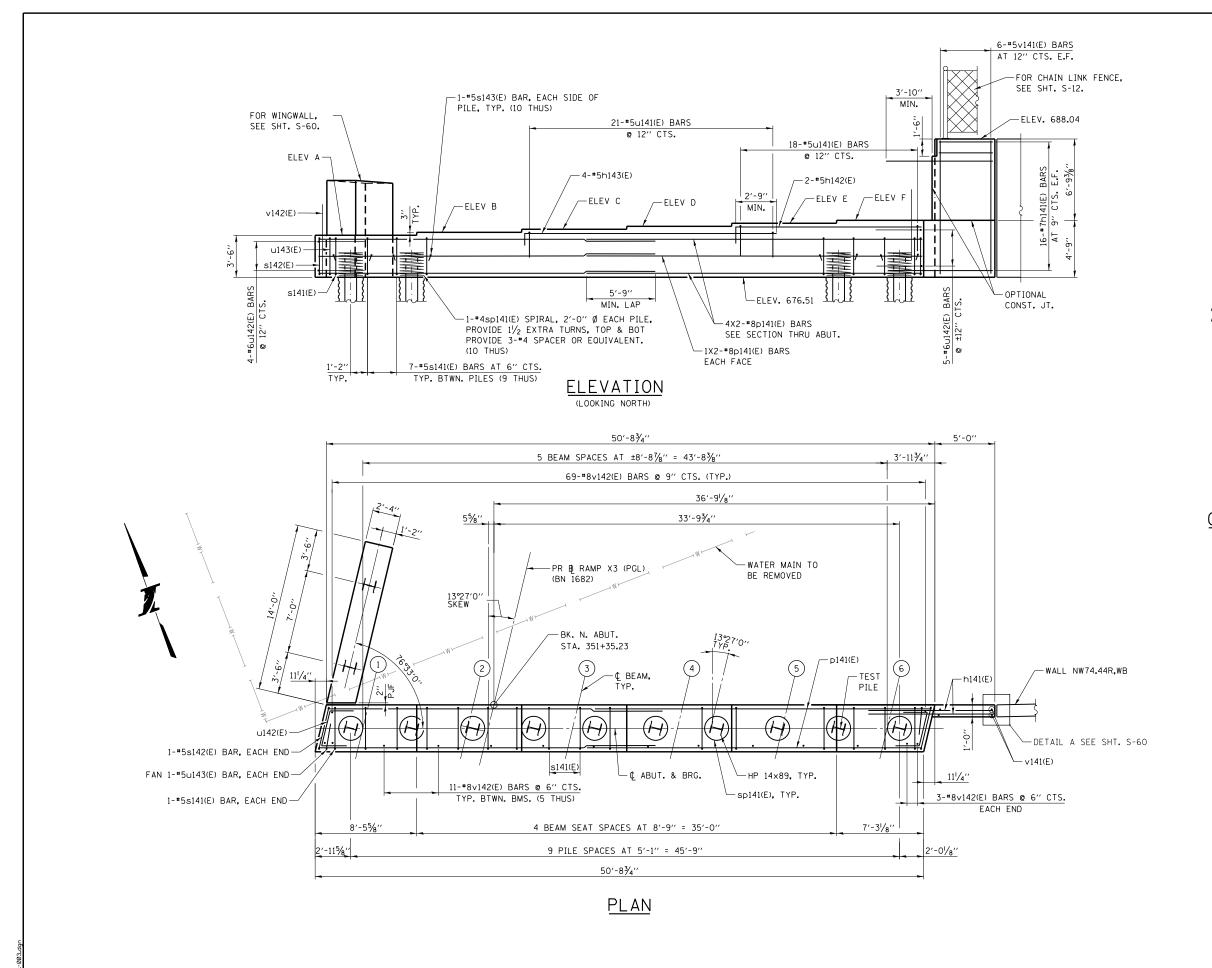
- 1. SEE MSE WALL SHEETS S-11 TO S-13 FOR ABUTMENT DRAINAGE DETAILS.
- 2. SEE MSE WALL SHEET S-13 FOR TYPICAL SECTION THRU ABUTMENT AND MSE WALL.

DATE 06/12/18 DRAWN BY JC DATE 06/12/18 CHECKED BY BGK



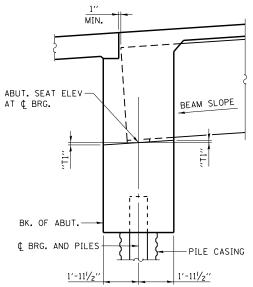
THE	ILLINOIS STATE TOLL HIGHWAY AUTHORITY
<b>2</b>	ILLINOIS STATE TOLL HIGHWAY AUTHORITY 2 7 0 0 0 0 D E N A V E N U E D 0 W N E R S G R 0 V E, I L L I N O I S 6 0 5 1 5
	ILLINOIS 60515

REVISIONS	CONTRACT NO. I-18-4694	S-57	
O. DATE DESCRIPTION	CUNTRACT NO. 1-10-4634	3-51	
	BRIDGE NO. 1682	DRAWING NO.	
		165 <i>o</i> ≥ 220	
	SOUTH ABUTMENT DETAILS	163 OF 220	



#### ABUTMENT SEAT ELEVATION & SEAT SLOPE TABLE

DESIGNATION	ELEVATION	T1
А	680.01	3/8′′
В	680.26	3/8′′
С	680.51	3/8′′
D	680.76	3/8′′
E	681.01	1/4′′
F	681.26	1/4′′



#### CROSS SECTION THRU ABUTMENT

#### NOTES:

- 1. FOR PILE DETAILS, SEE SHEET S-10.
- 2. CONCRETE SEALER TO BE APPLIED ON FRONT FACE AND ENDS OF ABUTMENT CAP & DIAPHRAGM AND FRONT FACE OF WINGWALLS.
- 3. POUR STEPS MONOLITHICALLY WITH CAP.
- 4. CUT TO FIT h141(E) AT 2" PJF.
- 5. FOR ABUTMENT DIAPHRAGM AND BEARING PAD DETAILS, SEE SHEET S-78.

#### PILE DATA - WINGWALL

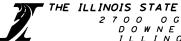
PILE TYPE: HP 14×89 WITH PILE SHOES NOMINAL REQUIRED BEARING: 216 KIPS FACTORED RESISTANCE AVAILABLE: 49 KIPS EST. PILE LENGTH: 58 FEET NO. PRODUCTION PILES: 2 NO. TEST PILES: 0

#### PILE DATA - ABUTMENT

PILE TYPE: HP 14×89 WITH PILE SHOES NOMINAL REQUIRED BEARING: 687 KIPS FACTORED RESISTANCE AVAILABLE: 309 KIPS EST. PILE LENGTH: 104 FEET NO. PRODUCTION PILES: 9 NO. TEST PILES: 1

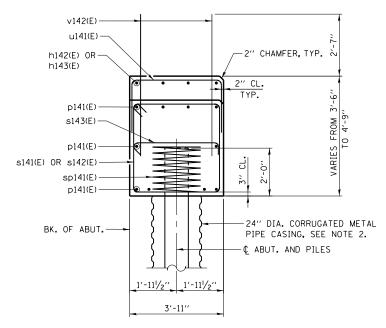
JC DATE 06/12/18 DRAWN BY DATE 06/12/18 CHECKED BY BGK

**GARZA KARHOFF** TENGINEERING, LLC

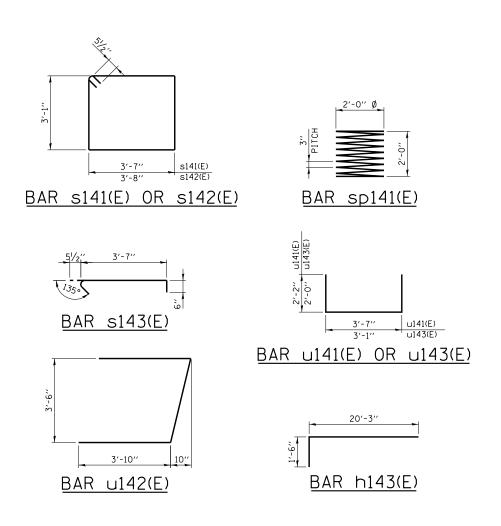


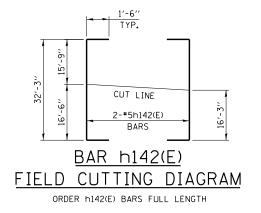
THE ILLINOIS STATE TOLL HIGHWAY AUTHORITY NO. 2700 OGDEN AVENUE DOWNERS GROVE, ILLINOIS 60515

REVISIONS		REVISIONS	CONTRACT NO. I-18-4694	C EO
	DATE	DESCRIPTION	CUNTRACT NO. 1-18-4694	S-58
			BRIDGE NO. 1682	DRAWING NO.
				166 OF 220
			NORTH ABUTMENT PLAN & ELEVATION	100 OF 220



## CROSS SECTION THRU ABUT. (DIMENSIONS AT RIGHT ANGLE TO ABUTMENT)





## BILL OF MATERIAL

BAR	No.	SIZE	LENGTH	SHAPE
h141(E)	32	#7	9'-0''	
h142(E)	2	<b>#</b> 5	35'-3''	
h143(E)	4	#5	21'-9''	
h144(E)	12	#7	13′-6′′	
h145(E)	12	#7	12'-11''	
p141(E)	20	#8	28'-1''	
s141(E)	65	<b>#</b> 5	14'-3''	
s142(E)	2	#5	14'-5''	
s143(E)	20	<b>#</b> 5	4'-7''	
sp141(E)	10	#4	2'-0''	<b>E</b>
u141(E)	39	#5	7′-11′′	
u142(E)	9	#6	11'-4''	5
u143(E)	2	#5	7'-1''	
v141(E)	12	#5	11'-2''	
v142(E)	130	#8	5′-11′′	
∨143(E)	36	#5	7'-7''	
	 DESCRIPTIO	 N	UNIT	QUANTITY
CONCRET	E STRUCTU	RES	CU YD	42.1
	CEMENT BAR		POUND	7,010
FURNISHI HP14X89	NG STEEL	PILES	FOOT	1,052
DRIVING	PILES		FOOT	1,052
TEST PIL	E STEEL H	HP14X89	EACH	1
PILE SHO	DES		EACH	12
CONCRET	E SEALER	SQ FT	708	
GEOCOMP	GEOCOMPOSITE WALL DRAIN			55
GRANULAR BACKFILL FOR STRUCTURES			CU YD	128
PIPE UNI STRUCTU	PIPE UNDERDRAINS FOR STRUCTURES 4"			56
PILE CAS METAL P	SING, CORRI IPE, 24''	UGATED	FOOT	216

\* LENGTH IS HEIGHT OF SPIRAL.

#### NOTES:

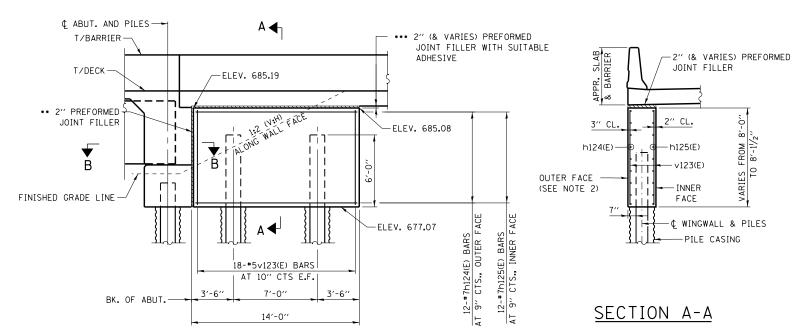
- 1. SEE MSE WALL SHEETS S-11 TO S-13 FOR ABUTMENT DRAINAGE DETAILS.
- 2. SEE MSE WALL SHEET S-13 FOR TYPICAL SECTION THRU ABUTMENT AND MSE WALL.

DRAWN BY JC DATE 06/12/18
CHECKED BY BGK DATE 06/12/18



THE	ILLINOIS STATE TOLL HIGHWAY AUTHORITY
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	ILLINOIS 60515

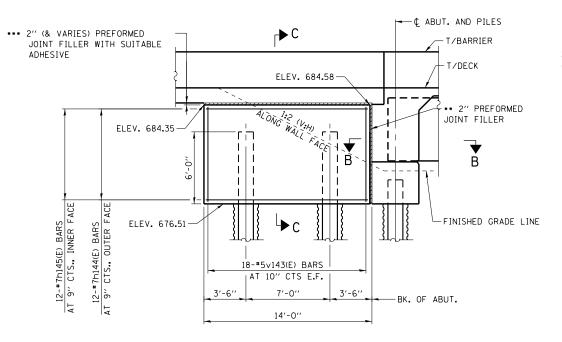
REVISIONS	CONTRACT NO 1-19-4604	S-59	
DESCRIPTION	CUNTRACT NO. 1-10-4634	3-53	
	BRIDGE NO. 1682	DRAWING NO.	
		167 of 220	
	NORTH ABUTMENT DETAILS	167 of 220	

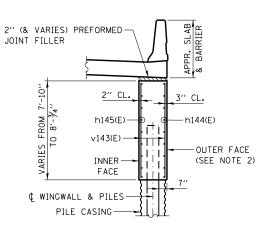


#### SOUTHWEST WINGWALL ELEVATION

(LOOKING EAST)

- •• PREFORMED JOINT FILLER WITH CONCRETE FLAT HD. C.S. 21/2"
  LONG NAILS @ 12" STAGGERED CTS. VERTICALLY.
  ••• ADHESIVE MUST BE COMPATIBLE WITH PREFORMED JOINT FILLER
- MATERIAL AND CONCRETE. SURFACE PREPARATION SHALL BE CONDUCTED IN ACCORDANCE WITH MANUFACTURER'S GUIDELINE.



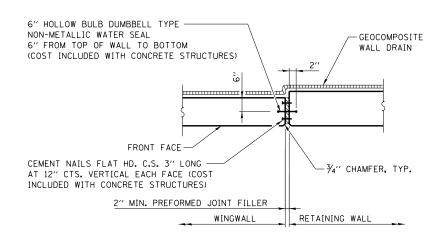


SECTION C-C

#### GEOCOMPOSITE WALL DRAIN-3/8"×1'-4" NEOPRENE SHEET (55 DUROMETER) ATTACHED -FULL HEIGHT AT EDGES TO THE END DIAPHRAGM/BACKWALL AND WINGWALL WITH A 3/8"×5" STEEL PLATE AND 1/2" Ø ANCHOR BOLTS, NUTS AND WASHERS AT 12" CTS. VERTICALLY. INCLUDED WITH THE COST OF HIGH PERFORMANCE CONCRETE SUPERSTRUCTURE. 2" MIN. PREFORMED JOINT FILLER END DIAPHRAGM/ WINGWALL

SECTION B-B NORTH ABUTMENT SECTION SHOWN, SOUTH ABUTMENT SECTION SIMILAR.

BACKWALL



#### DETAIL NORTH ABUTMENT DETAIL SHOWN. SOUTH ABUTMENT DETAIL SIMILAR.

## NOTES:

- 1. FOR REBAR DETAILS AND BILL OF MATERIAL, SEE SHEETS S-57 AND S-59.
- 2. FOR FORM LINER DETAILS, SEE SHEET S-05.
- 3. FOR PILE CASING DETAILS, SEE SHEET S-13.

## NORTHWEST WINGWALL ELEVATION

(LOOKING EAST)

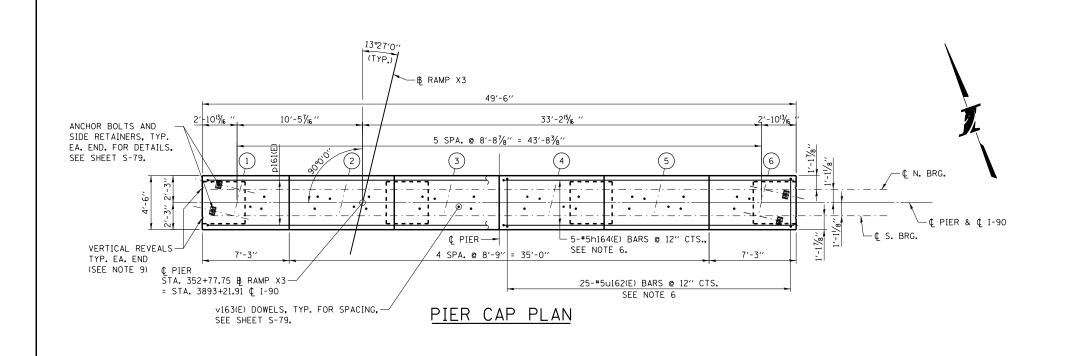
- •• PREFORMED JOINT FILLER WITH CONCRETE FLAT HD. C.S. 21/2" LONG NAILS @ 12" STAGGERED CTS. VERTICALLY.
- \*\*\* ADHESIVE MUST BE COMPATIBLE WITH PREFORMED JOINT FILLER MATERIAL AND CONCRETE. SURFACE PREPARATION SHALL BE CONDUCTED IN ACCORDANCE WITH MANUFACTURER'S GUIDELINE.

THE ILLINOIS STATE TOLL HIGHWAY AUTHORITY 2700 OGDEN AVENUE DOWNERS GROVE, ILLINOIS 60515

CONTRACT NO. I-18-4694 S-60 DESCRIPTION DATE BRIDGE NO. 1682 DRAWING NO. 168 of 220 WINGWALL DETAILS

DATE 06/12/18 DATE 06/12/18 CHECKED BY BGK

**GARZA KARHOFF** ENGINEERING, LLC

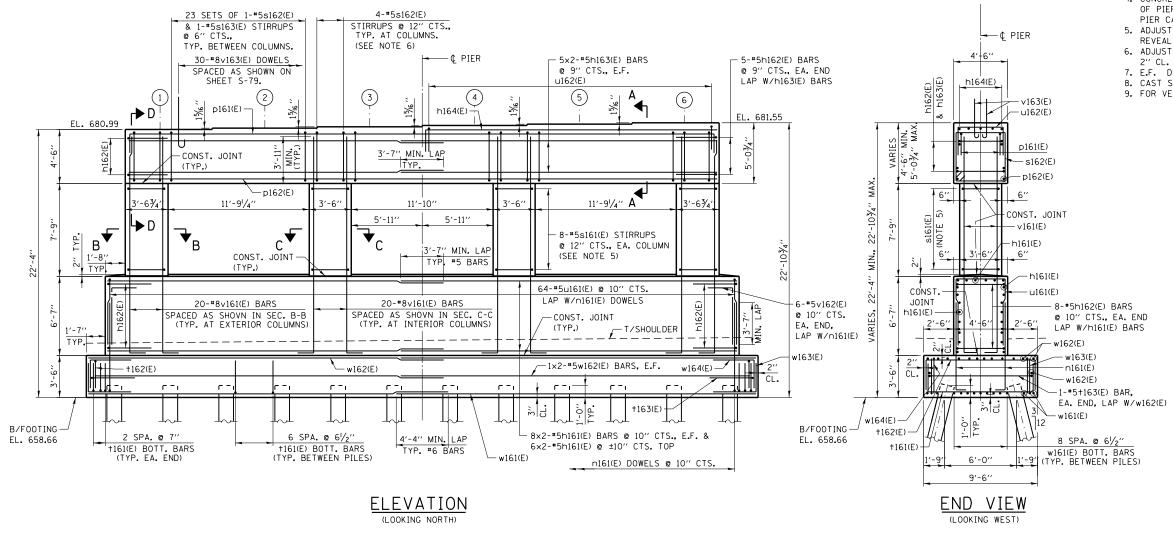


# BRIDGE SEAT ELEVATIONS

BEAM	ELEV. AT	ELEV. AT
NO.	¢ S. BRG.	¢ N. BRG.
1	680.99	680.99
2	681.10	681.10
3	681.21	681.21
4	681.33	681.33
5	681.44	681.44
6	681.55	681.55

#### NOTES:

- 1. FOR BILL OF MATERIAL, SEE SHEET S-62.
- 2. FOR PIER DIAPHRAGM AND BEARING PAD DETAILS, SEE SHEET S-79.
- 3. FOR SECTIONS A-A, B-B, C-C & D-D, SEE SHEET S-62.
- 4. CONCRETE SEALER SHALL BE APPLIED ON VERTICAL FACES AND ENDS OF PIER CAP, COLUMNS AND CRASH WALL AND ON UNDERSIDE OF PIER CAP AND ON TOP OF CRASH WALL.
- ADJUST STIRRUP SPACING IN COLUMN NEAR THE HORIZONTAL REVEAL AS REQUIRED TO MAINTAIN 2" MIN. CONC. COVER.
- 6. ADJUST BAR SPACING IN PIER CAP AS REQUIRED TO MAINTAIN 2" CL. MIN. BETWEEN BAR AND ANCHOR BOLT.
- 7. E.F. DENOTES EACH FACE.
- 8. CAST STEPS MONOLITHICALLY WITH CAP.
- 9. FOR VERTICAL AND HORIZONTAL REVEAL DETAILS, SEE SHEET S-05.



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2 7 0 0 0 G D E N A V E N U E

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CONTRACT NO. I-18-4694 S-61

BRIDGE NO. 1682

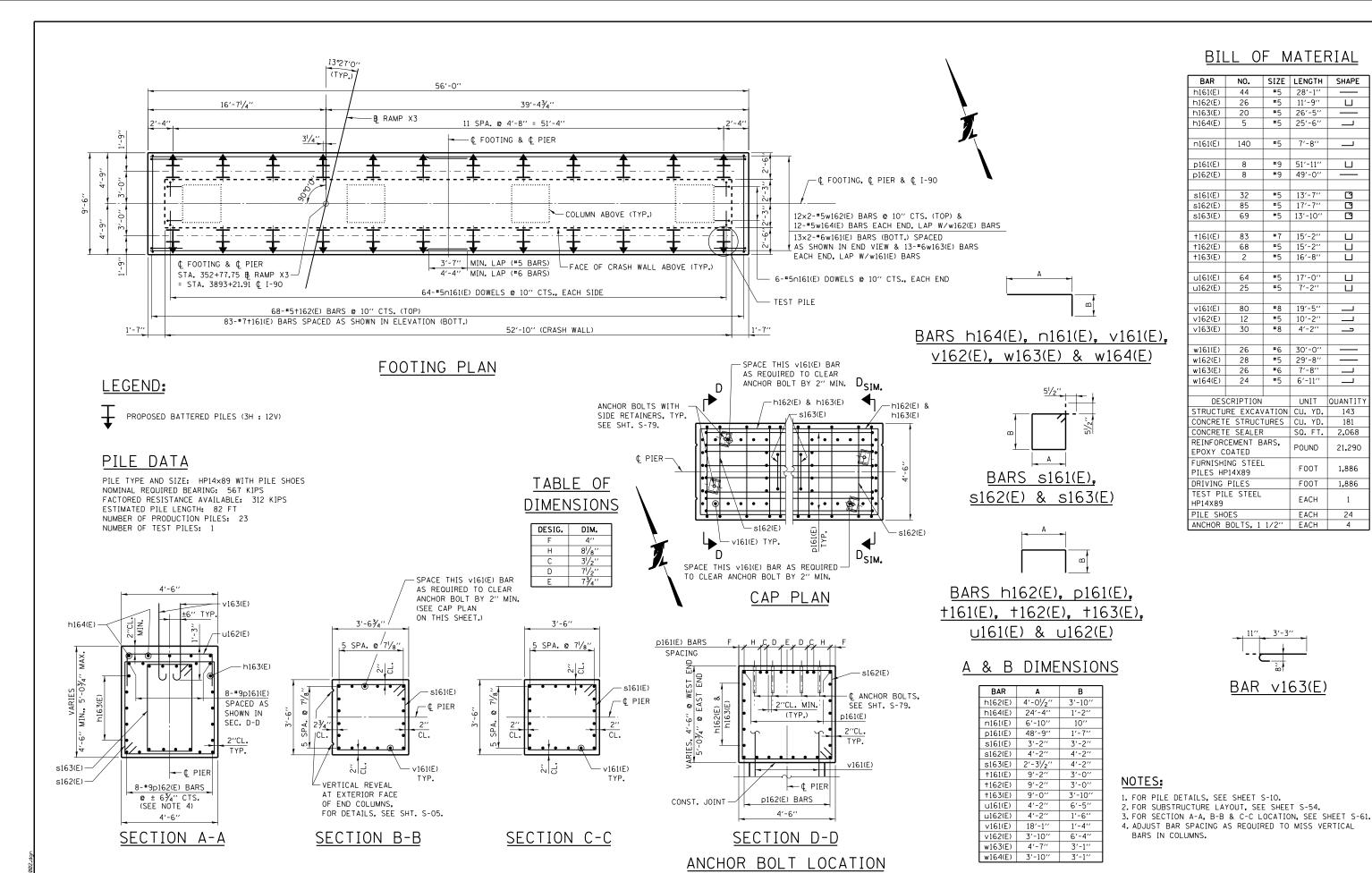
PIER PLAN & ELEVATION

REVISIONS

CONTRACT NO. I-18-4694

S-61

DRAWING NO.
169 OF 220



DRAWN BY FD DATE 06/12/18
CHECKED BY KK DATE 06/12/18

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INDUSTRIAL-INFRASTRUCTURE-SUSTAINABILITY

THE ILLINOIS STATE TOLL HIGHWAY AUTHORITY

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I L L I N O I S 6 0 5 1 5

NO. DATE DESCRIPTION

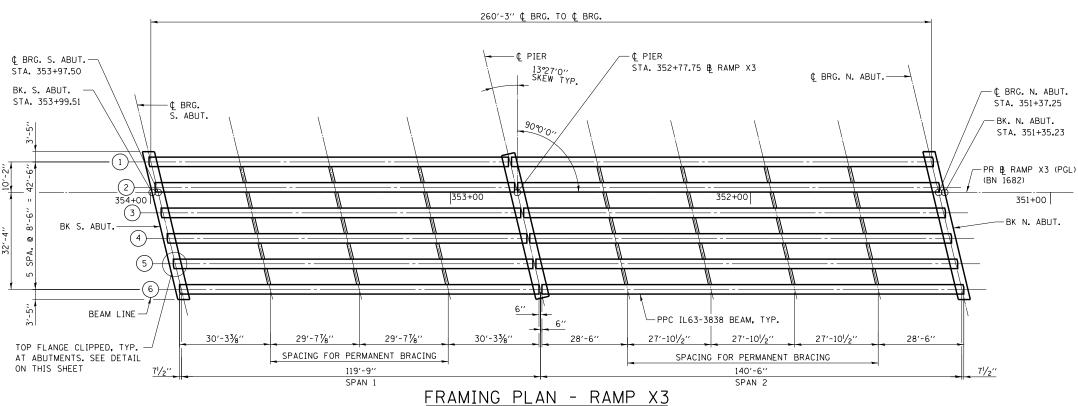
CONTRACT NO. I-18-4694 S-62

BRIDGE NO. 1682

PIER DETAILS

CONTRACT NO. I-18-4694

DRAWING NO. 170 OF 220



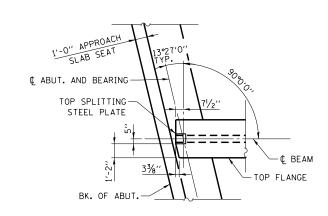
MING PLAN - RAMP X3
(BN 1682)

INTERIOR BEAM MOMENT TABLE				
		0.45 SPAN 1	PIER	0.55 SPAN 2
I	(IN.4)	527,741	527,741	527,741
I'	(IN.4)	1,125,914	1,125,914	1,125,914
Sb	(IN.3)	18,688	18,688	18,688
Sb'	(IN.3)	26,232	26,232	26,232
S†	(IN.3)	15,182	15,182	15,182
St'	(IN.3)	56,071	56,071	56,071
DC1	(K/')	1.979	1.979	1.979
M DC1	('K)	3,512	0	4,834
DC2	(K/')	0.182	0.182	0.182
M DC2	(′K)	146	-393	268
DW	(K/')	0.425	0.425	0.425
M DW	('K)	342	-917	626
M LL + IM	(′K)	2,103	-2,341	2,434

	INTERIOR BEAM REACTION TABLE					
			S. ABUT.	PIER	PIER	N. ABUT.
				SPAN 1	SPAN 2	
	R DC1	(K)	118.5	118.5	139.0	139.0
• •	R DC2	(K)	7.6	14.9	14.9	10.0
*	R DW	(K)	17.8	34.8	34.8	23.3
• •	R LL + IM	(K)	104.7	107.5	107.5	110.2
	R TOTAL	(K)	248.6	275.7	296.2	282.5

- \* BASED ON HL-93.
- •• AT CONTINUOUS PIER(S), REACTIONS FROM COMPOSITE LOADS ARE ASSUMED TO BE EQUALLY DISTRIBUTED TO EACH BEARING LINE.

- I NON-COMPOSITE MOMENT OF INERTIA OF BEAM SECTION (IN.4).
- I' COMPOSITE MOMENT OF INERTIA OF BEAM SECTION (IN.4).
- Sb Non-composite section modulus for the Bottom fiber of the PRESTRESSED BEAM (IN. $^{3}$ ).
- Sb' COMPOSITE SECTION MODULUS FOR THE BOTTOM FIBER OF THE PRESTRESSED BEAM (IN.  $^{\!3}\!$  ).
- S+ NON-COMPOSITE SECTION MODULUS FOR THE TOP FIBER OF THE PRESTRESSED BEAM (IN. 3).
- S+' COMPOSITE SECTION MODULUS FOR THE TOP FIBER OF THE PRESTRESSED BEAM (IN. 3).
- DC1 UN-FACTORED NON-COMPOSITE DEAD LOAD (KIPS/FT.).
- M DC1 UN-FACTORED MOMENT DUE TO NON-COMPOSITE DEAD LOAD (KIP-FT.).
- DC2 UN-FACTORED LONG-TERM COMPOSITE (SUPERIMPOSED EXCLUDING FUTURE WEARING SURFACE) DEAD LOAD (KIPS/FT.).
- M DC2 UN-FACTORED MOMENT DUE TO LONG-TERM COMPOSITE (SUPERIMPOSED EXCLUDING FUTURE WEARING SURFACE) DEAD LOAD (KIP-FT.).
- DW UN-FACTORED LONG-TERM COMPOSITE (SUPERIMPOSED FUTURE WEARING SURFACE ONLY) DEAD LOAD (KIPS/FT.).
- M DW UN-FACTORED MOMENT DUE TO LONG-TERM COMPOSITE (SUPERIMPOSED FUTURE WEARING SURFACE ONLY) DEAD LOAD (KIP-FT.).
- M LL + IM UN-FACTORED LIVE LOAD MOMENT PLUS DYNAMIC LOAD ALLOWANCE (IMPACT) (KIP-FT.).



TOP FLANGE PLAN - CLIPPED

NOTE:

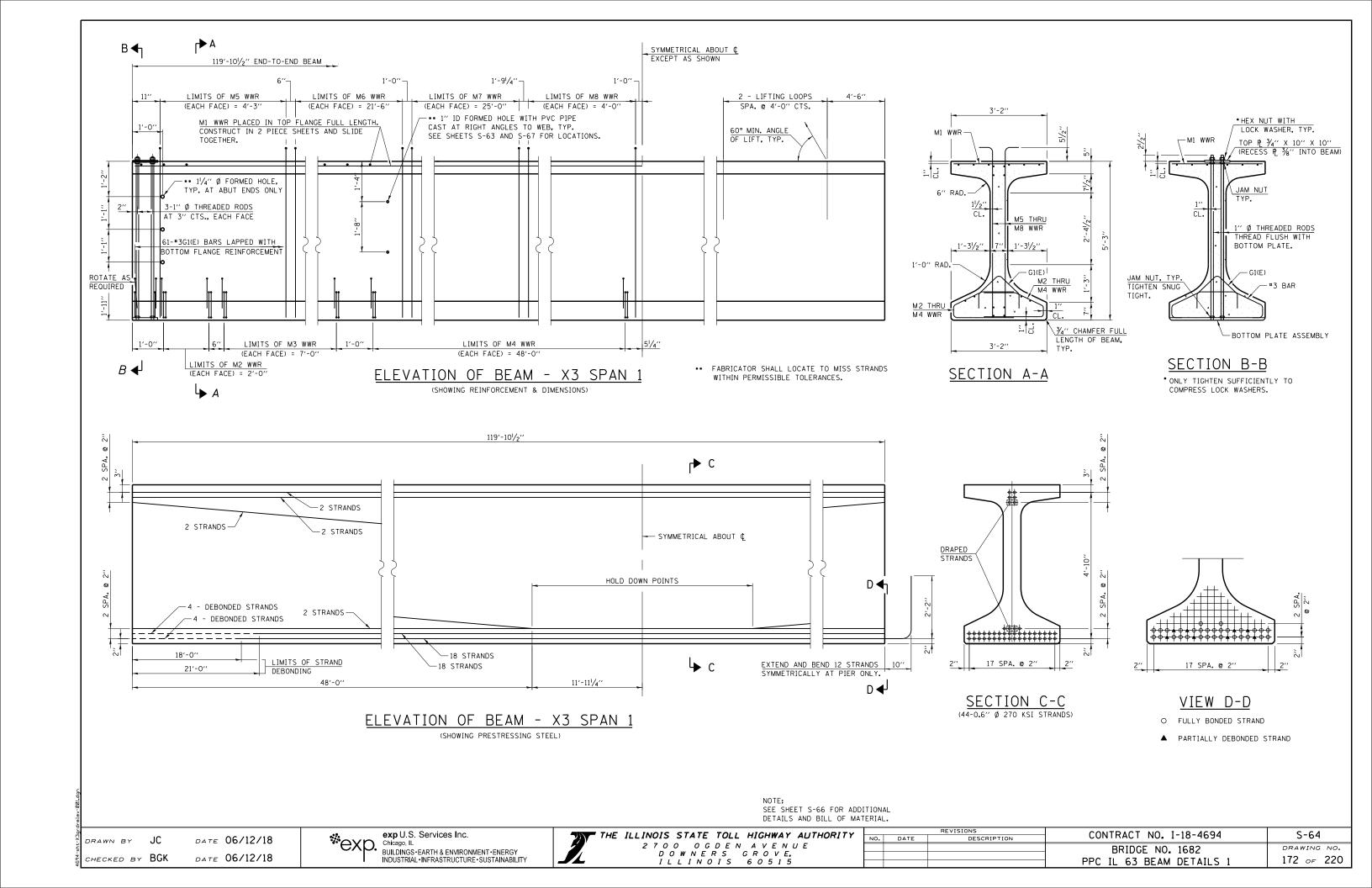
FOR INTERMEDIATE DIAPHRAGM DETAILS, SEE SHEET S-67.

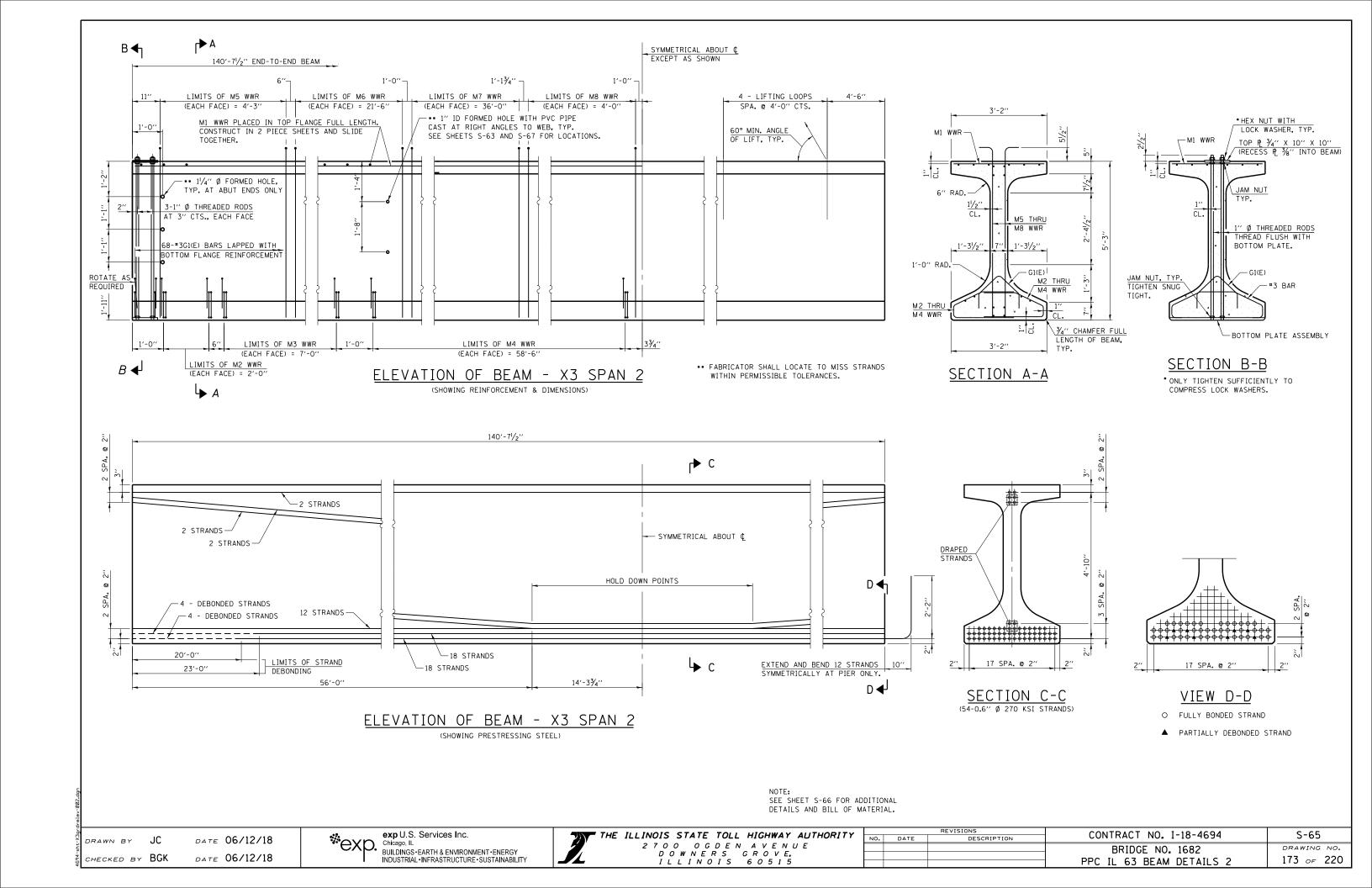
DRAWN BY JC DATE 06/12/18
CHECKED BY BGK DATE 06/12/18

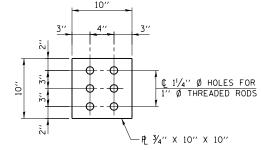
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THE ILLINOIS STATE TOLL HIGHWAY AUTHORITY
2700 OGDEN AVENUE
DOWNERS GROVE,
ILLINOIS 60515

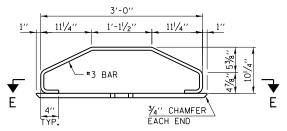
REVISIONS			CONTRACT NO. I-18-4694	S-63	
NO.	DATE	DESCRIPTION	CONTRACT NO. 1-10-4634	3-63	
			BRIDGE NO. 1682	DRAWING NO.	
				171 220	
			FRAMING PLAN	171 of 220	



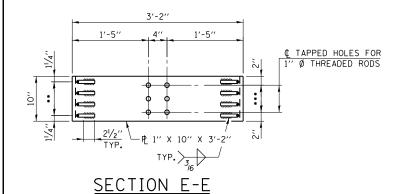


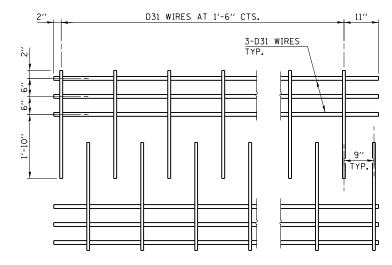


#### PLAN - TOP PLATE



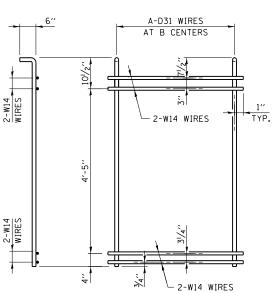
ELEVATION - BOTTOM PLATE ASSEMBLY





#### M1 WWR DETAIL

WHEN MULTIPLE SHEETS OF M1 WWR ARE REQUIRED ALONG THE BEAM LENGTH, #5(E) BARS (5'-0" LONG) SHALL BE USED TO SPLICE THE LONGITUDINAL D31 WIRES TOGETHER (MIN. LAP 2'-2").

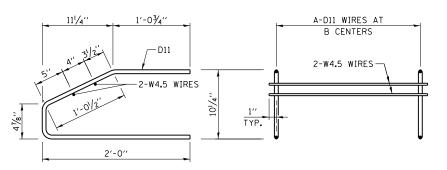


#### M5 THRU M8 WWR DETAIL (SEE TABLE OF DIMENSIONS)

#### TABLE OF DIMENSIONS

$\leq$	<u>SPAN</u>	<u>1</u>
WWR	А	В
M2	9	3′′
М3	15	6′′
M4	33	1'-6''
M5	18	3''
М6	44	6′′
М7	26	1'-0''
М8	3	2'-0''

SPAN 2					
WWR	Α	В			
M2	9	3"			
М3	15	6′′			
M4	40	1'-6''			
M5	18	3′′			
М6	44	6"			
М7	37	1'-0''			
М8	3	2'-0''			

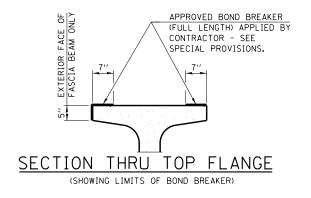


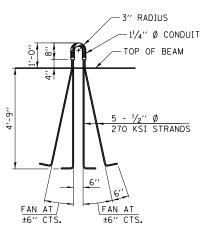
## M2 THRU M4 WWR DETAIL

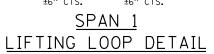
(SEE TABLE OF DIMENSIONS)

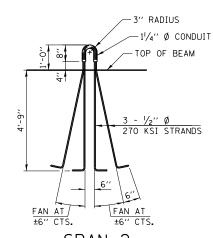
#### NOTES:

- 1. INSERTS FOR  $\frac{3}{4}$ "  $\phi$  THREADED DOWEL RODS, WHEN SPECIFIED, ARE TO BE TWO STRUT, FERRULE TYPE FOR INTERIOR BEAMS AND SINGLE FERRULE, FLARED LOOP TYPE FOR EXTERIOR BEAMS.
- 2. PRESTRESSING STEEL SHALL BE UNCOATED HIGH STRENGTH, LOW RELAXATION 7-WIRE STRAND, GRADE 270. THE NOMINAL DIAMETER FOR BEAM STRANDS SHALL BE 0.6" AND THE NOMINAL CROSS-SECTIONAL AREA SHALL BE 0.217 SO. IN. THE NOMINAL DIAMETER FOR LIFTING LOOPS SHALL BE 1/2" AND THE NOMINAL CROSS SECTIONAL AREA SHALL BE
- 3. THE BEAMS SHALL HAVE A FINAL CONCRETE COMPRESSIVE STRENGTH, f'c, OF 8500 PSI AND A RELEASE CONCRETE COMPRESSIVE STRENGTH, f'ci, OF 7000 PSI.
- 4. A MINIMUM  $2^{1}\!/_{2}$ "  $\emptyset$  LIFTING PIN SHALL BE USED TO ENGAGE THE LIFTING LOOPS DURING HANDLING.
- 5. BEND THE EXTENDED STRANDS INWARD ON THE FASCIA BEAMS TO MAINTAIN  $1/\!\!/_2$ " CLEARANCE INSIDE THE PIER DIAPHRAGM.
- 6. THE TOP AND BOTTOM PLATES SHALL BE AASHTO M270 GRADE 50.
  7. THE TOP PLATES AND BOTTOM PLATE ASSEMBLIES SHALL BE GALVANIZED ACCORDING TO AASHTO M111. THE THREADED RODS, NUTS AND WASHERS SHALL BE GALVANIZED ACCORDING TO AASHTO M232.
- 8. THREADED RODS SHALL BE ASTM F 1554 GRADE 55.
  9. BEAMS SHALL NOT BE RELEASED FROM THE FABRICATOR UNTIL THEY HAVE ATTAINED 45 DAYS OF AGE OR OLDER.
- 10.WELDED WIRE REINFORCEMENT (WWR) SHALL CONFORM TO ASTM A884 WITH A CLASS A, TYPE 1 EPOXY COATING.





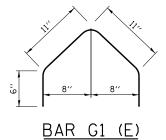




SPAN 2 LIFTING LOOP DETAIL

#### BILL OF MATERIAL

ITEM	UNIT	QUANTITY
FURNISHING AND ERECTING PRECAST PRESTRESSED CONCRETE BEAMS, IL63	FOOT	1563



\*\* 3 SPACES AT 21/2" = 71/2" \*\*\* 2 SPACES AT 3" = 6"

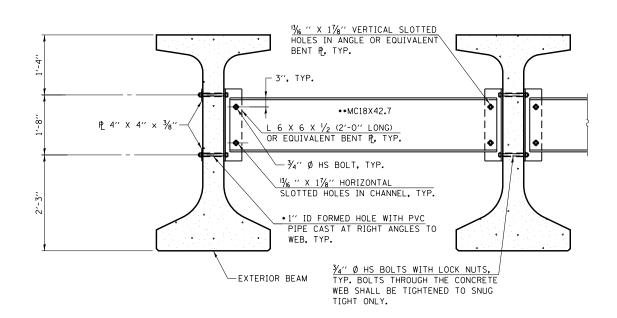
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THE ILLINOIS STATE TOLL HIGHWAY AUTHORITY 2700 OGDEN AVENUE DOWNERS GROVE, ILLINOIS 60515

REVISIONS			CONTRACT NO. I-18-4694	5-66
NO.	DATE	DESCRIPTION	CUNTRACT NO. 1-10-4694	S-66
			BRIDGE NO. 1682	DRAWING NO.
				174 of 220
			PPC IL 63 BEAM DETAILS 3	114 of 220

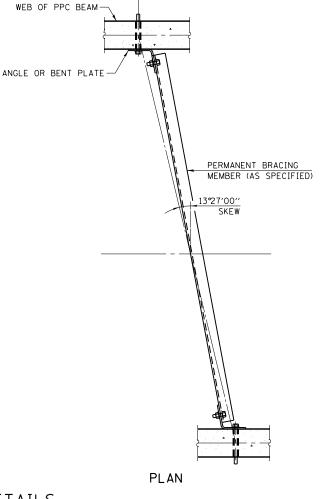
DATE 06/12/18 DATE 06/12/18 CHECKED BY BGK

\*\*exp. Chicago, IL BUILDINGS



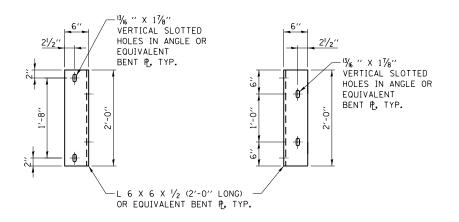
### NOTES:

- 1. ALL MATERIAL FOR BRACING SHALL BE HOT DIP GALVANIZED ACCORDING TO AASHTO M111 UNLESS OTHERWISE NOTED.
  2. TWO HARDENED WASHERS ARE REQUIRED FOR EACH SET OF OVERSIZED HOLES.
- 3. ALL HOLES SHALL BE  $^{1}\!\%$  " Ø UNLESS OTHERWISE NOTED. 4. % " X 3" X 3" PLATE WASHERS ARE REQUIRED OVER ALL
- SLOTTED HOLES.
- 5. ALL BOLTS SHALL BE GALVANIZED ACCORDING TO AASHTO M232. 6. BRACING SHALL BE INSTALLED AS BEAMS ARE ERECTED AND
- TIGHTENED AS SOON AS POSSIBLE DURING ERECTION.
  7. PERMANENT BRACING SHALL NOT BE PAID FOR SEPARATELY, BUT SHALL BE INCLUDED IN THE COST OF FURNISHING AND ERECTING PRECAST PRESTRESSED CONCRETE BEAMS.
- \* FABRICATOR SHALL LOCATE TO MISS STRANDS WITHIN PERMISSIBLE TOLERANCES.
- \*\* ALTERNATE MC18X45.8 CHANNELS ARE PERMITTED TO FACILITATE MATERIAL ACQUISITION.



¢ INSERTS AND BOLTS-

PERMANENT BRACING DETAILS



BEAM FACE

DIAPHRAGM FACE

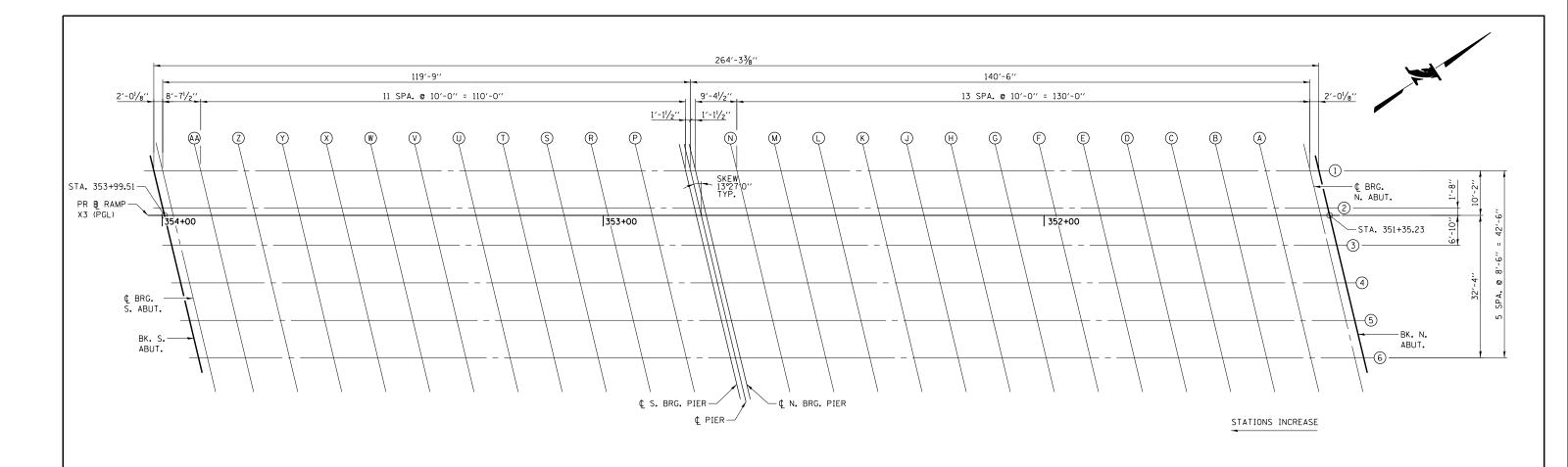
DIAPHRAGM SUPPORT

JC DRAWN BY CHECKED BY BGK DATE 06/12/18 DATE 06/12/18 \*\*CXP. Chicago, IL BUILDINGS

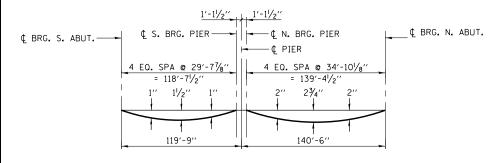
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REVISIONS			CONTRACT NO. I-18-4694	S_67
NO.	DATE	DESCRIPTION	CUNTRACT NO. 1-10-4694	S-67
			BRIDGE NO. 1682	DRAWING NO.
				175 oF 220
			PPC IL 63 BEAM DETAILS 4	113 of 220



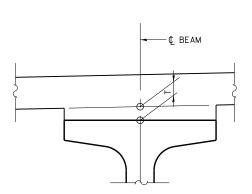
#### PLAN - TOP OF SLAB ELEVATIONS



#### DEAD LOAD DEFLECTION DIAGRAM

(INCLUDES WEIGHT OF CONCRETE ONLY.)

NOTE:
THE ABOVE DEFLECTIONS ARE NOT TO BE USED IN THE FIELD IF THE ENGINEER IS WORKING FROM THE GRADE ELEVATIONS ADJUSTED FOR DEAD LOAD DEFLECTIONS AS SHOWN ON SHEETS S-69 TO S-71.



TO DETERMINE "T": AFTER ALL BEAMS HAVE BEEN ERECTED, ELEVATIONS OF THE TOP FLANGES OF THE BEAMS SHALL BE TAKEN AT INTERVALS SHOWN ABOVE. THESE ELEVATIONS SUBTRACTED FROM THE "THEORETICAL GRADE ELEVATIONS ADJUSTED FOR DEAD LOAD DEFLECTION" SHOWN ON SHEETS S-69 TO S-71, MINUS SLAB THICKNESS, EQUALS THE FILLET HEIGHTS "T" ABOVE TOP FLANGE OF BEAMS.

## FILLET HEIGHTS

DRAWN BY EG DATE 06/12/18
CHECKED BY CCE DATE 06/12/18

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THE ILLINOIS STATE TOLL HIGHWAY AUTHORITY

2 7 0 0 0 G D E N A V E N U E

D O W N E R S G R O V E,

I L L I N O I S 6 0 5 1 5

REVISIONS			CONTRACT NO. I-18-4694	S-68
NO.	DATE	DESCRIPTION	CONTRACT NO. 1-10-4634	3-60
			BRIDGE NO. 1682	DRAWING NO.
				176 of 220
			TOP OF SLAB PLAN	176 of 220

BEAM 2 X3 PGL BEAM 1

			1	
LOCATION	STATION	OFFSET	THEORETICAL GRADE ELEVATIONS	THEORETICAL GRADE ELEVATIONS ADJUSTED FOR DEAD LOAD DEFLECTION
BK. N. ABUT.	351+37.67	10.17	686.12	686.12
CL BRG. N. ABUT.	351+39.68	10.17	686.15	686.15
А	351+49.68	10.17	686.30	686 <b>.</b> 35
В	351+59.68	10.17	686.43	686 <b>.</b> 53
С	351+69.68	10.17	686.56	686.70
D	351+79.68	10.17	686.67	686.85
Е	351+89.68	10.17	686.76	686.97
F	351+99.68	10.17	686.84	687.06
G	352+09.68	10.17	686.91	687.13
Н	352+19.68	10.17	686.96	687.18
J	352+29.68	10.17	687.01	687.21
K	352+39.68	10.17	687.05	687.22
L	352+49.68	10.17	687.08	687.21
М	352+59.68	10.17	687.10	687.20
N	352+69.68	10.17	687.12	687.17
CL N. BRG. PIER	352+79.05	10.17	687.13	687.13
CL PIER	352+80.18	10.17	687.13	687.13
CL S. BRG. PIER	352+81.30	10.17	687.13	687.13
Р	352+91.30	10.17	687.13	687.16
R	353+01.30	10.17	687.13	687.18
S	353+11.30	10.17	687.12	687.19
T	353+21.30	10.17	687.10	687.19
U	353+31.30	10.17	687.07	687.18
٧	353+41.30	10.17	687.04	687.15
W	353+51.30	10.17	687.00	687.10
Χ	353+61.30	10.17	686.95	687.05
Υ	353+71.30	10.17	686.90	686.97
Z	353+81.30	10.17	686.84	686.89
AA	353+91.30	10.17	686.77	686.79
CL BRG. S. ABUT.	353+99.93	10.17	686.70	686.70
BK. S. ABUT.	354+01.94	10.17	686.69	686.69

LOCATION	STATION	OFFSET	THEORETICAL GRADE ELEVATIONS	THEORETICAL GRADE ELEVATIONS ADJUSTED FOR DEAD LOAD DEFLECTION
BK. N. ABUT.	351+35.63	1.67	686.44	686.44
CL BRG. N. ABUT.	351+37.65	1.67	686.46	686.46
A	351+47.65	1.67	686.57	686 <b>.</b> 62
В	351+57.65	1.67	686.67	686.78
С	351+67.65	1.67	686.77	686.92
D	351+77.65	1.67	686.86	687.04
E	351+87.65	1.67	686.94	687.15
F	351+97.65	1.67	687.01	687.23
G	352+07 <b>.</b> 65	1.67	687.07	687.30
Н	352+17.65	1.67	687.12	687.35
J	352+27.65	1.67	687.17	687.38
K	352+37.65	1.67	687.21	687.39
L	352+47.65	1.67	687.24	687.38
М	352+57 <b>.</b> 65	1.67	687 <b>.</b> 27	687.37
N	352+67.65	1.67	687.29	687.33
CL N. BRG. PIER	352+77.02	1.67	687.30	687.30
CL PIER	352+78.15	1.67	687.30	687.30
CL S. BRG. PIER	352+79.27	1.67	687.30	687.30
Р	352+89.27	1.67	687.30	687.33
R	352+99.27	1.67	687.30	687.36
S	353+09.27	1.67	687.29	687.37
T	353+19.27	1.67	687 <b>.</b> 27	687.37
U	353+29 <b>.</b> 27	1.67	687.25	687.36
V	353+39.27	1.67	687.21	687.33
w	353+49.27	1.67	687.18	687.29
X	353+59 <b>.</b> 27	1.67	687.13	687.23
Y	353+69.27	1.67	687.08	687.16
Z	353+79 <b>.</b> 27	1.67	687.02	687 <b>.</b> 07
AA	353+89.27	1.67	686.95	686.98
CL BRG. S. ABUT.	353+97.90	1.67	686.89	686.89
BK. S. ABUT.	353+99.91	1.67	686.87	686.87

LOCATION	STATION	OFFSET	THEORETICAL GRADE ELEVATIONS	THEORETICAL GRADE ELEVATIONS ADJUSTED FOR DEAD LOAD DEFLECTION
BK. N. ABUT.	351+35.23	0.00	686.50	686.50
CL BRG. N. ABUT.	351+37.25	0.00	686.52	686 <b>.</b> 52
Α	351+47.25	0.00	686.63	686.68
В	351+57.25	0.00	686.72	686.83
С	351+67.25	0.00	686.81	686.96
D	351+77.25	0.00	686.89	687.08
E	351+87.25	0.00	686.97	687.18
F	351+97.25	0.00	687.04	687.27
G	352+07.25	0.00	687.10	687.33
н	352+17.25	0.00	687.15	687.38
J	352+27.25	0.00	687.20	687.41
К	352+37.25	0.00	687.24	687.42
L	352+47.25	0.00	687.27	687.42
М	352+57 <b>.</b> 25	0.00	687.30	687.40
N	352+67.25	0.00	687.32	687.37
CL N. BRG. PIER	352+76.62	0.00	687.33	687.33
CL PIER	352+77.75	0.00	687.33	687.33
CL S. BRG. PIER	352+78.87	0.00	687.33	687.33
Р	352+88.87	0.00	687.34	687.37
R	352+98.87	0.00	687.33	687.39
S	353+08.87	0.00	687.32	687.40
Т	353+18.87	0.00	687.30	687.41
U	353+28.87	0.00	687.28	687.39
V	353+38.87	0.00	687.25	687.37
w	353+48.87	0.00	687.21	687.32
X	353+58.87	0.00	687.17	687.27
Υ	353+68.87	0.00	687.11	687.19
Z	353+78.87	0.00	687.05	687.11
АА	353+88.87	0.00	686.99	687.01
CL BRG. S. ABUT.	353+97.50	0.00	686.92	686.92
BK. S. ABUT.	353+99.51	0.00	686.91	686.91

DRAWN BY EG CHECKED BY CCE DATE 06/12/18 DATE 06/12/18

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REVISIONS		REVISIONS	CONTRACT NO. I-18-4694	S-69
NO.	DATE	DESCRIPTION	CUNTRACT NO. 1-10-4634	3-63
			BRIDGE NO. 1682	DRAWING NO.
				177 OF 220
			TOP OF SLAB ELEVATIONS 1	111 OF 220

BEAM 4 BEAM 3 BEAM 5

LOCATION	STATION	OFFSET	THEORETICAL GRADE ELEVATIONS	THEORETICAL GRADE ELEVATIONS ADJUSTED FOR DEAD LOAD DEFLECTION
BK. N. ABUT.	351+33.60	-6.83	686.77	686.77
CL BRG. N. ABUT.	351+35.61	-6.83	686.78	686.78
А	351+45.61	-6.83	686.86	686.91
В	351+55.61	-6.83	686.93	687.03
С	351+65.61	-6.83	686.99	687.14
D	351+75.61	-6.83	687.05	687.24
Е	351+85.61	-6.83	687.11	687.32
F	351+95.61	-6.83	687.17	687.40
G	352+05.61	-6.83	687.23	687.46
Н	352+15.61	-6.83	687.28	687.51
J	352+25.61	-6.83	687.33	687.54
K	352+35.61	-6.83	687.37	687 <b>.</b> 55
L	352+45.61	-6.83	687.40	687 <b>.</b> 55
М	352+55.61	-6.83	687.43	687 <b>.</b> 53
N	352+65.61	-6.83	687.45	687.50
CL N. BRG. PIER	352+74.99	-6.83	687.46	687.46
CL PIER	352+76.11	-6.83	687.47	687.47
CL S. BRG. PIER	352+77.24	-6.83	687.47	687.47
Р	352+87.24	-6.83	687.47	687.50
R	352+97.24	-6.83	687.47	687 <b>.</b> 53
S	353+07.24	-6.83	687.46	687 <b>.</b> 55
T	353+17.24	-6.83	687.44	687 <b>.</b> 55
U	353+27.24	-6.83	687.42	687 <b>.</b> 54
٧	353+37.24	-6.83	687.39	687.51
W	353+47.24	-6.83	687.35	687.47
X	353+57.24	-6.83	687.31	687.41
Υ	353+67.24	-6.83	687.26	687.34
Z	353+77.24	-6.83	687.20	687.26
AA	353+87.24	-6.83	687.14	687.16
CL BRG. S. ABUT.	353+95.86	-6.83	687.07	687.07
BK. S. ABUT.	353+97.88	-6.83	687.06	687.06

LOCATION	STATION	OFFSET	THEORETICAL GRADE ELEVATIONS	THEORETICAL GRADE ELEVATIONS ADJUSTED FOR DEAD LOAD DEFLECTION
BK. N. ABUT.	351+31.57	-15.33	687.11	687.11
CL BRG. N. ABUT.	351+33 <b>.</b> 58	-15.33	687.12	687.12
A	351+43 <b>.</b> 58	-15.33	687.16	687.22
В	351+53.58	-15.33	687.20	687.30
С	351+63.58	-15.33	687 <b>.</b> 23	687.38
D	351+73.58	-15.33	687.26	687.45
E	351+83.58	-15.33	687.30	687.51
F	351+93.58	-15.33	687.34	687 <b>.</b> 57
G	352+03.58	-15.33	687.39	687.62
Н	352+13.58	-15.33	687.44	687.67
J	352+23.58	-15.33	687.49	687.70
К	352+33.58	-15.33	687 <b>.</b> 53	687.71
L	352+43.58	-15.33	687 <b>.</b> 57	687.71
M	352+53 <b>.</b> 58	-15.33	687.60	687.69
N	352+63.58	-15.33	687.62	687.67
CL N. BRG. PIER	352+72.96	-15.33	687.63	687.63
CL PIER	352+74.08	-15.33	687.63	687.63
CL S. BRG. PIER	352+75.21	-15.33	687.63	687.63
P	352+85.21	-15.33	687.64	687.67
R	352+95.21	-15.33	687.64	687.70
S	353+05.21	-15.33	687.63	687.72
Т	353+15 <b>.</b> 21	-15.33	687.62	687.72
U	353+25.21	-15.33	687.60	687.71
V	353+35.21	-15.33	687.57	687.69
W	353+45.21	-15.33	687 <b>.</b> 53	687.65
X	353+55.21	-15.33	687.49	687.59
Y	353+65 <b>.</b> 21	-15.33	687.44	687.52
Z	353+75 <b>.</b> 21	-15.33	687.38	687.44
AA	353+85 <b>.</b> 21	-15.33	687.32	687.35
CL BRG. S. ABUT.	353+93.83	-15.33	687.26	687.26
BK. S. ABUT.	353+95.84	-15.33	687.24	687.24

LOCATION	STATION	OFFSET	THEORETICAL GRADE ELEVATIONS	THEORETICAL GRADE ELEVATIONS ADJUSTED FOR DEAD LOAD DEFLECTION
BK. N. ABUT.	351+29.53	-23.83	687.47	687.47
CL BRG. N. ABUT.	351+31.55	-23.83	687.47	687.47
A	351+41.55	-23.83	687.48	687 <b>.</b> 53
В	351+51.55	-23.83	687.48	687.59
С	351+61.55	-23.83	687.48	687.63
D	351+71.55	-23.83	687.48	687.66
E	351+81.55	-23.83	687.49	687.70
F	351+91.55	-23.83	687.51	687.74
G	352+01.55	-23.83	687.55	687.78
н	352+11.55	-23.83	687.60	687.83
J	352+21 <b>.</b> 55	-23.83	687.65	687.86
К	352+31.55	-23.83	687.69	687.87
L	352+41.55	-23.83	687.73	687.87
M	352+51 <b>.</b> 55	-23.83	687.76	687.86
N	352+61 <b>.</b> 55	-23.83	687.78	687.83
CL N. BRG. PIER	352+70.92	-23.83	687.80	687.80
CL PIER	352+72.05	-23.83	687.80	687.80
CL S. BRG. PIER	352+73.17	-23.83	687.80	687.80
Р	352+83.17	-23.83	687.81	687.84
R	352+93.17	-23.83	687.81	687.87
S	353+03.17	-23.83	687.80	687.89
Т	353+13.17	-23.83	687.79	687.90
U	353+23.17	-23.83	687.77	687.89
V	353+33.17	-23.83	687.74	687.86
w	353+43.17	-23.83	687.71	687.83
X	353+53.17	-23.83	687.67	687.77
Y	353+63.17	-23.83	687.62	687.70
Z	353+73.17	-23.83	687.56	687.62
АА	353+83.17	-23.83	687.50	687.53
CL BRG. S. ABUT.	353+91.80	-23.83	687.44	687.44
BK. S. ABUT.	353+93.81	-23.83	687.43	687.43

DRAWN BY EG

CHECKED BY CCE

DATE 06/12/18 DATE 06/12/18

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INDUSTRIAL-INFRASTRUCTURE-SUSTAINABILITY



		REVISIONS	CONTRACT NO. I-18-4694	S-70
NO.	DATE	DESCRIPTION	CUNTRACT NO. 1-10-4634	3-10
			BRIDGE NO. 1682	DRAWING NO.
				170 220
			TOP OF SLAB ELEVATIONS 2	178 of 220

#### BEAM 6

LOCATION	STATION	OFFSET	THEORETICAL GRADE ELEVATIONS	THEORETICAL GRADE ELEVATIONS ADJUSTED FOR DEAD LOAD DEFLECTION
BK. N. ABUT.	351+27.50	-32.33	687.84	687.84
CL BRG. N. ABUT.	351+29 <b>.</b> 51	-32.33	687.84	687.84
Α	351+39 <b>.</b> 51	-32.33	687.81	687.86
В	351+49.51	-32.33	687.78	687.88
С	351+59 <b>.</b> 51	-32.33	687.74	687.89
D	351+69.51	-32.33	687.71	687 <b>.</b> 89
E	351+79.51	-32.33	687.69	687.90
F	351+89.51	-32.33	687.69	687.91
G	351+99.51	-32.33	687.72	687.94
Н	352+09.51	-32.33	687.76	687.98
J	352+19.51	-32.33	687.81	688.01
К	352+29.51	-32.33	687.86	688.03
L	352+39.51	-32.33	687.89	688.03
М	352+49.51	-32.33	687.93	688.02
N	352+59.51	-32.33	687.95	688.00
CL N. BRG. PIER	352+68.89	-32.33	687.97	687.97
CL PIER	352+70.01	-32.33	687.97	687.97
CL S. BRG. PIER	352+71.14	-32.33	687.97	687.97
Р	352+81.14	-32.33	687.98	688.01
R	352+91.14	-32.33	687.98	688.04
S	353+01.14	-32.33	687.98	688.06
Т	353+11.14	-32.33	687.96	688.07
U	353+21.14	-32.33	687.95	688.06
V	353+31.14	-32.33	687.92	688.04
W	353+41.14	-32.33	687.89	688.00
X	353+51.14	-32.33	687.85	687.95
Y	353+61.14	-32.33	687.80	687 <b>.</b> 88
Z	353+71.14	-32.33	687.75	687.80
AA	353+81.14	-32.33	687.69	687.71
CL BRG. S. ABUT.	353+89.76	-32.33	687.63	687.63
BK. S. ABUT.	353+91.78	-32.33	687.61	687.61

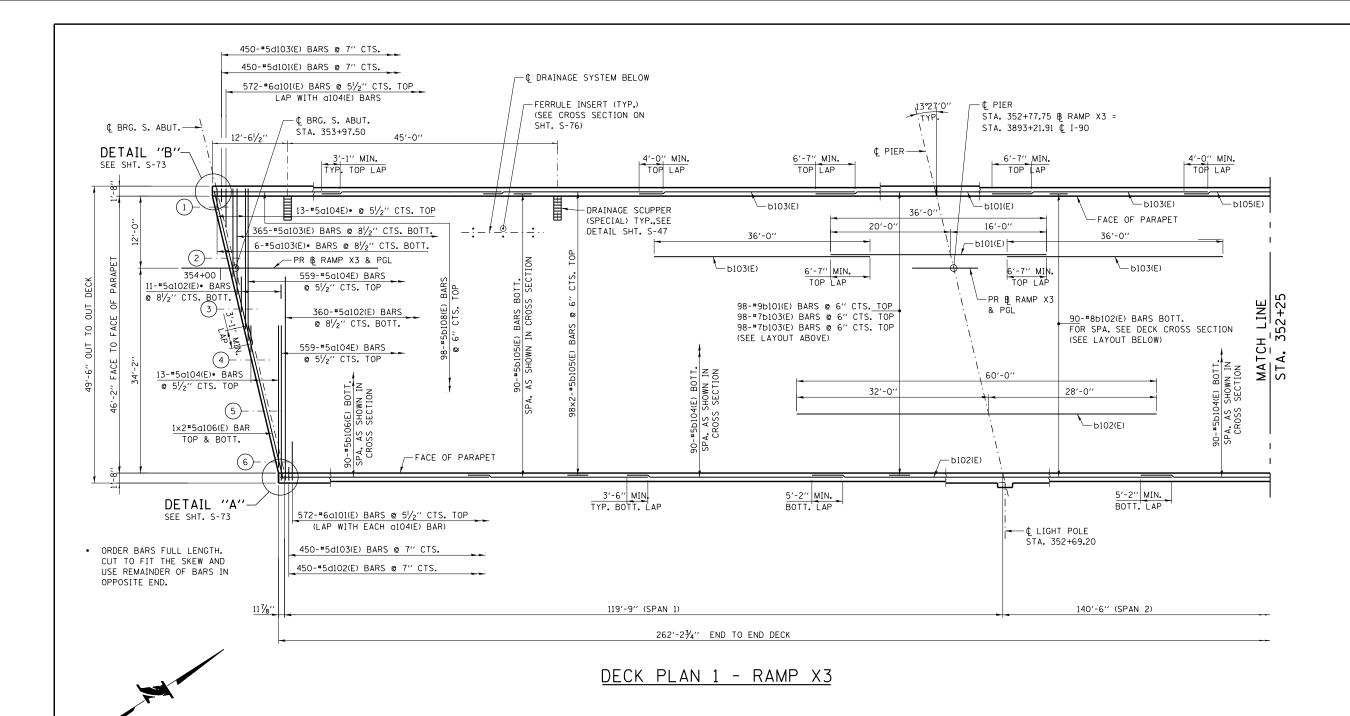
DRAWN BY EG CHECKED BY CCE

DATE 06/12/18 DATE 06/12/18

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REVISIONS		REVISIONS	CONTRACT NO. I-18-4694	S-71
NO.	DATE	DESCRIPTION	CONTRACT NO. 1-10-4634	3-71
			BRIDGE NO. 1682	DRAWING NO.
				179 oF 220
			TOP OF SLAB ELEVATIONS 3	119 of 220



NOTES:

FOR NOTES SEE SHT. S-73

DRAWN BY HBJ DATE 06/12/18
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THE ILLINOIS STATE TOLL HIGHWAY AUTHORITY

2 7 0 0 0 G D E N A V E N U E

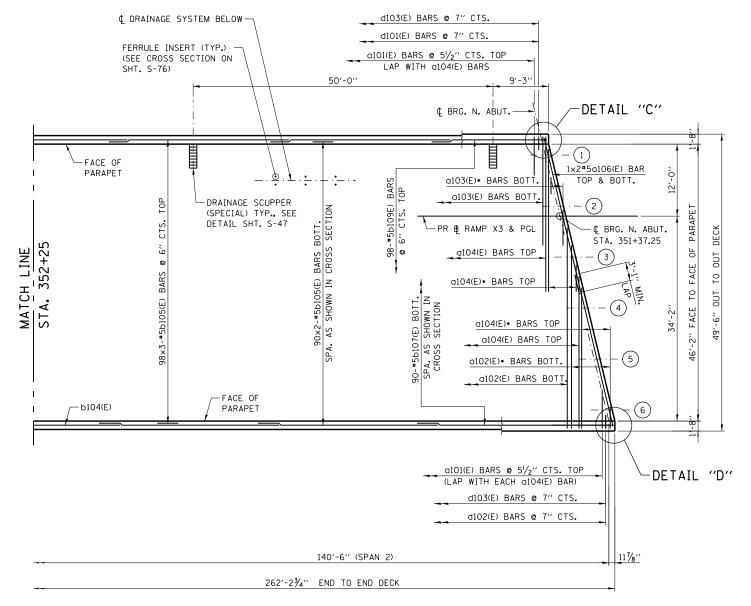
D O W N E R S G R O V E.

I L L I N O I S 6 0 5 1 5

CONTRACT NO. I-18-4694 S-72

BRIDGE NO. 1682

DECK PLAN 1 180 of 220

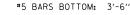


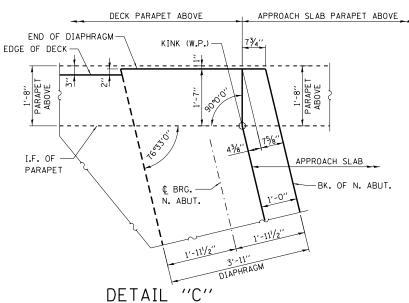
DECK PLAN 2 - RAMP X3



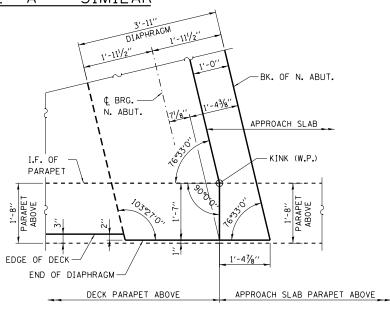
#### NOTES:

- 1. SEE SHT. S-76 FOR DECK CROSS SECTION.
- 2. SEE SHT. S-81 FOR SUPERSTRUCTURE DETAILS.
- 3. SEE SHT. S-82 FOR BAR LIST AND SUPERSTRUCTURE
- 4. SEE SHT. S-77, S-78 AND S-79 FOR DIAPHRAGM DETAILS.
- 5. SEE SHT. S-80 FOR PARAPET REINFORCEMENT.
- 6. SEE SHT. S-81 FOR PARAPET DETAILS AT LIGHT POLES.
- 7. SEE SHT. S-46 AND S-47 FOR DRAINAGE AND SCUPPER DETAILS.
- 8. BARS INDICATED THUS 69x3-#5 ETC. INDICATES 69 LINES OF BARS WITH 3 LENGTHS PER LINE.
- 9. LAP SPLICES SHALL BE AS FOLLOWS UNLESS NOTED OTHERWISE:
  - #5 BARS TOP: 3'-1"

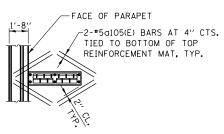




DETAIL "A" - SIMILAR



DETAIL "D" <u>DETAIL "B" - SIMILAR</u>



NOTE: CUT LONGITUDINAL REINFORCEMENT TO CLEAR DRAINAGE SCUPPERS.

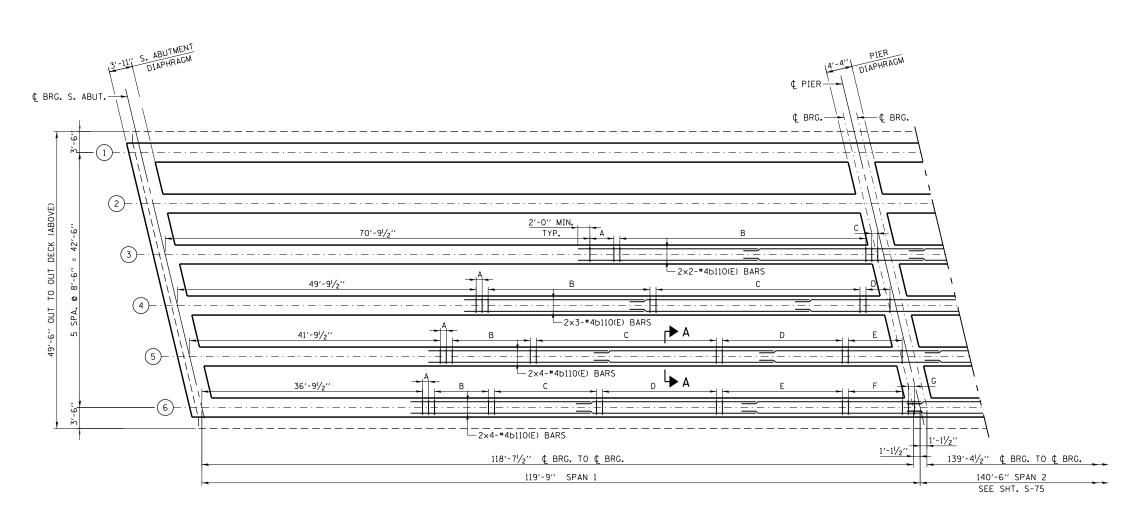
DETAIL "E" - ADD'L REINF. AT SCUPPER

HBJ DATE 06/12/18 DRAWN BY DATE 06/12/18 CHECKED BY CCE

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CONTRACT NO. I-18-4694 S-73 DATE DESCRIPTION DRAWING NO. BRIDGE NO. 1682 181 of 220 DECK PLAN 2





# REFLECTED DECK PLAN - SPAN 1

# TABLE OF STIRRUPS - SPAN 1 \*

BEAM	Α	В	С	D	E	F	G
1	-	-	-	-	-	-	-
2	-	-	-	-	-	-	-
3	5-#4s101(E)	42-#4s102(E)	2-#4s103(E)	-	-	-	-
4	2-#4s101(E)	28-#4s102(E)	35-#4s103(E)	5-#4s104(E)	-	-	-
5	2-#4s101(E)	14-#4s102(E)	31-#4s103(E)	21-#4s104(E)	10-#4s105(E)	-	-
6	2-#4s101(E)	10-#4s102(E)	18-#4s103(E)	20-#4s104(E)	21-#4s105(E)	10-#4s106(E)	2-#4s107(E)

\* STIRRUPS SPACED @ 12" CTS. TYP.

# FILLET WIDTH D(E) BARS SEE DECK PLANS FOR BAR DESIGNATIONS TOP/BEAM SECTION A-A SECTION A-A

#### NOTES:

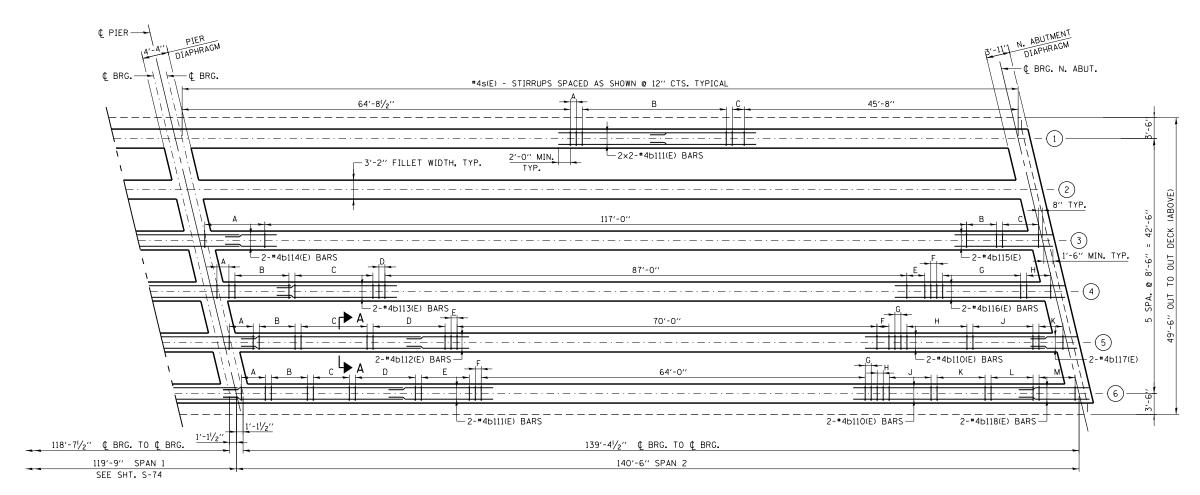
- 1. STIRRUPS SPACED @ 12" CTS. TYP.
- 2. SEE SHT. S-72 FOR DECK SLAB REINFORCEMENT.
- 3. SEE SHT. S-82 FOR BAR LIST AND BILL OF MATERIAL.
- 4. BARS INDICATED THUS 2×3-\*5 ETC. INDICATES 2 LINES OF BARS WITH 3 LENGTHS PER LINE.
- 5. LAP SPLICES SHALL BE AS FOLLOWS UNLESS NOTED OTHERWISE: #4 BARS: 2'-5"

DRAWN BY HBJ DATE 06/12/18
CHECKED BY CCE DATE 06/12/18

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	REVISIONS	CONTRACT NO. I-18-4694	S-74
DATE	DESCRIPTION	CUNTRACT NO. 1-10-4634	3-14
		BRIDGE NO. 1682	DRAWING NO.
			182 OF 220
		DECK PLAN 3	182 of 220





# REFLECTED DECK PLAN - SPAN 2

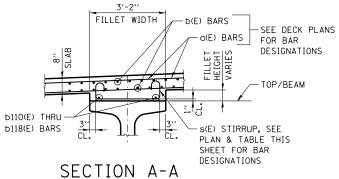
# TABLE OF STIRRUPS - SPAN 2 \*

BEAM	Α	В	С	D	E	F	G	н	J	К	L	М
1	2-#4s101(E)	25-#4s102(E)	3-#4s101(E)	-	-	-	-	-	-	-	-	-
2	-	-	-	-	-	-	-	-	-	-	-	-
3	11-#4s102(E)	6-#4s101(E)	7-#4s103(E)	-	-	-	-	-	-	-	-	-
4	3-#4s104(E)	10-#4s103(E)	14-#4s102(E)	2-#4s101(E)	4-#4s101(E)	2-#4s102(E)	14-#4s103(E)	5-#4s105(E)	-	-	-	-
5	5-#4s105(E)	7-#4s104(E)	12-#4s103(E)	13-#4s102(E)	2-#4s101(E)	3-#4s101(E)	2-#4s102(E)	11-#4s103(E)	11-#4s105(E)	5-#4s107(E)	-	-
6	5-#4s106(E)	7-#4s105(E)	7-#4s104(E)	11-#4s103(E)	9-#4s102(E)	2-#4s101(E)	2-#4s101(E)	2-#4s102(E)	8-#4s103(E)	9-#4s105(E)	8-#4s107(E)	7-#4s108(E)

• STIRRUPS SPACED @ 12" CTS. TYP.

#### NOTES:

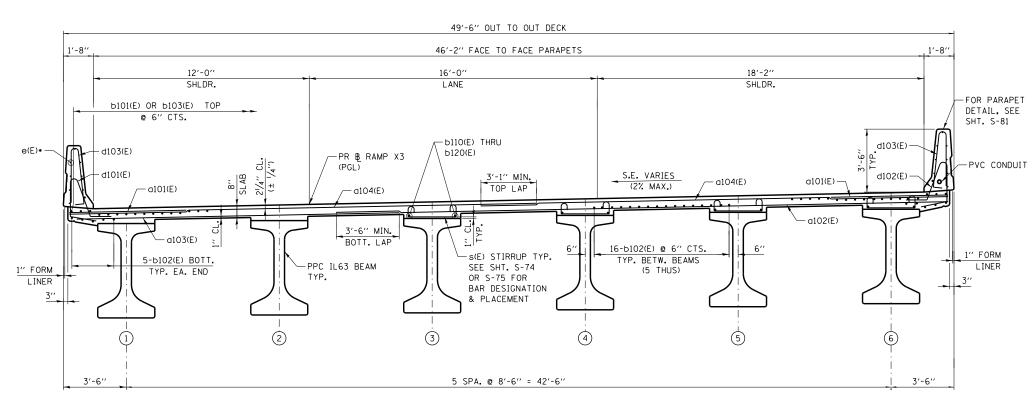
- 1. STIRRUPS SPACED @ 12" CTS. TYP.
- 2. SEE SHTS. S-72 & S-73 FOR DECK SLAB REINFORCEMENT.
- 3. SEE SHT. S-82 FOR BAR LIST AND BILL OF MATERIALS.
- 4. BARS INDICATED THUS 2×3-#5 ETC. INDICATES 2 LINES OF BARS WITH 3 LENGTHS PER LINE.
- 5. LAP SPLICES SHALL BE AS FOLLOWS UNLESS NOTED OTHERWISE: #4 BARS : 2'-5"



HBJ DATE 06/12/18 DATE 06/12/18 CHECKED BY CCE

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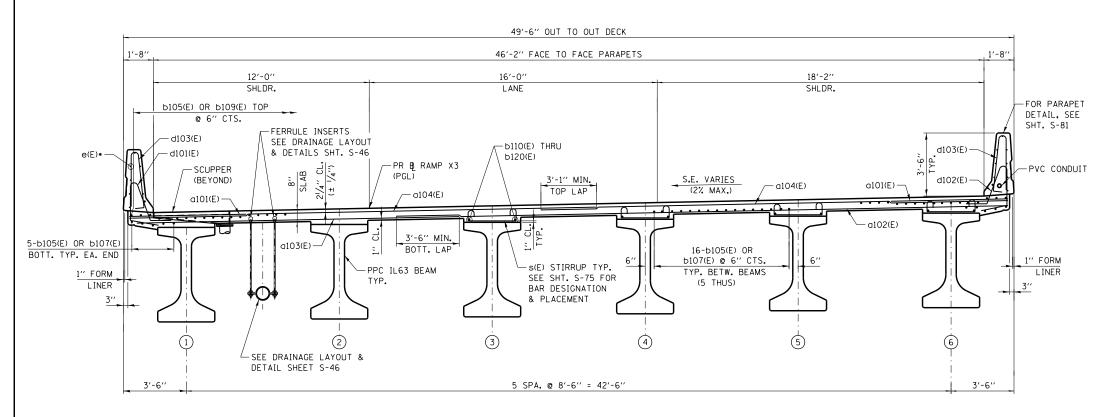
CONTRACT NO. I-18-4694 S-75 DATE DESCRIPTION DRAWING NO. BRIDGE NO. 1682 183 of 220 DECK PLAN 4



# CROSS SECTION - NEAR PIER

• SEE SHT. S-80 AND S-81 FOR PARAPET BAR DESIGNATIONS.

LOOKING NORTH



## CROSS SECTION - NEAR ABUTMENT

LOOKING NORTH

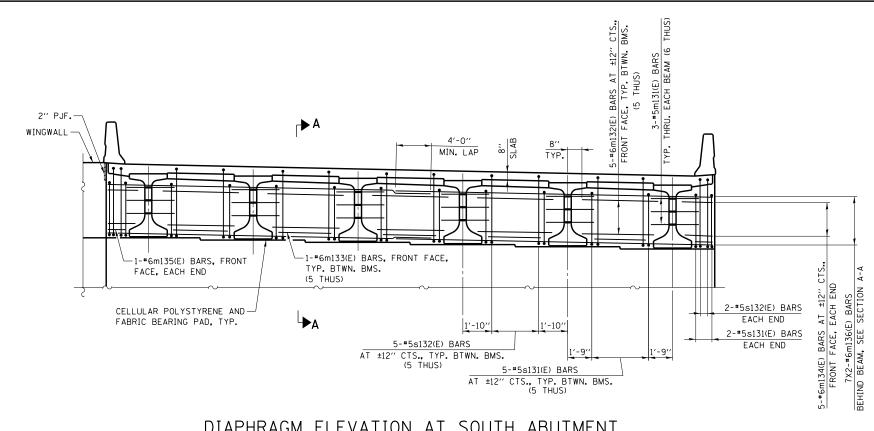
DRAWN BY HBJ DATE 06/12/18

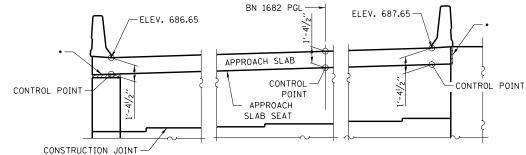
CHECKED BY CCE DATE 06/12/18

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THE	ILLINOIS STATE TOLL HIGHWAY AUTHORITY
	2700 OGDEN AVENUE DOWNERS GROVE.
	ILLINOIS 60515

	REVISIONS		CONTRACT NO. I-18-4694	S-76
NO.	DATE	DESCRIPTION	CONTRACT NO. 1-10-4634	3-16
			BRIDGE NO. 1682	DRAWING NO.
			DECK CROSS SECTION	184 of 220

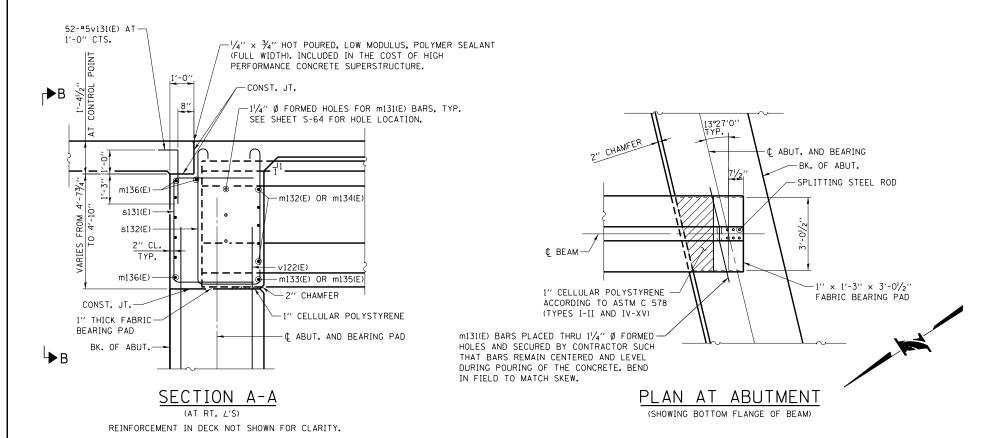




# SECTION B-B

• 2" MIN. PJF (PER ARTICLE 1051.09 OF THE STANDARD SPECIFICATIONS) BONDED TO WINGWALL WITH SUITABLE ADHESIVE AS RECOMMENDED BY SUPPLIER.

# DIAPHRAGM ELEVATION AT SOUTH ABUTMENT



# NOTES:

- 1. FOR BAR LIST IN DIAPHRAGM SEE SHEET S-82.
- 2. FOR BILL OF MATERIAL SEE SHEET S-82.
- 3. FOR DETAILS OF BARS s131(E), s132(E) AND v131(E) SEE SHEET S-82.
- 4. THE s131(E), AND s132(E) BARS SHALL BE PLACED PARALLEL TO THE BEAMS. SPACING FOR THESE BARS SHALL BE RIGHT ANGLES TO THE
- 5. THE APPROACH SLAB SEAT SHALL HAVE A CONSTANT SLOPE DETERMINED FROM THE CONTROL POINTS SHOWN.
- 6. COST OF CELLULAR POLYSTYRENE IS INCLUDED WITH HIGH PERFORMANCE CONCRETE SUPERSTRUCTURE.

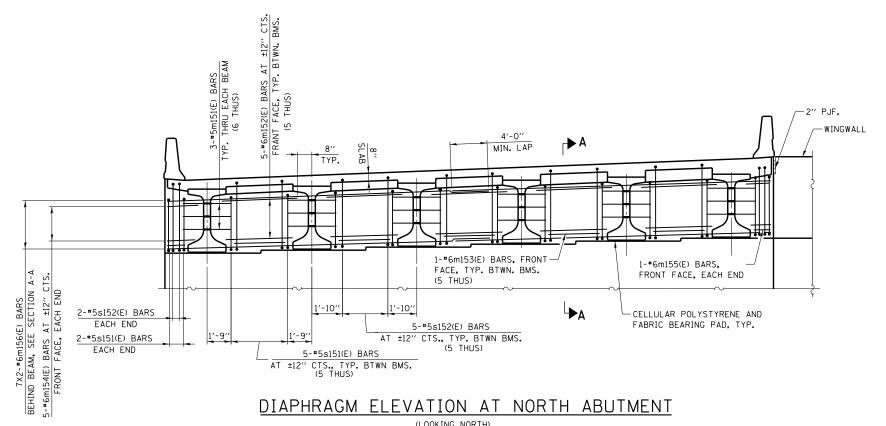
DATE 06/12/18 DATE 06/12/18 CHECKED BY BGK

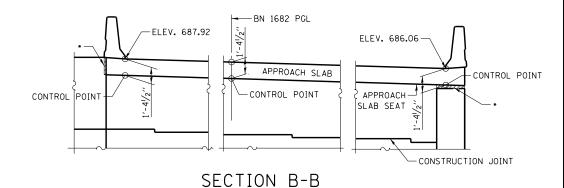
**GARZA KARHOFF** ENGINEERING, LLC



DESCRIPTION DATE

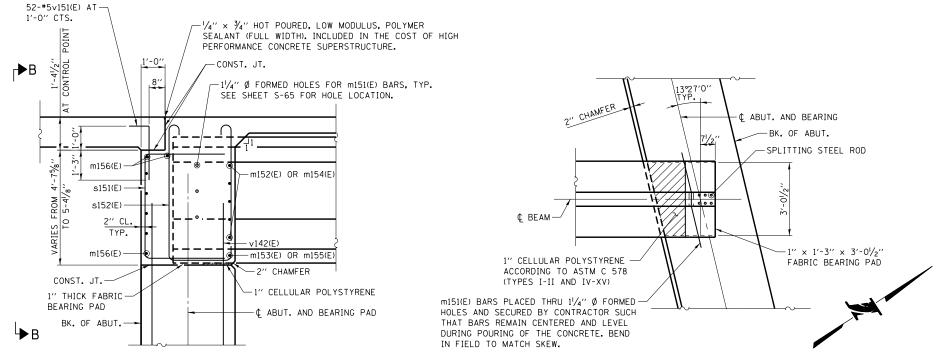
CONTRACT NO. I-18-4694 S-77 BRIDGE NO. 1682 DRAWING NO. 185 OF 220 SOUTH ABUTMENT DIAPHRAGM DETAILS





\* 2" MIN. PJF (PER ARTICLE 1051.09 OF THE STANDARD SPECIFICATIONS) BONDED TO WINGWALL WITH SUITABLE ADHESIVE AS RECOMMENDED BY SUPPLIER.

# DIAPHRAGM ELEVATION AT NORTH ABUTMENT



# PLAN AT ABUTMENT

(SHOWING BOTTOM FLANGE OF BEAM)

# NOTES:

- 1. FOR BAR LIST IN DIAPHRAGM SEE SHEET S-82.
- 2. FOR BILL OF MATERIAL SEE SHEET S-82.
- 3. FOR DETAILS OF BARS s151(E), s152(E) AND v151(E) SEE SHEET S-82.
- 4. THE s151(E), AND s152(E) BARS SHALL BE PLACED PARALLEL TO THE BEAMS. SPACING FOR THESE BARS SHALL BE RIGHT ANGLES TO THE
- 5. THE APPROACH SLAB SEAT SHALL HAVE A CONSTANT SLOPE DETERMINED FROM THE CONTROL POINTS SHOWN.
- 6. COST OF CELLULAR POLYSTYRENE IS INCLUDED WITH HIGH PERFORMANCE CONCRETE SUPERSTRUCTURE.

JC DATE 06/12/18 DRAWN BY DATE 06/12/18 CHECKED BY BGK

SECTION A-A

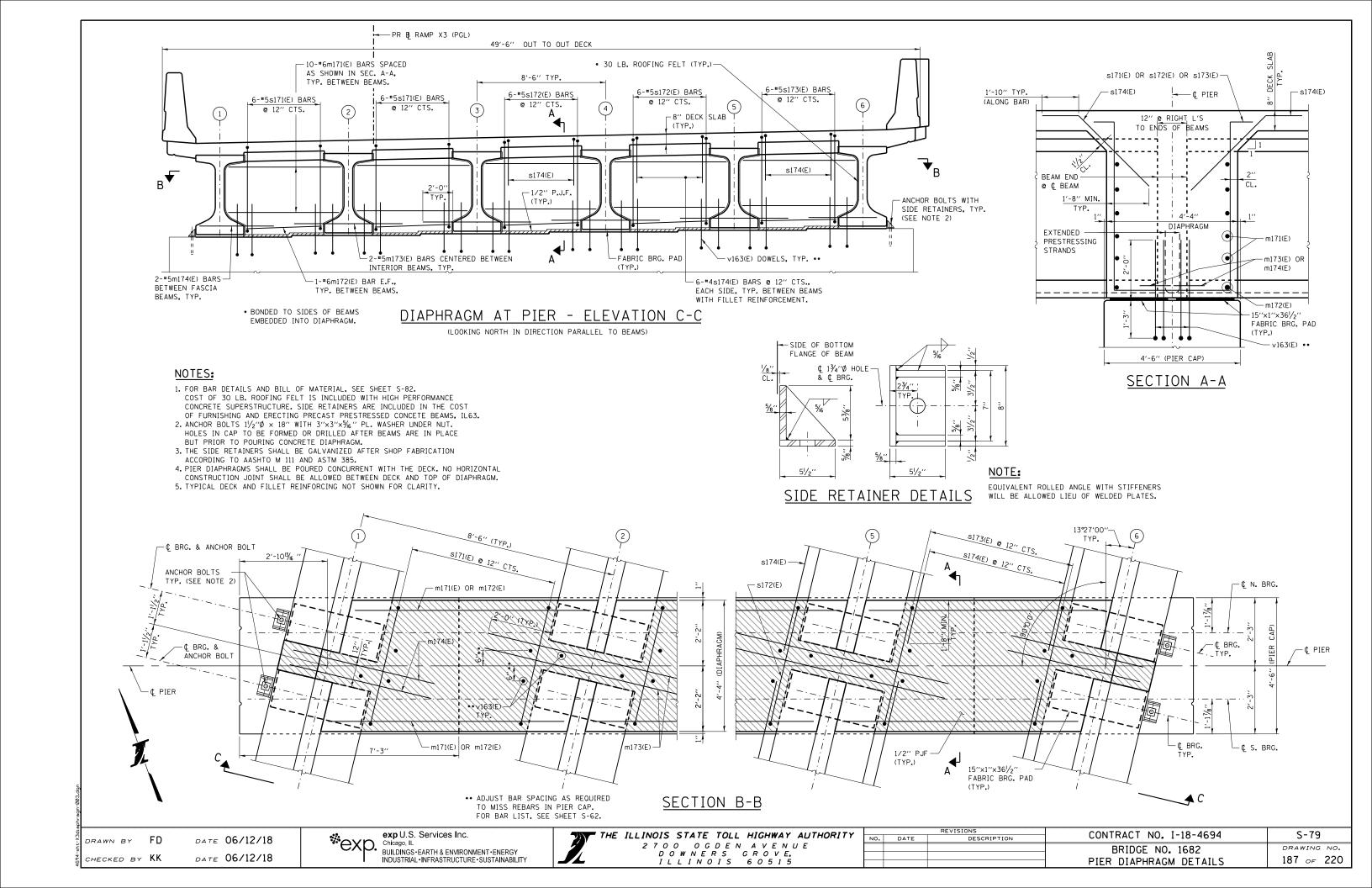
REINFORCEMENT IN DECK NOT SHOWN FOR CLARITY.

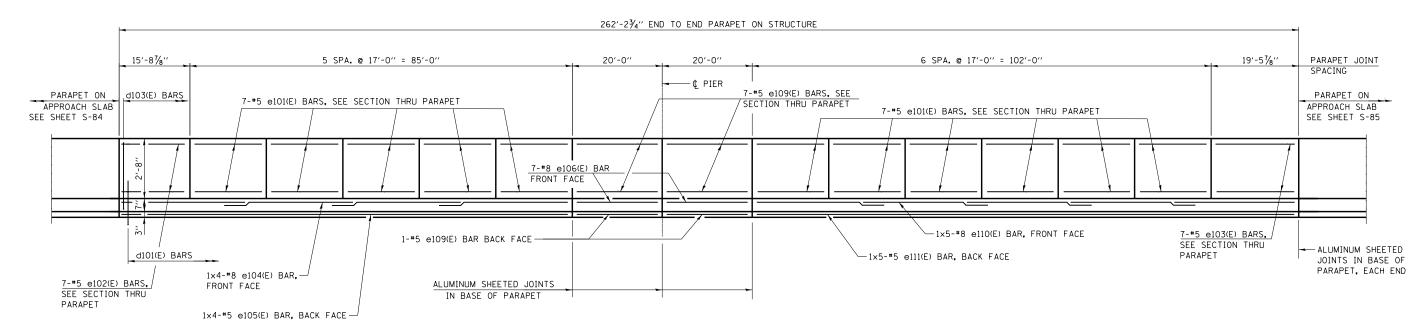
(AT RT. L'S)

GARZA KARHOFF ENGINEERING, LLC



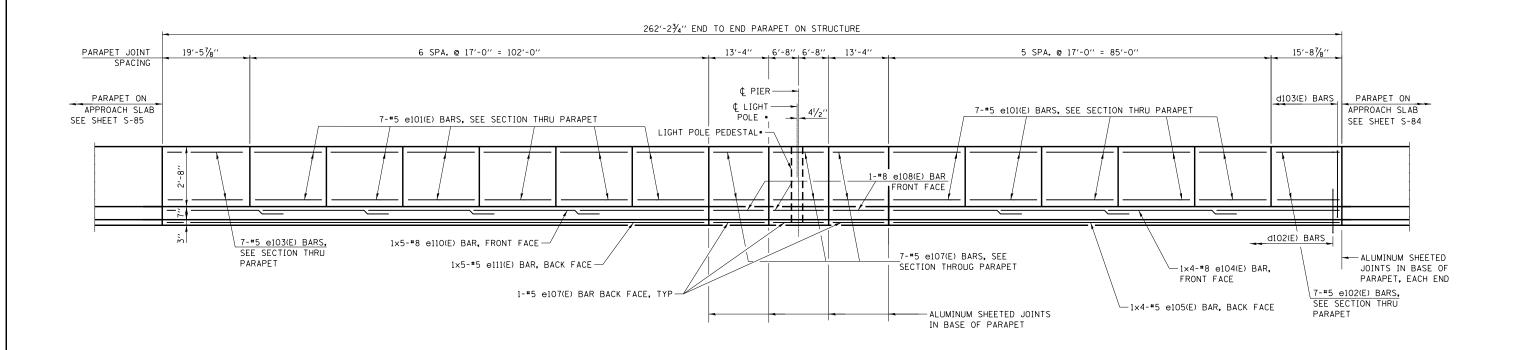
REVISIONS			CONTRACT NO. I-18-4694	S-78
NO.	DATE	DESCRIPTION	CUNTRACT NO. 1-10-4634	3-10
			BRIDGE NO. 1682	DRAWING NO.
				186 of 220
			NORTH ABUTMENT DIAPHRAGM DETAILS	186 of 220





# INSIDE ELEVATION OF PARAPET

(WEST PARAPET, LOOKING WEST)



# INSIDE ELEVATION OF PARAPET

(EAST PARAPET, LOOKING EAST)

MIN LAP #8 BARS = 5'-11" #5 BARS = 3'-0"

• FOR ADDITIONAL LIGHT POLE PEDESTAL REINFORCEMENT SEE SHEET S-81.

DRAWN BY EG DATE 06/12/18

CHECKED BY KK DATE 06/12/18

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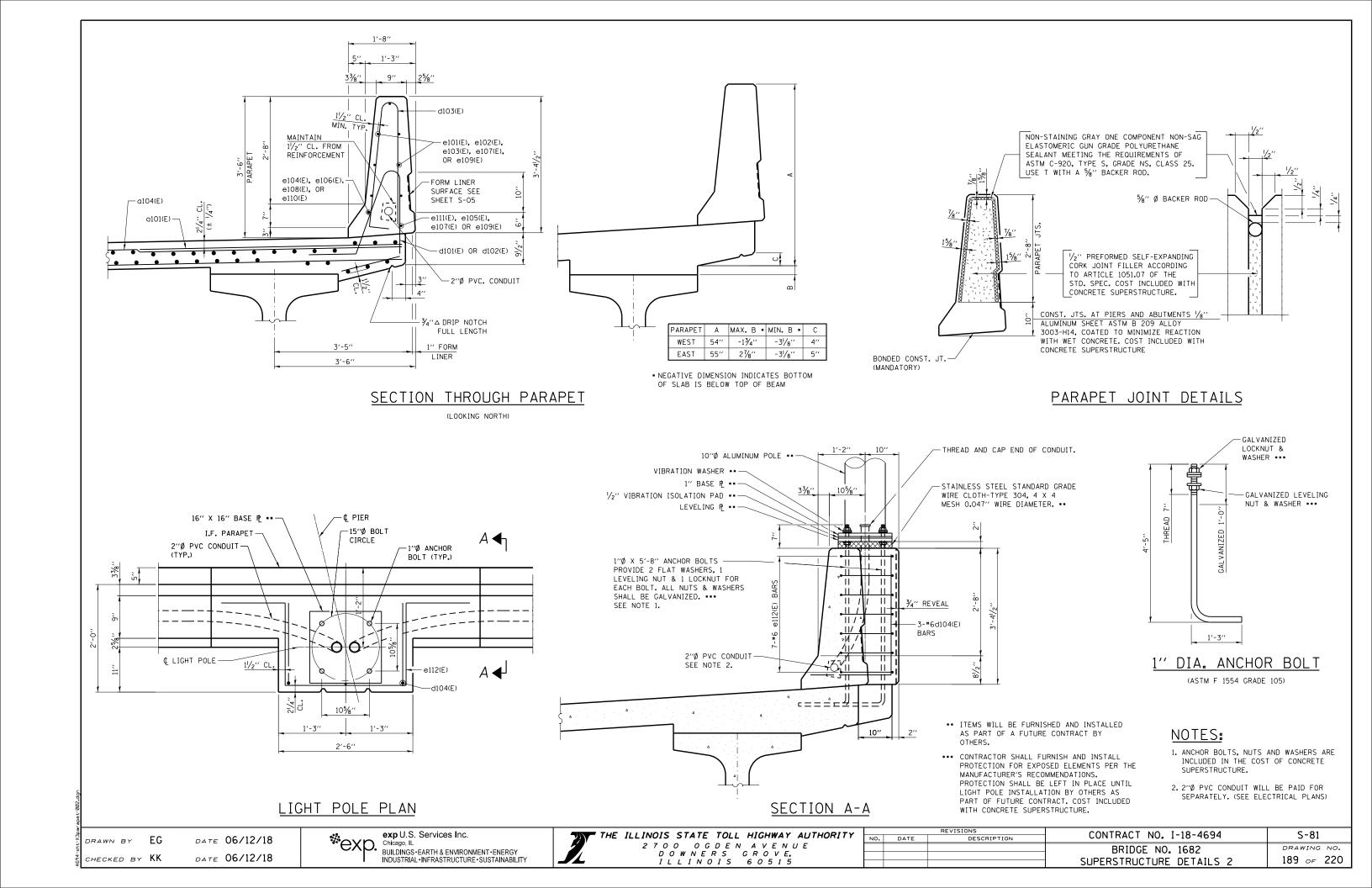
THE ILLINOIS STATE TOLL HIGHWAY AUTHORITY

2 7 0 0 0 G D E N A V E N U E

D 0 W N E R S G R O V E,

I L L I N O I S 6 0 5 1 5

REVISIONS		REVISIONS	CONTRACT NO. I-18-4694	S-80	
o. c	DATE	DESCRIPTION	CUNTRACT NO. 1-10-4634	3-60	
			BRIDGE NO. 1682	DRAWING NO.	
				188 <i>o</i> 220	
			SUPERSTRUCTURE DETAILS 1	100 OF 220	



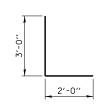
# BILL OF MATERIAL

#### DECK

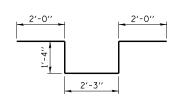
BAR	No.	SIZE	LENGTH	SHAPE
a101(E)	1144	#6	6′-6′′	
a102(E)	371	#5	34'-0''	
a103(E)	371	#5	18'-2"	
a104(E)	1144	#5	26'-0"	
a105(E)	32	#5	4'-0''	
a106(E)	6	#5	26'-6''	
	_			
b101(E)	98	#9	36'-0''	
b102(E)	90	#8	60'-0''	
b103(E)	196	#7	36'-0''	
b104(E)	180	#5	36'-0''	
b105(E)	760	#5	30'-0''	
b106(E)	90	#5	35'-0''	
b107(E)	90	#5	33'-3''	
b108(E)	98	#5	21'-6''	
b109(E)	98	#5	19'-6''	
b110(E)	30	#4	30'-0''	
b111(E)	6	#4	17'-9''	
b112(E)	2	#4	10'-6''	
b113(E)	2	#4	20'-0"	
b114(E)	2	#4	9'-0''	
b115(E)	2	#4	16′-6′′	
b116(E)	2	#4	28'-6''	
b117(E)	2	#4	8'-0''	
b118(E)	2	#4	12'-0''	
d101(E)	450	#5	7'-1''	<u> </u>
d102(E)	450	#5	7'-1''	<u> </u>
d103(E)	900	#5	6′-10′′	I)
s101(E)	37	#4	4'-9''	
s102(E)	172	#4	4'-10''	
s103(E)	159	#4	4'-11''	ر ا
s104(E)	63	#4	5′-0′′	ر ا
s105(E)	68	#4	5′-1′′	_ ر
s106(E)	15	#4	5′-2′′	ر ا
s107(E)	15	#4	5′-3′′	<u> </u>
s108(E)	7	#4	5′-5′′	
	LCODIDII:	LINITT	OLIANITATIV	
	DESCRIPTION	UNIT	QUANTITY	
HIGH PER SUPERST	FORMANCE RUCTURE	CU.YD.	360.7	
REINFORC COATED	EMENT BAR	POUND	160290	
PROTECT:	IVE COAT	SQ.YD.	1,345	
BRIDGE D	ECK GROOV	ING	SQ.YD.	1,287

#### PARAPETS

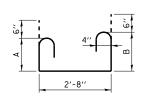
BAR	No.	SIZE	LENGTH	SHAPE
e101(E)	154	#5	16'-8''	
e102(E)	14	#5	15'-4''	
e103(E)	14	<b>#</b> 5	19'-2''	
e104(E)	8	#8	29'-7''	
e105(E)	8	<b>#</b> 5	27′-5′′	
e106(E)	2	#8	19'-8''	
e107(E)	24	<b>#</b> 5	13'-0''	
e108(E)	3	#8	13'-0''	
e109(E)	16	<b>#</b> 5	19'-8''	
e110(E)	10	#8	29'-0''	
e111(E)	10	<b>#</b> 5	26′-8′′	
e112(E)	7	#6	8'-11''	
d104(E)	3	#6	5′-0′′	L
DI	ESCRIPTIO	N	UNIT	QUANTITY
CONCRETE SUPERSTRUCTURE			CU YD	71.6
REINFORCEMENT BARS, EPOXY COATED			POUND	6070
PROTEC	TIVE COA	Т	SQ YD	257



BAR d104(E)



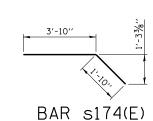
BAR e112(E)



BARS s101(E) THRU s108(E)

BAR	Α	В
s101(E)	6''	7''
s102(E)	7''	7''
s103(E)	7''	8′′
s104(E)	8′′	8′′
s105(E)	8′′	9′′
s106(E)	9''	9′′
s107(E)	9''	10′′
s108(E)	10′′	11''

SHAPE	
4	
L	
QUANTITY	
71.6	
6070	



PIER DIAPHRAGM

#6

#5

#5

#5

SIZE LENGTH SHAPE 7′-9′′

5′-1′′ 7′-3′′

5′-0′′

17'-0''

17'-3'' 17'-5''

5′-8′′

UNIT

CU YD

POUND

T T

QUANTITY

36

1410

No.

50

12

12

36

DESCRIPTION

HIGH PERFORMANCE CONCRETE SUPERSTRUCTURE

REINFORCEMENT BARS, EPOXY COATED

BAR

m171(E)

m172(E)

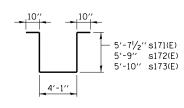
m173(E)

m174(E)

s171(E)

s172(E)

s173(E) s174(E)



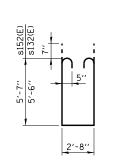
BARS s171(E), s172(E) & s173(E)

#### SOUTH ABUTMENT DIAPHRAGM

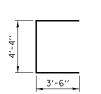
BAR	No.	SIZE	LENGTH	SHAPE
m131(E)	18	#5	4'-0''	
m132(E)	25	#6	7′-5′′	
m133(E)	5	#6	5′-3′′	
m134(E)	10	#6	2'-8''	
m135(E)	2	#6	1'-6''	
m136(E)	14	#6	27'-3''	
s131(E)	29	#5	11'-4''	
s132(E)	29	#5	14'-10''	二
∨131(E)	52	#5	3′-1′′	
D	ESCRIPTIO	N	UNIT	QUANTITY
HIGH PERFORMANCE CONCRETE SUPERSTRUCTURE			CU YD	35.4
REINFOR EPOXY C	CEMENT BA	ARS,	POUND	1970

#### NORTH ABUTMENT DIAPHRAGM

BAR	No.	SIZE	LENGTH	SHAPE
m151(E)	18	#5	4'-0''	
m152(E)	25	#6	7′-5′′	
m153(E)	5	#6	5′-3′′	
m154(E)	10	<b>#</b> 6	2'-8''	
m155(E)	2	#6	1'-6''	
m156(E)	14	#6	27'-3''	
s151(E)	29	#5	11'-4''	
s152(E)	29	#5	15′-0′′	二
∨151(E)	52	<b>#</b> 5	3'-1''	
D	ESCRIPTIO	N	UNIT	QUANTITY
	RFORMANCE E SUPERS	CU YD	37 <b>.</b> 3	
REINFOR EPOXY C	CEMENT BA	POUND	1970	



BARS v151(E) & v131(E)



BARS s152(E) & s132(E) BARS s151(E) & s131(E)

BARS d101(E & d102(E)

BAR d103(E)

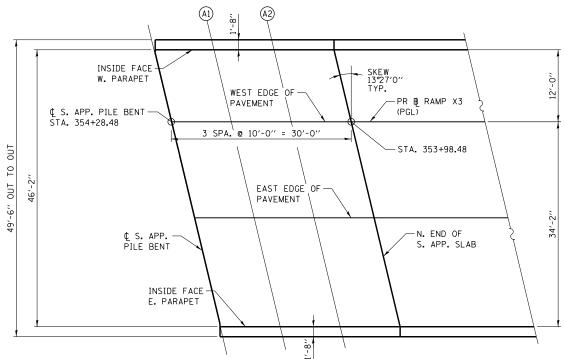
57/8"

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REVISIONS			CONTRACT NO. I-18-4694	S-82
NO.	DATE	DESCRIPTION	CUNTRACT NO. 1-10-4634	3-62
			BRIDGE NO. 1682	DRAWING NO.
			BITIDOE TOOL	
			SUPERSTRUCTURE DETAILS 3	l 190 <i>o</i> ⊧ 220
			SUPERSTRUCTURE DETAILS S	130 07 220





SKEW
1327'0
TYP. WEST EDGE OF
PAVEMENT
STA. 351+36.26

NORTH APPROACH SLAB

(SHOWING GEOMETRY AND SCREED LINES)

# SOUTH APPROACH SLAB

(SHOWING GEOMETRY AND SCREED LINES)

# <u>PLAN</u>

# INSIDE FACE WEST PARAPET

LOCATION	STATION	OFFSET	THEORETICAL GRADE ELEVATIONS	
C/L S. APP. PILE BENT	354+31.35	12.00	686.40	
A1	354+21.35	12.00	686.49	
A2	354+11.35	12.00	686.57	
N. END S. APP. SLAB	354+01.35	12.00	686.65	
S. END N. APP. SLAB	351+39.13	12.00	686.07	
A3	351+29.13	12.00	685.91	
Α4	351+19.13	12.00	685.75	
C/L N. APP. PILE BENT	351+09.13	12.00	685.58	

# WEST EDGE OF PAVEMENT & PR B RAMP X3 (PGL)

LOCATION	STATION	OFFSET	THEORETICAL GRADE ELEVATIONS		
C/L S. APP. PILE BENT	354+28.48	0.00	686.67		
A1	354+18.48	0.00	686.75		
A2	354+08.48	0.00	686.84		
N. END S. APP. SLAB	353+98.48	0.00	686.92		
S. END N. APP. SLAB	351+36.26	0.00	686.51		
A3	351+26.26	0.00	686.40		
Α4	351+16.26	0.00	686.28		
C/L N. APP. PILE BENT	351+06.26	0.00	686.16		

# EAST EDGE OF PAVEMENT

LOCATION	STATION	OFFSET	THEORETICAL GRADE ELEVATIONS
C/L S. APP. PILE BENT	354+24.66	-16.00	687.02
A1	354+14.66	-16.00	687.10
A2	354+04.66	-16.00	687.19
N. END S. APP. SLAB	353+94.66	-16.00	687.26
S. END N. APP. SLAB	351+32.44	-16.00	687.14
A3	351+22.44	-16.00	687.09
Α4	351+12.44	-16.00	687.04
C/L N. APP. PILE BENT	351+02.44	-16.00	686.98

# INSIDE FACE EAST PARAPET

LOCATION	STATION	OFFSET	THEORETICAL GRADE ELEVATIONS
C/L S. APP. PILE BENT	354+20.31	-34.17	687.42
A1	354+10.31	-34.17	687.51
A2	354+00.31	-34.17	687.59
N. END S. APP. SLAB	353+90.31	-34.17	687.66
S. END N. APP. SLAB	351+28.09	-34.17	687.92
А3	351+18.09	-34.17	687.94
Α4	351+08.09	-34.17	687.96
C/L N. APP. PILE BENT	350+98.09	-34.17	687.97

DRAWN BY EG

DATE 06/12/18

DATE 06/12/18

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THE ILLINOIS STATE TOLL HIGHWAY AUTHORITY

2 7 0 0 0 G D E N A V E N U E

D O W N E R S G R O V E.

I L L I N O I S 6 0 5 1 5

CONTRACT NO. I-18-4694 S-83

BRIDGE NO. 1682

TOP OF APPROACH SLAB ELEVATIONS 191 of 220

#### BILL OF MATERIAL BILL OF MATERIAL FOR APPROACH SLAB 30'-0" BRIDGE APPROACH SLAB BAR SI7F LENGTH SHAPE BAR No. #6 d150(E) a149(F) 119 6'-6" 14'-95/8" 15'-0" #5 a150(E) 12'-0" d151(E) #5 #5 a152(E) 18'-4" e150(E) 59#6a149(E) BARS @ 6" CTS. TOP LAP WITH a153(E) OR a156(E) BARS •• 54 #5 -CONSTRUCTION JOINT a153(E) 13'-3" e151(E) ¢ PILE BENT-(TYP.) (SLAB) a154(E) 53 #5 15'-8'' e152(E) 1'-8" SINGLE FACE BARRIER (TYP.) a155(E) **#**5 52 19'-5' e153(E) (TYP.) a156(E) #5 15'-2" a157(E) #5 16'-0'' #5 g158(E) 21'-2" CONCRETE SUPERSTRUCTURE #5a156(E) BARS @ 6" CTS. TOP a159(E) 176 #8 29'-1" REINFORCEMENT BARS, EPOXY COATED b150(F) #5 29'-7' 1-#5a150(E) BAR @ EACH END TOP 99 PROTECTIVE COAT 74 #9 24'-5" b152(E) 75 #9 32'-0" 54-#5a153(E) BARS @ 6" CTS. TOP 27-#5b150(E) BARS @ 6" CTS. TOP DESCRIPTION UNIT QUANTITY -SAWED LONGITUDINAL BRIDGE APPROACH SLAB SQ.YD. 165 ¢ PILE BENT & ₽ RAMP X3 — 5-#5a156(E) BARS @ 6" CTS. TOP JOINT. TYP STA. 354+28.48 REINFORCEMENT BARS. LBS. 35320 -PR B RAMP X3 (PGL) SEE NOTE 3 EPOXY COATED PROTECTIVE COAT SQ.YD. 154 BRIDGE DECK GROOVING SQ.YD. 147 ¢ JOINT & ₿ RAMP X3 #5a157(E) BARS @ 6" CTS. TOP STA. 353+98.48 • FOR INFORMATION ONLY CONSTRUCTION JOINT (SLAB) PARAPET ON DECK 1'-3" -1-#5a151(E) BAR @ EACH END TOP 53-#5a154(E) BARS @ 6" CTS. TOP OUT AS SHOWN ON PLAN VIEW 75-#9b152(E) BARS @ 8" -32-#5b150(E) BARS @ 6" CTS. TOP CTS. BOT. SEE NOTE 1 MIN. LAP ALUMINUM SHEETED JOINT 4-#5 e150(E) BARS (BARRIER) EACH FACE 1 7-#5a157(E) BARS @ 6" CTS. TOP 1-#6 e151(E) OR e153(E) SEE NOTE 3 BAR FACH FACE 29'-11'' #5a158(E) BARS @ 6" CTS. TOP CONSTRUCTION -JOINT (SLAB) 25-#5 d150(E) BARS -1-#5a152(E) BAR @ EACH END TOP 52-#5a155(E) BARS @ 6" CTS. TOP @ 7" CTS. (TOP) 25-#5 d151(E) BARS @ 7" CTS. (BOT) BN 1682 DECK 40-#5b150(E) BARS @ 6" CTS. TOP APPROACH SLAB BARRIER ELEVATION (EAST BARRIER SHOWN, WEST BARRIER SIMILAR) 74-#9b151(E) BARS @ 8" CTS. SPACED BETWEEN b152 (E) BOT. 8-#5a158(E) BARS @ 6" CTS. TOP NOTES: SEE NOTE 3 1. TILT HOOK OF #9 BARS FOR MINIMUM $2^{1}/4^{"}$ CLEARANCE. 2" FROM EDGE OF APP. SLAB, TYP. 1/4" OPEN JT., TYP. 2. USE 4'-0" MIN. LAP FOR #5 BARS. USE 7'-10" MIN. LAP FOR #8 BARS. TATOL MUNIMULIA -3. CUT REINFORCEMENT IN THE FIELD TO FIT THE SKEW AND USE REMAINDER IN 15′-0¾ 15'-0'' (TYP.) (BARRIER) OPPOSITE END. COAT CUT ENDS WITH EPOXY. 60#6a149(E) BARS @ 6" CTS. TOP 4. FOR SECTIONS A-A AND D-D SEE SHEET S-86. LAP WITH a155(E) OR a158(E) BARS \*\* 5. PROTECTIVE COAT SHALL BE APPLIED TO TOP AND TRAFFIC FACES OF BARRIERS. 88X2#8a159(E) BARS @ 4" CTS. BOT. 6. TOOL EDGES OF EXPANSION JOINTS TO 1/4" RADIUS. \*\* ORDER BARS FULL LENGHT. CUT TO FIT THE SKEW. 7. CONCRETE BARRIERS SHALL BE CONSTRUCTED & PAID FOR IN ACCORDANCE WITH

SOUTH APPROACH SLAB

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DRAWN BY EG

CHECKED BY KK

DATE 06/12/18

DATE 06/12/18

CONTRACT NO. I-18-4694 THE ILLINOIS STATE TOLL HIGHWAY AUTHORITY S-84 DESCRIPTION DATE 2 7 0 0 O G D E N A V E N U E BRIDGE NO. 1682 DRAWING NO. DOWNERS GROVE. ILLINOIS 60515 192 of 220 SOUTH APPROACH SLAB

SECTIONS 503 AND 508 OF THE STANDARD SPECIFICATIONS.

8. FOR BARS b152(E), d150(E), AND d151(E) SEE SHEET S-86.

BILL OF MATERIAL FOR BARRIERS

SIZE

#5

#6

#5

#6

-¢ ¼" OPEN JOINT AT MIDPOINT OF BARRIER

AS SHOWN ON PLAN VIEW

BARS EACH FACE

25-#5 d150(E) BARS

@ 7" CTS. (TOP)

25-#5 d151(E) BARS @ 7" CTS. (BOT)

-4-#5 e150(E) OR e152(E)

No.

100

100

24

4

DESCRIPTION

LENGTH

6'-10''

8'-4"

14'-8''

29'-8"

14'-5''

29'-6"

UNIT

CU.YD.

LBS.

SQ.YD.

SHAPE

QUANTITY

8.2

2420

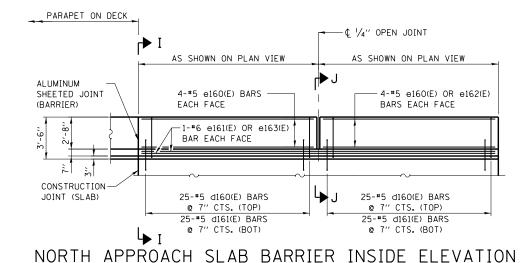
29

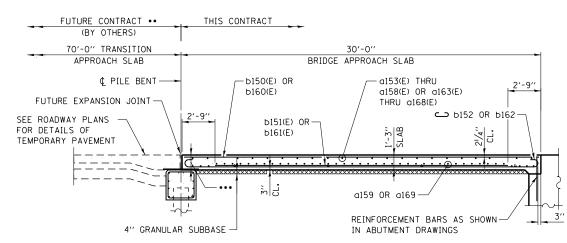
### BILL OF MATERIAL

BILL	OF MATE	RIAL FOR	APPROACH	SLAB
BAR	No.	SIZE	LENGTH	SHAPE
a160(E)	2	#5	12'-0''	I ——
a161(E)	2	#5	16′-1′′	
a162(E)	2	#5	18'-4''	
a163(E)	54	#5	13'-3''	
a164(E)	53	#5	15'-8''	
a165(E)	52	<b>#</b> 5	19'-5''	
a166(E)	5	#5	15'-2''	
a167(E)	7	<b>#</b> 5	16'-0''	
a168(E)	8	#5	21'-2''	
a169(E)	176	#8	29'-1''	
a170(E)	119	#6	6′-6′′	
b160(E)	99	#5	29'-7''	
b161(E)	74	#9	24'-5''	l ——
b162(E)	75	#9	32'-0"	
DE	SCRIPTION	l	UNIT	QUANTITY
BRIDGE AF	PPROACH S	SLAB	SQ.YD.	165
REINFORCE EPOXY CO		LBS.	35320	
PROTECTI'	VE COAT		SQ.YD.	154
BRIDGE DE	ECK GROO	/ING	SQ.YD.	147

BILL OF MATERIAL FOR BARRIERS							
BAR	No.	SIZE	LENGTH	SHAPE			
d160(E)	100	<b>#</b> 5	6′-10′′	1			
d161(E)	100	<b>#</b> 5	8'-4''				
e160(E)	24	<b>#</b> 5	14'-8''				
e161(E)	4	#6	29'-8''				
e162(E)	8	<b>#</b> 5	14'-5''				
e163(E)	4	#6	29'-6''				
DE:	SCRIPTION	l	UNIT	QUANTITY			
CONCRETE	SUPERST	CU.YD.	8.2				
REINFORCE EPOXY CO		LBS.	2420				
PROTECTIV	/E COAT		SQ.YD.	29			

\* FOR INFORMATION ONLY





(WEST BARRIER SHOWN, EAST BARRIER SIMILAR)

# LONGITUDINAL CROSS SECTION

\*\* SEE ROADWAY PLANS FOR TEMPORARY PAVEMENT AND INTERIM GRADING

\*\*\* OKAY TO VARY APPROACH SLAB THICKNESS AT THE PILE BENT TO MATCH TOP OF APPROACH SLAB ELEVATIONS ON SHEET S-83

2. FOR BARS b162(E), d160(E), AND d161(E) SEE SHEET S-86

DOWNERS GROVE, ILLINOIS 60515

THE ILLINOIS STATE TOLL HIGHWAY AUTHORITY CONTRACT NO. I-18-4694 S-85 DESCRIPTION DATE 2 7 0 0 O G D E N A V E N U E DRAWING NO. BRIDGE NO. 1682 193 of 220 NORTH APPROACH SLAB

88X2#8a169(E) BARS @ 4" CTS. BOT.

NORTH APPROACH SLAB

30'-0" BRIDGE APPROACH SLAB

15'-03/8'

-¢ PILE BENT

#5a166(E) BARS @ 6" CTS. TOP

-C PILE BENT & B RAMP X3

PR B RAMP X3 (PGL)

#5a168(E) BARS @ 6" CTS. TOP

2" FROM EDGE OF APP. SLAB, TYP.

STA. 351+6.26

1'-3"

1'-3"

#5a167(E) BARS @

40-#5b160(E) BARS @ 6" CTS. TOP

-1/4" OPEN JT., TYP.

14'-91/2'

74-#9b161(E) BARS @ 8" CTS.
SPACED BETWEEN b162 (E) BOT.

6" CTS. TOP

59-#6a170(E) BARS @ 6" CTS. TOP LAP WITH a163(E) OR a166(E) BARS

1'-8" SINGLE FACE

53-#5a164(E) BARS @ 6" CTS. TOP

29'-11'

15'-0"

60-#6a170(E) BARS @ 6" CTS. TOP

LAP WITH a165(E) OR a168(E) BARS

52-#5a165(E) BARS @ 6" CTS. TOP

BARRIER (TYP.)

54-#5a163(E) BARS @ 6" CTS. TOP

75-#9b162(E) BARS

@ 8" CTS. BOT. SEE NOTE 1

15'-0"

(TYP.)

CONSTRUCTION JOINT -

(TYP.) (SLAB)

5-#5a166(E) BARS @ 6" CTS. TOP SEE NOTE 3 ON SHEET S-84

1-#5a160(E) BAR @

27-#5b160(E) BARS € 6" CTS. TOP

¢ JOINT & B RAMP X3-STA. 351+36.26

7-#5a167(E) BARS @ 6" CTS. TOP

SEE NOTE 3 ON SHEET S-84

1-#5a161(E) BAR @ EACH END TOP

CONSTRUCTION JOINT (SLAB)-

8-#5a168(E) BARS @ 6" CTS. TOP

SEE NOTE 3 ON SHEET S-84

BN 1682 DECK

1-#5 a162 (E) BAR @ EACH END TOP

32-#5b160(E) BARS @ 6" CTS. TOP

16'-0" LANE

EACH END TOP

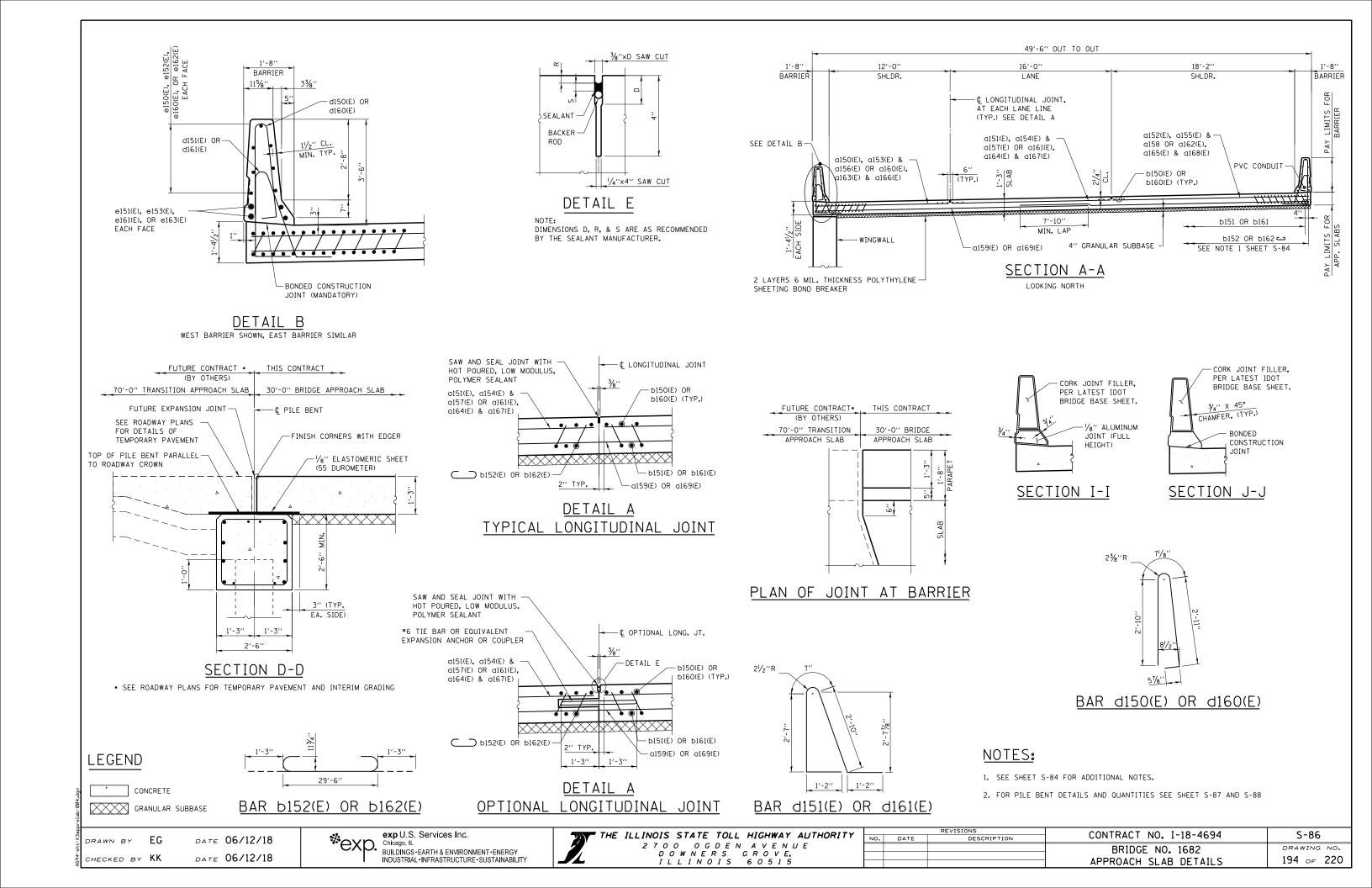
DRAWN BY EG DATE 06/12/18 DATE 06/12/18 CHECKED BY KK

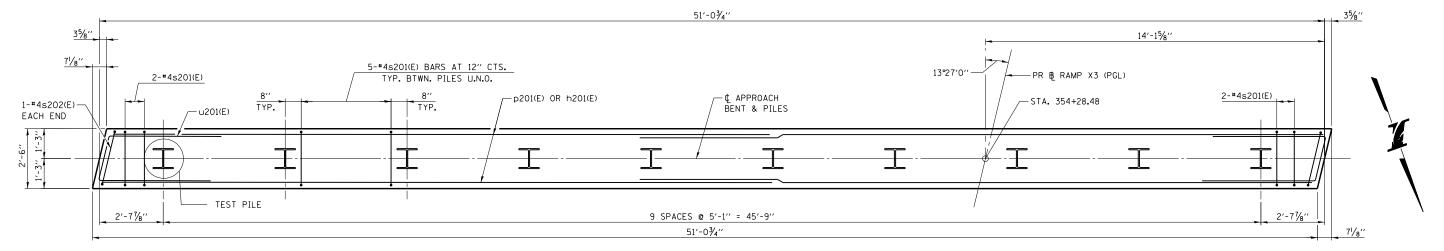
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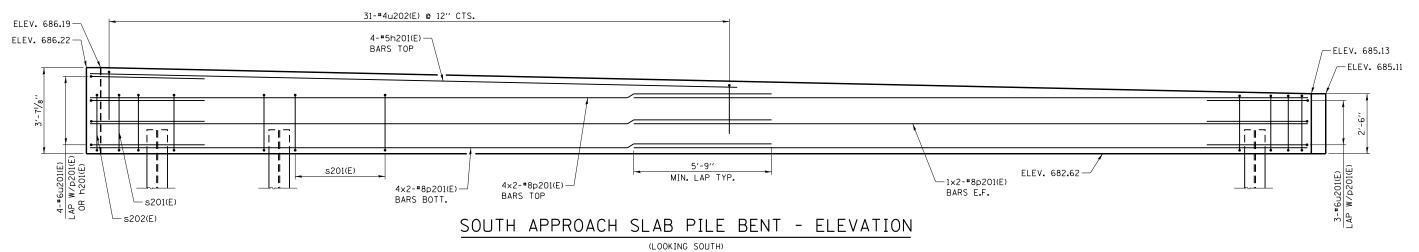
NOTES

1. SEE SHEET S-84 FOR NOTES



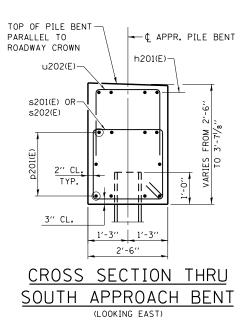


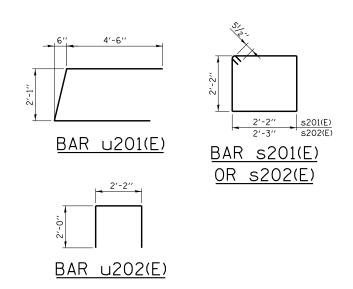
# SOUTH APPROACH SLAB PILE BENT - PLAN



# BILL OF MATERIAL

BAR	No.	SIZE	LENGTH	SHAPE	
h201(E)	4	#5	30′-0′′		
p201(E)	20	#8	28'-4''		
s201(E)	49	#4	9′-7′′		
s202(E)	2	#4	9'-9''		
u201(E)	7	#6	11'-3''		
u202(E)	31	#4	6'-2''		
[	DESCRIPTION	١	UNIT	QUANTITY	
CONCRETE	STRUCTUR	ES	CU YD	14.5	
REINFORC EPOXY CO	EMENT BARS	POUND	2220		
FURNISHIN PILES HP		FOOT	351		
DRIVING I	PILES		F00T	351	
TEST PIL	E STEEL HF	°10×42	EACH	1	
PILE SHO	ES		EACH	10	





# PILE DATA

TYPE: HP10×42 WITH PILE SHOES NOMINAL REQUIRED BEARING: 181 KIPS FACTORED RESISTANCE AVAILABLE: 100 KIPS EST. LENGTH: 39' NO. PRODUCTION PILES: 9 NO. TEST PILES: 1

# NOTES

- 1. FOR PILE DETAILS, SEE SHEET S-10.

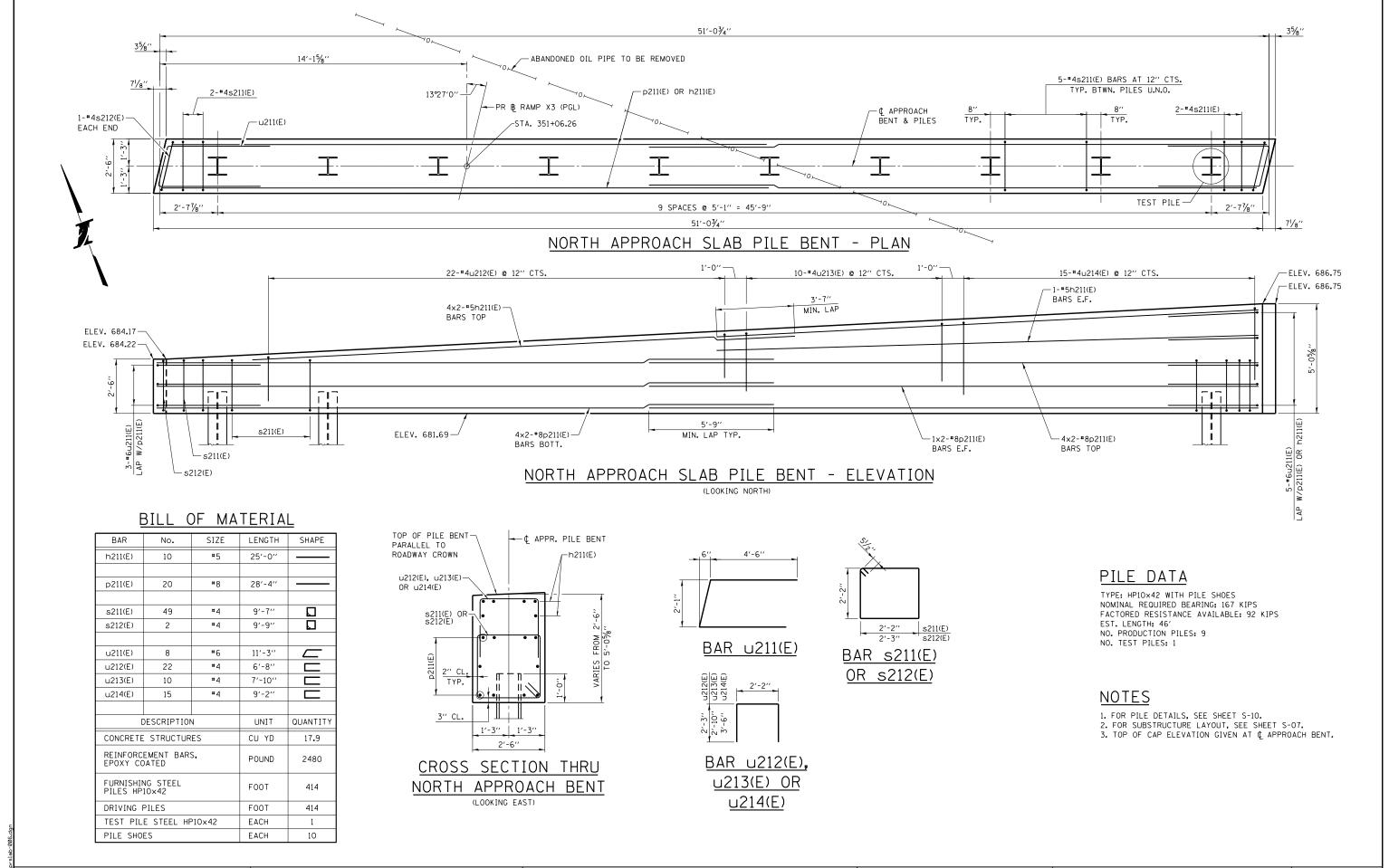
2. FOR SUBSTRUCTURE LAYOUT, SEE SHEET S-07. 3. TOP OF CAP ELEVATION GIVEN AT ¢ APPROACH BENT.

EG DATE 06/12/18 DRAWN BY DATE 06/12/18 CHECKED BY KK

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CONTRACT NO. I-18-4694 S-87 DESCRIPTION DATE DRAWING NO. BRIDGE NO. 1682 195 OF 220 SOUTH APPROACH PILE BENT



DRAWN BY EG DATE 06/12/18
CHECKED BY KK DATE 06/12/18

exp U.S. Services Inc.
Chicago, IL
BUILDINGS-EARTH & ENVIRONMENT-ENERGY
INDUSTRIAL-INFRASTRUCTURE-SUSTAINABILITY

THE ILLINOIS STATE TOLL HIGHWAY AUTHORITY

2 7 0 0 0 G D E N A V E N U E

D O W N E R S G R O V E,

I L L I N O I S 6 0 5 1 5

REVISIONS			CONTRACT NO. I-18-4694	S-88
ο.	DATE	DESCRIPTION	CONTRACT NO. 1-10-4634	3-00
			BRIDGE NO. 1682	DRAWING NO.
			NORTH APPROACH PILE BENT	196 <i>of</i> 220



						Date 3/7/18
	ROUTE DE	SCR	IPTIOI	ISTI N	HA Cor	ntract: I-17-4676, Elgin-C'Hare Western Access (EOWA) LOGGED BY MC
	SECTION		OCA	TION_	SE 1/4	, SEC. 25, TWP. T41N, RNG. R11E, 3 <sup>rd</sup> PM
	COUNTY Cook DRILLIN	G ME	тноп			HSA/Rotary HAMMER TYPE CME Automatic
	STRUCT. NO.	D E P T H	B L O W S	U C S Qu	M O I S T	Surface Water Elev.   n/a   ft   D   B   U   M
1	Ground Surface Elev. 669.30 ft TOPSOIL-black	(ft)	(/6")	(tsf)	(70)	After Hrs. ft (ft) (/6") (tsf) (%)  CLAY LOAM-brown & gray-stiff to
	CLAY LOAM-brown, gray & spotted black-very stiff (Fill)	Ξ	6 6 7	3.0 P	29 18	very stiff (continued) 4 6 3.0 21 10 B
		=	3	2.0	17	CLAY-gray-very stiff 4 7 2.4 22
		-5	6	Р		25 10 B
GPJ 3/22/18		=	6 8 12	2.0 B	17	CLAYEY SAND & 643.80
3L0GS117034_L00		-10	4 5 10	3.7 B	17	SILT-gray-medium dense
MERCHANGE (#-17-4676)/17034 BORING LOGS/17034_LOG.GPJ 3/2/18			5 5 12	3.7 B	17	SILTY LOAM-gray-medium dense
90 INTERCHANGE	450.00	-15	3 4 8	3.2 B	25	5 
7034 EXP. 1480_1-	CLAY LOAM-brown & gray-stiff to very stiff	=	4 5 9	1.8 B	24	
Z:/PROJECTS!2017/17034 EXP		-20	7 9 12	3.6 B	18	

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



GSI Job No. \_\_\_\_17034\_\_ **SOIL BORING LOG** 

	ROUTE	DESCR	IPTIO	ISTH N	IA Cor	ntract: I-17-4676, Elgin-C Access (EOWA)	D'Hare Western	LOGG	ED BY		IC .
	SECTION	ι	OCA1	ION_	SE 1/4	, SEC. 25, TWP. T41N,	RNG, R11E, 3 <sup>rd</sup> P	и			
	COUNTY Cook DRIL	LING ME	THOD			HSA/Rotary	HAMMER TYPE	:	CME A	utoma	tic
	STRUCT. NO.     Station     BORING NO.     4676-BSB-001   Station     420+92   Offset     9,80ft Right	P T H	B L O W S	S On	M O I S T	Surface Water Elev Stream Bed Elev Groundwater Elev.: First Encounter Upon Completion	<u>n/a</u> ft  Dry to 10.0' ft	D E P T H	B L O W S	U C S Qu	M O I S T
	Ground Surface Elev. 669.30 SILTY LOAM-gray-medium dense	ft (ft)	(/6")	(tsf)	(%)	After Hrs CLAY LOAM-gray-stiff	ft	(ft)	(/6")	(tsf)	(%)
	(continued)	7.30	5	2.8	18	(continued)			7 9	2.6	13
		45	16	В				-65	16	В	
1480_1-90 INTERCHANGE (+17-4878)/17034 BORNGLOSS/17034_LOG.GPJ 322/18		-50	5 8 11	1.2 B	18			-70	10 14 21	4.5 P	12
HANGE (F-17-4678) 17034 BOR	SILTY CLAY-gray-hard	7.30	6						11		
00_1-80 INTERC		-55	10 20	4.0 P	22			-75	14 27	4.5 P	13
4EXP.	CLAY LOAM-gray-stiff to hard	2.30	6 8	2.4	16				20	2.1	12
ZIPRO		-80	17	2.4 B	16			-80	14 16	2.1 B	12

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



GSI Job No. \_\_\_\_17034

**SOIL BORING LOG** 

Page <u>3</u> of <u>3</u> Date \_\_\_3/7/18\_\_

ISTHA Contract: I-17-4676, Elgin-O'Hare Western
DESCRIPTION Access (EOWA) LOGGED BY MC

SECTION LOCATION SE 1/4, SEC. 25, TWP. T41N, RNG. R11E, 3<sup>rd</sup> PM COUNTY Cook DRILLING METHOD HSA/Rotary HAMMER TYPE CME Automatic

Stream Bed Elev. <u>n/a</u> ft BORING NO. 4676-BSB-001 (ft) (/6") (tsf) (%) 11 14 1.7 14 85 18 B SILTY CLAY LOAM with GRAVEL-gray-dense to very dense

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

DRAWN BY AMF DATE 06/12/18 DATE 06/12/18 CHECKED BY BGK

Chicago, IL
BUILDINGS-EARTH & ENVIRONMENT-ENERGY
INDUSTRIAL-INFRASTRUCTURE-SUSTAINABILITY exp U.S. Services Inc.



		REVISIONS	CONTRACT NO. I-18-4694	S-89
ю.	DATE	DESCRIPTION	CONTRACT NO. 1-10-4634	3-09
			BRIDGE NOS. 1681 & 1682	DRAWING NO.
				197 of 220
			BORING LOG	191 OF 220



								Date	3/8	3/18
ROUTE	DE	SCR	IPTIOI	ISTI N	IA Cor	ntract: I-17-4676, Elgin-C'Hare Western Access (EOWA)	.ogg	ED BY	rN	IC
SECTION		_ ı	OCA1	ION_	SE 1/4	, SEC. 25, TWP. T41N, RNG. R11E, 3 <sup>rd</sup> PM	1			
COUNTY Cook DRII	LLING	S МЕ	THOD			HSA/Rotary HAMMER TYPE	(	CME A	utoma	tic
STRUCT. NO.   Station	-	DEPTH (ft)	B L O W s	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev.	D E P T H	B L O W s	U C S Qu (tsf)	M O I S T
TOPSOIL-black						CLAY LOAM-brown &				
	72.70		_		21	gray-medium stiff to hard (Fill) (continued)	_			
SANDY CLAY LOAM with Gravel-dark brown & gray-loose to		_	3		19	,	_	7	2.8	24
medium dense (Fill)		_	5		19		_	9	2.0 B	24
			5			850.70 SANDY CLAY LOAM with Gravel-dark brown & gray-medium	, –	7		
		-5	6 6	3.3 P	14	dense (Fill)	-25	10 13	1.8 P	14
CLAY LOAM-brown & gray-medium stiff to hard (Fill)	68.20		6 8 8	2.1 B	16		=	6 7 7	2.0 P	12
		_	5	4.1	12	CLAY LOAM-brown & gray-very stiff		5	2.1	19
		-10	10	В			-30	14	В	
			4 6 8	2.2 B	16	641.70 SILT-gray-loose to medium dense	, <u> </u>			
		_	6				_	6		
		-15	10 13	0.7 B	18		-35	7 5		19
		_	5	0.9	20					
		_	11	В			Ξ			
		_	6				Ξ	6		
		_	8	2.0	22			6		21

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



GSI Job No. \_\_\_\_17034\_\_ **SOIL BORING LOG** 

ROUTE		DESC	CRIPTIO	ISTI N	HA Cor	ntract: I-17-4676, Elgin-C Access (EOWA)	O'Hare Western	LOGG	ED BY	rN	IC
SECTION			LOCA	TION_	SE 1/4	, SEC. 25, TWP. T41N, I	RNG. R11E, 3 <sup>rd</sup> P	м			
COUNTY	Cook [	DRILLING	метног			HSA/Rotary	HAMMER TYPE	:	CME A	utoma	tic
Station BORING NO Station Offset	4676-BSB-002 420+73 42.70ft Left ace Elev. 673,7		D B L P O T W S	U C S Qu (tsf)	M O I S T	Surface Water Elev. Stream Bed Elev. Groundwater Elev.: First Encounter Upon Completion After Hrs.	ft Dry to 10.0'ft ft	D E P T H	B L O W S	U C S Qu (tsf)	M O I S T
	e to medium dense		6 5 4		19	CLAY LOAM-gray-med hard (continued)			7 11 18	1.9 B	23
91/22/5 (AS) 50/1 PROL 1/550 15/NB/06 PROL 1/5/29/1/19 30/NB/108/108/108/108/108/108/108/108/108/108	ray-stiff	626.70	4 8 8 16	1.6 B	19				7	1.8 B	13
	iray-medium stiff to	616.70	7 8 55 10 - - 7 - 11 - - - 22	1.5 B	16				15 22 30 14 18 23	3.3 B	13

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



GSI Job No. \_\_\_\_17034

**SOIL BORING LOG** 

Page <u>3</u> of <u>3</u> Date \_\_\_3/8/18\_\_

ISTHA Contract: I-17-4676, Elgin-O'Hare Western

DESCRIPTION Access (EOWA) LOGGED BY MC

SECTION LOCATION SE 1/4, SEC. 25, TWP. T41N, RNG. R11E, 3<sup>rd</sup> PM COUNTY \_\_\_\_\_ Cook \_\_\_\_ DRILLING METHOD \_\_\_ HSA/Rotary HAMMER TYPE CME Automatic

Surface Water Elev. <u>n/a</u> ft Stream Bed Elev. <u>n/a</u> ft BORING NO. 4676-BSB-002 Station 420-73 H S Qu T First Encounter Dry to 10.0' ft Upon Completion N/a ft After Hrs. ft CLAY LOAM-gray-medium stiff to hard (continued)

14 19 4.1 13 -85 22 B

End Of Boring @ -95.0\*. Boring backfilled with cuttings.

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

DRAWN BY AMF DATE 06/12/18 DATE 06/12/18 CHECKED BY BGK

Chicago, IL
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							Date	3/9	9/18
ROUTE [	ESCR	IPTIOI	ISTI N	HA Cor	ntract: I-17-4676, Elgin-C'Hare Western Access (EOWA) L	OGGI	ED BY	N	IC
SECTION	'	OCA	LION _	SE 1/4	, SEC. 25, TWP. T41N, RNG. R11E, 3 <sup>rd</sup> PN	<u> </u>			
COUNTY Cook DRILLI	NG ME	THOD			HSA/Rotary HAMMER TYPE		ME A	utoma	tic
	D E P T H	B L O W S	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev.	D E P T H (ft)	B L O W S (/6")	U C S Qu (tsf)	M O I S T (%)
CLAYEY TOPSOIL with GRAVEL	_			19	CLAY LOAM-brown & gray-stiff to hard (Fill) (continued)	_			
CLAY LOAM-dark brown &	50	4		19	Train (1 my (continuou)	$\dashv$	3		
gray-very stiff (Fill)	_	5	3.3	18		$\dashv$	6	1.8	20
	_	6	P			$\neg$	7	В	20
SANDY CLAY LOAM with GRAVEL-dark brown &	50					_			
gray-medium dense (Fill)		7		10		_	4	2.8	20
g.c,	_	9		10		_	8	2.0 P	20
	5	9		$\vdash$		-25	0	Р	-
CLAY LOAM-brown & gray-stiff to	00	1				$\dashv$			
hard (Fill)		4				$\neg$	6		
	_	8	4.5	15			8	2.0	23
		12	Р				11	Р	
					649.50	_			
					CLAY LOAM-brown-very stiff	$\neg$			
		4					9		
	_	7	4.5	12		_	9	3.3	19
	10	14	Р			30	11	В	
	_					_			
		5				-			
	_	6	2.5	16	645.50				
		9	В		CLAY-gray-stiff	$\neg$			
	_				1				
		1							
		4					4		
		8	4.3	17			7	1.9	25
	-15	13	В			-35	12	В	
	_					_			
	_	4				-			
	_	5	1.5	17		_			
	_	8	P.5	l ''	SILT-gray-medium dense	$\dashv$			
	-	Ť	ı.	$\vdash$		$\dashv$			
	_	1				$\dashv$			
		7					10		
		9		NR			9		19

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



**SOIL BORING LOG** 

	ROUTE	DESCR	IPTIOI	ISTH	HA Cor	ntract: I-17-4676, Elgin-O'Hare Western Access (EOWA)	LOGG	ED BY		IC
	SECTION	ι	OCA	ION _	SE 1/4	, SEC. 25, TWP. T41N, RNG. R11E, 3 <sup>rd</sup>	PM			
	COUNTY Cook DRIL	LING ME	THOD			HSA/Rotary HAMMER TY	PE	CME A	utoma	tic
	STRUCT. NO.   Station	D E P T H	B L O W S	U C S Qu (tsf)	M O I S T	Surface Water Elev.   n/a   ft	D E P T H	B L O W S	U C S Qu (tsf)	M O I S T
	Ground Surface Elev. 677.50  SILT-gray-medium dense (continued)	ft (#)	(,, ,	((31)	(70)	After Hrs. ft  CLAY LOAM-gray-very stiff (continued)	(10)	(,0,)	(tsi)	(70)
		5.50	3	0.7	19	SILTY LOAM-gray-dense	.50	20		28
		-45	7	0.7 В	19		-65	24		20
4676)117034 BORNG LOGS/117034_LOG.GPJ 3/22/18			3 6 8	1.2 B	19	CLAY LOAM-gray-medium stiff to hard	.50	6 8 13	0.9 B	17
	SILTY LOAM-gray-medium dense	5,50	4					13		
	62 CLAY LOAM-gray-very stiff	0.50	9 17		26		-75	19 23	3.0 B	15
Z1PROJECTS(2017)1		-80	5 6 9	2.0 B	16		-80	11 19 23	3.0 B	13

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



SILTY CLAY LOAM with FRACTURED ROCK-gray-very dense

SECTION

GSI Job No. \_\_\_\_17034\_\_

GSI Job No. \_\_\_\_17034\_\_

**SOIL BORING LOG** 

Page <u>3</u> of <u>3</u>

ISTHA Contract: I-17-4676, Elgin-O'Hare Western

DESCRIPTION Access (EOWA)

Date \_\_\_3/9/18\_\_ LOGGED BY MC

LOCATION SE 1/4, SEC. 25, TWP. T41N, RNG. R11E, 3<sup>rd</sup> PM

COUNTY Cook DRILLING METHOD HSA/Rotary HAMMER TYPE CME Automatic Stream Bed Elev. <u>n/a</u> ft BORING NO. 4676-BSB-003 (ft) (/6") (tsf) (%)

15 | 17 | 5.2 | 12 | 85 | 20 | B

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

DRAWN BY AMF DATE 06/12/18 DATE 06/12/18 CHECKED BY BGK

Chicago, IL
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Najperville, IIII)uls (0565 (050) 365-28/8				SO	IL BORING LOG		Page	1 (	or <u>3</u>
							Date	3/5	/18
ROUTE DE	SCR	IPTIO	ISTI N	HA Cor	htract: I-17-4676, Elgin-C'Hare Western Access (EOWA) LO	GGE	D BY	M	IC
SECTION	_ '	LOCA	TION _	SE 1/4	, SEC. 25, TWP. T41N, RNG. R11E, 3 <sup>rd</sup> PM				
COUNTY Cook DRILLIN	G ME	ЕТНОЕ			HSA/Rotary HAMMER TYPE _	С	ME A	utomat	tic
STRUCT. NO	D E P T H	o w	U C S Qu	M 0 8 T	Surface Water Elev.	D E P T H	B L O W S	U C S	M O I S T
Ground Surface Elev. 664.30 ft	(ft)	(/6")	(tsf)	(%)	Upon Completion ft After Hrs ft	(ft)	(/6")	(tsf)	(%)
TOPSOIL-black 663.30 CLAY LOAM-brown, gray &	_	6		28	CLAY LOAM-brown & gray-stiff to very stiff (continued)	$\exists$	6		
spotted black-stiff to hard (Fill)	_	8		13		_	7 10	1.9 B	19
	-5	5 7 11		11	becoming gray @ -23.0'	-25	5 6 9	3.8 P	20
	=	5 12 10	4.5 P	17	SANDY LOAM-gray-medium dense	_	6 6 8		19
	Ξ	4 6 10	3.8 B	18	636.30 SILT-gray-medium dense		7 9 7		23
	10	5	1.6	19		-30	,		
	_	3 5	3.8	23			4		18
CLAY LOAM-brown & gray-stiff to very stiff	15	4	В			-35	8		
	Ξ	6 9	3.8 P	21	CLAY to CLAY LOAM-gray-stiff to very stiff	$\exists$			
	_	6				$\dashv$	5		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



GSI Job No. \_\_\_\_17034\_\_ **SOIL BORING LOG** 

ISTHA Contract: I-17-4676, Elgin-C'Hare Western Access (EOWA) LOGGED BY MC DESCRIPTION ROUTE SECTION LOCATION SE 1/4, SEC. 25, TWP. T41N, RNG. R11E, 3<sup>rd</sup> PM \_\_ DRILLING METHOD \_ HAMMER TYPE \_\_\_\_ CME Automatic COUNTY HSA/Rotary Stream Bed Elev. BORING NO. 4676-BSB-004 421+17 16.10ft Left After \_\_\_\_ Hrs.

CLAY to CLAY LOAM-gray-stiff to very stiff (continued) Ground Surface Elev. 664.30 ft CLAY to CLAY LOAM-gray-stiff to very stiff (continued) 14 3.0 13 -65 22 P 8 1.5 -45 11 P

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



GSI Job No. \_\_\_\_17034\_\_

**SOIL BORING LOG** 

ISTHA Contract: I-17-4676, Elgin-O'Hare Western Access (EOWA) DESCRIPTION LOGGED BY MC

SECTION LOCATION SE 1/4, SEC. 25, TWP. T41N, RNG. R11E, 3<sup>rd</sup> PM \_\_\_ DRILLING METHOD \_ COUNTY HSA/Rotary HAMMER TYPE CME Automatic

Stream Bed Elev. n/a ft BORING NO. 4676-BSB-004 S Qu First Encounter Dry to 10.0' ft
Upon Completion n/a ft
After Hrs. ft
End Of Boring @ -100.0'. Boring
backfilled with cuttings. 
 Station
 421+17

 Offset
 16.10ft Left
 (ft) (/6") (tsf) (%) Ground Surface Elev. 664.30 ft CLAY to CLAY LOAM-gray-stiff to very stiff (continued) CLAYEY GRAVEL with SAND-gray-very dense CLAY LOAM with GRAVEL-gray-very dense

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

DRAWN BY AMF CHECKED BY BGK

DATE 06/12/18 DATE 06/12/18

exp U.S. SCIVISES
Chicago, IL
BUILDINGS-EARTH & ENVIRONMENT-ENERGY
INDUSTRIAL-INFRASTRUCTURE-SUSTAINABILITY exp U.S. Services Inc.





GSI Job No. \_\_\_\_17034\_\_

**SOIL BORING LOG** ISTHA Contract: I-17-4676, Elgin-O'Hare Western Access (EOWA) DESCRIPTION LOCATION SE 1/4, SEC. 25, TWP. T41N, RNG. R11E, 3<sup>rd</sup> PM COUNTY DRILLING METHOD Surface Water Elev. \_\_\_\_ Stream Bed Elev. \_\_\_\_ \_\_\_\_\_ n/a\_\_ ft BORING NO. 4676-BSB-005 421+16 60.20ft Left Dry to 10.0' ft n/a ft (ft) (/6") (tsf) (%) Ground Surface Elev. 661.90 TOPSOIL-black CLAY LOAM-brown & gray-very stiff to hard (Fill) TOPSOIL-black CLAY LOAM-brown & gray-stiff to very stiff CLAY LOAM-gray-soft to very stiff SILTY LOAM-gray-loose to

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Pe The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, from 137 (Rev. 8-99)



GSI Job No. \_\_\_17034\_\_ **SOIL BORING LOG** 

ISTHA Contract: I-17-4676, Elgin-O'Hare Western Access (EOWA) LOGGED BY MC SECTION LOCATION SE 1/4, SEC. 25, TWP. T41N, RNG. R11E, 3<sup>rd</sup> PM COUNTY \_\_ \_\_ DRILLING METHOD \_\_\_\_\_ n/a\_\_ ft BORING NO. 4676-BSB-005 SILTY CLAY LOAM-gray-dense (continued) CLAY LOAM-gray-soft to very stiff (continued) CLAY LOAM with GRAVEL-gray-hard 8 1.3 -45 11 B 13 4.5 -65 18 P 10 2.3 15 -55 14 B 13 4.0 14 -75 15 P SILTY CLAY LOAM-gray-dens

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Pen The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, from 137 (Rev. 8-99)



Cook

CLAY LOAM with GRAVEL-gray-hard (continued)

COUNTY

GSI Job No. \_\_\_17034\_\_

**SOIL BORING LOG** 

ISTHA Contract: I-17-4676, Elgin-O'Hare Western Access (EOWA) DESCRIPTION

LOCATION SE 1/4, SEC. 25, TWP. T41N, RNG. R11E, 3rd PM

Stream Bed Elev. n/a ft BORING NO. 4676-BSB-005 First Encounter
Upon Completion
After \_\_\_\_ Hrs. 
 Station
 421+16

 Offset
 60.20ft Left
 \_\_\_ft (ft) (/6") (tsf) (%) Ground Surface Elev. 661.90

\_ DRILLING METHOD

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Pene The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, from 137 (Rev. 8-99)

DRAWN BY AMF DATE 06/12/18 DATE 06/12/18 CHECKED BY BGK

exp U.S. SCIVISES
Chicago, IL
BUILDINGS-EARTH & ENVIRONMENT-ENERGY
INDUSTRIAL-INFRASTRUCTURE-SUSTAINABILITY exp U.S. Services Inc.



CONTRACT NO. I-18-4694 S-93 DESCRIPTION DRAWING NO. BRIDGE NOS. 1681 & 1682 201 OF 220 BORING LOG



**SOIL BORING LOG** 

									Date	3/1.	2/18
ROUTE	DES	SCR	IPTIO	ISTI N	IA Cor	ntract: I-17-4676, Elgin-C'Hare West Access (EOWA)	ern L(	DGGI	ED BY	M	IC
SECTION		_ ı	OCA1	ION_	SE 1/4	, SEC. 25, TWP. T41N, RNG. R11E,	3 <sup>rd</sup> PM				
COUNTY Cook DRIL	LING	ME	THOD			HSA/Rotary HAMMER	TYPE		ME A	utomat	tic
STRUCT. NO.   Station   BORING NO.   4676-BSB-006   Station   353+72   Offset   22.40ft Left   Ground Surface Elev.   661.00	ft	DEPTH (ft)	B L O W s (/6")	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev.	_ft _ft _ft	D E P T H	B L O W s	U C S Qu (tsf)	M O I S T (%)
12.0" TOPSOIL-black						SILTY LOAM-gray-medium dense					
	0.00				32	(continued)		$\Box$			
CLAY LOAM-brown & gray-stiff (Fill)		_	2						5		
(FIII)			3	1.3 P	20			$\dashv$	5		22
		_	3	Р				_	5		
		_						-			
		_	2					$\dashv$	7		
			3	1.0	20				6		20
		-5	4	Р				-25	5		
65	5.50						635.50				
SILTY CLAY-dark brown to black-loose			_			CLAY LOAM-gray-soft to very stiff			_		
black-loose		_	3					_	2	0.7	
		_	3	2.3 P	26			$\dashv$	3 5	0.7 B	20
		_	5	Р				_	5	В	
CLAY LOAM-gray-soft to stiff	3.00	_						$\dashv$			
CEAT ECAM-gray-soit to sun		_	3					$\dashv$	2		
			7	4.5	16			-	4	0.4	20
		-10	9	Р				-30	7	В	
		-10						-00			
		_									
			4								
			6	1.7	19	l		$\Box$			
		_	8	В	_			_			
		_						$\dashv$			
		_	4			l		-	3		
		_	5	0.3	19			$\dashv$	8	2.3	17
		-15	9	В		l		-35	12	В	
64	5.50	-13						-00			
SILTY CLAY-brown & gray-very		_									
stiff			4			l					
			6	2.0	26		624.00				
		_	10	В	$\vdash$	SILTY CLAY-gray-stiff					
SILTY LOAM-gray-medium dense	3.00	_						$\dashv$			
SILT I LOAM-gray-medium dense		_	7			l		_	7		
		_	7		21	l		$\dashv$	7	1.3	15
		-20	7					-40	10	В	10

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



GSI Job No. \_\_\_\_17034\_\_ **SOIL BORING LOG** 

ISTHA Contract: I-17-4676, Elgin-O'Hare Western Access (EOWA) \_\_\_ LOGGED BY \_\_\_MC DESCRIPTION SECTION LOCATION SE 1/4, SEC. 25, TWP. T41N, RNG. R11E, 3rd PM COUNTY \_\_\_\_\_Cook \_\_\_\_ DRILLING METHOD \_ HSA/Rotary HAMMER TYPE CME Automatic Surface Water Elev.\_\_\_\_ Stream Bed Elev. BORING NO. 4676-BSB-006 353+72 22.40ft Left After Hrs.

CLAY LOAM-gray-stiff to hard (continued) Ground Surface Elev. 661.00 ft SILTY CLAY-gray-stiff (continued) CLAY LOAM-gray-stiff to hard 10 B

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



GSI Job No. \_\_\_\_17034\_\_

**SOIL BORING LOG** 

S Qu

ISTHA Contract: I-17-4676, Elgin-O'Hare Western Access (EOWA) DESCRIPTION\_ LOGGED BY MC

LOCATION SE 1/4, SEC. 25, TWP. T41N, RNG. R11E, 3<sup>rd</sup> PM COUNTY \_\_\_\_\_ Cook \_\_\_\_ DRILLING METHOD \_ HSA/Rotary HAMMER TYPE CME Automatic Stream Bed Elev. n/a ft

BORING NO. 4676-BSB-006 First Encounter Dry to 10.0' ft
Upon Completion n/a ft
After Hrs. ft
End Of Boring @ -100.0'. Boring
backfilled with cuttings. 
 Station
 353+72

 Offset
 22.40ft Left
 (ft) (/6") (tsf) (%) Ground Surface Elev. 661.00 ft CLAY LOAM-gray-stiff to hard SILTY CLAY LOAM with GRAVEL-gray-very dense

50/4" 4.5 -85 P

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

DRAWN BY AMF DATE 06/12/18 DATE 06/12/18 CHECKED BY BGK

exp U.S. SCIVISES
Chicago, IL
BUILDINGS-EARTH & ENVIRONMENT-ENERGY
INDUSTRIAL-INFRASTRUCTURE-SUSTAINABILITY exp U.S. Services Inc.



CONTRACT NO. I-18-4694 S-94 DESCRIPTION NO. DATE DRAWING NO. BRIDGE NOS. 1681 & 1682 202 OF 220 BORING LOG



GSI Job No. <u>17034</u> COLL DODING LOC

(030) 355-98/8					SU	IL BURING LUG	,		· ugo		- <u>-</u>
									Date	1/2	9/18
ROUTE	DE	SCR	IPTIOI	ISTI N	HA Cor	ntract: I-17-4676, Elgin-C'Hare Western Access (EOWA)	n _ LO	GGI	ED BY	N	IC
SECTION		_ ι	OCA1	ION_	SE 1/4	, SEC. 25, TWP. T41N, RNG. R11E, 3	rd PM				
COUNTY Cook DRIL	LING	э ме	THOD			HSA/Rotary HAMMER TY	YPE _	C	ME A	utoma	tic
STRUCT. NO		D E P T	B L O W	U C <b>S</b>	M 0 1 5	Surface Water Elev. n/a f Stream Bed Elev. n/a f Groundwater Elev.:	ft ft	D E P T	B L O W	U C S	M O I S
Station 422+55		Ĥ	s	Qu	Ť	First Encounter Dry to 10.0' f	ft	Н	s	Qu	Ť
Offset 7.90ft Right Ground Surface Elev. 665.00	ft	(ft)	(/6")	(tsf)	(%)	Upon Completion n/a f After Hrs f	ft	(ft)	(/6")	(tsf)	(%)
10.0" ASPHALT	4.17	_				CLAY LOAM-brown & gray-medium stiff to hard		_			
CRUSHED STONE-dense		_	7			(continued) becoming gray @ -20.5'		Ⅎ	5		
		_	17 15		12	woodining gray & 20.0		$\dashv$	7 15	0.9 B	20
CLAY LOAM-dark brown &	2.00	_				64 SANDY LOAM-gray-medium	42.00	_			
gray-very stiff to hard (Fill)		_	3			dense		_	8		
		-5	8 9	5.8 B	14			-25	9		25
		_				SILTY LOAM-gray-medium dense	39.50	_			
		_	3			Sierr Estim-gray-inculain delise		$\exists$	7		
		_	5 6	2.3 B	20			$\dashv$	7 8		23
		_				63 CLAY LOAM-gray-medium stiff	37.00	$\exists$			
		_	6	0.5	16	, , , , , , , , , , , , , , , , , , ,		_	3	0.0	14
		-10	9	6.5 B	16			-30	7	0.6 B	14
CLAY LOAM-brown &	4.50	_						$\overline{}$			
gray-medium stiff to hard		_	5	2.7	22			$\exists$			
		_	10	2.7 B	22	SILTY LOAM-gray-medium dense	33.00	$\exists$			
		_						-			
		_	6		14				9		19
		-15	14	5.1 B	14			-35	9		19
		_						-			
		_	5	4.0	20		-	コ			
		_	10	1.8 B	20	CLAY-gray-very stiff	28.00	$\exists$			
		_						$\dashv$			
		_	6	4.5	24			$\exists$	5	2.0	10
I			11	1.5	24				8	2.0	19

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



GSI Job No. \_\_\_\_17034\_\_ **SOIL BORING LOG** 

ISTHA Contract: I-17-4676, Elgin-C'Hare Western Access (EOWA) DESCRIPTION LOGGED BY MC ROUTE SECTION LOCATION SE 1/4, SEC. 25, TWP. T41N, RNG. R11E, 3<sup>rd</sup> PM \_\_ DRILLING METHOD \_ COUNTY HSA/Rotary HAMMER TYPE CME Automatic Stream Bed Elev. BORING NO. 4676-BSB-007 422+55 7.90ft Right After \_\_\_\_ Hrs.

CLAY LOAM-gray-stiff to hard Ground Surface Elev. 665.00 ft CLAY-gray-very stiff (continued CLAY LOAM-gray-stiff to very stiff 22 4.3 12 -85 32 B 8 2.5 .45 13 B CLAY-gray-very sti CLAY LOAM-gray-stiff to hard

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



CLAY LOAM with Fractured Rock-gray-hard

GSI Job No. \_\_\_\_17034\_\_

**SOIL BORING LOG** 

Page <u>3</u> of <u>3</u>

ISTHA Contract: I-17-4676, Elgin-O'Hare Western Access (EOWA) DESCRIPTION\_

LOGGED BY MC

546.50

LOCATION SE 1/4, SEC. 25, TWP. T41N, RNG. R11E, 3<sup>rd</sup> PM \_ DRILLING METHOD \_ COUNTY HSA/Rotary HAMMER TYPE CME Automatic

Stream Bed Elev. BORING NO. 4676-BSB-007 S Qu 
 Station
 422+55

 Offset
 7.90ft Right
 After Hrs.

CLAY LOAM with Fractured (ft) (/6") (tsf) (%) Ground Surface Elev. 665.00 ft CLAY LOAM-gray-stiff to hard

SILTY CLAY LOAM with Fractured Rock-gray-very dense SILT-gray-very dense

SILT-gray-very dense FRACTURED ROCK-gray-very

End Of Boring @ -118.5'. Boring backfilled with cuttings. The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

DRAWN BY AMF DATE 06/12/18 DATE 06/12/18 CHECKED BY BGK



exp U.S. SCIVISES
Chicago, IL
BUILDINGS-EARTH & ENVIRONMENT-ENERGY
INDUSTRIAL-INFRASTRUCTURE-SUSTAINABILITY exp U.S. Services Inc.



		REVISIONS	CONTRACT NO. I-18-4694	S-95
NO.	DATE	DESCRIPTION	CUNTRACT NO. 1-10-4634	3-35
			BRIDGE NOS. 1681 & 1682	DRAWING NO.
				203 of 220
			BORING LOG	203 of 220



**SOIL BORING LOG** 

			IST	HA Cor	ntract: I-17-4676, Elgin-C'Hare Weste	arm				
ROUTE	DESC	RIPTIO	N		Access (EOWA)	LO	GGI	ED BY	M	IC
SECTION		LOCA	TION_	SE 1/4	, SEC. 25, TWP. T41N, RNG. R11E,	3 <sup>rd</sup> PM				
COUNTY Cook DRILL	LING N	ETHO			HSA/Rotary HAMMER	TYPE _	C	ME A	utomat	tic
STRUCT. NO	E P T H	U W S	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev.   n/a	.ft .ft	D E P T H (ft)	B L O W S	U C S Qu (tsf)	M O I S T (%)
10.0" ASPHALT 663	3.37				SILTY LOAM-gray-medium dense	643.70	$\exists$			
CRUSHED STONE-medium dense		11 13 15		8	one in gray modum conto		$\exists$	4 10 14		18
CLAY LOAM-brown & gray-stiff to hard (Fill)	1.20	5 7 6 8	7.3 S	15				5 6 7		19
		4 7 7	1.6 B	21			-25	5 5 7		19
	-	6 9 0 11	6.5 S	18			-30	5 7 4		19
CLAY LOAM-brown & 653 gray-medium stiff to very stiff	3,70	4 7 10	1.7 B	27	SILTY CLAY-gray-soft	632.20	-30	,		
		6 10 5 15	2.3 B	20			-35	3 5 6	0.3 B	20
		4 7 10	0.9 B	19	SILTY CLAY LOAM-gray-medium	627.20				
	=	5 8 12	3.1 B	20	stiff to very stiff (A-4)		-40	5 8 12	1.2 B	19

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



GSI Job No. \_\_\_\_17034\_\_ **SOIL BORING LOG** 

ISTHA Contract: I-17-4676, Elgin-O'Hare Western Access (EOWA) LOGGED BY MC DESCRIPTION SECTION LOCATION SE 1/4, SEC. 25, TWP. T41N, RNG, R11E, 3<sup>rd</sup> PM Cook DRILLING METHOD COUNTY HSA/Rotary HAMMER TYPE CME Automatic Surface Water Elev.\_\_\_ Stream Bed Elev. \_\_\_ BORING NO. 4676-BSB-008 422+33 53.80ft Left After Hrs.

CLAY LOAM-gray-medium stiff to hard (continued) Ground Surface Elev. 664.20 ft SILTY CLAY LOAM-gray-medium stiff to very stiff (A-4) (continued) 8 0.6 -45 12 B CLAY LOAM-gray-medium stiff to hard

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



SECTION

GSI Job No. \_\_\_\_17034\_\_

**SOIL BORING LOG** 

ISTHA Contract: I-17-4676, Elgin-O'Hare Western Access (EOWA) DESCRIPTION\_

LOGGED BY MC

LOCATION SE 1/4, SEC. 25, TWP. T41N, RNG. R11E, 3<sup>rd</sup> PM

COUNTY \_\_\_\_\_ Cook \_\_\_\_ DRILLING METHOD \_ HSA/Rotary HAMMER TYPE CME Automatic

Stream Bed Elev. n/a ft BORING NO. 4676-BSB-008 S Qu First Encounter Dry to 10.0' ft
Upon Completion Na ft
After Hrs. ft
End Of Boring @ -100.0'; Boring
backfilled with cuttings. 
 Station
 422+33

 Offset
 53.80ft Left
 (ft) (/6") (tsf) (%) Ground Surface Elev. 664.20 ft CLAY LOAM-gray-medium stiff to hard (continued)

SILTY CLAY LOAM with Fractured Rock-gray-very dense

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

DRAWN BY AMF DATE 06/12/18 DATE 06/12/18 CHECKED BY BGK

exp U.S. SCIVISES
Chicago, IL
BUILDINGS-EARTH & ENVIRONMENT-ENERGY
INDUSTRIAL-INFRASTRUCTURE-SUSTAINABILITY exp U.S. Services Inc.





GSI Job No. \_\_\_\_17034\_\_ **SOIL BORING LOG** 

									Date	211	/18
ROUTE	DE	SCR	IPTIOI	ISTI V	HA Cor	ntract: I-17-4676, Elgin-C'Hare Western Access (EOWA)	LO	GGI	ED BY	N	IC
SECTION		_ ι	OCA1	ION_	SE 1/4	, SEC. 25, TWP. T41N, RNG. R11E, 3"	PM				
COUNTY Cook DRIL	LLING	э ме	THOD			HSA/Rotary HAMMER TY	PE_	С	ME A	utoma	tic
STRUCT. NO.   Station	ft	D E P T H	B L O W S	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev.	t	D E P T H	B L O W S	U C S Qu (tsf)	M O I S T (%)
10.0" ASPHALT	62.67	_			_	CLAY LOAM-brown-medium stiff to stiff (continued)		$\dashv$			
CRUSHED STONE-dense		_	14		$\vdash$	,	-	$\dashv$	3		
		$\equiv$	19		10		_	二	5	0.9	19
_		_	19		$\vdash$			4	6	В	-
CLAY LOAM-dark brown &	60.50					SILTY LOAM-gray-medium dense	10.50	$\dashv$			- 1
gray-very stiff to hard (Fill)		_	6					$\neg$	20		
		_	6 7	3.2 B	19			$\dashv$	6 8		18
		-5	,	ь	$\vdash$		-	-25	0		$\dashv$
											- 1
		_	5 6	2.8	21			$\neg$	11 8		19
		_	8	2.6 B	21		-	$\dashv$	8		19
		_				63	5.50	$\dashv$			$\neg$
1		$\neg$				SILTY CLAY LOAM-gray-loose		コ			- 1
		_	7	4.0	18		-	$\dashv$	3	1.0	16
		-10	8	В				-30	4	P	
							_				
TOPSOIL-black	52.50		7				_	$\neg$			- 1
TOPSOIL-Black		_	9	2.1	32	A2	1.50	$\dashv$			- 1
		$\overline{}$	12	В		CLAY-gray-stiff	1.00	コ			
	50.50						-	$\neg$			- 1
CLAY-dark brown & gray-stiff (Apparent Fill)		_	5					$\dashv$	3		- 1
		_	6	1.5	27		-	$\neg$	3	1.0	19
		-15	7	В			_	-35	5	Р	
CLAY LOAM-brown-medium stiff	48.00	-						$\dashv$			- 1
to stiff		_	2				-	$\dashv$			- 1
		_	4	2.2	18		6.50	二			
		_	6	В	$\vdash$	CLAY LOAM-gray-very stiff		$\dashv$			
becoming gray @ -18.0'		_					-	$\dashv$			
		_	4				_		7		
		_	8	1.8	17			$\exists$	11	2.8	19

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



**SOIL BORING LOG** 

								Date	2/	1/18
	ROUTE	DESCR	IPTIO	ISTI N	HA Cor	ntract: I-17-4676, Elgin-O'Hare Western Access (EOWA)	ogg	ED BY	rN	IC
	SECTION		LOCA	TION_	SE 1/4	, SEC. 25, TWP. T41N, RNG. R11E, 3 <sup>rd</sup> PI	И			
	COUNTY Cook DRIL	LING ME	ETHOE			HSA/Rotary HAMMER TYPE		CME A	utoma	tic
	STRUCT. NO. Station  BORING NO. 4676-BSB-009 Station 352+70	D E P T H	B L O W S	U C S	M O I S T	Surface Water Elev.         n/a ft         ft           Stream Bed Elev.         n/a ft         ft           Groundwater Elev.:         First Encounter         Dry to 10.0' ft	D E P T H	B L O W S	U C S Qu	M O I S T
	Offset 3.30ft Left  Ground Surface Elev. 663.50	ft (ft)	(/6")	(tsf)	(%)	Upon Completion ft After Hrs ft	(ft)	(/6")	(tsf)	(%)
	CLAY LOAM-gray-very stiff (continued)  82 SANDY CLAY	1.50				CLAY LOAM-gray-very stiff to hard (continued)	-			
	LOAM-gray-medium dense	-45	12 10 15	2.8 P	12		-65	16 18 31	4.5 P	11
M_L06.GPJ 322/18	SANDY LOAM-gray-dense	6,50				CLAY-gray-very stiff 596.5				
4676)117034 BORNG LOGS11703	61 CLAY-gray-very stiff	1.50	7 13 30		22	591.5 SANDY CLAY LOAM with		10 17 24	2.3 B	21
J-90 INTERCHANGE ¢-17.		-66	10 15 23	3.5 B	23	Gravel-gray-stiff to hard	-75	15 13 17	3.6 B	11
PROJECT S/2017/17054 EXP, 1490	60 CLAY LOAM-gray-very stiff to hard	6.50	11	2.2	13			13	1.6	13
ZVPR		-80	20	В			-80	27	В	

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



GSI Job No. \_\_\_\_17034

**SOIL BORING LOG** 

ISTHA Contract: I-17-4676, Elgin-O'Hare Western Access (EOWA) DESCRIPTION\_ LOGGED BY MC

	SECTION		ו	LOCA	TION _	SE 1/4	SEC. 25, TWP. T41N, I	RNG. R11E, 3 <sup>™</sup> PI	И		
	COUNTYC	ook DRILL	ING ME	ТНО			HSA/Rotary	HAMMER TYPE	CME A	utoma	tic
	STRUCT. NOStation	376-BSB-009 352+70	D E P T H	B L O W S	U S Qu	M O I S T	Surface Water Elev Stream Bed Elev Groundwater Elev.: First Encounter Upon Completion	n/a ft  Dry to 10.0' ft	D B E L P O T W H S	U C S Qu	M O I S T
	Ground Surface E	lev. 663.50	ft (ft)	(/6")	(tsf)	(%)	After Hrs. End Of Boring @ -100.	ft	(ft) (/6")	(tsf)	(%)
	Gravel-gray-stiff to h (continued)						backfilled with cuttings.				
			_	19 33		9			$\exists$		
			-85	38					-105		
ANGE (I-17-4676)17034 BORNG LOGS117034 LOG.GPJ 3/2/218	SILT-gray-very dens	e 571		17 23 42	4.5 P	10			_105		
<b>TERCH</b>			_	00.0		16					
OJECTS12017/17034 EXP, 1-490_1-90 III	SILTY CLAY LOAM- dense	560 gray-very	3.50	50/3"	4.5	9			-119		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

DRAWN BY AMF DATE 06/12/18 CHECKED BY BGK DATE 06/12/18

exp U.S. Services Inc.
Chicago, IL
BUILDINGS EARTH & ENVIRONMENT ENERGY
INDUSTRIAL INFRASTRUCTURE SUSTAINABILITY





SOIL BORING LOG

							Date	2/2	7/18
ROUTE	DESC	RIPTIO	N	HA Co	ntract: I-17-4676, Elgin-C'Hare Western Access (EOWA)	LOGG	ED BY	rN	1C
SECTION		LOCA	TION_	SE 1/4	, SEC. 25, TWP. T41N, RNG. R11E, 3 <sup>rd</sup> P	М			
COUNTY Cook DRI	LLING M	ЕТНО			HSA/Rotary HAMMER TYPE	:	CME A	utoma	tic
STRUCT. NO.   Station	- P	L 0 <b>W</b>	U C S Qu (tsf)	M O I S T	Surface Water Elev.	D E P T H	B L O W S	U C S Qu (tsf)	M O I S T (%)
TOPSOIL-black				23	CLAY LOAM-brown & gray-stiff to hard (continued)				
CLAY LOAM-dark brown & gray-medium stiff to very stiff (Fill)	68.20	3 3 3	0.5 P	20		Ξ	5 7 10	1.1 B	22
	-	4	Ť			_	5		
	_	5 5 9	2.3 B	16		-25	8 12	1.2 B	19
	=	8 12		12	becoming gray @ -25.5'		10	4.0	13
	=	14				Ξ	3	Р	
		9	3.5 P	15		-30	3	1.8 B	17
	-	4 4	1.6	22	637.2				
	=	7	В			Ξ	9		
		10		27	SILTY LOAM-gray-medium dense	-35	10		17
	_	5		33	,	=			
CLAY LOAM-brown & gray-stiff to	51.20	8				Ξ			
hard	-	7 9 0 10	3.2 B	18		-40	6 10		18

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



GSI Job No. \_\_\_\_17034\_\_ **SOIL BORING LOG** 

ISTHA Contract: I-17-4676, Elgin-O'Hare Western Access (EOWA) \_\_ LOGGED BY \_\_\_MC DESCRIPTION SECTION LOCATION SE 1/4, SEC. 25, TWP. T41N, RNG, R11E, 3<sup>rd</sup> PM Cook DRILLING METHOD COUNTY HSA/Rotary HAMMER TYPE CME Automatic Surface Water Elev.\_\_ Stream Bed Elev. \_\_ BORING NO. 4676-BSB-010 423+84 20.60ft Left Ground Surface Elev. 669.20 ft

SILTY LOAM-gray-medium dense (continued) After \_\_\_\_ Hrs. CLAY-gray-very stiff (continued) CLAY LOAM-gray-very stiff 10 3.4 -45 18 B 14 3.6 13 -65 29 B SILTY CLAY LOAM-gray-stif SILT-gray-very dens CLAY LOAM-gray-stif

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



GSI Job No. \_\_\_\_17034\_\_

**SOIL BORING LOG** 

ISTHA Contract: I-17-4676, Elgin-O'Hare Western Access (EOWA) DESCRIPTION LOGGED BY MC

SECTION LOCATION SE 1/4, SEC. 25, TWP. T41N, RNG. R11E, 3<sup>rd</sup> PM Cook DRILLING METHOD HSA/Rotary COUNTY HAMMER TYPE \_\_\_\_ CME Automatic

STRUCT. NO.   Station	D E P T H	B L O W S	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev.	D E P T H	B L O W s	U C S Qu (tsf)	(9
CLAY LOAM-gray-stiff (continued)	_				End of Boring @ -100.0'. Boring backfilled with cuttings.	_			

SILTY LOAM with GRAVEL-gray-very dens

CLAYEY SAND & GRAVEL-gray-very dense

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

DRAWN BY AMF DATE 06/12/18 DATE 06/12/18 CHECKED BY BGK

exp U.S. Services Inc.
Chicago, IL
BUILDINGS EARTH & ENVIRONMENT ENERGY
INDUSTRIAL INFRASTRUCTURE SUSTAINABILITY





GSI Job No. <u>17034</u> Page <u>1</u> of <u>3</u>

805 Amhe(st Court, 6MR 204 Naperville, Illinois 60565 (650) 355-2888					SC	IL BORING LOG		Page	1	of <u>3</u>
								Date	2/2	6/18
ROUTE	_ DE	SCR	IPTIOI	ISTI N	HA Cor	ntract: I-17-4676, Elgin-O'Hare Western Access (EOWA) L	ogg	ED BY	rN	IC
SECTION		_ ι	OCA	ION_	SE 1/4	, SEC. 25, TWP. T41N, RNG. R11E, 3 <sup>rd</sup> PN	1			
COUNTY Cook DR	ILLING	S ME	THOD			HSA/Rotary HAMMER TYPE	(	CME A	utoma	tic
STRUCT. NOStation	=	D E P	B L O	U C S	М О І	Surface Water Elev. <u>n/a</u> ft Stream Bed Elev. <u>n/a</u> ft	D E P	B L O	U C S	M O I
BORING NO.   4676-BSB-011   Station   423+68   Offset   68.60ft Left	_	H	w s	Qu	S T	Groundwater Elev.: First Encounter Dry to 10.0' ft Upon Completion n/a ft	H	w s	Qu	S T
Ground Surface Elev. 668.80	ft	(ft)	(/6")	(tsf)	(%)	AfterHrsft	(ft)	(/6")	(tsf)	(%)
TOPSOIL-black	667.80	-			25	CLAY-brown & gray-stiff to very	<u> </u>			
SILTY LOAM-brown-dense (Fill)	007.00		7			stiff	$\equiv$	6		
		_	13 18		14		_	6 5	3.3 P	20
SILTY CLAY LOAM-brown &	665.80	_					_			
gray-medium dense to dense (Fill)		_	8				_	4		
		-5	7 10		13		-25	7 9	1.9 B	21
		_								
			5				_	7		
		_	15 20	4.5 P	14		$\overline{}$	10 11	3.7 B	20
CH TV I CAM become 6 constitution	660.80	_		_		h	_		_	
SILTY LOAM-brown & gray-dense (Fill)		-	6			becoming gray @ -28.0'	-	4		
		_	14 19		11		_	5 9	1.4 B	22
	658.30	-10	15				-30	-		
CLAY LOAM-brown & gray spotted black-stiff to very stiff (Fill)		_	4				_			
		_	9	2.6	17	636.80	_			
		-	12	В		SILTY LOAM-gray-medium dense	_			
		_	5				$\equiv$	9		
		_	7	1.1	20		_	9		18
		-15	8	В			-35	9		
		_	_				_			
		-	6 8	1.9	23	631.80	. –			
		Ξ	13	В		CLAY to CLAY LOAM-gray-medium stiff to very	_			
SILTY CLAY-dark brown &	650.80	_				stiff	_			
gray-very stiff (Possible Fill)		_	7	2.5	27		$\overline{}$	3	0.7	16
		-20		2.5 B	21		-40	3	0.7 B	16

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



GSI Job No. \_\_\_\_17034\_\_ **SOIL BORING LOG** 

ISTHA Contract: I-17-4676, Elgin-O'Hare Western Access (EOWA) DESCRIPTION LOGGED BY MC SECTION LOCATION SE 1/4, SEC. 25, TWP. T41N, RNG, R11E, 3<sup>rd</sup> PM Cook DRILLING METHOD HAMMER TYPE \_\_\_\_ CME Automatic COUNTY HSA/Rotary Surface Water Elev.\_\_ Stream Bed Elev. \_\_ BORING NO. 4676-BSB-011 423+68 68.60ft Left After \_\_\_\_ Hrs. Ground Surface Elev. 668.80 ft CLAY to CLAY LOAM-gray-medium stiff to very stiff (continued) LOAM-gray-medium stiff to very stiff (continued) SILTY LOAM-gray-very dense CLAY LOAM-gray-hard

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



SILTY CLAY-gray-very der

SANDY CLAY LOAM with Gravel-gray-very dense

GSI Job No. \_\_\_\_17034\_\_

**SOIL BORING LOG** 

ISTHA Contract: I-17-4676, Elgin-O'Hare Western Access (EOWA) DESCRIPTION\_ LOGGED BY MC

LOCATION SE 1/4, SEC. 25, TWP. T41N, RNG. R11E, 3<sup>rd</sup> PM

\_\_\_\_ DRILLING METHOD \_ COUNTY HSA/Rotary HAMMER TYPE CME Automatic

Stream Bed Elev. n/a ft BORING NO. 4676-BSB-011 S Qu First Encounter Dry to 10.0' ft
Upon Completion Na ft
After Hrs. ft
End Of Boring @ -100.0'; Boring
backfilled with cuttings. 
 Station
 423+68

 Offset
 68.60ft Left

 Ground Surface Elev.
 668.80
 ft
 (ft) (/6") (tsf) (%) CLAY LOAM-gray-hard (continued) 28 4.3 -85 37 B

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

DRAWN BY AMF DATE 06/12/18 DATE 06/12/18 CHECKED BY BGK



exp U.S. SCIVISES
Chicago, IL
BUILDINGS-EARTH & ENVIRONMENT-ENERGY
INDUSTRIAL-INFRASTRUCTURE-SUSTAINABILITY exp U.S. Services Inc.





GSI Job No. \_\_\_\_17034\_\_ SOIL BORING LOG

									Date	2/1	6/18
ROUTE	DES	SCR	IPTIOI	ISTH N	HA Cor	ntract: I-17-4676, Elgin-C'Hare Weste Access (EOWA)	LC	OGG	ED BY	N	IC
SECTION		_ ı	OCA	ION_	SE 1/4	, SEC. 25, TWP. T41N, RNG. R11E,	3 <sup>rd</sup> PM				
COUNTY Cook DRIL	LLING	э ме	THOD			HSA/Rotary HAMMER 1	TYPE .	C	ME A	utoma	tic
STRUCT. NO.   Station   BORING NO.   4676-BSB-012   Station   351+17   Offset   31.10ft Left   Ground Surface Elev.   673.30	-	DEPTH (ft)	B L O W S	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev.   n/a	ft ft ft	DEPTH (ft)	B L O W s (/6")	U C S Qu (tsf)	M O I S T
TOPSOIL-black							652.80				
SANDY CLAY LOAM-brown & black-very stiff (Fill)	72.30		7 6 9	2.8 P	17	CLAY LOAM-brown & gray-stiff to very stiff			7 11 16	1.6 B	19
SILTY LOAM-brown & gray-medium dense (Fill)	70.30		6		15				7	2.3	20
CLAY LOAM-gray-stiff to hard (Fill)	87.80	-5	12 3 5	4.5	12		,	-25	7 9	P	23
			6	Р			,		13	В	
becoming brown & gray @ -10.5'		-10	8 10	2.8 B	13			-30	8 12	1.2 B	20
		_	7 10 15	1.8 B	19	SILT-gray-medium dense	641.30				
		-	8 11 13	1.1 B	19		638.30	-35	9 9		18
		-15	6			CLAY to CLAY LOAM-gray-medium stiff to stiff	338.30	-35			
			9 16	1.8 B	21		,				
		_	7 13 18	2.0 B	19			_	4 8	1.4 B	19

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



GSI Job No. \_\_\_\_17034\_\_ **SOIL BORING LOG** 

	ROUTE	DESCRIPTION	ISTI N	HA Cor	ntract: I-17-4676, Elgin-C Access (EOWA)	Hare Western	LOGG	ED BY		IC_
	SECTION	LOCA	TION_	SE 1/4	, SEC. 25, TWP. T41N,	RNG. R11E, 3 <sup>rd</sup> F	M			
	COUNTY Cook DRIL	LING METHO	D		HSA/Rotary	HAMMER TYP	E(	CME A	utoma	tic
	STRUCT. NO.   Station   BORING NO.   4676-BSB-012   Station   351+17   Offset   31.10ft Left	P O T W H S	U C S Qu (tsf)	M O I S T	Surface Water Elev Stream Bed Elev Groundwater Elev.: First Encounter! Upon Completion		D E P T H	B L O W S	U C S Qu (tsf)	M O I S T
	Ground Surface Elev. 673.30  CLAY to CLAY LOAM-gray-medium stiff to stiff (continued)	. ft (ft) (/6"	(tsi)	(70)	After Hrs. SILTY LOAM-gray-med (continued)	611.	=	(,0 )	(tai)	(70)
		3 - 3 -45 7	0.8 B	20	obvi zovan gray oan		-65	6 10 12	1.3 B	17
34_LOS.GPJ 372218										
1-890_J-90 INTERCHWINGE (#-17-4878)\17034 BORRING LOGS\17034_LOG.GPJ		-50 11 	0.7 B	19	SILTY LOAM-gray-ven	601. y dense	30	16 24 36	4.6 B	13
90_J-90 INTERCHANGE (J-1)		8 8 -55 9	1.2 B	17			-75	35 50/5"		16
PROJECTS/2017/17034 EXP., 148	SILTY LOAM-gray-medium dense	6.30			CLAY LOAM-gray-stiff	598. to hard	30	22		
ZIPROJE		-60 17		27			-80	28 31	4.3 B	10

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



ROUTE

GSI Job No. \_\_\_\_17034

SOIL BORING LOG

Page <u>3</u> of <u>3</u> Date \_\_2/16/18\_

ISTHA Contract I-17-4676, Elgin-O'Hare Western
DESCRIPTION Access (EOWA) LOGGED BY MC

SECTION			_ '	.UCA	HON_	SE 1/4	, SEC. 25, IWP. 141N,	RNG.R11E, 3 PM				_
COUNTY	Cook	DRILLING	S ME	THOD			HSA/Rotary	HAMMER TYPE		ME A	utoma	tic
STRUCT. NO.			D	В	U	М	Surface Water Elev.	n/a ft	D	В	U	

	STRUCT. NO	D E P T H	B L O W S	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev.	D E P T H	B L O W S	U C S Qu (tsf)	M O I S T
	CLAY LOAM-gray-stiff to hard (continued)		22 27	2.9	12	End Of Boring @ -100.0°. Boring backfilled with cuttings.				
S117034_LOG.GPJ 3/22/18		-85	15 19	B 1.7	14		-105			
1490_J-90 INTERCHANGE (F-17-4678)/17034 BORNGLOSS/17034_LOS.GPJ 3/22/18	SANDY CLAY LOAM with Fractured Rock-gray-dense		25 50/2*	В			-110			
P. 1490 J-90 INTERC		-95			10		-115			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

DRAWN BY AMF DATE 06/12/18 CHECKED BY BGK DATE 06/12/18

exp U.S. Services Inc.
Chicago, IL
BUILDINGS EARTH & ENVIRONMENT ENERGY
INDUSTRIAL INFRASTRUCTURE SUSTAINABILITY





**SOIL BORING LOG** 

ROUTE _		DE:	<b>s</b> cr	IPTIOI	ISTI N	HA Cor	ntract: I-17-4676, Elgin-C'Hare Weste Access (EOWA)	ern L(	ogg	ED BY		IC
SECTION			_ ι	OCA	TION_	SE 1/4	, SEC. 25, TWP. T41N, RNG. R11E,	3 <sup>rd</sup> PM				
COUNTY	Cook	DRILLING	3 МЕ	THOD			HSA/Rotary HAMMER	TYPE	(	CME A	utoma	tic
Station	NO		D E P	B L O	U C S	M 0 1	Surface Water Elev. n/a Stream Bed Elev. n/a	ft ft	D E P	B L O	U C S	M 0 1
Station . Offset	NO. 4676-BSB-0 351+37 5.50ft Left		H	s	Qu	S T	Groundwater Elev.: First Encounter Dry to 10.0' Upon Completion n/a	ft	H	s	Qu	S T
Ground TOPSOIL	Surface Elev. 670	.60ft	(ft)	(/6")	(tsf)	(%)	After Hrs CLAY LOAM-brown & gray	ft	(ft)	(/6")	(tsf)	(%)
		669.60	_			15	spotted black-stiff to hard (Fill) (continued)		_			
	AM-brown & um dense (Fill)		_	7		12	(continued)		_	8		34
			_	7		12			_	8		54
			_	9					_	3		
			_	9		10			_	4	1.0 P	30
		665,10	-5	12		$\vdash$		645.10	-25	,	-	-
	AM-brown & gray ack-stiff to hard (Fill)		Ξ	5 5 12	4.5 P	18	CLAY LOAM-brown-stiff to very stiff		Ξ	4 6	1.5 B	23
880			_	10	-				_	4	В	
1000 J			-10	8	2.7 B	14			-30	6	2.3 B	18
VIGA BONE			_	5					_			
11-616			Ξ	6 7	1.2 B	23	becoming gray @ -32.0'		Ξ			
4			_	5					_	5		
			<del>_</del>	7	3.1 B	16			-	7	1.2 B	20
5			-15						-35			=
<u>.</u>			_	5					_			
2012			_	8 12	2.0 B	19	SILTY LOAM to	633.60	_			
2 1 2 1			_	12	В	$\vdash$	LOAM-gray-medium dense		_			
99			_	7					-	8		
HONE.			_	9	2.9 B	25				10		21

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



GSI Job No. \_\_\_\_17034\_\_ **SOIL BORING LOG** 

ISTHA Contract: I-17-4676, Elgin-O'Hare Western Access (EOWA) DESCRIPTION LOGGED BY MC SECTION LOCATION SE 1/4, SEC. 25, TWP. T41N, RNG, R11E, 3<sup>rd</sup> PM Cook DRILLING METHOD HAMMER TYPE \_\_\_\_ CME Automatic COUNTY HSA/Rotary Stream Bed Elev. BORING NO. 4676-BSB-013 After Hrs.

CLAY LOAM-gray-stiff to very stiff Ground Surface Elev. 670.60 ft SILTY LOAM to CLAY LOAM-gray-stiff to very stiff SILTY CLAY-gray-hard 24 5.3 13 -70 32 B CLAY LOAM-gray-hard

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



SILTY CLAY-gray-stiff

SILTY LOAM with Gravel-gray-very dense

GSI Job No. \_\_\_\_17034\_\_

**SOIL BORING LOG** 

ISTHA Contract: I-17-4676, Elgin-O'Hare Western Access (EOWA) DESCRIPTION LOGGED BY MC

SECTION LOCATION SE 1/4, SEC. 25, TWP. T41N, RNG. R11E, 3<sup>rd</sup> PM Cook DRILLING METHOD COUNTY HSA/Rotary HAMMER TYPE CME Automatic

Stream Bed Elev. BORING NO. 4676-BSB-013 S Qu First Encounter Dry to 10.0' ft
Upon Completion Na ft
After Hrs. ft
End Of Boring @ -100.0'; Boring
backfilled with cuttings. 
 Station
 351+37

 Offset
 5.50ft Left
 (ft) (/6") (tsf) (%) Ground Surface Elev. 670.60 ft CLAY LOAM-gray-hard (continued)

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

DRAWN BY AMF DATE 06/12/18 DATE 06/12/18 CHECKED BY BGK

exp U.S. SCIVISES
Chicago, IL
BUILDINGS-EARTH & ENVIRONMENT-ENERGY
INDUSTRIAL-INFRASTRUCTURE-SUSTAINABILITY exp U.S. Services Inc.



		REVISIONS	CONTRACT NO. I-18-4694	S-101
NO.	DATE	DESCRIPTION	CONTRACT NO. 1-10-4634	3-101
			BRIDGE NOS. 1681 & 1682	DRAWING NO.
				209 <i>o</i> ≥ 220
			BORING LOG	209 of 220



**SOIL BORING LOG** 

							Date	2/1.	3/18
ROUTE	DESC	RIPTIO	ISTI N	1A Cor	ntract: I-17-4676, Elgin-C'Hare Western Access (EOWA) LO	GGE	D BY	A	D
SECTION		LOCA	TION_	SE 1/4	, SEC. 25, TWP. T41N, RNG. R11E, 3 <sup>rd</sup> PM				
COUNTY Cook DRIL	LING M	ETHOD			HSA/Rotary HAMMER TYPE	CI	ME A	utomat	tic
STRUCT. NO	P T H	B L O W S	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev.	D E P T H	B L O W S	U C S Qu (tsf)	M O I S T (%)
6.0" ASPHALT 67 CRUSHED STONE-very dense	0.50				650.50 SILTY LOAM to LOAM-dark brown	$\Box$			
	Ξ	35 45		8	& gray-medium dense (Fill)	4	5 7 9		18
SILTY CLAY LOAM-gray-medium dense (Fill)	8.00	5 5		6	CLAY LOAM-brown & gray-very stiff (Fill)	#	6	2.0	18
	_ <	7		ь		-25	8	2.0 B	10
CLAY LOAM-brown & gray-stiff to hard (Fill)	5.50	4 6 6	3.5 P	14		=	8 9 11	2.0 P	20
	=	4			SILTY SAND-dark gray	7	ST		21
	-10	10 12	2.5 P	14		-30			
	=	10 13 15	4.5 P	15	639.00 SILTY LOAM-gray-medium dense	╡			
	Ξ	4	1.4	18		∄	6		20
	15		B	10		-35	9		20
SILTY CLAY LOAM-dark brown,gray & black-medium dense (Fill)	5.50	6	3.5	26		=			
CLAY LOAM-dark brown & gray-very stiff (Fill)	3.00	5	Р			$\exists$	4		
	-21	7	2.4 B	21		-40	4		16

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



GSI Job No. \_\_\_\_17034\_\_ **SOIL BORING LOG** 

						Date	8
	ROUTE	DESC	RIPTIO	N_	HA Cor	ntract: I-17-4676, Elgin-O'Hare Western Access (EOWA) LOGGED BY AD	_
	SECTION		LOCA	TION_	SE 1/4	4, SEC. 25, TWP. T41N, RNG. R11E, 3 <sup>rd</sup> PM	_
	COUNTY Cook DRIL	LING N	IETHO	D		HSA/Rotary HAMMER TYPE CME Automatic	_
	STRUCT. NO.   Station	-   E	L O W I S	U C S Qu	M O I S T	Surface Water Elev.	6
	Ground Surface Elev. 671.00 SILTY LOAM-gray-medium dense	ft (f	t) (/6")	(tsf)	(%)	After Hrs ft (ft) (/6") (tsf) (%	-)
	(continued)	9.00	5 7	1.5 B	18	(continued)  OLAY LOAM-gray-very stiff to hard  7  12 3.1 13	3
BORNGLOGS117034_LOG.GPJ 3/22/18	SILTY LOAM-gray-medium dense	4.00	12 10 10	В	18		1
TERCHANGE (-17-4678) 117034 BORNG	CLAY LOAM-gray-very stiff	9.00	7 12	3.6 B	16		7
ZAPROJECTS(2017A) 7034 EXP., 1484 _ 1-90 INC	61 SILT-gray-medium dense	4.00	11 13 11	В	19		1

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



GSI Job No. \_\_\_\_17034

**SOIL BORING LOG** 

Page <u>3</u> of <u>3</u>

Date \_\_2/13/18\_ ISTHA Contract: I-17-4676, Elgin-O'Hare Western

DESCRIPTION Access (EOWA) LOGGED BY AD

SECTION LOCATION SE 1/4, SEC. 25, TWP. T41N, RNG. R11E, 3<sup>rd</sup> PM

COUNTY Cook DRILLING METHOD HSA/Rotary HAMMER TYPE CME Automatic Stream Bed Elev. <u>n/a</u> ft BORING NO. 4676-BSB-014 Station 424+22 H S Qu T First Encounter Upon Completion 12.40ft Right Ground Surface Elev. 671.00 ft (ft) (/6") (tsf) (%) End Of Boring @ -100.0" Boring (continued) (ft) (/6") (tsf) (%) CLAY LOAM-gray-very stiff to hard (continued)

25 5.6 -85 25 B SILTY CLAY LOAM with FRACTURED ROCK-gray-very

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

DRAWN BY AMF DATE 06/12/18 DATE 06/12/18 CHECKED BY BGK

exp U.S. SERVISE : Chicago, IL BUILDINGS EARTH & ENVIRONMENT ENERGY INDUSTRIAL INFRASTRUCTURE SUSTAINABILITY exp U.S. Services Inc.





	(030) 355-2818					SC	IL BORING LOG		ı ayı		oi <u>J</u>
									Date	2/1	4/18
	ROUTE	DESC	RII	PTIO	ISTI N	HA Cor	ntract: I-17-4676, Elgin-C'Hare Western Access (EOWA)	OGG			
	SECTION		L	OCAT	ION_	SE 1/4	, SEC. 25, TWP. T41N, RNG. R11E, 3 <sup>rd</sup> PN	<u> </u>			
	COUNTY Cook DRILL	LING	ME.	THOD			HSA/Rotary HAMMER TYPE		ME A	utoma	tic
	STRUCT. NO	- 1	Ē	B L	U C	M O	Surface Water Elev. <u>n/a</u> ft Stream Bed Elev. <u>n/a</u> ft	D E	B L	U	M O
	BORING NO. 4676-BSB-015	1	P T	o W S	S Qu	S T	Groundwater Elev.:	P T H	o W s	S Qu	S T
	Station         424+16           Offset         45.10ft Left           Ground Surface Elev.         671.30		- 1	(/6")	(tsf)	(%)	First Encounter		(/6")	(tsf)	(%)
	TOPSOIL-black		1			25	CLAY LOAM-brown & gray-stiff to very stiff (continued)	_			
	SANDY LOAM-brown-medium	0.30	+	4		25	very suit (conunaeu)	$\dashv$	4		
	dense (Fill)		$\forall$	9		9			6	2.5	17
			コ	16				$\neg$	8	В	
	SILTY CLAY-gray-stiff to hard (Fill)	3.30	Ⅎ	4				$\exists$	4		
		_	⇉	10	4.3	14		$\exists$	6	1.7	22
		-	-5	17	Р			-25	7	В	
2		-	$\dashv$	2				-	7		
325		_	╡	4 7	4.5 P	13		$\exists$	13 20	3.3 B	19
3		_	⇉	_	Ė			$\exists$	20		
3			Ⅎ	4				$\exists$	7		
3			10	10 12	4.0 P	20		-30	12 15	1.9 B	20
			$\exists$					$\exists$			
3		_	7	7	2.8	16	639.30	$\Box$			
7		_	⇉	18	В		SILTY LOAM-gray-medium dense				
1		-	Ⅎ					$\exists$			
		-	$\pm$	9		25		$\dashv$	10		19
2		_	15	14				-35	12		
		_	╡	ST				$\exists$			
Ś			+	٠.	1.5	26	634.30	, ⊢			
5		_	コ		Р		CLAY LOAM-gray-medium stiff to stiff	コ			
-	CLAY LOAM-brown & gray-stiff to	3.30	+			$\vdash$	Sull Sull	$\dashv$			
2	very stiff		┨	6				$\dashv$	6		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



GSI Job No. \_\_\_\_17034\_\_ Page <u>2</u> of <u>3</u> **SOIL BORING LOG** 

(620) 302-5099				U	IL DOMINO LOO		_		
DOUTE	DE	COIDT	IST	HA Cor	ntract: I-17-4676, Elgin-O'Hare Western Access (EOWA)				
		_			, SEC. 25, TWP. T41N, RNG. R11E, 3 <sup>rd</sup> P				
COUNTY Cook	DRILLING	METH	DD	_	HSA/Rotary HAMMER TYPE		ME A	utoma	atic
STRUCT. NO		D B		M O	Surface Water Elevn/a ft	D E	B L	U	1
		P O	S	i s	Stream Bed Elevft	P	o w	Š	]
8tation 45.10ft 45.10ft	3-015 16	H S		T	Groundwater Elev.: First EncounterDry to 10.0'ft	н	S	Qu	
Offset 45.10ft Ground Surface Elev, 6	Leftft	(ft) (/6	") (tsf)	(%)	Upon Completion ft After Hrs ft	(ft)	(/6")	(tsf)	(
CLAY LOAM-gray-medium s	11.00		1	` '	SILTY LOAM-gray-dense	1. /	. ,	. ,	ŀ.
stiff (continued)		$\dashv$			(continued)	$\dashv$			
		コ			609.3 CLAY LOAM-gray-very stiff	0			
		$\dashv$			CLAY LOAM-gray-very sum	$\dashv$			
		┒╻				$\neg$	9		
		7	1.3	18		$\exists$	12	2.7	ļ,
		-45 9	В	$\vdash$		-65	15	В	┝
		コ				$\exists$			
	624.30				604.3				
SILTY CLAY LOAM-gray-ve dense	ry	$\exists$			SANDY CLAY LOAM-gray-very dense	_			
		┪				コ			
		30		18		$\dashv$	20 40	4.5	١,
		<sub>-50</sub> 39	9	_		-70	47	Р	L
		╛							
	619.30	$\exists$			599.3	$\Box$			
CLAY LOAM-gray-very stiff	010.30	コ			CLAY LOAM-gray-stiff to hard	_			
		$\dashv$				$\dashv$			
		6 9	2.0	18		$\exists$	20 24	4.3	ļ,
		-55 15				-75	30	P.5	L
		$\dashv$				$\dashv$			
		コ				$\neg$			
SILTY LOAM-gray-dense	614.30	$\exists$				$\exists$			
		-				$\exists$			
			.	ı	I	$\neg$	20	ı	
		14		24		$\rightarrow$	26 32	4.5	1

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



COUNTY

SILTY CLAY LOAM with Gravel-gray-very dense

Cook DRILLING METHOD \_

GSI Job No. \_\_\_\_17034\_\_

HAMMER TYPE \_\_\_\_ CME Automatic

**SOIL BORING LOG** 

HSA/Rotary

Page <u>3</u> of <u>3</u> Date \_\_\_2/14/18\_\_

ISTHA Contract: I-17-4676, Elgin-O'Hare Western Access (EOWA) DESCRIPTION\_ LOGGED BY MC

SECTION LOCATION SE 1/4, SEC. 25, TWP. T41N, RNG. R11E, 3<sup>™</sup> PM

Stream Bed Elev. n/a ft BORING NO. 4676-BSB-015 First Encounter Dry to 10.0' ft
Upon Completion n/a ft
After Hrs. ft
End Of Boring @ -100.0'. Boring
backfilled with cuttings. (ft) (/6") (tsf) (%)

31 4.5 13 -85 39 P

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

DRAWN BY AMF DATE 06/12/18 DATE 06/12/18 CHECKED BY BGK

exp U.S. SERVISE : Chicago, IL BUILDINGS EARTH & ENVIRONMENT ENERGY INDUSTRIAL INFRASTRUCTURE SUSTAINABILITY exp U.S. Services Inc.





**SOIL BORING LOG** 

			ют				Date		2/10
ROUTE DE	SCR	IPTIO	N	HA CO	ntract: I-17-4676, Elgin-C'Hare Western Access (EOWA)	LOGG	ED BY	rN	1C
SECTION	_ 1	OCA	TION_	SE 1/4	, SEC. 25, TWP. T41N, RNG. R11E, 3 <sup>rd</sup> F	M			
COUNTY Cook DRILLIN	G ME	THOE			HSA/Rotary HAMMER TYP	Ē	CME A	utoma	tic
STRUCT. NO.   Station   BORING NO.   4676-BSB-016   Station   351+03   Offset   5.50ft Left	D E P T H	B L O W S	U C S	M O I S T	Surface Water Elev.	D E P T H	B L O W S	U C S Qu	M 0 8 T
Offset 5.50ft Left Ground Surface Elev. 674.70 ft	(ft)	(/6")	(tsf)	(%)	Upon Completionn/a_ft AfterHrsft	(ft)	(/6")	(tsf)	(%)
TOPSOIL-black 673.70 SILTY CLAY-brown-dark brown &	_	3		20	CLAY LOAM-brown & gray spotted black-stiff to very stiff (Fill) (continued)	Ξ	6		
black-stiff to hard (Fill)	_	4 7	4.3 P	13		=	11 14	2.8 B	20
	=	2			CLAY-brown & gray-stiff to very stiff	-	3		
	▼-5	3	1.5 P	15		-25	6 7	1.3 B	27
CLAY-gray-stiff to very stiff (Fill)	_	2	1.0	19		Ξ	4	1.4	21
	Ξ	3	P	19		Ξ	5	B	
	_	5 2 13	3.5 B	15		-30	4 7 11	2.1 B	19
664.20 CLAY LOAM-brown & gray spotted black-stiff to very stiff (Fill)	10	7	В			30	Ϊ	В	
	=	9	2.3 B	20	becoming gray @ -32.0'	_	1		
	Ξ	6	4.0	18		Ξ	4		20
	-15	14	1.8 B	18		-35	8	1.1 B	20
	=	12 14	2.3	19	637.	70 -	}		
	Ξ	17	В	$\vdash$	SILTY LOAM-gray-medium dense	Ξ	]		
	_	6 12	1.5	21		_	7 8		20
	-20	14	В	ı	I	-40	12		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



GSI Job No. \_\_\_\_17034\_\_ **SOIL BORING LOG** 

ISTHA Contract: I-17-4676, Elgin-O'Hare Western Access (EOWA) DESCRIPTION LOGGED BY MC SECTION LOCATION SE 1/4, SEC. 25, TWP. T41N, RNG, R11E, 3<sup>rd</sup> PM Cook DRILLING METHOD COUNTY HSA/Rotary HAMMER TYPE CME Automatic Stream Bed Elev. BORING NO. 4676-BSB-016 First Encounter 669.7 ft V
Upon Completion n/a
After Hrs. ft 351+03 5.50ft Left Ground Surface Elev. 674.70 ft
SILTY LOAM-gray-medium dense After \_\_\_\_ Hrs.

CLAY LOAM-gray-very stif CLAY-gray-medium stiff to very SILTY CLAY LOAM-gray-very SILTY CLAY LOAM-gray-very stiff SILTY CLAY-gray-stiff to hard CLAY LOAM-gray-very stiff

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



BORING NO. 4676-BSB-016

GSI Job No. \_\_\_\_17034\_\_

**SOIL BORING LOG** 

ISTHA Contract: I-17-4676, Elgin-O'Hare Western Access (EOWA) DESCRIPTION LOGGED BY MC

LOCATION SE 1/4, SEC. 25, TWP. T41N, RNG. R11E, 3<sup>rd</sup> PM Cook DRILLING METHOD COUNTY HSA/Rotary HAMMER TYPE CME Automatic Stream Bed Elev.

First Encounter 669.7
Upon Completion n/a
After Hrs.

End Of Boring @ -100.0'. Boring backfilled with cuttings. 
 Station
 351+03

 Offset
 5.50ft Left
 669.7 ft ▼ <u>n/a</u> ft (ft) (/6") (tsf) (%) Ground Surface Elev. 674.70 ft SILTY CLAY-gray-stiff to hard (continued) 31 3.8 -85 38 P CLAY LOAM-gray-very stir 29 2.4 SANDY CLAY LOAM with Fractured Rock-gray-very dense

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

DRAWN BY AMF DATE 06/12/18 DATE 06/12/18 CHECKED BY BGK

exp U.S. Services Inc. exp U.S. SCIVISES
Chicago, IL
BUILDINGS-EARTH & ENVIRONMENT-ENERGY
INDUSTRIAL-INFRASTRUCTURE-SUSTAINABILITY





									Date	3/5	5/18
ROUTE	DES	SCR	IPTIO	ISTH N	HA Cor	ntract: I-17-4676, Elgin-C'Hare West Access (EOWA)	ern LC	ogg	ED BY	N	IC
SECTION		_ ι	OCAT	ION_	SE 1/4	, SEC. 25, TWP. T41N, RNG. R11E	3 <sup>rd</sup> PM				
COUNTY Cook DRIL	LING	э ме	THOD		Hol	low Stem Auger HAMMER	TYPE .	(	ME A	utoma	tic
STRUCT. NO.   Station   BORING NO.   4676-RWB-001	- - - ft	DEPTH (f)	B L O <b>W</b> s (/6")	U S Qu (tsf)	M O I S T (%)	Surface Water Elev.	_ft. <b>▼</b> ft.	DEPTH (ft)	B L O W s (/6")	U C S Qu (tsf)	M O I S T
SILTY SAND & GRAVEL-gray (Fill)	24.00				13	SAND-dark grav-loose	641.50	_			
CLAY LOAM-brown & gray-very stiff (Fill)	31.00	Ξ	3 7 8	2.8 P	19	o, me dam gray 10000		Ξ	3 4 4		19
			4	2.8	23	SILTY LOAM-gray-medium dense	639.00		3		20
		-5	6	P.0	23			-25	5		20
		_	3			SILT-gray-medium dense	636.50	_	4		
95	54.00	_	4 6	2.0 P	19		634.00	_	5 7		19
SANDY CLAY LOAM-brown & gray-very stiff		_	4			SILTY LOAM-gray-loose		_	3		
		-10	6 8	2.0 P	17			-30	3 4		19
CLAY LOAM-brown & gray-very stiff	51.50	_	3	3.3	17		630.00	_			
		_	7	В		CLAY LOAM-gray-stiff	630.00	_			
		_	3	3.2	19			_	3	1.5	17
		-15	8	В				-35	6	В	
becoming gray @ -15.5'		_	5					_			
		_	8 10	2.3 B	18	SILTY CLAY LOAM-gray-medium dense	625.00	_			
SANDY LOAM-gray-medium dense	44.00	Y				delise		_			
delise		_	5 7 7		18	End Of Boring @ -40.0°. Boring		_	8	0.5 B	18

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



GSI Job No. \_\_\_\_17034\_\_ **SOIL BORING LOG** 

										Date	3/1	2/18
	ROUTE	_ DES	SCR	IPTIO	ISTH N	HA Cor	ntract: I-17-4676, Elgin-O'Hare West Access (EOWA)	em L(	ogg	ED BY	N	1C
	SECTION		_ ι	OCA1	ION_	SE 1/4	SEC. 25, TWP. T41N, RNG. R11E	. 3 <sup>rd</sup> PM				
	COUNTY Cook DR	ILLING	S ME	THOD		Hol	low Stem Auger HAMMER	TYPE	(	OME A	utoma	tic
	STRUCT. NOStation	=	D E P	B L O	U C <b>S</b>	M 0 1	Surface Water Elev. <u>n/a</u> Stream Bed Elev. <u>n/a</u>	ft ft	D E P	B L O	U C <b>s</b>	M O I
	BORING NO. 4676-RWB-002 Station 353+80 Offset 24.20ft Right	_	H	s	Qu	S T	Groundwater Elev.: First Encounter Dry Upon Completion 658.0	_ft _ft ▽	H	s	Qu	S T
	Ground Surface Elev. 661.00	ft	(ft)	(/6")	(tsf)	(%)	After Hrs.	ft	(ft)	(/6")	(tsf)	(%)
	12.0" TOPSOIL-black	860.00				32	SILTY LOAM-gray-medium dense	640.50				
	CLAYEY TOPSOIL-dark brown to black			1					_	5		
	DIACK		_	2		34			_	7 8		18
		7	∇				CLAY-gray-medium stiff	638.00	_			
	(	857.00		1					_	8		
	SILTY CLAY-dark brown & gray-medium stiff to stiff		-5	2	0.5 P	25			-25	8	0.9 B	19
			-0					635.50	-23			
92			_	2			SILTY CLAY LOAM-gray-stiff		_	3		
3/2/2/18			-	3	1.7	22	1		_	3	1.3	19
GP.			$\neg$	5	В				_	6	Р	
46L005117034_L00.GPJ			-				CLAY LOAM with	633.00				
11 703			$\equiv$	4			GRAVEL-gray-stiff		_	3		
88			40	7	1.9 B	26			-	5 7	1.3 B	13
SNS SNS		350.50	-10	_	_				-30	_		
34 BO	CLAY LOAM-brown & gray-stiff to very stiff		$\equiv$	5					_			
(-17-4676)\17034 BORB	· · · · · · · · · · · · · · · · · · ·		-	10	3.7	16		629.00	_			
7-467			$\neg$	12	В		CLAY-gray-medium stiff	020.00	_			
1-1			_						_			
HANG			_	5					_	3		
TERC			1	6 8	1.9 B	20			_	7	0.7 B	21
8			-15	0	В				-35	11	В	
8			_						_			
90			-	6	2.2	20			_			
7034			$\exists$	9	B	20	SILT-gray-dense	624.00				
1710	(	343.00							_			
CTS12	CLAY LOAM with GRAVEL-gray-medium dense		-	3					_	11		
PROJECT \$12017/17034 EXP			_	7	4.5 P	13	End Of Boring @ -40.0'. Boring backfilled with cuttings.		_	16 21		21
ź			-20	0	P		addining with country of	621.00	-40	21		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



SECTION

GSI Job No. \_\_\_\_17034

**SOIL BORING LOG** 

LOCATION SE 1/4, SEC. 25, TWP. T41N, RNG. R11E, 3<sup>rd</sup> PM

DESCRIPTION ISTHA Contract: I-17-4676, Elgin-O'Hare Western Access (EOWA) LOGGED BY MC

COUNTY Cook DRILLING	ME	THOD		Hol	llow Stem Auger	HAMMER TYPE	(	OME A	utoma	tic
Station	D E P T	B L O W	U C S	M 0 1 8	Surface Water Elev Stream Bed Elev	n/a ft n/a ft	D E P T	B L O W	U C S	M O I S
	Ĥ	s	Qu	Ť	First Encounter _	647.1 ft ▼	Ĥ	s	Qu	Ť
Grodina Garrago Elevi 000.10 It	(ft)	(/6")	(tsf)	(%)	Upon Completion _ After Hrs	ft ft	(ft)	(/6")	(tsf)	(%)
TOPSOIL-black						648.60	_			
668.10				34	SAND-brown-loose to	medium				
CLAY LOAM-brown & gray-very		5			dense			4		
stiff to hard (Fill)	П	7	4.5	13	1		•-	7	4.5	23

	-5	3 5 6	2.7 B	16	becoming gray @ -23.0'	-25	2 3 3	L
	=	3			SILT-gray-medium dense	$\exists$	4	
	=	5 7	3.5 B	17		긬	4 6	
SILTY CLAY-black-medium dense	-	23				╡	4	
	-10	7		25		-30	7 8	
CLAY LOAM-brown & gray-very stiff to hard		5				ᅻ		

End Of Boring @ -40.0'. Boring backfilled with cuttings. The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)

DRAWN BY AMF DATE 06/12/18 CHECKED BY BGK DATE 06/12/18

exp U.S. Services Inc.
Chicago, IL
BUILDINGS EARTH & ENVIRONMENT ENERGY
INDUSTRIAL INFRASTRUCTURE SUSTAINABILITY



CONTRACT NO. I-18-4694 S-105 DESCRIPTION DRAWING NO. BRIDGE NOS. 1681 & 1682 213 of 220 BORING LOG



GSI Job No. \_\_\_\_17034\_\_ **SOIL BORING LOG** 

									Date	2/2	8/18
ROUTE	DES	SCR	IPTIOI	ISTI N	HA Cor	ntract: I-17-4676, Elgin-C'Hare West Access (EOWA)	em L(	OGG	ED BY	N	IC
SECTION		_ ι	OCA	ION_	SE 1/4	, SEC. 25, TWP. T41N, RNG. R11E	. 3 <sup>rd</sup> PM				
COUNTY Cook DRI	LLING	S ME	THOD		Hol	low Stem Auger HAMMER	TYPE	C	CME A	utoma	tic
STRUCT. NO.   Station	-	DEPTH (ft)	B L O W s	U C S Qu (tsf)	M O I S T (%)	Surface Water Elev.	_ ft ▼ ft	DEPTH (ft)	B L O W s (/6")	U C S Qu (tsf)	M O I S T
TOPSOIL-black	59.10	_			44	CLAY LOAM-brown & gray-stiff to hard (continued)		_			
CLAY LOAM-brown & gray spotted black-stiff to hard (Fill)		Ξ	3 5 7	2.3 P	16			$\equiv$	4 5 8	1.2 B	17
			4	1.0					3		10
		-5	7 9	4.2 B	18			-25	4 6	1.2 B	19
		_	3 6 8	1.8 B	21				3 5 6	1.8 B	17
CLAY LOAM-brown & gray-stiff to hard	52.10	-10	6 9 12	9.2 B	15			-30	3 5 8	1.9 B	18
		_	5 9 13	5.8 B	18			_			
		-15	6 7 13	4.3 P	20			-35	5 8 13	4.3 B	17
becoming gray @ -15.5'			3 5 6	1.6 B	20						
		-20	4 5 4	2.3 B	17	End Of Boring @ -40.0'. Boring backfilled with cuttings.	620.10	<b>▼</b>	7 11 16	2.3 B	16

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)
BBS, from 137 (Rev. 8-99)



GSI Job No. \_\_\_\_17034\_\_ **SOIL BORING LOG** 

											Date	5/1	5/18
ROUTE	I-490	_ DE	SCRII	PTION	IST	HA Co	ntract: I-17-4676, Elgin- Access (EOWA)	-O'Hare Weste	LC	OGGE	D BY	R	RN
SECTION	I-490		_ ι	OCAT	ION _	SE 1/4	, SEC. 25, TWP. T41N,	, RNG. R11E, 3	<sup>™</sup> PM				
COUNTY	Cook DF	RILLING	MET	HOD		Ho	low Stem Auger	_ HAMMER T	YPE .	(	OME A	utoma	tic
BORING NO Station Offset Ground Surface	4676-RWB-005 353+94 90.30ft Right e Elev. 660.80		D E P T H	B L O W S	U C S Qu (tsf)	M O I S T	Surface Water Elev. Stream Bed Elev. Groundwater Elev.: First Encounter Upon Completion After Hrs.	641.8 n/a	ft ft <u>▼</u> ft	D E P T H	B L O W s	U C S Qu (tsf)	M O I S T
CRUSHED STO	NE		_			5	SANDY LOAM to		640.30	_			
CLAY LOAM wit		659.80		4		-	LOAM-gray-medium	dense			5		
brown & gray-ve	ry stiff to hard (Fill)		_	6	4.5 P	13				=	5 8		20
			_	0	Р	$\vdash$			637.80	-	0		
			$\overline{}$	5			SAND-gray-medium of	dense		_	7		
			_	6	4.5	11				-	9		1
			-5	8	Р					-25	10		
			_				SILTY LOAM-gray-mo	edium dense	635.30				
			$\overline{}$	4 6	2.1	18				$\neg$	3		1:
			_	6	В	10					8		"
			_				CLAY LOAM-gray-stif		632.80	_			
			_	5			CLAT LOAM-gray-sur	ii to very suii		_	4		
			_	6	2.8 B	19				_	7 11	2.5 B	1
		650.30	-10	-						-30			$\vdash$
CLAY LOAM-bro	own & gray-hard		_	5						=			
			_	10	4.5	16							
			_	14	Р					_			
			_										
			_	5	4.8	17				_	5	1.7	1
			-15	10	В					-35		В	
			_							_			
			_	6									
			_	13 17	6.2 B	17				_			
		642.80	_						622.80				
SILTY CLAY wit Seams-gray-stiff				7			SAND-gray-dense			_	7		
			¥	10	1.5	25	End Of Boring @ -40				17		1
			-20	11	Р		backfilled with cutting	S.	620.80	-40	14		$\Box$

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer) The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, from 137 (Rev. 8-99)



GSI Job No. \_\_\_\_17034\_\_

**SOIL BORING LOG** 

Date \_\_5/15/18\_

	ROUTE	1-490	)	DE	SCRI	PTION	IST	HA Co	ntract: I-17-4676, Elgin Access (EOWA)	-O'Hare Weste	em LC	OGGE	Date DBY		RN
				_					, <b>SEC.</b> 25, <b>TWP.</b> T41N		_				
	COUNTY _	Cook	DF	RILLING	MET	HOD	_	Hol	low Stem Auger	_ HAMMER	TYPE .	(	OME A	utoma	tic
	STRUCT. No Station	0		_	D E P	B L O	U C <b>S</b>	M 0 1	Surface Water Elev. Stream Bed Elev.	n/a n/a	_ft _ft	D E P	B L O	U C <b>S</b>	М О І
	Station _ Offset _	0. 4676-F 42 85.00	1+57 Ift Right		H	S	Qu	S T	Upon Completion	641.5 643.5	_ft <u>⊽</u>	H	S	Qu	S T
1	6.0" TOPSO	urface Elev		ft 662.00		(/6")	(tsf)	(%)	After Hrs. CLAY LOAM-brown 8	& gray-hard	_ ft 642.00	٠,	(/6")	(tsf)	(%)
	CLAY LOA! gray-mediur	M-dark brown m stiff (Fill)	&	002.00	_	3		24	(continued) SILTY LOAM-gray-m	-	$\overline{}$	▼_	3		
					_	3 4	0.8 P	17				$\exists$	4 6		25
	SANDY CL & gray-loose	AY LOAM-dar e (Fill)	k brown	659.50	_	4			SAND-gray-loose		639.50	$\exists$	2		
					-5	4 5	2.5 P	10				-25	3		21
5/30/18	CLAY LOA! hard (Fill)	M-brown & gra	ry-stiff to	657.00	_	2	4.0	20	SILT-gray-medium de	ense	637.00		3		21
					_	10	B	20				_	6		21
S117094_L					Ξ	2						$\exists$	3		
ING LOG					-10	5 7	1.9 B	19				-30	7		23
PROJECTS20171 7034 EXP. 1-400_1-400_1-400 INTERCHANGE (1-17-4676)(17034 BORNIG LOGS(17034_LOG-GPJ	CLAY LOA!	M-brown & gra	y-hard	652.00	- -	7	4.5	13			630.50	_			
HANGE (1-17-4					_	7	Р		CLAY to CLAY LOAN very stiff	M-gray-stiff to			3		
INTERC					-15	10 12	7.1 B	17				-35	5 5	1.0 B	19
P.1-600_100					_	7						$\exists$			
711 700M EX					_	10 15	5.7 B	19				$\exists$			
ECT5/20					ν-	9						$\exists$	4		
Z-PRO					-20	11 17	4.5 P	21	End Of Boring @ -40 backfilled with cutting		622.50	_ -40	8 12	2.4 B	18

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetron The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, from 137 (Rev. 8-99)

DRAWN BY AMF DATE 06/12/18 CHECKED BY BGK DATE 06/12/18

exp U.S. Services Inc.
Chicago, IL
BUILDINGS EARTH & ENVIRONMENT ENERGY
INDUSTRIAL INFRASTRUCTURE SUSTAINABILITY





GSI Job No. \_\_\_17034\_\_

**SOIL BORING LOG** 

ISTHA Contract: I-17-4676, Elgin-O'Hare Western Access (EOWA) LOGGED BY RN DESCRIPTION SECTION I-490 LOCATION SE 1/4, SEC. 25, TWP. T41N, RNG. R11E, 3<sup>rd</sup> PM COUNTY Cook DRILLING METHOD Hollow Stem Auger HAMMER TYPE CME Automatic Surface Water Elev. Stream Bed Elev. STRUCT. NO. <u>n/a</u> ft <u>n/a</u> ft

BORING NO. 4676-RWB-008 CLAY LOAM-dark brown & gray spotted black-very stiff to hard (Fill) becoming brown & gray @ -3.0'

becoming gray @ -23.0

SAND-gray-medium dens

CLAY LOAM-brown & gray-very stiff to hard

CLAY LOAM with Stone-dark

SILTY CLAY-gray-stiff

8 6.2 17 End Of Boring @ -40.0". Boring backfilled with cuttings. The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Pen The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, from 137 (Rev. 8-99)

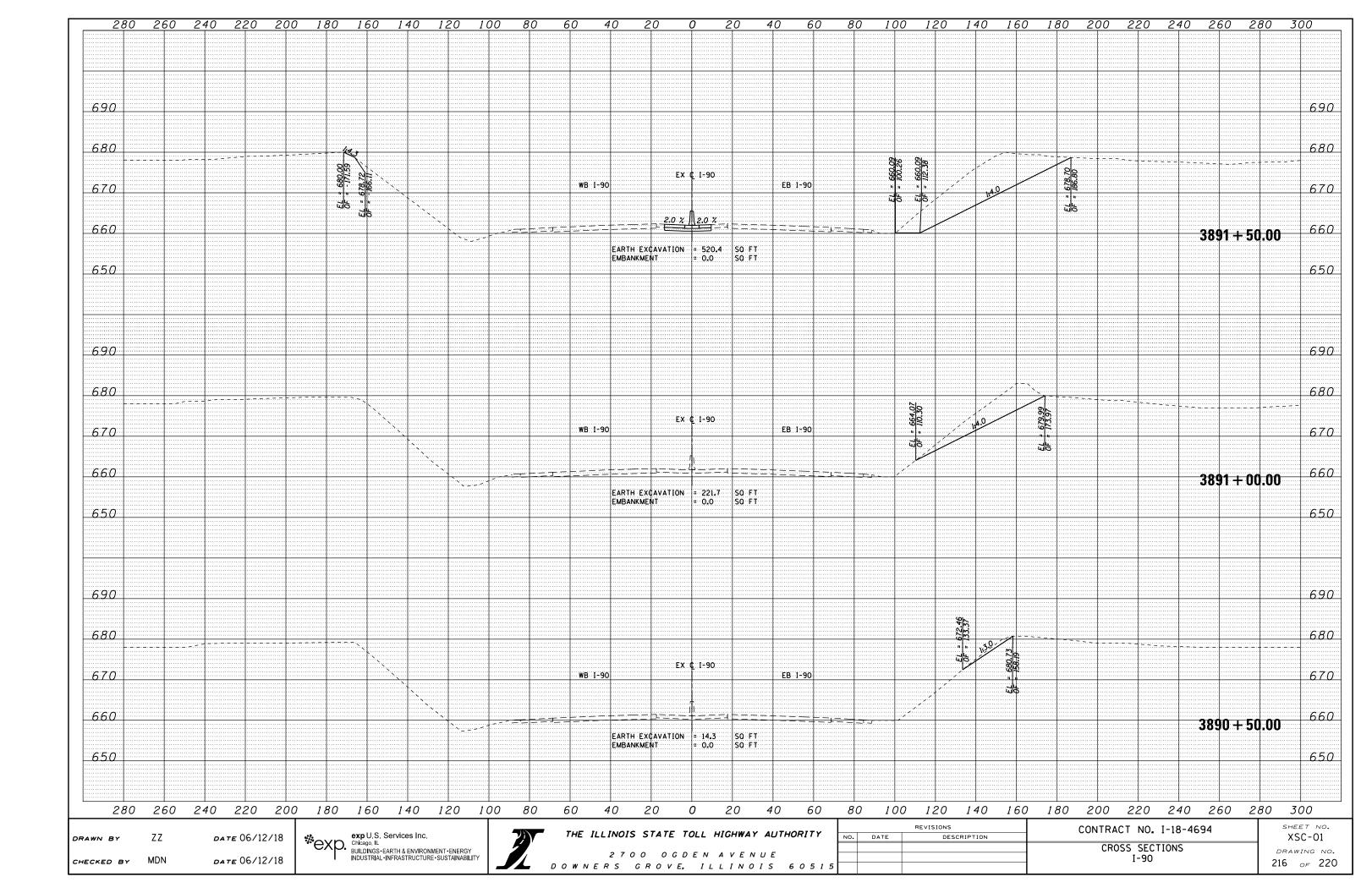
DRAWN BY AMF DATE 06/12/18 CHECKED BY BGK DATE 06/12/18

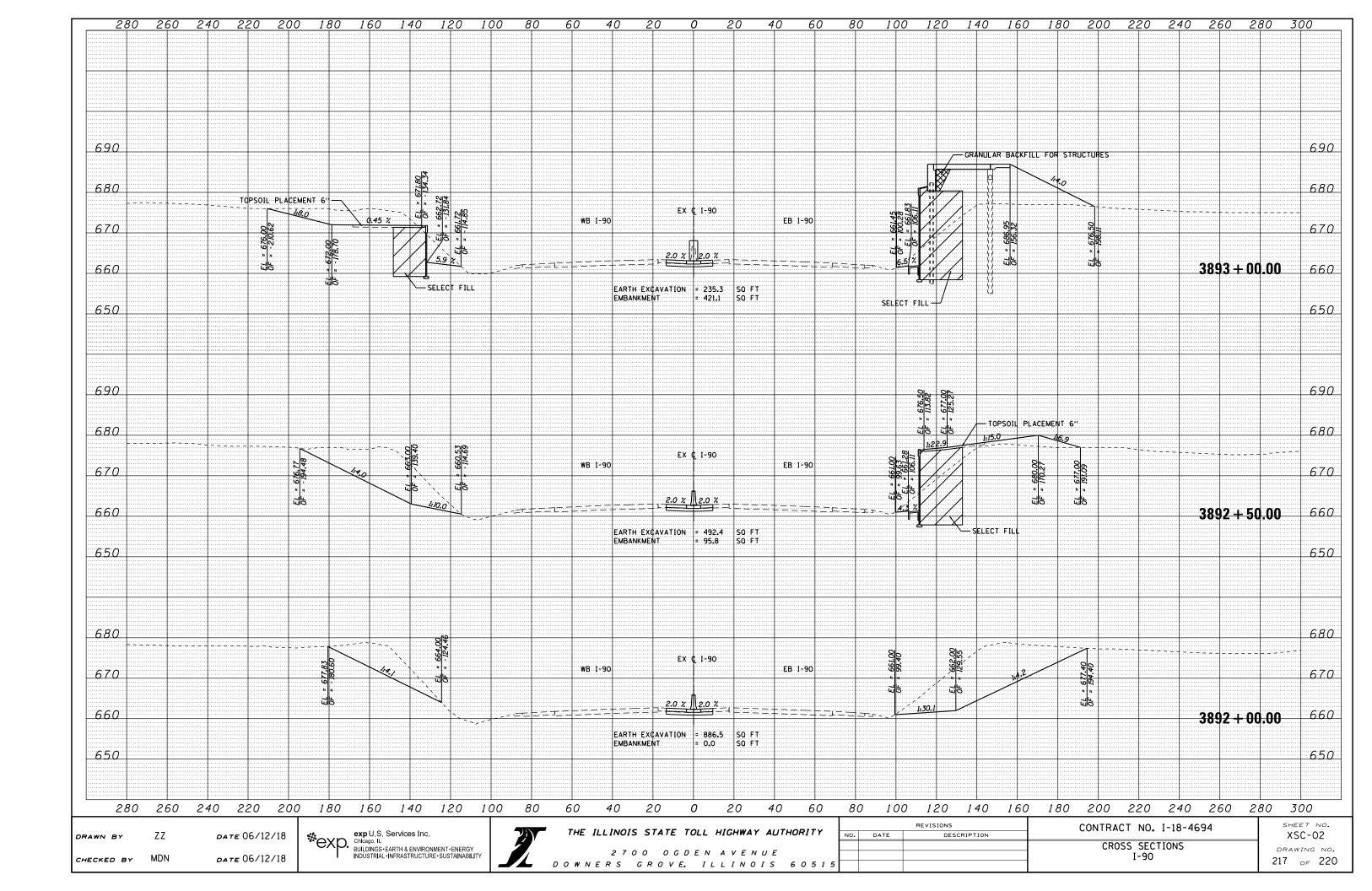
exp U.S. Services Inc. exp. Chicago, IL BUILDINGS. BUILDINGS • EARTH & ENVIRONMENT • ENERGY INDUSTRIAL • INFRASTRUCTURE • SUSTAINABILITY

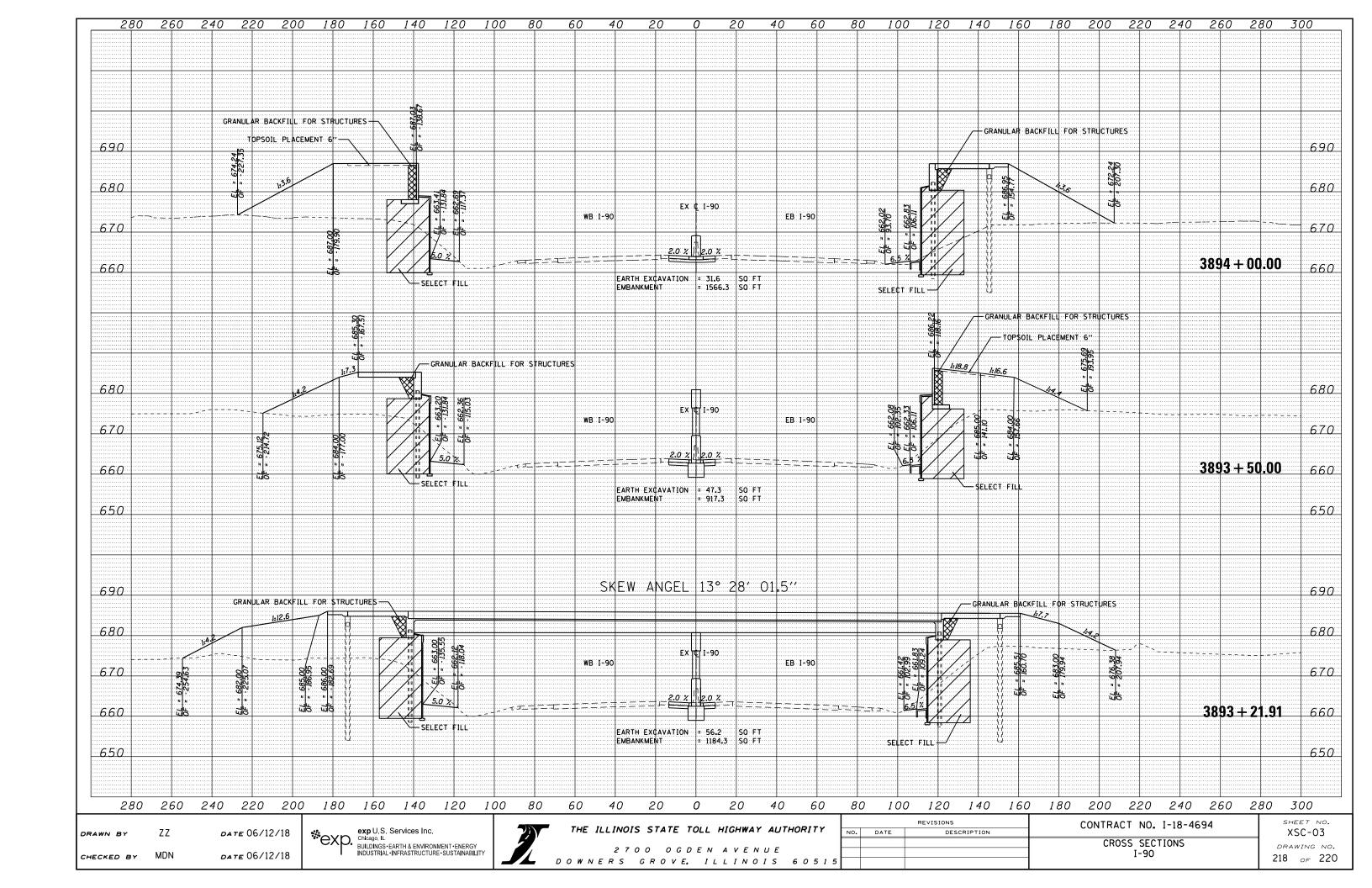


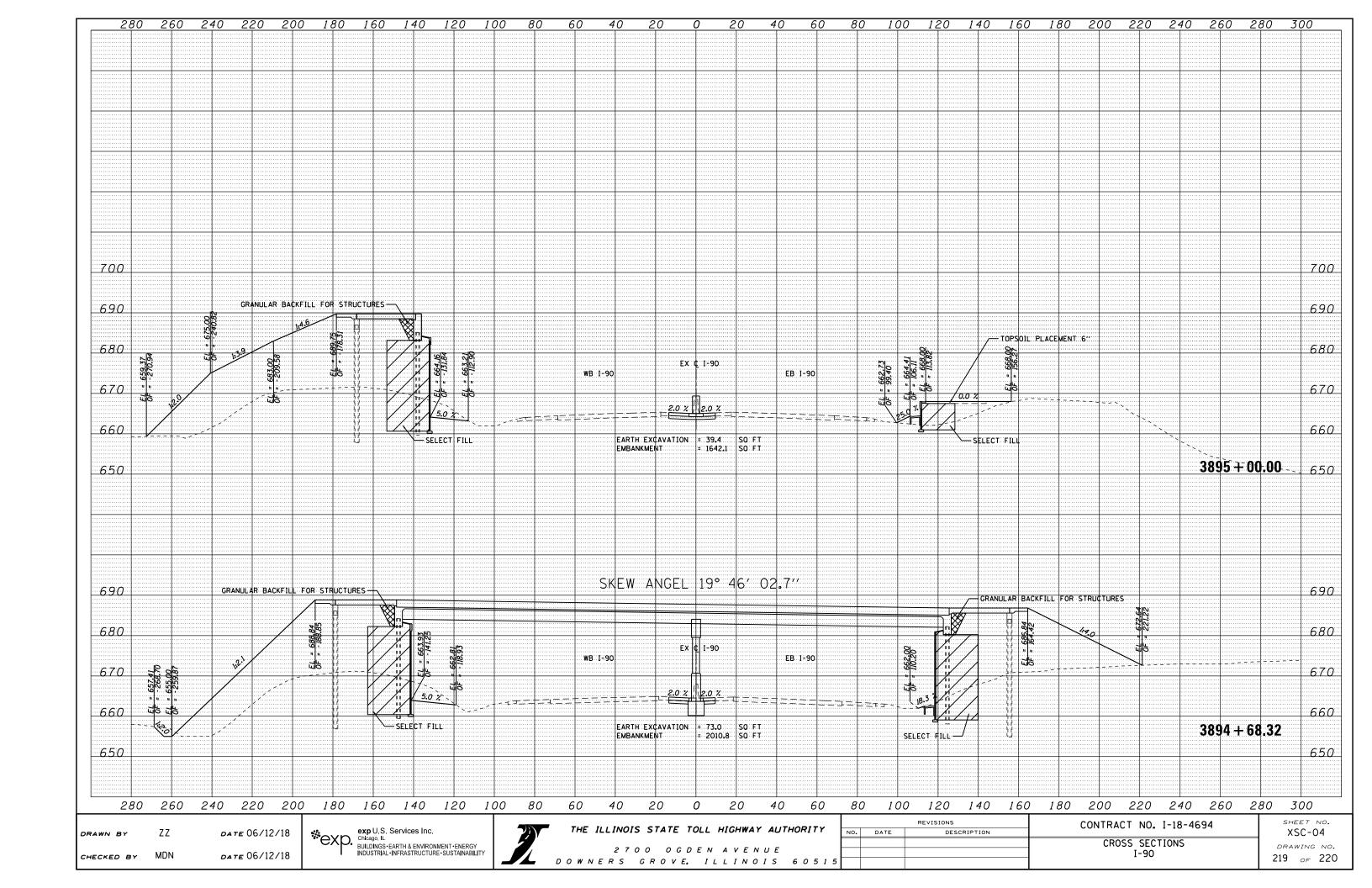
THE ILLINOIS STATE TOLL HIGHWAY AUTHORITY 2700 OGDEN AVENUE DOWNERS GROVE, ILLINOIS 60515

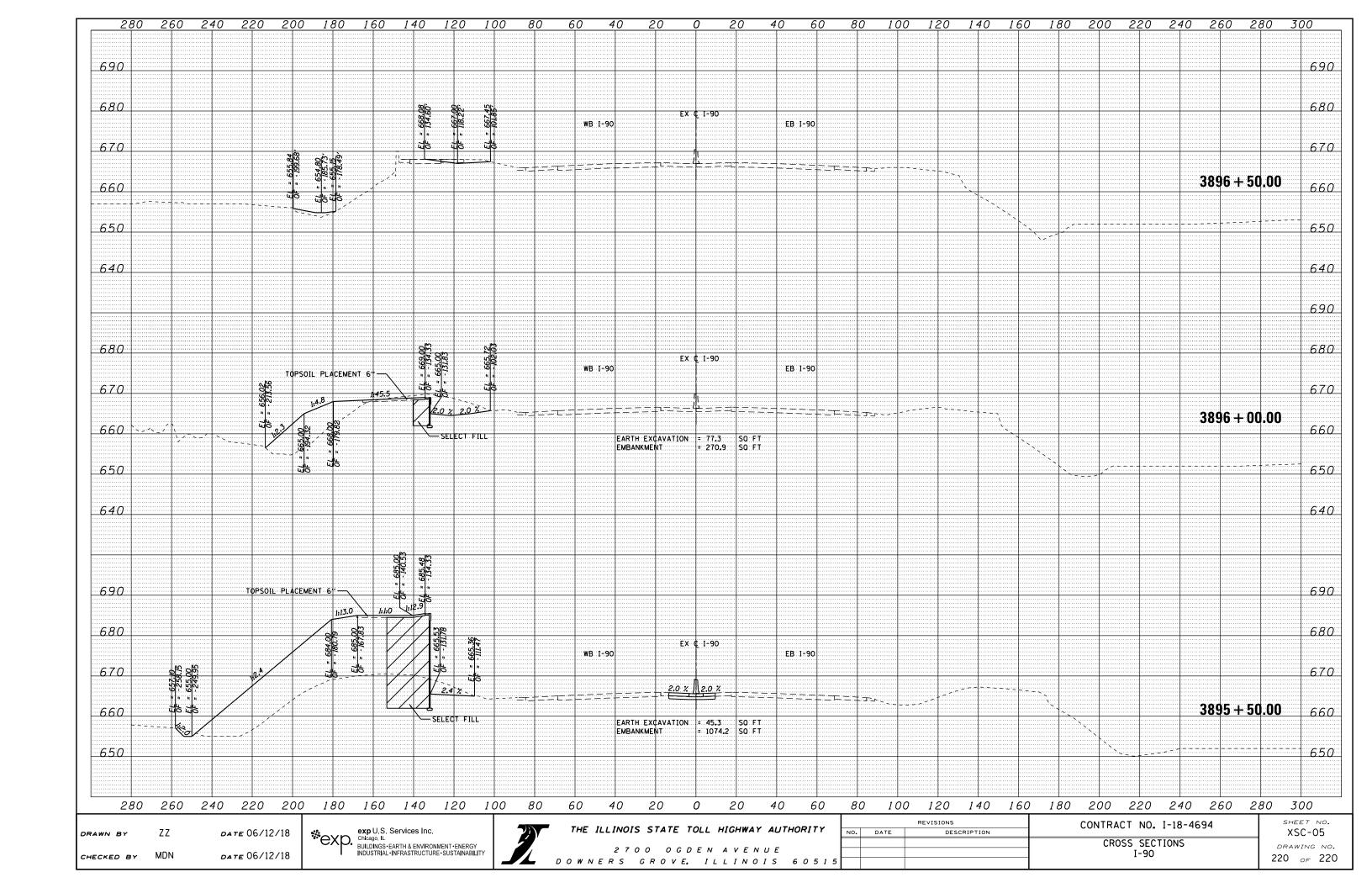
		REVISIONS	CONTRACT NO. I-18-4694	S-107
NO.	DATE	DESCRIPTION	CONTRACT NO. 1-10-4634	3-101
			BRIDGE NOS. 1681 & 1682	DRAWING NO.
				215 of 220
			BORING LOG	213 OF 220











ABV	ABOVE	CU YD	CUBIC YARD	HD	HEAD	PED	PEDESTAL	STD	STANDARD
A/C	ACCESS CONTROL	CULV	CULVERT	HDW	HEADWALL	PNT	POINT	SBI	STATE BOND ISSUE
AC	ACRE	C&G	CURB & GUTTER	HDUTY	HEAVY DUTY	PC	POINT OF CURVATURE	SR	STATE ROUTE
ADJ	ADJUST	D	DEGREE OF CURVE	ha	HECTARE	PΙ	POINT OF INTERSECTION OF HORIZONTAL	STA	STATION
AS	AERIAL SURVEYS	DC	DEPRESSED CURVE	НМА	HOT MIX ASPHALT		CURVE	SPBGR	STEEL PLATE BEAM GUARDRAIL
AGG	AGGREGATE	DET	DETECTOR	HWY	HIGHWAY	PRC	POINT OF REVERSE CURVE	SS	STORM SEWER
АН	AHEAD	DIA	DIAMETER	HORIZ	HORIZONTAL	PT	POINT OF TANGENCY	STY	STORY
APT	APARTMENT	DIST	DISTRICT	HSE	HOUSE	POT	POINT ON TANGENT	ST	STREET
ASPH	ASPHALT	DOM	DOMESTIC	IL	ILLINOIS	POLYETH	POLYETHYLENE	STR	STRUCTURE
AUX	AUXILIARY	DBL	DOUBLE	IMP	IMPROVEMENT	PCC	PORTLAND CEMENT CONCRETE	е	SUPERELEVATION RATE
AGS	AUXILIARY GAS VALVE (SERVICE)	DSEL	DOWNSTREAM ELEVATION	IN DIA	INCH DIAMETER	PP	POWER POLE OR PRINCIPAL POINT	S.E. RUN.	SUPERELEVATION RUNOFF LENGTH
AVE	AVENUE	DSFL	DOWNSTREAM FLOWLINE	INL	INLET	PRM	PRIME	SURF	SURFACE
AX	AXIS OF ROTATION	DR	DRAINAGE OR DRIVE	INST	INSTALLATION	PE	PRIVATE ENTRANCE	SMK	SURVEY MARKER
ВК	BACK	DI	DRAINAGE INLET OR DROP INLET	IDS	INTERSECTION DESIGN STUDY	PROF	PROFILE	T	TANGENT DISTANCE
В-В	BACK TO BACK	DRV	DRIVEWAY	INV	INVERT	PGL	PROFILE GRADELINE	T.R.	TANGENT RUNOUT DISTANCE
BKPL	BACKPLATE	DCT	DUCT	ΙP	IRON PIPE	PROJ	PROJECT	TEL	TELEPHONE
В	BARN	EA	EACH	IR	IRON ROD	P.C.	PROPERTY CORNER	TB	TELEPHONE BOX
BARR	BARRICADE	EB	EASTBOUND	JT	JOINT	PL	PROPERTY LINE	TP	TELEPHONE POLE
BGN	BEGIN	EOP	EDGE OF PAVEMENT	kg	KILOGRAM	PR	PROPOSED	TEMP	TEMPORARY
ВМ	BENCHMARK	E-CL	EDGE TO CENTERLINE	km	KILOMETER	R	RADIUS	TBM	TEMPORARY BENCH MARK
BIND	BINDER	E-E	EDGE TO EDGE	LS	LANDSCAPING	RR	RAILROAD	TD	TILE DRAIN
BIT	BITUMINOUS	EL	ELEVATION	LN	LANE	RRS	RAILROAD SPIKE	TBE	TO BE EXTENDED
ВТМ	BOTTOM	ENTR	ENTRANCE	LΤ	LEFT	RPS	REFERENCE POINT STAKE	TBR	TO BE REMOVED
BLVD	BOULEVARD	EXC	EXCAVATION	LP	LIGHT POLE	REF	REFLECTIVE	TBS	TO BE SAVED
BRK	BRICK	EX	EXISTING	LGT	LIGHTING	RCCP	REINFORCED CONCRETE CULVERT PIPE	TWP	TOWNSHIP
BBOX	BUFFALO BOX	EXPWAY	EXPRESSWAY	LF	LINEAL FEET OR LINEAR FEET	REINF	REINFORCEMENT	TR	TOWNSHIP ROAD
BLDG	BUILDING	E	EXTERNAL DISTANCE OF HORIZONTAL CURVE	L	LITER OR CURVE LENGTH	REM	REMOVAL	TS	TRAFFIC SIGNAL
CIP	CAST IRON PIPE	E _	OFFSET DISTANCE TO VERTICAL CURVE	LC	LONG CHORD	RC	REMOVE CROWN	TSCB	TRAFFIC SIGNAL CONTROL BOX
CB	CATCH BASIN	F-F	FACE TO FACE		LONGITUDINAL	REP	REPLACEMENT	TSC	TRAFFIC SYSTEMS CENTER
C-C	CENTER TO CENTER	FA	FEDERAL AID		LUMP SUM	REST	RESTAURANT	TRVS	TRANSVERSE
CL	CENTERLINE OR CLEARANCE	FAI FAP	FEDERAL AID INTERSTATE FEDERAL AID PRIMARY	MACH MB	MACHINE MAIL BOX	RESURF RET	RESURFACING RETAINING	TRVL TRN	TRAVEL TURN
CL-E CL-F	CENTERLINE TO EDGE CENTERLINE TO FACE	FAS	FEDERAL AID PRIMARY	MH	MANHOLE	RT	RIGHT	TY	TYPE
CTS	CENTERS	FAUS	FEDERAL AID SECONDARY	MATL	MATERIAL	ROW	RIGHT-OF-WAY	T-A	TYPE A
CERT	CERTIFIED	FP FP	FENCE POST	MED	MEDIAN	RD	ROAD	TYP	TYPICAL
CHSLD	CHISELED	FE	FIELD ENTRANCE	m MILD	METER	RDWY	ROADWAY	UNDGND	UNDERGROUND
CS	CITY STREET	FH	FIRE HYDRANT	METH	METHOD	RTE	ROUTE	USGS	U.S. GEOLOGICAL SURVEY
CP	CLAY PIPE	FL	FLOW LINE	M	MID-ORDINATE	SAN	SANITARY	USEL	UPSTREAM ELEVATION
CLSD	CLOSED	FB	FOOT BRIDGE	mm	MILLIMETER	SANS	SANITARY SEWER	USFL	UPSTREAM FLOWLINE
CLID	CLOSED LID	FDN	FOUNDATION		MILLIMETER DIAMETER	SEC	SECTION	UTIL	UTILITY
CT	COAT OR COURT	FR	FRAME	MIX	MIXTURE	SEED	SEEDING	VBOX	VALVE BOX
СОМВ	COMBINATION	F&G	FRAME & GRATE	мвн	MOBILE HOME	SHAP	SHAPING	VV	VALVE VAULT
С	COMMERCIAL BUILDING	FRWAY	FREEWAY	MOD	MODIFIED	S	SHED	VLT	VAULT
CE	COMMERCIAL ENTRANCE	GAL	GALLON	MFT	MOTOR FUEL TAX	SH	SHEET	VEH	VEHICLE
CONC	CONCRETE	GALV	GALVANIZED	N & BC	NAIL & BOTTLE CAP	SHLD	SHOULDER	VP	VENT PIPE
CONST	CONSTRUCT	G	GARAGE	N & C	NAIL & CAP	SW	SIDEWALK OR SOUTHWEST	VERT	VERTICAL
CONTD	CONTINUED	GM	GAS METER	N & W	NAIL & WASHER	SIG	SIGNAL	VC	VERTICAL CURVE
CONT	CONTINUOUS	GV	GAS VALVE	NOAA	NATIONAL OCEANIC ATMOSPHERIC	SOD	SODDING	VPC	VERTICAL POINT OF CURVATURE
COR	CORNER	GRAN	GRANULAR		ADMINISTRATION	SM	SOLID MEDIAN	VPI	VERTICAL POINT OF INTERSECTION
CORR	CORRUGATED	GR	GRATE	NC	NORMAL CROWN	SB	SOUTHBOUND	VPT	VERTICAL POINT OF TANGENCY
CMP	CORRUGATED METAL PIPE	GRVL	GRAVEL	NB	NORTHBOUND	SE	SOUTHEAST	WM	WATER METER
CNTY	COUNTY	GND	GROUND	NE	NORTHEAST	SPL	SPECIAL	WV	WATER VALVE
CH	COUNTY HIGHWAY	GUT	GUTTER	NW	NORTHWEST	SD	SPECIAL DITCH	WMAIN	WATER MAIN
CSE	COURSE	GP	GUY POLE	OLID	OPEN LID	SQ FT	SQUARE FEET	WB	WESTBOUND
XSECT	CROSS SECTION	GW	GUY WIRE	PAT	PATTERN	m 2	SQUARE METER	WILDFL	WILDFLOWERS
m <sup>3</sup>	CUBIC METER	HH	HANDHOLE	PVD	PAVENE	mm 2	SOUARE MILLIMETER	W	WITH
mm <sup>3</sup>	CUBIC MILLIMETER	HATCH	HATCHING	PVMT PM	PAVEMENT PAVEMENT MARKING	SQ YD STB	SOUARE YARD STABILIZED	WO	WITHOUT
1				ı IVI	I AVENIENT MANNING	טוט	3   ADILIZED		

Illinois Department of Transporta	tion
PASSED January 1, 2011	ISI
Michael Brand	ISSUED
ENGINEER OF POLICY AND PROCEDURES	
APPROVED January 1, 2011	1
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ENGINEER OF DESIGN AND ENVIRONMENT	

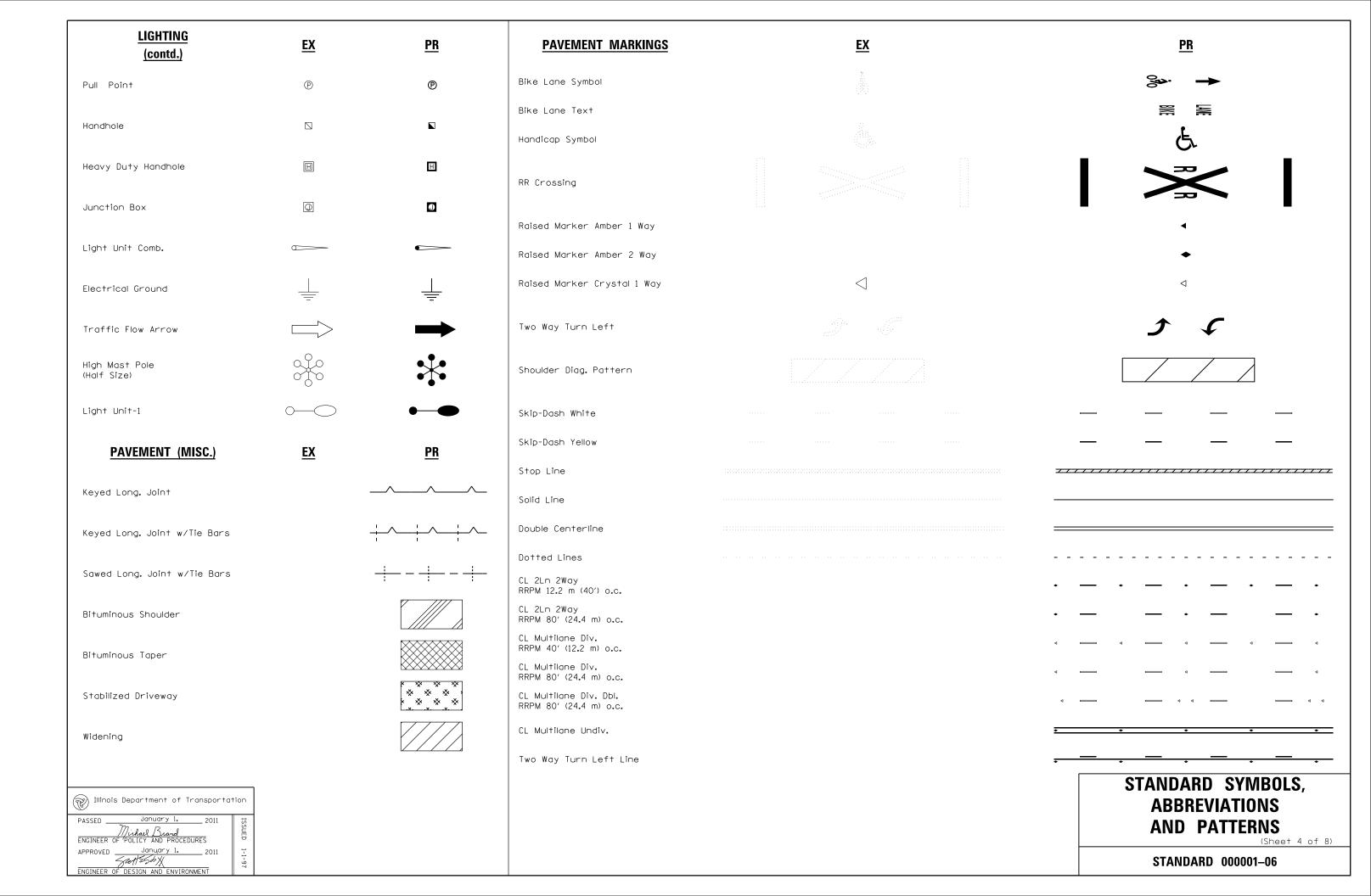
DATE	REVISIONS	
1-1-11	Updated abbreviations	
	and symbols.	
1-1-08	Updated abbreviations	
	and symbols.	
		1

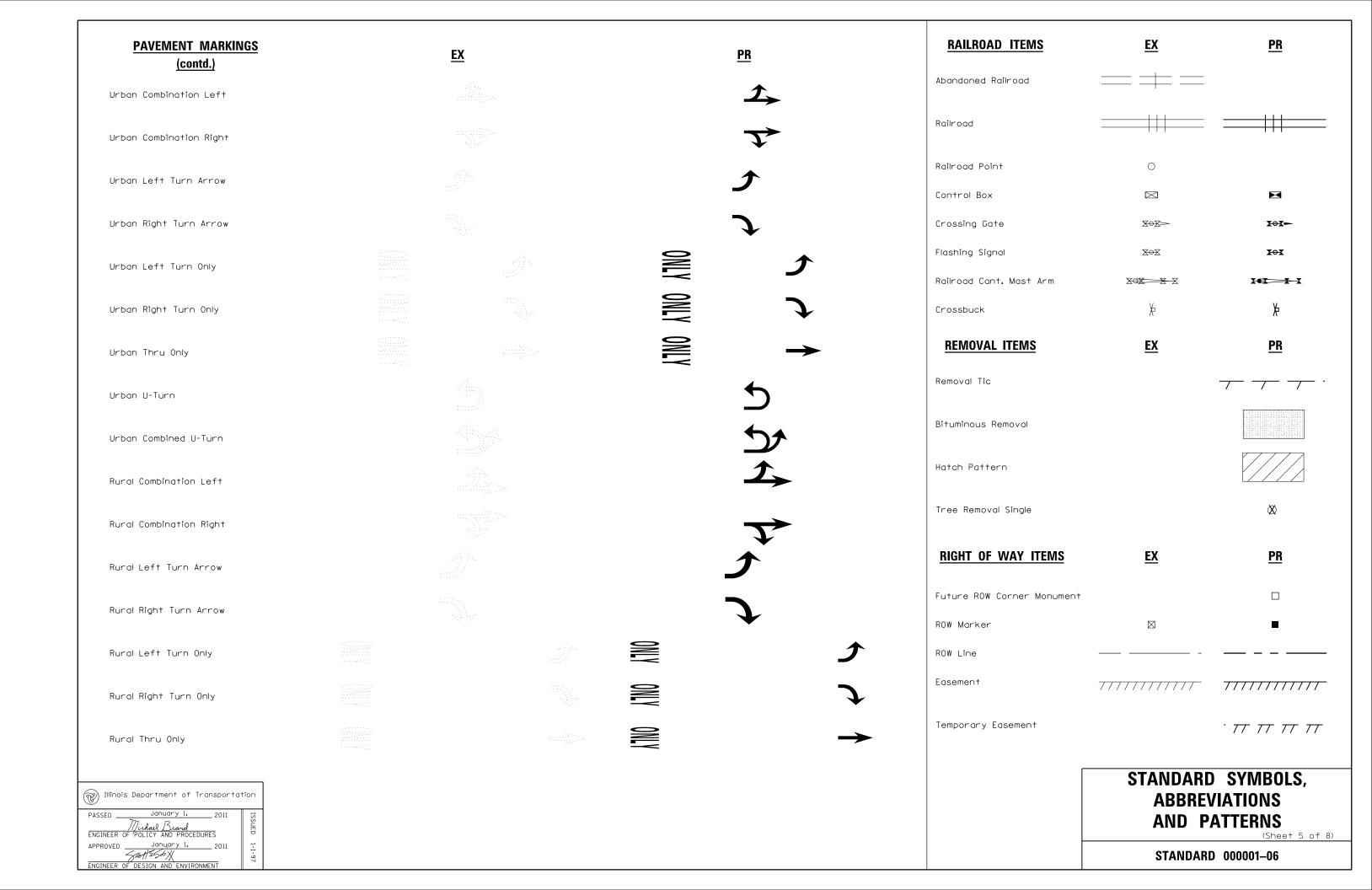
# STANDARD SYMBOLS, **ABBREVIATIONS** AND PATTERNS (Sheet 1 of 8)

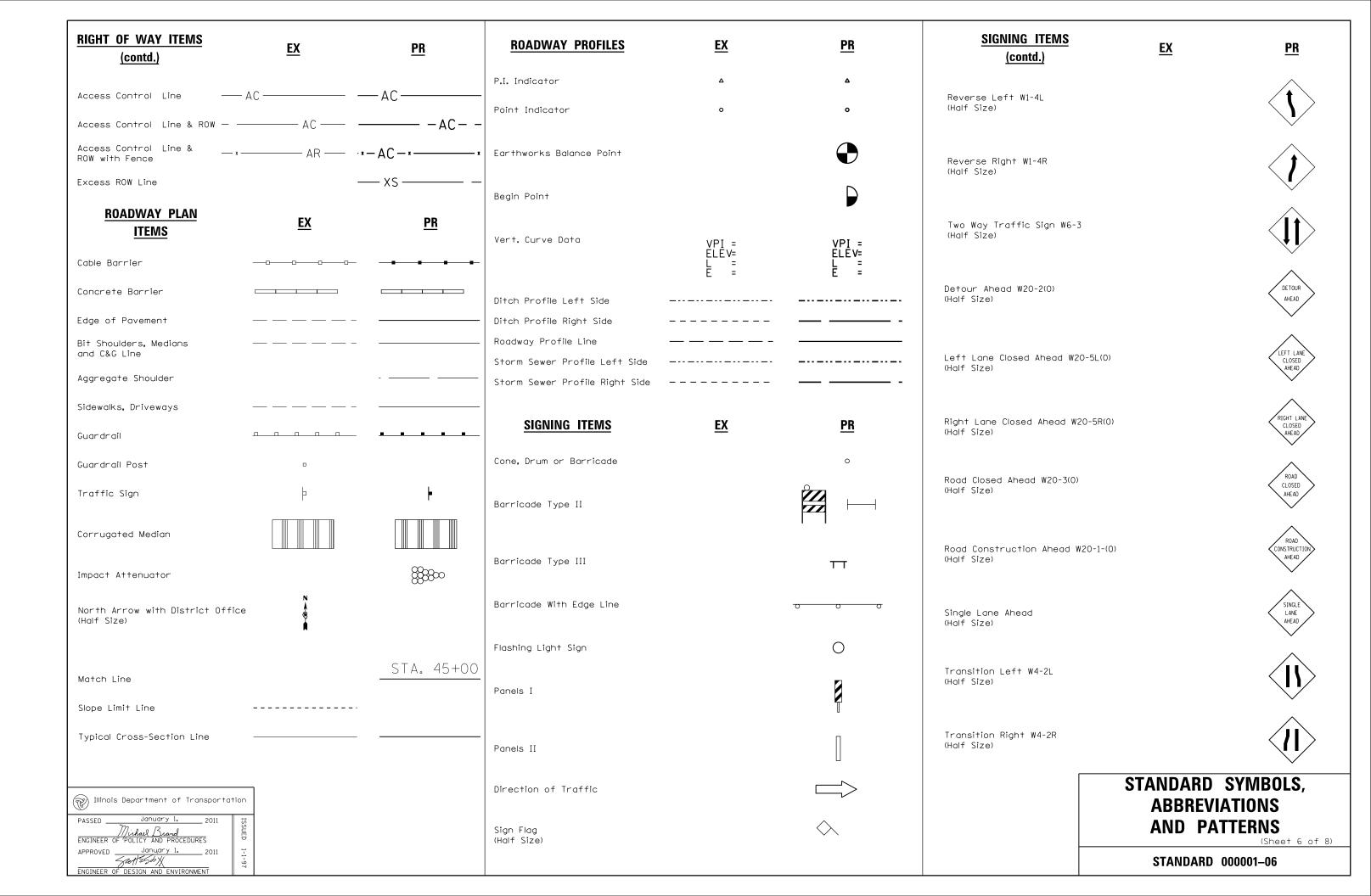
STANDARD 000001-06

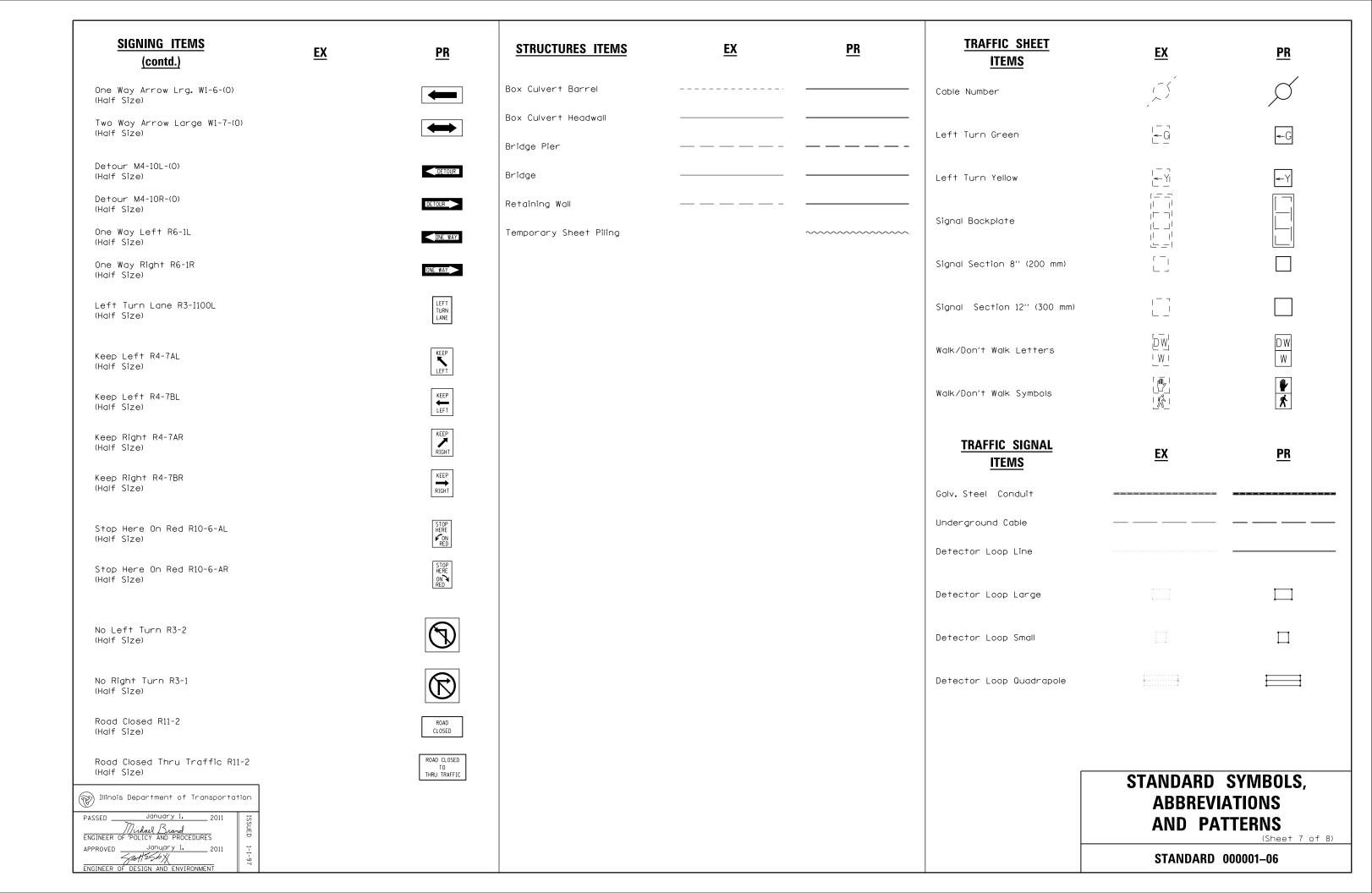
ADJUSTMENT ITEMS	<u>EX</u>	<u>PR</u>	ALIGNMENT ITEMS	<u>EX</u>	<u>PR</u>	CONTOUR ITEMS	<u>EX</u>	<u>PR</u>
Structure To Be Adjusted		ADJ	Baseline			Approx. Index Line		
			Centerline			Approx. Intermediate Line		
Structure To Be Cleaned		С	Centerline Break Circle	0	$\odot$	Index Contour		
Main Structure To Be Filled		FM	Baseline Symbol	B	B	Intermediate Contour		
			Centerline Symbol	<u>C</u>	<b></b>	DRAINAGE ITEMS	EX	PR
Structure To Be Filled		F	PI Indicator	Δ	Δ	Channel or Stream Line		<u>—</u>
Structure To Be Filled Special		FSP	Point Indicator	0	٥	Culvert Line	H	
Structure To Be Removed		R	Horizontal Curve Data (Half Size)	CURVE P.I. STA= △=	CURVE P.I. STA= △=	Grading & Shaping Ditches		
			(null Size)	D= R= T=	D= R= T=	Drainage Boundary Line		<del></del>
Structure To Be Reconstructed		REC		L= E= e= T R =	L= E= e= T.R.=	Paved Ditch	4444	A A A STATE A A A A STATE A A A A STATE A
Structure To Be Reconstructed Special		RSP		T.R.= S.E. RUN= P.C. STA= P.T. STA=	e= T.R.= S.E. RUN= P.C. STA= P.T. STA=	Aggregate Ditch	_Privility _Privility _Privility	Brown of the season
			BOUNDARIES ITEMS	<u>EX</u>	PR	Pipe Underdrain		
Frame and Grate To Be Adjusted		A	Dashed Property Line	<del></del>	<u></u>	Storm Sewer		<b></b>
Frame and Lid To Be Adjusted		A	Solid Property/Lot Line			Flowline	Æ	ŧ.
Domestic Service Box		$\wedge$	Section/Grant Line			Ditch Check		<b>—</b>
To Be Adjusted		A	Quarter Section Line			Headwall	_	
Valve Vault To Be Adjusted		A	Quarter/Quarter Section Line			Inlet		-
Special Adjustment		(SP)	County/Township Line			Manhole	0	•
Special Adjustilletti		(1)	State Line			Summit	<+->	$\longleftrightarrow$
Item To Be Abandoned		AB	Iron Pipe Found	0		Roadway Ditch Flow	<b>-</b> √>	<b>-</b> ∼>
Item To Be Moved		M	Iron Pipe Set	•		Swale		<b>→</b>
			Survey Marker			Catch Basin	0	•
Item To Be Relocated		REL	Property Line Symbol	P		Culvert End Section	⊲	•
Pavement Removal and Replacement			Same Ownership Symbol (Half Size)	7		Water Surface Indicator	<u></u>	
		<u> </u>	Northwest Quarter Corner			Riprap		00000 00000 000000
Illinois Department of Transportation  PASSED January 1. 2011   Midal Brand ENGINEER OF POLICY AND PROCEDURES			(Half Size)  Section Corner (Half Size)				STANDARD ABBREVI AND PA	ATIONS
APPROVED Jonuary 1. 2011  Fautter X  ENGINEER OF DESIGN AND ENVIRONMENT			Southeast Quarter Corner (Half Size)				STANDARD	

EROSION & SEDIMENT  CONTROL ITEMS	<u>EX</u>	<u>PR</u>	NON-HIGHWAY IMPROVEMENT ITEMS	<u>EX</u>	<u>PR</u>	EXISTING LANDSCAPING ITEMS	<u>EX</u>	<u>PR</u>
Cleaning & Grading Limits			Noise Attn./Levee			(contd.)  Seeding Class 5		
Dike			Field Line	—— E——		Second Green		
Erosion Control Fence Perimeter Erosion Barrier		~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	Fence	_ x x x x x		Seeding Class 7		
Temporary Fence		_ XXX _ XXX _ XXX _ XXX _ XXX -	Base of Levee			Seedlings Type 1		
Ditch Check Temporary		<del></del>	Mailbox	P		Seedlings Type 2		
Ditch Check Permanent		<b>—</b>	Multiple Mailboxes			Sodding		
Inlet & Pipe Protection		$\bigoplus$	Pay Telephone			Mowstake w/Sign		_•_
Sediment Basin			Advertising Sign	þ		Tree Trunk Protection		
Erosion Control Blanket		+++++	LANDSCAPING ITEMS	<u>EX</u>	<u>PR</u>	Evergreen Tree	=(E)	A
Fabric Formed Concrete Revetment Mat			Contour Mounding Line				$\mathcal{H}$	4
Turf Reinforcement Mat			Fence Fence Post		— x — x — x — x —	Shade Tree	E	+
Mulch Temporary			Shrubs Mowline			<u>LIGHTING</u>	<u>EX</u>	<u>PR</u>
Mulch Method 1		+ X + X + , -	Perennial Plants			Duc†		
Mulch Method 2 Stabilized		本本本本本本 本	Seeding Class 2			Conduit  Electrical Aerial Cable	A	A
Mulch Method 3 Hydraulic		444	Seeding Class 2A			Electrical Buried Cable	L	L
			Seeding Class 4			Controller	$\boxtimes$	<b>X</b>
						Underpass Luminaire Power Pole	-0-	<b>□</b>
PASSED January 1, 2011 IS Minday Brand ENGINEER OF POLICY AND PROCEDURES			Seeding Class 4 & 5 Combined				STANDARD ABBREV	SYMBOLS, IATIONS ATTERNS (Sheet 3 of 8)
APPROVED  January 1, 2011  January 1, 2011  Findinger of Design and Environment							STANDARD	000001–06



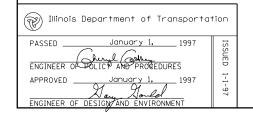






TRAFFIC SIGNAL ITEMS (contd.)	<u>EX</u>	<u>PR</u>	UNDERGROUND UTILITY ITEMS	<u>PR</u>	<u>ABANDONED</u>	UTILITY ITEMS (contd.)	<u>EX</u>	<u>PR</u>
Detector Raceway	"E"		Cable TV ———————————————————————————————————	стv	CTV	Traffic Signal	<b>;</b>	•
			Electric Cable ————————————————————————————————————	— — E—	<del>-</del> -/E/-	Traffic Signal Control Box	<b>]</b> *S]	
Aluminum Mast Arm	0		Fiber Optic — FO —	— F0 —	<del>-</del> -/ F0/	Water Meter	T	
Steel Mast Arm	0	•——	Gas Pipe ————————————————————————————————————	— G —	<del>-</del> -/   G	Water Meter Valve Box	0	•
			0il Pipe ————————————————————————————————————	— — o —	<del>-</del> -/	Profile Line		
Veh. Detector Magnetic		-	Sanitary Sewer ->>	->- <b>-&gt;&gt;-</b>	<b>&gt;-</b> /->/->/->/	Aerial Power Line	— А — — — А	— А ———
Conduit Splice	•	•	Telephone Cable ————————————————————————————————————	— — T—	<del>_</del>	VEGETATION ITEMS	EV	PR
Controller		$\blacksquare$	Water Pipe ────────────────────────────────────	— W —	— / W I W I	VEGETATION TIEMS	<u>EX</u>	<u>rn</u>
Gulfbox Junction	0	0				Deciduous Tree	©	
Wood Pole	8	•	<u>UTILITIES ITEMS</u>	<u>EX</u>	<u>PR</u>	Bush or Shrub	Q	
Temp. Signal Head		<b></b> }≫-	Controller	$\boxtimes$	×	Evergreen Tree	<b>©</b>	
Handhole		N	Double Handhole			Stump	風	
Double Handhole			Fire Hydrant	Ø	•	Orchard/Nursery Line		
Heavy Duty Handhole	H	н	GuyWire or Deadman Anchor	$\rightarrow$		Vegetation Line		
Junction Box	<b>O</b>	0	Handhole			Woods & Bush Line		
Ped. Pushbutton Detector	<b>©</b>	<b>©</b>	Heavy Duty Handhole		⊞	<u>WATER FEATURE</u> ITEMS	<u>EX</u>	<u>PR</u>
Ped. Signal Head	-0	4	Junction Box		0			
Power Pole Service	-0-	-	Light Pole	¤	*	Stream or Drainage Ditch		
Priority Veh. Detector	<b>«</b> <	<b>~</b>	Manhole	©	<b>⊙</b>	Waters Edge	<u></u>	
Signal Head	>	-	Pipeline Warning Sign	Ь		Water Surface Indicator		
Signal Head w/Backplate	+€>	+►	Power Pole	-0-	-	Water Point	<ul><li>○</li></ul>	
Signal Post	0	•	Power Pole with Light	ф———		Disappearing Ditch	<b>&lt;</b>	
Closed Circuit TV	[C]a	(C)(	Sanitary Sewer Cleanout	©		Marsh	بىللىر	
Video Detector System	(V)1	<u> </u>	Splice Box Above Ground		_	Marsh/Swamp Boundary		
		-	Telephone Splice Box	<b>⊞</b>	-		STANDARD SYN	/IROLS
PASSED January 1. 2011  Michael Brand ENGINEER OF POLICY AND PROCEDURES			Above Ground Telephone Pole	-0-	-◆-		ABBREVIATIO AND PATTER	NS RNS
ENGINEER OF POLICY AND PROCEDURES  APPROVED  January 1. 2011  Fatt Shill							STANDARD 00000	(Sheet 8 of 8)

									DECIMAL	OF AN	INC	Н А	ND OF A FOO	)T						
	А	В			А	В			А	В			A B			А	В		А	
1/64	0.0052 0.0104 0.015625 0.0208	1/16 1/8 3/16 1/4	3/1		0.171875 0.1771 0.1823 0.1875	2½8 2½8 2¾6 2½4		11/32	0.3385 0.34375 0.3490 0.3542	4 <sup>1</sup> / <sub>16</sub> 4 <sup>1</sup> / <sub>8</sub> 4 <sup>3</sup> / <sub>16</sub> 4 <sup>1</sup> / <sub>4</sub>		33/64	0.5052 0.5104 0.515625 0.5208	6 <sup>1</sup> / <sub>16</sub> 6 <sup>1</sup> / <sub>8</sub> 6 <sup>3</sup> / <sub>16</sub> 6 <sup>1</sup> / <sub>4</sub>	43%4 11/ <sub>16</sub>	0.671875 0.6771 0.6823 0.6875	8/ <sub>16</sub> 8/ <sub>8</sub> 8 <sup>3</sup> / <sub>16</sub> 8 <sup>1</sup> / <sub>4</sub>	27/32	0.8385 0.84375 0.8490 0.8542	10½6 10½8 10¾6 10½4
1/32	0.0260 0.03125 0.0365 0.0417	5/16 3/8 7/16 1/2	13/6	64	0.1927 0.1979 0.203125 0.2083	2 <sup>5</sup> / <sub>16</sub> 2 <sup>3</sup> / <sub>8</sub> 2 <sup>7</sup> / <sub>16</sub> 2 <sup>1</sup> / <sub>2</sub>		<sup>23</sup> / <sub>64</sub>	0.359375 0.3646 0.3698 0.3750	45/16 43/8 47/16 41/2		17/32	0.5260 0.53125 0.5365 0.5417	65/ <sub>16</sub> 63/ <sub>8</sub> 67/ <sub>16</sub> 61/ <sub>2</sub>	45%4	0.6927 0.6979 0.703125 0.7083	8 <sup>5</sup> / <sub>16</sub> 8 <sup>3</sup> / <sub>8</sub> 8 <sup>7</sup> / <sub>16</sub> 8 <sup>1</sup> / <sub>2</sub>	55/64 7/8	0.859375 0.8646 0.8698 0.8750	105/6 103/8 107/6 101/2
3/64 1/16	0.0521 0.0573	9/16 5/8 11/16 3/4	<i>7</i> /3	32	0.2135 0.21875 0.2240 0.2292	2 <sup>9</sup> / <sub>16</sub> 2 <sup>5</sup> / <sub>8</sub> 2 <sup>11</sup> / <sub>16</sub> 2 <sup>3</sup> / <sub>4</sub>		<sup>25</sup> ⁄64	0.3802 0.3854 0.390625 0.3958	4% 45/8 41/16 43/4		35%4 9/16	0.546875 0.5521 0.5573 0.5625	6% 65/8 6"/16 63/4	23/32	0.7135 0.71875 0.7240 0.7292	8% 85% 811/16 83/4	57/64	0.8802 0.8854 0.890625 0.8958	10%6 105/8 101/6 103/4
5/64	0.0677 0.0729 0.078125 0.0833	13/ <sub>16</sub> 7/ <sub>8</sub> 15/ <sub>16</sub> 1	15/		0.234375 0.2396 0.2448 0.2500	2 <sup>13</sup> / <sub>16</sub> 2 <sup>7</sup> / <sub>8</sub> 2 <sup>15</sup> / <sub>16</sub> 3	1	13/32	0.4010 0.40625 0.4115 0.4167	4 <sup>13</sup> / <sub>16</sub> 4 <sup>7</sup> / <sub>8</sub> 4 <sup>15</sup> / <sub>16</sub> 5		37/64	0.5677 0.5729 0.578125 0.5833	6 <sup>13</sup> / <sub>16</sub> 6 <sup>7</sup> / <sub>8</sub> 6 <sup>15</sup> / <sub>16</sub> 7	41/64 3/4	0.734375 0.7396 0.7448 0.7500	8 <sup>13</sup> / <sub>16</sub> 8 <sup>7</sup> / <sub>8</sub> 8 <sup>15</sup> / <sub>16</sub> 9	29/32	0.9010 0.90625 0.9115 0.9167	10 <sup>13</sup> / <sub>16</sub> 10 <sup>7</sup> / <sub>8</sub> 10 <sup>15</sup> / <sub>16</sub> 11
3/32	0.0885 0.09375 0.0990 0.1042	1½6 1½8 1¾6 1¼	17/6	64	0.2552 0.2604 0.265625 0.2708	31/16 31/8 33/16 31/4		<sup>27</sup> /64	0.421875 0.4271 0.4323 0.4375	5 <sup>1</sup> / <sub>16</sub> 5 <sup>1</sup> / <sub>8</sub> 5 <sup>3</sup> / <sub>16</sub> 5 <sup>1</sup> / <sub>4</sub>		19/32	0.5885 0.59375 0.5990 0.6042	7½6 7½8 7¾6 7½4	49%4	0.7552 0.7604 0.765625 0.7708	9½6 9½8 9¾6 9½4	5%4 15%6	0.921875 0.9271 0.9323 0.9375	11½6 11½8 11¾6 11½
7/64 1/8	0.1146 0.1198	15/16 13/8 17/16 11/2	9/3	32	0.2760 0.28125 0.2865 0.2917	35/16 33/8 31/16 31/2		<sup>29</sup> ⁄64	0.4427 0.4479 0.453125 0.4583	55/16 53/8 57/16 51/2		<sup>3</sup> % <sub>4</sub>	0.609375 0.6146 0.6198 0.6250	7 <sup>5</sup> / <sub>16</sub> 7 <sup>3</sup> / <sub>8</sub> 7 <sup>1</sup> / <sub>16</sub> 7 <sup>1</sup> / <sub>2</sub>	25/32	0.7760 0.78125 0.7865 0.7917	9 <sup>5</sup> / <sub>16</sub> 9 <sup>3</sup> / <sub>8</sub> 9 <sup>7</sup> / <sub>16</sub> 9 <sup>1</sup> / <sub>2</sub>	61/64	0.9427 0.9479 0.953125 0.9583	11 <sup>5</sup> / <sub>16</sub> 11 <sup>3</sup> / <sub>8</sub> 11 <sup>1</sup> / <sub>16</sub> 11 <sup>1</sup> / <sub>2</sub>
9/64	0.1302 0.1354 0.140625 0.1458	1%6 15/8 11/6 13/4	19/ <sub>0</sub>		0.296875 0.3021 0.3073 0.3125	3 <sup>9</sup> / <sub>16</sub> 3 <sup>5</sup> / <sub>8</sub> 3 <sup>11</sup> / <sub>16</sub> 3 <sup>3</sup> / <sub>4</sub>	I	15/32	0.4635 0.46875 0.4740 0.4792	5% 5% 51/16 53/4		41/64	0.6302 0.6354 0.640625 0.6458	7%6 7%8 711/16 73/4	51/ <sub>64</sub>	0.796875 0.8021 0.8073 0.8125	9%6 95%8 91%6 93/4	31/32	0.9635 0.96875 0.9740 0.9792	11 <sup>9</sup> / <sub>16</sub> 11 <sup>5</sup> / <sub>8</sub> 11 <sup>11</sup> / <sub>16</sub> 11 <sup>3</sup> / <sub>4</sub>
5/32	0.1510 0.15625 0.1615 0.1667	1 <sup>13</sup> / <sub>16</sub> 1 <sup>7</sup> / <sub>8</sub> 1 <sup>15</sup> / <sub>16</sub> 2	21/4	64	0.3177 0.3229 0.328125 0.3333	3 <sup>13</sup> / <sub>16</sub> 3 <sup>7</sup> / <sub>8</sub> 3 <sup>15</sup> / <sub>16</sub> 4		<sup>31</sup> /64	0.484375 0.4896 0.4948 0.5000	5 <sup>13</sup> / <sub>16</sub> 5 <sup>7</sup> / <sub>8</sub> 5 <sup>15</sup> / <sub>16</sub> 6		21/32	0.6510 0.65625 0.6615 0.6667	7 <sup>13</sup> / <sub>16</sub> 7 <sup>7</sup> / <sub>8</sub> 7 <sup>15</sup> / <sub>16</sub> 8	53/64	0.8177 0.8229 0.828125 0.8333	9 <sup>13</sup> / <sub>16</sub> 9 <sup>7</sup> / <sub>8</sub> 9 <sup>15</sup> / <sub>16</sub> 10	63% <sub>4</sub>	0.984375 0.9896 0.9948 1.0000	11 <sup>13</sup> / <sub>6</sub> 11 <sup>7</sup> / <sub>8</sub> 11 <sup>15</sup> / <sub>6</sub> 12



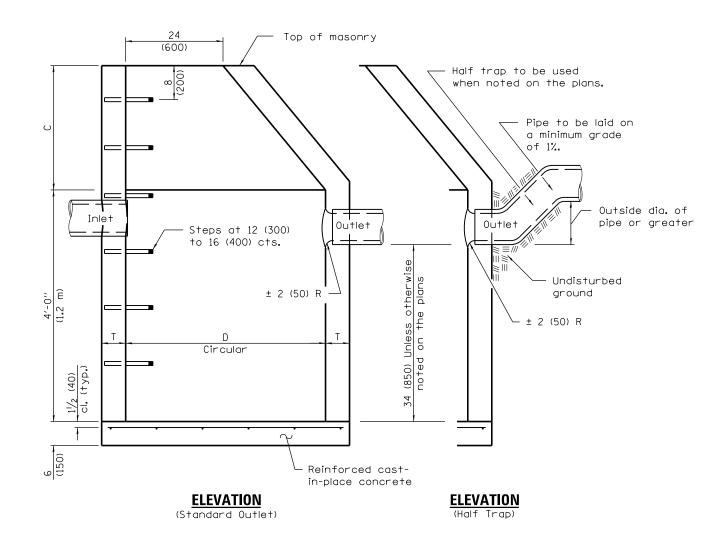
A = Fractions of Inch or Foot

B = Inch Equivalents to Foot Fractions

DATE	REVISIONS	
1-1-97	New Standard.	

DECIMAL OF AN INCH AND OF A FOOT

STANDARD 001006



Illinois Department of Transportation

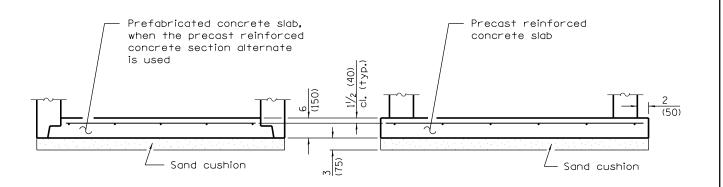
January 1,

ENGINEER OF DESIGN AND ENVIRONMENT

January 1,

PASSED

APPROVED



#### **ALTERNATE BOTTOM SLAB**

ALTERNATE MATERIALS FOR WALLS	D	C*	T (min.)
Concrete Masonry Unit	4'-0'' (1.2 m)	30 (750)	5 (125)
	5'-0'' (1.5 m)	3'-9'' (1.15 m)	5 (125)
Brick Masonry	4'-0'' (1.2 m)	30 (750)	8 (200)
	5'-0'' (1.5 m)	3'-9'' (1.15 m)	8 (200)
Precast Reinforced	4'-0'' (1.2 m)	30 (750)	4 (100)
Concrete Section	5'-0'' (1.5 m)	3'-9'' (1.15 m)	5 (125)
Cast-in-place Concrete	4'-0'' (1.2 m)	30 (750)	6 (150)
	5'-0'' (1.5 m)	3'-9'' (1.15 m)	6 (150)

• For precast reinforced concrete sections, dimension "C" may vary from the dimension given to plus 6 (150).

### **GENERAL NOTES**

Bottom slabs shall be reinforced with a minimum of 0.20 sq. in./ft (420 sq. mm/m) in both directions with a maximum spacing of 12 (300).

Bottom slabs may be connected to the riser as determined by the fabricator; however, only a single row of reinforcement around the perimeter may be utilized.

See Standard 602601 for optional precast reinforced concrete flat slab top.

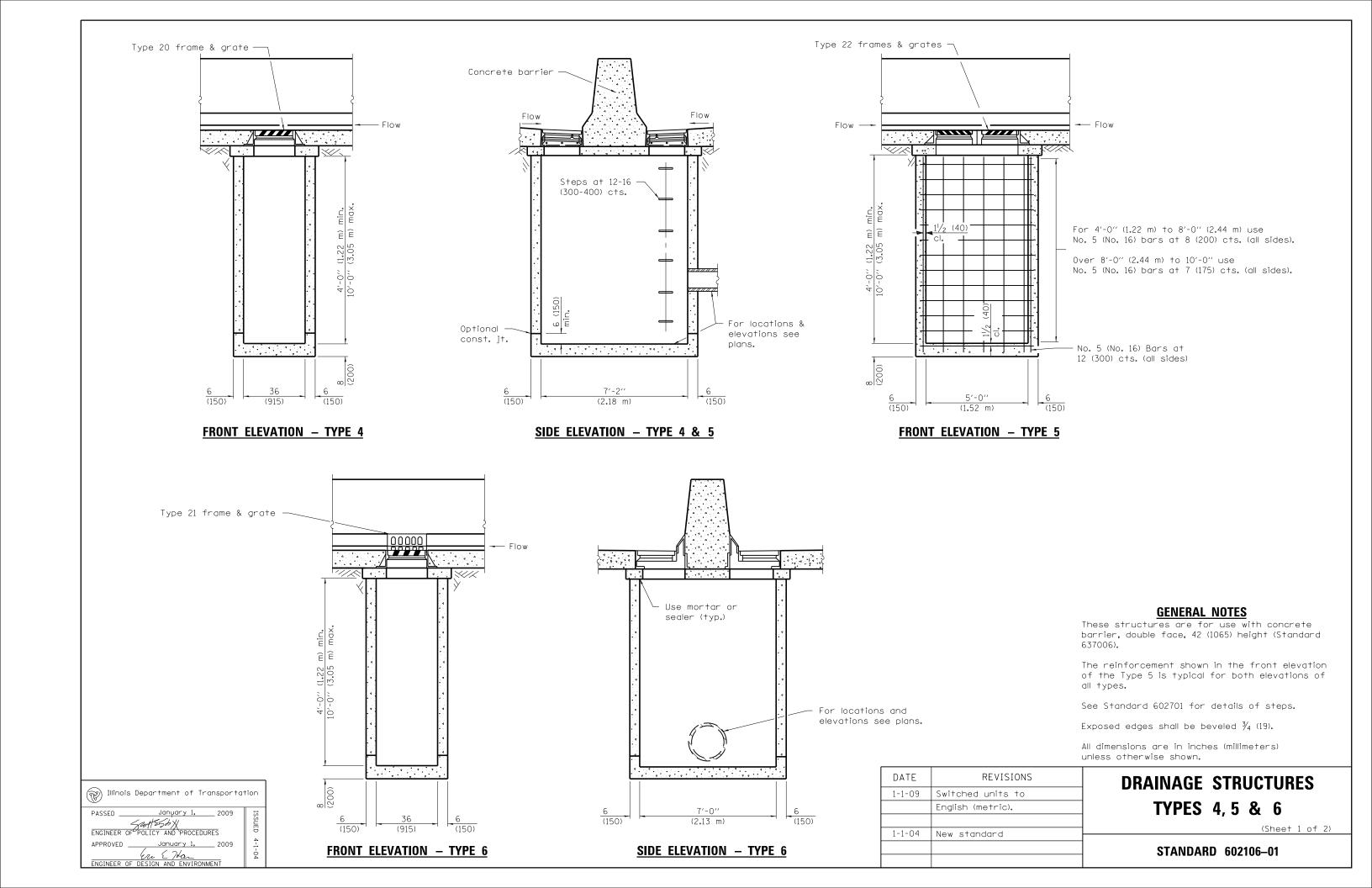
See Standard 602701 for details of steps.

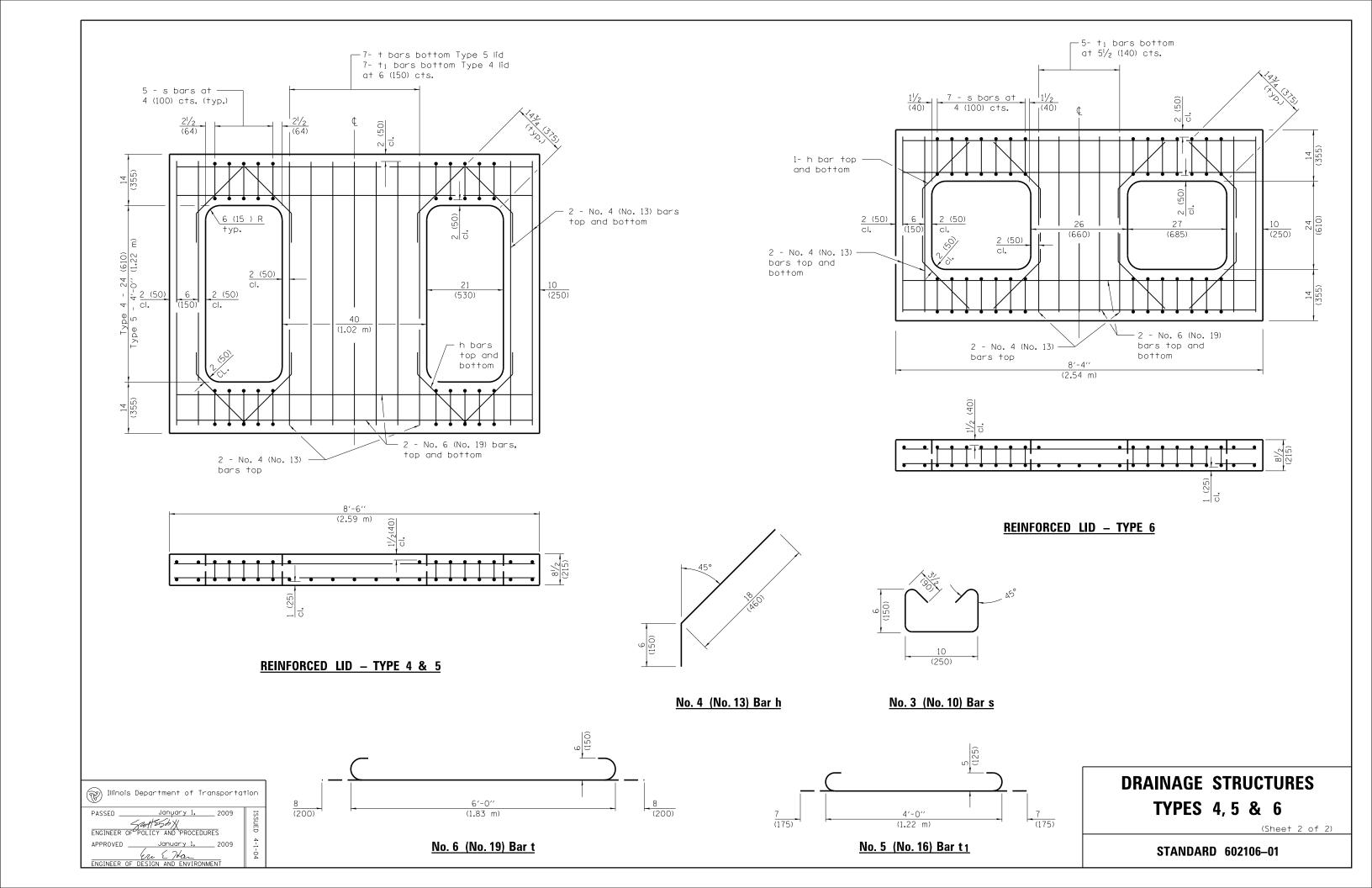
All dimensions are in inches (millimeters) unless otherwise shown.

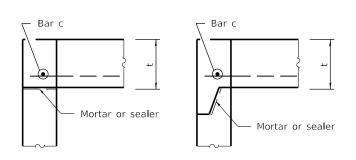
DATE	REVISIONS
1-1-11	Added 'Outside' to half trap
	note. Detail rein. in slabs.
	Revised general notes.
1-1-09	Switched units to
	English (metric).

# CATCH BASIN TYPE A

**STANDARD 602001–02** 

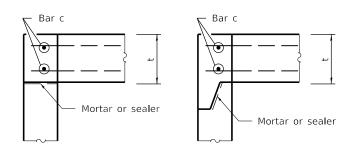






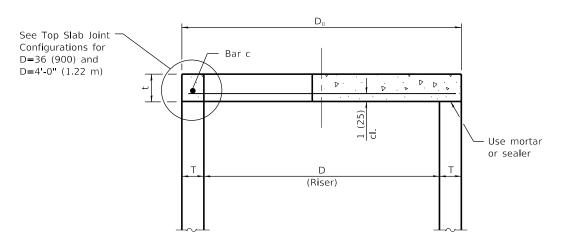
# TOP SLAB JOINT CONFIGURATIONS FOR D = 36 (900) AND D = 4'-0" (1.22 m)

(Shown at access hole)

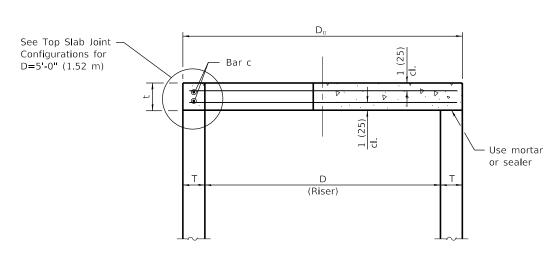


# TOP SLAB JOINT CONFIGURATIONS D = 5'-0" (1.52 m)

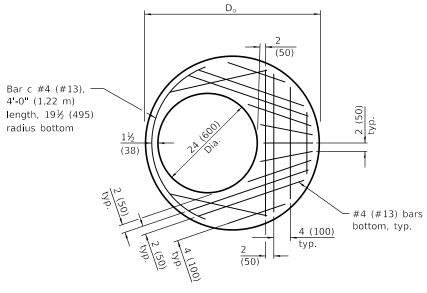
(Shown at access hole)



# TOP SECTION THRU INLET OR CATCH BASIN FOR D = 36 (900) AND D = 4'-0" (1.22 m)

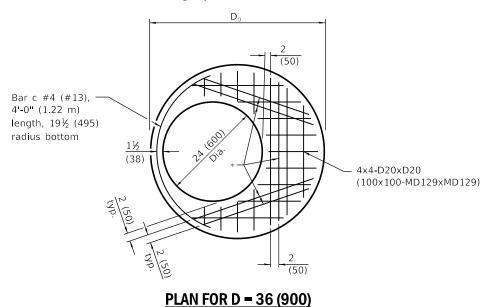


# $\frac{\text{TOP SECTION THRU CATCH BASIN}}{\text{FOR D = 5'-0" (1.52 m)}}$



## PLAN FOR D = 36(900)

(Showing Layout of Reinforcement Bars)



(Showing Layout of Welded Wire Reinforcement)

\* #4 (#13) bars bottom. Bundle first bar with WWR bar closest to the opening.

#### **GENERAL NOTES**

The flat slab top may be used in lieu of the tapered tops shown on Standards 602001, 602016, or 602306 at the option of the Contractor or when field conditions prohibit the use of tapered tops.

Lifting holes shall be located in the sections as per the manufacturer's recommendations and grouted prior to backfilling.

All dimensions are in inches (millimeters) unless otherwise shown.

		ui
DATE	REVISIONS	
1-1-18	Revised for compliance with	1
	LRFD.	]
		]
4-1-16	Changed terminology to	├
	'welded wire reinforcement'.	]
		1

PRECAST REINFORCED
CONCRETE FLAT SLAB TOP
(Sheet 1 of 2)

STANDARD 602601-05

# **TABLE**

D	Т	D₀ (min.)	t		
36 (900)	able ds	.	6 (150)		
4'-0" (1.2 m)	l l se applicable Standards	D + 2T	6 (150)		
5'-0" (1.5 m)	See Sta		8 (200)		

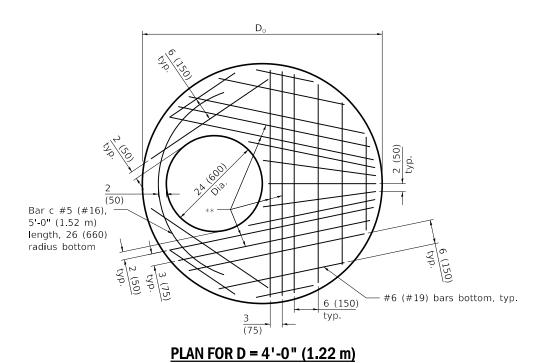
Illinois Department of Transportation

PASSED January 1. 2018

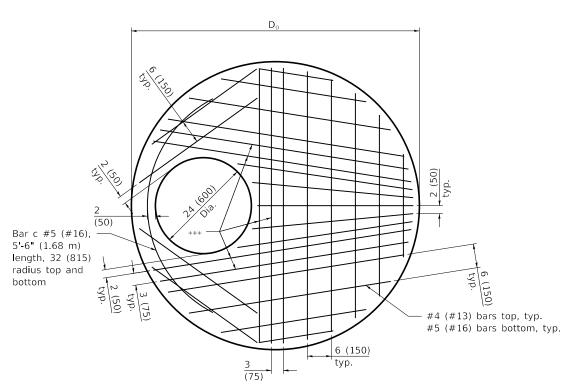
Michael Brand
ENGINEER OF POLICY AND PROCEDURES

APPROVED January 1. 2018

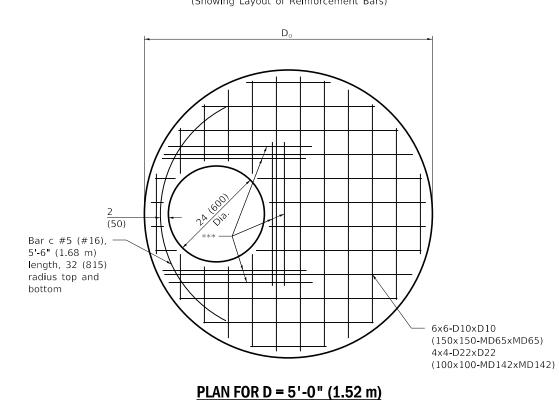
Mauen 2 2018
ENGINEER OF DESIGN AND ENVIRONMENT



(Showing Layout of Reinforcement Bars)



# PLAN FOR D = 5'-0" (1.52 m) (Showing Layout of Reinforcement Bars)



# PLAN FOR D = 4'-0" (1.22 m)

(100x100-MD194xMD194) bottom

(50)

Bar c #5 (#16),

length, 26 (660)

5'-0" (1.52 m)

radius bottom

(Showing Layout of Welded Wire Reinforcement)

\*\* #5 (#16) bars bottom. For WWR, bundlefirst bar with WWR bar closest to the opening.

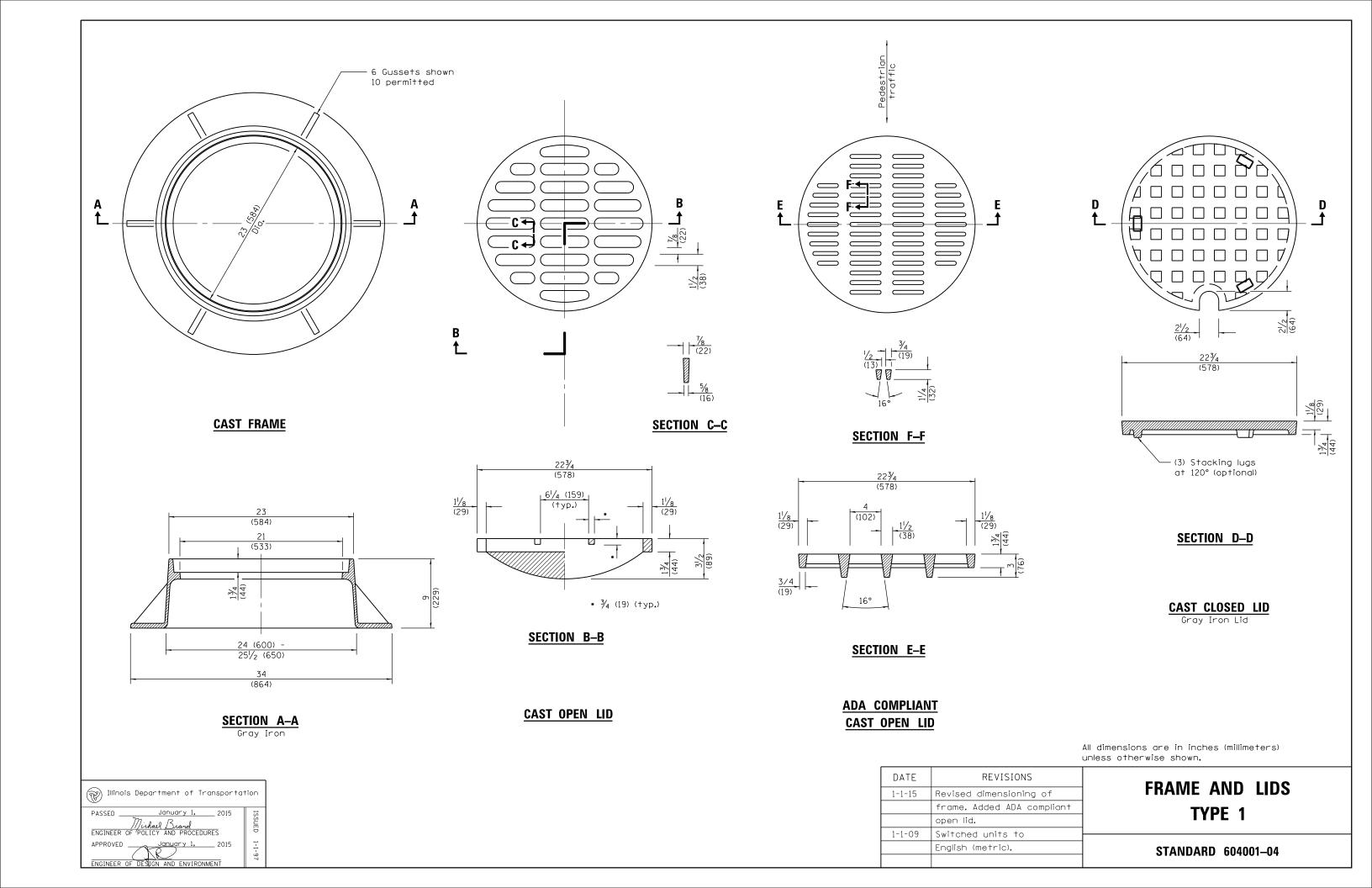
# Illinois Department of Transportation PASSED January 1. 2018 Mirkay Brand ENGINEER OF POLICY AND PROCEDURES APPROVED January 1. 2018 APPROVED January 1. 2018 ENGINEER OF DESIGN AND ENVIRONMENT

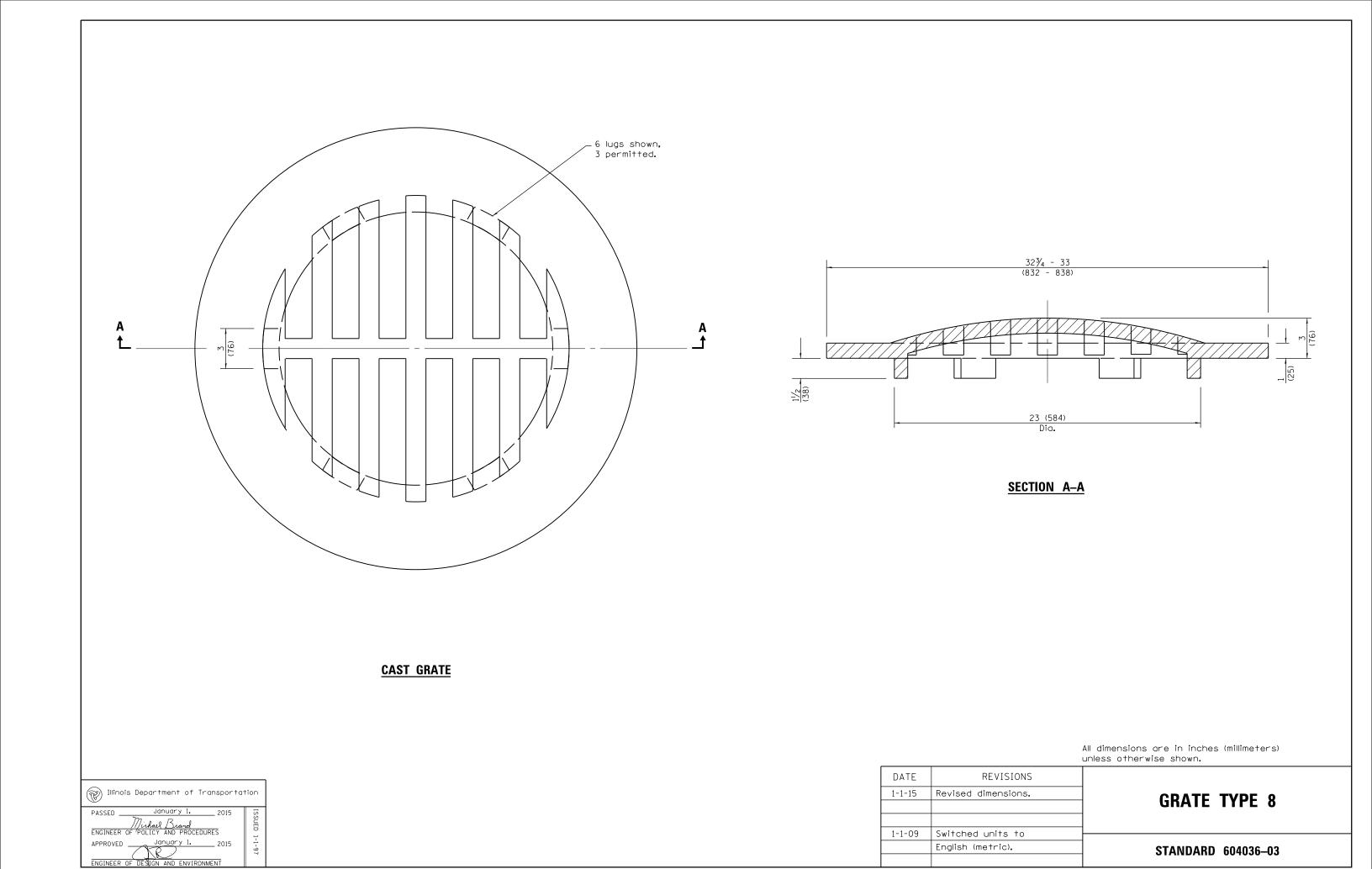
(Showing Layout of Welded Wire Reinforcement)

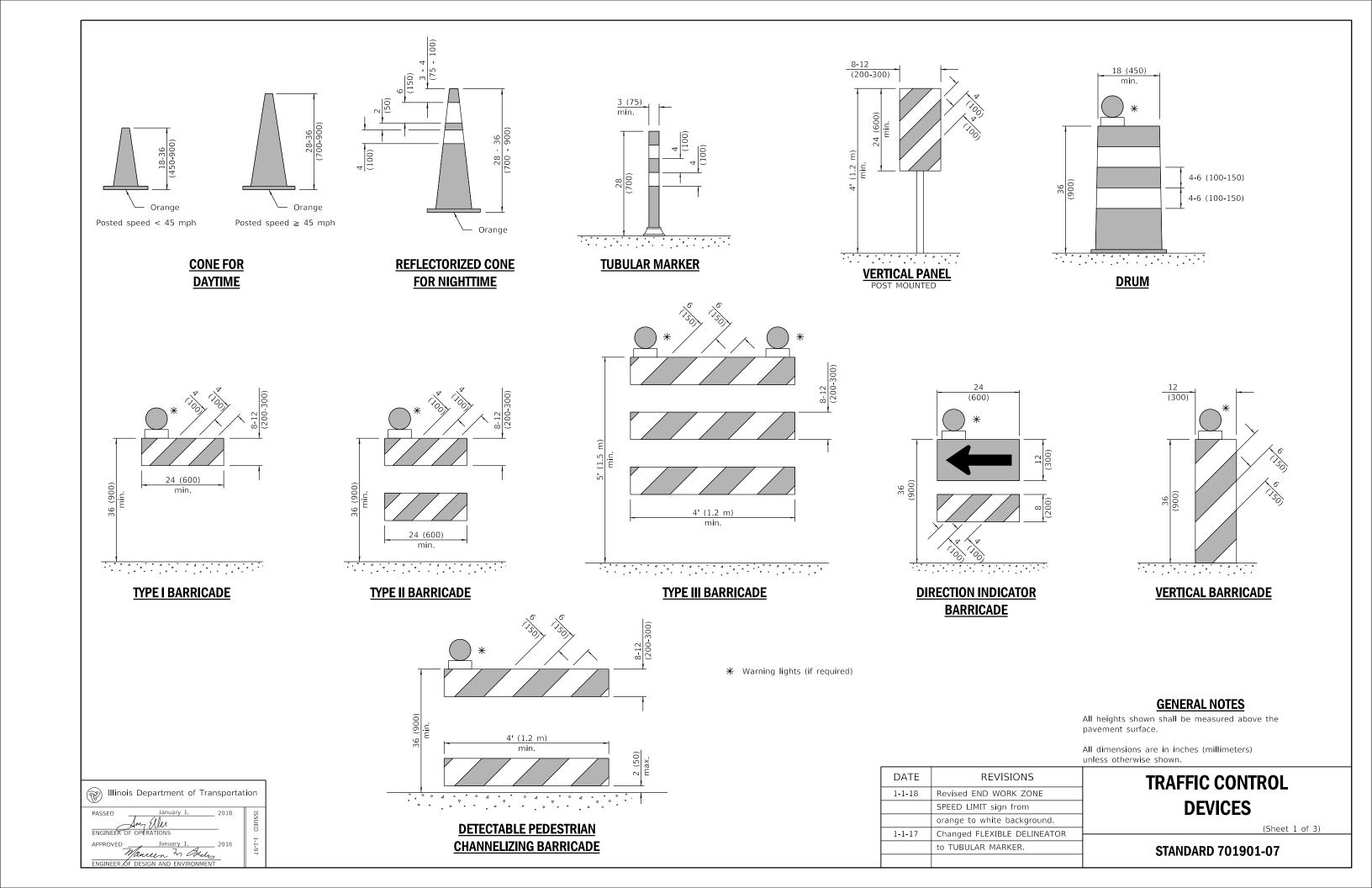
\*\*\* #5 (#16) bars top and bottom. For WWR,
bundle first bar with WWR bar closest to the
opening.

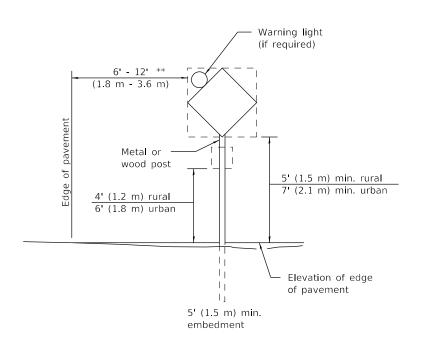
# PRECAST REINFORCED CONCRETE FLAT SLAB TOP

STANDARD 602601-05









#### **POST MOUNTED SIGNS**

WIDTH

**MILES** 

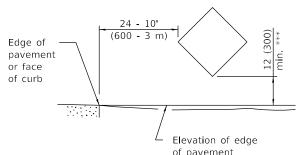
AHEAD

W12-I103-4848

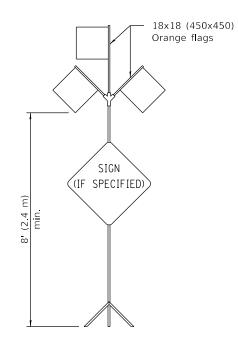
WIDTH RESTRICTION SIGN

XX'-XX" width and X miles are variable.

\*\* When curb or paved shoulder are present this dimension shall be 24 (600) to the face of curb or 6' (1.8 m) to the outside edge of the paved shoulder.



four days, this dimension shall be 5' (1.5 m) min. If located behind other devices, the height shall be sufficient to be seen completely above the devices.

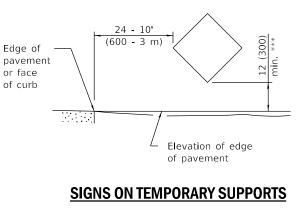


#### HIGH LEVEL WARNING DEVICE

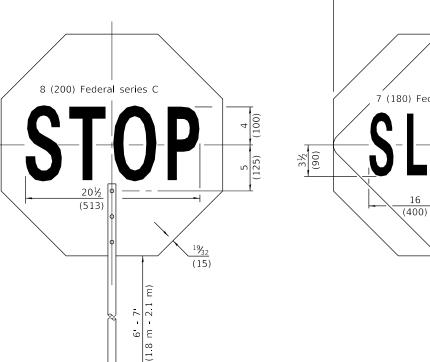
5 (125)

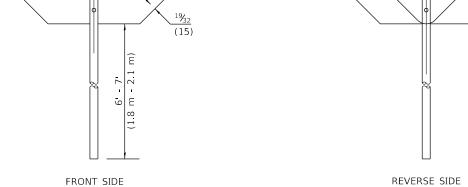
(175)

(600)



\*\*\* When work operations exceed





ROAD CONSTRUCTION NEXT X MILES

END CONSTRUCTION

G20-I104(0)-6036

G20-I105(0)-6024

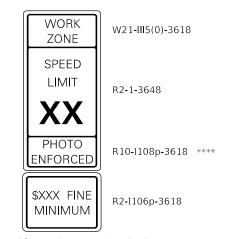
This signing is required for all projects 2 miles (3200 m) or more in length.

ROAD CONSTRUCTION NEXT X MILES sign shall be placed 500' (150 m) in advance of project limits.

END CONSTRUCTION sign shall be erected at the end of the job unless another job is within 2 miles (3200 m).

Dual sign displays shall be utilized on multilane highways.

#### **WORK LIMIT SIGNING**



Sign assembly as shown on Standards or as allowed by District Operations.



G20-I103-6036

This sign shall be used when the above sign assembly is used.

## HIGHWAY CONSTRUCTION **SPEED ZONE SIGNS**

\*\*\*\* R10-I108p shall only be used along roadways under the juristiction of the State.

# TRAFFIC CONTROL **DEVICES**

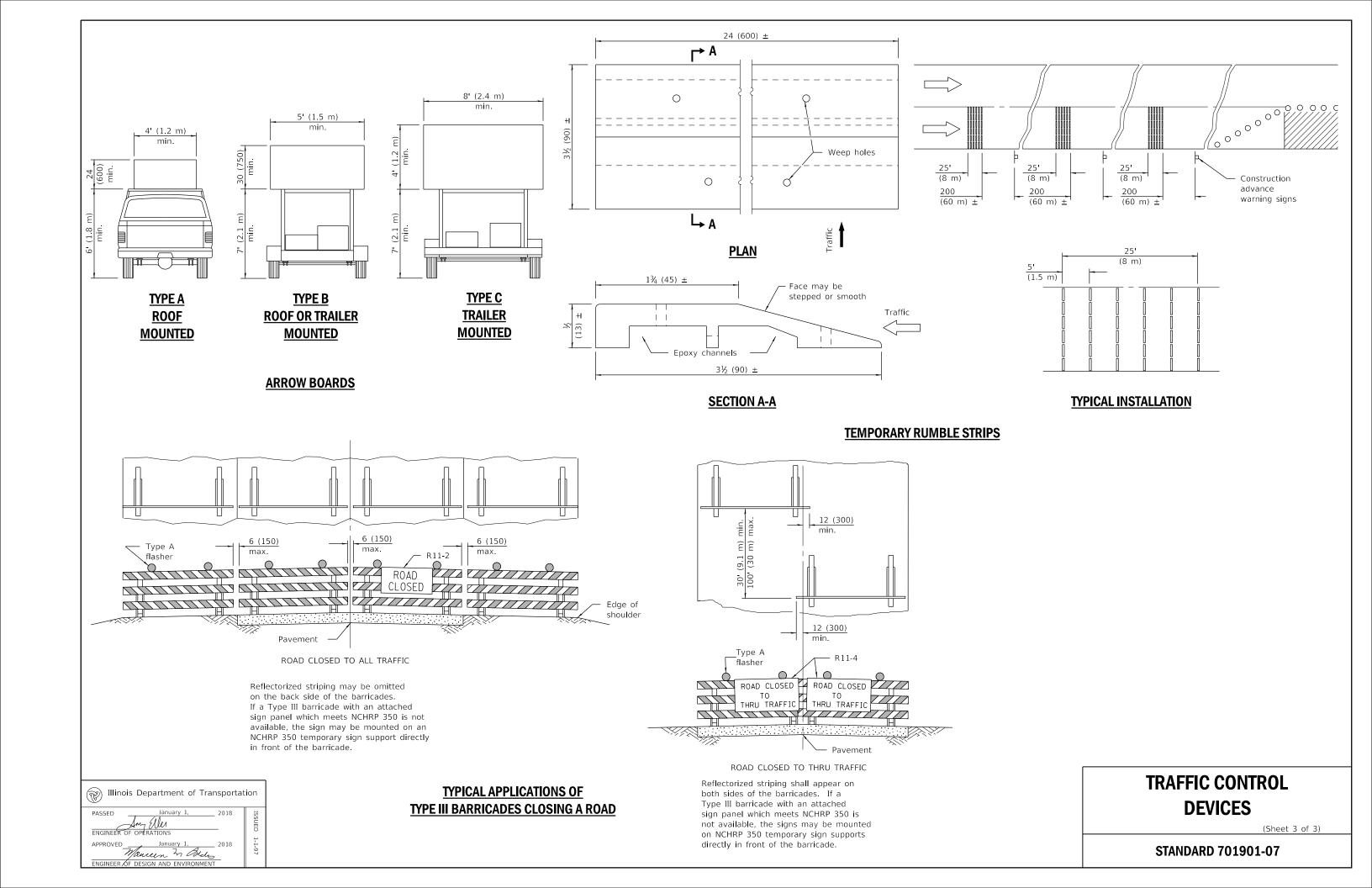
(Sheet 2 of 3)

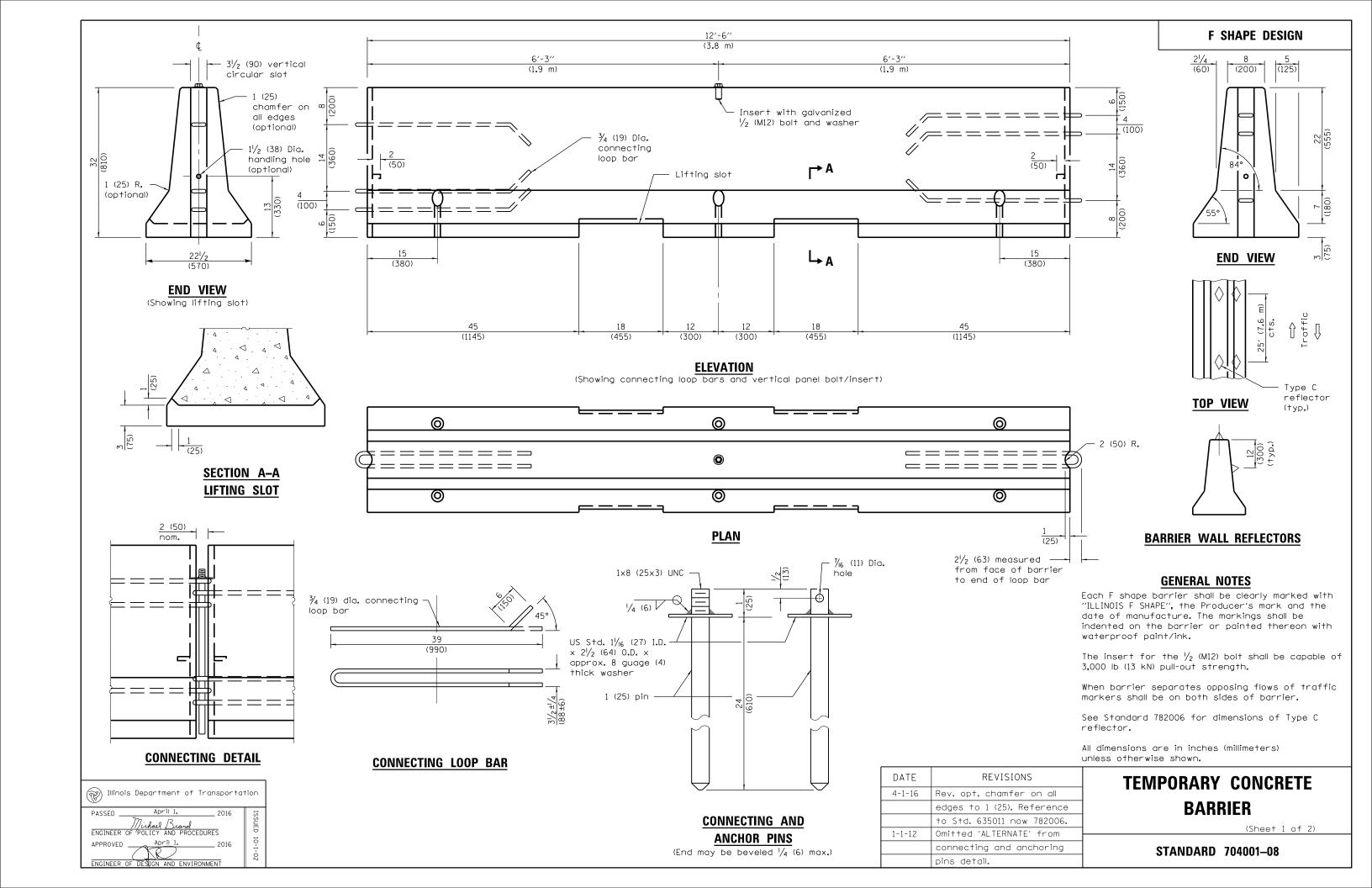
STANDARD 701901-07

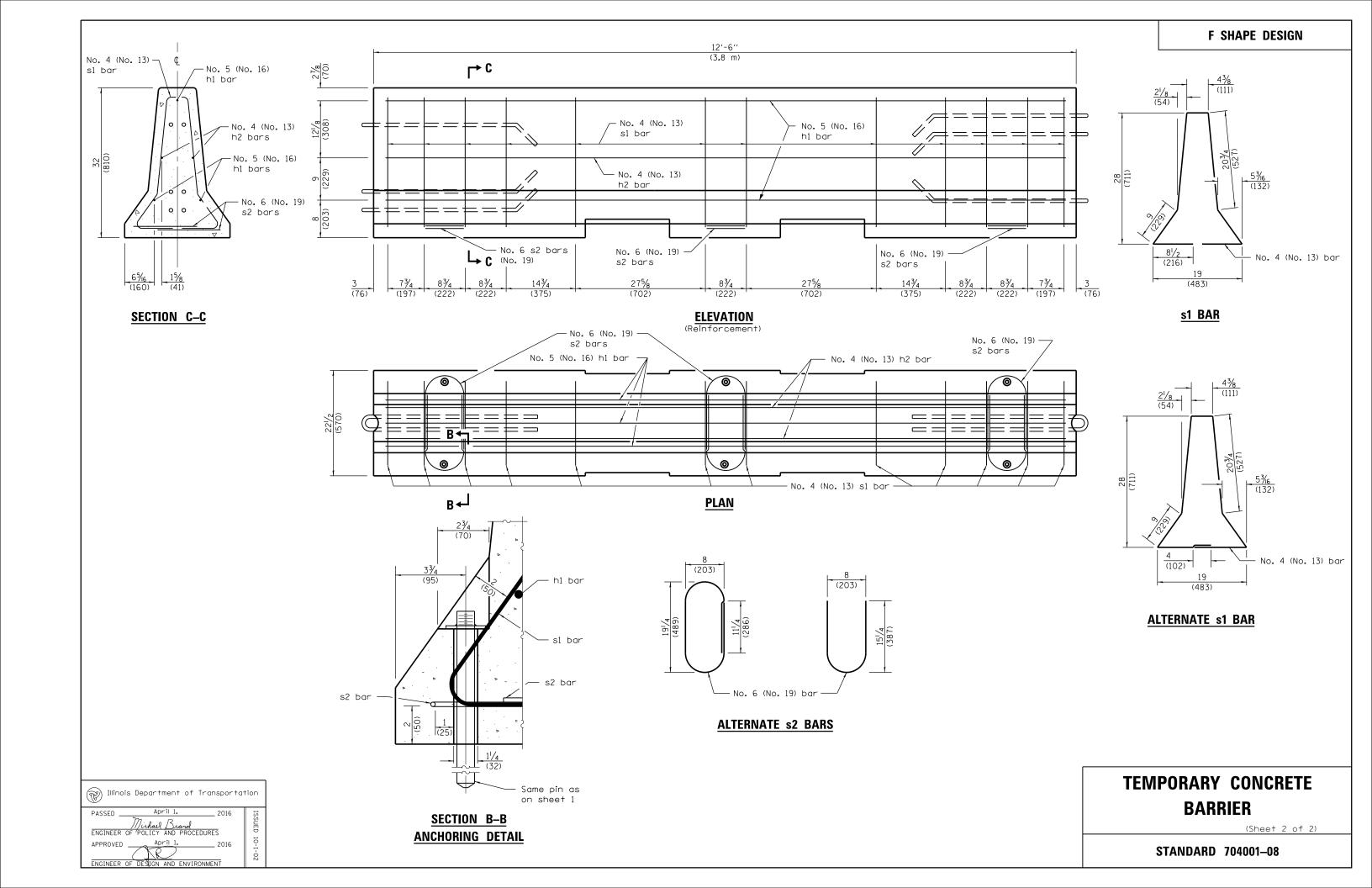
Illinois Department of Transportation

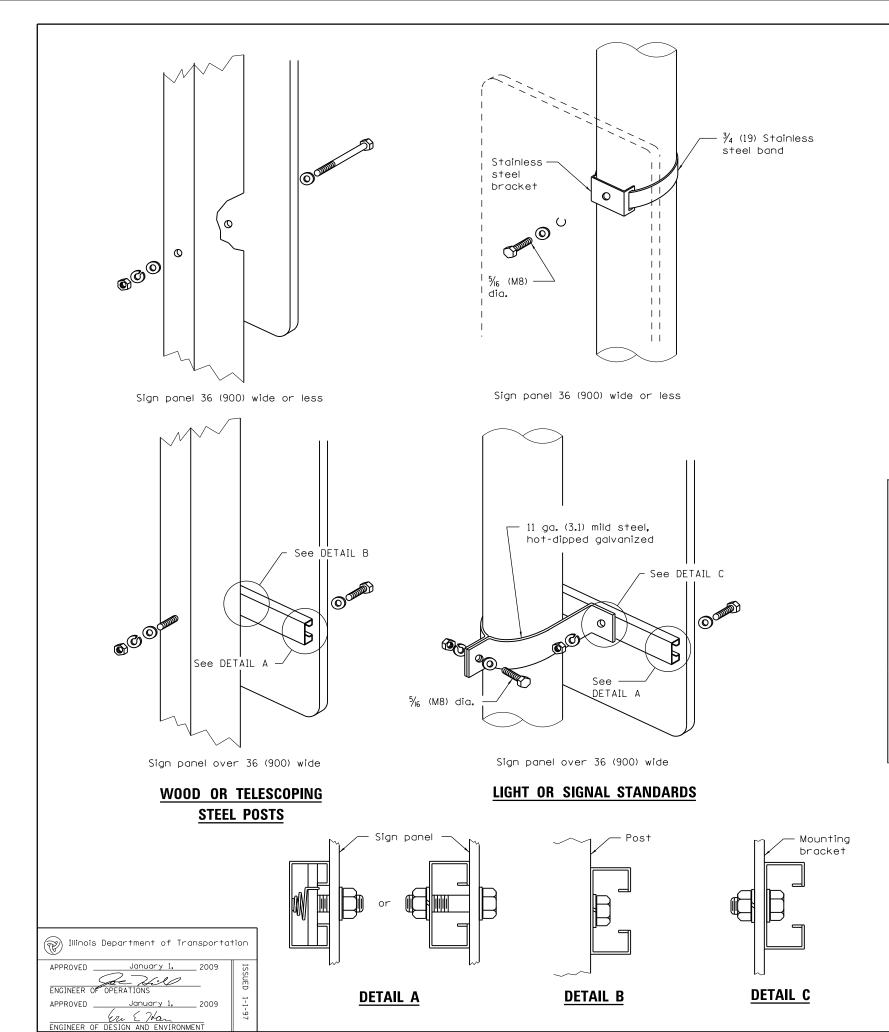
Manuern In Bell

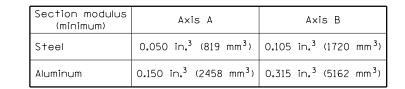
**FLAGGER TRAFFIC CONTROL SIGN** 

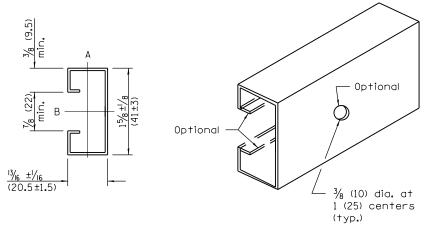




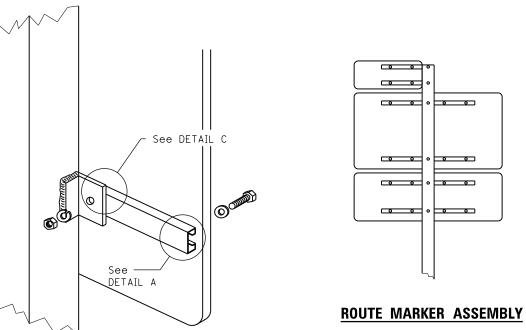








### **SUPPORTING CHANNEL DETAILS**



# BREAKAWAY STEEL TUBING POSTS

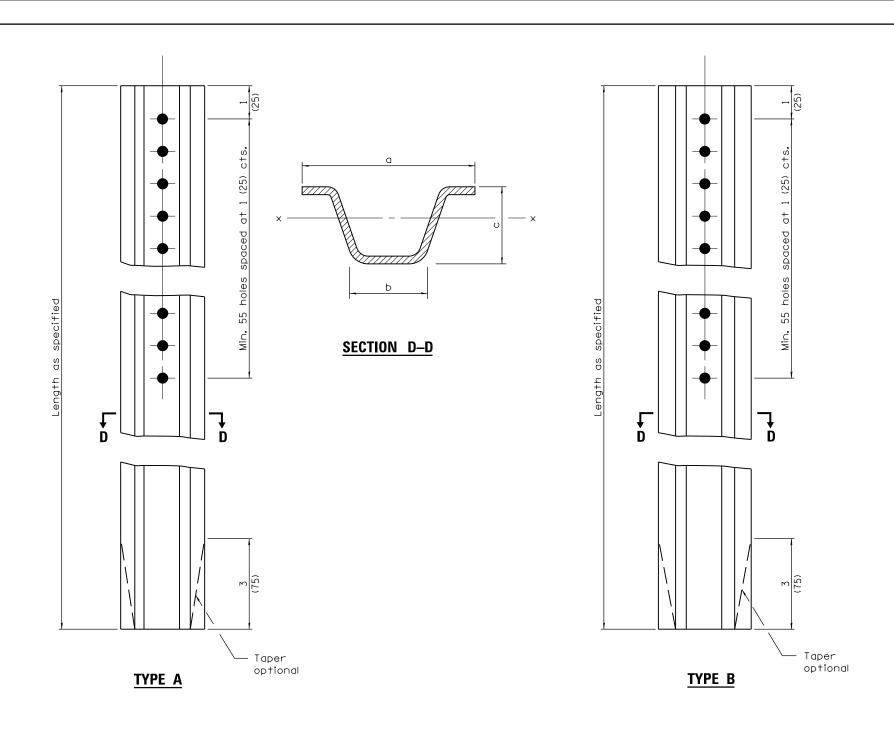
(All sign panel sizes)

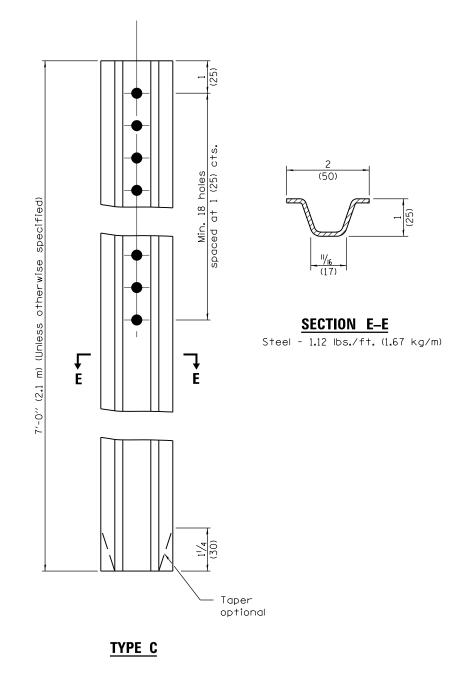
All dimensions are in inches (millimeters) unless otherwise shown.

DATE	REVISIONS	
1-1-09	Switched units to	
	English (metric).	
1-1-97	Renum. Standard 2319-6.	

# SIGN PANEL MOUNTING DETAILS

STANDARD 720001-01





#### **GENERAL NOTES**

Dimensions shown for cross sections are minimum.

All holes are  $\frac{3}{8}$  (10).

Sx-x is the minimum section modulus about the x-x axis of the post as shown. For posts in which holes are punched or drilled for more than half their length, Sx-x shall be computed for the net section.

All dimensions are in inches (millimeters) unless otherwise shown.

DATE	REVISIONS	
1-1-09	Switched units to	
	English (metric).	
1-1-97	Renum. Standard 2350-4.	H

# METAL POSTS FOR SIGNS, MARKERS & DELINEATORS

STANDARD 720011-01

		а	Ь	С	Sx-x in. <sup>3</sup> (mm <sup>3</sup> )	lbs./ft. (kg/m)
TYPE A	Steel	3½ (78)	1 <sup>1</sup> / <sub>4</sub> (32)	1½6 (37)	0.223 (3.654)	2.00 (2.98)
ITPE A	Aluminum	3½ (89)	15/8 (41)	1½ (48)	0.435 (7,128)	0.90 (1.34)
TABL B	Steel	3¾6 (81)	1 <sup>1</sup> / <sub>4</sub> (32)	1½ (38)	0.341 (5.588)	3.00 (4.46)
TYPE B	Aluminum	45/8 (118)	2 <sup>1</sup> / <sub>4</sub> (57)	23/8 (60)	0.888 (14 <b>.</b> 552)	1.30 (1.93)

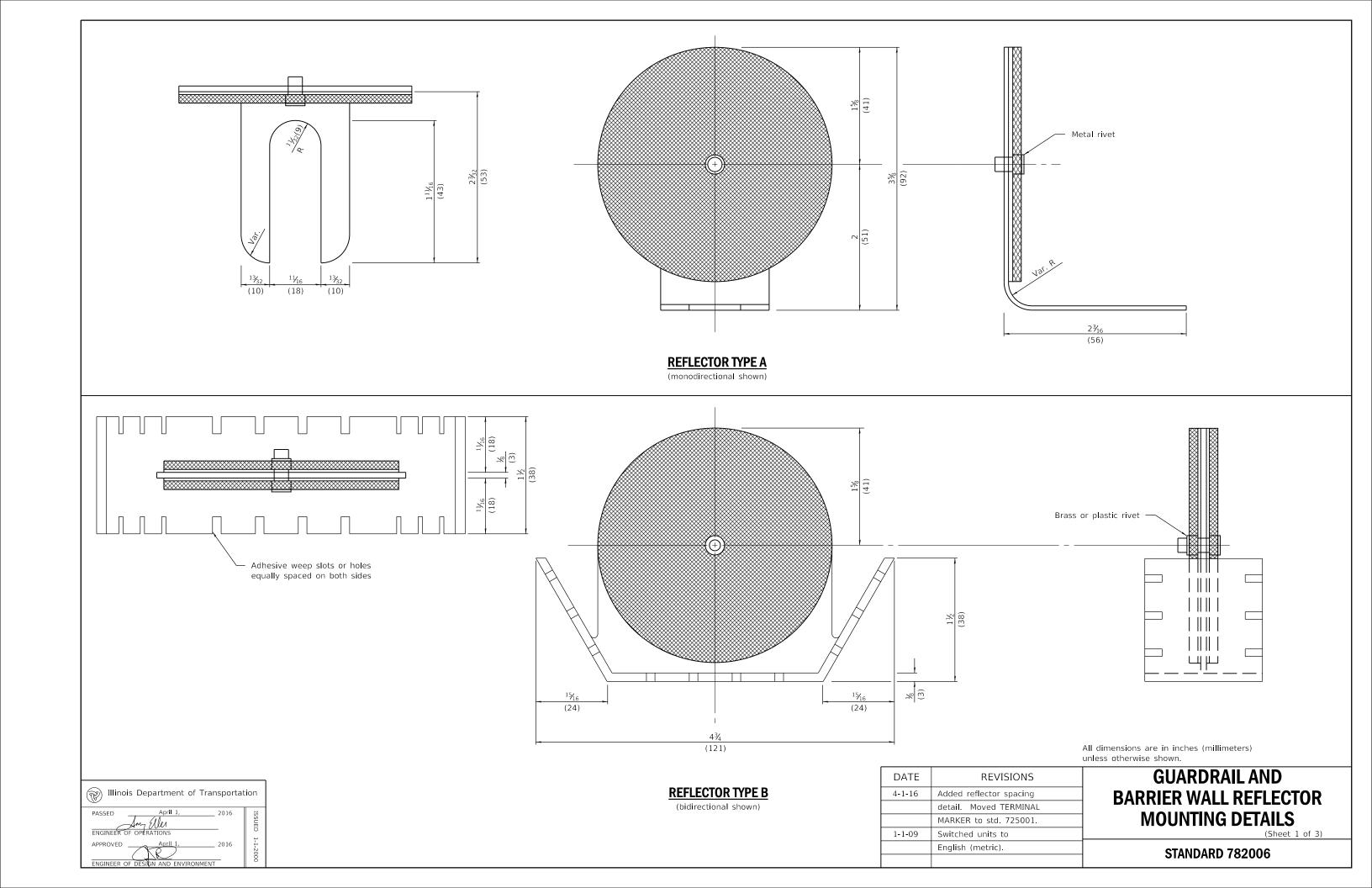
PASSED January 1, 2009

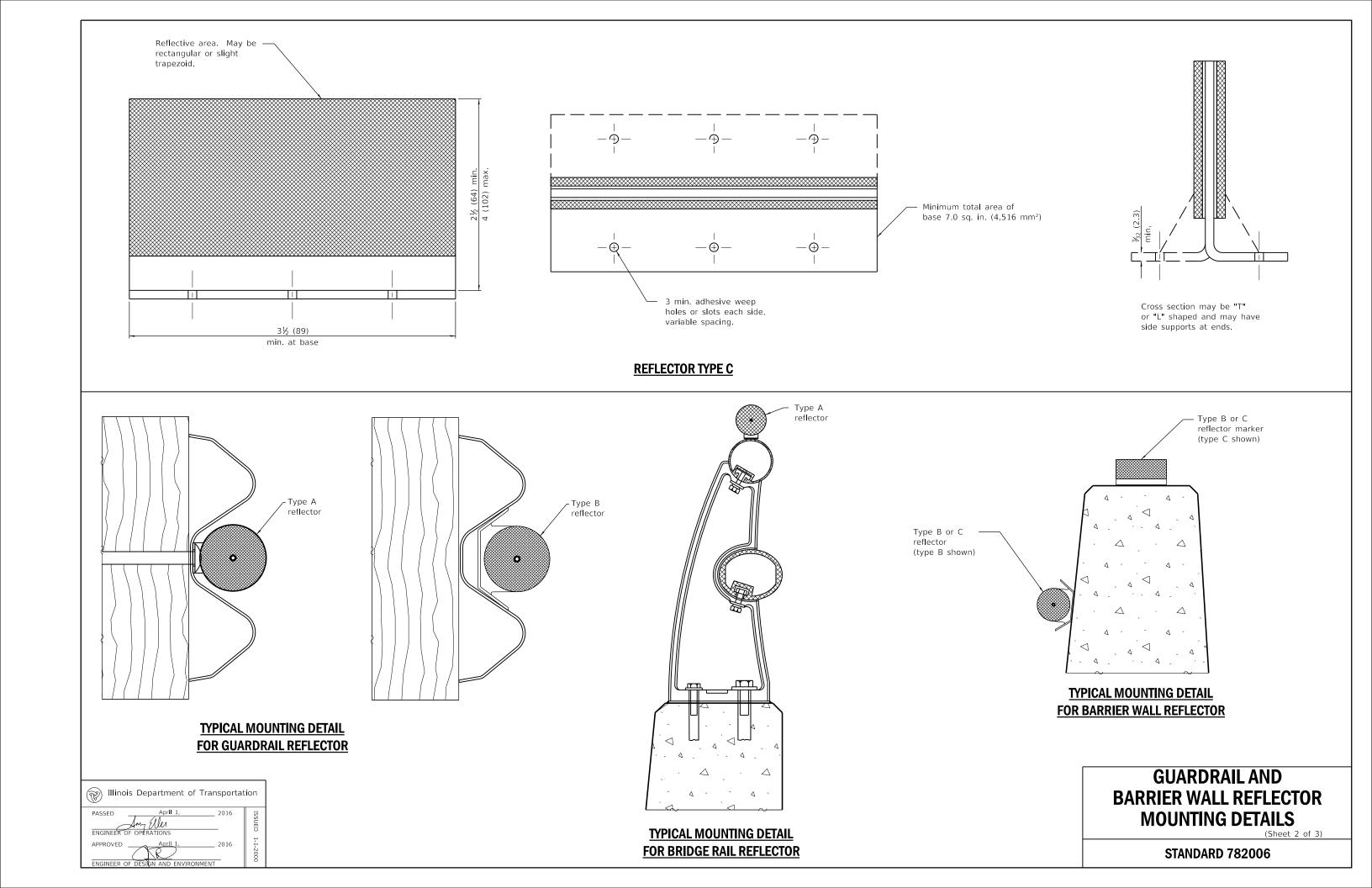
ENGINEER OF POLICY AND PROCEDURES

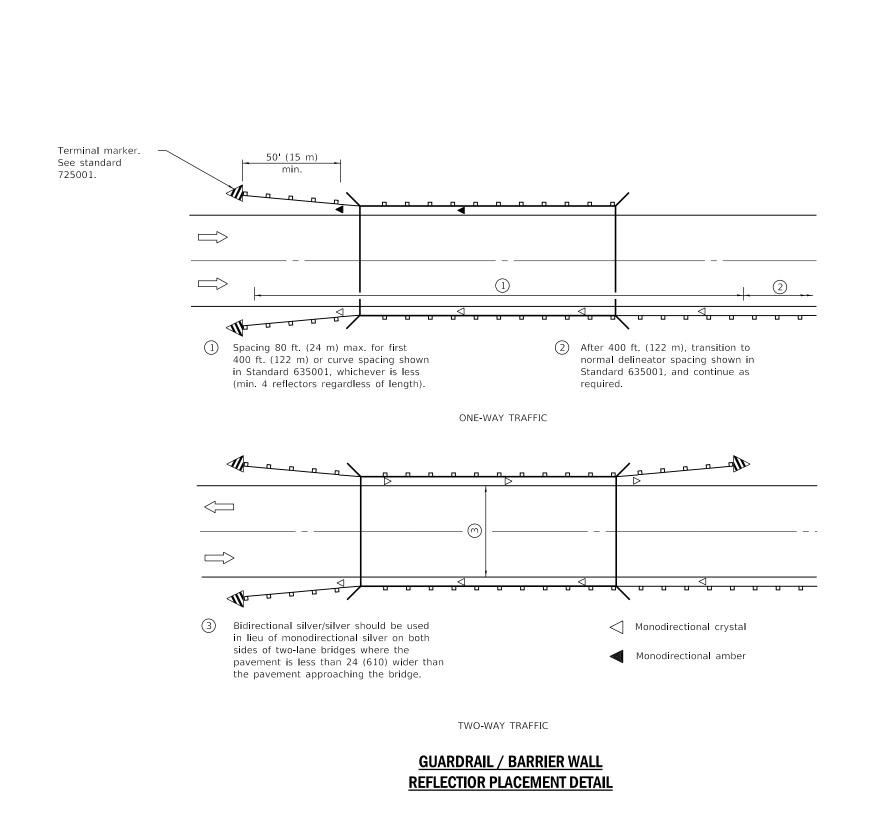
APPROVED January 1, 2009

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ENGINEER OF DESIGN AND ENVIRONMENT





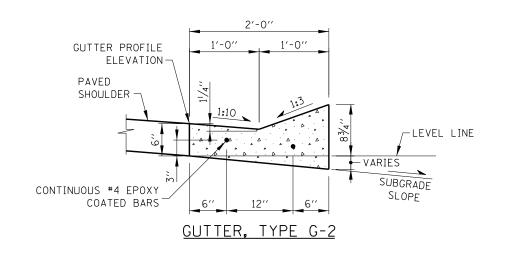


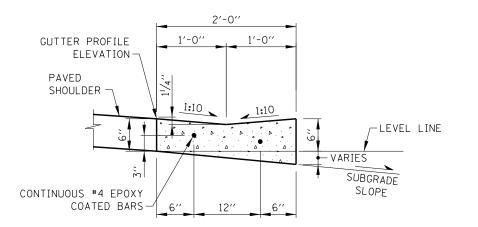
Illinois Department of Transportation

# GUARDRAIL AND BARRIER WALL REFLECTOR MOUNTING DETAILS

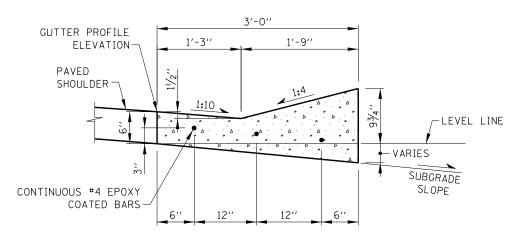
(Sheet 3 of 3)

**STANDARD 782006** 





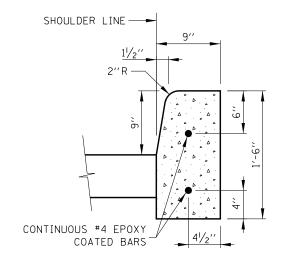
## GUTTER, TYPE G-2, MODIFIED

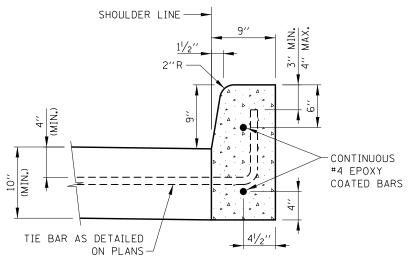


3'-0" GUTTER PROFILE 1'-3'' 1'-9'' ELEVATION PAVED SHOULDER LEVEL LINE VARIES SUBGRADE SLOPE CONTINUOUS #4 EPOXY COATED BARS 12''

GUTTER, TYPE G-3

GUTTER, TYPE G-3, MODIFIED





ADJACENT TO FLEXIBLE PAVEMENT

ADJACENT TO PCC PAVEMENT

CONCRETE CURB, TYPE C (RAMP TOLL PLAZAS ONLY)

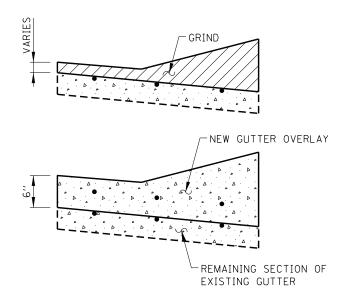
# DATE 2-7-2012

NOTES:

1. FOR CONCRETE CURB, TYPE C TRANSITIONS, THE LEADING ENDS OF CURB IN THE DIRECTION OF TRAFFIC SHALL BEGIN FLUSH WITH ADJACENT PAVEMENT OR SHOULDER SURFACE AND TRANSITION TO FULL HEIGHT AT THE RATE OF ONE INCH VERTICAL TO ONE FOOT HORIZONTAL.

2.	GUTTER TRANSITION DETAILS	STANDARD DRAWING
	TRAFFIC BARRIER TERMINAL TYPE T1 (SPECIAL)	B-28
	TRAFFIC BARRIER TERMINAL TYPE T1-A (SPECIAL)	B-29
	TRAFFIC BARRIER TERMINAL TYPE T10	B-2
	TRAFFIC BARRIER TERMINAL TYPE T6	B-3

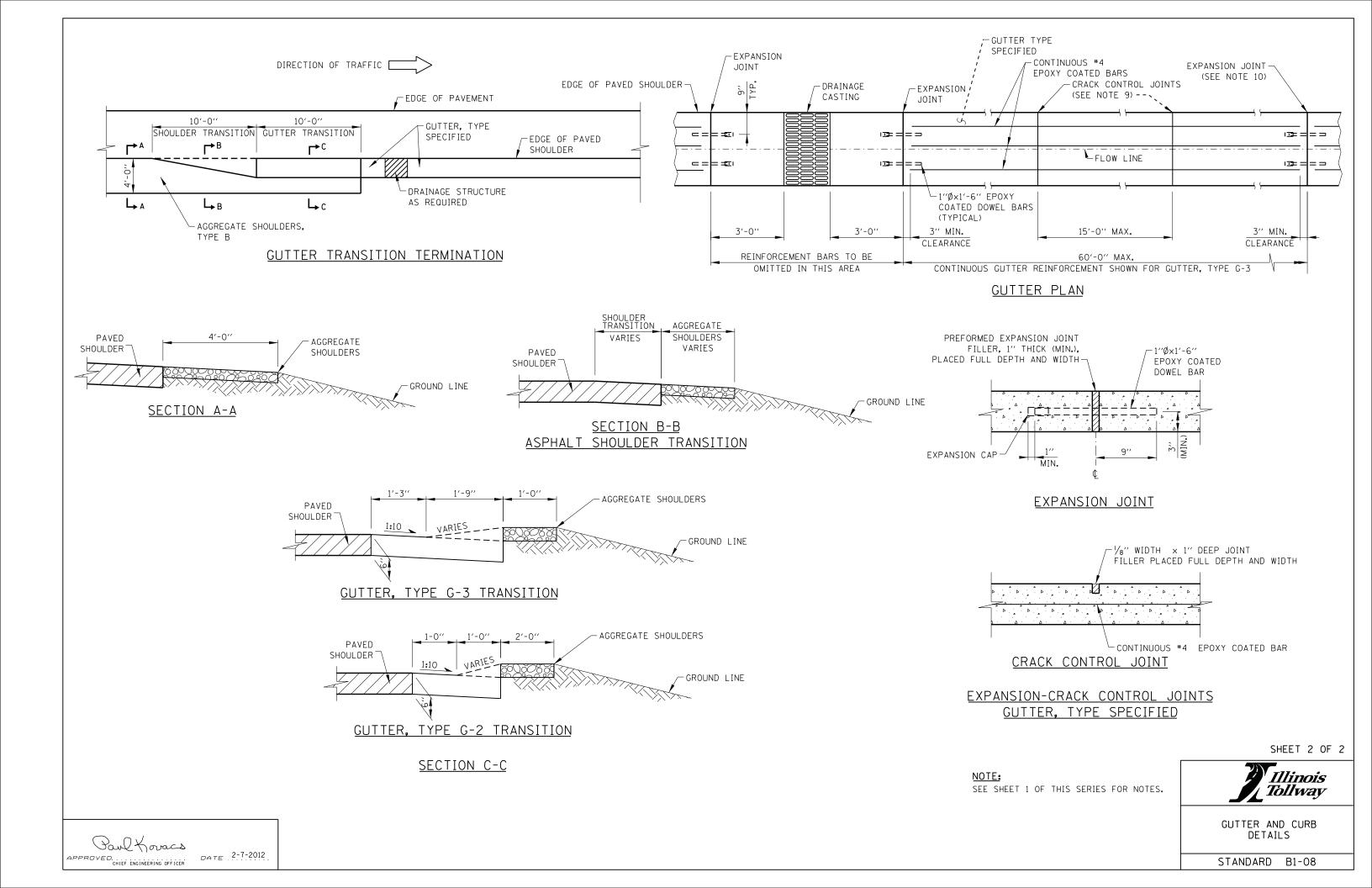
- 3. ALL SLOPES ARE EXPRESSED AS UNITS OF VERTICAL DISPLACEMENT TO UNITS OF HORIZONTAL DISPLACEMENT (V:H).
- 4. REINFORCEMENT STEEL SHALL BE ACCURATELY PLACED AND FIRMLY HELD IN THE POSITION SPECIFIED USING EPOXY COATED STEEL CHAIRS. CHAIR SPACING SHALL NOT EXCEED 4'-0".
- 5. GUTTER REINFORCEMENT SHALL BE PLACED 3" ABOVE BOTTOM OF GUTTER FOLLOWING THE SUBGRADE SLOPE.
- 6. OTHER GUTTER AND CURB TRANSITION DETAILS WILL BE SHOWN ON THE PLANS.
- 7. CONTINUOUS #4 BARS SHALL BE LAPPED A MINIMUM OF 1'-1".
- 8. FOR CONCRETE GUTTER OVERLAYS, CRACK CONTROL JOINTS SHALL BE PLACED AT LOCATIONS OF UNDERLYING JOINTS AND WORKING CRACKS.
- 9. GUTTER CRACK CONTROL JOINTS TO ALIGN IN PROLONGATION WITH PCC SHOULDER JOINTS WHERE EXISTING, CRACK CONTROL JOINTS SHALL BE SEALED FULL DEPTH AND WIDTH IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.
- 10. EXPANSION JOINTS SHALL BE CONSTRUCTED IN GUTTER AT MAXIMUM JOINT SPACING OF 60'-0", SEE EXPANSION JOINT DETAIL ON SHEET 2 OF THIS STANDARD.

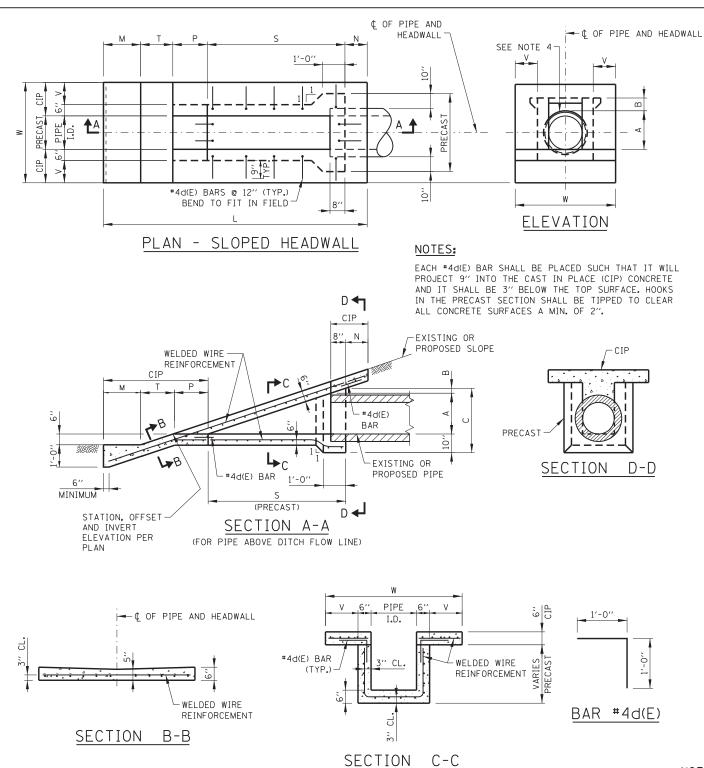


CONCRETE GUTTER OVERLAY

SHEET 1 OF 2

	A TE	25,45,50,45	Illinois Tollway
<i>D</i> 2	4 / E	REVISIONS	
2-0	07-12	REVISED NOTES	
11-0	01-12	ADDED CONCRETE GUTTER	GUTTER AND CURB
		OVERLAY, MODIFIED GUTTER	DETAILS
		CONTROL JOINT SPACING	]
3-11	-2015	REVISED DETAIL DESCRIPTIONS	
3-31	-2016	REVISED NOTE	STANDARD B1-08
3-01	-2018	REVISED NOTE	3 I AINDAIND DI-UO





# <u>DIMENSIONS AND QUANTITIES</u> FOR ONE SLOPED HEADWALL TYPE III

								<u>'</u>	טא נ	JINE	<u>SLUF</u>		HEADI	<u> </u>	ITE 1.	<u> </u>				
	PIPE						DIME	NSIONS						CAST-IN-	WELDED WIRE		REINF	ORCEMENT	BARS	
	I.D.	А	В	С	N	М	Т	Р	S	L	V	W	CONC. CU. YD.	PLACE CU. YD.	REINFORCEMENT SO. YD.	MARK(E)	SIZE	NO.	LENGTH	LB.
	6′′	9"	2¾′′	1'-9¾''	1'-0''	1'-8''	1'-6''	1'-6¾''	2'-111/4''	8′-8′′	1'-0''	3′-6′′	0.15	0.72	3.28	d6	#4	12	2'-0''	16
	12"	1'-31/2''	23/4′′	2'-41/4''	1'-0''	1'-8''	1'-6''	1'-6¾''	4'-6¾''	10'-31/2''	1'-0''	4'-0''	0.34	0.92	4.50	d12	#4	14	2'-0''	19
SLOPE	15"	1'-61/2"	23/4′′	2'-71/4''	1'-0''	1'-8''	1'-6''	1'-6¾''	5′-3¾′′	11'-1/2"	1'-0''	4'-3''	0.45	1.01	5.88	d15	#4	16	2'-0''	21
m	18"	1'-10''	23/4′′	2'-10¾''	1'-0''	1'-8''	1'-6''	1'-6¾''	6'-21/4"	11'-11''	1'-0''	4′-6′′	0.61	1.13	6.44	d18	#4	18	2'-0''	24
1 TO	21''	2'-1''	2¾′′	3'-1¾''	1'-0''	1'-9''	1'-6''	1'-6¾''	6'-11'/4''	12'-9"	1'-3''	5′-3′′	0.76	1.39	8.34	d21	#4	22	2'-0''	29
	24''	2'-41/2"	23/4′′	3'-51/4"	1'-0''	2'-0''	1'-6''	1'-6¾''	7′-9¾′′	13'-101/2''	1'-6''	6′-0′′	0.95	1.72	9.85	d24	#4	24	2'-0''	32
	27''	2'-71/2''	2¾′′	3'-81/4''	1'-11/2"	2'-3''	1'-6''	1'-6¾''	8'-6¾''	15'-0''	1'-9''	6′-9′′	1.14	2.07	13.54	d27	#4	24	2'-0''	32
	30''	2'-11''	2¾′′	3'-11¾''	1'-3"	2'-6''	1'-6''	1'-6¾''	9'-51/4"	16'-3''	2'-0''	7′-6′′	1.38	2.46	16.40	d30	#4	26	2'-0''	35
$\overline{\Box}$	PIPE						DIME	NSIONS					PRE CAST		WELDED WIRE		REINF	ORCEMENT	BARS	
	I.D.	А	В	С	N	М	Т	Р	S	L	٧	W	CONC.	PLACE CU. YD.	REINFORCEMENT SQ. YD.	MARK(E)	SIZE	NO.	LENGTH	LB.
	6′′	9"	2''	1'-9''	1'-0''	1'-8''	2'-0''	2'-1''	3′-8′′	10′-5′′	1'-0''	3′-6′′	0.17	0.83	4.07	d6	#4	12	2'-0''	16
	12''	1'-31/2''	2''	2'-31/2"	1'-0''	1'-8''	2'-0''	2'-1''	5′-10′′	12'-7''	1'-0''	4'-0''	0.41	1.07	5 <b>.</b> 50	d12	#4	16	2'-0''	21
SLOPE	15''	1'-61/2''	2''	2'-61/2"	1'-0''	1'-8''	2'-0''	2'-1''	6'-10''	13'-7''	1'-0''	4'-3''	0.55	1.18	6.63	d15	#4	18	2'-0''	24
4 SL(	18''	1'-10''	2"	2'-10''	1'-0''	1'-8''	2'-0''	2'-1''	8'-0''	14'-9''	1'-0''	4'-6''	0.74	1.32	8.60	d18	#4	22	2'-0''	29
10	21''	2'-1''	2''	3'-1''	1'-0''	1'-9''	2'-0''	2'-1''	9'-0''	15′-10′′	1'-3''	5′-3′′	0.93	1.63	11.03	d21	#4	24	2'-0''	32
-	24"	2'-41/2''	2"	3'-41/2"	1'-0''	2'-0''	2'-0''	2'-1''	10'-2''	17'-3''	1'-6''	6'-0''	1.18	2.00	13.88	d24	#4	28	2'-0''	37
	27"	2'-71/2''	2"	3'-71/2"	1'-11/2''	2'-3''	2'-0''	2'-1''	11'-2''	18'-71/2''	1'-9''	6′-9′′	1.42	2.41	14.83	d27	#4	30	2'-0''	40
	30"	2'-11''	2"	3′-11′′	1'-3''	2'-6''	2'-0''	2'-1''	12'-4''	20'-2"	2'-0''	7′-6′′	1.71	2.87	20.49	d30	#4	32	2'-0''	43
	PIPE						DIME	NSIONS					PRE CAST		WELDED WIRE		REINF	ORCEMENT	BARS	
	I.D.	А	В	С	N	М	Т	Р	S	L	٧	W	CONC.	PLACE CU. YD.	REINFORCEMENT SQ. YD.	MARK(E)	SIZE	NO.	LENGTH	LB.
	6′′	9"	11/2"	1'-81/2''	1'-0''	1′-8′′	3′-0′′	3′-0′′	5′-3′′	13'-11''	1'-0''	3′-6′′	0.23	1.07	5.29	d6	#4	16	2'-0''	21
	12''	1'-31/2''	11/2"	2'-3"	1'-0''	1′-8′′	3′-0′′	3'-0''	8'-6''	17'-2''	1'-0''	4′-0′′	0.57	1.38	8.62	d12	#4	22	2'-0''	29
SLOPE	15"	1'-61/2''	11/2"	2′-6′′	1'-0''	1′-8′′	3'-0''	3'-0''	10'-0''	18'-8''	1'-0''	4'-3''	0.77	1.53	10.35	d15	#4	26	2'-0''	35
9	18"	1'-10''	11/2''	2'-91/2''	1'-0''	1'-8''	3'-0''	3'-0''	11'-9''	20′-5′′	1'-0''	4′-6′′	1.04	1.70	12.47	d18	#4	28	2'-0''	37
1 TO	21"	2'-1''	11/2"	3'-01/2''	1'-0''	1'-9''	3′-0′′	3′-0′′	13'-3''	22'-0''	1'-3''	5′-3′′	1.31	2.11	15.77	d21	#4	34	2'-0''	45
	24''	2'-41/2''	11/2"	3'-4''	1'-0''	2'-0''	3'-0''	3'-0''	15′-0′′	24'-0''	1'-6''	6′-0′′	1.66	2.59	17.62	d24	#4	38	2'-0''	51
1				1					1				1	1						

#### NOTES:

 THE CAST IN PLACE (CIP) SLOPED HEADWALL SHALL BE CONSTRUCTED FLUSH WITH EXISTING OR PROPOSED SLOPE.

1'-11/2"

1'-3''

2'-3''

2'-6"

3'-0"

3'-0"

3'-0"

16'-6'' 25'-101/2"

28'-0"

18'-3''

1'-9"

2'-0"

6'-9''

7′-6′′

1.99

3.11

3.70

3'-7"

3'-101/2''

2. CLASS SI CONCRETE SHALL BE USED THROUGHOUT.

2'-71/2"

11/2"

11/2"

27"

- 3. WELDED WIRE REINFORCEMENT SHALL BE EPOXY COATED 6x6-W4xW4, 58 LBS. PER 100 SO.FT.
- 4. ALL REINFORCEMENT BARS SHOWN SHALL BE EPOXY COATED (E).
- 5. BAR BENDING DETAILS ARE DIMENSIONED OUT TO OUT OF BARS.
- 6. COVER FROM FACE OF CONCRETE TO FACE OF REINFORCEMENT BAR SHALL BE 3" FOR SURFACES FORMED AGAINST EARTH AND 2" FOR ALL OTHER SURFACES UNLESS OTHERWISE SHOWN
- 7. PRECAST UNIT USE IS OPTIONAL. THE ENTIRE STRUCTURE MAY BE CAST IN PLACE.
- 8. AFTER THE PRECAST SLOPED HEADWALL HAS BEEN PLACED, THE SPACE BETWEEN THE HEADWALL AND PIPE SHALL BE COMPLETELY FILLED WITH AN APPROVED NON-SHRINK GROUT WITH A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 5000 PSI.

9. THE SLOPED HEADWALL DETAILS SHOWN ON THIS DRAWING ARE FOR USE ONLY WITH PIPES HAVING DIAMETER OR SPAN OF 30" OR LESS.

d27

d30

#4

#4

40

44

2'-0"

2'-0"

53

59

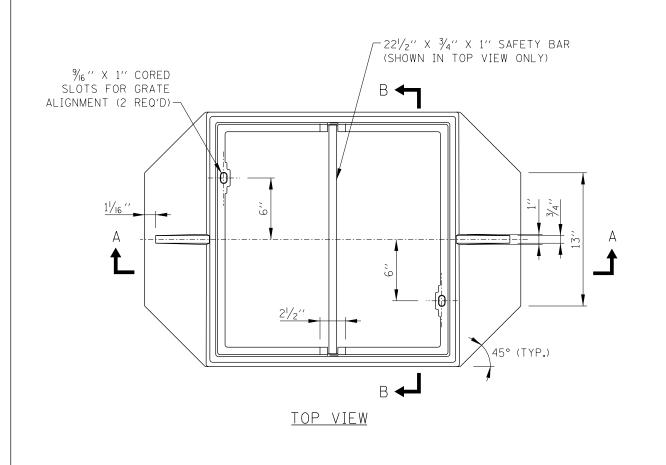
24.10

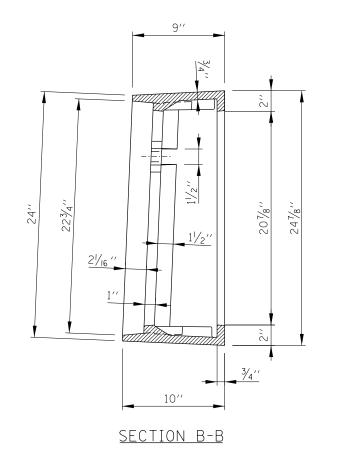
29.13

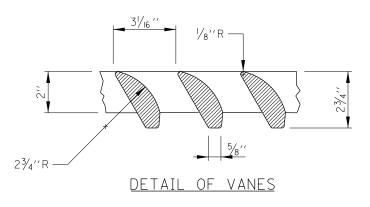
- 10. ALL SLOPES ARE EXPRESSED AS UNITS OF VERTICAL DISPLACEMENT TO UNITS OF HORIZONTAL DISPLACEMENT (V:H).
- 11. I.D. DENOTES INSIDE DIAMETER OF PIPE. O.D. DENOTES OUTSIDE DIAMETER OF PIPE.

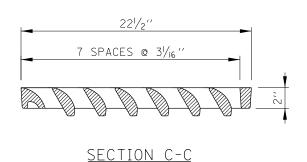
		Illinois Tollway
DATE	REVISIONS	
31-2014	REVISED QUANTITIES	
-11-2015	REVISED TABLES AND SECTIONS	SLOPED HEADWALLS
31-2016	CHANGED TERMINOLOGY TO	TYPE III DETAILS
	WELDED WIRE REINFORCEMENT	2
31-2017	REVISED TABLE (L)	
		STANDARD B10-09

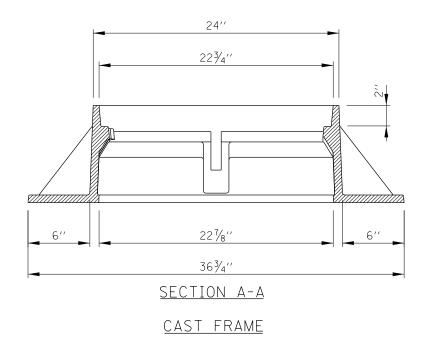


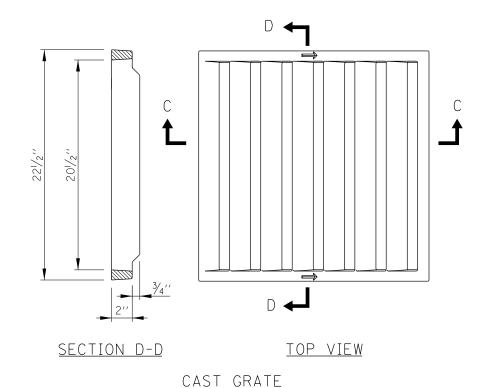












# NOTES:

- 1. ALL FRAMES AND GRATES SHALL CONFORM TO THE REQUIREMENTS OF ART. 1006.14 FOR GRAY IRON CASTINGS AND TO ART. 1006.15 FOR DUCTILE IRON CASTINGS.
- 2. FRAME AND GRATE TO BE NEENAH FOUNDRY COMPANY, NEENAH NO. R-3528-V, EAST JORDAN IRON WORKS 7535 OR APPROVED EQUAL.
- 3. GRATE SHALL NOT BE BOLTED TO FRAME.

Illinois Tollway	

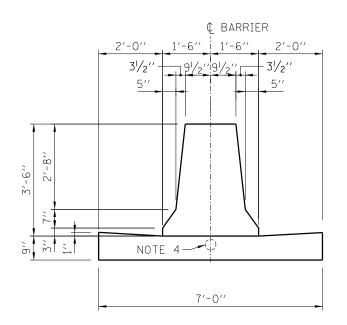
DATE REVISIONS

O3-31-14 ADDED FRAME AND GRATE
CASTINGS

FRAME AND GRATE
TYPE 20A

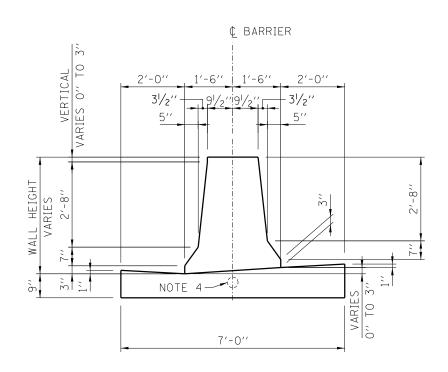
STANDARD B25-01

CHIEF ENGINEER DATE 6-30-2008



CONCRETE BARRIER, DOUBLE FACE, 42"

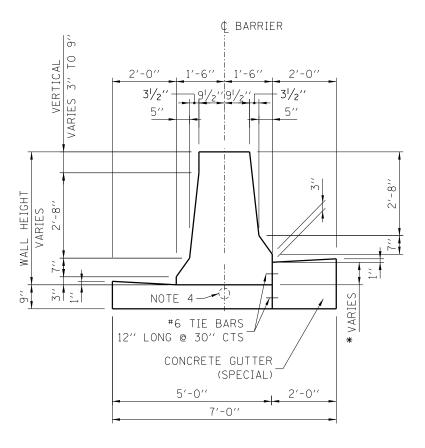
CONCRETE BARRIER BASE, 7'-0"



CONCRETE BARRIER, DOUBLE FACE, VARIABLE HEIGHT

CONCRETE BARRIER BASE, VARIABLE HEIGHT, 7'-O''

(BARRIER HEIGHT VERTICAL DIFFERENTIAL VARIES O'' TO 3'')



# CONCRETE BARRIER, DOUBLE FACE, VARIABLE HEIGHT CONCRETE BARRIER BASE, 5'-O''

(BARRIER HEIGHT VERTICAL DIFFERENTIAL VARIES 3" TO 9")
\*\*WHEN 6" OR GREATER ADD TOP TIE BAR.

#### NOTES:

- 1. 2" DEEP CONTRACTION JOINTS SHALL BE DONE BY SAWING AND SHALL BE CONSTRUCTED IN THE CONCRETE BARRIER WALL, CONCRETE BARRIER BASE, AND CONCRETE GUTTER (SPECIAL). CONTRACTION JOINTS SHALL ALSO BE CONSTRUCTED AT BOTH SIDES OF ALL DRAINAGE STRUCTURES. MAXIMUM CONTRACTION JOINT SPACING SHALL BE 30'-0". THE MINIMUM DISTANCE BETWEEN CONTRACTION JOINTS IN THE MEDIAN BARRIER WALL SHALL BE 2'-0". WHEN A DRAINAGE STRUCTURE FALLS WITHIN 2'-0" FROM AN EXPANSION JOINT (OR) CONTRACTION JOINT, THE NEAREST CONTRACTION JOINT SHALL BE OMITTED.
- 2. GUTTER PROFILE IN THE VICINITY OF SAG VERTICAL CURVES, ALONG FLAT GRADES AND AT THE MEETING OF PROPOSED AND EXISTING GUTTER, SHALL BE CAREFULLY CONTROLLED AND FIELD ADJUSTED IF NECESSARY TO ENSURE POSITIVE DRAINAGE AND AVOID PONDING.
- 3. IN AREAS OF RELATIVELY FLAT LONGITUDINAL PROFILE GRADES, THE 3" VERTICAL DIMENSION AT THE BOTTOM OF THE BARRIER CAN VARY FROM 2" TO 3"/4" TO CREATE AN ACCEPTABLE LONGITUDINAL GRADE IN THE GUTTER.
- 4. REFERENCE PLAN SHEET FOR TYPE, SIZE AND NUMBER OF CONDUITS. PROVIDE 11/2" (MIN.) CLEARANCE TO THE TOP OF CONDUIT AND 2" (MIN.) CLEARANCE TO THE BOTTOM OF THE CONDUIT.
- 5. WHEN VARIABLE HEIGHT VERTICAL DIFFERENTIAL EXCEEDS 9" SEE STRUCTURAL PLANS FOR DETAILS.
- 6. GUTTER SLOPE SHALL BE 4.17% SLOPED TOWARD THE MEDIAN UNLESS OTHERWISE NOTED. GUTTER SLOPE IS REVERSE PITCHED IN SUPERELEVATED SECTIONS. TRANSITION GUTTER SLOPE OVER 30'-0". GUTTER SLOPE TRANSITIONS ARE INCLUDED IN THE COST OF CONCRETE BASE AND/OR CONCRETE GUTTER (SPECIAL). SEE ROADWAY PLANS FOR LIMITS OF REVERSE PITCHED GUTTER AND TRANSITIONS.

	REVISIONS	DATE
	ADDED CONDUITS TO	2-07-2012
'	BARRIER BASE	
	ADDED GUTTER TRANSITION	11-01-2012
	TAPER DETAIL AND NEW	
	JOINT DETAIL	
	MODIFIED BARRIER BASE	3-31-2014

Illinois Tollway

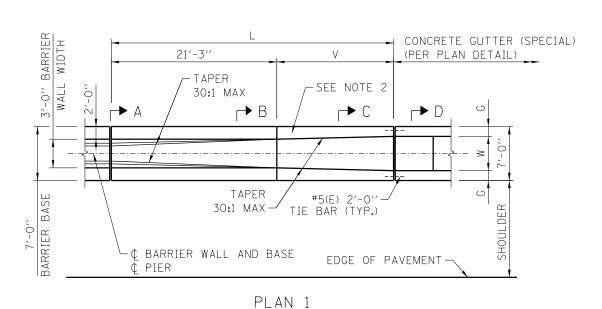
AND CONCRETE BARRIER, DOUBLE FACE, 42" AND VARIABLE HEIGHT

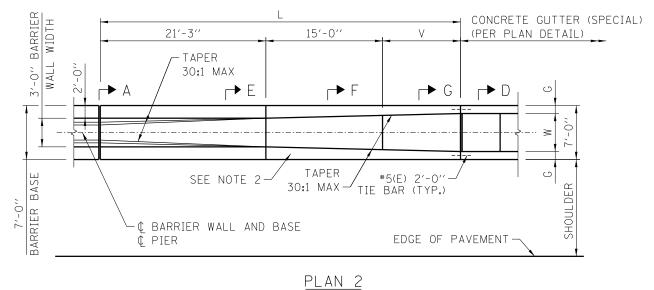
ISED NOTES STANDARD C5-05

\*WHEN 6" OR GREATE

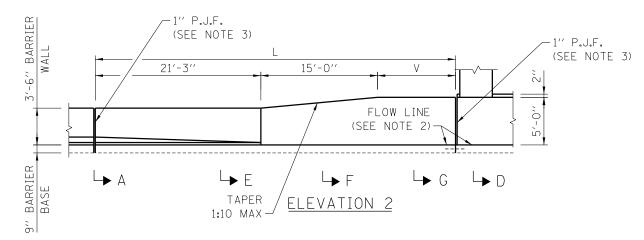
Paul Koracs

APPROVED. CHIEF ENGINEER DATE 2-7-2012





#### ' BARRIER WALL -1" P.J.F. (SEE NOTE 3) -1" P.J.F. (SEE NOTE 3) 21'-3'' -6,, FLOW LINE (SEE NOTE 2) BARRIER BASE **▶** B **└** C **└**▶ A TAPER ELEVATION 3 1:10 MAX-



CONCRETE MEDIAN BARRIER TRANSITION, TYPE V-F AT BRIDGE PIERS (FOR W ≤4'-0")

CONCRETE MEDIAN BARRIER TRANSITION, TYPE V-F
AT BRIDGE PIERS (FOR W >4'-0")

		TABLE OF	VARIABLES	
	W	L	V	G
	3'-0''	31′-3′′	10'-0''	2'-0''
A	3′-6′′	31′-3′′	10'-0''	1'-9''
٦	4'-0''	36′-3′′	15′-0′′	1'-6''
2	4'-6''	46′-3′′	10'-0''	1'-3''
	5′-0′′	51′-3′′	15′-0′′	1'-0''
PLAN	5′-6′′	58′-9′′	22′-6′′	9′′
	6'-0''	66′-3′′	30′-0′′	6′′

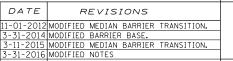
#### NOTES:

- 1. 2" DEEP CONTRACTION JOINTS SHALL BE DONE BY SAWING AND SHALL BE CONSTRUCTED IN THE CONCRETE BARRIER WALL, CONCRETE BARRIER BASE, AND CONCRETE GUTTER (SPECIAL). CONTRACTION JOINTS SHALL ALSO BE CONSTRUCTED AT BOTH SIDES OF ALL DRAINAGE STRUCTURES. MAXIMUM CONTRACTION JOINT SPACING SHALL BE 30'-0". THE MINIMUM DISTANCE BETWEEN CONTRACTION JOINTS IN THE MEDIAN BARRIER WALL SHALL BE 2'-0". WHEN A DRAINAGE STRUCTURE FALLS WITHIN 2'-0" FROM AN EXPANSION JOINT (OR) CONTRACTION JOINT, THE NEAREST CONTRACTION JOINT SHALL BE OMITTED.
- 2. GUTTER PROFILE IN THE VICINITY OF SAG VERTICAL CURVES, ALONG FLAT GRADES AND AT THE MEETING OF PROPOSED AND EXISTING GUTTER, SHALL BE CAREFULLY CONTROLLED AND FIELD ADJUSTED IF NECESSARY TO ENSURE POSITIVE DRAINAGE AND AVOID PONDING.
- 3. NON-STAINING GRAY ONE COMPONENT NON-SAG ELASTOMERIC GUN GRADE POLYURETHANE SEALANT MEETING THE REQUIREMENTS OF ASTM C-920, TYPE S, GRADE NS, CLASS 25, USE T WITH A BACKER ROD.

SHEET 1 OF 2

Illinois

Tollway

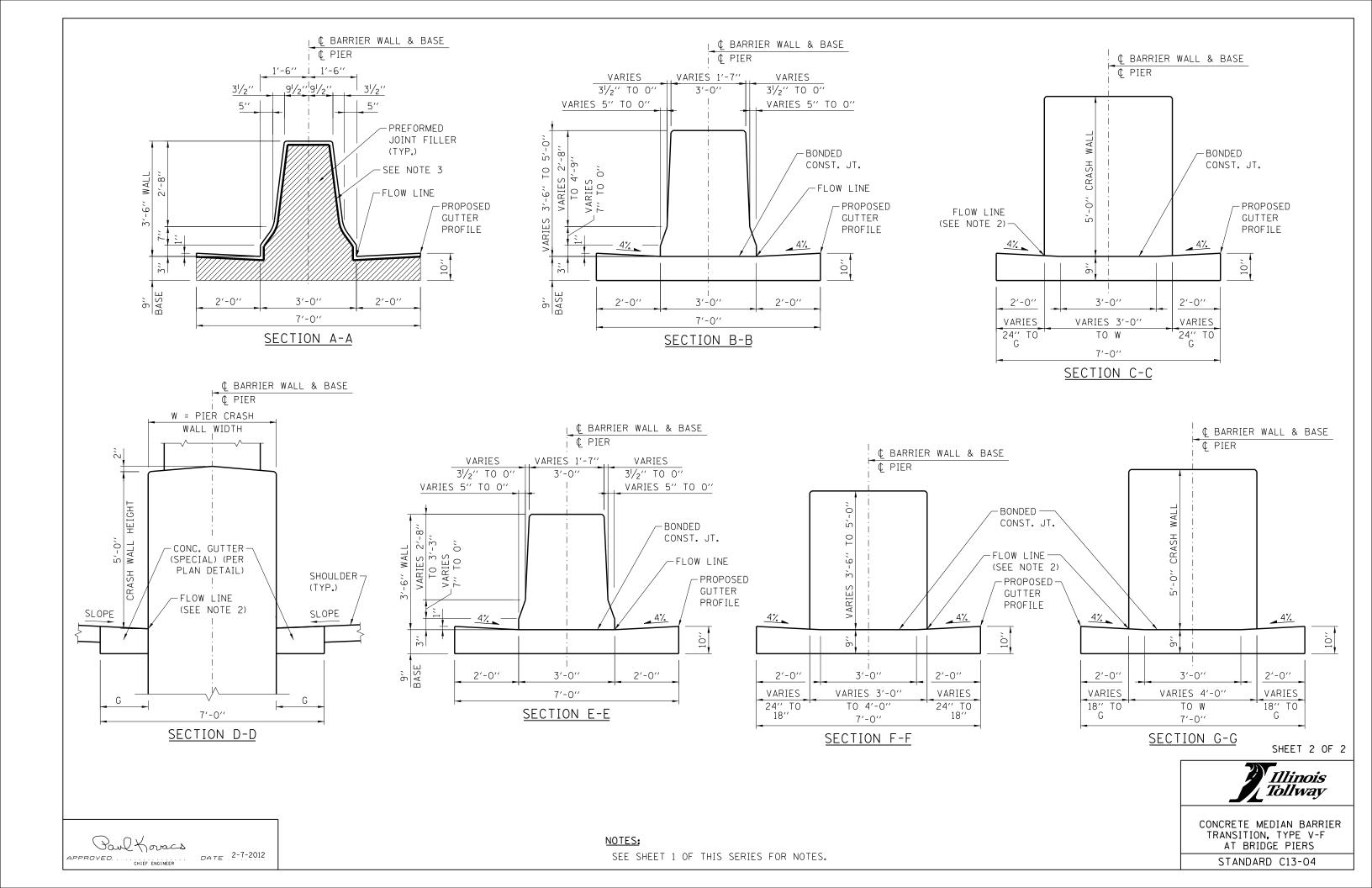


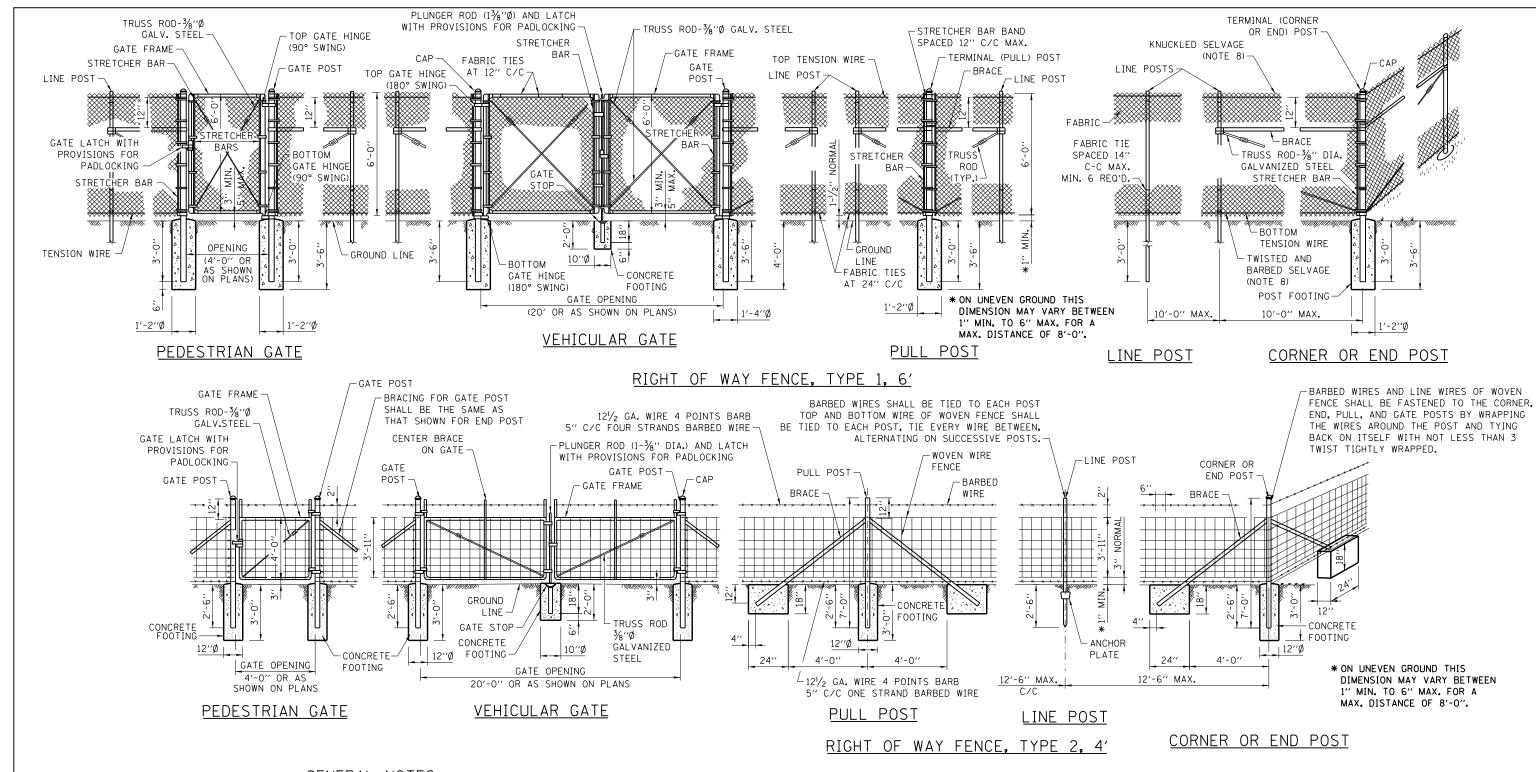
CONCRETE MEDIAN BARRIER TRANSITION, TYPE V-F AT BRIDGE PIERS

STANDARD C13-04

Paul Kovacs
APPROVED. CHIEF ENGINEER

DATE 2-7-2012

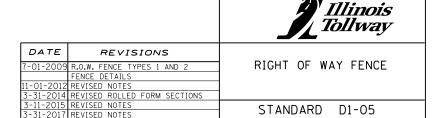




#### GENERAL NOTES

- ON STRAIGHT RUNS OF FENCE, PULL POSTS SHALL BE USED AT 500' CENTERS FOR TYPE 1 AND 330' CENTERS FOR TYPE 2.
- WHERE R.O.W. FENCE FOLLOWS R.O.W. LINE IT SHALL BE INSTALLED PARALLEL TO AND 6" INSIDE THE R.O.W. LINE ON ILLINOIS TOLLWAY PROPERTY.
- LINE POSTS AND BRACES SHALL BE ON ILLINOIS TOLLWAY SIDE OF FENCE FABRIC.
- 4. WHEN THE TENSION OF THE FENCE TENDS TO PULL THE POSTS FROM THE GROUND, THE LINE POSTS SHALL BE ANCHORED WITH ANCHORAGE SPECIFIED FOR CORNER POSTS.
- 5. WHEN THE FENCE LINE HAS A CHANGE IN DIRECTION OF 10° OR MORE, A CORNER POST SHALL BE PLACED AT THE POINT OF CHANGE. WHERE THE ANGLE OF CHANGE IS LESS THAN 10° A PULL POST SHALL BE USED.
- 6. WHERE GRADE LINE HAS A CHANGE IN SLOPE OF 10° OR MORE, A CORNER POST WITH BRACING AS REQUIRED SHALL BE PLACED. WHERE ANGLE IS LESS THAN 10° LINE POST MAY BE USED.
- 7. WHERE RIGHT-OF-WAY FENCE, TYPE 1 IS USED, THE FABRIC SHALL BE KNUCKLED SELVAGE ON TOP AND TWISTED AND BARBED SELVAGE ON BOTTOM.
- 8. PLACEMENT OF BRACED END POSTS OR CORNER POSTS WITHIN THE CLEAR ZONE SHALL BE AVOIDED.

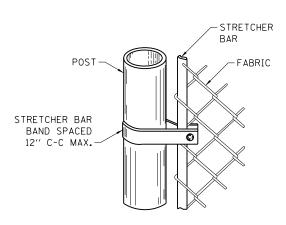
SHEET 1 OF 3



Poul Koracs

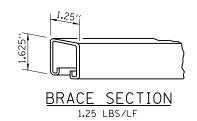
APPROVED. CHIEF ENGINEER

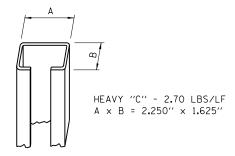
DATE 7-1-2009



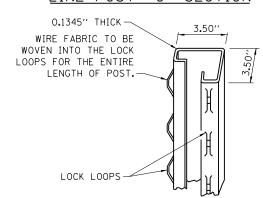
STRETCHER BARS SHALL BE GALVANIZED FLAT STEEL BAR NOT LESS THAN 1/4" × 3/4" AND THE STRETCHER BAR BANDS SHALL BE GALVANIZED FLAT STEEL BAR NOT LESS THAN 1/8" × 1" WITH A 3/8" GALVANIZED CARRIAGE BOLT.

# METHOD OF FASTENING STRETCHER BAR TO POST



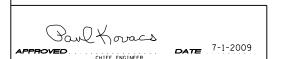


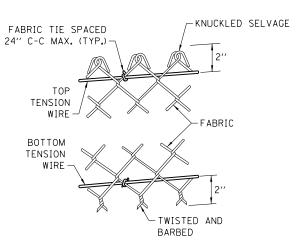
# LINE POST "C" SECTION



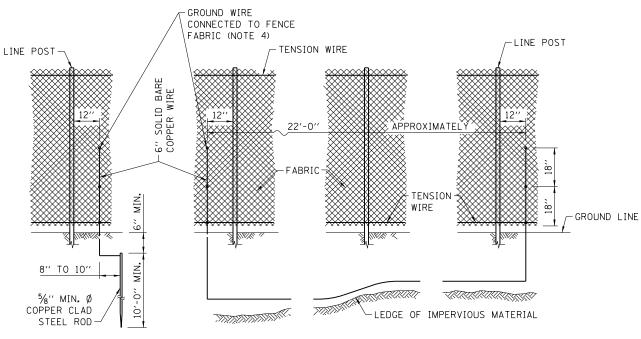
TERMINAL POST SECTION
5.10 LBS/LF

DETAILS OF ROLL FORMED SECTIONS





# METHOD OF TYING FABRIC TO TENSION WIRES



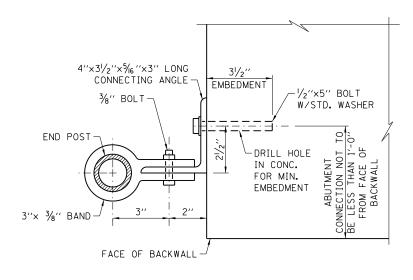
# STANDARD GROUND

#### COUNTERPOISE GROUND (ALTERNATE)

## NOTES FOR STANDARD AND COUNTERPOISE GROUND:

- 1. THE INTERVALS FOR GROUNDING CONTINUOUS FENCING SHALL NOT EXCEED 500 FEET IN URBAN AREAS AND 1000 FEET IN RURAL AREAS. FENCE ADJACENT TO A GATE SHALL BE GROUNDED A MAXIMUM DISTANCE 100 FEET EACH SIDE OF THE GATE.
- 2. FENCE CROSSING UNDER A POWER LINE SHALL BE GROUNDED, ONCE DIRECTLY UNDER THE CROSSING AND ONE ON EACH SIDE AT 25 TO 50 FEET AWAY. FENCE LOCATED DIRECTLY UNDER A TELEPHONE WIRE OR CABLE CROSSING SHALL HAVE A SINGLE GROUND.
- 3. COUNTERPOISE GROUNDS SHALL BE USED AT LOCATIONS WHERE GROUND RODS CAN NOT BE DRIVEN DUE TO IMPERVIOUS EARTH MATERIALS.
- 4. THE GROUND WIRES SHALL BE CONNECTED TO FENCE FABRIC AND GROUND ROD BY STAINLESS STEEL BOLTS AND WASHERS. THE LOWER CONNECTION OF THE GROUND WIRE SHALL BE MADE TO THE BOTTOM TENSION WIRE.

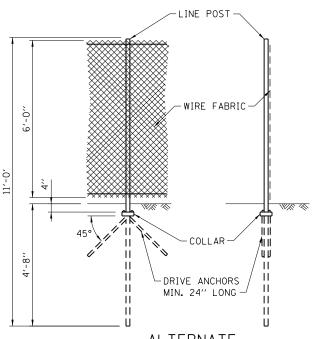
#### ELECTRICAL GROUNDING DETAILS



#### ABUTMENT CONNECTION DETAIL

#### NOTES FOR ABUTMENT CONNECTION:

1. WHEN ROLL FORMED SECTION IS USED IN LIEU OF PIPE AS END POST, THE POST SHALL BE BOLTED DIRECTLY TO THE ABUTMENT WALL WITH  $2^1\!/_2$ " x 5" BOLTS WITH STANDARD WASHERS MEETING THE APPROVAL OF THE ENGINEER.



ALTERNATE

DRIVEN LINE POST ANCHORAGE
WITH OR WITHOUT DRIVE ANCHORS

#### NOTE FOR FENCE POST:

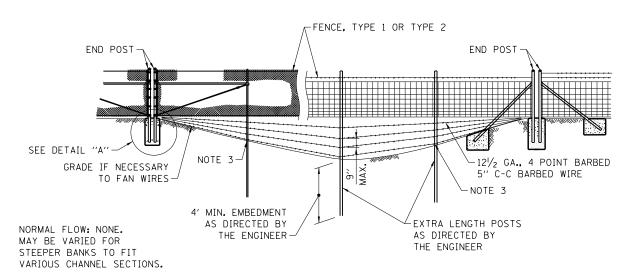
ALTERNATE DRIVEN LINE POST ANCHORAGE IS OPTIONAL. DRIVEN LINE POST ANCHORAGE WITHOUT DRIVE ANCHORS MAY BE USED IN AVERAGE TO GOOD SOIL CONDITIONS. WHEN SOIL IS WEAKER (OU < 1.25 TONS/ SO. FT.) AND STABILITY OF THE POST IS QUESTIONABLE, DRIVE ANCHORS SHALL BE USED. TYPES, SHAPES, DIMENSIONS AND COATING REQUIREMENTS OF DRIVE ANCHORS (ANCHOR BLADES AND COLLARS) FOR DIFFERENT TYPE OF POSTS SHALL BE AS RECOMMENDED BY THE MANUFACTURER.

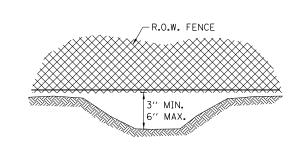
SHEET 2 OF 3



RIGHT OF WAY FENCE

STANDARD D1-05







-Ø SAME AS REGULAR FOOTING

— GROUND

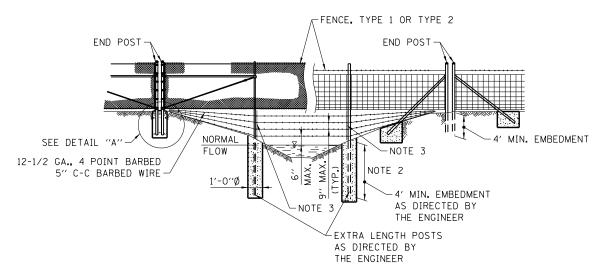
NOTE:

LINE

<u>FOOTING FOR POST WHEN</u> <u>ROCK LEDGE IS ENCOUNTERED</u>

# FENCE INSTALLATION OVER DITCH

#### STREAM CROSSING, TYPE 1



# STREAM CROSSING, TYPE 2

# END POST 4" MAX. END POST NOT CENTERED IN CONCRETE

## NOTES FOR STREAM CROSSING TYPE 1 AND TYPE 2:

- 1. THESE INSTALLATION CONDITIONS ARE TYPICAL AND ARE NOT TO BE CONSTRUCT AS REPRESENTATIVE OF ALL CONDITIONS WHICH WILL BE ENCOUNTERED. CONSTRUCTION WILL BE VARIED AS REQUIRED OR DIRECTED TO MEET FIELD CONDITIONS.
- 2. FOR STREAM CROSSING OF THE TYPE REQUIRED THE BOTTOM BARBED WIRE SHALL BE ANCHORED TO CONCRETE FOOTING OR TO HOLES DRILLED IN POSTS, AND INTERMEDIATE WIRES SHALL BE TIED TO THE BOTTOM WIRE AND TO POSTS IN AN EVENLY SPACED FASHION TO PREVENT SLIPPAGE.
- CONCRETE AND FITTINGS FOR ALL TYPES OF FENCE SHALL BE AS DETAILED FOR SIMILAR CONDITIONS PER STANDARD DRAWING.

THE FENCE FABRIC SHALL BE REPLACED BY BARBED WIRE STRANDS AT 12" MAXIMUM CENTERS BETWEEN THE END POSTS.

# DETAIL A

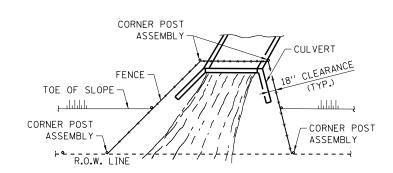


 THIS TYPE OF INSTALLATION IS TO BE USED ONLY WHEN SPECIFICALLY CALLED FOR IN THE CONTRACT PLANS.

NOTES FOR INSTALLATION AROUND HEADWALL:

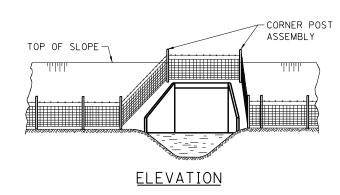
2. WHEN THE WIDTH OF THE CULVERT MAKES NECESSARY TO ANCHOR A POST TO THE TOP OF THE CULVERT, A CAST IRON SHOE OR OTHER DEVICE APPROVED BY THE ENGINEER SHALL BE USED.

# INSTALLATION AROUND HEADWALL



CONCRETE FOOTING -

PLAN AT HEADWALL



# SHEET 3 OF 3



RIGHT OF WAY FENCE

STANDARD D1-05

#### SURVEY AND ROADWAY ITEMS EROSION & SEDIMENT CONTROL, LANDSCAPING ITEMS **EXISTING PROPOSED EXISTING** PROPOSED **PROPOSED** EXISTING CLEARING & GRADING LIMITS CONSTRUCTION JOINT W/DOWEL BARS (LIMITS OF CONSTRUCTION) DIVERSION DIKE $\bowtie$ $\boxtimes$ EROSION CONTROL BLANKET BENCHMARK DRAINAGE DIVIDE DRAINAGE PATH CANTILEVER SIGN STRUCTURE OVER SEEDING CLASS B1 BUTTERFLY SIGN STRUCTURE SEDIMENT BASIN OVER SEEDING CLASS B2 • • DOUBLE COLUMN GROUND MOUNTED SIGN AGGREGATE BERM CULVERT INLET SINGLE COLUMN GROUND MOUNTED SIGN PROTECTION-STONE SEEDING CLASS A1 CULVERT INLET $\nabla$ SPAN TYPE SIGN STRUCTURE PROTECTION-FENCE DB SEEDING CLASS A2 DEWATERING BASIN TRIPLE COLUMN GROUND MOUNTED SIGN $\begin{bmatrix} 0 & 0 & 0 \end{bmatrix}$ - FIPB -FILTER FABRIC SEEDING CLASS A3 000000000 INLET PROTECTION, BASKET TYPE RUMBLE STRIP FILTER FABRIC DRAINAGE AND UTILITY ITEMS; ROADWAY LIGHTING AND SIGNS INLET PROTECTION, COVER TYPE SEEDING CLASS A4 — FB —— FB — FLOTATION BOOM PROPOSED EXISTING (C) INITIAL CONSTRUCTION ITEM SEEDING CLASS A5 -RIP-BOX CULVERT WITH HEADWALL RECTANGULAR INLET PROTECTION CABLE IN DUCT W/O GROUND SEEDING CLASS A6 LOW POINT TEMPORARY ROCK CHECK DAM OVERHEAD ELECTRICAL SEEDING CLASS D1 TEMPORARY DITCH CHECK OVERHEAD TELEPHONE PIPE CULVERT SODDING (SALT TOLERANT) Œ LAKE OR POND **(1)** QUARRY SEDIMENT BASIN TEMPORARY GROUND COVER STREAM SWAMP \* \* \* \* \* \* \* SILT FENCE $\langle A \rangle$ CABLE OR CONDUIT TAG ——SSF—— SUPER SILT FENCE TURF REINFORCEMENT MAT [E] $[\mathsf{E}]$ ELECTRICAL MANHOLE STABILIZED CONSTRUCTION ENTRANCE []LD LIGHT-DUTY BOX STONE OUTLET STRUCTURE SEDIMENT TRAP ROADWAY LUMINAIRE STREAM DIVERSION <u>\_\_\_\_\_</u> TEMPORARY PIPE SLOPE DRAIN M TEMPORARY RIPRAP STEEL TOWER -**√-**TS-**√-**[T]T TEMPORARY SWALE TELEPHONE MANHOLE 0 TREES AND STUMP UNDERPASS LUMINAIRE TREE PROTECTION SHEET 1 OF 3 0 WATER POINT [W] W WATERMAIN VALVE VAULT Illinois TEMPORARY STREAM CROSSING $\bigcirc$ *Tollway* WATER WELL $\otimes$ WOOD POLE DATFREVISIONS SYMBOLS AND PATTERNS REVISED SYMBOL & PATTERNS ADDED NEW SYMBOLS Paul Koracs 3-11-2015 ADDED NEW SYMBOL 3-31-2016 UPDATED DITCH CHECK SYMBO

**DATE** 7-1-2009

CHIEF ENGINEER

STANDARD D2-04

# ELECTRICAL AND MECHANICAL ITEMS

				EXISTING	PROPOSED	
	HOME RUN TO PANEL AS NOTED	G	STANDBY GENERATOR	——— А ———	A	COMPRESSED AIR (A)
<b>⊗</b>	INDICATES CIRCUIT TURNING DOWN  INDICATES CIRCUIT TURNING UP	A   -P	PANEL CIRCUIT BREAKER	AR	AR	ACID RESISTANT WASTE OR DRAIN
•	GROUND ROD	С	MECHANICALLY HELD LIGHTING COIL	ARV	ARV	ACID RESISTANT VENT
	GROUNDING TRIAD	CR	CONTROL RELAY COIL	DS	DS	STORM SEWER (DOWNSPOUT)
( <del>€) (</del> €)		\$	SINGLE-POLE SWITCH	G		GAS LINE
V/v	TRANSFORMER	$\Leftrightarrow$	DUPLEX RECEPTACLE	——— нс ———	——— нс ———	HOT GAS BYPASS LINE (HG)
	MOTOR	© c	4P, 4W, WEATHERPROOF RECEPTACLE WITH SPRING DOOR, BACK BOX, & ANGLE ADAPTER	——— ннwr ———	——— нн <b>w</b> r ———	HEATING HOT WATER RETURN (HHWR)
O O ATSAA	AUTOMATIC TRANSFER SWITCH (ATS)	$\bigcirc$ B	4P, 4W, WEATHERPROOF RECEPTACLE WITH SPRING DOOR & BACK BOX	——————————————————————————————————————	HHWS	HEATING HOT WATER SUPPLY (HHWS)
JB OR J	JUNCTION BOX  DISCONNECT SWITCH	GFI	DUPLEX RECEPTACLE WITH GROUND FAULT PROTECTION	IA	IA	DRY COMPRESSED AIR (IA-INSTRUMENT AIR)
<u> </u>		А	CONTROL BUILDING LIGHTING 1' X 4' INDUSTRIAL FLUORESCENT FIXTURE, PORCELAIN REFLECTOR, ELECTRONIC BALLAST.	——— Р ———	P	PROCESS WATER ("P" WATER) LINE
A		В	COMPACT WALL-MOUNTED LOW WATTAGE HPS FIXTURE WITH WIRE GUARD & SINGLE FACTORY INSTALLED FUSE	PW	——— РW ———	PROTECTED WATER OR PLANT WATER (PW)
A	CIRCUIT BREAKER	c 🖳	EMERGENCY LIGHT UNIT WITH 2-6 VOLT, 12 WATT SEALED BEAM HALOGEN LAMPS WITH WALL MOUNTING BRACKET	RD	RD	REFRIGERANT DISCHARGE LINE (RD)
A	MANUAL TRANSFER SWITCH	D	LANE LIGHTING - HEAVY DUTY ALUMINUM HOUSING WITH ENCLOSED REFLECTOR & TEMPERED GLASS LENS W/AUTO	RS	RS	REFRIGERANT SUCTION LINE (RS)
sw.		\\-	REGULATOR BALLAST. ASYMMETRIC PATTERN  WIRE	v	v	VENT LINE (V)
WH)	SELF CONTAINED UTILITY METERING	<u> </u>	CONDUIT			

SHEET 2 OF 3



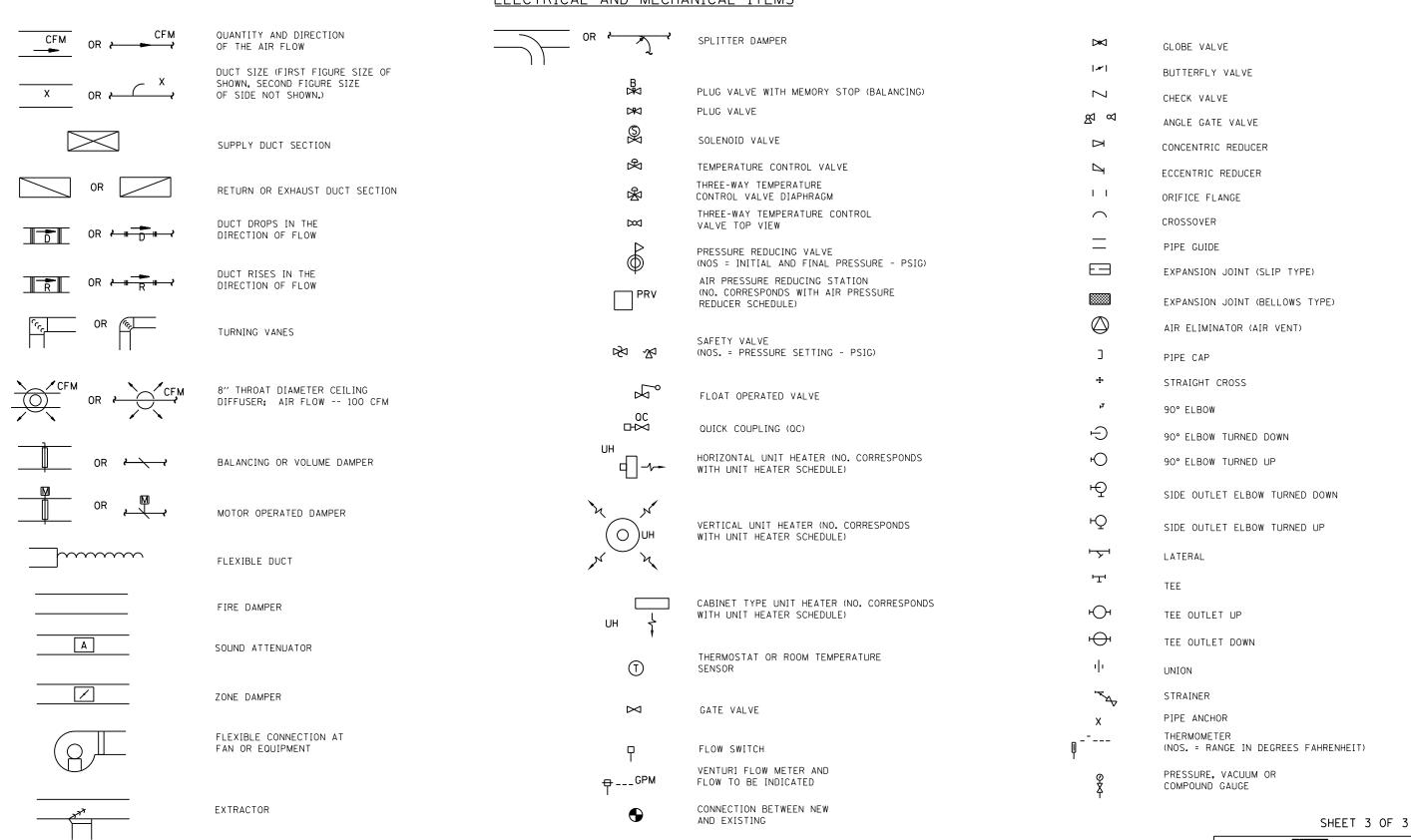
# NOTE:

ALL SYMBOLS AND PATTERNS ON THIS DRAWING ARE PROPOSED UNLESS OTHERWISE NOTED.

SYMBOLS AND PATTERNS

STANDARD D2-04

#### ELECTRICAL AND MECHANICAL ITEMS





#### NOTE:

ALL SYMBOLS AND PATTERNS ON THIS DRAWING ARE PROPOSED UNLESS OTHERWISE NOTED.

SYMBOLS AND PATTERNS

STANDARD D2-04

	PERI	MANENT DELINEATIO	N SPACING		
		MAII	NLINE	R.	AMP
	REFLECTORS	TANGENT	CURVE	TANGENT	CURVE
*	GUARDRAIL	100′	100′	100′	100' (R >= 1,050') 50' (R < 1,050')
*	BARRIER WALL (DOUBLE FACE)	100′	100′	100′	100' (R >= 1,050') 50' (R < 1,050')
*	BARRIER WALL (SINGLE FACE)	100′	100′	100′	100' (R >= 1,050') 50' (R < 1,050')
	SHOULDER NARROWING	3 @ 15′	3 @ 15′	3 @ 15′	3 @ 15′
	BRIDGE APPROACHES	3 @ 15′	3 @ 15′	3 @ 15′	3 @ 15′
*	BRIDGE PARAPET	50′	50′	50′	50′
*	NOISE ABATEMENT WALL (CRASH WORTHY)	100′	100′	100′	100' (R >= 1,050') 50' (R < 1,050')
	ROADWAY DELINEATORS	RA	AMP		
		TANGENT	CURVE	TANGENT	CURVE
	  POST MOUNTED DELINEATOR	200′	200′	200′	TABLE A

TEMPORARY DELINEATION SPACING								
	TANGENT	REVERSE CURVE	SHIFT	TAPER				
TEMPORARY CONCRETE BARRIER	50′	25′	25′	25′				

100'

NΑ

NA

100'

*	WHEN	ADJACENT	SHOULDER	IS	USED	AS	А	TRAVELED	LANE,	USE	SPACING	REQUIREMENTS	ΑS	SHOWN FO	)R
	TEMP(	DRARY DEL:	INEATION.												

TAB	LE A
REFLECTOR SPACING	ON RAMP-CURVES
RADIUS OF CURVE (FT.)	SPACING ALONG CURVE (FT.)
LESS THAN 1050	50
1050-1299	100
1300-1999	125
2000-2999	150
3000-3999	175
MORE THAN 3999	200

#### GENERAL NOTES:

EMERGENCY TURNAROUNDS DELINEATION-THE FOLLOWING DELINEATION SHOULD BE INSTALLED ON THE LEFT SIDE OF THE PAVEMENT APPROACHING EMERGENCY TURNAROUNDS.

- A. ONE-HALF OF A MILE IN ADVANCE OF THE EMERGENCY TURNAROUNDS ONE WHITE REFECTOR UNIT OVER THREE AMBER REFLECTOR UNITS.
- B. ONE-FOURTH OF A MILE IN ADVANCE OF THE EMERGENCY TURNAROUNDS ONE WHITE REFLECTOR UNIT OVER TWO AMBER REFLECTOR UNITS.
- C. AT A POINT NEAR THE INTERSECTION OF THE EDGE OF THE LEFT SHOULDER AND NEAR EDGE OF THE EMERGENCY TURNAROUNDS ONE WHITE REFLECTOR UNIT OVER ONE AMBER REFLECTOR UNIT.

#### NOTES FOR ROADWAY DELINEATORS, POST MOUNTED INSTALLATION:

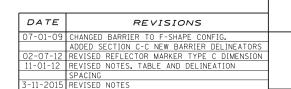
- 1. A. MAINLINE-SINGLE WHITE REFECTOR UNITS SHALL BE PLACED CONTINUOUSLY ON THE RIGHT AND SINGLE AMBER REFLECTOR UNITS SHALL BE PLACED ON THE LEFT ON MAIN LINE SECTIONS WITHOUT BARRIER WALL.
  - B. RAMPS-SINGLE REFLECTOR UNITS SHALL BE PLACED ON THE OUTSIDE OF ALL CURVED SECTIONS OF RAMPS, SINGLE WHITE SHALL BE PLACED ON THE RIGHT SIDE AND AMBER ON THE LEFT SIDE. THE DELINEATORS SHALL BE OVERLAPPED FOR A SHORT DISTANCE TO CLEARLY INDICATE WHERE DELINEATION ON ONE SIDE OF THE RAMP ENDS AND DELINEATION ON THE OTHER SIDE APPEARS.
  - C. DOUBLE WHITE REFLECTOR UNITS SHALL BE PLACED ON THE RIGHT AT ALL ACCELERATION AND DECELERATION LANES.
- 2. REFLECTORS SHALL BE MOUNTED ON SUPPORTS SUCH THAT THE TOP OF REFLECTORS IS FOUR FEET ABOVE THE ROADWAY EDGE AND TWO FEET OUTSIDE THE OUTER EDGE OF THE PAVED SHOULDER OR TWO FEET MINIMUM AND SIX FEET MAXIMUM OUTSIDE THE BACKS OF CURBS OR GUTTERS.
- 3. IN ALL CASES, THE COLOR OF THE REFLECTORS SHALL BE THE SAME AS THE ADJACENT EDGE LINE EXCEPT AS SPECIFIED IN GENERAL NOTES.
- 4. POST MOUNTED REFLECTORS SHALL BE PLACED CONTINUOUSLY AS NOTED ABOVE IN CONJUNCTION WITH GUARDRAIL INSTALLED.
- 5. THE PLACEMENT OF ROADWAY DELINEATOR "CIRCULAR REFLECTORS" SHALL BE USED FOR ALL MINOR PROJECTS WHICH HAVE A LENGTH OF LESS THAN 5 MILES. THE PLACEMENT OF ROADWAY DELINEATOR "RECTANGULAR REFLECTORS" SHALL BE USED FOR ALL MAJOR PROJECTS WHICH HAVE A LENGTH GREATER THAN 5 MILES. ALL ROADWAY DELINEATORS WITHIN A ROADWAY SEGMENT SHALL BE OF THE SAME TYPE.

#### NOTES FOR GUARDRAIL AND BARRIER WALL REFLECTOR:

1. REFLECTORS TYPE B AND TYPE C SHALL HAVE REFLECTIVE SURFACE ON ONE SIDE ONLY.

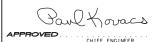
SHEET 1 OF 3

<sup>1</sup> Illinois Tollway



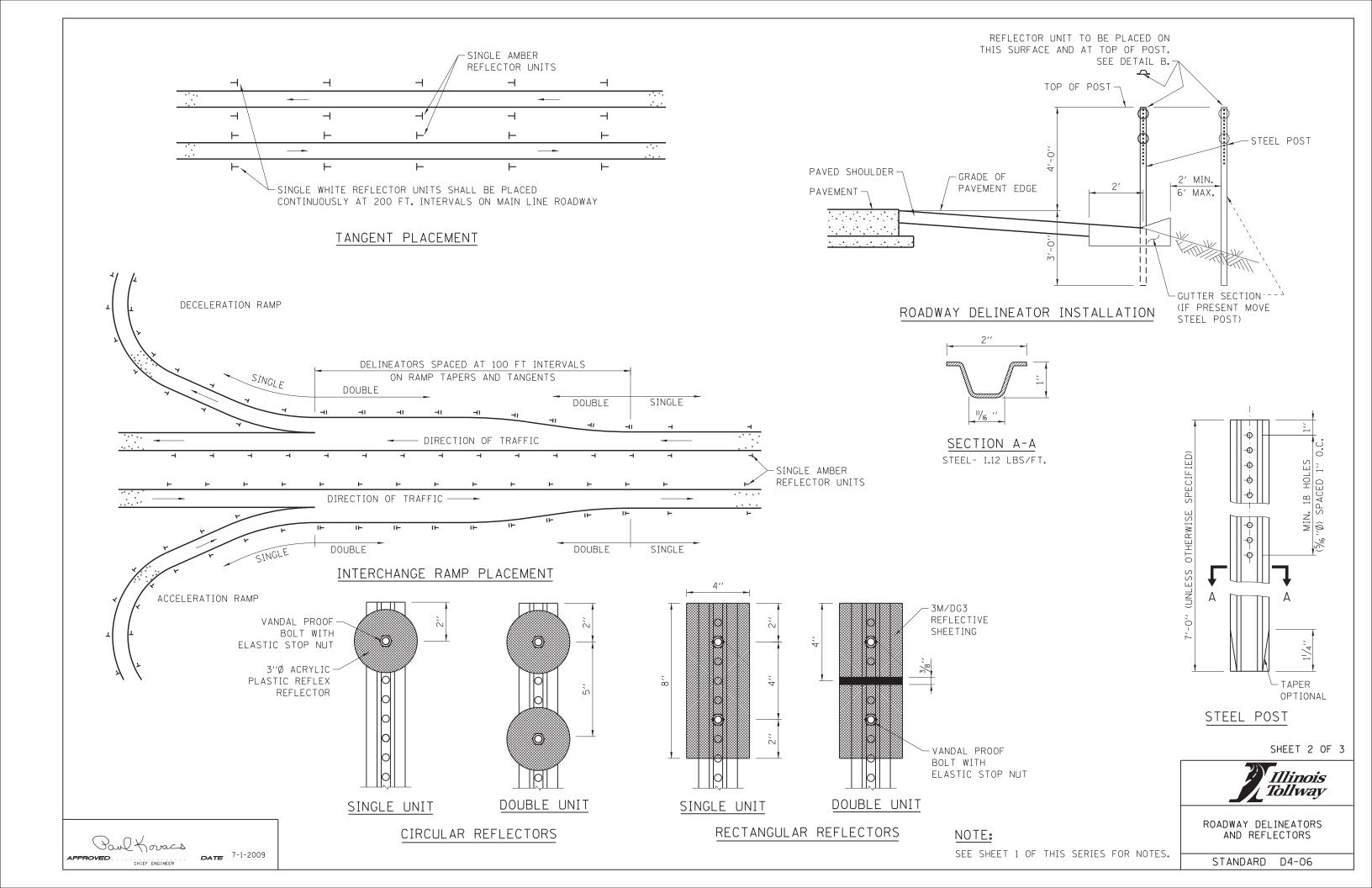
REVISED DELINEATOR ATTACHMENT TO POST REVISED PERM. DELINEATION SPACING TABLE ROADWAY DELINEATORS
AND REFLECTORS

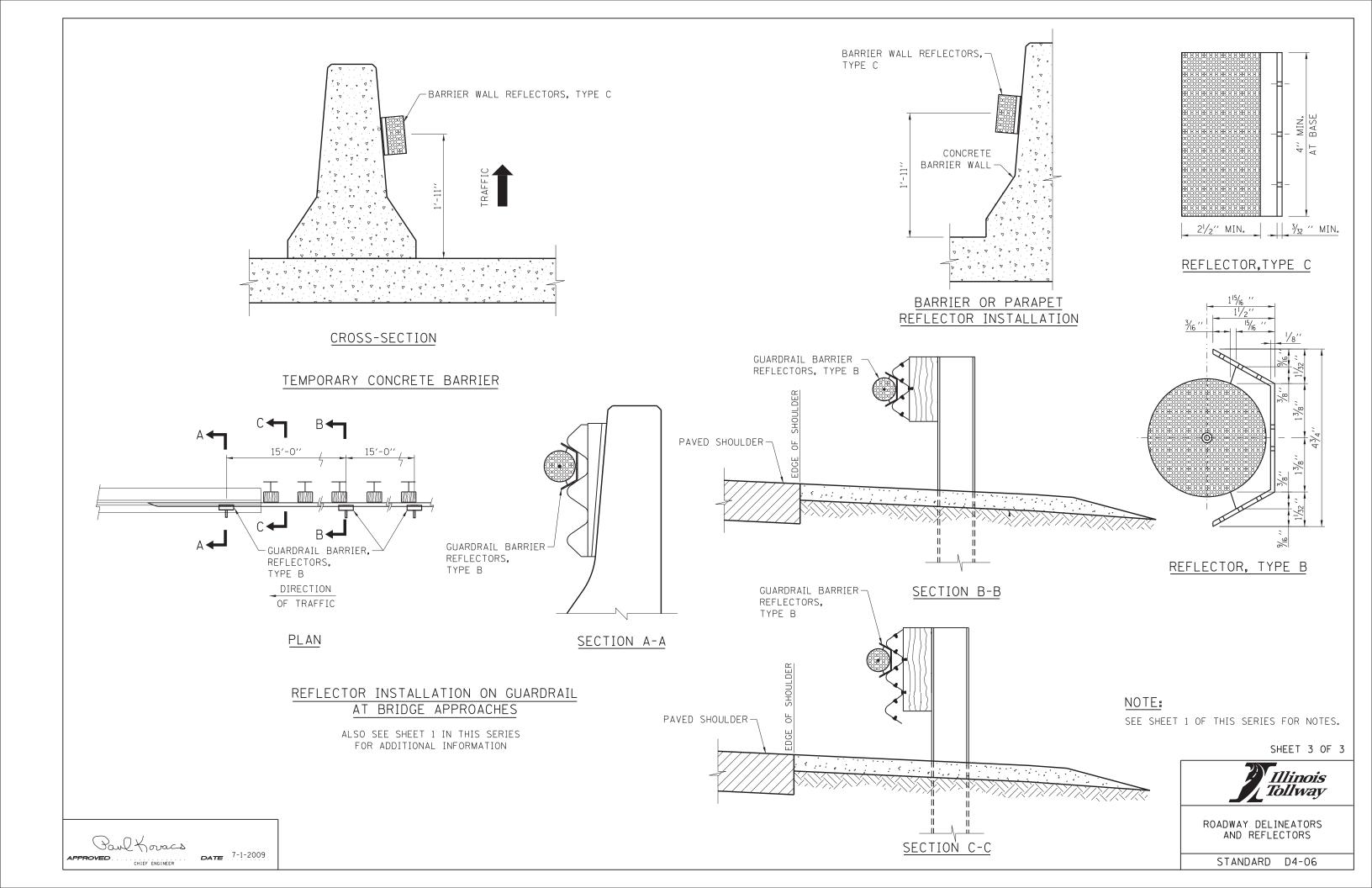
STANDARD D4-06

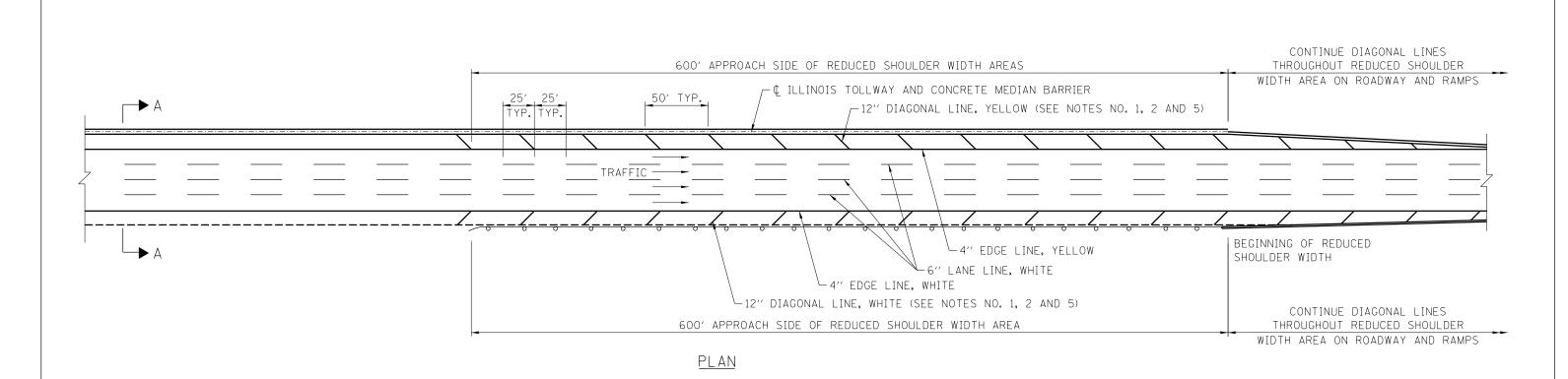


POST MOUNTED DELINEATOR

(RAMP TAPERS AND TANGENTS)







#### ¢ ILLINOIS TOLLWAY 49'-0" OUTSIDE MEDIAN SHOULDER SHOULDER 12'-0" 12'-0'' 12'-0'' 13'-0'' 4" SOLID YELLOW 4" SOLID WHITE (GROOVED) -(GROOVED) 6" WHITE SKIP DASH (GROOVED)

SECTION A-A

ROADWAY AND SHOULDER STRIPING - NEW CONSTRUCTION

#### GENERAL NOTES:

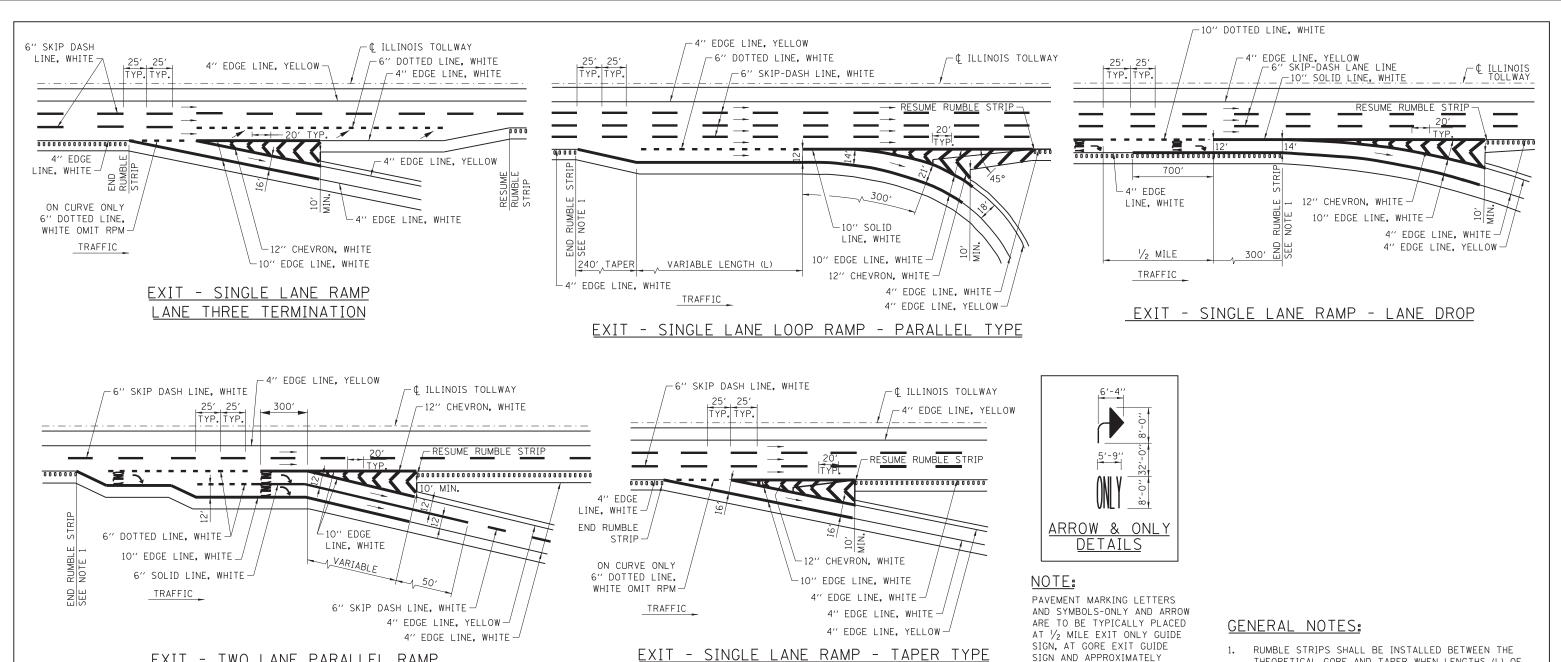
- 1. DIAGONAL SHOULDER STRIPING REQUIRED WHERE THE SHOULDER WIDTH IS LESS THAN STANDARD.
- 2. ROADWAY MARKING MATERIALS TO BE USED ON FINISHED CONCRETE SURFACE AND ASPHALT SURFACE SHALL BE AS SHOWN ON THE PLANS.
- 3. WHERE THE GUARDRAIL ENCROACHES ON THE SHOULDER THE DIAGONAL MARKINGS SHALL EXTEND AS CLOSE TO THE FACE OF THE RAIL AS POSSIBLE.
- 4. ALL PERMANENT LANE LINES AND EDGE LINES SHALL BE GROOVED, ON ROADWAY SURFACES, UNLESS OTHERWISE NOTED.
- 5. DIAGONAL STRIPING SHALL BE SURFACE APPLIED.
- 6. GORE STRIPING (CHEVRON) SHALL BE SURFACE APPLIED.
- 7. ALL LANE LINES AND EDGE LINES SHALL BE SURFACE APPLIED ON BRIDGES.
- 8. PAVEMENT MARKINGS SHALL NOT BE GROOVED AT THE CASH SIDE OF MAINLINE TOLL PLAZAS OR THE OPEN ROAD TOLLING (ORT), 100' CONTINUOUSLY REINFORCED CONCRETE (CRC) PAVEMENT SECTION OF MAINLINE UNDER MONOTUBES.

		Illinois Tollway
DATE	REVISIONS	
7-01-09	ADDED LINE GROOVING NOTES	PERMANENT PAVEMENT
2-07-12	REVISED NOTES	MARKINGS
11-01-12	REVISED EDGELINE OFFSET, REVISED NOTES	
3-31-14	REVISED NOTES	
3-31-16	REVISED NOTES	STANDARD D5-06
		JIANDAND DO 00

POUL KOVACS

APPROVED.... CHIEF ENGINEER

DATE 7-1-2009

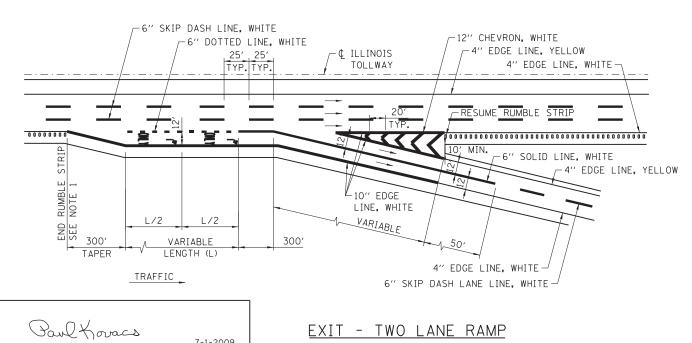


# EXIT - TWO LANE PARALLEL RAMP

**DATE** 7-1-2009

CHIEF ENGINEER

#### EXIT - SINGLE LANE RAMP - TAPER TYPE



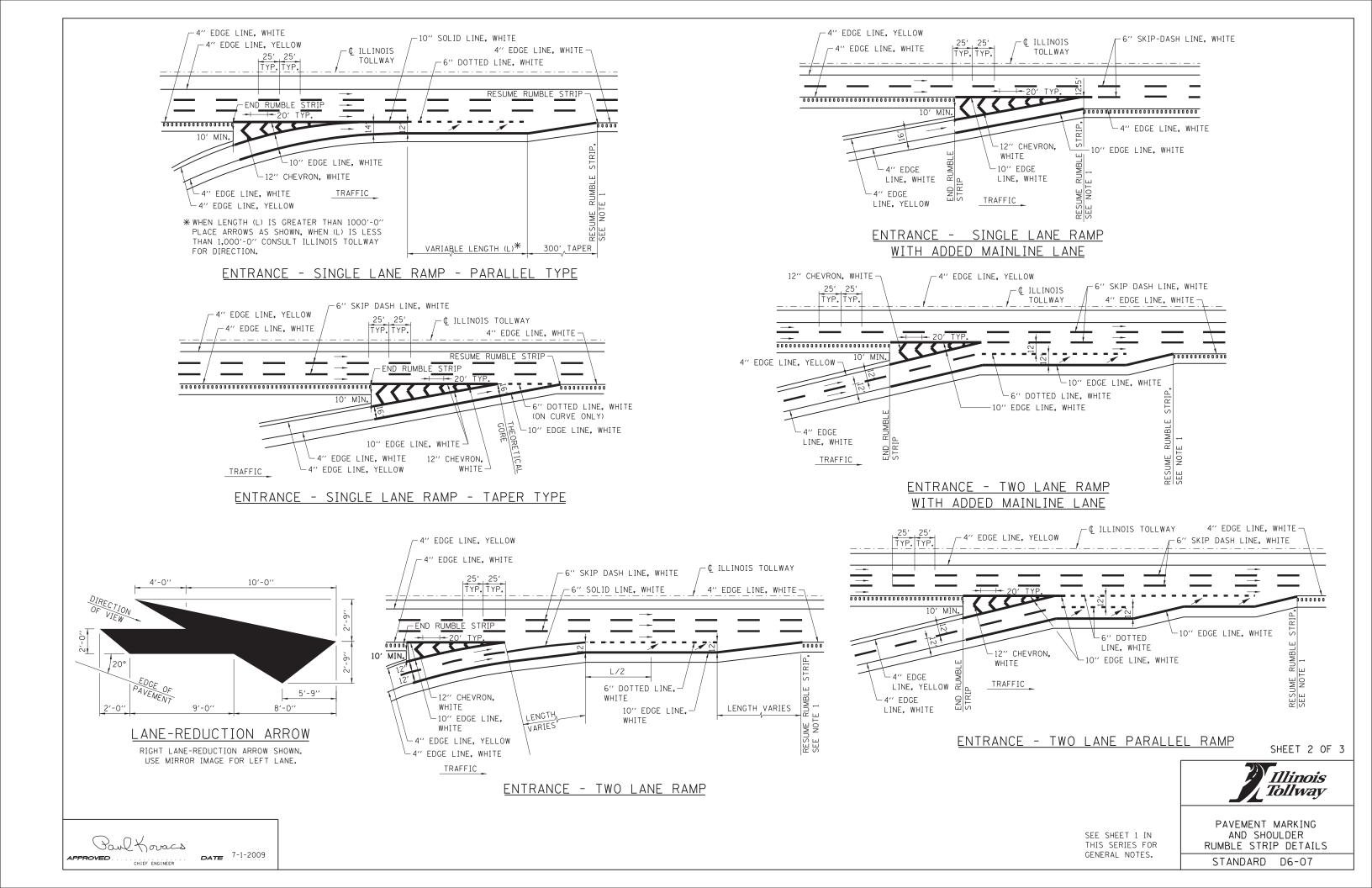
- RUMBLE STRIPS SHALL BE INSTALLED BETWEEN THE THEORETICAL GORE AND TAPER WHEN LENGTHS (L) OF AUXILIARY LANES, ACCELERATION LANES OR DECELERATION LANES, ARE GREATER THAN 1000'.
- ROADWAY MARKING MATERIALS TO BE USED ON FINISHED CONCRETE SURFACE AND ASPHALT SURFACE SHALL BE AS SHOWN ON THE PLANS.
- 3. ALL LANE LINES AND EDGE LINES SHALL BE GROOVED.
- GORE STRIPING (CHEVRON) SHALL BE SURFACE APPLIED.
- LETTERS AND SYMBOL MARKING SHALL BE SURFACE APPLIED.
- 6. DOTTED LINES SHALL CONSIST OF 3' LINE AND 9' GAPS.

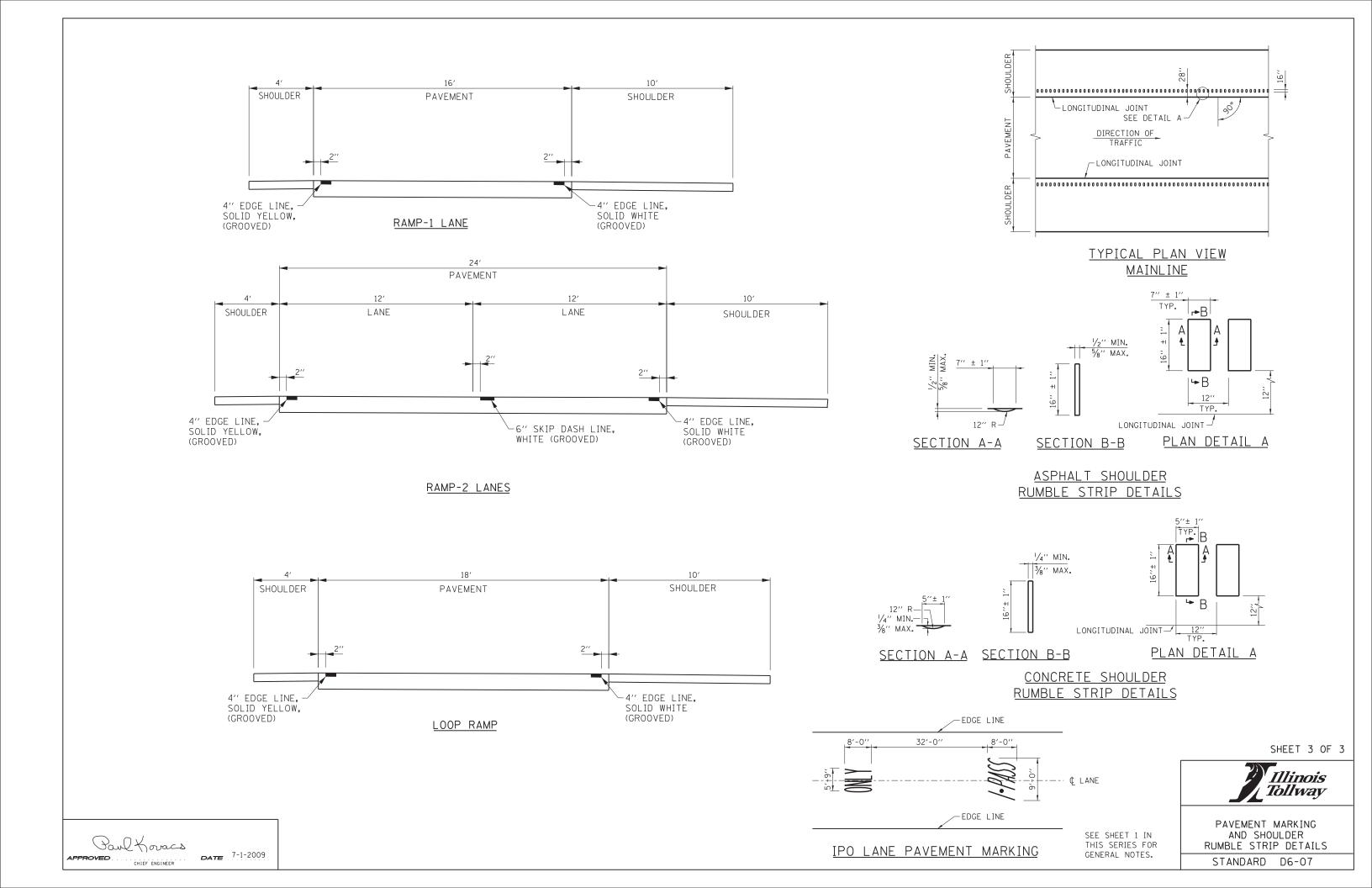
SHEET 1 OF 3

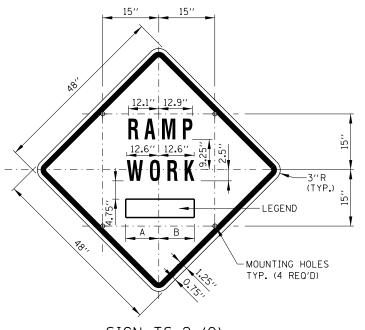
DATE	REVISIONS	Illinois Tollway
11-01-12	REVISED NOTES AND ADDED DOTTED LINE	
03-01-13	REVISED SINGLE LANE LOOP RAMP DETAILS	DAVENENT MARKEN
03-31-14	ADDED LANE REDUCTION MARKINGS	PAVEMENT MARKING
3-11-2015	REVISED DETAILS, ADDED LANE-REDUCTION	AND SHOULDER
	ARROWS AND SHEET 3	RUMBLE STRIP DETAILS
3-31-2016	REVISED NOTES, ADDED IPO PAVEMENT MARKING	RUMBLE SIRIF DETAILS
	DETAIL.	STANDARD D6-07
3-31-2017	REVISED NOTES	3 I ANDARD DO-01

HALFWAY BETWEEN THE TWO.

EXIT - TWO LANE RAMP







SIGN NO.	LEGEND	Α	В
TS-2A	AHEAD	15.50"	15.50"
TS-2B	500 FT	14.25''	15.13"
TS-2C	1000 FT	14.88′′ <i>L</i> 2	15.75" L2
TS-2D	1500 FT	14.88" L2	15.75" L2
TS-2E	√ <sub>2</sub> MILE	15.75" L3	15.75" L3
TS-2F	1 MILE	13.06′′	13.06′′

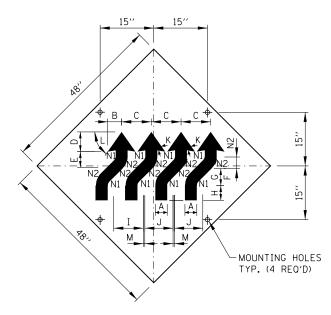
#### SIGN TS-2 (0)

COLOR: BACKGROUND - FLUORESCENT ORANGE (0) BORDER AND SYMBOL - BLACK

SIZE: 48"×48"

LETTERING: 7" FEDERAL SERIES D

MOUNTING HOLES: 16" DIA., 4 HOLES SPACED AS SHOWN

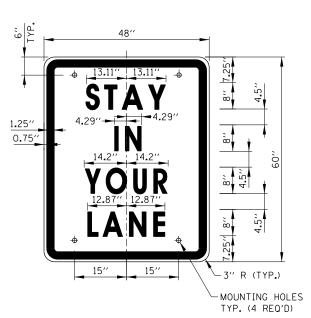


Α	41/2"
В	4 <sup>1</sup> / <sub>2</sub> '' 5 <sup>3</sup> / <sub>4</sub> ''
С	121/2"
D	73/4′′
Ε	61/2"
F	4 <sup>1</sup> / <sub>2</sub> '' 6 <sup>1</sup> / <sub>2</sub> ''
G	61/2"
Н	6′′
I	123/4′′
J	12''
K	45°
L	55°
М	3/4′′
N1	2''
N2	61/2′′

#### SIGN W1-4dR (0)

COLOR: BACKGROUND-FLUORESCENT ORANGE (0) TYPE A REFLECTIVE SHEETING PER STANDARD SPECIFICATIONS (\* A) BORDER AND LETTERS-BLACK

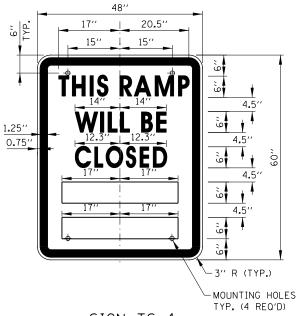
MOUNTING HOLES:  $\frac{7}{16}$ " DIA., 4 HOLES SPACED AS SHOWN.



## SIGN TS-3

COLOR: BACKGROUND - WHITE (REFLECTORIZED) ( \*A) BORDER AND LETTERS - BLACK

LETTERING: LEGEND - 8" FEDERAL SERIES D MOUNTING HOLES:  $\frac{7}{6}$ " DIA., 4 HOLES, SPACED AS SHOWN



#### SIGN TS-4

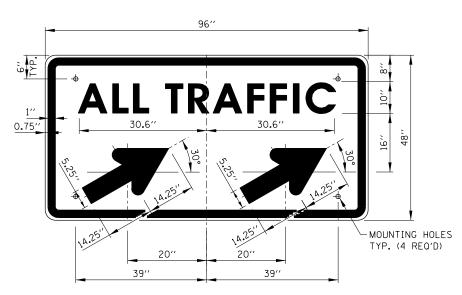
COLOR: BACKGROUND - WHITE (REFLECTORIZED)( \* A) BORDER AND LETTERS - BLACK

SIZE: 48"x60"

LETTERING: LEGEND - 6" FEDERAL SERIES C MOUNTING HOLES: 1/6" DIA., 4 HOLES, SPACED AS SHOWN

#### RAMP CLOSURE ADVANCE INFORMATION SIGN

THE VARIABLE MESSAGE WITH DATES FOR THE BOTTOM TWO LINES SHALL BE DETERMINED BY THE ENGINEER AND GIVEN TO THE CONTRACTOR BEFORE THE REQUIRED FIELD ERECTION DATE.



#### SIGN TS-5a & TS-5b

COLOR: BACKGROUND - WHITE (REFLECTORIZED)( \* A) BORDER AND LETTERS - BLACK

ARROW - BLACK

SIZE: 96"×48"

LETTERING: 10" FEDERAL SERIES D

MOUNTING HOLES:  $\frac{7}{16}$ " DIA., 4 HOLES, SPACED AS SHOWN NOTE: SIGN TS-5a IS SHOWN, SUBSTITUTE

LEGEND "#" FOR "##" FOR SIGN TS-5b

DATE

REVISIONS

DED SIGN COLOR DESIGNATION
LETED SIGN TS-1

REVISED FINE SIGN NUMBER AND DDED LED SPEED LIMIT DISPLAY REVISED NOTES

REVISED END WZSL SIGN COLOR

#### NOTES:

- ALL LETTERING IS DESIGNATED BY SIZE AND SERIES IN ACCORDANCE WITH THE LATEST EDITION OF "STANDARD ALPHABETS FOR HIGHWAY SIGNS AND PAVEMENT MARKINGS" AS PUBLISHED BY THE U.S. DEPARTMENT OF TRANSPORTATION. LETTERING SPACING SHALL BE IN ACCORDANCE WITH THIS GUIDE EXCEPT WHERE NOTED.
- 2. SYMBOLS AND ARROWS SHALL CONFORM TO THE DETAILS SHOWN IN THE LATEST EDITION OF "STANDARD HIGHWAY SIGNS" AS PUBLISHED BY THE U.S. DEPARTMENT OF TRANSPORTATION.
- 3. SEE THE CONTRACT REQUIREMENTS FOR ADDITIONAL NOTES AND SPECIFICATIONS. FLUORESCENT ORANGE REFLECTIVE SHEETING PER THE STANDARD SPECIFICATIONS.
  - (\*A) REFLECTIVE SHEETING PER THE STANDARD SPECIFICATIONS.
- 4. DIMENSIONS INDICATED THUS L ARE BASED ON A REDUCTION IN STANDARD LETTERING SPACING AS SHOWN BELOW:
  - L1 SPACING REDUCED BY 25%
  - L2 SPACING REDUCED BY 40%
  - L3 SPACING REDUCED BY 50%

SHEET 1 OF 2

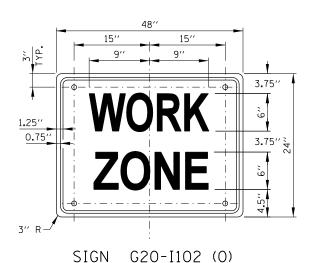


CONSTRUCTION SIGNS

STANDARD E1-06



DATE 5-1-2009

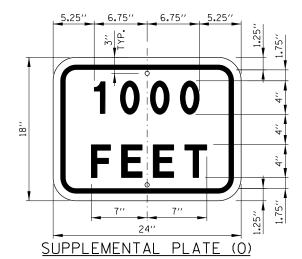


COLOR: BACKGROUND - FLUORESCENT ORANGE (0) BORDER AND LETTERS - BLACK

SIZE: 48"x24"

LETTERING: 6" FEDERAL SERIES C

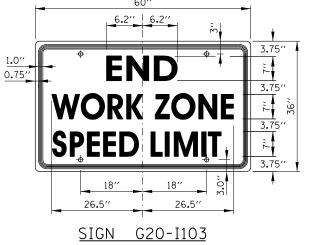
MOUNTING HOLES:  $\frac{1}{16}$ " DIA., 4 HOLES SPACED AS SHOWN



BACKGROUND - FLUORESCENT ORANGE (0) BORDER AND LETTTERS - BLACK

SIZE: 24"×18"

LETTERING: 4" FEDERAL SERIES D MOUNTING HOLES: 1/16" DIA., 2 HOLES SPACED AS SHOWN

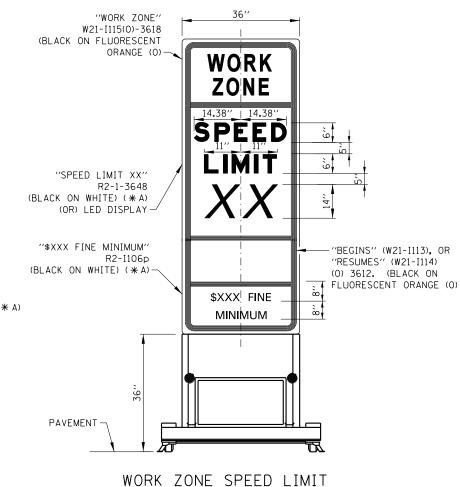


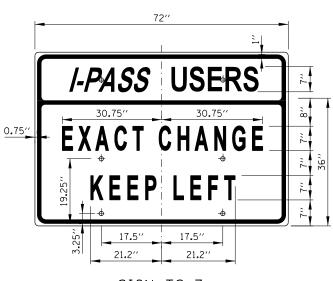
COLOR: BACKGROUND - WHITE (REFLECTORIZED) (\* A) BORDER AND LETTERS - BLACK

SIZE: 60"x36"

LETTERING: 6" FEDERAL SERIES C

MOUNTING HOLES: 7/6" DIA., 4 HOLES SPACED AS SHOWN





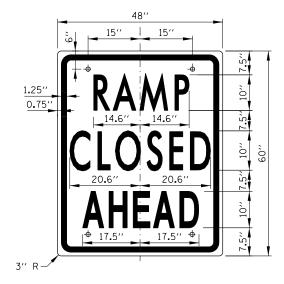
# SIGN TS-7

COLOR: BACKGROUND - WHITE (REFLECTORIZED) (\* A) BORDER AND LETTTERS - BLACK

SIZE: 72"x36"

LETTERING: 7" FEDERAL SERIES C

MOUNTING HOLES: 1/16" DIA., 4 HOLES SPACED AS SHOWN



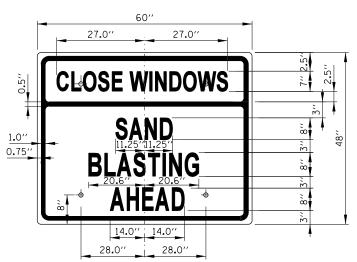
#### SIGN TS-9

COLOR: BACKGROUND - WHITE (REFLECTORIZED) (\* A)

BORDER AND LETTTERS - BLACK

SIZE: 48"x60"

LETTERING: 10" FEDERAL SERIES C
MOUNTING HOLES: 76" DIA., 4 HOLES SPACED AS SHOWN

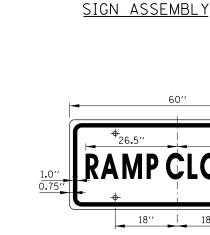


## SIGN TS-10 (0)

COLOR: BACKGROUND - FLUORESCENT ORANGE (0) BORDER AND LETTTERS - BLACK

SIZE: 60"x48"

LETTERING: 8" FEDERAL SERIES C, 7" FEDERAL SERIES B MOUNTING HOLES: 76" DIA., 4 HOLES SPACED AS SHOWN



# SIGN TS-6

COLOR: BACKGROUND - WHITE (REFLECTORIZED) (\* A) BORDER AND LETTTERS - BLACK

SIZE: 60"x24" LETTERING: 8" FEDERAL SERIES C

MOUNTING HOLES: 1/6" DIA., 4 HOLES SPACED AS SHOWN

SHEET 2 OF 2



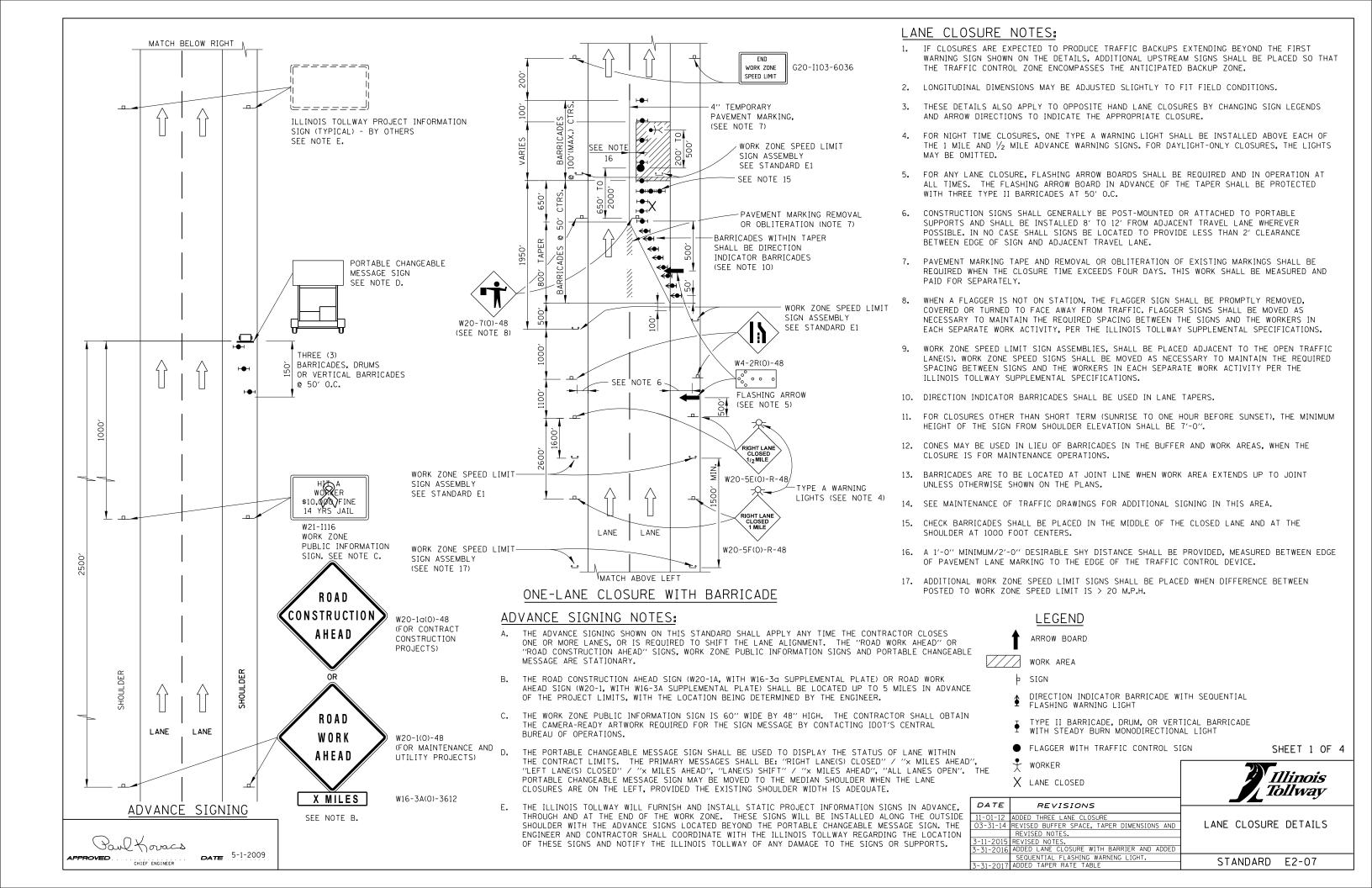
CONSTRUCTION SIGNS

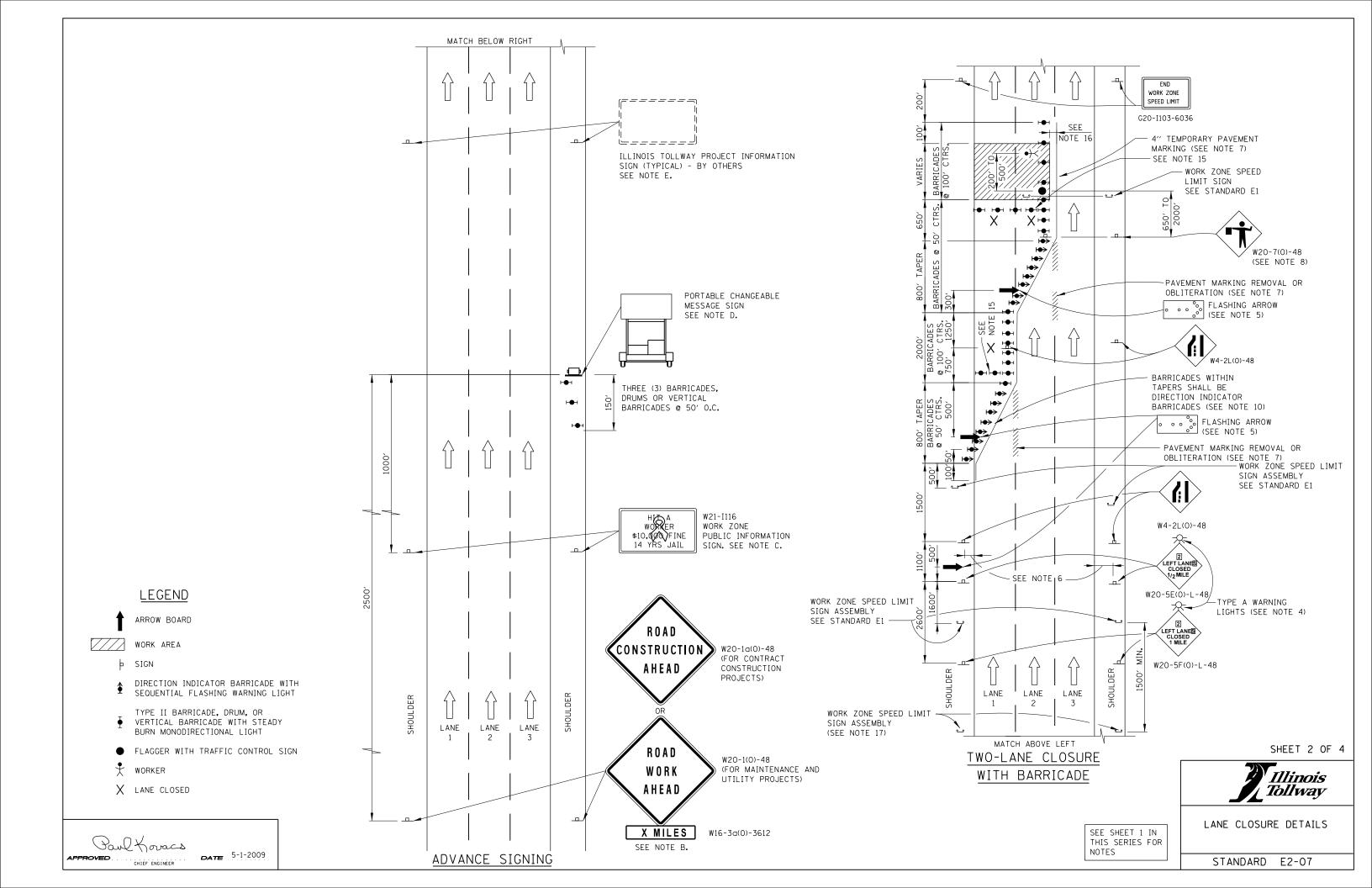
SEE SHEET 1 OF THIS SERIES FOR NOTES.

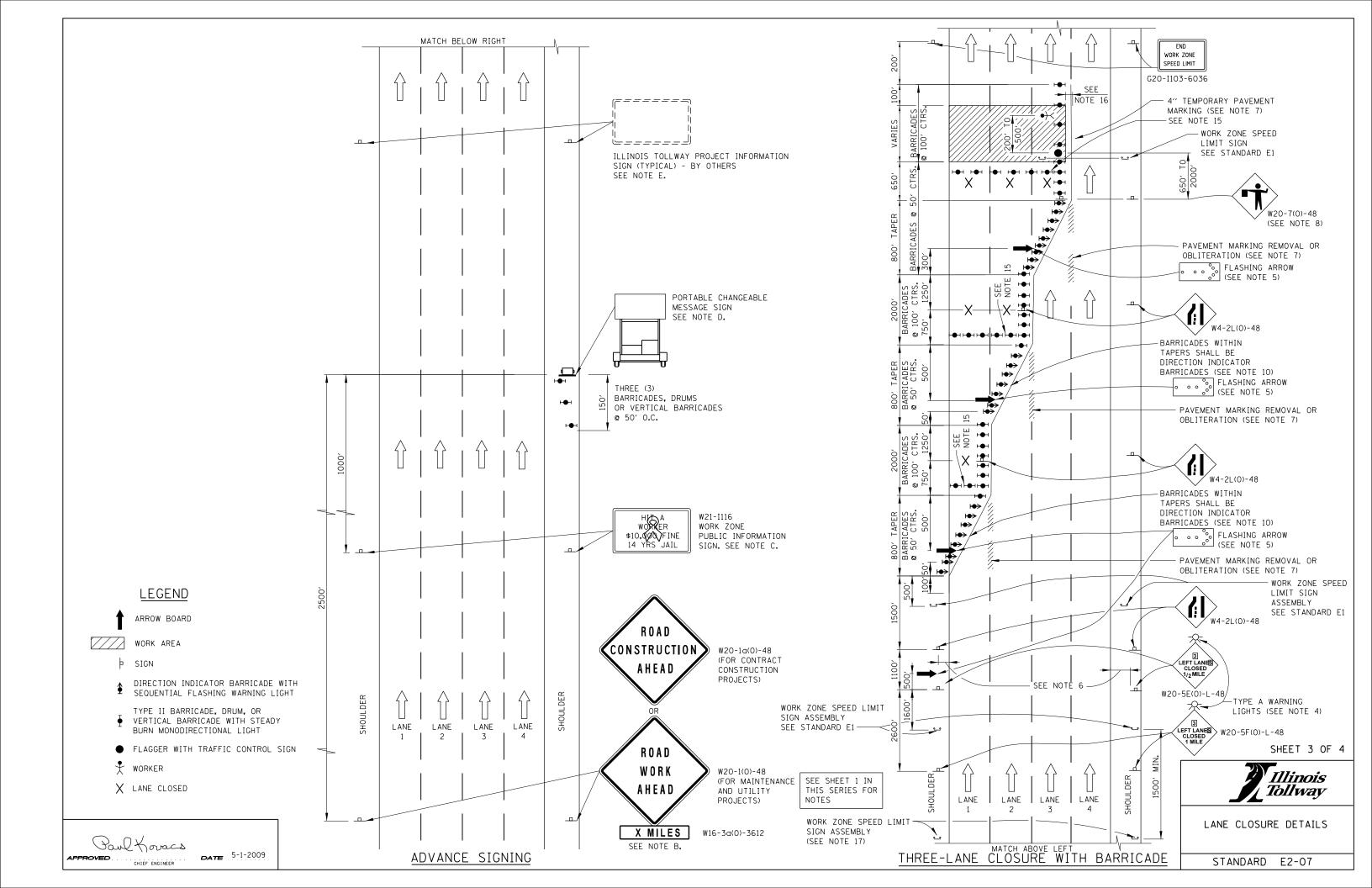


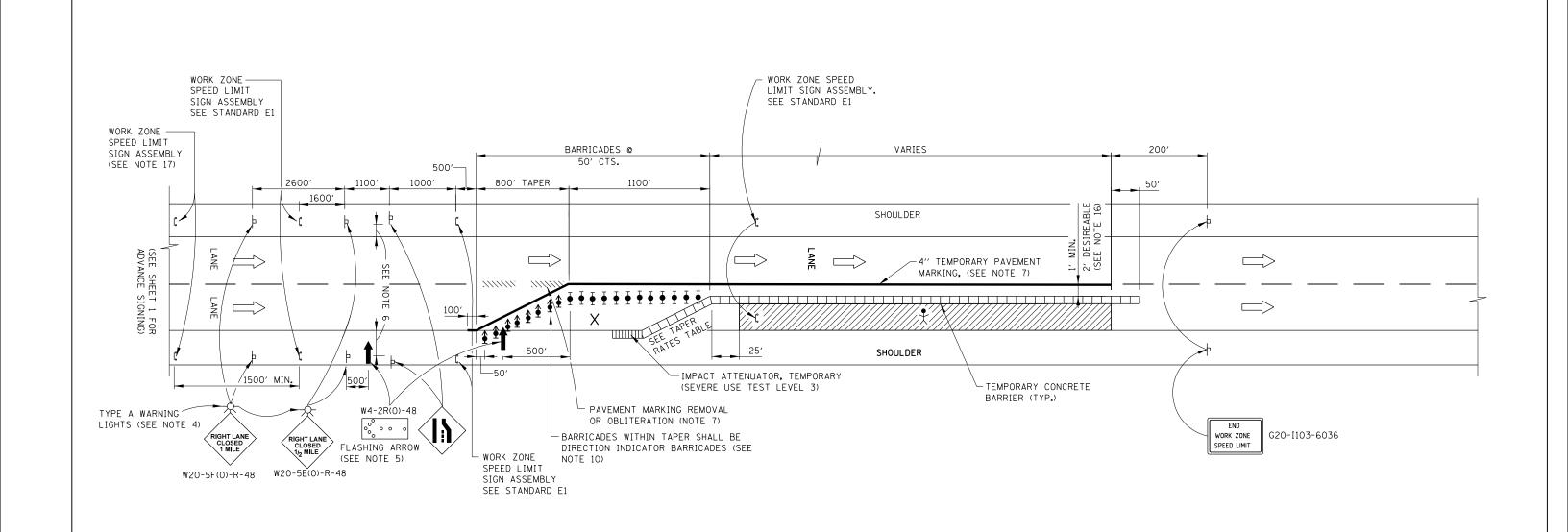
DATE 5-1-2009

STANDARD E1-06









#### ONE-LANE CLOSURE WITH BARRIER

#### TAPER RATES

WORK			BARRIER
ZONE		BARRIER	AT OR
SPEED	SHY LINE	INSIDE	BEYOND
(mph)	(f+.)	SHY LINE	SHY LINE
65	8.5	28:1	19:1
60	8	26:1	18:1
55	7	24:1	16:1
50	6.5	21:1	14:1
45	6	18:1	12:1
40	5	16:1	10:1
35	4.5	15:1	9:1
30	4	13:1	8:1

LEGEND ARROW BOARD

WORK AREA

⊨ SIGN

PORTABLE CHANGEABLE MESSAGE

DIRECTION INDICATOR BARRICADE WITH SEQUENTIAL FLASHING WARNING LIGHT

TYPE II BARRICADE, DRUM, OR VERTICAL BARRICADE WITH STEADY BURN MONODIRECTIONAL LIGHT

★ WORKER

X LANE CLOSED

NOTE:

SEE SHEET 1 OF THIS SERIES FOR NOTES.

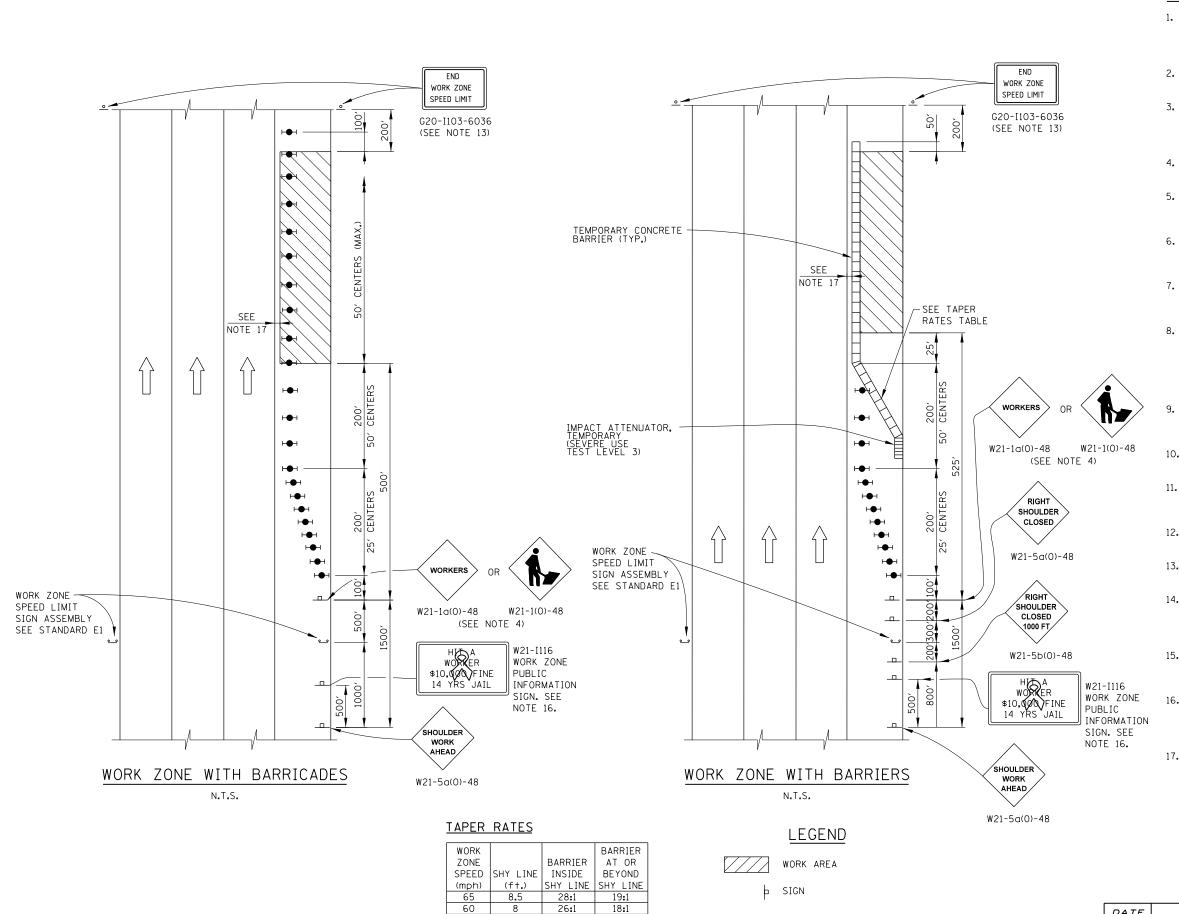
SHEET 4 OF 4



LANE CLOSURE DETAILS

STANDARD E2-07





24:1

21:1

18:1

16:1

15:1

13:1

50

45

40

35 30

Paul Koracs

CHIEF ENGINEER

DATE 5-1-2009

6.5

4.5

16:1

14:1

12:1

10:1

9:1

8:1

#### GENERAL NOTES:

- 1. THE SHOULDER SHALL BE CLOSED WHEN A WORK ACTIVITY REQUIRING 15 OR MORE MINUTES IS PERFORMED AT A DISTANCE WHICH IS LESS THAN 15 FEET BUT NO CLOSER THAN 2 FEET FROM THE EDGE OF PAVEMENT.
- 2. THE ADJACENT EXTERIOR LANE SHALL BE CLOSED WHEN WORK IS PERFORMED WITHIN 2 FEET FROM THE EDGE OF PAVEMENT.
- 3. THE CHANNELIZING DEVICES WHICH SEPARATE THE WORK SPACE FROM THE ADJACENT TRAVEL LANE SHALL BE SPACED AT 25' FOR (200 FEET) AND AT A MAXIMUM OF 50' FOR ALL ADDITIONAL DEVICES.
- WHEN THE WORKSITE IS UNATTENDED, SUBSTITUTE -"SHOULDER WORK AHEAD" SIGN.
- 6. WORKER SIGNS OR SHOULDER WORK SIGNS AND
  CHANNELIZATION DEVICES ARE PLACED ONLY ON THE SIDE OF
  THE ROADWAY ON WHICH THE ACTIVITY IS PERFORMED.
- 6. FOR SHOULDER CLOSURE EXTENDING OVERNIGHT, BARRICADE TYPE II WITH STEADY BURNING LIGHT, TYPE C SHALL BE
- 7. FOR SHORT TERM CLOSURE (SUNRISE TO ONE HOUR BEFORE SUNSET) NOT EXTENDING INTO DARKNESS, CONES MAY BE USED.
- ONE WORK ZONE SPEED LIMIT SIGN ASSEMBLY SHALL BE PLACED AT A DISTANCE OF 500' TO 2,500' MAXIMUM IN ADVANCE OF WORKERS THROUGHOUT THE SHOULDER CLOSURE. MOVING OPERATIONS MAY REQUIRE CONTINUOUS ADJUSTMENT OF THE SIGN ASSEMBLY LOCATION TO MAINTAIN THE ABOVE INTERVAL.
- AN ADDITIONAL SIGN ASSEMBLY SHALL BE PLACED 500' BEYOND THE LAST ENTRANCE RAMP FOR EACH INTERCHANGE THAT FALLS WITHIN THE 2,500'.
- 10. THE SIGN ASSEMBLY SHALL BE PLACED NO CLOSER THAN 500' TO ANY OTHER SIGN.
- 11. THE WORK ZONE SPEED LIMIT SIGNS AND SIGN ASSEMBLY SHALL BE PROMPTLY REMOVED OR COVERED WHEN SHOULDER CLOSURE IS NOT IN USE.
- ALL CONFLICTING SPEED LIMIT SIGNS SHALL BE COVERED OR REMOVED.
- 13. "END WORK ZONE SPEED LIMIT" SIGNS SHALL BE IN PLACE ONLY WHEN THE EXISTING POSTED SPEED > 55MPH.
- 4. FOR SHOULDER REPAIRS OR REPLACEMENT THE CHANNELIZING DEVICES SHALL BE PLACED AT THE EDGE OF PAVEMENT WHENEVER THE WORK ACTIVITIES RESULT IN A DROPOFF AT THE EDGE OF PAVEMENT.
- 15. ANY UNATTENDED OBSTACLE OR EXCAVATION LEFT ON THE SHOULDER OVERNIGHT SHALL BE IN COMPLIANCE WITH THE ROADWAY TRAFFIC CONTROL AND COMMUNICATIONS MANUAL.
- 6. THE WORK ZONE PUBLIC INFORMATION SIGN IS 60" WIDE BY 48" HIGH. THE CONTRACTOR SHALL OBTAIN THE CAMERA-READY ARTWORK REQUIRED FOR THE SIGN MESSAGE BY CONTACTING IDOT'S CENTRAL BUREAU OF OPERATIONS.
- 17. A 1'-0" MINIMUM/2'-0" DESIRABLE SHY DISTANCE SHALL BE PROVIDED, MEASURED BETWEEN EDGE OF PAVEMENT LANE MARKING TO THE EDGE OF THE TRAFFIC CONTROL DEVICE.

TYPE II BARRICADE, DRUM, OR VERTICAL BARRICADE WITH STEADY BURN MONODIRECTIONAL LIGHT



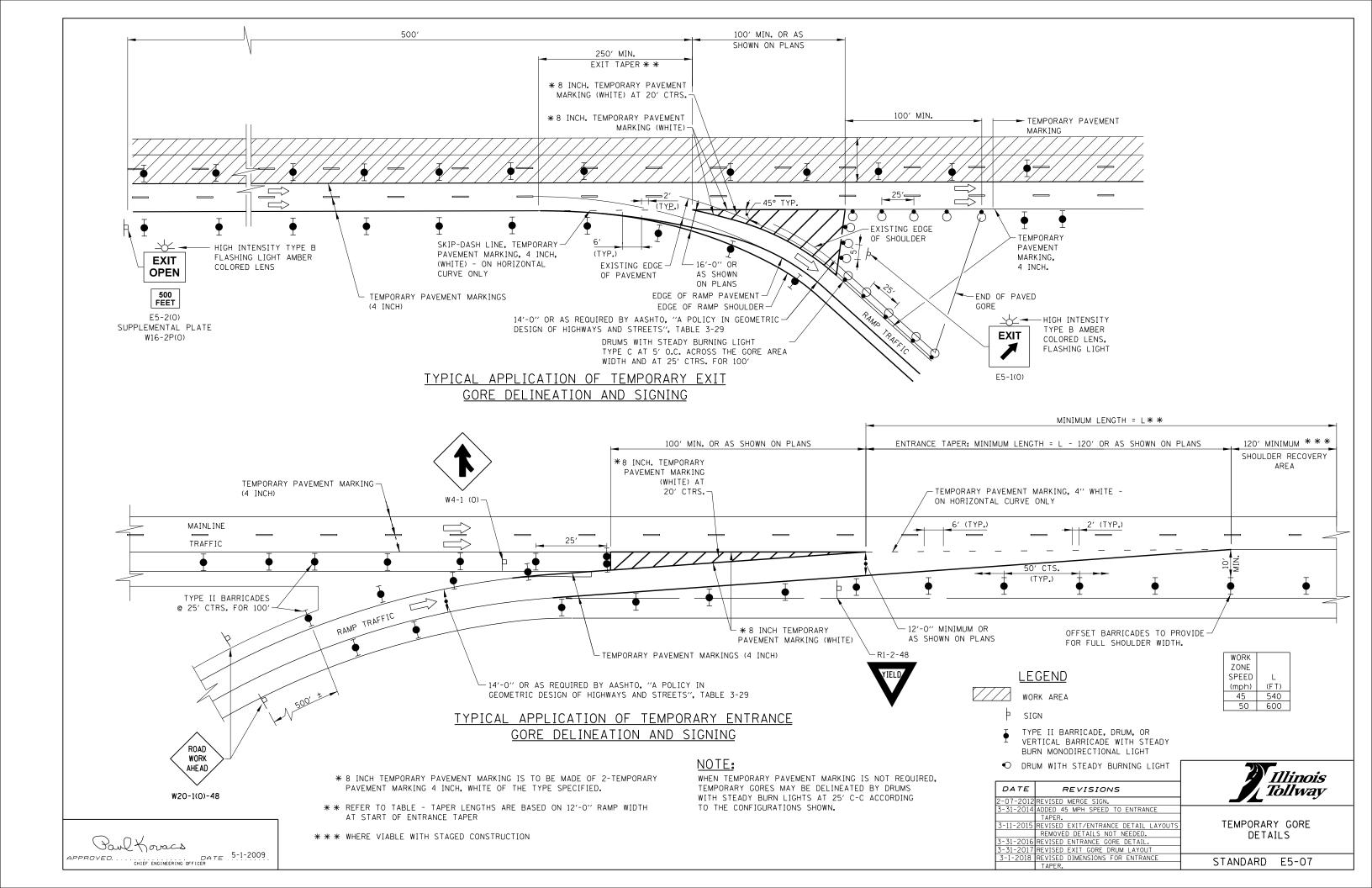
MUTCD" AND REVISED NOTES.

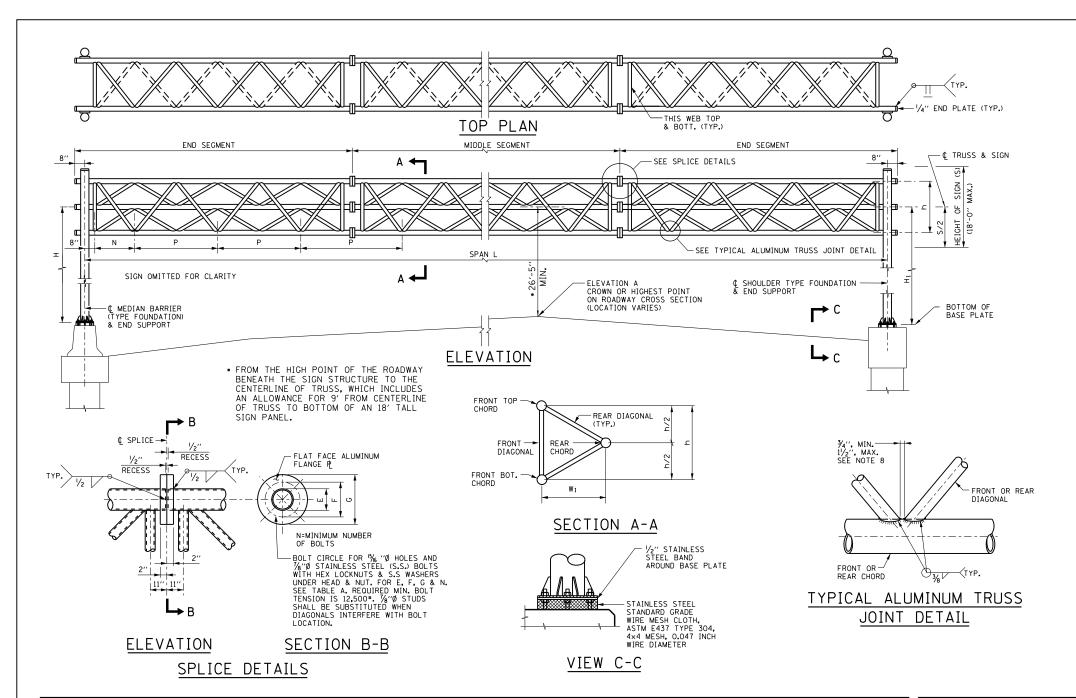
EVISED NOTES ADD WORK ZONE WITH BARRIERS SHOULDER CLOSURE DETAILS

Illinois

*Tollway* 

STANDARD E3-06





	SIGN STRUCTURE MEMBER SCHEDULE													
			DIME	NSIO	N S			ALUMIN	UM TRU	s s *			STEEL END	SUPPORT
TRUSS						MAXIMUM			MIDDLE SEGMENT OR END SEGMENT				PIPE COLUMN (NOMIN	IAL DIAMETER)
NO.	TRUSS SPAN L	Р	N	h	w <sub>1</sub>	ALLOWABLE SIGN PANEL	DL (TRUSS) DEFLECTION	CHORD	(O.D.)	DIAGONAL (	O.D.)	w	10" X.X.S. (104.13#/FT.)	12" X.X.S. (125.49#/FT.)
						AREA	DEI EEC I TOIN	FRONT	REAR	FRONT	REAR		H OR H <sub>1</sub>	H OR H <sub>1</sub>
T-80	80'-0''	9'-0''	3'-4''	4'-6''	3′-10¾′′	900 S.F.	1"	51/2"Ø x1/2"	51/2"Ø x1/2"	21/2"Ø x1/4"	21/2"Ø x1/4"	5′-9′′	32'-0" (MAX)	38'-0" (MAX)
T-85	85'-0''	9′-6′′	3′-10′′	4'-9''	4'-13/8''	955 S.F.	11/16′′	61/8"Ø ×1/2"	61/8"\$ ×1/2"	3"ø ×¹/₄"	3''Ø x'/4''	6′-7′′	31'-0'' (MAX)	38'-0" (MAX)
T-90	90'-0''	10'-0''	4'-4''	5′-0′′	4'-4''	1010 S.F.	11/8′′	61/8"Ø x1/2"	61/8"Ø ×1/2"	3'' Ø ×¹/₄''	3'' Ø ×¹/₄''	6′-7′′	31'-0'' (MAX)	38'-0" (MAX)
T-95	95′-0"	10'-6''	4'-10''	5′-3′′	4'-65/8''	1065 S.F.	1 <sup>3</sup> / <sub>16</sub> ''	61/8"\$ x1/2"	61/8"Ø ×1/2"	3"∅ ×¹/₄"	3" Ø x¹/₄"	6′-7′′	31'-0" (MAX)	38'-0" (MAX)
T-100	100'-0''	11'-4''	4'-0''	5′-8′′	4'-107/8''	1125 S.F.	11/4′′	7'' Ø ×'/2''	7''ø ×1/2''	31/2"Ø ×1/4"	31/2"Ø x1/4"	7′-5′′	31'-0" (MAX)	38'-0" (MAX)
T-105	105'-0''	12'-0''	3′-10′′	6′-0′′	5′-2¾′′	1180 S.F.	15/6′′	7''Ø x'/2''	7''Ø x1/2''	31/2''Ø x1/4''	31/2"Ø x1/4"	7′-5′′	31'-0" (MAX)	38'-0" (MAX)
T-110	110'-0''	12'-6''	4'-4''	6′-3′′	5′-5′′	1200 S.F.	13/8′′	7''Ø x <sup>1</sup> / <sub>2</sub> ''	7''Ø ×1/2''	31/2''Ø ×1/4''	31/2"Ø x1/4"	7′-5′′	31'-0" (MAX)	38'-0" (MAX)
T-115	115′-0′′	13′-0′′	4'-10''	6′-6′′	5′-75⁄8′′	1200 S.F.	11/2′′	7½''ø ×½''	7½''Ø x½''	31/2′′∅ ×1/4′′	31/2''ø ×1/4''	10'-2"	34'-0'' (MAX)	40'-0" (MAX)
T-120	120'-0''	13'-8''	4'-8''	6′-10′′	5′-11′′	1200 S.F.	1%6′′	71/2''Ø ×1/2''	71/2''Ø x1/2''	31/2"Ø ×1/4"	31/2"Ø ×1/4"	10'-2"	34'-0" (MAX)	40'-0" (MAX)
T-130	130'-0''	15'-0''	4'-4''	7′-6′′	6′-57⁄8′′	1200 S.F.	1%6′′	9''ø x½''	9″ø ×½″	4''Ø x <sup>1</sup> / <sub>4</sub> ''	4''ø ×¹/₄''	10'-2''	NOT APPLICABLE	40'-0" (MAX)
T-140	140'-0''	16′-3′′	4'-4''	8'-2''	7′-07⁄8′′	1200 S.F.	1"/16"	10" Ø x <sup>1</sup> /₂"	10'' ø ×¹/₂''	4''ø x¹/₄''	4''ø ×¹/₄''	10'-2"	NOT APPLICABLE	40'-0" (MAX)
T-150	150'-0''	17′-6′′	4'-4''	8'-10''	7'-73/4''	1200 S.F.	1 <sup>13</sup> / <sub>16</sub> ''	11"ø x½"	11'' Ø ×1∕2''	41/2''Ø x1/4''	41/2"Ø x1/4"	10'-2"	NOT APPLICABLE	40'-0" (MAX)

#### TABLE Α CHORD O.D. E G 51/2′′Ø 13" 6%''Ø & 7''Ø 111/2" | 141/2" 10 121/2" | 151/2" | 12 71/2′′Ø 131/2" 161/2" 14 9′′Ø 151/2" 181/2" 16

PROVIDE THE ABOVE CAMBER AT MIDDLE OF SPAN OF STRUCTURES

CAMBER

CAMBER IN INCHES

11/2"

15/8"

11/8"

11/8"

21/8'

171/2" | 201/2" | 18

SPAN IN FEET

80 THRU 95

96 THRU 110

111 THRU 120

121 THRU 130

131 THRU 140 141 THRU 150

11''Ø

\* SUBSTITUTION OF LARGER TRUSS SIZE IS ACCEPTABLE.

# NOTES:

2. A PAIR OF MAIN PIPE COLUMN SIZES FOR EACH SUPPORT SHALL BE SELECTED INDEPENDENTLY BASED ON SPECIFIC NEEDS.

#### **GENERAL NOTES:**

- 1. WORK THIS SHEET WITH OVERHEAD SIGN STRUCTURES SPAN TYPE SUMMARY AND TOTAL BILL OF MATERIAL.
- 2. AFTER ADJUSTMENTS TO LEVEL TRUSS AND ENSURE ADEQUATE VERTICAL CLEARANCE, ALL TOP AND LEVELING NUTS SHALL BE TIGHTENED AGAINST THE BASE PLATE WITH A MINIMUM TORQUE OF 200 LB.-FT. STAINLESS STEEL MESH SHALL THEN BE PLACED AROUND THE PERIMETER OF THE BASE PLATE. SECURE TO BASE PLATE WITH STAINLESS STEEL BANDING.
- 3. SIGN SUPPORT STRUCTURES MAY BE SUBJECT TO DAMAGING VIBRATIONS AND OSCILLATIONS WHEN SIGN PANELS ARE NOT IN PLACE DURING ERECTION OR MAINTENANCE OF THE STRUCTURE. TO AVOID THESE, ATTACH TEMPORARY BLANK SIGN PANELS OR OTHER BRACING TO THE STRUCTURE UNTIL PERMANENT SIGNS
- 4. TRUSS SEGMENTS SHALL BE SHIPPED INDIVIDUALLY WITH ADEQUATE PROVISION TO PREVENT DETRIMENTAL MOTION DURING TRANSPORT. THE CONTRACTOR IS RESPONSIBLE FOR MAINTAINING THE CONFIGURATION AND PROTECTION OF THE TRUSSES.
- 5. ONLY SIGN PANELS ARE PERMITTED TO BE MOUNTED ON THIS TRUSS.

#### DESIGN SPECIFICATIONS:

1. 2013 AASHTO STANDARD SPECIFICATIONS FOR STRUCTURAL SUPPORTS FOR HIGHWAY SIGNS, LUMINAIRES AND TRAFFIC SIGNALS, 6TH EDITION.

#### CONSTRUCTION SPECIFICATIONS:

1. ALL MATERIALS, EXCEPT AS SHOWN, FABRICATION, ERECTION AND CONSTRUCTION REQUIREMENTS SHALL BE IN ACCORDANCE WITH SECTION 733 OF THE LATEST ILLINOIS TOLLWAY SUPPLEMENTAL SPECIFICATIONS.

#### LOADING:

- 1. BOTH END SUPPORTS ARE DESIGNED FOR 60% OF THE TOTAL LOAD.
- 2. WIND LOADING SHALL BE A MINIMUM OF 35 PSF ON SIGN PANELS AND 10 PSF ON GROSS AREAS DEFINED BY THE PERIMETER OF TRUSS MEMBERS NOT COVERED BY SIGN PANEL AREAS.
- 3. THE AASHTO GROUP II AND III ALLOWABLE STRESS SHALL BE 133% (ALLOWABLE STRESS DESIGN).

#### FABRICATION NOTES:

- 1. NO SPLICES SHALL BE LOCATED WITHIN 0.1xL OF THE CENTERLINE OF THE SPAN.
- 2. MATERIALS: ALUMINUM SHALL CONFORM TO ASTM B221, ALLOY 6061 TEMPER T6. ALL STRUCTURAL STEEL PIPE SHALL BE ASTM A53 GRADE B OR A106 GRADE B. ALL STRUCTURAL STEEL PLATES AND SHAPES SHALL CONFORM TO AASHTO M2TO GR. 36 OR GR. 50. STAINLESS STEEL FOR SHIMS, SLEEVE AND HANDHOLE COVERS SHALL BE ASTM A240, TYPE 302 OR 304, OR ANOTHER ALLOY SUITABLE FOR EXTERIOR EXPOSURE AND ACCEPTABLE TO THE ENGINEER, THE STEEL PIPE AND STIFFENING RIBS AT THE BASE PLATE FOR THE COLUMN SHALL HAVE A MINIMUM LONGITUDINAL CHARPY V-NOTCH (CVN) ENERGY OF 15 LB.-FT. AT 40° F. (ZONE 2) BEFORE GALVANIZING.
- 3. WELDING: ALL WELDS TO BE CONTINUOUS UNLESS OTHERWISE SHOWN. ALL WELDING TO BE DONE IN ACCORDANCE WITH CURRENT AWS DI.1 AND DI.2 STRUCTURAL WELDING CODES (STEEL AND ALUMINUM) AND THE STANDARD SPECIFICATIONS. ALUMINUM WELD FILLER SHALL BE ALLOY 5556.
- 4. FASTENERS FOR ALUMINUM TRUSSES: HIGH STRENGTH BOLTS MUST SATISFY THE REQUIREMENTS OF AASHTO MIG4 (ASTM A325), OR APPROVED ALTERNATE, AND MUST HAVE MATCHING LOCK NUTS.
  THREADED STUDS FOR SPLICES (IF MEMBERS INTERFERE) MUST SATISFY THE REQUIREMENTS OF ASTM A449,
  ASTM A193, GRADE B7, OR APPROVED ALTERNATE, AND MUST HAVE MATCHING LOCK NUTS. BOLTS AND LOCK
  NUTS NOT REQUIRED TO BE HIGH STRENGTH MUST SATISFY THE REQUIREMENTS OF ASTM A307. ALL BOLTS
  AND LOCK NUTS MUST BE HOT DIP CALVANIZED PER AASHTO M232, EXCEPT STAINLESS STEEL FASTENERS. AND LOCK NUTS MUST BE HOT DIF GALVANIZED FER ASSISTED ME32, EXCEPT STRINLESS STEEL FASTENERS, NUTS AND WASHERS. THE LOCK NUTS MUST HAVE NYLON OR STEEL INSERTS. A STAINLESS STEEL FLAT WASHER CONFORMING TO ASTM A240 TYPE 302 OR 304, IS REQUIRED UNDER BOTH HEAD AND NUT OR UNDER BOTH NUTS WHERE THREADED STUDS ARE USED. HIGH STRENGTH BOLT INSTALLATION SHALL CONFORM TO ARTICLE 505.04 (F) (2)D OF THE IDOT STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION. ROTATIONAL CAPACITY ("ROCAP") TESTING OF BOLTS WILL NOT BE REQUIRED.
- 5. U-BOLTS: U-BOLTS MUST BE PRODUCED FROM ASTM A276 TYPE 304, 304L, 316 OR 316L, CONDITION A. COLD FINISHED STAINLESS STEEL, OR AN EQUIVALENT MATERIAL ACCEPTABLE TO THE ENGINEER.

  ALL NUTS FOR U-BOLTS MUST BE LOCK NUTS EQUIVALENT TO ASTM A307 WITH NYLON OR STEEL

  INSERTS AND HOT DIP GALVANIZED PER AASHTO M232. A STAINLESS STEEL FLAT WASHER CONFORMING TO ASTM A240, TYPE 302 OR 304, IS REQUIRED UNDER EACH U-BOLT LOCK NUT.
- 6. GALVANIZING: ALL STEEL GRATING, PLATES, SHAPES AND PIPE SHALL BE HOT DIP GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH AASHTO M111. PAINTING IS NOT PERMITTED.
- 7. SEE TABLE "SIGN STRUCTURE MEMBER SCHEDULE" FOR "W" AND "W,".
- 8. DIAGONALS SHALL BE DETAILED TO MINIMIZE OFFSET FOR THEORETICAL PANEL POINT AND PROVIDE 34 TO 1/2 INCH CLEARANCE BETWEEN DIAGONALS AND PROVIDE CLEARANCE FOR U-BOLT CONNECTIONS OF SIGNS OR WALKWAY BRACKETS.

SHEET 1 OF 5

DATE	REVISIONS	
2-07-2012	REVISED FOUNDATIONS AND REVISED NOTES.	
2-01-2013	REVISED TABLES, ELEVATION, AND NOTES.	
12-12-2013	REVISED TABLES AND NOTES.	
3-31-2014	REVISED SIGN STRUCTURE DETAILS.	
7-01-2014	REVISED FOUNDATION CONCRETE.	
3-11-2015	REVISED NOTES.	
3-31-2016	REVISED FOUNDATION NOTE AND REVISED	
	BASE PLATE DIMENSIONS.	
3-31-2017	COLUMN MEMBER ADJUSTMENTS AND	
	FOUNDATION REINFORCEMENT.	
3-01-2018	REVISED VER. CLEARANCE, AND ADDED NOTE	

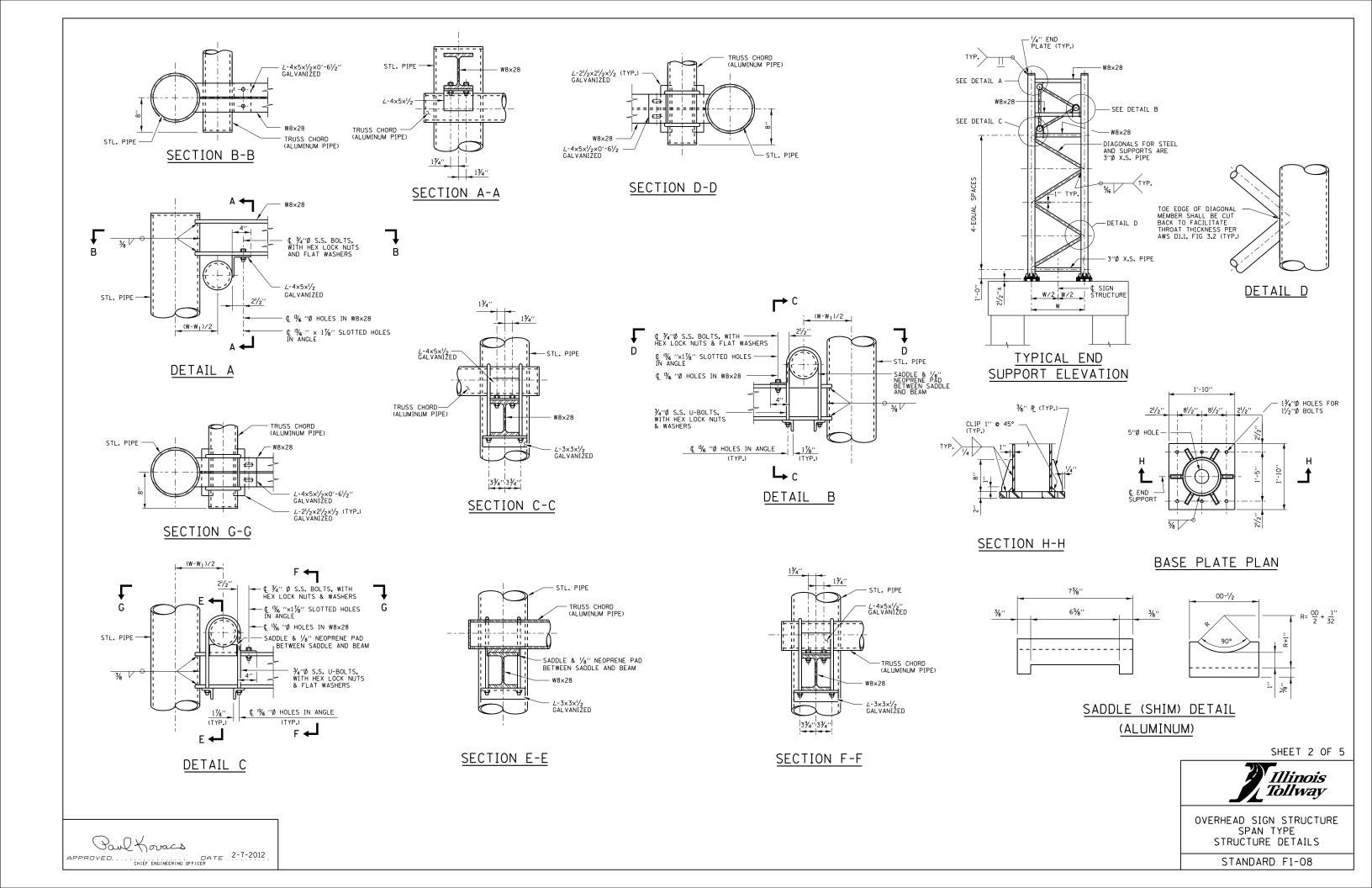
Illinois *Tollway* 

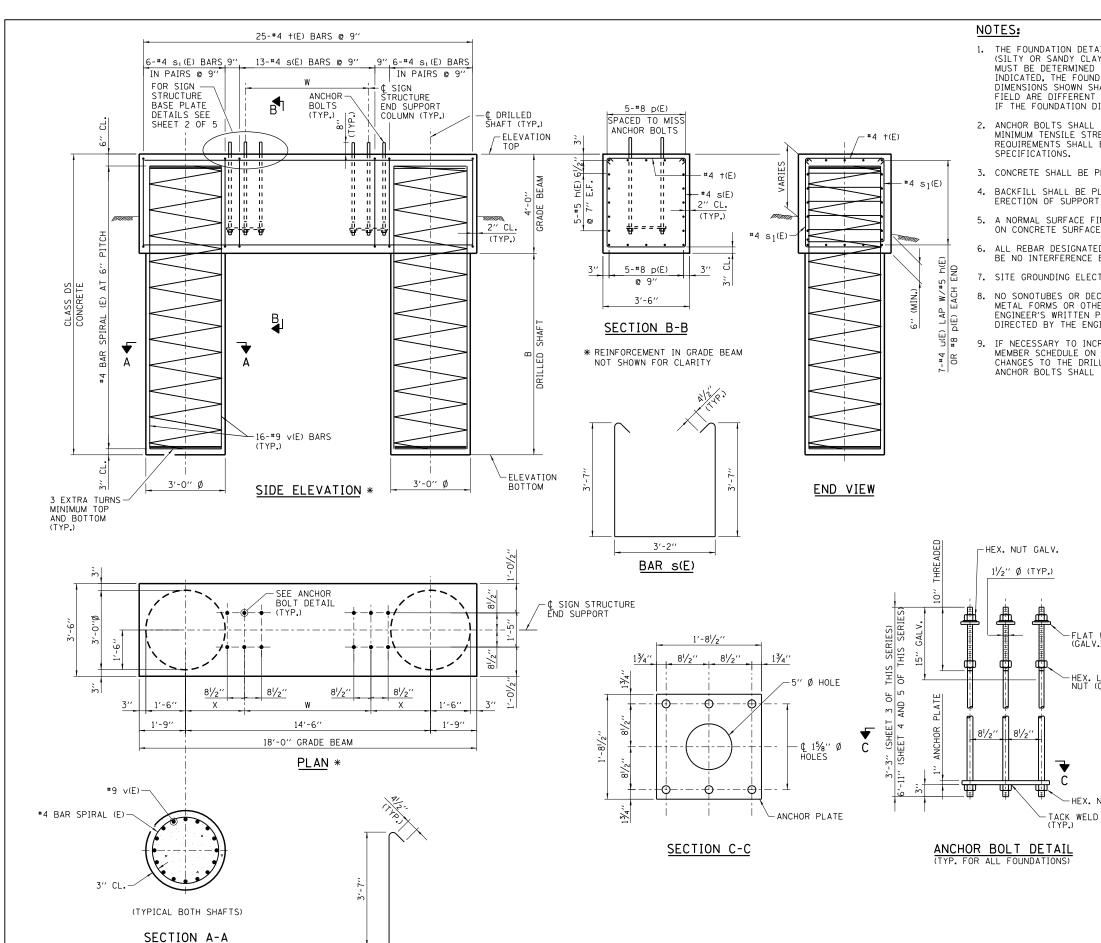
OVERHEAD SIGN STRUCTURE SPAN TYPE STRUCTURE DETAILS

STANDARD F1-08

Paul Koracs CHIEF ENGINEERING OFFICER 2-7-2012 APPROVED....

1. XXS DENOTES DOUBLE EXTRA STRONG PIPE.





3'-0"

BAR s<sub>1</sub>(E)

Paul Kovacs

CHIEF ENGINEERING OFFICER 2-7-2012

1. THE FOUNDATION DETAILS SHOWN ARE BASED ON THE PRESENCE OF MOSTLY COMMON COHESIVE SOIL CONDITIONS (SILTY OR SANDY CLAY), WITH AN AVERAGE UNCONFINED COMPRESSIVE STRENGTH (QU) > 1.25 TON/SQ. FT. WHICH MUST BE DETERMINED BY PREVIOUS SOIL INVESTIGATIONS AT THE JOBSITE. WHEN OTHER CONDITIONS ARE

INDICATED, THE FOUNDATION DIMENSIONS SHOWN SHALL BE INCLUDED IN THE PLANS AND THE FOUNDATION DIMENSIONS SHOWN SHALL BE THE RESULT OF SITE SPECIFIC DESIGNS. IF CONDITIONS ENCOUNTERED IN THE FIELD ARE DIFFERENT THAN THOSE INDICATED, THE CONTRACTOR SHALL NOTIFY THE ENGINEER TO DETERMINE IF THE FOUNDATION DIMENSIONS NEED TO BE MODIFIED.

- 2. ANCHOR BOLTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M314 OR ASTM F1554 GRADE 55, WITH A MINIMUM TENSILE STRENGTH OF 75,000 PSI. ALL OTHER MATERIAL, FABRICATION, AND CONSTRUCTION REQUIREMENTS SHALL BE IN ACCORDANCE WITH SECTION 734 OF THE ILLINOIS TOLLWAY SUPPLEMENTAL
- 3. CONCRETE SHALL BE PLACED MONOLITHICALLY, WITHOUT CONSTRUCTION JOINTS UNLESS NOTED OTHERWISE.
- 4. BACKFILL SHALL BE PLACED PER SECTION 502 OF THE IDOT STANDARD SPECIFICATION AND PRIOR TO ERECTION OF SUPPORT COLUMN.
- 5. A NORMAL SURFACE FINISH FOLLOWED BY A CONCRETE SEALER APPLICATION WILL BE REQUIRED ON CONCRETE SURFACES ABOVE THE LOWEST ELEVATION 6" BELOW FINISHED GROUND LINE.
- 6. ALL REBAR DESIGNATED (E) SHALL BE EPOXY COATED. REBAR SHALL BE POSITIONED SO THAT THERE WILL BE NO INTERFERENCE BETWEEN VERTICAL REINFORCEMENT AND ANCHOR BOLTS.
- 7. SITE GROUNDING ELECTRODE SYSTEM TO BE PROVIDED AS INDICATED ON THE PLANS.

3'-2"

BAR + (E)

3'-0"

BAR u(E)

FLAT WASHER

HEX. LEVELING NUT (GALV.)

HEX. NUT

(GAL V.)

- NO SONOTUBES OR DECOMPOSABLE FORMS SHALL BE USED 6" BELOW THE FINISHED GROUND LINE. PERMANENT METAL FORMS OR OTHER SHIELDING SHALL NOT BE LEFT IN PLACE BELOW THE ELEVATION WITHOUT THE ENGINEER'S WRITTEN PERMISSION. EXCAVATIONS SHALL BE DEWATERED BEFORE CONCRETE PLACEMENT IF DIRECTED BY THE ENGINEER AT NO ADDITIONAL COST.
- IF NECESSARY TO INCREASE STEEL END SUPPORT HEIGHT ABOVE THE LIMITATIONS SHOWN IN SIGN STRUCTURE MEMBER SCHEDULE ON SHEET 1 OF THIS SERIES, GRADE BEAM DEPTH SHALL BE INCREASED UP TO 6'-O" WITHOUT CHANGES TO THE DRILLED SHAFT DESIGN. GRADE BEAM REINFORCEMENT, CONCRETE VOLUME AND LENGTH OF ANCHOR BOLTS SHALL BE REVISED ACCORDINGLY.

TRUSS No.	W	Х	В	CLASS DS CONC. CY	REINF. BARS POUND		
T-80	5′-9′′	4'-5''	40′-0′′	30.3	6650		
T-85	6'-7''	4'-1''	50'-0''	35.5	7940		
T-90	6'-7''	4'-1''	50'-0''	35.5	7940		
T-95	6'-7''	4'-1''	50'-0''	35 <b>.</b> 5	7940		
T-100	7′-5′′	3'-7''	50′-0′′	35 <b>.</b> 5	7940		
T-105	7′-5′′	3'-7''	50′-0′′	35.5	7940		
T-110	7′-5′′	3'-7''	50'-0''	35.5	7940		
T-115	10'-2"	2'-2''	50'-0''	35.5	7940		
T-120	10'-2''	2'-2''	50′-0′′	35 <b>.</b> 5	7940		
T-130	10'-2"	2'-2''	55′-0′′	38.1	8590		
T-140	10'-2''	2'-2''	55′-0′′	38.1	8590		
T-150	10'-2"	2'-2''	55′-0′′	38.1	8590		

DESIGN TABLE FOR DRILLED SHAFTS IN COHESIVE SOILS

BAR LIST - EACH FOUNDATION (2 SHAFT AND 1 GRADE BEAM)

NUMBER SIZE LENGTH SHAPE

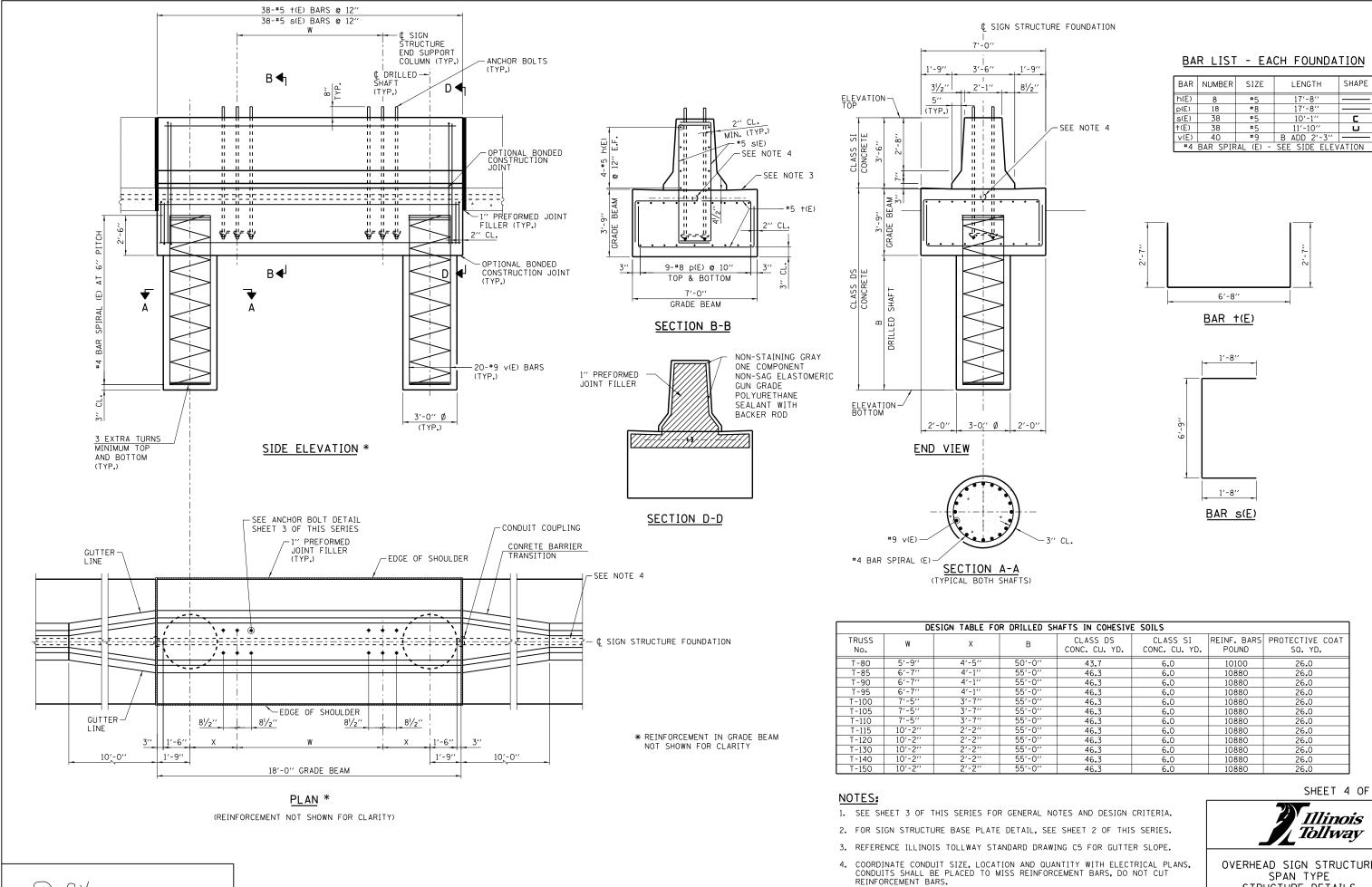
	HONDEN	0122	22.10	0		
h(E)	10	#5	17'-8''			
p(E)	10	#8	17'-8''			
s(E)	13	#4	11'-1''			
s <sub>1</sub> (E)	24	#4	6'-111/2''			
†(E)	25	#4	3'-11''	Ĵ		
u(E)	14	#4	7'-0''			
v(E)	32	#9	B ADD 3'-3"			
#4 BAR SPIRAL (E) - SEE SIDE ELEVATION						

SHEET 3 OF 5



OVERHEAD SIGN STRUCTURE SPAN TYPE STRUCTURE DETAILS

STANDARD F1-08



Paul Koracs

APPROVED. CHIEF ENGINEERING OFFICER 2-7-2012

Illinois

SHEET 4 OF 5

SHAPE

<u>\_</u>\_\_\_\_

LENGTH

10′-1′

*Tollway* OVERHEAD SIGN STRUCTURE SPAN TYPE

26.0

26.0

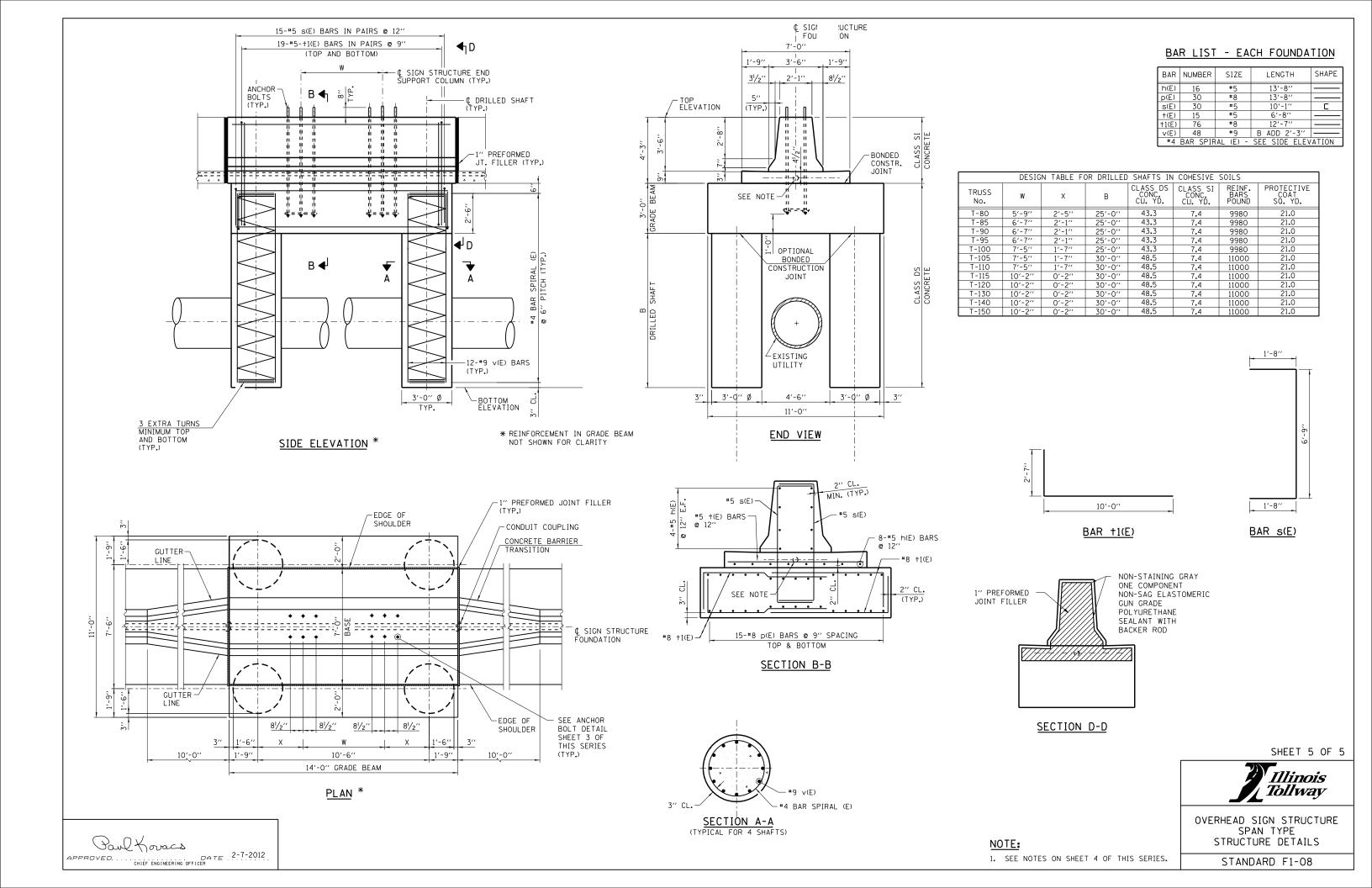
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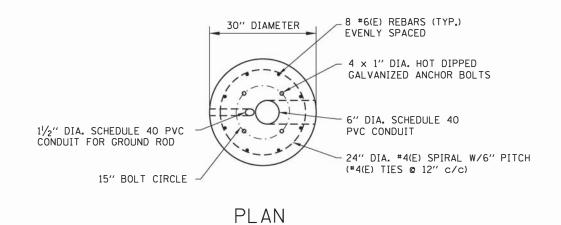
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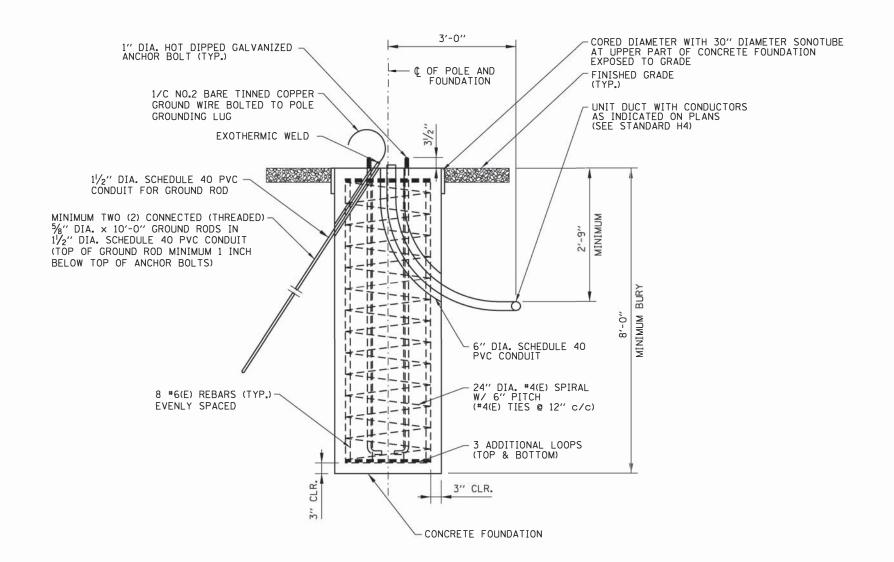
26.0

STRUCTURE DETAILS STANDARD F1-08

5. PROTECTIVE COAT SHALL BE APPLIED TO THE TRAFFIC AND TOP FACES OF THE BARRIER AND TOP FACE OF GUTTER.

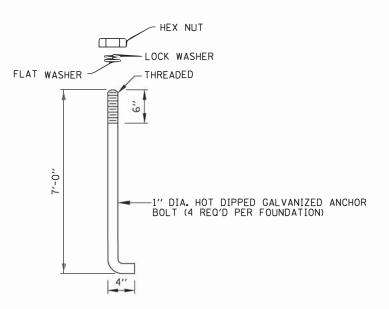






#### NOTES:

- AT LOCATIONS NOT SHIELDED BY GUARDRAIL, THE LIGHT POLE FOUNDATION SHALL BE FLUSH WITH SURROUNDING GRADED ON ALL SIDES. THE SURROUNDING AREA SHALL BE A LEVEL GRADED AREA CONSTRUCTED OF AGGREGATE SHOULDERS WITH FILTER FABRIC,
- PROVIDE SEEDING, POTASIUM FERTILIZER NUTRIENT, AND EROSION CONTROL BLANKET AS REQUIRED.
- THE TOP OF FOUNDATION SHALL BE AT THE SAME ELEVATION AS THE ADJACENT TOP OF GUTTER OR WHEN ADJACENT TO AGGREGATE SHOULDER, AT THE SAME ELEVATION AS THE OUTSIDE EDGE OF THE AGGREGATE SHOULDER SLOPED A MAXIMUM 6% AWAY FROM THE PAVED SHOULDER.
- ALL SLOPES ARE EXPRESSED AS UNITS OF VERTICAL DISPLACEMENT TO UNITS OF HORIZONTAL DISPLACEMENT (V:H).
- ALL GROUND MOUNTED LIGHT POLES SHALL BE PROVIDED WITH AN ACCEPTED FHWA BREAKAWAY BASE OR DEVICE PER THE ILLINOIS TOLLWAY SUPPLEMENTAL SPECIFICATIONS SECTION 1070.
- FOR DETAILS OF FUSE HOLDER, POLE BASE WIRING AND CONDUCTOR SPLICE SEE STANDARD H2.
- ALL REINFORCEMENT BARS SHALL BE EPOXY COATED.
- ALL EQUIPMENT SHALL BE GROUNDED AND BONDED IN ACCORDANCE WITH THE NATIONAL ELECTRICAL CODE AND THE NATIONAL ELECTRICAL SAFETY CODE.
- FOR ALL MEDIAN BARRIER FOUNDATIONS, THE ANCHOR BOLTS SHALL BE CENTERED AROUND THE MEDIAN BARRIER WALL CENTERLINE.



ANCHOR BOLT DETAIL

ELEVATION

LIGHT STANDARD FOUNDATION DETAILS - CONCRETE

Paul Horacs APPROVED. CHIEF ENGINEERING OFFICER DATE 2-7-2012 (GROUND MOUNTED UNITS)

DATE REVISIONS 1-01-2012 ADDED CONTROLLER NUMBER 3-31-2014 REVISED HELIX FOUNDATION, NEW DETAIL "A", AND GRADED AREA 3-11-2015 MOVED MEDIAN BARRIER MOUNTED FOUNDATION DETAILS. ADDED HELIX FOUNDATION DEPTH INFORMATION.

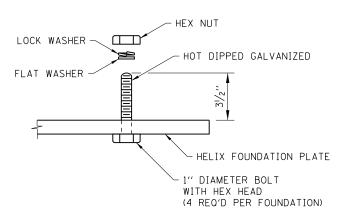
3-01-2018 INCREASED POLE SETBACK.

Illinois Tollway LIGHT STANDARD

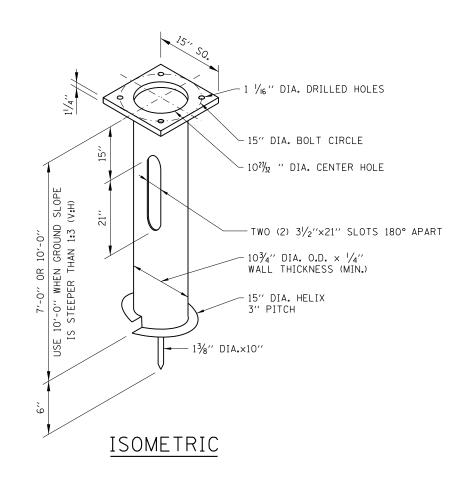
SHEET 1 OF 9

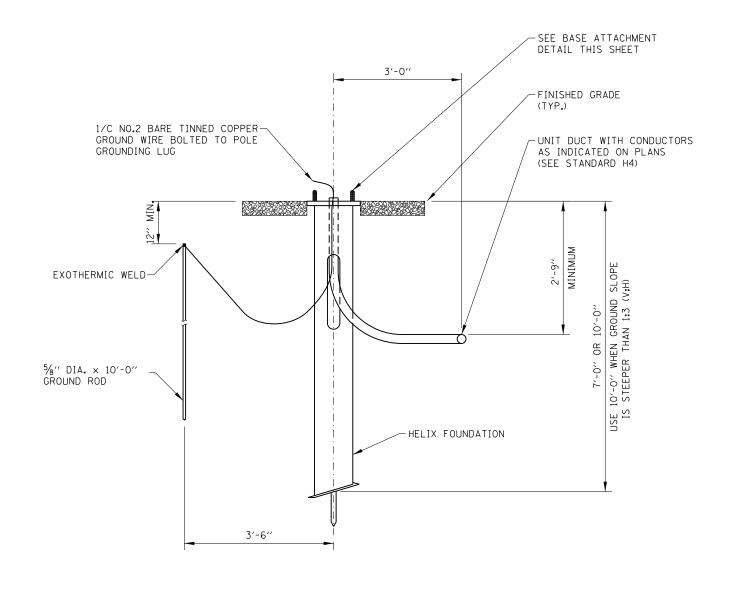
STANDARD H1-07

FOUNDATION



# BASE ATTACHMENT DETAIL





ELEVATION

NOTES:

SEE SHEET 1 OF THIS SERIES FOR NOTES.

SHEET 2 OF 9

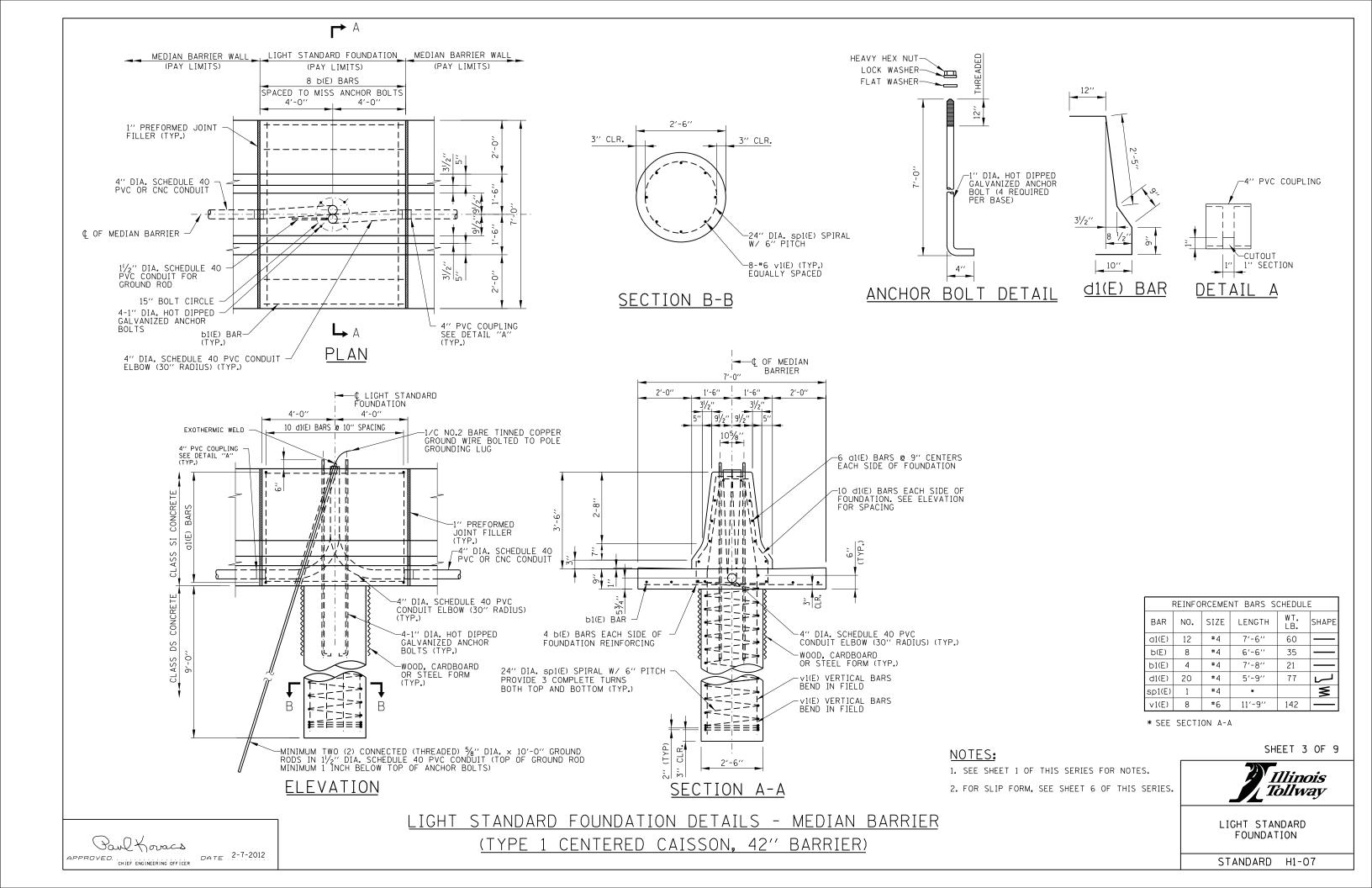


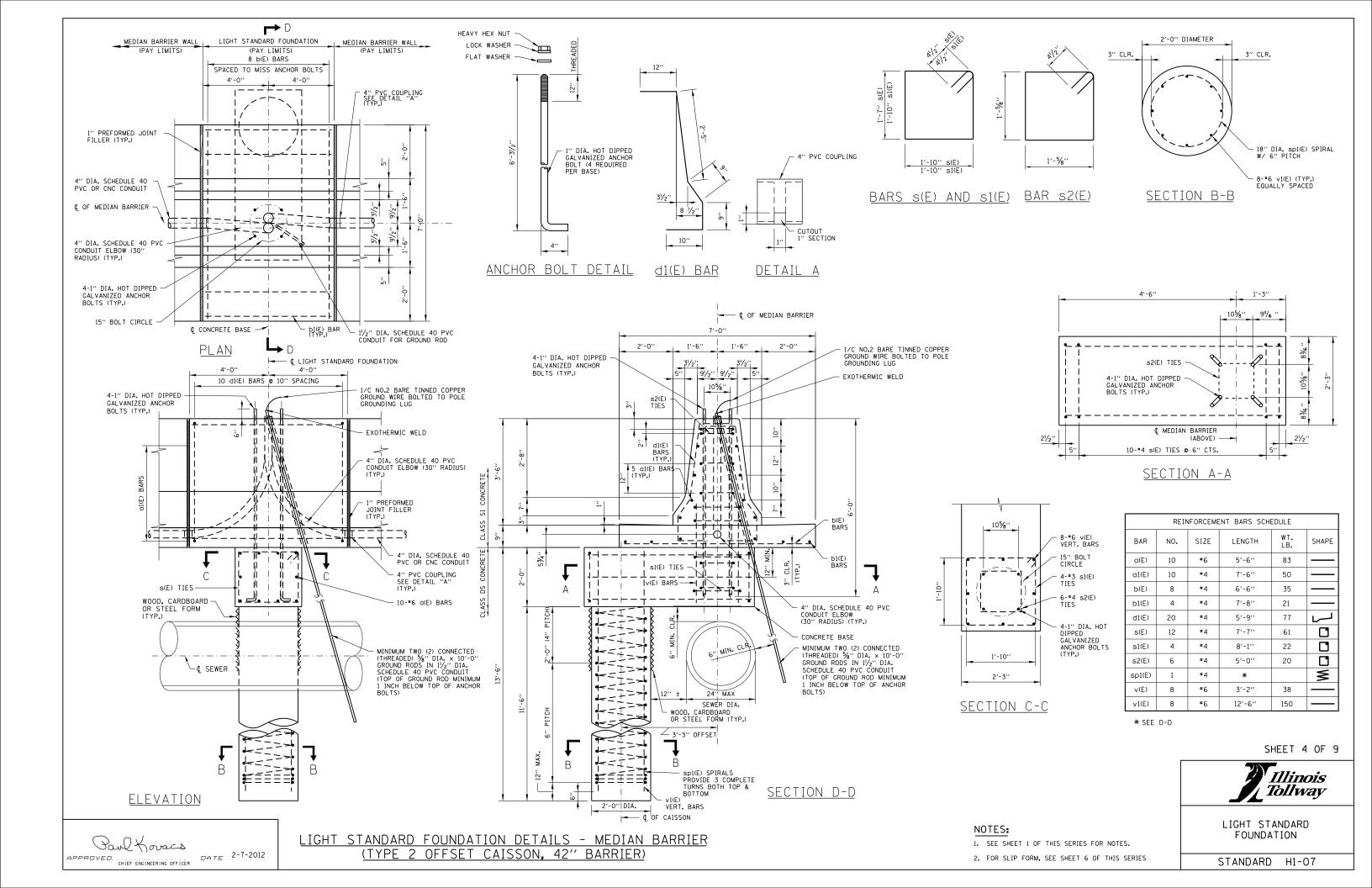
LIGHT STANDARD FOUNDATION

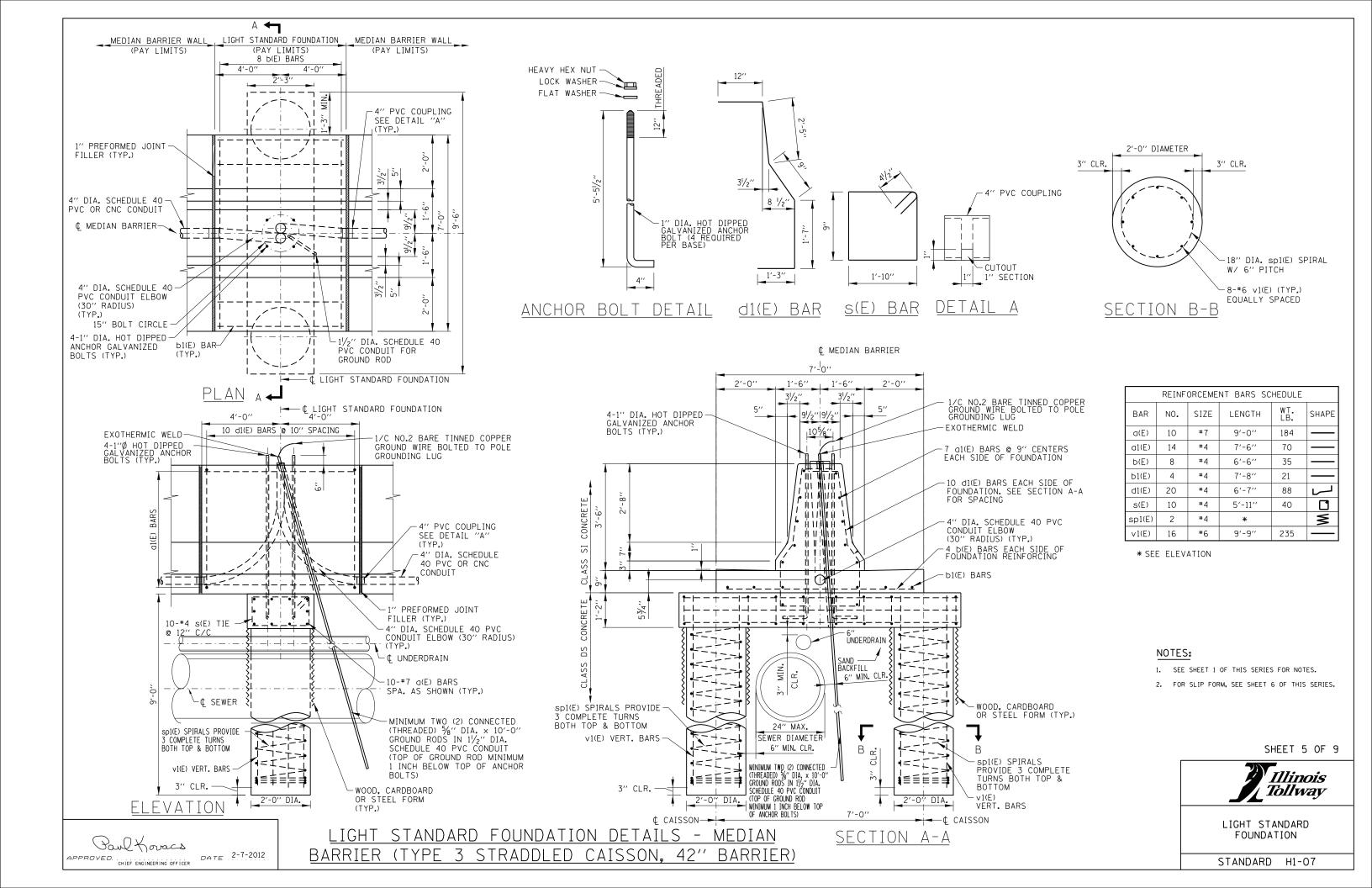
STANDARD H1-07

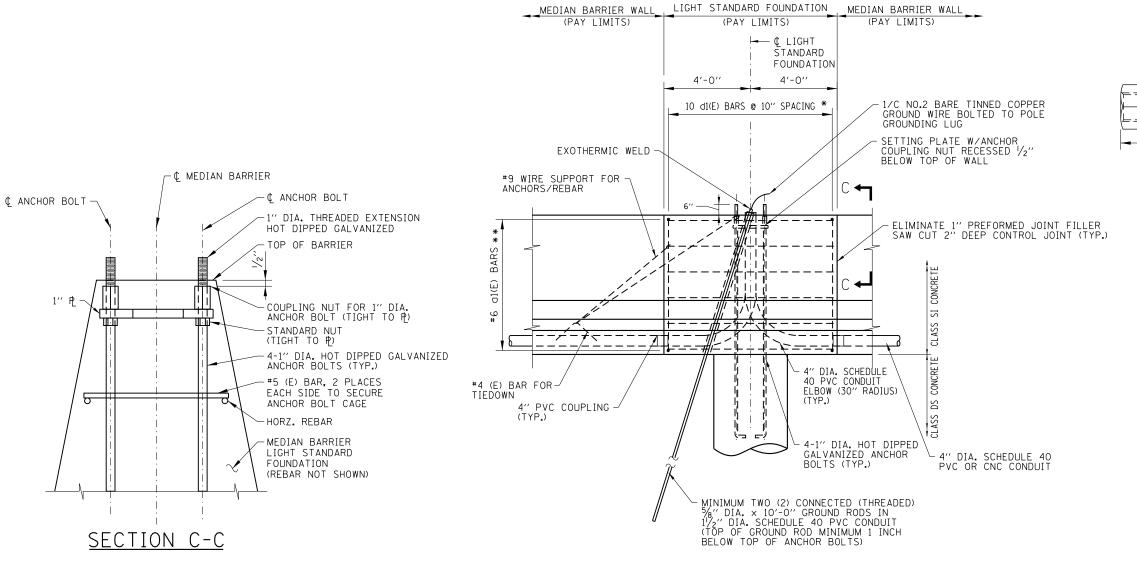
FOUNDATIO

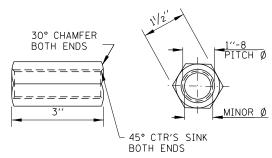
LIGHT STANDARD FOUNDATION DETAILS - HELIX (GROUND MOUNTED UNITS)



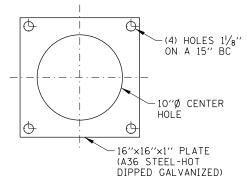








# COUPLING NUT



SETTING PLATE

ELEVATION

\* #6 d1(E) BAR REPLACES #4 d1(E) BAR \*\* #6 a1(E) BAR REPLACES #4 a1(E) BAR

NOTES:

- 1. SEE SHEET 1 OF THIS SERIES FOR NOTES.
- 2. PLUG TOP OF COUPLER WITH PLASTIC PLUG OR COVER WHILE PLACING CONCRETE.

SHEET 6 OF 9

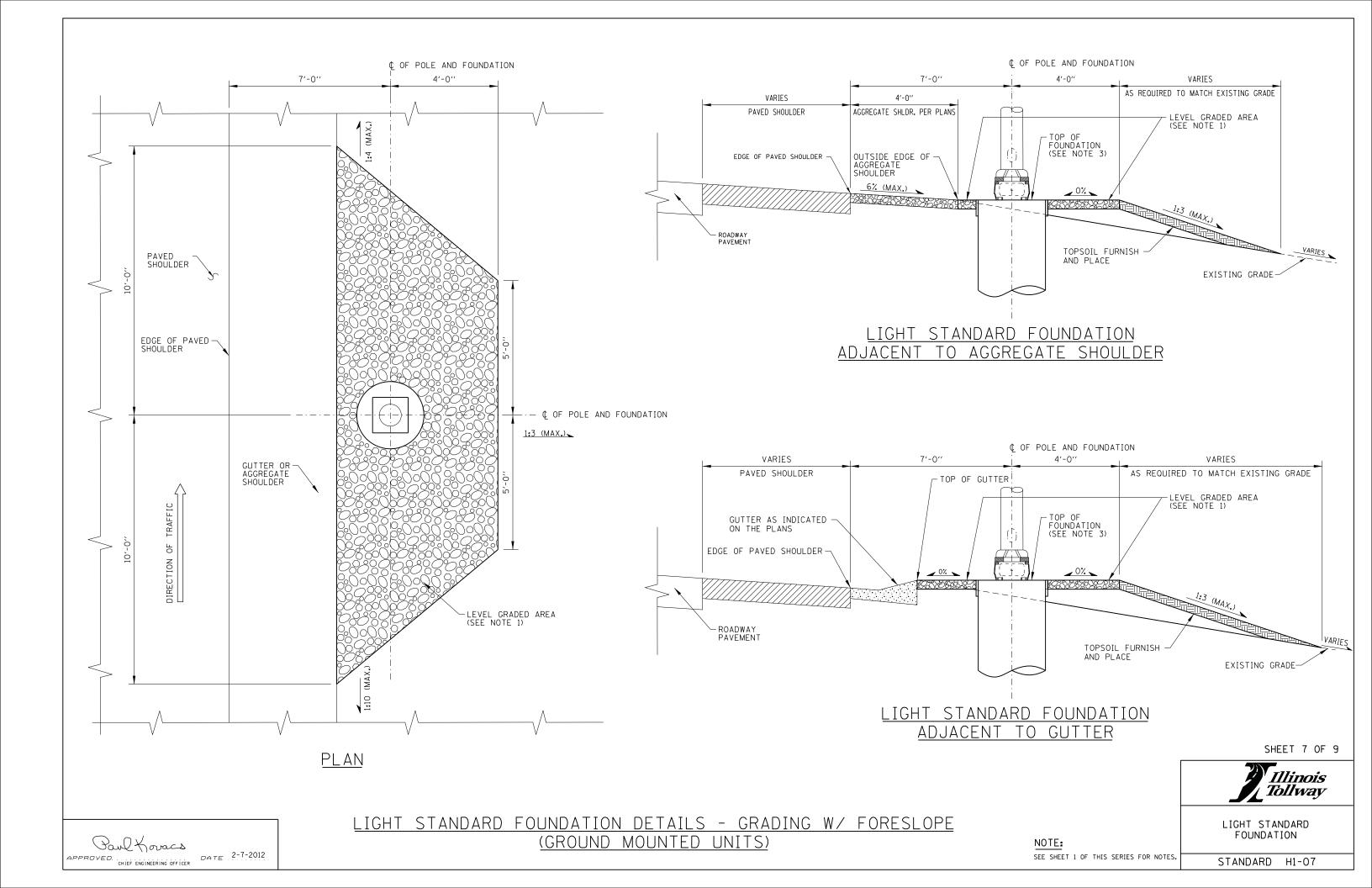
Illinois Tollway

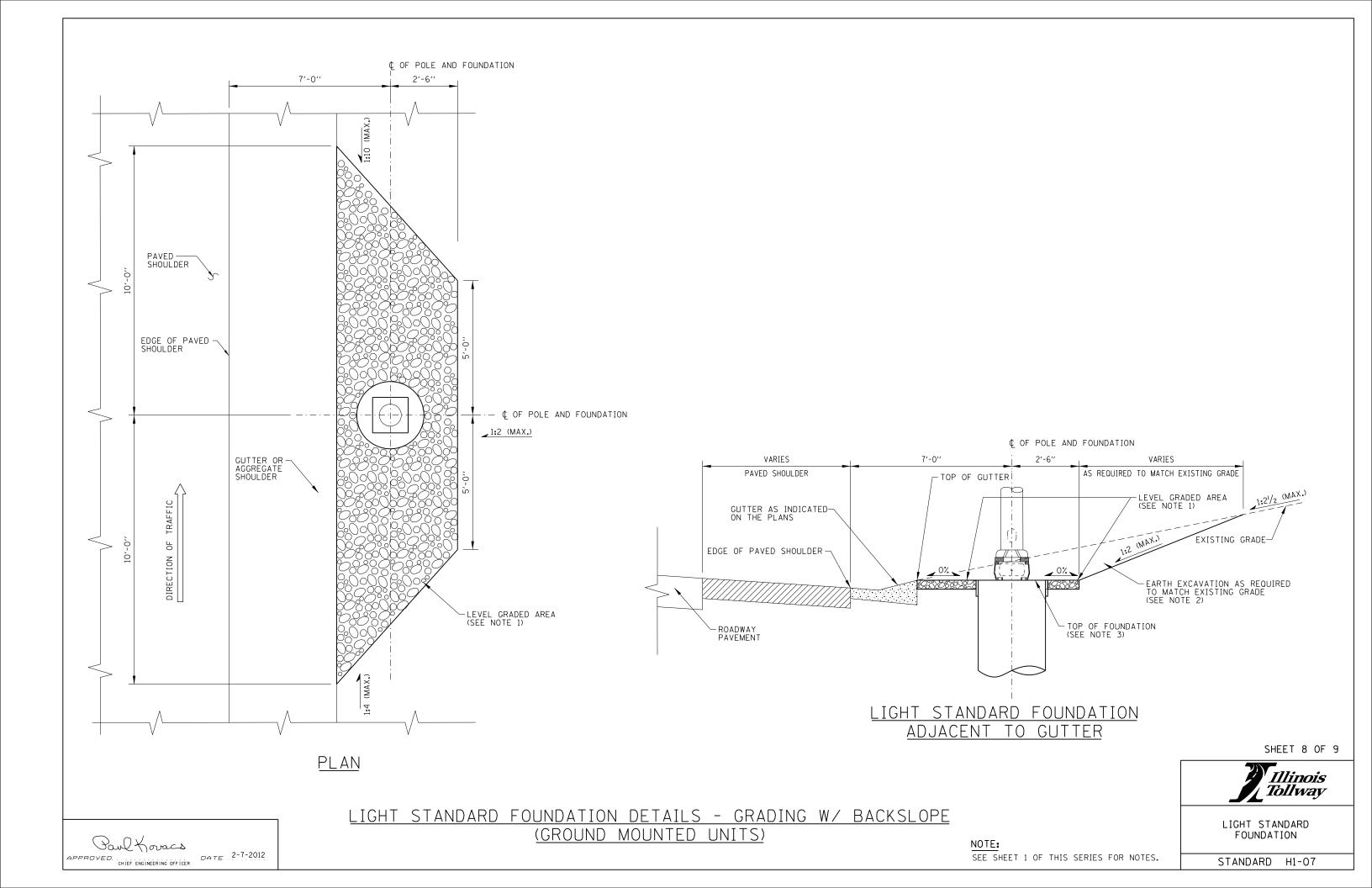
LIGHT STANDARD
FOUNDATION

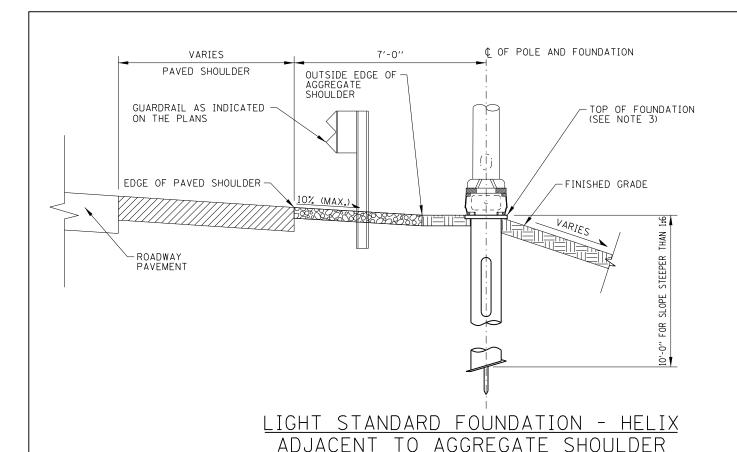
STANDARD H1-07

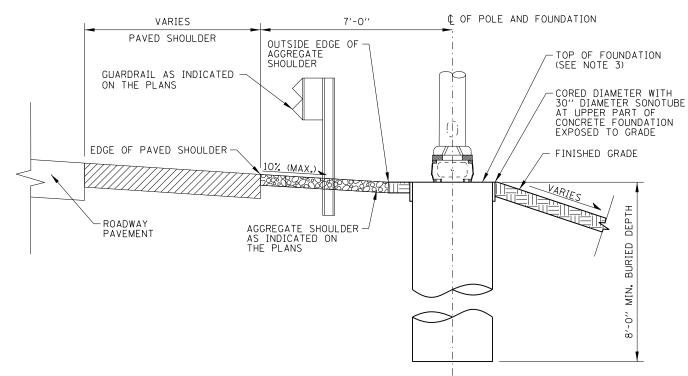
Paul Kovacs
APPROVED. CHIEF ENGINEERING OFFICER
DATE 2-7-2012

LIGHT STANDARD FOUNDATION DETAILS - MEDIAN BARRIER (MODIFICATIONS FOR SLIPFORM POUR, 42" BARRIER)

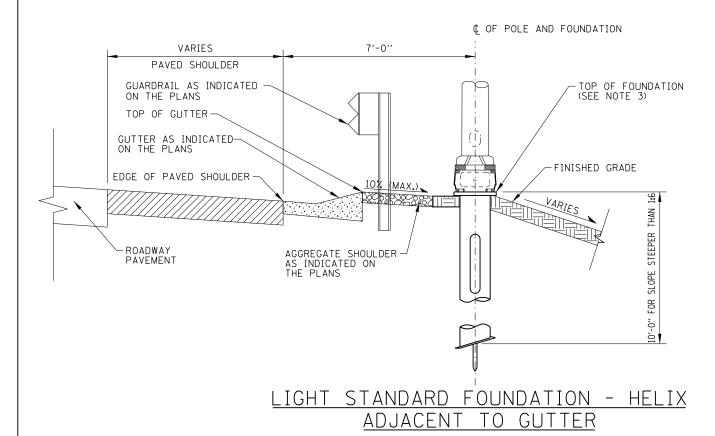


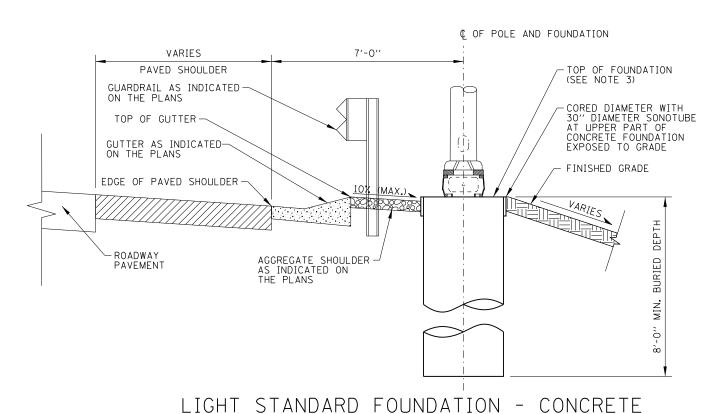






# LIGHT STANDARD FOUNDATION - CONCRETE ADJACENT TO AGGREGATE SHOULDER





LIGHT STANDARD FOUNDATION DETAILS - ADJACENT TO GUARDRAIL (GROUND MOUNTED UNITS)

ADJACENT TO GUTTER

SEE SHEET 1 OF THIS SERIES FOR NOTES.

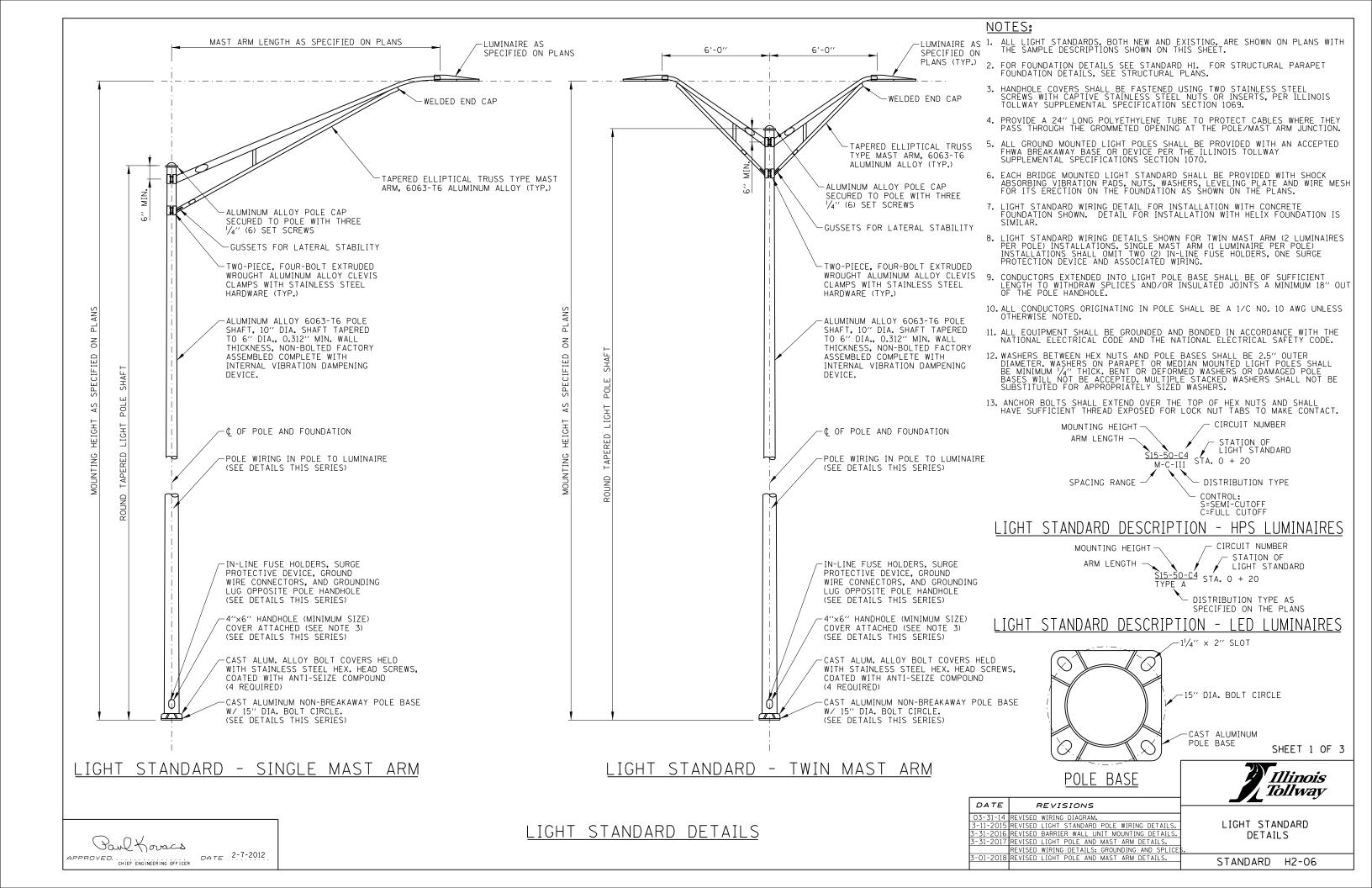
Illinois *Tollway* 

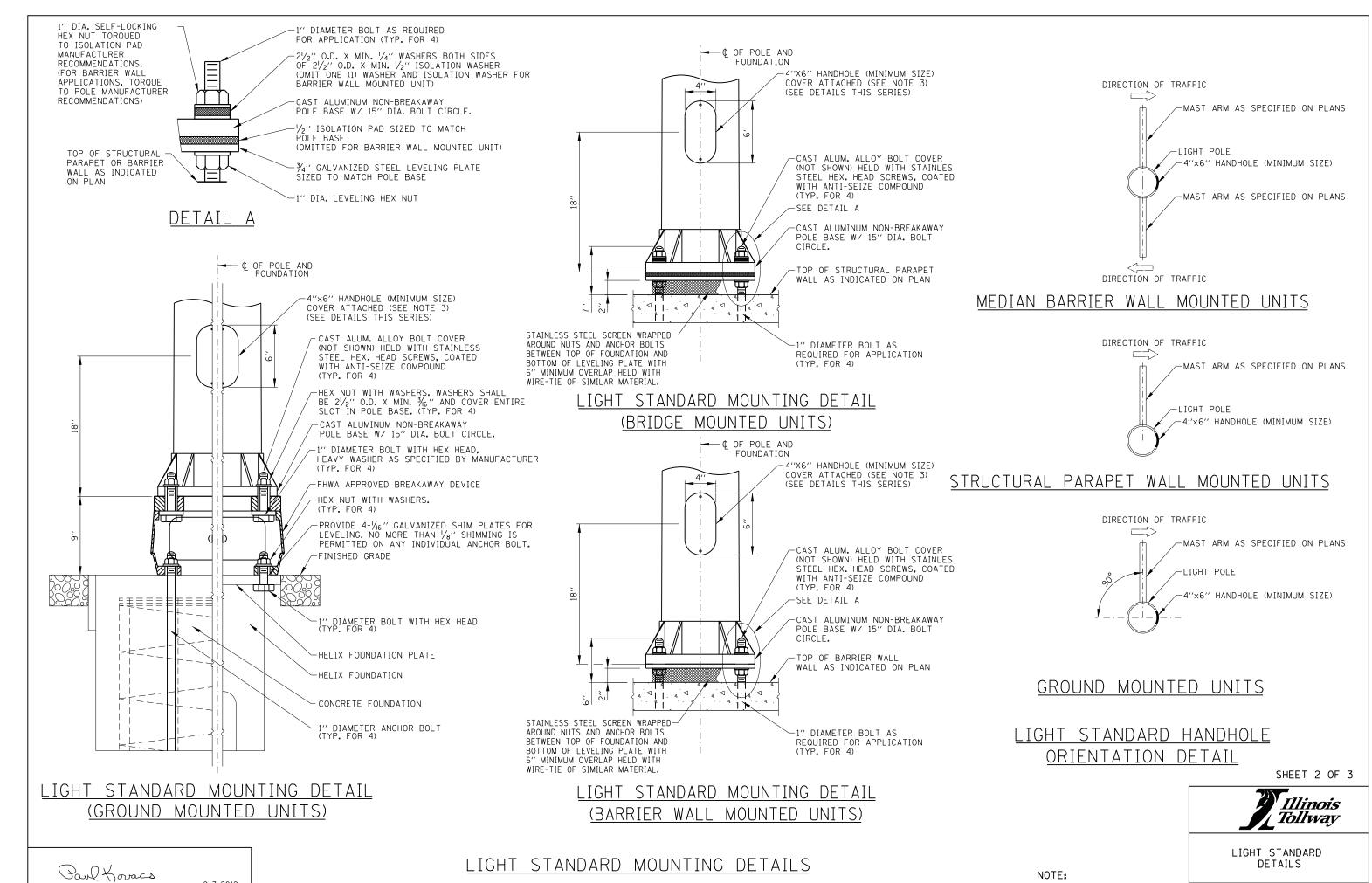
SHEET 9 OF 9

LIGHT STANDARD FOUNDATION

STANDARD H1-07

Paul Koracs APPROVED. CHIEF ENGINEERING OFFICER

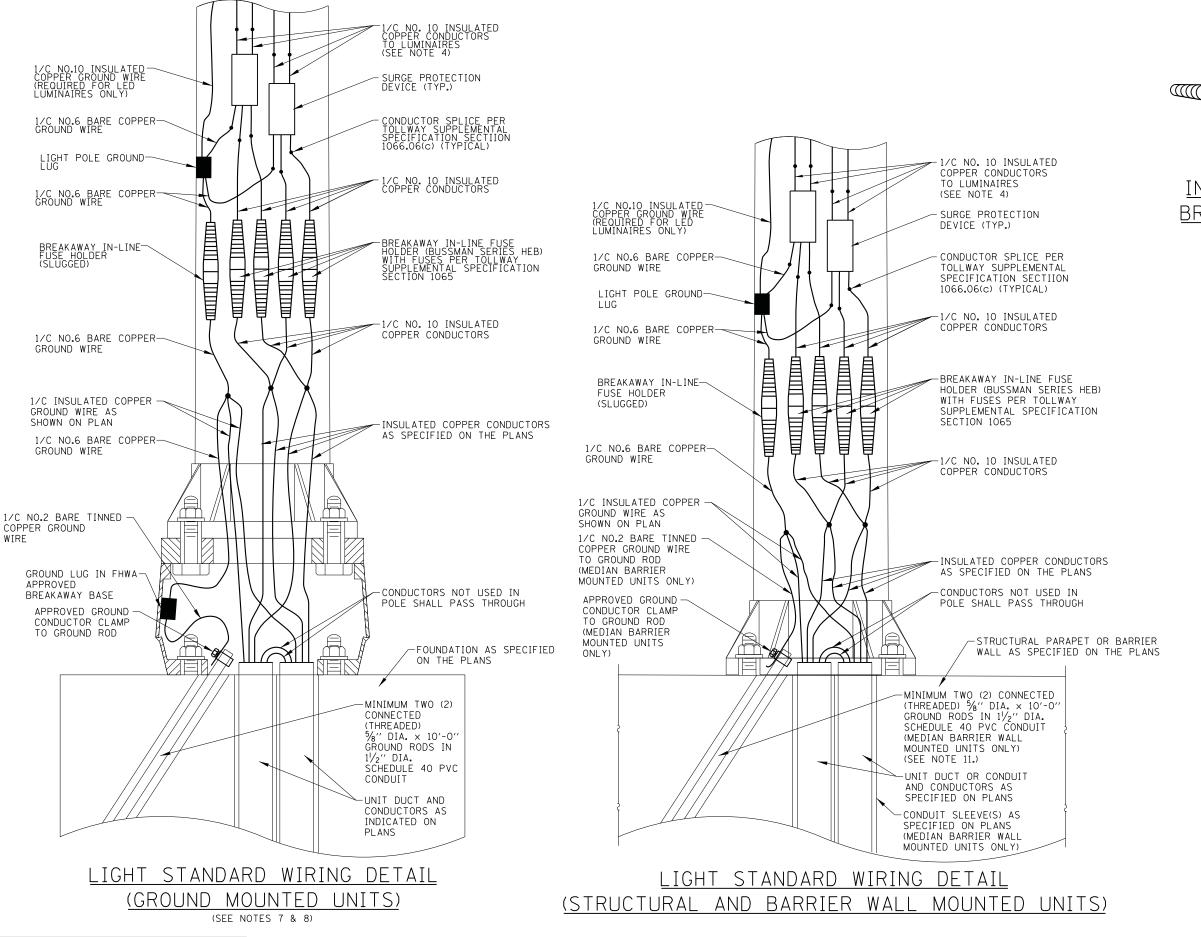




APPROVED. CHIEF ENGINEERING OFFICER DATE 2-7-2012

SEE SHEET 1 OF THIS SERIES FOR NOTES.

STANDARD H2-06



'A' OR 'B' TYPE BREAKAWAY RECEPTACLE

FUSE HOLDER L-TYPE INSULATING BOOT

IN-LINE FUSE HOLDER WITH BREAKAWAY FEATURE DETAIL

SHEET 3 OF 3



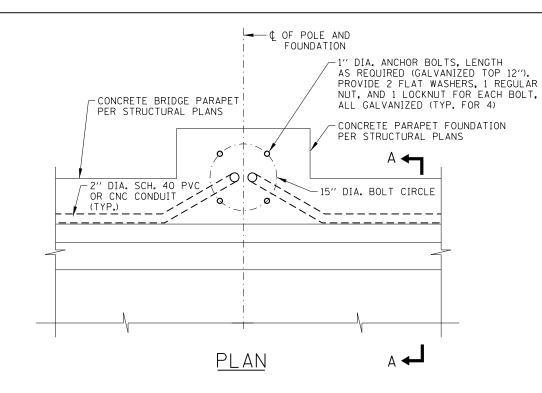
LIGHT STANDARD DETAILS

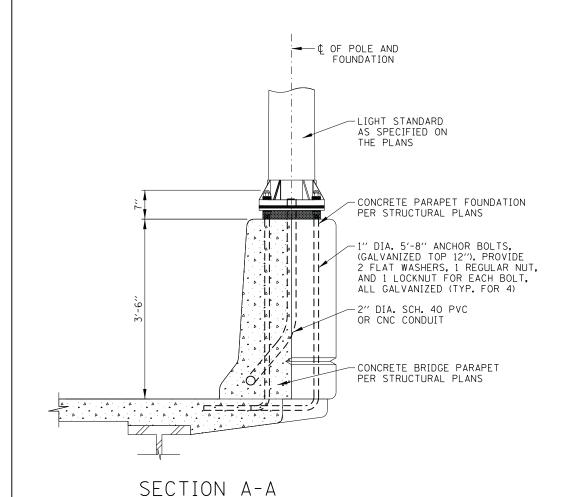
STANDARD H2-06

NOT

SEE SHEET 1 OF THIS SERIES FOR NOTES.

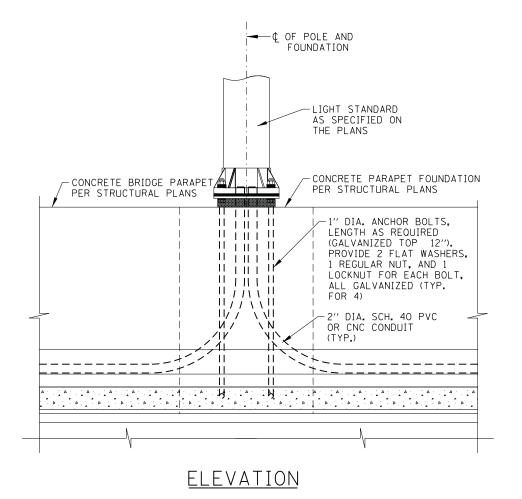
LIGHT STANDARD WIRING DETAILS





Paul Koracs

APPROVED CHIEF ENGINEERING OFFICER



# NOTES:

- FOR STRUCTURAL PARAPET FOUNDATION DETAILS, SEE STRUCTURAL PLANS.
- 2. THE END 4'-O'' SECTION OF WINGWALL/PARAPET SHALL BE KEPT FREE FROM ANY ATTACHMENTS TO AVOID CONFLICT FROM TRAFFIC BARRIER TERMINAL.
- 3. ALL CONDUIT, JUNCTION BOXES AND APPURTENANCES MOUNTED TO STRUCTURE SHALL BE OFFSET FROM THE FACE OF THE STRUCTURE A MINIMUM OF ONE (1) INCH BY MEANS OF A STAINLESS STEEL C-CHANNEL. C-CHANNEL SHALL BE SECURED TO BRIDGE PARAPET WITH 1/2" DIA. EXPANSION ANCHORS (MIN. 2" LONG). EXPANSION ANCHORS SHALL BE HOT DIPPED GALVANIZED AFTER FABRICATION AND SHALL BE MADE BY PARABOLT, KWICK-BOLT OR WEJ-IT. CONDUIT SHALL BE SECURED WITH APPROVED CLAMPS A MINIMUM OF 5 FEET FROM CENTER AND A MINIMUM OF 2 FEET FROM ANY CHANGE IN DIRECTION OR JUNCTION BOX.
- 4. THE BARREL IN THE EXPANSION JOINT FITTING SHALL BE FULLY EMBEDDED IN THE CONCRETE ON ONE SIDE OF THE EXPANSION JOINT. ONE HALF THE LENGTH OF THE DEFLECTION FITTING SHALL BE EMBEDDED IN THE CONCRETE ON THE OTHER SIDE OF THE EXPANSION JOINT.
- 5. EXPANSION/DEFLECTION JOINTS SHALL BE PROVIDED AT ALL BRIDGE EXPANSION JOINTS.
- 6. ALL CLAMPS AND HARDWARE FOR CONDUIT MOUNTING SHALL BE OF LIKE MATERIAL AS THE CONDUIT.
- 7. ALL EQUIPMENT SHALL BE GROUNDED AND BONDED IN ACCORDANCE WITH THE NATIONAL ELECTRICAL CODE AND THE NATIONAL ELECTRICAL SAFETY CODE.

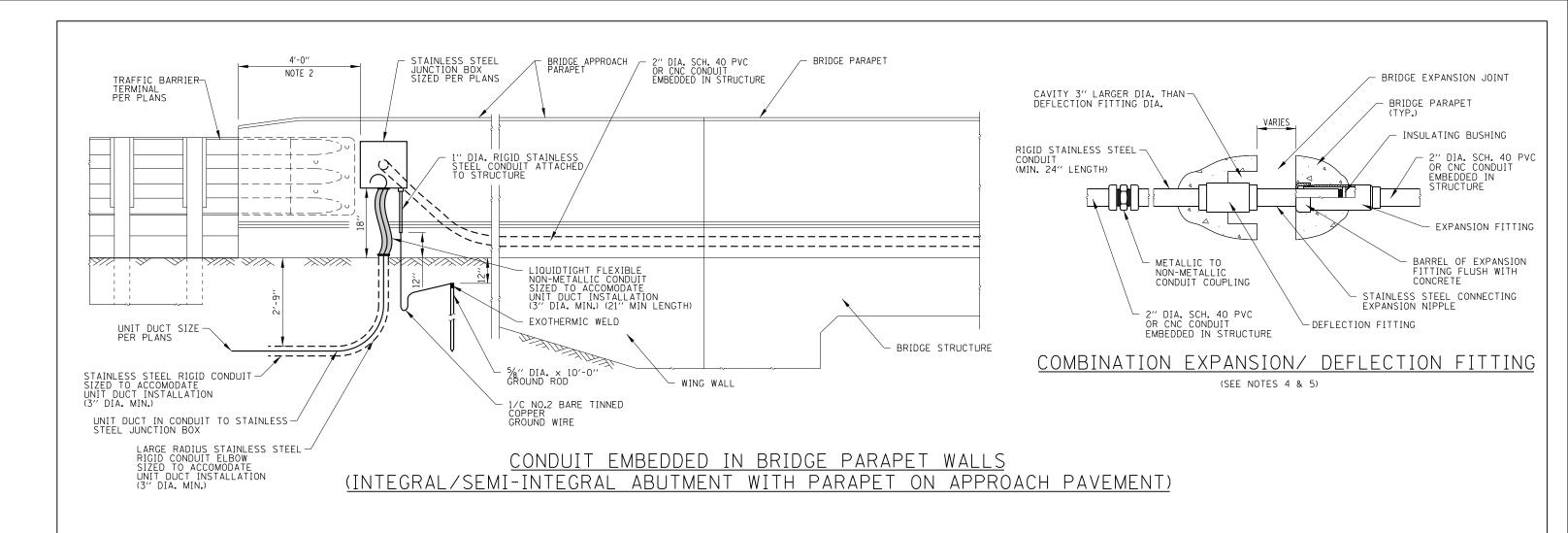
SHEET 1 OF 4

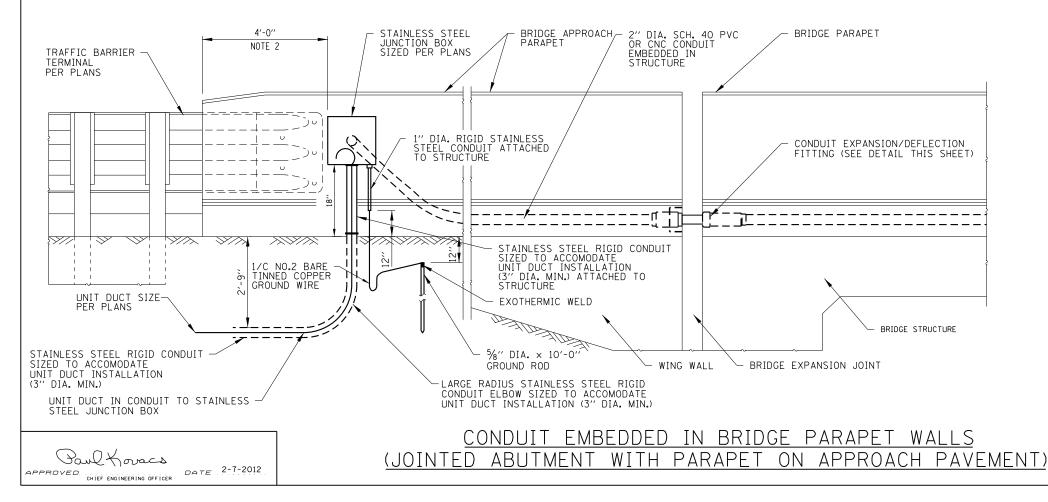


DATE REVISIONS
P-07-2012 REVISED NOTES
1-01-2012 REVISED JUNCTION BOX
3-11-2015 ADDED BRIDGE CONDUIT DETAILS
3-31-2017 REVISED NOTES
REVISED NOTES
REVISED APPROACH PARAPET CLEAR AREA DIM.
3-01-2018 TYPOGRAPHICAL CORRECTIONS.

STANDARD H3-05

CONDUIT EMBEDDED IN BRIDGE PARAPET





SHEET 2 OF 4

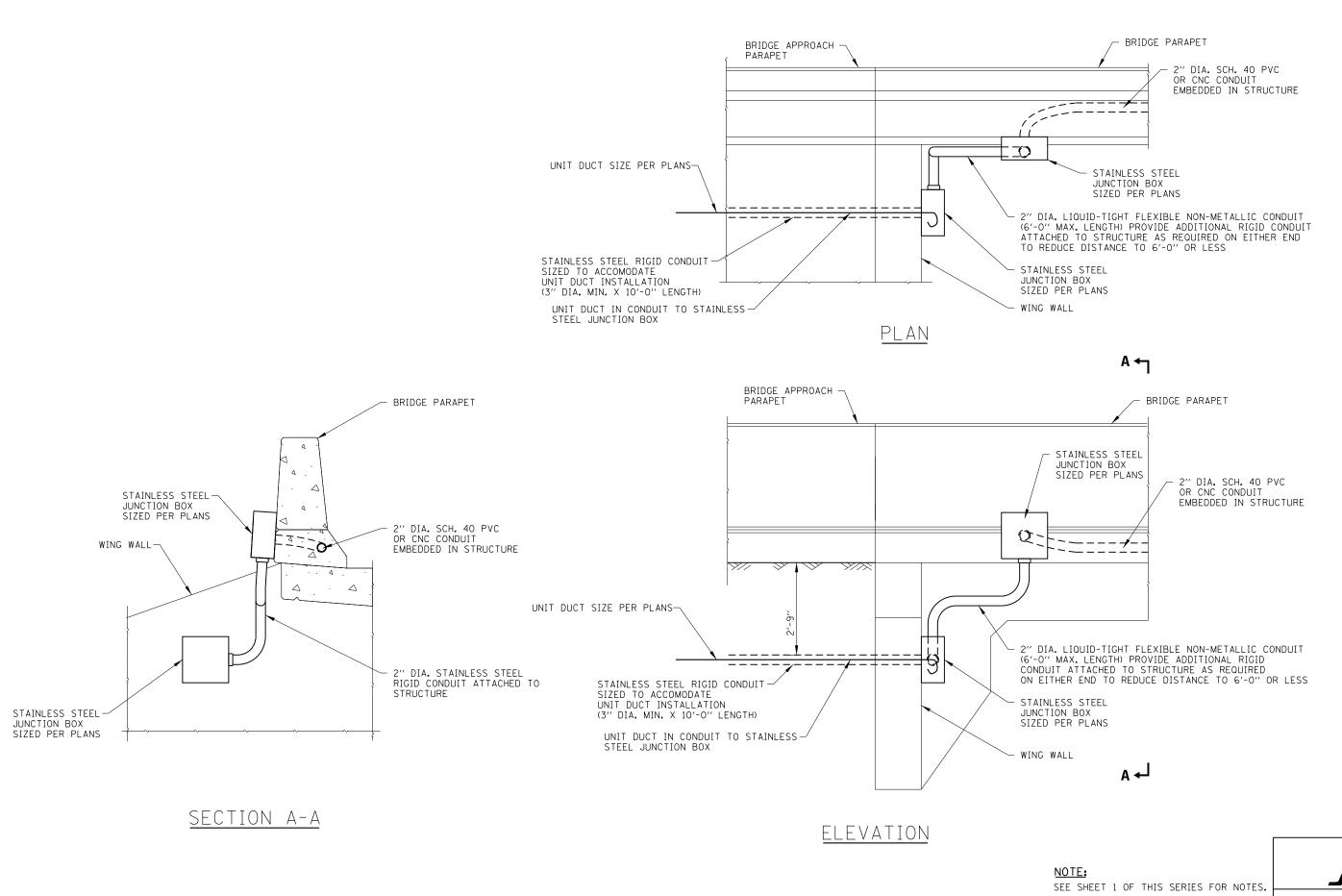


BRIDGE CONDUIT DETAILS

NOTE:

SEE SHEET 1 OF THIS SERIES FOR NOTES.

STANDARD H3-05



CONDUIT EMBEDDED IN BRIDGE PARAPET WALLS
(INTEGRAL/SEMI-INTEGRAL ABUTMENT WITH PARAPET ENDING ON BRIDGE DECK)

Paul Koracs

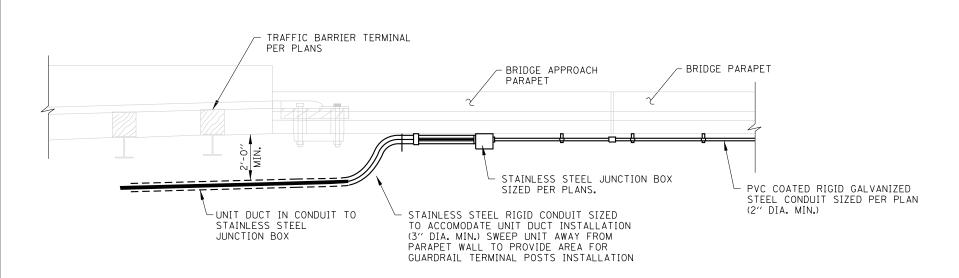
APPROVED CHIEF ENGINEERING OFFICER

SHEET 3 OF 4

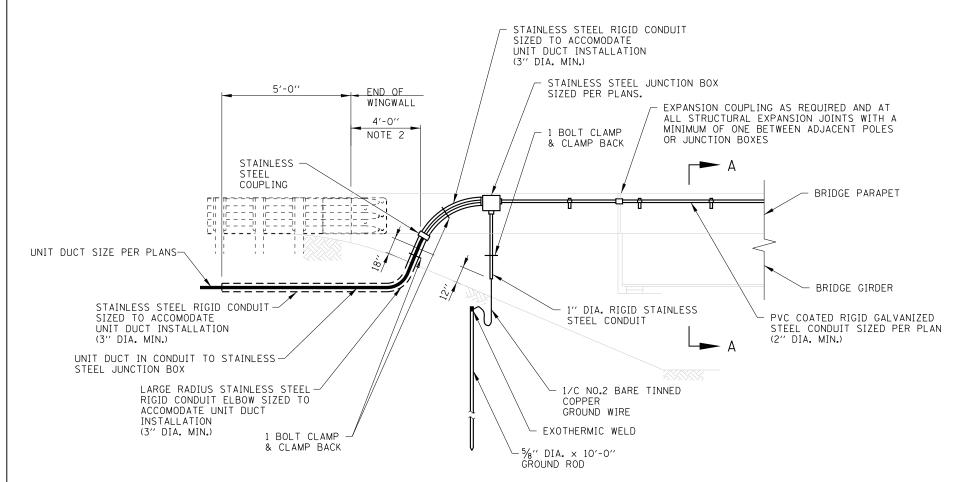


BRIDGE CONDUIT DETAILS

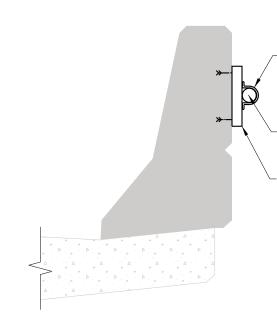
STANDARD H3-05



# PLAN VIEW



ELEVATION OF TYPICAL WINGWALL CONDUIT TRANSITION



PIPE SUPPORT (PVC COATED GALVANIZED STEEL), MINIMUM SIZE EQUAL TO PIPE DIAMETER. MOUNT TO CHANNEL WITH TWO 3/8" STAINLESS STEEL CLAMPING NUTS, HEX HEAD CAP SCREW & LOCK WASHER, MOUNTED ON 5 FEET CENTERS

PVC COATED RIGID GALVANIZED STEEL CONDUIT SIZED PER PLAN (2" DIA. MIN.)

STAINLESS STEEL C-CHANNEL, 10" LONG MOUNTED EXTERNALLY ON BRIDGE PARAPET ON 5'-0" CENTERS. ATTACH TO BRIDGE PARAPET WITH 1/2" DIA. EXPANSION ANCHORS (MIN. 2" LONG). EXPANSION ANCHORS SHALL BE HOT DIPPED GALVANIZED AFTER FABRICATION AND SHALL BE MADE BY PARABOLT, KWIK-BOLT OR WEJ-IT

SECTION A-A

SHEET 4 OF 4



BRIDGE

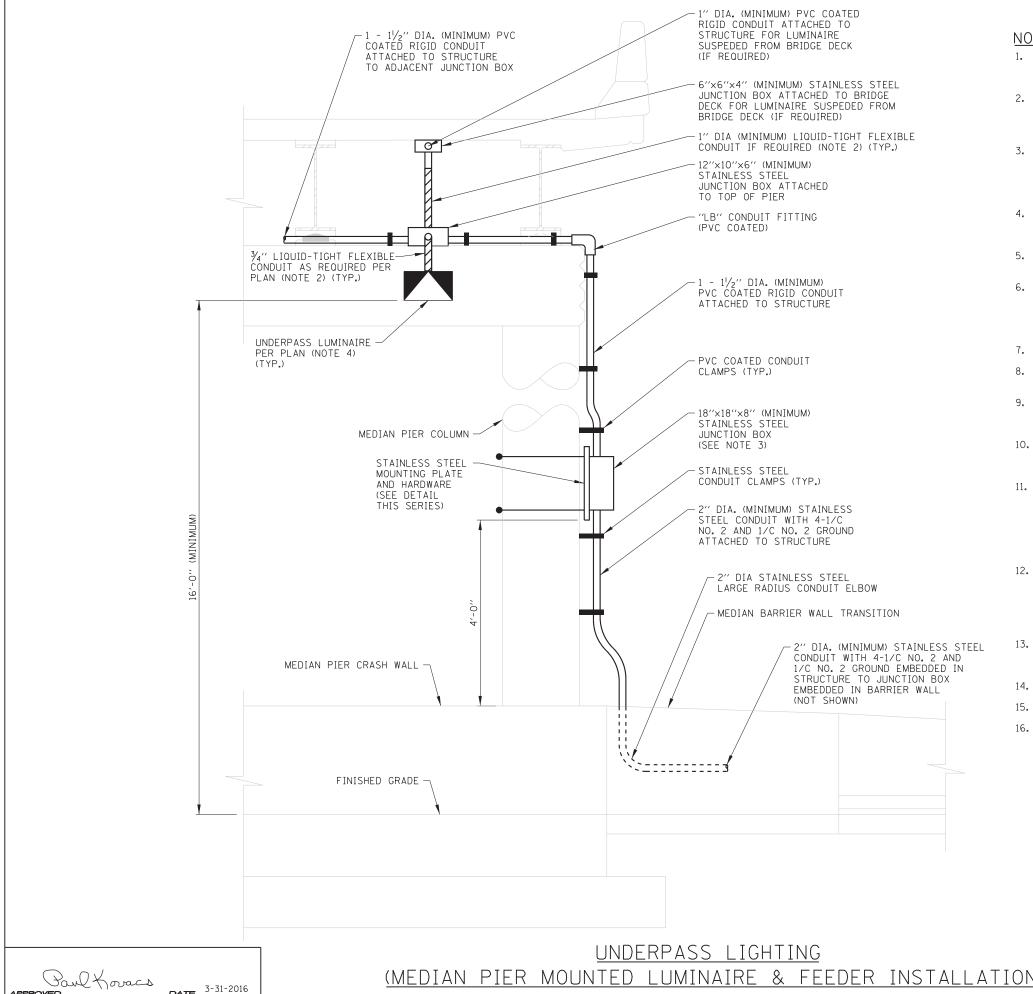
CONDUIT DETAILS

CONDUIT ATTACHED TO BRIDGE PARAPET

Paul Koracs APPROVED CHIEF ENGINEERING OFFICER

SEE SHEET 1 OF THIS SERIES FOR NOTES.

STANDARD H3-05



DATE 3-31-2016

APPROVED.

CHIEF ENGINEER

# NOTES:

- USE OF THIS STANDARD DETAIL IS LIMITED TO THE INSTALLATION OF LIGHT EMITTING DIODE LUMINAIRES ONLY. FOR INSTALLATION OF OTHER LIGHT SOURCE TYPES, REFER TO PLAN DETAILS
- LIQUID-TIGHT FLEXIBLE CONDUIT, MAXIMUM LENGTH 6'-O", TYPICAL FOR EACH INSTANCE AS SHOWN, PROVIDE SUFFICIENT LENGTH OF PVC COATED RIGID GALVANIZED STEEL CONDUIT AS REQUIRED CONDUIT AS REQUIRED SO THE MAXIMUM LENGTH OF REQUIRED LIQUID-TIGHT DOES NOT EXCEED 6'-O". LIQUID-TIGHT FLEXIBLE CONDUIT.
- PROVIDE TWO (2) 2-POLE 30A, 600 VOLT CIRCUIT BREAKERS (EATON HFD OR APPROVED EQUAL), TWO (2) SURGE PROTECTION DEVICES (IN ACCORDANCE WITH ARTICLE 1065.02 OF THE STANDARD SPECIFICATIONS) AND SUFFICIENT 30 AMPERE, 600 VOLT TERMINAL BLOCKS TO SPLIT 480 VOLT WIRING FROM CIRCUIT BREAKER TO TWO (2) NO. 10 WIRES FOR EACH LUMINAIRE.
- WIRING SHALL BE 2-1/C NO. 10 WITH 1/C NO. 10 GROUND OR AS INDICATED ON THE PLANS TERMINATING AT EACH LUMINAIRE. SEE PLANS FOR REMAINING WIRING
- THE CONTRACTOR SHALL PROVIDE EXPANSION/DEFLECTION FITTINGS (O-Z/GEDNEY TYPE AXDX) WHERE CONDUITS CROSS STRUCTURE EXPANSION JOINTS.
- IN NEW BRIDGE DECKS, PROVIDE STAINLESS STEEL SINGLE COIL, FLARED LOOP INSERTS CAST IN THE DECK FOR  $\frac{3}{4}$ " DIAMETER STAINLESS STEEL THREADED RODS. IN EXISTING BRIDGE DECKS, PROVIDE DRILLED STAINLESS STEEL EXPANSION TYPE ANCHORS FOR 3/4" DIAMETER STAINLESS STEEL THREADED RODS. EXPANSION TYPE ANCHORS SHALL HAVE A MINIMUM OF 500 POUNDS CAPACITY EACH.
- ALL ITEMS MOUNTED TO BRIDGE PIER SHALL BE OFFSET FROM THE STRUCTURE A MINIMUM OF ONE (1) INCH BY USE OF STAINLESS STEEL C-CHANNEL.
- WHERE BEAM DEPTH EXCEEDS FIVE (5) FEET, THE DESIGNER SHALL PROVIDE A METHOD FOR ATTACHMENT OF THE HANGER ASSEMBLIES SUCH THAT THE LENGTH OF THE ASSEMBLIES DO NOT EXCEED FIVE (5) FEET.
- DETAILS SHOWN ARE FOR UNDERPASS LIGHTING INSTALLATIONS FED FROM THE MEDIAN BARRIER WALL. FOR INSTALLATIONS FED FROM A BRIDGE ABUTMENT, REFER TO THE
- UNDERPASS LUMINAIRES SUSPENDED FROM BRIDGE DECK SHALL BE INSTALLED CENTERED BETWEEN THE BRIDGE BEAMS. THE LUMINAIRE SHALL BE LOCATED SUCH THAT IT IS SETBACK A MINIMUM OF 1 FOOT FROM THE OUTSIDE EDGE OF THE SHOULDER PAVEMENT WITH THE TOP OF THE LUMINAIRE MOUNTING PLATE A MAXIMUM OF 1 INCH FROM THE BOTTOM OF THE BRIDGE BEAM. IN NO CASE SHALL ANY PORTION OF THE SUSPENDED LUMINAIRE OR SUPPORTING HARDWARE BE LOWER THAN 14'-6" WHEN MEASURED TO THE OUTSIDE EDGE OF THE ADJACENT SHOULDER PAVEMENT.
- IN NO INSTANCE SHALL ANY UNDERPASS LUMINAIRE OR ANY OTHER ELECTRICAL EQUIPMENT BE INSTALLED BELOW THE ELEVATION OF THE BOTTOM OF THE BRIDGE BEAM WHEN OVER ANY PAVEMENT (ROADWAY OR SHOULDER) WITH EXCEPTION OF THOSE MOUNTED TO THE MEDIAN PIER AT WHICH CASE THE MINIMUM HEIGHT SHALL BE 16'-0" WHEN MEASURED TO THE LOWEST PORTION OF THE LUMINAIRE OR SUPPORTING HARDWARF.
- 13. LUMINAIRE MOUNTING PLATE FOR LUMINAIRES SUPENDED FROM BRIDGE DECK SHALL BE OF THE DIMENSIONS NECESSARY AND FIELD DRILLED TO ACCOMODATE THE SPECIFIC LUMINAIRE PROVIDED AND ASSOCIATE LUMINAIRE HANGER ASSEMBLIES.
- 14. SEE PLANS FOR UNDERPASS LUMINAIRE LOCATIONS AND MOUNTING HEIGHTS.
- 15. SEE STRUCTURAL DRAWINGS FOR SPECIFIC STRUCTURE DETAILS.
- ALL EQUIPMENT SHALL BE GROUNDED AND BONDED IN ACCORDANCE WITH THE NATIONAL ELECTRICAL CODE AND THE NATIONAL ELECTRICAL SAFETY CODE.

03-31-17 Revised Notes to remove

SHEET 1 OF 3

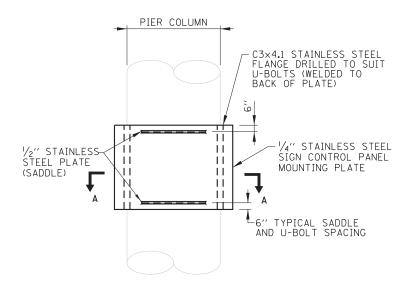
Illinois *Tollway* DATE REVISIONS

(MEDIAN PIER MOUNTED LUMINAIRE & FEEDER INSTALLATION)

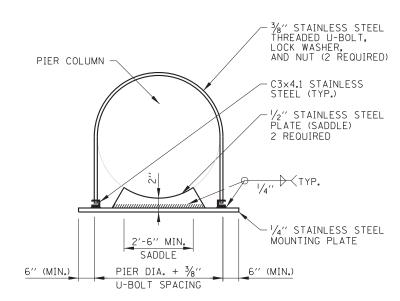
STANDARD H9-01

UNDERPASS LIGHTING

INSTALLATION DETAILS

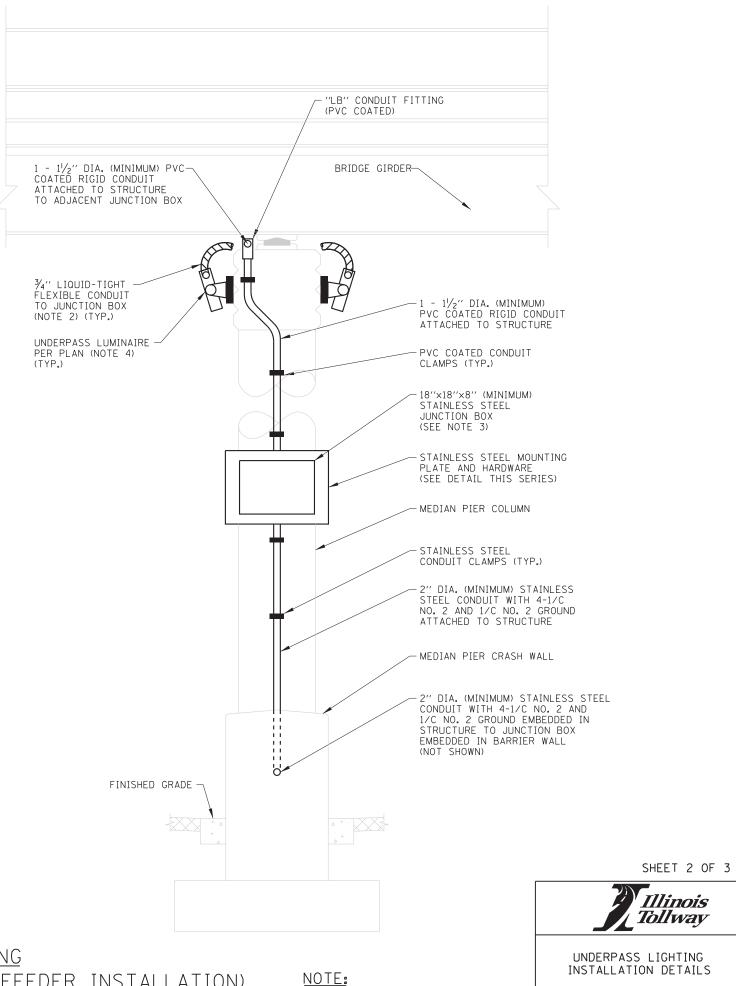


# ELEVATION



SECTION A-A

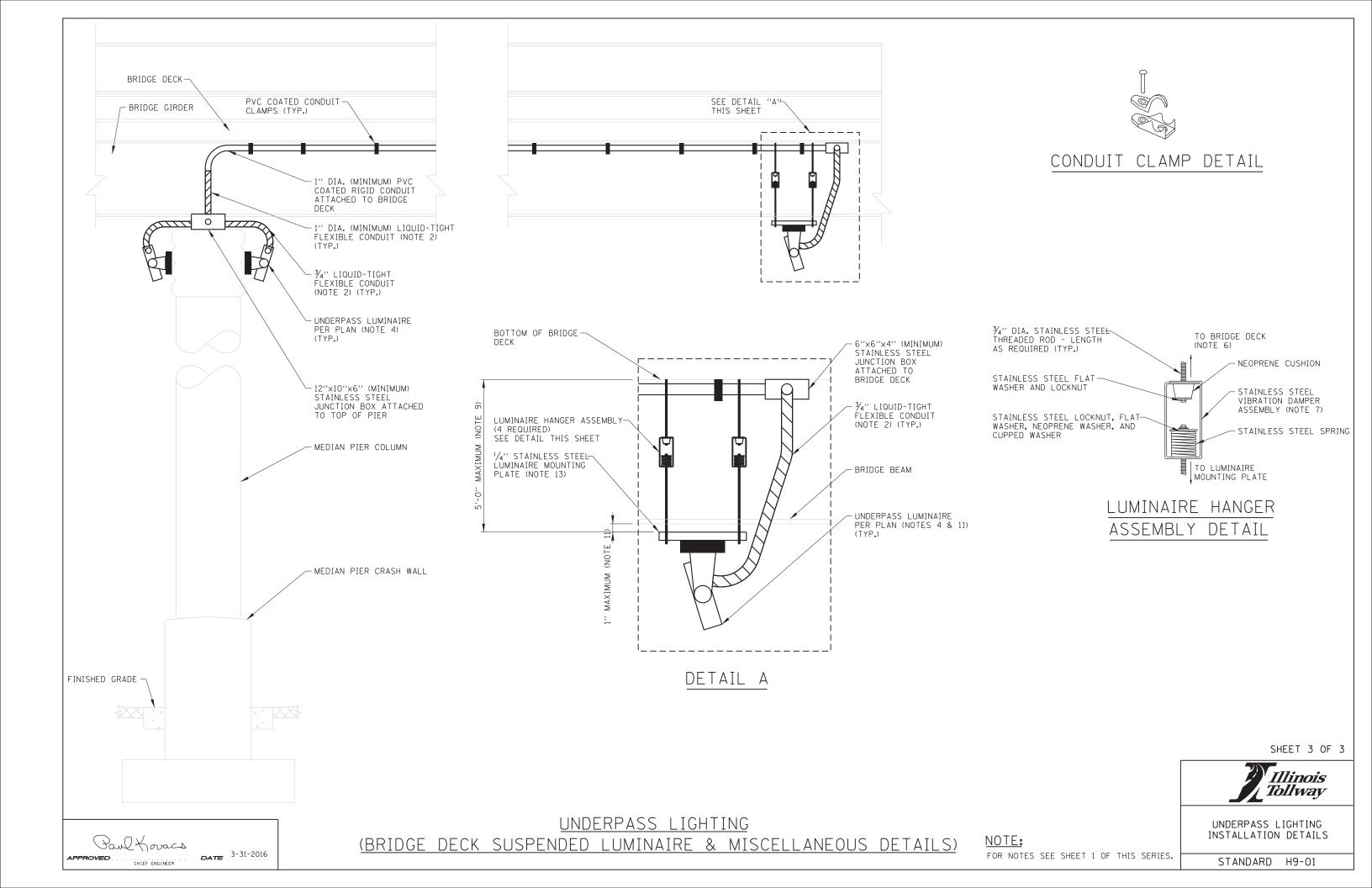
MEDIAN PIER JUNCTION BOX MOUNTING PLATE DETAIL



UNDERPASS LIGHTING (MEDIAN PIER MOUNTED LUMINAIRE & FEEDER INSTALLATION)

FOR NOTES SEE SHEET 1 OF THIS SERIES.

STANDARD H9-01



- 1. THE WORK DESCRIBED ON THESE DRAWINGS IS AN INTEGRAL PART OF THE STORM WATER POLLUTION PREVENTION PLAN USED TO OBTAIN A NPDES PERMIT FROM IEPA FOR THE CONSTRUCTION OF THIS PROJECT.
- 2. THE PURPOSE OF THE EROSION AND SEDIMENT CONTROL MEASURES INCLUDED FOR THIS PROJECT IS TO LIMIT THE SEDIMENT POLLUTION IMPACT OF ANY STORM WATER DISCHARGES THAT ORIGINATE ON THIS SITE OR OFF-SITE FLOWS THAT FLOW OVER THE DISTURBED AREAS.
- 3. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ENSURE THAT SEDIMENT TRANSPORT OFF THE SITE IS REDUCED BY A COMBINATION OF MINIMIZATION OF EROSION AT THE SOURCE AND INSTALLATION OF SPECIFIC MEASURES TO CONTROL OR REDUCE THE TRANSPORT OF SEDIMENT. A COPY OF THE EROSION AND SEDIMENT CONTROL PLAN, NOI, SWPPP, AND INSPECTION LOG BEING IMPLEMENTED BY THE CONTRACTOR SHALL BE ON THE CONSTRUCTION SITE AT ALL TIMES.
- 4. TO THE MAXIMUM EXTENT POSSIBLE EROSION SHALL BE MINIMIZED AT THE SOURCE. ALL FLOWS ORIGINATING OFF THE CONSTRUCTION SITE SHALL BE DIVERTED AROUND DISTURBED AREAS OR SHALL BE CONVEYED THROUGH THE SITE IN A MANNER THAT UNTREATED ON-SITE RUNOFF, SHALL BE MINIMIZED AND DOES NOT MIX WITH THE OFF-SITE RUNOFF.
- 5. ALL RUNOFF ORIGINATING ON DISTURBED AREAS ASSOCIATED WITH THIS PROJECT WILL PASS THROUGH ONE OR MORE MEASURES THAT WILL MINIMIZE THE OFF-SITE SEDIMENT IMPACTS OF THE CONSTRUCTION ACTIVITY.
- 6. ALL PERMANENT SEDIMENT BASINS, PERMANENT STORM WATER CONTROL MEASURES, AND RUNOFF CONTROL MEASURES REQUIRED TO KEEP OFF-SITE RUNOFF FROM FLOWING OVER THE CONSTRUCTION AREA WILL BE INSTALLED BEFORE CLEARING AND STRIPPING OF THE SITE PROCEEDS. PRIOR TO PROCEEDING WITH EARTHWORK ON A PROJECT THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER A PROPOSED EARTHWORK AND STABILIZATION SCHEDULE FOR REVIEW AND APPROVAL.
- 7. A MAXIMUM OF 10 ACRES IS ALLOWED TO BE IN SOME STAGE OF GRADING AT A SINGLE TIME. ADDITIONAL AREAS (UP TO 10 ACRES) MAY BE CLEARED BUT SHALL NOT BE STRIPPED OF VEGETATION UNTIL THE GRADED AREAS HAVE BEEN PROTECTED FROM EROSION THROUGH INSTALLATION OF EITHER TEMPORARY OR PERMANENT MEASURES. WHENEVER POSSIBLE, THE GRADING SHALL BE COMPLETED TO THE DESIGN GRADE AND THE PERMANENT VEGETATION PLAN IMPLEMENTED PRIOR TO STARTING GRADING ACTIVITIES ON THE NEXT SITE.
  - A. WHEN BALANCING EARTHWORK (BORROW FROM A CUT USED AS FILL AT A LOCATION DISTANT FROM THE CUT) THE CHIEF ENGINEER WILL CONSIDER ALLOWING MORE THAN 10 ACRES OF CONSTRUCTION WORK AREAS AND STORAGE AREAS.
  - B. WHERE NEW INTERCHANGES ARE BEING CONSTRUCTED THE ALLOWABLE AREA BEING GRADED MAY BE LARGER THAN 10 ACRES WHEN THE CONTRACT DRAWINGS AND SWPPP DEFINE SUCH INCREASES.
  - C. VARIATIONS TO THE ABOVE MAY BE CONSIDERED BY THE CHIEF ENGINEER UNDER ALL THE FOLLOWING CONDITIONS:
    - IF THE CONTRACTOR FALLS BEHIND SCHEDULE THROUGH NO FAULT OF HIS OWN.
    - THE CONTRACTOR MUST PRESENT A SCHEDULE DEMONSTRATING THE NEED FOR SUCH VARIATION IN ORDER TO COMPLETE THE WORK ON TIME.
    - THE CONTRACTOR MUST COMPLY WITH ALL OTHER CONTRACT AND PERMIT REQUIREMENTS.
- 8. DISTURBED AREAS ARE TO BE PROTECTED FROM EROSION IN A TIMELY MANNER. UPON COMPLETION OF GRADING OR CONSTRUCTION, THE AREA SHALL BE STABILIZED (USING PERMANENT MEASURES WHEN POSSIBLE) WITHIN 7 CALENDAR

Paul Kovacs

APPROVED. .... CHIEF ENGINEERING OFFICER

DATE 2-7-2012

## GENERAL NOTES - EROSION AND SEDIMENT CONTROLS

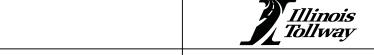
DAYS. TEMPORARY STABILIZATION THROUGH USE OF GROUND COVER, MULCHING, OR OTHER APPROVED MEASURES WILL BE INSTALLED WHENEVER SITE DEVELOPMENT WORK, GRADING OR OTHER EARTH DISTURBING ACTIVITIES CEASE TO BE CONTINUOUS FOR A PERIOD EXCEEDING 14 CALENDAR DAYS. THE 7/14 DAY REQUIREMENT IS TAKEN TO MEAN THAT THE STABILIZATION OPERATION IS COMPLETE OR NEARING COMPLETION IN THE DEFINED TIME.

- 9. STABILIZATION OF CUT OR FILL SLOPES WITH TEMPORARY OR PERMANENT EROSION CONTROL MEASURES IS REQUIRED WHENEVER THE CUT OR FILL ACTIVITY REACHES 15 FEET VERTICALLY OR THE FINISHED SLOPE EQUALS 50 FEET, WHICHEVER IS MORE RESTRICTIVE. ONCE THE STABILIZATION MEASURES ARE INSTALLED, THE PLACEMENT OF FILL OR EXCAVATION ACTIVITIES ARE ALLOWED TO PROCEED.
- 10. THE CONTRACTOR SHALL DESIGNATE ONE OF HIS EMPLOYEES AS EROSION AND SEDIMENT CONTROL MANAGER. THIS PERSON WILL BE RESPONSIBLE FOR IMPLEMENTATION OF THE EROSION AND SEDIMENT CONTROL PLAN ON ALL DISTURBED AREAS. THIS PERSON SHALL POSSESS THE NECESSARY TRAINING AND CERTIFICATION ON EROSION AND SEDIMENT CONTROL MEASURES FOR ACCEPTANCE BY THE ILLINOIS TOLLWAY. THIS EMPLOYEE IS TO HAVE THE AUTHORITY TO CARRY OUT THE IMPLEMENTATION OF ANY INSTRUCTIONS CONCERNING THE EROSION AND SEDIMENT CONTROL PLAN GIVEN BY THE ENGINEER. ALL MEASURES WILL BE INSPECTED BY THIS INDIVIDUAL AND THE ENGINEER ON A REGULAR BASIS (AT LEAST ONCE EVERY 7 DAYS) AND AFTER ANY RAINFALL EVENT GREATER THAN 0.5 INCHES, OR EQUIVALENT SNOWFALL (I.E. + 5").
- 11. SEDIMENT TRAPS, SEDIMENT BASINS, DITCHES, SILT FENCES, FENCES, STONE OUTLET STRUCTURES, EARTH BERMS, ETC. SHALL BE MAINTAINED DURING THE CONSTRUCTION SEASON AS WELL AS THE WINTER MONTHS AND OTHER TIMES WHEN THE PROJECT IS CLOSED DOWN. TRAPS WILL BE CLEANED WHEN THEY ARE 50% FILLED. SILT FENCE AND STONE OUTLET STRUCTURES SHALL HAVE SEDIMENT REMOVED WHEN IT REACHES 50% THE HEIGHT OF THE CONTROL DEVICE. THESE SPOILS WILL BE REMOVED TO AN APPROVED SITE.
- 12. SALVAGED TOPSOIL SHALL BE PLACED ON WELL DRAINED LAND AWAY FROM INTERMITTENT AND LIVE STREAMS OR WETLANDS WITH THE APPROPRIATE RUNOFF CONTROL AND SEDIMENT CONTROL MEASURES INSTALLED AROUND THE STORAGE SITE. SALVAGED TOPSOIL SHALL BE STABILIZED WITH STRAW MULCH IMMEDIATELY AFTER SHAPING OF THE PILE IN ACCORDANCE WITH THE ILLINOIS TOLLWAY SUPPLEMENTAL SPECIFICATIONS. SILT FENCE SHALL BE PROVIDED AT THE PERIMETER OF THE STOCKPILE.
- 13. MATERIALS EXCAVATED FOR THE CONSTRUCTION OR CLEAN OUT OF SEDIMENT TRAPS SHALL NOT BE STOCKPILED IN THE VICINITY OF THE TRAP. IT SHALL BE PLACED IN AN EMBANKMENT OR WASTED AS DIRECTED BY THE ENGINEER.
- 14. EXCAVATION TO BE USED FOR EMBANKMENTS SHALL NOT BE STOCKPILED UNLESS PERIMETER CONTROLS ARE UTILIZED. WHEN THIS MATERIAL IS STOCKPILED FOR THE CONVENIENCE OF THE CONTRACTOR THE COST OF PROVIDING THE CONTROLS ARE THE RESPONSIBILITY OF THE CONTRACTOR. IF THE MATERIAL IS STOCKPILED AT THE DIRECTION OF THE ENGINEER THE ILLINOIS TOLLWAY WILL ASSUME THE COSTS OF THE CONTROLS.
- 15. SEDIMENT LADEN DEWATERING DISCHARGE MUST BE DIRECTED TO AN APPROVED SEDIMENT TRAPPING MEASURE PRIOR TO RELEASE FROM THE SITE.
- 16. ALL EROSION AND SEDIMENT CONTROL MEASURES ARE TO BE CONSIDERED TEMPORARY. THESE MEASURES WILL BE REMOVED BY THE CONTRACTOR AS DESIGNATED ON THE PLANS OR AS DIRECTED BY THE ENGINEER. DISTURBED AREAS ARE TO BE RESTORED UPON REMOVAL.

- 17. WHEN THE CONTRACTOR REQUESTS A CHANGE TO POSTPONE COMPLETION OF THE EXCAVATION OF A SPECIFIC AREA AS A CONTINUOUS OPERATION AND PLACING THE TOPSOIL AS DEFINED IN THE STANDARD SPECIFICATIONS, THE ENGINEER MAY ALLOW THE CONTRACTOR TO STABILIZE THE AREA USING TEMPORARY STABILIZATION WITH STRAW MULCH PROVIDING THE FOLLOWING CONDITIONS ARE MET:
  - A. ALL AREAS BEING STABILIZED ARE 1:3 (V:H) SLOPES OR FLATTER.
  - B. THE COST OF PREPARING THE SEED BED AND STABILIZING THE AREA WITH TEMPORARY STABILIZATION WITH STRAW MULCH IS THE RESPONSIBILITY OF THE CONTRACTOR.
  - C. ALL REQUIRED SEDIMENT CONTROL MEASURES FOR THE SECTION OF ROAD IN QUESTION HAVE BEEN INSTALLED AND ARE BEING MAINTAINED.
- 18. THE CONTRACTOR SHALL PREPARE A SKETCH SHOWING DIMENSIONS FROM TWO ADJACENT OBJECTS TO ALL DRAINAGE STRUCTURES THAT HAVE BEEN PROTECTED. THIS IS TO LOCATE THE STRUCTURE IN CASE OF HEAVY RAINFALL AND THE STRUCTURE IS BLOCKED OR FLOODED. THE ENGINEER SHALL BE PROVIDED WITH A COPY OF THE SKETCH.
- 19. THE CONTRACTOR SHALL CONDUCT HIS OPERATIONS IN ACCORDANCE WITH THE STANDARD DRAWINGS AND SPECIAL PROVISION (S.P.) 111, STORM WATER POLLUTION PREVENTION PLAN INCLUDING CONTROLS AND SPILL PREVENTION-MATERIAL MANAGEMENT PRACTICES. THE CONTRACTOR AND ALL SUBCONTRACTORS SHALL SIGN THE CONTRACTOR'S CERTIFICATION STATEMENT. LIST THE MATERIALS OR SUBSTANCES EXPECTED TO BE PRESENT ON-SITE IN THE INVENTORY FOR POLLUTION PREVENTION PLAN AND SHALL NAME TWO ADDITIONAL INDIVIDUALS TO ASSIST IN SPILL PREVENTION AND CLEAN UP AT THE PRECONSTRUCTION CONFERENCE. SEE S.P. 111.
- 20. AT THE TIME OF THE PRECONSTRUCTION CONFERENCE, THE CONTRACTOR SHALL SUBMIT FOR APPROVAL THE PROPOSED CONCRETE TRUCK WASHOUT LOCATIONS AS REQUIRED IN SPECIAL PROVISION 111. RUNOFF FROM WASH AREAS SHALL BE CONTAINED IN DESIGNATED AREAS SO THAT RUNOFF DOES NOT REACH THE STORM SEWER OR DITCH SYSTEMS. WASHOUT WATER SHALL BE TAKEN TO AN APPROVED DISCHARGE LOCATION.
- 21. IF AN ALTERNATIVE SIZE DITCH CHECK IS PROPOSED BY THE CONTRACTOR FOR USE ON THE PROJECT, A CONTRACT DITCH CHECK SPACING WILL NEED TO BE RECALCULATED BY THE CONTRACTOR IN ACCORDANCE WITH THE ILLINOIS TOLLWAY EROSION AND SEDIMENT CONTROL, LANDSCAPE DESIGN CRITERIA MANUAL. ANY RESULTING QUANTITY CHANGES MUST BE APPROVED BY THE ENGINEER PRIOR TO START OF WORK.
- 22. ALL RUNOFF, EROSION AND SEDIMENT CONTROL MEASURES SHALL BE LOCATED OUTSIDE THE CLEAR ZONE. THE CONTRACTOR SHALL REVIEW THE LOCATIONS OF ALL MEASURES AND PERFORM A BARRIER WARRANT ANALYSIS IF NECESSARY TO ENSURE ROADSIDE OBSTACLES ARE NOT CREATED.
- 23. ALL SLOPES ARE EXPRESSED AS UNITS OF VERTICAL DISPLACEMENT TO UNITS OF HORIZONTAL DISPLACEMENT (V:H).

SHEET 1 OF 9

STANDARD K1-07



	DATE	REVISIONS	
3.	-31-2014	REVISED GENERAL NOTES.	TEMPORARY EROSION
3	-11-2015	REVISED NOTES.	AND SEDIMENT CONTROLS
3-	-31-2016	REMOVED TEMPORARY DITCH CHECKS	7.1.1.B 02.B 1.1.1.2.1.1 00.1.1.1.02.0
3-	-01-2018	REVISED BUFFER WIDTHS AND DETAIL.	

# STANDARD SYMBOLS

CLEARING & GRADING LIMITS -----(LIMITS OF CONSTRUCTION)

 $\bowtie$ 

CULVERT INLET PROTECTION-FENCE



CULVERT INLET PROTECTION-STONE



DEWATERING BASINS



DIVERSION DIKE



DRAINAGE DIVIDE



-- ► EXISTING DRAINAGE PATH



FILTER FABRIC INLET PROTECTION, COVER TYPE



FILTER FABRIC INLET PROTECTION, BASKET TYPE





INITIAL CONSTRUCTION ITEM



PROPOSED DRAINAGE PATH



RECTANGULAR INLET PROTECTION



SEDIMENT BASIN AGGREGATE BERM



SEDIMENT BASIN



SILT FENCE



STABILIZED CONSTRUCTION ENTRANCE



STONE OUTLET STRUCTURE SEDIMENT TRAP



STREAM DIVERSION



SUPER SILT FENCE



TEMPORARY DITCH CHECK



TEMPORARY PIPE SLOPE DRAIN



TEMPORARY RIPRAP



TEMPORARY ROCK CHECK DAM



TEMPORARY STREAM CROSSING



TEMPORARY SWALE



TREE PROTECTION

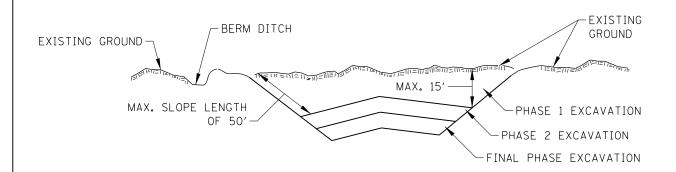
SHEET 2 OF 9



TEMPORARY EROSION AND SEDIMENT CONTROLS

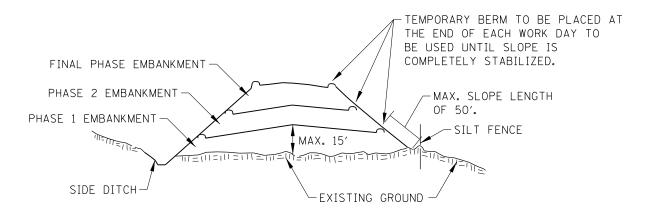
STANDARD K1-07

Paul Kovacs APPROVED. DATE 2-7-2012



- 1. ALL CUT SLOPES SHALL BE EXCAVATED AND STABILIZED (PLACE TOPSOIL, PREPARE SEEDBED, APPLY SEED, PROTECT SLOPE WITH MULCH OR EROSION BLANKET) AS THE WORK PROGRESSES.
- 2. CONSTRUCTION SEQUENCE:
  - A) EXCAVATE AND STABILIZE BERM, SIDE AND OUTLET DITCHES, PROVIDE SEDIMENT TRAPS FOR DITCHES.
  - B) PERFORM PHASE 1 EXCAVATION AND STABILIZE SLOPES WITH PERMANENT SEEDING.
  - C) PERFORM PHASE 2 EXCAVATION AND STABILIZE SLOPES WITH PERMANENT SEEDING. OVER SEED PHASE 1 SLOPES, IF REQUIRED.
  - D) PERFORM FINAL PHASE EXCAVATION, DRESS, SEED AND MULCH SLOPES WITH PERMANENT SEEDING. STABILIZE SURFACE DRAIN DITCHES. OVER SEED PHASE 1 & 2 SLOPES, IF REQUIRED, AS DETERMINED BY THE ENGINEER.
- 3. IF PERMANENT SEEDING CANNOT BE PLACED DUE TO CONTRACT REQUIREMENTS REGARDING PLANTING SEASONS, THE CUT SLOPE IS TO HAVE TOPSOIL PLACED AND SEEDING PREPARED PRIOR TO USING TEMPORARY STABILIZATION WITH STRAW MULCH OR TEMPORARY SEEDING WITH EROSION BLANKET.
- 4. THE CONTRACTOR HAS THE OPTION OF DELAYING TOPSOIL SEEDING BEYOND THE 15 FOOT LIMITATION. IF THIS OPTION IS CHOSEN, THE CUT SLOPE MUST BE "TEMPORARY STABILIZED" AT NO COST TO THE ILLINOIS TOLLWAY.
- 5. ONCE THE EXCAVATION WITHIN A SPECIFIC AREA HAS BEGUN, THE OPERATION SHALL BE CONTINUOUS FROM STRIPPING THROUGH THE COMPLETION OF THE GRADING AND PLACEMENT OF SLOPE STABILIZATION MEASURES. ANY INTERRUPTIONS IN THE OPERATION OF 14 DAYS OR MORE MUST BE APPROVED BY THE ENGINEER. ANY VIOLATION OF THIS REQUIREMENT WILL RESULT IN THE CONTRACTOR ASSUMING THE RESPONSIBILITY OF PLACING TEMPORARY STABILIZATION AT HIS OWN COST AND EXPENSE.

EXCAVATION PHASING PLAN - CUT SECTION



### NOTES:

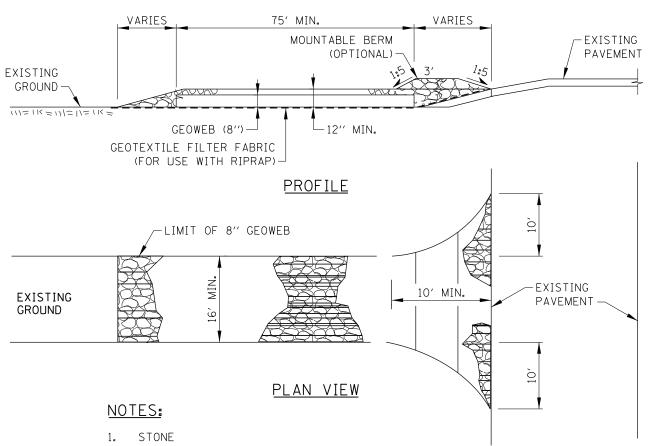
- 1. THE EMBANKMENT WILL BE MADE IN STAGES NOT TO EXCEED 15' IN HEIGHT OR 50' IN SLOPE LENGTH. THE EMBANKMENT SLOPES WILL BE STABILIZED USING TEMPORARY MEASURES BEFORE BEGINNING NEXT STAGE.
- 2. AT THE END OF EACH WORK DAY TEMPORARY BERMS (EARTH) AND TEMPORARY PIPE SLOPE DRAINS WILL BE CONSTRUCTED ALONG THE TOP EDGE(S) OF THE EMBANKMENT TO INTERCEPT SURFACE RUNOFF.
- 3. CONSTRUCTION SEQUENCE:
  - A) EXCAVATE AND STABILIZE SIDE DITCH AND/OR INSTALL PROPOSED PERIMETER CONTROLS AT THE TOE OF SLOPE.
  - B) PLACE PHASE 1 EMBANKMENT AND STABILIZE WITH TEMPORARY SEEDING AND MULCH.
  - C) PLACE PHASE 2 EMBANKMENT AND STABILIZE WITH TEMPORARY SEEDING AND MULCH.
  - D) PLACE FINAL PHASE EMBANKMENT AND STABILIZE WITH PERMANENT VEGETATIVE PLAN ON THE ENTIRE SLOPE.
- 4. ONCE THE PLACEMENT OF FILL WITHIN A SPECIFIC AREA HAS BEGUN, THE OPERATION SHALL BE CONTINUOUS FROM STRIPPING THROUGH THE COMPLETION OF THE GRADING AND PLACEMENT OF PERMANENT VEGETATIVE PLAN. ANY INTERRUPTIONS IN THE OPERATION OF 14 DAYS OR MORE MUST BE APPROVED BY THE ENGINEER. ANY VIOLATION OF THIS REQUIREMENT WILL RESULT IN THE CONTRACTOR ASSUMING THE RESPONSIBILITY OF PLACING TEMPORARY STABILIZATION AT HIS OWN COST AND EXPENSE.

EMBANKMENT PHASING PLAN - FILL SECTION

SHEET 3 OF 9



TEMPORARY EROSION AND SEDIMENT CONTROLS

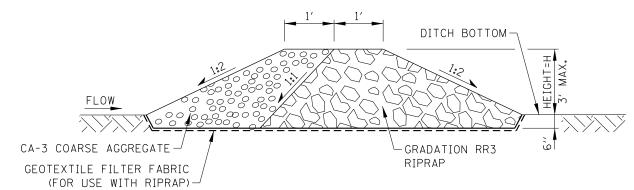


- A. STONE SIZE CA-3
- B. LENGTH AS REQUIRED, BUT NOT LESS THAN 75'.
- C. THICKNESS NOT LESS THAN 4" ABOVE TOP OF GEOWEB.
- 2. WIDTH 16' MINIMUM FOR ONE WAY TRAFFIC: 24' MINIMUM FOR TWO-WAY TRAFFIC: BUT NOT LESS THAN THE FULL WIDTH AT POINTS WHERE INGRESS OR EGRESS OCCURS.
- 3. GEOWEB NOT LESS THAN 8" IN DEPTH WILL BE PLACED OVER THE ENTIRE AREA PRIOR TO PLACING OF STONE.
- 4. SURFACE WATER ALL SURFACE WATER FLOWING OR DIVERTED TOWARD CONSTRUCTION ENTRANCES SHALL BE PIPED ACROSS THE ENTRANCE. IF PIPING IS IMPRACTICAL, A MOUNTABLE BERM WITH 1:5 SLOPES WILL BE PERMITTED.
- 5. MAINTENANCE THE ENTRANCE SHALL BE MAINTAINED IN A CONDITION WHICH SHALL PREVENT TRACKING OR FLOWING OF SEDIMENT ONTO PUBLIC RIGHTS-OF-WAY. THIS MAY REQUIRE PERIODIC TOP DRESSING WITH ADDITIONAL STONE AS CONDITIONS DEMAND AND REPAIR AND/OR CLEAN OUT OF ANY MEASURES USED TO TRAP SEDIMENT. ALL SEDIMENT SPILLED. DROPPED. WASHED OR TRACKED ONTO PUBLIC RIGHTS-OF-WAY SHALL BE REMOVED IMMEDIATELY.
- PERIODIC INSPECTION AND NEEDED MAINTENANCE SHALL BE PROVIDED AFTER HEAVY USE AND EACH RAINFALL EVENT.
- 7. TO BE USED TO REDUCE OR ELIMINATE TRACKING OF SEDIMENT ONTO PUBLIC STREETS. PLACE AT ALL POINTS OF CONSTRUCTION INGRESS AND EGRESS. DISTURBED AREAS TO BE RESTORED UPON REMOVAL.

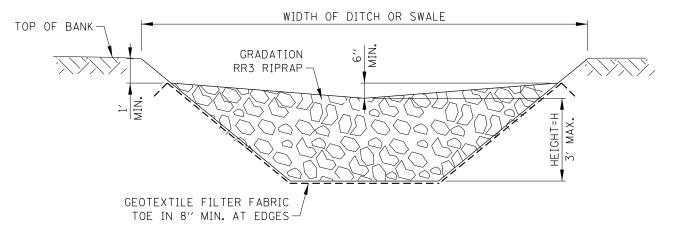
# STABILIZED CONSTRUCTION ENTRANCE

STANDARD SYMBOL





PROFILE



# CROSS SECTION CENTERLINE LOOKING DOWNSTREAM

#### NOTES:

- 1. FOR LOCATIONS AND HEIGHTS OF ROCK CHECK DAMS REFER TO CONSTRUCTION DRAWINGS.
- 2. TEMPORARY ROCK CHECK DAMS SHALL BE REPLACED WHEN THEY CEASE TO FUNCTION AS INTENDED DUE TO WASHOUT OR CONSTRUCTION TRAFFIC DAMAGE.
- 3. SEDIMENT SHALL BE REMOVED WHEN IT REACHES 50% OF DAM HEIGHT. THIS PRACTICE IS NOT A SUBSTITUTE FOR MAJOR PERIMETER TRAPPING SUCH AS A TEMPORARY SEDIMENT TRAP OR BASIN.
- 4. SPACING BETWEEN DAMS SHALL BE SUCH THAT THE TOE OF THE UPSTREAM DAM IS AT THE SAME ELEVATION AS TOP OF RIPRAP AT THE CENTER OF THE DOWNSTREAM DAM.
- 5. WHEN A TEMPORARY ROCK CHECK DAM IS IN THE CLEAR ZONE, IT MUST BE MADE TRAVERSABLE TO AN ERRANT VEHICLE. THE MAXIMUM UNSHIELDED TRANSVERSE SLOPE ALLOWED TO FACE TRAFFIC SHALL BE 1:10 (V:H) AND THE MAXIMUM TRANSVERSE FACING AWAY FROM TRAFFIC SHALL BE 1:4 (V:H). AN UNSHIELDED TEMPORARY ROCK CHECK DAM SHALL HAVE AN ADDITIONAL LAYER OF CA-3 COURSE AGGREGATE (6" MIN.) PLACED ON THE DOWNSTREAM SIDE OF THE ROCK CHECK DAM. THE GEOTEXTILE FILTER FABRIC SHALL BE PLACED ALONG THE ENTIRE BASE OF THE TEMPORARY ROCK CHECK DAM.

Illinois *Tollway* 

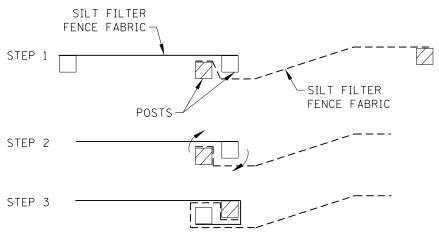
SHEET 4 OF 9

TEMPORARY EROSION AND SEDIMENT CONTROLS

STANDARD K1-07

TEMPORARY ROCK CHECK DAM STANDARD SYMBOL

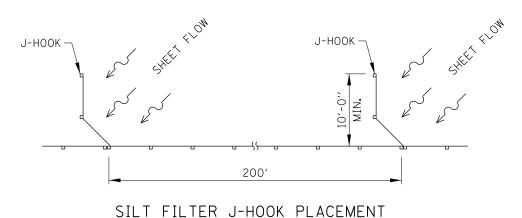
CHIEF ENGINEERING OFFICER

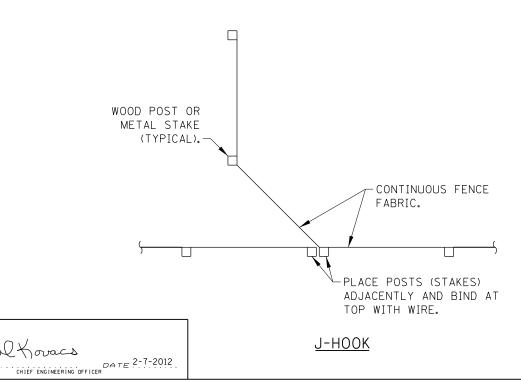


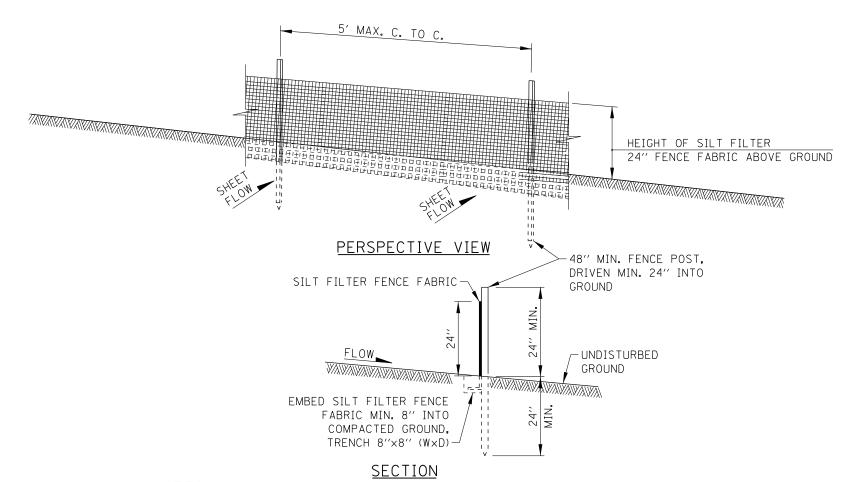
Paul Koracs

- 1. PLACE THE END POST OF THE SECOND FENCE INSIDE THE END POST OF THE FIRST FENCE.
- 2. ROTATE BOTH POSTS AT LEAST 180 DEGREES IN A CLOCKWISE DIRECTION TO CREATE A TIGHT SEAL WITH THE FABRIC MATERIAL.
- 3. DRIVE BOTH POSTS A MINIMUM OF 24" INTO THE GROUND.

# ATTACHING TWO SILT FENCES







# NOTES:

- 1. SILT FILTER FENCE FABRIC TO BE FASTENED SECURELY TO FENCE POSTS.
- 2. WHEN TWO SECTIONS OF SILT FILTER FENCE FABRIC ADJOIN EACH OTHER THEY SHALL BE SECURELY FASTENED PER THE DETAIL ATTACHING TWO SILT FENCES.
- 3. MAINTENANCE SHALL BE PERFORMED AS NEEDED. SILT BUILD UP AGAINST FENCE SHALL BE REMOVED WHEN "BULGES" DEVELOP IN THE SILT FENCE, OR WHEN SILT REACHES 50% OF FENCE HEIGHT.
- 4. FENCE POSTS: 2"x2" (NOMINAL) HARDWOOD OR SCHEDULE 40 METAL PIPE OR 1.33 LB/FT MIN. STANDARD T OR U SECTION STEEL POSTS.
- 5. THIS DEVICE IS TO CONTROL SHEET FLOW ONLY. DO NOT USE FOR CONCENTRATED FLOWS, DRAINAGE CHANNELS, ABOVE OR BELOW DRAINAGE PIPES.

SILT FENCE (SF) STANDARD SYMBOL

SHEET 5 OF 9



TEMPORARY EROSION AND SEDIMENT CONTROLS

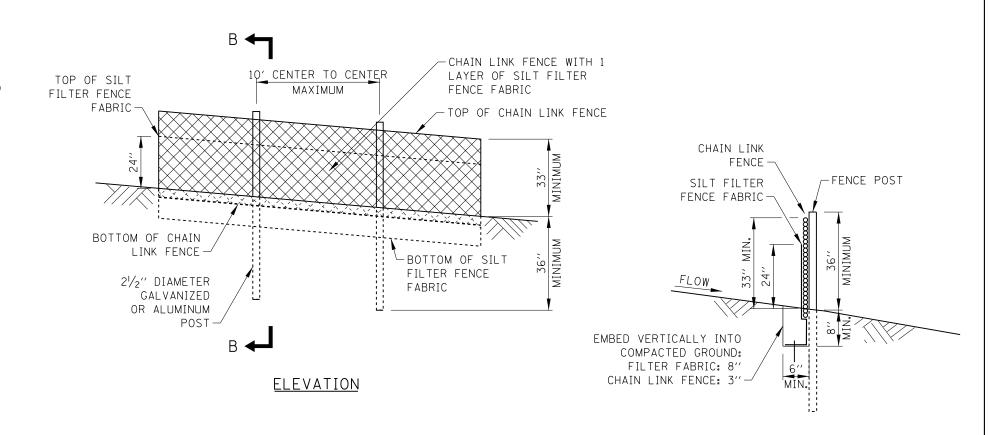
STANDARD K1-07

- 1. FENCING SHALL BE 36" IN HEIGHT AND CONSTRUCTED IN ACCORDANCE WITH ILLINOIS TOLLWAY STANDARD DRAWING D1, RIGHT-OF-WAY FENCE, TYPE 1. THE SPECIFICATION FOR A 6" FENCE SHALL BE USED, SUBSTITUTING 36" FABRIC AND 6" LENGTH POSTS.
- 2. CHAIN LINK FENCE SHALL BE FASTENED SECURELY TO THE FENCE POSTS WITH WIRE TIES. THE LOWER TENSION WIRE, BRACE AND TRUSS RODS, DRIVE ANCHORS AND POST CAPS ARE NOT REQUIRED. PULL POSTS, CORNER POSTS, HORIZONTAL BRACING AND TIE RODS ARE NOT REQUIRED.
- 3. SILT FILTER FENCE FABRIC SHALL BE FASTENED SECURELY TO THE CHAIN LINK FENCE WITH TIES SPACED EVERY 24" AT THE TOP AND MID SECTION.
- 4. WHEN TWO SECTIONS OF SILT FILTER FENCE FABRIC ADJOIN EACH OTHER, THEY SHALL BE OVERLAPPED 2' HORIZONTALLY.
- 5. MAINTENANCE SHALL BE PERFORMED AS NEEDED. SILT BUILD-UP AGAINST FENCE SHALL BE REMOVED WHEN SILT REACHES 50% OF FENCE HEIGHT.
- 6. SUPER SILT FENCE IS TO BE USED TO PROTECT ENVIRONMENTALLY SENSITIVE AREAS AND CONTROL SEDIMENT RUNOFF FROM CONSTRUCTION SITES WHEN ADDITIONAL REINFORCEMENT IS REQUIRED DUE TO SLOPE OF SITE OR VOLUME OF STORM WATER RUNOFF.

# SUPER SILT FENCE (SSF)

STANDARD SYMBOL

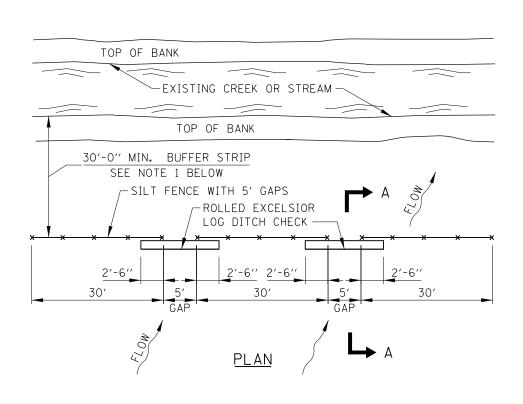
\_\_\_\_\_ SSF \_\_\_\_\_

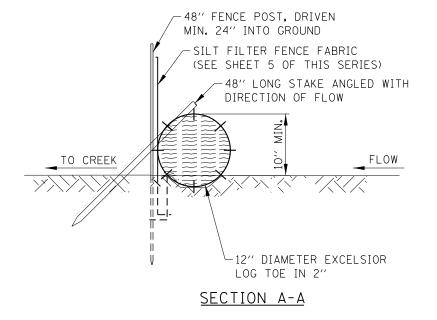


## SECTION B-B

#### NOTES:

- 1. A MINIMUM 30' WIDE VEGETATED BUFFER STRIP SHALL BE PRESERVED AND/OR RE-ESTABLISHED WHERE POSSIBLE ALONG EXISTING CHANNELS.
  - a. FOR ANY WATERS OF THE U.S. DETERMINED TO BE A HIGH-QUALITY AQUATIC RESOURCE, THE BUFFER MUST BE A MINIMUM OF 100'.
  - b. FOR ANY WATERS OF THE U.S. THAT DO NOT QUALIFY AS WETLAND (FOR EXAMPLE LAKES, RIVERS, PONDS, ETC.), THE BUFFER MUST BE A MINIMUM OF 50' FROM THE ORDINARY HIGH WATER MARK (OHWM).
  - c. FOR ANY JURISDICTIONAL WETLAND FROM 0.25 ACRES UP TO 0.50 ACRES IN SIZE, THE BUFFER MUST BE A MINIMUM OF 30'.
  - d. FOR ANY JURISDICTIONAL WETLAND OVER 0.50 ACRES IN SIZE, THE BUFFER MUST BE A MINIMUM OF 50'.
- 2. THE 5' GAPS IN THE SILT FENCE AND THE 20"
  DIAMETER TEMPORARY DITCH CHECKS ARE TO ALLOW
  FLOODWATER FLOW INTO THE CREEK FROM THE SITE
  WITHOUT DAMAGE TO THE SILT FENCE.
- 3. MAINTENANCE SHALL BE PERFORMED AS NEEDED AND SILT SHALL BE REMOVED WHEN IT REACHES 50% OF ROLL HEIGHT. WHEN ROLLED EXCELSIOR LOG BECOMES LESS THAN 10" IT SHALL BE REPLACED.





SHEET 6 OF 9



TEMPORARY EROSION AND SEDIMENT CONTROLS

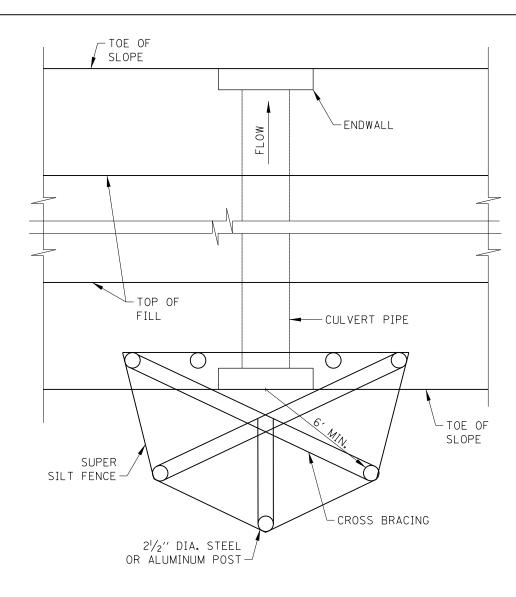
STANDARD K1-07

Paul Kovacs

APPROVED. .... CHIEF ENGINEERING OFFICER

DATE 2-7-2012

CREEK BUFFER STRIP AND SILT FENCE



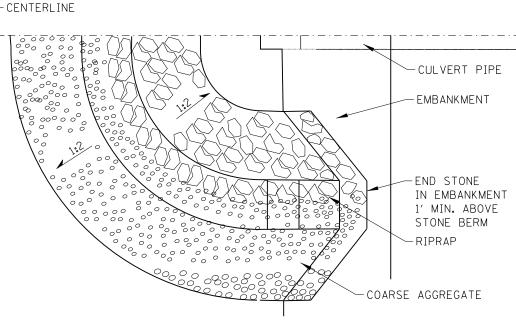
#### PLAN VIEW

#### NOTES:

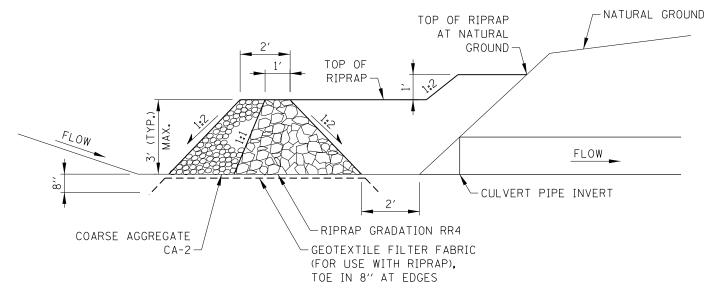
- 1. CONSTRUCT SUPER SILT FENCE PER SHEET 6 IN THIS SERIES, EXCEPT THE MAXIMUM POST SPACING SHALL BE 3 FEET AND THE TOPS OF POSTS SHALL BE CROSSED BRACED.
- 2. MAINTENANCE SHALL BE PERFORMED AS NEEDED. SEDIMENT SHALL BE REMOVED WHEN IT REACHES 50% OF THE FENCE HEIGHT.
- 3. THE CULVERT INLET PROTECTION AND SEDIMENT SHALL BE REMOVED WHEN CONSTRUCTION IS COMPLETE.
- 4. THE CULVERT INLET PROTECTION FENCE TO BE MEASURED AND PAID FOR AS SUPER SILT FENCE.

# CULVERT INLET PROTECTION - FENCE STANDARD SYMBOL





# HALF PLAN VIEW



#### CENTERLINE CROSS SECTION

# NOTES:

- 1. MAINTENANCE SHALL BE PERFORMED AS NEEDED. SEDIMENT SHALL BE REMOVED WHEN IT REACHES 50% OF THE STONE HEIGHT.
- 2. THE CULVERT INLET PROTECTION AND SEDIMENT SHALL BE REMOVED WHEN CONSTRUCTION IS COMPLETE.
- 3. THE CULVERT INLET PROTECTION STONE TO BE MEASURED AND PAID FOR AS TEMPORARY RIPRAP.

CULVERT INLET PROTECTION - STONE
STANDARD SYMBOL





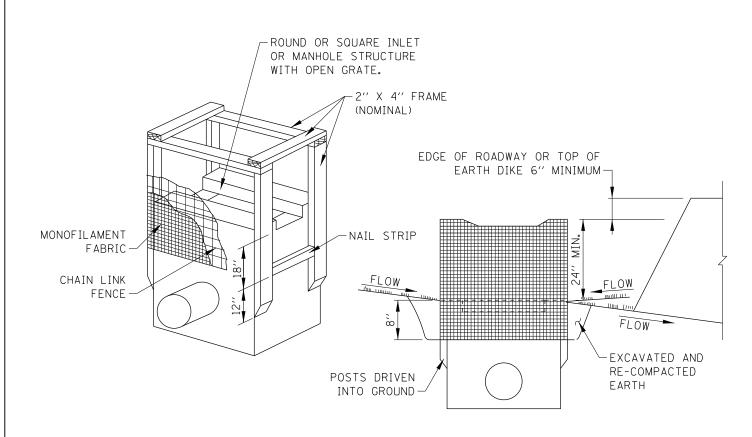
SHEET 7 OF 9

TEMPORARY EROSION AND SEDIMENT CONTROLS

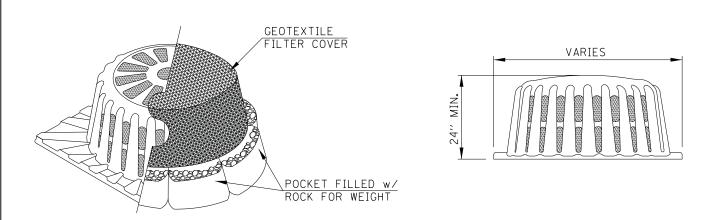
STANDARD K1-07

Paul Kovacs
APPROVED. ... CHIÉF ENGINÉERING OFFICER

DATE 2-7-2012







#### POLYETHYLENE FRAME

#### NOTES:

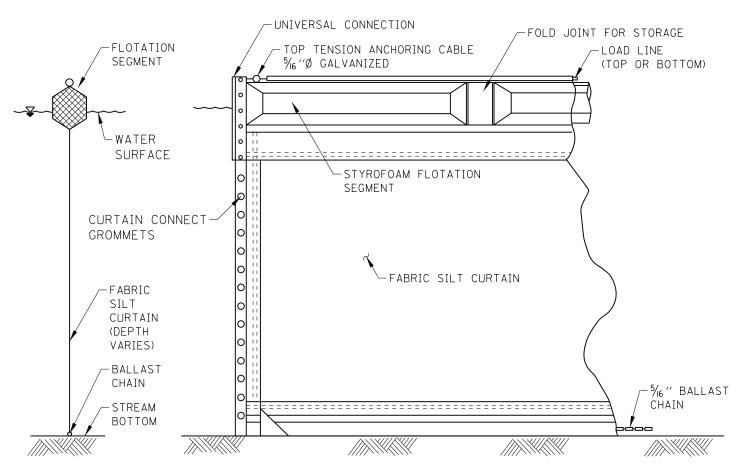
- WOODEN FRAME IS TO BE CONSTRUCTED OF 2"x4" CONSTRUCTION GRADE LUMBER. IF CONTRACTOR PREFERS, SUPER SILT FENCE CAN BE CONSTRUCTED AROUND THE INLET PER SHEET 6 IN THIS SERIES.
- MAINTENANCE SHALL BE PERFORMED AS NEEDED AND SILT REMOVED WHEN IT REACHES 50% OF FENCE HEIGHT.
- TO BE USED TO PROTECT EXISTING AND NEW INLETS, CATCH BASINS AND MANHOLES WITH OPEN LIDS IN NON-PAVED AREAS.

#### RECTANGULAR INLET PROTECTION

STANDARD SYMBOL

Paul Koracs APPROVED. CHIEF ENGINEERING OFFICER 2-7-2012





# SECTION

#### ELEVATION

#### NOTES:

- FLOTATION BOOM FOR USE IN MOVING WATER SHALL BE ANCHORED TO PREVENT DRIFT SHOREWARD OR DOWNSTREAM. ANCHORAGES SHALL BE INSTALLED ON BOTH SHORE AND STREAM SIDE. BOOMS ARE NOT TO BE INSTALLED ACROSS FLOWING BODY OF WATER.
- SHORE ANCHORS SHALL CONSIST OF A POST WITH DEADMAN OR APPROVED EQUAL. STREAM ANCHORS SHALL BE OF SUFFICIENT SIZE TO STABILIZE THE BARRIER WITH NUMBER AND SPACING DEPENDENT ON WATERWAY VELOCITIES.
- FABRIC SECTIONS SHALL BE CONNECTED END TO END WITH MINIMUM %" DIAMETER POLYPROPYLENE ROPE.
- DESIGN OF BOOM AND ANCHORAGE SHALL BE IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS. BOTTOM OF BOOM SHALL REACH BOTTOM OF WATERWAY USING ONE VERTICAL SECTION AS REQUIRED.
- MAINTENANCE SHALL BE PERFORMED AS NEEDED. CONTRACTOR SHALL REMOVE THE BOOM AT COMPLETION OF WORK IN A MANNER THAT WILL PREVENT SILTATION OF THE WATERWAY.
- CONSTRUCTION DEBRIS/MATERIALS SHALL BE REMOVED IMMEDIATELY TO PREVENT DAMAGE TO THE CURTAIN AND ENTRY INTO THE WATERWAY.
- FLOTATION BOOMS TO BE USED TO CONTROL TURBIDITY WHEN WORKING IN WATERWAYS.

FLOTATION BOOM

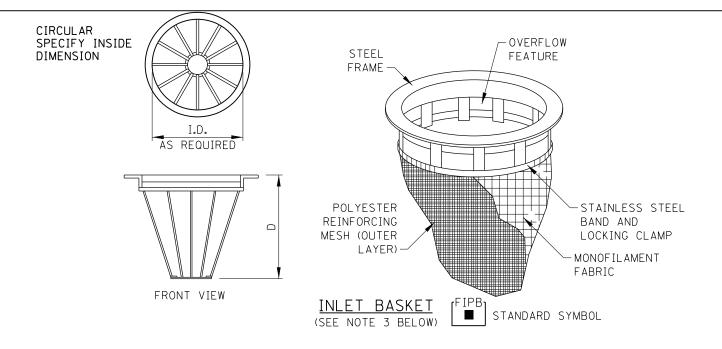
STANDARD SYMBOL -FB----FB-

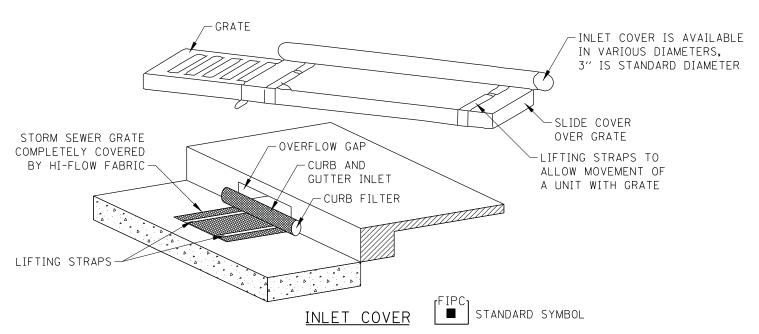
TEMPORARY EROSION AND SEDIMENT CONTROLS

SHEET 8 OF 9 Illinois

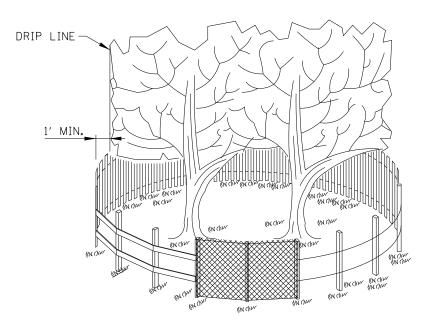
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STANDARD K1-07

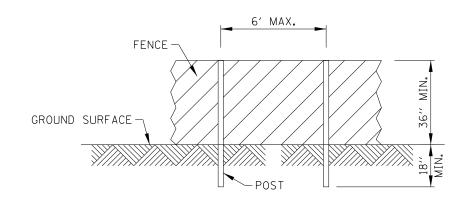




- 1. MONOFILAMENT FABRIC INLET PROTECTION SHALL CONSIST OF INLET BASKET, FRAME AND FABRIC INSERT.
- 2. DEVICE SHALL BE EQUIPPED WITH AN OVERFLOW FEATURE SO DRAINAGE TO INLET IS NOT COMPLETELY BLOCKED IF DEVICE IS FULL OF SILT.
- 3. INLET BASKET IS AVAILABLE TO FIT ROUND, RECTANGULAR, BEEHIVE OR CURB INLET CASTINGS.
- 4. MAINTENANCE SHALL BE PERFORMED AS NEEDED. REMOVE SILT FROM FABRIC INSERT WHEN 50% OF CAPACITY IS REACHED. REMOVE SILT FROM INTERIOR AND EXTERIOR OF INLET COVER WHEN 50% OF COVER HEIGHT IS REACHED.



SIDE VIEW



POST AND FENCE DETAIL

# NOTES:

- 1. THE FENCE SHALL BE LOCATED 1 FOOT MINIMUM OUTSIDE THE DRIP LINE OF THE TREE TO BE SAVED AND IN NO CASE CLOSER THAN 5 FEET TO THE TRUNK OF ANY TREE.
- 2. THE FENCE SHALL BE HIGH VISIBILITY PLASTIC OR WOOD LATH SNOW FENCE TO CLEARLY DELINEATE THE PROTECTION AREA.
- 3. USED TO PROTECT TREES FROM DISTURBANCE AND FROM EQUIPMENT TRAVELING OVER THE ROOT ZONE.

TREE PROTECTION STANDARD SYMBOL





SHEET 9 OF 9

TEMPORARY EROSION
AND SEDIMENT CONTROLS

STANDARD K1-07

FILTER FABRIC INLET PROTECTION